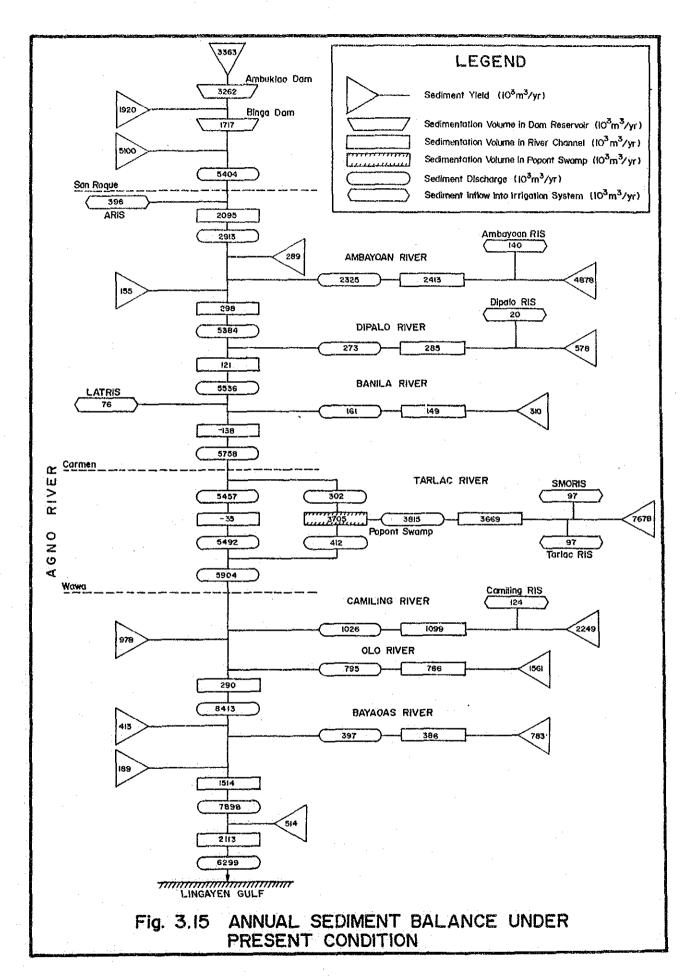
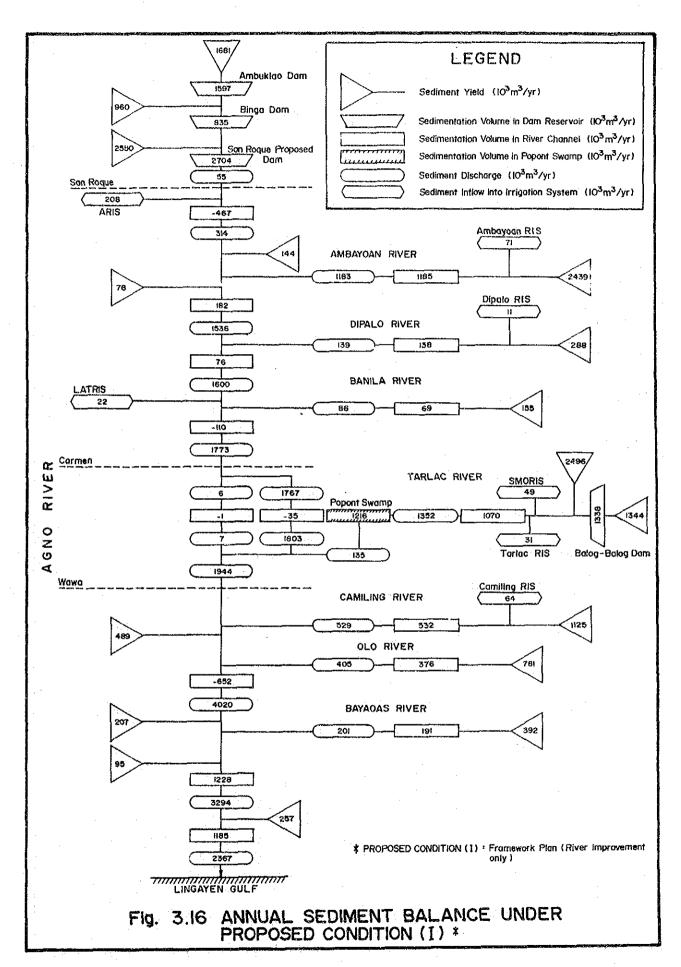
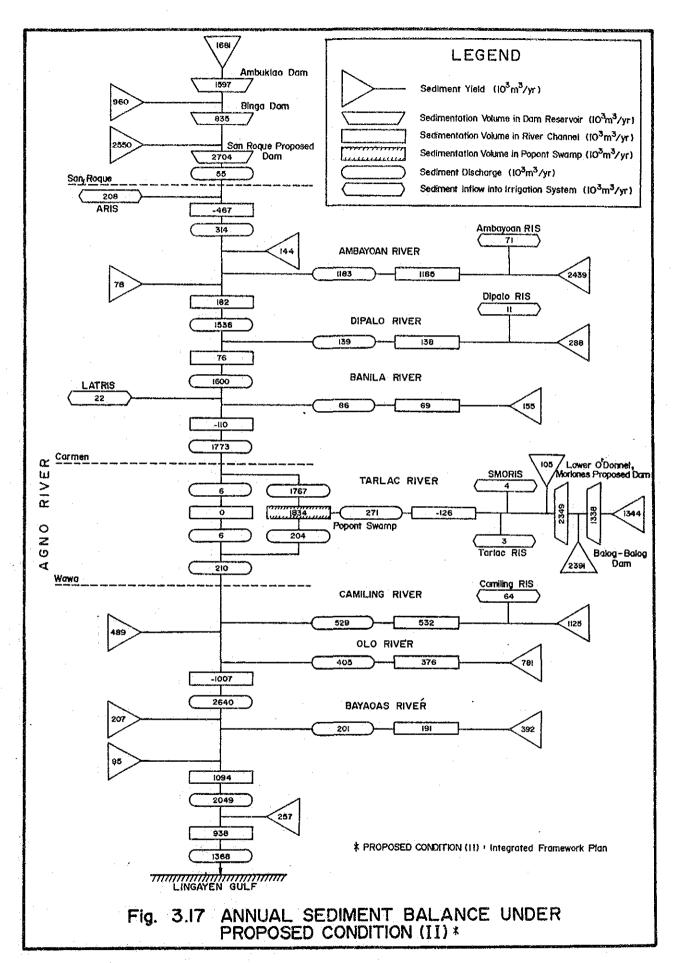
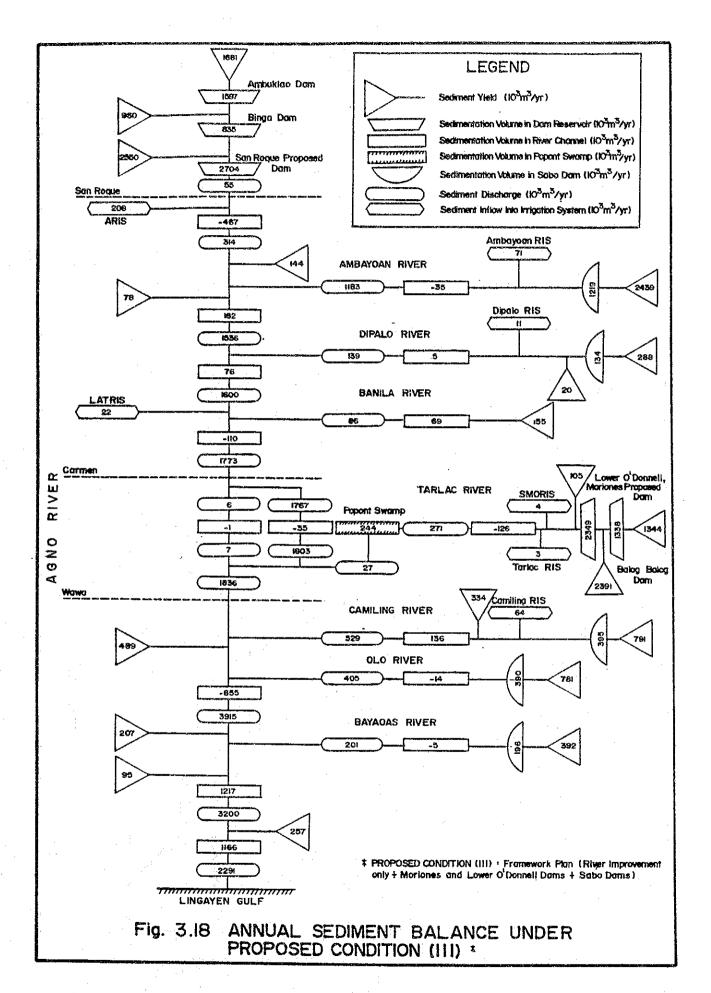


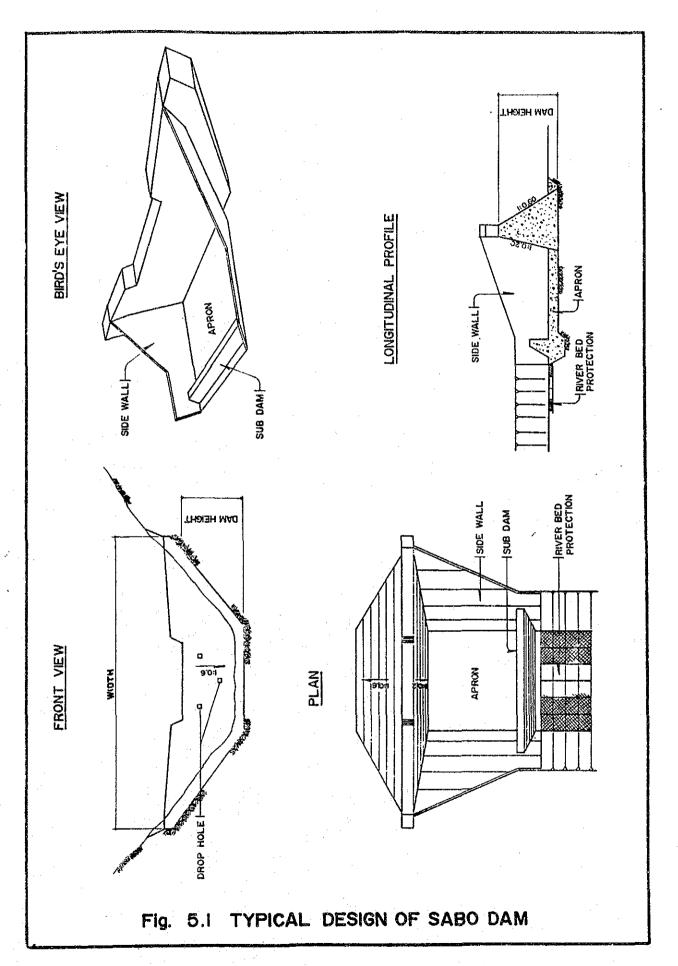
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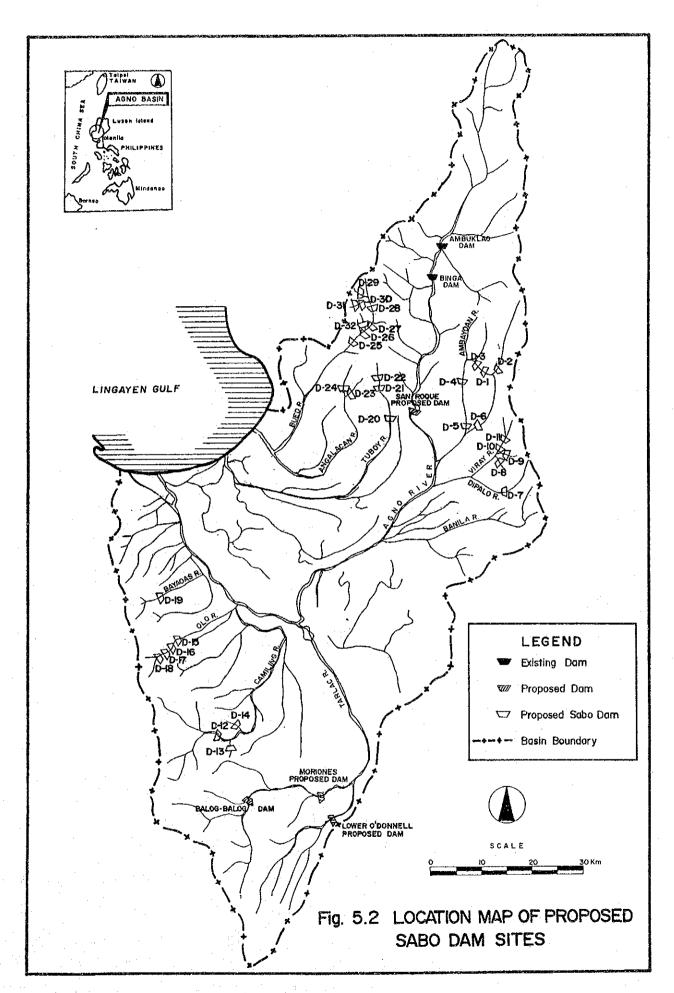












7. RV RIVER IMPROVEMENT PLAN

RV : RIVER PLANNING

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ABBREVIATIONS

1. NAME OF PHILIPPINE GOVERNMENT AGENCIES

GOP Government of the Philippines

DPWH Department of Public Works and Highways

DPWH-PMO DPWH Project Management Office

NIA National Irrigation Administration

NAPOCOR National Power Corporation

PAGASA Philippine Atmospheric, Geophysical and Astronomical

Services Administration

NAMRIA National Mapping and Resource Information Authority

BFAR Bureau of Fishery and Aquatic Resources

AFCS Agno River Flood Control System

ARIS Agno River Irrigation System

2. NAME OF JAPANESE GOVERNMENT AND OTHER OFFICIAL AGENCIES AND ORGANIZATION

GOJ Government of Japan

JICA Japan International Cooperation Agency

MOC Ministry of Construction, Japan

OECF Overseas Economic Cooperation Fund, Japan

UN United Nations

3. MEASUREMENT UNITS

| (Length) | • | (Weight) | |
|----------|---------------|----------|------------------|
| mm | millimeter(s) | gr(grs) | gramme(s) |
| cm | centimeter(s) | kg(kgs) | kilogramme(s) |
| m | meter(s) | ton(s) | ton(s), eq'vt to |
| lem . | leilomotor(a) | | 1 0001ca |

| Area) | | (Time) | |
|-----------------|----------------------|-----------|-----------|
| mm ² | square millimeter(s) | sec | second(s) |
| cm^2 | square centimeter(s) | min | minute(s) |
| m^2 | square meter(s) | hr(hrs) | hour(s) |
| km^2 | square kilometer(s) | dy(dys) | day(s) |
| ha(has) | hectare(s) | mth(mths) | month(s) |
| | | yr(yrs) | year(s) |

1. INTRODUCTION

This Supporting Report presents the result on the study of the present river condition, river improvement and floodway plans for the Framework Plan and Long Term Plan.

For the present river condition study, the following items were carried out.

- Characteristics of river form and longitudinal and cross-sectional form.
 - Carrying capacity of existing river channel.
 - Condition of river bed fluctuation and scouring of banks and dikes.
 - Existing river facilities.

Regarding the river improvement and floodway plans for the Framework Plan and Long Term Plan, the following studies were carried out.

- River improvement only as one plan of the alternative Framework Plans.
- Conceivable floodway plan to be incorporated in the alternative Framework Plan.
- River improvement and floodway plan for the selected Framework Plan.
 - River improvement and floodway plan for Long Term Plan as a stage development plan for Framework Plan.

2. PRESENT RIVER CONDITIONS

2.1 River Systems

The Agno River basin subject to the Study is surrounded by the Cordillera Central Mountains on the north and the Zambales Mountains on the southwest and is adjoining the Pampanga River basin on the southeast, thus it forms the northern part of the Central Plains of Luzon.

In the Study area are the basins of the Agno River and its neighboring rivers named as Allied Rivers as shown in Fig. 2.1. The total basin area is about $7,640 \text{ km}^2$. The river system diagram and the physical features of rivers are shown in Fig. 2.2 and Table 2.1 respectively.

The Agno River is ranked as the fifth largest scale river in the country with a basin area of about 5,910 km2 and river length of 221 km. It originates in the Cordillera Central Mountains and flows southward in the mountainous area. Then it debauches into a vast alluvial plain and flows down toward Bayambang, located about 50 km upstream from the rivermouth, collecting runoff from the left bank tributaries and joins the Tarlac River, a major tributary of the Agno River.

The confluence of the Agno and Tarlac Rivers forms a depression known as the Poponto swamp. These areas are usually flooded during the rainy season and function as a natural retarding basin thus somehow aiding in reducing flood peak to the downstream.

After joining the Tarlac River, the Agno River turns to the northwestward, collecting runoff from the northeastern slope of the Zambales Mountains, and finally empties into the Lingayen Gulf.

The principal tributaries of Agno River are the Ambayaoan, Viray-Dipalo, Banila and Camiling Rivers which drain from the north.

The catchment area and river length at major points of these rivers are as follows:

| River | Catchment area (km ²) | River length (km) |
|-----------------------|---------------------------------------|----------------------|
| Agno River | | |
| Whole river basin | 5,907 | 221.0 |
| Poponto floodway site | 2,477 | 119.0 |
| Major tributaries | · · · · · · · · · · · · · · · · · · · | |
| Ambayaoan | 367 | 62.2 |
| Viray-Dipalo | 135 | 21.2 |
| Banila | 309 | 39.0 |
| Tarlac | 1896 | 93.0 |
| Camiling | 604 | 64.0 |

The Allied Rivers, independent rivers from the Agno River, mainly consist of the Cayanga-Patalan and the Panto-Sinocalan River systems. The catchment areas of these rivers are $618~{\rm km}^2$ and $1,115~{\rm km}^2$ respectively. The streams of the Allied Rivers are called by different names in each municipality area.

The Cayanga-Patalan River originates in the northern mountains near Baguio City and flows southwest before turning on a northwestern course, and finally empties into the Lingayen Gulf. After debauching into the plain, it is joined by the Aloragat and Bued Rivers which also drain from the northern mountainous area. The Bued River is one of the major tributaries of the Cayanga-Patalan River, having about 45% of the total basin area. At a point about 16 km upstream of the Bued River from the confluence with the Cayanga-Patalan River, the Bued River is naturally connected with the Aloragat River. A part of flood water of the Bued River is thus diverted.

The Panto-Sinocalan River heads in the northern mountains and takes the same flow direction as the Cayanga-Patalan River, and also empties into the Lingayen Gulf. The River holds the tributaries such as; the Macalong, Ingalera and Dagupan Rivers which drain from its left bank. The major portion of the watershed of these tributaries is located in the plain area.

The catchment area and river length at major points of the Allied Rivers are as follows:

| River | Catchment River area (km²) length (km) |
|---------------------------|--|
| Cayanga-Patalan-Angalacan | River |
| Whole river basin | 61.0 |
| Aloragat | 116 31.0 |
| Bued | 286 54.0 |
| | |
| Pantal-Sinocalan-Tagamusi | ng River |
| Whole river basin | 1,115 75.5 |
| Mitura-Macalong | 141 31.0 |
| Ingalera | 197 32.5 |
| Dagupan | 273 |

2.2 Channel Conditions

2.2.1 General Features of Stream Channels

From a viewpoint of stream form, river channel condition may be classified into meandering, braided and straight ones. Through the site inspection together with the data of river cross sections and topographical maps, channel condition and river bed configuration of the Agno and the Allied Rivers might be roughly described as follows:

Straight channels are only found in the short stretches near the rivermouth and cut-off channels of the Agno River. Braided channels are readily observed in the upper reaches of the main Agno between the confluences with the Viray-Dipalo River and the ARIS dam, in the Ambayaoan River, in the upper reaches of the Viray-Dipalo and the Banila Rivers, and in the Bued and Tuboy Rivers. These stretches are formed of randomly interconnected channels separated by bars and therefore stream flow tends to be wide spread and water depth becomes relatively shallow. The lower reaches of the main Agno between 5

km upstream of the rivermouth and the confluence with the Tarlac River shows a meandering channel. The upper main Agno between the inlet of Poponto floodway and the confluence with the Viray-Dipalo River and the Talrac River have also a meandering channel with developed sand bars. Other meandering channels are observed in the Camiling, the main Cayanga-Patalan, and in the main Pantal-Sinocalan Rivers. Fig. 2.3 shows the historic shifting of the river course of the main Agno River from 1947 based on several topographic maps.

The general features of present river channels of the Agno River and its major tributaries and the Allied Rivers are summarized below:

| | • | | | | |
|------------|----------------------|-----------------------|---------------------------|-------------------------|--------------------|
| River | Stretch | River width (m) | Low Water width (m) | Channel depth (m) | River Bed Slope |
| Agno | (Mouth-Tarlac) | 4,000-1,500 | 550~100 | 8.0-4.0 | 1/7,000-1,650 |
| | (Tarlac-ARIS dam) | 2,400-450 | 350- 75 | 5.5-3.0 | 1/1,650- 200 |
| Tarlac | (8 kmUs-Tarlac town) | 1,500-600 | 550- 60 | 3.5-2.5 | 1/1,200- 750 |
| Ambayaoan | (Ds.end-9 kmUs) | 450-150 | 75- 60 | 2.5-1.5 | 1/200- 150 |
| Viray-Dip | alo (Ds.end-8km Us.) | 450-250 | 120- 55 | 4.0-3.0 | 1/400- 250 |
| Banila | (Ds.end-30km Us.) | 120- 30 | 120- 25 | 4.0-1.5 | 1/850- 100 |
| Camiling | (Camiling-Mayantoc) | 120- 50 | 120~ 50 | 6.0-5.0 | 1/2,000- 250 |
| Pantal-Si | nocalan-Tagamusing | 300- 35 | 160- 10 | 7.0-1.5 | 1/1,750- 70 |
| Cayanga-Pa | atalan-Angalacan | 300- 35 | 170- 20 | 6.0-2.0 | 1/1,300- 140 |
| | | | | | |

Note: Ds.: Downstream Us.: Upstream

2.2.2 Carrying Capacity

The carrying capacity of the existing river channel is estimated for the Agno River, its major tributaries and the Allied Rivers by means of non-uniform flow or uniform flow calculations.

The carrying capacity is herein defined as the discharge at the water level below the dike crest or the bank by a free board.

The estimated carrying capacity is shown in Figs. 2.4 and 2.5, and is summarized below:

| River/stretch | Carrying Capacity | | | |
|-----------------------------|-------------------------------|---------------------------------------|----------|--|
| | Discharge (m ³ /s) | Return Period | (yr) | |
| A Diagram | | | | |
| Agno River | 2 500 8 000 | 2 - 25 | 1000 | |
| River mouth-19km Us. | 2,500 - 8,000 | 5 - 40 | i | |
| 19km Us 44km Us. | 4,500 - 13,500 | 3 - 40 | | |
| 44km Us 50km Us. | 1,000 - 2,000 | 3 - 5 | | |
| (Bayambang) | | | | |
| 50km Us 90km Us. | 1,500 - 14,000 | 2 - 100 | 7.5 | |
| 90km Us 99km Us. | 1,000 - 11,000 | 2 - 100 | | |
| Tarlac River | | | | |
| 8km Us 33km Us. | 600 - 7,500 | 3 - 100 | | |
| 33km Us 37km Us. | 3,200 - 5,800 | 60 - 100 | | |
| Camiling River | | | | |
| Camiling-Mayantoc | 500 - 1,400 | 5 - 50 | | |
| Banila River | | | F | |
| Ds. end - 24km Us. | 70 - 400 | 6 | • . | |
| Viray - Dipalo River | and the same of | | 5. | |
| Ds. end - 8km Us. | 400 - 1,500 | 10 - 100 | g in the | |
| Ambayaoan River | | e general effect | | |
| Ds. end - 9km Us. | 800 - 1,800 | 6 - 100 | | |
| Pantal-Sinocalan-Tagamusing | g River | and the second | | |
| River mouth - 49km Us. | 120 - 500 | 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | | |
| Cayanga Patalan River | in the second | | | |
| River mouth - 38km Us. | 180 - 830 | 1.5 - 3 | | |
| Aiver mouth - John Us. | 200 030 | | | |

Note: Us.: Upstream Ds.: Downstream

2.2.3 River Bed Fluctuation and Caving

River bed fluctuation

River bed fluctuation was studied by using data of river flow at water level gauging stations and the river cross-section surveys in 1981 and 1989. Fig. 2.6 shows secular variation of annual lowest water level at stations and change of deepest river bed estimated based on the cross-section data.

In regards to the tendency of the longitudinal variation of the main Agno, it seems that river bed in the lower reaches from confluence of the Tarlac River and the upper reaches from the confluence of the Banila River is aggradating, while the middle reaches is degradating but their change is not so

high. In the Tarlac River, aggradation is observed in the stretch from the confluence of main Agno up to 15 km upstream.

Caving

The existing dike and bank are eroded by severe scouring action for almost the whole stretches. The condition of breaches, gaps and scours caused by past floods in the period from 1984 to 1986 is summarized in Table 2.2. These locations are shown in Fig.2.7.

As shown in Fig.2.7, many banks located in the stretches having braided form and meandering with developed bars have caved.

2.2.4 Sediment

Fig. 2.8 shows sediment deposition volume in the Agno and Tarlac Rivers, which is estimated by comparing river cross-section data surveyed in 1981 and 1989 with due consideration of the excavated volume for dredging works conducted so far.

As shown in the above Figure, the Agno main river course is broadly divided into sediment deposition and transportation stretches; the former is located at stretches from the river mouth to the confluence with the Tarlac River and from the confluence with the Banila River to foot of mountain near the ARIS dam, and the latter is observed at stretches between the confluences of the Tarlac River and the Banila River. The Tarlac River has a tendency that the sediment transported from the upstream basin is deposited copiously around the Poponto swamp.

Sediment transport capacity in the existing channel is estimated as presented in Fig.2.9. The estimation is carried out by the use of sediment formula based on the data on river bed materials and hydraulic character on each channel condition. Sato-Kikkawa-Ashida's formula for the bed load and Lane-Kalinske's formula for the suspended load are employed for the calculation of sediment discharge. Details of sediment calculation is described in the Supporting Report (SD). Annual transport capacity of the Agno is summarized as follows:

- From river mouth to junction with the Tarlac : $80 \times 10^3 700 \times 10^3 \text{m}^3/\text{yr}$
- From junction with the Tarlac to one
- with the Banila : $700 \times 10^3 100 \times 10^3 \text{m}^3/\text{yr}$
 - From junction with the Banila to ARIS dam : $100 \times 10^3 220 \times 10^3 \text{m}^3/\text{yr}$

2.2.5 Rivermouth Clogging

No serious problems about rivermouth clogging has been reported for the Agno River and the Allied Rivers, but sand bars are developing at the rivermouth of the left course of the Agno River and the Cayanga River.

The following are findings through field investigations for the rivermouth clogging.

Agno River Main Course

The Agno River bifurcates into the left and main courses at about 5 km upstream from its rivermouth. The rivermouth of the main course at present is 400 - 500 m in width and the river bed near the rivermouth is aggradating. However a rivermouth clogging is not reported.

According to the interview survey with the residents near the rivermouth, before 1940 the channel of present main courses was narrow with a width of about 50 m and most of the flood water flowed into the Lingayen Gulf through the present left course and the Pantal River. After dredging and widening of the main course conducted around 1940, flood water flowed into the main course and thus the channel gradually became wider.

Moreover, a large quantity of sediment from the upstream is deposited and the rivermouth is moved towards the sea. The annual extension rate of sand bar estimated by comparing the aerophotos taken in 1964 and 1989 is about 40 m/yr at the left side.

As for the left course of the Agno River, the shoreline near the rivermouth is coming near the land since the supply of sediment from the upstream was reduced by the change of main flow to the present main course. Comparing the aerophotos taken in 1964 and 1989, the withdrawal of the shoreline of this left side is about 200 m for 25 years. In order to prevent further erosion, a breakwater is at present being constructed along the shoreline. Dredging was conducted from 1981 to 1984 to remove a sand bar developed from the left side, but the sand bar at the right side is developing and has narrowed the river width to only 10 m at present.

Pantal River

The width of the rivermouth of the Pantal River is stable at about 300 m. However, the water depth sometimes becomes shallow due to sedimentation. From 1987 to 1988, the river channel 1,500 m in length was dredged from 400 m upstream of the rivermouth.

Cayanga River

A sand bar is developing on the right side narrowing the rivermouth to a width of about 50 m. In the ensuing interview survey with the residents it was found out that there is a big difference in the width of rivermouth between the dry and the wet season. The river width is only 15 m in the dry season, but it becomes 85 m in the wet season. However, no inundation caused by rivermouth clogging has been experienced yet because the sand bar is washed out during floods.

