11. COST ESTIMATES

11.1 Cost Estimation Criteria

The cost estimation criteria presented herein are prepared for comparative study of alternative and proposed flood control plans.

11.1.1 Constitution of Project Cost

Basic Conditions

In estimating the project cost, the following basic conditions are assumed.

- (1) The construction works are to be procured by bidding.
- (2) Unit cost of each construction work item is estimated on a unit price basis, except for some work items to be estimated on a lump sum/percentage basis.
- (3) Unit prices are based on the price level of June 1989.
- (4) Exchange rates of the foreign and local currencies are US\$1.00 = P21.30 = ¥132.00.

Constitution of Project Cost

The project cost is composed of main construction cost, compensation cost, administration and engineering services, and contingencies. The detailed constitution of the project cost is shown in Figure 11.1.1. The project cost is classified into two categories: financial cost and economic cost. The financial cost is the budgetary cost required for implementation of the project, and the economic cost is used for economic evaluation of the project.

11.1.2 Main Construction Cost

The main construction cost consists of the cost of preparatory works, main works and miscellaneous works.

Preparatory Works

The cost of preparatory works is usually in the range of 5 to 10% of the cost of main construction works for flood control and river improvement depending on the project study status. Therefore, in this estimation, 10% which is onin the higher side of the range is applied.

Main Works

The cost of main works is computed as the unit cost multiplied by the work quantity. The unit cost of each work item consists of a direct cost and indirect cost. The direct cost in the unit cost consists of materials cost, squipment expenses, and labor cost. These costs and expenses are estimated on unit prices based on the "Agno Flood Control System CY-1989 Regular Infrastructure Program" of DPWH-PMO AFCS and similar projects in the Study area.

The indirect cost consists of (1) overhead, contingencies and miscellaneous (OCM), (2) profit, (3) mobilization and demobilization for contractor, and (4) Value Added Tax (VAT). Each item of the indirect cost is computed in percentage according to the guideline of DPWH.

The percentages of indirect cost are as follows.

- (1) Overhead, Contingencies : 9% of sum of estimated direct cost and Miscellaneous
- (2) Profit : 7% of sum of estimated direct cost
- (3) Mobilization and : 5% of sum of estimated direct cost Demobilization
- (4) Value Added Tax : 10% of equipment expenses and labor cost in direct cost

Miscellaneous Works

This study is made at the master plan level. Therefore, 15% of the sum of prepatory works and main works is adopted as the rate of miscellaneous works.

11.1.3 Compensation

The cost of compensation is estimated on a unit basis divided into land acquisition and house evacuation, which are classified as follows.

Land Acquisition : Land 1 (farmland)

Land 2 (non-farmland)

House Evacuation : Building 1 (light)

Building 2 (strong-framed)

The following unit costs, which represent average values in the project area obtained from the provincial assessors, are adopted in the estimate of cost of compensation.

Commercial area : P400,000/ha

Residential area : P150,000/ha

Farmland (Irrigated) area : P10,000/ha

Non-farmland area : P7,000/ha

Building 1 : P40,000/unit

Building 2 : P80,000/unit

11.1.4 Administration and Engineering Services

The administration cost of the government is computed at 5% of the sum of the main construction and compensation costs according to the standard criteria of DPWH.

The cost of engineering services herein estimated covers the detailed design and construction supervision. Therefore, 16% of the cost of main construction works is adopted as the rate of the engineering cost.

11.1.5 Project Contingency

The project contingency consists of physical contingency and price escalation contingency.

The physical contingency is usually estimated at 10 to 20% depending on the project study status. In this study, 15% which is the average value of the above percentage is applied.

The price escalation rates adopted are 6% for local currency and 3% for foreign currency which are currently used in the projects financed by OECF loan.

11.1.6 Currency Proportion

The financial project cost is estimated assuming that the majority of it will be financed by international financing agencies. The assumed currency components of unit costs are as follows:

	Portion of	f Unit Cost	
Particulars —	Foreign Currency	(%) Local Currency (Z)
(1) Labor Cost	0	100	
(2) Equipment Cost	100	0	
(3) Material Cost			
(a) Fuel	50	50	
(b) Cement	65	35	
(c) Re-bar	65	35	
(d) Structural Steel	100	0	
(e) Others	1 r 70 -	100	
(4) Overhead (Excl. VAT)	$(2+3) \times 21$	$(1+3) \times 21$	
(5) Value Added Tax (VAT)	. 0	$(1+3) \times 10$	
(6) Compensation (land acqui-	0	100	
sition and resettlement)			
(7) Administration	0	100	4.
(8) Engineering Services	90	10	

11.2 Unit Costs

11.2.1 Financial Unit Costs

The financial unit costs of respective construction work items of river and dam works are preliminary prepared based on the foregoing criteria and are listed in Tables 11.2.1 for river works and 11.2.2 for dam works.

11.2.2 Economic Unit Costs

The economic cost is the financial cost less government tax and profit of the contractor, and price escalation contingency.

The economic unit costs of respective construction work items are shown in Table 11.2.3 for river works and Table 11.2.4 for dam works.

11.2.3 Lump Sum Unit Costs

The cost of revetments, groins, sluice ways, water gates, bridges and fixed weirs are estimated by lump sum unit costs listed in Table 11.2.5. These lump sum unit costs are prepared based on the standard design.

11.3 Cost Estimates

11.3.1 Project Cost

The economic project cost and the financial project cost of the Framework Plan and the Long Term Plan were estimated at 1989 constant price level based on the cost estimation criteria and the unit prices specified in Sections 11.1 and 11.2.

Those project costs and work quantities presented in Chapters 9 and 10 were estimated by river stretches and/or river systems constituted in Figure 11.1.2. Corresponding location map is presented in Figure 11.1.3.

Further details of the project cost and the work quantities of the alternative Framework Plans and the Long Term Plan are presented in the Supporting Report, CP: Construction Plan and Cost Estimates.

11.3.2 Financial Project Cost for Long Term Plan

The financial project cost of the proposed Long Term Plan is summarized as follows:

			(Unit Mill	. Pesos)
	River	Foreign Currency Portion	Local Currency Portion	Total
ı.	Agno River (25-year design	flood)		
;	1. Lower Agno River	4,048	2,248	6,296
	2. Poponto Stretch	761	366	1,127
	3. Upper Agno River	1,393	811	2,204
	Sub-Total	6,203	3,424	9,627
	4. Tarlac River	903	518	1,421
	5. Tributaries	937	703	1,640
	Total of Agno River	8,043	4,645	12,688
II.	Allied Rivers (10-year des	ign flood)	ta e e e e e e e e e e e e e e e e e e e	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	1. Panto-Sinocalan River	1,311	849	2,160
	2. Cayanga-Patalan River	615	511	1,126
	Total for Allied Rivers	1,926	1,360	3,286
	Grand Total	9,969	6,005	15,974

The cost and work quantities are listed in Tables 11.2.6 (1/2) - (2/2) by stretches or tributaries.

Further details of the financial project cost are compiled in Table 2.19 (1-22) of the Supporting Report, CP: Construction Plan and Cost Estimates. The quantity and the cost of compensation for the proposed Long Term Plan is presented in Table 11.2.7.

Table 11.2.1 UNIT COSTS OF RIVER CONSTRUCTION WORKS (FINANCIAL COST)

												(Unit : Pesos)
			Direct Cos	ost	:				Indirect Cost	:		
them Item of Work No.	Cait	Material	Equipment	Labor	Total	Overhead Cont'cy	Frofit	Mob. & Demoh	Value Added Tax	Total	Cost	Remarks
1. Excavation 1	Cu.m	7.49	35.04	3.57	46.10	4.15	3.23	2.30	3.87	13.55	8	Common Soil
2. Excavation 2	CW.TH	9.36	40.59	4.17	54.12	4.78	3.72	2.65	4.48	15.63	8	Stony with Boulder
3. Dredging	CU.TT	8.10	17.88	1.48	27.46	2.47	1.92	1.37	1.9	7.70	98	Fine Sand
4. Embanicment 1	cu.m	12.53	49.31	5.74	67.58	6.08	4.73	3.38	5.52	19.71	88	Excavated Materials
5. Embankment 2	Cu.m	20.76	76.27	8.16	105.19	9.46	7.37	5.27	8.44	30.54	136	Borrow Materials
6. Stone Masoury	cu.m	531.53	79.11	138.04	748.68	67.38	52.41	37.43	21 72	178.94	626	Rubble Concrete Class B
7. Backfilting Gravel	CII.III	15.87	31.00	87.13	134.00	12.06	9.38	6.70	11.81	39.95	174	Hauling Distance: 20km
8. Sodding	SQ.FI	1.00	0.00	6.88	7.88	0.71	0.55	0.39	0.67	2.32	10	Native Grass
9. Concrete (210 kg/cu.cm)	Cu.m	1,467.19	62.98	199.12	1,729.29	132.05	121.05	86.46	26.21	365.77	2,095	Class A
10. Concrete (140 kg/cu.cm)	Cu.m	1,274.53	62.98	199.12	1,536.63	138.30	107.56	76.83	26.21	348.90	1,896	Class C
11. Reinforcing Steel Bar	<u>\$9</u>	18.80	1.50	2.47	22.77	2.05	1.59	1:14	0.40	5.18	83	Including Fabrication
12. Stone Spurdike	G.33	26.31	38.75	68.67	133.73	12.04	9.36	69.9	10.74	38.83	173	Boulder, 20-40 cm in diameter
13. Gatzion Cylinder	E	95.18	0.00	29.71	124.89	11.24	8.74	6.24	2.97	29.19	154	0.45 m in diameter, L=5.00 m
14. Gabion Cybrider	E	136.89	0.00	44.21	181.10	16.30	12.68	9.06	4.42	42.46	222	0.60 m in diameter, L=5.00 m
15. Gabion Mattress	SQ.m.	229 13	0.00	35.31	264.44	23.80	18.51	13.22	3.53	59.06	324	0.50 x 1.20 x 3.00 m
16. RC Pile	8	656.02	195.73	267.05	1,118.80	100.69	78.32	55.94	46.28	281.23	1,400	0.40 x 0.40 m
17. Wooden Pile 1	E	47.46	6.16	22.16	75.78	6.82	5.30	3.79	2.83	18.74	ጽ	0.15 m in diameter
18. Wooden Pile 2	Ħ	84.00	8.62	31.03	123.65	11.13	8.66	6.18	3.97	29.94	15.	0.20 m in diameter
19. Steel Pipe Pile	8	3,027.45	284.57	116.57	3,428.59	308.57	240.00	171.43	40.11	760.11	4,189	0.60 m in character
20. RC Sheet Pile	SQ.m	831.12	445.16	310.03	1,586.31	142.77	111.04	79.32	75.52	408.65	1,995	$0.45 \times 0.20 \text{ m}$ in diameter
21. Steel Sheet Pile	sq.m	3,917.43	53.04	11.64	3,982.11	358.39	278.75	1199.11	6.47	842.72	4,825	Type III
22. Shrice Gate-1	ij	194,000.00	39,000.00	30,000.00	263,000.00	24,000.00	18,000.00	13,000.00	7,000.00	62,000.00	325,000	1.5 x 1.5 m Steel Gate
23. Shrice Gate-2	ឌ្គ	580,000.00	120,000.00	93,000.00	793,000.00	73,000.00	54,000.00	40,000.00	21,000.00	188,000.00	981,000	2.0 x 2.0 m Steel Gate
24. Stoel Roller Gate	G	140,000.00	13,000.00	30,000.00	183,000.00	17,000.00	13,000.00	9,000.00	5,000.00	44,000.00	227,000	0.8 Usq.m w/Guide & Hoist
25. Bridge	₩. ₩.	7,844.00	1,159.00	1,074.00	10,077.00	907.00	705.00	504.00	223.00	2,339.00	12,420	RC Type
26. Demolishment, Concrete	CIT	132.86	334.42	726.72	1,194.00	107.46	83.58	59.70	106.11	356.85	1,550	Reinforced Structure
27. Demolishment, Metal	ğ	\$6.09	1,600.29	96.40	1,752.78	157.75	122.69	87.64	169.67	537.75	2,290	Metal Structure

Table 11.2.2 UNIT COSTS OF DAM CONSTRUCTION WORKS (FINANCIAL COST)

	•		Direct Cost	z.				Indirect Cost	a A			
Item No. Item of Work	Unit	Matcriel	Equipment	Labor	Total	Overhead Cont'cy & Misc.	Profit	Mob. & Demob.	Value Added Tax	Total	Unit	Remarks
1. RIVER DIVERSION WORKS												
1.1 Excavation (Common)	8	œ	42	9	56	. S	4	m	s	. 17	73	Soil & Riverbed Materials.
1.2 Excavation (Rock)	5	84	4	80	200	18	71	10	15	272	257	Sound Rock
1.3 Excavation (Tunnet)	E T	245	650	<u></u>	806	82	8	45	8	257	1,165	
1.4 Concrete (Infer & Outlet)	8	1.470	600	197	2.067	981	14	103	ŝ	493	2.560	
1.5 Concrete (Timpel Lining)	1 2	1.470	455	90	2 225	200	156	111	9/	\$	2.768	
16 Concrete (Plus)		270	400	S -	280	162	126	8		433	223	
T. Constitution Const	3	1,4,0	3 8	2 5	305	304	3	2 2	3 =	100	1000	
1.1 Consolingation Groun	8 .	J.5.50	₹ -	8 '	5	3	50.5	Ç.•	ĵ.	0.00	1,830	
1.8 Kemiorcing Meel Bar	ž,	41	٠.		SI.	7	7	-	- .,	۰	23	
1.9 Cofferdam	E Co	15	\$ *	9	117	01	oo	φ.	Ξ,	33	132	
2. DAM										<i>i.</i>		
2.1 Excavation (Common)	ETTO	90	ቲ	9	98	Ŕ	4	ť	Y)	.17	73	Soil & Riverbed Materials.
2.2 Excavation (Rock)	E 10	48	3	. 00	200	82	4	10	15	2.5	257	Sound Rock
2.3 Excavation (Core)	ē	12	8	·vc	11	. <u>.</u>	**	, v c		35	143	
2.4 Embankment (Filter)	1	7	: 5		88	2	· <u>c</u>	۰۰۰	: 5	Š	8 %	
2 C Entrated (Best)		6	25	•	3	•	2 :	٠,	2 4) ²		
C. Edibanaluciji (AOCK)	3	ጻ (5	1 4	66	s >	1 6	07	2 2	8 8	3 5	
2.0 embantment (Kiprap)	e d	75	767	× į	767	9 ?	RZ :	2 ;	\$;	2	377	
2.7 Curtain Grout	£	1,800	120	Q	2,000	8	140	8	200	620	2,520	
2.8 Blanket/Consolidation Grout	E	1,350	8	8	1,500	135	105	75	15	330	1,830	
2.9 Mass Concrete (Concrete Dam)		ļ	3				;	į	. ;		;	
(1) Volume: 100,000 cu.m	G.T	276	436	256	1,268	151	8	\$	6	393	1,661	
(2) Volume: 250,000 cu.m	8	561	425	249	1,235	147	88	85	63	382	1,617	
(3) Volume: 500,000 cu.m	3	\$40	409	240	1,189	142	83	79	\$9	369	1,558	
(4) Volume: 1,000,000 cu.m	Corne	510	386	227	1,123	13	79	74	19	348	1,471	
(5) Volume: 2,000,000 cu.m	E TO	488	370	217	1,075	128	75	11.	26	333	1,408	
3. SPIILWAY					÷							
3.1 Excavation (Common)	87.00	œ	42	9	56	S	4	ო	٧	17	73	Soil & Riverbed Materials.
3.2 Excavation (Rock)	Contra	43	144	00	200	81	14	10	15	53	257	Sound Rock
3.3 Concrete	ខ្ល	1,315	350	175	1.840	162	128	8	53	433	2,273	
3.4 Reinforcing Steel Bar	DQ M	. 19	, ,,,	co :	£	~	7	.		9	8	
3.5 Bridge	sdru	6,500	986	880	8,350	743	578	418	185	1,924	10,274	RC T-beam Type
4. OUTLET FACILITIES				•	1							
4.1 Excavation (Common)	8	90	. 24	9	95	v o	4	ĸ	V)	17	73	Soil & Riverbed Materials.
4.2 Freevation (Rock)	8	84	114	•	170	18	14	10	15	57	722	Sound Rock
4.3 Concrete	8	1.470	9	161	2.067	186	4	103	8	493	2,560	
4.4 Reinforcing Steel Bar	2	19	7 -4		83	8	64	, pud	***	9	8	
	•		•				٠.				i	
METAL WORKS		8	ć	7	ç	5		•	٠	ç	9,5	
2,1 Liverson Chemic One	9	8 5	,	7 %	9 9	3 5	,	.	ሳነ	તું :	ę į	0.9 Usq.m, Wisheel log
5.2 Spillway Gate	5	3	2	ê,	183	11	E	σ.	'n	4	122	0.8 Usq.m, w/guide and hoist
5.3 Intake Gate	5	\$	E	8	183	11	13	•	νn .	4	227	2.4 t/sq.m. w/guide and hoist
5.4 Valve	203	210	8 2	42	2	Z,	2	74	•	63	333	40 kg/mm, w/guard gate and hoist

Table 11.2.3 UNIT COSTS OF RIVER CONSTRUCTION WORKS (ECONOMIC COST)

(Unit: Pesos)		Remarks		52 Common Soil	60 Stony with Boulder	30 Fine Sand	6 Excavated Materials	8 Borrow Materials	3 Rubble Concrete Class B	1 Hauling Distance: 20km	9 Native Grass	4 Class A	6 Class C	24 Including Fabrication	9 Boulder, 20-40 cm in diameter	2 0.45 m in diameter, L=5.00 m	1 0.60 m in diameter, L=5.00 m	5 0.50 x 1.20 x 3.00 m	1 0.40 x 0.40 m	1 0.15 m in diameter	1 0.20 m in diameter	3 0.60 m in diameter	4 0.45 x 0.20 m in diameter	3 Type III	0 1.5 x 1.5 m Steel Gate	0 20x20 m Steel Gate		4 RC Type		
		Cost		Ϋ́	'	m	7	118	793	151		1.804	1,606	2	149	132	191	275	1,201	81	131	3,563	1,714	4,093	278,000	838,000	192,000	10.594	1,346	
		Total		6.35	7.32	3.72	9.29	14.44	97.38	18.54	1.09	221.56	197.29	2.92	18.36	16.15	23.44	33.82	147.45	9.94	16.13	437.62	210.45	502.65	34,000.00	103,000.00	23,000.00	1,301.00	165.30	
	Indirect Cost	Value Added Tax	· :	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	00.00	0.00	0.00	0.00	
:	· 144	Mob. & Demob.		2,27	2.61	1.33	3.32	5.16	34.78	6.62	0.39	79.13	70.46	1.04	92'9	5.77	8,37	12.08	52.66	3.55	5.76	156.29	75.16	179.52	12,000.00	37,000.00	8,000.00	465.00	59.04	
		Profit		0.00	0.00	0.00	0.00	00.0	000	0.00	0.00	0.00	000	0.00	0.00	0.00	0.00	000	0.00	000	000	0.00	0.00	0.00	000	0.00	000	0.00	0.00	- :
		Overhead Cont'cy & Misc.		4.08	4.71	2.39	5.97	9.28	62.60	11.92	0.70	142.43	126.83	1.88	11.80	10.38	15.07	21.74	94.79	6.39	10.37	281.33	135.29	323.13	22,000.00	00:000'99	15,000.00	836.00	106.26	
		Total		45.35	52.28	26.65	66.33	103.11	695.53	132.41	7.78	1,582.57	1,409.18	20.89	131.10	115.37	167.41	241.53	1,053.20	71.03	115.25	3,125.85	1,503.20	3,590.37	244,000.00	735,000.00	169,000.00	9,293.00	1,180.71	-
	Š	Labor	;	3.57	4.17	1.48	5.74	8.16	138.04	87.13	6.88	199.12	199.12	2.47	19.89	29.71	44,21	35.31	267.05	22.16	31.03	116.57	310.03	11.64	30,000,00	93,000.00	30,000.00	1,074.00	726.72	
	Direct Cost	Equipment		35.04	40.59	17.88	49.31	76.27	79.11	31.00	0.00	62.98	62.98	1.50	38.75	0.00	000	0.00	195.73	6.16	8.62	284.57	445.16	53.04	39,000.00	120,000.00	13,000.00	1,159.00	334.42	
		Material		6.74	7.52	7.29	11.28	18.68	478.38	14.28	0.90	1,320.47	1,147.08	16.92	23.68	85.66	123.20	206.22	590.42	42.71	75.60	2,724.71	748.01	3,525.69	175,000.00	522,000.00	126,000.00	7,060.00	119.57	
		Crit	1	cu.m	cu.m	ca.m	cu.m	св.т	CD.III	cu.m	sq.m	cu.m	cu.m	χς G	cn.m	8	Ħ	85.EB	Ħ	E	E .	æ	8. E.	a.ç	33	se	ton	m.ps		
		item item of Work No.		1. Excavation 1	2 Excavation 2	3. Dredging	4. Embankment 1	5. Embankment 2	6. Stone Masorary	7. Backfilling Gravel	8. Sodding	9. Concrete (210 kg/cu.cm)	10. Concrete (140 kg/cu.cm)	11. Reinforcing Steel Bar	12. Stone Spurdike	13. Gabion Cylinder	14. Gabion Cylinder	15. Gabion Mattress	16. RC Pile	17. Wooden Pile 1	18. Wooden Pile 2	19. Steel Pipe Pile	20. RC Sheet Pile	21. Steel Sheet Pile	22. Sluice Gate-1	23. Shice Gate-2	24. Steel Roller Gate	25. Bridge	26. Demolishment, Concrete	

