The salient features of the conveyance sewer are shown in Table 5.5. The sewer alignment is shown in Fig. 5.6, and its longitudinal profile is shown in Fig. 5.7.

5.3 Treatment Plant

The aerated lagoon treatment plant will be constructed at Pluit Pond. The pond will be used for a multipurpose of flood control and wastewater treatment. The treatment plant will treat wastewater of the integrated area of the Project Area and JSSP Area by 2000. Design influent wastewater to the treatment plant is determined at 441,000 m³/d for the target year 2000 and 529,000 m³/d for the year 2010. This design wastewater discharge includes wastewater of the JSSP Area of 124,800 m³/d for 2000 and 136,000 m³/d for 2010 respectively. Design water quality of influent and effluent wastewater is 200 mg/l and 30 mg/l as BOD respectively. Design water quality of influent and effluent of wastewater is 200 mg/l and 30 mg/l as BOD respectively.

The proposed treatment plant includes inflow pump station, aerated lagoon, facultative pond, disinfection facilities and drying bed. Among them, disinfection facilities will not be installed until 2000.

(1) Inflow Pump Station

A pump station of 454 m³/min. capacity will be installed by the year 2000. Additional pumps with a total capacity of 98 m³/min. will further be provided by 2010. The design pump head is 20m. The salient features of the pump station are shown in Table 5.6. Layout of the pump station is shown in Fig.5.8(1)-5.8(4).

(2) Aerated Lagoon and Facultative Pond

The wastewater will be treated initially by aerated lagoon with a storage capacity of 1,075,000m3 and a surface area of 21.5 ha, and finally with facultative pond. For aeration, 24 units of aerator will be installed by 2000, which would be increased to 59 units by 2010.

The aerated lagoon is designed in such a manner that the existing flood control function of the Pluit Pond remain unaffected. Design water level and other structure level of the aerated lagoon are as follows.

(Unit: P.P.m)

| High water level | : | 0.90 |
|--|---|-------|
| Normal water level | : | -1.00 |
| Aerated lagoon operation level | : | -1.00 |
| Low water level | : | -1.90 |
| Crown elevation of aerated lagoon embankment | : | +1.50 |
| Elevation of acrated lagoon weir | : | -1.90 |

The construction of aerated lagoon includes embankment of 1,600m in length and dredging of 340,000m³.

The effluent of aerated lagoon will finally be treated by the facultative pond with a storage capacity of 2,096,000m³ having a surface area of 52.4 ha.

The salient features of the aerated lagoon are shown in Table 5.6. Layout of the aerated lagoon is shown in Fig. 5.9. Flow-diagram of the treatment plant is shown in Fig. 5.10. Design water level of the treatment plant and pump station is shown in Fig. 5.11.

Table 5.1 Numbers of House Connection by Sub-Zone

| | Total | 25,200 | 12,600 | 4,500 | 009'6 | 41,100 | 11,600 | 23,400 | 128,000 |
|------|----------|--------|--------|-------|-------|--------|--------|--------|----------|
| 2010 | Others | 7,000 | 3,000 | 400 | 1,700 | 11,100 | 2,200 | 009 | . 26,000 |
| | Domestic | 18,200 | 009'6 | 4,100 | 7,900 | 30,000 | 9,400 | 22,800 | 102,000 |
| | Total | 21,000 | 11,200 | 3,400 | 8,800 | 37,800 | 10,200 | 22,600 | 115,000 |
| 2000 | Others | 4,500 | 2,000 | 300 | 1,300 | 8,700 | 1,600 | 009 | 19,000 |
| | Domestic | 16,500 | 9,200 | 3,100 | 7,500 | 29,100 | 8,600 | 22,000 | 000'96 |
| Sub | Zone | ¥. | Ø | O | Q | 禸 | ŢĿ | ტ | Total |

Table 5.2 Tertiary and Secondary Sewers in Sewerage Area

| | Area (ha) | Earth | | Sewer Lin | Sewer Line Length | Upper: (m) | (m) (m/ha) | Nos. of Manhole |
|-------------------|-----------|-----------------|----------|-----------|-------------------|------------|---------------|-----------------|
| | () | Depth (m) | ø 150 mm | ø 200 mm | ø 250 mm | ø 300 mm | Total | Lower:(Unit/ha) |
| | | H<20m | 3 605 | 4315 | C | C | 7.920 | 901 |
| Conventional | | | (35.4) | (42.4) | (0.0) | (0.0) | (77.8) | (1.1) |
| Area | | 2.0 < H < 4.0 m | 0 | 955 | 1,715 | 755 | 3,425 | 48 |
| (Type = A) | 101.8 | | (0.0) | (6.4) | (16.8) | (7.4) | (33.6) | (0.5) |
| Kel. Gondang Dia) | | | 3,605 | 5,270 | 1,715 | 755 | 11,345 | 157 |
| | | Total | (35.4) | (51.8) | (16.8) | (7.4) | (111.4) | (1.5) |
| | | | 0.050 | 2 075 | ¥ | C | 12 980 | 010 |
| A S. C. | | III 0:7 / 11 | 5,7,7 | 5 6 6 | 3 6 | 9 6 | (120.4) | 2 6 |
| Arca | | | (100.0) | (27.2) | (0.3) | (0.0) | (130.4) | (7.7) |
| (Type B) | 99.5 | 2.0 < H < 4.0 m | 0 | 1,220 | 1,700 | 1,225 | 4,145 | 70 |
| Kwl. Cideng | | | (0.0) | (12.3) | (17.1) | (12.3) | (41.7) | (0.7) |
| | | | 9,950 | 4,195 | 1,755 | 1,225 | 17,125 | 289 |
| | | Total | (100.0) | (42.2) | (17.6) | (12.3) | (172.1) | (2.9) |
| Tutercentor | | H < 2.0 m | • | • | 1 970 | 840 | 2.810 | 44 |
| Area | 73.4 | | | | (26.8) | (11.4) | (38.2) | (0.6) |
| Kel. Duri Utara | | 2.0 < H < 4.0 m | , | ı | 80 | 1,325 | 1,405 | 22 |
| Kel. Duri Selatan | | | | | (1.1) | (18.1) | (19.2) | (0.3) |
| | | | | , | 2,050 | 2,165 | 4,215 | 99 |
| | | Total | | | (27.9) | (29.5) | (57.4) | (6.0) |

Table 5.3 Proposed Collection Sewer by Sub-Zone

| | | | | | | | |) | unit: m) |
|----------|--------------------|--------|--------|--------|--------|---------|--------|----------|----------|
| | Sub-zone | ¥ | В | C | Ω | ш | Ţ, | Ð | Total |
| | Sewer Size | | | | | | | New York | |
| | (mm) | | | | | | | | . |
| | 150 | 31,400 | 12,400 | 19,900 | 20,500 | 61,200 | 14,900 | 43,800 | 204,100 |
| | 200 | 27,200 | 5,200 | 8,400 | 8,700 | 25,800 | 6,300 | 18,500 | 100,100 |
| | 250 | 15,000 | 5,700 | 3,900 | 7,100 | 35,400 | 6,300 | 10,200 | 83,600 |
| | 300 | 10,800 | 5,200 | 2,800 | 6,200 | 33,500 | 5,700 | 8,000 | 72,200 |
| | Secondary/Tertiary | 84,400 | 28,500 | 35,000 | 42,500 | 155,900 | 33,200 | 80,500 | 450,000 |
| | 350 | 2,105 | 275 | 1,390 | 735 | 875 | 315 | | 5,695 |
| | 400 | 1,385 | 710 | 450 | 1,120 | 2,915 | 550 | 1,490 | 9,070 |
| | 450 | 086 | 1,200 | | 635 | 5,930 | 805 | 800 | 10,350 |
| | 500 | 1,360 | 360 | | 460 | 3,320 | 490 | 1,370 | 7,360 |
| | 009 | 2,430 | 1,060 | -13 | 485 | 4,020 | 2,445 | 920 | 11,360 |
| | 700 | 2,535 | 1,080 | | 1,245 | 1,605 | 860 | 1,620 | 8,945 |
| <u>.</u> | 800 | 1,085 | 250 | 069 | 1,470 | 2,630 | 360 | 069 | 7,175 |
| | Main | 12,330 | 4,935 | 2,530 | 6,150 | 21,295 | 5,825 | 6,890 | 59,955 |
| | 006 | 1,995 | 35 | 1,285 | 1,150 | 2,545 | 85 | 1,420 | 8,515 |
| | 1000 | | | | 20 | 2,015 | | 955 | 2,990 |
| | 1100 | | | | | 1,525 | | | 1,525 |
| | 1200 | | ., | | | 120 | | 1,090 | 1,210 |
| | 1350 | | | | | 2,780 | | 150 | 2,930 |
| | 1500 | | | | | 120 | | | 120 |
| | Trunk | 1,995 | 35 | 1,285 | 1,170 | 9,105 | 85 | 3,615 | 17,290 |
| | Force Main | 500 | | | | | ı | 240 | 740 |
| | | (| i c | 0 | 6 | 6 | 1 | | |
| | Total | 99,225 | 33,470 | 38,815 | 49,820 | 186,300 | 39,110 | 91,245 | 537,985 |

Table 5.4 Lift Pump Station at Kel. Jelambar Baru

| Year Design Discharge | 2000 1.051 m3/sec | 2010 1.188 m3/sec |
|--------------------------|---|----------------------|
| | 36 m3/min x (1 unit + 1 unit standby 18 m3/min x 2 units | _ |
| | With hydraulic head of 17 | m |

