The riverbed width reaches about 50 m between EL. 240 m and EL. 730 m but becomes wider in the area above EL. 730 m as much as 100 m. Topography of the area below EL. 240 m shows an aspect of valley-bottom plain and the river width is as wide as 150 m (the valley width is some 1 km). Distribution of the largest grain diameter varies in places at EL. 70 m and EL. 180 m.

The K. Glidik basin was hardly hit by the disaster of May 14, 1981 but the left bank where the elevation is as low as EL. 70 m to EL. 180 m was hit by the disaster on January 18, 1982. The area between EL. 350 m and EL. 630 m forms a V shaped valley, and the riverbed there is formed by consecutive base rock. The riverbeds below EL. 350 m and above EL. 630 m are covered with loose Lahar (or Ladu) sediment.

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4. RIVERBED MATERIALS

4.1 ITEMS OF INVESTIGATION

The grain size distribution test and the physical characteristics test were conducted on riverbed materials, hillside materials and flowing debris materials to provide data for the understanding of sediment transfer and the riverbed fluctuation simulation.

Table-4.1 shows the item of investigation.

Table-4.1 Items of Riverbed Material Investigation

			Test	: .
Classification	Time and Place of Collection	Grain Size Distri- bution	Specific Density	Field Density
	K. Mujur (6 Locations)	0	X	×
Riverbed Materials	K. Rejali (6 Locations)	0	0	0
	K. Lengkong (2 Locations)	0	X	×
Hillside Materials	Landslide area of BS. Tunggeng	0	0	X
Flowing Debris	K. Lengkong 11:20 Feb. 9, 1983	0	O	X
	K. Lengkong 14:00 Feb. 9, 1983	О	0	×
Suspénded Materials	K. Mujur 17:25 May 1, 1983	0	0	x
	K. Lengkong 14:00 Feb. 15, 1983	O	0	X

O: Data collected X: Data not collected

4.2 GRAIN SIZE DISTRIBUTION

The result of the grain size distribution test on riverbed and other materials is shown in Table-4.2 and 4.3 and Fig.-4.1.

The values shown in these Tables and Figure are the average values of three testing materials.

4.3 SPECIFIC GRAVITY AND FIELD DENSITY

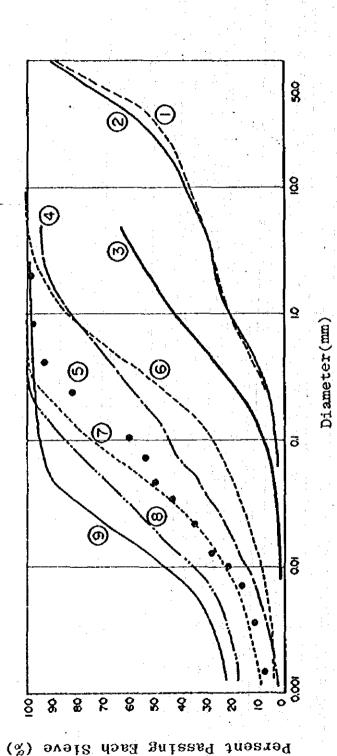
The result of the specific gravity test and the field density test is shown in Table-4.4.

Table-4.2 Grain Size Distribution

1	1			·	T	Γ.	l	<u> </u>	F		F			
0.0013			0.17	0.16				V 1 /	130	80.0		i veg	97.0	3.70
			0.20	0.20		•				0.10			0.42	5.91
0.006 0.003			0.33	0.31						0.22			0.68	10,20
0.009			0.43	0.45		<u>.</u>				06.0			0.80	16.90 13.28 10.20
0.012		1	0.65	0.61))		57.0			1.16	
0.020			0.98	1,07			. Le fa e			21.0		1 1	1.99	21.82
0.041			1.78	1.92			, single			1.34			4-06	31.85
0.106 0.075 0.054			2.29	2.54						1.65		188 188 188	4.70	35,53
0.075	1.62	0.48	3,16	3.63	1.20	67.0	1.07	1.42	1.32	2.33	1.33	0.92	5.97	SE 27
0.250 0.106	2.23	0.78	4.24	4.79	2.08	0.85	1.60	2.11	2.01	3-21	2.25	1.43	8.18	46.33
0.250	6.25	3.56	9.22	10.52	9.22	4.89	5.46	7.13	7.90	7.98	9.05	6.38	19.49	59.19
.425	10.37	7.31	12.36	14.22	15.76	9.87	8.71	16.09	13,84	12.20	13.75	11.99	27.24	68.38
1.00	22.84	17.24	18.27	20.42	25.56	21.90	15.26	26.99	19.84	23.90	20.45	22.91	42.61	82.58
8	32.31	22.98	21.63	23.90	30.05	29.16	19.84	34.03	23.31	31.16	24.54	27.84	52.73	92.10
4.75 2	38.58	27.52	24.55	27.26	34.10	34.89	25.12	38.24	27.20	35.54	29.55	32.09	62.86	94. 22
10	46.55	35,43	30,93	36.62	42.21	43.12	32,74	42.22	35.14	42.50	37.52	39.28		
8	51.03		37.24	46.62	47.75	50.44	37,70 32,7	49.43 46.22	39.41	86.33	43.97	77,77	3 - s - 4	
8	54.74 51.03 46.5	47.43 41.72	43.93	94.27 71.42 62.18 53.12 46.62 36.62	51.81	72.42 65.79 56.55	52.81 48.85 41.59		43_37	49.57	92,35 66.88 60,85 49,77	06.87		
07	81.66 65.24 61.29	58.30	59,33	62.18	62,32	62.79	48.85	54,25	51.63	86.38	60,85	58,57	94 O	11 / 13 1
8	65.24		90_64 68_12	71.42	67.72	72.42	52,81	59.17	88.37 57.10	86.42 60.73	98.39	90-79		ę.
100	81.66	93.67 64.3	90.64	94.27	88.00	91.95	88.32	84.24	88.37	86.42	92.35	91.34	100 mm	2011
Diameter (um)	1						,-ga-a,#2-ab		, / q.	Def pag	1. 1			
Test (R-2	Ž.	Å.	R-10	17-2	R-12	1	¥-5	%-3	. S- M	6 -1 4	01-H	G-1 (Pronofitwo)	. C-2 (Jengkong fan)

0.0071 0.0036 0.0015 9.01 4.4 7.1 22.2 18.1 14.0 0.9 23.3 29.0 11.3 28.89 18,6 7.6 15.9 37.0 0.010 35.6 9.5 23.6 20.7 45.0 0.014 54.8 11.7 29.5 45.2 29.1 0.0374 0.024 14.6 54.3 35,8 6.89 37.1 1.74 17.9 63.9 43.5 80 0 0.074 0.0521 6 68 50.5 21.2 57.2 71.9 23.8 66.3 53.9 9.76 77.4 0.105 28.4 59.9 76.5 87.5 92.6 48.0 8-96 0.25 99.3 81.5 7.96 99.2 0.42 61.6 92.5 8. 76 100 79.2 0.84 97.9 8-96 Š 4.46 0.66 2.00 99.2 99.5 9.66 4.76 100 9.25 100 90 Diameter (mm) upstream of BS. Tunggeng 15, 1983) Lahar at K. Lengkong Lahar at K. Lengkong Hillside material at Lahar at K. Lengkong (14:00 Feb. 9, 1983) (II:20 Feb. 9, 1983) (17:25 May 1, 1983) Lahar at K. Mujur Tested Naterials (14:00 Feb.

Table-4.3 Grain Size Distribution



Suspended Sediment of K. Mujur Flowing Lahar Materials (At 14:00 on Feb. 9, 1983) Flowing Lahar Materials (At 11:20 on Feb. 9, 1983) at the Mujur Bridge (At 17:25 on May 1, 1983) 0 @ **©** Riverbed Deposit of K. Glidik (Pronojiwo) Brosion Sediment at the Upper Stream of Riverbed Deposit of K. Rejali Riverbed Deposit of K. Mujur Deposit of K. Lengkong Fan BS. Tunggeng 4 (0) **(** (m)

regend:

Suspended Sediment of K. Lengkong

(At 14:00 on Feb. 15, 1983)

Pronojiwo)

<u>ඉ</u>

Fig. - 4.1 Grain size distribution curves

Table-4.4 Sepcific Gravity and Field Density

Classifi-	Location		Specific	Field	*Grain Concentra-
cation		Position No.		Density	tion by Volume
Riverbed		R-2	2,777	1,535	0.55
	K. Rejali	R-4	2,793	1,675	0.60
		R-12	2,824	1,435	0.51
Flowing Debris	K. Lengkon 11:20 Feb.		2,751	_	
K. Lengkong 14:40 Feb. 9			2,739	**	
Suspended Materials	K. Lenkgon 14:00 Feb.		2,718	-	
	K. Mujur 17:25 May	1, 1983	2,806	-	·
Hillside Materials	Landslide BS. Tungge		2,727	-	

^{*} Given from specific gravity and field density.

4.4 GRAIN CONCENTRATION OF LAHAR BY VOLUME

The volume density of debris observed in 1983 is shown in Table-4.5.

Table-4.5 Debris Density of Flowing Lahar

:				1911	. <u> </u>			
d Value	α©	1:75	1.85	1.08	1.14	1.02	1.02	00-τ
Estimated Value	PG 🗇	0.41	0.49	0.02	0.13		10.0	00.0
me	③Water (gr)	315.125	270.075	1862,800	1652.075	535.950	524.625	1995,450
Observed Volume	© Soil (gr)	612.350	710.875	186,050	516.800	7.200	18.375	10.525
sqo	water Discharge © Total (m /s) Volume (m)	530	230	1900	1900	530	530	2000
	water Discharge (m /s)	598.0	598.0	19.2	3.78 (14:15)	T T	1-1	10.8
	Station	Pronojiwo Bridge II	Pronojiwo Bridge I	K. Lengkong II	K. Lengkong I	K. Leprak II	K. Leprak I	Mujur Bridge
	Time	11:20	11:20			15:00	16:00	15:30
	Date	Feb. 9, 1983	Feb. 9, 1983	Feb. 15, 1983 13:00	Feb. 15, 1983 14:00	Feb. 8, 1983	Feb. 8, 1983	Mar. 16, 1983

D: Specific Gravity of Lahar $D = \frac{(J + J)}{(1)}$

Cd: Grain Concentration of Lahar by Volume Dd = (1)

Estimated Value from Flood Mark

4.5 CHARACTERISTICS OF RIVERBED MATERIALS

The characteristics of riverbed materials and others are described below on the basis of the investigation data mentioned above.

- (1) The grainsize distribution of riverbed materials of the K. Mujur and the K. Rejali is almost uniform from the upper stream to the river mouth. The average grain size of the K. Mujur and the K. Rejali are dm = 40 mm and dm = 30 mm respectively. Silt with a diameter of less than 0.074 mm is almost non-existant at less than 2%.
- (2) The riverbed materials of the K. Glidik are generally finer than those of the other two rivers but the volume of silt is still low at less than 6%.
- (3) The grain size distribution of lahar deposit in the K. Lengkong fan (accumulated in 1976) shows a slight shift towards the finer grain size in comparison with the distribution of riverbed materials mentioned above and is similar to the distribution of hillside sediments and flowing debris. Silt constitutes 40% by volume.
- (4) The grain size distribution of hillside sediments and flowing debris shows a shift to much finer grains than riverbed materials. The hillside sediments are finr than the flowing debris as shown in the ratio of silt at 54% and 24% respectively.
- (5) The grain size distribution of suspended materials at times other than flood shows the strong presence of finer grains, as silt constitutes 70% 90%.

(6) The density of flowing debris is quite high. At Pronojiwo of the K. Glidik (riverbed incline i=1/20), the volume density is 44% and the weight density is 1.80 t/m.

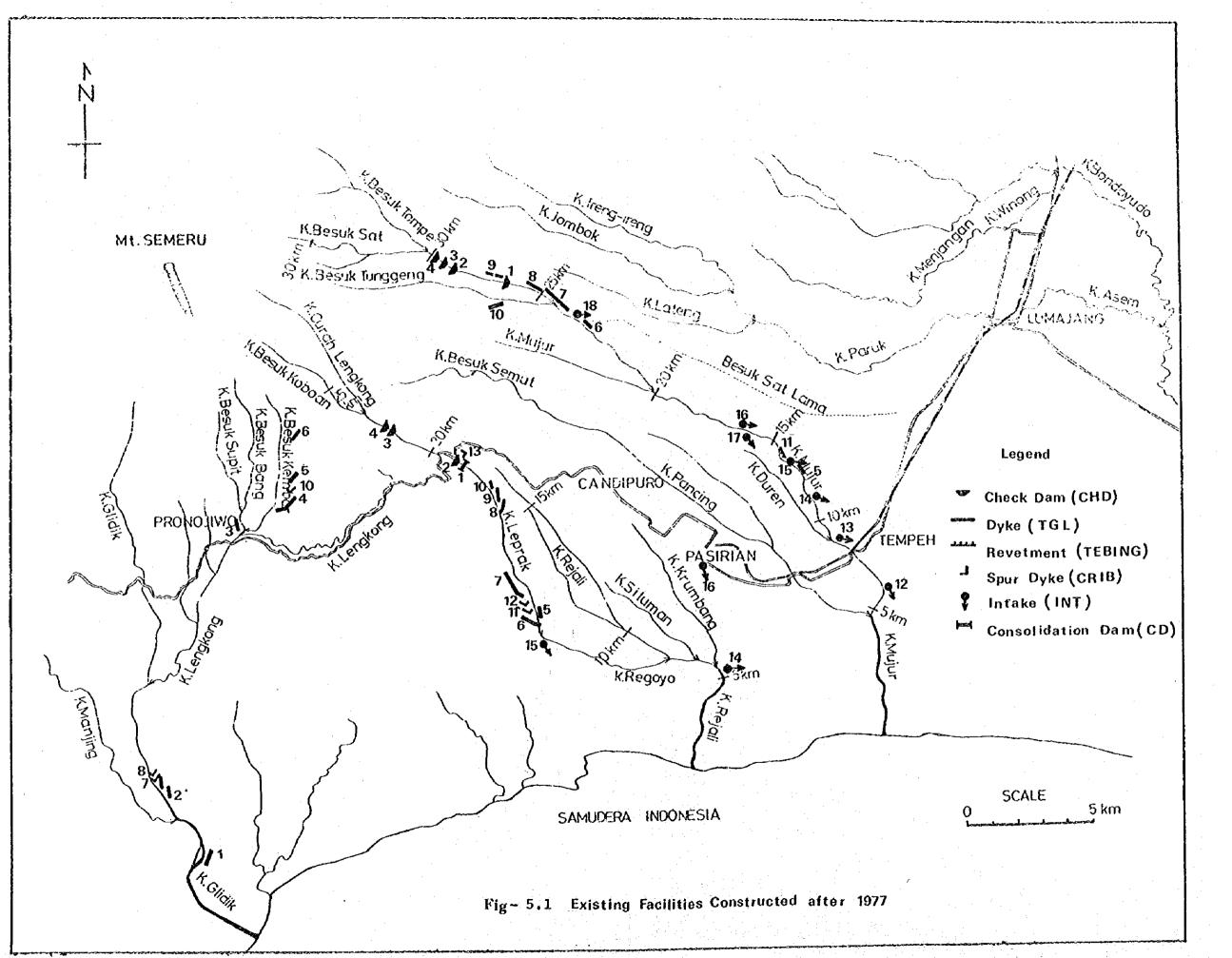
5. EXISTING FACILITY

The facility existing along the river channels in the study area is classified as follows for the use of planning:

- 1 Disaster prevention works executed by Mt. Semeru Project Office after 1977 in which it was established.
- (2) Disaster prevention work executed before 1977.
- (3) Intakes for irrigation.

Existing facilities belonging to group (1) and (3) are shown in Fig.-5.1 and Table-5.1 - 5.3. Existing facilities belonging to group (2), which were executed along K. BS. Sat and K. BS. Sat lama before 1977, are shown in Table-5.4 and Fig.-5.2.

The simbols which are used in tables and figures have the following means.