2.3 River Facilities and Structures

Existing river facilities for flood control in the Agno and Allied Rivers are earth and concrete dikes, groins, revetments and diversion channels. Location and length of the major facilities are shown in Fig. 2.10 and Table 2.3 respectively.

Diking system is one of the most progressive flood control facilities in the Agno River. The length of existing dike of major rivers are summarized below:

| | | Length of diking | system (km) |
|------------------|----------------------------------|------------------|-------------|
| River | Stretch | Right bank | Left bank |
| Agno River | | | |
| Main Agno | River mouth-Bayambang (50km Us.) | 40.50 | 16.30 |
| | Bayambang-ARIS Dam (99km Us.) | 47.80 | 28.80 |
| Ambayaoan | Confluence-8.7km Upstream | 0.00 | 3.50 |
| Viray-Dipalo | Confluence-8.5km Upstream | 5.70 | 7.40 |
| | Viray River stretch (L=3.9km) | 3.30 | 0.00 |
| | Dipalo River stretch (L=8.1km) | 0.00 | 7.00 |
| Banila | Confluence-30.9km Upstream | 0.00 | 9.30 |
| Tarlac | Confluence-TARIS Dam (37.0km Us. |) 29.60 | 25.50 |
| Camiling | Confluence-Mayantoc (20.8km Us.) | 0.00 | 0.00 |
| Allied Rivers | | | • |
| Cayanga-Patalan | River mouth-37.5 km Upstream | 0.00 | 0.00 |
| | Tributaries | 0.00 | 0.00 |
| Pantal-Sinocalan | River mouth-49.4km Upstream | 2.50 | 1.30 |
| | Tributary | 0.00 | 0.00 |

The by-pass floodway channel was designed to divert a part of flood water of the main Agno to the Poponto swamp to improve the hydraulic bottle-neck of the existing river channel at Bayambang. Construction works was commenced in 1975. The major structures of the floodway are a pilot channel, overflow concrete spillway and parallel dikes. The dimension of existing structures are as follows:

| Floodway | : Length | of stretch | | 5,000 | m |
|----------------|----------|---|----------------|----------|------|
| | Width o | f floodway | | 800-1000 | m |
| 2 2 3 | Width o | f low waterchan | nel | 30-50 | m |
| | Length | of Dike stretch | (Right) | 5,000 | m |
| and the second | | ditto - | (Left) | 4,500 | m |
| Spillway | Length | | | 1,020 | m |
| | Crest e | levation | | 18.75 | EL.m |
| | Crest h | eight | *, * | 3.55 | m, |
| 5-23-6 | 1.5 | at the second | and the second | | |

The other river facilities in the study stretches are bridges and intake water facilities. The location and main features of existing bridges are described in the Supporting Report (DS). Fig. 2.11 shows the location of irrigation area and intake water facilities.

3. REVIEW OF PREVIOUS RIVER CONTROL WORKS AND PLAN

3.1 Previous River Control Works

As early as the 1930's Government of the Philippines started flood control study on the Pampanga and Agno River basins. Due to habitual flooding in the Agno River basin in the 1930's, construction works of earth dike was commenced on the downstream reaches of the Agno River in 1938. By the year 1960, construction of earthdike 100 km in length, improvement works of 65 km of river channels and 10 km of revetment were completed. Tables 3.1 and 3.2 shows the completed river control projects since 1968, and the work quantities and construction costs since 1972 respectively.

3.2 On-going River Control Works and Five-Year Program

Major flood control plans for on-going river improvement works in the Agno River are comprised of:

- (1) Master Plan of Agno Flood Control System
- (2) Five-year DPWH Infrastructure Program
- (3) Regular Infrastructure Program
- (4) Rehabilitation Program

Despite of these plans, a master plan with overall and long-term goals on the Agno River basin has not yet been established. At present, it might be said that these plans mainly focus on rehabilitation and maintenance works of existing river channels.

According to the Five-year DPWH Infrastructure Program (1989-1993), on-going river control works planned are 22 projects in the Agno River, 10 projects in major tributaries and 9 projects in the Allied Rivers. Almost all of the projects are the small-scale works to be implemented urgently, and these works mainly consists of bank protection ones.

The proposed projects in the Five-year DPWH Infrastructure Program are listed in Table 3.3.

4. PLAN FORMULATION METHODOLOGY AND DESIGN CRITERIA

4.1 Methodology of Plan Formulation

The Master Plan is composed of the two stage plans, Framework Plan and Long Term Plan. The Framework Plan is defined as a master plan with overall and long-range goals of basin-wide flood control plan, taking into account the social and economic conditions in the Agno River basin, which is to be achieved in an unspecified future. The Long Term Plan is defined as an stage development plan of the Framework Plan which is to be achieved at the target year of 2010.

Under the channel phase of flood control, the river improvement measures contemplated are the following:

- Improvement of channel alignment including short-cut
 - Construction of diking system
 - Deepening and widening of river channel
 - Protection of bank and dike from erosion
 - Other appropriate facilities

Another measure is flood diversion system to reduce flood discharge in the lower reaches.

4.2 River and its Stretches Subject to Master Plan

The flood control target by structural measures of river improvement and floodway are specified below.

| Planning | Scale |
|----------|---------|
| (Return | Period) |

River

Framework Plan

- Main Agno River and Tarlac River 100 year

- Other major tributaries of Agno River 50 year

50 year

Long Term Plan

- Agno River Feasible scale

- Allied Rivers Feasible scale

River stretches subject to Master Plan are shown in Table 4.1, and the length of stretches are as follows:

| | L | ength of | |
|-----------------------------------|--------|----------------|---|
| River | Subjec | t Stretch (km) | |
| Agno River | | | |
| - Main Agno | | 98.85 | |
| - Tarlac River | . * | 37.05 | 1 |
| - Camiling River | | 20.00 | |
| - Banila River | | 30.90 | |
| - Viray-Dipalo River | | 20.05 | |
| - Ambayaoan River | | 8.70 | |
| Allied Rivers | | | |
| - Cayanga-Patalan-Angalacan River | | 37.50 | |
| - Bued River | | 19.80 | |
| - Aloragat River | | 19.70 | |
| - Pantal-Marusay-Sinocalan- | | 49.40 | |
| Tagamusing-Tuboy River | | | |
| - Dagupan-San Juan-Elang River | | 27.60 | |
| - Ingalera River | | 37.50 | |
| - Mitura-Macalong River | 4 | 21.00 | |
| | | | |

4.3 Design Criteria of River Improvement and Floodway

The following are the criteria applied to the design of river improvement plans and floodway.

(1) Design flood

- Agno River : A return period of 100-year is adopted for main Agno and Tarlac Rivers, and the floodway in these rivers. The 50-year flood for other tributaries.
- Allied Rivers: A return period of 50 years is adopted for main streams, tributaries and floodway in these rivers.

(2) River bed profile

- Average river bed profile under present condition is considered as the designed river profile. (Average river bed elevation but not the deepest bed is considered as the planned river bed.)

(3) Channel section

- Compound section with high-water and low-water channels confined by dikes are adopted in principle.

(4) River width (High-Water Channel)

- River width should be decided referring to the data on rivers in Japan shown in Fig. 4.1 and by Regime theory.
- Ideal river width thus calculated should be planned as the future river alignment (Framework Plan) for stable stretches of channel in case the existing river width is larger than the above, However for the upper reaches having braided stream pattern the existing river width should be maintained.
- In case existing river width is less than the above, it should be widened.
- High-water channel elevation is set at present channel or ground level.

(5) Low-water channel (width of bed)

- Low water channel is designed with its capability to carry an approximate 2.0-year flood.
- In case the existing low-water channel has less than a 2.0-year flood capacity, the existing channel should be widened.
- In case the existing low-water channel has a greater than 2.0-year flood capacity, the existing river channel should be maintained as it is.

(6) Alignment of low-water channel (cut-off channel)

- Cut-off channel should be adopted in case the existing channel length is more than 2.5 times of the proposed cut-off channel length.

(7) High water level

- High water level is decided by hydraulic calculation.

- With regard to Manning's coefficient of roughness for water level calculation, 0.028 to 0.035 for low-water channel and 0.040 to 0.045 for high-water channel are adopted for the channel conditions. (Refer to Table 4.2 and Fig. 4.2.)

(8) Dike

- Type:

Earth embankment is adopted in principle in case there is no special restriction of available land area for constructing earth dikes.

Existing concrete dike should be maintained as it is. (Concrete parapet wall in Bayambang for example)

- Free board:

In accordance with the Standard for River and Sabo Engineering by MOC of Japan. (Refer to Fig. 4.4.)

- Crown width:

Ditto

- Slope:

Ditto

- (9) Revetment
 - Type:

Concrete revetment in principle for both the low-water and high-water level channels

- Height (LW channel): From river bed up to the top of low-water channel in case of low-water revetment
- Height (HW channel): From high-water channel bottom up to Flood

 Water level below dike crest and its free
 board
- Length:

In principle, the total length of revetment is assumed at 20% of twice the length of river channel, while in the Upper Agno it is 40% of 2 times the length of the river channel.

(10) Groin

-Type:

Wooden pile groin in principle except for special site.

Concrete pile groin or concrete block groin in case of heavy scouring and meandering sites.

- Length/Interval:

In accordance with the Standard for River and Sabo Engineering by MOC of Japan. (50m interval and 30m length)

(11) Other structures

- Bridges:

Existing bridges should be raised together with the necessary free board if it is found to be insufficient.

- Water intake:

Replacement or modification of existing intake structures should be considered when necessary.

5. RIVER IMPROVEMENT ONLY AS ALTERNATIVE PLAN FOR FRAMEWORK PLAN

5.1 Features of River Improvement Only Alternative

5.1.1. Design Flood Discharge

The design flood discharge distribution for the river improvement only alternative plan for the Framework Plan is shown in Fig. 5.1.

5.1.2. Features of Design Channel

(1) Agno River

The river improvement only alternative plan of the Agno River system comprises of low-water channel improvement (river training), construction of diking system, structures for bank and dike protection and drainage facilities such as sluice way and water gate.

The following are principal features of this plan.

| | Improvement | Diking system (km) | | | |
|--------------|---------------|--------------------|-------------|----------|--|
| River | of L.W.C (km) | New dike | Heightening | Existing | |
| Main Agno | 98.85 | 82.50 | 98.50 | 3.00 | |
| Tarlac | 37.04 | 17.50 | 49.00 | 6.50 | |
| Camiling | 18.80 | 31.50 | 0.00 | 0.00 | |
| Banila | 23.90 | 46.10 | 1.90 | 7.40 | |
| Viray-Dipalo | 20.10 | 0.00 | 7.40 | 16.80 | |
| Ambayoan | 8,70 | 12.50 | 3.50 | 0.00 | |

Note; L.W.C.: Low-water channel

There exists in the main Agno a hydraulic bottle-neck point at Bayambang between the confluence with the Tarlac River and the inlet of the Poponto floodway. In this stretch, all of the flood water of the 100-yr probable flood should be diverted to the Poponto swamp through the floodway and a closure dike should be constructed at the upper and lower end of the river channel at Bayambang. The reason for this is that the river flow cannot be controlled due to high back-water levee at the confluence of the Tarlac

River.

At river stretches between 5.0 km downstream and 1.5 km upstream of the Plaridel bridge in Carmen, backward displacement of the right dike with a length of 2.8 km is planned in order to obtain a sufficient river width the design discharge.

Principal features of designed channel of the Agno river system are summarized in Tables 5.1 and 5.2. The longitudinal profiles of the main Agno and the Tarlac Rivers are presented in Figs. 5.2 and 5.3 respectively. For the profiles of other tributaries, refer to Figs. 7.11 to 7.14.

(2) Allied Rivers

Principal features of the river improvement plan for the Allied River system are summarized as follows:

In this plan formulation, the Bued closure dike is proposed so as not to divert the flood flow to the Aloragat River. Basic concept of this plan is discussed in succeeding Chapter 6.

| River | Improvement of L.W.C (km) | Diking system (km) New dike Heightening |
|-----------------------|---------------------------|--|
| Cayanga-Patalan-Angal | acan River | |
| Main stream | 37.50 | 70.30 0.00 |
| Bued | 15.10 | 21.10 0.00 |
| Aloragat | 19.70 | 8.50 0.00 |
| Pantal-Sinocalan-Taga | musing River | |
| Main stream | 49.40 | 89.10 3.80 |
| Dagupan | 21.70 | 56.00 0.00 |
| Ingalera | 37.50 | 63.00 0.00 |
| Macalong | 21.00 | 40.00 0.00 |

Principal features of designed channel of the Allied Rivers system are summarized in Table 5.3. The longitudinal profiles of the two main streams are presented in Figs. 5.4 and 5.5. For the profiles of tributaries, refer to Figs. 7.24 to 7.30.

5.2 Work Quantities and Construction Cost

(1) Agno River

The work quantities and economic cost of river improvement only alternative plan for the Agno River system are summarized below:

| Items | Unit | Main Agno | Tarlac River | Other Tributaries |
|-------------------|---------------------|--------------|-----------------|---------------------------------------|
| Excavation | 1,000m ³ | 28,225 | 5,050 | 2,083 |
| Dredging | 1,000m ³ | 17,075 | . 0 | 0 |
| Embankment | 1,000m ³ | 25,651 | 4,302 | 3,370 |
| Revetment | 1,000m ² | 517 | 97 | 190 |
| Groin | PC. | 958 | 244 | 1,070 |
| Drainage facility | PC | 20 | , 3 | , , , 26 |
| Bridge | PC | 5 | 3 | 14 |
| Fixed weir | PC | 0 | 0 | 0 |
| Intake facility | PC | 0 | 0 | 4 |
| Main construction | Million | | | · · · · · · · · · · · · · · · · · · · |
| cost (Economic) | Pesos | 7,710 | 1,067 | 1,293 |

The details of the above work quantities and construction cost are shown in Tables 7.1 and 7.3 respectively.

(2) Allied Rivers

The work quantities and economic cost of river improvement only alternative plan for the Cayanga-Patalan and Panto-Sinocalan River systems are summarized below:

| Item | Unit | Cayanga- Patalan R. | Pantal- Sinocalan R. |
|-------------------|---------------------|------------------------|-------------------------|
| Excavation | 1,000m ³ | 2,221 | 6,376 |
| Dredging | 1,000m ³ | 390 | 113 |
| Embankment | 1,000m ³ | 1,331 | 8,058 |
| Revetment | 1,000m ² | 194 | 400 |
| Groin | PC. | 1,095 | 960 |
| Drainage facility | PC. | 16 | 39 |
| Bridge | PC. | 8 | 26 |
| Main construction | Million | | |
| cost (Economic) | Pesos | 777 ···· | 1,897 |

The details of the above work quantities and construction cost are shown in Tables 7.2 and 7.4 respectively.

6. FLOODWAY ALTERNATIVE PLANS FOR FRAMEWORK PLAN

6.1 Identification of Floodway Alternatives

The following floodway schemes are studied for the purpose of reducing flood discharge of the existing channel in the lower reaches.

(1) Agno Floodway: To divert the flood from the Allied Rivers gathered in the Agno River to Lingayen Gulf.

(2) San Manuel Floodway: To divert the Tuboy River flood (upstream of Sinocalan River) to the Agno River.

(3) Binalonan Floodway: To divert the Tuboy River flood to the Angalacan River, a tributary of the Cayanga River.

(4) Aloragat Floodway: To divert the Aloragat River flood to the Angalacan River.

(5) Bued Closure dike: To prevent the excess flood water from the Bued River to flow into the Aloragat River.

Diagram of the above conceivable schemes is shown in Fig. 6.1.

6.2 Features of Floodway Alternatives

6.2.1 Design Flood Discharge

The design flood discharge distribution of the floodway alternatives are shown in Figs. 6.2 to 6.6. As for levels of design flood, the Agno Floodway is adopted for a 100-yr flood and the other four floodways for 50-yr flood.

6.2.2 Features of Designed Floodways

Principal features of the floodway alternatives are summarized below:

Agno Floodway

This floodway intends to divert flood runoff of 6,400 m³/s from the upper Agno at the point 2.0 km downstream of existing ARIS Dam, directly into Lingayen Gulf through diversion channel consisting of new channel stretch of 20 km long and improvement channel stretches of 24 km long of Upper Sinocalan River and middle and lower Cayanga River. The river width and width of lowwater channel bed 1,000 m and at 80-60 m respectively. Fig. 6.7 shows principal features of the design channel.

San Manuel Floodway

This floodway intends to divert flood runoff of 550 m³/s from the Tuboy River at the point 8.0 km upstream of Binalonan town into the Agno River. The length of whole floodway is 12.5 km. The river width and width of low-water channel bed ranges 80-60 m and 25-10 m respectively. Fig. 6.3 shows the principal features of the design channels.

Binalonan Floodway

This floodway intends to divert flood runoff of 650 m³/s from the Tuboy River at the point 7.0 km upstream of Binalonan, into the Angalacan River through diversion channel having length of 6.7 km. The river width and width of low-water channel bed is 60 m and 15 m respectively. Fig. 6.4 shows the principal features of the design channel.

Aloragat Floodway

This floodway intends to divert flood runoff of 750 m³/s from the Aloragat River at the point 7.5 km upstream of confluence with the Cayanga-Patalan River, into the Angalacan River through diversion channel having a length of 1.5 km. The river width and width of low-water channel bed is 80 m and 25 m respectively.

Bued Closure Dike

This scheme intends to enable flood water of the Bued River to flow down through the main stream of the Bued by constructing a 2 km long closure dike at the left bank connecting naturally with the Aloragat River.

The effect of reduction of flood discharge by the closure dike is expected for the stretches of whole main Aloragat and the main Patalan between the confluence with the Aloragat and the Bued. The total length of the stretches is a 31.8 km. On the other hand, on a 16.5 km long stretch of the Bued. flood water increases.

6.2.3 Work Quantities and Construction Cost

The work quantities and economic cost of each floodway alternative shown in Tables 6.1 to 6.5.

6.3 Selection of Floodway Plan for Alternative Framework Plans

Main construction cost of the floodway alternatives are given below in comparison with the river improvement only alternative plan.

| | Economic Cost (Mill. Pesos) | | | | |
|-----------------------------|-----------------------------|-----------|---------|--------|---------|
| Alternatives | Floodway | Main Agno | Cayanga | Panta1 | (Total) |
| - River improvement only* | 0 | 7,706 | 777 | 1,897 | 10,380 |
| - Agno floodway* | 4,423 | 5,948 | 330 | 1,540 | 12,241 |
| - San Manuel floodway* | 342 | 7,845 | 777 | 1,540 | 10,504 |
| - Binalonan floodway* | 166 | 7,706 | 837 | 1,549 | 10,258 |
| - Aloragat floodway | 31 | 7,706 | 912 | 1,897 | 10,546 |
| - Without Bued closure dike | - | 7,706 | 928 | 1,897 | 10,531 |

Note; *: With Bued closure dike

As seen in the above summary, the Binalonan floodway and Bued closure dike schemes would be competitive with river improvement only alternative.

Therefore, these two schemes are incorporated in the alternative Framework Plans. All other schemes are excluded from the alternative plans for Framework Plan.

7. RIVER IMPROVEMENT AND FLOODWAY PLAN FOR FRAMEWORK PLAN

7.1 Alternative Framework Plans

7.1.1 Selected Alternative Plans

Combining river improvement, floodway, retarding basins and dams selected as a conceivable flood control method by sectorial studies, the following alternative plans were examined.

(1) Agno River

- Alternative-AG1 : A plan consisting of river improvement only
 - Alternative-AG2: A plan consisting of river improvement and Poponto natural retarding basin
 - Alternative-AG3: A plan consisting of river improvement, Poponto natural retarding basin and Moriones-O'Donnell dam
 - Alternative-AG4 : A plan consisting of river improvement and Moriones-O'Donnell dam

(2) Allied rivers

- Alternative-AL1: A plan consisting of river improvement and Bued closure dike
- Alternative-AL2: A plan consisting of river improvement, Bued closure dike and Binalonan Floodway

7.1.2 Design Flood Discharge

The design flood discharge distribution for each alternative plan is shown in Fig. 7.1.

7.1.3 Comparison of River Improvement Plan for Alternative Plans

(1) Principal features of design channel for each alternative plan

For the Agno River, the design features on river width, low water channel, river bed profile and bank protection works of low water channel are similar to those of the river improvement only alternative plan. The difference of design channel features for each alternative plan depends on high-water level, dike section and other facilities related to high-water level.

For the Allied Rivers, river width and low water channel is designed for each alternative plan, because the design discharge of each alternative plan has great difference.

(2) Work quantities for each alternative plan

The work quantities for the Agno and the Allied Rivers are shown in Tables 7.1 and 7.2, and are summarized below:

Agno River

| | | 2 | Alte | rnative | |
|-------------------|----------------------|--------|--------|---------|--------|
| Items | Unit | -AG1 | -AG2 | -AG3 | -AG4 |
| Excavation | 1,000 m ³ | 35,358 | 36,008 | 35,258 | 34,608 |
| Dredging | $1,000 \text{ m}^3$ | 17,075 | 17,075 | 17,075 | 17,075 |
| Embankment | 1,000 m ³ | 33,323 | 26,731 | 25,095 | 30,436 |
| Revetment | 1,000 m ² | 805 | 878 | 874 | 799 |
| Groin | PC. | 2,272 | 2,272 | 2,272 | 2,277 |
| Drainage facility | PC | 49 | 46 | 46 | 49 |
| Bridge | PC | 22 | 22 | 22 | 22 |
| Fixed weir | PC | 0 | · 1 | 1 | 0 |
| Intake facility | PC | 4 | 4 | 4 | .4 |
| | | , | * | | |

Allied Rivers

| | | Alternative | | |
|-------------------|----------------------|-------------|-------|--|
| Items | Unit | -AL1 | -AL2 | |
| Excavation | 1,000 m ³ | 8,597 | 8,073 | |
| Dredging | 1,000 m ³ | 503 | 478 | |
| Embankment | 1,000 m ³ | 9,389 | 8,287 | |
| Revetment | 1,000 m ² | 593 | 664 | |
| Groin | PG. | 2,057 | 1,849 | |
| Drainage facility | PC- | 69 | 63 | |
| Bridge | PC | 34 | 33 | |
| Fixed weir | PC | 0 | . 0 | |
| Intake facility | PC | .0 | 4 | |

(3) Construction cost

The estimated economic construction costs for each alternative plan are shown in Tables 7.3 and 7.4 and are summarized below:

Agno River

(million Pesos)

| River | | Alternative | | | | |
|-------------|---------|-------------|--------|--------|--------|--|
| | | - AG1 | - AG2 | - AG3 | - AG4 | |
| | | | | | | |
| Main Agno | (1/100) | 11,472 | 10,700 | 10,485 | 11,202 | |
| Tarlac R. | (1/100) | 1,587 | 1,288 | 1,061 | 1,265 | |
| Other | | | • | · | | |
| tributaries | (1/50) | 1,925 | 1,925 | 1,925 | 1,925 | |
| Total | | 14,984 | 13,913 | 13,471 | 14,392 | |

Note: Construction cost for river improvement and/or floodway only

(million Pesos)

| | Alternative | | |
|---------------------------|-------------|-------|--|
| River | -AL1 | -AL2 | |
| | | | |
| Pantal-Sinocalan R.(1/50) | 2,824 | 2,305 | |
| Cayanga-Patalan R. (1/50) | 1,158 | 1,246 | |
| Binalonan Floodway (1/50) | 0 | 248 | |
| Total | 3,982 | 3,799 | |
| | | | |

7.2 Features of River Improvement and Floodway Plan for Framework Plan

7.2.1 Design Flood Discharge

The design flood discharge distribution for the Framework Plan is shown in Fig. 7.2.

7.2.2 Features of Design Channel and Floodway

(1) Main Agno

The outline of river improvement and floodway plan of Main Agno is as follows:

In the lower reaches from the river mouth to the confluence with the Tarlac River, river improvement works over a 43.8 km long stretch comprises of the following.

- Channel improvement on the main stream over a length of 43.8 km including cut-off channel on the three stretches with a total length of over 6.0km and bank protection works.
- Diking system the both banks of the main stream over a total length of 86.6 km and a 38km long in the back-water stretches for the small scale tributaries.

- Included in the above diking system along the main stream is the construction and heightening of the dike on the right bank over 23.5 km long and 19.9 km long respectively, and on the left bank over 33.9 km long and 9.3 km long respectively.
- In the stretch 3.7km upstream of the river mouth to the junction with the Bayaoas River, a 19km right bank is newly constructed creating a 1.5km river width, and the existing dike in the stretch is planned as a secondary dike.
- Reconstruction of 10 sluice ways, 2 water gates and 4 bridges in connection with the river improvement.

In the middle reaches between the junction with the Tarlac River and the inlet of Poponto floodway, the said floodway is planned as a main channel. On the other hand, the existing main channel at Bayambang will replace as a bypass channel in order to divert a part of flood water from upstream of the Agno directly to the junction with the Tarlac River. The improvement of the 10.7 km long floodway and the 10.5 km long existing main channel comprise of the following works:

- Channel improvement on Poponto floodway over a length of 10.7km and a 1.0 km channel of downstream of the existing main channel including bank protection works.
- Diking system on the right bank over a length of 7.8 km and on the left bank over a length of 4.8 km at the Poponto floodway, and on the backwater stretches of the existing main channel over a total length of 20.3 km.
- Construction of new dike on the floodway over a 3.8 km long right dike and raising of a 4.0 km long existing right dike and a 4.8 km long existing left dike, and new dike on both bank along the existing main channel over a total length of 9.0 km.
- Construction of a fixed weir with a 3.2 km closure dike on the right bank of upstream of the Poponto floodway, and demolition of the existing side overflow weir at the inlet of the floodway.

- Construction of 2 sluice ways.

In the upper reaches from the inlet of Poponto floodway to ARIS dam, the river improvement over a 44.4 km long stretch comprises the following works:

- Channel improvement on the main stream over a length of 44.4 km including bank protection works.
- Diking system on the right bank over a length of 44.4 km and on the left bank over a length of 37.4 km.
- Backward displacement of the right dike with a total length of 2.8km at Carmen, Rosales.
- Construction of new dike on the right bank over a 9.8 km long and on the left bank over 7.5 km long, and heightening of a 30.6 km long existing right dike and a 29.9 km long existing left dike.
- In the upstream from the open levee at Asingan, the right dike is designed as a super-dike with a 50m wide dike crown and also considering removal of surplus soils.
- Construction of 4 sluice ways and raising of Plaridel bridge at Carmen, Rosales.

Alignment, longitudinal profile and typical cross-section of the Agno River improvement plans are shown in Figs. 7.4, 7.9 and 7.15 respectively. Principal features of the design channel and the floodway are summarized in Table 7.5.

(2) Tarlac River

Tarlac River improvement over a 37.0 km long stretch is composed of the following works:

- Channel improvement on the main stream over a length of 28.9km including bank protection works, and on the natural retarding stretch over a 8.1 km long low-water channel.

- Diking system over a length of 28.9km on the right bank through the whole stretch from 8.1km upstream of the confluence with the Agno to TARIS dam and on the left bank over a length of 27.9 km.
- To be incorporated to the above diking system is the heightening of the existing dike on the right bank over a length of 23.3 km and on the left bank over a length of 22.2 km, and construction of a 1.3 km new dike.
- Construction of 2 sluice ways and 3 bridges in connection with river improvement.

Alignment, longitudinal profile and typical cross-section of the Tarlac river improvement plans are shown in Figs. 7.5, 7.10 and 7.16 respectively. Principal features of the design channel are summarized in Table 7.6.

(3) Camiling River

Camiling river improvement works over a 20.0 km long stretch is composed of the following:

- Improvement of the confluence with the Agno River at 3.5km downstream of the junction with the Tarlac River.
- Channel improvement on the main stream over a length of 18.8km including cut-off channel on the two stretches over a total length of 1.5km and bank protection works.
 - Construction of diking System on the right bank over a length of 15.3 km and on the left bank over a length of 16.2 km. In the downstream from the confluence with the Bayating River and in the upstream from the confluence with the Mamair River, diking system is planned on one side of the bank only.
 - Construction of 4 sluice ways and Camiling bridge in connection with the river improvement.

Alignment, longitudinal profile and typical cross-section of the Camiling river improvement plans are shown in Figs. 7.6, 7.11 and 7.17 respectively. Principal features of the design channel are summarized in Table 7.6.