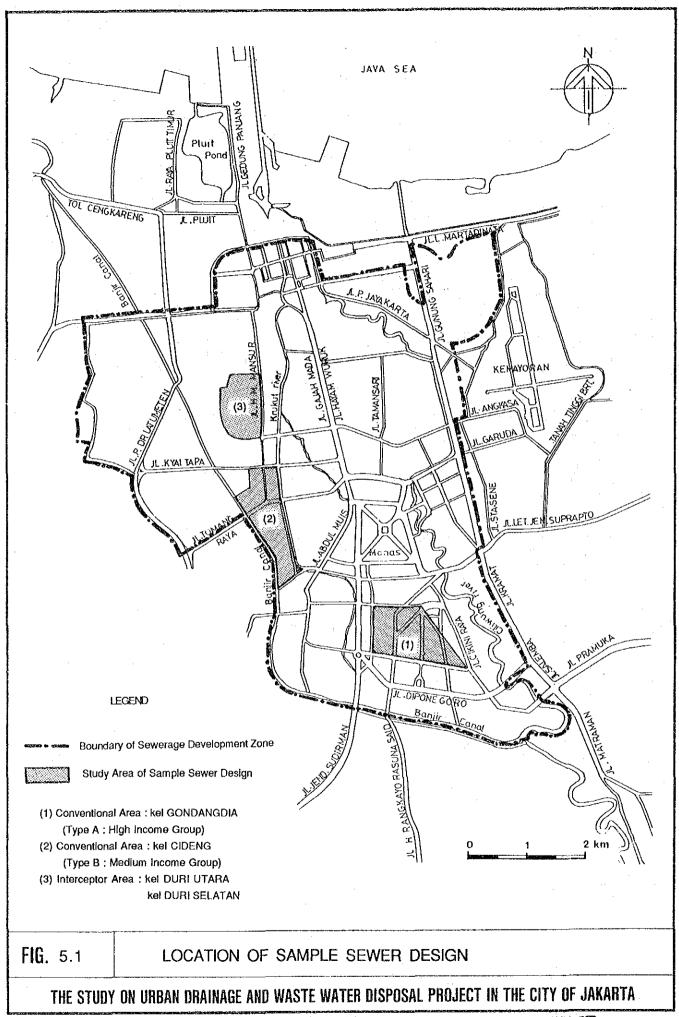
Table 5.5 Proposed Conveyance Sewer

| | | Location | | | | Design |
|------|--------------------------|--------------------------|----------|--------|--------|------------|
| Line | | | Diameter | Length | Slope | Wastewater |
| No. | Origin | End | (w w) | (m) | 1/1000 | (m3/s) |
| S1 | Jl. Madiun | Hotel Indonesia | 1,900 | 1,385 | 1.2 | 3.494 |
| \$2 | Hotel Indonesia | Jl. Kebon Sirih | 2,100 | 1,110 | 1.2 | 4.077 |
| S3 | Jl. Kebon Sirih | Jl. Medan Merdeka Utara | 2,200 | 1,460 | 1.2 | 4.721 |
| S4 | Jl. Medan Merdeka Utara | Jl. Sukarjo Wiryopranoto | 2,300 | 1,300 | 1.2 | 5.357 |
| S5 | Jl. Sukarjo Wiryopranoto | • | 2,400 | 1,110 | 1.2 | 6.299 |
| S6 | JI. Raya Mangga Besar | : | 2,600 | 1,320 | | 7.333 |
| 87 | Ji. Jembatan Bambu Ji | Jl. Kunir | 2,700 | 260 | 1.1 | 8.350 |
| S8 | Ji. Kunir | Jl. Pejagalan | 2,700 | 099 | 1.1 | 8.549 |
| 68 | Jl. Pejagalan | Treatment Plant | 2,900 | 1,435 | 1.0 | 9.695 |
| | | | Total | 10,340 | • | - |

Note: Design wastewater is nourly maximum includung groundwater infiltration.

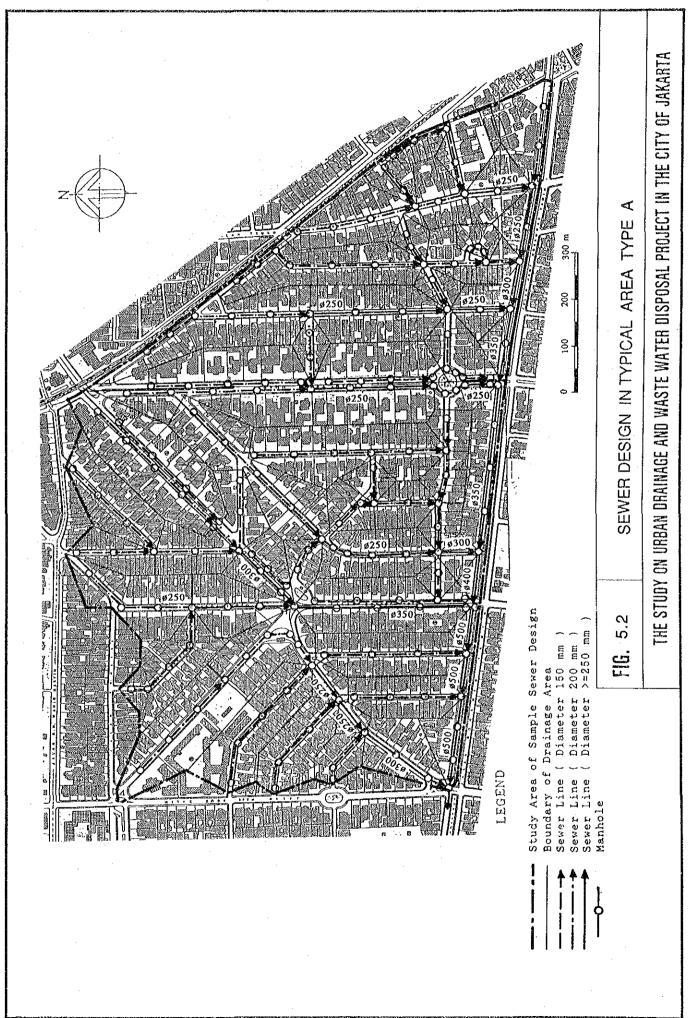
Table 5.6 Treatment Plant at Pluit Pond

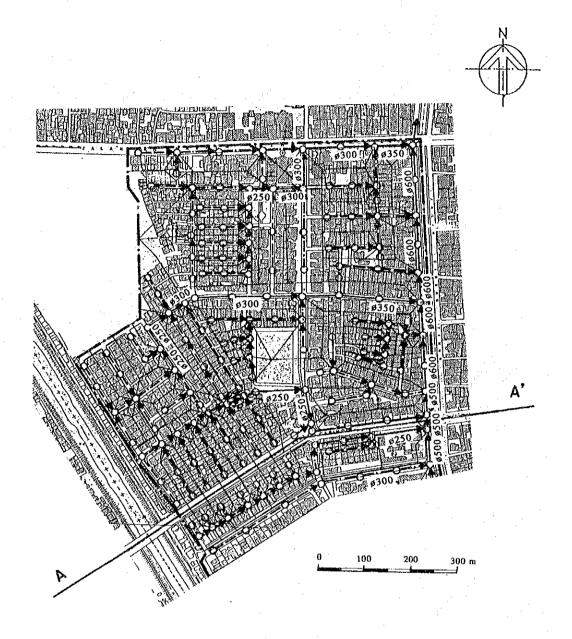
| | Year | 2000 | 2010 |
|----------|-------------------------------|----------------------------|----------------------------|
| (1) | Design Wastewater | 441,000 m3/d | 529,000 m3/d |
| | (including desludge from | (190 m3/d) | (411 m3/d) |
| | on site facilities) | | • |
| | Wastewater Quality in BOD | | |
| | Influent | 200 mg/l | 200 mg/l |
| ļ | Effluent | 30 mg/l | 30 mg/l |
| (2) | Inflow Pump Station | | |
| | Space | 2 Station | 2 Station |
| | • | 21m x 37m | 21m x 37m |
| | Inflow pump | | |
| | | ø 900 x 98 m3/min x 20m(H) | ø 900 x 98 m3/min x 20m(H) |
| | | 3 units + 1 unit standby | 4 units + 2 units standby |
| | | ø 600 x 40 m3/min x 20m(H) | ø 600 x 40 m3/min x 20m(H) |
| ļ | | 4 units | 4 units |
| (3) | Aerated Lagoon | Arca; 21.5 ha, Capa | acity; 1,075,000 m3 |
| | Excavation | 340,000 m3 | |
| | Embankment | 1, | 600 m |
| \ | Operational water level | P.P | r, -1.00 m |
| | Bottom elevation | • | 6.00 m |
| | Crown elevation of embankment | | . +1.50 m |
| | Elevation of weir | | 1.90 m |
| | Aerator | 24 units x 75 kw | 59 units x 75 kw |
| 1 | Retention time | 1 day | 2 days |
| | | for aerated lagoon and 1.4 | |
| | | days for Faculltative pond | |
| (4) | Facultative pond | | acity; 2,096,000 m3 |
| İ | Operational water level | P.P. | -1.00 m |
| | Effective water depth | | 4 m |
| } | Retention time | 5 days | 4 days |
| | | for maturation pond | |
| (5) | Disinfection Building | | 660 m2 |
| (6) | Chlorine Injection | | 4 units + 1 unit standby |
| 1 | · | | 0~3.8 1 / min / unit |
| (7) | Drying Bed | 2,000 m2 | 6,000 m2 |