Table 11.2.4 UNIT COSTS OF DAM CONSTRUCTION WORKS (ECONOMIC COST)

			Reco coalin	ផ				indirect Cost				
Item No. Item of Work	Unit	Material	Equipment	Labor	Total	Overhead Confry &	t Profit	Mob.	Value Added Tax	Total	Cost	Remarks
1. RIVER DIVERSION WORKS												
1.1 Excavation (Common)	6		4	٠	55	*.	No.	0	0	•	63	Soil & Riverbed Materials.
1.2 Excevation (Rock)	8	43	77	00	195			01	0		223	Sound Rock
1.3 Excavation (Tunnel)	G	72	059	<u> </u>	. Z			4		124	1,008	
14 Concrete (Inles & Confes)	. 5	333	ξ.	6	1 830		er.				2,180	
1 & Concrete (Purpel I (mine)		2	35.	200	2000			2			3,50	
The Country of the Co	3	30.	3 8	85	4,070						. 000	
1.0 Condete (ring)	E 2	1,143	96	1.50	1,0,1		, a				2,508	
1.7 Corsolidation Grout	E	1,215	8	ક	1,365		Ęħ.	0	0	57	1,556	
1.8 Reinforcing Steel Bar	er Se	. 13	-	ب	23	-		0	0	m	74	
1.9 Cofferdem	COL.TO	11	83	9	116			0	0	16	132	
2 DAM										**		
2 Totalistics (Commence)	į	r	ξ	*	٧		v	•	·c		63	Coll P. Dissalad I Condition
2.0 The street of Continued		· ç		, ,	3 5		· ·	9		į	9 6	See of Mive Social Manual See.
Z.z. Excavation (Rock)	5 5	54	<u>4</u>	×		*,	×o. •	0		8	577	Sound Kock
2.3 Excavation (Core)	CI TO	11	જ	9	116		10	9	0		132	
2.4 Embankment (Filter)	CILTS	<u>6</u>	156	•	181		16	6			508	
2.5 Embankment (Rock)	Curt	፠	157	4	195		. 81	0 10			223	٠
2.6 Embankment (Riprap)	EL.US	47	232	90	287	52	9	0 14			327	
2.7 Curtain Grout	E	1.620	120	08	1.820		4	0	0	C	2.075	
2.8 Blanket/Consolidation Gross	ε	1215	8	Ş	1 365	-	çı				1 596	
2.9 Mass Construct (Construct Dam)				3			,	}				
(1) Vehime: 100 000 mm	į	, v	763	356	0101			. 6			377	
(1) Votable: 100,000 Cu.m	CH.131	0.00	000	200	777		- 1	\$ 8	•	=	3.5	
	E E	S S	3	749	1,179		۱ بر	0 82		677	804,1	
(3) Volume: 500,000 cu.m	E 13	486	£04	240	1,135		79	0 79			1,356	
(4) Volume: 1,000,000 cu.m	E 10	2	386	227	1,072	2	4	74			1,280	
(5) Volume: 2,000,000 cu.m	10.10	439	370	217	1,026		œ:	0 71	о _.		1,225	
3. SPILLWAY												-
3.1 Excavation (Common)	8	**	42	9	55	-	•	0	.·	· oc	.63	Soil & Riverbed Materials.
3.2 Percevesion (Rock)	E	E.P	144		100					× ×	2	Sound Rock
2 2 Contracts		761	5	74	200		3 5			,	10	Tools pinos
2.4 Deleganian Start Day		1011) ·		5						e č	
S. + Reparenting Section	¥°	100	4 6,0		7 .		4 (Š	H ()
3.5 Bridge	S H	2,830	\$	2	3.	66	2	280		1,078	8,778	KC 1-ocam 1ype
4. OUTLET FACILITIES						-						
4.1 Excavation (Common)	E TO	7	42	9	55		٠,	0	J		8	Soil & Riverbed Materials.
4.2 Excavation (Reck)	E	43	14.		195		18	01		83	223	Sound Rock
4.3 Contracts	8	1323		5	1 920			96	φ	260	2.189	
A & Dainfouring Creel Box	į	7		"	21	100		c			20	
The same of the sa	۶	2. • • • • • • • • • • • • • • • • • • •	• • • • •	•	:		Š	• · · · · · · · · · · · · · · · · · · ·		s	i	
5. METAL WORKS												
5.1 Diversion Closure Gate	200	88	6:	77		1	11	0	0	•	135	0.9 t/sq.m, w/steel log
5.2 Spillway Gate	9	126	13	8			'n	0	0		28	0.8 t/sq.m, w/guide and hoist
5.3 Intake Gate	20	126	13	8	691		15	8	0	23	192	2.4 t/sq.m, w/guide and hoist
		***	٠	<			•	•			66	and the fact that the same of
5.4 Valve	٤		<u> </u>	77						1	507	A LORDIN WISHER PAIR

Table 11,2.5 STANDARD UNIT CONSTRUCTION COSTS OF FLOOD CONTROL WORKS AND FACILITIES

(Unit : Pesos) Financial Cost Work Items Unit Foreign Local Total Economic Remarks Item Cost Cost No. Cost 1. Excavation 1 47 13 60 52 Common soil cu.m 54 15 69 60 Stony with boulder 2. Excavation 2 cu.m 27 30 Fine sand 3. Dredging cu.m 9 36 4. Embankment 1 67 88 76 Excavated materials 21 cù.m 5. Embankment 2 104 32 136 118 Borrow materials cu.m 6. Sodding sq.m 0 10 10 9 Native grass 7. Revetment (L.W.C.) Gabion type Type-A 284 346 630 539 sq.m Туре-В 191 233 424 363 sq.m 8. Revetment (H.W.C.) Wet masonry type 302 370, 672 575 Type-A sq.m 239 291 530 453 Type-B sq.m 9. Groin (L.W.C.) 97,140 131,000 112,000 Wooden pile type Type-A pc. 33,860 575,000 Concrete frame type 287,000 390,000 677,000 Type-B pc. 10. Groin (H.W.C.) Type-A 33,500 61,500 95,000 80,000 Wooden pile type pc. 465,000 Concrete frame type Type-B pc. 232,000 317,000 549,000 11. Sluice Way Type-A 1,161,000 549,000 1,710,000 1,450,000 Culvert, 1,5x1.5 m pc. Type-B 1,736,000 775,000 2,511,000 2,128,000 Culverts, 1.5x1.5 m x 2 pc. 12. Water Gate 14,730,000 5,881,000 20,611,000 17,459,000 Slide gate, 10.0x7.0 m Type-A pc. Type-B 31,174,000 11,172,000 42,346,000 36,161,000 Slide gate, 20.0x8.0 m pc. 13. Demolishment Concrete 485 1,065 1,550 1,346 cu.m 1,970 320 2,290 1,992 Metal ton 10,594 Concrete type 14. Bridge 6,620 5,800 12,420 sq.m 15. Fixed Weir 44,490,000 36,403,000 80,893,000 69,300,000 pc.

Table 11.2.6 FINANCIAL PROJECT COST OF LONG TERM PLAN (1/2)

Nilmoyayayiyay (ACC) Circ. Saybayay (ACC) ACC (Acc) (A	ACC TO COLUMN A STATE OF THE ST		(Unit:	.000 Pesos)
River	Length (km)	F.C.	L.C.	Total
I. Agno River				
1. Lower Agno River				
(1) RM-AG045 (2) AG045-AG122 (3) AG122-AG282	6.9 25.1 11.9	955,609 1,958,053 979,063	679,183 963,113 519,039	1,634,792 2,921,166 1,498,102
Sub-total of 1	43.9	3,892,725	2,161,335	6,054,060
2. Poponto Stretch				
(1) Bayambang Stretch(2) Poponto Floodway	10.5 10.7	76,139 685,298	53,450 312,500	129,589 997,798
Sub-total of 2	21.2	761,437	365,950	1,127,387
3. Upper Agno River		÷		
(1) AG309-AG351 (2) AG351-AG405 (3) AG405-AG473	14.3 10.6 19.5	299,418 222,559 871,344	225,551 155,322 429,655	524,969 377,881 1,300,999
Sub-total of 3	44.4	1,393,321	810,528	2,203,849
Total of I	109.5	6,047,483	3,337,813	9,385,296
II. Tarlac River				
(1) AG180-TA200 (2) TA200-TA265	8.1 29.0	456,111 446,532	184,589 333,839	640,700 780,371
Total of II	37.1	902,643	518,428	1,421,071
III. Agno River Tributary	**			
(1) Camiling River(2) Banila River(3) Viray-Dipalo River(4) Ambayoan River	20.0 30.9 20.1 8.7	225,737 459,202 150,801 101,274	161,015 314,534 149,433 78,013	386,752 773,736 300,234 179,287
Total of III	79.7	937,014	702,995	1,640,009
GRAND TOTAL (1+11+111)	226.3	7,887,140	4,559,236	12,446,376

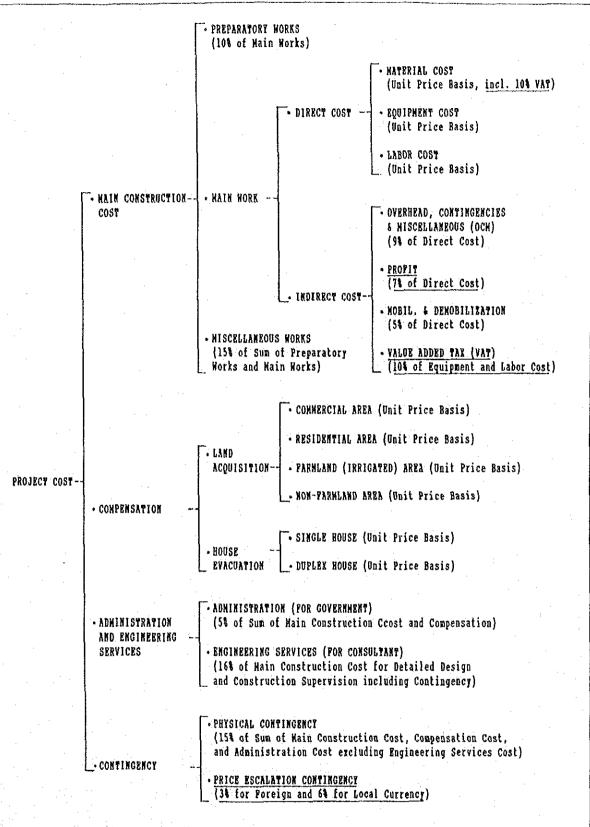
Table 11.2.6 FINANCIAL PROJECT COST OF LONG TERM PLAN (2/2)

The state of the s		and the state of t	(Unit:1	.000 Pesos)
River	Length (km)	F.C.	L.C.	Total
I. Pantal-Sinocalan River	and Property and classification of the Conference of the Conferenc		ales allan essa del 1900 de la composició de la colonida de la colonida de la colonida de la colonida de la co	
 (1) Pantal-Sinocalan River (2) Dagupan River (3) Ingalera River (4) Macalong River (5) Binalonan Floodway 	49.8 27.6 37.5 22.0	539,589 379,441 334,582 57,757	376,417 207,483 219,499 45,235	916,006 586,924 554,081 102,992
Sub-total of I	136.9	1,311,369	848,634	2,160,003
II. Cayanga-Patalan River				
(1) Cayanga-Pantalan River(2) Bued River(3) Aloragat River	37.5 19.0 21.3	338,684 214,179 61,882	262,748 161,985 86,802	601,432 376,164 148,684
Sub-total of II	77.8	614,745	511,535	1,126,280
Total of I and II	214.7	1,926,114	1,360,169	3,286,283

Table 11.2.7 QUANTITY AND COST OF COMPENSATION

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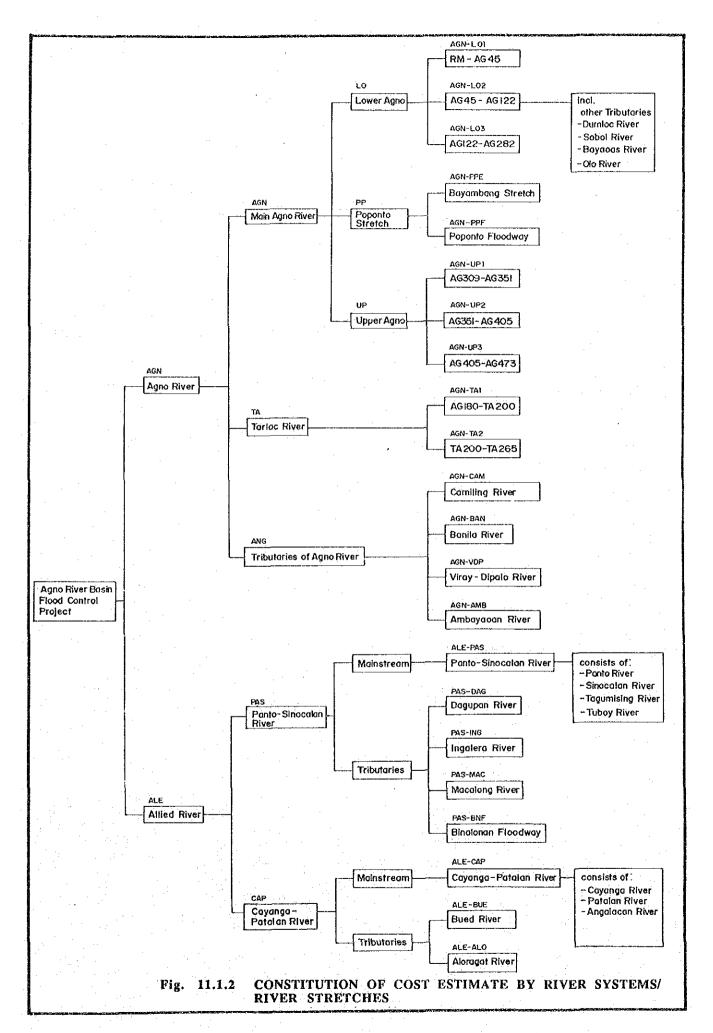
R-ANGERSCHEINE STEINE STEINE SEINE SEINE IN DES FLIEREN VON DER STEINE SEINE S			
River/River Stretch	Land (ha)	House (Nos)	Lost (Mill. Pesos)
A. D.	:		
Agno River	1,290	1 270	158
RM-AG045		1,370	and the second s
AG045-AG122	7,489	750	120
AG122-AG282	2,232	900	76
AG282-AG307	941	190	21
AG307-AG309	1,010	100	16
AG309-AG351	1,860	190	30
AG351-AG405	2,438	250	39
AG405-AG473	2,940	150	30
Total	20,200	3,900	490
I. Tarlac River			1 17
AG180-TA200	885	90	14
TA200-TA265	2,768	90	30
Total	3,653	180	44
II. Tributaries of Agno River			
Ambayoan River	334	0	3
Viray-Dipalo River	651	. 0	3 7
Banila River	555	0	6
Camiling River	408	0	4
Canning Kivei	100	· ·	
Total	1,948	0	20
V. Allied River			
Cayanga-Patalan River	850	. 0	9
Bued River	910	0	
Aloragat River	206	0	9 2
Total	1,966	0	20
. Pantal-Sinocalan River	•		
Pantal-Sinocalan River	880	0	9
Binalonan Floodway	68	60	á
Ingalera River	402	0	4.
	445	Ö	4
Dagupan River Macalong River	495	0	5
Total	2,290	60	26
•	•		

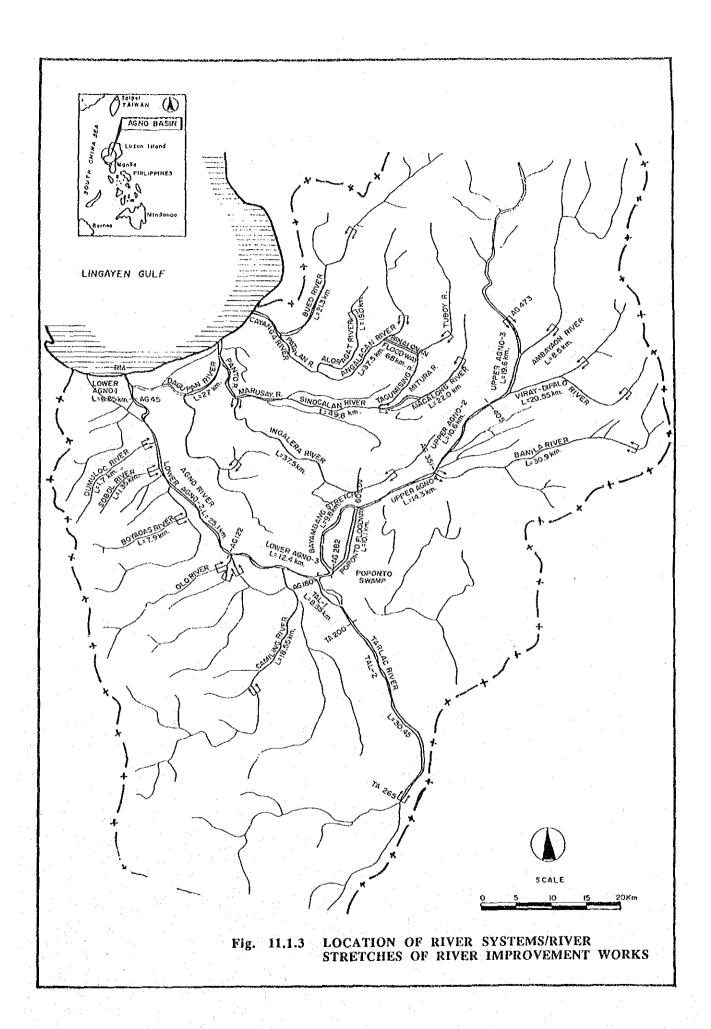


Note: For Financial Project Cost, all Items are included.

For Economic Project Cost, underlined items are excluded.

Fig. 11.1.1 CONSTITUTION OF PROJECT COST (CONTRACT SYSTEM)





12. ECONOMIC EVALUATION

12. ECONOMIC EVALUATION

12.1 Benefit

The cumulative annual average probable flood damages under the 1989 condition without the project are estimated for the flood frequencies of 1.05, 2, 5, 10, 25, 50 and 100 years as discussed in Section 5.4.

The amount of these annual average probable flood damages which can be reduced by the flood control structural measures (river improvement, dams, retarding basins, floodways) is accounted for by the flood control benefit at 1989 constant price level. The annual average flood control benefit is compiled in Table 12.1.1 for respective river stretches and basins by different flood frequencies.

The study employs the following two benefit flows during the project life of 50 years:

a) Current Development Level (Constant Benefit Flow)

In this case it is assumed that the damageable assets and indirect damages in the Study Area are kept as they are now throughout the project life, computed at 1989 constant price.

b) Future Development Level (Future Growth of Benefit Flow)

In this case the region's damageable assets and indirect damages in the Study Area will increase at the same rate as the region's likely GRDP growth rate (hereinafter called low growth case).

Since the benefit flow of the current development level does not reflect the actual condition of the future damages in the Study Area correctly, the benefit flow of the future development level is adopted for assessing viability of the project, while that of current development level is used as supplemental information.

For projecting the future damageable assets and indirect damages, cases of high growth and low growth are reviewed. The high growth case assumes

that the region's damageable assets and indirect damages will increase at the same rate as the region's targeted GRDP growth rate (projected by NEDA) while the low growth case assumes that those of the region will increase at the same rate taken from the actual performance of the region's economy during the period 1975-1982. The projected growth factors are as follows:

	1989	1990	1991	2000	2001	2010	2044
High Growth Case		. 1.			1.		* 1 . ·
.GRDP Growth Rate(%/yr)	5.9	5.9	5.2	5.2	5.6	5.6	5.6
.Growth Factor	1.000	1.059	1.114	1.850	1.953	3.368	22.68
Low Growth Case				-		*	
.GRDP Growth Rate(%/yr)	5.2	5.2	5.2	5.2	5.2	5.2	5.2
.Growth Factor	1.0	1.052	1.107	1.837	1.933	3.209	18.92
the gath each resign of the						7	

In this study the of low GRDP growth case is adopted because it will reflect more realistic future damages in the Study Area.

12.2 Economic Benefit Cost Analysis

Optimization of Long Term Plan

This study employs the normative economic evaluation of benefit cost analysis. The economic internal rate of return (EIRR) is calculated as the optimization criteria. The Long Term Plan is considered as an independent flood control and protection project for the flood prone areas in the Study Area.