K. Mujur

1. CHD Besuk Sat I

2. VI

III 3.

II

5. TGL Mujur

" Tunggeng Bawah

" Kertosari I+II

" Leces

Besuk Sat

" Sumbersari

11. TEBING Mujur

12. INT Pandanwangi

" Soponyono

" Kedung Caring

" Klerek

" Lobang 1

17. " Lobang 2

K. Rejali

1. CD Leprak 1

2. CHD Leprak 2

3. CD Curah kobo'an 2

4. CHD Curah kobo'an 1

5. TGL Leprak 10

" Leprak 9

" Leprak 8

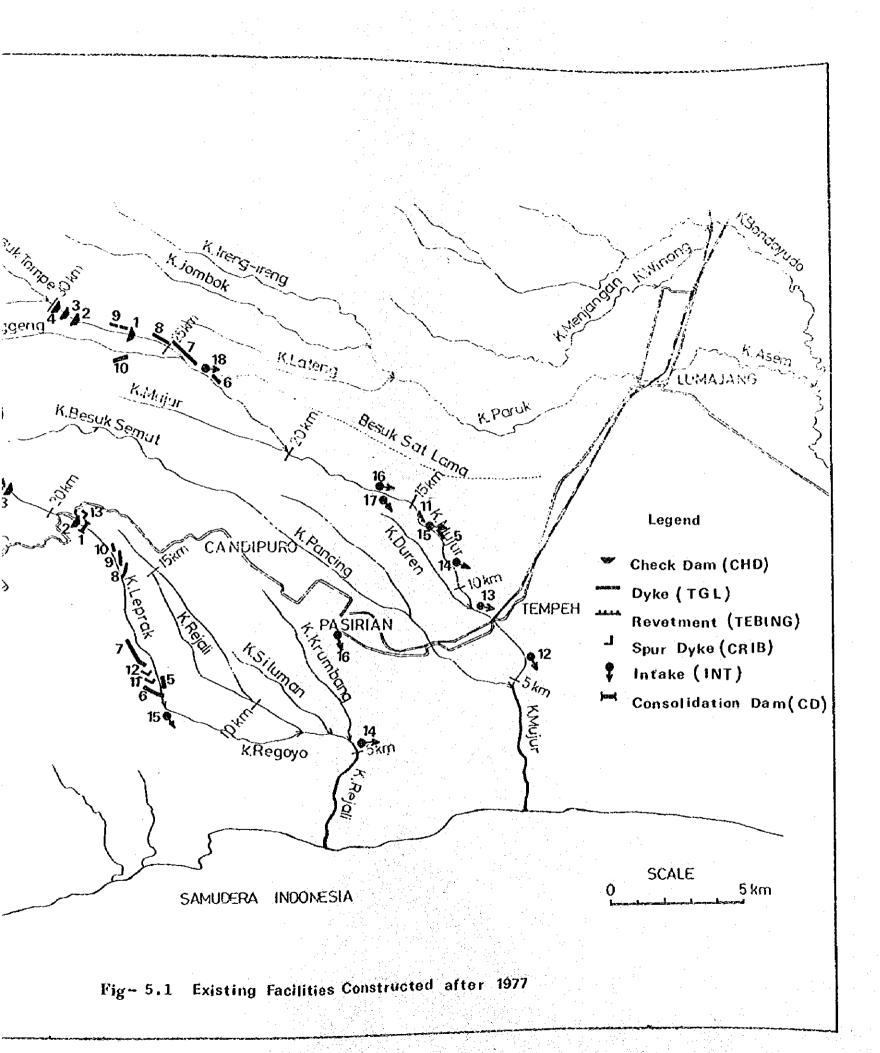
" Leprak 2+3+5+6+11

" Leprak 1+7

" Leprak 4

KRIB Swakelola

Swakelola



K. Mujur

13. KRIB Leprak 1. CHD Besuk Sat Banjr Scherm 14. INT VI 15 • " Rahayu 3. III 16. " Talang II 5. TGL Mujur K. Glidik " Tunggeng Bawah 1. TGL Umbul Sari Kertosari I+II

" Leces 2. " Wareng
" Besuk Sat 3. " Besuk Bang

4. " Besuk Sarat 2+3

5. " Besuk Sarat 1

6. Besuk Sarat 4+5
7. KRIB Wareng 1

Besuk Sarat.

" Soponyono 7. KRIB wareng 1
" Kedung Caring 8. " Wareng 2

15. " Klerek

16. " Lobang 1

10. "Sumbersari

12. INT Pandanwangi

11. TEBING Mujur

17. " Lobang 2

K. Rejali

1. CD Leprak 1

2. CHD Leprak 2

3. CD Curah kobo an 2

4. CHD Curah kobo'an 1

5. TGL Leprak 10

6. " Leprak 9

7. "Leprak 8

8. " Leprak 2+3+5+6+11

9. " Leprak 1+7

10. " Leprak 4

11. KRIB Swakelola

12. " Swakelola

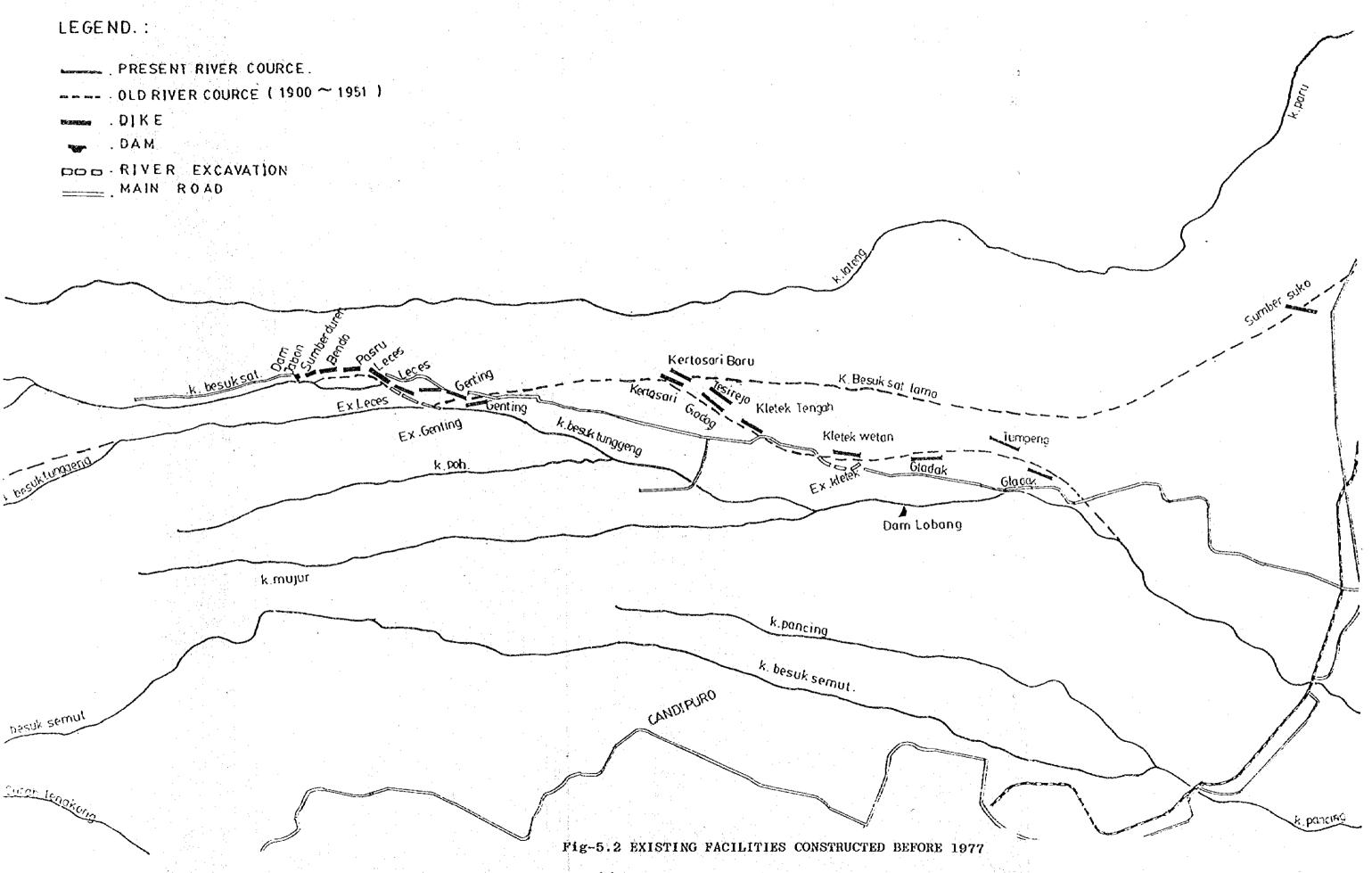


Table-5.1 Existing Facility in K. Mujuz

						and the second s		
River	#	Tributary	Facility	Facility Name	Completed	Elements	Constructor	Remarks
Name		Name			Year	general de la companya de la company La companya de la co		The second of th
	×	K. Besuk Sat	Check dam	CED. BS. SAT I	64. 44.	H=7.3,Bl=28,B=129,h=4.0	Mt. Semeru	destroyed at May 1981
				CED BS SAT 4	08,	E=10.5,Bl=62.14,B=203,b=5.6	Project Office	
				CED. BS. SAT 3	08.	н 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
				CED.BS.SAT 2	64.	Bmll.0,Bl=48,B#197,h#5.25	•	
-	X.	K. Mujur		TOT. MUJUR	18.	L=86,H=2.5,bl=2.5		
	X F	Tunggeng		TCL.TUNGGENG BAWAE	18.	L=190,E=5,bl=5.5,b2=15.5		
3	× 8	Besuk Sat	Dyke	mer. lecres	.78	I=160,H=4.0	*	partially destroyed
į								
TuCum				TGL.KERTOSARI 1+2	18.	L=835, N=5, DL=4, DZ=L5		
	<i>;</i>			TGL.BS.SAT	08.	L=200,E=3,b1=2.5	2	destroyed at May 1981
	×	Tunggeng		TGL. SUMBERSARI	18.	L#275,H#4,bl=5.5,b2=15.5	t	
	X. X	K. Mujur	Revetment	TEBING MUJUR	84.	L-120, H-4	*	
				INT. PANDANWANGI		тесн		
				* SOPONYONO				
	X X	K. Mujur	Intake	* KEDUNG				
				CARING		•		
·				INT. KLEREK				
	· .			" LOBANG 2				
· ·	: *			" LOBANG 1		•		
	X .A	Tunggeng	. :	INT. ROWOGEDANG		HALF-TECH		

Table-5.2 Existing Facility in K. Rejali

								1
River	Tributary Name	Facility	Facility Name	Completed Year	Elements	Constructor	Remarks	
	K. Leprak		CHD.Leprak 1	.82	H=8,B=30,Bl=18,h=7,5	Mt. Semeru Project		1
			CD.Leprak 2	182		E .	under construction	1
	K. Kobo'an	Check dam	CD.Kobo'an 2	08,	E=6.5,B=56,Bl=20,h=6.5	# 1	The second of th	T
			CD.Kobo'an 1	180	H=11.5,B1*30,b*7	* 12 12 12 12 12 12 12 12 12 12 12 12 12		т-
			TGL.Leprak 1	.78	L-250,B=3	*		7
			4	.80	L=30,H+4,3,bl=4,b2=12,3			Τ
			. 2	84.	L=157,b=3.5,b1=3,b2=8	B Company		,
			. 3	.78	L=98.5,H=3.0,bl=4.0,b2=7.0	# Company (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)		} -
		·	\$	64.	L=125,H=2,bl=3,b2=8			<u>2</u>
×	K. Leprak	Dyke	9	08.	L=49,E=2,bl=3,b2=11	* · · · · · · · · · · · · · · · · · · ·		4
Rejaii			11	.82	L=185,H=3	*	· · · · · · · · · · · · · · · · · · ·	т
				84.	L=160,H=45,bl=3,b2=11	the best with a contrast of the contrast of th	As a desired of the second of	1
		· · · · · · · · · · · · · · · · · · ·	8	.80	L=308, H=2.5, bl=3, b2=8	A distribution of the second o	the state of the s	7
			6	181	L=132,E=3,bl=2.5,b2=6	Addition of the second	destroyed at May 1981	स
		•	or	78.	1-215,8-3,b1-3,b2-10		and department of the second o	<u> </u>
: N			TGL.Jugosari l	100	A CANADA A	Hall Market of the state of th	destroyed at May 1981	ī
			TGL.Jugosari 2			The Art of the Control of the Contro	on the second control of the second control	T
and the control of th		Spur dyke	KRIB. Leprak	artis di talanda di sanciano del	existence of the spot to be a second of the	er erene er er er grunder i Bereigene ming van der gebruik in der	port of Manacol of the large of same by a major processing parameter	1
			XRIB. Swakelola	184		The second secon		
A Property of the Control of the Con	K. Rejali	Intake	INT. Banjir SCHERM	manifest the training and the second	the field between the arthurs and the second state of the second s		All the second s	<u> </u>
			INT.Talang		TECH			
		·	INT. Rahayu		HALF-TECH			f
								ı

Table-5.3 Existing Facility in K. Glidik

Facility	Facility Name	Completed	Blements	Constructor	Remarks
TGL. Besuk Band		Year	1=275	Mt. Semeru	
			H1=3.0, b1=2, b2=4	Project	
MGL. Besuk Sarat 3		181	L=350 E]=3.5, b]=3, b2=10		
TGL. Besuk Sarat 2		. 08.	1=58 By=2.0 by=3 b2=7	•	
TGL. Besuk Sarat l		64.	L=136 H1=2.0 b1=3 b2=7	•	
TGL. Besuk Sarat		•82	L=121, E1=6.0	g E	
TGL. Wareng		182	H1=3.0, Lell6	*	
TGL. Umbulsari		181	Hl=3.0, L=42	3	
KRIB Besuk Sarat	۲.			•	
KRIB Wareng 1		181	H1=3, L=102	E	
KRIB Wareng 2		18,	H)=3, L=41	•	

Table - 5.4 Existing Facilities Constructed before 1977

Facility name	Constructed	Facility name	Constructed year
	year		
Jabon dam	1951	penutup baru dyke	1913
Lobang dam		Kertosari "	1910
Leces excava- tion	1912	Kertosari baru "	1912
Genting "	1909	Tesirejo "	1912
Kletek "	1910	Glodog "	
Sumber Duren dyke	1913	Kletek tengah "	1910
Bendo dyke	1922	Kletek wetan "	1910
Pasru dyke	1914	Tumpeng "	1912
Leces dyke	1913	Gladak "	
Genting dyke	1910	Sumber suko "	before 1910

6. PROMISING SITE FOR SEDIMENT CONTROL FACILITIES

6.1 ITEMS AND METHOD OF INVESTIGATION

Data was collected during the field investigation on the following items in view of finding possible construction sites of check dam(s), consolidation dam(s) and dike(s).

- . Height of both banks
- . Nature of the foundation
- . Appropriate facilities; type, height and length
- . Expected effect of facilities
- . Aspects to be considered in view of constructing these facilities

6.2 INVESTIGATION RESULT

The result of the investigation on promising sites for new sediment control facilities has been reflected in Supporting Report (I).

Promising sites for new sediment control facilities in each catchment area are described below.

(1) K. Mujur

The founation of the river below EL. 900 m consists of gravel and, therefore, there is no promising site for a large-scale check dam ($H \ge 25$ m).

The section between BL. 900 m and 640 m enjoys a bank height of more than 20 m which makes the construction of a small check dam possible. There is not an effective site in this section except at the confluence of the BS. Tompe and the BS. Sat as the river channel maintains a U shape all the way through.

The bank height lowers to 3 - 10 m on both sides of the river below EL. 640 m which makes it impossible to build a check dam. A possible sediment control facility, if any, will be a sand pocket where the height of banks will be heightened by dikes.

Two promising sites for the sand pocket construction were found at about EL. 580 m and about EL. 250 m as the sand pocket is suited to the inflection point of the riverbed incline.

