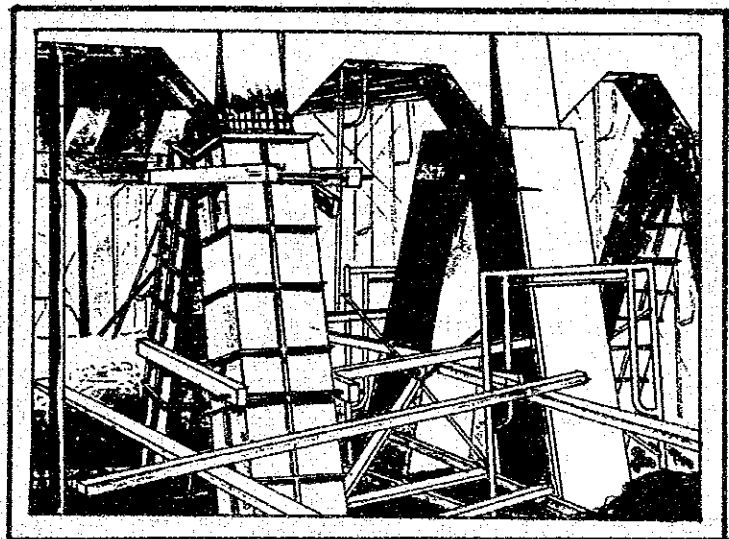
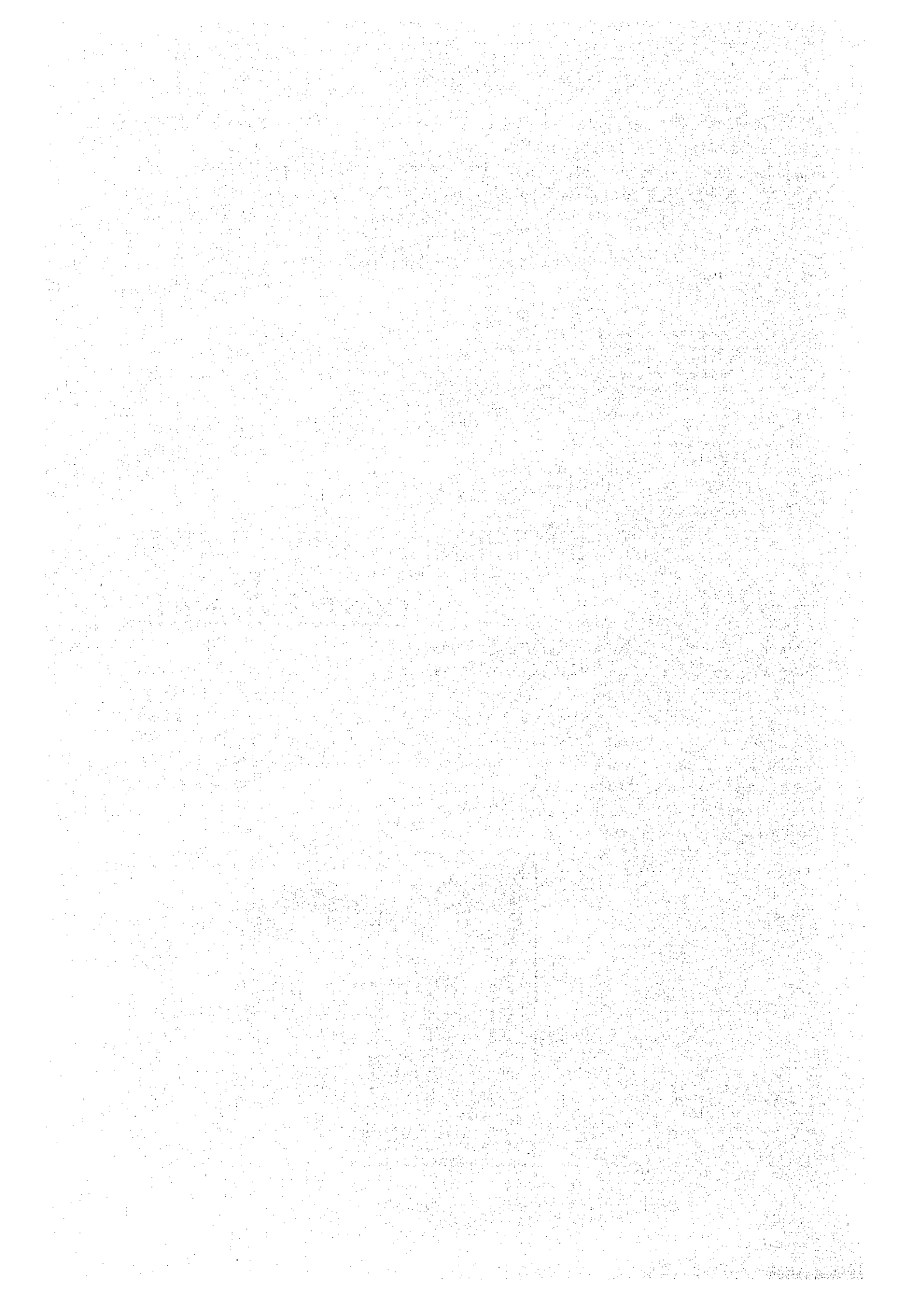


# **CHAPTER 14**

## **COST ESTIMATE**





## CHAPTER 14

### COST ESTIMATE

#### 14.1 General

The cost estimate started with an extensive data collection exercise and field survey at several existing bridge sites and at a few bridge construction sites as well as interview surveys with some local contractors to ensure that the unit price analysis and results were firmly based on the real situation in Malaysia.

Objective of the study is to estimate the maintenance and rehabilitation project cost for each of the study bridges, amounting to 205 bridges.<sup>(1)</sup>

To achieve the above objective, the following procedures were applied.

- Assessment of the indirect cost items and project cost composition and determination of multiplier factors of these items.
- Review of main work items derived from the preliminary design of 20 bridges and identification of the associated subsidiary work items.
- Analysis of the unit price of each pay item (or each rehabilitation method) based on above review results.
- Establishment of standard unit prices applicable to all the bridges based on assessment of the unit prices analyzed.
- Estimate of the maintenance and rehabilitation project cost covering all the study bridges.

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Note:

(1.) After commencement of the Study, 11 number of the bridges have been replaced or being constructed by the Government. Thus, the total number of the bridges has been reduced from 216 bridges to 205 bridges.

## **14.2 Unit Price Analysis for the 20 Bridges**

### **14.2.1 Basic Condition**

Following basic conditions were applied in the cost estimate:

- Price level of labour, material and equipment is based on December, 1991.
- The unit rates are derived from market investigation and from JKR.
- The unit rates are in and around Kuala Lumpur and the local deviation of the rates is not considered in the estimate.
- Except for the direct cost, other costs such as contractor's overhead and profit, detailed design and supervision cost are computed using the multiplier factors.
- Production rate in the unit price analysis is basically based on standard production rates of various work items in Japan after some modification was made with due consideration of Malaysian local conditions.

### **14.2.2 Structure of Project Cost**

Project cost, in general, consists of construction cost, land acquisition and compensation, engineering cost, administration cost and contingency. The construction cost is divided into prime construction cost and contractor's overhead and profit. The prime construction cost is further subdivided into direct cost comprising of labour cost, material cost and equipment cost and indirect cost such as field supervision cost and common preliminary work cost.

Structure of the project cost is depicted in Figure 14-1 and the main cost items are briefly described below.

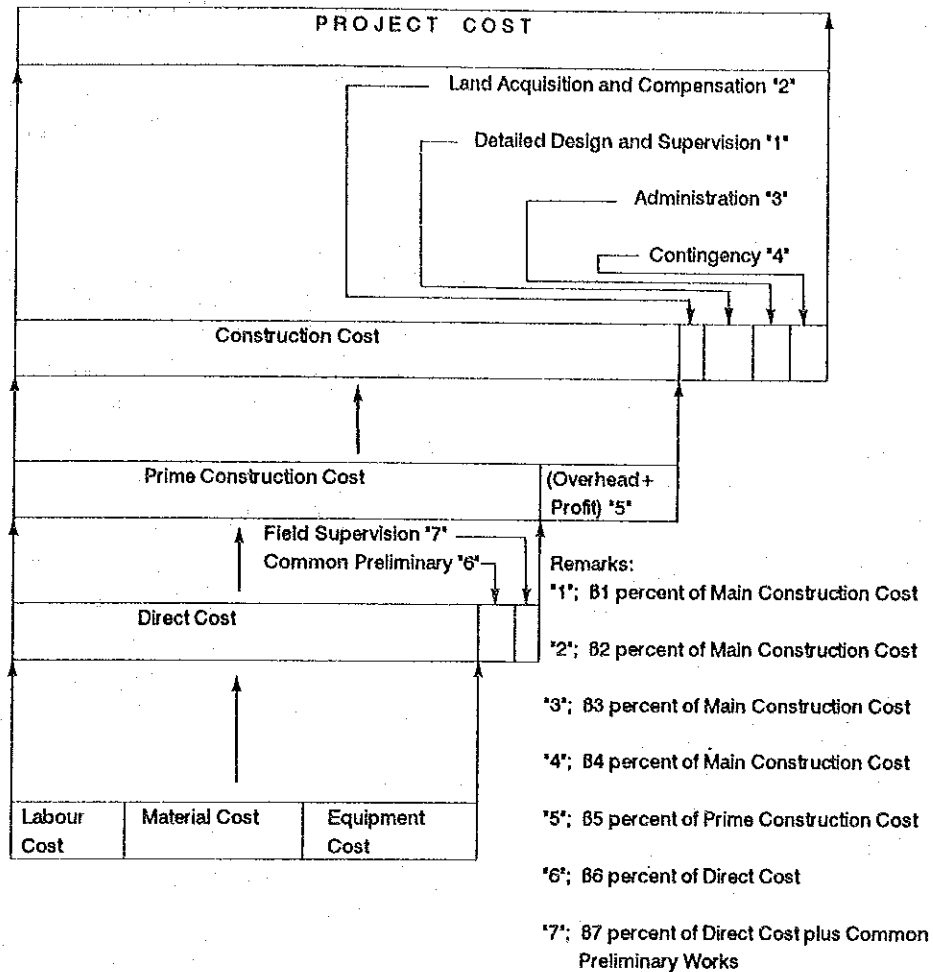
#### **(1) Direct Cost**

The direct cost of each pay item is estimated based on the quantities derived from the preliminary design. This comprises of labour cost, material cost and equipment cost.

##### **▪ Labour Cost**

Labour cost includes wages, income tax and all fringe benefits such as vacation, sick leave, medicare, EPF contributions and workmen's compensation. All these follow government regulations. The labour cost is estimated on the basis of data researched from the market investigation.

**Figure 14-1 Components of Project Cost**



▪ **Material Cost**

All materials for the rehabilitation works can be procured from the Malaysian market. The materials cost to be used in the cost estimate are adjusted according to the prevailing escalation rate and on the basis of price level at the end of 1991.

▪ **Equipment Cost**

The cost of construction equipment is estimated on the basis of market investigation. It is considered that the equipment expense per unit per hour includes depreciation cost, operator's wages, cost of maintenance, fuel and lubricants necessary for equipment operation and repair.

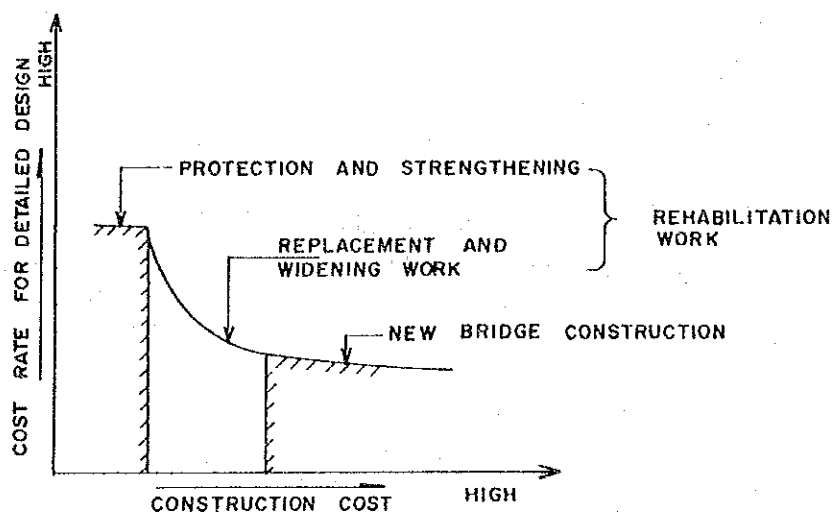
The unit rate of labour, material and equipment applied in the Study are shown in Appendix R of Volume III.

## (2) Engineering Cost

Engineering cost consists for detailed design and construction supervision and it is assumed to be 10% (6% for the detailed design and 4% for supervision) of the construction cost.

In general, cost rate of detailed design is inversely proportional to the construction cost. Furthermore, among bridge projects, rehabilitation design cost is relatively higher than that for a new bridge construction because the rehabilitation work is complicated and requires special techniques. These concepts can be depicted as a cost rate curve shown in Figure 14-2.

**Figure 14-2 Relationship between Construction Cost and Cost Rate for Detail Design**



From the above, 6% for the detailed design is adopted in this Study instead of 4% as used in NALS.

While for construction supervision, 4% is applied in this Study which is 2% lower than that for NALS, assuming that management of the service is required for scattered project locations.

## (3) Land Acquisition and Compensation Cost

In a new bridge construction, a considerable amount of land acquisition and compensation costs are required. While, in the rehabilitation work no additional land acquisition cost is necessary, only a small amount of compensation cost may be required where detour road is provided. However, these costs can be incorporated into

the construction cost. Therefore in the Study, the land acquisition and compensation cost are neglected i.e. considered as zero percent.

**(4) Administration Cost**

Administration cost is expenses by the Government arising from implementation of the project and is assumed to be 3% of the construction cost.

**(5) Contingency**

Contingency is divided into physical contingency and price contingency as described below:

- Physical contingency is mainly to cover unforeseeable or unavoidable design and cost estimate items, generally depending on the level of study. For feasibility study level, 15% of the construction cost is considered in the study.
- Price contingency allows for future price escalation and fluctuation of exchange rates. Therefore, at this stage price contingency is not considered.

**(6) Overhead and Profit**

This item including taxes is assumed to be 20% of the prime construction cost through the market study.

**(7) Common Preliminary Works**

This cost is to cover expenditure required to run contractor's site office, warehouse, laboratory and for other common temporary works. In this study, 5% of the direct cost is adopted since transportation and individual temporary works have been incorporated in the direct cost.

**(8) Field Supervision**

Contractor's field supervision cost covering mainly contractor's field management staff cost is assumed to be 4% of the direct cost.

A summary of the multiplier factors applicable to this study is tabulated in Table 14-1 together with comparison of the figures adopted in NALS.

**Table 14-1 Summary of Applicable Multiplier Factors  
Compared with Those Adopted in NALS**

Factor	Description	Axle Load Study (%)	This Study (%)
β 1	Detailed Design and Supervision	4	6
		6	4
β 2	Land Acquisition and Compensation	Excluded in the study	0
β 3	Administration	5	3
β 4	Contingency		
	i) Physical Contingency ii) Price Contingency	- 15	15 See (5)
β 5	Overhead + Profit	30	20
β 6	Common Preliminary Works	10	5
β 7	Field Supervision	6	4

Consequently the project cost is given by the following computation based on the structure of project cost and is the one hundred sixty eight (168) percent of the Direct Cost.

$$\begin{aligned}
 \text{Project Cost} &= \text{Direct Cost} \times \{ 1 + \beta_6 + (1 + \beta_6) \beta_7 \} \times (1 + \beta_5) \times \\
 &\quad (1 + \beta_4 + \beta_3 + \beta_2 + \beta_1) \\
 &= \text{Direct Cost} \times \{ 1 + 0.05 + (1 + 0.05) \times 0.04 \} \times (1 + 0.20) \times \\
 &\quad (1 + 0.15 + 0.03 + 0 + 0.10) \\
 &= \text{Direct Cost} \times 1.68
 \end{aligned}$$

### 14.2.3 Unit Price Analysis

In order to analyze unit prices of the respective rehabilitation work items, required materials, equipment and labour are listed and the unit quantity of each item (or subitem) is firstly calculated based on standard construction methods. The unit price consisting of basically labour cost, material cost and equipment cost is in general given by the following formula.

$$\text{U.P.} = \sum_i^m \alpha_i L_i + \sum_i^n \beta_i M_i + \sum_i^o \gamma_i E_i$$

Where:

- U.P. : Unit Price
- $L_i$  : Labour unit rate
- $M_i$  : Material unit rate
- $E_i$  : Equipment unit rate
- $\alpha_i, \beta_i, \gamma_i$  : Production rate of each item

It is, however, difficult to estimate the accurate production rates of respective items in the rehabilitation works because of the lack of the rehabilitation work records in Malaysia.



To this end, the following procedures are taken so as to ensure the analysis results are as precise as possible.

- (1) The production rate stated in the Cost Estimate Manual published by the Ministry of Construction in Japan is modified taking into account labours' skillfulness, unit material usage, efficiency of equipment, operator's capability and so on in Malaysia as well as based on those in similar projects in ASEAN countries.
- (2) Applying the modified production rates of various items, the unit prices are calculated using the above mentioned formula.
- (3) The production rate applied in the above calculation is calibrated based on comparison with unit prices obtained from JKR and market research.
- (4) Finally the unit price of each work item is recalculated using the calibrated production rate.

Example calculation sheets which elaborate the above methodology for derivation of unit prices for several work items are enclosed in Appendix-P of Volume III. A summary of unit price for each pay item under 20 bridges is tabulated in Table 14-2.

### **14.3 Standard Unit Prices of Respective Rehabilitation Methods**

One of the main purposes of the Study is to establish an implementation program for the bridge rehabilitation covering all the study bridges which in turn requires cost estimate of all the bridges.

In this regard, rehabilitation methods and the work quantities for the rest of the bridges, which were discarded either from the visual inspection or from the detailed survey, were designated during the supplemental survey and are presented in Chapter 13.

Therefore, standard unit prices applicable to the other bridges are prepared in this section based on the assessment of the unit price for the 20 bridges.

#### **14.3.1 Classification of Unit Prices for the 20 Bridges**

The unit prices analyzed for 20 bridges were reviewed from an applicability viewpoint to the other bridges and were classified into the following four categories.

**Category A :** Where the unit prices are not affected by the job site condition and work the sequences are similar in every bridge. Thus the unit price will be applicable to the rehabilitation method in other bridges without any modification.



**Category B :** Where the unit prices are slightly affected by the job site condition but the work sequences are similar in every bridge. Thus average unit price of those for 20 bridges is considered as the standard unit price. These items are replacement to R.C. Slab, concrete wall lining and detour roads. In the calculation of the average, the extremely low or high unit prices due to special site condition are excluded.

**Category C :** Where the unit prices are mainly for functional rehabilitation works. Thus these unit prices are only applicable to the other bridges provided that rehabilitation method, the work scale and bridge type of the other bridge are the same as those in the original unit price.

**Category D :** Where the unit prices are considerably affected by the job site condition and size of the bridge member to be rehabilitated. These are for concrete pile lining and hydraulic rehabilitation works and are not applicable to other bridges without standardization of these unit prices.

#### **14.3.2 Standard Unit Price Analysis**

The unit prices under Category A are applicable to other bridges with the same rehabilitation methods and the average of the unit prices under Category B is also applicable to other bridges with the same rehabilitation methods.