(4) Banila River

Banila river improvement over a 30.9 km long stretch is composed of the following works:

- Improvement of alignment of the confluence with the Agno.
- Channel improvement on the main stream over a length of 23.9 km and bank protection works.
- Diking system on the right bank over a length of 24.5 km and on the left bank over a length of 30.9km. In the upper reach, from 2.5km down stream of Banila bridge at Umingan, only left dike is planned considering the present condition of landuse, insufficient effect of flood mitigation by confining dike due to run-off from slope of mountain area on the right bank, and characteristics of regime of the main stream.
 - Included also to the above diking system is the construction of dike over a total length of 48.0 km including the heightening of a 2.0 km existing dike.
- Construction of 14 sluice ways, 4 intake facilities and 7 bridges in connection with the river improvement.

Alignment, longitudinal profile and typical cross-section of the Banila river improvement plans are shown in Figs. 7.7, 7.12 and 7.18 respectively. Principal features of the design channel are summarized in Table 7.6.

(5) Viray-Dipalo River

Viray-Dipalo river improvement over a 20.1 km long stretch is composed of the following works:

- Channel improvement on the Viray-Dipalo River over a length of 7.9 km, on the Viray River over a length of 4.5 km and on the Dipalo River over a length of 7.7 km including bank protection works.

- Diking system on both banks of the Viray-Dipalo River over a length of 13.0 km, on the right bank of the Viray River over a length of 4.1 km and on the left bank of the Dipalo River over a length of 7.1 km.
- From among the preceeding stretches, a 7.4 km long dike is to be raised in the existing one.
- Construction of 4 sluice ways and 4 bridges in connection with the river improvement.

Alignment, longitudinal profile and typical cross-section of the Viray-Dipalo river improvement plans are shown in Figs.7.8, 7.13, and 7.19 respectively. Principal features of the design channel are summarized in Table 7.6.

(6) Ambayaoan River

Ambayaoan River improvement over a 8.7 km long stretch is composed of the following works:

- Channel improvement on the main stream over a length of 8.7 km including bank protection works.
- Construction of diking system on both banks over a total length of 16.0km including the heightening of a 3.5km existing left dike.
- Construction of 4 sluice ways and the bridge at San Nicolas in connection with the river improvement.

Alignment, longitudinal profile and typical cross-section of the Ambayaoan river improvement plans are shown in Figs. 7.8, 7.14 and 7.20 respectively. Principal features of the design channel are summarized in Table 7.6.

(7) Cayanga-Patalan-Angalacan River system

This river improvement is composed of the following works:

Cayanga-Patalan-Angalacan River

- Channel improvement on the main stream over a length of 37.5 km including bank protection works.
- Construction of diking system on the whole stretch of the Cayanga-Patalan River over a total length of 29.6 km, and in the Angalacan River a 22.7 km long right dike and a 18.0 km long left dike.
- Construction of 16 sluice ways and 6 bridges in connection with the river improvement.

Bued River

- Channel improvement on the main stream over a length of 19.8 km including bank protection works. On a 4.7 km long portion upstream the NIA dam, only a bank protection work is to be done.
- Construction of diking system on the right bank over a length of 14.1 km and on the left bank over a length of 7.0 km including a 2.0 km long closure dike.
- Construction of 6 sluice ways and San Vicente bridge in connection with the river improvement.

Aloragat River

- Channel improvement on the Aloragat River over a length of 19.7 km including bank protection works.
- Construction of diking system on the Bobonan River new main stream after construction of Bued closure dike, over a total length of 8.5 km.
- Reconstruction of two bridges in connection with the river improvement.

Alignment, longitudinal profile and typical cross-section are shown in Fig.7.22, Figs. 7.23 to 25 and Figs. 7.31 to 7.33 respectively. Principal features of the design channel are summarized in Table 7.7.

(8) Pantal-Sinocalan River System

This river improvement is composed of the following works:

Pantal-Sinocalan - Tagamusing River

- Channel improvement on the main Pantal-Sinocalan channel over a length of 33.0 km including bank protection works and on the whole stretch of the Tagamusing River, only bank protection works.
- Construction of diking system on both banks of the stretch of the Pantal-Sinocalan River over a total length of 68.0 km including heightening of the 3.8 km existing dike.
- Construction of 16 sluice ways and 6 bridges in connection with the river improvement.

Binalonan Floodway and Tuboy River

- Construction of a 6.7 km long diversion channel with the parallel dike over a total length of 13.4 km.
- Construction of 2 sluice ways and a new bridge at the inlet of the floodway.
- Channel improvement on the Tuboy River over a length of 5.9 km including bank protection works.
- Construction of diking system on the left bank of Tuboy over a length of 5.9 km.

Dagupan-San Juan-Elang River

- Channel improvement on the main Dagupan-San Juan River over a length of 21.7 km and on the downstream of the Elang over a length of 5.9 km including bank protection works.

- Construction of diking system on the above stretches over a total length of 56.0 km.
- Construction of 7 sluice ways and 6 bridges in connection with the river improvement.

Ingalera River

- Channel improvement on the main stream over a length of 37.5 km including bank protection works.
- Construction of a 25.0 km long right dike and a 38.0 km long left dike.
- Construction of 8 sluice ways and 8 bridges in connection with the river improvement.

Mitura-Macalong River

- Channel improvement on the mainstream over a length of 21.0 km including bank protection works.
- Construction of a 19 km long right dike and a 21.0 km long left dike.
- Construction of 8 sluice ways and 3 bridges in connection with the river improvement.

Alignment, longitudinal profile and typical cross-section are shown in Fig. 7.22, Figs. 7.26 to 7.30 and Fig. 7.34 to 7.38 respectively. Principal features of the design channel are summarized in Table 7.7.

7.3 Work quantities and Construction Cost

(1) Agno River

The work quantities and economic cost of the river improvement plan for the Agno River system are summarized below:

| | | Main | Tarlac | Other |
|-------------------|---------------------|--------|--------|-------------|
| Items | Unit | Agno | River | tributaries |
| Excavation | 1,000m ³ | 28,875 | 4,300 | 2,083 |
| Dredging | 1,000m ³ | 17,075 | 0 | . 0 |
| Embankment | 1,000m ³ | 20,370 | 1,355 | 3,370 |
| Revetment | 1,000m ² | 588 | 96 | 190 |
| Groin | Pc. | 958 | 244 | 1,070 |
| Drainage facility | Pc. | 18 | 2 | 26 |
| Bridge | Pc. | 5 | 3 | 14 |
| Fixed weir | Pc. | 1 | 0 | 0 |
| Intake facility | Pc. | 0 | 0 | 4 |
| Main construction | Million | 7,046 | 713 | 1,293 |
| cost | Pesos | | | |
| Total cost | Million | 10,485 | 1,061 | 1,925 |
| | Pesos | · | • | |

The details of the above are shown in Tables 7.1 and 7.3.

(2) Allied Rivers

The work quantities and economic cost of the river improvement and floodway plan for the Cayanga-patalan and Pantal-Sinocalan River systems are summarized belows:

| | | Cayanga-Patalan | Pantal-Sinocalan |
|-------------------|---------------------|-----------------|------------------|
| Items | Unit | River | River |
| Excavation | 1,000m ³ | 2,361 | 5,712 |
| Dredging | 1,000m ³ | 440 | 38 |
| Embankment | 1,000m ³ | 1,773 | 6,515 |
| Revetment | 1,000m ² | 194 | 470 |
| Groin | Pc. | 1,095 | 754 |
| Drainage facility | Pc. | 22 | 41 |

| Bridge | Pc. | 9 | 24 |
|-------------------|---------|-------------|-------|
| Intake facility | Pc. | 0 | 4 |
| Main construction | Million | 837 | 1,715 |
| cost | Pesos | | |
| | | *** | |
| Total cost | Million | 1,246 | 2,553 |
| 2 m | Pesos | to division | |
| 7:1 | | | |

The details of the above are shown in Tables 7.2 and 7.4.

- 8. RIVER IMPROVEMENT AND FLOODWAY PLAN FOR LONG TERM PLAN
- 8.1 Features of River Improvement and Floodway Plan
- 8.1.1 Design Flood Discharge

The design flood discharge distribution for Long Term Plan is shown in Fig. 8.1.

8.1.2 Features of Design Channel and Floodway

Principal feature of the river improvement and floodway plan for Long Term Plan are summarized below.

(1) Main Agno

Lower reaches

- Stretch of river improvement : 43.8 km long

- Channel improvement : 43.8 km long

- Diking system : 43.4 km long on the right bank

: 43.2 km long on the left bank

Right bank : New dike of 5.0 km long

Heightening of 27.4 km long

Existing dike of 11.0 km long

Left bank : New dike of 33.9 km long

Heightening of 8.8 km long

Existing dike of 0.5 km long

- Construction of other : 10 sluice ways

facilities 2 water gates for intake water 4 bridges

Middle reaches

- Length of stretch : 10.7 km long on the Poponto floodway

10.5 km long on the existing main stream

- Channel improvement : 10.7 km long on the floodway

: 1.0 km long on the existing main stream

- Diking system along : 7.8 km long on the right bank

the floodway 4.8 km long on the left bank

Right bank

: New dike of 3.8 km long Heightening of 4.0 km long

Left bank

: Heightening of 4.8 km long

- Diking system along

: 11.7 km long on the right bank

the existing main channel

8.6 km long on the left bank

Right bank

: New dike of 4.3 km long (on the back-water stretch) Existing dike of 7.4 km long

Left bank

: New dike of 4.2 km long

(on the back-water stretch)
Existing dike of 4.4 km long

- Construction of other facilities

: 1 fixed weir with a 3.2 km long dike

2 sluice ways

Upper reaches

- Stretch of river improvement : 44.4 km long

- Channel improvement

: 44 4 km long

- Diking system

: 44.4 km long on the right bank 37.4 km long on the left bank

Right bank

: New dike of 9.8 km long including backward displacement of 2.6 km dike.

Heightening of 27.2 km long
Existing dike of 7.4 km long

Left bank

: New dike of 7.5 km long
Heightening of 26.4 km long
Existing dike of 3.5 km long

- Construction of other facilities

: 4 sluice ways

1 bridge

Alignment, longitudinal profile and typical cross-section of the Agno river improvement plans are shown in Figs. 7.4, 7.9 and 7.15 respectively. Principal features of the design channel and floodway are summarized in Table 8.1.

(2) Tarlac River

- Stretch of river improvement : 37.0 km long including the stretch with the low-water channel improvement only on the retarding area ($L=8.1\ km$)

- Channel improvement

: 37.0 km long

- diking system

: 28.9 km long on the right bank 27.9 km long on the left bank

Right bank

: Heightening of 23.3 km long

Existing dike of 5.6 km long

Left bank

: New dike of 1.3 km long
Heightening of 22.2 km long
Existing dike of 4.4 km long

- Construction of other

: 2 sluice ways

3 bridges

facilities

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Alignment, longitudinal profile and typical cross-section of the Tarlac river improvement plans are shown in Figs. 7.5, 7.10 and 7.16 respectively. Principal features of the design channel are summarized in Table 8.2.

(3) Camiling River

- Stretch of river improvement : 20.0 km long

- Channel improvement

: 18.8 km long including 1.5 km long cut-

off channel

- Diking system

: 15.1 km long on the right bank (new dike)

16.0 km long on the left bank (new dike)

- Construction of other

: 4 sluice ways

facilities

1 bridge

Alignment, longitudinal profile and typical cross-section of the Camiling River improvement plan are shown in Figs. 7.6, 7.11 and 7.17 respectively. Principal features of the design channel are summarized in Table 8.2.

(4) Banila River

- Stretch of river improvement : 30.9 km long

- Channel improvement

: 23.9 km long

- Diking system

: 24.5 km long on the right bank

30.9 km long on the left bank

Right bank

: New dike of 24.5 km long

Left bank

: New dike of 21.6 km long

L Dank . NC

Heightening of 1.7 km long

Existing dike of 7.6 km long

- Construction of other facilities

: 14 sluice ways 7 bridges

Alignment, longitudinal profile and typical cross-section of the Banila River improvement plan are shown in Figs. 7.7, 7.12 and 7.18 respectively. Principal features of the design channel are summarized in Table 8.2.

(5) Viray-Dipalo River

- Stretches of river

: 7.9 km long on the Viray-Dipalo

improvement

: 4.5 km long on the Viray : 7.7 km long on the Dipalo

- Channel improvement

: The above whole stretch over a total

length of 20.1 km

- Diking system

: 13.0 km long on both banks of the Viray-Dipalo

4.1 km long on the right bank of the Viray

7.1 km long on the left bank of the Dipalo

Right bank

: Heightening of 3.6 km long

Existing dike 7.0 km long

Left bank

: Heightening of 3.6 km long

Existing dike 10.0 km long

- Construction of other

: 4 sluice ways

facilities

4 bridges

Alignment, longitudinal profile and typical cross-section of the Viray-Dipalo River improvement plans are shown in Figs. 7.8, 7.13 and 7.19 respectively. Principal features of the design channel are summarized in Table 8.2.

(6) Ambayaoan River

- Stretch of river improvement : 8.7 km long

- Channel improvement

: 8.7 km long

- Diking system

: 8.0 km long on the right bank

8.0 km long on the left bank

Right bank

: New dike of 8.0 km long

Left bank

: New dike of 4.5 km long

Heightening of 3.5 km long

- Construction of other

: 4 sluice ways

facilities

1 bridge

Alignment, longitudinal profile and typical cross-section of the Ambayaoan River improvement plans are shown in Figs. 7.8, 7.14 and 7.20 respectively. Principal features of the design channel are summarized in Table 8.2.

(7) Cayanga-Patalan-Angalacan River system

Cayanga-Patalan-Angalacan River

- Stretch of river improvement : 37.5 km long

- Channel improvement : 37.5 km long

- Diking system (new dike) : 37.5 km long right dike

32.6 km long left dike

- Construction of other

: 16 sluice ways

facilities

6 bridges

Bued River

- Stretch of river improvement : 19.8 km long

- Channel improvement : 15.1 km long

- Diking system (new dike) : 14.1 km long right dike

7.0 km long left dike

including a 2.0 km closure dike

- Construction of other

: 6 sluice ways

facilities

1 bridge

Aloragat River

- Stretch of river improvement : 19.7 km long

- Channel improvement : 19.7 km long

- Diking system (new dike) : 4.3 km long right dike

4.2 km long left dike

- Construction of other: 2 bridges

facilities

Alignment, longitudinal profile and typical cross-section of the river improvement plans for the Cayanga River system are shown in Fig. 7.22, Figs. 7.23, 7.25 and Figs. 7.31 to 7.33 respectively. Principal features of the design channel are summarized in Table 8.3.

(8) Pantal-Sinocalan-Tagamusing River system

Pantal-Sinocalan-Tagamusing River

- Stretch of river improvement: 43.5 km long

- Channel improvement : 33.0 km long

- Diking system : 34.0 km long right dike

34.0 km long left dike

Right bank : New dike of 31.5 km long

Heightening of 2.5 km long

Left bank : New dike of 32.7 km long

Heightening of 1.3 km long

- Construction of other : 16 sluice ways

facilities 6 bridge

Dagupan-San Juan-Elang River

- Stretch of river improvement : 27.6 km long

- Channel improvement : 27.6 km long

- Diking system (new dike) : 27.6 km long right dike

27.6 km long left dike

- Construction of other : 7 sluice ways

facilities 6 bridges

Ingalera River

- Stretch of river improvement : 37.5 km long

- Channel improvement : 37.5 km long

- Diking system (new dike) : 21.0 km long right dike

34.0 km long left dike

- Construction of other : 8 sluice ways

facilities 8 bridges

Mitura-Macalong River

- Stretch of river improvement : 21.0 km long

- Channel improvement : 21.0 km long

- Diking system (new dike) : 15 km long right dike

17 km long left dike

- Construction of other : 8 sluice ways

facilities : 2 bridges

Alignment, longitudinal profile and typical cross-section are shown in Fig. 7.22, Figs. 7.26 to 7.30 and Figs. 7.34 to 7.38 respectively. Principal features of the design channel are summarized in Table 8.3.

8.2 Work Quantities and Construction Cost

(1) Agno River

The work quantities and financial cost of river improvement plan for the Agno River system are summarized below:

| Items | Unit | Main Agno | Tarlac River | Other Tributaries |
|-------------------|---------------------|--------------|-----------------|----------------------|
| | 3 | | · | |
| Excavation | 1,000m ³ | 24,673 | 4,300 | 1,200 |
| Dredging | 1,000m ³ | 13,027 | 0 | 0 |
| Embankment | 1,000m ³ | 15,269 | 1,355 | 2,581 |
| Revetment | 1,000m ² | 514 | 96 | 190 |
| Groin | Pc. | 958 | 244 | 1,070 |
| Drainage facility | Pc. | 18 | 2 | . 26 |
| Bridge | Pc. | 5 | 3 | 14 |
| Fixed weir | Pc. | . 1 | 0 | 0 |
| Intake facility | Pc. | 0 | . 0 | 4 |
| Foreign currency | Million Pesos | 6,047 | 903 | 937 |
| Local currency | Million Pesos | 3,338 | 518 | 703 |
| Total cost | Million Pesos | 9,385 | 1,421 | 1,640 |

The details of the above rivers are shown in Tables 8.4, and 8.6.

(2) Allied Rivers

The work quantities and economic cost of the river improvement and floodway plan for the Cayanga-Patalan and Pantal-Sinocalan River systems are summarized below:

| en e | Ca | yanga-Patalan | Pantal-Sinocalan |
|--|---------------------|--------------------|------------------|
| Items | Unit | River | River |
| Excavation | 1,000m ³ | 1,842 | 4,216 |
| Dredging | 1,000m ³ | 260 | 38 |
| Embankment | 1,000m ³ | 718 | 4,012 |
| Revetment | 1,000m ³ | 193 | 373 |
| Groin | Pc. | 1,095 | 952 |
| Drainage facility | Pc. | *: 22 : * * | 39 |
| Bridge | Pc. | . 9 | 22 |
| Intake facility | | 0 | |
| | | | |
| Foreign currency | Million Peso | s 615 | 1,311 |
| Local currency | Million Peso | s 511 | 849 |
| | \$ 4 | | |
| Total cost | Million Peso | s 1,126 | 2,160 |

The details of the above rivers are shown in Tables 8.5 and 8.7.

9. REVIEW OF AGNO RIVER IMPROVEMENT PLAN FOR FRAMEWORK AND LONG TERM PLANS

9.1 Review of River Improvement Plan Required

The Master Plan Study was executed in the period, May 1989 - February 1990. After this study the following inspection and survey were conducted; 1) Inspection of earthquake damages which occurred on July 16, 1990 and 2) Additional topographic and cross-section survey for Feasibility Study area selected.

In accordance with the assessment of results of the above inspection and additional survey, the following study for review of Master Plan is required;

- Review of the selection of Feasibility Study area in consideration of the earthquake-damaged river facilities
- Review of the storage function of Poponto natural retarding basin based on the new topographic map
- Review of the probable flood discharge in the downstream reach of Agno River and revision of that design flood discharge distribution
- Review of the comparative study for alternatives of Agno flood control plan based on the revised probable flood discharge.

9.2 Proposed River Improvement Study Stretches for Feasibility Study

Among the earthquake-damaged river facilities the most serious damages have been identified over the diking system of the middle and upper reaches of the main Agno River and the whole reaches of the Tarlac River. In the Allied Rivers the damaged river facilities were bank protection ones. The extent of dike damaged in the Agno River Basin in summarized as follows:

| River/Stretch (Right/Left Bank) | River Length (km) | Length of Damage Extent Earth dike Conc. di Revetme | | | |
|------------------------------------|-------------------------|---|------|--|--|
| Main Agno River | | | | | |
| River mouth - Wawa(R) | 45.0 | 2.32 | 0.11 | | |
| (L) | | 0 | 0.99 | | |
| Wawa - ARIS dam (R) | 54.0 | 16.60 | 0.20 | | |
| (L) | | 19.56 | 0.99 | | |
| Poponto Floodway (R/L) | 6.0 | 1.31 | 0 | | |
| Tarlac River (R/L) | 37.0 | 44.92 | 0.50 | | |
| Camiling River (R/L) | 20.0 | 0 | 3.74 | | |
| Banila River (R/L) | 30.9 | 2.12 | : 0 | | |
| Viray-Depalo River(R/L) | 20.1 | 1.30 | 0 | | |
| Ambayoan River (R/L) | 8.7 | 0 | .0 | | |

PMO-AFCS Flood Control Program for Earthquake-Damage Rehabilitation/ Restoration was formulated in order to rehabilitate all the damaged flood control facilities, especially dikes to the original conditions before the flood season of 1991.

Therefore, the Feasibility Study areas selected in the Master Plan Study are adopted unchanged; i.e., the Upper Agno River and the Pantal-Sinocalan River.

9.3 Revision of the Probable Flood Discharge in the Downstream of Main Agno River

The storage function of the Poponto natural retarding basin was reviewed by using the new topographic map with scale of 1/25,000 and the revised design high water level is as follows:

- Framework Plan (100-year): H.W.L = 16.70
- Long Term Plan (25-year) : H.W.L = 16.00

The revised design flood discharge distribution of Framework and Long Term Plans are shown in Figs. 9.1 and 9.2 respectively.

9.4 Review of the Comparative Study of Alternatives for Agno Flood Control
Plan

Based on the revised probable flood discharge, the comparative study on least costly alternative of Agno River were reviewed, and the result is described in the Main Report.

Of the four alternatives for the Agno River, the combination plan of river improvement, the Poponto natural retarding basin and the Moriones-O'Donnel dam which was selected originally as the Framework Plan, is reassessed to be the case of least project cost.

9.5 Modification in Agno River Improvement Plan for Framework and Long Term Plans

The features of the modified part of the design channel of Agno River Improvement Plan for Framework and Long Term Plans are summarized in Tables 9.1 and 9.2 respectively.

The modified parts are as bellow:

- In the downstream reach from the Poponto natural retarding basin, the design high water level and the design elevation of dike crown are heightened due to increase of the design flood discharge.
- In the upstream reach, the followings are modified;
 - 1) The design high water level in the back-water stretch in Bayambang due to the revised design high water level of the Poponto natural retarding basin.
 - 2) The design river width of the Poponto floodway (Set-back levee plan proposed) in consideration of the channel and dike stabilization.
 - 3) The design dike alignment from Asingan to San Manuel in consideration of dike stabilization.

Figs. 9.3 and 9.4 show the modified design plan which covers the upper stretch of the Poponto natural retarding basin, and the revised design longitudinal profile in the whole study area.

TABLES

Table 2.1 PHYSICAL FEATURES OF RIVER BASIN

| RIVER | CATCHMENT AREA (km2) | RIVER LENGTH (km) |
|-------------------------|---|---------------------------------------|
| | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | |
| AGNO RIVER BASIN | | |
| AGNO RIVER | | |
| Ambuklao Dam | 617 | 58.5 |
| Binga Dam | 860 | 73.5 |
| San Roque Dam | 1,250 | 119.0 |
| Poponto Floodway Site | 2,477 | 165.0 |
| River Mouth (Whole) | 5,907 | 221.0 |
| TARLAC RIVER | ~, ~· | second of |
| O'Donnell River | 303 | 50.3 |
| Moriones River | 558 | 64.8 |
| (Whole) | 1,896 | 93.0 |
| AMBAYOAN RIVER | 367 | 62.2 |
| VIRAY-DIPALO RIVER | | |
| Viray River | 70 | 8.0 |
| Dipalo River | 65 | 14.0 |
| (Whole) | 135 | 21.2 |
| BANILA RIVER | 309 | 39.0 |
| CAMILING RIVER | 604 | 64.0 |
| OLO RIVER | 140 | 35.7 |
| BAYAOAS RIVER | 72 | 21.2 |
| | | e e e e e e e e e e e e e e e e e e e |
| ALLIED RIVER BASIN | | . 4 . |
| | | |
| CAYANGA-PATALAN RIVER | | |
| (Whole) | 618 | 61.0 |
| Angalacan River | <u>1</u> 44 | 25.5 |
| Aloragat River | 116 | 31.0 |
| Bued River | 286 | 54.0 |
| PANTO-SINOCALAN RIVER | | |
| (Whole) | 1,115 | 75.5 |
| Mitura-Macalalong River | 141 | 31.0 |
| Tagumising River | 182 | 44.5 |
| Ingalera River | 197 | 32.5 |
| Dagupan River | 273 | 32.0 |

Table 2.2 SUNMARY OF BREACHES/GAPS AND SCOURS CAUSED BY FLOODS (1984-1988)

| RIVER | DAMAGE | | | -FLOOD | | 5-F1.00D | | -FLOOD | |
|--------------|--------------------------|------------------------|------|-------------|---------|-----------------|---|---|--|
| (Reaches) | TYPE | Aug. 28-30 | | | e 22-24 | | July 9-11 | | |
| | | | ("Ma | ring") | | uring") | | ading") | |
| • " | , | | | | | e 28-30 | - | .30- | |
| | | | | | ("D | aling") | | t. 5 | |
| | | • | | | | | ("M | iding") | |
| 1000 07000 | PANTANTAE (DEUFT | (624) | 0 | 0.760 | 0 | 2 200 | 10 | 5,340 | |
| AGNO RIVER | EARTHDIKE/REVET. | (Site, m) | 8 | 2,760 | . 8 | 2,290 17,888 | 10 | 55,402 | |
| (Upper) | BREACHES/GAPS SCOURED | (P:1,000) (Site, m) | 0 | 52,972 0 | 0 | 17,000 | 3 | 2,90 | |
| | 2000KED | (P:1,000) | v | 0 | | 0 | J | 2,750 | |
| | DAMAGED SPURDIKE | (Site, Unit) | 2 | 56 | 0 | 0 | 7 | 18 | |
| ŧ, | DAMAGED SLOUDING | (P:1,000) | ۲. | 1,400 | · | 0 | | 3,81 | |
| | | (111,000) | | 11-100 | | • | | | |
| | | | | | • | | v.** | | |
| AGNO RIVER | EARTHDIKE/REVET. | (Site, m) | 1 | 110 | 0 | 0 | 1 | 50 | |
| (Lower) | BREACHES/GAPS | (P:1,000) | | 20,000 | , | 0 | 1. | 12,000 | |
| • | SCOURED | (Site, m) | 0 | 0 | 0 | 0 | 0 | • | |
| | • | (P:1,000) | | 0 | | 0 | | (| |
| | DAMAGED SPURDIKE | (Site, Unit) | 0 | 0 | 0 | 0 | 1 | 4 | |
| | | (P:1,000) | | 0 | | 0 | | 8,12 | |
| | | | | | | ٠ | | | |
| TARLAC RIVER | EARTHDIKE/REVET. | (Site, m) | 0 | 0 | 3 | 70 | 5 | 49 | |
| | BREACHES/GAPS | (P:1,000) | | 0 | | 660 | | 5,84 | |
| | SCOURED | (Site, m) | 0 | 0 | 0 | 0 | 0 | 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | |
| | | (P:1,000) | | 0 | | 0 | | (| |
| | DAMAGED SPURDIKE | (Site, Unit) | 0 | 0 | 0 | 0 | 3 | 40 | |
| | | (P:1,000) | | 0 | | 0 | | 1,28 | |
| | • | | | | | | | | |
| RIBUTARIES | EARTHDIKE/REVET. | (Site, m) | 0 | . 0 | . 3 | 140 | . * * 7 | 1,08 | |
| OF AGNO | BREACHES/GAPS | (P:1,000) | | 0 | | 1,000 | | 8,30 | |
| RIVER | SCOURED | (Site, m) | 0 | 0 | 0 | 0 | 0 | | |
| | | (P:1,000) | | 0 | | 0 | $(x,y)\in \mathbb{R}^{\frac{1}{2}} \times \mathbb{R}^{2}$ | | |
| | DAMAGED SPURDIKE | (Site, Unit) | 0 | 0 | 0 | 0 | 8 | 61 | |
| | | (P:1,000) | | 0 | | 0 | | 1.74 | |
| • | | • .• | • | | | | | | |
| TFIED | EARTHOIKE/REVET. | (Site, m) | 4 | 976 | 1 | 40 | 9 | 1,82 | |
| RIVER | BREACHES/GAPS | (P:1,000) | • | 8,166 | • | 231 | ertige | 8,76 | |
| 10 70.01 | SCOURED | (Site, m) | 0 | 0.100 | 0 | 0 | 0 | 0,70 | |
| | COUNTR | (P:1,000) | • | 0 | | . 0 | • | | |
| | DAMAGED SPURDIKE | (Site, Unit) | 0 | .0 | 2 | 10 | 3 | 48 | |
| | PIRRIORD OF OURTHE | (P:1,000) | , | 0 | | 576 | J | 997 | |
| | | | | • | | | · | | |
| | | (P:1,000) | | | | | | | |