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A.D 1991





LEGEND

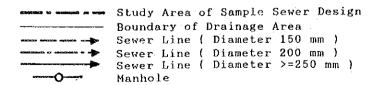


FIG. 5.3(1)

SEWER DESIGN IN TYPICAL AREA TYPE B (1)

THE STUDY ON URBAN DRAINAGE AND WASTE WATER DISPOSAL PROJECT IN THE CITY OF JAKARTA

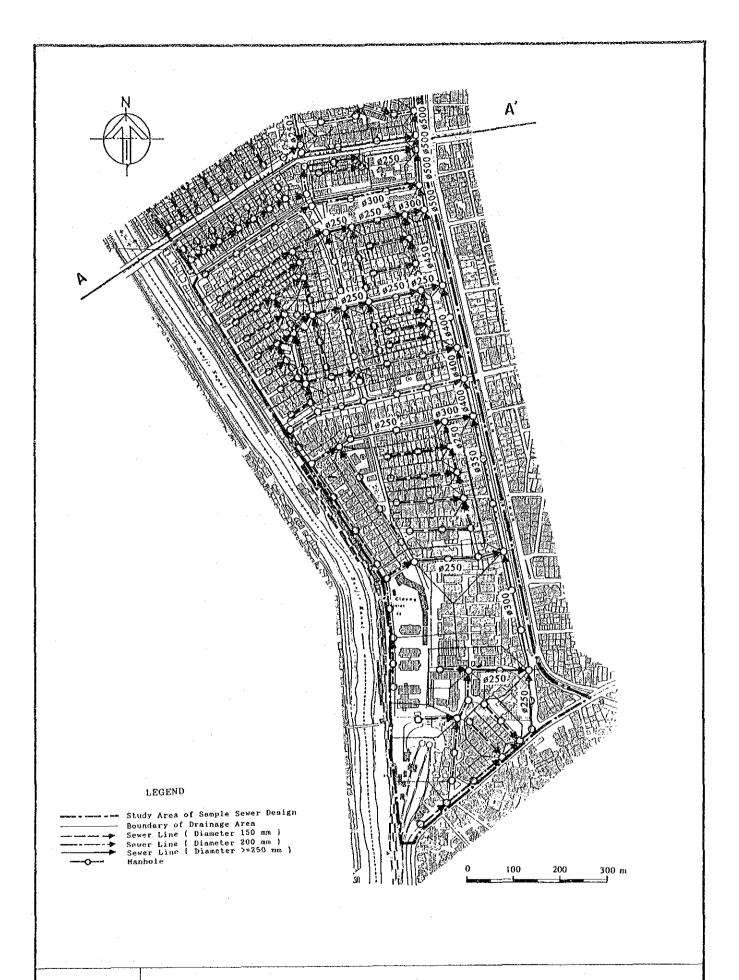


FIG. 5.3(2)

SEWER DESIGN IN TYPICAL AREA TYPE B (2)

THE STUDY ON URBAN DRAINAGE AND WASTE WATER DISPOSAL PROJECT IN THE CITY OF JAKARTA





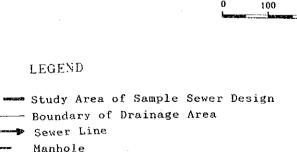
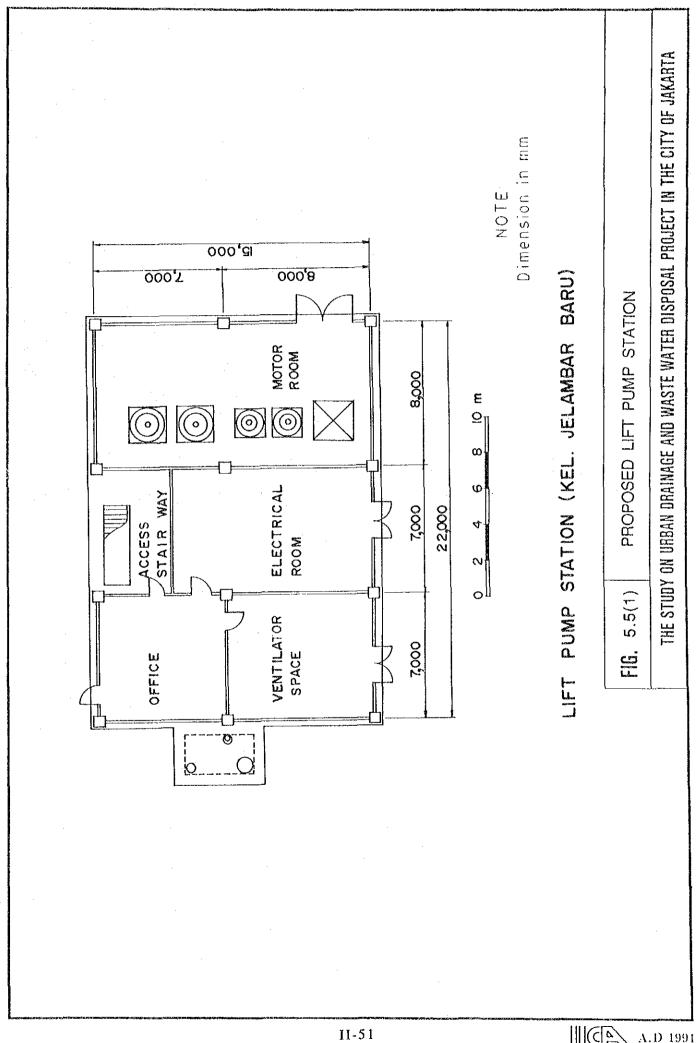
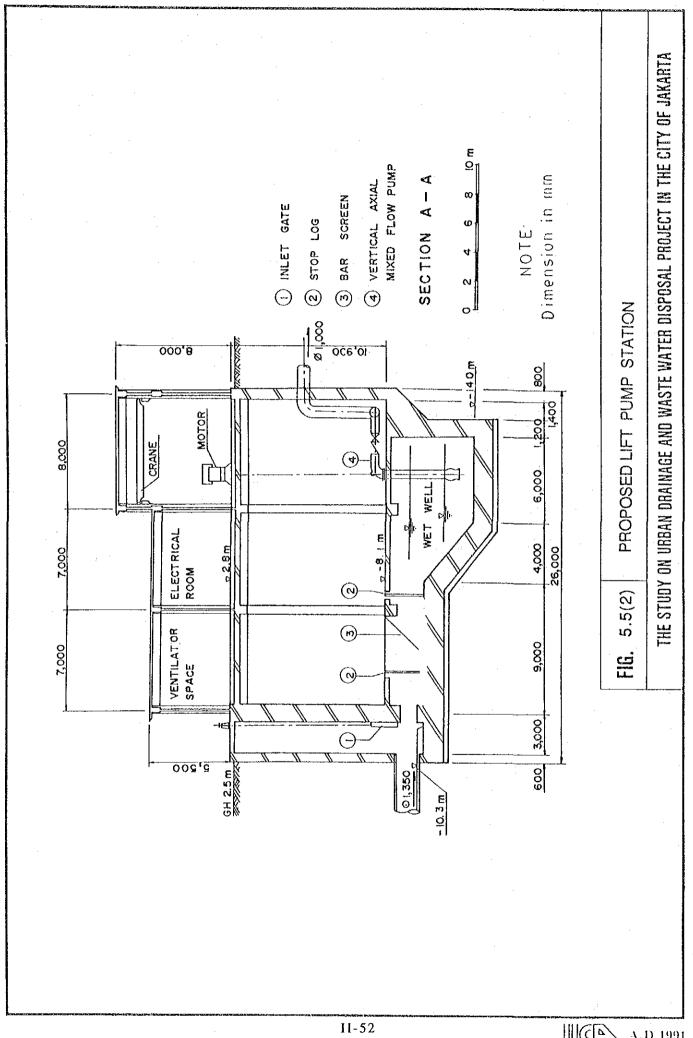


