				1		
	Com	partment		<u>Facility</u>		Main Feautres
					-	
	1.	Northern Compt.				:
		(DC-1)	1.	Embankment	L≖	14.00 km (E.33+200~E.69)
			2.	Sub-Embankment (SA)	L=	6.40 km (SA.0~SA.16)
			3.	Flood Wall (R)	L≔	5.85 km (R.16+150~R.22)
			4.	Sluice Gate	No=	4 Places (Main Emb.3, Sub-Emb.1)
				• •		
	2.	Central Compt.				
	-11	(DC-2)	1.	Embankment	L=	6.00 km (E.18+200~E.33+200)
			2.	Sub-Embankment (SA)	L=	· -
			3.	Flood Wall (R)	L=	4.85 km (R.11+300~R.16+150)
			4.	Sluice Gate	No=	1 Place
					•	
	3,	Southern Compt1				
		(DC-3)	. 1.	Embankment	L=	2.97 km (E.11+150~E.18+200)
			2.	Sub-Embankment	L=	4.71 km (SB.0~SB.12)
			3.	Flood Wall (R)	L=	2.50 km (R.8+300~R.11+300)
	•		4.	Sluice Gate	No=	1 Place
	4.	Southern Compt2			-	
		(DC-4)	1.	Embankment	L=	4.55 km (E.0~E.11+150)
			2.	Sub-Embankment (SA)	L=	6.31 km (SC.0~SC.13)
			3.	Flood Wall (R)	L=	8.07 km (R.0~R.8+800)
			4.	Sluice Gate	No=	1 Place
· · · ·				· · ·		
		Total				
·		(DC 1-4)	1.	Embankment	I.=	27.52 km (E.0~E.69)
			2.	Sub-Embankment (SA)	L=	17.42 km (3 Sub-Embankments)
			3.	Flood Wall (R)	L=	21.27 km (R.0~R.22)
			4.	Sluice Gate	No=	7 Place
	· •					

TABLE E.3.3 PROPOSED FLOOD MITIGATION FACILITY : DHAKA EAST

TABLE E.3.4 LAND USE AND POPULATION OF GREATER DHAKA EAST

Year		1990		2000		2010	
Item	Drainage Area (km2)	Built-up Area (km2)	Population (million people)	Built-up Area (km2)	Population (million people)	Built-up Area (km2)	Population (million people)
Greater Dhaka East (F/S Area)	118.62	23.11	0.638	50.27	1.151	85.50	2.202
Part of Greater Dhaka West (Drainage Related Area)	47.74	43.60	1.512	45.38	1.806	46.90	2.268
Total (Study Area)	166.36	66.71	2.150	95.65	2.957	132.40	4.470

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TABLE E.3.5 MAIN HYDRAULIC FEATURES OF EXISTING MAJOR KHALS : GREATER DHAKA EAST

Dhaka East Zone (DC)

				Max. S	ection			Min. Section							
Khal	Length	Catchment	Width	Depth	Slope	Discharge	Specific	Catchment	Width	Depth	Slope	Discharge	Specific		
	• •	Атеа	•			Capacity	Discharge	Area				Capacity	Discharge		
No.							Capacity						Capacity		
	(m)	(km2)	(m)	(m)	(%)	(m3/s)	(m3/s/km2)	(km2)	(m)	(m)	(%)	(m3/s)	(m3/s/km2		
					a second						:				
К 1	820	2.50	21.60	3.30	0.050	30,9	12.3	0.80	5.80	1.10	0.050	- 1.3	1.		
K 2	1,640	5.95	6.80	1.00	0.025	0.9	0.2	2.00	2.00	0.40	0.025		0.		
K 3	4,100	3.77	82.20	3.60	0.017	81.4	21.6	2.50	35.60	1.50	0.017	8.2	3.		
K 4	440	11.49	13.80	1.40	0.017	2.8	0.2	10.00	9,60	0.80	0.017	0.8	0.		
K 5	3,040	18.34	38.60	2.80	0.025	30,3	1.7	1.76	20.40	0.90	0.025				
K 6	1,940	1.00	13.60	1.20	0.017	2.1	2.1	7,30		0.20	0.017		. 0.		
K 7	640	18.00	12.00	1.30	0.017	2.1	0.1	20.00		1.00	0.017		0.		
K 8	3,540	26.00	11.60	3.30	0.017	9.1	0.3	24.50	9.80	0.90	0.017	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.		
K 9	1,760	0.50	15.40	1.10	0.017	2.1	4.2	3.00		0.90	0.017	i	0.		
K 10	1,600	30.80	14.20	1.80	0.017	4.3	0.1	30.00		1.00	0.017	1 S. 1 S. 1998	0.		
K 11	1,620	6.30	19.20	1.30	0.017	3.5	0.6	1		0.80	0.017	· ·	0.		
K 12	740	38,40	18.60	2.00	0.017	6.8	0.2	l sau		1.60	0.017		0.		
K 13	1,700	4.00	12.40	1.80	0.017	3.8	0.9	1		1.30	0.017	l i i i i i i i i i i i i i i i i i i i	0.		
K 14	740	1.50	10.40	1.60	0.017	2.6	1.7			1.20	0.017	1.1	2.		
K 15	2,600	47.90	39.60	2.60	0.017	22.7	0.5	39.00	10,60	1.40	0.017		<u>,</u> 0.		
K 16	2,200	3.50	43.20	2.90	0.017	29.7	8.5	1.00		1.30		•	1.		
<u>K 17</u>	800	1.00	13.20	1.70	0.017	3.7	3.7			0.50		ş	0.		
K 18	1,920	1.50	13.60	1.50	0.017	3.1	2.1	0.50		0.50			0.		
K 19	3,220	1.00	26.60	1.10	0.017	3.6			3	0.60					
K 20	1,640	33.50	25.40	2.00	0.025	11.4	0.3	32.00		1.70	0.025		0.		
K 21	2,600	40.90	42.60	3.20	0.017	34.5	0.8	34.00	22.00	2.30	0.017	1)		
K 22	5,320	14.00	17.00	3.50	0.025	18.5	1.3	13.50	E		0.025		0.		
K 23	4,820	8.20	34.20	2.50	0.017	18.4	2.2		1 .		0.017	· ·	0.		
K 24	3,420	5.50	18.00	3.10	0.017	13.4	2.4	2.00	7.60	1.10	0.017	1.0	1		
K 25	2,420	1.00	41.40	1.80	0.017	12.9	12.9	8.00	13.20	2.00	0.017	4.8			
K 26	1,800	4.10	29.00	2.70	0.017	17.6	4.3	3.00					l		
K 27	2,400	45.00	57.00	5.30	0.017	106.6	2.4	44.00	56.80	2.70	0.017	34.8	0.		
													<u> </u>		

Note: Roughness Coefficient (n) = 0.035

	s										.*				
	Remarks						EAST		Remarks						
		Мах.	5.15	4.90	4.60	4.55	K DHAKA				al yarold be		-		
· · · · · · · · · · · · · · · · · · ·	E E	Design	3.25	3.15	3.05	3.00	TABLE E.3.6(2) HYDRAULIC REQUIREMENTS OF PROPOSED RETARDING POND : GREATER DHAKA EAST	m, PWD)	T.W.L	3.00	3.00	3.00	3.00	3.00	3.00
	ler	T.W.L	3.00	3.00	3.00	3.00	UNO DND	Design Water Level (m, PWD)	Ţ.	0	0		0	0	0
Design water Level (III, F WD)	Inner	H.W.L	4.00	4.00	4.00	4.00	RETARDI	Design W	H.W.L	4.00	4.00	4.00	4.00	4.00	4.00
Proven 11		T.W.L	3.00	3.00	3.00	3.00	ROPOSED	Storage Capacity	(x 10 ⁶ m ³)	1.38	1.27	5.75	5.59	1.99	2.97
ŗ	Outer	H.W.L	6.25	6.15	6.05	0.00	NTS OF P	Storage	(x1			5	5		5
		H.H.W.L	8.15	7.90	7.60	7.55	TABLE E.3.6(2) HYDRAULIC REQUIREMENTS OF PROPOSED RETARDING POND :	Pond Area	(ha)	138	127	575	339	199	297
LISCHARGE	Capacity	(m ³ /s)	25.60	54.60	53.10	47.20	AULICR				• • • •		*. •		
-	Area	(km ²)	22.11	47.88	46.58	41.34	6(2) HYDI	Drainage	Zone	DC-1	DC-1	DC-2	DC-3	DC-4	DC-4
LTAINAGE LONE	No.		DC-1	DC-2	DC-3	DC-4	ABLE E.3.		pq						
Proposed	Pumping	Station	P 5	P6	P 7A	P 7B	AT .	Proposed	Retarding Pond	RP 5-1	RP 5-2	RP 6	RP 7A	RP 7B-1	RP 7B-2
			1							L		-	÷		

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TABLE E.3.6(1) HYDRAULIC REQUIREMENTS OF PROPOSED PUMPING STATION : GREATER DHAKA EAST

TABLE E.3.7 LAND USE AND POPULATION BY DRAINAGE ZONE IN 1990 AND 2010

33.75 37.52 10.65 8.20 2.79 3.949 3.349 13.10 13.10 13.10 13.396 8.32 8.18 8.18 36.34 3.39 2.98 7.50 66.71 90.36 2.15 4.47 9.29 99.65 166.36 Total 1.74 11.71 9.59 1.29 3.33 9.38 9.38 9.38 9.38 5.68 5.68 41.34 2.35 19.17 20.30 0.97 4.26 3.71 6.68 1.88 1.88 1.87 Total 1.68 5.41 0.31 0.01 0.01 19.67 19.67 19.67 19.67 19.67 21.47 0.96 7.92 8.66 1.17 1.17 1.17 1.17 1.17 0.156 6.20 6.20 0.00 0.00 5.66 5.66 0.43 31.32 DC 4 4-8 $\begin{array}{c} 3.79\\ 0.93\\ 0.12\\ 1.77\\ 1.77\\ 3.18\\ 3.18\\ 0.00\\ 0.00\\ 0.02\\ 0.02\end{array}$ 0.78 0.54 2.58 1.47 1.27 1.27 1.57 0.28 9.32 9.32 9.32 0.07 0.07 10.02 4-A 9.28 9.28 2.17 2.13 2.18 2.18 2.18 2.18 0.00 1.70 6.4 4 4 46.58 4.00 4.00 14.80 1.42 2.69 4.68 31.65 1.408 1.085 1.085 1.4.93 0.95 Total 2.87 2.70 0.59 0.27 0.27 0.27 0.00 5.66 5.66 1.41 10.70 2.46 13.16 0.21 14.57 0.56 0.02 DC 3 ц т 1.49 0.93 9.60 6.58 1.58 1.85 2.18 9.44 9.44 0.00 0.78 0.78 4.06 3.44 13.95 1.42 1.42 2.69 4.68 30.24 8.015 0.15 1.62 32.01 3-A 0.48 0.15 5.11 10.21 1.86 1.37 1.37 0.03 31.25 5.93 5.93 16.63 0 0.45 10.73 34.82 2.33 2.33 37.15 0.31 9.96 0.01 47.88 Total 0.26 0.0 2.57 6.65 0.88 0.71 0.71 4.71 15.52 15.52 10.70 5.82 16.52 32.04 0.20 3.15 0.01 3.36 26.99 1.69 28.68 ç ç 0.22 2.49 3.51 0.93 0.03 0.03 0.03 0.03 0.07 0.07 ğ 0.6 0.01 3.33 6.41 6.80 10.13 0.11 0.11 3.21 2-B 0.0 0.05 0.05 0.05 0.05 0.05 0.05 0.00 0.04 0.04 5.71 3.60 0.44 1.42 0.25 1.67 2-A 4.46 8.44 1.38 1.38 1.38 5.53 2.5.35 2.5.35 2.5.35 2.5.35 2.5.35 2.81 5.21 0.55 0.08 0.16 4.90 0.08 0.02 5.16 24.39 30.56 1.01 0 0 ñ Residencial (high)
Residencial (middle)
Residencial (low)
Residencial (low)
Commercial
Industry
Industry Residencial (high)
Residencial (middle)
Residencial (low)
Residencial (low)
Commercial
Industry
Industry
Industry
Industry
Industry
Industry
Nub-Total
Sub-Total
Sub-Total Residencial (middle) Sub-Total (buil-up area Population (x10 6) Population (x10 6) (7) Agriculture(8) Water BodySub-Total Area Land Use (km2) Land Use (km2) Area (km2) Item E E ε E

TABLE E.3.8 RUN-OFF COEFFICIENT : GREATER DHAKA EAST

Dhaka East Zone (DC)

		Run-off				Land Use (%)				
Block	Area	Coefficient		Residential		Commercial	Industrial	Institutioni	Open Space	Waterbodies
No.			High Density	Midium Density			· · ·		/Agrecultural 0.20	1.00
	km2	1	0.50	0.50	0.30	0.65	0.55	0.30	0.20	1.00
DC-1-1	2.53	0.39	12.03	24,05	24.05	5.70	1.11	17.50		1.31
DC-1-2	3.95	0.40				5.35	0.96	17.75	11.23	1.16
DC-1-2	5.01	0.44				4.19	0.45	18.57		0.63
DC-1-3	1.76	0.38		29,16		2.80	0.22	17.58	24.97	0.71
DC-1-5	5.09	0.33		19.70		2.81	0.49	11.78	42.42	0,90
DC-1-5	3.77	0.32				3.51	0.68	10.77	47.23	0.80
DC-1-7	3.38	0.39				5.70	1.11	17.50	14.25	1.3
		0.39				5,70	1.11	17.50	14.25	1.3
DC-1-8 DC-1-9	2.57 2.50	0.39			-	4.82	0.15	33.09	1.17	0.1

	(DC-2)								1997 - 1997 1997 - 1997	
······································		Run-off				Land Use (%)				
Block	Area	Coefficient		Residential		Commercial	Industrial	Institution	Open Space	Waterbodias
No.			High Density	Midium Density	Low Density				/Agrecultural	
	km2	1	0.50	0.50	0.30	0.65	0.55	0.30	0.20	1.00
DC-2-1	5.71	0.38	18.18	13.92	3.57	4.08	0.00	59.43	0.00	0.82
	1.79	0.40			3.85	4.72	0.00	55.25	0.00	1.48
DC-2-2	4.08	0.40			10.38	4.19	0.31	22.23	0.00	0.45
DC-2-3		0.45			11.45	4.03	0.38	. 18.69	0.00	0.55
DC-2-4	2.97						0.18	38.20	0.00	1.05
DC-2-5	1.29	0.42		15.81	3.36			29.76	33.05	1.36
DC-2-6	4.54	0.34		21.27	4.51	4.24	0.00	41.18		1.40
DC-2-7	3.52	0.40			4.20		0.00	14.35		1.27
DC-2-8	2.36	0.35			5.59		0.00	20.40		1,29
DC-2-9	3.33	0,40					0.00	18.22		
DC-2-10	1.25	0.38			5.09		0.00	18.22		1.26
DC-2-11	4.98	0.38						18.22		1.28
DC-2-12	1.34	0.38	15.26				0.00			1.01
DC-2-13	1.23	0.35	12.03	24.10		2.36	0.00	14.37		
DC-2-14	4.48	0.24	3,84	7.67	1.28	0.10	0.00	3.86		0.54
DC-2-15	5.01	0.25	3.94	7,88	1.31	0.10	0.00	3.96	82.24	0.56

	(DC-3)			•						
·		Run-olf				Land Use (%)				
Block	Area	Coefficient		Residential		Commercial	Industrial	Institution	Open Space	Waterbodies
No.			High Density	Midium Density	Low Dansity				/Agrecultural	
• • • •	km2	f	0.50	0.50	0.30	0.65	0.55	0.30	0.20	1.00
	8,81	0.47	40.87	9,45	0.32	10.51	6.63	30.02	0.00	2.20
DC-3-1	3.23	0.50			4.26		24,96	10.00	0.00	2.79
DC-3-2	5.54	0.42		21.06	7.71	3.47	0.00	35,98	4.67	2.30
DC-3-3	7.24	0.43			5.21	4.52	0.17	36.68	0.00	2.28
DC-3-4	6.29	0,43			4.06		7.16	38.88	0.00	1.55
DC-3-5		0.43			6.76		0,00	25.04	10.61	1.94
DC-3-6	2.38	0.38			4.96		0.00	18.13	27.79	1.14
DC-3-7	3.42	0.34			3.71	2.42	0.00	13.57	45.97	0.86
DC-3-8	2.09	0.34					0.10	9.98	59.98	0.52
DC-3-9	1.86				5.29		0.00	9.03	68.27	0.43
DC-3-10	4.10	0.27			14.70		0.00	25.08		1.19
DC-3-11 DC-3-12	0.69 0.93	0.40 0.27			5.62		0.00	9.59		0.45

		Run-oll				Land Use (%)			<u> </u>	
Block	Area	Coefficient		Residential		Commercial	Industrial	Institution	Open Space	Waterbodie
No.			High Density	Midium Density					/Agrecultural	
	km2	f	0.50	0.50	0.30	0.65	0.55	0.30	0.20	1.00
DC-4-1	7.25	0.47	35.05	5,94	0.45	21.78	2.79	33.99	0.00	.00
DC-4-2	2.11	0.48			2.41	5.68	0.61	12.84	0.00	0.7
DC-4-2 DC-4-3	3.67	0.47			4.34	6.97	0.22	15.71	0.00	0.5
	1.03	0.43			3.58		0.18	12.97	17.48	0.4
DC-4-4		0.44			0.04		0.00	41.81	0.12	0.9
DC-4-5	2.72	0.44			0.04		0.00	41.81	0.12	0.9
DC-4-6	0.61				4.01	8.95	0.15	22.52	2.82	0.6
DC-4-7	2.49	0.46			5.48		0.13	9.77	60.00	0.5
DC-4-8	0.70			30.91	9.27		0.22	16.52		. 0.8
DC-4-9	1.39						0.79	19.16		0.8
DC-4-10	5.30						0.52	14.70		0.7
DC-4-11	3.24						1.13	31.90		
DC-4-12	2.91	0.44					0.68	19.09		
DC-4-13	2.56				7.45		0.12	8.96	and the second	0.4
DC-4-14	0.30						0.12	5.94		0.3
DC-4-15	1.11	0.26						20.62		
DC-4-16	1.10	0.43	11.23				0.42			
DC-4-17	0.30	0.46	14.91				0.68	19.09		
DC-4-18	1.96	0.44	27.04	29.90			1.13	31.90		
DC-4-19	0.59	0.46	14.91	52.18	7.45	4.39	0.68	19.09	0.00	- 1.e

(00-1)