For the optimization of the development scale (flood control scale) of the Long Term Plan the following evaluation criteria are adopted for the benefit cost analysis:

(1)	Base Year for B/C Analysis	Beginning of 1990
(2)	Project Life	50 year (from 1995-2044)
(3)	Start of Construction	Beginning of 1995
(4)	Construction Period	15 years (from 1995-2009)

(5)	Disbursement Schedule	Uniform distribution of total
		project cost during the
	and the second of the second of the second	construction period
(6)	Annual Operation and	0.5% of main construction cost
	Maintenance Cost	and physical contingency of
		completed works
(7)	Price Escalation Rate	No escalation
(8)	Timing of Benefits (a)	River improvement and floodway:
		in proportion to the completion rate
		of the main construction works.
	(b)	Dam: the year the whole dam is
•		completed.
(9)	Growth Factor (GF) of (a)	Current development level:
	Future Benefits	constant (GF=1.0)
		Future development level:
		low growth of GRDP (see Section
		12.1)
	•	

The results of the benefit cost analysis are summarized in Table 12.2.1 and the optimization result is illustrated in Figure 12.2.1. The optimum development scale is assessed for the case of future development level in terms of EIRR as set out below:

Agno River with Tarlac	25 year flood	16.6 (3.9)
Agno River Tributaries	25 year flood	15.5 (3.1)
Allied Rivers	10 year flood	33.8 (15.2

The EIRR in the parentheses is the case of current development level.

The cash flows of the optimum Long Term Plan are shown in Table 12.2.2 for the Agno River, Table 2.2.3 for the Agno River Tributaries and Table 2.2.4 for the Allied Rivers.

Priority Project Areas

The priority flood control areas identified in Section 13.1.2 are assessed for the 10-year flood in terms of ETRR assuming the same evaluation criteria. The calculation results of the benefit cost analysis are summarized in Table 13.1.1.

12.3 Project Evaluation

The result of the economic benefit cost analysis presented in Section 12.2 justifies that the Long Term Plan is economically viable with the economic internal rate of return of 20% in the whole Study Area.

If all the river improvement works of the Long Term Plan are implemented by the end of year 2009, the monetary and non-monetary benefits will be attained as summarized below.

Flood Protection

- . Agno River and its tributaries; 25-year flood
- . Pantal-Sinocalan River and

Cayanga-Patalan River

; 10-year flood

Benefited Area (protected area)

. 1,400 km²

; 57% of the potential inundation area

Benefited Resident (protected people)

. 2.1 million at 2010 ; 65% of the residents in the Study Area or 90% in the potential inundation area

Annual Average Economic Benefit

- . 1.03 billion pesos/year at 1989
- . 3.31 billion pesos/year at 2010 (1989 price level)

Other Non Monetary Benefits

- . Reduction in water-borne parasitic diseases including malaria which usually break out after large floods
- . Reduction in loss of lives
- . Enhancement of socio-economic stability in the Study Area
- . Creation of job opportunities to the local people

Table 12.1.1 ECONOMIC FLOOD CONTROL BENEFIT BY FLOOD FREQUENCIES

		<u> </u>	Jnit:millio	n pesos/yr
	100-year Flood	50-year Flood	25-year Flood	10-year Flood
River	4			· · · · · · · · · · · · · · · · · · ·
AGNO RIVER			ing the state of t	
- Agno Main Stream - Tarlac River	532 45	503 42	456 37	346 26
Total	577	545	493	372
AGNO TRIBUTARIES	· · —	76	70	58
ALLIED RIVERS		-		
- Cayanga-Patalan River - Pantol-Sinocalan River		100 406	97 400	90 381
Total	-	506	497	471
Study Area	· -	1,127	1,060	901

Table 12.2.1 ECONOMIC COST, BENEFIT AND INTERNAL RATE OF RETURN OF THE ALTERNATIVE LONG TERM PLANS

	100 Year Flood (Million Pesos)	50 Year (Million	Pesos)	25 Year (Million	Pesos)	10 Year (Million	Pesos
AGNO RIVER MAINSTREAM					~~~~~		
AGNO KIVEK DAINSIKEAD	4 052	* *	£ 054		5,528		4,811
(1) Main Construction Cost (2) Total Project Cost	0,932		6,254				
(2) Total Project Cost	10,340		9,400		8,394		7,413
(3) Benefit (Annual)	532		503		456		346
TARLAC RIVER							
(1) Main Construction Cost	897		792		713		612
(2) Total Project Cost	1,288		1,170		1,061	1	923
(3) Benefit (Annual)	45		42		37	1. 1.	26
AGNO TRIBUTARIES							
(1) Main Construction Cost			1,293		1,012		893
			1,925		1,506		1,330
(2) Total Project Cost			76		70		58
(3) Benefit (Annual) (4) EIRR		(2.1%)		(3.1%)		(2.7%)	14.9
AGNO MAIN AND TARLAC RIVERS	_ 1.					1 .	c
(1) Main Construction Cost			7,056		6,241		5,423
(2) Total Project Cost	11,628]	0,570		9,455		8,336
(3) Benefit (Annual)	577		545		493	4	372
(4) EIRR		(3.8%)	16.5%	(3.9%)	16.67	(2.8%)	15.1
AGNO MAIN, TRIBUTARIES AND TARLAC RIVERS							
(1) Main Construction Cost			8,349		7,253		6,316
(2) Total Project Cost			2,495		10,961		9,666
(3) Benefit (Annual)		•	621		563		430
ALMANAL BARRATAN BANER		with floo	diron	with flo	odway .	elth flo	വർയാഗ
CAYANGA-PATALAN RIVER		WICH IIO	837	WICH \$10	777	WICH IIO	715
(1) Main Construction Cost					1.150		1 000
(2) Total Project Cost			1,246		1,159		1,000
(3) Benefit (Annual)			100		97		90
PANTAL-SINOCALAN RIVER	,	with floo	dway	with flo			
(1) Main Construction Cost			1,715		1,546		1,319
(2) Total Project Cost			2,553		2,303		1,965
(3) Benefit (Annual)			406		400		381
ALLIED RIVERS		with floo	dway	with flo	odway	with flo	odway
(1) Main Construction Cost			2,552		2,323		2,027
			3,799		3,462	4	3,020
(2) Total Project Cost			506		497		471
(3) Benefit (Annual) (4) EIRR		(12.9%)		(13.9%)		(15.2%)	
	n n Turen a						
AGNO MAIN, TARLAC, AND ALLIE (1) Main Construction Cost		•	9.608		8,564		7,450
(2) Total Project Cost		. 1	4,369		12,917		11,356
(3) Benefit (Annual)			1,051		990		843
(4) BIRR		(6.4%)	20.4%	(6.9%)	21.0%	(6.6%)	20.6
AGNO MAIN, TARLAC, TRIBUTARI	ES						
AND ALLIED RIVERS (STUDY A	and the second s	_					
(1) Main Construction Cost			0,901		9,576		8,343
(2) Total Project Cost		1	6,294		14,423	•	12,686
(3) Benefit (Annual)		•	1,127		1,060		901
(4) EIIR		(6.0%)	19.7%	(6.5%)	20.5%	(6.27)	20.0

Remarks

a) Cost and benefit at constant 1989 prices.
b) Economic Internal Rate of Return (EIRR) in parentheses are for the case of current development level, i.e. constant benefit.

c) EIRR values without parentheses are for the case of future development level, i.a. increasing at the growth rate of the GDP of the inundated area under the "low growth" scenario.

Table 12.2,2 CASH FLOW AND ECONOMIC BENEFIT COST ANALYSIS FOR AGNO RIVER: LONG TERM PLAN (CURRENT LEVEL) (1/2)

AGNO RIVER BASIN FLOOD CONTROL STUDY			ALUATION PE	
**************************************	Main	Other	Total	
CASE NO. LAG-A2-25 Constant Benefit	Const.	Costs	Cost	Unit : Million Pesos
RIVER IMPROVEMENT WORK	6241	3214	9455	
DAM CONSTRUCTION WORK	Ö	O	0	A Park
ANNUAL BENEFIT	493 mil	. Pesos		· ·
GROWTH FACTORS 2000	1.000			.9
2010	1.000			
2044	1,000			

CALCULATED EIRR

2040

2041

2042

2043

2044

51 52

53

54

55

3,86%

		C	ost Stream	n.			•	service of		
No.	Year	River	Dam	ON River	OM Dam	Total	Benefit	B-C		GROWTH FACTOR
0	1989			<u> </u>		0.00	0.00	0.00		1.000
1	1990				100	0.00	0.00	0.00		1.000
2	1991	100				0.00	0.00	0.00		1.000
3	1992	111			71	0.00	0.00	0.00		1.000
4	1993				:	0.00	0.00	0.00	. :	1.000
5	1994		- :		**	0.00	0.00	0.00		1.000
6	1995	630.33	0.00	0.00		630.33	0.00	-630.33		1.000
7	1996	630.33	0.00	2.39		632.73	32.87	-599.86		1.000
8	1997	630.33	0.00	4.78		635.12	65.73	-569.38		1.000
9	1998	630.33	0.00	7.18		637.51	98.60	-538.91		1.000
10	1999	630.33	0.00	9.57		639.90	131.47	-508.44		1.000
11	2000	630.33		11.96	0.00	642.30	164.33	-477.96	43.	1.000
12	2001	630.33	•	14.35	0.00	644.69	197.20	-447.49		1.000
13	2002	630.33	** ***	16.75	0.00	647.08	230.07	-417.01	1.44	1.000
14	2003	630.33		19.14	0.00	649.47	262.93	-386.54		1.000
15	2004	630.33		21.53	0.00	651.86	295.80	-356.06		1.000
16	2005	630.33		23.92	0.00	654.26	328.67	-325.59		1.000
17	2006	630.33		26.32	0.00	656.65	361.53	-295.12	•	1.000
18	2007	630.33		28.71	0.00	659.04	394.40	-264 64		1.000
19	2008	630.33	• *	31.10	0.00	661.43	427.27	-234.17	. :	1.000
20	2009	630.33		33.49	0.00	663.83	460.13	-203.69		1.000
21	2010			35.89	0.00	35.89	493.00	457.11		1.000
22	2011			35.89	0.00	35,89	493.00	457.11	1.1	1.000
23	2012			35.89	0.00	35.89	493.00	457.11		1.000
24	2013			35.89	0.00	35.89	493.00	457.11	• • •	1.000
25	2014	*, *,		35.89	0.00	35.89	493.00	457.11		1.000
26	2015			35.89	0.00	35.89	493.00	457.11		1.000
27	2016			35.89	0.00	35.89	493.00	457 11	1111	1.000
28	2017			35.89	0.00	35.89	493.00	457.11		1.000
29	2018	- :-		35.89	0.00	35.89	493.00	457.11	. *. * .	1,000
30	2019		100	35.89	0.00	35.89	493.00	457 11		1.000
31	2020		4 -	35.89	0.00	35.89	493.00	457.11	: *	1.000
32	2021			35.89	0.00	35.89	493.00	457.11		1.000
33	2022			35.89	0.00	35.89	493.00	457.11		1.000
34	2023	1.0		35.89	0.00	35.89	493.00	457.11	**	1.000
35	2024	25%		35.89	0.00	35.89	493.00	457.11		1.000
36	2025	1000	**	35.89	0.00	35.89	493.00	457.11		1.000
37	2026			35.89	0.00	35.89	493.00	457.11		1.000
38	2027		100	35.89	0.00	35.89	493.00	457.11	2.5	1.000
39	2028	700	1.4 (2)	35.89	0.00	35.89	493.00	457.11		1.000
40	2029			35.89	0.00	35.89	493.00	457.11		1,000
41	2030	100		35.89	0.00	35.89	493.00	457.11	1.1	1.000
42	2031		, a to sassi	35.89	0.00	35.89	493.00	457.11		1.000
43	2032		grafier in the	35.89	0.00	35.89	493.00	457.11		1.000
44	2033	1000	1000	35.89	0.00	35.89	493.00	457.11	100	1.000
45	2034	1995年第	all of the state	35.89	0.00	35.89	493.00	457.11	1000	1.000
46	2035	* * * * * * * * * * * * * * * * * * *	14.35	35.89	0.00	35.89	493.00	457.11	44.5	1.000
47	2036	500	- 1 - 4 - 4	35.89	0.00	35.89	493.00	457.11		1.000
48	2037	10,700	The state	35.89	0.00	35.89	493.00			1.000
49	2038	25 July 1	, the said	35.89	0.00	35.89	493.00	457.11	Michigan	1.000
50	2039		i kalendari	35.89	0.00	35.89	493.00	457.11	. 3 1 4 .	1.000
JU Ri	2040	12.41.1		35.89	0.00	35.89	493.00	457.11		1.000

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Table 12.2.2 CASH FLOW AND ECONOMIC BENEFIT COST ANALYSIS FOR AGNO RIVER: LONG TERM PLAN (CURRENT LEVEL) (2/2)

AGNO RIVER BASIN FLOOD CONTROL STUDY	ECONOMIC EVALUATION PROGRAM (at 1989 Constant Prices)
CASE NO. LAG-A2-25 Low Growth RIVER IMPROVEMENT WORK DAM CONSTRUCTION WORK	Main Other Total Const. Gosts Cost Unit: Million Pesos 6241 3214 9455 0 0 0
ANNUAL BENEFIT	493 mil. Pesos
GROWTH FACTORS 2000	1.837
2010	3,209
2044	18.920

CALCULATED EIRR

16.57%

		C	Cost Strea	m							
No.	Year	River	Dam	OM River	OM Dam	Total		Benefit	B-C		GROWTH FACTOR
								0.00	0.00		1.000
0	1989					0.00		0.00	0.00		1.076
1	1990	2	•			0.00		0.00	0.00	,	1.152
. 2	1991					0.00		0.00	0.00	1.5	1.228
3	1992	11				0.00		0.00	0.00		1.304
4	1993					0.00		0.00	0.00	1.5	1.380
5 6	1994 1995	630.33	0.00	0.00		630.33			-630.33	•	1.457
. 7	1996	630.33	0.00	2.39	. •	632.73		50.37	-582.35	1.	1.533
8	1997	630.33	0.00	4.78		635.12		105.75	-529.37		1.609
. 9	1998	630.33	0.00	7.18		637.51		166.12	-471.39		1.685
. 10	1999	630.33	0.00	9.57		639.90		231.50	-408.40		1.761
11	2000	630.33		11.96	0.00	642.30		301.88	-340.41		1.837
12	2001	630.33		14.35	0.00	644-69		389.31	-255.38		1.974
13	2002	630.33		16.75	0.00	647.08	٠.	485.76	-161.32 -58.24	•:	2.111 2.249
14	2003	630.33		19.14	0.00	649.47		591.23 705.72	53.85		2.386
15	2004	630.33		21.53	0.00	651.86		829.23	174.97		2.523
16	2005	630.33		23.92	0.00	654.26		961.75	305.10		2.660
17	2006	630.33		26.32	0.00	656.65 659.04		1103.29	444.25		2.797
18	2007	630.33		28.71 31.10	0.00	661.43		1253.86	592.42		2.935
19	2008 2009	630.33 630.33		33.49	0.00	663.83		1413.44	749.61		3.072
20	2010	620.33		35.89	0.00	35.89		1582.04	1546.15	4.5	3.209
21 22	2011			35.89	0.00	35.89		1809.85	1773.96		3.671
23	2012			35.89	0.00	35.89	1	2037.66	2001.77		4,133
24	2013		•	35.89	0.00	35.89		2265.47	2229.58		4.595
25	2014			35.89	0.00	35.89		2493.28	2457.39		5.057
26	2015			35.89	0.00	35.89		2721.08	2685.20	$\lambda = T$	5.519
27	2016		-	35.89	0.00	35.89		2948.89	2913.01		5.982
28	2017			35.89	0.00	35.89		3176.70	3140.82		6.444 6.906
29	2018			35.89	0.00	35.89		3404.51	3368.63		7.368
. 30	2019			35.89	0.00	35.89		3632.32 3860.13	3596.44 3824.25	11/1/20	7.830
- 31	2020			35.89	0.00	35.89 35.89	100	4087.94	4052.06		8.292
: 32	2021			35.89	0.00 0.00	35.89	•	4315.75	4279.87	2.4	8.754
33	2022			35.89 35.89	0.00	35.89	100	4543.56	4507.67		9.216
34	2023			35.89	0.00	35.89		4771.37	4735.48		9.678
35 36	2024 2025			35.89	0.00	35.89		4999.18	4963.29		10.140
37	2026			35.89	0.00	35.89		5226.99	5191.10		10.602
38	2027			35.89	0.00	35.89		5454.80	5418.91	4 1	11.065
39	2028			35.89	0.00	35.89	100	5682.61	5646.72		11.527
40	2029			35.89	0.00	35.89		5910.42	5874.53	1.5	11.989
41	2030			35.89	0.00	35.89		6138.23	6102.34	4.0	12.451
42	2031			35.89	0.00	35.89		6366.04	6330.15	100	12.913
43	2032	1.5		35.89	0.00	35.89	7.5	6593.85	6557.96	1 1 1 1 1 1 1 1	13.375
- 44	2033			35.89	0.00	35.89		6821.66	6785.77	477.7	13.837 14.299
45	2034			35.89	0.00	35.89	*	7049.47	7013.58		14.761
46	2035			35.89	0.00	35.89		7277.27	7241.39 7469.20	10 miles	15.223
47	2036			35.89	0.00	35.89 35.89			7697.01	144	15.685
48	2037			35.89	0.00	35.89			7924.82		16.147
49	2038			35.89 35.89	0.00	35.89	. 3	8188.51			16.610
50 51	2039 2040			35.89	0.00	35.89	1.		8380.44		17.072
51 52	2040			35.89	0.00	35.89			8608.25	1	17.534
53	2042			35.89	0.00	35.89	1.		8836.06	1000	17.996
54	2043	* *		35.89	0.00	35.89			9063.86	1.16.	18.458
55	2044			35.89	0.00	35.89		9327.56	9291.67	9.4	18.920
	77.1						· · ·			·	

Table 12.2.3 CASH FLOW AND ECONOMIC BENEFIT COST ANALYSIS FOR AGNO RIVER TRIBUTARIES: LONG TERM PLAN (CURRENT LEVEL) (1/2)

AGNO RIVER BASIN FLOOD CONTROL STUDY					ALUATION P		
	CASE NO. LAT-AT-25 Constant Benef RIVER IMPROVEMENT WORK DAM CONSTRUCTION WORK ANNUAL BENEFIT	it	Main Const. 1012 0 70 mil.	Other Costs 494	Total Cost 1506 0	•	Million Pesos
	20	000 10 144	1.000 1.000 1.000	·	¥ .		