Since there are many intakes along the K. Mujur, construction of consolidation dams should be necessary to protect these intakes for the river section below EL. 220 m.

A promising site for a check dam is almost non-existent along the BS. Tunggeng, the right side tributary, as its width is narrow (river width = 10 - 20 m) and its cross sectionshows a U shape.

(2) K. Rejali

The foundation mostly consists of gravel except the section between EL. 500 m and EL. 660 m and the section over EL. 900 m. As such, a check dam of over 25 m in height can only be constructed in these sections. However, the section over EL. 900 m has a riverbed incline of i = 1/10 and is not suitable for a large check dam as this teep incline reduces the sediment control efficiency of a check dam.

Moreover, the section between EL. 500 m and EL. 600 m forms a narrow neck section with steep cliffs on both sides and makes it practically impossible to construct a check dam. On the contrary, a large check dam can be constructed for the section between EL. 600 m and EL. 660 m as it has a solid rock riverbed.

The section below EL. 500 m forms an alluvial fan and the construction of a check dam is impossible as the bank

height is as low as 2 - 10 m on both sides. However, a sand pocket at around EL. 400 m will be a suitable choice in this section as the best place for a sand pocket is at the inflection point.

The Butter Bright of Green Company of the Street Company

(3) K. Glidik

The construction of a large check dam is possible in the section between EL. 280 m and EL. 630 m as it has a solid rock riverbed. However, the construction of access roads will be quite difficult there because of the deep valley with 200 m-high sheer cliffs on both sides. A promising site for a check dam, therefore, should be limited to either the top of the upper stream or the bottom of the lower stream.

The west bank of the K. Lengkong over EL. 630 m is lower than the east bank and, therefore, renders it impossible to build a high dam. The construction of consolidation dam is nevertheless possible at any point along the river.

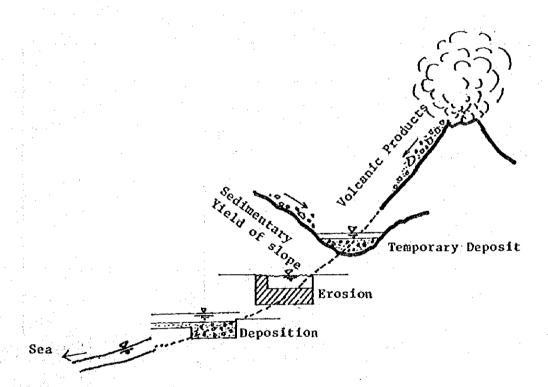
7. RIVERBED FLUCTUATION SIMULATION

7.1 METHOD OF ANALYSIS

The riverbed fluctuation simulation was conducted to understand the sediment transfer from the valley head to the river mouth at the time of flooding.

As Fig.-7.1 shows, sediment is directly produced at the crator and the slope and temporarilly accumulated on the riverbed. Later, it is transported to the lower stream area by water flow when flooding occurs.

Fig.-7.1 Transportion of Sediment



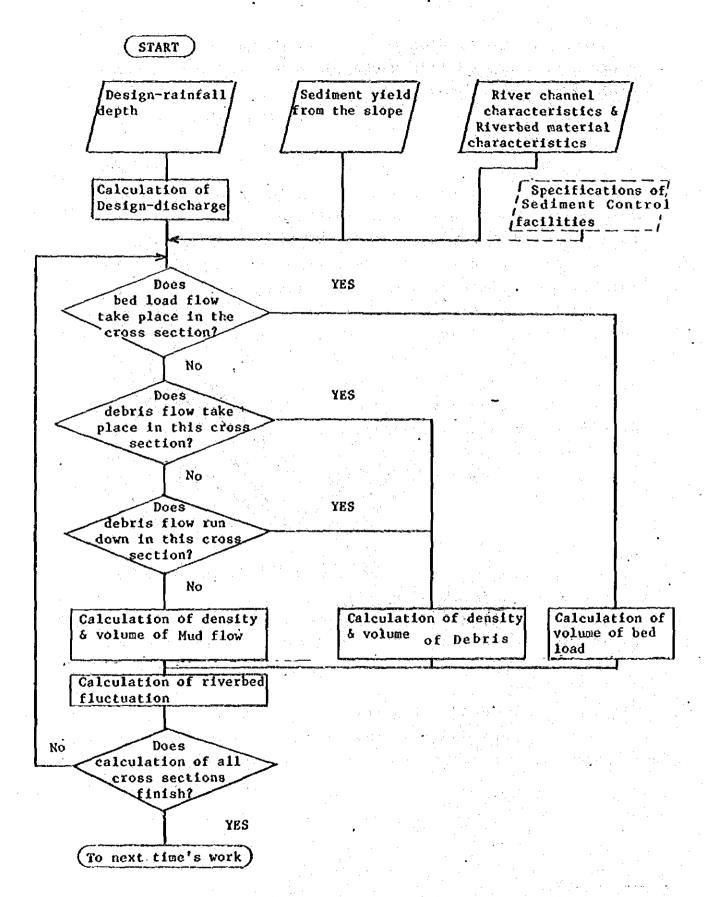


Fig. - 7.2 Flow Chart of Riverbed Fluctuation Simulation

During this transportation, erosion of the riverbed or deposition of sediment will occur. In short, therefore, sediment will flow down while its actual volume is fluctuating.

The erosion and accumulation within the above-mentioned channels are calculated by the riverbed fluctuation simulation, which is shown in Figure-7.2. The simulation is based on the understanding that the sediment is transported by tractive force. In addition, it is understood that the sediment tractive force of water varies according to the different types of sediment flow, where there are three types of flow, i.e. debris flow, mud flow and bed load flow.

As there is no small grain size sediment (cf. figure-4.1) at the riverbeds of the K. Mujur, K. Rejali and K. Glidik, the suspended load flow is considered to have been discharged into the sea, and therefore is not included in the simulation.

The method of detail calculation and its formula are described below.

(1) Calculation of Design Discharge

The design discharge is calculated by the Kinematic Wave Method using the design rainfall depth as input. The detail of the Kinematic Wave Method and of the calculation results are shown in Part - 5 HYDROLOGY.

(2) Sediment Yield from Slopes

The types of sediment yield from slopes in this basin are as follows:

Direct Yield from volcanic activities
Yield from breaking, landslides and surface erosion

Although it is not easy to estimate the volume of direct yield from volcanic activities, it should be of a large quantity as Mt. Semeru is so active that it erupts every ten minutes or so. Therefore, the volume of the sediment yield at the most upper-stream areas of the following branch rivers, where the direct ejecta from the crators will be discharged, is assumed to be more than the sediment tractive force at these places.

K. Besuk Bank (Branch of K. Glidik)

K. Besuk Kembar (Branch of K. Glidik)

K. Curah Kobo'an (Branch of K. Rejali)

Data concerning breaking, landslides and surface erosion around Mt. Semeru is non existent. Therefore, data on bare-land surface erosion on the slopes where volcanic ash falls for active volcanoes in Japan and the American continent (cf. Figure-7.2 and -7.3) is used as a reference to achieve the following formula:

$$Qe = 0.35.a.sin^3 9.C*$$
 (7.1)

Where 0: Average Gradient of Slope

a: Bare-land Size (m²)

C*: Grain Concentration by Volume

Qe: Eroded Sediment Volume per Year (m³/year)

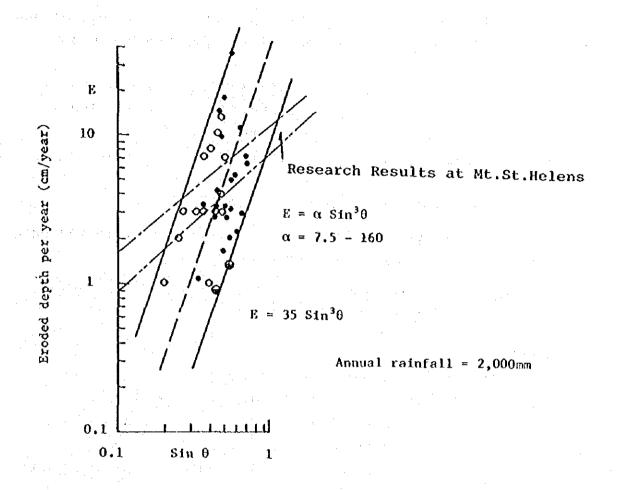


Fig.-7.3 Erosion Depth of Bare-Land in Japan

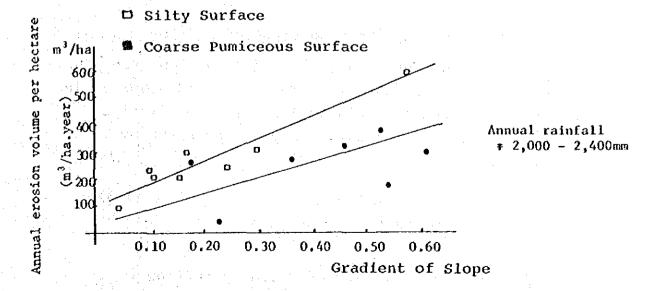


Fig. -7.4 Erosion of Volcanic Deposit at Mt. St. Helens

The bare-land size of the slope (An) is given from Fig.-2.2 in Part E, LAND USE. The bare-land size (An) and the gradient of the bare-land slope (Is) are as follows:

Curah Lengkong	An = 2.62 km^2 ,	Is = 0.255
B. Sat	An = 1.38 km^2 ,	1s = 0.350
B. Tengah	An = 0.9 km^2 ,	1s = 0.370
B. Tunggeng	An = 1.5 km^2 ,	Is = 0.289

(3) Occurrence Conditions for Debris Flow

The following (T. Takahashi's) formula is used to indicate the occurrence conditions for debris flow.

$$\tan \theta \ge \frac{C^* \cdot (\sigma - f)}{C^* \cdot (\sigma - f) + f(1 + \frac{h}{d_2})} \tan \theta \qquad (7.2)$$

Where,

C*: Grain Concentration by Volume

5: Specific Gravity of Particle

P: Specific Gravity of Water

h: Water Depth

do: Mean Diameter of Riverbed Material at its surface

0: Riverbed Gradient

Ø: Friction Angle of Riverbed Material

(4) Formula to Show Stopping Conditions of Mudflow

The term mudflow here means the bed load flow type debris flow in the terminology of T. Takahashi and others. The smallest gradient for this type of flow to exist is calculated by the following formula, suggested by T. Takahashi.

$$\tan \theta \Rightarrow \frac{C^* \cdot (\delta - P2) \cdot \tan \theta}{C^* \cdot (\delta - P2) + (1 + \frac{h}{d_2})}$$
 (7.3)

Where,

P2: Specific Gravity of Fluid in Mudflow

(5) Calculation of Density and Volume of Debris

The density of debris flow and the volume of debris discharged by running water are given by the following (T. Takahashi's) formula.

$$Cd = \leq 0.8 C*$$

$$Q_{\bullet} = \frac{C^{*}}{C^{*} - Cd} \cdot Cd \cdot Qw \qquad (7.5)$$

Where,

Cd: Debris Flow Density

Qw: Water Discharge

Qs: Sediment Discharge

(6) Calculation of Sediment Discharge Volume by Mudflow

The intermediate type of flow, during the transformation
of debris flow to bed load flow, is called mudflow here.

The sediment discharge volume by mudflow is given by T. Mizuyama's formula.

$$Q_{B} = 5.5 \tan^{2}\theta \cdot Q_{W} \tag{7.6}$$

(7) Calculation of Sediment Discharge Volume by Bed Load Flow The Meyer-Peter-Mülür formular is used to calculate the sediment discharge volume by bed load flow.

$$\frac{q_{\delta}}{(6/9, -1) \ gd_{1}} = 8.0 \ (T* - 0.047)^{1.5}$$
 (7.7)

$$\mathcal{L}_{*} = \frac{gh \tan \theta}{(\mathscr{G}_{\beta_{i}} - 1) gd_{i}} \tag{7.8}$$

$$Q_{\bullet} = q_{\bullet} \frac{Q_{W}}{h}$$
 (7.9)

d₁ : Average Grain Size of Riverbed Material

(8) Calculation of Riverbed Fluctuation

The sediment fluctuation volume (AQ₈) at a certain section of a river channel is shown as the difference between the inflowing and the outflowing sediment discharge volume to and from the section, and is as shown in Fig.-7.5. In addition, the height of the riverbed fluctuation (Ah) is given by the following formula.

$$\Delta Q \delta = Q \delta z - Q \delta o \tag{7.10}$$

$$\Delta h = \frac{\Delta Q b}{B \cdot L \cdot C^*} \tag{7.11}$$

The sediment discharge volume given in (7.5) - (7.7) must be smaller than the accumulated sediment volume of the upperstream section.

The initial value of accumulated sediment volume for each of these sections is given while the bed rock, which will not be eroded, is used as the input data.

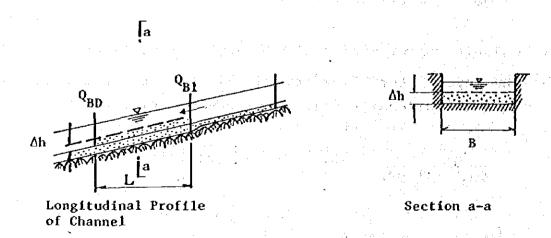


Fig. -7.5 Calculation of Riverbed Fluctuation

7.2 Input Data for the Riverbed Fluctuation Calculation

The input data for each of the previous calculations is as follows:

(1) Design Discharge

The calculation results in Part 5, HYDROLOGY are used. The water depth (h) is calculated from the design discharge and the river channel characteristics shown below.

(2) River Channel Characteristics

The longitudinal profiles and the lateral profiles of the river channels shown by the study results in section 3.2 are used here. The elevation of the riverbed, the width of the river channel, section distance and height of bank are as shown in Table 7.1 - Table 7.5. The elevation and the width indicated in these tables are the mean elevation of the riverbed and the mean river width, calculated based on our survey. The riverbed gradient (0) can be calculated from the section distance and the elevation of each riverbed.

(3) Characteristics of Riverbed Material

The characteristics of riverbed material is given as follows, based on the survey results shown in Chapter 4.

Grain Concentration by Volume	C*	=	0.55
Specific Gravity of Particle	૮	=	2.8 t/m^3
Specific Gravity of Water	۶,	=	1.0 t/m^3
Specific Gravity of Fluid in Debris Flow	∫ 2	=	1.56 t/m^3
Mean Diameter of Riverbed Materials	d ₁	==	0.1 m
Mean Diameter of Materials on Riverbed	-		
Surface	d ₂	=	0.03 m
Friction Angle of Riverbed Material	ø	=	20 ^O

Among the values shown above, f_2 and g are given by the formula (7.4), based on the specific gravity of lahar = 1.8 t/m³ (cf. Table-4.5), measured at Pronojiwo Bridge on February 9, 1983.