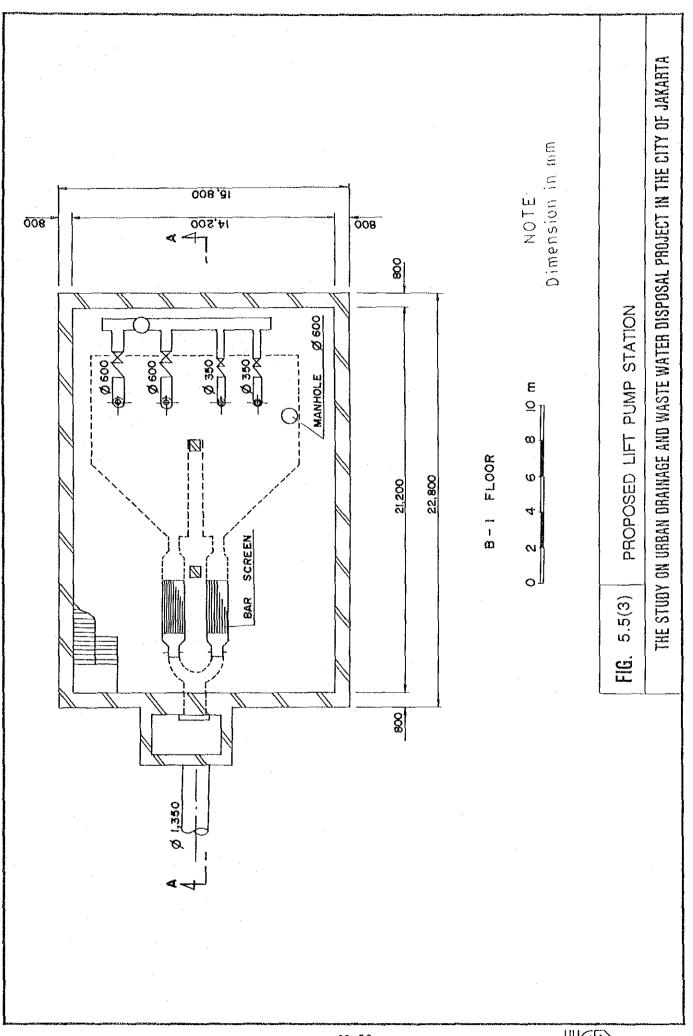
FIG. 5.4

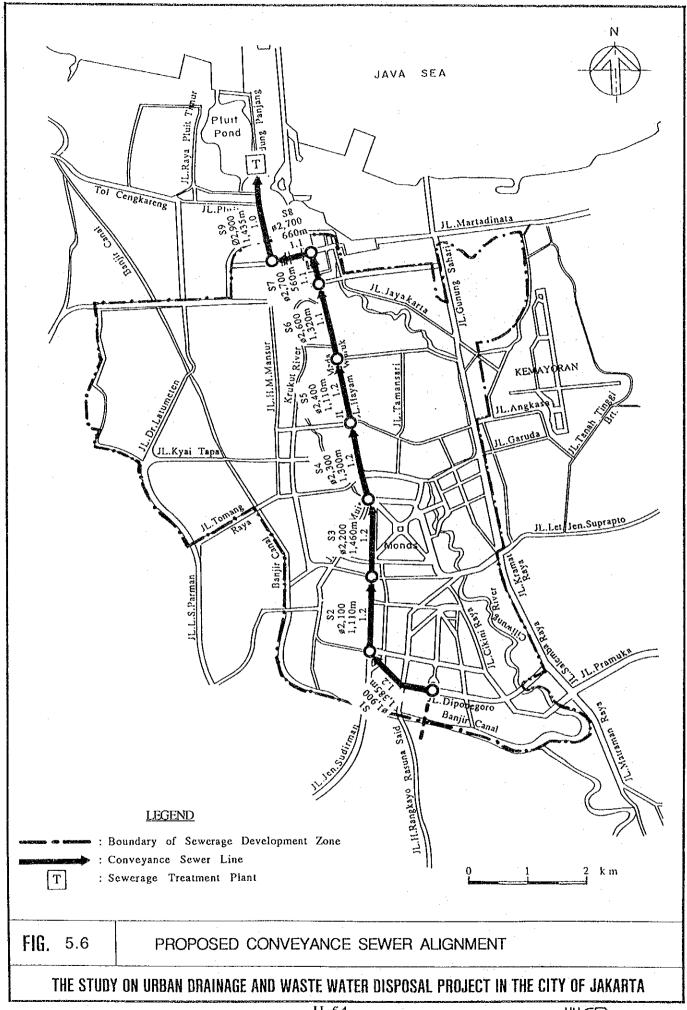
SEWER DESIGN IN TYPICAL AREA TYPE (C)

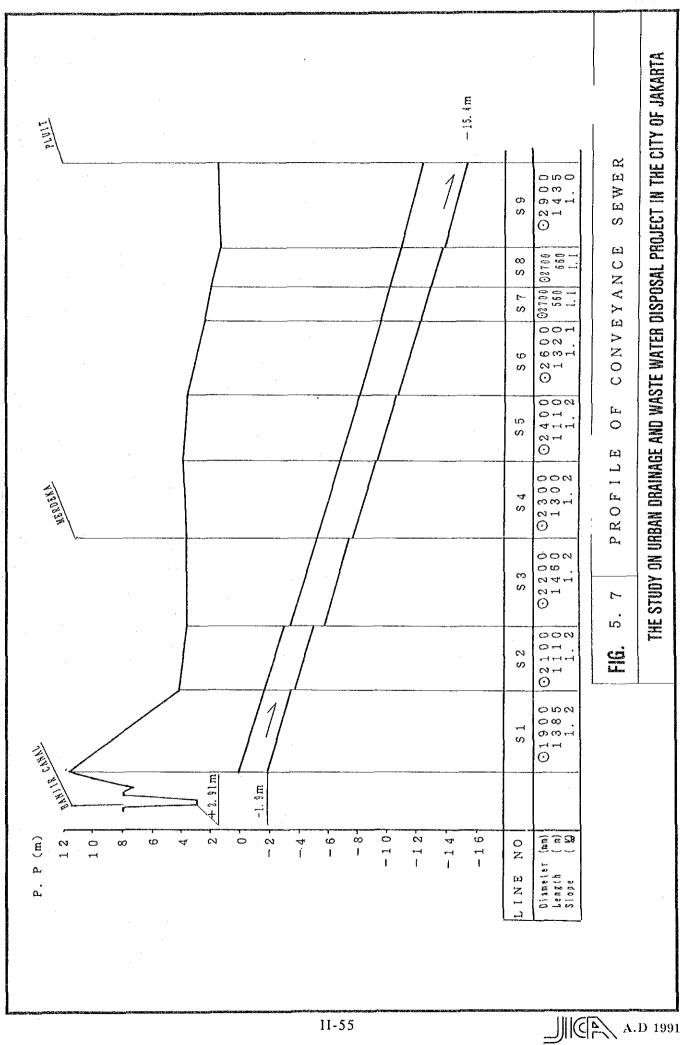
THE STUDY ON URBAN DRAINAGE AND WASTE WATER DISPOSAL PROJECT IN THE CITY OF JAKARTA