Condition Block	19: Inlet Time Drainage		Run ofi	Coefficier	Lengi	h		Time of	Rainfal	Areal	Design
No.	Individual Ac	cumlated	Individual Accumlated		Individual Accumlated		Velocity	Concentration	Intensity	Reduction	Discharge
	а	A	1	F.	1	L	v	1	r	Factor	
	(km2)	(km2)			(km)	(km)	(m/s)	(min)	(mm/hr)	(m3/s/km)	(m3/s)
DC-1-1	2.53	2,53	0.39	0.39	2.30	2.30	0.80	68	76.37	0.98	20.5
DC-1-2	3.95	6.48	0.40	0,40	1.70	4.00	0.80	103	58.73	0.96	40.3
DC-1-3	5.01	11.49	0.44	0.42	2.30	6,30	0.80	151	46.71	0.94	58.5
DC-1-4	1.76	1.76	0.38	0.38	2.00	2.00	0.80	62	80.64	0.99	14.9
DC-1-5	5.09	18.34	0,33	0.39	1.90	8,20	0.80	191	40.67	0.92	74.5
DC-1-6	3.77	22.11	0.32	0.38	0.50	8.70	0.80	201	39.33	.0.91	83.2
DC-1-7	3.38	3.38	0.39	0.39	2.50	2.50	0.80	72	73,76	0.98	26.5
DC-1-8	2.57	5.95	0.39	0.39	1.30	3.80	0.80	99	60.37	0,96	37.3
DC-1-9	2.50	2.50	0.42	0.42	2.20	2.20	0.80	66	77.74	0.99	22.6

(DC-2)

Block	Drainage	Area	Run-of	Coefficier	Leng	(ի 🦷		Time of	Rainfal	Areal	Design
No.	Individual Ac	cumlated	Individual Ad	cumlated	Individual Ac	cumlated	Velocity	Concentration	Intensity	Reduction	Discharge
	a	A	1997 - 1 99	F	1 E	L	utu V	t E	t station	Factor	2.1.1
<u> </u>	(km2)	(km2)			(km)	<u>(km)</u>	(m/s)	(min)	(mm/hr)	(m3/s/km)	(m3/s)
DC-2-1	5.71	5.71	0.38	0.38	3.20	3.20	0.80	87	65.89	0.96	38.
DC-2-2	1.79	7.50	0.40	0.39	1.40	4.60	0.80	116	54.30	0.95	41.6
DC-2-3	4.08	4.08	0.44	0.44	2.10	2.10	0.80	64	79.16	0.97	38.4
DC-2-4	2.97	7.05	0.45	0.45	2.00	4.10	0.80	105	57.94	0.96	48.
DC-2-5	1.29	15.84	0.42	0,42	1.00	5.60	0.80	137	49.42	0.92	83,
DC-2-6	4,54	20.38	0.34	0.40	2.20	7.80	0.80	183	41,81	0.91	86.
DC-2-7	3.52	3.52	0.40	0.40	3.10	3,10	0.80	85	66.91	0.97	25.5
DC-2-8	2.36	26.26	0,35	0.40	2.00	. 9.80	0.80	224	36.67	0.68	93.0
DC-2-9	3.33	3.33	0.40	0.40	2.40	2.40	0.80	70	75.04	0.98	27.
00-2-10	1.25	30.84	0.38	0.40	1.40	11.20	0,80	253	33.77	0.86	98.
00-2-11	4.98	4.98	0.38	0.38	2.20	2.20	0.80	66	77.74	0.97	40.
C-2-12	1.34	6.32	0.38	0.38	1.60	3.80	0.80	99	60,37	0.96	. 39.1
C-2-13	1.23	38.39	0.35	0.39	0.70	11.90	0.80	268	32.48	0.85	115.
00-2-14	4.48	4,48	0.24	0.24	2.80	2.80	0.80	78	70.17	0.97	20.3
00-2-15	5.01	47.88	0.25	0,36	2.50	14.40	0.80	320	28.59	0.83	114.0

(DC-3)

Block	Drainage	Area	Run-off	Coefficier	Long	th		Time of	Rainfal	Areat	Design
No.	Individual Ac	cumlated	Individual Ac	cumlated	Individual Ac	cumlated	Velocity	Concentration	intensity	Reduction	Discharge
	a	A	f	F	₽ °	L	· V	t	r	Factor	
	(km2)	<u>(km2)</u>			(km)	<u>(km)</u>	(m/s)	(min)	(mm/hr)	(m3/s/km)	(m3/s)
DC-3-1	8.81	8.81	0.47	0.47	4.07	4.07	1.00	88	65.33	0.95	71.3
DC-3-2	3.23	12.04	0.50	0.48	2.40	6.47	1.00	128	51.22	0.94	77.0
DC-3-3	5.54	5.54	0.42	0.42	5.20	5.20	0,80	128	51.11	0.96	31.4
DC-3-4	7.24	7.24	0.43	0.43	4.60	4.60	0.80	116	54.30	0.96	45.5
DC-3-5	6.29	6.29	0.43	0.43	3.20	3.20	0.80	87	65.89	0.96	47.8
DC-3-6	2.38	33.49	0.42	0.45	2.30	8.77	0,80	203	39.15	0.86	139,6
DC-3-7	3.42	3.42	0.38	0.38	2,60	2.60	0.80	74	72.52	0.97	25.5
DC-3-8	2.09	5.51	0.34	0.36	1.80	4.40	0.80	112	55,70	0.96	29.8
DC-3-9	1.86	40.86	0.30	0.43	2.70	11.47	0.80	259	33.26	0.85	137.4
DC-3-10	4.10	44.96	0.27	0.41	2.20	13.67	0.80	305	29.63	0.83	127.1 (120.
DC-3-11	0.69	0.69	0.40	0.40	1.20	1.20	0.80	45	94.79	1.00	7.2
DC-3-12	0.93	1.62	0.27	0.33	1.20	2.40	0.80	70	75.04	0.99	10.9

(DC-4)

Conditio Block	Drainage /	20min Vea	Bun-of	Coefficier	Leng	th		Time of	Rainfal	Areal	Design
No.	Individual Acc		Individual Ad	_			Velocity	Concentration	Intensity	Reduction	Discharge
an lan	a (km2)	A (km2)	,Ejs El	F	l (km)	.L. (km)	V (m/s)	t (min)	r (mm/hr)	Factor (m3/s/km)	(m3/s)
DC-4-1	7.25	7.25	0.47	0.47	4.00	4.00	1.00	87	65.89	0.95	58.6
DC-4-2	2.11	9.36	0.48	0.47	1.40	5.40	1.00	110	56.28	0.94	64.5
DC-4-3	3.67	3.67	0.47	0.47	3.30	3,30	0.80	. 89	64.90	0.97	30.4
DC-4-4	1.03	14.06	0.43	0.47	2.20	7.60	0.80	178	42.40	0.93	71.9
DC-4-5	2.72	2.72	0.44	0.44	1.90	1.90	0.80	60	82.17	0.98	26.5
DC-4-6	0.61	3.33	0.44	0.44	1.30	3.20	0.80	87 🖈	65.89	0.98	26.0
DC-4-7	2,49	5.82	0,46	0.44	1.10	4.30	0.80		56.43	0.96	38.9
DC-4-8	0.70	20.58	0.29	0.45	1.00	8.60	0.80	199	39.59	0.91	93.6
DC-4-9	1.39	1.39	0.36	0.36	1.90	1,90	0.80	60	82.17	0.99	11.3
DC-4-10	5.30	5,30	0.37	0.37	2.70	2.70	0.80	76	71.33	0.96	36.8
DC-4-11	3.24	8.21	0.35	0.39	2.20	4.90	0.80	122	52.46	0.95	44.6 (25.0
DC-4-12	2,91	2.91	0.44	0.44	3.00	3.00	0.80	83	67.96	0,98	23.8
DC-4-13	2.56	5.47	0.46	0.45	2.20	5.20	0.80	128	51.11	0,96	33.5
DC-4-14	0,30	14.31	0.29	0.99	0.30	5,50	0.60	135	49.83	0.93	72.5 (35.0
DC-4-15	1.11	37.39	0.26	0.42	1.90	10.50	0.80	239	35.16	0.85	131.0 (115.
DC-4-16	1.10	1.10	0.43	0.43	1,70	1.70	0.80		85.42	0.99	11.0
DC-4-17	0.30	38,79	0.46	0.42	0.50	11.00	0.80		34.15	0.85	132.1 (120.
DC-4-18	1.96	1.96	0.44	0.44	1.60	1.60	0.80		87.15	0.99	20.8
DC-4-19	0,59	2.55	0.46	0.45	1.20	2.80	0.80		70.17	0.98	21.8

Note: The figures in parenthesis indicate the design discharge estimated in hydraulic simulation by Mike 11 model. considering the storage effect of the retarding area.

TABLE E.3.10 HYDRAULIC DESIGN OF KHAL IMPROVEMENT : GREATER DHAKA EAST

Dhaka East Zone (DC)

Khal	Design		Section		Roughness	Slope	Vetosity	Discharge
: ¹	Discharge	Bottom Wid.	Upper Wid.	Height	Coefficient	i (%)		Capacity
No.	(m3/s)	(m)	(m)	(m)			(m/s)	(m3/s)
						·.	· · · · ·	
(DC-1) KD-1-1	83.2	20.00	34.80	3,70	0,035	0.022	0,84	85.
	74.6	17.50	32.30	3.70	0.035	0.022	0.83	76.
KD 1-2			27.80	3.70	0.035	0.022	0.80	60.
KD-1-3	58.5	13.00					0.80	40.
KD-1-4	40.3	7.50	22.30	3.70	0.035	0.022		40. 22.
KD-1-5	20.5	2.00	16.80	3.70	0.035	0.022	0.65	
KD-2	14.9	2.00	16.32	3.58	0.035	0.025	0.67	22.
KD-3-1	37.3	5.00	21.00	4.00	0.035	0.022	0.74	38.
KD-3-2	26.5	2.00	18.00	4.00	0.035	0.022	0,68	27.
KD-4	22.6	2.50	10.50	4.00	0.025	0.022	0.91	23.
(DC-2)	:							
KD-5-1	115.5	27.50	43.50	4.00	0.035	0.018	0.82	117.
KD-5-2	115.5	27.50	43.50	4.00	0.035	0.018	0.82	117.
KD-5-3	98.5	23.00	39.00	4.00	0.035	0.018	0.81	100
KD-5-4	93,1	21.50	37.50	4.00	0.035	0.018	0.80	94
KD-5-5	86.2	19.50	35.50	4.00	0.035	0.018	0.79	87.
KD-5-6	83.2	16.00	31.68	3.92	0.035	0.025	0.89	83.
KD-5-7	41.6	6.50	21.70	3.80	0.035	0.025	0.78	41.
KD-5-8	38.6	6.00	21.00	3.75	0.035	0.025	0.77	39.
KD-6	20.7	2.00	17.40	3.85	0.035	0.025	0.70	26.
KD-7-1	40.1	7.00	23.00	4.00	0.035	0.018	0.69	41.
KD-7-2	40.1	6.00	21.28	3.82	0.035	0.025	0.78	40.
KD-8	27.5	2.50	17.90	3,85	0.035	0.025	0.72	28.
KD-9	25.2	2.00	17,68	3.92	0.035	0.025	0.71	27.
KD-10-1	48.6	9.00	24.00	3,75	0.035	0.025	0.81	50.
KD-10-2	38.6	7.00	14.16	3.58	0.025	0.025	1.07	40
(DC-3)			· ·					
KD-11-1	120.0	24.50	41.70	4.30	0.035	0.018	0.85	120.
KD-11-2	139.6	29.00	46.20	4.30	0.035	0.018	0.86	139.
KD-11-3	139.6	26.50	42.98	4.12	0.035	0.025	0.98	140.
KD-12-1	10.9	2.00	17.20	3.80	0.035	0.025	0.70	25.
KD-12-2	7.2	2.00	15.04	3.26	0.035	0.025	0.64	17.
KD-13-1	29.8	2.00	18.60	4.15	0.035	0.025	0.74	31.
KD-13-2	25.5	2.00	18.20	4.05	0.035	0.025	0.73	29.
(DC-4)	20.0	2.00	10.20	4.00	0.000	0.020	0.10	
KD-14-1	120.0	24.50	41.70	4.30	0.035	0.018	0.85	120.
KD-14-7 KD-14-2	115.0	23.50	40.70	4.30	0.035	0.018	0.84	116.
		18.50	35.70	4.30	0.035	0.018	0.82	95.
KD-14-3	93.6				0.035	0.018	1.09	74.
KD-14-4	71.9	11.50	20.10	4.30				73.
KD-14-5	71,9	10.00	18.36	4.18	0.025	0.025	1.23	
KD-15-1	21.8	2.00	18.80	4.20	0.035	0.025	0.74	32.
KD-15-2	20.8	2.00	16.48	3.62	0.035	0.025	88.0	22.
KD-16	11.0	2.00	16.40	3.60	0.035	0.025	0.68	22.
KD-17-1	35.0	3.00	20.00	4.25	0.035	0.025	0.77	37.
KD-17-2	25.0	2.00	18.28	4.07	0.035	0.025	0.73	30.
KD-17-3	36.8	5.00	20.36	3.84	0.035	0.025	0.76	37.
KD-18-1	33.6	3.00	19.28	4.07	0.035	0,025	0.75	34.
KD-18-2	23.8	2.00	16.80	3.70	0.035	0.025	0.69	23.
KD-19	11.3	2.00	16.04	3.51	0.035	0.025	0.67	21.
KD-20-1	38.9	3.50	20.34	4.21	0.035	0.025	0.78	38.
KD-20-2	26.5	3.00	11.20	4.10	0.025	0.025	1.00	29.
KD-20-3	26.5	3.00	11.00	4.00	0.025	0.025	0.99	27.

TABLE E.3.11

PROPOSED KHAL IMPROVEMENT WORKS : GREATER DHAKA EAST

Dhaka East Zone (DC)

	:		Required		pen		rered	Buidan	Aquedua	Dredging	Maintenance	Land
Zone	Khal	Khal	Hydraulic	and the second sec	hannel		innel	Bligge	Aqueduct	oradging	Road	Acquisitio
		Length	Section	Brick	Sodding	Box	Brick	0	(1)		rioau	Requisitio
			Wb x Wu x H	Protection		Culvert	Pipe	(Places)	(Piaces)	(1000m3)	(km)	(ha)
·	No.	<u>(km)</u>	(m x m x m)	(km)	<u>(km)</u>	<u>(km)</u>	<u>(km)</u>	<u>+</u>		(1000113)		<u>(ia/</u>
	KD-1-1	0.50	20.0 34.8 3.7		0.50			_	-	25.10	0.50	1.88
	KD-1-2	1.90	17.5 32.3 3.7		1.90				-	36.90	1.90	2.35
	KD-1-2	2.30	13.0 27.8 3.7		2.30		-	_		65,95	2.30	7.47
-	KD-1-4	1.70	7.5 22.3 3.7		1.70	-	-	-	_	32.30	1.70	6.72
DC-1	KD-1-5	1.00	2.0 16.8 3.7		1.00	-	_	l .	_	22.90	1.00	2.93
	KD-1-3	1.40	2.0 16.3 3.6	_	1.40		-	-		1.86	1.40	1.66
	KD-3-1	1.30	5,0 21.0 4.0		1.30	_	_			0.00	1.30	3.88
	KD-3-2	1.40	2.0 18.0 4.0		1.40	_	-		-	0.00	1.40	3.85
	KD-4	1.20	2.5 10.5 4.0	1.20			-] .	-	3.86	1.20	1.63
	Sub-Total	12.70	2.0 10.0 1.0	1.20	11.50	0.00	0.00	0	0	188.87	12.70	32.37
	000-10181	12.70	· · · · · · · · · · · · · · · · · · ·			0.00		<u> </u>	-			
											0.50	
	KD-5-1	2.50	27.5 43.5 4.0	-	2.50	-	-	-	•	148.44	2.50	8.01
	KD-5-2	0.70	27.5 43.5 4.0	-	0.70	-	-	•	-	49.47	0.70	3.18
	KD-5-3	1.40	23.0 39.0 4.0		1.40	-	-	•	-	79.81	1.40	5.55
	KD-5-4	2.00	21.5 37.5 4.0		2.00	-	-	-	-	81.70	2.00	6.80
	KD-5-5	2.20	19,5 35.5 4.0	-	2.20	-	-	-	·	61.03	2.20	8.67
00-2	KD-5-6	1.00	16.0 31.7 3.9	-	1.00	-	-	•	-	32.40	1.00	4.40
	KD-5-7	1.40	6.5 21.7 3.8	-	1.40	-	-	-	-	18.80	1.40	5.18 3.08
	KD-6	1.80	2.0 17.4 3.9		1.80		-		-	13.62	1.80	1
	KD-7-1	1.60	7.0 23.0 4.0		1.60		-	- ·	•	27.68	1.60	3.53
	KD-7-2	2.20	6.0 21.3 3.8		2.20	- ·	• •	-	· •	23.40	2.20	4.54 2.85
	KD-8	1.80	2.5 17.9 3.9	-	1.80		-	-	-	6.91	1,80	
	KD-9	1.00	2.0 17.7 3.9	-	1.00	-	-		1	1.59 39.34	1.00	1.59 7.06
•	KD-10-1	2.00	9.0 24.0 3.8	-	2.00		-	3				7.29
	KD-10-2	2.10	7.0 14.2 3.6	2.10	-			5	0	154.25 738.44	2.10 23.70	71.73
1	Sub-Total	23.70	• •	2.10	21.60	0.00	0.00		Ŭ.	/ 30.44	23.10	1.73
										÷.		
	KD-11-1	2.20	24.5 41.7 4.3	-	2.20	-	-	• 1	- .	135.81	2.20	2.38
	KD-11-2	2.70	29.0 46.2 4.3	· -	2.70	-	-	-		155.96	2.70	5.49
	KD 11-3	1.70	26.5 43.0 4.1	-	1.70	-	-	-	-	110,73	1.70	6.66
DC-3	KD-12-1	1.20	2.0 17.2 3.8	-	1.20	-	-	-	-	37.82	1.20	2.97
	KD-12-2	1.30	2.0 15.0 3.3	· ·	1.30	-	-	1	-	35.89	1.30	4.44
	KD-13-1	1.80	2.0 18.6 4.2	-	1.80	-	-	· ·	· •	14.81	1.80	2.73
	KD-13-2	1.20	2.0 18.2 4.1	· ·	1.20					4.01	1.20	1.23
	Sub-Total	12.10		0.00	12.10	0.00	0.00	0	0	495.03	12.10	25.90
<u> </u>					<u> </u>	<u> </u>		<u> </u>	¦	. :		
	KD-14-1	0.50	24.5 41.7 4.3	· -	0.50	- ⁻	-		1.1	159.04	0.50	5.60
	KD-14-2	1.90	23.5 40.7 4.3	1 · · -	1.90	-	ļ .	1	-	153.30	1.90	8.62
	KD-14-3	1.00	18.5 35.7 4.3	· -	1.00	-	-	.	+	117.04	1.00	6.41
	KD-14-4	0.70	11.5 20.1 4.3	0.70	-	-		-	-	45.15	0.70	1.27
	KD-14-5	1.50	10.0 18.4 4.2	1.50	-	-	i -	1	•	111.59	1.50	4.32
	KD-15-1	1.20	2.0 18.8 4.2	- 1	1.20	-	-	-	- 1	61.56	1.20	2.96
•	KD-15-2	1.40	2.0 16.5 3.6	-	1.40	-	-	-	-	86.64	1.40	5.05
DC-4	KD-16	1.70	2.0 16.4 3.6		1.70	-	-	-	-	78.98	1.70	5.14
	KD-17-1	0.60	3.0 20.0 4.3	1	0.60	-	-	-		6.40	0.60	0.36
	KD-17-2	2.20	2.0 18.3 4.1		2.20		· - ·	1	•	81.88	2.20	3.46
	KD-17-3	2.70	5.0 20.4 3.8		2.70	. •	-	-	1 - ¹	117.99	2.70	4.47
:	KD-18-1	2.20	3.0 19.3 4.1		2.20	· -	-	-	-	56.14	2.20	4.23
	KD-18-2	0.90	2.0 16.8 3.7		0.90	1 ² -		-	-	19.80	0.90	1.87
5.1	KD-19	1.90	2.0 16.0 3.5	-	1.90	-	- 1	-	-	14.16	1.90	1.65
	KD-20-1	1.10	3,5 20,3 4.2	-	1.10		· .	1		47.40	1.10	3.45
	KD-20-2	1.30	3.0 11.2 4.1	1,30	•	-	.	-	-	34.40	1.30	3.14
	KD-20-2	1.30	3.0 11.0 4.0	1.30	-	-	i -	-	.	46.24	1,30	4.53
ļ.	Sub-Total	24.10		4.80	19.30	0.00	0.00	4	0	1237.71	24.10	66.53
										0000.07		100 50
	Total	72.60		8.10	64.50	0.00	0.00	12	0	2660.05	72.60	196.