MARTINE AND THE SECOND AND ASSESSMENT

Table-7.1 River Channel Characteristics of K. Mujur (1)

		<u> </u>		
No. of Section	Elevation of Riverbed	Height of Bank	Distance	Width of River Channel
TENG NO.13	1,003.31	50.0	397	18
TENG NO.9+43	963.31	50.0	323	18
TENG NO.6	934.63	50.0	400	20
TENG NO.3	905.77	50.0	185	29
TUN NO.45	1,000.00	25.0	900	35
TUN NO.44	887.00	25.0	2,000	35
TUN NO.43	704.33	25.0	370	21
TUN NO.39+65	688.70	25.0	215	24
TUN NO.38-85	668.72	14.0	550	31
TUN NO.34-35	637.24	14.0	270	26
TUN NO.31+95	625.20	14.0	1,075	28
TUN NO.25+20	581.02	8.0	290	31
TUN NO.23-70	570.32	8.0	360	12
TUN NO.21-30	555.16	8.0	350	12
TUN NO.19+20	541.06	10.0	890	28
TUN NO.11+65	500.93	5.0	1,165	23
SA NO.325	1,035.27	50.0	553	63
SA NO.319+53	990.13	50.0	491	78
SA NO.314	953.34	50.0	482	27
SA NO.307	902.80	50.0	185	37
SA NO.304	886.82	50.0	718	38
SA NO.296	836.22	50.0	102	28
SA NO.293+4	829.96	50.0	449	36
SA NO. 288	799.22	50.0	169	62
SA NO.285+84	789.68	50.0	715	157
SA NO. 277+34	736.22	16.0	245	74
SA NO.276-81	724.44	16.0	344	94
SA NO.273+20	707.90	16.0	486	123
SA NO.266+25	678.26	13.0	342	75
SA NO.262-50	662.52	13.0	250	201
SA NO.260	643.02	11.0	112	87
SA NO.259	644.79	14.0	280	93
SA NO.255+48	629.19	14.0	844	200
SA NO. 247+15	586.10	7.0	712	96
SA NO. 240	551.44	10.3	657	98
SA NO.231+5	523.94	16.0	1,416	73
SA NO.213+30	460.71	10.0	154	63
SA NO.211+90	447.90	15.0	402	40
SA NO.207+25	433.30	12.7	492	41
SA NO.201+70	416.76	9.5	559	68
SA NO.193+50	396.98	9.5	917	37
SA NO.186	366.44	50.0		
SA NO.183	355.59	50.0	307 571	152 84
SA NO.178	332.90	50.0	294	121
SA NO.176	323.84	50.0	463	208
SA NO.171+86	304.41	11.8	132	99
SA NO.171+66 SA NO.170+55	302.19	1.71	132 145	130
	296.69	6.8		
SA NO.169			300	158
SA NO.166	286.79	9.7	692	118
SA NO.160+62	272.24	3.3	394	161
SA NO.157+10	258.11	13.9	432	114

Table-7.2 River Channel Characteristics of K. Mujur (2)

No. of Section	Blevation of Riverbed	Height Bank	of Distance	Width of River Channel
SA NO.153+50	248.73	3.0	571	142
SA NO.148+30	237.52	4.0	345	174
SA NO.145	230.35	2.5	428	258
SA NO.141	216.22	2.28	833	118
SA NO.134+10	191.07	1.83	548	145
SA NO.128+65	178.08	4.4	302	100
SA NO.126+15	173.27	4.4	275	107
SA NO.123	167.96	4.4	548	265
SA NO.117+67	158.78	4.4	394	156
SA NO.113+95	150.34	4.4	659	$\overline{137}$
SA NO.107+30	141.01	4.4	638	188
SA NO.102	130.28	6.0	162	133
SA NO.100+30	128.62	3.0	441	265
SA NO.95	118.08	3.1	324	123
SA NO.92+60	110.74	3.16	833	79
SA NO.84+25	96.86	2.64	1,145	130
SA NO.73	81.25	2.25	385	156
SA NO.68+20	75.22	2.08	832	173
SAT NO.61	65.04	2.3	313	110
SAT NO.58	62.75	2.90	348	108
SAT NO.55	60.03	2.67	373	124
SAT NO.52	56.04	2.5	327	39
SAT NO.49	53.36	3.42	325	119
SAT NO.46	49.09	3.24	400	114
SAT NO.42	46.12	3.2	378	105
SAT NO.39	42.23	4.10	297	78
SAT NO.36	40.33	3.75	333	86
SAT NO.33	37.07	3.0	289	112
SAT NO.31	35.25	8.0	327	146
SAT NO.29	32.64	8.0	370	100
SAT NO.26	28.98	8.0	364	112
SAT NO.23	27.69	8.0	393	89
SAT NO.20	22.89	8.0	510	145
SAT NO.17	20.62	8.0	369	383
SAT NO.15	18.55	8.0	407	419
SAT NO.13	15.44	8.0	482	333
SAT NO.13	12.34	8.0	513	453
SAT NO.7	8.42	0.85	471	296
SAT NO.4	5.68			
	2.40	1.0	390 261	690
SAT NO.2	3.40	1.27	361	664

Table-7.3 River Channel Characteristics of K. Rejali (1)

No. of Section	Elevation of H Riverbed	eight of Bank	Distance	Width of River Channe
LENGKONG 66	1,500.0	20	1,900	20
LENGKONG 65	1,200.0	20	2,970	20
LENGKONG 35	960.0	20	1,100	20
LENGKONG 24	880.0	8	330	20
LENGKONG 21	855.0	20	300	28
LENGKONG 18	836.3	20	800	35
LENGKONG 10	785.6	6	500	35
LENGKONG 5	752.5	15	500	14
LENGKONG 154	1,500.0	40	1,620	: 55
LENGKONG 153	1,190.0	40	500	55
LENGKONG 152	1,150.0	40	1,100	55
LENGKONG 141	1,020.0	70	700	30
LENGKONG 135	952.8	60	430	50
LENGKONG 131	930.0	50	440	28
LENGKONG 126	890.0	40	390	26
LENGKONG 122	860.0	40	590	26
LENGKONG 117	820.0	30	600	55
LENGKONG 111	780.0	20	650	60
LENGKONG 104	744.0	15	500	80
LENGKONG 99	718.0	40	200	170
LENGKONG 97	707.7	40	710	170
LENGKONG 90	670.0	30	90	60
LENGKONG 89	660.0	45	700	60
LENGKONG 82	610.0	30	130	30
LENGKONG 80	590.0	100	1,070	70
LENGKONG 70	523.41	100	391	36
LENGKONG 67	493.93	50	286	57
LENGKONG 65	487.18	31	695	57
Lengkong 60	443.52	7.2	221	80
LENGKONG 58	432.42	1.2	858	56
LENGKONG 51	408.67	1.2	883	121
LENGKONG 43	368.94	1.2	1,357	127
LENGKONG 31	326.46	2.6	667	130
LENGKONG 25	306.96	4.0	399	106
LENGKONG 21	283.06	5.2	742	106

Table-7.3 River Channel Characteristics of K. Rejali (2)

No. of Section	Elevation of Riverbed	Height Bank	of Distance	Width of River Channel
K.LEP NO.14	259.86	5.2	519	106
K.LEP NO.10	241.54	2.0	570	119
K.LEP NO. 5	221.59	2.0	496	72
K.LEP NO. 0	209.32	1.2	478	141
K.LEP CP56 A	195.8	2.4	772	. 205
K.LEP CP57	179.07	1.6	709	173
K.LEP CP57 A	166.11	2.8	490	197
K.LEP CP58	156.20	1.8	860	229
K.LEP CP58 A	144.18	2.8	510	261
K.LEP CP59	137.54	6.4	505	352
K.LEP CP59 A	129.09	2.8	485	128
K.LEP CP60	120.45	2.0	425	92
K.LEP CP60 A	113.67	10.0	490	223
K.LEP CP61	103.93	4.0	505	63
K.LEP CP61 A	98.02	4.0	440	76
K.LEP CP62	89.85	3.8	505	80
K.LEP CP62 A	83.85	4.0	495	244
K.LEP CP63	74.29	6.0	395	76
K.LEP CP63 A	66.34	6.0	490	107
K.LEP CP64	57.16	3.2	1,095	41
K.LEP CP64 A	41.53	4.8	550	258
K.LEP CP65	31.23	1.6	540	149
K.LEP CP65 A	22.45	3.2	440	163
K.LEP CP66	18.85	2.4	530	184
K.LEP CP66 A	12.45	2.4	950	123
K.LEP CP67	3.61	1.2	950	477

Table-7.3 River Channel Characteristics of K. Glidik

No. of Section	Elevation of Riverbed	Height of Bank	Distance	Width of River Channe
BS.BANG 58	1,200.0	50	1,530	45
BS.BANG 57	936.12	50	430	45
BS.BANG 53	886.37	50	463	60
BS.BANG 47	861.41	50	358	64
S.BANG 44	826.05	50	577	130
S.BANG 37	801.63	50	371	41
S.BANG 31	783.62	50	162	95
S.BANG 29	758.97	50	862	45
BS.BANG 25	751.75	50	647	37
BS.BANG 19	726.57	11.0	717	66
S.BANG 12	692.91	8.6	838	75
BS.BANG 4	650.48	14.0	299	93
BS.BANG 1	634.59	11.5	112	104
K.LENG 240	767.0	15	1,120	36
K.LENG 238	739.5	10	473	36
LENG 237	727.24	10	676.2	36
K.LENG 236	721.86	7	581.3	23
C.LENG 235	719.22	6	538.1	25
LENG 234	717.99	2.8	594.4	22
LENG 231	717.91	1.5	964	94
C.LENG 220	715.91	3.3	1,073	67
LENG 206	704.41	3.5	1,686	95
LENG 186	695.63	0.9	1,343	37
(.LENG 171	689.68	4.5	875	104
LENG 161	694.66	3.4	1,016	289
LENG 151	688.70	2.6	462	113
LENG 147	674.68	3.3	322	82
LENG 144	662.60	4.5	78	56
C. LENG 143	658.83	8.2	507	94
C.LENG 138	634.59	11.5	615	104
	593.47	100		
			130	50 .
C.LENG 129	466.26	100	586	50
C.LENG 124	425.86	100	618	60
K.LENG 119	390.92	100	751	38
K.LENG 113	372.44	100	379	40
K.LENG 109	355.87	100	500	41
LENG 104	339.28	100	625	38
K.LENG 98	318.97	100	1,418	59
LENG 87	270.71	100	297	94
LENG 85	262.27	100	304	44
LENG 83	255,53	100	754	87
LENG 78	223.69	100	390	262
LENG 74	209.32	2.4	946	208
.GLIDIK 65	183.25	1.9	462	296
GLIDIK 64	178.33	1.9	280	296
GLIDIK 62	173.68	3.8	904	355
	148.66	2.6	and the second s	336
COLDIN 54	136.76	2.0	439 772	
GLIDIK 51	199 9E			246
GLIDIK 44	123.35	0.8	443	289
GLIDIK 40	113.10	0.8	609	207
GLIDIK 34	101.48	1.0	522	223
GLIDIK 30	90.34	0.4	597	319
GLIDIK 24	80.75	1.4	1,149	191
GLIDIK 14	59.54	1.4	1,082	261
GLIDIK 4	48.88	2.0	2,400	313
(.GLIDIK 0	21.92	1.9	2,400	267

7.3 FINDINGS OF THE RIVERBED FLUCTUATION CALCULATION

(1) Riverbed Fluctuation Tendency

The probability rainfall used in the riverbed fluctuation calculation was set for the probability of exceedence at 1/3, 1/5, 1/10, 1/20, 1/40, 1/70 and 1/100.

Fig.-7.6 and 7.8 show the calculation results for the probability one hundredth. The fluctuation tendency shown there is the same when the probability of exceedence is small.

The simulation shows that each river causes flooding in the alluvial fan and the major flooding points are as follows:

K. Mujur The section between 10 and 20 km from the river mouth;

K. Rejali The sections between 10 and 19 km and between 3.5 and 10.5 km from the river mouth.

(2) Design Sediment Volume

The harmful sediment volume, in other words, the sediment volume to be controlled (Q_{BD}) , for each design size should be given. Their actual values are given by the following formula.

$$QeD = Qin - Qout$$

$$C^*$$
(7.12)

 Q_{RD} : Design Sediment Volume

Q in : Passing Sediment Volume at Supplementary Reference

Q out: Passing Sediment Volume at Design Reference Point

C* : Grain Concentration by Volume C* = 0.55

The design sediment volume which is given by the formula (7.12), based on the results of river fluctuation simulation, is shown in Table-7.6.

			·
River System Return Period (Year)	K. Mujur (m ³)	K. Rejali (m ³)	K. Glidik (m ³)
3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	250,000	1,610,000	1,510,000
5	270,000	1,940,000	1,830,000
10	330,000	2,390,000	2,310,000
20	1,250,000	3,020,000	3,200,000
40	2,070,000	3,680,000	3,200,000
70	3,480,000	4,510,000	4,200,000
100	5,040,000	5,220,000	4,500,000
Supplementary reference point	No.11+65 No.277+34	No.80	No.74
Design reference point	No.29	No.62	No.4

Table-7.6 Design Sediment Volume

(3) Riverbed Fluctuation Tendency with the Planned Control Facilities

Fig.-7.9 through Fig.-7.11 show the result of the riverbed fluctuation calculation after completion of the planned sediment control facilities.

In this calculation, the computed facilities are those which are described as the first and the second steps in the master plan. Furthermore, it is assumed, for the purpose of calculation, that the check dams are filled to their capacity by sediment and that the sand pockets are empty.

The prospect is clearly shown that with the completion of the planned sediment control facilities in the master plan, sediment flooding will be practically contained. (4) Control Sediment Volume by Facilities

The control volume by check dam (Vc) is given by the following formula.

Vc = [(Run-off Sediment Volume without Facilities) - (Run-off Sediment Volume with facilities)] . $\frac{1}{C}$ * (7.13)

The control sediment volume given above consists of the run-off sediment regulation volume and the yield suppression sediment volume.

The value of Vc, given by the formula (7.13), varies according to the design magnitude. However, values for the 40-100 return period plan do not show any substantial difference. Moreover, as the value of Vc attained here will be also used for the economic evaluation of sediment control facilities, the value of Vc to be used is determined in consideration of the storage capacity of the check dams and of their efficiency as a group, while based on the control volume at the return period of 40 years.

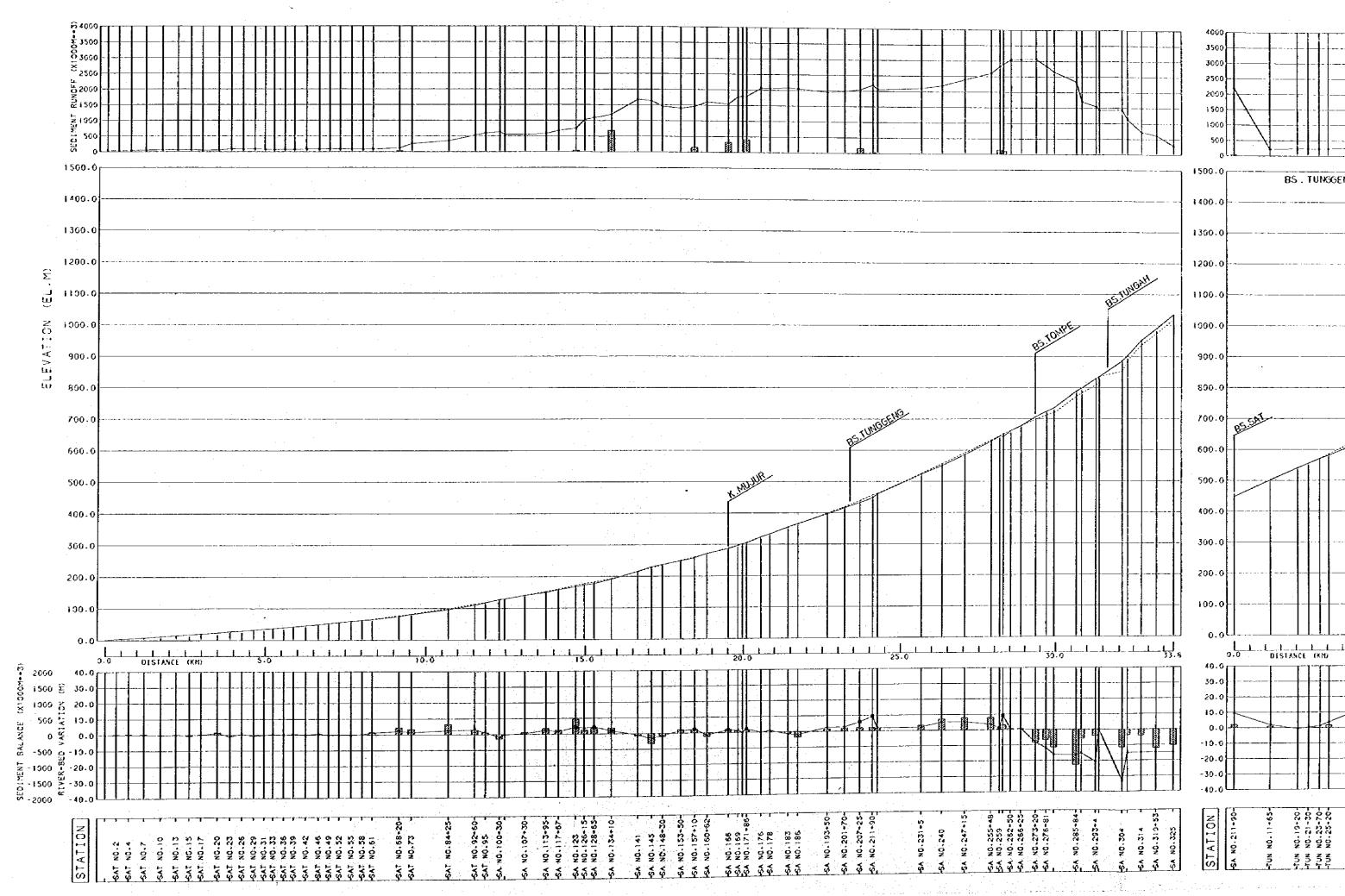
At the K. Glidik, however, the 100 years return period plan is used as the amount of sediment discharge is so small that it does not reach to the dam at the lower stream if the 40 years return period plan is used.