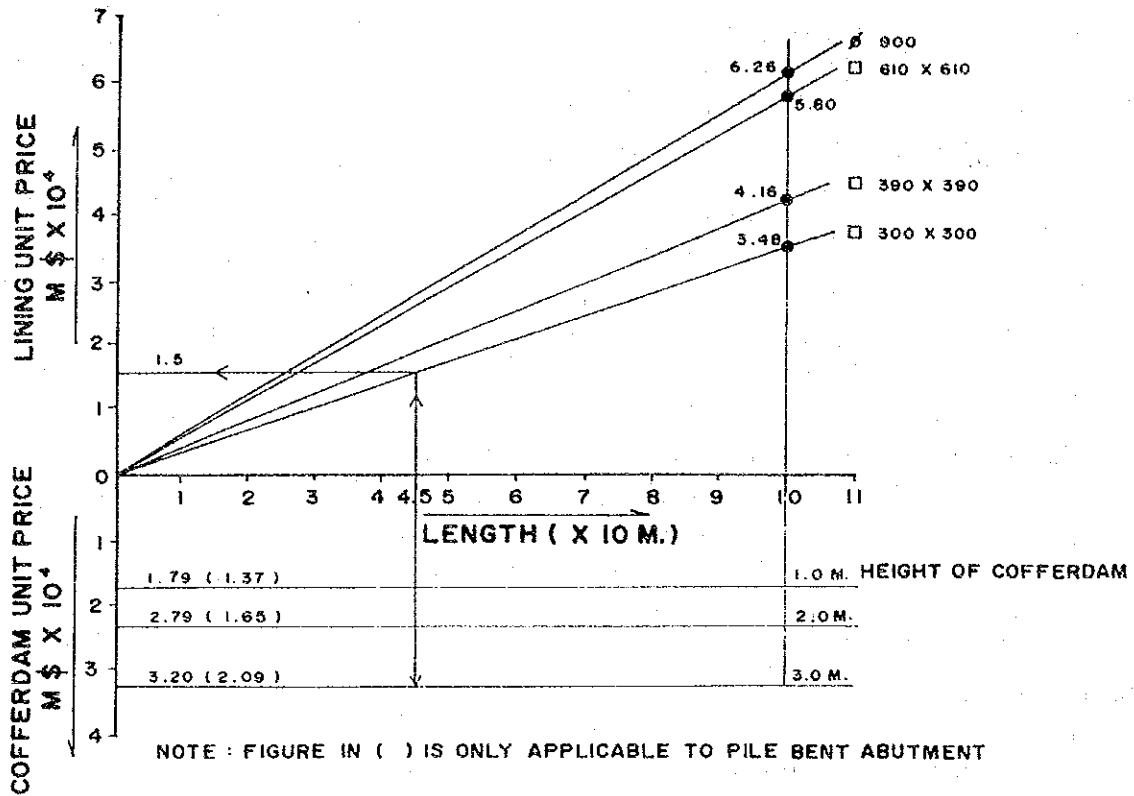
While the unit prices under Category C are only applicable to other bridges if conditions of the original estimate meet those of other bridges. If not, individual unit price analysis is required to estimate the cost of the bridge.

For the unit prices under Category D covering concrete pile lining and hydraulic rehabilitation plan, standardization of these unit prices is presented below;

##### **(1) Concrete pile lining**

Each unit price for the concrete lining which has been analyzed in the cost estimate for 20 bridges is plotted in a X-Y graph on a different pile size basis to derive a relationship between pile length and the construction cost with the cofferdam. After calibration by interpolation method, standard unit price of the pile lining for different size of piles was obtained as shown in Figure 14-3.

Figure 14-3 Relationship for Standard Unit Price of Concrete Pile Lining



## (2) Hydraulic Rehabilitation Plan

Hydraulic rehabilitation plan consists of slope protection, footing protection, river bed protection and river realignment and furthermore each plan has different work methods as mentioned in Chapter 12.

To this end, standard unit quantities of each rehabilitation method are firstly estimated referring to the standard design presented in section 12.4. Based on these quantities, standard unit price of each hydraulic rehabilitation method is estimated using the formula stated in 14.2.3.

Consequently, based on the above mentioned exercises, the standard unit price of the individual rehabilitation method which will be used to estimate the cost of the rest of the bridges is prepared and is summarized in Table 14-3.

Table 14-3 SUMMARY OF STANDARD UNIT PRICE FOR EACH REHABILITATION METHOD

	REHABILITATION ITEMS	UNIT	CATEGORY	UNIT PRICE (M\$)		REMARKS
				DIRECT COST (1)	PROJECT COST (2)	
Super - Structure	1. Guniting	m2	a	620.00	1040.00	Cement exceptionally developed for guniting
	2. Guniting with rebar	m2	a	760.00	1280.00	Cement exceptionally developed for guniting
	3. Patching (Type A)	m2	a	220.00	370.00	Depth < 25 mm
	4. Patching (Type B)	m2	d	270.00	450.00	25mm < depth < 50mm
	5. Prepacked lining	m2	a	2980.00	5010.00	Depth = 100mm
	6. Prepacked lining w/ rebar	m2	a	3160.00	5310.00	Depth = 100mm
	7. Epoxy injection (Type A)	m	a	120.00	200.00	Width = 0.2 ~ 0.6mm
	8. Epoxy injection (Type B)	m	a	140.00	230.00	Width = 0.6 ~ 3.0mm
	9. Protective coating	m2	a	32.40	54.40	Cleaning and 3-coatings (acrylic resin)
	10. Waterproof layer	m2	a	47.80	80.40	4-coatings(chloroprene) and pavement (50mm)
	11. Steel bonding plate	m2	a	930.00	1560.00	Grouting epoxy resin
	12. Repainting	m2	a	58.00	97.50	Blasting and 3-coatings (epoxy resin)
	13. Adding cross beam (steel)	t	a	4760.00	7990.00	Added at span center
	14. Attachment of steel plate	t	a	930.00	1570.00	Bolted cover plate
	15. Replacement to R.C Slab	m2	b	560.00	940.00	Buckle plate bridge 950\$/m2 * with additional girder
Sub - Structure	1. Epoxy injection (Type A)	m	a	120.00	200.00	Width = 0.2 ~ 0.6mm
	2. Epoxy injection (Type B)	m	a	140.00	230.00	Width = 0.6 ~ 3.0mm
	3. Protective Coating	m2	a	32.40	54.40	Cleaning and 3-coatings (acrylic resin)
	4. Concrete lining (Wall)	m2	b	190.00	320.00	Dry work, 1390\$/m2 * with cofferdam(H=1.0)
	5. Concrete lining (Column)					
	(a) 410 diameter (Steel Piles)	m	d	-	-	Refer to Fig 14-3
	(b) 300 x 300 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(c) 310 x 310 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(d) 360 x 360 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(e) 380 x 380 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(f) 390 x 390 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(g) 550 x 550 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(h) 610 x 610 (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	(i) 900 diameter (R.C Piles)	m	d	-	-	Refer to Fig 14-3
	6. Crosshead lining (prepacked)					
	(a) Prepacked lining + rebar	m2	a	3160.00	5310.00	Depth = 100 mm
	(b) Prepacked lining	m2	a	2980.00	5010.00	Depth = 100 mm
	7. Patching (Type A)	m2	a	220.00	370.00	Depth < 25 mm
	8. Patching (Type B)	m2	a	270.00	450.00	25 mm < Depth < 50 mm
	9. Replacement of Abut. by rigid frame.	m	c	8550.00	14390.00	Portal type pier, PC pile 500x500x41.0 m
Incidental Facilities	1. Extension drainage pipes.	No	a	390.00	660.00	Extension and binding with metal strip
	2. Water drop	m	a	93.40	160.00	Bonding strip (F R P)
	3. Expansion joints (Type A)	m	a	3020.00	5080.00	Rubber seal joint (Span > 10m)
	4. Expansion joints (Type B)	m	a	1190.00	2000.00	Blind type (Span < 10m)
Temporary Work	1. Detour road	m	b	590.00	1000.00	Embankment height 4.0 ~ 5.0m, 330\$/m2 for height 1.0m
	2. Temporary bridge	m	a	8780.00	11390.00	Steel frame with wooden deck
Scaffolding	1. Substructure -- ground support	m3	a	12.10	20.40	Prefabricated pipe support
	2. Superstructure -- "	m3	a	12.10	20.40	Prefabricated pipe support
	3. Superstructure -- Hanging	m2	a	21.30	35.90	Steel tube with wooden planks
Functional Rehabilitation	1. Adding sidewalk (Concrete)3	m2	c	1560.00	2620.00	B=2x2m, L=3@12.1m, 760\$/m2 * for Superstructure
	2. Adding sidewalk (steel)3	m2	c	1040.00	3260.00	B=2x2m, L=21.0m, 960\$/m2 * for Superstructure
	3. Widening carriageway	m2	c	2440.00	4110.00	B=2x3.55m, L=2@6.24m, 370\$/m2 * for Superstructure
	4. Raising grade	m2	c	1480.00	2490.00	B=9.11m, L=50.0m, Superstructure alone
Total Replacement		m2	c	1990.00	3340.00	B=12.6m, L=15.0m, 830\$/m2 * for Superstructure
River Training	1. Slope Protection					
	(a) Type A	m2	d	140.00	230.00	Height=3m, Slope 1:15 Stone Masonry
	(b) Type B	m2	d	200.00	340.00	Height=3m, Slope 1:1 Concrete Block Masonry
	(c) Type C	m2	d	140.00	240.00	Height=3m, Slope 1:2 Concrete Block Pitching
	(d) Type D	m2	d	120.00	200.00	Height=3m, Slope 1:2 Concrete Frame
	2. Foot Protection					
	(a) Type A	m2	d	16.40	27.50	Depth = 1m, Dumped Stone
	(b) Type B	m2	d	33.40	56.10	Depth = 1m, Gablon
	(c) Type C	m2	d	180.00	290.00	Depth = 1m, Concrete Block
	(d) Type D	m2	d	590.00	990.00	Depth = 2m, Sheet Pile alone
	3. Riverbed protection					
	(a) Type A	m2	d	260.00	440.00	Depth = 1m, Gablon
	(b) Type B	m2	d	320.00	540.00	Depth = 3m, Dumped Stone and Gablon
	4. River Alignment					
(a) Type A	m	d	310.00	510.00	Spur Dike High = 2.0m	
(b) Type B	m	d	320.00	530.00	Groyne by Concrete Pile Block	

Remark : Number 1) indicate the unit price for Direct Cost  
 2) indicates Unit Price for Project Cost (=1.68 x Direct Cost)  
 3) indicates that the width is the total width of walkway including curb  
 4) indicate the width of carriageway  
 Symbol (\*) indicates that the Unit Cost is Direct Cost

#### 14.4 Project Cost Estimate

In the supplemental bridge survey covering 199 bridges, rehabilitation methods for each bridge were identified and the corresponding work quantities were also estimated accordingly. The standard unit prices applicable to most of the rehabilitation methods of those bridges were analyzed in the previous section based on the assessment of the unit prices covering various rehabilitation methods for 20 bridges from the detailed survey.

However, some of these standard unit prices are not applicable to several rehabilitation methods such as widening of carriageways, adding sidewalks, raising bridge grade as well as total replacement, the unit price of which depends on the bridge type and configuration of the bridge. Those unit prices are individually estimated referring to breakdown of unit prices of similar rehabilitation methods.

The project cost of each bridge can be obtained by adding each rehabilitation work item amount assigned to the bridge, the amount was calculated by multiplying the estimated quantity of rehabilitation work by the corresponding standard unit price.

The project cost of each bridge is shown in Table 14-4 and the total project cost covering 205 bridges amounts to M\$58,148,268, while the cost breakdown of each bridge is attached in Appendix-R of Volume III.

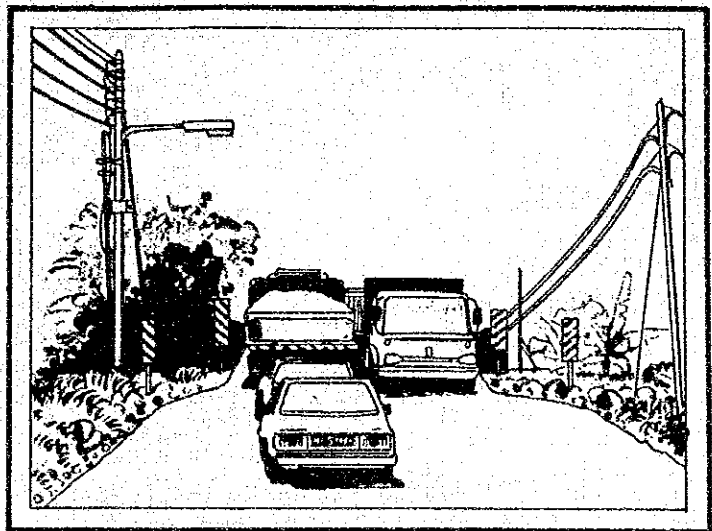


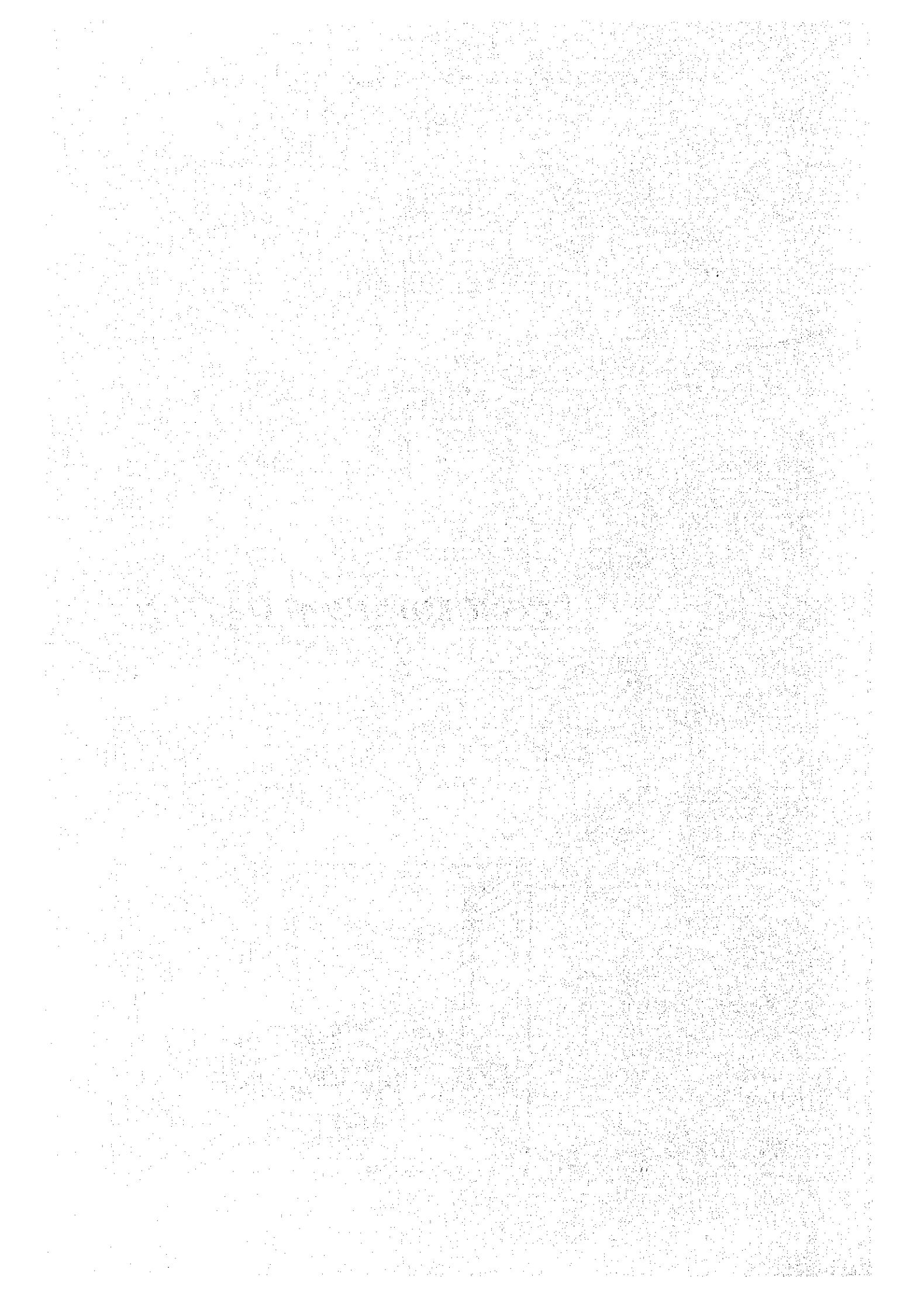




# CHAPTER 15

## ECONOMIC EVALUATION





# CHAPTER 15

## ECONOMIC EVALUATION

### 15.1 General

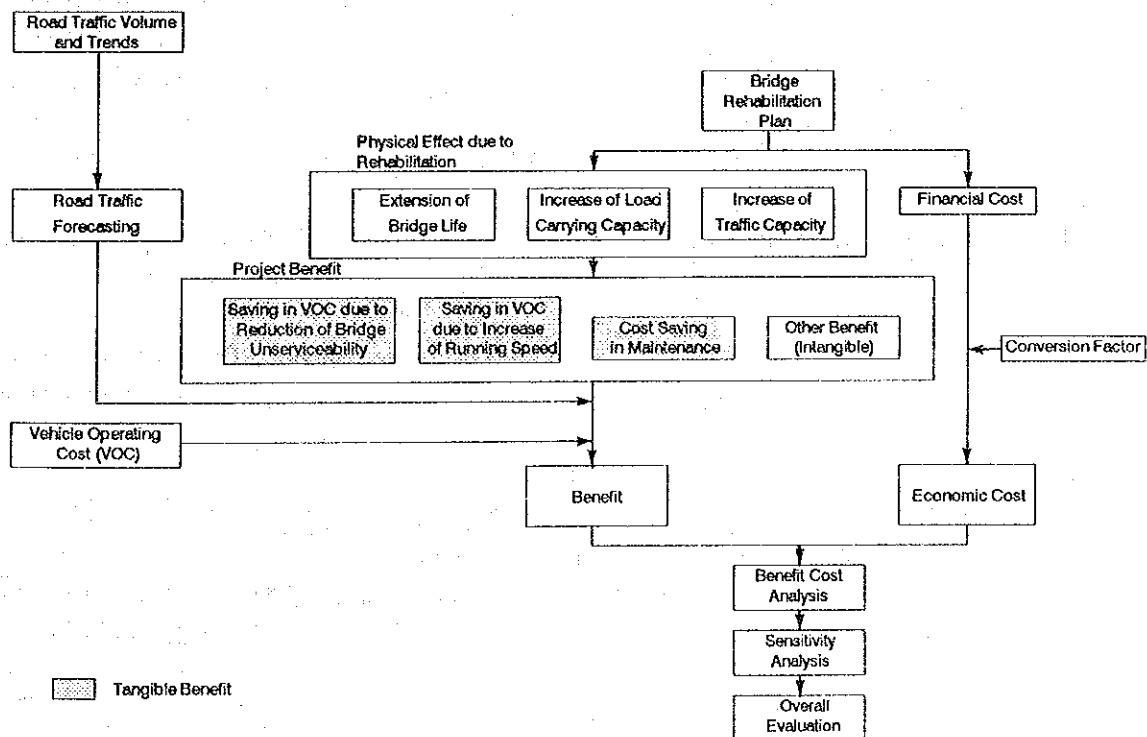
This chapter aims to evaluate the economic viability and to determine the priority of each of the 203 bridges out of 216 study bridges because 11 bridges have been replaced by the Government after this study started and two bridges have no rehabilitation work requirement.

The study methodology was determined based on the following principles:

- The method of evaluation shall be simplified due to the large number of bridges to be evaluated and to the relatively small cost of individual bridge rehabilitation work.
- Future traffic volume shall be estimated by utilizing existing traffic data to the maximum extent.
- The result of economic evaluation shall be easily reviewed by JKR corresponding to the possible changes in the future traffic situation.