TABLE E.4.1 PROPOSED FLOOD MITIGATION FACILITY : DND AREA

<u>Route</u> (Total

Facility

Main Features

1 place

. : -

<u>Ro</u> (To	<u>ite</u> tal Length)		Facility	• •	Main Peatures
1.	Chasara to Buriganga Bridge (DW)	1)	Flood Wall Construction	•	
	(L= 10.63 km)	2)	Rehabilitation Work		
		~)	(1) Foot Protection	•	L = 3.63 km
			(2) Flood Wall Raising	:	L = -
		3)	Stop Log Structure	:	14 places
2.	Buriganga Bridge to Demra (DN)	1)	Flood Wall Construction	•	L = 0.58 km
	(L= 8.58 km)				
		2)	Rehabilitation Work		
			(1) Foot Protection	:	L = 5.60 km
			(2) Flood Wall Raising	:	L = 4.40 km
		3)	Stop Log Structure	:	17 places
3.	Chasara to Hajiganj (DS)	1)	Flood Wall Construction	•	L = 1.75 km
	(L= 2.15 km)		· · · · · · · · · · · · · · · · · · ·		
4.	Hajganj to Demra (DE)	1)	Flood Wall Construction	:	L = 1.05 km
	(L= 10.16 km)				
		2)	Rehabilitation Work	:	
			(1) Foot Protection	•	L = 8.40 km
			(2) Flood Wall Raising	:	L = 3.20 km
		3)	Stop Log Structure	:	27 places
		4)	Sluice Gate	•	1 place
	Total			·:.	
		1)	Flood Wall Construction	•	3.38 km
		2)	Rehabilitation Work	•	
			(1) Foot Protection	•	17.63 km
			(2) Flood Wall Raising	•	7.60 km
		3)	Stop Log Structure	•	58 places

4) Sluice Gate

Drainage		1990		2000		2100
Area (km ²)	Buil-up Area (km ²)	Population ' (million people)	Buil-up Area (km ²)	Population (million people)	Buil-up Area (km ²)	Population (million people)
56.79	21.74	0.449	36.14	0.880	42.70	1.314

TABLE E.4.2 LAND USE AND POPULATION OF DND

TABLE E.4.3 MAIN HYDRAULIC FEATURES OF EXISTING MAJOR KHALS : DND

DND Project Area (NA)

					19 - A				<u>.</u>				
1.1.1				Max. S	ection						Section	·	
Khal	Length	Catchment	Width	Depth	Slope	Discharge	Specific	Catchment	Width	Depth	Slope	Discharge	Specific
		Area				Capacity	Discharge	Area				Capacity	Discharge
No.							Capacity						Capacity
	(m)	(km2)	(m)	(m)	(%)	(m3/s)	(m3/s/km2)	(km2)	(m)	(m)	(%)	(m3/s)	(m3/s/km2)
K 1	3,200	2.80	5.60	1.20	0.017	0.8	0.3	0.80	3.20	0.60	0.017	0.2	0.2
K 2	3,280	1.50	23.80	2.80	0.050	26.2	17.4	7.00	3.60	0.90	0.050	0.6	0.1
K 3	2,200	9,50	10.20	2.00	0.017	3.6	0.4	10.60	5.60	1.40	0.017	1.1	0.1
K 4	1,660	21.00	14.00	2.00	0.017	5.1	0.2	21.30	12.00	0.90	0.017	1.2	0.1
K 5	1,200	24.00	43.20	4.80	0.017	68.2	2.8	25.00	19.20	2.70	0.017	11.5	0.5
K 6	900	25.00	58.40	6.90	0.017	168.4	6.7	25.00	31.40	2.30	0.017	14.7	0.6
K 7	3,600	1.00	15.60	2.40	0.017	7.6	7.6	2.30	10.40	0.90	0.017	- 1.0	0.4
K 8	1,000	1.40	18.60	3.00	0.050	22.6	16.1	0.70	6.40	1.40	0.050	1	3.0
K 9	2,480	7.00	14.40	2.90	0.017	9.5	1.4	8.00	8.60	1.40	0.017	1.7	0.2
K 10	820	11.00	11.80	1.50	0.017	2.7	0.2	11.30	10.60	1.30	0.017	1.9	
K 11	3,000	25.00	26.60	3.60	0.017	25.8	1.0	1.50	13.60	1.00	0.017		1.1
K 12	2,240	13.50	7.60	· 1.90	0.017	2.4	0.2	12.50	7.00	1.00	0.017	0.8	
K 13	320	55.80	2.40	0.40	0.017	0.1	0.0	55.80	2.40	0.40	0.017	E -	0.0
K 14	1,380	1.80	6.80	1.80	0.017	2.0	1.1	0.80	4.60	0.70			0.4
K 15	2,200	10.00	23.00	1.00	0.017	2.7	0.3	12.80	4.80	0.70	0.017	i i	0.0
K 16	1,400	5.00	10.20	3.00	0.017	6.8	1.4	7.60	6.20			•	0.0
K 17	2,600	2.50	29.40	2.90	0.025	24.4	9.7	3.70	10.40	1.10	0.025		0.5
K 18	1,600	0.20	16.00	3.00	0.050	19.2	96.2	1.00	13.00	2.20	0.050	9.4	9.4
			and the second								L	l	

Note : Roughness Coefficient (n) = 0.035

TABLE E.4.4

		1										
		То	tal opera	tion of Pu	imp in ho	our	Water	level			(feet P)	WD)
Year	Α	В	C	A+B	B+C	A+B+C	Rive	r side	Count	ry side	Main	Canal
	Gate	Gate	Gate	Gate	Gate	Gate	max.	min.	max.	min.	max.	min.
1970	2,166	327	3,995	2,493	4,322	6,488	18.70	2.00	8.80	2.30	15.60	<u>13.50</u>
1971	2,066	316	6,638	2,382	6,954	9,020	18.20	1.60	9.70	1.60	16.20	11.00
1972	3,654	528	2,160	4,182	2,688	6,342	16.00	1.20	7.60	1.60	15.40	10.50
1973	2,958	393	5,306	3,351	5,699	8,657	17.30	0.90	8.70	1.00	15.50	12.00
1974	2,944	481	6,370	3,425	6,851	9,795	19.70	1.70	8.20	1.40	15.60	13.20
1975	3,804	499	4,439	4,303	4,938	8,742	16.50	0.80	9.40	2.50	15.50	8.60
1976	3,623	712	4,588	4,335	5,300	8,923	16.30	1.00	8.80	1.60	15.80	9.00
1977	2,925	670	4,147	3,595	4,817	7,742	17.20	0.30	8.50	2.00	15.90	-
1978	3,824	604	4,873	4,428	5,477	9,301	16.10	0.05	8.40	2.70	15.90	11.40
1979	4,381	718	4,856	5,099	5,574	9,955	16.30	1.00	9.00	1.60	15.70	11.30
1980	3,597	653	4,530	4,250	5,183	8,780	18.30	1.00	7.50	0.00	15.80	11.50
1981	3,026	735	4,132	3,761	4,867	7,893	16.70	1.00	7.90	2.00	15.50	10.90
1982	3,404	528	2,703	3,932	3,231	6,635	15.90	0.00	7.90	1.40	15.90	11.50
1983	2,428	544	5,814	2,972	6,358	8,786	17.20	0.00	9.70	0.50	15.60	11.50
1984	3,168	332	7,921	3,500	8,253	11,421	18.10	0.00	9.40	0.00	15.70	11.00
1985	2,560	533	3,645	3,093	4,178	6,738	16.50	1.00	11.30	1.30	16.60	6.00
1986	3,483	237	5,216	3,720	5,453	8,936	15.40	0.20	10.00	0.80	16.00	9.00
1987	3,040	191	5,914	3,231	6,105	9,145	19.00	0.40	10.10	0.70	16.00	11.50
1988	2,272	191	6,712	2,463	6,903	9,175	20.90	0.80	9.80	1.00	15.25	12.80
1989	2,957	436	2,597	3,393	3,033	5,990	15.90	0.00	7.40	0.00	15.25	12.00
Average	3,114	481	4,828	3,595	5,309	8,423	17.31	0.75	8.91	1.30	15.74	10.41
	(49)	(366)	(4,223)	(415)	(4,589)	(4,638)						

Note: 1) Case A means irrigation supply by pump from the Lakhya River.

2) Case B means irrigation supply by pump from the drainage channel of country side.

3) Case C means pump drainage to the Lakhya River.

4) The figures in parenthesis show the value between June and October.

TABLE E.4.5(1) HYDRAULIC REQUIREMENTS OF PROPOSED PUMPING STATION : DND

. 1⁹

Proposed	Drainag	te Zone	Drainage Zone Discharge			Design W	Design Water Level (m, PWD)	(m, PWD)	Static Head	cad	
Pumping	No.	Area	Capacity		Outer		Inner	ler	(m)		Remarks
Station		(km^2)	(m ³ /s)	TMT TMH TMT TMH TMHH	H.W.L	L.W.L	H.W.L	L.W.L	Design	Max.	
P 10	NA-1	25.10	14.50	ł	5.75	3.00	1.80	1.00	4.75	:	Existing Pumping
P 11	NA-2	31.69	50.20	7.10	5.65	3.00	4.00	3.00	2.65	4.10	Station
Note: $1 ext{ II II N/I}$ and $ ext{ II N/I}$ of mitter decign water	puo I/XX E	n W T	היהה לפיוח		al means th	at of 100-w	Par and 2-W	ear frequen	lavel means that of 100-year and 2-year frequency flowd respectively.	ectively	

Note: 1. H.H.W.L. and H.W.L. of outer design water level means that of 100-year and z-year irequency mood respectively

TABLE E.4.5(2) HYDRAULIC REQUIREMENTS OF PROPOSED RETARDING POND : DND

	Remarks						
vel (m, PWD)	T.W.T	3.00	3.00	3.00	3.00	3.00	3.00
Design Water Lev	ТМН	4.00	4.00	4.00	4.00	4.00	4.00
Storage Capacity Design Water Level (m, PWD)	(x 10 ^{6m3})	1.96	0.45	0.60	06.0	2.25	0.66
Pond Area	(ha)	196	45	60	06	225	66
Drainage	Zone	NA-1	NA-1	NA-1	NA-2	NA-2	NA-2
Proposed	Retarding Pond	RP 10-1	RP 10-2	RP 10-3	RP 11-1	RP 11-2	RP 11-3

TABLE E.4.6 RUN-OFF COEFFICIENT : DND

DND Project Area (NA)

(NA-1)

		Run-oll				Land Use (%)				
Block	Area	Coefficient		Residential		Commercial	Industrial	Institution	Open Space	Waterbodies
No.			High Density	Midium Density	Low Density				/Agrecultural	
	<u>km2</u>	<u> </u>	0.50	0.50	0.30	0.65	0.55	0.30	0.20	1.00
NA-1-1	0.97	0.50	46.29	8.17	0.00	3,15	19.16	18,90	0.00	
NA-1-2	1.17	0.50			0.00		15.42	16.21	0.00	
NA-1-2	2,70	0.51	53.82		0.00		11.51	12.33		
NA-1-4	2,15	0.40			2.79	2.76	7.85	20.21	25,60	
NA-1-5	1.62	0.30			2.10	0.90	3,31	9.47	62.97	
NA-1-5	0.71	0.29			1.03	1.10	4.00	8.64		
NA-1-7	1.32	0.44			5.29		6.00	24.38		
NA-1-8	1.15	0.48			2.43		10.78	21.88		
NA-1-9	2.30	0,44			5.53	4.52	3.73	27,75	5.57	
NA-1-10	3.43	0.44			5.53		3.73	27.75	5.57	
NA-1-11	0.53	0,45			8.52		13.50	22.44	7.14	: 3.7
NA-1-12	1.62	0.44			5.53		3.73	27.75	5.57	3.1
	1.23	0.39			4,41		2.97	22.11	24.77	2.5
NA-1-13		0.39			7.01		11.41	17.30	27.01	2.6
NA-1-14	0.69	0.40			5.69		4,66	26.33	8.65	3.0
NA-1-15	1.59				9.50		16.32	21.21		3.7
NA-1-18	0.94	0.45			4.77		10.69	10.36		1.9
NA-1-17	0.98	0.34		1.92	4.77	2.00				

(NA-2)

		Run-off				Land Use (%)				
Block	Area	Coefficient		Residentia		Commercial	Industrial	Institution		Waterbodies
No.			High Density	Midium Dans	ity Low Density				/Agrecultural	
	km2	f	0.50	0.50	0.30	0,65	0.55	0.30	0.20	1.00
						4.99	9.05	32.64	0.00	1.5
NA-2-1	2.02	0.44	24.11	21.3			8,66			
NA-2-2	.1.67	0.45		23.3			14.57	25.60		
NA-2-3	1.09	0,46		19.						
NA-2-4	2.77	0.38		14.			10.54			
NA-2-5	2.43	0.41	15.87	22.			8.20			
NA-2-6	1.57	0.35	10.85	13.			6,16			
NA-2-7	1.29	0.26	4.47	4.	40 1.29	0.72	4.19			
NA-2-8	1.36	0.45	23.66	20.1	22 3.29	1.87	12.02			
NA-2-9	1.61	0.42		19.	39 3.35	1.72	10.03			
NA-2-10	1.19	0.43		25.	4.69	1.89	7.62			
NA-2-11	1.39	0.44		25.	2 4.89	2.23	10.28	31.40		
NA-2-12	0.71	0.43		25.		1.94	7.96	22.32		
NA-2-13	1.78	0.43		25.	-		7.60	20.70	18.18	5.3
	2.63	0.40		20.	A CONTRACTOR OF		7.89	19.24	27.07	4.7
NA-2-14		0.36		15.			6.22	13.32	:44.74	4.3
NA-2-15	0.67			15.			5.22		47.79	3.2
NA-2-16	1.25	0.34		6.			3.65			1.9
NA-2-17	1.56	0.28					13,96			
NA-2-18	0.90	0.41		14.			14.10			
NA-2-19	1.51	0.44		17.			21,63			
NA-2-20	1.76	0.41	16.59	9.				12.74		
NA-2-21	0.53	0,45	18.52	8.	42 6.74	4.48	31.11	12.74	13.10	2.0

TABLE E.4.7 DESIGN DISCHARGE : DND

Condition Block	s: Inlet Time; Drainage A		Run-aff	Coefficier	Len	ath		to emit	Rainfal	Areal	Design
No.	Individual Acc		Individual Acc		Individual Ad		Velocity	Concentration	Intensity	Reduction	Discharge
	a	A ·	(F	1	L,	V	t t	r	Factor	
1. 	(km2)	(km2)			(km)	(km)	(m/s)	(min)	(mm/hr)	(m3/s/km)	(m3/s)
						1.00	0.80	41	99.14	1.00	13.3
NA-1-1	0.97	0.97	0.50	0.50	1.00	2.20	0.80		77.74	0.99	23.0
NA-1-2	1.17	2.14	0.50	0.50	1.20		0.80	78	70.17	0.98	26.4
VA-1-3	2.70	2.70	0.51	0.51	2.80	2.80		108	57.17	0.96	50.5
VA-1-4	2.15	6,99	0.40	0.47	1.40	4.20	0.00	72	73.76	0.99	9.9
A-1-5	1.62	1,62	0.30	0.30	2.50	2.50	0.80	108	57.17	0.99	10.9
VA-1-8	0.71	2.33	0.29	0.30	1.70	4.20	0.80		47.83	0.94	57.3 (30
NA-1-7	1.32	10.64	0.44	0.43	1.80	6.00	0.80	145		0.99	12.7
8-1-8	1.15	1.15	0.48	0.48	1.80	1.80	0.80		83.77		27.3
NA-1-9	2.30	3.45	0.44	0.45	1.50	3.30	0.80		\$4.90	0.97	
A-1-10	3.43	5,88	0.44	0.45	1.50	. 4.80	0.80		52.97	0.96	43.5
A-1-11	0.53	18.05	0.45	0.44	0.60	6.60	0.80		45.64	0.92	92.2 (50
IA-1-12	1.62	1.62	0.44	0.44	1.60	1.60	0.80	53	87.15	0.99	17.1
A-1-13	1.23	2.85	0.39	0.42	1.60	3.20	0.80	87	65.89	0.98	21.4
łA-1-14	0.69	21.59	0,40	0.43	1.00	7.60	0.80	178	42.40	0.91	100.4 (60
A-1-15	1,59	1.59	0.43	0.43	1.80	1.80	0.80	58	83.77	0.99	15.7
A-1-16	0.94	24.12	0.45	0.43	0.70	8.30	0.80		40.39	0.90	105.8 (65
A-1-10	0.98	2-4.12	0.34	0.40	1.10	-	0.80		-	•	33.8

(NA-2)

Condition Block	Drainage /	20min Area	Run-off	Coefficier	Leng			time of	Rainfal	Areal	Design
No.	Individual Ac		Individual Ac		Individual Ac	cumlated	Velocity	Concentration	Intensity	Reduction	Discharge
	a	A	4.1	, F :	1	L	. V	t	r	Factor	(-0/-)
	(km2)	(km2)			<u>(km)</u>	<u>(km)</u>	(m/s)	(min)	(mm/hr)	(m3/s/km)	(m3/s)
NA-2-1	2.02	2.02	0.44	0.44	2.20	2.20	0.80	66	77.74	0.99	19.1
NA-2-2	1,67	3.69	0.45	0.44	1.80	4.00	0.80	103	58.73	0.97	25.9
NA-2-2	1.09	1.09	0.46	0.46	1.60	1.60	0.80	53	87.15	0.99	12.1
NA-2-4	2.77	7.55	0.38	0.42	1.50	5.50	0.89	135	49.83	0.95	42.1
NA-2-5	2.43	2.43	0.41	0.41	2.30	2.30	0.80	68	76.37	0.98	20.7
NA-2-5	1.57	11.55	0,35	0.41	1.20	6.70	0.80	160	45.29	0.94	56.1
NA-2-7	1.29	12.84	0.26	0.40		8.00	0.80	187	41.23	0.93	54.1
NA-2-8	1.36	1.36	0.45	0.45	2.20	2.20	0.80	66	77.74	0.99	13.1
NA-2-0	1.61	1.61	0.42	0.42	1.80	1.80	0.80	1	83.77	0.99	15.6
NA-2-9	1.19	4,16	0.43	0.43	1.40	3,60	0.80		52.10	0.97	30.1
A-2-11	1.39	1.39	0.44	0.44	1.80	1.80	0.80	58	83.77	0,99	14.1
A-2-12	0.71	2.10	0.43	0.44	1.50	3.30	0.80	89	64.90	0.99	16.4
	1.78	8.04	0.43	0.43	1.20	4.80	0.80	120	52.97	0.95	48.7
IA-2-13	2.63	2.63	0.40	0.40	2.60	2.60			72.52	0.98	20.8
IA-2-14		11.34	0.36	0.42	0.80	5.60			49,42	0.94	61.6
A-2-15	0.67	1.25	0.34	0.34	1.60	1.60			87.15	0.99	10.2
IA-2-16		14,15	0.28	0.40	2.40	8.00			41.23	0.93	60.0
A-2-17	1.56	25.02	0.41	0.43	1.00	9.30			37.83	0.89	71.5 (40
A-2-18	0.90			0.43	2.20	2.20	+		77.74	0.99	14.2
IA-2-19	1.51	1.51	0.44	0.44	1.80	11.10			33.96	0.88	71.6
A-2-20	1.76	28.29	0.41	0.43	0.60	11.70			32.84	0.82	143.5

Note: The figures in parenthesis indicate the design discharge estimated in hydraulic simulation by Mike 11 model, considering the storage effect of the retarding area.