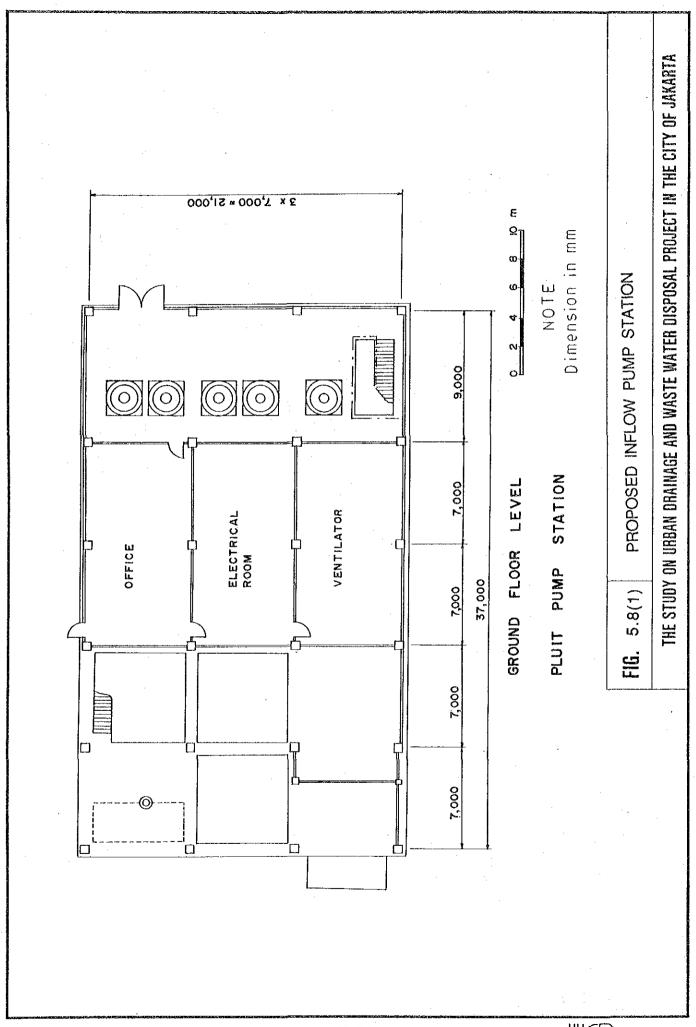


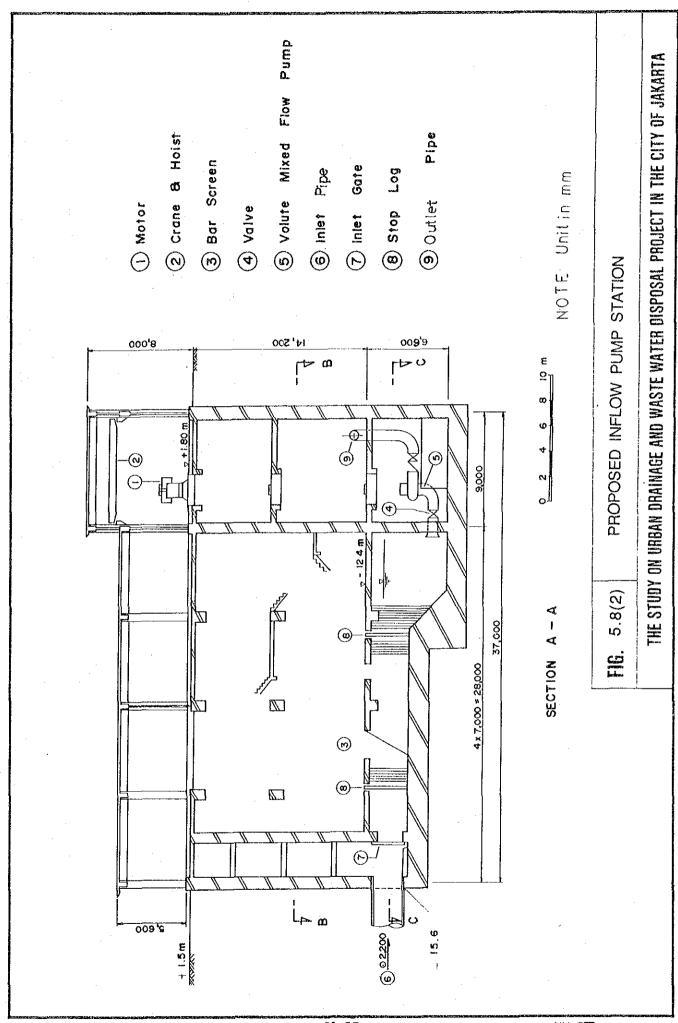


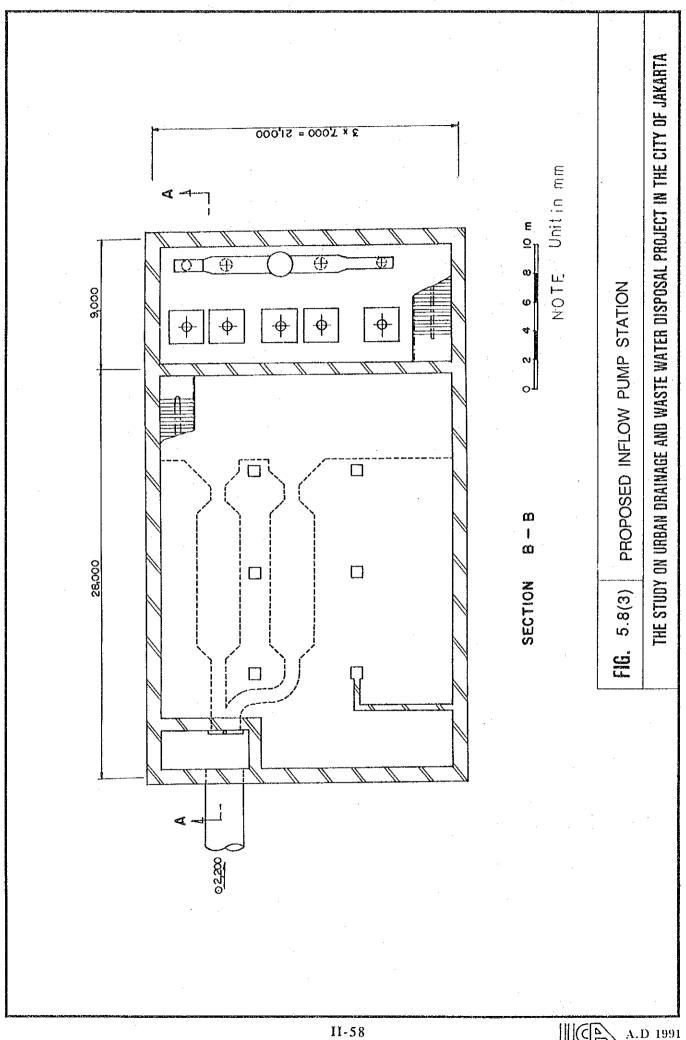


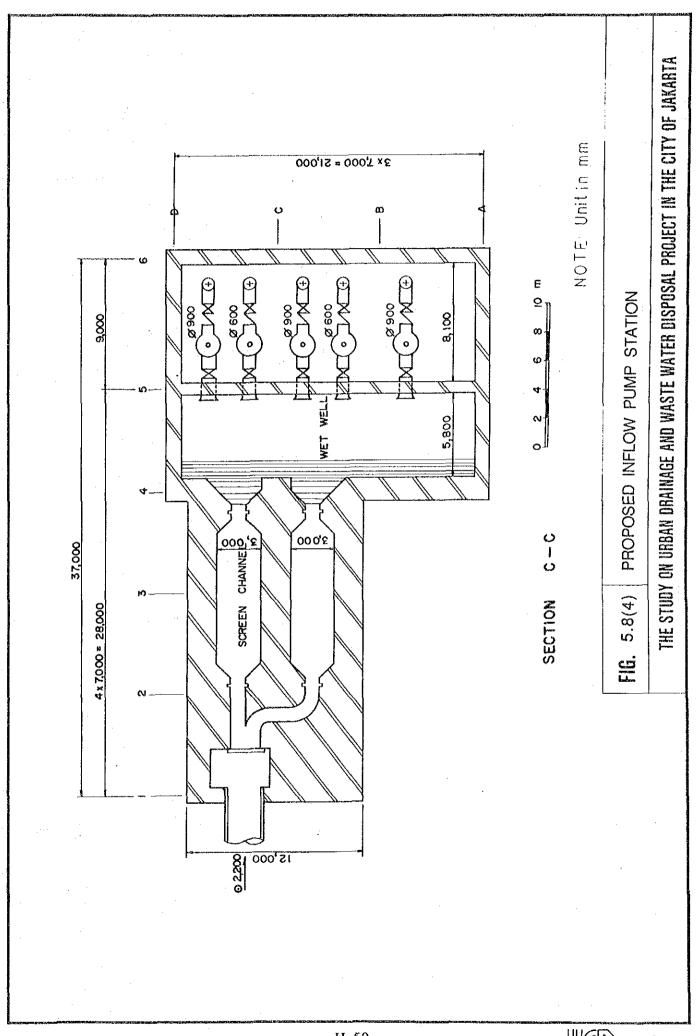


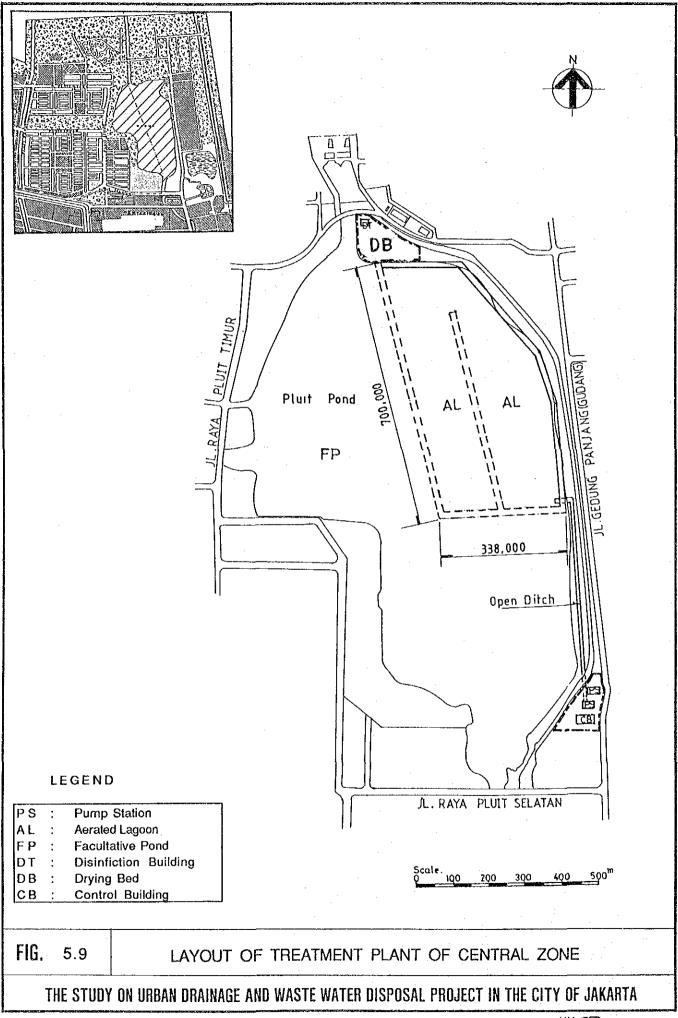


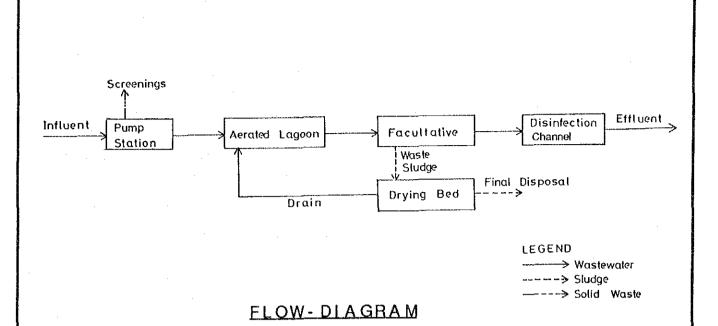












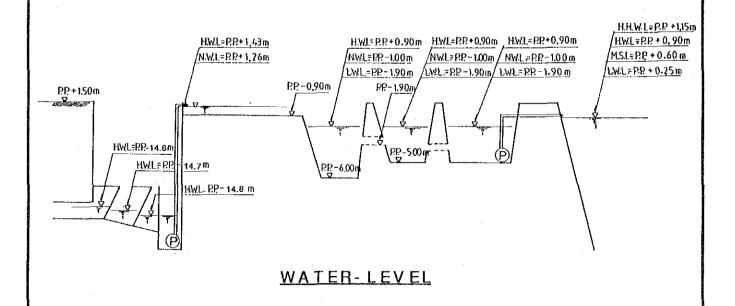
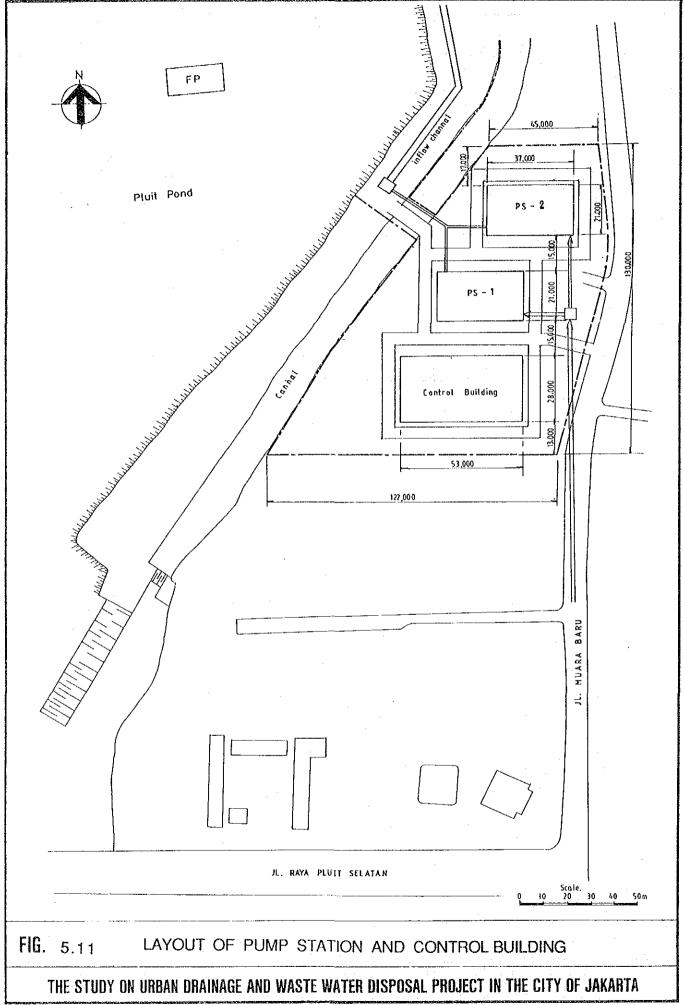
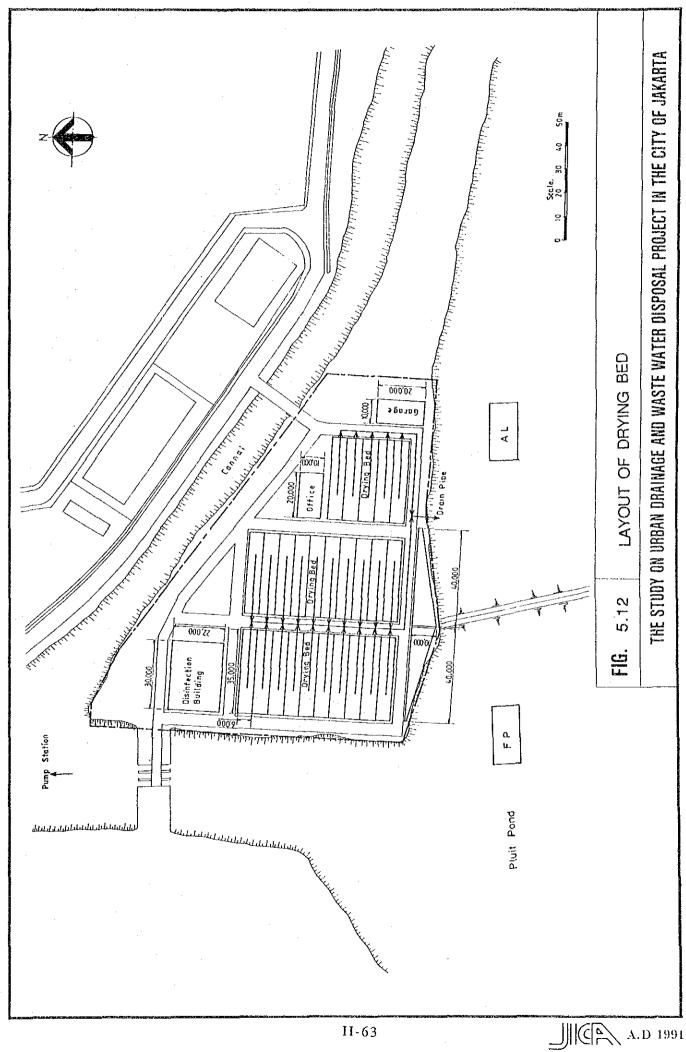
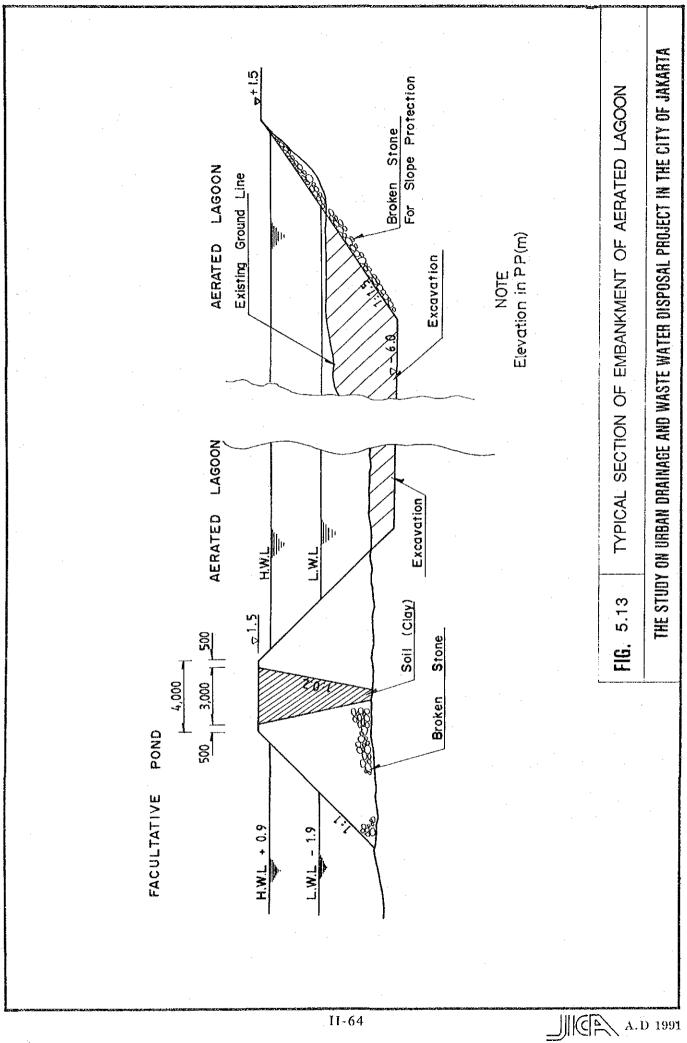


FIG. 5.10 FLOW-DIAGRAM AND WATER-LEVEL OF PLUIT TREATMENT PLANT
THE STUDY ON URBAN DRAINAGE AND WASTE WATER DISPOSAL PROJECT IN THE CITY OF JAKARTA







Chapter 6 PROJECT AND COST ESTIMATE

6.1 Construction Plan

6.1.1 Geology and Topography

The Project Area is in the Jakarta plain, and its geological condition is primarily deltaic. Most of the area is covered by either alluvium or young rocks. The alluvium soils spread mostly along the rivers while the young volcanic rocks cover the rest of the Project Area.

The ground surface in the northern part is almost flat with a low elevation, that declines toward north with a slope in the range of $0.2 \,\mathrm{m} \sim 0.3 \,\mathrm{m}$ per thousand meter. The level of groundwater table is high, especially in the northern coastal area. While in the southern part of the Project Area, the ground slope is rather steeper with a surface slope of one (1) to two (2) meters per thousand meters.

In the Project Area, geological survey at Pluit Pond and along Krukut River and Abdul Mus Rd. were conducted by local consultants in the year 1986 and 1987 respectively.