SOURCE: AGNO FLOOD CONTROL SYSTEM OFFICE, Rosales

Table 2.3 EXISTING RIVER CONTROL FACILITIES

| PROJECT | EARTH- DIKE | REVET./ GRAVITY | CUT-OFF CHANNEL /RIVER IMPV'T. | REMARKS (Construction of |
|---|----------------|--------------------|---|-----------------------------|
| | (km) | WALL (km) | (1968-88) (km) | Earthdike, 1968-88) (km) |
| 1. AGNO RIVER CONTROL PROJECT | | | *************************************** | |
| (UPPER REACHES) Bayambang-Baby Dike Section | 0.98 (R) | | | 0.98 |
| Bayambang-Villasis Earthdike Section | 18.37 (R) | 1.89 | • | 0.90 |
| Villasis-Asingan Earthdike Section | 12.00 (R) | 0.40 | : - | 12.00 |
| Asingan-Sn. Manuel Earthdike Section | 12.00 (R) | 2.20 | - . | 7.50 |
| Anulid-Bautista Earthdike Section | 7 7 | - | - | 5.80 |
| · · · · · · · · · · · · · · · · · · · | 5.80 (L) | 1 | 1 02 bm/Cn 111n) | 4.7 |
| Anulid-Poponto Earthdike/Flood Way | 4.67 (L) | - | 1.02 km(Spillway) | 4.67 |
| Al a grant bite Carlin | 6.03 (R) | - | 10.85 | 6.03 |
| Alcala-Sto. Tomas Dike Section | 7.20 (L) | 4.73 | 0.99 | 1.02 |
| Rosales-Lagasit Dike Section | 4.00 (L) | | - | |
| Lagasit-Sta. Maria Section | 12.06 (L) | - | F | |
| Sta. Maria-Tayug Section (LOWER REACHES) | - (L) | - | • | |
| Lingayen-Urbiztondo-Bayambang | 40.50 (R) | _ | - | 8.50 |
| Bugallon-Labrador Earth Section | 0.53 (L) | | - ' | 0.53 |
| Bugallon-Aguilar Earthdike Section | 11.35 (L) | | . · | 11.35 |
| Cupang Parallel Earthdike Section | 2.05 (L) | - | . . | 2.05 |
| | 1.84 (R) | - | • | 1.84 |
| Sobol Parallel Earthdike Section | 1.60 (L) | - | · <u>-</u> | 1.60 |
| | 1.80 (R) | - | | 1.80 |
| 2. AMBAYOAN RIVER CONTROL PROJECT | 2.50 (L) | _ | • | 2.50 |
| 3. VIRAY-DIPALO RIVER CONTROL PROJECT | 14.44 (L) | - | 8.80 | 14.44 |
| en e | 9.00 (R) | | - | 9.00 |
| 4. TOTONOGEN RIVER CONTROL PROJECT | 2.50(L+R) | _ | • | 2.50 |
| 5. BANILA RIVER CONTROL PROJECT | 9.33 (L) | - | - - | 9.33 |
| 6. TARLAC RIVER CONTROL PROJECT | 22.54 (L) | 3.00 | | 22.54 |
| or marity marity sommer morest | 25.00 (R) | 4.62 | · _ | : - |
| 7. O'DONNELL RIVER CONTROL PROJECT | 9.33 (R) | - | | 9.33 |
| 8. MORIONES RIVER CONTROL PROJECT | 5.55 (it) | | _ | - |
| 9. BATACAN RIVER CONTROL PROJECT | | 0.30 | | ** |
| O. OLO RIVER CONTROL PROJECT | 3.45 | | 프로프 - | 3.45 |
| 1. BAYAOAS RIVER CONTROL PROJECT | 3173 | | | - |
| | 0.85 | _ | 0.84 | - |
| 2. BEI RIVER CONTROL PROJECT | 0.05 | - | - | 0.85 |
| 3. PANTAL RIVER CONTROL PROJECT | - . | 1.10 (R) | ~ | 4.03 |
| 4. CAYANGA RIVER CONTROL PROJECT | | 0.80 (R) | · | - |
| E DUED DIVED CONTOOL DOOLES | | | | |
| 5. BUED RIVER CONTROL PROJECT | . - | 3.69 (R) | • | - |
| 6. ANGALACAN RIVER CONTROL PROJECT | | - | - | - |
| 7. ALORAGAT RIVER CONTROL PROJECT | - | - | | - |
| 8. PANTO RIVER CONTROL PROJECT | 0 50 (0) | - | - · | - 3 PA |
| | 2.50 (R) | - | 1.60 | 2.50 |
| O. SINOCALAN RIVER CONTROL PROJECT | 1.26 | _ | · - | 1.26 |
| 1. TAGAMUSING RIVER CONTROL PROJECT | .= | 0.78(L+R | | - |
| 2. MITURA RIVER CONTROL PROJECT | - | 0.22 (R) | | - |

Note : R : Right Bank, L : Left Bank

Table 3.1 (1/2) LIST OF COMPLETED RIVER CONTROL PROJECTS 1968-1988 (EARTHDIKE/CUT-OFF CHANNEL/RIVER IMPROVEMENT WORKS)

| | PROJECT | LOCATION | YEAR | REMARKS |
|----|---|--------------------------|---------|--|
| 1 | STO.TOMAS EARTHDIKE | Sta. 3.000-10.662 | 1968 | |
| | | Sta. 9.550- 9.745 | 1973 | (Raising) |
| | into a contract of the contrac | Sta. 10.380-11.360 | 1973 | (-ditto-) |
| 2 | ASINGAN-SAN MANUEL EARTHDIKE | Sta. 18.200-23.220 | 1978 | |
| | (SETBACK LEVEE) | Sta. 23.220-25.700 | 1982 | |
| 3 | VILLASIS-ASINGAN SETBANK | Sta. 0.000-12.000 | 1973/74 | |
| 4 | BAYAMBANG BABY DIKE | Sta. 19.400-20.383 | 1980/81 | |
| 5 | STA.MARIA-TAYUG EARTHDIKE | Sta. 0.000-12.000 | 1975/76 | et e e |
| 6 | ALCALA EARTHDIKE | Sta. 10.662-11.680 | 1972 | |
| 7 | ALCALA CUT-OFF CHANNEL | Sta. 0.000- 0.992 | 1981 | |
| 8 | POPONT SWANP FLOODWAY(SPILLWAY) | Sta. 0.000-1.020 | 1977 | The second of the A |
| | ANULID-POPONT PILOT CHANNEL | Sta. 0.000- 7.000 | 1978 | 15 15 |
| 10 | ANULID-POPONT RIGHT EARTHDIKE | Sta. 0.000- 4.780 | 1975/76 | • |
| | $(-\infty,+\infty) = (-\infty,+\infty) = (-\infty,$ | Sta. 4.780- 5.840 | 1983 | server of the con- |
| | | Sta, 5.840-6.025 | 1984 | tian and the first t |
| 11 | ANULID-POPONT LEFT EARTHDIKE | Sta. 11.000-13.500 | 1975/76 | All Control of the Control |
| | | Sta. 13.500-15.390 | 1978/79 | general de la company |
| | | Sta. 15.390-15.674 | 1988 | As the Bill Ville |
| 12 | ANULID-BAUTISTA EARTHDIKE | Sta. 0.000-4.920 | 1975/76 | Martin Desire |
| | | Sta. 5.120- 5.800 | 1978 | tion at employed |
| 13 | ROSARIO-LINGAYEN EARTHDIKE | Sta. 32.000-40.500 | 1973/74 | The state of the pro- |
| | AGUILAR-BUGALLON EARTHDIKE | Sta. 0.000-11.355 | 1976/77 | |
| | SOBOL PARALLEL EARTHDIKE | Sta. 0.000-1.600(L) | 1979 | |
| | | Sta. 0.000-1.800(R) | 1979 | 200 |
| 16 | CUPANG PARALLEL EARTHDIKE | Sta. 0.000- 2.050 | 1980/81 | |
| | | Sta. 0.000-1.840 | 1980/81 | e eta la production |
| 17 | BUGALLON-LABRADOR EARTHDIKE | Sta. 11.500-12.025 | 1979/80 | |
| | AMBAYOAN EARTHDIKE | Sta. 3.000- 3.400 | 1977/78 | |
| | VIRAY-DEPALO EARTHDIKE | Sta.(-)3.000-6.000(R) | 1975/76 | Section 1 |
| | | Sta.(-)2.840-6.930 | 1975/76 | the second second |
| 20 | VIRAY-DEPALO PILOT CHANNEL | Sta. 0.000- 6.800 | 1980 | $(m_{ij}, m_{ij}, m_{$ |
| | SN.QUINTIN EARTHDIKE(DEPALO R.) | Sta.(-)3.340-(-)8.620(L) | 1979/80 | |
| | SN.QUINTIN PILOT CHANNEL | Sta.(-)3.340-(-)5.340 | 1980 | $((x_0)^{k}(t)^{k}) = ((t_0)^{k}(t_0)^{k})$ |

SOURCE: YEAR-END REPORTS AND/OR LIST OF COMPLETED PROJECTS, AFCS OFFICE

Table 3.1 (2/2) LIST OF COMPLETED RIVER CONTROL PROJECTS 1968-1988 (EARTHDIXE/CUT-OFF CHANNEL/RIVER IMPROVEMENT WORKS)

| PROJECT | LOCATION | YEAR | REMARKS |
|--|-------------------------------|--------------|--|
| 23 TOTONOGEN EARTHDIKE | Sta. 0.000-1.150 | 1988 | ************************************** |
| • | Sta. 0.000-1.350 | 1988 | |
| | Sta. 10.380-11.360 | 1973 | |
| 24 BANILA EARTHDIKE | Sta. 0.000- 9.332 | 1977/78 | |
| 25 TARLAC LEFT DIKE | Sta. 0.000-1.350 | 1970 | |
| | Sta. 1.350- 1.780 | 1972/73 | |
| | Sta.(-)0.000-(-)2.000 | - | |
| | Sta. 1.780- 5.000 | · - - | |
| • | Sta. 5.000- 8.000 | 1974 | |
| Sta.4.260- | 5.240/5.380-5.440/6.320-8.000 | 1982 | (Raising) |
| | Sta. 8.000- 9.780 | 1982/83 | |
| | Sta. 9.000-19.000 | 1988 | |
| | Sta.(~)2.000-(~)3.355 | 1988 | |
| 25 ARMENIA EARTHDIKE(O'DONNELL R.) | Sta. 0.000- 5.530 | 1974 | |
| | Sta. 5.530- 9.300 | 1988 | |
| 26 SAPANG PILOT CHANNEL | Sta. 0.000- 4.000 | 1977/78 | |
| 27 CALAPAN CUT-OFF CHANNEL | Sta. 0.000-1.680 | 1979/80 | · |
| 28 TABLANG-BACAO R.I. | Sta. 0.000- 4.000 | 1979/80 | . * |
| 29 CAMANGAAN-CALAPAN R.I. | Sta. 0.000- 5.080 | 1979/80 | • |
| 30 CUT-OFF CHANNEL | Sta. 0.000- 0.560 | 1979/80 | 1 1 |
| 31 OLO RIVER EARTHDIKE | Sta. 1.380- 2.160 | 1981 | |
| | Sta. 0.000-1.380 | 1982 | (Raising) |
| | Sta. 1.160- 2.640 | 1981 | |
| The state of the s | Sta. 2.640- 3.450 | 1982 | |
| 32 BEI EARTHOIKE | Sta. 0.800- 1.650 | 1984/85 | |
| 33 BEI CUT-OFF CHANNEL | Sta. 0.420- 0.861 | 1988 | 100 |
| 34 SOBOL CREEK EARTHDIKE | (L=2,170m) | 1979 | • |
| 35 QUIBAOL CUT OFF CHANNEL | Sta. 0.000- 1.750 | 1979/80 | 1.5 |
| 36 MARUSAY EARTHDIKE | Sta. 0.000- 1.260 | 1983 | |
| (STA.BARBARA-CALASIAO) | Sta. 1.260- 1.360 | 1984 | |
| 37 MARUSAY CUT-OFF CHANNEL | Sta. 4.740- 5.160 | 1982 | |
| (CALASIAO) | Sta. 5.184- 5.470 | 1982 | |
| | Sta. 7.620- 8.230 | 1983 | |
| 38 SINOCALAN EARTHDIKE | Sta. 0.000- 1.260 | 1983 | |
| | Sta. 0.000- 0.400 | 1986 | • |
| | Sta. 0.400- 1.100 | 1988 | |

SOURCE : YEAR-END REPORTS AND/OR LIST OF COMPLETED PROJECTS, AFCS OFFICE

Table 3.2 SUMMARY OF ACCOMPLISHMENT OF RIVER CONTROL WORKS (1972-1988)

| YEAR | EARTH DIKE | W/BOULDER FACING/ APRON | REVETMENT GRAVITY WALL | BOULDER Spur Dike | BOULDER DIKE | GRAVEL SUR- FACING | CUT-OFF CHANNEL | DRAINAGE GATE | SPILLWAY | TOTAL COST |
|-------|---------------|-------------------------------|---------------------------------------|-------------------------|-----------------|--------------------------|--------------------|----------------------------|---------------------------------------|---------------|
| | (m) | (m) | (m) | (TINU) | (m) | (m) | (m) | (UNIT) | (m) | (P) |
| 1972 | 3,234 | 0 | 225 | 3 | | 15,000 | 0 | 0 | 0 | 305,956 |
| 1973 | 3,322 | 0 | * * * * * * * * * * * * * * * * * * * | 155 | 0 | 0 | , | 0 | 0 | 12,186,174 |
| 1974 | 28,855 | 0 | 3,852 | 0 | 0 | 1,452 | 7,000 | 1 | 0 | 4,211,700 |
| 1975 | 69,915 | 0 | 2,765 | 1 | 0 | 10,248 | 460 | 2 . 2 | s 0 | 22,006,479 |
| 1976 | 12,015 | 2,050 | 3,100 | 83 | 0 | 12,142 | . 0 | 1 | 0 | 16,043,439 |
| 1977 | 12,607 | 0 | 75 × 1 = 75 | 26 | 0 | 0 | 3,500 | 1 | 1,020 | 19,157,660 |
| 1978 | 25,295 | 0 | 116 | 0 | 180 | 65,816 | 8,100 | 1 | 0 | 23,651,927 |
| 1979 | 20,631 | . 0 | 0 | 29 | 720 | 0 | 0 | 4 | 0 | 9,099,154 |
| 1980 | 6,993 | 1,160 | 1,999 | 30 | 0 | 4,250 | 8,740 | . 0 | 0 | 8,610,694 |
| 1981 | 10,805 | 0 | 843 | 0 | 0 | 0 | 992 | 0 | 0 | 8,492,994 |
| 1982 | 5,686 | 0 | 1,176 | 85 | 0 | 32,887 | 12,021 | 2 | 6 | 12,104,140 |
| 1983 | 10,484 | 0 | 856 | 138 | 0 | 280 | 9,599 | 0 | 0 | 15,859,494 |
| 1984 | 2,091 | - | 441 | 52 | · - | 18,788 | 10,000 | : · · · · · · - | 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | 10,603,756 |
| 1985 | 1,304 | 194 | 2,458 | 64 | 2 | 0 | 7,426 | 0 | 0 | 11,583,600 |
| 1986 | 3,965 | 1,524 | 922 | 120 | 759 | 48,395 | 600 | 2 | 0 | 22,252,572 |
| 1987 | 7,750 | 3,633 | 1,910 | 182 | 686 | 1 : · · · <u>-</u> | 4,703 | 2 | 0 | 33,933,000 |
| 1988 | 4,235 | 9,492 | 2,347 | 387 | 793 | 0 | 5,896 | 0 | 0 | 40,763,288 |
| TOTAL | 229,187 | 18,053 | 23,085 | 1,355 | 3,138 | 209,258 | 79,037 | 16 | 1,020 | 270,866,027 |

SOURCE: AGNO FLOOD CONTROL SYSTEM OFFICE, Rosales

Table 3.3(1/2) FIVE-YEAR INFRASTRUCTURE PROGRAM OF AGNO FLOOD CONTROL SYSTEM

| | PROJECT LOCATION | SCOPE OF WORK AND PHYSICAL TARGET | | FUNDIN | G REQU | IREMEN | TS (10 | ^6P) '93 |
|--------|--|---|------------------------------------|--------|--------|--------|---------|-------------|
| | | | | | , | | | |
| | RIVER CONTROL PROJECT | | | | | | | |
| 1. | Asingan-Sn. Manuel | Restoration of damaged dike Spur dike, apron | Const./Impv't./ Rehab.ON-GOING | 4.75 | 5.00 | 5.40 | 5.60 | 5.90 |
| 2. | Villasis | Spur dike, Revetment | -ditto- | 1.20 | 1.30 | 1.90 | 1.20 | 1.50 |
| 3. | Sto. Tomas | Restoration of boulder dike | -ditto- | 1.20 | 1.20 | 2.10 | 2.40 | 2.50 |
| | | Bank protection, Spur dike | -ditto- | 2.70 | 2.00 | 2.60 | 1.00 | 0.60 |
| 4. | Alcala | Restoration of damaged dike Bank protection, Spur dike | -ditto- | 2.70 | 2.00 | 2.60 | 1.00 | 0.60 |
| 5. | Urbiztondo | Spur dike. Concrete revetment. River improvement | -ditto- | 2.70 | 2.80 | 3.30 | 0.60 | 2.20 |
| . 6 | Brgy. Sanchez, Asingan | | -ditto- | 1.00 | 1.00 | 1.10 | 1.30 | 0.90 |
| | Alcala - Bayambang | Earthdike, Spur dike, | -ditto- | 0.90 | - | | 1.30 | |
| | | Bank protection | | | | | | |
| 8. | Brgy. Daraoey. Bayambang | Spur dike, Drainage system, Bank protection | -ditto- | 1.20 | - | - | 1.30 | 1.30 |
| 9. | Brgy, Quibad, | Cut-off channel (COC), | Const./Impv't./ | 1.40 | 1.40 | - | ' | 1.50 |
| | = | Spur dike | Rehab. NEW PROJECT | | | | | |
| | Brgy. Quibad, Lingayen | Spur dike, Channel impv't. | Const./Impv't./ Rehab. ON-GOING | 2.90 | 2.00 | 2.00 | 2.30 | 2.30 |
| 11 | 7 3 | Bank protection | | | 2.00 | _ | | 2.00 |
| | San Carlos City | COC, Spur dike | | 1.00 | | | | |
| | | Protec. of damaged dike with | | | | 1.30 | | - |
| 13. | kusa les-sta. Maria | · · · · · · · · · · · · · · · · · · · | -ditto- | 1.20 | 1.30 | 1.30 | 1.40 | - |
| | mark a | boulder spur dike | | ۰۸ ۵۰ | 0 50 | 0.40 | 0.40 | ۰. |
| | the state of the s | Spur dike | -ditto- | | | 0.40 | | |
| | Sto. Tomas-Alcala | Spur dike, COC | | 1.20 | | | | |
| | Bautista | COC, Spur dike, Flood gate | -ditto- | | | 2.20 | | |
| | Buga I lon | Earthdike, COC, Flood gate | -ditto- | | | 1.20 | | |
| | - | Earthdike, Spur dike, Flood gate | | 1.20 | | | 1.40 | • |
| | Sta. Maria | Spur dike, Channel impv't. | -ditto- | - | 1.20 | | 1.00 | |
| | | Spur dike, Channel impv't. | -ditto- | - | 1.00 | | 1.00 | |
| | Aguilar | | -ditto- | 0.50 | 1.20 | | 1.00 | |
| 22. | Brgy. Domalanoan, | COC, Spur dike | -ditto- | - | 0.80 | 1.50 | | 1.80 |
| 1000 | Lingayen | | | | | • | | |
| AMBAY | YOAN R.C.P. | | | | | | | |
| | | Spur dike, Channel imprv't. | Const./Impv't./ | 1.00 | 1,00 | 1.00 | 1,20 | 1.20 |
| | | | Rehab. ON-GOING | | | | | |
| VADAY. | Y-DIPALO R.C.P. | | the second second section | | | | | ٠. |
| | Tayug-Natividad | Restoration of damaged dike | -ditto- | 1.20 | 1.20 | 1.40 | 1.40 | 1.50 |
| 25. | San Quintin | Spur dike, Channel impv't. Rest. of damaged dike with | -ditto- | 1.20 | 1.20 | 1.20 | 1.40 | 1.50 |
| | | Spur dike | | | | | | : |
| TOTO | NOGEN R.C.P | | | ٠. | | | e e | |
| 26. | Rosales | Revetment, Spur dike | -ditto- | - | 0.90 | 0.90 | 0.90 | 1.00 |
| BANTI | A R.C.P. | | The transfer of the second | | | | | |
| | Umingan | Spur dike, Channel impv't. | -ditto- | 0.50 | 0.40 | 0.50 | 0.60 | 0.70 |
| | Ba lungao | Bank protection, Spur dike, | -ditto- | | | 1.00 | | |
| 20. | កក rangas | River impv't., Earthdike | -41 C LV* | 1.00 | 1.00 | 7.00 | 1.00 | 1.00 |
| ٠. | | Wines limbs co. equalification | | | | | | • . |

Table 3.3(2/2) FIVE-YEAR INFRASTRUCTURE PROGRAM OF AGNO FLOOD CONTROL SYSTEM

| PROJECT LOCATION | SCOPE OF WORK AND PHYSICAL TARGET | SUTATS | FUNDIN '89 | G REQU | | • |)^6P) '93 |
|--|---|--|------------------|--------|----------------|------------------------|---------------------------------------|
| ng mag-api gapi ngi pili fidi ber mag api ma mag api di di Pili fine Ses E.a. al ^a ^{ma} 10 46 mer mer me | | · · | 20 Ar | 20.45 | | | |
| TARLAC R.C.P. | Concrete revetment, floodgate boulder apron, earthdike | | 39.45 | 39.45 | , - | • | . - |
| D'DONNEL R.C.P. | Earthdike, River impv't | and the profit of the second s | 8.66 | 8.30 | - . | e L e st | . • |
| HORIONES R.C.P. | Concrete revetment | | 3.50 | 3.50 | <u>.</u> . | | - |
| DLO R.C.P. Mangatarem | Earthdike, River impv't. | -ditto- | 1.50 | _ | _ | 1.00 | 1.00 |
| ridiga tar em | Eur Citation interest info | | | | | | ., |
| BAYAOAS R.C.P. Aguilar | Concrete revetment with | -ditto- | | 0.40 | - | 0.50 | 0.50 |
| | boulder apron | in the second of | | | | | |
| BEI R.C.P. Bugallon | Earthdike, Spur dike | -ditto- | 0.20 | 0.20 | 0 1 € • 7 | 0.30 | 0.50 |
| CAYANGA R.C.P. | | i i a i shi shi | Τ'. | | • • | | 1 1 1 |
| Sn. Fabian | COC, Concrete revetment | Const./Impv't./ Rehab. ON-GOING | 1.00 | 1.00 | | 1.00 | |
| uro n e h | | A State of the Sta | | 5,1 ×1 | | 1 112 | 1.3 |
| UED R.C.P. Sison | Revetment | -ditto- | · · | - | 2.00 | 2.00 | - |
| Sn. Fabian | Concrete revetment, Bank protec | ditto- | 0.90 | . • • | 1.35 | | 0.90 |
| WED-ALORAGAT R.C.P. | | | | | | | |
| $\delta = -\delta \frac{\mathbf{a}}{\mathbf{a}} = -\frac{1}{2} \left(\frac{1}{2} - \frac{1}{2} \right)$ | Bank protection, Spur dike | -ditto- | 0.50 | 0.50 | 0.50 | 1.20 | 1.30 |
| NGALACAN R.C.P. | • • | | | ± *** | | 4 | ٠. |
| Hapandan | Spur dike, Bank protection | | 0.50 | | | | - |
| Mapandan - | Spur dike, Concrete revetment Spur dike, River impv't. | -ditto- -ditto- | 0.55 0.50 | 0.50 | -1 | 1.10 0.70 | 0.80 |
| 71/00a1 all D. O. O. | • | • | | | | | |
| INOCALAN R.C.P. Sta. Barbara-Urdaneta | Spur dike, COC | Const./Impv't./ Rehab. NEW PROJECT | | - | 1.50 | 2.00 | · · · · · · · · · · · · · · · · · · · |
| ARUSAY R.C.P. | | Tionast Henry (Noozon | • | | | | |
| Sta. Barbara | Earthdike, Spur dike, Bank protection | Const./Impv't./ Rehab. ON-GOING | : | 1.00 | 2.10 | 2.20 | • • |
| AGAMUSING R.C.P. | | | | | .* | | |
| Sumabnit | Spur dike, Revetment. River impv't. | -ditto- | 7.1 - | 1.20 | - | 1.10 | - |
| Binalonan | Spur dike, Concrete revetment, | -ditto- | 1.50 | 1.00 | 0.50 | 0.40 | - |
| OLONG-MITURA R.C.P. Urdaneta | Revetment, Spur dike | -ditto- | 0.40 | 1.00 | 0.90 | | |
| HRVV O C D | | | | | | | |
| UBOY R.C.P. San. Manuel | Spur dike | -ditto- | : | 0.50 | 0.40 | 0.40 | |

Table 4.1 RIVER STRETCHES SUBJECT TO MASTER PLAN

| River System | | River and Stretch | Length (km |
|----------------|--------------------------------|--|------------|
| Agno | Main Agno | : From rivermouth to the ARIS dam | 98.85 |
| 4 11 14 | Tarlac | : From confluence with the Agno to the TARIS dam | 37.05 |
| 0.0 | | A Section 1 | 7.5 |
| | Camiling | : From the 15km Upstream of the confluence with the Agno to the Mayantoc | 20.00 |
| 11 | Banila | : From confluence with the Agno to mountain-foot | 30.90 |
| | Viray-Dipalo | : From confluence with the Agno to the 8.5km Upstream (Main river) | 7.90 |
| 11. | | From confluence with the Dipalo to mountain-foot (Viray river) | 4.45 |
| | | From confluence with the Viray to mountain-foot (Dipalo river) | 7.70 |
| ed Victoria | Ambayoan | : From confluence with the Agno to mountain-foot | 8.70 |
| ayanga | Main Cayanga (Cayanga-Pata) | : From rivermouth to the 37.5km Upstream an-Angalacan River) | 37.50 |
| | Bued | : From confluence with the Cayanga to the 0.9km Upstream of Bued bridge | 19.80 |
| | Aloragat | : From confluence with the Patalan to the 2km Upstream of Bobonan bridge | 19.70 |
| anto | Main Panto (Panto-Marusay | : From rivermouth to mountain-foot -Sinocalan-Tagumising-Tuboy River) | 49.40 |
| | Dagupan | : From confluence with the Panto to San Carlos | 27.60 |
| | (Dagupan-San J | uan-Elang River) | |
| | Ingalera | : From confluence with the Sinocalan to the 1.5km Upstream of San Nicolas bridge | 37.50 |
| | Mitura-Macalon | g : From confluence with the Sinocalan to San Manuel | 21.00 |