In accordance with the above principles for the evaluation, the following procedure was taken as shown in Figure 15-1.

Figure 15-1 Flowchart of Economic Evaluation



- 1) Select a traffic count station corresponding to each bridge from the traffic census data.
- 2) Estimate the future traffic volume of each bridge using traffic growth rate included in the above.
- 3) List possible benefits derived from the proposed bridge rehabilitation work.
- 4) Formulate an evaluation model to quantify benefits included in the above.
- 5) Calculate economic benefits using future traffic volume and vehicle operating cost.
- 6) Convert financial cost into economic cost.
- 7) Work out economic evaluation by benefit cost analysis.
- 8) Conduct sensitivity analysis to test evaluation stability.
- 9) Judge project feasibility considering indirect aspects.

## 15.2 Traffic Projection

In general, the following three methods can be considered applicable in traffic projection:

- Four-step Model
- Model using Economic Indicators as Parameters
- Trends Model

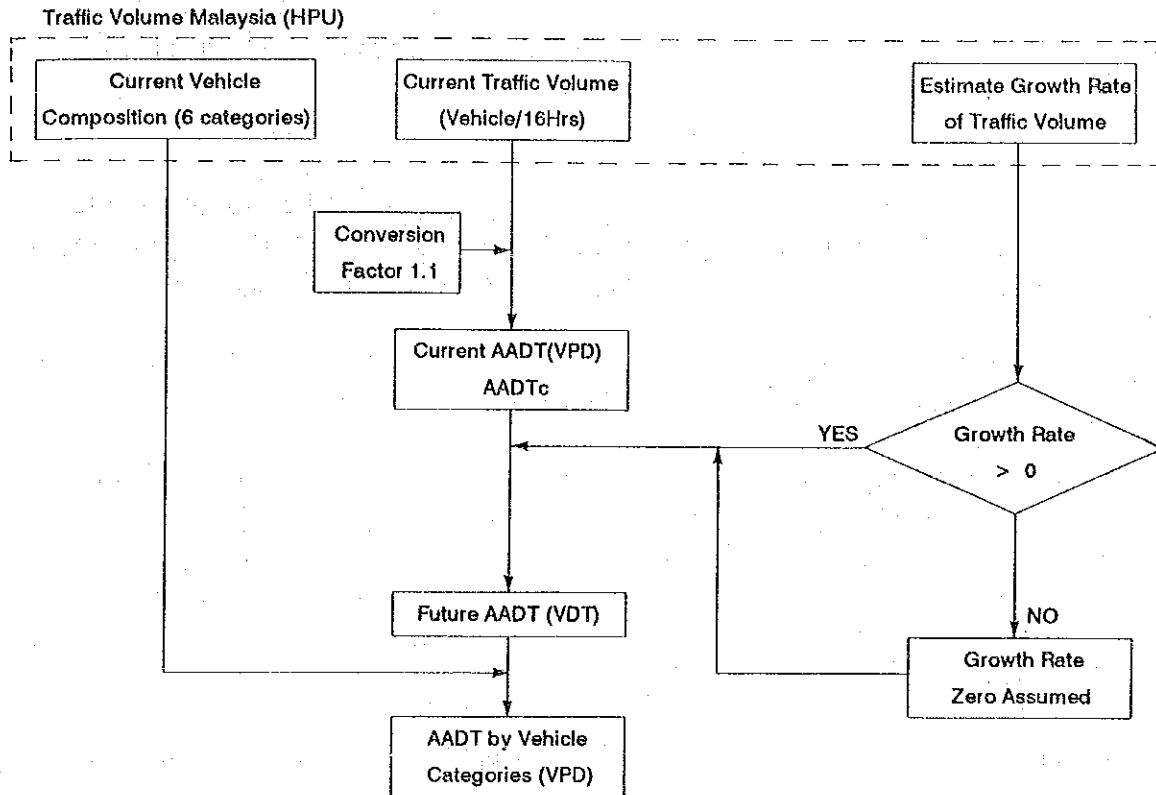
Considering that the study bridges are scattered widely over the country, traffic volume would have to be forecasted in a national scale. The four-step model or the model using economic indicators as parameters, however, would require a considerable amount of computation work and would produce relatively inaccurate results in terms of the traffic volume at bridge site. In Malaysia, traffic count data have been compiled for more than 450 stations in the Peninsular over past 10 years and, therefore, a simple and accurate forecast has become possible using the data as compared to the above stated models. The trends model based on the existing traffic data was adopted for traffic forecast in this study.

The procedure to forecast traffic using the trends model presented in Figure 15-2 is as follows:

- 1) Identify a traffic count station corresponding to each bridge.
- 2) Determine traffic volume (16 hours) by vehicle type and growth rate for each bridge.
- 3) Convert 16-hour traffic volume to daily value (VPD).
- 4) Set growth rate at zero if it is calculated negative.
- 5) Calculate future traffic volume using the growth rate.
- 6) Break down daily traffic volume into those by vehicle type using the present modal shares.

In this Study, "Traffic Volume Malaysia 1989" (HPU, Sept. 1989) was referred to for traffic count data. The data used to estimate future traffic volume for each bridge are shown in Appendix-S of Volume III.

**Figure 15-2 Flow of Traffic Projection**



### 15.3 Economic Costs

#### (1) Cost Components

The cost components to be considered in this study are:

- o **Rehabilitation Cost**

Investment cost in order to improve bridge durability and to enhance bridge functions.

- o **Maintenance Cost**

Continuously required cost in order to keep bridge serviceability after the rehabilitation.

#### (2) Conversion to Economic Costs

The economic cost of the project is normally calculated from the estimated financial cost by eliminating taxes and transfers and by applying, if necessary, the shadow prices in cost

component. In order to standardize the process, authorized conversion factors from financial to economic prices are published by the Economic Planning Unit (EPU), the Prime Minister's Department. In this study, these factors were taken.

Using the conversion factor, economic cost is derived according to the following formula:

$$\begin{array}{rcccl} \text{Economic Cost} & & \text{Conversion Factor} & & \text{Financial Cost} \\ \text{( EC )} & = & \text{( CF )} & \times & \text{( FC )} \end{array}$$

Since the conversion factor that can be directly applied to the bridge rehabilitation works was not shown in "National Parameters for Project Appraisal" (EPU, 1986), the average of the following works was adopted as the conversion factor in the study:

Reinforced Concrete Piling	0.78
Excavation and Embankment Building	0.80
Road Surfacing	0.87
Concrete Work for Civil Eng. Structure	0.77
<u>Structural Steel Work</u>	<u>0.78</u>
Average	0.80

### (3) Economic Costs

The rehabilitation cost estimated for all the bridges in 1991 prices is converted from financial to economic cost as follows:

Project Cost (Financial)	: M\$58,148,267
Conversion Factor	: 0.80
Project Cost (Economic)	: M\$46,518,614

The maintenance cost varies depending on natural and socio-economical conditions of the area where the bridge is located. In the absence, however, of the unit maintenance cost identified as standards in Malaysia, actual examples in the OECD countries were studied. According to the study results, annual maintenance cost ranges between 0.1 and 2.0 % of the initial construction cost of bridges. Hence, annual maintenance cost was assumed to be 1 % of the new bridge construction cost in this study. In addition, the standard construction cost of a new bridge in Malaysia is about M\$2,500 per squaremeter of deck in 1991 market prices.

Maintenance cost after rehabilitation proposed in this study was assumed as follows:

- i) **Reconstruction and widening :**  
2.5 % of rehabilitation cost for 5 years (0.5%/year)

- ii) **Reinforcement :**  
5.0 % of rehabilitation cost for 5 years (1.0%/year)
- iii) **Protection :**  
10.0 % of rehabilitation cost for 5 years (2.0%/year)

These maintenance costs are also subject to conversion.

## **15.4 Benefit Measurement**

### **15.4.1 Benefit due to Bridge Rehabilitation**

Benefits accrued from bridge rehabilitation are :

- User Benefit :** Benefit of bridge users and adjacent inhabitants directly brought by bridge rehabilitation.
- Social Benefit :** Indirect social benefit including stabilization of society and improvement of living environment.
- Supplier Benefit :** Benefit of bridge supplier/administrator including maintenance cost savings.

For each of the benefits above, there are tangible and intangible portions, as described below.

#### **(1) User Benefit**

- **Tangible Benefit**

Benefit of bridge users occurs mainly from the savings in vehicle operating cost. More concretely, the following benefits can be quantified:

- i) **Savings in vehicle operating cost due to a reduction in unservice durations of bridges.**

Improvement of bridge durability reduces number of days of bridge unservice (extends bridge life) and, therefore, saves vehicle operating cost required for detours.

- ii) **Savings in vehicle operating cost due to an increase of vehicle speed**

Bridge widening makes it possible for vehicles to keep running speed on and near the bridge.

- **Intangible Benefit**

- i) **Reduction of traffic accidents**

Bridge widening facilitates separation of vehicles and pedestrians. This reduces traffic accidents, though unquantifiable.

- ii) **Improvement in Traffic Security**

Improved service level increases punctuality and safety of traffic reducing idle time, though quantification is difficult.

**(2) Social Benefit and Supplier Benefit**

- **Tangible Benefit**

- i) **Maintenance Cost Savings**

The maintenance cost savings can be expected on the bridge administrator side. The benefit comes from the difference in maintenance cost between "with" and "without" project cases.

- **Intangible Benefits**

- i) **Access to Public Facilities**

Public facilities such as high schools, hospitals, recreational and social facilities are relatively fewer in the rural areas than in urban. However, people who live on the opposite side of the bridges leading to these public facilities will lose their access to this facilities, if the bridge fails.

- ii) **Improvement of Social Environment**

If a bridge is unusable, it is difficult for the military and police authorities to capture criminal elements. The rehabilitation of bridges may have an important contribution to mobility of these government authorities and assist in the improvement of social environment including peace and security.

- iii) **Access to Market**

Those who live on the opposite side of the unserviceable bridges may lose the opportunity to sell their agricultural products and to buy daily necessities or production inputs.



#### iv) Access to Development Program

Many rural development programs may be conducted to encourage greater production and improved lived conditions. The unserviceability of bridges may reduce benefits from these programs.

### 15.4.2 Model for Bridge Life and Unservice

In order to quantify the benefit of reducing unservice duration of bridges, a probabilistic model was introduced.

#### (1) Bridge Life

Usually it is said that a newly constructed bridge has a 40 to 80-year life span. For example, a 50-year life span is expected in Japan. This life span is mainly fitted for calculating depreciation and taxation and does not reflect the real bridge life span until a bridge is physically unusable. If a bridge is well maintained and repaired, its physical life span could be longer than 50 years. A bridge will be unserviceable because of social factors such as width or loading capacity for increased traffic volume and other administrative reasons.

Therefore, in the study, the bridge life span was defined as the bridge age by which a half of the bridges population would have been statistically unserviceable.

#### (2) Probability Model for Bridges to be Unusable

In the absence of statistical data as to bridge life in Malaysia, a study in Japan was referred to in this study.

"Statistical Analysis on Bridge Life" (H. Iizuka, JSCE, 1988) dealt with 4,377 bridges in Niigata Prefecture in Japan and obtained a reliability function  $R(t)$  based on reliability theory.

Using  $R(t)$ , the unreliability function  $F(t)$  can be expressed as  $F(t) = 1 - R(t)$ . Therefore, the "Unserviceability Probability Density"  $f(t)$  is:

$$f(t) = dF(t)/dt = -dR(t)/dt$$

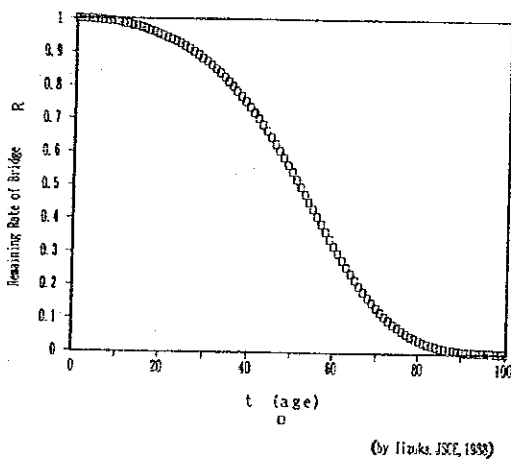
Using  $R(t)$  shown in Figure 15-3,  $f(t)$  can be calculated. As shown in Figure 15-4, "Unserviceability Probability Density"  $f(t)$  can be approximated by the "normal distribution" with an average of 50 years and a standard deviation of 16.7 years. Hence, bridge life applied in this study was determined at 50 years. The following equation shows unserviceability probability density for newly constructed bridges.

$$f(t) = \frac{1}{\sqrt{2\pi}\delta} e^{-(t-m)^2/2\delta^2} = N [m, \delta^2] = N [50, 16.7^2]$$

Hence, the cumulative probability that a newly constructed bridge becomes unserviceable by the year t is expressed as follows:

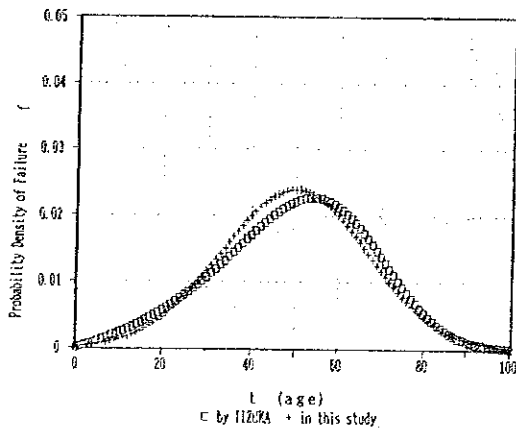
$$F(t) = \int_0^t f(t) dt$$

Figure 15-3 A Example of Reliability Function R(t) in Japan



t	R(t)	t	R(t)	t	R(t)	t	R(t)	t	R(t)
0	1.000	21	0.946	41	0.720	61	0.297	81	0.032
1	1.000	22	0.940	42	0.703	62	0.276	82	0.027
2	0.999	23	0.933	43	0.684	63	0.256	83	0.023
3	0.999	24	0.926	44	0.665	64	0.236	84	0.019
4	0.998	25	0.918	45	0.646	65	0.217	85	0.016
5	0.997	26	0.910	46	0.626	66	0.199	86	0.013
6	0.995	27	0.902	47	0.605	67	0.182	87	0.011
7	0.994	28	0.893	48	0.584	68	0.165	88	0.009
8	0.992	29	0.883	49	0.563	69	0.150	89	0.007
9	0.990	30	0.873	50	0.541	70	0.135	90	0.006
10	0.987	31	0.862	51	0.519	71	0.121	91	0.005
11	0.985	32	0.851	52	0.497	72	0.108	92	0.004
12	0.982	33	0.839	53	0.474	73	0.096	93	0.003
13	0.978	34	0.826	54	0.452	74	0.085	94	0.002
14	0.975	35	0.813	55	0.429	75	0.075	95	0.002
15	0.971	36	0.799	56	0.406	76	0.066	96	0.001
16	0.967	37	0.785	57	0.384	77	0.058	97	0.001
17	0.962	38	0.770	58	0.362	78	0.050	98	0.001
18	0.957	39	0.754	59	0.340	79	0.043	99	0.001
19	0.952	40	0.737	60	0.318	80	0.037	100	0.001

Figure 15-4 Probability Density of Bridge Unserviceability



t	f(t)	t	f(t)	t	f(t)	t	f(t)	t	f(t)
Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study	Iizuka Study
0	0.000	21	0.001	41	0.018	61	0.021	81	0.005
1	0.000	22	0.001	42	0.018	62	0.020	82	0.004
2	0.001	23	0.001	43	0.019	63	0.020	83	0.004
3	0.001	24	0.001	44	0.019	64	0.019	84	0.003
4	0.001	25	0.001	45	0.020	65	0.018	85	0.003
5	0.001	26	0.001	46	0.021	66	0.017	86	0.002
6	0.001	27	0.001	47	0.021	67	0.016	87	0.002
7	0.002	28	0.001	48	0.021	68	0.016	88	0.002
8	0.002	29	0.001	49	0.022	69	0.015	89	0.001
9	0.002	30	0.001	50	0.022	70	0.014	90	0.001
10	0.002	31	0.001	51	0.022	71	0.013	91	0.001
11	0.003	32	0.002	52	0.023	72	0.012	92	0.001
12	0.003	33	0.002	53	0.023	73	0.011	93	0.001
13	0.004	34	0.002	54	0.023	74	0.010	94	0.001
14	0.004	35	0.002	55	0.023	75	0.009	95	0.001
15	0.004	36	0.002	56	0.022	76	0.008	96	0.001
16	0.004	37	0.002	57	0.022	77	0.008	97	0.000
17	0.005	38	0.002	58	0.022	78	0.007	98	0.000
18	0.005	39	0.002	59	0.022	79	0.006	99	0.000
19	0.006	40	0.002	60	0.021	80	0.005	100	0.000

### (3) Probability Model for Bridges to be Unusable after Rehabilitation

In order to quantify the benefit of rehabilitation works, a function for bridge unavailability probability density after rehabilitation should be assumed.

#### • Protection

Protection work is defined to be the work to maintain the design load-carrying capacity of the bridge and to ensure the safety of road users. This effect was assumed based on an engineering study and it found that the probability for a bridge to be unusable will be kept constant over 5 years at the same service level as that at the time protection work completed. In formula :

General formula of probability

$$f_0(t) = N [50, 16.7^2]$$

For a bridge of age  $n$ , this can be modified to the following since the bridge is existing without falling down in spite of the probability of falling down in the past:

$$f_1(t) = k_1 \cdot f_0(t) \quad (t > n)$$

$$\text{where: } k_1 = 1 / (1.0 - F_0(t < n))$$

$$F_0(t < n) = \int_0^n f_0(t) dt$$

The effect of protection was assumed to extend the bridge durability over 5 years.

Probability after protection

$$f_2(t) = k_2 \cdot f_1(n) \quad (n < t < n+5)$$

$$f_2(t) = k_2 \cdot f_1(t-5) \quad (t > n+5)$$

$$\text{where: } k_2 = 1 / (5 \cdot f_1(n) + F_1(t > n))$$

$$F_1(t > n) = \int_n^\infty f_1(t) dt$$

#### • Reinforcement

Reinforcement work is defined to be the work that enables a bridge to carry heavier loads by strengthening the structure. The effect of bridge reinforcement is to increase bridge durability and to extend bridge life. In this study, the following assumption was adopted:

- The probability for a bridge to be unusable will be kept constant over 5 years at the same level as that at the time work completed. This is the same assumption as "protection work".
- Bridge loading capacity will be improved by 20 %. Hence the probability for a bridge to be unusable will be reduced as much as 80 %.

In formula:

Probability of bridge durability rehabilitated

$$f_3(t) = N [m, \delta^2] = N [50, 20.9^2]$$

Cumulative probability

$$F_3(t) = \int_0^t f_3(t) dt$$

Assuming new age  $n_3$  from the following:

$$F_3(n_3) = F_0(n)$$

Therefore, the bridge gains an extended life of  $(n-n_3)$  years by reinforcement.

Probability after reinforcement:

$$\begin{aligned} f_4(t) &= k_4 \cdot f_3(n_3) && (n < t < n+5) \\ f_4(t) &= k_4 \cdot f_3(t-5-(n-n_3)) && (t > n+5) \end{aligned}$$

$$\text{Where, } k_4 = 1 / (5 \cdot f_3(n_3) + F_3(t > n_3))$$

#### • Reconstruction

The age of a bridge becomes zero upon reconstruction.

In formula:

Probability after reconstruction

$$f_5(t) = f_0(t-n) = N [50+n, 16.7^2]$$

Figure 15-5 presents schematically the effects of rehabilitation works in terms of probability for bridges to be unusable for ages 20, 30, 40 and 45 years.