TABLE E.4.8 HYDRAULIC DESIGN OF KHAL IMPROVEMENT : DND

DND Project Area (NA)

Khal	Design		Section		Roughness	Slope	Velosity	Discharge
	Discharge	Bottom Wid.	Upper Wid.	Height	Coefficient	1 (%)		Capacity
No.	(m3/s)	(m)	(m)	<u>(m)</u>			(m/s)	(m3/s)
(NA-1)								
KN-1-1	33.8	5,50	13.50	4.00	0.025	0.017	0.89	33.8
KN-1-2	65.0	14.50	30.50	4.00	0.035	0.017	0.73	65.6
KN-1-3	60.0	13.00	29.00	4.00	0.035	0.017	0.72	60.4
KN-1-4	50.0		26.00	4.00	0.035	0.017	0.69	50.0
KN-1-5	30.0	10.00	26.00	4.00	0.035	0.017	0.69	50.0
KN-1-6	50.6	8.50	24.02	3.88	0.035	0.025	0.82	51.7
KN-1-7	23.0	3.50	17.02	3.38	0.035	0.025	0.68	23.7
KN-1-8	13.3	2.50	8.78	3.14	0.025	0.025	0.85	15.0
KN-2-1	21.4	2.00	17.48	3.87	0.035	0.025	0.71	26.6
KN-2-2	17.1	2.50	9.40	3.45	0.025	0.025	0.89	18.3
KN-3	10.9	2.00	17.44	3.86	0.035	0.025	0.70	26.4
KN-13	15.8	2,00	17.60	3,90	0.035	0.025	0.71	27.1
KN-14-1	43.5	6.50	22.02	3.88	0.035	0.025	0.79	43.8
KN-14-2	27.4	3.00	18.00	3.75	0.035	0.025	0.72	28.2
KN-14-3	12.7	2.00	8.16	3.08	0.025	0.025	0.81	12.7
KN-15	9.9	2.00	14.76	3.19	0.035	0.025	0.63	16.8
KN-16	26.4	4.00	11.40	3.70	0.025	0.025	0.99	28.3
NA-2)							· · · · ·	1. 1. A. A.
KN-4-1	143.5	33.50	41.84	4.17	0.025	0.010	0,92	144.0
KN-4-2	56.1	10.50	27.18	4.17	0.035	0.017	0.72	56.7
KN-4-3	56.1	10.50	27.18	4.17	0.035	0.017	0.72	56.7
KN-4-4	42.1	5.50	21.66	4.04	0.035	0.025	0.79	43.5
KN-4-5	26.0	4.00	11.18	3,59	0.025	0.025	0.98	-26.7
KN-4-6	19.1	3.50	10.04	3.27	0.025	0.025	0.91	20.2
KN-5-1	71.6	22.50	38.70	4.05	0.035	0.010	0.60	74.5
KN-5-2	40.0	9.50	17.50	4.00	0.025	0.010	0.76	40.8
KN-6	14.2	2.00	17.92	3.98	0.035	0.025	0.72	28.4
KN-7-1	61.6	12.00	28.68	4.17	0.035	0.017	0.73	62.3
KN-7-2	61.6	12.00	28.68	4.17	0.035	0.017	0.73	62.3
KN-7-3	48.7		15.14	4.07	0.025	0.025	1 14	51.6
KN-7-4	30.1	4.50	11.94	3.72	0.025	0.025	1.02	31.1
KN-7-5	13.1	2.00	8.82	3.41	0.025	0.025	0.86	15.9
KN-8	10.2	2.00	18,36	4.09	0.035	0.025	0.73	30.4
KN-9	20.8	2.00	18.28	4.07	0.035	0.025	0.73	30.0
KN-10	15.6	2.00	16.28	3.57	0.035	0.025	0.67	21.9
KN-11	20.7	2.00	18.20	4.05	0.035	0.025	0.73	29.7
KN-12	12.1	2.00	9.28	3.64	0.025	0.025	0.89	18.3
KN-17	16.4	2.00	15.80	3.45	0.035	0.025	0.66	20.2

TABLE E.9.9 PROPOSED KHAL IMPROVEMENT WORKS : DND

DND Project Area (NA)

-	14-1	Mhail	Required		pen		ered Innel	Bridge	Aqueduct	Dredging	Maintenance	Land
Zone	Khal	Khal	Hydraulic :		nannel	· · · · · · · · · · · · · · · · · · ·	Brick	Diluge	Aquoduct	Dioghuð	Road	Acquisition
		Length	Section	Brick	Sodding	Box	Pipe	(Placet)	(Places)		1 Koud	And a lotter
	· . · ·	1	Wb x Wu x H	Protection	. (1	Culvert (km)	(km)	(Flaces)	(Fiaces)	(1000m3)	(km)	(ha)
	<u>No.</u>	<u>(km)</u>	(mxmxm)	(km)	(km)	<u>(Kin)</u>	<u>(Kui)</u>			(1000110)		1104/
	KN-1-1	2.10	5.5 13.5 4.0	2.10		_	_	-	-	22,27	2.10	0.61
	KN-1-2	0.70	14.5 30.5 4.0		0,70	-		1	-	34.44	0.70	1.72
	KN-1-3	1.00	13.0 29.0 4.0		1.00	-	_	1		43.02	1.00	2.54
	KN-1-4	0.60	10.0 26.0 4.0		0.60	.		-		11.90	0.60	0.82
.]	KN-1-5	1.80	10.0 26.0 4.0		1.80	-	-	1	- 1	76.88	1.80	4.83
· ·]	KN-1-6	1.40	8.5 24.0 3.9		1.40		-	1		49.86	1.40	3.98
	KN-1-7	1.20	3.5 17.0 3.4		1.20	-		-	i -	29.51	1.20	2.67
NA-1	KN-1-8	0.60	2.5 8.8 3.1	0.60	-			-	-	5.91	0.60	0.22
100-1	KN-2-1	1.60	2.0 17.5 3.9	0.00	1,60	_	-	3	1	34.36	1.60	3.70
	KN-2-2	1.60	2.5 9.4 3.5	1.60	-	-	-	2	-	21.32	1.60	2.37
	KN-2-2	1.70	2.0 17.4 3.9	-	1.70	-	-	1	- 1	16.78	1.70	2.92
	KN-3 KN-13	1.20	2.0 17.6 3.9	· ·	1.20		-	1	<u> </u>	39.35	1.20	3.49
	KN-14-1	1.50	6.5 22.0 3.9	-	1.50		_ .	•	- 1	48.18	1.50	4.92
	KN-14-2	1.50	3.0 18.0 3.8	1 L	1.50	-	-	-		36.30	1.50	4.45
	KN-14-3	1.60	2.0 8.2 3.1	1.60	-	-	-	1	· ·	27.68	1.60	3.18
	KN 15	1.60	2.0 14.8 3.2	_	1,60			1	. I	11.69	1.60	4.32
	KN-16	2.20	4.0 11.4 3.7	2.20	-		·		-	43.04	2.20	4.68
	Sub-Total	23.90		8.10	15.80	0.00	0.00	13	1	552.49	23.90	51.42
		20.00	· ·						[:		
			· · · ·						1	1	1	
	KN-4-1	1.80	33.5 41.8 4.2	1.80	-	-	-	3	-	229.56	1.80	4.37
	KN-4-2	1.30	10.5 27.2 4.2	•	1.30			-		118.68	1.30	4.93
	KN-4-3	1.20	10.5 27.2 4.2	-	1.20	- 1	- ·	2	-	61.65	1.20	2.61
	KN-4-4	1.50	5.5 21.7 4.0	- 1	1.50	-	-	2		73.93	1.50	4.16
	KN-4-5	1.80	4.0 11.2 3.6	1.80		-		4		24.91	1.80	1.27
	KN-4-6	0.80	3.5 10.0 3.3	0.80	-	•	-	1	- I	20.80	0.80	0.67
	KN-5-1	1.80	22.5 38.7 4.1	-	1.80	-	-	-	-	256.96	1.80	6.03
NA-2	KN-5-2	1.00	9.5 17.5 4.0	1.00	18.2		-		- 1	67.23	1.00	2.40
	KN-6	0.90	2.0 17.9 4.0	-	0.90	-	-	-	-	18.44	0.90	1.36
	KN-7-1	2.40	12.0 28.7 4.2	•	2.40	-	-	1	1	196.23	2.40	7.92
	KN-7-2	0.80	12.0 28.7 4.2	-	0.80	1.11	· ·		-	42.41	0.80	2.52
	KN-7-3	1.20	7.0 15.1 4.1	1.20	-	-	- 1	-	-	24.82	1.20	1.36
	KN-7-4	1.40	4.5 11.9 3.7	1.40	.		-	6	-	34.30	1.40	1.18
	KN-7-5	0.80	2.0 8.8 3.4	0.80	- 1		l -	1	-	8.70	0.80	0.58
	KN-8	1.00	2.0 18.4 4.1	-	1.00	-	-	-	-	20.54	1.00	1.65
	KN-9	1.30	2.0 18.3 4.1	-	1.30		-	1	-	31.32	1.30	2.20
	KN-10	1.80	2.0 16.3 3.6		1.80	-	-	2		20,00	1.80	2.43
	KN-11	1.40	2.0 18.2 4.1		1.40	-	-	-	-	30.19	1.40	3.14
	KN-12	1.60	2.0 9.3 3.6	1.60		-	-	1	1 · •	8.96	1.60	0.76
	KN-17	1.50	2.0 15.8 3.5		1.50	-	-	· 1 ·	-	47.32	1.50	4.18
÷.	Sub-Total	27.30		10.40	16.90	0.00	0.00	25	1	1336.95	27.30	55.72
					ļ		ļ	ļ	ļ	ļ		
	Total	51.00		18.50	32.70	0.00	0.00	38	2	1889.44	51.20	107.14
	Total	. 51.20		10.00	32.10	0.00	0.00		1	1003.44		1

TABLE E.5.1 PROPOSED FLOOD MITIGATION FACILITY : NARAYANGANJ WEST

Route (Total Length)		Facility			Main Features
1. Narayanganj to					
Panchabati (NW)	1)	Road-Cum-Embankment	:	4.10 km	(NW.8+100~ NW.29)
(L = 5.64 km)	2)	Embankment	:	1.54 km	(NW.0~NW.8+100)
	3)	Sluice Gate	:	2 places	
2. Narayanganj to					
Demra (NE) (L = 21.83 km)	1)	Flood Wall	:	11.48 km	(NE.0~NE.48, NE.55~NE.62, NE.87~NE.88)
	2)	Embankment	:	10.35 km	(NE.48~NE.55, NE.62~NE.87)
	3)	Sluice Gate	:	12 places	
	4)	Stop Log Structure	:	17 places	· · · · · · · · · · · · · · · · · · ·
Total	1)	Road-Cum-Embankment	:	4.10 km	
	2)	Embankment	:	11.89 km	
	3)	Flood Wall	:	11.48 km	
	3)	Shuice Gate	:	14 places	
	4)	Stop Log Structure	:	17 places	:

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TABLE E.5.2 LAND USE AND POPULATION OF NARAYANGANJ WEST

Drainage		1990		2000	and a state of the	2100
Area (km ²)	Buil-up Area (km ²)	Population (million people)	Buil-up Area (km ²)	Population (million people)	Buil-up Area (km ²)	Population (million people)
		nganalas de tato mante de la propositiva de la construcción de la mange en que se a que				
18.36	13.12	0.470	17.20	0.696	17.20	0.927

TABLE E.5.3

MAIN HYDRAULIC FEATURES OF EXISTING MAJOR KHAL: NARAYANGANJ WEST

Narayanganj West Zone (NB)

				Max. S	ection					Min S	Section		
Khal	Length	Catchment	Width	Depth	Slope	Discharge	Specific	Catchment	Width	Depth	Slope	Discharge	Specific
		Area				Capacity	Discharge	Area				Capacity	Discharge
No.		an Ar Ataria an Ara					Capacity						Capacity
	(m)	(km2)	(m)	(m)	(%)	(m3/s)	(m3/s/km2)	(km2)	(m)	(m)	(%)	(m3/s)	(m3/s/km2
K 1	420	0.50	11.20	1.70	0.050	5.3	10.6	1.70	5.00	0.90	0.050	0.8	0.
K 2		1.20	3.00	3.50	0.050	2.6	2.2	1.20	3.00	3.50	0.050	2.6	.2.
К 3	1,220	55.80	44.00	6.60	0,050	199.9	3.6	55.80	13.20	0.40	0.050	0.6	0.
K 4	520	0.30	11.20	5.60	0.050	31.6	105.3	0.70	5.20	3.60	0.050	6.2	8
K 5	600	0.70	25.60	5.20	0.050	76.4	109.2	0.20	20.40	4.50	0.050	47.5	237.
K 6	360	0.50	15.40	3.10	0.050	19.4	38.9	0.50	15.40	3.10	0.050	19.4	38.
K 7	1,240	2.40	14.40	3.00	0.017	10.0	4.2	2.00	7.60	0.90	0.017	0.7	0
K 8	1,400	4.60	16.40	5.20	0.025	32.6	7.1	1.30	12.20	2.40	0.025	7.1	5.

Note: Roughness Coefficient (n) = 0.035

	Remarks							Remarks						
ad		Max.	4.35	3.75	4.10	3.47								
Static Head	(n) (n)	Design	2.80	2.20	2.50	1.95	(m, PWD)	L.W.L	3.00	3.50	3.00	3.50	3.50	
(האיז, ש	er	T.W.L	3.00	3.50	3.00	3.50	Design Water Level (m, PWD)	Ţ.				<u> </u>	0	
Lesign water Level (III, FWD)	Inner	H.W.L	4.20	4.60	4.50	4.60	Design W	H.W.L	4.20	4.60	4.50	4.60	4.60	
LOOLEN W		L.W.L	3.00	3.50	3.00	3.50	Storage Capacity	(x 10 ⁶ m ³)	0.21	23	0.28	26	0.30	
	Outer	H.W.L	5.80	5.70	5.50	5.45	Storage	(x1	Ó	Ö	, O			:
		H.H.W.L	7.35	7.25	7.10	6.97	Pond Area	(ha)	21	23	28	26	30	
Discharge	Capacity	(m ³ /s)	2.00	2.20	2.70	5.30				- - -				
	Area	(km ²)	1.73	1.92	2.36	4.65	Drainage	Zone	NB-1	NB-2	NB-4	NB-5	NB-5	
Drainage Zone	No.		NB-1	NB-2	NB-4	NB-5		nd		· · ·		•		
Proposed	Pumping	Station	P 12	P 13	P 14-A	P 14-B	Proposed	Retarding Pond	RP 12	RP 13	RP 14-1	RP 14-2	RP 14-3	

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TABLE E.5.4(1) HYDRAULIC REQUIREMENTS OF PROPOSED PUMPING STATION : NARAYANGANJ WEST

TABLE E.5.5 RUN-OFF COEFFICIENT : NARAYANGANJ WEST

Narayanganj West Zone (NB)

		Run-off				Land Use (%)				
Block	Area	Coefficient	Illah Darahu	Residential Midium Density	Low Donaity	Commercial	Industrial	Institution	Open Space /Agrecultural	Waterbodie
No.	km2	- f	High Densky 0.50	0,50	0.30	0.65	0.55	0.30	0.20	1.00
NB-1-1	0.57	0.45	23.44	19.60	5,95	5.21	7.83			4.(
NB-1-2	1.73	0.42			5,23	4.58	6.88	27.49		3.5
NB-2-1	1.19	0.43			3.29	4.65	25,76	17.61	19.37	2.0
NB-2-2	0.73	0.49			4.08	6.02	31.93	21.83	0.05	2.5
NB-2-3	2.07	0.49			4.05	.6.02	31.93	21.83	0,05	2.5
NB-3-1	0.95	0.45			6.30	3.61	19.18	30,37	0.18	1.9
NB-3-2	0.91	0.45			6.30	3.61	19,18	30.37	0.18	1.5
NB-3-3	0.73	0,45			6.30	3.61	19.18	30.37	0.18	1.
N8-3-4	0.70	0.48	39,92		1.18	7.44	19.65	25.46	0.00	1.1
N8-3-5	0.89	0.48	39.92		1.18	7.44	19.65	25.46	0.00	1.
NB-3-6	0.40	0.53	35.09		0.00	33.90	6.04	17.02	0.00	2,1
NB-3-7	0.51	0.53			0.00	33.90	6.04	17.02	0.00	2.
NB-3-8	0.24	0.53	35.09		0.00		6.04	17.02	0.00	2.
NB-4-1	1.07	0.49			2.48	10.02	12.28	22.71	0.41	4.9
NB-4-2	0.54	0.45	29.25		2.11	8.54	10.46	19.34	15.16	3.1
NB-4-3	0.75	0.41	25.18		1.82	7.35	9.01	16.65	26,96	
NB-5-1		0.50	42.91			11.96	3.38	15,13	0.41	4.
N8-5-2	1.01	0.40			2,43	7.82	2.21	9.89		2.
		0.46	37.43		3,25	10.44	2.95	13.20		
N9-5-3	1.33		38.90		3.38	10.85	3.07	13.72		4.
NB-5-4	1.39	0.48			3.72		3.38	15.13		4.
NB-5-5	0.17	0.50	42.91	17,94	3.72	11.00	0.00	10.10		