Table 4.2 APPLIED MANNING'S COEFFICIENT BY RIVER STRETCH

| - 1013 aut | Manning's | Manning's Coefficient | | | |
|---|--|-----------------------|--|--|--|
| River/Stretch | | Kigh-water Channel | | | |
| Main Agno River | | | | | |
| Rivermouth - Conf. of Olo River | 0.028 | 0.040 | | | |
| Conf. of Olo River - Conf. of Viray-Dipalo River | 0.030 | 0.040 | | | |
| Conf. of Viray-Dipalo River - Conf. of Ambayoan River | 0.033 | 0.040 | | | |
| Conf. of Ambayoan River - San Roque Dam | 0.035 | 0.045 | | | |
| Tarlac River | | | | | |
| Conf. of Agno River - Conf. of O'Donnell River | 0.030 | 0.040 | | | |
| O'Donell River | 0.033 | 0.040 | | | |
| Camiling River | 0.030 | 0.040 | | | |
| Banila River | | | | | |
| Conf. of Agno River - Conf. of Karayogan River | 0.033 | 0.040 | | | |
| Upstream of Conf. of Karayogan River | 0.035 | 0.045 | | | |
| Viray - Dipalo River | | | | | |
| Conf. of Agno River - Conf. of Dipalo River | 0.033 | 0.040 | | | |
| Upstream of Conf. of Dipalo River | 0.035 | 0.045 | | | |
| Ambayoan River | 0.035 | 0.045 | | | |
| Cayanga River - Patalan River | | | | | |
| Rivermouth - Conf. of Bued River | 0.028 | 0.040 | | | |
| Conf. of Bued River - Conf. of Angalacan River | 0.030 | 0.040 | | | |
| Bued River | ***** | ***** | | | |
| | 0.033 | 0.040 | | | |
| Upstream of San Fabian Weir | 0.035 | 0.045 | | | |
| Aloragat River | 0.000 | ****** | | | |
| Conf. of Patalan River - Amagbagan | 0.033 | 0.040 | | | |
| · · | 0.035 | 0.045 | | | |
| Upstream of Amagbagan Amgalacan River | 0,000 | 0.015 | | | |
| Angalacan River Conf. of Patalan River - Maraboc | 0.033 | 0.040 | | | |
| | 0.035 | 0.045 | | | |
| Upstream of Maraboc Panto River Sinocalan River | 0.000 | 0.075 | | | |
| | 0.028 | 0.040 | | | |
| Rivermouth - Conf. of Ingalera River Conf. of Ingalera River - Conf. of Tagamusing River | 0.030 | 0.040 | | | |
| | 0.030 | 0.040 | | | |
| Tagamusing River | 0.030 | 0.040 | | | |
| Conf. of Sinocalan River - Yatyat | and the second s | | | | |
| Yatyat - Cili | 0.033 | 0.040 | | | |
| Upstream of Cili | 0.035 | 0.045 | | | |
| Mitura River | 0.030 | 0.040 | | | |
| Magalong River | 0.030 | 0.040 | | | |
| Ingalera River | 0.030 | 0.040 | | | |
| Dagupan River | 0.028 | 0.040 | | | |
| Capangbogan River | 0.030 | 0.040 | | | |

Table 5.1 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR RIVER IMPROVEMENT ONLY ALTERNATIVE

River: AGNO RIVER Design Flood: 100-yr

| | Agno R. | | | | | | |
|-------------------------|---------|---------------|----------------|-----------------|------------------|-----------------------|-----|
| Item | Unit | R.M - AG45 | AG45 - AG65 | AG65 - AG109 | AG109 - AG177 | AG177 - AG180+1.4k | |
| Design Discharge | m3/s | 17400 | 17400 | 17400 | 15700 | 14900 | |
| Distance | m | 6850 | 9050 | 15150 | 10500 | 2800 | 1.0 |
| Gradient of River Bed | _ | 1/6500 | 1/6500 | 1/3500 | 1/2000 | 1/2000 | |
| River Width | m | 1500 | 1500 | 1500 | 1500 | 1500 | • |
| Width of Channel Bed | m | 400-300 | 300 | 240 | 200 | 200 | |
| Dike Height (Ave.) | m | 6.6 | 7.0 | 7.5 | 6.8 | 6.5 | |
| Water Depth | m | 9.23-11.13 | 11.13-12.0 | 12.0 | 12.0-10.66 | 10.66-10.38 | |
| Low Channel Height (Ave | .) m | 6.5 | 6.5 | 6.5 | 6.5 | 6.0 | |

| | ٠ | Flood way | • | Agno R. | • | | • |
|--------------------------|------|-----------------------|------------------|------------------|------------------|------------------|---|
| Item | Unit | AG180+1.4k - AG324 | AG324 - AG351 | AG351 - AG367 | AG367 - AG414 | AG414 - AG453 | |
| Design Discharge | m3/s | 9200 | 9200 | 8200 | 8200 | 8200 | |
| Distance | m | 12100 | 12300 | 7650 | 7700 | 5300 | |
| Gradient of River Bed | | 1/1550 | 1/1550 | 1/1000 | 1/700 | 1/370 | |
| River Width | m | 1000-830 | 900-2500 | 1000-3200 | 1050-2500 | 1250-2400 | |
| Width of Channel Bed | m | 180 | 180 | 180 | 180 | 150 | |
| Dike Height (Ave.) | m | 6.5 | 5.5 | 5.2 | 4.0 | 3.3 | |
| Water Depth | m | 10.38-8.0 | 8.0 | 8.0-5.5 | 5.5 | 4.3 | |
| Low Channel Height (Ave. |) m | 4.5 | 4.0 | 3.0 | 3.0 | 3.0 | 1 |

| | | . Ag | gno R. | |
|---------------------------|------|-------------------|------------------|--|
| Item | Unit | AG453 - AQG459 | AG459 - AG473 | |
| Design Discharge | m3/s | 6400 | 6400 | |
| Distance | m | 3000 | 6450 | |
| | :3 : | 1/370 | 1/210 | |
| River Width | m | 1000-1900 | 300-1300 | |
| Width of Channel Bed | m | 150 | 150 | |
| Dike Height (Ave.) | m | 2.8 | 2.8-4.0 | |
| Water Depth | m. | 4.3 | 4.3-5.5 | |
| Low Channel Height (Ave.) | m | 3.0 | 3.0 | |

Table 5.2(1/2) FEATURES OF DESIGN CHANNEL OF TRIBUTARIES OF AGNO RIVER FOR RIVER IMPROVEMENT ONLY ALTERNATIVE

River: TARLAC RIVER Design Flood: 100-yr

| | | Tarlac R. | | | | | | | |
|-------------------------|------|-----------------------|------------------|------------------|------------------|----------------------|-------|--|--|
| Item | Unit | AG180+0.8k - TA187 | TA187 - TA200 | TA200 - TA227 | TA227 - TA251 | TA251 - TARIS DAM | | | |
| Design Discharge | m3/s | 6800 | 4000 | 4000 | 4000 | 3200 | | | |
| Distance | m | 2050 | 6050 | 13000 | 11800 | 4150 | | | |
| Gradient of River Bed | _ | 1/1850 | 1/1850 | 1/1300 | 1/760 | 1/692 | • | | |
| River Width | m | 1000 | 1000 | 1700-640 | 1600-600 | 600-270 | | | |
| Width of Channel Bed | m | 180 | 160 | 160 | 160 | 140 | | | |
| Dike Height (Ave.) | m | 7.5 | 7.4 | 5.0 | 4.2 | 2.5 | F171 | | |
| Water Depth | m | 10.3-9.49 | 9.49-6.22 | 6.22-4.7 | 4.7 | 4.7-4.4 | * * * | | |
| Low Channel Height (Ave | | 5.0-4.0 | 4.0-2.0 | 2.0 | 2.0 | 3.5 | | | |

River: CAMILING RIVER Design Flood: 50-yr

| | | Camiling R. | | | | | | | | |
|-------------------------|------|--------------------------|-----------------------|------------------|------------------|------------------|------------------|--|--|--|
| Item | Unit | AG143+1.0k CA156+0.3k | CA156+0.3k - CA162 | CA162 - CA167 | CA167 - CA172 | CA172 - CA173 | CA173 - CA175 | | | |
| Design Discharge | m3/s | 2200 | 1550 | 1550 | 1550 | 1150 | 1150 | | | |
| Distance | m | 3550 | 4650 | 4300 | 4950 | 1300 | 2050 | | | |
| Gradient of River Bed | _ | 1/2000 | 1/2000 | 1/1000 | 1/550 | 1/300 | Existing | | | |
| River Width | m | 250 | 180 | 180 | 180 | 130 | 130 | | | |
| Width of Channel Bed | m | 60 | 50 | 50 | 50 | 35 | Existing | | | |
| Dike Height (Ave.) | m | 6.8 | 5.0 | 3.6 | 2.8 | 1.8 | 1.8-0.0 | | | |
| Water Depth | m | 10.11-8.96 | 8.96-7.5 | 7.5-7.1 | 7.1-5.42 | 5.42-5.22 | 5.22-4.8 | | | |
| Low Channel Height (Ave | | 4.7 | 4.7 | 4.7 | 4.5 | 4.5 | 4.0 | | | |

River: BANILA RIVER Design Flood: 50-yr

| Item | | | | Banila R. | | | |
|-------------------------|------|----------------------|-----------------------|------------------|------------------|------------------|------------------|
| | Unit | AG349- AG349+3.7k | AG349+3.7k - BN381 | BN381 - BN386 | BN386 - BN394 | BN394 - BN397 | BN397 - BN401 |
| Design Discharge | m3/s | 1400 | 1400 | 950 | 440 | 440 | 340 |
| Distance | m | 3700 | 8050 | 4550 | 7600 | 2900 | 4100 |
| Gradient of River Bed | _ | 1/1295 | 1/835 | 1/520 | 1/265 | Existing | Existing |
| River Width | m | 180 | 180 | 120 | 120 | 120 | 120 |
| Width of Channel Bed | m | 30 | 30 | 20 | 10 | Existing | Existing |
| Dike Height (Ave.) | m | 3.6 | 3.2 | 2.9 | 2.4 | 2.1 | 1.3 |
| Water Depth | m | 7.6 | 7.0 | 7.0-6.42 | 6.42-3.14 | 3.14-1.5 | 1.5 |
| Low Channel Height (Ave | .) m | 5.0 | 4.8 | 4.8 | 4.8-2.5 | 1.0 | 1.0 |

Table 5.2(2/2) FEATURES OF DESIGN CHANNELEL OF TRIBUTARIES OF AGNO RIVER FOR RIVER IMPROVEMENT ONLY ALTERNATIVE

River: VIRAY-DIPALO RIVER

Design Flood: 50-yr

| | | Viray-Dipalo R. | | | | Viray R. | | | |
|-------------------------|-------|-----------------|-----------------|-----------------|----------------------|----------------------|---------------------|--|--|
| Item | Unit | AG414- VD425 | VD425- VD428 | VD428- VD430 | VD430- VD430+0.6k | VD430+0.6k -VD433 | VD433 VD434+0.5k | | |
| Design Discharge | m3/s | 750 | 750 | 750 | 750 | 370 | 370 | | |
| Distance | m | 2800 | 3100 | 2000 | 600 | 2400 | 1450 | | |
| Gradient of River Bed | ₩, | 1/375 | 1/300 | 1/250 | 1/127 | 1/127 | 1/75 | | |
| River Width | · · m | 300-290 | 320-270 | 320-260 | 300 | 150 | 150 | | |
| Width of Channel Bed | · m | 30 | 30 | 30 | - 30 | 15 | 15 | | |
| Dike Height (Ave.) | m i | 1.7 | 1.7 | 1.7 | 1.7 | 0.9 | 0.9 | | |
| Water Depth | m | 4.0 | 4.0 | 4.0 | 4.0 | 2.9 | 2.9 | | |
| Low Channel Height (Ave | m (.e | 3.3 | 3.3 | 3.3 | 3.3 | 2.8 | 2.8 | | |

| | Dipalo R. | | | | | | |
|--------|--------------------------|--|---|---|-----------------|-----------------|--|
| Unit V | D430+0.6k -VD436 | VD436- VD437 | VD437- VD439 | VD439- VD441 | VD441- VD442 | | |
| m3/s | 350 | 350 | 210 | 210 | 210 | | |
| m | 1500 | 700 | 1950 | 1950 | 1000 | | |
| - | 1/170 | 1/125 | 1/125 | 1/80 | 1/68 | | |
| m | 100 | 100 | 100 | 100 | 100 | | |
| m | 15 | . 15 | 10 | -10 | 10 | | |
| m | 2.6 | 2.6 | 2.3 | 2.1 | 1.9 | | |
| m | 3.8 | 3.0 | 2.5 | 2.3 | 2.1 | | |
| .) m | 2.0 | 1.2 | 1.0 | 1.0 | 1.0 | | |
| | m3/s m - m m | m3/s 350 m 1500 - 1/170 m 100 m 15 m 2.6 m 3.8 | Unit VD430+0.6k VD436VD436 VD437 m3/s 350 350 m 1500 700 - 1/170 1/125 m 100 100 m 15 15 m 2.6 2.6 m 3.8 3.0 | Unit VD430+0.6k VD436- VD437VD436 VD437 VD439 m3/s 350 350 210 m 1500 700 1950 - 1/170 1/125 1/125 m 100 100 100 m 15 15 15 10 m 2.6 2.6 2.3 m 3.8 3.0 2.5 | Unit VD430+0.6k | Unit VD430+0.6k | |

River: AMBAYOAN RIVER Design Flood: 50-yr

| Ambayoan R. | | | | | | | | | | | |
|------------------------|--------------|----------------------|-----------------------|-----------------------|---|--|--|--------|--|--|--|
| Item | | AG461- AM444+0.5k | AM444+0.5k - AM448 | AM448 - AM451+0.4k | | | | | | | |
| Design Discharge | m3/s | 1750 | 1750 | 1750 | | | | ****** | | | |
| Distance | m | 1800 | 3350 | 3350 | ÷ | | | | | | |
| Gradient of River Bed | . - . | 1/390 | 1/205 | 1/150 | | | | | | | |
| River Width | m | 400 | 400 | 400 | | | | | | | |
| Width of Channel Bed | m | 50 | 50 | 50 | | | | | | | |
| Dike Height (Ave.) | w | 4.2 | 2.2 | 2.0 | | | | | | | |
| Water Depth | m | 5.5 | 3.7 | 3.5 | | | | | | | |
| Low Channel Height (Av | e.) m | 2.8 | 2.5 | 2.5 | | | | ٠ | | | |

Table 5.3(1/3) FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS FOR RIVER IMPROVEMENT ONLY ALTERNATIVE

River: CAYANGA-PATALAN-AN-ANGALACAN-RIVER Design Flood: 50-yr (with Closure Dike)

| | | Cayanga R. | Patalan R. | Angalacan River | | | | |
|-------------------------|------|------------------|-----------------------|-------------------------|-------|---------------------|----------------------|--|
| Item | Unit | R.M - Bued R. | Bued R Aloragat R. | Aloragat R. - 21.0 k | | Maraboc - 27.0 k | 27.0 k - Bugayong | |
| Design Discharge | m3/s | 2600 | 1300 | 700 | 700 | 500 | 500 | |
| Distance | m | 6500 | 8300 | 6200 | 2800 | 3200 | 3300 | |
| Gradient of River Bed | | 1/1300 | 1/1100 | 1/650 | 1/460 | 1/460 | 1/230 | |
| River Hidth | mi | 500 | 150 | 120 | 100 | 100 | 80 | |
| Width of Channel Bed | m | 60 | 40 | 30 | 30 | 25 | 20 | |
| Dike Height (Ave.) | m | 2.7 | 2.8 | 1.4 | 1.1 | 0.7 | 0.3 | |
| Water Depth | m | 8.0 | 6.8 | 5.4 | 5.1 | 4.7 | 4.3 | |
| Low Channel Height (Ave | .) m | 6.5 | 5.0 | 5.0 | 5.0 | 5.0 | 4.0 | |

| Angalacan River | | | | | | | | | | |
|-------------------------|------|------------------------|---------------------|-----|--|--|--|--|--|--|
| Item | Unit | Bugayong -Killo Br. | Killo Br. -37.5k | | | | | | | |
| Design Discharge | m3/s | 370 | 370 | | | | | | | |
| Distance | m | 2700 | 4500 | | | 11:00 | | | | |
| Gradient of River Bed | _ | 1/190 | 1/140 | + 1 | | | | | | |
| River Width | m | 60 | 50 | F | | | | | | |
| Width of Channel Bed | m | 15 | 15 | 1 | | $(-\infty,+\infty) = \{ (-\infty,+\infty) \mid x \in \mathbb{R}^n \mid x \in \mathbb{R}^n : x \leq n \}$ | | | | |
| Dike Height (Ave.) | m | 0.4 | 1.1 | | | | | | | |
| Water Depth | m | 3.6 | 3.3 | | | a Augh | | | | |
| Low Channel Height (Ave | .) m | 4.0 | 3.0 | | | and the second of the second | | | | |

River: BUED RIVER

Design Flood: 50-yr (with Closure Dike)

| Item | Unit | Junction - 2.0 k | 2.0 k 4.0 k | 4.0 k NIA Dam | | 11.9 k 16.5 k | 16.5 k - 19.7 k |
|-------------------------|-------|---------------------|----------------|------------------|-------|------------------|--------------------|
| Design Discharge | m3/s | 1300 | 1300 | 1300 | 1300 | 1000 | 1000 |
| Distance | · m | 2000 | 2000 | 3300 | 4600 | 4600 | 3200 |
| Gradient of River Bed | _ | 1/400 | 1/280 | 1/170 | 1/143 | 1/140 | 1/70 |
| River Width | m | 400 | 400 | 400 | 400 | 400 | 400 |
| Width of Channel Bed | m | 30 | 20 | 20 | 20 | 20 | 20 |
| Dike Height (Ave.) | m | 4.2-2.0 | 2.1 | 2.1 | 1.9 | 1.9 | 1.4 |
| Water Depth | m | 8.0-5.8 | 5.6 | 3.3 | 2.4 | 2.1 | 1.9 |
| Low Channel Height (Ave | e.) m | 5.0 | 3.5 | 2.0 | 1.5 | 1.5 | 1.5 |

Table 5.3(2/3) FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS FOR RIVER IMPROVEMENT ONLY ALTERNATIVE

River: ALORAGAT RIVER

Design Flood: 50-yr (with Bued Closure Dike)

| | * * | ALORAGAT RIVER | | | | | | | | | |
|------------------------|------|----------------|-------|--------|--------|------|--|--|--|--|--|
| Item | Unit | Junction | 7.0k- | 11.5k- | 17.0k- | | | | | | |
| | | -7.0k | 11.5k | 17.0k | 19.7k | | | | | | |
| Design Discharge | m3/s | 470 | 470 | 250 | 170 | | | | | | |
|)istance | m | 7000 | 4500 | 5500 | 2700 | • | | | | | |
| Gradient of River Bed | | 1/680 | 1/355 | 1/355 | 1/185 | | | | | | |
| liver Width | m | 90 | - 80 | 50 | 45 | | | | | | |
| lidth of Channel Bed | . m | 30 | 20 | 10 | 10 | 1= 1 | | | | | |
|)ike Height (Ave.) | m | .0.0 | 0.0 | 1.3 | 1.4 | | | | | | |
| later Depth | m | 6.8-4.2 | 4.0 | 4.0 | 2.8 | | | | | | |
| ow Channel Height (Ave | .) m | 5.5 | 5.0 | 3.5 | 2.0 | • | | | | | |

River: PANTO-MARUSAY-SINOCALAN-TUBOY RIVER

Design Flood: 50-yr

| | | PANTO R. | MARUSAY | R. | SINOCALAN R. | | | |
|------------------------|-------|--------------------|---------------------|-----------------------|--------------------|--------------------|--------------------|--|
| Item | Unit | R.M- Dagupan R. | Dagupan R. -4.0k | 4.0k - Ingalera R. | Ingalera -18.0k | R. 18.0k- 25.5k | 25.5k- Mitura R | |
| Design Discharge | m3/s | 3300 | 2250 | 2250 | 1650 | 1250 | 1250 | |
| Distance | m | 2500 | 1500 | 4300 | 9700 | 7500 | 5500 | |
| Gradient of River Bed | | 1/1750 | 1/1750 | 1/1750 | 1/1750 | 1/1450 | 1/1100 | |
| River Width | m | 400 | 120 | 250 | 250 | 200 | 150 | |
| Width of Channel Bed | m | - 80 | 80 | 60 | 40 | 40 | 30 | |
| Dike Height (Ave.) | m | 3.9 | 3.6 | 3.6 | 3.4 | 3.3 | 3.2 | |
| Water Depth | m | 8.2 | 7.9 | 7.9 | 7.9 | 7.8 | 7.2 | |
| Low Channel Height (Av | e.) m | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.0 | |

| | | | TAGUMISING R. | | TUBOY R | • | |
|-------------------------|-------|---------------------|----------------------|----------------------|---------|----------------|--|
| Item | Unit | Mitura R. -36.7k | 36.7k- Sta. Maria | Sta. Maria -43.5k | 47.4k | 47.4k- 49.8 | |
| Design Discharge | m3/s | 800 | 800 | 650 | 550 | 550 | |
| Distance | m | 5700 | 4700 | 2100 | 3900 | 2000 | |
| Gradient of River Bed | - | 1/700 | 1/430 | 1/350 | 1/190 | 1/143-1/67 | |
| River Width | m | 100 | 100 | 80 | 60 | 60 | |
| Width of Channel Bed | m | 20 | 20 | 20 | . 15 | 15-10 | |
| Dike Height (Ave.) | m | 2.5 | 2.3 | 1.7 | 1.7 | 1.7-0.3 | |
| Water Depth | m | 6.5 | 5.8 | 5.2 | 4.7 | 4.7-3.3 | |
| Low Channel Height (Ave | e.) m | 5.0 | 4.5 | 4.5 | 4.0 | 4.0 | |

Table 5.3(3/3) FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS FOR RIVER IMPROVEMENT ONLY ALTERNATIVE

River: DAGUPAN RIVER Design Flood: 50-yr

| | | DAGUPAN R. | SAN | JUAN R. | ELANG R. | |
|-------------------------|-------|-------------------|----------------|--------------------|--------------------|-------------------------------------|
| Item | Unit | Junction -7.5K | 7.5K- 12.7K | 12.7K- Elang R. | San Juan -27.6k | |
| Design Discharge | m3/s | 1100 | 900 | 650 | 310 | |
| Distance | m | 7500 | 5200 | 9000 | 5900 | - |
| Gradient of River Bed | | 1/5000 | 1/5000 | 1/5000 | 1/5000 | 100 |
| River Width | m | 250 | 100 | 100 | 50 | 66 9 |
| Width of Channel Bed | m | 60 | 30 | 30 | 20 | $(x_1,\dots,x_{n-1},\dots,x_{n-1})$ |
| Dike Height (Ave.) | m | 3.9-3.2 | 3.6 | 4.1 | 3.3 | 1.1.1.1 |
| Water Depth | · M | 8.2-7.7 | 7.6 | 7.6 | 7.0 | |
| Low Channel Height (Ave | e.) m | 5.5 | 5.0 | 4.5 | 4.5 | n kantan di Kabupatèn Kabupatèn |

River: INGALERA RIVER Design Flood: 50-yr

| | | | INGALERA | RIVER | | | |
|------------------------|---------|------------------------|----------------------|--------|----------------------|-------|-----------------------|
| Item | | Junction -Malasigui | Malasigui - 26.0k | 32.0k | 32.0k- San Nicols | | , |
| Design Discharge | m3/s | 600 | 460 | 260 | 260 | 150 | |
| Distance | m | 13300 | 12700 | 6000 | 4000 | 1500 | |
| Gradient of River Bed | · · _ · | 1/3600 | 1/1800 | 1/1000 | 1/700 | 1/700 | Depth (|
| River Hidth | m | 100 | 60 | 50 | 50 | 40 | |
| Width of Channel Bed | m | 25 | 15 | 15 | 10 | 10 | |
| Dike Height (Ave.) | ·m | 3.6-3.0 | 2.2 | 1.3 | 1.7 | 1.3 | |
| Water Depth | m | 7.9-7.5 | 6.9 | 5.5 | 4.9 | 4.2 | ; |
| Low Channel Height (Av | e.) m | 5.5 | 5.5 | 5.0 | 4.0 | 3.5 | esta e La contrata |

River: MITURA-MAGALONG RI RIVER

Design Flood: 50-yr

| | | MITURA R. | | MAGALONG R | IVER | |
|-------------------------|-------|-------------------|----------------|------------------|------------------|--------------------|
| Item | Unit | Junction -5.3k | 5.3k- Taboy | Taboy - 19.0k | 19.0k - 21.0k | |
| Design Discharge | m3/s | 250 | 250 | 180 | 140 | |
| Distance | m | 5300 | 8900 | 4800 | 2000 | |
| Gradient of River Bed | : L | 1/800 | 1/460 | 1/460 | 1/250 | |
| River Width | · m | 50 | 40 | 35 | 30 | |
| Width of Channel Bed | m | 10 | 8 | 6 | a 1 4 , | |
| Dike Height (Ave.) | m | 3.0-1.0 | 1.5 | 1.4 | 1.3 | |
| Water Depth | m | 7.2-5.2 | 4.7 | 4.3 | 3.7 | and the second |
| Low Channel Height (Ave | e.) m | 5.0 | 4.0 | 3.5 | 3.0 | and the second and |
| | | | | | * : | |

Tabel 5.4 SUMMARY OF WORK QUANTITIES OF RIVER IMPROVEMENT ONLY ALTERNATIVE

| 64 CH 137 VA | Work Item | Unit | Main | Tarlac | Tributaries | Allied 1> |
|--------------|----------------------------|------------|------------|-----------|-------------|-----------|
| | MOLK TCOM | OHLE | Agno | River | of Agno | Rivers |
| | | | | | | |
| (1) | Excavation 1 | cu.m | 25,375,000 | 5,050,000 | 1,949,000 | 8,408,800 |
| ` ' | Excavation 2 | cu.m | 2,850,000 | 0 | 134,000 | 188,000 |
| | Total of (1) | cu.m | 28,225,000 | 5,050,000 | 2,083,000 | 8,596,800 |
| (2) | Dredging | cu.m | 17,075,000 | 0 | 0 | 503,000 |
| (3) | Embankment 1 | | • | | | |
| | Left Dike | cu.m | 5,647,100 | 1,630,000 | 1,667,400 | 4,634,200 |
| | Right Dike | cu.m | 4,055,100 | 2,671,700 | 1,689,900 | 4,634,200 |
| | Embankment 2 | | | | | |
| | Left Dike | cu.m | 6,820,000 | 0 | 13,000 | 39,600 |
| | Right Dike | cu.m | 9,129,000 | 0 | . 0 | 80,600 |
| | Total of (3) | cu.m | 25,651,200 | 4,301,700 | | 9,388,600 |
| (4) | Sodding | sq.m | 6,547,000 | 1,667,690 | 1,670,200 | 4,844,300 |
| (5) | Revetment (L.W.C.) | | | 12.2 | | |
| | Type-A | sq.m | 232,100 | 76,700 | 170,700 | 319,100 |
| | Type-B | sq.m | 130,200 | 12,100 | 3,500 | 235,900 |
| | Revetment (H.W.C.) | 1. | 4 | | أنمت من | |
| | Type-A | sq.m | 63,700 | 8,300 | 16,020 | 38,000 |
| | Type-B | sq.m | 91,300 | 0 | 0 | 0 |
| | Total of (5) | sq.m | 517,300 | 97,100 | 190,220 | 593,000 |
| (6) | Groin (L.W.C.) | | CEO | | 1 070 | 0.057 |
| | Type-A | pc. | 658 | 244 | 1,070 | 2,057 |
| | Type-B | pc. | 0 | . 0 | · Q | 0 |
| | Groin (H.W.C.) | | 140 | 0 | . ^ | Ċ |
| | Type-A | pc. | 148 | 0 | 0 | 0 |
| | Type-B | pc. | 152 958 | 0 | 1 070 | • |
| 175 | Total of (6) Sluice Way | pc. | 930 | 244 | 1,070 | 2,057 |
| (7) | Type-A | 200 | 7 | . 0 | 23 | 52 |
| | Type-B | pc. | 11 | 0 | 3 | 32 |
| | Total of (7) | pc. pc. | 18 | 2 | 26 | 55 |
| (8) | Water Gate | he. | 20 | ő | 20 | 33 |
| (0) | Type-A | pc. | 0 | 0 | 0 | 0 |
| | Type-B | pc. | 2 | 1 | . 0 | . 0 |
| | Total of (8) | pc. | 2 | 1 | Ö | 0 |
| (9) | Bridge | Por | _ | 0 | | • |
| () | Newly Const. | sq.m | 51,750 | 13,500 | 20,100 | 20,471 |
| | Rehabilit. | sq.m | . 0 | 0 | 0 | 3,426 |
| | Demolishment | - 11 | | . 0 | | -, |
| | Concrete | cu.m | 9,600 | 2,500 | 4,200 | 10,720 |
| | Metal | ton | 2,360 | 0 | 0 | 0 |
| (10) | Fixed Weir | pc. | 0 | 0 | . 0 | 0 |
| | Others | L.S | 1 | 0 | 1 | 1 |