At the estuary of Pluit Pond, the soil condition of the topsoil between ground surface of P.P. + 1.50 m and at level of P.P. - 5.50 m is sandy silt with N-value of zero (0). The subsoil strata between P.P. - 5.50 m and P.P. - 16.5 m is predominantly clay with some gravels and silty clay having an average N-value of five (5). At depths deeper than P.P. - 16.5 m, the strata is of very hard silty clay with N-value of more than 50. This layer is considered as the bearing stratum for structures.

The geologic conditions along the proposed conveyance sewer are as follows.

The uppermost layer of 0.5 to 1.5 meters thickness has a variety of soils; organic humus, silty sand, clayey silt, sandy silt and sandy clay. The soil consistency varies from very soft to soft.

- The thickness of subsoil layer at the southern part of the Project Area ranges from nine (9) to 13 meters. However, it increases to more than 30 meters between Kh. Hasyin Asyhari Rd. and the southern edge of the Pluit Pond.
- The subsoil strata consists of silty clay, silty sand, organic clay, sandy clay, sandy silt and tuffaceous silt. Consistency of subsoil is soft with N-value of seven (7) in an average.
- Bearing strata at the southern part consists of tuff, tuffaceous silt and tuffaceous sand. N-value varies from 60 to more than 100.

Location of soil survey conducted in 1986 and 1987 and geological profile are shown in Figs. 6.1 and 6.2.