TABLE E.5.6 DESIGN DISCHARGE : NARAYANGANJ WEST

Narayanganj West Zone (NB)

Block	Drainage A	lrea		Coefficier	Leng			Time of	Raintal	Areal	Design
No.	Individual Acc	umlated	Individual Ac		Individual Acc	umlated	Velocity	Concentration	Intensity	Reduction	Discharge
	a	A		F.	1	Ļ	V	t	r.	Factor	(_0,)
	(km2)	(km2)			(km)	(km)	(m/s)	(min)	(mm/hr)	(m3/s/km)	(m3/s)
NB-1-1	0.57	0.57	0.45	0.45	0.90	0.90	0.80	39	101.46	1.00	7.
NB-1-2	1.73	1.73	0.42	0.42	1.80	1.80	0.80	- 58	83.77	0.99	16.
N8-2-1	1.19	1.19	0.43	0.43	1.50	1.50	0.80	51	88.94	0.99	12.
NB-2-2	0.73	0.73	0.49	0.49	1.80	1.80	0.80	58	83.77	- 1.00	8.
NB-2-3	2.07	2.07	0.49	0.49	2.10	2.10	0.80	64	79.16	0,99	21.
NB-3-1	0.95	0.95	0.45	0.45	1.50	1.50	0.80	51	88.94	1.00	10.
NB-3-2	0.91	0.91	0.45	0.45	1.40	1.40	0.80	49	90.81	1.00	10.
NB-3-3	0.73	0.73	0.45	0.45	1.10	1.10	0.80	43	96.91	1.00	8.
N8-3-4	0.70	0.70	0.48	0.48	1.00	1,00	0.80	41	99.14	1.00	9.
NB-3-5	0.89	0,89	0.48	0.48	1.50	1.50	0.80	51	88.94	1.00	10.
NB-3-6	0.40	0.40	0.53	0.53	0.80	0.80	0.80	37	103.90	1.00	6.
NB-3-7	0.51	0.51	0.53	0.53	1.20	1.20	0.80	45	94.79	1.00	7.
NB-3-8	0.24	0.24	0.53	0.53	0.60	0.60	0.80	33	109.15	1.00	э.
NB-4-1	1.07	1.07	0.49	0.49	1.70	1.70	0.80	55	85.42	0.99	12.
NB-4-2	0.54	0.54	0.45	0.45	0.50	0.50	0.80	30	111.98	1.00	7.
NB-4-3	0.75	2.36	0.41	0.46	0.90	1.40	0.80	49	90.81	0.99	27.
NB-5-1	1.01	1.01	0.50	0.50	1.40	1.40	0.80	49	90.81	0.99	12.
NB-5-2	0.75	1.76	0.40	0.46	0.80	2,20	0.80	66	77.74	1.00	17.
NB-5-3	1.33	1.33	0.46	0,46	1.50	1.50	0.80	51	88.94	0.99	15.
NB-5-4	1.39	1.39	0.48	0.48	2.00	2.00	0.80	62	80.64	0.99	14.
NB-5-5	0.17	4.65	0.60	0.47	0.30	2.50	0.80	72	73.76	0.97	43.

TABLE E.5.7 HYDRAULIC DESIGN OF KHAL AND TRUNK DRAIN IMPROVEMENT : NARAYANGANJ WEST

Narayanganj West Zone (NB)

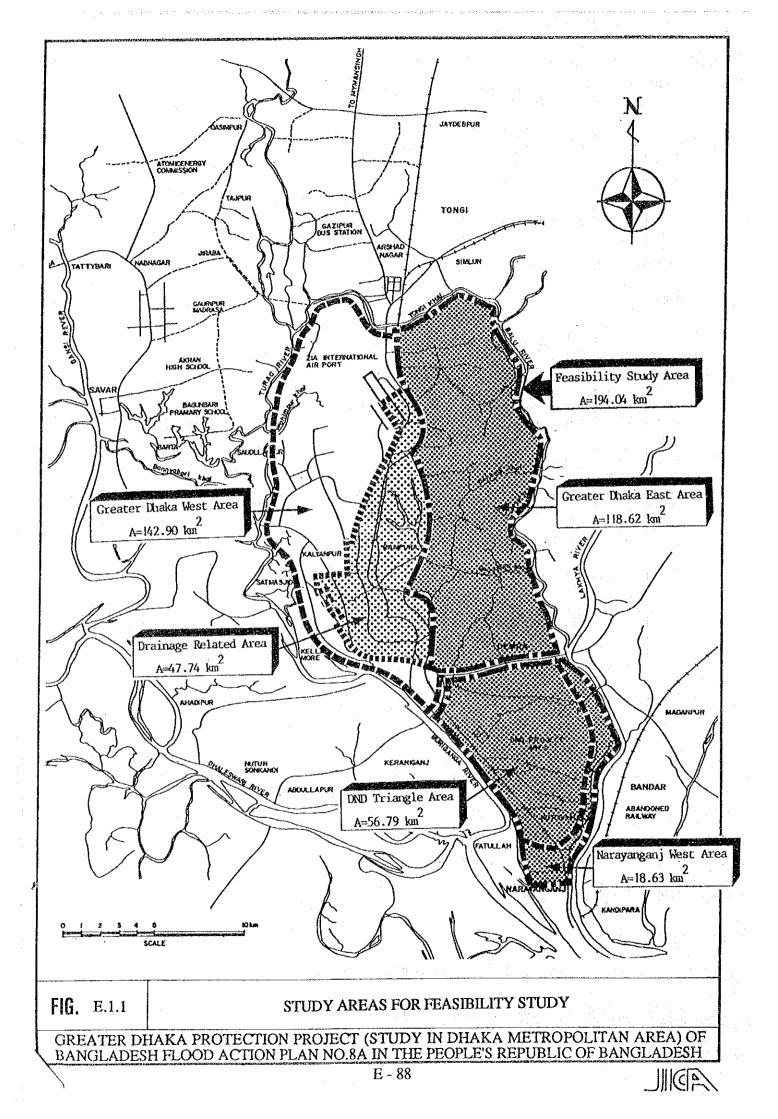
Khal	Design		Section		Roughness	Slope	Velosity	Discharge
	Discharge	Bottom Wid.	Upper Wid.	Height	Coefficient	i (%)		Capacity
No.	(m3/s)	<u>(m)</u>	(m)	(m)		anda ahkifaan waxaa ahkifaa da'a	(m/s)	(m3/s)
(NB-1)								· · · · ·
KN-18	7.2	2.00	7.00	2,50	0.025	0.033	0.84	9.5
KN-19	16.7	2.00	14.00	3.00	0.035	0.033	0.70	16.8
(NB-2)	10.1	2.00	14.00	0.00				
KN-20	12.5	3.00	13.00	2,50	0.035	0.033	0.66	13.1
KN-21	8.3		7.00	2.50	0.025	0.033	0.84	9.5
KN-22	21.9	4.50	10.50	3.00	0.025	0.033	1.05	23.7
(NB-3)								
KN-23	10.5	2.50	7.50	2.50	0.025	0.033	0.87	10.9
KN-24	10.3		7.50	2.50	0.025	0.033	0.87	10.9
KN-25	8.8	2.00	7.00	2.50	0.025	0.033	0.84	9.5
KN-26	9.2	2.00	7.00	2.50	0.025	0.033	0.84	9.5
KN-27	7.2	2.00	7.00	2.50	0.025	0.033	0.84	9.5
S-1	10.5	3.00	· •.	3.00	0.015	0.033	1.22	11.0
S-2	6.2	-	2.50	-	0.015	0.067	1.26	6.2
S-3	3.9	-	2.20	-	0.015	0.067	1.16	4.4
(NB-4)					· .			
KN-28-1	27.0	6.00	18.00	3.00	0.035	0.033	0.79	28.3
KN-28-2	7.5	2.00	13.32	2.83	0.035	0.033	0.68	14.7
KN-29	12.4	2.00	8.00	3.00	0.025	0.033	0.93	13.9
NB-5)								:
KN-30-1	43.2	9.50	15.50	3.00	0.025	0.033	1.19	44.7
KN-30-2	15.1	2.50	8.50	3.00	0.025	0.033	0.96	15.8
KN-31-1	.17.4	2.50	14.50	3.00	0.035	0.033	0.71	18.2
KN-31-2	12.7	2.00	8.00	3.00	0.025	0.033	0.93	13.9
KN-32	14.6	2.50	8.50	3.00	0.025	0.033	0.96	- 15.8

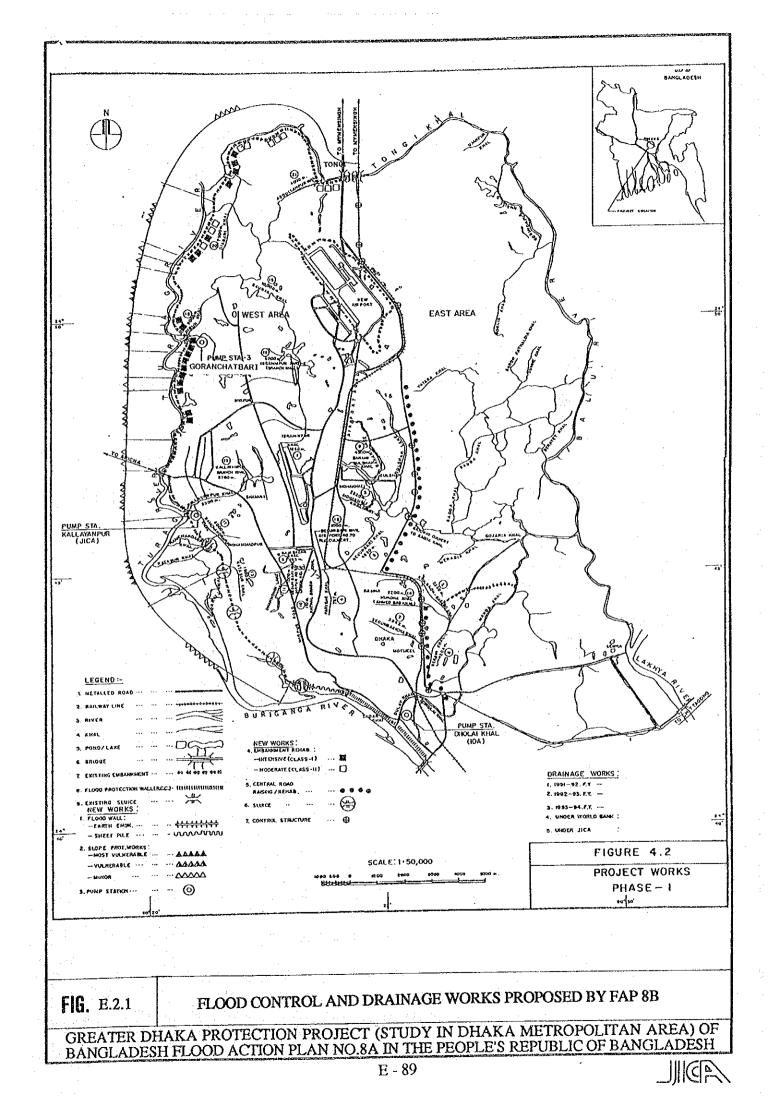
TABLE E.**5,8**

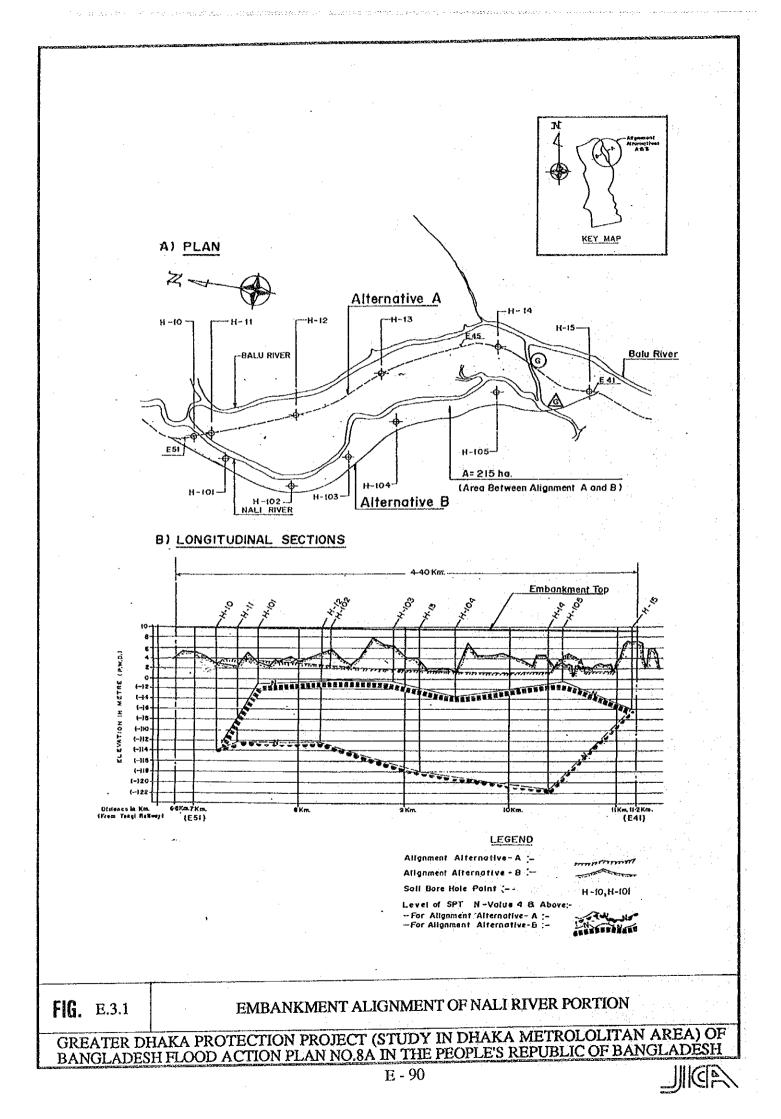
PROPOSED KHAL AND TRUNK DRAIN IMPROVEMENT WORKS : NARAYANGANJ WEST

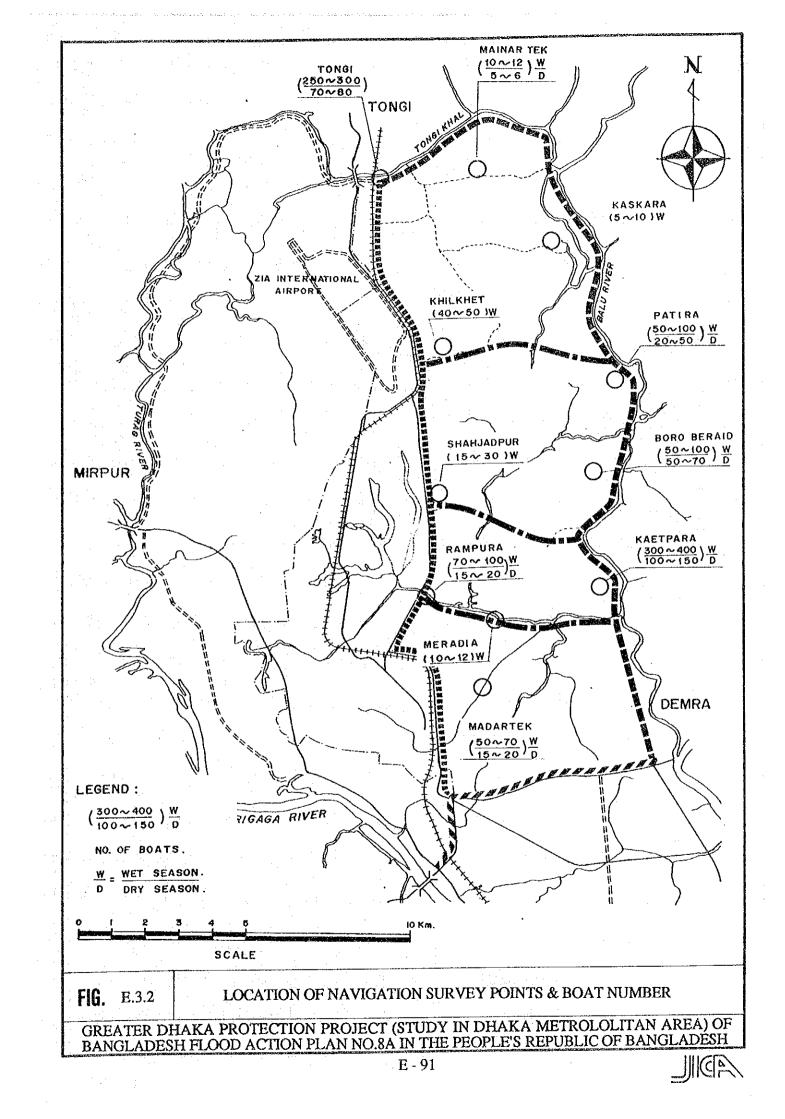
Narayanganj West Zone (NB)