Figure 15-5 Probability Density of Bridge Unserviceability after Rehabilitation

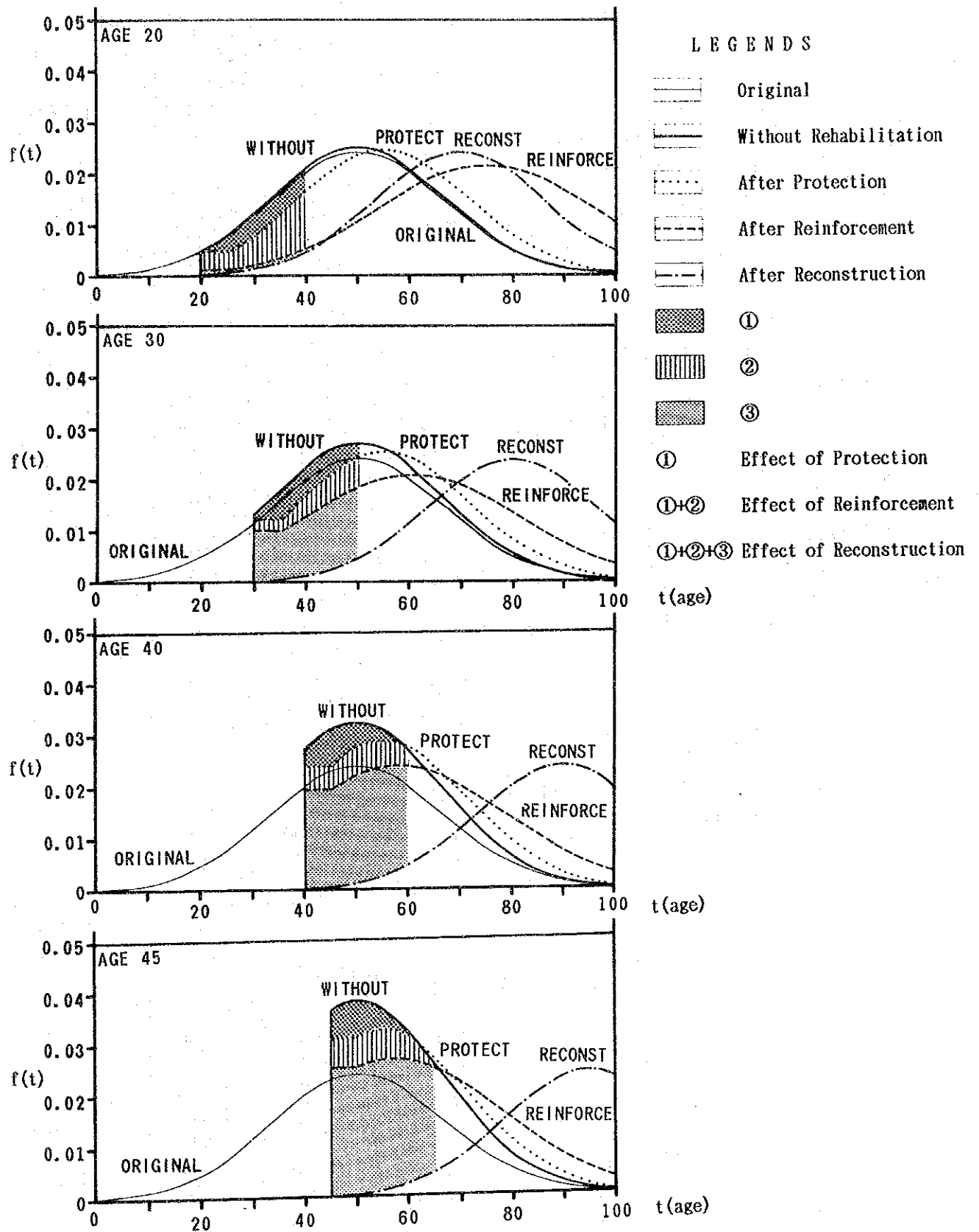


Table 15-1 Probability Density of Bridge Unserviceability after Rehabilitation

EQUIVALENT AGE = 20

	Original	Without	Protect	Reinforce	Reconst
20	0.476%	0.493%	0.482%	0.122%	0.027%
21	0.523%	0.548%	0.482%	0.122%	0.032%
22	0.586%	0.607%	0.482%	0.122%	0.038%
23	0.647%	0.670%	0.482%	0.122%	0.046%
24	0.711%	0.737%	0.482%	0.122%	0.054%
25	0.779%	0.807%	0.482%	0.122%	0.063%
26	0.851%	0.882%	0.536%	0.137%	0.074%
27	0.925%	0.959%	0.594%	0.153%	0.087%
28	1.003%	1.040%	0.655%	0.171%	0.101%
29	1.083%	1.123%	0.721%	0.190%	0.117%
30	1.166%	1.209%	0.790%	0.211%	0.136%
31	1.251%	1.296%	0.862%	0.234%	0.156%
32	1.336%	1.385%	0.938%	0.258%	0.179%
33	1.423%	1.475%	1.017%	0.285%	0.205%
34	1.510%	1.565%	1.098%	0.313%	0.234%
35	1.596%	1.654%	1.182%	0.344%	0.266%
36	1.681%	1.742%	1.267%	0.376%	0.301%
37	1.764%	1.829%	1.354%	0.411%	0.339%
38	1.845%	1.913%	1.442%	0.448%	0.381%
39	1.923%	1.993%	1.530%	0.487%	0.427%
40	1.997%	2.069%	1.617%	0.528%	0.476%
41	2.066%	2.141%	1.704%	0.572%	0.529%

EQUIVALENT AGE = 30

	Original	Without	Protect	Reinforce	Reconst
30	1.166%	1.312%	1.234%	1.001%	0.027%
31	1.251%	1.407%	1.234%	1.001%	0.032%
32	1.336%	1.503%	1.234%	1.001%	0.038%
33	1.423%	1.601%	1.234%	1.001%	0.046%
34	1.510%	1.698%	1.234%	1.001%	0.054%
35	1.596%	1.795%	1.234%	1.001%	0.063%
36	1.681%	1.891%	1.323%	1.060%	0.074%
37	1.764%	1.985%	1.414%	1.119%	0.087%
38	1.845%	2.076%	1.505%	1.180%	0.101%
39	1.923%	2.163%	1.597%	1.241%	0.117%
40	1.997%	2.246%	1.688%	1.302%	0.136%
41	2.066%	2.324%	1.778%	1.362%	0.156%
42	2.130%	2.396%	1.867%	1.423%	0.179%
43	2.188%	2.461%	1.952%	1.482%	0.205%
44	2.240%	2.519%	2.034%	1.541%	0.234%
45	2.284%	2.569%	2.112%	1.598%	0.266%
46	2.321%	2.611%	2.185%	1.654%	0.301%
47	2.351%	2.644%	2.253%	1.707%	0.339%
48	2.372%	2.668%	2.315%	1.759%	0.381%
49	2.385%	2.682%	2.369%	1.807%	0.427%
50	2.389%	2.687%	2.416%	1.853%	0.476%
51	2.385%	2.682%	2.456%	1.896%	0.529%

EQUIVALENT AGE = 40

	Original	Without	Protect	Reinforce	Reconst
40	1.997%	2.720%	2.400%	1.953%	0.027%
41	2.066%	2.815%	2.400%	1.953%	0.032%
42	2.130%	2.902%	2.400%	1.953%	0.038%
43	2.188%	2.981%	2.400%	1.953%	0.046%
44	2.240%	3.051%	2.400%	1.953%	0.054%
45	2.284%	3.112%	2.400%	1.953%	0.063%
46	2.321%	3.162%	2.483%	2.010%	0.074%
47	2.351%	3.202%	2.560%	2.065%	0.087%
48	2.372%	3.231%	2.630%	2.117%	0.101%
49	2.385%	3.249%	2.692%	2.164%	0.117%
50	2.389%	3.254%	2.746%	2.208%	0.136%
51	2.385%	3.249%	2.790%	2.247%	0.156%
52	2.372%	3.231%	2.826%	2.282%	0.179%
53	2.351%	3.202%	2.851%	2.312%	0.205%
54	2.321%	3.162%	2.866%	2.337%	0.234%
55	2.284%	3.112%	2.872%	2.357%	0.266%
56	2.240%	3.051%	2.866%	2.372%	0.301%
57	2.188%	2.981%	2.851%	2.381%	0.339%
58	2.130%	2.902%	2.826%	2.385%	0.381%
59	2.066%	2.815%	2.790%	2.384%	0.427%
60	1.997%	2.720%	2.746%	2.377%	0.476%
61	1.923%	2.620%	2.692%	2.364%	0.529%

EQUIVALENT AGE = 45

	Original	Without	Protect	Reinforce	Reconst
45	2.284%	3.638%	3.086%	2.536%	0.027%
46	2.321%	3.697%	3.086%	2.536%	0.032%
47	2.351%	3.744%	3.086%	2.536%	0.038%
48	2.372%	3.778%	3.086%	2.536%	0.046%
49	2.385%	3.798%	3.086%	2.536%	0.054%
50	2.389%	3.805%	3.086%	2.536%	0.063%
51	2.385%	3.798%	3.137%	2.574%	0.074%
52	2.372%	3.778%	3.176%	2.607%	0.087%
53	2.351%	3.744%	3.205%	2.634%	0.101%
54	2.321%	3.697%	3.222%	2.655%	0.117%
55	2.284%	3.638%	3.228%	2.671%	0.136%
56	2.240%	3.567%	3.222%	2.680%	0.156%
57	2.188%	3.485%	3.205%	2.683%	0.179%
58	2.130%	3.393%	3.176%	2.680%	0.205%
59	2.066%	3.291%	3.137%	2.671%	0.234%
60	1.997%	3.180%	3.086%	2.656%	0.266%
61	1.923%	3.063%	3.026%	2.635%	0.301%
62	1.845%	2.939%	2.956%	2.608%	0.339%
63	1.764%	2.810%	2.878%	2.575%	0.381%
64	1.681%	2.678%	2.792%	2.537%	0.427%
65	1.596%	2.542%	2.698%	2.494%	0.476%
66	1.510%	2.405%	2.598%	2.446%	0.529%

### 15.4.3 Vehicle Operating Costs (VOC)

The vehicle operating cost plays an essential role in estimating road user savings. This consists of 1) running cost and 2) time cost. NALS developed the evaluation model for vehicle operating cost in Malaysia. In this study, the model (Econom V5.1) was adopted with the following minor modifications:

#### 1) Updated VOC

Using the consumer price indices by sector, VOC was updated to the 1991 level.

#### 2) VOC of Heavy Lorries

Although NALS calculated VOC separately for 3-axle lorry and 4-axle lorry, the traffic count data does not discriminate between these categories. Hence, in this study an average VOC was calculated for "heavy lorries" assuming a combination of 85 % 3-axle and 15 % 4-axle lorries.

#### 3) Vehicle Running Speed

In detour routes which are required when a bridge is unusable, the slowest "flow group" in the model "Econom" was assumed while the fastest "flow group" was adapted on regular routes. In addition, the running speed on bridges before widening was assumed to be half of that on regular routes.

**Table 15-2 Vehicle Running Speed assumed for Estimation of VOC**

(km/hr)

Vehicle Type	Cars	Light	Medium (2 Ax)	Heavy 1 (3 Ax)	Heavy 2 (4 Ax)
Regular Route	74.38	66.04	55.70	40.91	42.97
Detour Route	54.68	49.89	43.68	35.20	36.34
Before Widening	37.19	33.02	27.85	20.45	21.49

**Table 15-3 Vehicle Operating Costs by Vehicle Type**

(M\$/km)

Vehicle Type	Motor-cycles	Cars & Taxis	Buses	S.Vans & Utilities	Medium Lorries	Heavy Lorries
Regular Route	0.046	0.184	1.517	0.498	0.785	1.059
Detour Route	0.055	0.220	1.859	0.632	0.911	1.147
Before Widening	0.066	0.266	2.761	0.867	1.248	1.579

#### 15.4.4 Benefit Measurement

##### (1) Equivalent Age of Bridge

Residual life of a bridge differs by structure, traffic volume, geography, present structural conditions and other factors even if physical age is the same. In order to assess residual life of a bridge, a concept of "equivalent age of bridge" was introduced in relation to unserviceability probability based on overall rating from a safety viewpoint, traffic volume and year built. It can be defined as a normalized age on the unserviceability probability density function with an average of 50 years. Hence:

$$\text{Residual Life of Bridge} = \frac{\text{Life of Newly Constructed Bridge (50 years)}}{\text{Equivalent Age of Bridge}}$$

**Table 15-4 Assumed Equivalent Age of Bridge**

Overall Rating From Safety viewpoint (R)	Traffic Volume (AADT : Vehicle /day)	Year Built	
		Before 1945	After 1945
4.0 ≤ R	all	45	40
3.5 ≤ R < 4.0	AADT ≥ 9,000	45	40
	AADT ≤ 9,000	40	30
R < 3.5	AADT ≥ 9,000	40	30
	AADT ≤ 9,000	30	20

##### (2) Unservice Duration of Bridges

In order to estimate the unservice duration of bridges, the number of months required for bridge construction was assumed as a function of bridge length as follows:

$$\log (M) = 0.572 \log (L) + 0.043$$

where, M : Standard number of months required for bridge construction  
L : Bridge Length (m)

Using "M", number of days of bridge unservice is derived as follows:

$$d = f \times M \times (365/12)$$

where, f : probability for a bridge to be unusable  
d : number of days for a bridge to be unusable



### (3) Equation for Benefit Calculation

The equations developed for calculating benefits are based on two options, namely 1) bridge rehabilitation (with project) and 2) do nothing (without project).

#### • Protection

$$B1 = \sum_x \sum_i (f_o(x) - f_{wp}(x)) \cdot DBU \cdot AADTx_i \cdot (DL_o \cdot VOC_{io} - DL_w \cdot VOC_{iw})$$

where, B1 : Benefit from Protection (\$)  
f<sub>o</sub>(x) : Probability for a bridge to be unusable in year x without rehabilitation  
f<sub>wp</sub>(x) : Probability for a bridge to be unusable in year x after protection  
DBU : Number of days required for bridge construction (day)  
AADTx<sub>i</sub> : Average Annual Daily Traffic of Vehicle type i in year x (vehicles/day)  
DL<sub>o</sub> : Length of detour route (km)  
DL<sub>w</sub> : Length of regular route (km)  
VOC<sub>io</sub> : Vehicle Operating Cost of Vehicle type i in detour route (\$/km)  
VOC<sub>iw</sub> : Vehicle Operating Cost of Vehicle type i in regular route (\$/km)

#### • Reinforcement

$$B2 = \sum_x \sum_i (f_o(x) - f_{wr}(x)) \cdot DBU \cdot AADTx_i \cdot (DL_o \cdot VOC_{io} - DL_w \cdot VOC_{iw})$$

where, B2 : Benefit from Reinforcement (\$)  
f<sub>wr</sub>(x) : Probability for a bridge to be unusable in year x after reinforcement

#### • Reconstruction

$$B3 = \sum_x \sum_i (f_o(x) - f_{wc}(x)) \cdot DBU \cdot AADTx_i \cdot (DL_o \cdot VOC_{io} - DL_w \cdot VOC_{iw})$$

where, B3 : Benefit from reconstruction (\$)  
f<sub>wc</sub>(x) : Probability for a bridge to be unusable in year x after reconstruction

#### • Widening

$$B4 = \sum_x \sum_i (AADTx_i \cdot (BL+200)/1000 \cdot (VOC_{io} - VOC_{iw}))$$

where, B4 : Benefit from widening (\$)  
 BL : Length of bridge (m)

• **Cost Saving in Maintenance**

$$B5 = \sum_i (BL \cdot BW \cdot UCC \cdot Pm)$$

where, B5 : Benefit from cost saving in maintenance (\$)  
 BW : Width of bridge (m)  
 UCC : Unit cost of bridge construction (\$/m<sup>2</sup>)  
 Pm : Rate of annual maintenance cost against to initial construction cost (0.01)

Input data for benefit calculations are shown in Appendix-S1 for traffic volume and in Appendix-S2 for dimensions, rehabilitation methods and estimated costs of bridges. Appendix-S4 shows examples of benefit calculation for 4 bridges of different type in addition to the calculation examples of economic evaluation.

**15.5 Economic Evaluation**

**15.5.1 Parameters of Economic Evaluation**

Economic evaluation quantifies cost and benefit and assesses both in comparison. This is usually called "Benefit Cost Analysis". In this analysis, the following three parameters are generally used for decision making and for determining priority:

i) **Benefit Cost Ratio (BCR)**

This ratio is calculated by dividing benefit by cost in terms of net present value:

$$BCR = \left( \sum_t Bt / (1+i)^t \right) / \left( \sum_t Ct / (1+i)^t \right)$$

where, Bt : Benefit in year t  
 Ct : Cost in year t  
 i : Discount rate

This quantifies the magnitude of net present benefit per net present cost.

ii) **Net Present Value (NPV)**

Unlike benefit cost ratio, this quantifies the magnitude of net present benefit less net present cost.

$$NPV = \left( \sum_t Bt / (1+i)^t \right) - \left( \sum_t Ct / (1+i)^t \right)$$

### iii) Internal Rate of Return (IRR)

This rate is defined to be a discount rate or an interest rate where net present value of the project becomes zero. In other words, this is the highest interest rate that makes the project economically feasible.

$$\text{When } \sum_t (Bt - Ct) / (1+i)^t = 0$$

This "i" is called "Internal Rate of Return".

### 15.5.2 Discount Rate and Project Life

Discount rate is a parameter to calculate net present value of benefit and cost and is also a criterion to judge project feasibility by comparing it with the internal rate of return of the project. It is, in general, a ratio between present value of goods and future value of the same goods after one year; value of a goods obtained in the future should be less than the value of the same goods readily available at present.

In 1984, the Economic Planning Unit proposed a discount rate of 13 % per annum. Due, however, to the recently achieved economic development, a discount rate of 11 % per annum was taken in this study.

The project life over which economic evaluation is carried out was determined at 20 years starting from 1994, due to the following reasons:

- i) Life of bridge materials replaceable or repairable including deck slab that directly receives vehicle load is considered to be about 20 years.
- ii) In the calculation using discount rate, the values becomes negligible in comparison with the possible errors included in the accumulated cost after 20 years.
- iii) Accuracy of the traffic forecast is considered to be no more than 20 years.

### 15.5.3 Economic Evaluation Results

Calculation examples are shown in Appendix-S4. The results of the benefit cost analysis for the 203 bridges to be rehabilitated are as follows:

## (1) Entire Project

The flow of cost and benefit of the project as a whole is shown in Table 15-5.