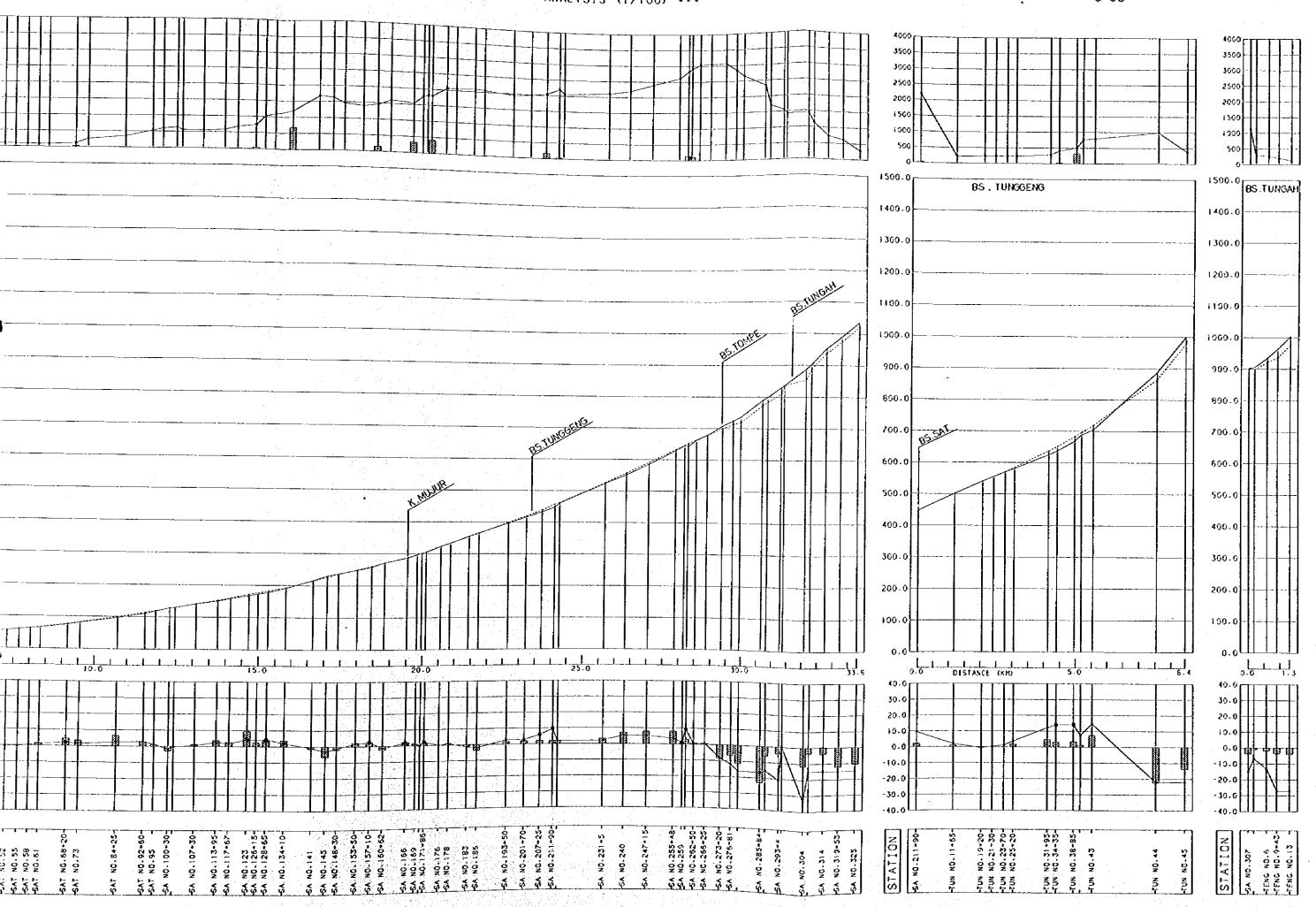
The run-off storage sediment volume is used as the control volume for sand pockets, as such maintenance work as removing deposits is expected to be carried out.

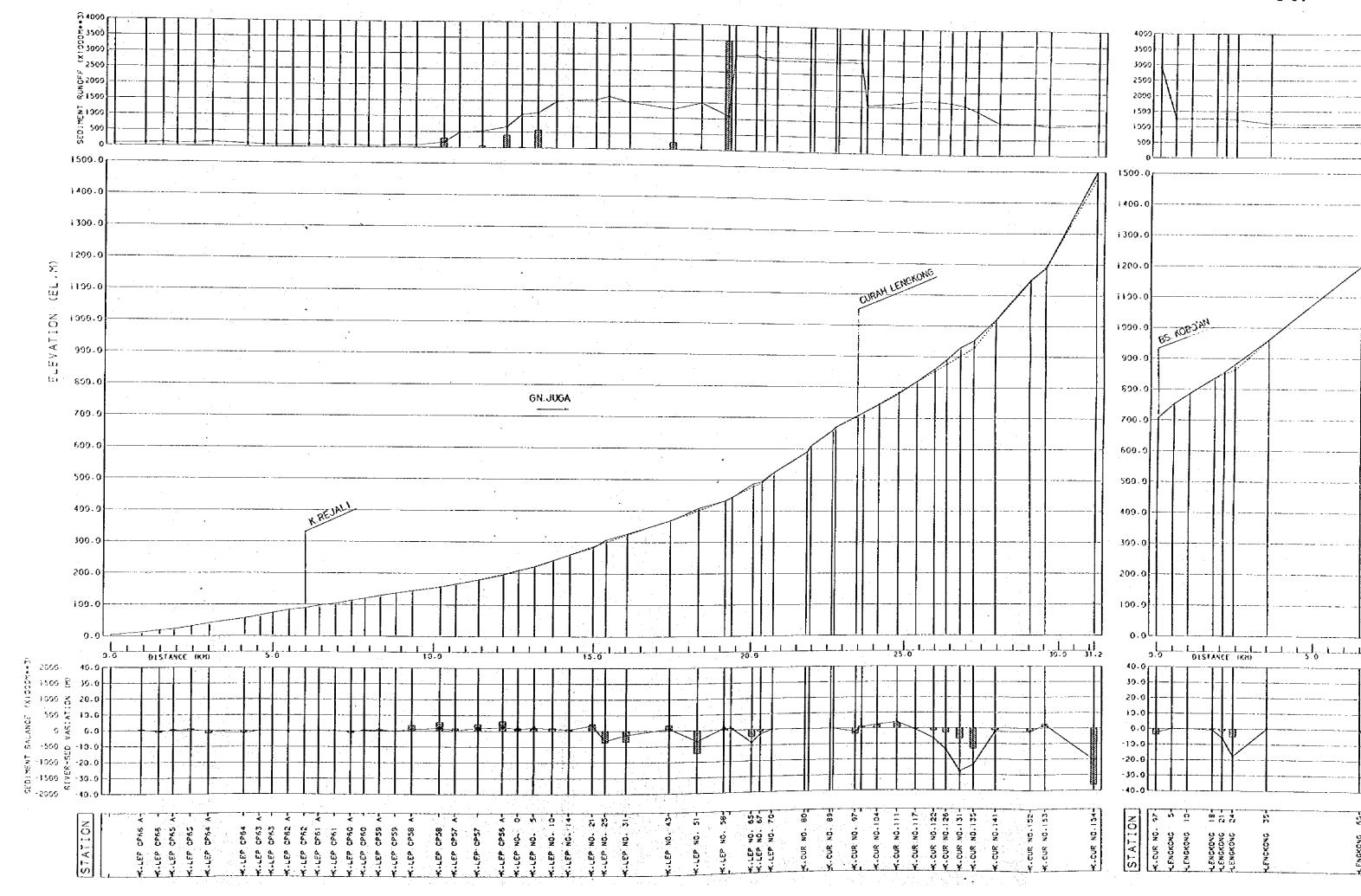
The effects of the sediment control facilities are as shown in Table-7.7.

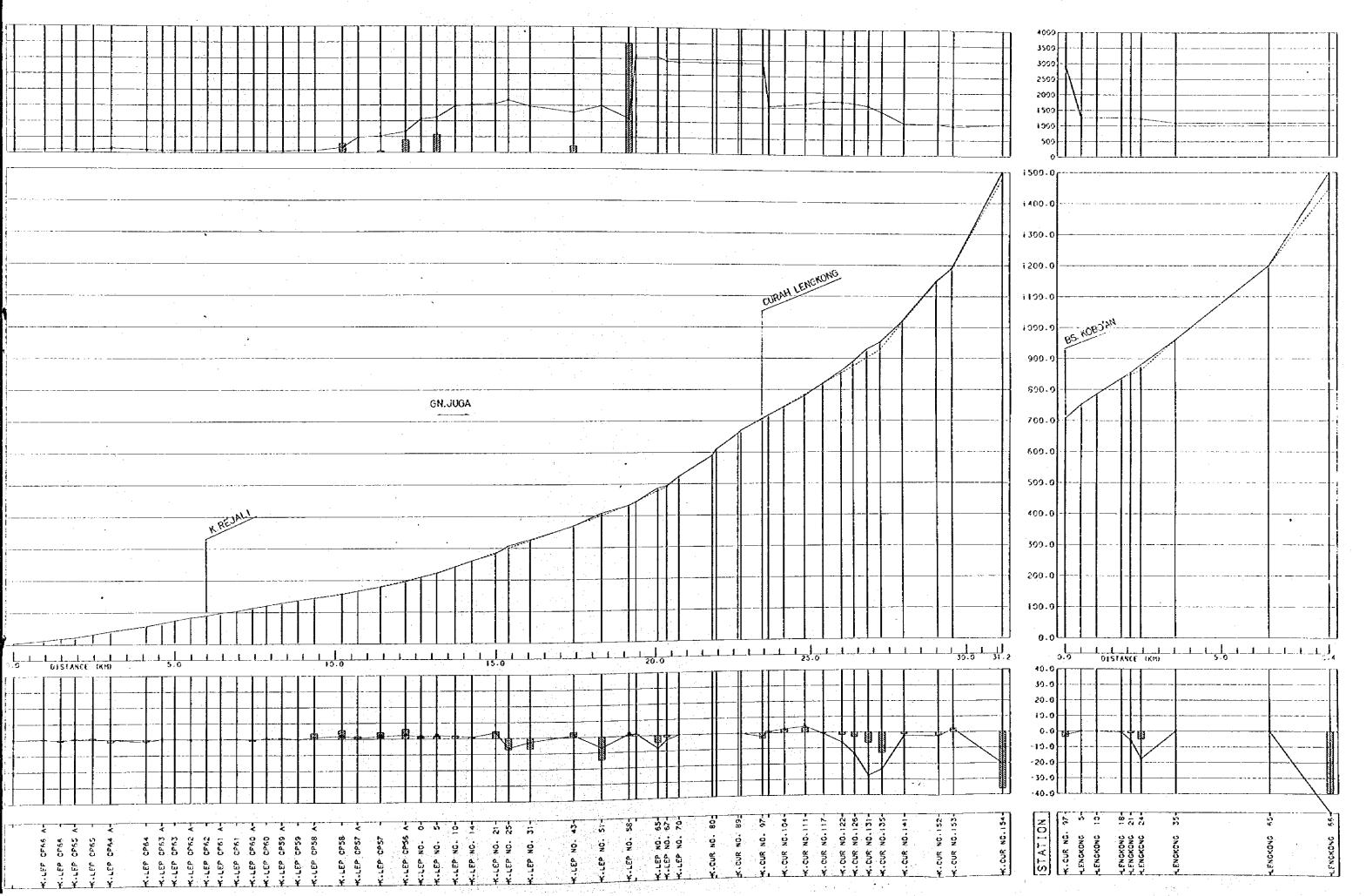
Table-7.7 Effects of Sediment Control Facilities

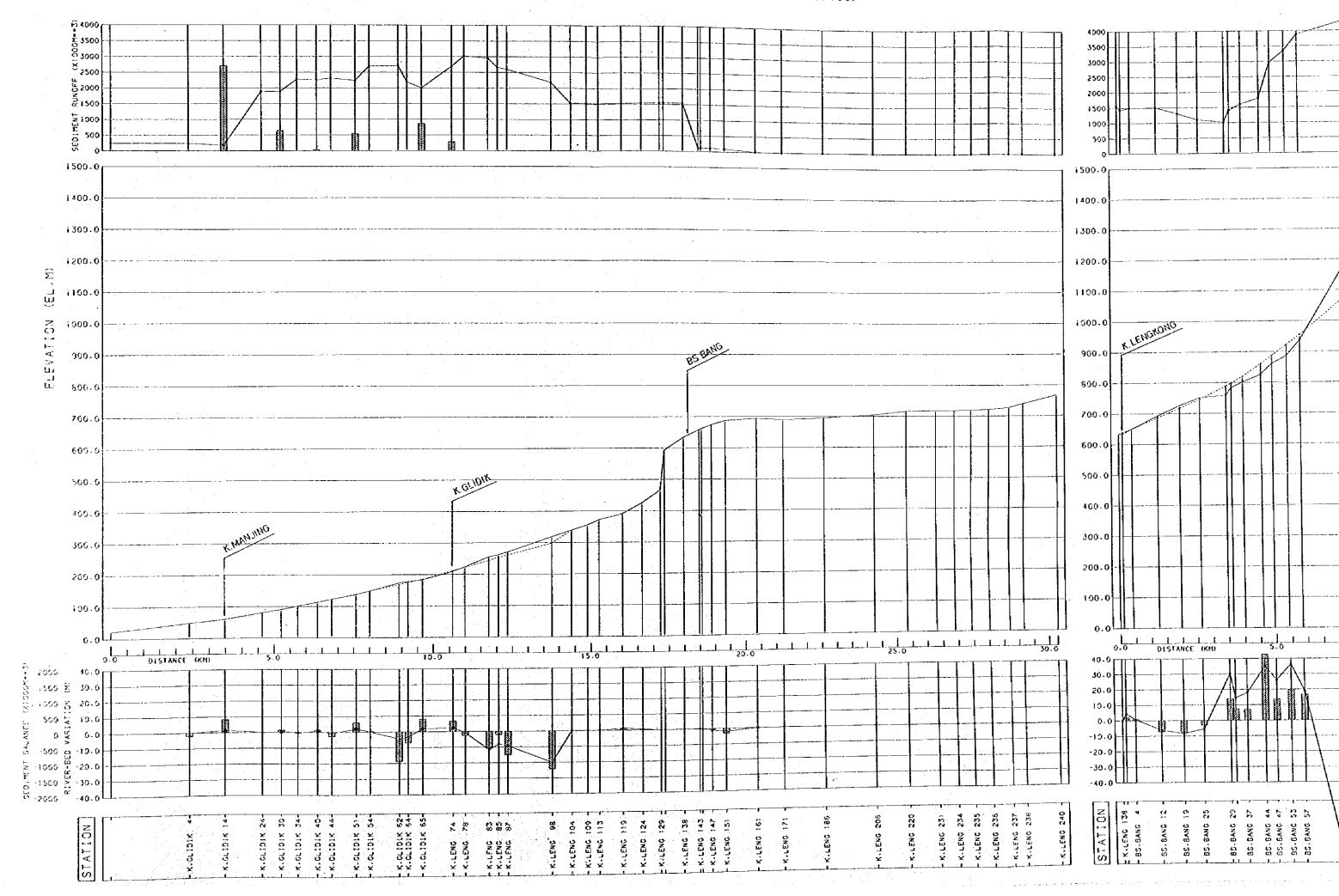
River System	Type of Work	Function	Name of Facility	Controllable volume (10 ³ m ³)
	Minimum, apiness v servite, lite in televise papagan ing s	-militari massangan masangan kantangan penangan pengangan pada dalah berahilangan pengangan pengangan pendadah	BS. Sat Check Dam-4	15
. 1			BS. Sat Check Dam-5	30
		Sediment yield suppression	• 6	130
٠٠	Check dam	Regulation of	7	1,050
		tunoff sediment	* 8	240
			· • 9	340
K. Hujur			10	276
			Sember sari check dam	2,117
			Kertosari Sand	1,414
	Sand pocket	Storage of runoff sediment	Pocket Keloposavit Sand	313
			Pocket Benda Sand Pocket	423
	Check dan	Sediment yeild	BS. Sat Check Dam-2	164
:	(Under construction)	suppression Control of runoff sediment	BS. Sat Check Dam-3	94
-		Total		6,608
		Suppression of sediment yeild	BS. Kobo'an Check Dam-3	90
·	Check dam	Regulation of runoff sediment	* 4 * 5	660 90
-		tanott seouseat	• 6	J
	_		• 7	430 300
K, Rejali	·		Curah Lengkong Check Dam-1	160
			• 2	. 80
	Diversion channel	Reduction of runoff sediment	Diversion channel	2,220
	Sand pocket	Storage of runoff sediment	K. Leprak Sand Pocket-1	250
			• 2	730
:			. 3	360
		Total		5,370
	Check Dam	Suppression of sediment yield suppression	Check Dam-7	2 165
K. Glidik		Regulation of runoff sediment	5	22 12
	•		* 3 * 2 * 1	360 2,100 440
			K. Glidik 2 Check Dam-1	480 - 980
i		Total		4,561