CALCULATED EIRR

3.08%

No. Year River Dam OH River Dam OH Total Benefit B-C CROWTE				Cost Stre	am							
1 1990	No.	Year	River	Dam			Total	· .	Benefit	B-C		
1 1990	0	1989	. :		· · · · · · · · · · · · · · · · · · ·		0.00		0.00	0.00		1.000
2			· · ·									
3 1992												
4 1993				-								
5												
6 1995 100.40 0.00 0.00 100.40 0.00 -1.00.40 1.000 7 1996 100.40 0.00 0.39 100.79 4.67 -96.12 1.000 8 1997 100.40 0.00 0.78 101.18 9.33 -91.84 1.000 10 1998 100.40 0.00 1.16 101.15 14.00 -87.56 1.000 10 1999 100.40 0.00 1.55 101.95 18.67 -83.29 1.000 11 2000 100.40 2.33 0.00 102.34 23.33 -79.01 1.000 12 2001 100.40 2.33 0.00 102.73 28.30 -74.73 1.000 13 2002 100.40 2.72 0.00 103.12 32.67 -70.45 1.001 14 2003 100.40 3.10 0.00 103.50 37.33 -66.17 1.000 15 2004 100.40 3.49 0.00 103.50 37.33 -66.17 1.000 16 2005 100.40 3.89 0.00 104.28 46.67 -57.61 1.000 17 2006 100.40 4.27 0.00 104.88 42.00 -61.89 1.000 18 2007 100.40 4.66 0.00 105.06 56.00 -49.06 1.000 19 2008 100.40 5.04 0.00 105.44 66.67 -44.78 1.000 19 2008 100.40 5.04 0.00 105.83 65.33 -49.06 1.000 20 2009 100.40 5.43 0.00 105.83 65.33 -40.50 1.000 21 2010 5.82 0.00 5.82 70.00 64.18 1.000 22 2011 5.82 0.00 5.82 70.00 64.18 1.000 23 2012 5.82 0.00 5.82 70.00 64.18 1.000 24 2013 5.82 0.00 5.82 70.00 64.18 1.000 25 2014 5.82 0.00 5.82 70.00 64.18 1.000 26 2015 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 29 2018 5.82 0.00 5.82 70.00 64.18 1.000 20 2009 100.40 5.83 65.82 70.00 64.18 1.000 21 2000 5.82 70.00 64.18 1.000 22 2010 5.82 0.00 5.82 70.00 64.18 1.000 23 2012 5.82 0.00 5.82 70.00 64.18 1.000 24 2013 5.82 0.00 5.82 70.00 64.18 1.000 25 2014 5.82 0.00 5.82 70.00 64.18 1.000 26 2015 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 29 2018 5.82 0.00 5.82 70.00 64.18 1.000 20 2009 5.82 70.00 64.18 1.000 21 2000 5.82 70.00 64.18 1.000 21 2000 5.82 70.00 64.18 1.000 22 2013 5.82 0.00 5.82 70.00 64.18 1.000 23 2020 5.82 0.00 5.82 70.00 64.18 1.000 24 2033 5.82 0.00 5.82 70.00 64.18 1.000 25 2015 5.82 0.00 5.82 70.00 64.18 1.000 26 205 5.82 0.00 5.82 70.00 64.18 1.000 27 206 5.82 70.00 64.18 1.000 28 207 5.82 0.00 5.82 70.00 64.18 1.000 29 5.82 0.00 5.82 70.00 64.18 1.000 20 20 5.82 70.00 64.18 1.000 20 20 5.82 70.00 64.18 1.000 2	5		1	2.4								
7 1996 100.40 0.00 0.79 100.79 4.67 -96.12 1.000 9 1998 100.40 0.00 0.78 101.18 9.33 -91.84 1.000 10 1999 100.40 0.00 1.15 101.95 18.67 -83.29 1.000 11 2000 100.40 1.94 0.00 102.34 23.33 -9.01 1.000 12 2001 100.40 2.73 0.00 102.73 28.00 -74.73 1.000 14 2003 100.40 2.72 0.00 103.12 32.67 -70.45 1.000 15 2004 100.40 3.49 0.00 103.89 42.00 -61.89 1.000 16 2005 100.40 3.88 0.00 104.67 51.33 -33.33 1.000 17 2006 100.40 4.66 0.00 105.46 66.67 -55.61 1.000 18 2			100.40	0.00	0.00	÷ .					1.	
8 1997 100.40 0.00 0.78 101.18 9.33 -91.84 1.000 9 1998 100.40 0.00 1.155 101.95 18.67 -83.29 1.000 10 1999 100.40 0.00 102.34 22.33 -79.01 1.000 12 2001 100.40 2.72 0.00 103.12 22.60 -74.73 1.000 13 2002 100.40 2.72 0.00 103.12 32.67 -70.45 1.000 14 2003 100.40 3.10 0.00 103.18 32.67 -70.45 1.000 15 2004 100.40 3.49 0.00 103.89 42.00 -61.89 1.000 16 2005 100.40 4.27 0.00 104.67 51.33 -53.33 1.000 17 2006 100.40 4.66 0.00 105.06 56.00 -49.06 1.000 20 2009 <t< td=""><td>7</td><td>1996</td><td>100.40</td><td>0.00</td><td>0.39</td><td>*</td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	7	1996	100.40	0.00	0.39	*						
10	8	1997	100.40	0.00	0.78		101.18	1	9,33	-91.84	1.14	1.000
11 2000 100.40 1.94 0.00 102.34 23.33 -79.01 1.000 12 2001 100.40 2.33 0.00 102.73 28.00 -74.73 1.000 13 2002 100.40 2.72 0.00 103.12 32.67 -70.45 1.000 14 2003 100.40 3.10 0.00 103.50 37.33 -66.17 1.000 15 2004 100.40 3.49 0.00 103.89 42.00 -61.89 1.000 16 2005 100.40 3.88 0.00 104.28 46.67 -57.61 1.000 17 2006 100.40 4.27 0.00 104.67 51.33 -55.33 1.000 18 2007 100.40 4.66 0.00 105.06 56.00 -90.66 1.000 20 2009 100.40 5.43 0.00 105.44 60.67 -44.78 1.000 20 2009 100.40 5.43 0.00 105.44 60.67 -44.78 1.000 21 2010 5.82 0.00 5.82 70.00 64.18 1.000 22 2011 5.82 0.00 5.82 70.00 64.18 1.000 22 2011 5.82 0.00 5.82 70.00 64.18 1.000 24 2013 5.82 0.00 5.82 70.00 64.18 1.000 25 2014 5.82 0.00 5.82 70.00 64.18 1.000 25 2014 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 29 2018 5.82 0.00 5.82 70.00 64.18 1.000 30 2019 5.82 0.00 5.82 70.00 64.18 1.000 31 2020 5.82 0.00 5.82 70.00 64.18 1.000 31 2020 5.82 0.00 5.82 70.00 64.18 1.000 31 2020 5.82 0.00 5.82 70.00 64.18 1.000 31 2020 5.82 0.00 5.82 70.00 64.18 1.000 31 2024 5.82 0.00 5.82 70.00 64.18 1.000 31 2024 5.82 0.00 5.82 70.00 64.18 1.000 31 2024 5.82 0.00 5.82 70.00 64	9	1998	100.40	0.00	1.16	100	101.56	400	14:00	-87.56		1.000
12 2001 100.40 2.33 0.00 102.73 28.00 -74.73 1.000 13 2002 100.40 2.72 0.00 103.12 32.67 -70.45 1.000 14 2003 100.40 3.10 0.00 103.15 37.33 -66.17 1.000 15 2004 100.40 3.48 0.00 103.89 42.00 -61.89 1.000 16 2005 100.40 3.88 0.00 104.28 46.67 -57.61 1.000 17 2006 100.40 4.66 0.00 105.06 56.00 -49.06 1.000 18 2007 100.40 4.66 0.00 105.06 56.00 -49.06 1.000 19 2008 100.40 5.04 0.00 105.44 60.67 -44.78 1.000 20 2009 100.40 5.82 0.00 5.82 70.00 64.18 1.000 21 2010 5.82 0.00 5.82 70.00 64.18 1.000 22 2011 5.82 0.00 5.82 70.00 64.18 1.000 24 2013 5.82 0.00 5.82 70.00 64.18 1.000 25 2014 5.82 0.00 5.82 70.00 64.18 1.000 26 2015 5.82 0.00 5.82 70.00 64.18 1.000 22 2016 5.82 0.00 5.82 70.00 64.18 1.000 23 2012 5.82 0.00 5.82 70.00 64.18 1.000 24 2013 5.82 0.00 5.82 70.00 64.18 1.000 25 2014 5.82 0.00 5.82 70.00 64.18 1.000 27 2016 5.82 0.00 5.82 70.00 64.18 1.000 28 2017 5.82 0.00 5.82 70.00 64.18 1.000 29 2018 5.82 0.00 5.82 70.00 64.18 1.000 30 2019 5.82 0.00 5.82 70.00 64.18 1.000 31 2020 5.82 0.00 5.82 70.00 64.18 1.000 32 2021 5.82 0.00 5.82 70.00 64.18 1.000 33 2029 5.82 0.00 5.82 70.00 64.18 1.000 34 2023 5.82 0.00 5.82 70.00 64.18 1.000 35 2024 5.82 0.00 5.82 70.00 64.18 1.000 36 2025 5.82 0.00 5.82 70.00 64.18 1.000 37 2026 5.82 0.00 5.82 70.00 64.18 1.000 38 2027 5.82 0.00 5.82 70.00 64.18 1.000 39 2028 5.82 0.00 5.82 70.00 64.18 1.000 44 2033 5.82 0.00 5.82 70.00 64.18 1.000 45 2034 5.82 0.00 5.82 70.00 64.18 1.	10	1999	100.40	0.00	1.55	4.	101.95	:	18.67	-83.29		1.000
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5.82 0.00 5.82 70.00 64.18 1.000							1.0	100			- 1	
The state of the s						4.5		7.00			47	and the second second
20.00 04.18 1.000												
	55	2044			3.62	0.00	3.02		70.00	04.18		1.000

Table 12.2.3 CASH FLOW AND ECONOMIC BENEFIT COST ANALYSIS FOR AGNO RIVER TRIBUTARIES: LONG TERM PLAN (CURRENT LEVEL) (2/2)

AGNO RIVER BASIN FLOOD CONTROL STUDY ECONOMIC EVALUATION PROGRAM

CASE NO. LAT-AT-25 Low Growth		Main	Other	Constant Total Cost		: Million Pesos
RIVER IMPROVEMENT WORK	1	Const.	Costs 494	1506	OHLL	. Intigation 1 does
DAM CONSTRUCTION WORK		0		. 0		
ANNUAL BENEFIT		70	mil. Pesos	;		•
GROWTH FACTORS	2000	1.837		1	*	Si 4
	2010	3.209				
	2044	18.920				

CALCULATED EIRR

15.46%

Cost Stream

No.	Year	River	Dam	OM River	OM Dam	Total		Benefit	B-C	9.5	GROWTH FACTOR
					 				A 20		. 000
. 0	1989		•			0.00		0.00	0.00		1.000
ī	1990				,	0.00		0.00	0.00	100	1.076
2	1991	100				0.00		0.00	0.00		1.152 1.228
3	1992	1.0	* 1		N	0.00		0.00	0.00		1.228
4	1993					0.00		0.00			1.380
5	1994	400.40				0.00		0.00	0.00 -100.40	1	1.457
6	1995	100.40	0.00	0.00	• •	100.40		7.15	-93.64		1.533
7	1996	100.40	0.00	0.39		100.79		15.01	-86.16		1.609
8.	1997	100.40	0.00	0.78		101.18 101.56		23.59	-77.98		1.685
9	1998	100.40	0.00	1.16	**		•	32.87	-69.08		1.761
10	1999	100.40	0.00	1,55	0.00	101.95			-59.48		1.837
11	2000	100.40		1.94	0.00	102.34		42.86 55.28	-47.45		1.974
12	2001	100.40	٠	2.33	0.00	102.73		68.97	-34.14		2.111
13	2002	100.40		2.72	0.00	103.12	*	83.95	-19.56		2.249
14	2003	100.40		3.10	0.00	103.50		100.20	-3.69		2.386
15	2004	100.40	÷	3.49	0.00	103.89			13.46	10.5	2.523
16	2005	100.40		3.88	0.00	104.28		117.74	31.89		2.660
- 17	2006	100.40		4.27	0.00	104.67		136.56		A1 4 1	2.797
7 18	2007	100.40		4.66	0.00	105.06	1.5	156.65	51.60		2.935
: 19	2008	100.40		5.04	0.00	105.44		178.03	72.59	1	
20	2009	100.40		5.43	0.00	105.83		200.69	94.86		3.072
21	2010			5.82	0.00	5.82		224.63	218.81	1174	3.209
22	2011	100		5.82	0.00	5.82		256.98	251.16	. •	3.671
23	2012		•	5.82	0.00	5.82		289.32	283.50	100	4.133
24	2013			5.82	0.00	5.82		321.67	315.85		4.595
25	2014		14	5.82	0.00	5.82		354.01	348.20		5.057
26	2015		***	5.82	0.00	5,82		386.36	380.54		5.519
27	2016		**	5.82	0.00	5.82		418.71	412.89		5.982
28	2017	: .	٠.	5.82	0.00	5.82		451.05	445.23		6.444
29	2018		•	5.82	0.00	5.82	•	483.40	477.58		6.906
30	2019			5.82	0.00	5.82		515.75	509.93	100	7.368
31	2020			5.82	0.00	5.82		548.09	542.27		7.830
32	2021	4.1		5.82	0.00	5.82		580.44	574.62	1.4	8.292
33	2022	+		5.82	0.00	5.82		612.78	606.97		8.754
34	2023	:	-	5.82	0.00	5.82		645.13	639.31		9.216
35	2024		7	5.82	0.00	5.82		677.48	671.66		9.678
36	2025	*	* -	5.82	0.00	5,82		709.82	704.00		10.140
37	2026			5.82	0.00	5.82		742.17		1. 1.	10.602
- 38	2027	1		5.82	0.00	5.82		774.52	768.70	21-1	11.065
39	2028			5.82	0.00	5.82		806.86	801.04		11.527
40	2029			5.82	0.00	5.82		839,21	833.39		11.989
41	2030	* .		5.82	0.00	5.82		871.55	865.73	1. Thinks	12.451
42	2031	100	•	5.82	0.00	5.82		903.90	898.08		12.913
43	2032			5.82	0.00	5.82		936.25	930.43		13.375
44	2033			5.82	0.00	5.82	* •	968.59	962.77		13.837
45	2034		2	5.82	0.00	5.82		1000.94	995.12	11 a	14.299
46	2035		1.	5.82	0.00	5.82		1033.28	1027.47	1. 1.	14.761
47	2036		.* . *	5.82	0.00	5.82		1065.63	1059.81	1.34	15.223
48	2037			5.82	0.00	5.82	*	1097.98	1092.16		15.685
49	2038	4.5		5.82	0.00	5.82		1130.32	1124.50	algarita e	16.147
50	2039	4		5.82	0.00	5.82	1	1162.67	1156.85	100000	16.610
51	2040			5.82	0.00	5.82		1195.02	1189.20	14.74%	17.072
52	2041		44	5.82	0.00	5,82		1227.36	1221.54	1 Jan 1975	17.534
∘. 53	2042		4	5.82	0.00	5.82		1259.71	1253.89	1.1.4.3	17.996
54	2043	-		5.82	0.00	5.82		1292.05	1286.23	1. 35	18.458
55	2044			5.82	0.00	5.82		1324.40	1318.58	14,00	18.920
									· · · · · · · · · · · · · · · · · · ·	100	1

Table 12.2.4 CASH FLOW AND ECONOMIC BENEFIT COST ANALYSIS FOR ALLIED RIVERS: LONG TERM PLAN (CURRENT LEVEL) (1/2)

		AGNO RIVER DASIN FLOOD CONTROL STUDY				(at 1989 Constant Prices)					
		CASE NO. LA RIVER IMPRO DAM CONSTRU	OVEMENT W	ORK	Benefit	Main Const. 2027 0	Costs 993 O	Cost 3020 3020	Unit	: Million Pasos	
		ANNUAL BENT GROWTH FACT			2000	471 mil. 1.000	. Pesos				
		GROWIN PAG.	iono		2010	1.000			,		
					2044	1.000					
		CALCULATED			•	15.187			٠.		
		Co	ost Stream	n 	~ = W = 2						
No.	Year	River	Dam	OM	ЮW	Total	Benef	lt	B-C	GROWTH	
•				River	Dam				•	YACTOR	
0	1989		 			0,00		.00	0.00	1.000	
1	1990					0.00		.00	0.00	1.000	
2	1991					0.00	0	•00	0.00	1.000	
` 3	1992					0.00	0	.00	0.00	1.000	
4	1993					0.00	0	.00	0.00	1.000	
5	1994		_			0.00	0	.00	0.00	1.000	
6	1995		0.00	0.00		201.33			,	1.000	
7	1996	201.33	0.00	0.78		202.11			-170.71	1.000	
. 8	1997	201.33	0.00	1.55		202.89			-140.09	1.000	
9	1998	201.33	0.00	2.33		203.66			-109.46	000.1	
10	1999	201.33	0.00	3.11	0.40	204.44	125		-78.84	1.000	
. 11	2000	201.33		3.89	0.00	205.22	157		-48.22	1.000	
12	2001	201.33		4.66	0.00	206.00	188 219		-17.60	1.000	
13 14	2002 2003	201.33 201.33		5.44 6.22	0.00	206.77 207.55	251		13.03 43.65	1.000 1.000	
15	2004	201.33		6.99	0.00	208.33	282		74.27	1.000	
16	2005	201.33		7.77	0.00	209.10		.00	104.90	1,000	
: 17	2006	201.33		8.55	0.00	209.88		40	135.52	1.000	
18	2007		`	9.32	0.00	210.66	376		166 14	1.000	
19	2008	201.33		10.10	0.00	211,43	408		196.77	1.000	
20	2009	201.33		10.88	0.00	212.21	439	.60	227.39	1.000	
21	2010			11.66	0.00	11.66	471	.00	459.34	1.000	
22	2011			11.66	0.00	11.66	471	.00	459.34	1.000	
23	2012		4	11.66	0,00	11.66	471	.00	459.34	1,000	
24	2013	•		11.66	0.00	11.66	471		459.34	1,000	
25	2014			11.66	0.00	11.66	471		459.34	1.000	
26	2015	•		11.66	0.00	11.66	471		459.34	1.000	
27	2016			11.66	0.00	11.66	471		459.34	1.000	
28	2017	•		11.66	0.00	11.66	471		459.34	1.000	
29	2018			11.66	0.00	11.66		.00	459.34	1.000	
30	2019			11.66	0.00	11.66 11.66		.00	459,34	1.000	
31	2020			11.66	0.00	11.66	471 471		459.34 459.34	1.000	
32	2021			11.66	0.00	11.66				1.000	
33 34	2022 2023			11.66 11.66	0.00	11.66	. 471 471		459.34 459.34	1.000	
35	2023			11.66	0.00	11.66	471		459.34	1.000	
36	2025			11.66	0.00	11.66	471		459.34	1.000	
37	2026			11,66	0.00	11.66	471		459.34	1,000	
38	2027			11.66	0.00	11,66	471		459.34	1.000	
39	2028			11.66	0.00	11.66	471	.00	459.34	1.000	
40	2029			11.66	0.00	11.66	471	.00	459.34	1.000	
41	2030			11.66	0.00	11.66	471		459.34	1.000	
42	2031			11.66	0.00	11.66	471		459.34	1.000	
43	2032			11.66	0.00	11.66	471		459.34	1.000	
44	2033	•		11.66	0.00	11.66	471		459.34	1.000	
` 45	2034	•		11.66	0.00	11.66	471		459.34	1.000	
46	2035		_	11.66	0.00	11.66 11.66	471 471		459.34	1.000	
47	2036	•		11.66	0.00				459.34	1.000	
48	2037 2038			11.66 11.66	0.00	11.66 11.66	471 471		459.34 459.34	1.000 1.000	
49 50	2038			11.66	0.00	11.66	471		459.34	1.000	
50 51	2039	•		11.66	0.00	11.66	471		459.34	1.000	
52	2040	Ĭ		11.66	0.00	11.66	471		459.34	1.000	
52 53	2042			11.66	0.00	11.66	471		459.34	1,000	
54	2043			11.66	0.00	11.66	471		459.34	1.000	
55	2044	• ,	•	11.66	0.00	11.66	471		459.34	1.000	
, 7											

Table 12.2.4 CASH FLOW AND ECONOMIC BENEFIT COST ANALYSIS FOR ALLIED RIVERS: LONG TERM PLAN (CURRENT LEVEL) (2/2)

AGNO RIVER BASIN FLOOD CONTROL STUDY

ECONOMIC EVALUATION PROGRAM (at 1989 Constant Prices)