**Table 15-5 Flow of Total Cost and Benefit**

(Unit: 1,000M\$)

	Cost Flow			Benefit									
	Project Cost	Maintenance Cost	Cost Flow	Protection, Reinforcement & Reconstruction						Widening	Maintenance (without Flow)	Benefit Flow	
				cars	small vamed.	lorrhvy.	lorr	buses	M cycle				
1994	46,519	0	46,519	0	0	0	0	0	0	0	0	564	564
1995	0	0	0	3,760	3,434	2,837	2,082	2,022	738	8	508	15,388	
1996	0	0	0	4,583	4,144	3,400	2,552	2,452	899	7	458	18,495	
1997	0	0	0	5,300	4,766	3,885	2,961	2,824	1,040	7	412	21,196	
1998	0	0	0	5,909	5,299	4,293	3,311	3,137	1,161	6	371	23,488	
1999	0	1,469	1,469	6,411	5,741	4,624	3,602	3,394	1,262	6	335	25,373	
2000	0	0	0	6,171	5,553	4,440	3,470	3,258	1,216	5	301	24,414	
2001	0	0	0	5,902	5,337	4,238	3,323	3,108	1,164	5	272	23,348	
2002	0	0	0	5,608	5,097	4,021	3,163	2,947	1,107	5	245	22,193	
2003	0	0	0	5,294	4,838	3,793	2,993	2,776	1,045	4	220	20,965	
2004	0	872	872	4,966	4,563	3,556	2,815	2,599	980	4	199	19,682	
2005	0	0	0	4,627	4,277	3,313	2,632	2,418	913	4	179	18,362	
2006	0	0	0	4,282	3,983	3,067	2,445	2,234	844	3	161	17,020	
2007	0	0	0	3,935	3,685	2,821	2,258	2,051	774	3	145	15,673	
2008	0	0	0	3,591	3,387	2,578	2,071	1,869	705	3	131	14,334	
2009	0	517	517	3,252	3,091	2,340	1,886	1,691	637	3	118	13,018	
2010	0	0	0	2,922	2,801	2,108	1,706	1,518	571	2	106	11,735	
2011	0	0	0	2,604	2,519	1,885	1,532	1,352	507	2	96	10,497	
2012	0	0	0	2,300	2,248	1,672	1,365	1,194	446	2	86	9,312	
2013	0	0	0	2,012	1,988	1,470	1,205	1,044	389	2	78	8,188	
	46,519	2,859	49,377	83,428	76,753	60,341	47,371	43,887	16,401	80	4,984	333,245	
	94.2%	5.8%	100.0%	25.0%	23.0%	18.1%	14.2%	13.2%	4.9%	0.0%	1.5%	100.0%	

$$NPV = 333,245 - 49,377 = 283,868 \text{ (x MS1000)}$$

$$BCR = 333,245 / 49,377 = 6.75$$

$$IRR = 57.84 \%$$

As a whole, most of the benefit is derived from the reduction of the duration of bridge unservice. The benefit is large enough as compared to the cost with a benefit cost ratio of 6.75 and an internal rate of return of 58 %. Hence, the project is considered to be feasible as a whole.

## (2) Individual Bridge

The result of economic evaluations for individual bridges is shown in Table 15-6. Appendix-S3 also shows the same result according to the value order of internal rate of return.

The internal rate of return exceeds 11 % for 197 bridges out of 203. For the remaining 6 bridges, it was also observed that the benefit grew by retarding rehabilitation for 5 years and that the internal rate of return was improved; 3 bridges became feasible with an internal rate of return of more than 11 % and other 3 bridges gained an internal rate of return of at least 6 %.

The project is considered to be carried out for all the bridges including the above 6 for the following reasons:







- i) The entire project is considered to be fully justified because 97 % bridges are feasible for rehabilitation in 1994 and 99 % in 1999.
- ii) For the 3 bridges with an IRR of less than 11 % for rehabilitation in 1999, intangible benefits not included in the calculation in this study can be added due to the service level and reliability of nation-wide road network improved by the proposed rehabilitation.

Figure 15-6 shows the distribution of IRR as classified into 4 categories by bridge. In general, the IRRs on the same routes are similar over a 100 kilometer section, presumably due to the same bridge type, structural condition, maintenance history and similar traffic volume and detour route condition.

In this connection, if the implementation program is formulated only taking into account the values of IRR, bridge rehabilitation works will be concentrated on the same route. Hence, the following adverse effects will arise consequently.

- i) Traffic flows in and around a particular route will be largely hindered.
- ii) Work capacity of local contractors available will be exceeded.
- iii) JKR State and District engineers who manage and supervise the project may be in shortage.

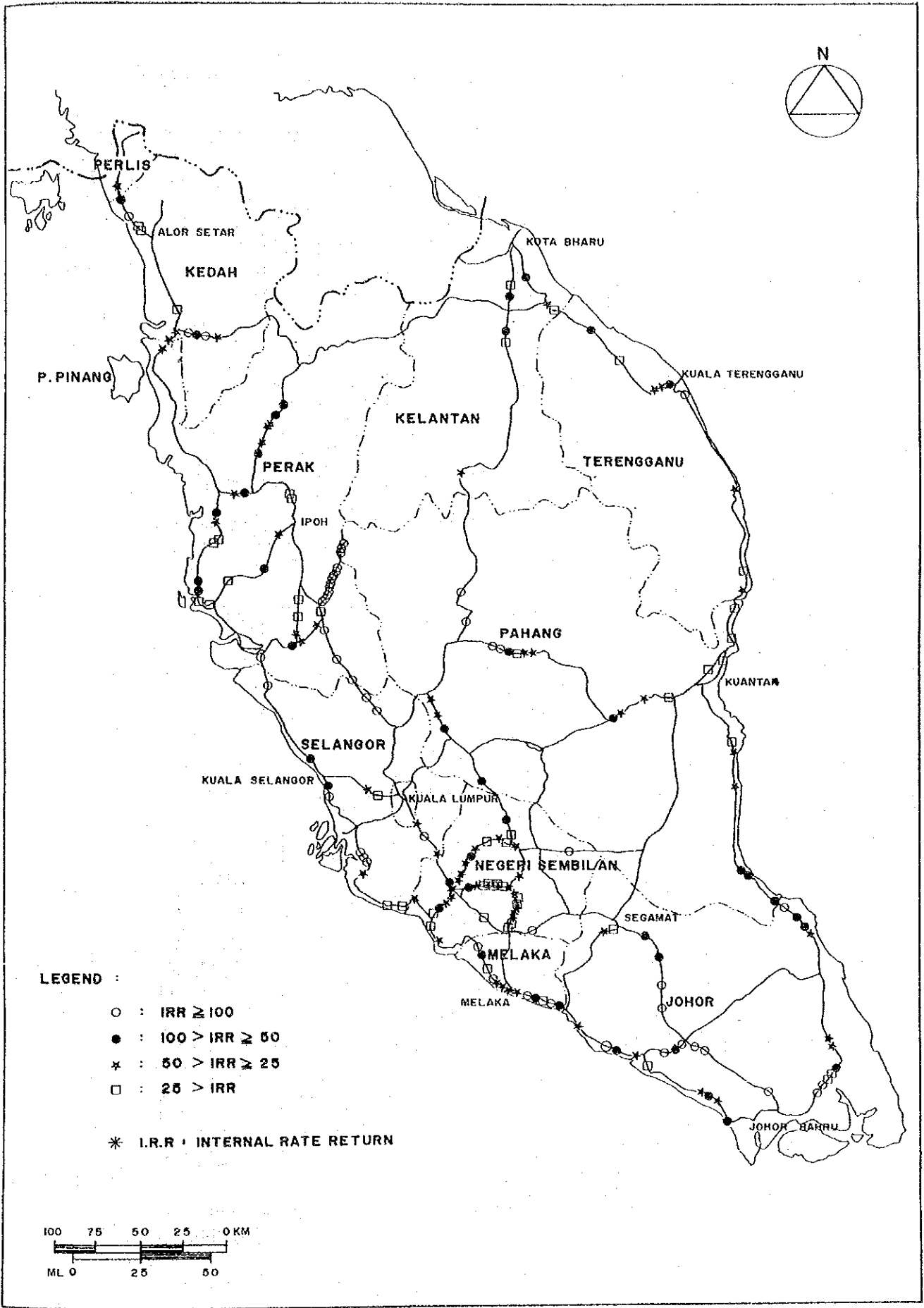
### (3) By State

Economic evaluation results were compiled by State as shown in Table 15-7. Figure 15-7 presents IRR with project cost by State. The IRR by state ranges between 26 and 99 %, indicating that the project packaged by State is also feasible. The reason for the highest IRR of 99 % in the Perak State is the lack of detour route in case the federal road No.59 becomes impassable. The Pahang State also has a high IRR at 82 % due to the low density of arterial roads. For the Trengganu State, the lowest IRR at 26 % can be attributed to 2 bridges that need reconstruction and large-scale rehabilitation of 10 % and 20 % respectively of the project cost.

The project cost by State is 35 % for Trengganu, 15 % for Pahang, 12 % for Perak and Johore. The Perlis State shares only 0.1 %.

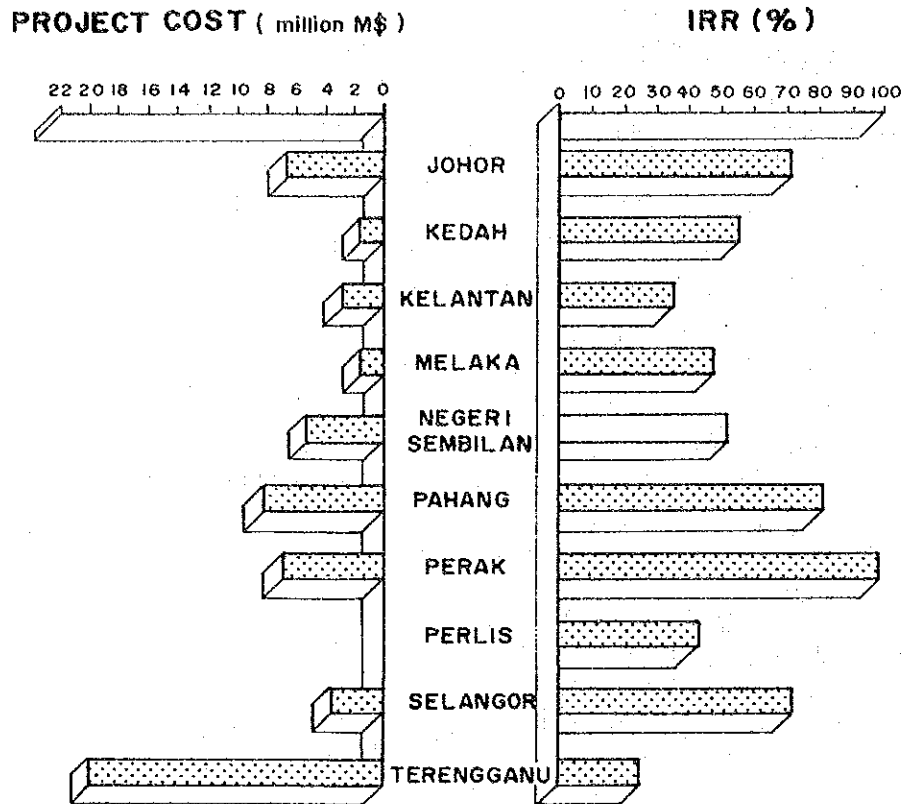
When the implementation program is made only based on the priority by State thus determined, a number of problems as described above might occur considering the large difference in IRR and project cost by State. This might also cause a problem in budget allocation system which needs to be stable and constant by State.





**Figure 15-6 IRR Distribution for Individual Bridges**

**Figure 15-7 IRR and Project Cost by State**



**Table 15-7 IRR and Project Cost by State**

STATE	NPV	BCR	IRR	PROJECT COST
Johor	44,324,310	8.4	72.90 %	6,827,790
Kedah	7,246,862	6.0	57.01 %	1,671,516
Kelantan	6,375,410	3.6	36.43 %	2,882,385
Melaka	5,663,845	5.1	48.94 %	1,625,577
N.Sembilan	22,787,010	5.9	53.01 %	5,429,610
Pahang	75,086,380	11.4	81.50 %	8,437,400
Perak	75,822,980	13.7	99.27 %	7,076,861
Perlis	148,106	3.9	43.69 %	57,705
Selangor	21,033,490	7.5	72.43 %	3,765,783
Terengganu	25,379,130	2.5	25.67 %	20,373,640
TOTAL	283,867,600	6.7	57.84 %	58,148,267

#### **15.5.4 Sensitivity Analysis**

In this study, available traffic data and standardized project cost estimate of respective bridges were adopted considering the enormous number of bridges totalling 203 bridges. This simplification, however, might mislead the conclusion due to over or underestimate of traffic volume and cost. Therefore, these values were intentionally changed and their influence was checked in order to test the stability of the project feasibility. This is called "sensitivity analysis" in general.

The cases of this sensitivity analysis were selected by changing traffic volume and cost as follows:

- 1) Cost increase by 15 %
- 2) Cost decrease by 15 %
- 3) Traffic volume increase by 15 %
- 4) Traffic volume decrease by 15 %
- 5) Cost increase by 15 % and traffic volume decrease by 15 %

The result is summarized in Table 15-8. The following can be pointed out:

- Cost decrease improves IRR more rather than traffic volume increase.
- Traffic volume decrease worsens IRR more rather than cost increase.
- With a cost increase of 15 % and a traffic volume decrease of 15 %, the IRR reduces to a considerable extent 76 % of the initial value. This implies that the bridges of an IRR of less than 14.5 % might not be regarded feasible in the worst case. The number of corresponding bridges, however, is 12, still showing a high feasibility as a whole.

#### **15.5.5 Conclusion**

The following is concluded as a result of economic evaluation for the 203 bridges. (13 bridges were excluded from the 216 bridges).

- 1) Implementation of the project for the 203 bridges is well justified because 97 % of bridges are feasible for rehabilitation in 1994 and 99 % in 1999.
- 2) 6 individual bridges have an IRR less than 11 % for rehabilitation in 1994. Due, however, to possible intangible benefits including to keep entity of the road network, these bridges have been incorporated in the proposed project. Moreover, the IRR of these bridges can be easily improved by retarding rehabilitation work.

Table 15-8 Sensitivity to Fluctuation in Cost and Traffic

No.	Key	Equip Age	Project Cost	16-hr Traffic (1988)	Base Case IRR(%)	Cost increased		Cost decreased		Traffic increased		Traffic decreased		Cost increase 15%		Cost decrease 15%	
						by 15% IRR(%)	% of Base IRR(%)	by 15% IRR(%)	% of Base IRR(%)	by 15% IRR(%)	% of Base IRR(%)	by 15% IRR(%)	% of Base IRR(%)	by 15% IRR(%)	% of Base IRR(%)	by 15% IRR(%)	% of Base IRR(%)
1	114920	30	189,701	7,758	86.6%	77.8%	89.9%	98.0%	113.2%	96.1%	111.0%	76.7%	88.6%	69.0%	79.7%		
2	161140	40	676,314	8,987	20.9%	17.7%	84.7%	25.0%	119.8%	24.3%	116.4%	17.3%	82.9%	14.5%	69.2%		
3	166510	40	843,424	2,946	48.7%	42.6%	87.4%	57.0%	116.9%	55.7%	114.3%	41.7%	85.6%	36.5%	74.8%		
4	237200	30	375,724	12,593	21.2%	18.6%	87.6%	24.5%	115.6%	23.9%	112.4%	18.4%	86.6%	15.9%	75.1%		
5	317000	20	3,449,112	2,064	71.5%	63.8%	89.3%	81.8%	114.4%	79.8%	111.6%	63.1%	88.2%	56.5%	79.0%		
6	319110	20	492,789	2,064	72.8%	66.2%	90.9%	81.5%	111.9%	79.4%	109.1%	65.8%	90.5%	59.9%	82.2%		
7	341800	30	1,335,912	4,206	13.2%	11.1%	84.2%	15.8%	119.8%	15.3%	116.3%	10.9%	82.3%	8.9%	67.6%		
8	346740	30	5,568,216	8,656	31.4%	28.1%	89.7%	35.5%	113.3%	34.9%	111.1%	27.7%	88.3%	24.8%	79.0%		
9	520850	40	40,599	13,061	170.6%	150.4%	88.1%	197.9%	116.0%	193.1%	113.2%	147.9%	86.7%	130.5%	76.5%		
10	546560	20	36,715	7,986	126.4%	108.8%	86.1%	151.3%	119.8%	141.2%	111.8%	111.4%	88.1%	96.0%	76.0%		
11	546980	40	1,146,765	7,986	56.8%	49.6%	87.4%	66.3%	116.8%	64.8%	114.1%	48.6%	85.7%	42.5%	74.8%		
12	563880	20	205,278	3,755	16.0%	13.8%	86.1%	18.8%	117.5%	17.8%	111.5%	14.0%	87.6%	11.9%	74.3%		
13	567840	30	415,115	20,315	87.7%	78.9%	90.0%	99.3%	113.2%	97.4%	111.1%	77.7%	88.5%	69.9%	79.7%		
14	834850	30	561,414	7,016	21.0%	18.5%	88.1%	24.2%	115.1%	23.7%	112.6%	18.2%	86.6%	15.9%	75.7%		
15	5001070	40	128,231	10,064	102.2%	89.8%	87.9%	118.7%	116.2%	116.1%	113.6%	88.1%	86.3%	77.5%	75.9%		
16	5303340	20	109,435	4,288	39.2%	35.2%	89.8%	44.4%	113.4%	43.5%	111.1%	34.7%	88.6%	31.2%	79.6%		
17	5903120	30	423,163	5,031	164.7%	146.3%	88.8%	189.5%	115.0%	185.4%	112.6%	143.9%	87.3%	128.1%	77.8%		
AVERAGE			15,997,907				88.0%		115.8%		112.6%		87.0%		76.3%		
STD. DEVI							1.8%		2.4%		1.9%		2.0%		3.6%		

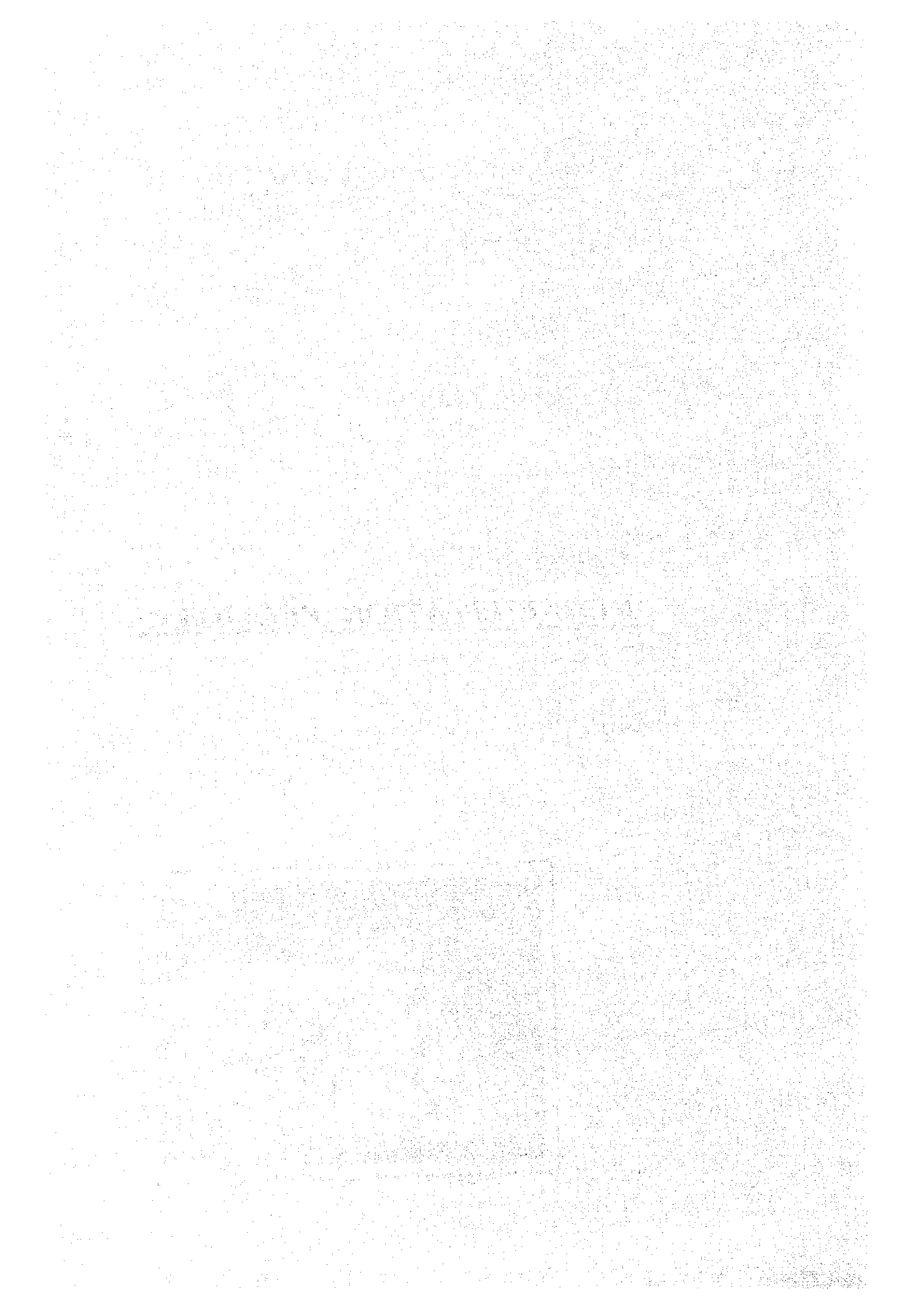
- 3) If an implementation program is formulated by route or by State automatically, only based on the calculated IRR or other parameters, some problems including work concentration can be anticipated as described below. These problems should be avoided by careful arrangement of the program.
- o Traffic flows in and around a particular route will be largely hindered.
  - o Work capacity of local contractors could be exceeded.
  - o JKR State and District local engineers who manage and supervise the work may be in shortage.