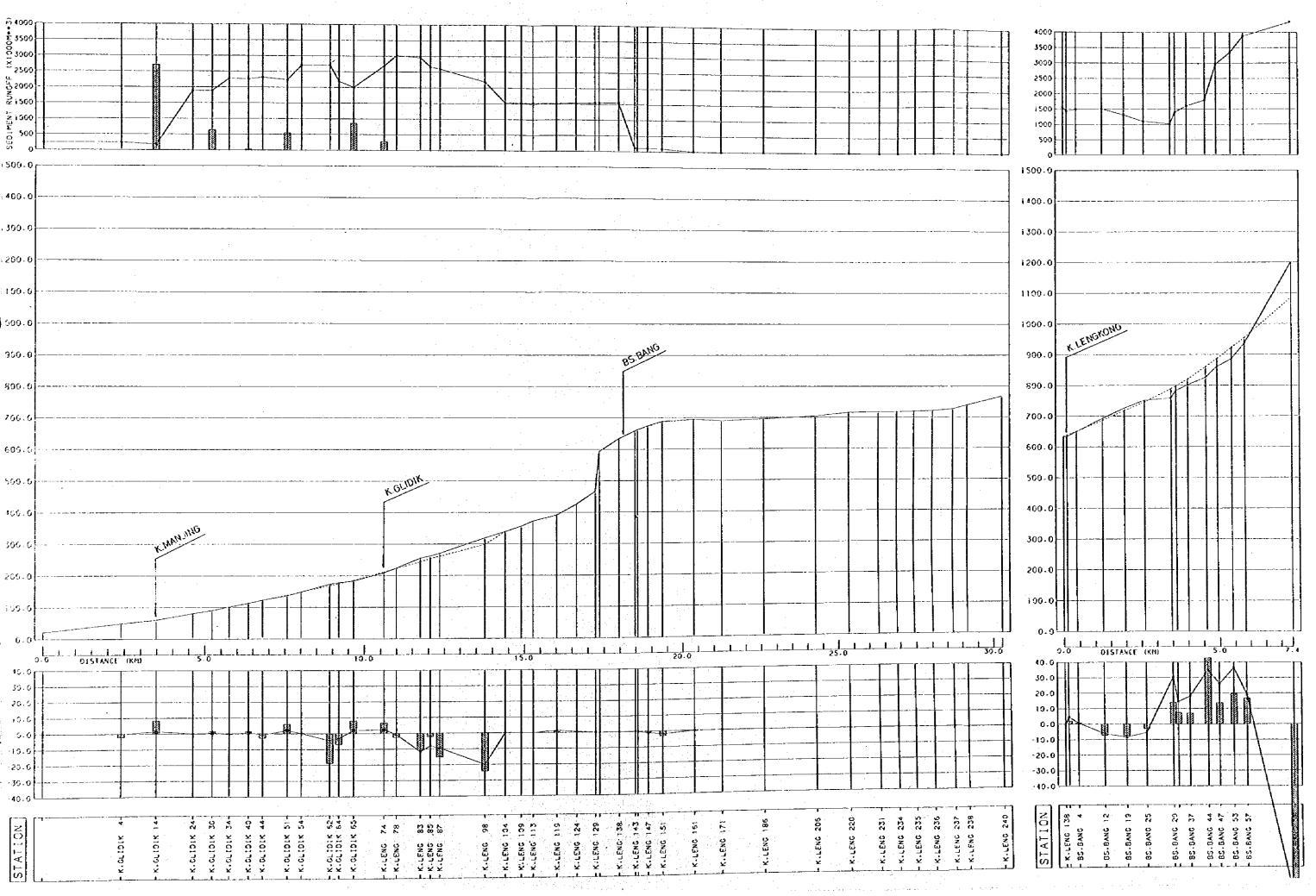










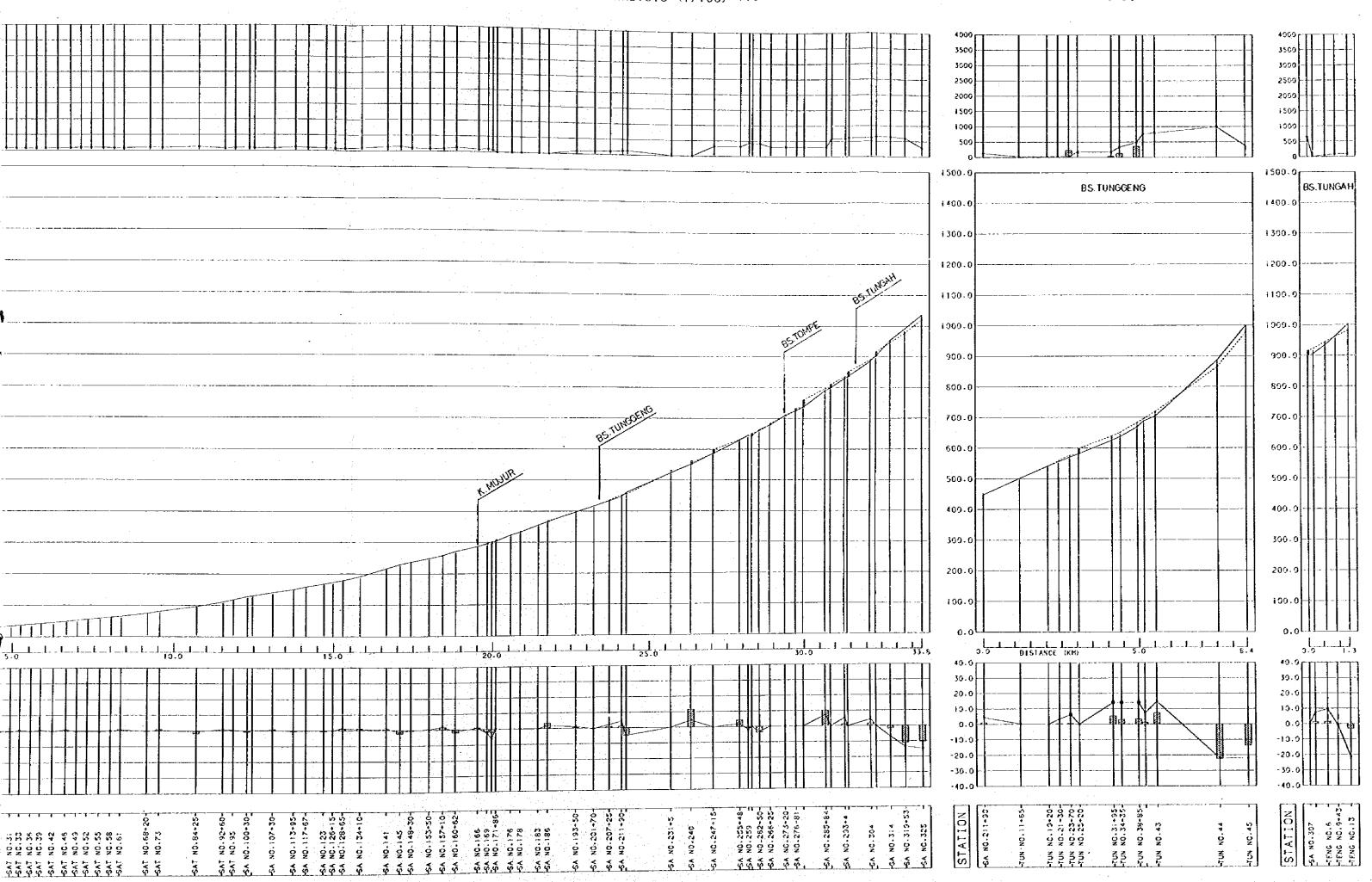


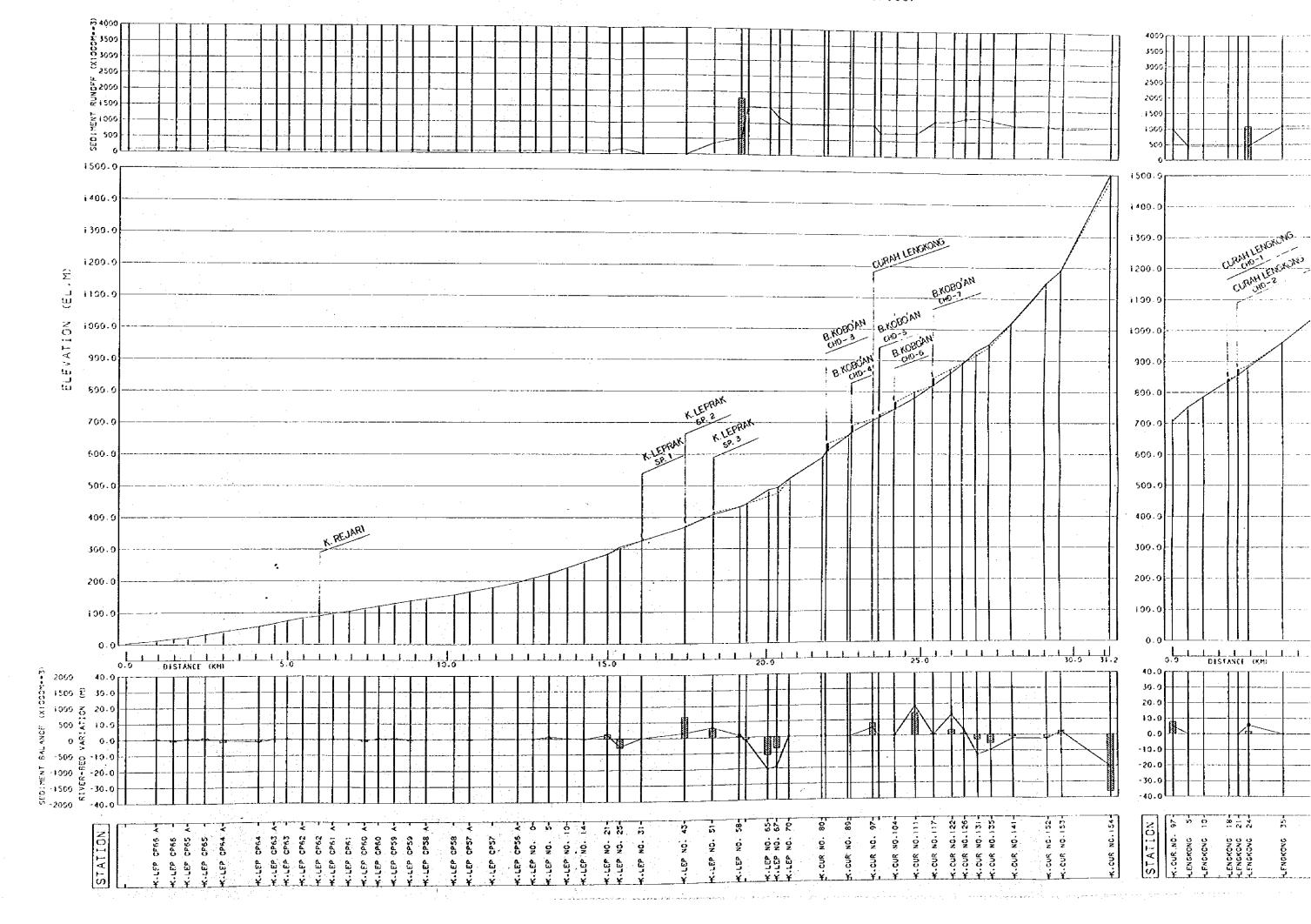
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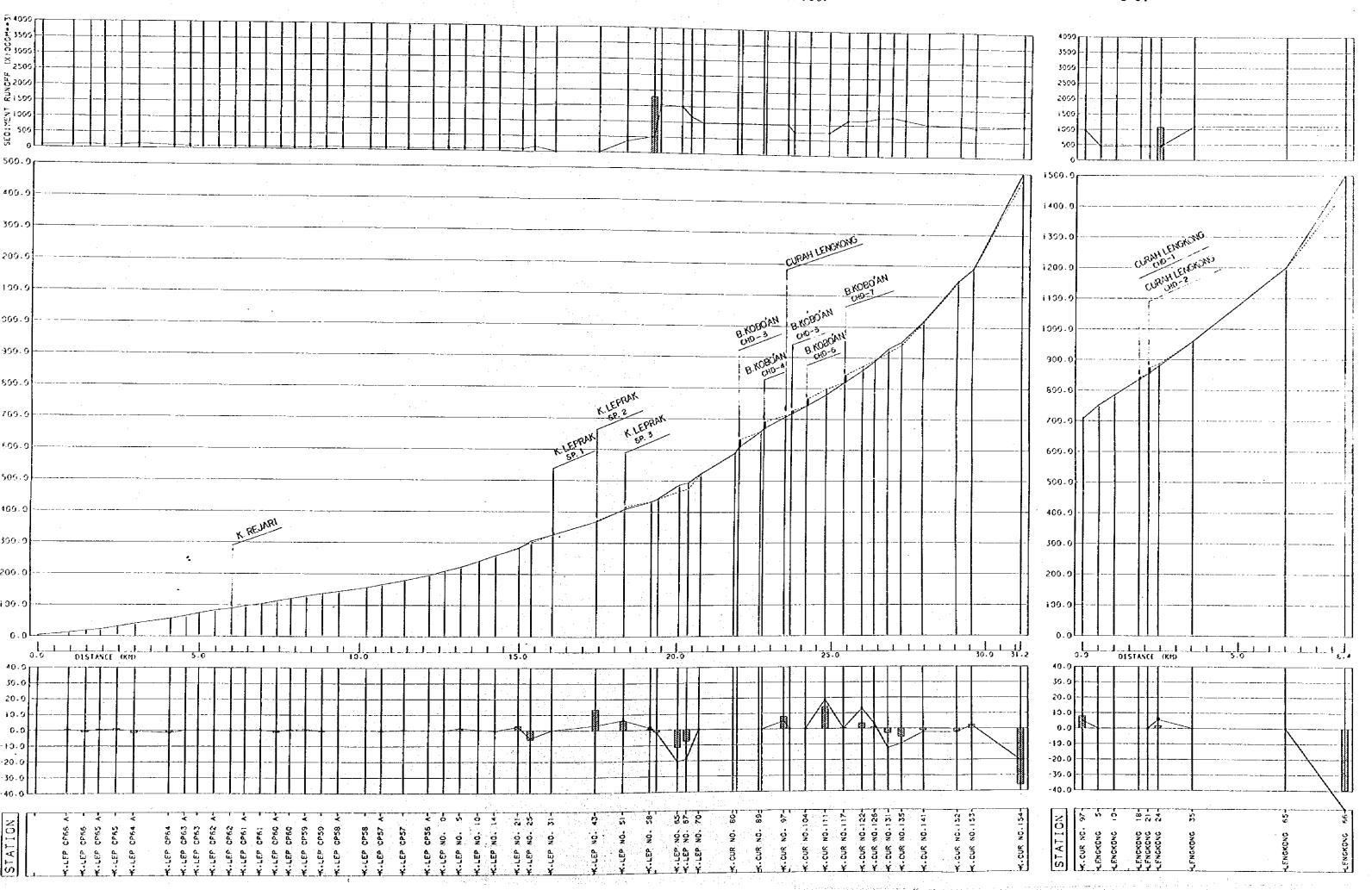
ELEVATION