6.1.2 Construction Method

(1) Sewer Pipe Installation

Open trench method is adopted for installation of secondary & tertiary, main and trunk sewers in principle. All the secondary & tertiary sewers of 460 km is installed by open trench method. The portions of main and trunk sewers with a total length of 2,650 m those cross rivers, main roads and railways at 47 locations will be constructed by micro tunnelling method. The remaining 74.6 km of main and trunk sewers be constructed by open trench method. At two (2) locations of crossing Bangir Canal and Credang River of Pangeran Tubagus Angke Rd., pipe beam bridges with length of 50 m and 30 m respectively are applied. Conveyance sewers of 10.34 km in length with diameter ranging from 1,900 mm to 2,900 mm are constructed by shield tunnelling method.

(2) Treatment Plant

Proposed treatment plant of aerated lagoon is constructed at Pluit Pond. The baffle and partition wall embankments of aerated lagoon and facultative pond, inside the Pluit Pond, will be constructed with broken stones and clay core. The underwater excavation works of aerated lagoon will be conducted after the completion of embankments.

6.1.3 Required Major Construction Equipment

Major construction works of sewerage development is installation of sewer pipes which require earth works. Closed face type mechanical shield tunnelling machine is required for conveyance sewer construction. While for the secondary & tertiary, main and trunk sewers, heavy equipment such as backhoe, vibro hammer, and truck crane are required for trench digging, setting and removing of sheet piles and pipe installation.

Dragline with boat or dredges will also be required for dredging of aerated lagoon.

6.1.4 Construction Schedule

(1) Workable Days

Annual workable days is estimated to be 240 days based on the following considerations:

Sunday per annum : 12 months x 4 days = 48 days

National holiday per annum: about 20 days

Rainy day per annum : 57 days (more than 10 mm/day

rainfall)

Total work suspension

days per annum : 125 days

(2) Work Time

Sewer installation works by open trench method along main roads be conducted during night time only. Trench is covered by steel deck in day-time for traffic use. Construction of conveyance sewers by shield tunnelling method is conducted all day with three (3) shifts, each of eight (8) working hours in order to ensure a continued work pace.

Construction works of pump station and treatment plant is conducted for eight (8) hours during day time only.

6.2 Project Cost Estimate

6.2.1 Basis of Cost Estimate

Based on facility plans, the project costs are estimated under the following conditions.

- (1) It is assumed that all construction works will be contracted to general contractors by international tender.
- (2) All base costs are expressed under the economic conditions that prevailed in August, 1990.
- (3) Overhead is assumed at 20% of the total cost of equipment and civil works and incorporated in the direct construction cost.
- (4) Engineering service and administration costs are assumed respectively at 7% and 1.5% of the total direct construction cost.
- (5) Physical contingency allowance at 10% of the direct construction cost is assumed.

6.2.2 Estimated Project Cost

The total project cost, consisting of direct construction cost, land acquisition cost, administration cost, engineering cost and physical contingency, amounts to Rp. 445.3 billion at 1990 price. Its breakdown is shwon in Table $6.1 \sim 6.16$.

6.2.3 Estimated Operation and Maintenance Cost

The annual operation and maintenance cost for the Project Area in 2000, consisting of sewer maintenance, O&M of lift pump station and treatment plant, is estimated at Rp. 3.6 billion at 1990 price. The annual O&M cost for the Central Sewerage Zone covering JSSP Area is estimated to be Rp. 7.0 billion at the year 2010. The cost breakdown is shown in Table 6.17 and Table 6.18.

Table 6.1 Project Cost of Sewerage Development

| (Uni | Unit: billion Rp.) |
|----------------------------|--------------------|
| A. Direct Costruction Cost | 375.3 |
| (1) Collection Sewer Line | 334.8 |
| (2) Lift pump Station | 4.1 |
| (3) Treatment Plant | 36.4 |
| B. Land Acquisition cost | 9.0 |
| C. Administration Cost | 5.6 |
| D. Engineering Cost | 26.3 |
| E. Physical Contingency | 37.5 |
| Total | 445.3 |
| | |

Table 6.2 Breakdown of Direct Construction Cost

| (Unit : billion Rp.) | Plant Total | 84.1 | 71.6 | 62.1 | 117.0 | 334.8 | 4:1 | | 22.5 | 13.9 | 36.4 36.4 | 36.4 375.3 |
|----------------------|-------------|--------------------------------|-----------------------|--------------------------|------------------------------|--------------------------------|----------------------|--------------------|--------------|-----------------------|-----------|------------|
| F | S | 14.0 | 10.6 | 16.6 | | 41.2 | 4.1 | | | : | | 45.3 |
| | H | 6.1 | 5.8 | 0.3 | | 12.2 | | | | | | 12.2 |
| | E | 29.7 | 22.7 | 31.7 | 62.4 | 146.5 | | - | | <u>.</u> | | 146.5 |
| Sub-2000 | Ω | 7.6 | 6.6 | 5.0 | 14.3 | 36.8 | | | | | | 3 4 8 |
| 0. | C | 6.0 | 2.9 | 3.9 | 15.5 | 28.3 | | | | | | 283 |
| | B | 5.3 | 4.9 | 0.1 | 11,4 | 23.2 | | | | | | 23.2 |
| | A | 15.4 | 13.3 | 2.4 | 13.4 | 46.6 | | | | a | | 46.6 |
| | Itme | A. Sewer Secondary/Tertiary | (ф150 ~ ф350) Main | (\$350 ~ \$800) Trunk | (4900 ~ 41500) Conveyance | (\$1900 ~ \$2900) Sub-Total | B. Lift Pump Station | C. Treatment Plant | Pump Station | Aerated lagoon/others | Sub-Total | Total |

Table 6.3 Construction Cost of Secondary/Tertiary Sewer

| | | | Conventional Area | Area | | Interceptor Area | rea | Construction |
|----------|----------|-------------|-------------------|---------------|-------|------------------|---------------|---------------|
| Sub-Zone | Area | | Unit Cost | Construction | Area | Unit Cost | Construction | Cost |
| | (ha) | | (million | Cost | (ha) | (million | Cost | (million Rp.) |
| | | | Rp./ha) | (million Rp.) | | Rp./ha) | (million Rp.) | |
| ¥ | (Type A) | 377 | 20.0 | 7,540 | | | | |
| | (Type B) | 181 | 29.1 | 5,267 | 196 | 13.5 | 2,646 | 15,453 |
| В | (Type B) | 124 | 29.1 | 3,608 | 124 | 13.5 | 1.674 | 5,282 |
| U | (Type B) | 199 | 29.1 | 5,791 | 13 | 13.5 | 175 | 5,966 |
| Q | (Type B) | 205 | 29.1 | 5,966 | 126 | 13.5 | 1,701 | 7,667 |
| E | (Type B) | 612 | 29.1 | 17,809 | 881 | 13.5 | 11,894 | 29,703 |
| Ц | (Type B) | 149 | 29.1 | 4,336 | 132 | 13.5 | 1,782 | 6,118 |
| Ð | (Type B) | 438 | 29.1 | 12,746 | 06 | 13.5 | 1,215 | 13,961 |
| Total | | 2.285 | | 63,063 | 1.562 | | 21.087 | 84 150 |

Table 6.4 Construction Cost of Main / Trunk Sewer

| | | | | | (unit: | million Kp.) |
|----------|--------|---------------------|--------|--------|------------------------|--------------|
| Sub-Zone | Main | Main (ø 350~800 mm | (m | Trunk | Trunk (ø 900~1,500 mm | mm) |
| - | Sewer | Manholc | Total | Sewer | Manhole | Total |
| | 12,750 | 521 | 13,271 | 4,452 | 8.3 | 4,535 |
| | 6,179 | 174 | 6,353 | 76 | 5 | 81 |
| | 2,816 | 88 | 2,904 | 3,851 | 67 | 3,918 |
| | 9,620 | 291 | 9,911 | 4,872 | 78 | 4,950 |
| | 21,967 | 796 | 22,763 | 31,313 | 432 | 31,745 |
| | 5,515 | 284 | 5,799 | 255 | 9 | 261 |
| | 10,318 | 308 | 10,626 | 16,377 | 202 | 16,579 |
| Total | 69,165 | 2,462 | 71,627 | 61,196 | 873 | 62,069 |

Table 6.5 Construction Cost of Conveyance Sewer

| Line No. | Diameter | Length | Unit Cost | Construction Cost |
|----------|----------|--------|-------------------|-------------------|
| ,,, | (mm) | (m) | (million Rp./m) | (billion Rp.) |
| S1 | 1,900 | 1,385 | 6.7 | 13.4 |
| \$2 | 2,100 | 1,110 | 10.3 | 11.4 |
| \$3 | 2,200 | 1,460 | 10.6 | 15.5 |
| 84 | 2,300 | 1,300 | 11.0 | 14.3 |
| SS | 2,400 | 1,110 | 11.4 | 12.7 |
| . 9S | 2,600 | 1,320 | 11.8 | 15.6 |
| S7 | 2,700 | 260 | 12.3 | 6.9 |
| 88 | 2,700 | 099 | 12.3 | 8.1 |
| 89 | 2,900 | 1,435 | 13.3 | 19.1 |
| Totai | | 10.340 | | 117.0 |

Table 6.6 Construction Cost of Lift Pump Station

| | | | Unit Cost | Construction Cost |
|----|---------------------------|----------|------------------------------------|-------------------|
| | Item | Quantity | (million Rp./m3) (million Rp.) | (million Rp.) |
| | Civil and Architect | | | |
| | Temporary Work | 6,750 m3 | 0.13 | 877 |
| | Concrete Work | 1,780 m3 | 0.35 | 623 |
| 2. | Mechamical and Electrical | | | |
| | Equipment | 1 18 | i | 2,600 |
| ω. | 3. Total | | | 4,100 |