Cost

3020

0

CASE NO. LAL-AT-10 Low Growth RIVER IMPROVEMENT WORK DAM CONSTRUCTION WORK ANNUAL BENEFIT GROWTH FACTORS

Total Main Other Const. Costs 2027 993 0 0 471 mil. Pesos

Unit : Million Pesos

2000 1,837 2010

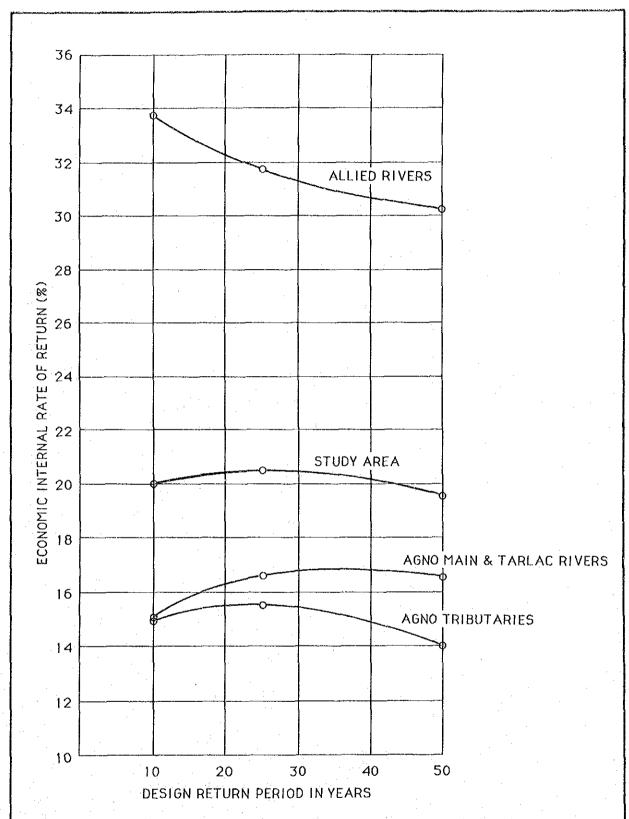
3.209 2044 18.920

CALCULATED KIRR

33.77%

Cost Stream

			OSC OFICE	ш						
No.	Year	River	Dam	OM River	OM Dam	Total		Benefit	B-C	GROWTH FACTOR
1. 1. 11.				· ·					· · · · · · · · · · · · · · · · · · ·	
0	1989					0,00		0.00	0.00	1.000
1	1990					0.00		0.00	0.00	1.076
2	1991					0.00		0.00	0.00	1.152
3	1992				-	0.00		0.00	0.00	1.228
. 4	1993				400	0.00		0.00	0.00	1.304
- 5	1994		24 12			0.00		0.00	0.00	1.380
6	1995	201.33	0.00	0.00		201.33		0.00		1.457
1 J. 7	1996	201.33	0.00	0.78	100	202.11		48.12	-153.99	1.533
. 8	1997	201.33	0.00	1.55		202.89		101.03	-101.86	1.609
9	1998	201.33	0.00	2.33		203.66		158.71	-44.95	1,685
10	1999	201.33	0.00	3.11		204.44		221.17	16.73	1.761
11	2000	201.33		3.89	0.00	205.22		288.41	83.19	1.837 1.974
12	2001	201.33		4.66	0.00	206.00		371.94	165.94 257.31	2.111
. 13	2002	201.33		5.44	0.00	206.77		464.09	357.30	2.249
14	2003	201.33		6.22	0.00	207.55		564.85		
15	2004	201.33	:	6.99	0.00	208.33		674.23	465,90	2.386
16	2005	201.33		7.77	0.00	209.10		792.22	583.12	2.523
17	2006	201.33	•	8.55	0.00	209.88		918.83	708.95	2.660
18	2007	201.33		9.32	0.00	210.66	* *	1054.06	843.40	2.797
19	2008	201.33		10.10	0.00	211.43	•	1197.90	986.47	2.935
20	2009	201.33	-	10.88	0.00	212.21		1350.36	1138,15	3.072
21	2010	•	*	11.66	0.00	11.66		1511.44	1499.78	3.209 3.671
22	2011			11.66	0.00	11.66		1729.08 1946.73	1717.43	4.133
23	2012			11.66	0.00	11.66		2164.37	1935.07 2152.71	4.595
24	2013			11.66	0.00	11.66		2382.01		5.057
25	2014			11.66	0.00	11.66			2370.36	5.519
26	2015			11.66	0.00	11.66 11.66		2599.66 2817.30	2588.00	5.982
27	2016			11.66		11.66		3034.94	2805.65 3023.29	6.444
28	2017			11.66 11.66	0.00			3252.59	3240.93	6.906
29 30	2018			11.66	0.00	11.66 11.66		3470.23	3458.58	7.368
	2019 2020		•	11.66	0.00	li.66		3687.87	3676.22	7.830
31 32	2021			11.66	0.00	11.66		3905.52	3893.86	8.292
33	2021			11.66	0.00	11.66		4123.16	4111.51	8.754
34	2023			11.66	0.00	11.66		4340.81	4329.15	9.216
35	2024			11.66	0.00	11.66			4546.79	9.678
36	2025			11.66	0.00	11.66		4776.09	4764.44	10.140
37	2026			11.66	0.00	11.66	٠.	4993.74	4982.08	10.602
38	2027			11.66	0.00	11.66		5211.38	5199.72	11.065
39	2028			11.66	0.00	11.66		5429.02	5417.37	11,527
40	2029		•	11.66	0.00	11.66		5646.67	5635.01	11.989
41	2030			11.66	0.00	11.66		5864.31	5852.65	12,451
42	2031			11.66	0.00	11.66		6081.95		12,913
43	2032			11.66	0.00	11.66		6299,60	6287.94	13,375
44	2033			11.66	0.00	11.66		6517.24	6505.59	13.837
45	2034			11.66	0.00	11.66		6734.88		14.299
46	2035			11.66	0.00	11.66		6952.53	6940.87	14.761
47	2036	* -		11.66	0.00	11.66		7170.17	7158.52	15.223
48	2037			11.66	0.00	11.66		7387.82	7376.16	15.685
49	2038			11.66	0.00	11.66		7605.46	7593.80	16.147
50	2039			11.66	0.00	11.66		7823.10	7811.45	16.610
51	2040			11.66	0.00	11.66		8040.75	8029.09	17.072
52	2041		•	11.66	0.00	11.66		8258.39	8246.73	17.534
52 53	2042	1.5		11.66	0.00	11.66	•	8476.03	8464.38	17.996
54	2042		5 (11.66	0.00	11.66		8693.68	8682.02	18.458
55	2044		J.	11.66	0.00	11.66		8911.32	8899.66	18.920
33	4047			21.00	4.00					



Remarks: The minimum requirement of the flood control scale is set at 10-year flood.

Fig. 12.2.1 OPTIMIZATION OF DEVELOPMENT SCALE FOR LONG TERM PLAN

13. IMPLEMENTATION PROGRAM

13. IMPLEMENTATION PROGRAM

13.1 Priority Project Areas Subject to Feasibility Study

13.1.1 Alternative Priority Flood Control Areas

Among the areas under the Long Term Plan formulated in Section 10.3.2, the priority flood control areas are to be identified for pursuing the Feasibility Study. The priority flood control areas are defined as:

- a) The areas where existing and potential flood damages are high, and flood protection measures are required to be implemented with the highest priority.
- b) The areas where flood control performance is high in terms of economic efficiency and social and regional impact.
- c) The areas whose discharge carrying capacity is less than the 10year design flood.

The Long Term Plan areas, therefore, are assessed for the 10-year design flood in terms of flood control efficiency (EIRR; economic internal rate of return). The main stream of the Agno River is divided into four stretches for assessment; the downstream stretch (river mouth-AG180), Bayambang-Poponto swamp area (AG180-AG309), the upstream stretch (AG309-AG473) and the Tarlac River (AG180-TA265). The four Agno River tributaries (Camiling, Banila, Viray-Dipalo, Ambayoan) are assessed independently. The Pantal-Sinocalan and Cayanga-Patalan Rivers are also assessed independently.

Since construction of a new diking system in the upstream stretch (AG309-AG473) of the Agno River will confine flood runoff inside the new river area and induce significant increase of flood discharge in the downstream stretches (river mouth to AG309) as illustrated in Figure 13.1.1 and Figure 13.1.2, improvement of the upstream stretch can only be implemented together with the improvement of the Bayambang stretch including the new Poponto floodway or the whole river improvement. Under this precondition, three alternatives, Case 1 to 3 are formulated for the Agno River main stream (river mouth to AG473).

The river stretches which have discharge carrying capacity less than 10-year design flood, in particular in the significantly affected region are identified as the stretches subject to first priority flood control measures. Such river stretches of the Agno River are illustrated in the longitudinal profile in Figure 13.1.4. The location and the corresponding length of the stretches and their carrying capacities are assessed as summarized in Table 13.1.2 for the Agno River and Table 13.1.3 for the Allied Rivers.

13.1.2 Selection of Priority Project Areas

The result of economic assessment tabulated in Table 13.1.1 indicates that the combined river improvement from the Bayambang stretch with the Poponto retarding basin (AG180) to the upstream end (AG473) is the most significant in the Agno River, while the Pantal-Sinocalan River gains the highest EIRR in the Study Area. The following three significant flood control areas are identified:

- No.1 Upper Agno River: Case 2, Bayambang stretch with Poponto retarding basin to upstream end
- No.2 Pantal-Sinocalan River
- No.3 Cayanga-Patalan River with Bued River

The location of these areas is shown in Figure 13.1.3.

Among the foregoing three projects, Project-A, Upper Agno River and Project-B, Pantal-Sinocalan River are chosen as the Priority Project Areas to be retained for Feasibility Study taking account of the economic efficiency and regional significance of flood control.

- A. Upper Agno River; Bayambang stretch with Poponto retarding basin (AG180) to the San Manuel stretch (AG473)
 - . Improvement of Bayambang stretch of the main Agno River

100,004,04

- . Improvement of Poponto floodway
- . Improvement of Upper Agno stretch

- B. Pantal-Sinocalan River; River mouth to the upstream to protect Dagupan city, Calasiao and Santa Barbara towns
 - . Improvement of the main Pantal-Sinocalan River
 - . Improvement of Dagupan River
 - . Improvement of Ingalera River

The economic internal rate of return is preliminarily estimated to be about 24% for Project-A and 40% for the Project-B under future development level.

13.2 Implementation Schedule

Alternative implementation programs for the Long Term Plan are formulated for the target year 2010 and 2020 as shown in Figure 13.2.1 and Figure 13.2.2 respectively. The total project cost of the Long Term Plan, which is estimated to be 15,974 million pesos at 1989 constant price level, corresponds to about 2.5% of the projected cumulative GRDP of the Study Area in the period 1995-2009 (16.9 billion pesos).

Since this amount of public fund required for the flood control works is very high if it is compared with the present level (some 0.5% of GRDP), the study recommends implementing the Long Term Plan by the end of the year 2019 as shown in Figure 13.2.2.

Table 13.1.1 ASSESSMENT OF PRIORITY FLOOD CONTROL AREAS

10-year Flood Protection

	Project Cost (million pesos)	Annual Benefit (million pesos)	ETRR (%)	Order of Flood Control Efficiency	Selected Priority Project Area	Weight of River Importance
AGNO MAIN STREAM						1
Case 1: Lower Agno (RM-AG282)	5,069 (4,685)	95.5	9.3	7	÷.	
Case 2: Poponto Stretch (AG180-AG309) and Upper Agno (AG309-AG473)	3,102 (2,728)	250.4	23.6	2	No.1	
Case 3: The Whole River (RM-AG473)	8,170 (7,413)	345.9	15.5	4		
TARLAC RIVER (AG180-TA265)	1,221 (923)	25.8	11.3	6		2
AGNO TRIBUTARIES	1,455 (1,330)	58.1	14.9	5	•	5
. Camiling River	303 (278)	9.3	12.7			
. Banila River	694 (636)	31.3	16.0			
. Viray-Dipalo River	291 (264)	12.1	15.3	• .		t to see
. Ambayoan River	167 (152)	5.4	13.1			
PANYAL-SINOCALAN RIVER	2,160 (2,000)	391.0	39.9	; 1	No.2	3
CAYANGA-PATALAN RIVER	1,126 (1,020)	79.7	21.3	3	No.3	4

Remarks:

⁽¹⁾ The project cost is the financial cost at 1989 constant price level.

The project cost in the parentheses is the economic cost.

⁽²⁾ Annual benefit is the economic price at 1989 constant level.

⁽³⁾ EIRR is the economic internal rate of return for the case of future increase of benefit under lower economic growth.

Table 13.1.2 ASSESSMENT OF CARRYING CAPACITY OF PRIORITY AREAS OF AGNO RIVER AND TARLAC RIVER

River/Stretch	Length of	Design			ng Carrying Capacity			
	stretch	Discharge	Discharge (and the second second	Q<10yr 10	• •	Q>25yr	Length
	(km)	(m3/s)	Max,	Min.	Lengt	h of strete	ch (km)	(km)
. Agno River		10-year						
 Lower Agno River 				1.				
(1) R.M-AG45	6.05	6500	R 6000	2500	6.85	0.00	0.00	6.85
			L 6000	2500	6.85	0.00	0.00	6.85
(2.1) AG45-AG65	9,05	6500	R 8200	3300	2,50	6.05	0.50	2.50
			L 4800	1200	6.10	1.50	1.45	4.20
(2.2) AG65-AG122	16.05	6500	R 13800	6400	1.20	6.45	8.40	1.20
	:		L 4000	1000	16.05	0.00	0.00	0.00
(3) AG122-AG202	12.40	5900-5500	R 8400	4400	8.30	4.10	0.00	8.30
			L 3400	1600	12.40	0.00	0.00	0.00
Sub-total of 1	44,35	· •	R		18.85	16,60	8.90	18,85
			Ĺ -		41.40	1,50	1.45	11.05
	•							1 1
2. Poponto Stretch	5.50	1600	2000	1100	4.50	1.00	0.00	3.40
(1.1) D/S of Bayamban (1.2) U/S of Bayamban	4.95	1600	3000	1500	1.00	2.95	1.00	0.00
	5.50	-	3000		5.50	0.00	0.00	5.50
(2.1) Retarding Basin	4.60	2400	3200	700	3.60	0.50	0.50	3.60
(2.2) Floodway	4.60	, 2400	3200	700	3.60	V.50	0.50	3.00
Sub-total of 2	20.55	-		-	14.60	4.45	1.50	12,50
							* .	
Upper Agno River		7		3.11	12112		132 124	1.14.
(1) AG309-AG351	14.30	4000	R 8400		4.00	7.90	2,40	4 -00
4.5			L 14000	1300	6.00	3.00	5.30	6.00
(2) AG351-AG405	10,60	3500	R 12900	1600	2,00	4.10	4.50	2.00
- 1		1	L 8200	2600	1.50	4.10	5.00	1.50
(3.1) AG405-AG453	10.05	3500	R 5800	1100	2.50	0.75	6.80	2.50
	_		L 6400	900	7.55	2.00	0.50	2.00
(3.2) AG453-AG473	9.45	2400	R 11200	1000	3.50	1.45	4.50	3.50
			L 12000	1000	3.65	3.00	2.80	0.00
Sub-total of 3	44.40	• .	R	•	12.00	14.20	18.20	12.00
	•		L -	-	19.70	12,10	13.60	9,50
otal of X	109.30	_	R -	-	45.45	35.25	28.60	43.3
OFFICE OF A	205,30		L -	-	74.70	18.05	16.55	33,0
			-				general garage	20,70
I. Tarlac River		10-year						
(1) AG180-TA200								
Ratarding Basin	8,10	_		-	8.10	0.00	0.00	0.00
(2.1) TA200-TA251	24.80	1700	7500	1300	7.50	6.30	11.00	7.50
(2.2) 251-TA265	4.15	1350	5800	1900	0.00	0.00	4.15	0.00
otal of II	37.05				15.60	6.30	15,15	7.50
				٠,	*			

Remarks: R= Right bank;L=Left bank;Q=Discharge

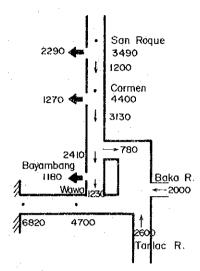
Table 13.1.3 ASSESSMENT OF CARRYING CAPACITY OF PRIORITY AREAS OF ALLIED RIVERS

:						
	River/Stretch	Length	Design Discharge	Existing Capacity	carrying (Minimum)	Requirement of Improvement
		Stretch (km)	(m3/s)	Discharge (m3/s)	Probability (vr)	Length (km)
			Ç,			
-	Fanto-Sinocalan River		10-year			
	(1) 0k-2.5k	2.50	1,900	500 (2)	1.5-2yr	2.50
	(2) 2.5k-8.3k	5.80	1250			5.80
	(3) 8.3k-18.0k	9.70	006	120(4)	1.00yr	9.70
	(4) 18.0k-31.0k	13.00	650			0.00
	(5) Tagumising River	18.40	160	180 (3)	1.5yr	0,00
	Sub-total of 1	49.40				18.00
	2. Dagupan River	27.60	700-190		1	22.00
-	3. Ingalera River	37,50	360-80	75 (3)	אַסס.נ	(Dike for backwater stretch of Panto river)
	4. Macalone River	21.00	130-70	١	ī	(Dike for backwater stretch of Panto river)
	5. Binalonan Flood	0	! !	1	t t	00.0
	Total of I	135.50				52.00
HI.	O.		10-year			
	. 1. Cayanga-Patalan River (1) Ok-6.5k	9,50	1500	830 (2)	3 - 6	O (၁ (၁
	(2) 6.5k-14.8k	8,30	800	260(2)	1.5-2yr	00.0
	(3) Angalacan River	22.70	400-50	180(3)	1.5-2yr	00.0
	Sub-total of 1	37.50				6.50
	 Bued River Aloragat River 	19.80	750-500	380(2)	3-42	15.10
	Total of II	77.00				21.60
	Total of I and II	212.50			÷.	73.60

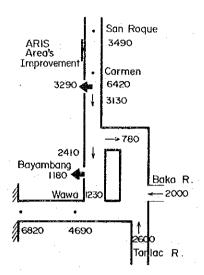
The value in parenthesis is available number of river cross-section for estimation of carrying capavity. Q-Discharge Note: Remarks:

- 278 -

Present Condition (Without any measures)



With Improvement of Breaches in the Upstream Only



With Improvement of Upstream Breaches and Poponto Floodway Only

ARIS Area's Improvement 3490

Carmen 6420
3130

Poponto Floodway 3190

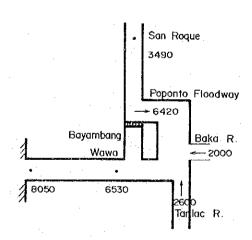
Bayambang Baka R. 2000

7400

5450

Tar lac R.

With Improvement of Breaches, Other Upstream Dikes and Poponto Floodway



Conditions:

- (1) 25-year probable flood
- Unit: m³/sec
- (2) without San Roque dam
- (3) start dike breaching with the water level

over 50% of the freeboad

Fig. 13.1.1 PROBABLE FLOOD DISCHARGE DISTRIBUTION OF AGNO RIVER WITH/WITHOUT RIVER IMPROVEMENT WORKS

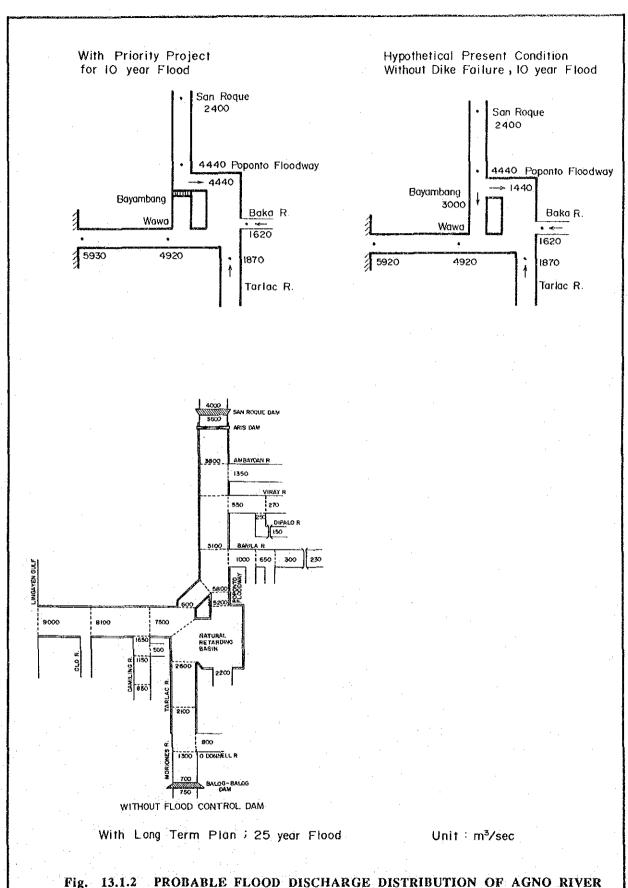
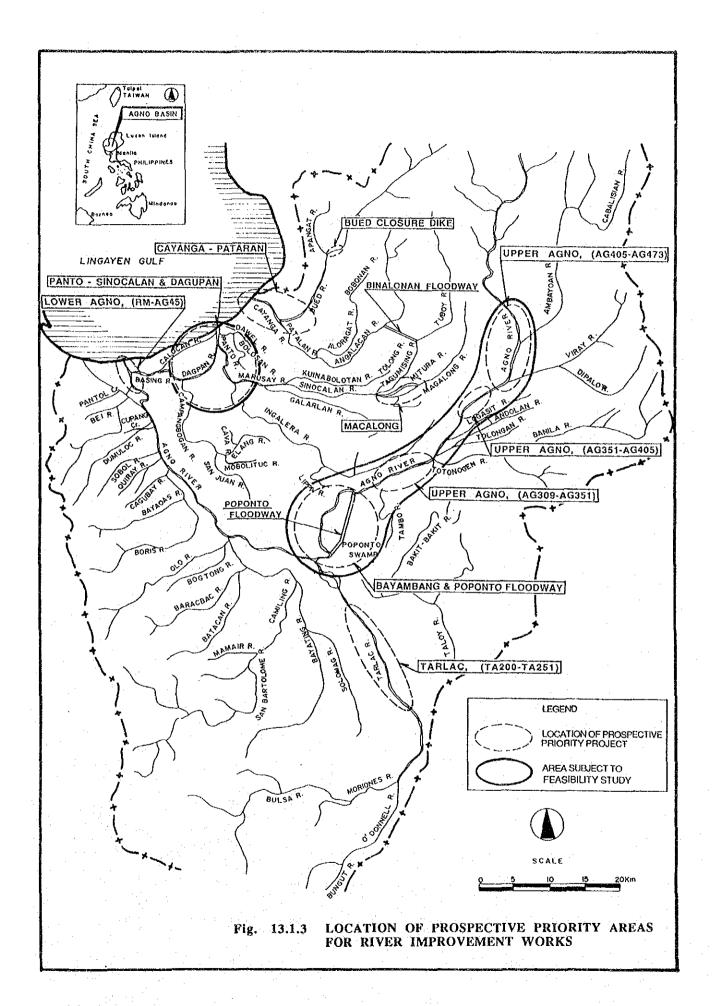
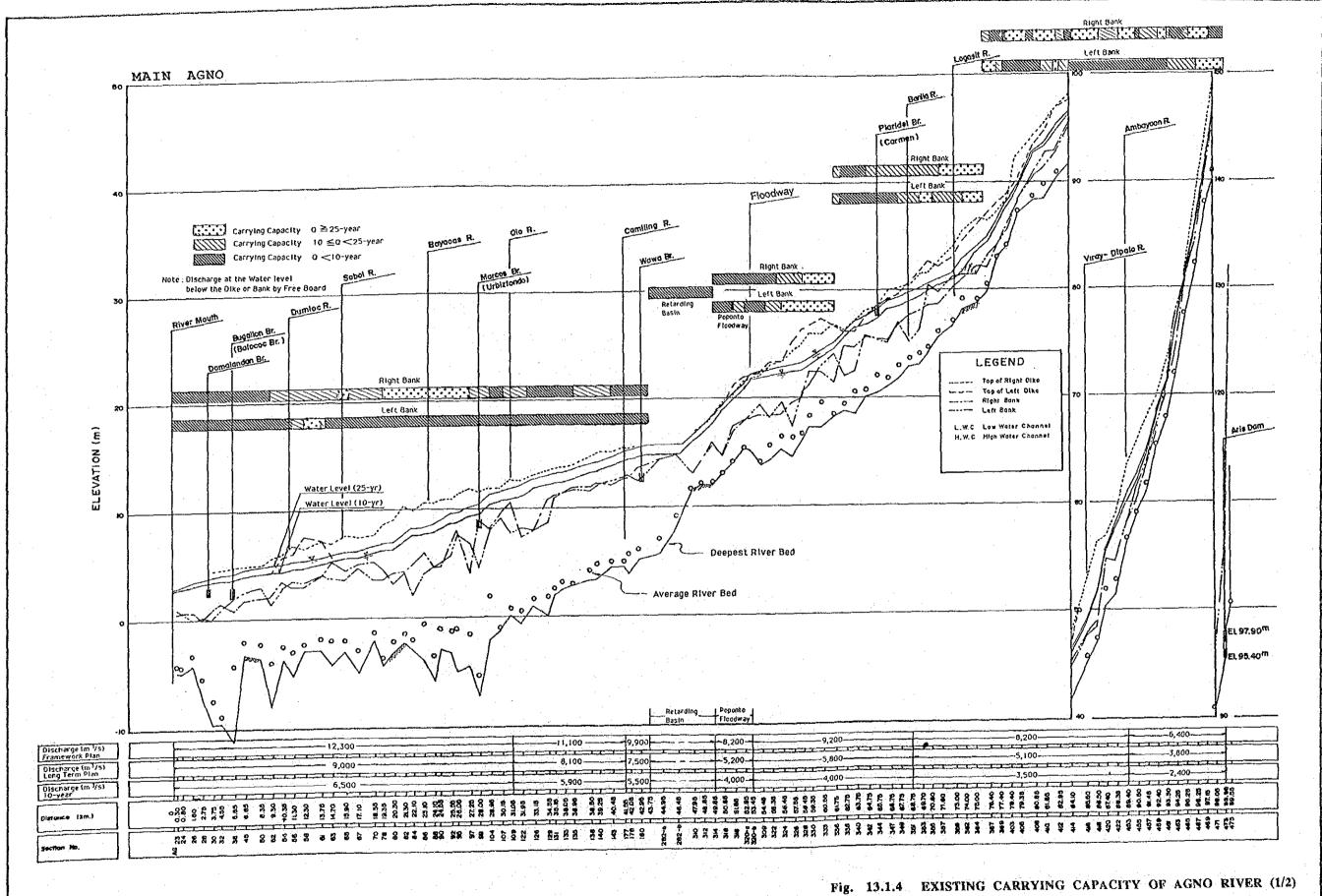