^{1&}gt; : River Improvement + Bued Closure Dike

Table 6.1 SUMMARY OF WORK QUANTITIES AND COST FOR AGNO FLOODWAY ALTERNATIVE

| | | | | | Sinocalan R. | |
|--|----------------------|--------------------------------|---------------------------------------|--|--------------|--------------|
| ltem | Unit | Agno Floodway | Bued River | Aloragat River | (Whole) | Main Anno |
| Work Quantities | | 00 00 to 50 to 24 ac as 12 has | me Get stad find den was who five dat |) They spic spic spic them were that the "ener i | | |
| Excavation | 1,000 m3 | 11,157 | 372 | 300 | 5,125 | 41,700 |
| Embankment | 1,000 m3 | 5,327 | 235 | · · · · · · · · · · · · · · · · · · · | 6,115 | 14,150 |
| Sodding | 1,000 m2 | 2,600 | 152 | . 0 | 3,400 | 3,900 |
| Revetment (L.W.C) | 1,000 m2 1,000 m2 | 651 62 | 40 2 | 41 | | 333 145 |
| Groin | Pc. | 542 | 281 | 272 | 740 | 732 |
| Sluiceway | Pc. | 40 | 6 | 0 | 33 | 46 |
| Water Gate | Pc. | 0 | 0 | 0 | 0 | 3 |
| Br idge | Pc. 100 m2 | 8 850 | 1 30 | 2 | | 3 456 |
| Intake | Pc. | 17 | 0 | 0 | 0 | 0 |
| Others | Pc. | 0 | 1 | 0 | . 0 | 1 |
| 2 Per 194 Not 196 Not 196 Not 196 AP 195 Not 197 NOT 198 NOT 197 NOT 198 NOT 197 NOT 198 NOT 197 NOT 198 NOT 1 | | | | 40 to 100 to 100 to 40 list on 2 | | |
| Main Construction Cost (Economic) | Pesos | | | 90 | 1,540 | 5,948 |

Note: Main Construction Cost - Preparatory Works + Main Works + Misellaneus Works

Table 6.2 SUMMARY OF HORK QUANTITIES AND COST FOR SAN MANNUEL FLOODWAY ALTERNATIVE

| Item | Unit | Cayanga-Patal | | | | |
|--|----------|---------------|-------|-------------------------|------------------|--------------|
| ing the second of the second o | | (Whole) | | San Mannuel Floodway | Other Stretch | Main Agno |
| ork Quantities | | | | | | |
| Éxcavation | 1,000 m3 | | 2,611 | 1,312 | 5,125 | 45,300 |
| Embankment | 1,000 m3 | | 1,331 | 1,084 | 6,115 | 26,550 |
| Sodding | 1,000 m2 | | 690 | 488 | 3,400 | 6,776 |
| Revetment (L.W.C) | 1,000 m2 | | 186 | 115 | 361 | 362 |
| (H.W.C) | 1,000 m2 | | 8 | 0 | 25 | 160 |
| Groin | Pc. | | 1,095 | 0 | 740 | 958 |
| Sluiceway | Pci | | 16 | 2 | 33 | 18 |
| Water Gate | Pc. | | 0 | . 0 | 0 | |
| Bridge | Pc. | | 8 | 1 | 23 | |
| | 100 m2 | | 71 | 30 | 142 | 518 |
| Intake | Pc. | | 0 | 3 | 0 | |
| Others | Pc. | | 1 | 0 | 0 | |
| Main Construction Cost (Economic) | Million | . <u></u> | 777 | 342 | 1,540 | 7,84! |

Note: Main Construction Cost = Preparatory Works + Main Works + Misellaneus Works

Table 6.3 SUMMARY OF HORK QUANTITIES AND COST FOR BINALONAN FLOODWAY ALTERNATIVE

| Item | Unit | Cayanga-Patal | Agno R. | | | |
|--------------------------------------|-----------------------|---------------|---------|-----------------------|-------|-------------------|
| | | | | Binalonan Floodway | | |
| Work Quantities | , | | | • | | |
| Excavation | 1,000 ₁ m3 | | 2,801 | 605 | 5,145 | 45,300 |
| Embankment | 1,000.m3 | | 1,773 | 308 | 6,207 | 25,651 |
| Sodding | 1,000 m2 | | 883 | 107 | 3,419 | 6,547 |
| Revetment (L.W.C) | 1,000 m2 | * * . | 186 | : 77: | 369 | 362 |
| | 1,000 m2 | · • | 8 | | 25 | 159 |
| Groin | Pc. | , | 1,095 | 0 | . 754 | 958 |
| Sluiceway | Pc. | | 16 | 2 | 31 | 18 |
| Water Gate | Pc. | .; .; | 0 | 0 | 0 | |
| Bridge | Pc. | • | 9 | 1 | 23 | Satissee • |
| | 100 m2 | | 74 | 8 | 142 | 518 |
| / Intake | Pc. | : | 0 | 4 | 0 | · · · · · · · · · |
| Others | Pc. | ' : | 1 | .0 | 0 | |
| Main Construction Cost (Economic) | Million Pesos | | 837 | 166 | 1,549 | 7,706 |

Note: Main Construction Cost = Preparatory Works + Main Works + Misellaneus Works

Table 6.4 SUMMARY OF WORK QUANTITIES AND COST FOR ALORAGAT FLOODWAY ALTERNATIVE

| Item | | 100 at 40 41 <u>- o</u> o a a a a a a | | Sinocalan R. | Agno R. | | |
|--------------------------------------|------------|---------------------------------------|---------------|----------------------|-------------------|---------|---|
| | | Main Stream | Bued River | Aloragat Floodway | Aloragat River | (Whole) | Agno |
| fork Quantities | | | | | | | |
| Excavation | 1,000 m3 | 2,130 | 372 | 168 | 300 | 6,490 | 45,300 |
| Embankment | 1,000 m3 | 1,440 | 85 | 48 | 437 | 8,058 | 25,65 |
| Sodding | 1,000 m2 | 615 | 38 | 19 | 175 | 4,154 | 6,54 |
| Revetment (L.W.C |) 1.000 m2 | 106 | 40 | 26 | 36 | 369 | 36: |
| |) 1,000 m2 | 6 | 2 | 0 | 0 | | 15 |
| Groin | Pc. | 542 | 281 | 0 | 250 | 962 | 95 |
| Sluiceway | Pc. | 10 | 6 | 1 | 3 | 39 | 1 |
| Water Gate | Pc. | 0 | 0 | 0 | . 0 | 0 4 | |
| Bridge | Pc. | 5 | 1 | · 1 | : 2 | 26 | |
| | 100 m2 | 39 | 30 | 5 | 3 | 167 | 51 |
| Intake | Pc. | 0 | 0 | .0 | 0 | 0 - 1 | - 1 · 1 · 1 · 1 · 1 · 1 · 1 · 1 · 1 · 1 |
| Others | Pc. | . 0 | 0 | . 0 | 0 | 0. | *: * |
| Main Construction Cost (Economic) | | 551 | 223 | 31 | 138 | 1,897 | 7.70 |

Note: Main Construction Cost = Preparatory Works + Main Works + Misellaneus Works

Table 6.5 SUMMARY OF WORK QUANTITIES AND COST FOR BUED CLOSURE DIKE ALTERNATIVE

| ~ C * * * * * * * * * * * * * * * * * * | 20. 30 MI 20 MI 40 EE 54 AF 44 | Cay | anga-Pata | lan R. | Sinocalan R. | Agno R. Main Agno | |
|---|--------------------------------|-----------|-----------|---------|--------------|-------------------------|--|
| Item | Unit | Stream | River | River | (Whole) | | |
| Work Quantities | | | | | | e e e | |
| Excavation | 1,000 m3 | 2,130 | 372 | 300 | 6,490 | 45,300 | |
| Embankment | 1,000 m3 | 1,380 | 85 | 643 | 8,058 | 25,651 | |
| Sodding | 1,000 m3 | 590 | 38 | 260 | 4,154 | 6,547 | |
| Revetment (L.W.C) (H.W.C) | | 106 6 | 40 2 | 41 0 | | 362 155 | |
| Groin | Pc. | 542 | 281 | 272 | 962 | 958 | |
| Sluiceway | Pc. | 10 | 6 | 3 | 39 | 18 | |
| Water Gate | Pc. | . 0 | . 0 | . 0 | 0 | 2 | |
| Bridge | Pc. 100 m2 | . 5 39 | 1 30 | 2 | | 4 518 | |
| Intake | Pc. | 0 | 0 | 0 | 0 | 0. | |
| Others | Pc. | • 0 | 0 | 0 | . 0 | 1 | |
| Main Construction Cost (Economic) | Pagne | 546 | | | 1,897 | 7,706 | |

Note: Main Construction Cost - Preparatory Works + Main Works + Misellaneus Works

Table 7.1 (1/5) WORK QUANTITIES OF RIVER IMPROVEMENT OF AGNO RIVER FOR ALTERNATIVE FRAMEWORK PLANS

River : Agno and Tarlac rivers
Study : Framework Plan
Alternative : River Improvement Only (AG-1)
Return Period : 1/100 - year

| • | | , | | Ag | no River Ha | in Stream | | | | Tarlac River | |
|--|----------|----------|-----------------|------------------|-----------------|--------------------------|--------------------|---------------------------|---|--------------------|-------------------------|
| Work Item | 1 | Unit | Lower Agno | Po | ponto Stret | ch | Upper Agno | Total of | Confluence | Upper Stretch | Total of |
| a de la companya de l | | ** | RH-AG282 (1) | Bayambang (2) | Floodway (3) | Sub-total (2)+(3)-(4) | AG309-AG473 (5) | Agno River (1)+(4)+(5) | AG180-TA200 | TA200-TA265 (7) | Tarlac River (6)+(7) |
| (1) Excavation 1 | l | cu.m | 15,275,000 | 0 | 6,800,000 | 6,800,000 | 3,300,000 | 25,375,000 | 2,600,000 | 2,450,000 | 5,050,000 |
| Excavation 2 | <u> </u> | Cu.m | . 0 | . 0 | 0 | 0 | 2,850,000 | 2,850,000 | | 0 | . 0 |
| Totoal of (| (1) . | cu.m | 15,275,000 | . 0 | 5,800,000 | 6,800,000 | 6,150,000 | 28,225,000 | 2,600,000 | | 5,050,000 |
| (2) Dredging | | cu.m | 17,075,000 | 0 | 0 | 0 | 0 | 17,075,000 | 0 | | 0 |
| (3) Embankment 1 | l | | | | | | | | | | |
| Left Dike | • | cu.m | 2,392,000 | 483,500 | 1,609,600 | 2,093,100 | 1,162,000 | 5,647,100 | 936,000 | 694,000 | 1,630,000 |
| Right Dik | (e | cu.m | 1,499,000 | 0 | 852,100 | | 1,704,000 | 4,055,100 | 1,755,000 | | 2,671,700 |
| Embankment 2 | | | | - | | | -1 | 1.1.2.2.1.2.2 | 4,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | 220,100 | 210717700 |
| Left Dike | | cu.m | 6,377,000 | 0 | 0 | 0 | 443,000 | 6,820,000 | 0 | 0 | 0 |
| Right Dik | | cu,m | 6,629,000 | 0 | 0 | | 2,500,000 | 9,129,000 | 0 | - | 0 |
| Total of (| | cu.m | 16,897,000 | 483,500 | 2,461,700 | | 5,809,000 | 25,651,200 | 2,691,000 | _ | 4,301,700 |
| (4) Sodding | | cu.m | 3,838,000 | 0 | 717,700 | | 1,991,300 | 6,547,000 | 523,890 | | 1,657,690 |
| (5) Revetment (L | | | ,, | • | 717,700 | 727,770 | 110711000 | 01247,1000 | 323,020 | 1,145,000 | 1,007,030 |
| Type-A | | sq.m | 40,000 | 0 | 59.800 | 59,800 | 132,300 | 232,100 | 18,400 | 58,300 | 76,700 |
| Type-8 | | m.p2 | 130,200 | 0 | 05,400 | | 0 | 130,200 | 12,100 | | 12,100 |
| Revetment (I | 1.N.C.) | | 100/200 | . • | | · | · | 150,200 | 10,100 | | 12,100 |
| Type-A | , | sq.m | 0 | . 0 | 0 | 0 | 63,700 | 63,700 | 0 | 8,300 | 8,300 |
| Type-B | | sd-w | 67,600 | 23,700 | 0 | _ | 05,705 | 91,300 | . 0 | | . 0,500 |
| Totoal of (| 51 | sq.m | 237,800 | 23,700 | 59.800 | | 196,000 | 517,300 | 30,500 | • | 97,100 |
| (6) Groin (L.W.C | | adina | 101.000 | 20,,00 | 33,000 | 037000 | 110,000 | 517,500 | 30,500 | 00,000 | 37,100 |
| Type-A | | pc. | 460 | 0 | 0 | . 0 | 198 | . 658 | 0 | 244 | 244 |
| Type-B | -1 | pe. | | . 0 | ů | - | 0 | . 030 | 0 | | 0 |
| Groin (H.H.C | :.) | | Ψ, | • | • | • | · | • | | | |
| Type-A | , | pc. | 0. | 0 | 0 | 0 | 148 | 148 | 0 | . 0 | . 0 |
| Туре-В | | pc. | 0 | 0 | 0 | | 152 | 152 | C | - | . 0 |
| Total of (| (6) | pc pc | 460 | 0 | 0 | - | 19 <i>e</i> 498 | 958 | | • | 244 |
| (7) Slutce Way | (0) | pc. | 400 | U | Ū | U | 430 | 930 | • | 294 | |
| • • | | | _ | | | | _ | | | | . 0 |
| Type-A | | pc . | . 2 | 1 | 1 | | 3 | 7 | 0 | | 2 |
| Туре-В | | pc. | 8 | 2 | 0 | - | 1 | 11 | . 0 | • | C |
| Totoal of (| (7) | pc. | 10 | . 3 | 1 | 4 | 4 | 18 | . 0 |] 2 | 2 |
| (8) Water Gate | | | | _ | | | | | | | . 0 |
| Type-A | | pc. | 0 | 0 | . 0 | | 0 | 0 | 0 | • | 0 |
| Type-B | | pc . | 2 | 0 | . 0 | | 0 | 2 | 1 | | 1 |
| Totoal of (| (8) | pc. | 2 | 0 | 0 | . 0 | 0 | . 2 | 1 | 0 | 1 |
| (9) Bridge | | | | _ | _ | _ | | | _ | | 0 |
| Hewly Con | | sq.m | 45,000 | 0 | . 0 | | 6,750 | 51,750 | | | 13,500 |
| Rehabilit | | 20.m | ø | 8 | 0 | • 0 | 0 | 0 | (| 6 | 0 |
| Demo i i siment | τ | | | | | _ | | | | | |
| Concreto | | cu.m | 5,800 | . 0 | 0 | | 3,800 | 9,600 | (| | 2,500 |
| Hetal | | ton | 1,060 | . 0 | . 0 | | 1,300 | 2,360 | , , | 0 | 0 |
| (10) Fixed Weir | | pc. | 0 | 0 | 0 | | 0 | 0 | (| | 0 |
| (11) Others | | L.S | 1 | 0 | 0 | 0 | . 0 | 1 | (|) , 0 | 0 |

(File cord : WQ-AGNF1)

Table 7.1 (2/5) WORK QUANTITIES OF RIVER IMPROVEMENT OF AGNO RIVER FOR ALTERNATIVE FRAMEWORK PLANS

River

: Agno and Tarlac rivers

Study Alternative : Framework Plan

: River Improvement and Natural Retarding Baisn (AG-2)

Return Period : 1/100 - year

| | | | Ą | no River Ma | in Stream | | | | Tarlac River | |
|-------------------------------|-------|---|------------------|-----------------|--------------------------|--------------------|---------------------------|--------------------|--------------------|-----------|
| Rork Item | Vnit | Lower Agno | Po | ponto Stret | ch | Upper Agno | Total of | Conf luence | Upper Stretch | Total |
| | • . | RM-AG282 (1) | Bayambang (2) | Floodway (3) | Sub-total (2)+(3)=(4) | AG309-AG473 (5) | Agno River (1)+(4)+(5) | AG180-TA200 (6) | TA200-TA265 (7) | |
| (1) Excavation 1 | Ct.m | 15,275,000 | 650,000 | 6,800,000 | 7,450,000 | 3,300,000 | 26,025,000 | 2,600,000 | 2,450,000 | 5,050,000 |
| Excavation 2 | cu.m | 0 | 0 | 0 | 0 | 2,850,000 | 2,850,000 | . 0 | 0 | 0 |
| Totosl of (1) | çu.m | 15,275,000 | 650,000 | 6,800,000 | 7,450,000 | 6,150,000 | 28,875,000 | 2,600,000 | 2,450,000 | 5,050,000 |
| (2) Dredging (3) Embankment 1 | cu.m | 17,075,000 | 0 | 0 | 0 | . 0 | 17,075,000 | | 0 | 0 |
| Left Dike | cu.m | 1,812,700 | 343,000 | 268,100 | 631,100 | 1,162,000 | 3,605,800 | 936,000 | 694,000 | 1,630,000 |
| Right Dike Embankment 2 | Çu.n | 934,400 | 374,000 | 1,431,900 | 1,805,900 | 1,704,000 | 4,444,300 | 0 | 916,700 | 916,709 |
| Left Dike | m.uo | 4,754,200 | 0 | 0 | o o | 443,000 | 5,197,200 | 0 | 0 | . 0 |
| Right Dike | Cu.m | 5,067,100 | . 0 | 0 | ō | 2,500,000 | 7,567,100 | G | _ | 0 |
| Totoal of (3) | cu.m | 12,568,400 | 717,000 | 1,720,000 | 2,437,000 | 5,809,000 | 20,814,400 | 936,000 | 1,610,700 | 2,546,700 |
| (4) Sodding | cu in | 3,156,000 | 237,200 | 402,000 | 639,200 | 1,991.300 | 5,786,500 | 523,890 | | 1,667,690 |
| (5) Revetment (L.W.C.) | ÇGILI | 5,,55,,755 | , | | 337,433 | .,,. | | | ********* | -1 |
| Type-A | sq.m | 40,000 | 3,150 | 59,800 | 62,950 | 132,300 | 235,250 | 18,400 | 58,300 | 76,700 |
| Type-B | SQ.m | 130,200 | | 0 | 4,670 | 0 | | 12,100 | | 12,100 |
| Revetment (H.W.C.) | Jq. | , | ., | · | ., | | | , | • | |
| Type-A | sq.m | Ó | 0 | 0 | 0 | 63,700 | 63,700 | 0 | 8,390 | 8,300 |
| Type-B | sq m | 53,300 | 23,700 | 79,800 | 103,500 | 0 | 156,800 | 0 | | 0 |
| Total of (5) | so m | 223,500 | 31,520 | 139,600 | 171,120 | 196,000 | 590,620 | 30,500 | 66,600 | 97,100 |
| (6) Groin (L.W.C.) | • 4 | | | • | • | | | | • | |
| Type-A | pc. | 460 | 0 | . 0 | 0 | 198 | 658 | .0 | 244 | 244 |
| Type-B | pc. | 0 | 0 | 0 | . 0 | . 0 | 0 | 0 | | 0 |
| Groin (H.W.C.) | • | | | | | | | | | |
| Type-A | pc. | 0 | 0 | 0 | 0 | 148 | 148 | 0 | 0 | . 0 |
| Type-8 | pc. | 0 | 0 | . 0 | 0 | 152 | 152 | 0 | υ | ` ' 0 |
| Totoal of (6) | pc. | 460 | . 0 | 0 | 0 | 498 | 958 | 0 | 244 | 244 |
| (7) Sluice Way | - | | | | | | | | | . 0 |
| Type-A | pc. | 2 | . 1 | ì | 2 | . 3 | 7 | 0 | . 2 | 2 |
| Type-B | pc. | . 8 . | 0 | . 0 | 0 | 1 | 9 | Û | 0 | 0 |
| Totoal of (7) | pc. | 10 | . 1 | 1 | . 2 | 4 | 16 | 0 | - 2 | . 2 |
| (8) Water Gate | | | | | | | | | | 0 |
| Type-A | pc. | 0 | 0 | 0 | 0 | 0 | . 0 | 0 | 0 | 0 |
| Type-B | pc. | 2 | 0 | 0 | 0 | Ō | | 1 | : 0 | 1 |
| Totoal of (8) | pc. | 2 | 0 | 0 | . 0 | . 0 | . 2 | . 1 | 0 | . 1 |
| (9) Bridge | • | | | | | | | | | 0 |
| Newly Const. | sq.m | 45,000 | . 0 | 0 | 0 | 6,750 | 51,750 | 0 | 13,500 | 13,500 |
| Rehabilit. Demolishment | sq.m | 0 | 0 | . 0 | 0 | 0 | 0 | . 0 | . 0 | 0 |
| Concrete | CU.A | 5,800 | 0 | 0 | 0 | 3,800 | 9,600 | . 0 | 2,500 | 2,500 |
| Metal | ton | 1,060 | 0 | 0 | . 0 | 1,300 | 2,360 | 0 | . 0 | 0 |
| (10) Fixed Heir | pc. | 0 | 0 | 1 | 1 | 0 | . 1 | .0 | . 0 | . 0 |
| (11) Others | i.s | 1 | . 0 | . 0 | 0 | 0 | 1 | 0 | . 0 | : 0 |

(File cord : WO-AGNF2)

Table 7.1 (3/5) WORK QUANTITIES OF RIVER IMPROVEMENT OF AGNO RIVER FOR ALTERNATIVE FRAMEWORK PLANS

Kiver

: Agno and Tarlac rivers

Study

: Framework Plan

Alternative : River Improvement, Natural Retarding Baisn and Dams (AG-3)

Return Period: 1/100 - year

| | | | Λg | no River Ha | in Stream | | | 1 | îarlac River | |
|------------------------|-------|-----------------|------------------|-----------------|--------------------------|--------------------|---------------------------|--------------------|--------------------|------------------------|
| Hork Item | Vnit | Lower Agno | Po | ponto Stret | ch | Upper Agno | Total of | Conf luence | Upper Stretch | Total of |
| | | RH-AG282 (1) | Bayambang (2) | Floodway (3) | Sub-total (2)+(3)-(4) | AG309-AG473 (5) | Agno River (1)+(4)+(5) | AG180-TA200 (6) | TA200-TA265 (7) | Tarlac Rive (6)+(7) |
| (1) Excavation 1 | cu.m | 15,275,000 | 650,000 | 6,800,000 | 7,450,000 | 3,300,000 | 26,025,000 | 2,600,000 | 1,700,000 | 4,300,000 |
| Excavation 2 | çu.m | . 0 | 0 | 0 | 0 | 2,850,000 | 2,850,000 | 0 | 0 | 1,300,000 |
| Totoal of (1) | cu.m | 15,275,000 | 650,000 | 6,800,000 | 7,450,000 | 6,150,000 | 28,875,000 | 2,600,000 | 1,700,000 | 4,300,000 |
| (2) Oredging | Cu.m | 17,075,000 | 0 | 0 | 0 | 0 | 17,075,000 | 0,000,000 | 1,700,000 | 4,300,000 |
| (3) Embankment 1 | | | | | | . • | 17,075,000 | | U | v |
| Left Dike | cu.m | 1,754,400 | 343,000 | 288,100 | 631,100 | 1,162,000 | 3,547,500 | 558,000 | 215 500 | 020 000 |
| Right Dike | CG.W | 849,900 | 374,000 | 1,431,900 | 1,805,900 | 1,704,000 | | 220,000 | 315,500 | 873,500 |
| Embankment 2 | Cuin | 045,300 | 374,000 | 1,451,100 | 1,005,900 | 1,704,000 | 4,359,800 | U | 481,600 | 481,600 |
| | | 4 506 600 | | _ | | *** | | | | |
| Left Dike | cu.m | 4,586,900 | 0 | 0 | . 0 | 443,000 | 5,029,900 | 0 | 0 | . 0 |
| Right Dike | Cu-m | 4,932,800 | 0 | 0 | 0 | 2,500,000 | 7,432,800 | . 0 | 0 | Q |
| Totoal of (3) | cu.m | 12,124,000 | 717,000 | 1,720,000 | 2,437,000 | 5,809,000 | 20,370,000 | 558,000 | 797,100 | 1,355,100 |
| (4) Sodding | Cu.m | 3,265,500 | 237,200 | 402,000 | 639,500 | 1,991,300 | 5,896,000 | 153,200 | 979,800 | 1,133,000 |
| (5) Revetment (L.H.C.) | | | | | | | • | | | |
| Type-A | sq.m | 40,000 | 3,150 | 59,800 | 62,950 | 132,300 | 235,250 | 18,400 | 58,300 | 76,700 |
| Type-9 | sq.m | 130,200 | 4,570 | 0 | 4,670 | 0 | 134,870 | 12,100 | . 0 | 12,100 |
| Revetment (H.W.C.) | | 1 | | | | | | | | |
| Type-A | sq.m | 0 | . 0 | . 0 | 0 | 63,700 | 63,700 | . 0 | 6.800 | 6.800 |
| Type-B | m.pz | 50,600 | 23,700 | 79.800 | 103,500 | 0 | 154,100 | 0 | 0,000 | 0,000 |
| Totoal of (5) | sq.m | 220,800 | 31,520 | 139,600 | 171,120 | 196,000 | 587,920 | 30,500 | 65,100 | 95,600 |
| (6) Groin (L.H.C.) | - 4 | | | , | .,-,,,,, | ,,,,,, | 407,520 | 30,300 | 03,100 | 33,000 |
| Type-A | DC. | 460 | . 0 | . 0 | 0 | 198 | 658 | Ò | 244 | 244 |
| Type-B | DC. | 0 | Ö | 0 | . 0 | 190 | 036 | | | 244 |
| Groin (H.W.C.) | pc, | ··· V. | | . υ | | v | ٠,٠ | 0 | .0 | . 0 |
| | 50 | . 0 | 0 | | | | | _ | _ | 100 |
| Type-A | pc. | . 0 | 0 | 0 | 0 | 148 | 148 | 0 | 0 | 0 |
| Туре-В | p¢. | | | . 0 | 0 | 152 | 152 | 0, | . 0 | 0 |
| Totoal of (5) | pc. | 460 | 0 | 0 | 0 | 498 | 958 | 0 | 244 | 244 |
| (7) Sluice Way | | | | | 2 | | | | | 0 |
| Type-A | pc. | 2, | . 1 | . 1 | . 2 | . 3 | 7 | 0 | 2 | 5 |
| Туре-В | DC. | 8 | 0 | 0 | 0 | 1 | 9. | 0 | 0 | . 0 |
| Totoal of (7) | pt. | 10 | 1 | . 1 | 2 | 4 | 16 | .0 | . 2 | 2 |
| (8) Water Gate | | | | | | | | | | . 0 |
| Type-A | pc. | 0 | . 0 | 0 | 0 | 0 | . 0 | . 0 | 0 | i i i |
| Type-B | pc. | 2 | 0 | . 0 | 0 | 0 | 2 | 0 | | ū |
| Totoal of (8) | pc. | 2 | ŏ | | • | ő | 2 | 0 | | |
| (9) Bridge | p. i | | • | Ü | v | U | | v | ٧. | ::: 6 |
| Hewly Const. | SQ.ms | 45.000 | 0 | ø | G | 6,750 | 51,750 | ·a | 13.500 | |
| Rehabilit. | • | 45,000 | 0 | 0 | | 0,750 | 51,750 | 0 | 13,500 | 13,500 |
| Demolishment | sq.m | . 0 | U | . 0 | U | u | U | Ü | v | |
| · · | | £ 5 | _ | _ | _ | | * 46- | _ | | |
| Concrete | CU m | 5,800 | 0 | 0 | | 3.800 | 9,600 | . 0 | | 2,500 |
| Heta1 | ton | 1,060 | 0 | 0 | - | 1,300 | 2,360 | 0 | = | |
| (10) Fixed Helr | pc. | 0 | 0 | 1 | | 0 | 1 | 0 | _ | C |
| (11) Others | L.S | 1 | . 0 | 0 | 0 | 0 | 1 | . 0 | 0 | C |

(File cord : WQ-AGNF3)