## CHAPTER 16

### REHABILITATION PROGRAM







## **CHAPTER 16**

### **REHABILITATION PROGRAM**

#### **16.1 General**

It was concluded, based on the economic evaluation carried out in Chapter 15, that all the study bridges are technically and economically viable and the implementation of a bridge rehabilitation project covering all the bridges shall be carried out within the earliest time according to the Government policy as emphasized in "Sixth Malaysia Plan 1991-1995". However, all the study bridges totaling 203 numbers are extensively scattered over the whole Peninsular and the extent of rehabilitation work required for each bridge also varies widely from only simple protection work as part of the structural rehabilitation to total bridge replacement or combined nature consisting of structural, functional and hydraulic rehabilitation works.

Accordingly, in order for the project implementation to be materialized effectively and smoothly, this Chapter presents the implementation schedule with a basic framework for the programming, the funding schedule, the further engineering design and the project management and organization.

#### **16.2 Framework for Programing**

Prior to establishment of the implementation schedule, basic frameworks for the programming such as the implementation period and the project packaging are elaborated below.

##### **16.2.1 Implementation Period**

In general, the implementation period of a project which is technically and economically feasible will be determined based on the period of preparatory works such as detailed engineering design, tendering activity, land acquisition and the physical construction period. Furthermore contractors' capability, the number of qualified contractors available and government's financial arrangement capability to allocate the required project funds shall be taken into account.

Among the above elements, the critical elements to decide the implementation period in this project are mainly the Government financial arrangement capability and technical aspects that are gradually increasing the risk of loss of structural safety and traffic hazards on the study bridges.

From the financial viewpoint, it is a fact that the Government has allocated M\$5,577.6 million for federal roads and bridges development in the "Sixth Malaysia Plan" covering five years from 1991 to 1995. On the other hand, the total project cost<sup>(1)</sup> amounts to M\$58 million, which is equivalent to about 1.0% of the development funds allocated in the "Sixth Malaysia Plan", assuming five year implementation period. Therefore, it is conclusive that the Government presumably has enough capability to allocate the required project funds.

The annual expenditure on bridge maintenance and rehabilitation in Japan is about 4.5% of the total roads and bridges development funds. Therefore, one twelfth<sup>(1)</sup> of the 4.5% in Japan is equivalent to 0.38%, as compared to the similar funding required for the bridge maintenance and rehabilitation in Malaysia of 1.0%. In comparison between 1.0% in Malaysia and 0.38% in Japan, it can be said that the 1.0% is a remarkably high value taking into account the differences of the present development levels and total number of bridges in these two countries. Nevertheless, the 1.0% is likely to be an acceptable level considering the accumulative maintenance and rehabilitation bills resulted from almost no practical maintenance and rehabilitation works being undertaken since the bridges were built in Malaysia.

From the technical viewpoint, the study bridges were rated as 3, 4 or SSAL in NALS which indicates the necessity or the urgency of rehabilitation work and the Study results also revealed that some of the study bridges have been critically deteriorated/damaged and most of them have suffered various advanced defects. Accordingly it is strongly recommended that immediate rehabilitation work is required to eliminate loss of traffic safety and to decrease risk of bridge failure.

Consequently, it is recommended that five years is suitable as an implementation period mainly from the Government's financial arrangement capability and the technical aspects.

### **16.2.2 Packaging**

With respect to the enormous number of the bridges totaling 203 bridges covered in the Study, it is essential to divide them into five packages taking into consideration the five year implementation period. For the packaging, the economic evaluation results pinpointed various adverse effects if the packaging was done on a road link basis or on a state basis. Those effects are unbalanced budget distribution, causing hindrance of smooth traffic flow, a possibility of shortage of the government management staff on the project, exceeding available contractors' capability in terms of work volumes and decreasing effectiveness of technology transfer from the federal level to the state and the district levels.

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Note : (1) The project covers one twelfth of total number of the federal bridges in Malaysia.

Consequently, the following concepts which overcome the above effects are taken into account in the packaging;

- Priority of each bridge implementation is basically determined by the corresponding economic index of IRR.
- In a package, the bridge arrangement within a certain stretch or area shall not be concentrated in order to prevent any hindrance of the present smooth traffic flow, to overload certain contractors and to overcome the shortage of Government staff for management and supervision.
- It is essential to transfer the maintenance and rehabilitation technology from the federal JKR to the state and district JKRs in the project implementation to enhance the JKR engineers and technicians' capability as well as to train a sufficient number of staff. In the transfer of technology, the Study Team believes that continuous training is more effective to ensure the final result than the concentrated type. Therefore, in order to create the project participation opportunity in every project year, the project bridges within a State or District shall be allocated equally in terms of number of bridges throughout the five year implementation period.

Based on the above concepts, the 203 bridges are divided into five packages and a summary of each package is shown in Table 16-1 together with the overall economic index of each package. A list of the bridges and the location map of each package are attached in Table 16-2 to 16-6 and Figure 16-1 to 16-5 respectively.

**Table 16-1 Summary of Each Package**

Package No.	No's of Bridges	Total Project Cost (M\$)	Overall Economic Index		
			IRR	BCR	
I	64	10,480,400	94.9%	12.03	
II	46	11,306,579	77.7%	10.01	
III	37	13,998,253	45.3%	5.14	
IV	29	11,508,034	38.7%	4.13	
V	27	10,855,002	28.6%	2.82	
<b>TOTAL</b>	<b>5</b>	<b>203</b>	<b>58,148,268</b>	<b>57.8%</b>	<b>6.75</b>

Table 16-2 List of Bridges Under Package I

No.	Key	State	District	Year Built	Max. Span (m)	No of Spans	Bridge Length (m)	Type of Bridge	Rehabilitation Plans <1			Direct Cost (RM)	Project Cost (RM)	IRR (%)	Total Amount of Each State (RM)
									RC	WD	PR				
1	00102350	Johor	J. BAHRU	1955	1.80	2	3.60	BOX				3,820	5,433	2571.08	1,404,835
3	00106960	Johor	MUANG	1937	2.19	1	2.19	BOX				3,792	5,354	1221.65	
43	00316745	Johor	MERSING	1905	3.67	1	3.67	RCS				88,533	143,377	437.04	
34	00112430	Johor	BATU PAHAY	1950	8.27	1	8.27	RCS				26,882	45,129	243.18	
144	00035565	Johor	K. YAGGI	1940	4.36	1	4.36	SBB				48,790	79,507	107.01	
3	00113760	Johor	BATU PAHAY	1950	4.74	1	4.75	SBB				83,553	108,450	158.84	
59	00055500	Johor	SEGAMAT	1953	8.83	3	26.34	RCS				142,271	238,013	111.71	
75	00016850	Johor	PONTIAN	1958	11.98	4	47.52	RCS				595,354	659,252	77.21	
194	08701200	Kedah	MUAR	1968	8.33	3	17.82	RCS				84,418	108,210	29.58	
109	00701810	Kedah	K. MUHA/SIK	1930	8.05	1	8.05	RCS				2,284	3,937	1032.41	
24	03184900	Kedah	RBO. PASU	1970	33.52	3	49.60	RCS				115,558	184,950	107.58	
122	00035800	Kalantan	KOTA SEJAR	1950	3.20	1	3.20	RCS				80,803	101,915	41.74	
65	00088500	Kalantan	MACHANG	1950	8.69	2	12.12	RCS				47,648	60,585	83.24	
121	00048850	Kalantan	P. PUTEH	1953	4.64	2	9.88	RCS				115,323	183,753	91.18	
77	00016850	Kalantan	KUALA KRAU	1960	3.34	1	3.34	RCS				49,807	85,676	50.90	
88	00033300	Melaka	JASIN	1940	4.93	1	4.95	PR				2,631	4,824	799.29	
80	00020950	Melaka	METAKA TGH	1950	9.33	1	9.33	SBB				3,910	8,989	217.42	
66	00020950	Melaka	JASIN	1950	4.27	1	4.27	SBB				24,188	40,549	170.59	
67	00024420	Melaka	ALOR GAJAH	1950	1.88	1	1.88	BOX				4,438	7,438	114.35	
84	00022780	Melaka	MELAKA TGH	1950	3.90	1	3.90	RCS				58,181	87,744	78.29	
137	01105770	N.Sembilan	M.P. MELAKA	1930	7.47	1	7.47	SBB				23,979	40,265	29.98	
10	00129234	N.Sembilan	JEMPUL	1970	8.18	3	18.32	PR				48,743	81,882	633.60	
135	00060420	N.Sembilan	YAMPIN	1940	8.70	1	8.70	RCS				3,928	8,598	315.68	
211	00020780	N.Sembilan	JELUPU	1935	10.70	3	38.70	SBB				197,913	332,326	88.00	
146	00101580	N.Sembilan	SEREMBAN	1950	3.70	1	3.70	SBB				30,783	54,735	81.75	
158	00030860	N.Sembilan	SEREMBAN	1940	3.31	1	3.31	SBB				51,337	88,297	79.59	
153	00103300	N.Sembilan	F. DICKSON	1950	6.27	1	6.27	SBB				60,139	101,034	62.31	
127	00001980	N.Sembilan	K. PILAH	1958	8.62	2	16.08	SBB				84,362	158,328	48.59	
11	00128234	N.Sembilan	K. PILAH	1950	9.07	2	18.14	SBB				162,113	300,950	45.41	
181	00080010	N.Sembilan	NEMBAU	1930	9.58	1	9.58	SBC				27,478	48,160	24.49	
183	00080010	N.Sembilan	NEMBAU	1930	8.35	1	8.35	SBB				52,457	86,128	875.99	
169	00033300	N.Sembilan	NEMBAU	1950	8.05	1	8.05	SBB				20,030	27,350	327.21	
117	00013470	Pahang	JERANTUT	1930	11.81	1	11.81	SBB				67,027	112,605	112.64	
45	00319118	Pahang	K. LIPIS	1950	11.67	1	11.67	PR				108,183	178,394	107.91	
150	00011950	Pahang	BOMPIN	1952	30.48	7	121.98	PCB				283,427	492,799	72.79	
27	00028540	Pahang	BENTONG	1951	10.77	4	32.66	SBB				186,462	279,856	71.13	
29	00030550	Pahang	MARAN	1955	6.26	1	6.26	SBB				59,280	80,572	87.12	
114	00030550	Pahang	RUANTAN	1957	6.40	1	6.40	SBB				41,724	70,060	48.37	
48	00038620	Pahang	KARIS	1957	9.04	2	18.08	SBB				133,807	224,480	54.83	
15	00149220	Perak	PEKAH	1945	5.73	1	5.73	PR				58,172	97,728	39.84	
189	00001000	Perak	BTG PADANG	1963	12.08	3	36.24	"				48,859	81,747	506.77	
14	00149600	Perak	BTG PADANG	1950	4.88	1	4.88	SBC				30,941	51,981	485.83	
102	00058290	Perak	BTG PADANG	1962	2.40	1	2.40	BOX				2,158	3,823	453.32	
201	07002330	Perak	H. PERAK	1989	2.48	2	4.92	BOX				36,877	59,837	323.80	
192	00009970	Perak	K. KANGGAR	1930	6.35	1	6.35	SBB				48,347	79,071	91.31	
105	00027640	Perak	MANJUNG	1950	3.14	1	3.14	SBB				132,117	221,857	89.14	
21	00168220	Perak	KRTA	1950	8.08	2	12.12	PR				247,082	415,115	87.70	
137	00006050	Perak	LAY MAYANG	1945	5.87	1	5.87	SBB				87,063	148,318	80.41	
208	07008390	Perak	LAMSELAMA	1950	5.09	1	5.09	SBB				73,563	124,241	76.65	
196	00001310	Perak	HULU PERAK	1950	3.07	1	3.07	SBB				37,591	63,153	73.98	
111	00703330	Perak	HULU PERAK	1950	3.80	1	3.80	SBB				47,569	80,470	89.89	
12	00143100	Perlis	PERLIS	1945	24.90	1	24.90	PCB				34,349	87,108	43.58	
96	00041210	Selangor	UDU SYGOR	1925	1.26	1	1.26	SBB				1,616	2,715	2408.45	
88	00040810	Selangor	K. LANGAY	1950	4.73	1	4.73	SBB				52,711	88,534	523.68	
154	00029510	Selangor	K. LANGAY	1950	8.23	1	8.23	SBB				88,657	143,820	219.87	
89	00048090	Selangor	K. LANGAY	1950	1.80	2	3.20	BOX				2,408	4,717	181.83	
164	00040590	Selangor	K. SELANGOR	1989	10.64	3	30.94	RCS				21,864	36,715	126.35	
32	00034370	Selangor	PETALING	1950	6.56	1	6.56	RCS				43,308	72,388	29.87	
57	00058450	Terengganu	SEPAK	1980	8.32	4	32.54	RCS				240,817	404,373	18.89	
58	00058790	Terengganu	K. TEPEKGGAMU	1983	5.25	3	17.68	PR				57,151	98,014	112.61	
62	00086930	Terengganu	K. TEPEKGGAMU	1959	5.90	9	53.10	PR				46,560	78,271	63.84	
55-1	00048740	Terengganu	BESUT	1995	5.84	1	5.84	PR				38,081	63,878	58.10	
54	00041800	Terengganu	DUNGUN	1973	30.50	3	152.25	PCB				942,824	1,849,044	31.36	
64	00041800	Terengganu	KEMAMAN	1955	12.10	3	36.14	RCS				798,186	1,335,812	13.18	
Grand Total											8,238,332	10,480,400		10,480,400	

Note : <\*1 RC Means Reconstruction (Total Replacement)  
 WD Means Widening Carriageway or Adding Sidewalk  
 RF Means Reinforcement Work  
 PR Means Protection Work  
 <\*2 Construction of this bridge is divided into two subpackages consisting of bridge and hydraulic rehabilitation.  
 This work is carried out in two consecutive years.