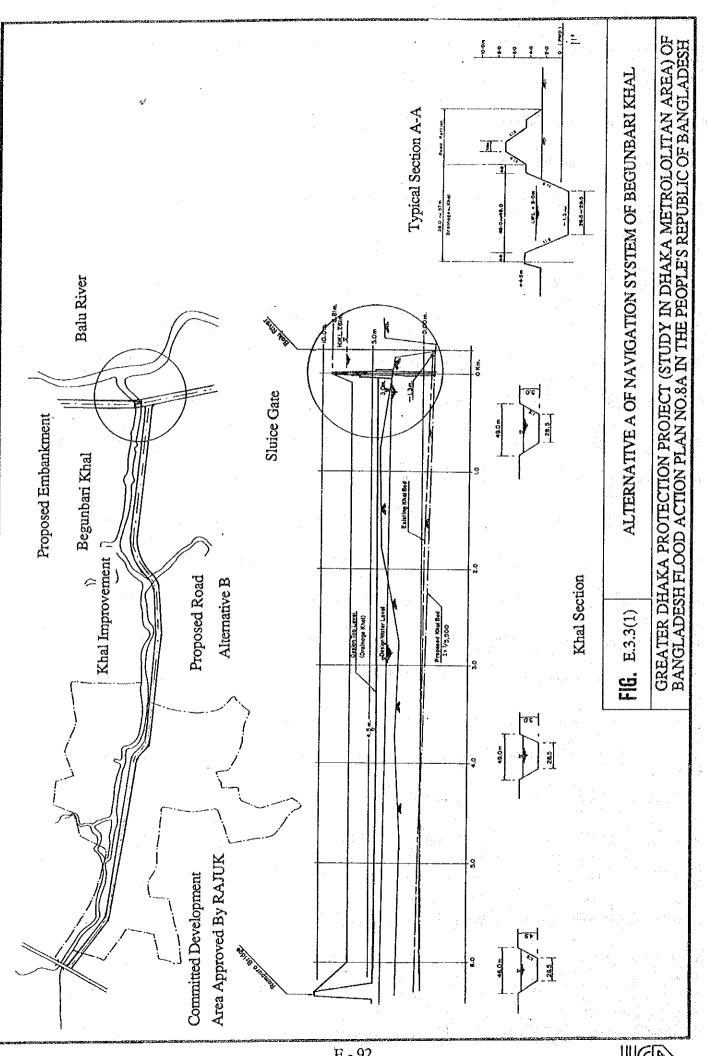
, i			Required	1	Opan		ered			.		· •
Zone	Khal	Khal	Hydraulic		hannel		nnel	Bridge	Aqueduc	Dredging	Maintenance	ł
		Length	Section	Brick	Sodding	Box	Brick				Road	Acquisitio
	tsus and		Wb x Wu x H	Protection		Culvert	Pipe	(Places)	(Places)			
	No.	<u>(km)</u>	(m x m x m)	(km)	(km)	(km)	(km)			(1000m3)	(km)	(ha)
1. 												
	KN-18	0.40	2.0 7.0 2.5	0.40	-		-	•	-	5.52	0.40	0.65
NB-1	KN-19	1.20	2.0 14.0 3.0	-	1.20		-	1	-	29.85	1.20	2.43
	Sub-Total	1:60		0.40	1.20	0.00	0.00	1	0	35.37	1.60	3.08
		· · · · · · · · · · · · · · · · · · ·										
	KN-20	0.90	3.0 13.0 2.5	-	0.90	•	-	-	•	8.46	0.90	0.90
NB-2	KN-21	1.40	2.0 7.0 2.5	1.40	-	· -	•	1		26.90	1.40	2.50
	KN-22	0.80	4.5 10.5 3.0	0.80	-	- 1	-	2	-	9.84	0.80	1.60
	Sub-Total	3.10		2.20	0.90	0.00	0.00	3	0	45.20	3.10	5.00
								L		i	L	↓
	KN-23	0.60	2.5 7.5 2.5	0.60	. •	-	1.1	1		7.80	0.60	1.02
	KN-24	0.70	2.5 7.5 2.5	0.70	•	-	+	1	-	11.20	0.70	1.20
	KN-25	0.40	2.0 7.0 2.5	0.40	· -	-	-	-	•	0.00	0,40	0.37
NB-3	KN-26	0.60	2.0 7.0 2.5	0.60		-	·	-	-	0.00	0.60	0.12
	KN-27	0.30	2.0 7.0 2.5	0.30	-	-	-		-	1.80	0.30	0.31
	S-1	0.90	3.0 3.0	1	•	0.90	-	1	-	21.69	0.90	1,50
	S-2	0.30	2.5			· -	0.30	-		5.76	0.30	0.46
	S-3	0.20	2.2			-	0.20	-	1 1	2.44	0.20	0.29
	Sub-Total	4.00		2.60	0.00	0.90	0.50	2	0	50.69	4.00	5.27
											1	
	KN-28-1	0.90	6.0 18.0 3.0	1 1 1	0.90		- '			10.25	0.90	1.65
NB-4	KN-28-2	0.50	2.0 13.3 2.8		0.50	-	-	-		4.33	0.50	1.04
	KN-29	1.40	2.0 8.0 3.0	1.40	. :	- '	-	2	· •	35.40	1.40	2.70
	Sub-Total	2.80		1.40	1.40	0.00	0.00	2	0	49.98	2.80	5.39
				1 :								
				· .		1		1	· ·	~	1	
	KN-30-1	0.30	9.5 15.5 3.0	0.30			-	1	· •	13.52	0.30	0.41
	KN-30-2	1.50	2.5 8.5 3.0	1.50			- 1	- 1	-	13.67	1.50	1.26
NB-5	KN-31-1	0.80	2.5 14.5 3.0		0.80	· _	-	1	-	22.80	0.80	1:53
	KN-31-2	1.30	2.0 8.0 3.0	1,30	-	1	-	2	-	33.30	1.30	3.01
	KN-32	1.80	2.5 8.5 3.0	1.80	. [']	1 _ ·		2	- L	37.60	1,80	3.45
	Sub-Total	5.70		4.90	0.80	0.00	0.00	6	. 0	120.89	5.70	9.66
	Total	17.20		11.50	4.30	0.90	0.50	14	0	302.13	17.20	28.40





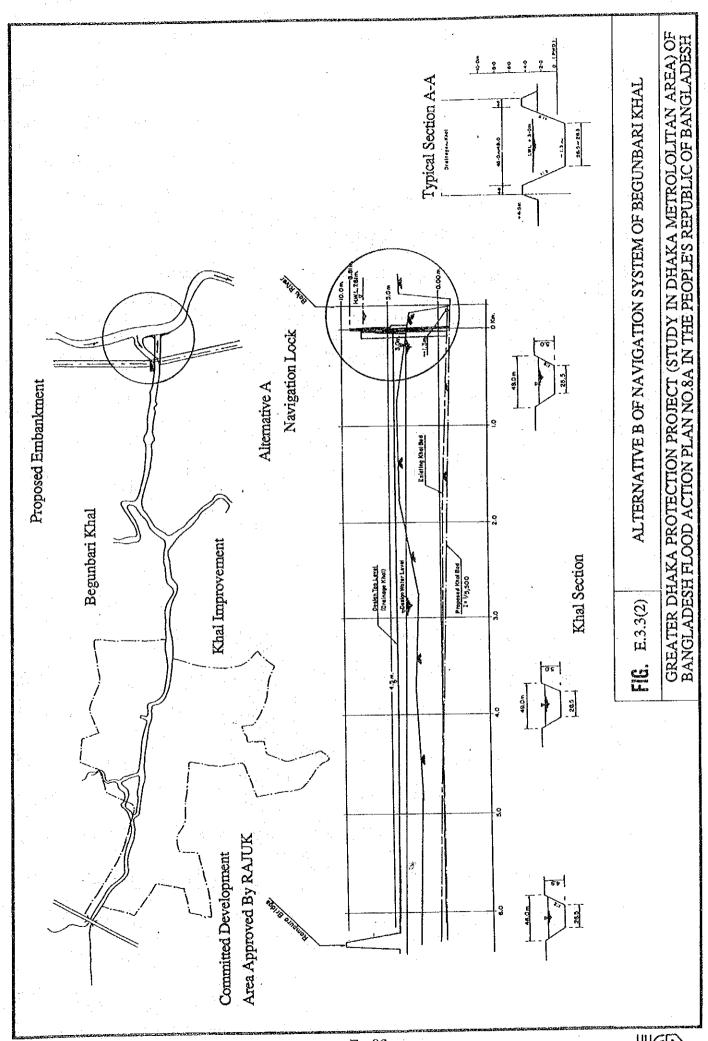




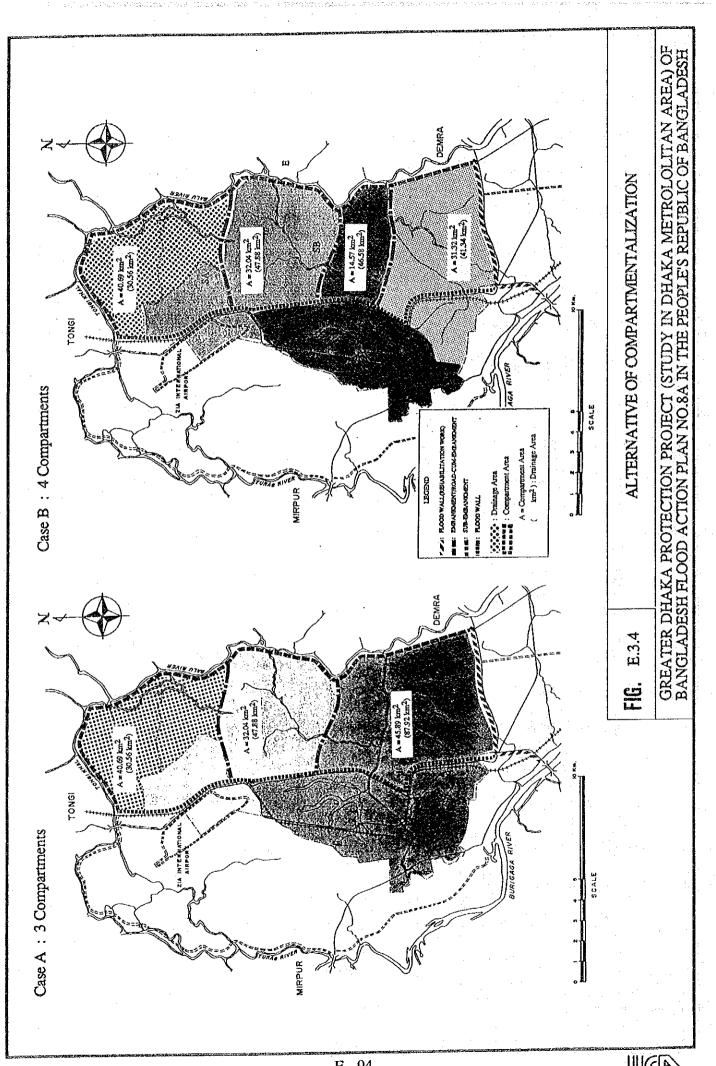


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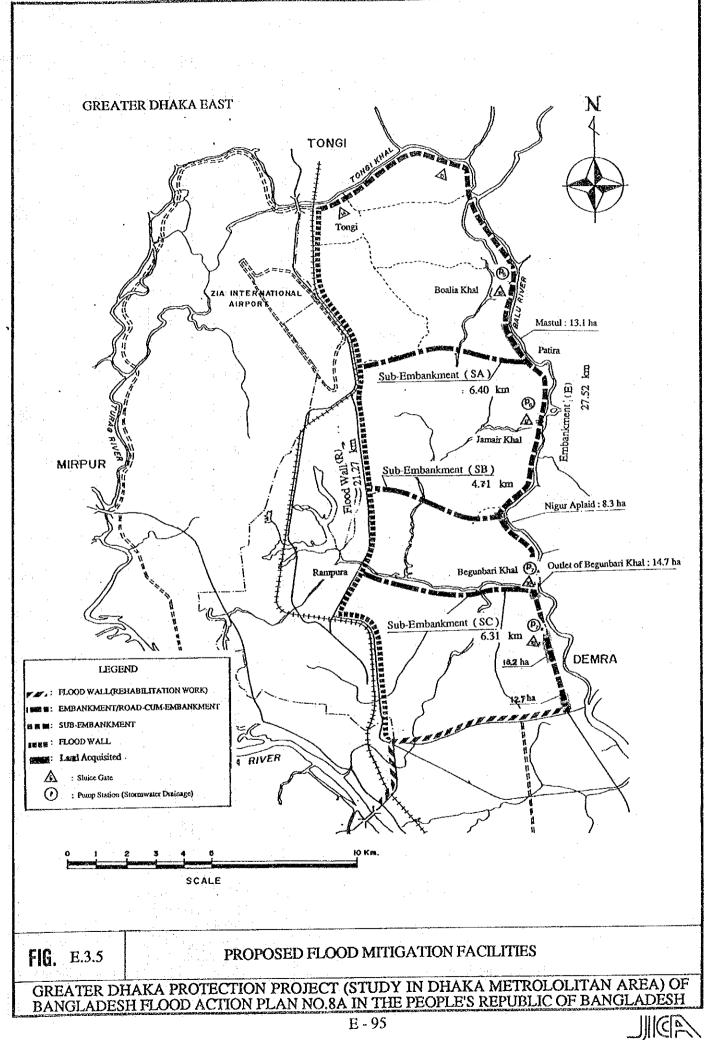


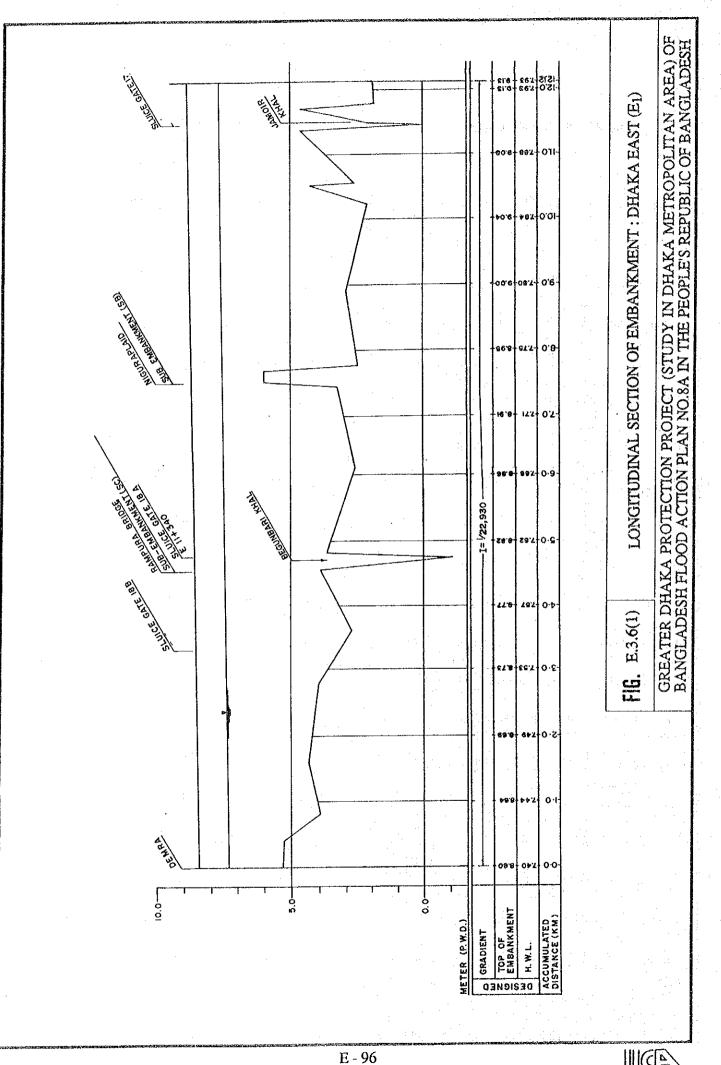
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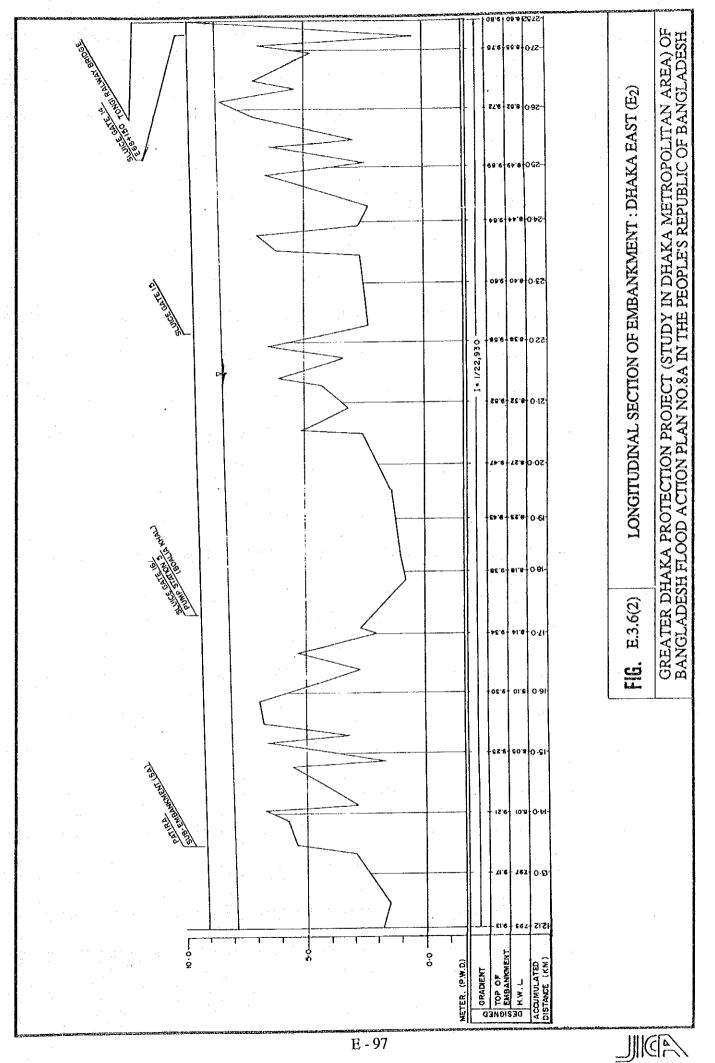
E - 94