Table 7.1 (4/5) WORK QUANTITIES OF RIVER IMPROVEMENT OF AGNO RIVER FOR ALTERNATIVE FRAMEWORK PLANS

River Study

: Agno and Tarlac rivers : Framework Plan

Alternative : River Improvement and Dams
Return Period : 1/100 - year, Q-16,200 m3/sec

| e de la companya de | | 4 | Agi | no River Ha | in Stream | · · · · · · · · · · · · · · · · · · · | | | Tartac River | |
|---|-------|-----------------|------------------|-----------------|--------------------------|---------------------------------------|---------------------------|--------------------|--|------------------------|
| Hork Item | Unit | Lower Agno | Poj | ponto Stret | ch | Upper Agno | Total of | Conf luence | Upper Stretch | ĭotal of |
| | | RM-AG282 (1) | Bayambang (2) | Floodway (3) | Sub-tota1 (2)+(3)-(4) | AG309-AG473 (5) | Agno River (1)+(4)+(5) | AG180-TA200 (6) | TA200-TA265 (7) | Tarlac Rive (6)+(7) |
| (1) Excavation 1 | Cu.m | 15,275,000 | 0 | 6,800,000 | 6,800,000 | 3,300,600 | 25,375,000 | 2,600,000 | 1,700,000 | 4,300,000 |
| Excavation 2 | ÇU,M | 15,275,000 | ō | 0 | 0 | 2,850,000 | 2.850.000 | 0 | | 0 |
| Totoal of (1) | cu.sı | 15,275,000 | Ŏ | 6,800,000 | 6,800,000 | 6,150,000 | 28,225,000 | 2,600,000 | | 4,300,000 |
| | | 17,075,000 | 0 | 0,000,000 | 0,000,000 | 0,130,000 | 17,075,000 | 0.000,000 | | 1,000,000 |
| (2) Dredging (3) Embankment 1 | cu.m | | | | • | | | | | |
| Left Dike | Cu.m | 2,259,500 | 483,500 | 1,609,600 | 2,093,100 | 1,162,000 | 5,514,600 | 558,000 | A Company of the Comp | 873,500 |
| Right Dike | cu.m | 1,341,600 | 0 | 852,100 | 852,100 | 1,704,000 | 3,897,700 | 1,046,000 | 481,600 | 1,527,600 |
| Embankment 2 | | • | | | | 15 | | | | |
| Left Dike | cu.m | 6,003,000 | . 0 | 0 | . 0 | 443,000 | 5,446,000 | 0 | 0 | ., . · · · (|
| Right Dike | cu.m | 6,306,200 | 0 | . 0 | . 0 | 2,500,000 | 8,806,200 | 0 | 0 | |
| Totoal of (3) | cu.m | 15,910,300 | 483,500 | 2,461,700 | 2,945,200 | 5,809,000 | 24,664,500 | 1,604,000 | 797,100 | 2,401,100 |
| (4) Sodding | cu.m | 3,776,300 | . 0 | 717,700 | 717,700 | 1,991,300 | 6,485,300 | 437,800 | 979,800 | 1,417,600 |
| (5) Revetment (L.W.C.) | | ********* | | | | | | | • | 1.0 |
| Type-A | sq.m | 40,000 | . 0 | 59,800 | 59,800 | 132,300 | 232,100 | 18,400 | 58,300 | 76,700 |
| Туре-В | sq.m | 130,200 | . 0 | 0 | 0 | 0 | 130,200 | 12,100 | | 12,100 |
| Revetment (H.W.C.) | | 107,117 | | | | | | | • | |
| Type-A | sq.m | . 0 | 0 | 0 | 0 | 63,700 | 63,700 | 0 | 6,800 | 6,800 |
| Type-8 | sq.m | 63,200 | 23,700 | . 0 | 23,700 | 0 | 86,900 | . 0 | | 0 |
| Totoal of (5) | sq.m | 233,400 | 23,700 | 59,800 | 83,500 | 196,000 | 512,900 | 30,500 | 65,100 | 95,600 |
| (6) Grain (L.H.C.) | 24.01 | 233,400 | 20,100 | 07,000 | 55,552 | | | | | |
| Type-A | pc. | 460 | . 0 | 0 | 0 | 198 | 658 | 0 | 244 | 244 |
| Туре-В | pc. | 0 | . 0 | 0 | Ö | 0 | 0 | 0 | . 0 | |
| Groin (H.W.C.) | pc. | • | - | | | • | | • | | |
| Type-A | D.C. | . 0 | . 0 | 0 | 0 | 148 | 148 | 0 | 0 | |
| ** | pc. | 0 | 0 | . 0 | 0 | 152 | 152 | ŏ | | |
| Type-8 | pc. | | | | | | 14 | | | a · |
| Totoal of (6) | pc. | 460 | 0 | 0 | 0 | 498 | 958 | 0 | 244 | 244 |
| (7) Stutce Way | | | | | | | | | : | |
| Type-A | pc. | 2 | 1 | 1 | 2 | . 3 | . 7 | 0 | 2. | |
| Type-B | pc. | 8 | 2 | 0 | 2 | 1 | 11 | 0 | . 0 | |
| Totoal of (7) | pc. | 10 | 3 | 1 | 4 | 4 | 18 | 0 | 2 . | 2 |
| (8) Water Gate | | | | | | | | | | (|
| Type-A | pc. | 0 | . 0 | 0 | 0 | 0 | 0. | 0 | . 0 | C |
| Туре-8 | pc. | 2 | 0 | 0 | . 0 | . 0 | 2 | 1 | 0 | 1 |
| Totoal of (8) | pc. | 2 | 0 | 0 | 0 | 0 | Ż | 1 | 0 | |
| (9) Bridge | • . | | | | | | | | | |
| Kewly Const. | sq.m | 45,000 | 0 | 0 | 0 | 6,750 | 51,750 | . 0 | 13,500 | 13,500 |
| Reliabilit. | sq.m | 0 | 0 | . 0 | 0 | 0 | 0 | ò | 0 | C |
| Demolishment | • | | | | | | | | | Ç |
| Concrete | cu.m | 5,800 | 0 | 0 | 0 | 3,800 | 9,600 | 0 | 2,500 | 2,500 |
| Metal | ton | 1,060 | 0 | 0 | . 0 | 1,300 | 2,360 | . 0 | 0 | |
| 10) Fixed Keir | DC. | 0 | 0 | . 0 | 0 | . 0 | 0 | 0 | 0 | |
| 11) Others | ί.\$ | 1 | 0 | 0 | 0 | : 0 | 1. | 0 | 0 | |

(File cord : WO-AGNF4)

Table 7.1(5/5) WORK QUANTITIES OF RIVER IMPROVEMENT OF AGNO RIVER FOR ALTERNATIVE PLANS

Filename : Tributaries of Agno River

Study : Framework Plan

Alternative : River Inprovement Only

Return Period : 50 - year

| | ** | المامالة | | Tribut | aries of Agno | River | |
|------|--------------------|---------------|-------------------|-----------------|--|--------------------|-------------------------|
| | Work Item | Unit | Camiling River | Banila River | Viray-Dipalo River | Ambayaoan River | Total of Tributaries |
| (1) | Excavation 1 | cu.m | 845,000 | 919,000 | 185,000 | 0 | 1,949,000 |
| | Excavation 2 | cu.m | . 0 | 49,000 | | 85,000 | 134,000 |
| 1 | Total of (1) | Cu.m | 845,000 | 968,000 | | 85,000 | |
| (2) | Dredging | cu.m | . 0 | 0 | | 0 | 0 |
| | Embankment 1 | | | | | | |
| , | Left Dike | cu.m | 642,600 | 797,200 | 62,800 | 164,800 | 1,667,400 |
| | Right Dike | cu.m | 586,200 | 854,600 | • | 167,700 | 1,689,900 |
| | Embankment 2 | * | | | | | |
| | Left Dike | cu.m | 0 | 13,000 | 0 | . 0 | 13,000 |
| | Right Dike | cu.m | 0 | 0 | | 0 | 0 |
| | Total of (3) | cu.m | 1,228,800 | 1,664,800 | 144,200 | 332,500 | 3,370,300 |
| (4) | Sodding | cu.m | 537,100 | 827,200 | | 171,500 | · · |
| | Revetment (L.W.C.) | | | | | - • | |
| | Type-A | sq.m | 48,100 | 67,000 | 39,700 | 15,900 | 170,700 |
| | Туре-В | sq.m | . 0 | 0 | | 3,500 | |
| | Revetment (H.W.C.) | | | | | | |
| | Type-A | sq.m | 11,800 | 0 | 220 | 4,000 | 16,020 |
| | Туре-В | sq.m | 0 | 0 | | 0 | • |
| | Total of (5) | sq.m | 59,900 | 67,000 | | 23,400 | |
| (6) | Groin (L.W.C.) | 4 | , , | .,, | 35,525 | | |
| \-/ | Type-A | pc. | 276 | 420 | 286 | 88 | 1,070 |
| | Туре-В | pc. | 0 | 0 | | . 0 | - |
| • | Groin (H.W.C.) | | • | | | _ | |
| | Type-A | pc. | 0 | 0 | 0 | 0 | . 0 |
| V | Туре-В | pc. | Ō | Ō | | 0 | |
| | Total of (6) | pc. | 276 | 420 | | 88 | |
| (7) | Sluice Way | | 2.0 | 120 | 200 | • | 2,0.0 |
| (// | Type-A | pc. | 1 | 14 | 4 . | 4 | 23 |
| | Type-8 | pc. | 3 | 0 | | 0 | |
| | Total of (7) | pc. | 4 | 14 | _ | 4 | 26 |
| (8) | Water Gate | P -0-0 | | • | • | • | |
| (4) | Туре-А | pc. | 0 | 0 | . 0 | 0 | 0 |
| | Туре-В | pc. | · 0 | 0 | | 0 | 0 |
| | Total of (8) | pc. | Ô | 0 | | 0 | 0 |
| (9) | Bridge | PVV | | | ŭ | • | • |
| (3) | Newly Const. | sq.m | 2,300 | 8,600 | 6,200 | 3,000 | 20,100 |
| | Rehabilit. | sq.m | 2,500 | 0,000 | | 0,000 | |
| | Demolishment | | | . • | | v | J |
| | Concrete | cu.m | 1,100 | 2,300 | 600 | 200 | 4,200 |
| | Metal | ton | 0 | 2,300 | | 200 | |
| (10) | Fixed Weir | pc. | 0 | 0 | | 0 | |
| | Others | L.S. | 0 | 1 | and the second s | 0 | |
| (-1) | | | | | · · · · · · · · · · · · · · · · · · · | | |

Table 7.2 (1/2) WORK QUANTITIES OF RIVER IMPROVEMENT OF ALLIED RIVER FOR ALTERNATIVE FRAMEWORK PLANS

River

Study

: Allied rivers : Franchork Plan : River Improvement with Bued Closure Dike (AL-1) Alternative

Return Period : 1/50 - year

| | | | | | | | Allied River | | | | | Total of |
|------------------------|---|-------------------------|------------------|-------------------|-------------------|-----------------------|--------------|--------------------------|---------------|-------------------|------------------|-----------------|
| Nork Item | Unit | | | Panto - Sino | calan River | | | (| ayanga - Pa | talan River | . 1 <u> </u> | Allied River |
| | | *Panto- Sinocalan R. | Dagupan River | Ingalera River | Kacalong River | Binalonan Floodway | Sub-total | **Cayanga- Patalan R. | Bued River | Aloragat River | Sub-total | RIFE |
| (1) Excavation 1 | cu.m | 2,968,000 | 1,399,000 | 1,815,000 | 194,000 | 0 | 6,376,000 | 1,549,000 | 183,800 | 300,000 | 2,032,800 | 8,408,80 |
| Excavation 2 | cu.m | 0 | | 0 | . 0 | 0 | 0 | 0 | 188,000 | . 0 | 188,000 | 188,00 |
| Total of (1) | CU.M | 2,958,000 | 1,399,000 | 1,815,000 | 194,000 | 0 | 6,376,000 | 1,549,000 | 371.800 | 300.000 | 2.220,800 | 8,596,80 |
| (2) Dredging | CU.M | 113,000 | . 0 | 0 | 0 | . 0 | 113,000 | 390,000 | 0 | 0 | 390,000 | 503,00 |
| (3) Embankment 1 | | • | | | | | | | | | | |
| Left Dike | cu.m | 1,508,000 | 1,428,100 | 871,600 | 221,300 | 0 | 4,029,000 | 547,800 | 57,400 | 0 | 605,200 | 4,634,20 |
| Right Dike | ÇU.M | 1,508,000 | 1,428,100 | 871,600 | 221,300 | 0 | 4,029,000 | 547,800 | 57,400 | 0 | 605,200 | 4,634,20 |
| Embankment 2 | | | | | 1.5 | | | | | | | |
| Left Dike | cu.m | · 5 0 | Ö | 0 | 0 | . 0 | 0 | 0 | 39,600 | 0 | 39,600 | 39,60 |
| Right Dike | cu.n | 0 | . 0 | 0 | 0 | . 0 | 0 | 0 | 80,500 | 0 | . 60,600 | 80,60 |
| Total of (3) | Cu.m | 3,016,000 | 2,856,200 | 1,743,200 | 442,600 | . 0 | 8,058,000 | 1,095,600 | 235,000 | . 0 | 1,330,600 | 9,388,60 |
| (4) Sodding | cu.m | 1,489,600 | 1,228,000 | 1,179,500 | 257,000 | 0 | 4,154,100 | 538,700 | 151,500 | . 0 | 690,200 | 4,844,30 |
| (5) Revetment (L.W.C.) | **** | .,, | | | | | | | | | \$ 100 N | |
| Type-A | sq.m | 73,000 | 33,800 | 18,900 | 37,700 | · D | 163,400 | 75,200 | 39,700 | 40,800 | 155,700 | 319.1 |
| Type-B | sq.m | 38,000 | 43,400 | 124,200 | 0.,, | . 0 | 205,600 | 30,300 | 0 | 0 | 30,300 | 235,9 |
| Revetment (H.W.C.) | 24.11 | | 13,100 | 10,000 | | | | | - | | | |
| * 12 | | 30,500 | o o | 0 | 0 | 0 | 30,500 | 5.800 | 1,700 | 0 | 7,500 | 38.0 |
| Type-A | sq.m | | | 0 | 0 | o. | 30,300 | 0.00 | 0 | . 0 | 0 | 3010 |
| Type-8 | zd-w | 0 | 0 | | | . 0 | 399,500 | 111,300 | 41,400 | 40,800 | 193,500 | 593,0 |
| Total of (5) | sq.a | 141,500 | 77,200 | 143,100 | 37,700 | v | 288,500 | 111,300 | 41,400 | 40,000 | 193,300 | 393,00 |
| (6) Groin (L.W.C.) | 13.33 | | | | | .: 0 | | 11 11 045 | 281 | 272 | 1,095 | 2,0 |
| Type-A | pc. | . 566 | 100 | 242 | 54 | - | 962 | 542 | 201 | 212 | 1,033 | 2.0 |
| Туре-В | pc. | 0 - | 0 | 0 | 0 | 0 | 0 | . 0 | . • | V. | U | |
| Groin (H.W.C.) | | | | | | : _: | 0 | . 0 | . 0 | .0 | . 0 | |
| Type-A | DC. | | 0 | 0 | 0 | . 0 | - | | 0 | • | 0 | |
| Турс-В | PC . | 0 | . 0 | , 0 | 0 | 0 | 0 | 0 | _ | 0 | | 7.0 |
| Total of (6) | DC | 566 | 100 | 242 | 54 | . 0 | 962 | 542 | 281 | 272 | 1,095 | 2,0 |
| 7) Sluice Way | | | | | | | | | **** | | | |
| Type-A | pc. | 16 | 4 | 8 | - 8 | (+ 0) | 36 | 10 | 6 | 0 | 16 | |
| Type-B | pc. | 0 | 3 | 0 | Đ | , O | 3 | Đ | a | Đ | Ď | |
| Total of (7) | pc. | - 16 | 7 | 8 | 8 | 0 | 39 | 10 | 6 | 0 | - 15 | : |
| 8) Water Gate | | | | | | | | | 1. | 1 1 1 1 1 1 | | |
| Type-A | pc. | . 0 | ,0 | 0 | . 0 | 0 | 0 . | 0 | 0 | 0 | 0 | |
| Туре-В | pc. | . 0 | 0 | 0 | 0 | Ó | 0 | 0 | .0 | .0 | 0 | |
| fotal of (8) | pc. | 0 | . 0 | 0 | 0 | . 0 | 0 | Q. | 0 | 0 | 0 | |
| 9) Bridge | | | | | | | 1,5 | | | | +. | |
| Newly Const. | a.pz | 8,000 | 3,905 | 3,900 | 193 | 0 | 15,998 | 1,200 | 3,000 | 263 | 4, 163 | 20,4 |
| Rehabilit. | \$Q.m | 338 | . 0 | 0 | 413 | . 0 | 751 | 2,675 | 0 | .0 | 2,675 | 3,4 |
| Demolishment | • | 3 | | | | | | | | | 7.3 | |
| Concrete | Eu .in | 4.590 | 1,200 | 1,700 | 1.030 | 0 | 8,520 | 1,700 | 300 | 200 | 2,200 | 10,7 |
| Hetal | ton | 1,350 | 0 | 0 | 0 | ō | 0,520 | . 0 | 0 | . 0 | 0 | - / • · |
| | | . 0 | 0 | 0 | 0 | . 0 | 0 | . 0 | ō | ŏ | ō | |
| 10) Fixed Keir | pc. | 0 | 0 | 0 | 0 | 0 | 0 | . 0 | 1 | ŏ | ĭ | |
| 11) Others | L.S. | U | U | Ų | Ų | U | | v | | | | |

Remarks: * Panto-Singcalan River Consists of Panto, Marusay, Singcalan, Tagunising and Tuboy Rivers.
** Cayanga-Patalan River consists of Cayanga, Patalan and Angalacan Rivers.

(File cord : WQ-ALEA1)

Table 7.2 (2/2) WORK QUANTITIES OF RIVER IMPROVEMENT OF ALLIED RIVER FOR ALTERNATIVE FRAMEWORK PLANS

River

: Allied rivers

: Framework Plan

Alternative : River Improvement with Bued Closure Dike/Dinalonan Floodway (AL-2)

| • | | | | | | | Allied River | | | | | |
|-------------------------------------|------|---|---------------------------------------|-------------------|-------------------|---------------------------|---------------------------------------|--------------------------|-----------------|-------------------|-----------|-----------------------------|
| Work Item | Unit | *************************************** | · · · · · · · · · · · · · · · · · · · | Panto - Sino | alan River | | | C | ayanga - Pa | talan River | | Total of Allied River |
| | | *Panto- Sinocalan R. | Qagupan River | ingalera River | Hacalong River | 8 In a lonan F loodway | Sub-total | **Cayanga- Patalan R. | . Bued River | Aloragat River | Sub-total | River |
| (1) Excavation 1 | cu.m | 1,699,000 | 1,399,000 | 1,815,000 | 194,000 | 604,800 | 5,711,800 | 1,689,500 | 183,809 | 300,000 | 2,173,300 | 7,885,10 |
| Excavation 2 | CG.M | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 168,000 | . 0 | 188,000 | 188,00 |
| Total of (1) | cu.m | 1,699,000 | 1,399,000 | 1,815,000 | 194,000 | 604,800 | 5,711,800 | 1,689,500 | 371,800 | 300,000 | 2,361,300 | 8,073,10 |
| (2) Oredging (3) Embankment 1 | CQ.M | 38,000 | 0 | 0 | 0 | 0 | 38,000 | 440,000 | 0 | 0 | 440,000 | 478,00 |
| Left Dike | cu.m | 660,400 | 1,350,000 | 871,600 | 221,300 | 154,000 | 3,257,300 | 768,800 | 57,400 | .· O | 826,200 | 4,083,50 |
| Right Dike Embankment 2 | CU.D | 660,400 | 1,350,000 | 871,600 | 221,300 | 154,000 | 3,257,300 | 768.000 | 57,400 | 0 | 825,200 | 4,083,50 |
| Left Olke | çu.n | 0 | 0 | 0 | O. | 0 | 0 | . 0 | 39,600 | 0 | 39,600 | 39,60 |
| Right Oike | Cu.m | 0 | 0 | 0 | 0 | 0 - | 0 | 0 | 80,600 | O | 80,600 | 80,60 |
| Total of (3) | Cu.M | 1,320,800 | 2,700,000 | 1,743,200 | 442,600 | 308,000 | 6,514,600 | 1.537,600 | 235,000 | 0 | 1,772,600 | 0,287,2 |
| 4) Sodding 5) Revetment (L.W.C.) | CU.M | 821,000 | 1,161,000 | 1,179,500 | 257,000 | 107,000 | 3,525,500 | 731,600 | 151,500 | 0 | 893,100 | 4,408,6 |
| Type-A | sq.m | 73,000 | 33,800 | 18,900 | 37,700 | 76,800 | 240,200 | 75,200 | 39,700 | 40,800 | 155,700 | 395,9 |
| Type-B Revetment (H.W.C.) | sq.m | 38,000 | 43,400 | 124,200 | 0 | 0 | 205,600 | 30,300 | 0 | ,0 | 30,300 | 235,9 |
| Type-A | sq.m | 24,600 | 0 | 0 | . 0 | 0 | 24,600 | 6,100 | 1,700 | 0 | 7,800 | 32,4 |
| Type-8 | sq.m | 0 | .0 | .0 | 0 | 0 | . 0 | 0 | 0 | 0 | 0 | |
| Total of (5) 6) Groin (L.M.C.) | sq.m | 135,600 | 77,200 | 143,100 | 37,700 | 76,800 | 470,400 | 111,600 | 41,400 | 40,800 | 193,800 | 664.2 |
| Type-A | pc. | 358 | 100 | 242 | 54 | 0 | 754 | 542 | 281 | 272 | 1,095 | 1,8 |
| Type-8 Groin (II.R.C.) | pc. | 0 | . 0 | 0 | 0 | 0 | • • • • • • • • • • • • • • • • • • • | 0 | . 0 | 0 | 0 | |
| Type-A | р¢. | . 0 | 0 | C | . 0 | . 0 | 0 | 0 | 0 | . 0 | 0 | |
| Type-B | pc. | 0 | 0 | 0 | 0 | 0 | . 0 | 0 | 0 | 0 | 0 | |
| Total of (6) 7) Slutce Way | pc. | 358 | 100 | 242 | 54 | . 0 | 754 | 542 | 281 | 272 | 1,695 | 1,8 |
| Type-A | pc. | 8 | 4 | 8 | . 8 | | 30 | 10 | 6 | 0 | 16 | |
| Туре-В | ж. | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | .0 | 0 | |
| Total of (7) | pc . | 8 | 7 | 8 | 8 | 2 | 33 | 10 | 6 | 0 | 16 | |
| B) Kater Gate | | | _ | _ | _ | | _ | _ | _ | | _ | |
| Type-A | pc. | 0 | 0 | 0 | 0 | 0 | . 0 | 0 | 0 | 0 | 0 | |
| Type-0 | bc. | 0 | . 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Total of (8) | pc. | . 0 | Û | 0 | 0 | 0 | 0 | . 0 | 0 | | 0 | |
| 9) Bridge | | 2 | | | | | | | | 1 | | |
| Newly Const. | sq.m | 6,195 | 3,905 | 3,900 | 193 | 780 | 14,973 | 1,135 | 3,000 | 263 | 4,398 | 19,3 |
| Rehabilit. | sq.m | . 0 | 0 | 0 | 413 | 0 | 413 | 3,040 | 0 | 0 | 3,040 | 3,4 |
| Depol ishment | : . | | | | | | | | | | | |
| Concrete | cu.m | 3,430 | 1,200 | 1,709 | 1,030 | 400 | 7,760 | 1,150 | 300 | 200 | 1,650 | 9,4 |
| Heta! | ton | 0 | 0 | Ċ | 0 | 0 | 0 | 0 | . 0 | 0 | 0 | |
| 0) Fixed Welr | pc. | . 0, | . 0 | 0 | 0 | . 0 | 0. | 0 | 0 | 0 | 0 | |
| 11) Others | L.S. | 0 | 0 | 0 | . 0 | . 1 | | 0 | 1 | . 0 | 1 | |

Remarks: * Panto-Sinocalan River Consists of Panto, Marusay, Sinocalan, Tagumising and Tuboy Rivers.

** Cayanga-Patalan River consists of Cayanga, Patalan and Angalacan Rivers.

(CE-AG100)

Table 7.3(1/2) PROJECT ECONOMIC COST OF AGNO RIVER FOR ALTERNATIVE FRAMEHORK PLANS

| | | | ž | Main Agno River | ver | | , , ; * | Tarlac River | ٤., | Dams | |
|--------|--|--------------------------|-----------|---------------------|-------------------------------------|------------|--|--------------|--------------------------|--------------------|----------------|
| | Alternatives | | ! | Poponto Stretch | | 9 1 2 2 | | | | | į |
| Return | | Lower Lower Agno River B | Bayambang | Poponto Floodway | - upper Agno Rive AG309-AG473 | Agno River | Conf luence AG180-TA200 TA200-TA265 | TA200-TA265 | iotai oi Tarlac River | and and L'Odonnell | urand Total |
| 1/100 | 1/100 Alternative-AG1 River Improvement | 8,070 | 80 | 1,027 | 2,282 | 11,472 | 741 | 845 | 1,587 | f | 13,059 |
| 1/100 | 1/100 Alternative-AG2 River Improvement and Matural Retarding Basin | 7,170 nd asin | 166 | 1,082 | 2,282 | 10,700 | 442 | 846 | 1,288 | • | 11,988 |
| 1/100 | Alternative-AG3 River Improvement, Natural Retarding Basin and Dams | 6,955 asin | 166 | 1,082 | 2,282 | 10,485 | 393 | 658 | 1,061 | 1,811 | 13,357 |
| 1/100 | 1/100 Alternative-AG4 River Improvement and Dams | 7,800 | 83 | 1,027 | 1,027 | 11,202 | 597 | 999 | 1,265 | 1,811 | 14,278 |

Table 7.3(2/2) PROJECT ECONOMIC COST OF AGNO RIVER FOR ALTERNATIVE FRAMEWORK PLANS

(Unit: Million Pesos)

| Return Period | · | Work Item | Camiling River | Banila River | Viray- Dipalo River | Ambayoan River | Totál |
|------------------|------|------------------------|-------------------|-----------------|---------------------------|-------------------|-------|
| 1/50 | Ι. | Main Construction Cost | | | | | |
| | | 1. Preparatory Works | 24 | 54 | 15 | ,9 | 102 |
| | | 2. Main Works | 239 | 543 | 148 | 92 | 1,022 |
| | | 3. Miscellaneous Works | 40 | 90 | 24 | 15 | 169 |
| | | Total of I. | 303 | 687 | 187 | 116 | 1,293 |
| | н. | Compensation | 30 | 69 | 19 | 12 | 130 |
| | III. | Administration | 17 | 38 | 10 | 6 | 71 |
| | IV. | Engineering Services | . 48 | 110 | 30 | 19 | 207 |
| 1 : 1 | ٧. | Physical Contingency | 53 | 119 | 32 | 20 | 224 |
| - | • | Grand Total | 451 | 1,023 | 278 | 173 | 1,925 |

Table 7.4(1/2) PROJECT ECONOMIC COST OF ALLIED RIVERS FOR ALTERNATIVE FRAMEWORK PLANS

Alternative-AL1: River Improvement with Sued Closure Dike

River : Panto-Sinocalan River

(Unit: Million Pesos)

| Return Period | | Work Item | Panto- Sinocalan River | Dagupan River | Ingalera River | Macalong River | · | Total |
|------------------|------|------------------------|------------------------------|------------------|-------------------|-------------------|---------|-------|
| 1/50 | Ι. | Main Construction Cost | | | | | | |
| • | | 1. Preparatory Works | 65 | 40 | 38 | 7 | | 150 |
| | | 2. Main Works | 652 | 401 | 375 | 71 | | 1,499 |
| | | 3. Miscellaneous Works | 108 | 66 | 62 | 12 | | 248 |
| | | Total of I. | 825 | 507 | 475 | 90 | · · · . | 1,897 |
| | II. | Compensation | 83 | 51 | 48 | 9 | | 191 |
| | III. | Administration | 45 | 28 | 26 | 5 | | 104 |
| | IV. | Engineering Services | 132 | 81 | 76 | 14 | | 303 |
| | ٧. | Physical Contingency | 143 | 88 | 82 | 16 | | 329 |
| 25.3 | | Grand Total | 1,228 | 755 | 707 | 134 | | 2,824 |

River: Cayanga-Patalan River

(Unit: Million Pesos)

| Return Period | | Work Item | Cayanga- Pataran River | Bued River | Aloragat River | | Total |
|------------------|------|------------------------|------------------------------|---------------|-------------------|---|-------|
| 1/50 | ı. | Main Construction Cost | | | | | ·. |
| | | 1. Preparatory Works | - 35 | 19 | 7 | | 61 |
| | | 2. Main Works | 354 | 190 | . 71 | | 615 |
| | | 3. Miscellaneous Works | 58 | 31 | 12 | - | 101 |
| | | Total of I. | 447 | 240 | 90 | | 777 |
| | II. | Compensation | 45 | 24 | 9 | | 78 |
| | III. | Administration | 25 | 13 | . 5 | | 43 |
| | IV. | Engineering Services | 72 | 38 | 14 | | 124 |
| | ٧. | Physical Contingency | 78 👵 | 42 | 16 | | 136 |
| | | Grand Total | 667 | 357 | 134 | | 1,158 |