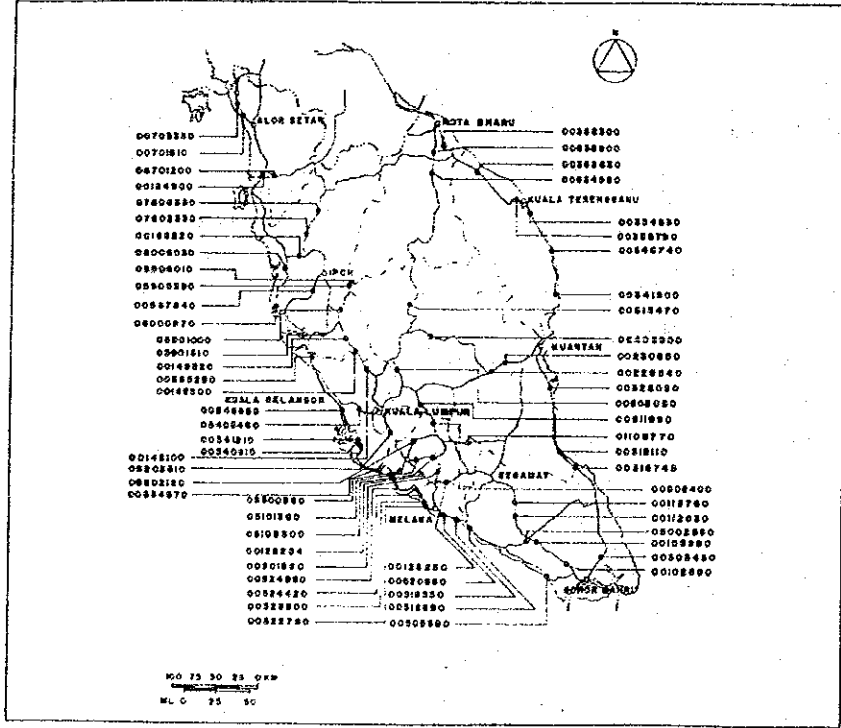


Figure 16-1 Location Map of Bridges Under Package I

Table 16-3 List of Bridges Under Package II

No.	Key	State	Dist/pt	Year Built	Max. Span (m)	No of Spans	Bridge Length (m)	Type of Bridge	Rehabilitation Plans <#1				Direct Cost (M\$)	Project Cost (M\$)	IRR (%)	Total Amount of Each State (M\$)
									RC	WD	RF	PR				
2	00108100	Johor	KLUANG	1954	15.80	3	27.40	RCB				77,104	129,535	147.07	1,388,723	
33	00300220	Johor	K. TINGGI	1940	4.94	1	4.84	SBB			*	49,955	83,924	138.19		
73	00514370	Johor	BATU PAHAT	1950	8.31	1	8.31	RCB			*	42,332	71,151	108.50		
142	00501070	Johor	BATU PAHAT	1919	4.77	1	4.73	SBB			*	78,329	128,231	102.15		
8	00114620	Johor	SEGAMAT	1955	6.43	2	12.66	RCB			*	112,917	183,701	86.53		
41	00313520	Johor	MERSING	1960	1.80	2	3.80	RCB			*	114,000	192,543	88.18		
42	00314150	Johor	MERSING	1954	5.50	2	11.00	PRB			*	231,031	389,132	82.28		
37	00304360	Johor	K. TINGGI	1928	3.35	1	3.35	SBB			*	90,971	152,831	55.08		
7	00118360	Johor	SEGAMAT	1947	2.44	2	4.88	BOX			*	31,254	52,873	53.94		
196	00701800	Kedah	K. MUDA/SIK	1968	30.64	3	91.52	PCB			*	187,911	282,060	101.63		357,772
197	00702080	Kedah	BALING	1950	7.16	1	7.16	SBB			*	45,040	75,682	40.42		
84	00309990	Kalantan	P. PUTEH	1951	4.28	2	8.58	RCB			*	135,089	228,950	49.56	604,654	
119	00321340	Kelantan	GLA MUSANG	1962	31.13	3	83.31	PCB			*	224,524	377,704	48.95		
76	00518390	Malaka	JASIN	1955	6.22	7	42.70	PCB			*	224,236	378,800	87.43	569,314	
78	00520130	Malaka	JASIN	1960	6.46	1	6.46	PRB			*	7,449	12,514	54.32		
154	00200280	N. Sembilan	SEPEMBAN	1932	4.68	1	4.68	SBB			*	77,989	130,820	55.35	1,064,410	
147	00101460	N. Sembilan	SEPEMBAN	1950	3.28	1	3.28	SBB			*	48,315	81,188	48.61		
132	00604330	N. Sembilan	K. PILAH	1950	7.77	1	7.77	SBB			*	52,964	86,980	37.63	37.78	
91	00534450	N. Sembilan	P. DICKSON	1965	8.83	4	35.32	RCB			*	158,905	265,794	37.78		
215	00603990	N. Sembilan	JELEBU	1930	9.62	1	9.62	SBB			*	81,442	103,223	35.60	31.00	
128	00601700	N. Sembilan	K. PILAH	1950	5.83	1	5.83	SBB			*	38,678	64,979	31.00		
129	00602270	N. Sembilan	K. PILAH	1950	3.11	1	3.11	SBB			*	36,635	61,547	28.08	27.01	
212	00602900	N. Sembilan	JELEBU	1950	3.00	1	3.00	SBB			*	158,988	268,898	27.01		
179	00505010	Pahang	IPAH	1961	30.74	4	122.36	PCB			*	171,952	286,711	286.57	1,401,261	
180	00403300	Pahang	JERANTUT	1950	12.31	1	12.31	SBB			*	73,843	123,720	108.97		
116	00810120	Pahang	K. LEKIS	1950	5.80	1	5.80	SBB			*	50,191	84,321	100.42	71.02	
113	00800350	Pahang	BENTONG	1950	3.47	1	3.47	SBB			*	36,647	61,903	71.02		
190	00404270	Pahang	JERANTUT	1930	10.91	1	10.91	SBB			*	63,637	106,310	70.68	44.30	
46	00318680	Pahang	POMPIN	1960	5.67	2	11.34	PRB			*	84,013	141,142	84.99		
28	00228970	Pahang	MARAN	1965	3.03	1	3.03	BOX			*	62,255	106,188	45.82	44.30	
182	00409650	Pahang	JERANTUT	1930	8.31	1	8.31	SBB			*	48,001	80,642	44.30		
32	00237200	Pahang	KUANTAN	1960	8.80	3	26.70	SBC			*	223,845	375,724	21.21	1,418,938	
18	00151300	Perak	BTG PADANG	1960	9.08	7	63.56	RCB			*	90,341	151,773	380.89		
172	00201580	Perak	BTG PADANG	1950	7.83	1	7.83	SBC			*	25,989	43,494	363.77	380.63	
178	00302660	Perak	BTG PADANG	1950	6.80	1	6.80	SBC			*	23,288	42,386	380.63		
103	00506900	Perak	H. PERAK	1960	7.33	1	7.33	RCB			*	45,136	75,828	207.62	83.22	
163	00501330	Perak	MANJUNG	1960	5.02	1	5.02	RCB			*	26,402	44,325	83.22		
205	07804750	Perak	HULU PERAK	1950	9.34	1	9.34	SBB			*	88,581	111,822	63.64	48.74	
22	00188510	Perak	LRT MATANG	1935	10.72	1	10.72	6BG			*	502,038	843,424	48.74		
188	00303740	Perak	LAMA BELAMA	1960	5.90	3	21.95	RCB			*	22,558	37,896	45.51	37.56	
108	00586850	Perak	KINTA	1950	2.63	1	2.63	SBB			*	40,380	67,855	37.56		
13	00149900	Selangor	ULU S'GOR	1965	12.13	3	25.91	IT			*	58,283	97,915	183.86	534,077	
97	00541000	Selangor	K. LANGAT	1950	3.24	1	3.24	SBB			*	98,542	165,551	134.50		
101	00546250	Selangor	K. SELANGOR	1965	12.81	8	83.58	SBC			*	181,078	270,611	98.22	46.52	
60	00387270	Terengganu	K. TERENGGANU	1957	5.88	2	11.79	PRB			*	46,374	77,908	46.52		
59	00387200	Terengganu	K. TERENGGANU	1956	5.84	3	5.84	PRB			*	53,748	85,233	33.38	31.36	
55-2	00346740	Terengganu	DUNKUN	1973	30.50	9	152.28	PCB			*	2,371,580	3,884,271	31.36		
48 Bridges											Grand Total	8,750,108	11,308,579		11,308,579	

Note: <#1 RC Means Reconstruction (Total Replacement)  
 WD Means Widening Carriageway or Adding Sidewalk  
 RF Means Reinforcement Work  
 PR Means Protection Work  
 <#2 Construction of this bridge is divided into two subpackages consisting of bridge hydraulic rehabilitations.  
 These works are carried out in two consecutive years.

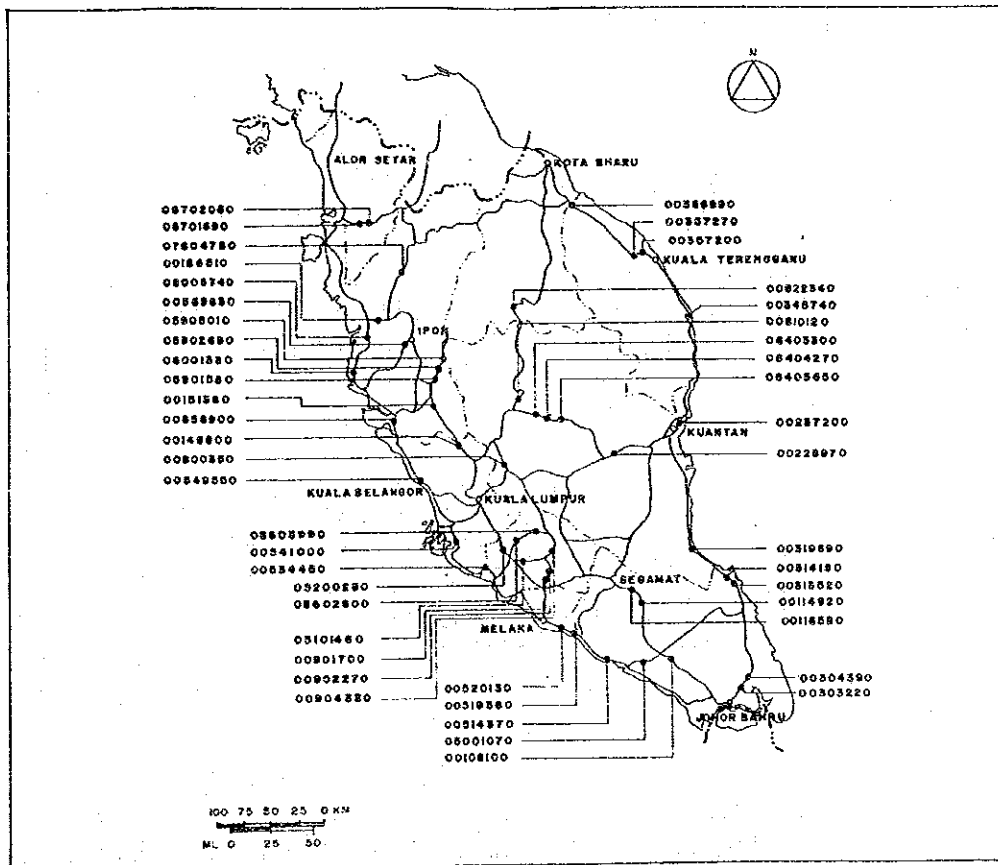


Figure 16-2 Location Map of Bridges Under Package II

Table 16-4 List of Bridges Under Package III

No.	Key	State	District	Year Built	Max. Span (m)	No of Spans	Bridge Length (m)	Type of Bridge	Rehabilitation Plans <#>				Direct Cost (M\$)	Project Cost (M\$)	IRR (%)	Total Amount of Exch State (M\$)
									RO	WD	RF	PR				
72	00514300	Johor	BATU PAHAT	1990	10.43	3	22.07	IT			*	80,263	134,842	70.31	1,365,101	
143	00601800	Johor	BATU PAHAT	1950	5.05	1	5.05	SBB			*	70,310	118,131	67.69		
68	00607230	Johor	PONTIAN	1965	11.77	3	35.21	PCB			*	172,968	239,951	50.59		
38	00300390	Johor	K. TINGGI	1974	16.57	5	84.57	IT			*	108,077	312,452	47.35		
40	003113150	Johor	MERSING	1950	4.40	1	4.40	SBE	*		*	111,852	187,852	43.62		
39	00308710	Johor	K. TINGGI	1969	19.90	7	51.96	IT			*	203,821	342,063	40.15		
185	06701230	Kedah	K. MUDA/SIK	1940	6.13	2	12.26	RCB			*	34,137	57,350	64.91	367,214	
110	00702830	Kedah	KBO. PASU	1960	9.54	1	9.54	RCB			*	71,353	119,873	53.35		
23	00184400	Kedah	KOTA BETAR	1950	2.61	2	12.20	RCB			*	113,060	189,691	25.77		
120	00034950	Kelantan	KUALA KRAJ	1960	4.63	3	13.71	RCS			*	304,175	501,414	21.02	501,414	
83	00521090	Melaka	MELAKA TGH	1960	7.13	2	14.28	RCB			*	154,388	259,368	38.49		
91	00521300	Melaka	MELAKA TGH	1950	6.90	1	6.90	RCB			*	119,370	200,542	29.51	450,910	
181	00302000	N. Sembilan	SEMEMPAN	1950	8.45	1	8.45	SBB			*	89,120	116,122	45.94		
162	00302160	N. Sembilan	SEMEMPAN	1950	6.31	1	6.31	SBB			*	59,390	93,063	44.17	1,037,570	
208	00601160	N. Sembilan	SEMEMPAN	1950	4.84	1	4.84	SBB			*	47,465	79,741	44.12		
69	00502800	N. Sembilan	P. JACKSON	1950	3.05	1	3.05	SBB			*	82,282	106,834	36.88		
133	00608190	N. Sembilan	JEMPUL	1950	9.54	1	9.54	SBB			*	61,149	102,730	27.12		
131	00802440	N. Sembilan	K. PILAH	1960	3.10	1	3.10	SBB			*	46,179	77,578	29.84		
148	00102000	N. Sembilan	K. PILAH	1950	4.74	1	4.74	SBB			*	53,697	90,211	24.79		
149	00102200	N. Sembilan	K. PILAH	1960	4.81	1	4.81	SBB			*	42,119	70,758	22.12		
124	00601300	N. Sembilan	K. PILAH	1960	5.74	1	5.74	RCS			*	44,298	74,320	20.99		
218	00004640	N. Sembilan	JELEBU	1950	9.51	1	9.51	SBB	*	*	*	132,956	223,383	16.34		
44-1	00317000	Pahang	RCMPIN	1974	45.70	9	397.32	PCB <#2			*	1,411,392	2,371,139	71.49	2,371,139	
173	00601800	Perak	BTG PADANG	1950	9.53	1	9.53	SBC			*	73,848	123,725	310.49		
175	00602230	Perak	BTG PADANG	1950	8.21	1	8.21	SBC			*	32,430	54,482	310.16		
174	00602000	Perak	BTG PADANG	1950	3.58	1	3.58	SBC			*	23,220	38,010	272.27		
204	07604180	Perak	HULU PERAK	1950	3.23	1	3.23	SBB	*	*	*	69,558	100,054	37.97		
157	05601430	Perak	HLR PERAK	1950	3.67	1	3.67	SBS			*	69,077	116,049	35.16		
198	07600230	Perak	HLR PERAK	1950	3.98	1	3.98	SBB			*	82,381	155,200	32.41		
202	07802480	Perak	K. KANGSAR	1950	5.34	1	5.34	SBB			*	87,871	113,697	26.59		
17	00155500	Perak	KUNTA	1970	1.61	2	3.62	BOX			*	298,271	501,095	24.46		
185	00005220	Perak	LAMA SELAMA	1960	7.01	1	7.01	RCB			*	10,594	17,748	24.30		
139	01800370	Perak	MANJUNG	1950	4.78	1	4.78	SBC	*	*	*	140,296	235,660	19.85		
100	00540800	Selangor	K. SELANGOR	1969	10.64	3	30.94	RCB	*	*	*	662,668	1,148,785	58.77	1,148,785	
53	00336210	Terengganu	KEMAMAN	1963	15.22	10	152.20	PCB	*	*	*	668,494	1,123,070	38.98		
61	00381490	Terengganu	BESUT	1960	0.01	3	16.03	FRB	*	*	*	95,131	181,500	23.44		
52-1	00336500	Terengganu	KEMAMAN	1965	28.63	16	219.13	PCB <#3	*	*	*	2,306,600	3,827,840	20.04		
37 Bridges											Grand Total	8,332,264	13,968,253		13,968,253	

Note: <#1 RC Means Reconstruction (Total Replacement)  
 <#1 WD Means Widening Carriageway or Adding Sidewalk  
 <#1 RF Means Reinforcement Work  
 <#1 PR Means Protection Work  
 <#2 This bridge is constructed continuously over a two year period.  
 <#3 This bridge is constructed continuously over a three year period.

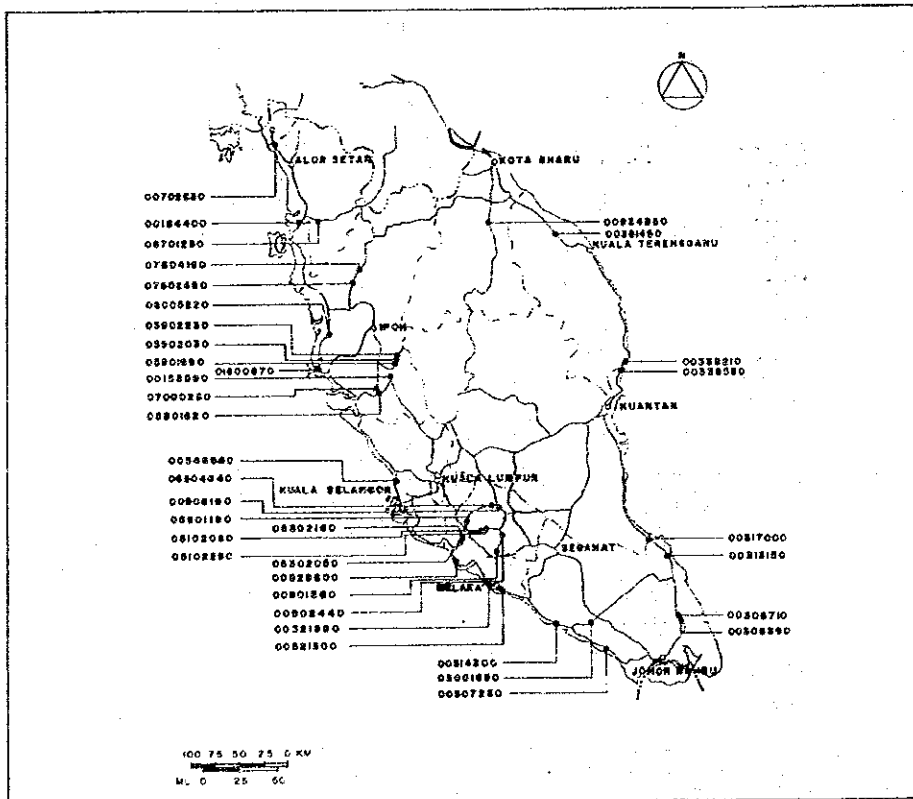
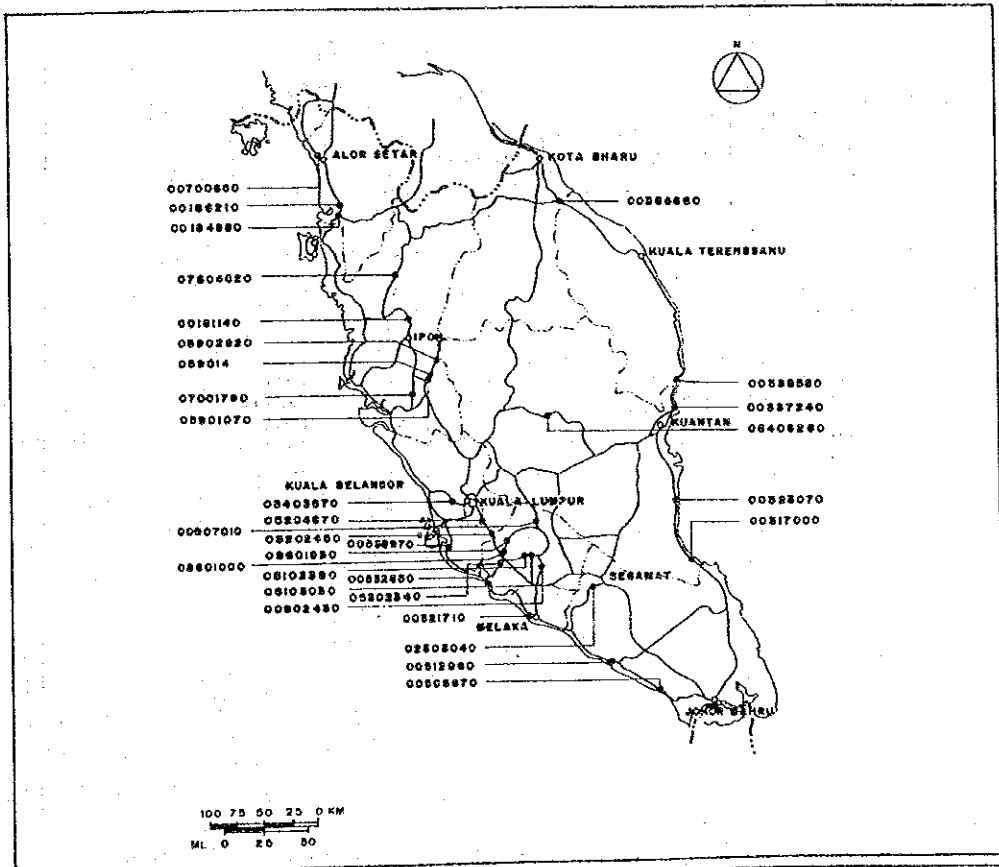


Figure 16-3 Location Map of Bridges Under Package III

**Table 16-5 List of Bridges Under Package IV**

No.	Key	State	District	Year Built	Max. Span (m)	No of Spans	Bridge Length (m)	Type of Bridge	Rehabilitation Plans <1>				Direct Cost (M\$)	Project Cost (M\$)	IRR (%)	Total Amount of Each State (M\$)
									RC	WD	RF	PR				
57	00506870	Johor	PONTIAN	1971	13.09	3	38.17	IT			*	201,999	339,350	45.75	1,237,301	
71	00512960	Johor	BATU PAHAT	1965	11.30	3	30.22	RCB			*	500,759	841,275	29.91		
140	02300940	Johor	SEGAMAT	1950	8.29	2	12.28	SBB			*	69,445	116,668	27.89		
25	00184900	Kedah	KOTA SETAR	1950	4.61	1	4.61	RCS			*	22,923	38,511	25.36	324,233	
28	00186210	Kedah	KOTA SETAR	1940	3.23	1	3.23	SBB			*	38,546	64,757	24.29		
107	00700800	Kedah	KOTA SETAR	1984	18.40	1	18.40	PCB			*	131,527	220,955	18.10		
63	00355580	Kalantan	P. PUTEH	1952	5.41	5	32.46	FRS			*	589,968	957,548	10.00	957,548	
82	00521710	Malaka	MELAKA TGH	1960	10.72	1	10.72	RCB			*	190,069	319,320	27.06		
210	00601830	N. Sembilan	SEREMBAN	1950	3.75	1	3.75	SBB			*	44,293	74,412	43.89	1,025,600	
207	00601000	N. Sembilan	SEREMBAN	1950	9.62	1	9.62	SBB			*	87,464	146,940	39.72		
163	05302340	N. Sembilan	SEREMBAN	1940	6.70	1	6.70	SBB			*	91,943	154,454	32.49		
90	00532850	N. Sembilan	P. DICKSON	1970	11.02	5	53.24	RCB			*	195,200	327,938	24.71	1,025,600	
152	05103030	N. Sembilan	K. PILAH	1950	3.79	1	3.79	SBB			*	42,514	71,424	19.56		
150	05102380	N. Sembilan	K. PILAH	1960	3.21	1	3.21	SBB			*	39,574	64,804	19.41		
130	00024300	N. Sembilan	K. PILAH	1950	3.10	1	3.10	SBB			*	61,829	103,873	14.09	1,025,600	
134	00607010	N. Sembilan	JELEBU	1930	8.38	1	6.36	SBB			*	48,659	81,747	14.01		
44-2	00317000	Pahang	ROMPIN	1974	45.78	9	397.32	PCB <*2			*	841,651	1,077,974	71.49		
193	00406260	Pahang	JERANTUT	1930	4.90	1	4.90	SBB			*	108,272	181,897	29.36	1,526,975	
47	00323070	Pahang	PEKAN	1965	10.42	3	31.28	RCB			*	108,330	181,894	29.17		
51	00337240	Pahang	KUANTAN	1957	8.58	1	6.56	RCS			*	50,661	85,110	19.11		
170	00901070	Perak	BTG PADANG	1950	4.71	1	4.71	SBC			*	41,341	69,453	184.30	1,420,900	
177	00902920	Perak	BTG PADANG	1950	8.77	1	6.77	SBC			*	66,158	111,145	178.23		
171	00901480	Perak	BTG PADANG	1950	1.95	2	3.90	SBC			*	86,229	144,685	170.47		
203	07804020	Perak	HULU PERAK	1950	6.35	1	6.35	SBB			*	124,277	208,795	29.42	1,420,900	
199	07001790	Perak	HULU PERAK	1970	14.80	3	44.36	IT			*	125,255	210,428	24.11		
19	00181140	Perak	KINTA	1950	9.77	2	19.11	SBS			*	402,568	676,314	20.89		
167	05204870	Selangor	U. LANGAT	1984	18.24	3	54.56	SBC			*	177,869	298,820	48.43	708,199	
153	05202450	Selangor	U. LANGAT	1955	12.11	1	12.11	RCB			*	33,110	58,625	49.13		
94	00506970	Selangor	K. LANGAT	1950	2.30	1	2.30	BOX			*	27,790	45,586	27.42		
185	05403570	Selangor	PETALING	1950	3.05	1	3.05	BOX			*	182,838	307,168	9.23	708,199	
52-2	00036580	Terengganu	KEMAMAN	1965	28.03	18	219.13	PCB <*3			*	2,338,000	3,927,840	20.04		
29	Bridges											6,850,021	11,508,034			11,508,034
									Grand Total							

Note : <1> RC Means Reconstruction (Total Replacement)  
 WD Means Widening Carriageway or Adding Sidewalk  
 RF Means Reinforcement Work  
 PR Means Protection Work  
 <2> This bridge is constructed continuously over a two year period.  
 <3> This bridge is constructed continuously over a three year period.