Fig. 13.1.2 PROBABLE FLOOD DISCHARGE DISTRIBUTION OF AGNO RIVER WITH/WITHOUT LONG TERM PLAN AND PRIORITY PROJECT





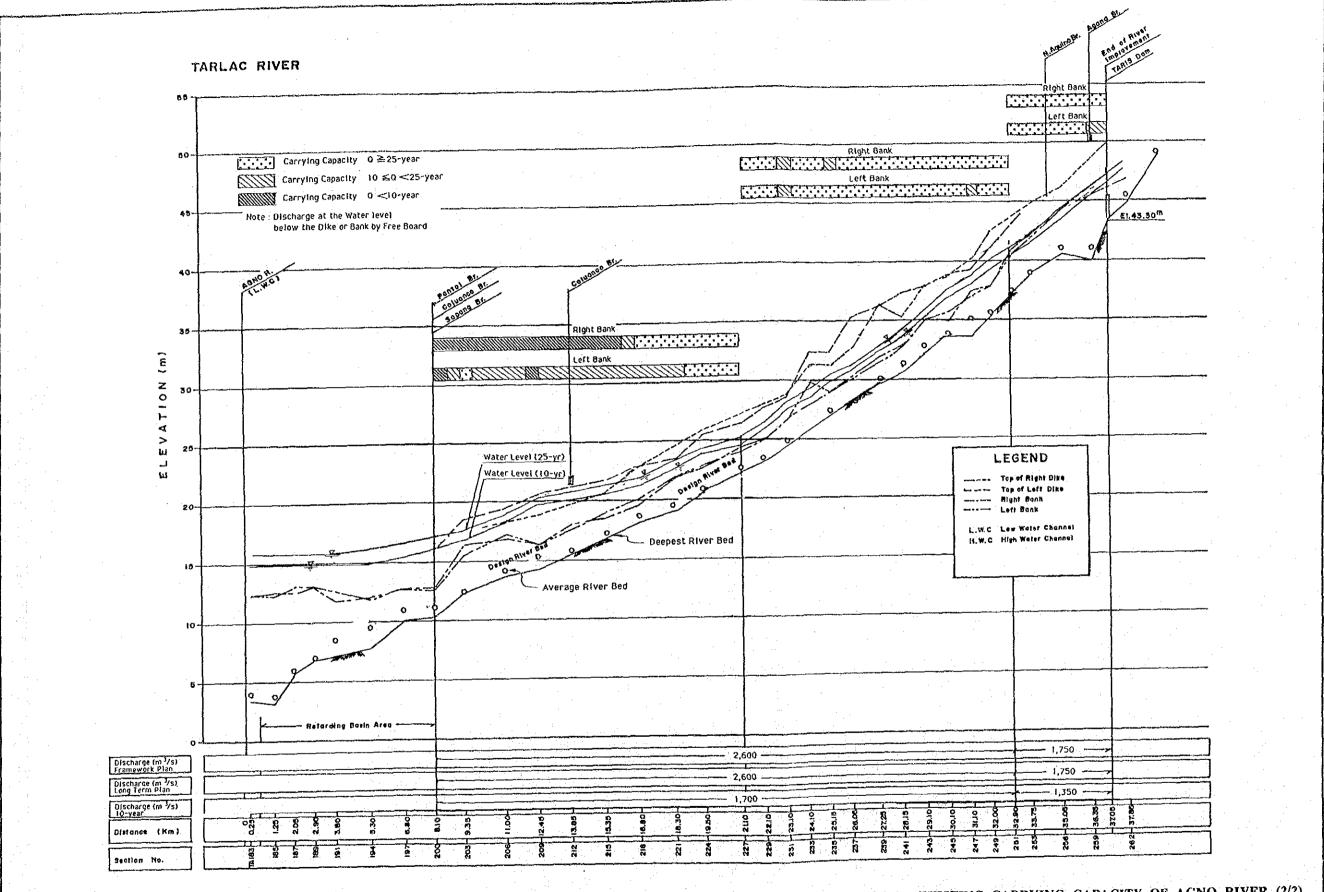


Fig. 13.1.4 EXISTING CARRYING CAPACITY OF AGNO RIVER (2/2)

COMPLETED PURE TO PURE		1005			
COMULATIVE PUBLIC FUND (million pesos)		1995	2000	2005 2009	
1% of cumulative GRDP		297 1,679	2,074 3,881	4,396 6,757	
2.5% of cumulative GRDP		742 4,199	5,185 9,702	10,490 16,893	
	1990		2000		2010 (Target year)
	0 1 2 3 4	56789	01234	56789	0
AGNO RIVER MAIN STREAM (P 9,627 million)					
1) Priority Project					e.
2) Long Term Plan					
TARLAC RIVER (P 1,421 million)					
1) Priority Project					
2) Long Term Plan			777777777	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
AGNO RIVER TRIBUTARIES (P 1,640 million)					
1) Priority Project					
2) Long Term Plan					
PANTAL-SINOCALAN RIVER (P 2,160 million)					
1) Priority Project		<i>11111111111</i>	:		
2) Long Term Plan					
CAYANGA-PATALAN RIVER (P 1,216 million)					
1) Priority Project					
2) Long Term Plan					
PRE-CONSTRUCTION PROCEDURE					

Fig. 13.2.1 ALTERNATIVE IMPLEMENTATION PROGRAM OF LONG TERM PLAN FOR TARGET YEAR 2010

COMULATIVE PUBLIC FUND (million pesos)		1995	2000	2005 2009	2010	2015 2019	2020
i					i		
1% of cumulative GRDP	: ,	297 . 1,679	2,074 3,881	4,396 6,757	7,431 10,494	11,362 15,308	16,426
2.5% of cumulative GRDP		742 4,199	5,185 9,702	10,490 16,893	18,578 26,235		
	1990		2000		2010		
	01234	5 6 7 8 9	0 1 2 3 4	56789	01234	56789	
AGNO RIVER MAIN STREAM (P 9,627 million)							
1) Priority Project							.*
2) Long Term Plan		. *					
TARLAC RIVER (P 1,421 million)	·						
Long Term Plan	·						
AGNO RIVER TRIBUTARIES (P 1,640 million)							.*
Long Term Plan				mamm	mmm	mmm	
PANTAL-SINOCALAN RIVER (P 2,160 million)							
1) Priority Project			aama				
2) Long Term Plan							
CAYANGA-PATALAN RIVER (P 1,216 million)							
Long Term Plan	·			:			
	:						
PRE-CONSTRUCTION PROCEDURE		7.0			+. 1	-	
(Feasibility Study, Detailed Design, Loan Application and Agreement, Bid Procedure, Compensation, etc)							

Fig. 13.2.2 PROPOSED IMPLEMENTATION PROGRAM OF LONG TERM PLAN

14. ENVIRONMENTAL ASSESSMENT

· 14. ENVIRONMENTAL ASSESSMENT

14.1 Objectives of the Environmental Study

The objectives of the Environmental Study on the Agno River Basin Flood Control in the Master Plan stage are as follows;

- (1) To identify impacts on the environment concerned by the Project,
- (2) To evaluate the magnitude/significance of the impacts,
- (3) To judge whether the proposed projects need further environmental study, and if so, to point out the effects to be studied in the Feasibility Study Stage.

14.2 Environmental Impact Assessment in the Master Plan Stage

14.2.1 Methodology of EIA for the Project

To attain the objectives of this environmental study in the Master Plan stage, an Initial Environmental Examination (IEE) is conducted. IEE is essentially an initial examination of the potential environmental effects of the proposed projects based mostly on the preliminary information readily obtainable. IEE is thus a first approach to EIA by scooping, to be carried out at a depth only to determine whether EIA will be required in the next Feasibility Study stage.

A checklist method is applied as a basic tool for IEE in this environmental study, because it is one of the useful initial tools for identification of impacts and evaluation of their significance. The checklist is prepared by placing major items of environmental effects in rows and major project components in columns. The expected effects are evaluated by significance ranging from A to C for each project component with classification as positive or negative. The checklist items are selected by the Study Team taking into consideration the features of the Project and the guidelines prepared by the Government of Philippines (GOP) and the Asian Development Bank (ADB). (Refer to Table 14.2.1)

14.2.2 Result of IEE for the Project

The results of the Initial Environmental Examination for the Framework Plan and the Long Term Plan are presented in Table 14.2.1.

Agno River

The major project components of flood control in the Agno River are the San Roque dam, the Moriones-O'Donnell dam, the river improvement works along the Agno River and the Poponto retarding basin. All of these components could be expected to cause relatively significant effects on the environment, in particular, social impact due to resettlement and evacuation.

First of all, issues of resettlement of the local people are expected especially in the inundation areas of the San Roque dam and Moriones-O'Donnell dam. Agricultural lands in the prospective reservoir areas are also affected by inundation. Secondly, erosion problems in the upstream and downstream areas are expected because San Roque dam and Moriones-O'Donnel dam are located in the erosion-prone area with a slope of 8° - 15°. Water quality deterioration may not be caused by these dams, but eutrophication and saline water intrusion might be expected.

As for the river improvement works in the Agno River and the Poponto retarding basin, these might have significant social impacts due to the acquisition of a right-of-way for new dike construction and the inundation by the basin.

Pantal-Sinocalan River

The major components of the Pantal-Sinocalan River flood control are the river improvement works and Binalonan floodway.

The river improvement works might have significant social impacts due to the acquisition of a right-of-way for new dike construction.

Although no crucial natural environmental issues are expected by the project, water quality deterioration in the downstream area of the Sinocalan

River might be caused by the diversion of flood water from the Tuboy River to the Angalacan River through the Binalonan floodway.

Cayanga-Patalan River

The major components of the Cayanga-Patalan River flood control are the river improvement works and the Bued closing dike. The river improvement works might have significant social impacts due to the acquisition of a right-of-way for new dike construction. The Bued closing dike is not planned to be constructed inside the river channel. It can, therefore, be considered that the natural and social environmental impacts caused by the closing dike are similar to those of the river improvement works except flood flow increases downstream of the dike.

Several environmental impacts are identified in the three project areas. The degree of social impacts due to the location might be significant. The natural environmental impacts could be reduced by taking proper countermeasures.

14.2.3 Conclusion of IEE for the Project

- (1) According to the EIA guidelines of DPWH, preparation of an EIA report is required for the project because it includes two large scale dams in the Framework Plan and the project area is considered prime agricultural land.
- (2) Among the proposed schemes of the Framework Plan, the construction of San Roque, and Moriones-O'Donnell dams, provision of new dikes and extension of Poponto retarding basin may have significant environmental impacts, such as resettlement problems and encroachment of agricultural lands. Thus, most careful attention shall be paid to those prospective socio-economic impacts.
- (3) As for the other schemes, no significant environmental effects would be expected under both the Framework Plan and Long Term Plan. However, some natural environmental impacts of low or medium significance may be expected. Further environmental study is, therefore required to visualize the expected impacts, and to find proper and possible countermeasures.

Table 14.2.1 RESULT OF IEE FOR THE PROJECT

		Agno Ri	ver		ĮΥ	lan River	Cavanga-Pa	talan River
Checklist Item	San Roque Dam	Moriones- O'Donnell Dam	River Improvement	Poponto Reterding Basin	River Improvement	Binalonan Floodway	River Improvement	Bued nent Closing Dike
A) Problems due to the Location								
1. Resettlement/evacuation	-/B	-/A	٧/-	-/A	-/A	-/A	-/A	-/B
2. Encroachment of cultural tribes	0	0	0 -	.: ; O :	0	0	0	0
3. Land value changes	۷ <u>د</u> ۲	۷.÷	∀ / +	¥ ;	∀ .	√. +	∀/+	∢ ?
4. Encroachment of agricultural lands	9/-	-/A	۸/-	۹/-	٧/-	9/-	-/A	2/-
5. Legiectation of forestry	ه ز	0	o (0 (0	۰,	0 (0
7 Encreachment of historical/cultural values	ر د ا	0 0	۰,	0 0	0 0	00	0 0	0 0
8. Watershed erosion/silt runoff	٥/-	U I	. 0	0	0		0	, 0
9. Effects on groundwater hydrology	11	, si	0	0	Ó	٥/-	0	• •
10. Impairment of navigation	0	0	٥/-	0	٥/-	. 0	D/-	0
 Encroachment of precious ecology 	0	0	С	0	0	0	0	0
12. Migrating valuable fish species	0	c	0	c	0	0	0	0
B) Problems related to the Design								
1. Road erosion	-/B	-/B	0	٥	•	٥		0
Water right conflicts)/-	٥	0	0	0	0		
3. Loss of community and recreation areas	0	-/B	٥/-	-/B	2/-	2/-	2/-	5/-
4. Intensification of traffic congestion	0	0	0	0	0	0	٥	o
5. Aesthetic and landscape	٥,	ه ر	0 (0.6	0 (٥ ،	0 (0
or frevention of accessionity	١.) -	o	5	0	o	6	0
C) Problems in Construction Stage	٠, ٢	ر	Ć				٠.	ć
2. Hazards to workers and nearby residents	, v	2/-	٥/-	. 0	ပ/-	'-	- '	0 0
3. Spread of communicable diseases	0	٥	0	0	, o ,	0	٥	0
4. Deterioration of water quality	0	oʻ	o.	•	0	· O	0	o
D) Problems in Operation Stage	Ç	Ç				- ,		
2. Deterioration of mater analysis	۲.	ر - -	5 0	5 6	> (ر د د	> 0	0 (
2. Intercion of caline water) II	O 0	.		>	2 0	o 'c	
4. Europhication	-/B	٥/-	0	ò	, 0	0	0	0
5. Encroachment of precious ecology	O	, oʻ	0	0	0	٥	0	٥
6. Deperciation of fisheries	2/÷		0	c.	o:	0	0.	0
7. Vector disease hazards	· o «	р () +/c) - -) + (0 () () (
o. Acsuicae and tanacapa	2	>	>	•	>			•

Note: (1) /: Upper side is the expected effect, and lower side is its significance.
(2) o: Noeffect expected,

+: Positive effect expected,
: Negative effect expected,
: Negative effect expected,
: there may be a change but such change will be neither benefical nor harmful,
A: Effect which has relatively high level of significance,
B: Effect which has relatively medium level of significance,
C: Effect which has relatively low level of significance,

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15. REVIEW OF MASTER PLAN AFTER RESUMPTION OF STUDY

15. REVIEW OF MASTER PLAN AFTER RESUMPTION OF STUDY

15.1 Suspension and Resumption of the Study

The Master Plan Study was executed in the period of May 25, 1989 - February 15, 1990. The Feasibility Study commenced on May 1, 1990 and was scheduled to be completed by the end of January, 1991. However, the basic study which was scheduled to start from August 1990 was suspended due to the occurrence of an earthquake on July 16, 1990 which inflicted heavy damages to the Study Area. After review of the results of the earthquake damage inspection conducted from September 9 to October 23, 1990, the homework of the Feasibility Study was resumed on November 26, 1990 while work in the Philippines re-started on December 4, 1990.

The Steering Committee and the Study Team confirmed the following basic principles for resumption of the study in the Joint Meeting held on December 12, 1990:

- a. The Master Plan of the Agno River Basin Flood Control established in the Interim Report is unchanged.
- b. The Feasibility Study areas selected in the Interim Report and accepted in the Sixth Joint Meeting held on March 1, 1990, are unchanged; the Upper Agno River and the Lower Pantal-Sinocalan Rivers.

15.2 Review of Framework Plan for Agno River

15.2.1 Existing Condition After the Earthquake

Among the river facilities damaged by the earthquake, the most serious damages have been identified over the diking system in the middle and upper reaches of the Agno River. In pursuing the feasibility studies, DPWH and the Study Team recognized in the Fifth Technical Meeting held on January 14, 1991 that the present condition of the flood control facilities shall be the condition immediately after the earthquake because the restoration works may not be realized due to the current economic condition. In the middle of February, however, DPWH issued the department order to restore all the

damaged flood control facilities, especially dikes to the their original condition before the 1991 flood season. Under this circumstance both DPWH and the Study Team agreed to set the condition after the programmed restoration works as the basic present condition for plan formulation.

15.2.2 Revision of Storage Capacity of Poponto Swamp

The storage area and capacity curves of the Poponto retarding basin were established anew using the new topographic maps with a scale of 1 to 25,000 and were compared with those made in the Master Plan stage using the old topographic maps with a scale of 1 to 50,000. The new topographic maps were made available in December, 1990.

Figure 15.2.1 indicates that the new storage area and storage capacity decrease by about 35% and 40% respectively at elevation 16.0 m. It is assessed that this decrease is brought about by the difference in accuracy of two different maps. Difference of contour lines of these maps is illustrated in Figure 15.2.2. The contour interval of the 1:50,000 maps is 20 m while that of the 1:25,000 maps is 2.5 m. The supplemental contour lines (2.5 m interval) of the 1:50,000 maps are interpolated by use of the point elevations on the maps. The study adopts the new area and capacity curves based on the following standpoints:

- (a) The ground elevation of the 1:50,000 maps was surveyed around 1950
 1960 and there is no way to verify its accuracy at present,
 while that of the 1:25,000 maps was surveyed in 1989.
 - (b) The elevation difference cannot be explained by historical ground transformation, such as sedimentation in the swamp area in the past 20 30 years.

15.2.3 Review of Framework Plan

The general layout of the Framework Plan for the Agno River shown in Figure 10.2.3. was reassessed by using the new high water level of the Poponto natural retarding basin which is made higher due to reduction of the new storage capacity curve. The design flood distribution of the Framework Plan shown in Case 3 of Figure 10.2.1 was modified as shown in Figure 15.2.3. The previous and revised discharges are summarized below.