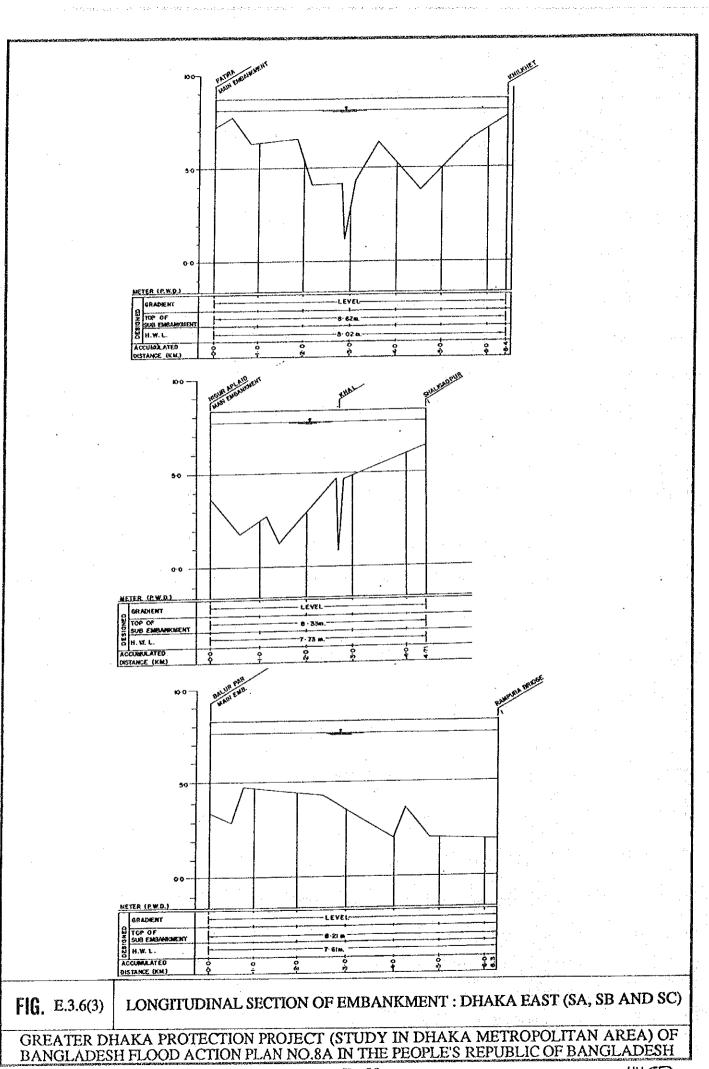


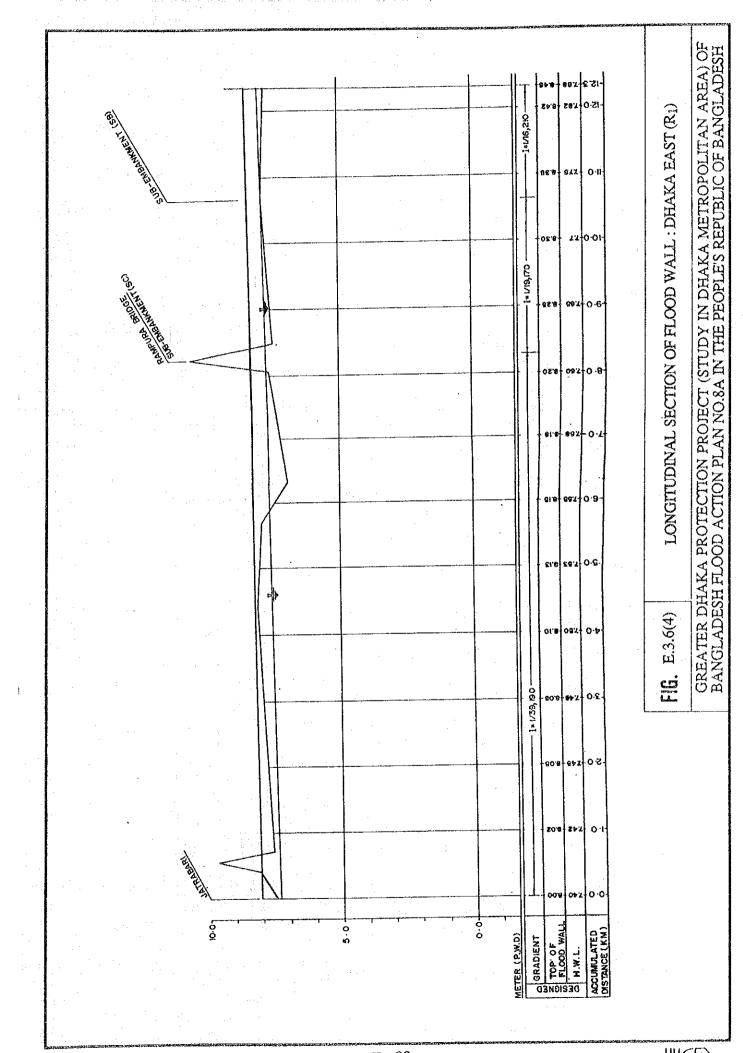




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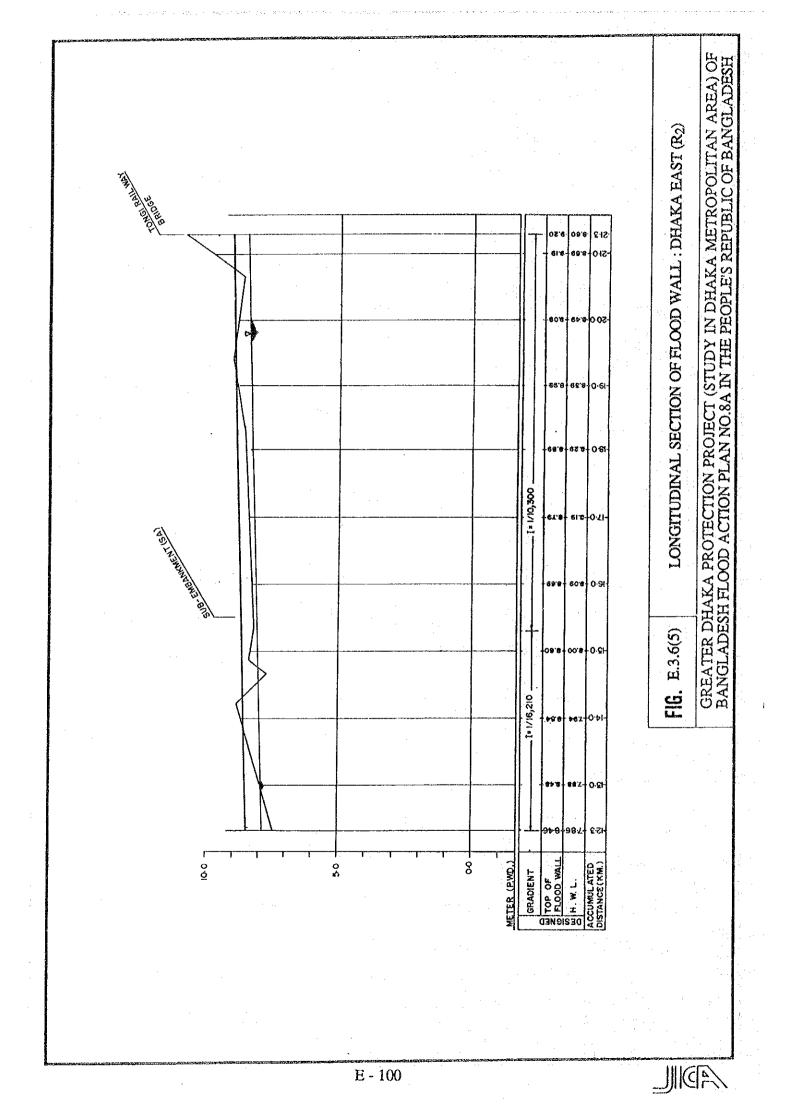


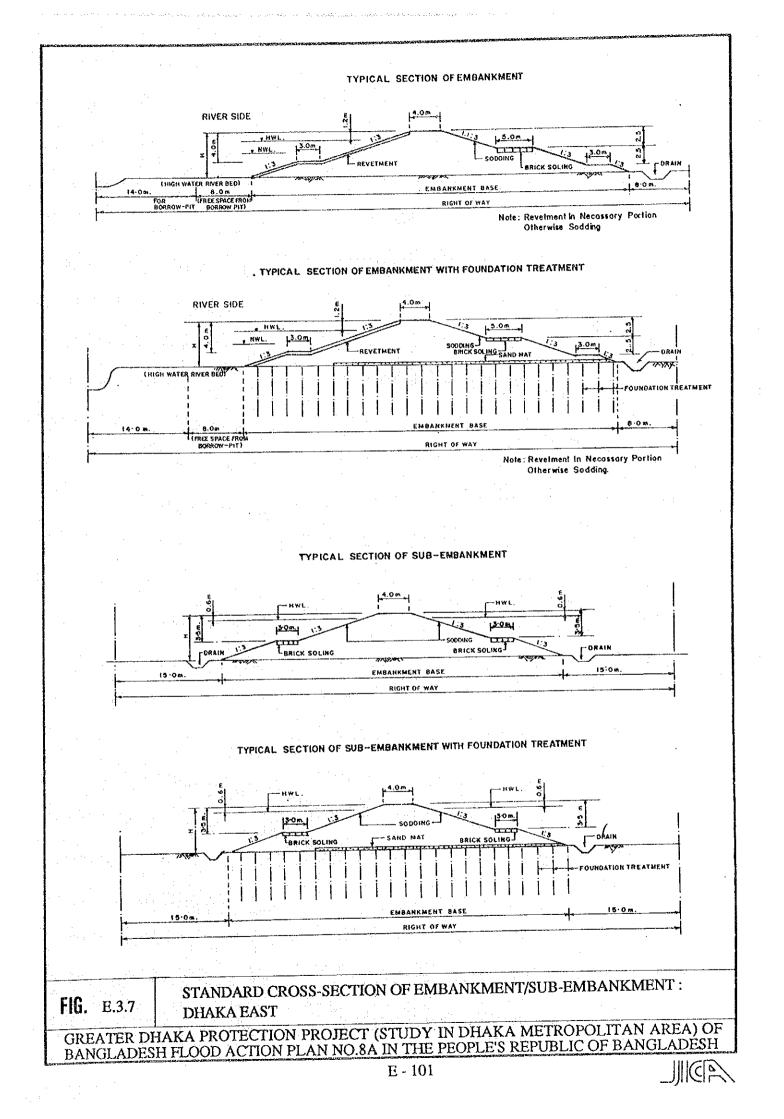


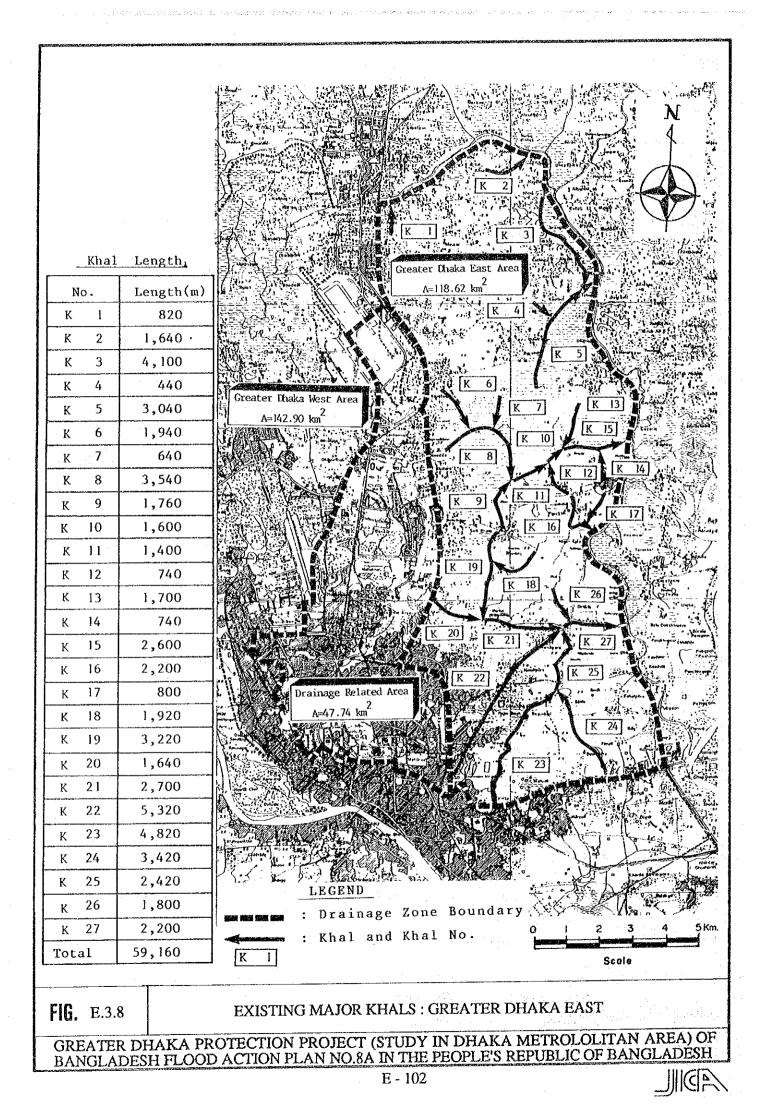


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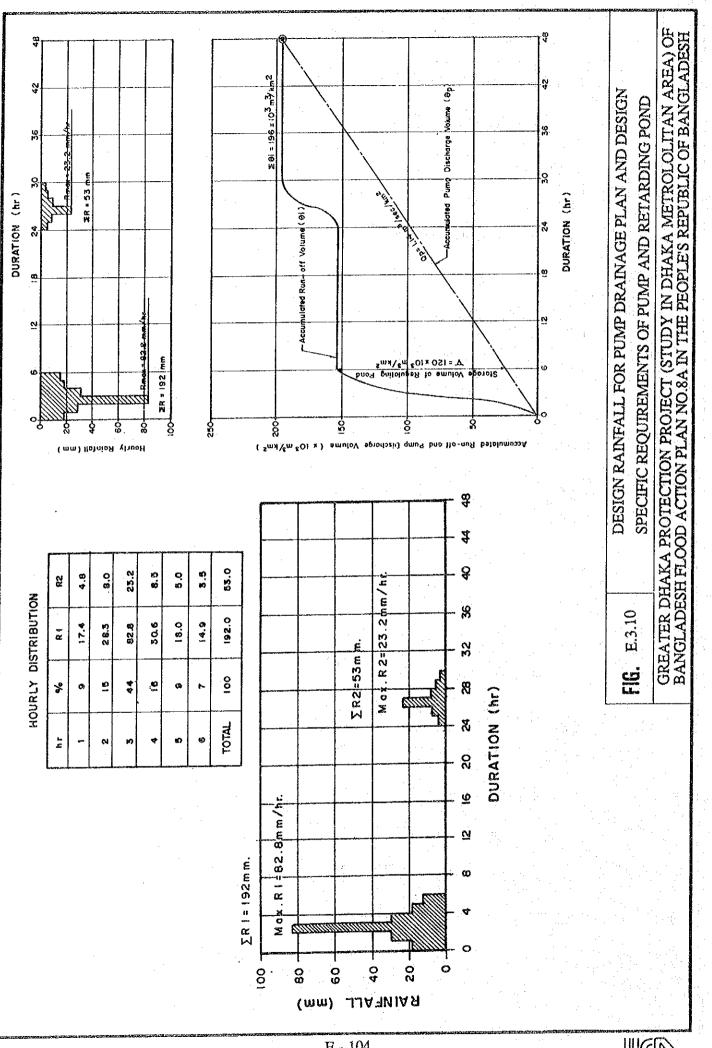
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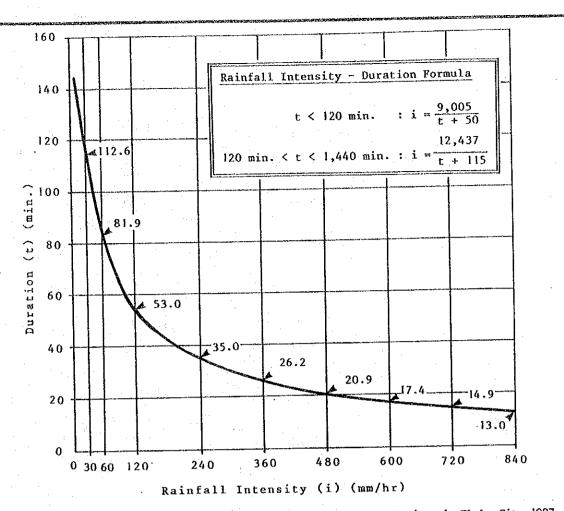


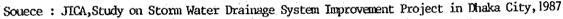


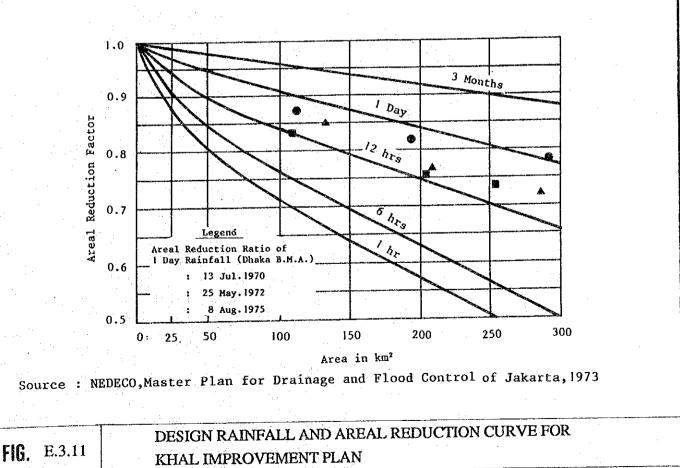


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	P*****	y			: 	r		Y			 1	r					*****		1	*** **	r	:		V) OF ESH
	Improved Length(m)	(2,220	1,000	2,366	195	110	Sub-total 5,891	Phase II Work (1992 - 1993)	3,035	1,204	1,576	1,000	4,500	767	12,082	(†	2,200	4,820	7,020	24,993		D BY ADB	OLITAN AREA) OF OF BANGLADESH
Phased Khal Improvement Work	Name of Khal	Phase I Work (1991 - 1992	Mohakhali Khal	BegunbariKhal	Segunbagicha Khal	Rajabazar Khal	Kathalbagan Khal			Khilgaon-Basabo Khal-	Gerani Khal	Segunbagicha Khal	Begunbari Khal	Banani-Gulshan Khal	Paribagh Khal	Sub-total	Phase III Work (1993 - 1994)	Mugda Khal	Begunbari Khal	Sub-total	Total		OSED IN DIFPP FINANCED BY ADB	IN DHAKA METROLOLI PEOPLE'S REPUBLIC OF
Ρι	Khal No		K 3	K11	K 7	K 5	K10			K 2	K 6	K 7	K 1 2	К 9	K 4		54	K 14	80 M				S PROP((STUDY IN THE
						and the second se			X8) A=46.58km ²			And the second									1 2 3 4 5 Km	Scole	FIG. E.3.9 KHAL IMPROVEMENT WORKS PROPOSED IN	GREATER DHAKA PROTECTION PROJECT (STUDY BANGLADESH FLOOD ACTION PLAN NO.8A IN THE
																					us manue : Drainage Zone Boundary	(KS) : Khal and Khal No.	red Fl	Protection,FAP-8R,Sep.1991
	E - 103													0.148.boX			5588.W 1144	1749DCT_236			<u></u>			