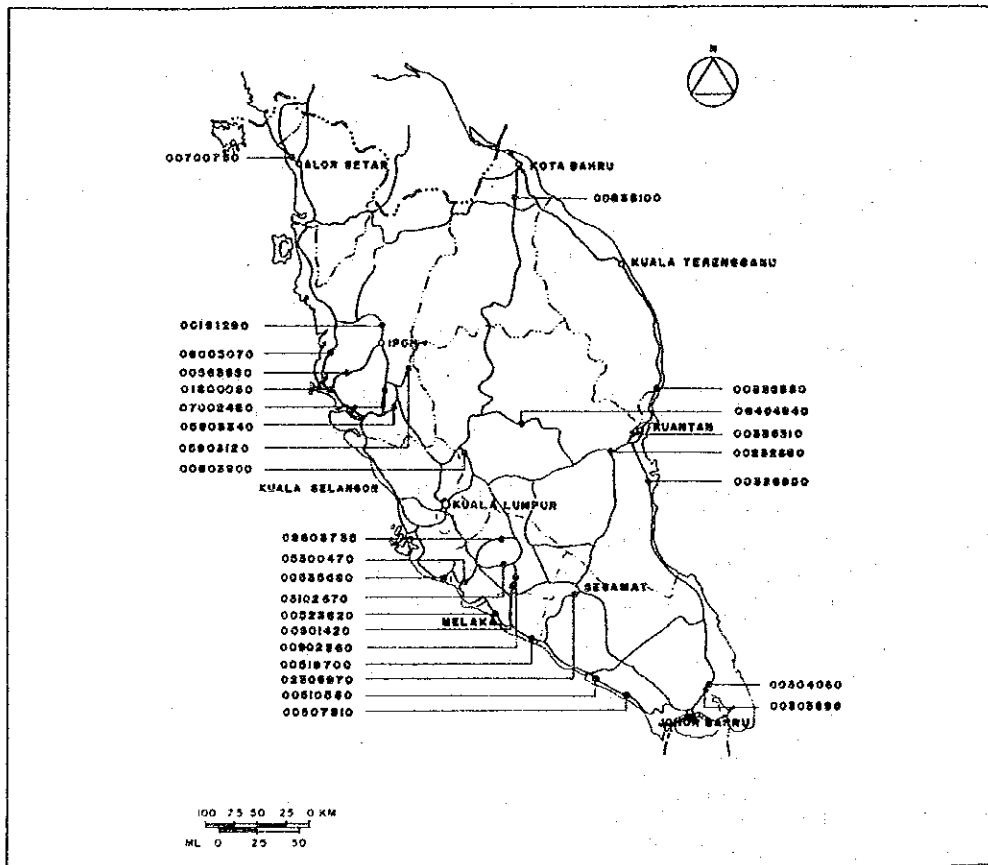


**Figure 16-4 Location Map of Bridges Under Package IV**

**Table 16-6 List of Bridges Under Package V**

No.	Key	State	District	Year Built	Max. Span (m)	No of Spans	Bridge Length (m)	Type of Bridge	Rehabilitation Plans <#1				Direct Cost (M\$)	Project Cost (M\$)	IRR (%)	Total Amount of Each State (M\$)
									RC	WD	RF	PR				
69	00507810	Johor	PONTIAN	1969	12.00	5	47.83	IT				342,078	574,693	35.26	1,351,831	
85	00303890	Johor	K. TINGGI	1940	4.50	2	9.18	RCS	*			101,419	170,364	24.85		
35	00304080	Johor	K. TINGGI	1963	36.65	2	92.25	RCS				148,830	246,874	22.63		
141	02303870	Johor	SEGAMAT	1950	5.88	2	7.60	RCS			*	70,879	119,077	20.97	1,452,792	
70	00510590	Johor	BATU PAHAT	1950	10.42	3	31.24	RCS			*	143,454	241,003	10.87		
108	00700750	Kedah	KOTA SETAR	1970	15.36	1	15.36	RCS			*	192,022	322,597	14.04		
123	00838100	Kelantan	MACHANG	1941	4.48	2	9.72	RCS			*	239,855	400,857	18.81	400,857	
78	00519700	Melaka	JASIN	1961	4.88	1	4.88	PRB	*		*	75,408	128,662	23.83	259,424	
80	00523820	Melaka	MELAKA TGH	1980	7.29	2	15.16	PRB			*	79,013	132,742	16.52	1,118,412	
150	05300470	N.Sembilan	P. DICKSON	1950	9.35	1	9.35	SBB			*	114,593	192,518	21.87		
125	00901420	N.Sembilan	K. PILAH	1950	3.24	1	3.24	SBB			*	80,512	150,380	14.06		
129	00902380	N.Sembilan	K. PILAH	1950	3.11	1	3.11	SBB			*	62,931	139,374	19.29	1,402,187	
214	00903735	N.Sembilan	JELIBU	1950	4.96	2	9.72	SBB	*		*	295,895	497,272	10.53		
161	05102870	N.Sembilan	K. PILAH	1960	3.21	1	3.21	SBB	*	*	*	61,470	136,870	9.96		
115	00903800	Pahang	PAUB	1952	5.47	2	10.94	SBB			*	152,885	256,813	27.60	1,402,187	
191	06401840	Pahang	JERANTUT	1930	6.21	1	6.21	SBB			*	103,268	173,487	23.78		
49	00329950	Pahang	PERAN	1965	5.89	4	23.52	PRB			*	184,087	275,853	17.89		
31	00232950	Pahang	KUANTAN	1963	11.08	1	11.08	PRB			*	153,880	258,182	13.76	1,452,792	
50	00303910	Pahang	KUANTAN	1959	12.00	3	36.00	RCS	*		*	280,757	436,072	5.00		
178	05803120	Perak	BTG PADANG	1950	10.88	3	23.18	SBC			*	251,883	423,183	164.70		
168	05803340	Perak	BTG PADANG	1950	4.97	1	4.97	SBB			*	85,140	109,435	39.16	1,452,792	
20	00181290	Perak	KHITA	1955	8.09	2	16.18	SBB	*	*	*	258,226	420,450	20.83		
200	07002400	Perak	BTG PADANG	1950	3.88	1	3.88	SBB	*	*	*	77,390	129,890	18.72		
136	01800060	Perak	MANJUNG	1980	3.68	1	3.68	RCS	*	*	*	81,984	104,133	17.50	1,452,792	
104	00543660	Perak	MANJUNG	1972	14.07	3	41.59	IT			*	122,189	205,278	15.89		
194	08005070	Perak	LAMESELAMA	1950	7.20	4	27.14	SBC			*	29,839	50,343	12.45		
80	00535860	Selangor	BSPANG	1960	14.70	5	61.34	RCS			*	389,820	620,692	7.83	820,982	
52--3	00338580	Terengganu	KEMAMAN	1965	28.03	16	219.13	PCB <#2	*			2,338,000	3,927,840	20.04	3,927,840	
27	Bridges								Grand Total				6,461,310	10,655,002		10,655,002

Note: <#1 RC Means Reconstruction (Total Replacement)  
 WD Means Widening Carriageway or Adding Sidewalk  
 RF Means Reinforcement Work  
 PR Means Protection Work  
 <#2 This bridge is constructed continuously over a three year period.



**Figure 16-5 Location Map of Bridges Under Package V**



### 16.3 Implementation Schedule

In principle, it is assumed that construction including tendering of each package shall be completed within one Malaysian fiscal year (which is from January 1st to December 31) because the project funds will be financed by the Government of Malaysia.

Main work items for the scheduling are detailed engineering design, tendering activities and construction. The outline and required time duration of each item are described hereinafter:

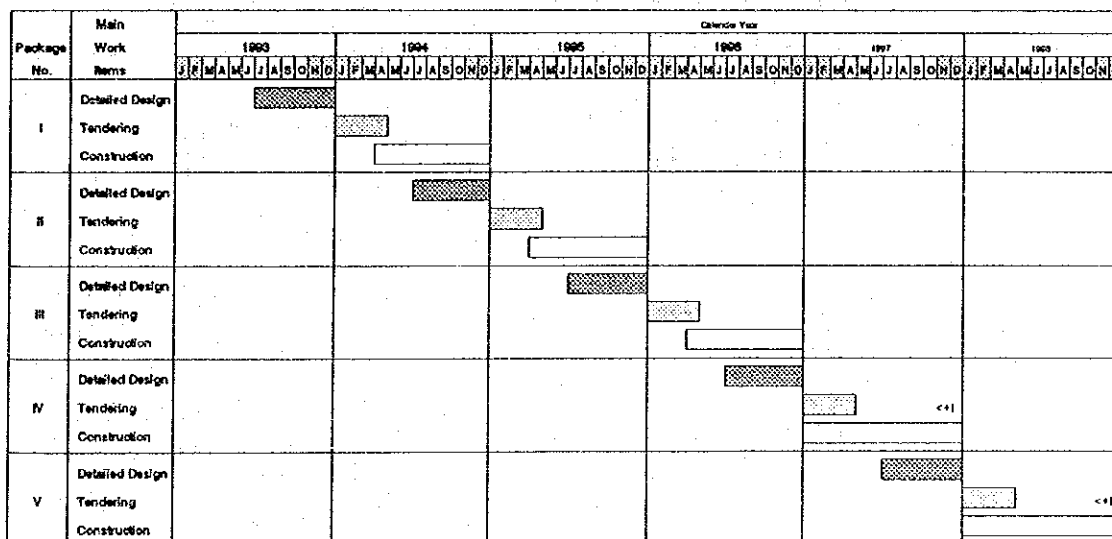
The detailed design which should be carried out one year in advance of the construction involves topographic survey, subsoil survey, structural survey, structural assessment, cost estimate and preparation of tender documents. It will take about six months to complete the design of each package.

The tendering activities including prebid conference, tender evaluation, and negotiation with the contractor shall commence on the first week of January of each fiscal year and will take four months to complete.

The construction of each package shall commence on the first week of April and be completed by the end of December in principle. Out of 203 study bridges, only two bridges, with construction period of more than 24 months, will be implemented for two consecutive years.

Assuming commencement of the package I in 1994, the project will be completed by the end of 1998. The overall implementation schedule is depicted in Figure 16-6.

Figure 16-6 Implementation Schedule



Note :   
 □ Indicate rainy season from November to February.   
 <-> Rehabilitation works for Bridge No. 317000 and 336540 are carried out continuously during the two or three years from 1996 to 1999   
 : Fiscal year in Malaysia from January 1 to December 31

## 16.4 Funding Schedule

The total project cost, as estimated in Chapter 14, is 58.148 million Malaysian Ringgit at December 1991 price level. In light of the implementation schedule, the project funding schedule of each year is shown in Table 16-7.

**Table 16-7 Project Funding Schedule**

Classification		Amount for Each Year (Million M\$)						Grand Total
		1993	1994	1995	1996	1997	1998	
Construction Cost		-	8.188	8.833	10.936	8.991	8.480	45.428
Engineering Cost	Detailed Design	0.678	0.343	1.075	0.305	0.325	0.0	2.726
	Supervision	-	0.328	0.353	0.437	0.360	0.339	1.817
Administration		-	0.246	0.265	0.328	0.270	0.254	1.363
Contingency		-	1.228	1.325	1.640	1.349	1.272	6.814
Grand Total		0.678	10.333	11.851	13.646	11.295	10.345	58.148

Note : Price level of those amounts is based on December, 1991.

## 16.5 Further Engineering Design

Prior to commencement of the tendering and construction of each package, detailed engineering design of each bridge must be carried out by the Bridge Unit in Federal JKR. The detailed design covers the following items;

- **Topographic Survey**
  - Topographic survey at bridge site
  - River cross section survey
- **Subsoil Investigation**
  - Mechanical boring at proposed pier and abutment sites
  - Field test and sampling
  - Laboratory test
  - Existing pile length measurement (if required)
- **River Hydrological Survey**
  - Field inspection from hydraulic viewpoint
  - Hydrological analysis
- **Detailed Structural Survey**
  - Structural details measurement
    - Structural dimension survey
    - Crack/corrosion mapping survey
    - Concrete cover, rebar size and spacing survey
    - Steel thickness measurement

- Material strength measurement
  - Concrete strength
  - Structural steel strength
  - Reinforcement bar strength
- Measurement of material deterioration degree
  - Carbonation test
  - Chloride test
  - Sulphate test
  - Rebar corrosion test
- Bridge function survey
- Full scale bridge loading test (if required)
- **Detailed Rehabilitation Design**
  - Establishment of design criteria
  - Structural assessment of bridges
  - Detailed rehabilitation design
  - Preparation of drawings
  - Estimate of work quantities
  - Construction planning
- **Cost Estimate**
  - Unit price analysis
  - Cost estimate of each project bridge
- **Preparation of Tender Documents**
  - Instructions to tenderers
  - Form of contract
  - General condition of contract
  - Technical specification
  - Bill of quantities
  - Tender drawings

## **16.6 Project Management and Organization**

JKR is the technological arm of the Government and serves as the implementing agency for carrying out infrastructure projects in the whole country. JKR is divided into three levels of management and organization structure that consist of the Federal, State and District levels.

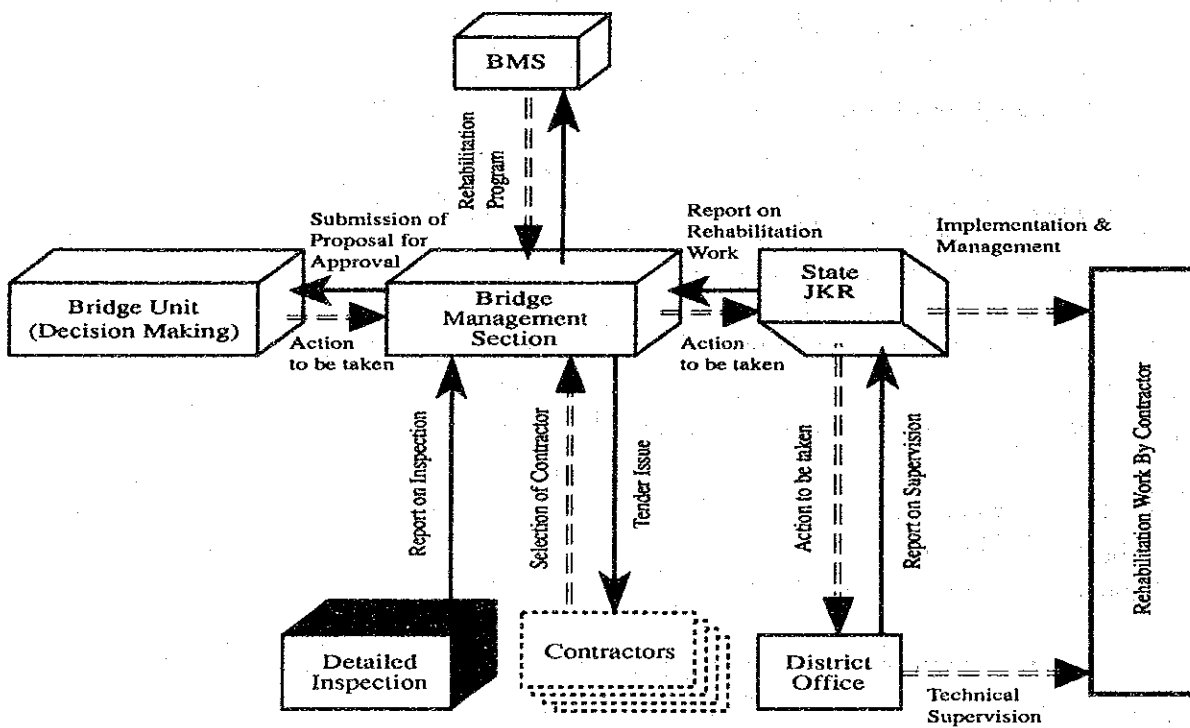
The Roads Branch out of 15 Branches in the Federal JKR is responsible for planning, construction and maintenance of the federal roads and bridges and development schemes as well as formulating roads standards, policies and advising the State JKRs on matters pertaining to the state roads. The Bridge Unit under the Roads Branch is responsible for establishing design standard and carrying out the design, construction and rehabilitation of the federal bridges.

The Federal JKR has 13 State JKR offices located in respective States which are responsible for the planning, implementation and maintenance of the state development projects as well as administration and monitoring the federal projects carried out in the State. Under the State JKRs, there are 72 District Offices on the Peninsular. Each of these is under a District Engineer who is responsible to the State Director for implementation and maintenance of projects in that District.

Under such situation, the Bridge Unit is the executing agency for the implementation of the project. The responsibilities of the agency are to carry out the detailed rehabilitation design and tendering activities. While the State JKR and the District JKR is responsible for construction management and monitoring of the project and the direct construction supervision respectively.

Schematic interrelationship between those agencies is depicted in Figure 16-7 which indicates the direction of flow and responsibility and function of the agencies concerned.

**Figure 16-7 Schematic Interrelationship between the Agencies**



In principle, the project shall be executed on a contract basis by contractors selected through competitive bidding. Therefore the Government must organize a construction supervision team(s) to manage and supervise the contractors' field works but the size of the staff of the organization depends on the number of bridges to be rehabilitated, their location and scale of the respective rehabilitation works. For reference purpose, a typical organization is shown in Figure 16-8, assuming it is required to manage and supervise a subproject covering about 15 to 20 bridges at a time.

**Figure 16-8 Typical Organization for Construction Supervision**