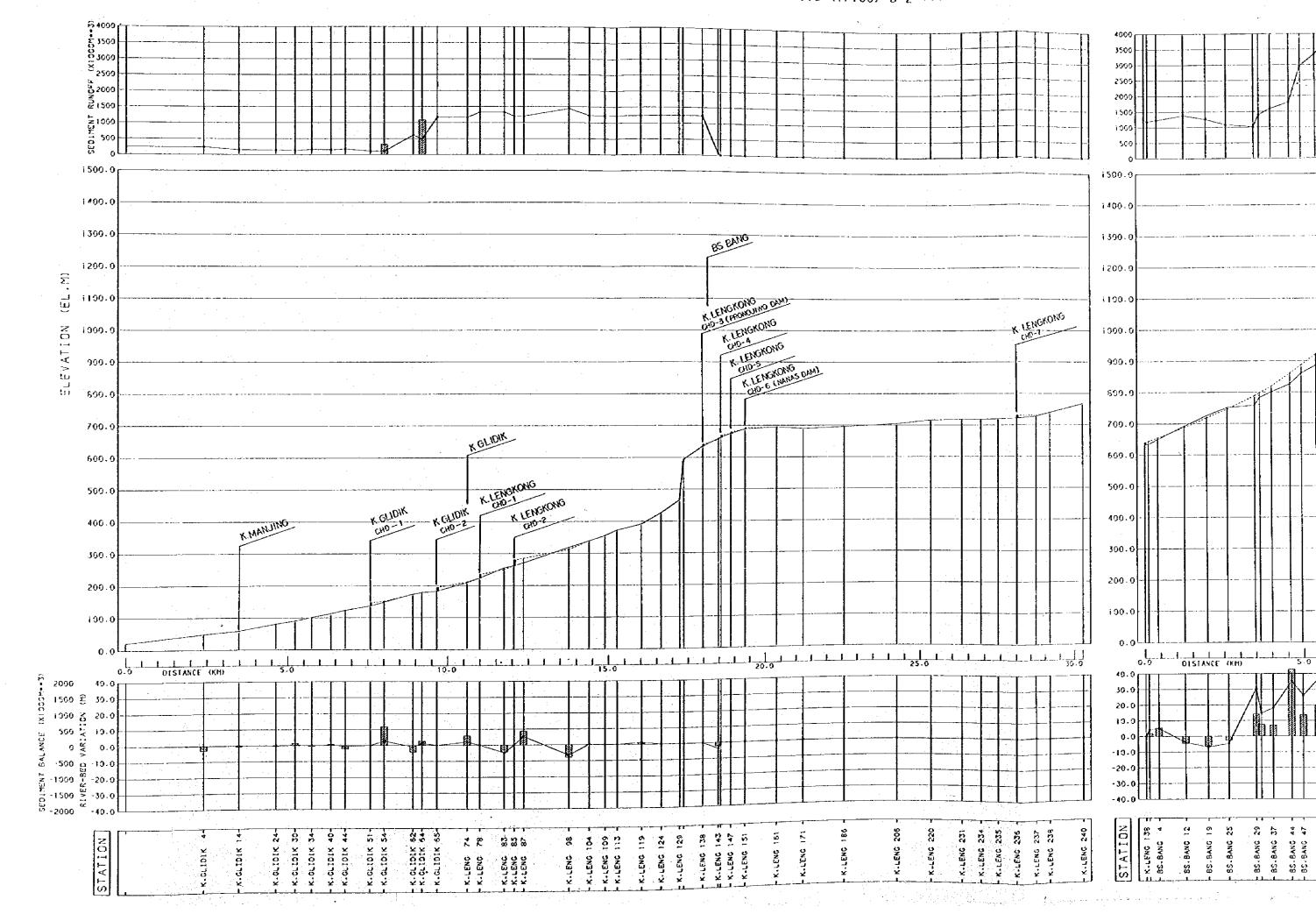
2000

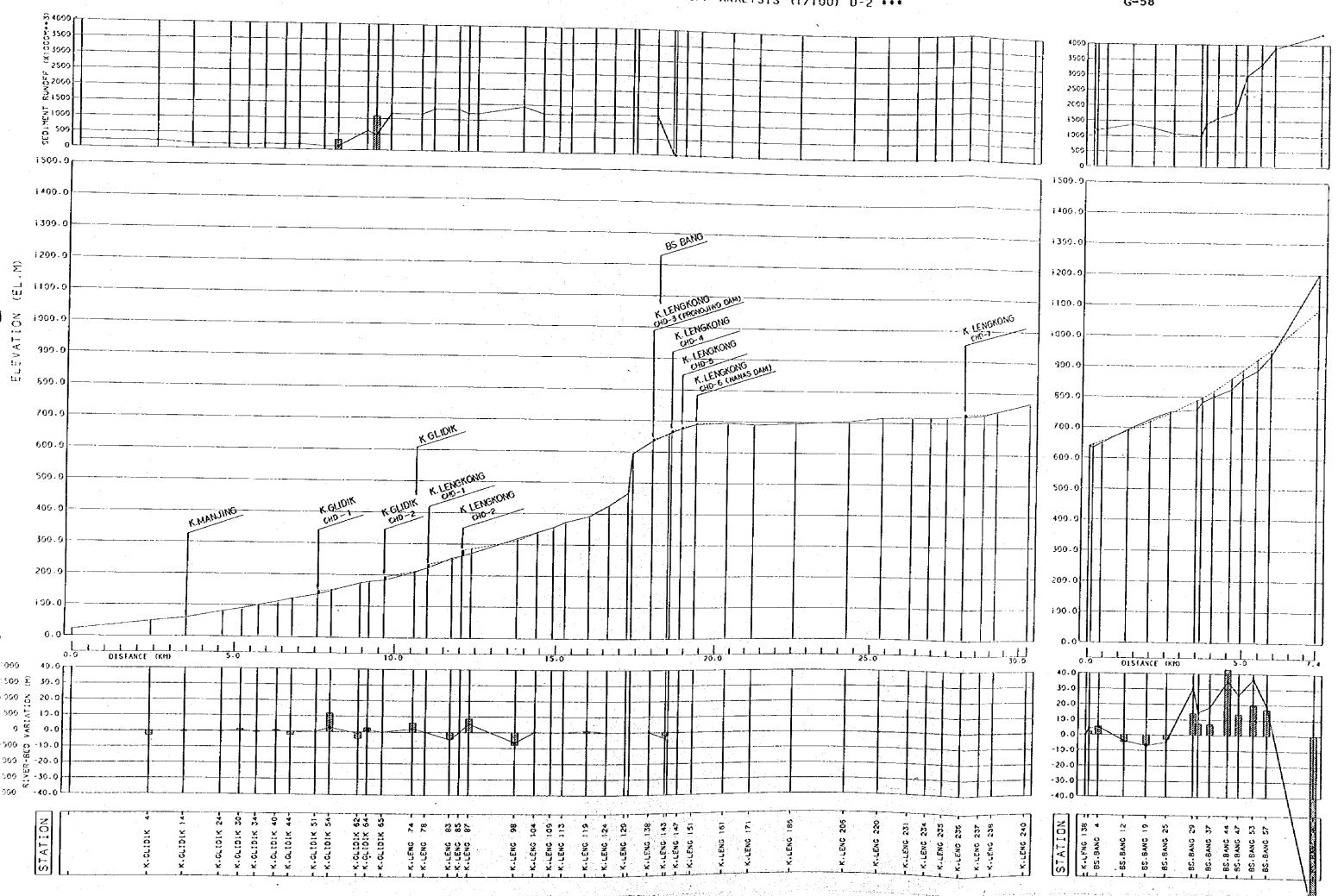
-1000 E1 -1500 -2000

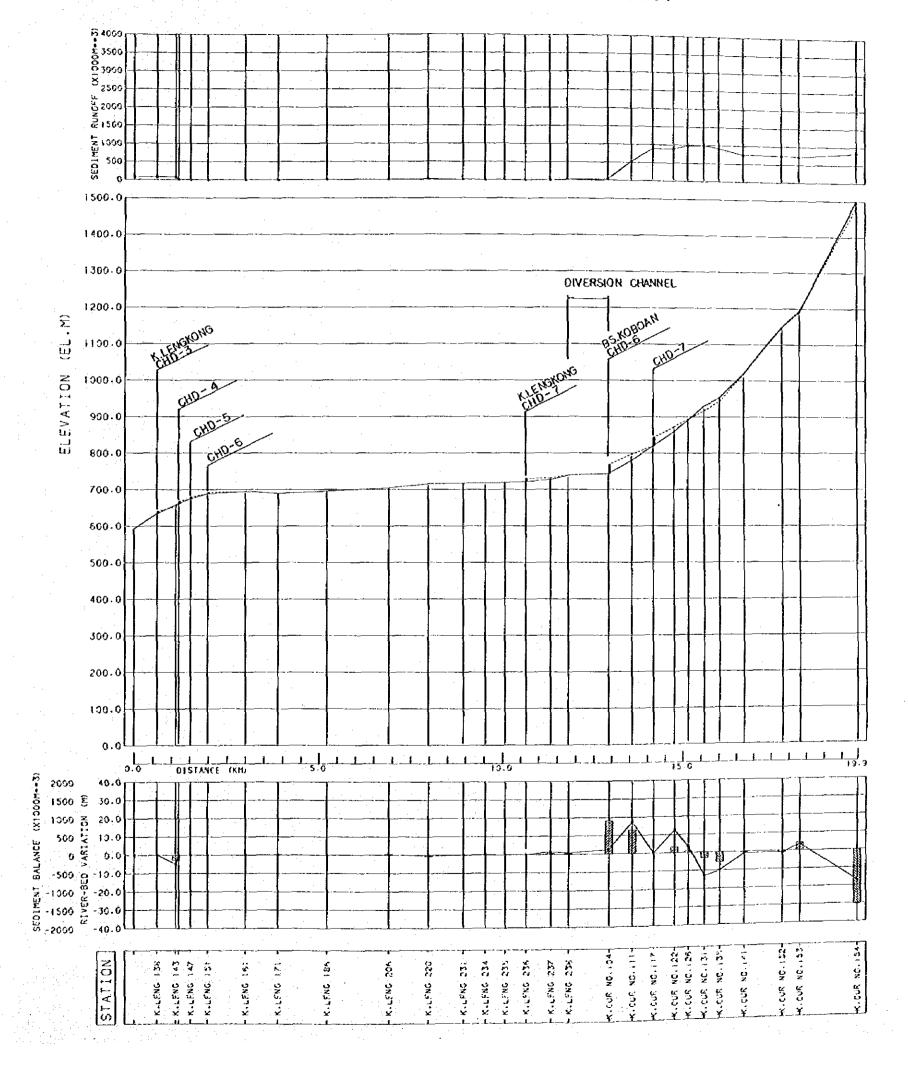












OUTPUT OF RIVERBED FLUCTUATION SIMULATION ON K. MUJUR

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ul L	ũ	3	ž.	ģ	4		٠.	ģ	ö	8	7.4	Ç			5	ġ	Š	128.011	ě	7	96.734	41.130	75.479	45.420	62.728	59.741	55,806	53.116	40.04	45.058	42.562	40-140	37.446	35.143	32.152	29.482	26.701	23.600	20.003	18.479	15.400	12 326	44.4	, V	,		1
इ.	ī	•		`. S.				Š	ੋ. ਤ	7.	7.	,	g				Š	124.620	ď	ċ	ć	41.250	75.220	65.040	62,750	00.030	56.040	53,300	000.04	44.120	47.230	40.330	37.070	35.250	32.640	24.980	27.690	22.890	20.620	18,550	15,440	12,340	00%	0.40	V 4	964	: 2)
NAME		6	1+/611	1,15345		844		1.614	1,134+1	겆	1.126+1	104	7. 1. 7. 4.A.	•	7	0 + 2 O 7 × F	701.0	001.0	0.0	9	40.84±2	VC.02	9102	10.07	3,02	9	07	9	4,0	9.02	ž	9	200	SAT NO. 31	N Q	Z,	2.02	NO.2	CON	92	QN	CON	Ş	SAT NO. 4	ž	2	

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	352.0 221.0 9.0 8.0	138.0 102.0	. 4	45.0	36.0 7	51.0 44.0	424.0 1235.0		0.4.0 1656.0		123.0													
本 ※ 本	0.04 0.04 0.04	12.0	> •	:	39.0	63.0	32.0		0.000		0000												-	
.YSIS (1/10)		000		. 194		;		 								•			•					12
HUNDFF ANALY	44. 000	0 0 0 0 0 0	. M .	70.0	0.77	31.0	2.0	25.0	239.0	0,0	320.0	68.0								•	-			
X N C C	21. 21.	046	9 0 0 9 M N	126.0	9 O	0 0 0 0	0.44	1 & C	0.44 0.00 0.00	0.00	542.0	76.0			-		:							
74 A. 00.00 A.	W. 0.0	4.	, m c	200	000 M	319.0	12.0	0	717.0	67.0	1058.0	×7.0				•	:		•	ŧ .				