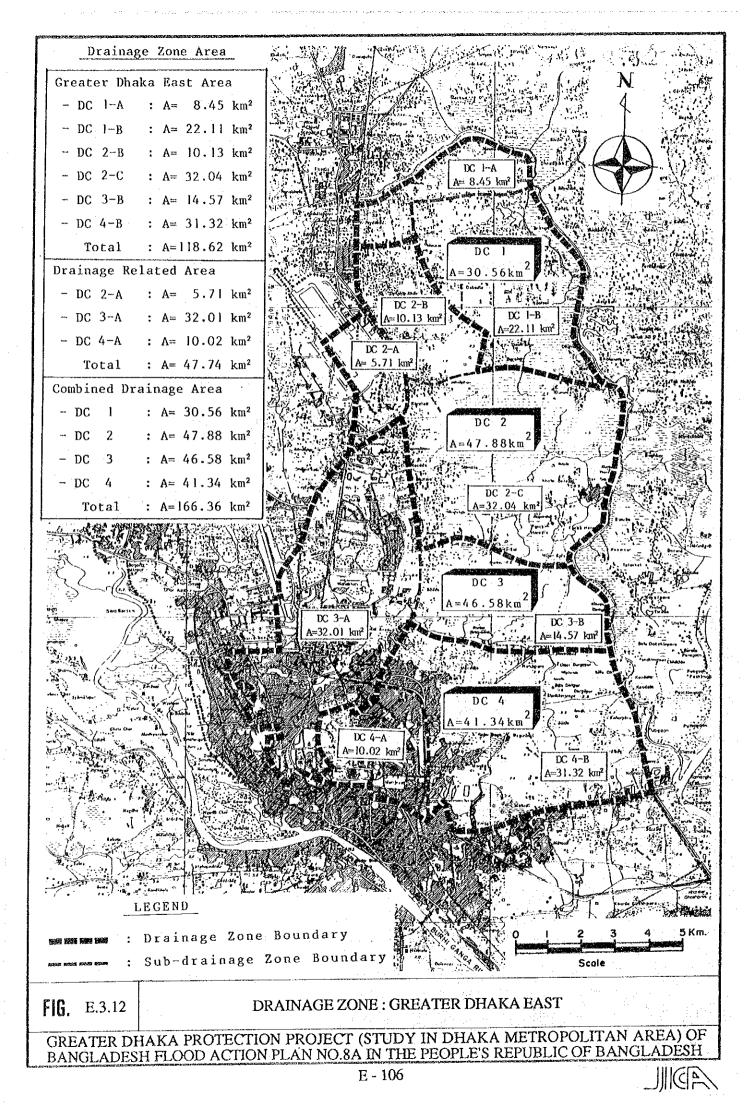


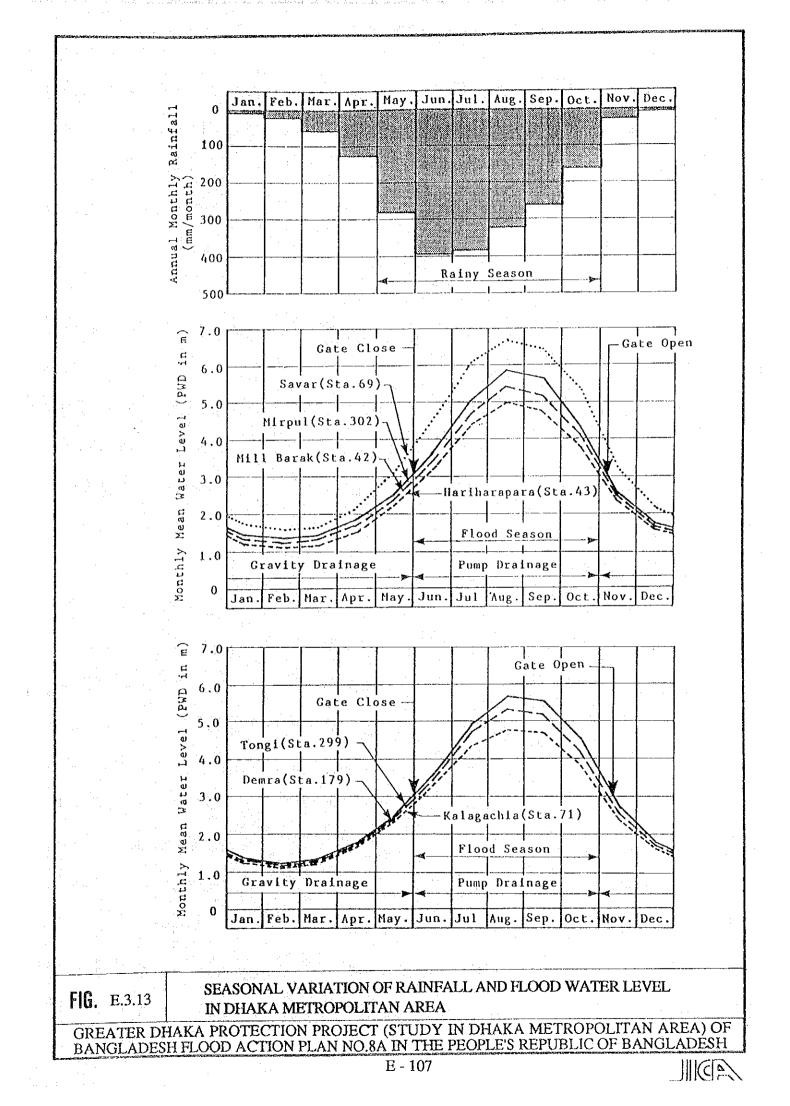


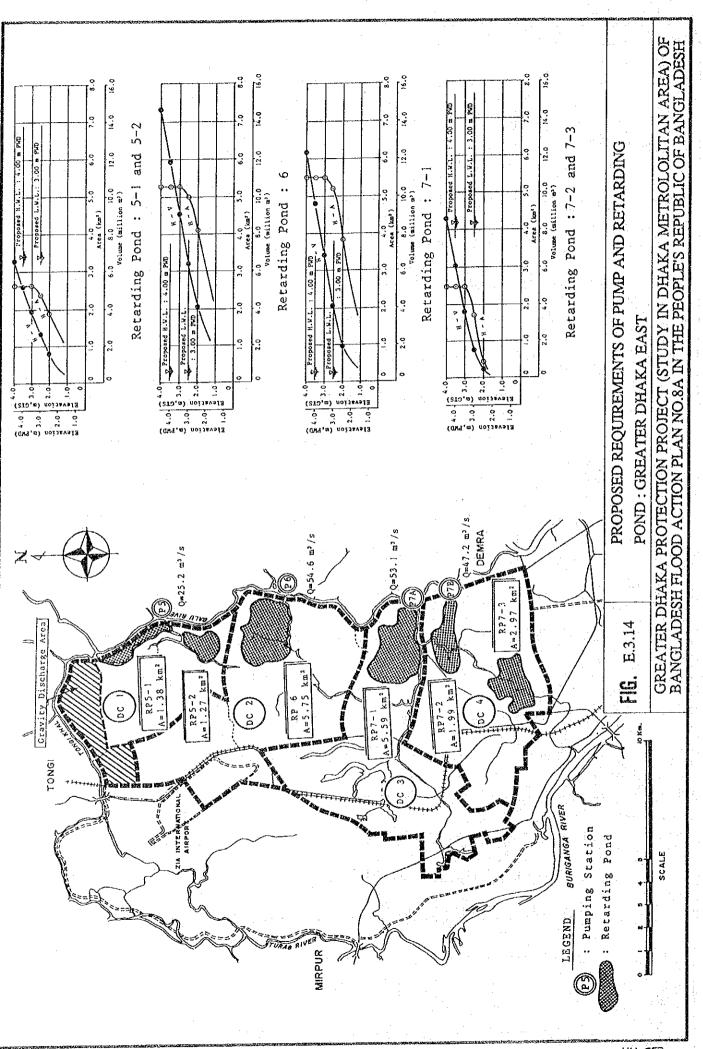




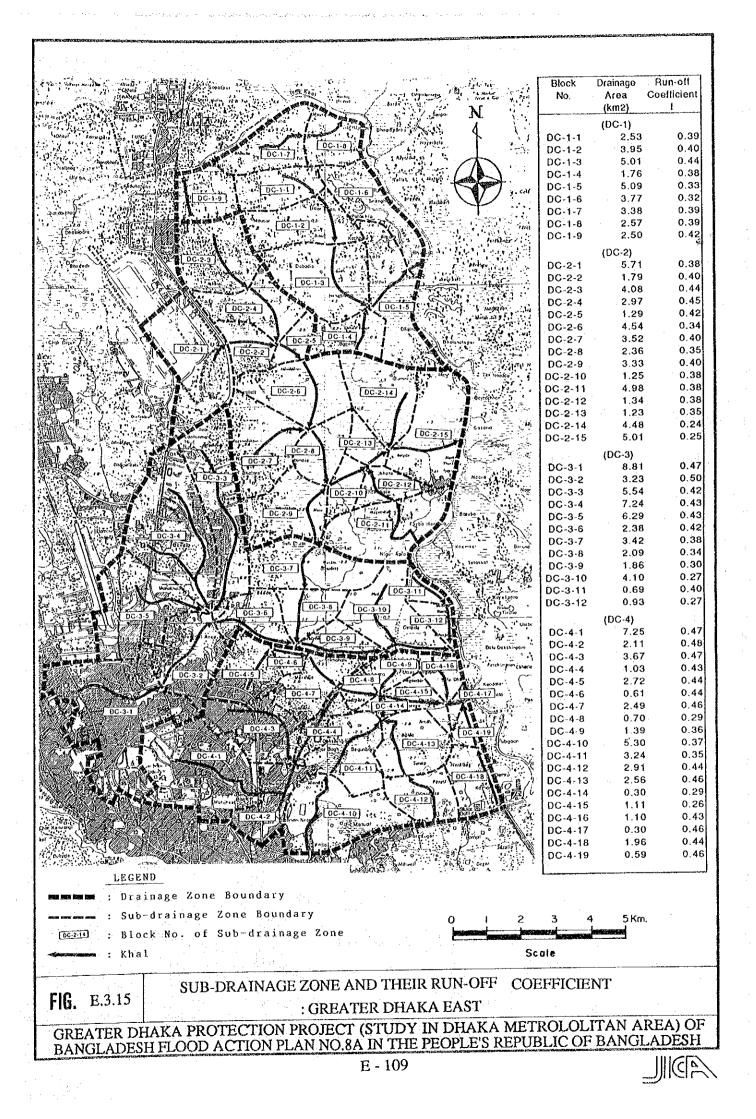
GREATER DHAKA PROTECTION PROJECT (STUDY IN DHAKA METROPOLITAN AREA) OF BANGLADESH FLOOD ACTION PLAN NO.8A IN THE PEOPLE'S REPUBLIC OF BANGLADESH

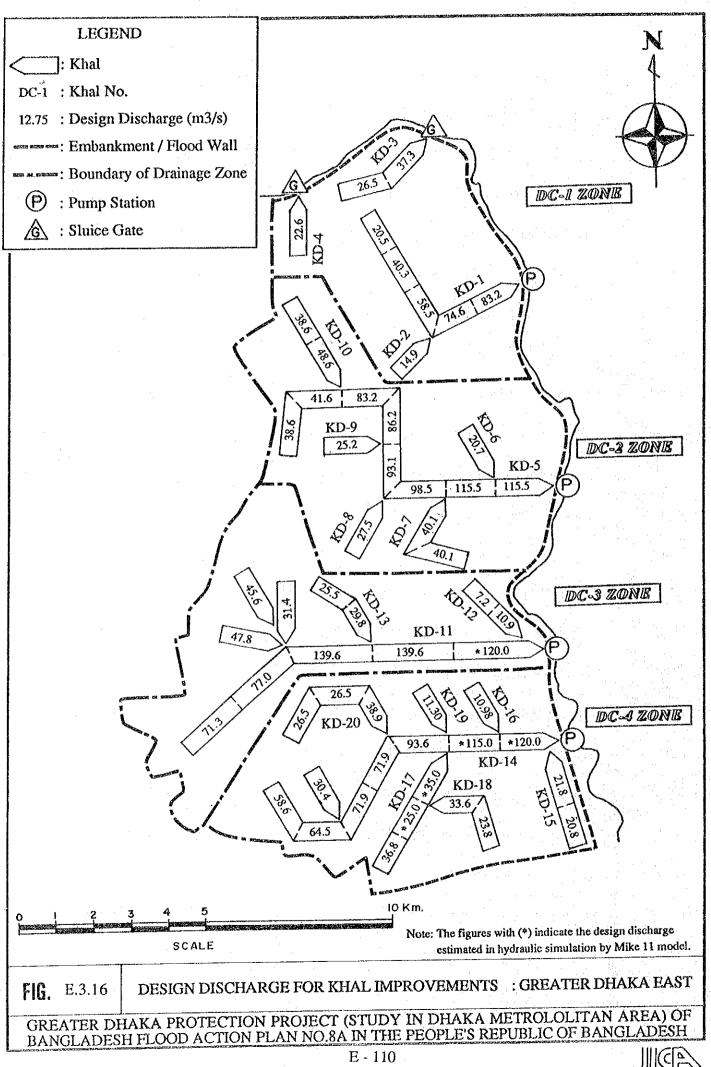


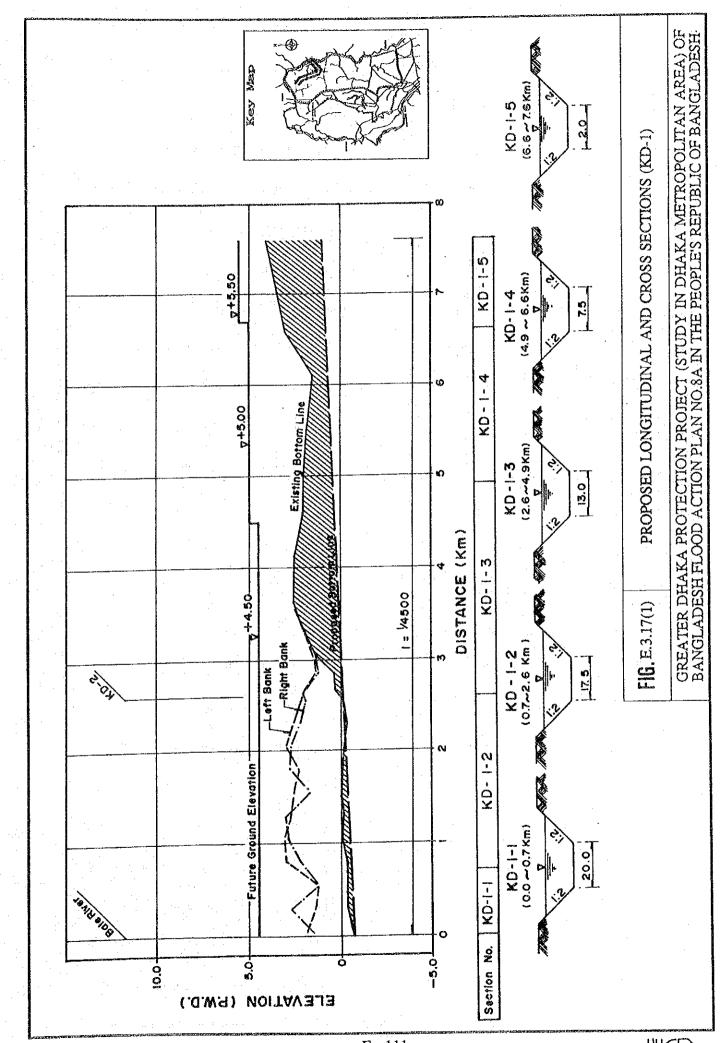




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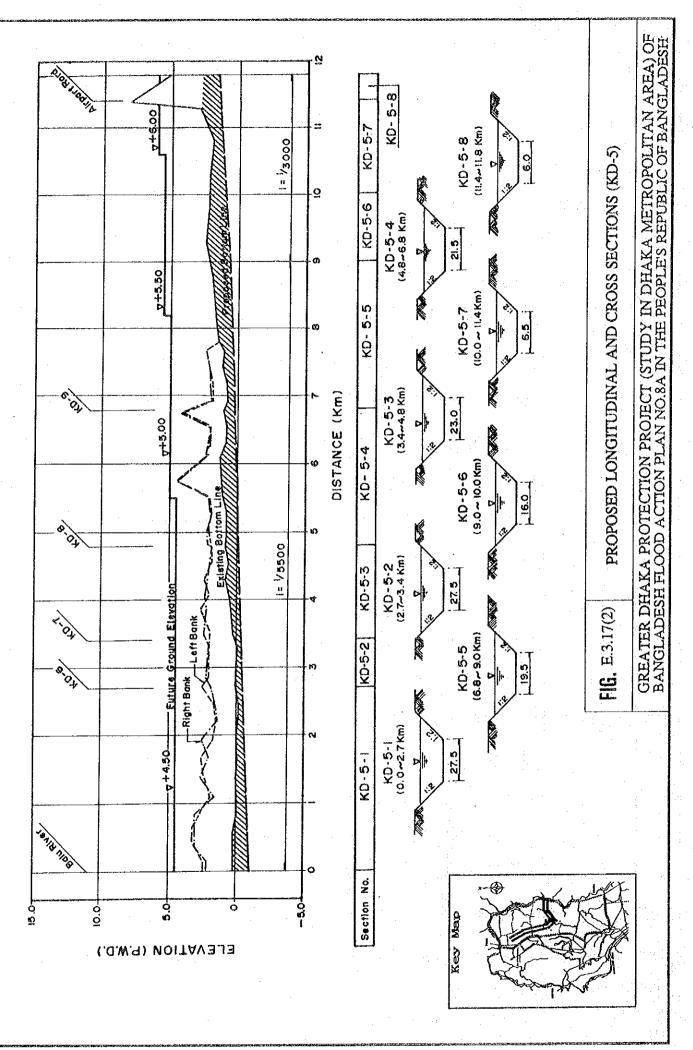




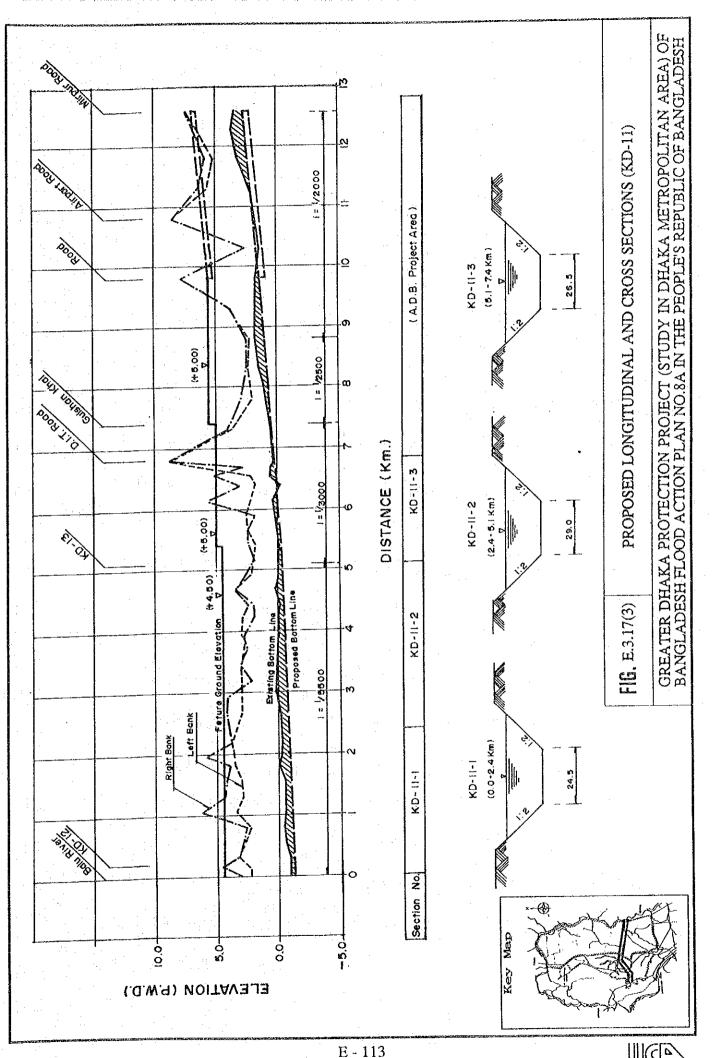
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