*	Adopted Frame	The Case of Sole	
Location	Previous discharge	Revised discharge	River Improvement
River mouth	12,300 m ³ /s	13,800 m ³ /s	17,310 m ³ /s
Wawa	$9,900 \text{ m}^3/\text{s}$	11,200 m ³ /s	14,820 m3/s
High Water Level			
of Swamp	El. 16.60m	El. 16.67m	

The project economic cost of the Alternative Framework Plans estimated in Table 10.2.1 was reviewed taking into account the cost increase of the lower reaches of the Agno River due to the increase is design flood discharge. The results summarized in Table 15.2.1 indicate that the least cost case is unchanged, and thus the adopted Framework Plan, combination of river improvement and Poponto natural retarding basin, is unchanged.

The general layout of the Framework Plan for the Allied Rivers shown in Figure 10.2.4 was reassessed by using the new river cross-sections, topographic maps and data. The Framework Plan, which provides the basin transfer of the upstream Tuboy River into the Angalacan River through the Binalonan floodway is unchanged. The design flood discharge distributions of the Framework Plan and the Long Term Plan are adopted unchanged as shown in Figure 10.2.2. and Figure 10.3.1 respectively.

15.2.4 Modification

For pursuing the Feasibility Study the following items were modified based on the foregoing revision:

- 1) Design Flood Distribution of the Agno River
 - . Framework Plan (100-year flood) Figure 15.2.3
 - . Long Term Plan (25-year flood) Figure 15.2.4
- 2) Storage Area and Capacity Curves of Poponto Swamp Figure 15.2.1
- 3) Project Economic Cost of Alternative Framework Plans Table 15.2.1

- 4) Project Financial Cost of the Long Term Plan

 Table 15.2.2 (1/2)-(2/2)
- 5) Features of Design Channel of Agno River for Framework Plan

 Table 15.2.3 (1/2)-(2/2)
- 6) Features of Design Channel of Agno River for Long Term Plan

 Table 15.2.4 (1/2)-(2/2)
- 7) Design Plan of Upper Agno River Figure 15.2.5 (1/5)(5/5)
- 8) Longitudinal Profile of Framework Plan Figure 15.2.6

Table 15.2,1 PROJECT ECONOMIC COST ALTERNATIVE FRAMEWORK PLANS

A. AGNO MAIN AND TARLAC RIVER

Unit: Million Pesos

				ALLEVA 16603
	Case 1 Solo River Improvement	Case 2 River Improvment & Natural Ratarding Basin	Case 3 River Improvement Natural Retarding Basin & Dam	Case 4 River Improvement & Dam
Agno Main Stream	11.472	11,050 (10,700)	10,810 (10,485)	11,202
Tarlac River Moriones-O. Dam	1,587	1,288	1,061 1,811	1,265 1,811
Sub-Total	13.059	*12,338 (11,988)	13,682 (13,357)	14,278
Production Foregone			340	340
Increase in Dredging Reduction in Dredging	2,166	<u>.</u>	-1,979	2 ,166 -1,979
Sub-Total	2,166		-1,639	14,805
Grand-Total	15,225	12,338 (11,988)	*12,043 (11,718)	14,805

Remarks

The values in the parentheses are the previous cost.

The case of the least cost

Table 15.2.2 PROJECT FINANCIAL COST OF LONG TERM PLAN FOR AGNO RIVER

			(Unit:	.000 Pesos)
River	Length (km)	F.C.	L.C.	Total
I. Agno River				
1. Lower Agno River				
(1) RM-AG045 (2) AG045-AG122 (3) AG122-AG282	6.9 25.1 11.9	993,833 2,036,375 1,018,226	706,350 1,001,638 539,801	1,700,184 3,038,013 1,558,026
Sub-total of 1	43.9	4,048,434	2,247,788	6,296,222
2. Poponto Stretch			e e e e e e e e e e e e e e e e e e e	
(1) Bayambang Stretch(2) Poponto Floodway	10.5 10.7	76,139 685,298	53,450 312,500	129,589 997,798
Sub-total of 2	21.2	761,437	365,950	1,127,387
3. Upper Agno River	* *** *			
(1) AG309-AG351 (2) AG351-AG405 (3) AG405-AG473	14.3 10.6 19.5	299,418 222,559 871,344	225,551 155,322 429,655	524,969 377,881 1,300,999
Sub-total of 3	44.4	1,393,321	810,528	2,203,849
Total of I	109.5	6,203,192	3,424,266	9,627,458
II. Tarlac River				
(1) AG180-TA200 (2) TA200-TA265	8.1 29.0	456,111 446,532	184,589 333,839	640,700 780,371
Total of II	37.1 .	902,643	518,428	1,421,071
III. Agno River Tributary				
(1) Camiling River(2) Banila River(3) Viray-Dipalo River(4) Ambayoan River	20.0 30.9 20.1 8.7	225,737 459,202 150,801 101,274	161,015 314,534 149,433 78,013	386,752 773,736 300,234 179,287
Total of III	79.7	937,014	702,995	1,640,009
GRAND TOTAL (I+II+III)	226.3	8,042,849	4,645,689	12,688,538

Table 15.2.3 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR FRAMEWORK PLAN (1/2)

River:Agno River Design Flood: 100-yr

	100	•	Agne	o R.		
Item	Unit RM AG45		AG45 - AG65 - AG65 AG109		AG109 - AG177	
esign Discharge	m3/s	13800	13800	13800	12700	
Distance	m	6850	9050	15150	10500	
Fradient of Channel Bed	-	1/6500	1/6500	1/3500	1/2000	
River width	m	400-300	1500	1500	1500	
lidth of Channel Bed	m	100	300	240	200	
lke Height (Ave.)	m	4.9	5.5	6.6	6.0	
Water Depth	T	8.73-9.75	9.75-11.1	11.1	11.1-9.74	
Low Channel Depth (Ave.)	10	6.5	6.5	6.5	6.5	

		Agno R	Retarding 1>	Floodway	Bayanbang 2
Item	Unit	AG177 - AG181	AG181 - (AG314	AG314 - AG320(b)	AG282(b)- AG307
Design Discharge	m3/s	11200	_	8200	1000
Distance	TO.	2200	7100	3800	9640
Gradient of Channel Bed	-	1/2000	1/1600	1/1600	1/1850
River width	m	1500	<u></u>	1200	250-1300
lidth of Channel Bed	n	200	180	180	80-100
lke Height (Ave.)	ш.	5.6	6.7	5.3	3.3
later Depth	т,	9.74-9.56	9.56-7.80	7.8	8.5-4.1
Low Channel Dapth (Ave.)	m	6.0	4.0	4.0	5.0

l>:Retarding Basin stretch

^{2&}gt;: Bayanbang Stretche of Agno R.

			no R.			
Item	Unit	AG320(b)- AG351	AG351 - AG367	AG367 - AG414	AG414 - AG453	
Design Discharge	m3/s	9200	8200	8200	8200	
Distance	m	15930	8170	8150	5330	
Gradient of Channel Bed		1/1600	1/1300	1/665	1/440	
River width	m	900-1900	1250-3000	3000-2000	2000-1200	
Width of Channel Bed	m ·	180	180	180	150	
Dike Height (Ave.)	m	5.3	4.6	3.9	3.4	
Water Depth	EQ.	7.8	7.8-5.4	5.4	4.9	
Low Channel Depth (Ave.)	m ·	4.0	3.5	3.0	3.0	

Table 15.2.3 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR FRAMEWORK PLAN (2/2)

River:Agno River Design Flood: 100-yr

Item		Agno R.				
	Unit	AG367 - AG460	AG460 - AG464	AG464 - AG469	AG469 - AG474	
Design Discharge	m3/s	6400	6400	6400	6400	
Distance	m	3120	1990	2420	2800	
Gradient of Channel Bed	· -	1/280	1/230	1/230	1/230	
River width	· m	1500-3000	3000-2200	2200-1100	1100-300	
Width of Channel Bed	m	150	150	150	150	
Dike Height (Ave.)	m	2.5	2.5	3.1	3.4	
Water Depth	. m	: 4.0	4.0	4.0-6.3	6.3-7.5	
Low Channel Depth (Ave.)	m	3.0	3.0	3.5	5.0	

Table 15.2.4 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR LONG TERM PLAN (1/2)

River:Agno River Design Flood: 25-yr

Itom	Agno R.						
	Unit	RM - AG45	AG45 - AG65	AG65 AG109	AG109 - AG177		
Design Discharge	m3/s	10100	10100	10100	9300		
Distance	YLL	6850	9050	15150	10500		
Gradient of Channel Bed	-	1/6500	1/6500	1/3500	1/2000		
River width	_ B	1500	(1500)	(1500)	(1500)		
lidth of Channel Bed	m.	360-250	240	200	200		
like Height (Ave.)	en	4.2	4.8	5.4	4.8		
ater Depth	101	8.2-9.2	9.2-10.4	10.4	10.4-9.1		
Low Channel Depth (Ave.)	Ta).	6.5	6.5	6.5	6.5		

	,	Agno R	Retarding 1>	Floodway	Bayanbang 2
Item	Unit	AG177 -	AG181 -	AG314 -	AG282(Ъ)-
	•	AG1,81	AG314	. AG320(Ъ)	AG307
esign Discharge	m3/s	8400		5200	600
istance	. 13	2200	7100	3800	9640
Gradient of Channel Bed	-	1/2000	1/1600	1/1600	1/1850
liver width	. 1m.	(1500)	_	1200	250-1300
lidth of Channel Bed	, z a,	200	180	180	80-100
like Height (Ave.)	m	4.4	4.7	4.2	2.3
later Depth	m	9.1-8.7	8.7-6.7	6.7	7.8-3.8
low Channel Depth (Ave.)	en	6.0	4.0	4.0	5.0

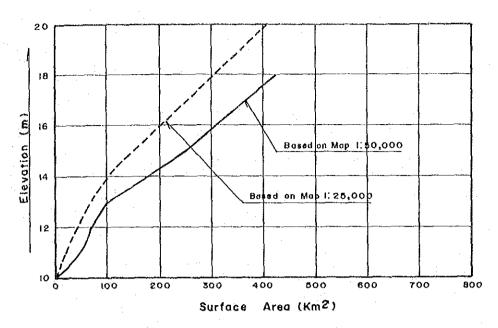
1	>:Retarding	Basin	stretch	2>:Bayanbang	Stretchc	of	Agno	R.

		Agno R.					
Item	Unit	AG320(b)- AG351	AG351 - AG367	AG367 - AG414	AG414 - AG453		
Design Discharge	m3/s	5800	5100	5100	5100		
Distance	m	15930	8170	8150	5330		
Gradient of Channel Bed	_ `	1/1600	1/1300	1/665	1/440		
River width	100	900-1900	1250-3000	3000-2000	2000-1200		
Width of Channel Bed	ш	180	180	180	150		
Dike Height (Ave.)	m	4.2	3.7	3.2	2.9		
Water Depth	m	6.7	6.7-4.7	4.7	4.4		
Low Channel Depth (Ave.)	m	4.0	3.5	3.0	. 3.0		

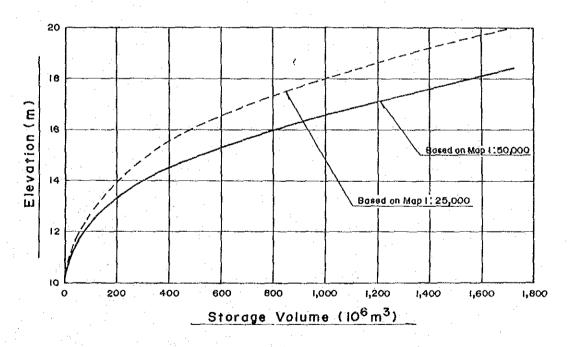
Table 15.2.4 FEATURES OF DESIGN CHANNEL OF AGNO RIVER FOR LONG TERM PLAN (2/2)

River:Agno River Design Flood: 25-yr

	Agno R.							
Item	Unit	AG367 AG460	AG460 ~ AG464	AG464 AG469	AG469 - AG474			
Design Discharge	m3/s	3800	3800	3800	3800			
Distance	m	3120	1990	2420	2800			
Gradient of Channel Bed	_	1/280	1/230	1/230	1/230			
River width	113,	1500-3000	3000-2200	2200-1100	1100-300			
Width of Channel Bed	tu	150	150	150	150			
Dike Height (Ave.)	. 10	1.8	1.8	2.2	2.0			
Water Depth	m	3.6	3.6	3.6-5.4	5.4-6.3			
Low Channel Depth (Avs.)	: m	3.0	3.0	3.5	5.0			