*** K.MUJUP SECTMENT RUNDEF ANALYSIS (1/10) ***

										•			
	SMAN	S.	a.	20	\$\$	38	Sas	Seo	U	808	SVB	SVO	VDAM
		(EL.M)	(EL.™)	£			(KH)	(KM)	(%)	(M3)	(M3)	(M3)	(H3)
		***	400	,	•	•							
	Ç	C 12 K C C	0000		2 C	10	1000	040011	•		174640		•
	O'CZ SZU	934.630	931.910	7	072	0.56	311F+0	1195+0		0005+0	0.1065+0	0000	٠.
	Š	905,770	900,508	3.43H	0.0161	042	0.1505-05	1165		3000	204E	0.0000-0	
	() () () () () () () () () ()	000	007.800	2.00		125	A+01-0-X	1000	4	0.000	402F+	00000	•
	2	200	4	30		100	0045	04004		1000			
		704 330	708 404	6		, 0	43.F+0	0 + 13 0 0 1	• , •	1000	1000	044000	•
	AA-ON CO MILE	000	480 000	, ~) C	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	04.00	•	14000	1000		•
	がいりない。これでは、これには、これには、これには、これには、これには、これには、これには、これに	668 720	672 473	1			00 P. F. C	0.00	• •	048000	1	044000	•
	ANTI- CA SEL	200	100	 			71.7	01110	•	0 (10 0 0	1000		•
	SOLVE SOLVE	200	426.522	16	0.0411		D 1 3 4 FF + 0 W	0.1245+07		0005400	0.6745+04	000000	> 0
	1.55 ×0.104 ×0.00	882,020	541,142			90	1178+0	1745	•	0.000	3016	044000	• •
	MAN NO. 77	570, 320	560,453	i X		039	134E+0	124E+0		000E+0	326E+	0+3000	
		555, 160	555,006	ક		0	136E+0	1246+0		-0005+0	.188E.	000E+0	
	11M NO.10+20	3	740.703	K		440,	1585+0	-124E+0	· •	-000E+0	.400E	0000 + O	•
	TUN NO TO +05	500, 430	501,050			040	.140E+0	.124F+0	•	-000E+0	3108+	.000E+0	0.0
	SCE UN VS		1033,770	•	C	•	2856	1776+	9	000E+	-0.518F	0005+0	
	SA NO.320-53	ò	988.630	500	o		3614	1836	4	000	-0. 607E+	044000	0
	SA 70 374		951.840	-1. V80	o		7276	1416+	4	÷3000	-0-197E+	000E+0	
	7.7	302.50c	901.300	-1.500	o		3040	299E+	ĸ	150E+	-0-185E+	000E+0	
		\$86,420	878.513	-8.307	ó	v	176E+	.314E+	٥.	-000E+	-0:142E+	-000E-0	0
	*	£X-220	8.4.220	000-0	۰	~	1765	3146	d A	-000E+	0.902E+	COOE	0
	SA NO.2031	22	324 755	->.204	Ö	~	Š.	3205.	ò	3060	-0.5136	000E+0	3
	NO. 1293	780.220	797,749	-1-471	ö.	•	į, N	3238	0	900	-0.275E+	0000	• •
	• • •	780, 580	787, 411	•	o s		200	* DAY 1	6 N	0000	-0-1205	0005+0	0.0
	STATE OF THE PARTY		407.404		, ç	-	7	7) v	9000	1071 D) c
	٠.	707	707	7	,		100	1	4				4
1	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	678.260	6.90 . 37.6		0			900	ر د د	000	0.6535	000E+0	
		662.520	541.843	Ť	0	•	1778	3045	Ň	DOOF	-0.385E	000E+0	0
1	3.3	643.020	906 879	•	Ó	_	125E-	557E+	۱۱۱ ا	000E	0.932E	0-30-00	· • ·
	100	544.790	643,132	-1 657	، ټ	~ .	1.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4		ν ν Λ. ς	9000 0000 0000	-0.297E	000	0 0
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	44 MC 240	551 440	557.016	0.576	. 0	-	1108	300		0000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	048000	0
		523,940	523,624	-0.316	Ģ	_	1225.	5.40F	~	.000E.	-0.215E	0.000	- 4
	SA NO.223+30	460,710	459.851	-0 x >0	ø	-	1456	*9269 <i>*</i>	, ,	.000E+	-0-416E	000E+0	0.0
-	SA NO.211490 #	447.900	453.377	5.477	ø.	_	1255	4 M	о <u>.</u>	140E	0.611E	0000	•
	SA NO.207-25	493, 500	-\$ +	2,740	Ó ¢	_	.071E	17.0	4	13000 F	0.507	0+1000	0 0
	NO.201-10	410.000	\$ * * * O T \$	0	٠ خ			307			1000		4
	00 10 10 10 10 10 10 10 10 10 10 10 10 1	004, 447	446	104	> <	_	300	10 TO TO	4 4	10000	36.00	044600	• '
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		332,400	333, 571		0		1026	8255		9000	0.361E	000E-0	1 4
	SA MO.176	324 A40	323,840		0	_	1025	£226	C+	9000	0.238E+	0000	
	NO. 17	304,410	305,255	7.	0		8	840E	-	OCC	0.253E	000E+0	
	NO.170	307.190	301.104	•	୍ଦ		984E	3008	-4	.000E-	-0.193E	000E+0	-
	NO.169	204,400	296.609	-0.041	c		SASS	800E	7	000E	-0.2208	0-3000	•
	100	284,790	288,139	o	0.0210	0.0237	0 - NS+m+0 + 0 + 0 + 0 + 0 + 0 + 0 + 0 + 0 + 0	0.9435-07	٠ ،	0.000E-00	V00FH0V	000000	0
	SA NO.160+62	272.240	271, (30	-0.0	=		1001		Ċ C	00.0	-0-42×E-	0.005-11	4

Ä	NAME	ES.	យ ប	20	SS	SE	Sas	San	U	SOR	8 × 8	SVO	VDAM
		(H. 19)	(BL.#)	- 3			(EM)	3 A	(%)	Ŷ		. 5	_
	1574.1	254.110	258,963	- 6	0.021	∹	.>67E-0	. 942E+0	ċ	.000E+0	* 40AE	.000E+0	Š
		Ž,	246.732	•	0.019	ဌ	- 369E-0	.995E+0	٠	.000E+0	.5595	0+3000.	8
	140+3	237,520	237,300	•	0.00	.023	.631E+0	1026+0	•	.000E+0	.1226-	-0005+0	ş
	1 50	ž	229.868	o	0.035	S	.891E+0	.104E+0	٠	.000E+0	.473E	0+3000	ş
	1	216.220	216.594	ö	0.030	030	,735€+0	1056+0	•	.0006+0	.283E•	.000E+0	٩
	34+1	٥	191.276	ó	0.023	025	-6178+0	.108F+0	•	0-3000°	.216E+	-0000-	S
			178.739		0.015	010	.461E+0	.110F+0	•	.000E+0	.283E+	.000E+0	٠.
	26+1	_	172.983	ó	0.039	910	. 50 HE . 0	.110E+0	•	000€+0	# 59E+	*000E+0	ď
	23	٥	167.779	ó	0.036	910	.6128+0	.111E+0		-0.00E+0	.190E.	0+3000	٩
	117+0	154.780	158.765	ó	0.02	.021	.415E+0	.114E+0	•	.000E+0	.501E	.0006+0	2
		ň	150.359	ó	0.014	710	. 00AE+0	.115F+0	٠	-0000°	-180E+	.000E+0	ď
	(07+3	141.610	140.861	•	0.016	ν,	.705E+0	.117F+0	٠	.000E+0	.176E+	.000E+0	ď
	ر ده	130.280	130.658		0.010	0.15	.590E+0	.119E+0	•	.000E+0	3000	043000	٠.
	80	128.120	128.201	•	0.023	.025	.7726.0	.120E+0	٠	.000E+0	.3326	-000E+0	٩
	95	114.050	116.371	•	0.022	₹	0-3969	.122E+0	•	.000E+0	.1395.	.0006+0	Ş.
	Š	110,740	111.466	•	0.016	017	-512E+0	1236+0	•	.0005+0	.334E	0000-	ç
	84+2	96.HO	96.692	•	0.013	.013	. 629E+0	.120E+0	•	-000E+0	.2115	.000E+0	8
	73	81.250	H1.342	-	0.015	015	.566E+0	13051-	•	-0006+0	.1126	-000E+0	٧,
		75.220	75,230	•	0.012	.01	.5296+0	.1316+0	6	-000E+0	.475E	043000	8
		65.040	65.508	•	0.007	400	3.566E+0	1335+0		-000E+0	.296E	-0005+0	္ပိ
		62.750	62.747	o	0.007	Õ	347E+0	1336+0	٠	.000E-0	.644E	.000E+0	ŝ
		60.030	20.00	•	0.010	010	440E+D	.133E+0		-000E+0	140E	.000E+0	S
		56.040	45.776	ö	H00-0	0	0+3041	1338+0	è	.000E+0	.357	.000E+0	ွ
		23.360	53.086	ċ	0.033	0.0	.5276+0	.1-33E+0	٠	.000E+0	3601.	.000E+0	Ş.
		000.07	40.704	ó	0.00	000	. SA7E+0	1335+0		,000E+0	-254E	.000E+0	٧.
		44.120	40.04	•	070	000	04 14 14 4 0	.133E+0	٠	.000E+0	.265E	-000E+0	ģ
		42.230	42.597	ó	0000	00	. 564E+0	.133E+0	•	0.000	.970E	0000	S
		40.330	40.170	٠.	0	Ö	. 587E +0	3	ě	0000	М О О	00E+0	
		37.070	37.469	o.	0.00	000	. 511E+0	*155E+0	•	0000±	A (0000	Š.
		00X 1 K	90,130	o,	0.00		0.40€±0.	135E+0	•	-000E	• 0.22E	0-3000 -	Š
		040.76	32.078	•	000	, oo		0+1401	٠	0+1000	1908	0000	Ş
		007.43	0/4	j.		> 0	0	1020	٠	0 + 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	, ,	1000 1000 1000	36
		000	0 40	•	3 6			0 - 50 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	٠			24000	3 6
		20170	07/10/	•	200	2	0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	\$ 100 to	٠.			0+1000+	3 6
		20.620	20.07		000	000	1.7.7E+0	1005		- 0000	1	-000F+0	2
		000.41	18.454	•	0.00	00	.265E-0	1.00E+0	٠	.000E+0	200	.000E+0	Š
		O## 161	15.481	•	0000	600	. 231E+0	0+10A1	٠	. 000E+0	7/10	*000E+0	3
		17.340	12.319	•	0.00	100	*257E+0	159E+0	٠	.000E+0	30/1	-000E+0	S;
		h.420	00 7 . a	•	6000	600	ZOIE+O	157E+0	٠	-000E-0	105	0000 + 0000 + 0	Ö
		7.480	5.728	•	\$00.0	00.	13251	1575+0	٠	.000E+0	W .	.000E+0	8
SAT NO	2.5	004.8	3,426	0.026	550010	0.000	0.50E-04	1556+0	٠,٠ ٥	0.000E+00	0 652E+04	9,	000
		1.520	1.622	•	0.004	.00.	24VE+0	Š	•	.000E+0	•461E	04300	0

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SVO	(M3)	0.000E+00 0.000E+00 0.000E+00			20000000000000000000000000000000000000
SVB	(3)	-0.3890 -0.3820 -0.2240 -0.435 -0.55	0.1138 0.1233 0.1333 0.	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	
SOS	(M3)	0.000E+0000.0			
Ų	%	NOVY			o o q o o o o o o o o o o o o o o o o o
San	(M3)	0.1396+07 0.1426+07 0.1426+07	000 000 000 000 000 000 000 000 000 00	00000000000000000000000000000000000000	
Ses	(M3)	0.214E+05 0.407E+05 0.291E+05	00000000000000000000000000000000000000	244422 2446222 2462222 2462222	000000000000000000000000000000000000
S Fi		0.1008	00000000000000000000000000000000000000	00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
SS		0.008 0.008 0.0721 0.0161	00000000000000000000000000000000000000	00000000000000000000000000000000000000	$\frac{1}{2}$
20	(H.)	-8.440 -8.1440 -8.132	444202 WWL 4 CW	000000	
3	(EL.M)	997.870 957.870 931.498 910.879	000 000 000 000 000 000 000 000 000 00	440000 644000	200 00 00 00 00 00 00 00 00 00 00 00 00
ES	⟨EL. M⟩	1007,310 965,310 934,630 905,770	000 000 000 000 000 000 000 000	00000 00000 00000 00000 00000	######################################
A (II)		TENG NO.13 TENG NO.0+43 TENG NO.6 TENG NO.3	TUN NO.45 TUN NO.45 TUN NO.39+65 TUN NO.38-45 TUN NO.38-45 TUN NO.38-35 TUN NO.23-70 TUN NO.23-70 TUN NO.23-70 TUN NO.23-70	NO.3255 NO.319+5 NO.314 NO.307 NO.306	SA NO. 1179 SA NO. 1188 SA NO.

K.MUJUP SEDIMENT RUNDEF ANALYSIS (1/20)

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VOAM	(M3)	000	0000	000.0	0	000	0000	00000	000	00000	000	000	0000	0000	000			> 0 > 0 > 0	000	000.0	0000	000	000	000	00000	000	000		000	0000	0000	0000	0000	0000	3	
SVO	(E &)	0.4996.409	0-3000	0-300C	0-900 1000 1000 1000 1000 1000 1000 1000	000000000000000000000000000000000000000	0000	0-3000	000E+0	300E+0	000E+0	00E+0	0005+0	0+3000	○ +Ⅲ000	000E+0	10000	0000	0+3000	300E+0	000E+0	000E+0	000E+0	0+3000	0+3000	049000	0.000.000		0+3000	200E+0	000E+0	000E+0	000E+0	000E+0	1000	0
SVB	(Ω Ε)	0.111901100	3725+0	1256+0	,304E+0	*101E*0	,455E+0	1965+0	.242E+0	5//E+0	247E+0	476E+0	332E+0	,292E+0	425E+0	120E+0	2112011	14.00	1686+0	.387E+0	.114E+0	,271E+0	1005 + 0	3625	1448+0	.750E+0	713E+0	3445	045404	.121E+0	1965+0	.765E+0	. 529E+0	.138E•0	1000	4 4 C C
SGR	CH3)	0.000	0000	.000E+0	.000E+0	00000	0.3000	-000E+0	0+3000-	-000E+0	00000	0000	0000	.000E+0	000E+0	0000	240000		000E+0	0.000	0.000	0000	000E+0	000E+0	-3000-	-000E+0	0 + 30 0 0 + 0 0 0 + 0 0 0 0 0 0 0 0 0 0		.000E+0	0.006+0	-3000°	-9000-	-000E+C	.000E+C	10000	
Ü	~.		٠ د د	0.0	φ.	10	ιή H	0.7	φ.	O	* *	0	0	4	ά ·	* «	† r) M	ń	†	4	0	0 0 4 m	M	M	M 1	Α C	, _K	0		0.2	د	٥.	0	4	,
800	က် ည	0.1248+08	127E+0	130E+0	131E+0	1 355 F + C	134E+0	1345+0	1376+0	1366+0	045091	1435+0	1468+0	1475+0	1516+0	190E+0	O - 0 / C 7 4	0+604 1446 1446	1665+0	166E+0	166E+0	1505.	100E+0	100E+0	166F+0	.166E+0	206E+0	2046	203F+0	201E+0	201F+0	1995+0	\$30E+0	1976+0	197E+0	L WC
Sas	(M3)	0 - 958/F + 00 0 - 918/F + 00	3115+0	. 560E-0	.363E+0	. W. C.	1956+0	.872E+0	.739E+0	. 257E+0	. 0 1 HE + 0	.866E-0	0+3789	+523E+0	7576+0	041.60	7000	10000	9 4	594E+0	•056E+0	.507E+0	VXIE+0	40.00	414E-0	.455E+C	6728-0	A 5 2 5 4 5	3215	2545+(-369C+	.3205+	.354E+(.2788+1	******************	
a a		0.020	024	.029	600	200	810	610.	050	2	C 4	9	5	0.17		0 4	73	5 8	9	80	ç	g:	0.000	8	0.0083	6	57))) (0000	000	00	é	3	S	5	4
5.5		75.00	.020A	0.0330	2000	7670	.0193	1.0164	0.214	N + 10 + 0	0.00	0239	0227	1.0167	0.136	70.0	7770	466	0.0107	2800.0	0.0131	7,000	0.0105	2600.0	0.0063	0.0000	00000	0000	0.0045	0.0056	0.0076	7900 0	0.0076	H\$00.0	0.0000	0000
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AAA K.MUJUR SEPIRETT RINDER ANALYSIS (1740) AAA

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<u></u>	(EL.M) 25%.110 24%.730	237:520	250.000	191.070	178.080	174.270	167.760	047.54	21011	130.280	124.620	040 all	110.740	96.860	81.250	74.220	65.040	62.750	60.030	54.040	53.760	40.000	46.120	42.230	500 CA	37.070	35.250	35.00	24.060	37.000	22,490	024. AS	14.550	15.440	12.840	2,400	1.180	3.400	1.520
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*** K.MUUR SEDIMENT RINDFF ANALYSIS (1/70) ***

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* *************************************	ENG NO.	34.35	4 27	9.040	1004	1008	74RE	179E+0	•	.000E+0	136640	000F+	ç
	ENG NO O	63,31	4.23	050.6	0888	0.635	14.7	.191E+0		.0.00E+0	1235+0	-3000	ô
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· • • • •	NO.193+5	30.	98.75	.7AB	.0333	0369	5366+0	.1385+0		000E+0	.493E+0	.000E+0	
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