Table 7.4(2/2) PROJECT ECONOMIC COST OF ALLIED RIVERS FOR ALTERNATIVE FRAMEWORK PLANS

Alternative-AL2: River Improvement with Bued Closure Dike and Binalonan Floodway

River : Panto-Sinocalan River

(Unit: Million Pesos)

| Return Period | • | Work Item | Panto- Sinocalan River | Dagupan River | Ingalera River | Macalong River | Binalona Floodway | Total |
|------------------|------|------------------------|------------------------------|------------------|-------------------|-------------------|----------------------|-------|
| 1/50 | I. | Main Construction Cost | | | | | | |
| | | 1. Preparatory Works | . 39 | 39 | 38 | 7 | 13 | 136 |
| | | 2. Main Works | 389 | 389 | 375 | 71 | 131 | 1,355 |
| | ٠. | 3. Miscellaneous Works | 64 | 64 | 62 | 12 | 22 | 224 |
| | | Total of I. | 492 | 492 | 475 | 90 | 166 | 1,715 |
| | 11. | Compensation | 49 | 49 | 48 | 9 | 17 | 172 |
| | III. | Administration | 27 | 27 | 26 | 5 | 9 | 94 |
| | IV. | Engineering Services | 79 | 79 | 76 | 14 | 27 | 275 |
| | ٧. | Physical Contingency | 85 | 85 | 82 | 16 | 29 | 297 |
| | | Grand Total | 732 | 732 | 707 | 134 | 248 | 2,553 |

River: Cayanga-Patalan River

(Unit: Million Pesos)

| Return Period | 2 - | Work Item | Cayanga- Patalan River | Bued River | Aloragat River | | | Total |
|------------------|-----|------------------------|------------------------------|---------------|-------------------|------|-----|-------|
| 1/50 | Ι. | Main Construction Cost | | | | H | | |
| | | 1. Preparatory Works | 40 | 19 | , 7 | | | 66 |
| | | 2. Main Works | 401 | 190 | 71 | | | 662 |
| | | 3. Miscellaneous Works | 66 | 31 | 12 | | | 109 |
| ÷ | | Total of I. | 507 | 240 | 90 | + %. | | 837 |
| | II. | Compensation | 51 | 24 | 9 | | | 84 |
| | ш. | Administration | 28 | 13 | 5 | | | 46 |
| | IV. | Engineering Services | 81 | 38 | 14 | | • | 133 |
| | ٧. | Physical Contingency | 88 | 42 | 16 | | · # | 146 |
| | | Grand Total | 755 | 357 | 134 | | | 1,246 |

Table 7.5 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR FRAMEWORK PLAN

River: AGNO RIVER Design Flood: 100-yr

| | | | : | Agno R. | | | 14. |
|--------------------------|------|--------------|----------------|-----------------|------------------|-----------------------|------------------------|
| ftem | Unit | RM - AG45 | AG45 - AG65 | AG65 - AG109 | AG109 - AG177 | AG177 - AG180+0.8k | * - 4 6 6 0 0 0 |
| Design Discharge | m3/s | 12300 | 12300 | 12300 | 11100 | 9900 | |
| Distance | m | 6850 | 9050 | 15150 | 10500 | 2200 | |
| Gradient of River Bed | - | 1/6500 | 1/6500 | 1/3500 | 1/2000 | 1/2000 | |
| River Width | m | 1500 | 1500 | 1500 | 1500 | 1500 | |
| Width of Channel Bed | m | 400-300 | 300 | 240 | 200 | 200 | |
| Dike Height (Ave.) | m | 4.7 | 5.3 | 6.2 | 5.6 | 4.8 | |
| Water Depth | m | 8.73-9.57 | 9.57-10.7 | 10.7 | 10.7~9.41 | 9,41-9.14 | |
| Low Channel Height(Ave.) | m | 6.5 | 6.5 | 6.5 | 6.5 | 6.0 | |

| | | Retarding 1 | > Floodway | Bayanba | Bayanbang 2> Agno R. | | | | |
|--------------------------|------|----------------------|---------------------|--------------------|----------------------|-----------|-----|--|--|
| Item | Unit | AG180+0.8k -AG314 | AG314 - AG320(b) | AG282(a)- AG307 | AG320(b) -AG351 | | | | |
| Design Discharge | m3/s | | 8200 | 1000 | 9200 | 8200 | | | |
| Distance | m | 6100 | 3600 | 10450 | 15300 | 7650 | | | |
| Gradient of River Bed | - | 1/1550 | 1/1550 | Existing | 1/1550 | 1/1000 | | | |
| River Width | m | | 1000-830 | Existing | 900~2500 | 1000-3200 | 4.5 | | |
| Width of Channel Bed | m | 180 | 180 | Existing | 180 | 180 | | | |
| Dike Height (Ave.) | m | 5.4 | 4.8 | 4.5-0.0 | 5.5 | 5.2 | | | |
| Water Depth | m | 9.14-7.66 | 7.66-8.00 | 8.0-4.0 | 8.0 | 8.0-5.5 | | | |
| Low Channel Height(Ave.) | m | 4.5 | 4.5 | 5.0 | 4.0 | 3.0 | 9 | | |

1>:Retarding Basin Stretch 2>:Bayanbang Stretch of Agno R.

| | | | Agno I | ξ. | | • | • | |
|-----------------------|-------|----------------|--------------------|------------------|------------------|---------------------------------------|----------|--|
| Item | Unit | AG367 AG414 | AG414 - - AG453 | AG453 - AG495 | AG459 - AG473 | 4 mm may not man and day 1997 per uni | . | |
| Design Discharge | m3/s | 8200 | 8200 | 6400 | 6400 | | | |
| Distance | m | 7700 | 5300 | 3000 | 6450 | 1 45 51 | | |
| Gradient of River Bed | _ | 1/700 | 1/370 | 1/370 | 1/210 | | | |
| River Width | II); | 1050-2500 | 1250-2400 | 1000-1900 | 300-1300 |) | | |
| Width of Channel Bed | m | 180 | 150 | 150 | 150 | | | |
| Dike Height | m | 4.0 | 3.3 | 2.8 | 2.8-4.0 | | | |
| Water Depth | m | 5.5 | 4.8 | 4.3 | 4.3-5.5 | | | |
| Low Channel Height | · · m | 3.0 | 3.0 | 3.0 | 3.0 | | * | |

Table 7.6(1/2) FEATURES OF DESIGN CHANNEL OF TRIBUTARIES OF AGNO RIVER FOR FRAMEWORK PLAN

River: TALRAC RIVER Design Flood: 100-yr

| | Ret | arding Basin | | Tarlac R. | | |
|-------------------------|------|---------------------|------------------|------------------|----------------------|------|
| Item | Unit | AG180+0.8k TA200 | TA200 - TA227 | TA227 - TA251 | TA251 - TARIS DAM | |
| Design Discharge | m3/s | - | 2600 | 2600 | 1750 | |
|)istance | m | 8100 | 13000 | 11800 | 4150 | |
| Gradient of River Bed | - | 1/1850 | 1/1300 | 1/760 | 1/692 | |
| River Width | m | | 1700-640 | 1600-600 | 600-270 | |
| lidth of Channel Bed | m | 160 | 160 | 160 | 140 | |
| like Height (Ave.) | m | 8.2 | 3.9 | 3.5 | 1.5 | 14 |
| later Depth | m | 8.9-4.82 | 4.82-4.0 | 4.0 | 4.0-3.5 | |
| ow Channel Height(Ave.) | m | 5.0-2.0 | 2.0 | 2.0 | 3.5 | |
| | | | | | | 2.45 |

River: CAMILING RIVER Design Flood: 50-yr

| | | er en er | en de la companya de | Camiling R. | | : | - |
|-------------------------|-------|--|--|------------------|------------------|------------------|------------------|
| Item | Unit | AG143+1.0k CA156+0.3k | CA156+0.3k - CA162 | CA162 - CA167 | CA167 - CA172 | CA172 - CA173 | CA173 - CA175 |
| Design Discharge | m3/s | 2200 | 1550 | 1550 | 1550 | 1150 | 1150 |
| Distance | m | 3550 | 4650 | 4300 | 4950 | 1300 | 2050 |
| Gradient of River Bed | | 1/2000 | 1/2000 | 1/1000 | 1/550 | 1/300 | Existing |
| River Width | Th. | 250 | 180 | 180 | 180 | 130 | 130 |
| Width of Channel Bed | n n | 60 | 50 | 50 | 50 | 35 | Existing |
| Dike Height (Ave.) | m | 5.3 | 4.2 | 3.6 | 2.8 | 1.8 | 1.8-0.0 |
| Water Depth | m | 8.86-7.71 | 7.71-7.5 | 7.5-7.1 | 7.1-5.42 | 5.42-5.22 | 5.22-4.8 |
| Low Channel Height(Ave. |) - m | 4.7 | 4.7 | 4.7 | 4.5 | 4.5 | 4.0 |

River: BANILA RIVER
Design Flood: 50-yr

| | | Banila R. | | | | | | | | |
|--------------------------|------|----------------------|-----------------------|------------------|------------------|------------------|------------------|--|--|--|
| · Item | Unit | AG349- AG349+3.7k | AG349+3.7k - 8N381 | BN381 - BN386 | BN386 - BN394 | BN394 - BN397 | BN397 - BN401 | | | |
| Design Discharge | m3/s | 1400 | 1400 | 950 | 440 | 440 | 340 | | | |
| Distance | m | 3700 | 8050 | 4550 | 7600 | 2900 | 4100 | | | |
| Gradient of River Bed | | 1/1295 | 1/835 | 1/520 | 1/265 | Existing | Existing | | | |
| River Width | m | 180 | 180 | 120 | 120 | 120 | 120 | | | |
| Width of Channel Bed | m | 30 | 30 | 20 | 10 | Existing | Existing | | | |
| Dike Height (Ave.) | m | 3.5 | 3.2 | 2.9 | 2.4 | 2.1 | 1.3 | | | |
| Water Depth | m | 7.5 | 7.0 | 7.0-6.42 | 6.42-3.14 | 3.14-1.5 | 1.5 | | | |
| Low Channel Height (Ave. |) m | 5.0 | 4.8 | 4.8 | 4.8-2.5 | 1.0 | 1.0 | | | |

Table 7.6(2/2) FEATURES OF DESIGN CHANNEL OF TRIBUTARIES OF AGNO RIVER FOR FRAMEWORK PLAN

River: VIRAY-DIPALO RIVEIVER

Design Flood: 50-yr

| | · | | Viray-Dipal | o R. | Viray R. | | |
|--------------------------|------|-----------------|-----------------|-----------------|----------------------|------------------------|----------------------|
| Item | Unit | AG414- VD425 | VD425- VD428 | VD428- VD430 | VD430- VD430+0.6K | VD430+0.6k -VD433 \ | VD433- /D434+0.5k |
| Design Discharge | m3/s | 750 | 750 | 750 | 750 | 370 | 370 |
| Distance | m | 2800 | 3100 | 2000 | 600 | 2400 | 1450 |
| Gradient of River Bed | _ | 1/375 | 1/300 | 1/250 | 1//127 | 1/127 | 1/75 |
| River Width | m | 380-290 | 320-270 | 320-260 | 300 | 150 | 150 |
| | m | 30 | 30 | 30 | 30 | 15 | 15 |
| Dike Height (Ave.) | m | 1.7 | 1.7 | 1.7 | 1.7 | 0.9 | 0.9 |
| Water Depth | m . | 4.0 | 4.0 | 4.0 | 4.0 | 2.9 | 2.9 |
| Low Channel Height (Ave. | | 3.3 | 3.3 | 3.3 | 3.3 | 2.8 | 2.8 |

State of the second

| | | | | Dipalo R. | | |
|--------------------------|------|----------------------|-----------------|-----------------|-----------------|-----------------|
| Item | Unit | VD430+0.6k -VD436 | VD436- VD437 | VD437- VD439 | VD439- VD441 | VD441- VD442 |
| Design Discharge | m3/s | 350 | 350 | 210 | 210 | 210 |
| Distance | m | 1500 | 700 | 1950 | 1950 | 1000 |
| Gradient of River Bed | _ | 1/170 | 1/125 | 1/125 | 1/80 | 1/68 |
| River Width | m | 100 | 100 | 100 | 100 | 100 |
| Width of Channel Bed | m | 15 | 15 | 10 | 10 | 10 |
| Dike Height (Ave.) | · m | 2.6 | 2.6 | 2.3 | 2.1 | 1.9 |
| Water Depth | m | 3.8 | 3.0 | 2.5 | 2.3 | 2.1 |
| Low Channel Height (Ave. |) m | 2.0 | 1.2 | 1.0 | 1.0 | 1.0 |

River: AMBAYOAN RIVER Design Flood: 50-yr

| | | | Ambayoan R. | | |
|--------------------------|------|----------------------|----------------------|----------------------|--|
| Item | Unit | AG461- AM444+0.5k | AM444+0.5k -AM448 | AM448- AM451+0.4k | |
| Design Discharge | m3/s | 1750 | 1750 | 1750 | |
| Distance | m | 1800 | 3550 | 3350 | |
| Gradient of River Bed | | 1/390 | 1/205 | 1/150 | |
| River Width | m | 400 | 400 | 400 | |
| Width of Channel Bed | m | 50 | 50 | 50 | |
| Dike Heightn (Ave.) | m | 4.2 | 2.2 | 2.0 | |
| Hater Depth | m | 5.5 | 3.7 | 3.5 | |
| Low Channel Height (Ave. |) m | 2.8 | 2.5 | 2.5 | |
| | _ | | | 50,00 | |

Table 7.7(1/4) FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS FOR FRAMEHORK PLAN

River: CAYANGA-PATALAN-ANGALACAN RIVER Design Flood: 50-yr (with Closure Dike)

| | : | Cayanga R. | Patalan R. Bued R Aloragat R. | Angalacan River | | | | |
|-----------------------|------|------------------|--------------------------------|-------------------------|--------------------|-------|----------------------|--|
| Item | Unit | R.M - Bued R. | | Aloragat R. - 21.0 k | 21.0 k- Maraboc | | 27.0 k - Bugayong | |
| Design Discharge | m3/s | 3100 | 1850 | 1250 | 1250 | 500 | 500 | |
| Distance | m | 6500 | 8300 | 6200 | 2800 | 3200 | 3300 | |
| Gradient of River Bed | - | 1/1300 | 1/1100 | 1/650 | 1/460 | 1/460 | 1/230 | |
| River Width | m | 500 | 200 | 150 | 120 | 100 | 80 | |
| Width of Channel Bed | m | 65 | 45 | 40 | - 35 | 25 | 20 | |
| Dike Height | m | 2.9 | 3.3 | 2.2 | 2.1 | 0.7 | 0.3 | |
| Water Depth | 10 | 8.2 | 7.3 | 6.2 | 6.1 | 4.7 | 4.3 | |
| Low Channel Height | m | 6.5 | 5.0 | 5.0 | 5.0 | 5.0 | 4.0 | |

| | | Anga 1 | acan R. | (x,y,y,y,y,y,y,y,y,y,y,y,y,y,y,y,y,y,y,y | |
|------------------------|------------|------------------------|---------------------|--|------------------------|
| Item | Unit | Bugayong -Killo Br. | Killo Br. -37.5k | | |
| Design Discharge | m3/s | 370 | 370 | | |
| Distance | m | 2700 | 4500 | | |
| Gradient of River Bed | · <u>-</u> | 1/190 | 1/140 | and the second | |
| River Width | ···m | 60 | 50 | | i |
| Width of Channel Bed | m | 15 | 15 | | Contract to the second |
| Dike Height (Ave.) | m | 0.4 | 1.1 | | the second second |
| Water Depth | m | 3.6 | 3.3 | | |
| Low Channel Height (Av | e.) m | 4.0 | 3.0 | | |

River: BUED RIVER

Design Flood: 50-yr (with Closure Dike)

| | | Bued River | | | | | | | |
|-------------------------|----------|-------------------|---------------|------------------|-------------------|-----------------|-----------------|--|--|
| Item | Unit | Junction -2.0k | 2.0k- 4.0k | 4.0k- NIA Dam | NIA Dam -11.9k | 11.9k- 16.5k | 16.5k- 19.7k | | |
| Design Discharge | m3/s | 1300 | 1300 | 1300 | 1300 | 1000 | 1000 | | |
| Distance | m | 2000 | 2000 | 3300 | 4600 | 4600 | 3200 | | |
| Gradient of River Bed | <u>-</u> | 1/400 | 1/280 | 1/170 | 1/143 | 1/140 | 1/70 | | |
| River Width | m | 400 | 400 | 400 | 400 | 400 | 400 | | |
| Width of Channel Bed | m | 30 | 20 | 20 | : 20 | - 20 | 20 | | |
| Dike Height (Ave.) | m | 4.4-2.0 | 2.1 | 2.1 | 1.9 | 1.6 | 1.4 | | |
| Water Depth | m | 8.2-5.8 | 5.6 | 3.3 | 2.4 | 2.1 | 1.9 | | |
| Low Channel Height (Ave | .) m | 5.0 | 3.5 | 2.0 | 1.5 | 1.5 | 1.5 | | |

Table 7.7(2/4) FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS
FOR FRAMEWORK PLAN

River: ALORAGAT RIVER

Design Flood: 50-yr (with Closure Dike)

| | | | | ALORAGAT R | IVER | | |
|-------------------------|-------|-------------------|-------|-----------------|-----------------|------------------------|------|
| Item | | Junction -7.0k | 11.5k | 11.5k- 17.0k | 17.0k- 19.7k | | |
| Design Discharge | m3/s | 470 | 470 | 250 | 170 | | |
| Distance | m | 7000 | 4500 | 5500 | 2700 | | * 2 |
| Gradient of River Bed | : = | 1/680 | 1/355 | 1/355 | 1/185 | $\{(-1,-1),\ldots,3\}$ | 4.4 |
| River Width | m | 90 | 80 | 50 | 45 | | 1.0 |
| Width of Channel Bed | m | 30 | 20 | 10 | 10 | | |
| Dike Height (Ave.) | m | 2.8-0.0 | . 0 | 1.3 | 1.4 | | |
| Water Depth | m | 7.3-4.2 | 4.0 | 4.0 | 2.8 | | 1000 |
| Low Channel Height (Ave | e.) m | 5.5 | 5.0 | 3.5 | 2.0 | | |

River: PANTO-MARUSAY-SINSINOCALAN-TUBOY RIVER

Design Flood: 50-yr (with Floodway)

| | | PANTO R. | MARUSAY | R. | SINOCALAN R. | | | |
|-------------------------|------|--------------------|---------------------|-----------------------|--------------|------------------------------------|--|--|
| Item | Unit | R.M- Dagupan R. | Dagupan R. -4.0k | 4.0k - Ingalera R. | -18.0k | R. 18.0k- 25.5k- 25.5k Mitura R | | |
| Design Discharge | m3/s | 2700 | 1650 | 1650 | 1000 | 650 650 | | |
| Distance | m | 2500 | 1500 | 4300 | 9700 | 7500 5500 | | |
| Gradient of River Bed | - | 1/1750 | 1/1750 | 1/1750 | 1/1750 | 1/1450 1/1100 | | |
| River Width | m | 400 | 120 | 220 | 220 | 150 100 | | |
| Width of Channel Bed | m | 70 | 60 | 50 | 30 | 30 25 | | |
| Dike Height (Ave.) | m | 3.7-3.4 | 3.0 | 3.0 | 2.6 | 2.4 2.0 | | |
| Water Depth | m | 8.0-7.7 | 7.5 | 7.5 | 7.1 | 6.9 6.0 | | |
| Low Channel Height (Ave | .) m | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 5.0 | | |

| | _ | | TAGUMISING | | | |
|-----------------------|------|---------------------|----------------------|----------------------|-----|--|
| Item | Unit | Mitura R. -36.7k | 36.7k- Sta. Maria | Sta. Maria -43.5k | | |
| Design Discharge | m3/s | 160 | 160 | 120 | | |
| Distance | m | 5700 | 4700 | 2100 | | |
| Gradient of River Bed | | 1/700 | 1/430 | 1/350 | | on a convert property section |
| River Width | m | 100 | 80 | 80 | | and the second of the second |
| Width of Channel Bed | m | - 10 | 10 | 10 | - P | |
| Dike Height | m | 0 | Ó | 0 | | 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1 |
| Water Depth | m | 4.0 | 3.3 | 3.0 | | |
| Low Channel Height | m | 5.0 | 4.5 | 4.5 | | entingen stage et |

Table 7.7(3/4) FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS FOR FRAMEWORK PLAN

River: DAGUPAN RIVER Design Flood: 50-yr

| | | DAGUPAN R. | SAN JUAN R. | | ELANG R. | |
|-----------------------|-------|-------------------|----------------|--------------------|---------------------|--|
| Item | Unit | Junction -7.5k | 7.5k- 12.7k | 12.7k- Elang R. | San Juan - 27.6k | to any art for the for the death and the said for and any fur- |
| Jesign Discharge | m3/s | 1100 | 900 | 650 | 310 | |
| istance | m | 7500 | 5200 | 9000 | 5900 | |
| radient of River Bed | | 1/5000 | 1/5000 | 1/5000 | 1/5000 | |
| tiver Width | m | 250 | 100 | 100 | 50 | |
| lidth of Channel Bed | 'n | 60 | 30 | 30 | 20 | |
| like Height (Ave.) | : · m | 3.2 | 3.6 | 4.1 | 3.3 | |
| later Depth | m | 7.7 | 7.6 | 7.6 | 7.0 | |
| ow Channel Height (Av | e.) m | 5.5 | 5.0 | 4.5 | 4.5 | |

River: INGALERA RIVER Design Flood: 50-yr

| | | | INGALERA | RIVER | | |
|-------------------------|----------------|------------------------|----------------------|------------------|--------------------|----------------------------|
| Item | . Unit | Junction -Malasigui | Malasigui - 26.0k | 26.0k - 32.0k | 32.0k- San Nico | San Nicolas Dlas -37.5k |
| Design Discharge | m3/s | 600 | 460 | 260 | 260 | 150 |
| Distance | m [:] | 13300 | 12700 | 6000 | 4000 | 1500 |
| Gradient of River Bed | - | 1/3600 | 1/1800 | 1/1000 | 1/700 | 1/700 |
| River Width | m | 100 | 60 | 50 | 50 | 40 |
| Width of Channel Bed | m | 25 | 15 | 15 | 10 | 10 |
| Dike Height (Ave.) | · m | 3.0 | 2.2 | 1.3 | 1.7 | 1.3 |
| Water Depth | m · | 7.5 | 6.9 | 5.5 | 4.9 | 4.2 |
| Low Channel Height (Ave | e.) m | 5.5 | 5.5 | 5.0 | 4.0 | 3.5 |
| | - | | | 1 | | |

River: MITURA-MAGALONG RIVER ER

Design Flood: 50-yr

| | | MITURA R. | | RIVER | | |
|-------------------------|-------|-------------------|----------------|------------------|------------------|---|
| Item | Unit | Junction -5.3k | 5.3k- Taboy | Taboy - 19.0k | 19.0k - 21.0k | |
| Design Discharge | m3/s | 250 | 250 | 180 | 140 | |
| Distance | m | 5300 | 8900 | 4800 | 2000 | |
| Gradient of River Bed | _ | 1/800 | 1/460 | 1/460 | 1/250 | |
| River Width | m | 50 | 40 | 35 | 30 | |
| Width of Channel Bed | TO TO | 10 | 8 | 6 | 4 | • |
| Dike Height (Ave.) | m | 2.0-1.0 | 1.5 | 1.4 | 1.3 | |
| Water Depth | m | 6.0-5.2 | 4.7 | 4.3 | 3.7 | |
| Low Channel Height (Ave | .) m | 5.0 | 4.0 | 3.5 | 3.0 | |

Table 7.7(4/4)

FEATURES OF DESIGN CHANNEL OF ALLIED RIVERS FOR FRAMEWORK PLAN

River: BINALONAN FLOODWAY/TUBOY RIVER

Design Flood: 50-yr

| Item | | Binalonan Floodway | | Tuboy R. | | | |
|------------------------|------|--------------------|---------------|----------------|-----------------|-----------|------|
| | Unit | Junction -1.8k | 1.8k- 6.7k | 6.7k- 10.6k | 10.6k- 12.2k | | |
| Design Discharge | m3/s | 650 | 650 | 550 | | | |
| Distance | m | 1800 | 4900 | 3900 | 2000 | | |
| Gradient of River Bed | | 1/400 | 1/355.5 | 1/190 | 1/143-1/67 | 4 pt - 15 | |
| River Width | m | 60 | 60 | 60 | 60 | 1.00 | 1000 |
| Width of Channel Bed | m | 15 | 15 | 15 | 15-10 | | |
| Dike Height (Ave.) | m | 2.5 | 2.4 | 1.7 | 1.7-0.3 | 12. | : |
| Water Depth | m | 6.1-6.0 | 6.0-5.7 | 4.7 | 4.7-3.3 | | 4.5 |
| Low Channel Height (Av | | 4.5 | 4.5 | 4.0 | 4.0 | • | |

Table 8.1 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR LONG TERM PLAN

River: AGNO RIVER Design Flood: 25-yr

| | | Agno R. | | | | | | | |
|-------------------------|------|---------------------|----------------|-----------------|------------------|-----------------------|-------|--|--|
| Item | Unit | Rivermouth -AG45 | AG45 - AG65 | AG65 - AG109 | AG109 - AG177 | AG177 - AG180+0.8k | ***** | | |
| Design Discharge | m3/s | 9000 | 9000 | 9000 | 8100 | 7500 | | | |
| Distance | m | 6850 | 9050 | 15150 | 10500 | 2800 | | | |
| Gradient of River Bed | | 1/6500 | 1/6500 | 1/3500 | 1/2000 | 1/2000 | | | |
| River Width | m | 1500 | (1500) | (1500) | (1500) | (1500) | | | |
| Width of Channel Bed | m | 360-250 | 240 | 200 | 200 | 200 | | | |
| Dike Height (Ave.) | m | 3.6 | 4.2 | 5.1 | :4.5 | 4.1 | | | |
| Water Depth | m | 8.1-9.0 | 9.0-10.1 | 10.1 | 10.1-8.8 | 8.8-8.4 | | | |
| Low Channel Height(Ave. | .) m | 6.5 | 6.5 | 6.5 | 6.5 | 6.0 | | | |

| | I | Retarding 1> | Floodway | Bayambang 2 | ?> Agn | o R. | V. |
|------------------------|------|----------------------|---------------------|---------------------|----------|------------------|-----|
| Item | Unit | AG180+0.8k -AG314 | AG314 - AG320(b) | AG282(a) - AG307 | | AG351 - AG367 | |
| Design Discharge | m3/s | | 5200 | 600 | 5800 | 5100 | |
| Distance | m | 6100 | 3600 | 10450 | 15300 | 7650 | |
| Gradient of River Bed | · / | 1/1550 | 1/1550 | Existing | 1/1550 | 1/1000 | |
| River Width | ·m | · | 1000-830 | 160-2000 | 600-2500 | 100-3200 | 100 |
| Width of Channel Bed | m | 180 | 180 | Existing | 180 | 180 | * * |
| Dike Height (Ave.) | m | 4.7 | 4.0 | 4.1-0.0 | 4.4 | 4.4 | |
| Water Depth | m | 8.4-7.0 | 7.0 | 7.6-3.4 | 6.9 | 6.9-4.8 | - , |
| Low Channel Height(Ave | .) m | 4.5 | 4.5 | 5.0 | 4.0 | 3.0 | 200 |

1>:Retarding Basin Stretch 2>:Bayambang Stretch of Agno R.

| • | | | Agno R. | * - 2* | | |
|--|-----------------|-----------|-----------|-----------|----------|--|
| ltem | Unit | AG367 - | AG414 - | AG453 - | AG459 - | |
| and the second of the second o | it. Libial | AG414 | AG453 | AG459 | AG473 | |
| Design Discharge | m3/s | | 5100 | 3800 | 3800 | |
|)istance | m | 7700 | 5300 | 3000 | 6450 | |
| Gradient of River Bed | 1 .7 <u>1</u> . | 1/700 | 1/370 | 1/370 | 1/210 | |
| liver Hidth | m | 1050-2500 | 1250-2400 | 1000-1900 | 300-1300 | |
| lidth of Channel Bed | m | 180 | 150 | 150 | 150 | |
| like Height (Ave.) | m | 3.3 | 2.8 | 2.0 | 2.0-3.0 | $\varphi(t) = e(\varphi_t) \wedge e(\varphi_t) + e(\varphi_t)$ |
| ater Depth | m | 4.8 | 4.3 | 3.8 | 3.8-4.8 | $\label{eq:controller} \begin{array}{cccccccccccccccccccccccccccccccccccc$ |
| ow Channel Height (Ave | e.) m | 3.0 | 3.0 | 3.0 | 3.0 | |