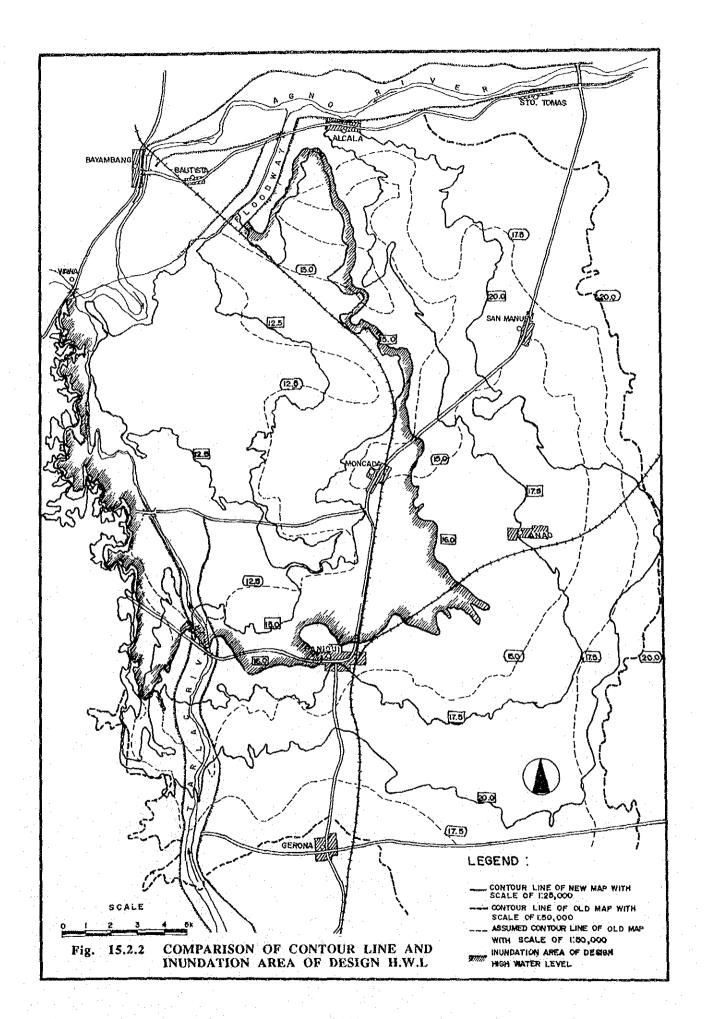


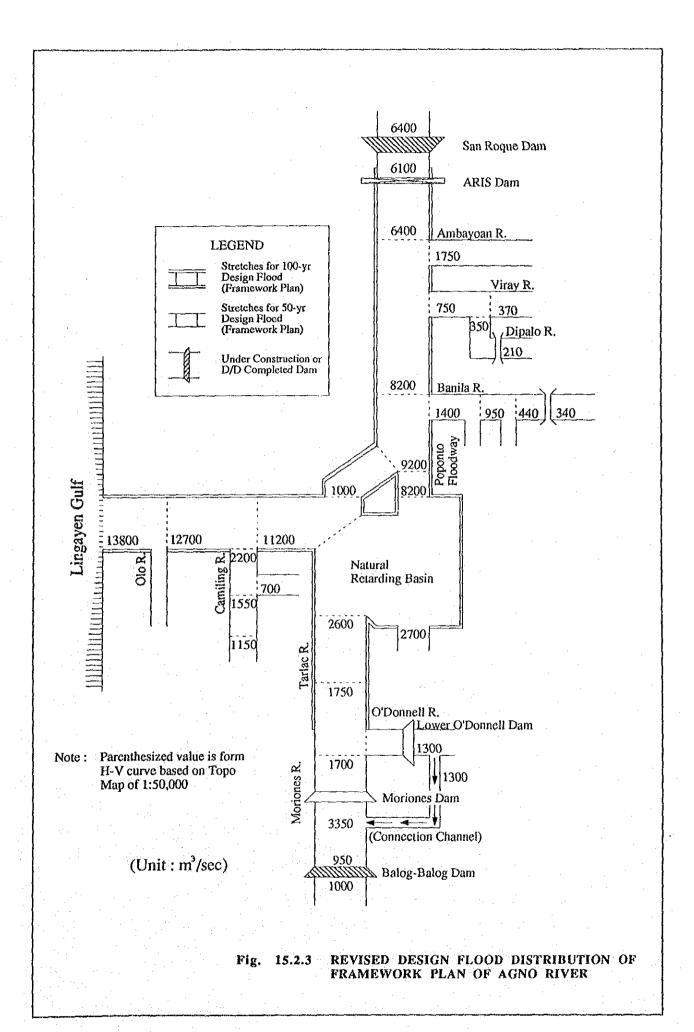




STORAGE CAPACITY CURVES

Fig. 15.2.1 COMPARISON OF NEW STORAGE AREA AND VOLUME CURVES OF POPONTO SWAMP WITH OLD ONES





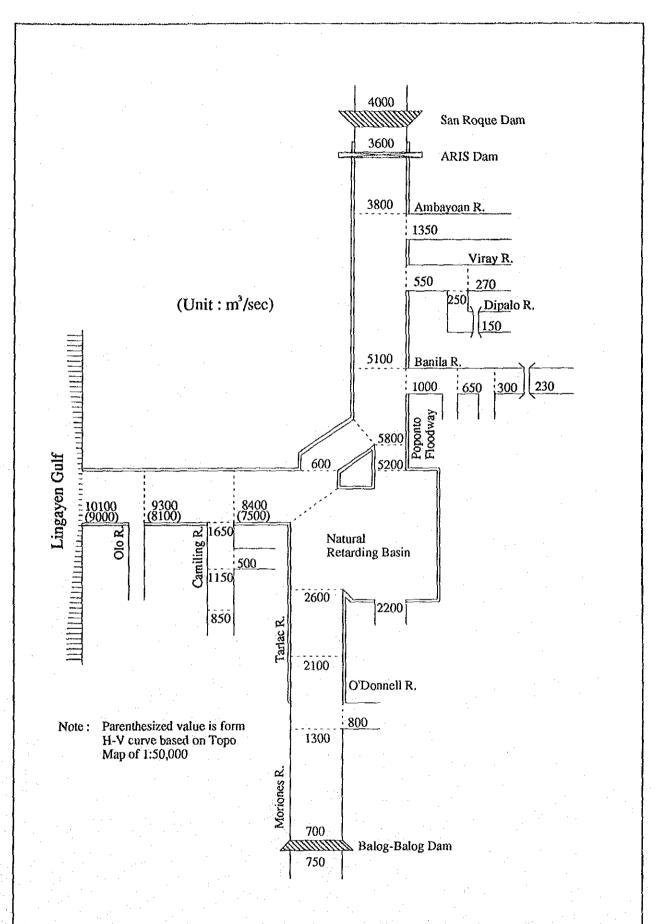
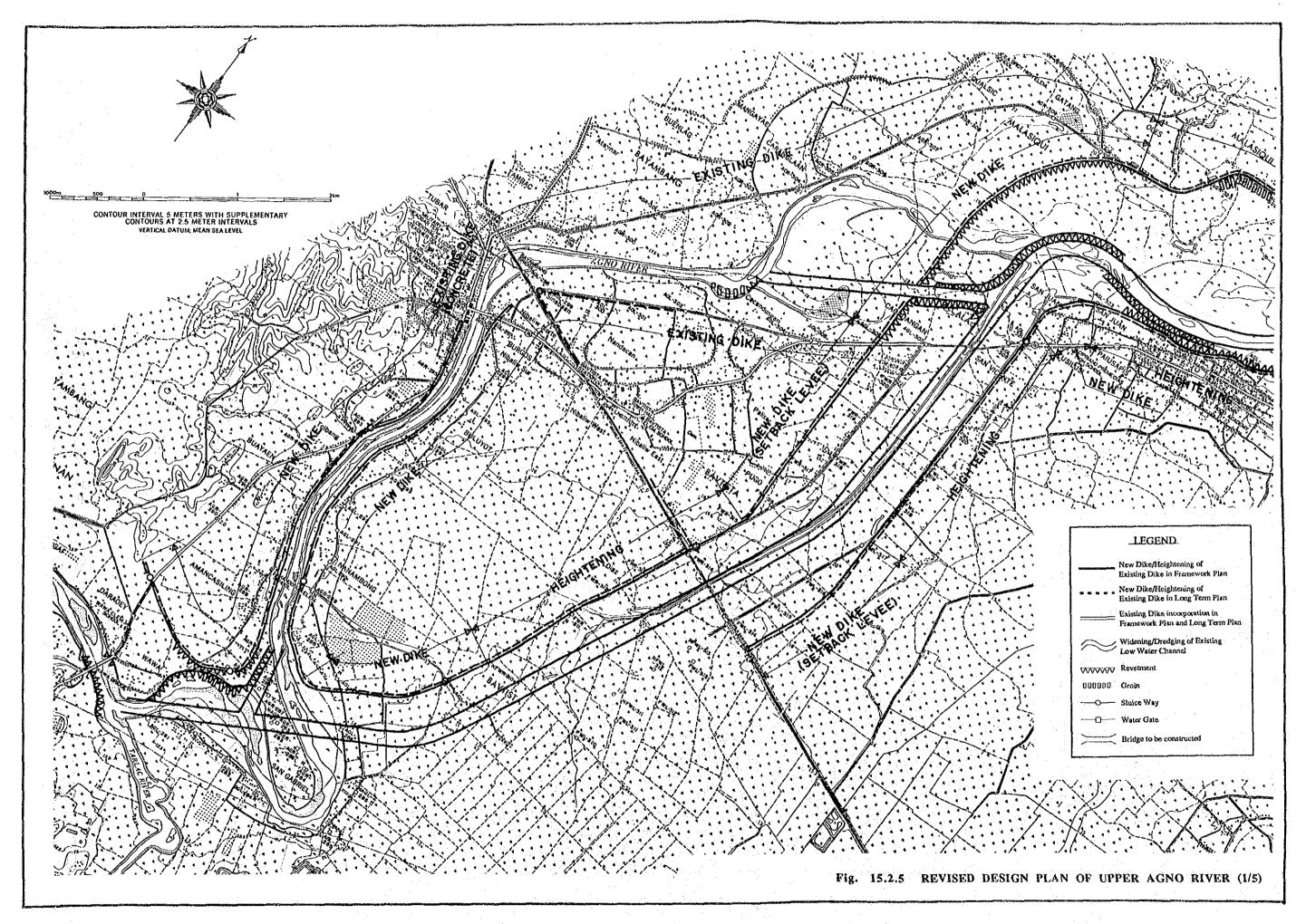
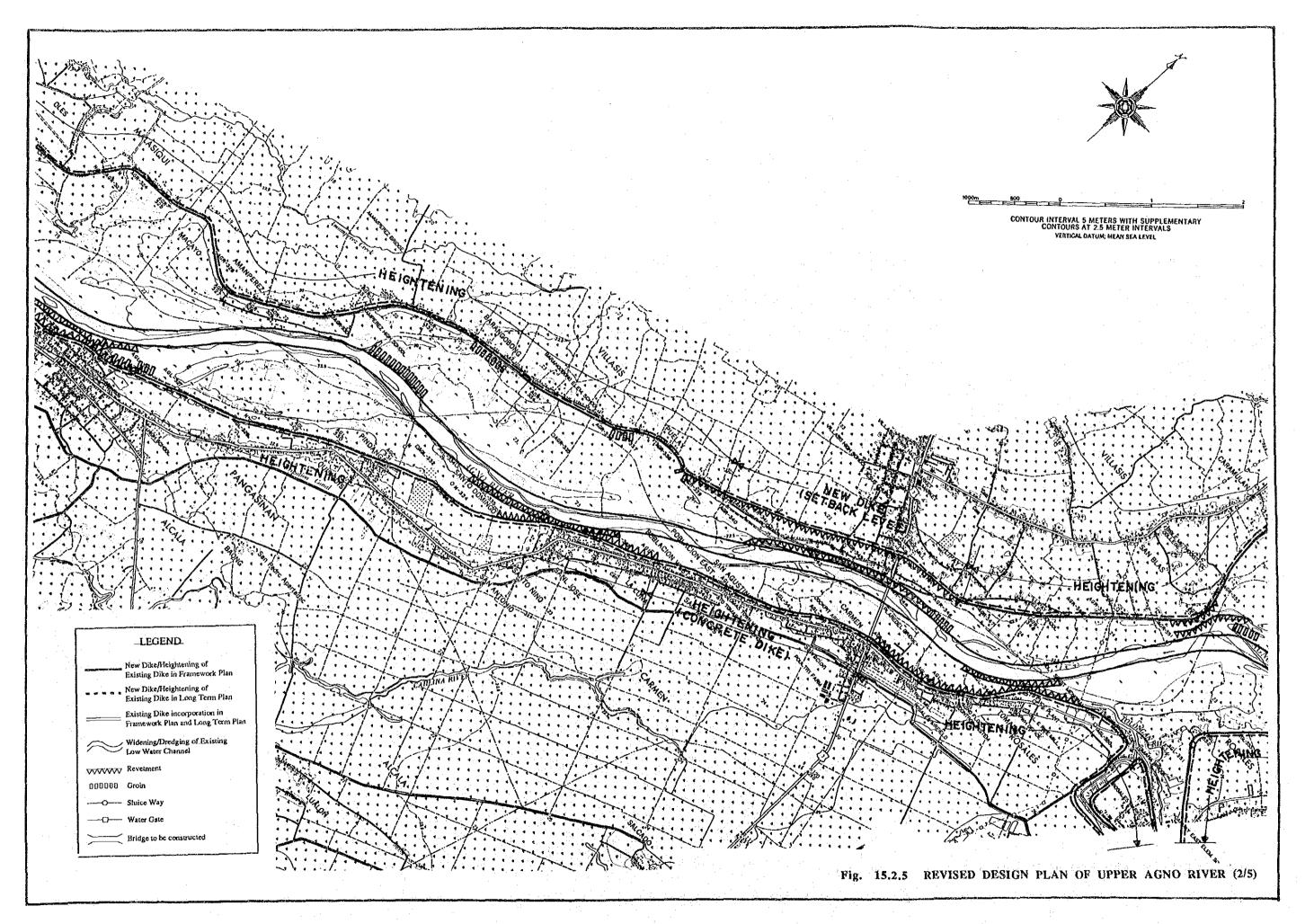
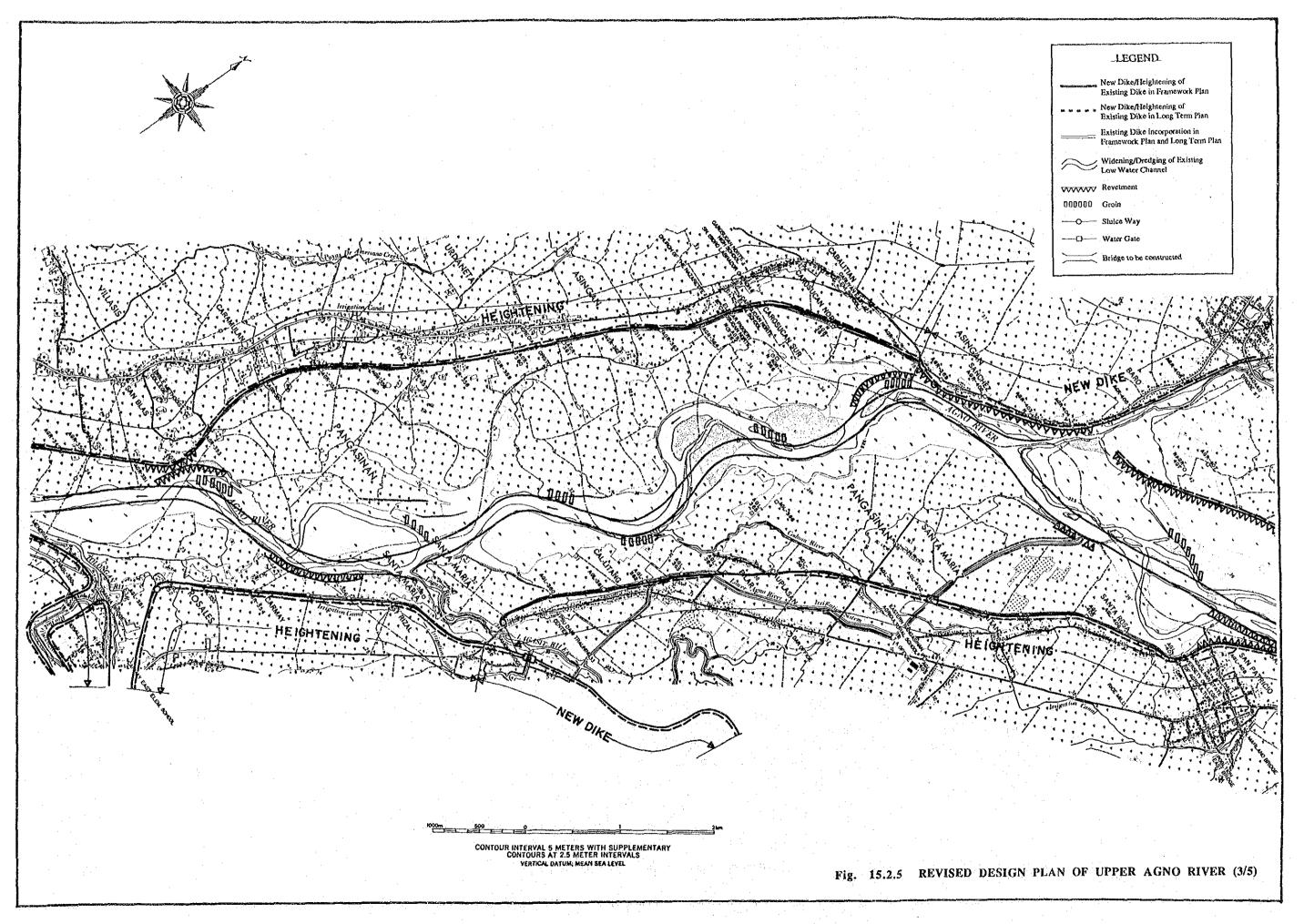


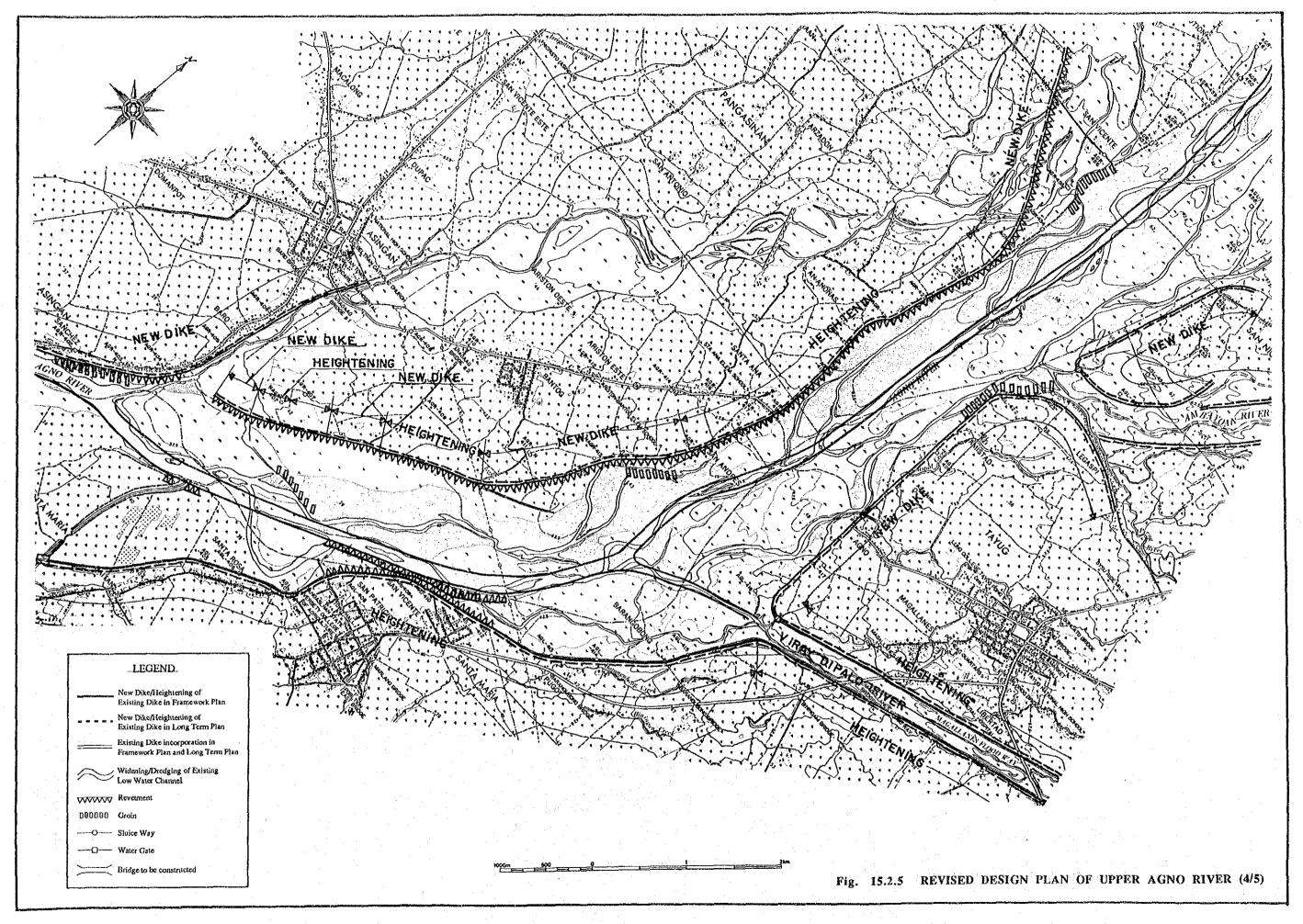
Fig. 15.2.4 REVISED DESIGN FLOOD DISTRIBUTION OF LONG TERM PLAN OF AGNO RIVER (25-YEAR FLOOD)



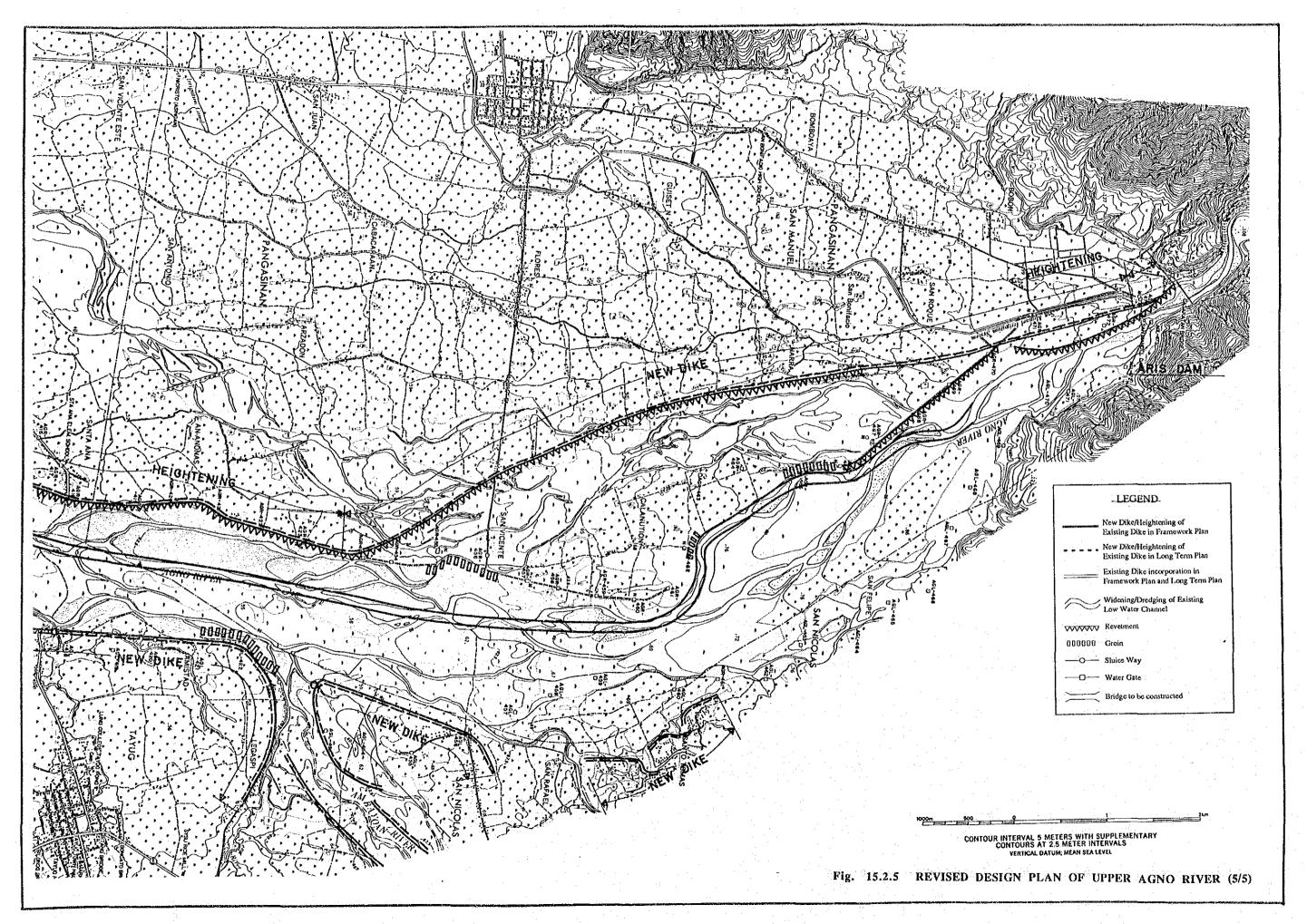




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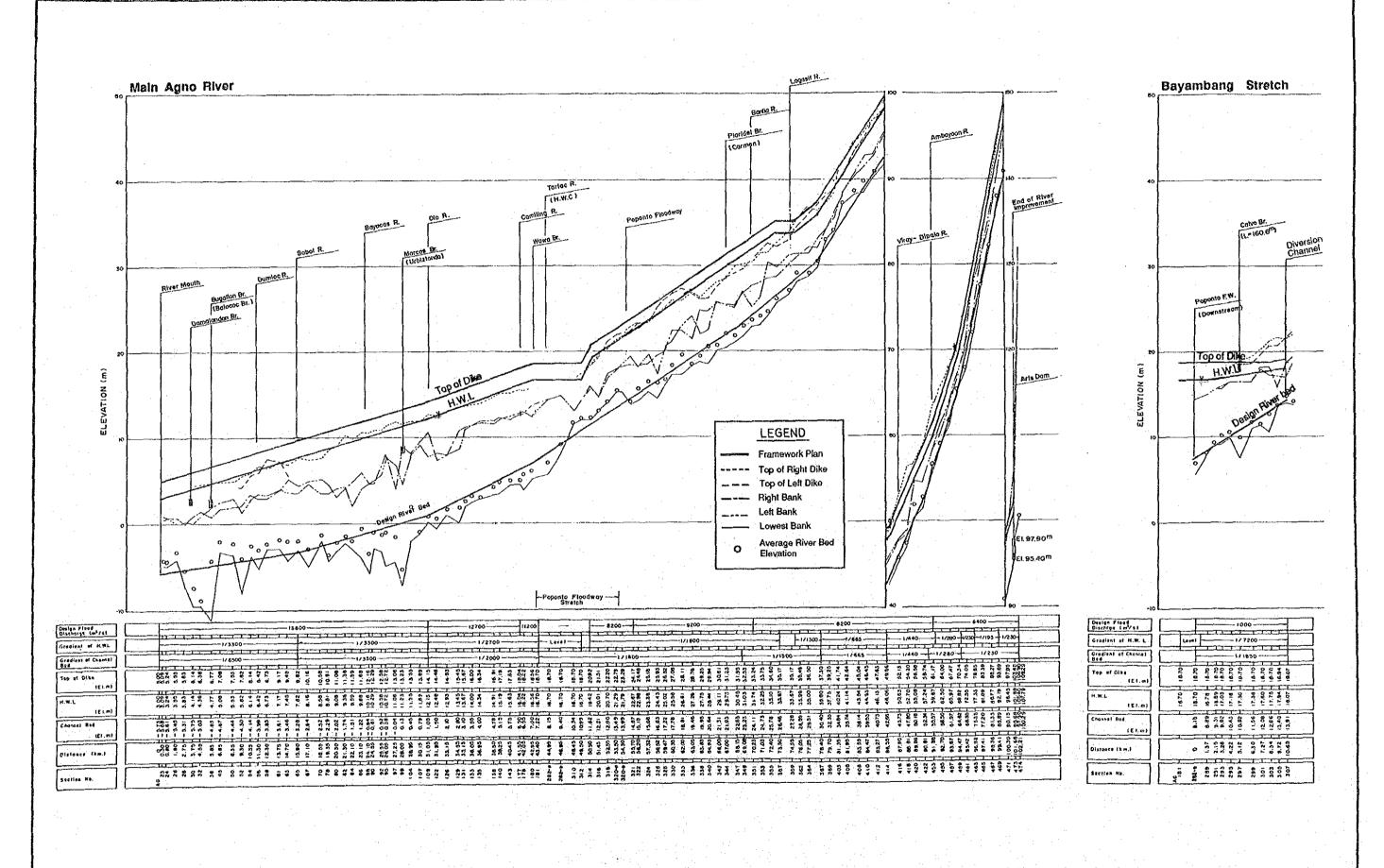


Fig. 15.2.6 REVISED LONGITUDINAL PROFILE OF FRAMEWORK PLAN