(Unit: million Rp.) Civil/Architect Mechanical/Electrical Unit Cost Quantity Unit Cost Const. Total Item Quantity Const. Cost Cost 1. Pumping St. (per 1 station) Temporary Work 17,280 m3 13,000 2,246 2,030 5,800 m3 35,000 Concrete Work/others 483 m2 30,000 145 Building 6,817 Mechanical/Electrical Work 1 ls Sub Total 4,421 6,817 22,476 $(4,421 + 6,817) \times 2$ Total of 2 Pump Stations 2. Split Manhole Temporary Work 1,716 m3 130,000 223 300,000 Concrete Work 520 156 Sub Total 379 379 3. Aerated Lagoon 340,000 m3 8,800 2,992 Excavation 4,740 120,000 m3 39,500 Broken Stone 18,000 m3 9,100 162 Backfill 194 13,640 m2 14,200 Slope Protection 147 3,528 Aerator (75 kw) 24 set 8,088 3,528 11,616 Sub Total 4. Other Facilities Drying Bed 2,000 m2 12,000 24 3,000 m2 300,000 900 Control House 924 924 Sub Total 5. Others 150,000 m2 2,000 300 Site Preparation 500 500 m 1,000,000 Open Channel 2,000,000 90 m 180 Force Main Sub Total 980 980 19,213 17,162 Total 36,375

Main Sewer Construction Cost by Sub-zone, by Diameter and by Earth Covering Depth Table 6.8 (1)

| Earth Covering | À | Length (m) | | Unit Cost | Con | Construction Co (million Rp.) | Cost |
|-------------------|----------------------|-------------------|-------------------|-------------------------|---------------------|----------------------------------|-------------------|
| | Ą | В | Ü | (million Rp./m) | ¥ | В | Ú |
| | 520 1,325 260 | 275 0 0 | 505 645 240 | E . | 107 400 427 | 57 0 0 | 104 195 394 |
| | 615 950 270 | 450 260 0 | 20 430 0 | 0.224 0.323 1.680 | 138 307 454 | 101 | 139 |
| | 25 500 455 | 625 275 300 | 000 | 0.253 0.354 1.729 | 1777 | 158 97 519 | 000 |
| | 585 710 65 | 155 105 100 | 000 | 0.278 0.382 1.774 | 163 271 115 | 43 40 177 | 000 |
| | \$25 550 1,355 | 1060 | 000 | 0.363 0.473 1.903 | 191 260 2,579 | 2,017 | 000 |
| | 1,300 1,235 0 | 65 645 370 | 000 | 0.851 1.993 2.794 | 1,106 2,461 0 | 1,285 1,034 | 000 |
| | 590 495 | 250 0 | 069 | 0.898 2.045 2.869 | 1,207 1,420 | 511 | 1,980 |
| | 200 | | | 0.349 | 175 | | |
| 1 1 | 12,830 | 4,935 | 2,530 | | 12,750 | 6,179 | 2,816 |

Table 6.8 (2) Main Sewer Construction Cost by Sub-zone, by Diameter and by Earth Covering Depth

| | Ö | 00 | 342 185 | 43 1,003 | 0 160 1,685 | 241 780 0 | 221 957 2,459 | 838 0 1,314 | 10,318 |
|--------------------------|--------------------|------------|-------------------------|-------------------------|-------------------------|-------------------------|---|-------------------------|--------|
| n Cost n Rp.) | [14 | 50 21 | 116 | 285 | 109 | 639 1,913 244 | 306 997 0 | 736 | 5,515 |
| Construction (million | 'n | 30 220 | 45 745 689 | 2,092 | 64 756 1,969 | 662 4,681 434 | 3,199 | 2,975 3,371 0 | 21,967 |
| | Q | 222 | 39 118 974 | 189 | 523 | 66 | 2,083 | 3,299 0 | 9,620 |
| Unit Cost | (million Rp./m) | 0.206 | 0.224 0.323 1.680 | 0.253 0.354 1.729 | 0.278 0.382 1.774 | 0.473 1.903 2.712 | 0.851 1.993 2.794 | 2.045 2.869 4.692 | |
| | Ü | 00 | 320 1,060 110 | 170 50 580 | 0 420 950 | 510 410 0 | 260 480 880 | 410 | 6,890 |
| Length (m) | ĮL, | 245 70 | 190 360 0 | 805 | 205 285 0 | 1,350 1,005 90 | 360 500 0 | 360 | 5,825 |
| Leng | Щ | 145 730 | 2,305 | 5,910 20 | 230 1,980 1,110 | 1,400 2,460 160 | 1,605 | 1,455 | 21,295 |
| | Q | 735 | 175 365 580 | 535 100 | 0 165 295 | 140 345 0 | 1,045 200 | 320 1,150 0 | 6,150 |
| Earth Covering | Depth (m) | 0-2 2-4 | 0-2 2-4 4-6 | 0-2 2-4 4-6 | 0-2 2-4 4-6 | 4-4 4-6 8-8 | 4-4-8-8-8-8-8-8-8-8-8-8-8-8-8-8-8-8-8-8 | 4-6 6-8 8< | Total |
| Diameter | (mm) | 350 | 400 | 450 | 500 | 009 | 700 | 800 | TC |

Trunk Sewer Construction Cost by Sub-zone, by Diameter and by Earth Covering Depth Table 6.9 (1)

| ŭ | 3,851 | | 3,851 |
|--------------------|--------------------------|---|---|
| В | 76 | | 76 |
| Ą | 3,973 | | 4,452 |
| (million Rp./m) | 2.165 | | 1 |
| Ü | 1285 | | 1,285 |
| В | 35 | | 35 |
| Ą | 1,835 | | 1,995 |
| Depth (m) | 4-6 6-8 | | Total |
| (mm) | 006 | | To |
| | Depth (m) A B C Rp./m) A | Depth A B C (million A B C (2.165) A B 6 (4-6 1,835 35 0 1285 2.997 479 0 | Depth A B C (million A B C (Rp./m) A B 6 (1,835 3.973 76 1.885 160 0 1285 2.997 479 0 |

Trunk Sewer Construction Cost by Sub-zone, by Diameter and by Earth Covering Depth Table 6.9 (2)

| | ****** | | | | | | | | | | | | | | |
|-----------------------|--------------------|-----|-----------------|-------|------------|-------|----------|-------|-------|-------|--------------|-------|----------|--|--------|
| | Ð | 671 | 2,008 | 0 | 1,001 | 0 | 0 | 5,748 | O | 0 | 836 | 0 | 816 | | 16,377 |
| n Cost n Rp.) | ţı. | | 255 | 0 | 00 | 0 | 0 | 0 | 0 | 0.0 | 00 | 0 | | | 255 |
| Construction (million | щ | 650 | 3,048 | 0 | 5,144 | 3,304 | 2,620 | 633 | 488 | 3,946 | 1,700 | 471 | | ************************************** | 31,313 |
| | Q | 0 | 3,483 | 0 | 100 | 0 | 0 | 0 | 0 | 00 | 00 | 0 | | | 4,872 |
| Unit Cost | (million Rp./m) | 1 | 4.838 | 2.283 | 3.127 | 3,255 | 5.138 | 5.273 | 1.478 | 2.769 | 5.574 | 3.922 | | | _ |
| | Ů | 310 | 670 440 | 0 | 320 | 0 | 0 | 1,090 | 0 | 00 | 150 | 0 | | (240 m) | 3,615 |
| Length (m) | ĬΤί | 0 | × 0 | 0 | 00 | 0 | 0 | 0 | 0 | 00 | 00 | 0 | <u> </u> | | 85 |
| Len | щ | 300 | 1,615 | 0 | 1,645 | 1,015 | 510 | 120 | 330 | 1,425 | 305 | 120 | | | 9,105 |
| | Q | 0 0 | 720 | 0 | 20 | 0 | 0 | 0 | 0 | 00 | 0 | 0 | | | 1,170 |
| Earth Covering | Depth (m) | 4-6 | × • × • × | 4-6 | %-% %-% | 8-9 | ∨ | % | 2-4 | 4-6 | | 8-9 | 0-2 | | tal |
| Diameter | (mm) | 006 | | 1000 | | 1100 | | 1200 | 1350 | , | Cid-11 to 12 | 1500 | 1000 | (bressured) | Total |

Table 6.10 (1) Manhole Construction Cost by Sub-zone, by Diameter and by Earth Covering Depth

| Sewer | Earth | | - | | Unit Cost | Cons | ruction | Cost |
|-----------|-----------|------|--------|-------|-------------|-------|----------|--------|
| Diameter | Covering | Nos. | of Mar | nhole | (million | | illion R | |
| (mm) | Depth | Α | В | C | Rp./unit) | Α | В | С |
| 350 | 0-2 | 7 | 3 | 6 | 1.11 | 7.8 | 3.3 | 6.7 |
| | 2-4 | 17 | | 8 | 2.22 | 37.7 | 0.0 | 17.8 |
| | 4-6 | 3 | | 3 | 2.77 | 8.3 | 0.0 | 8.3 |
| | | | | | | | | |
| 400 | 0-2 | 8 | 7 | 1 | 1.11 | 8.9 | 7.8 | 1.1 |
| | 2-4 | 14 | 3 | 5 | 2.22 | 31.1 | 6.7 | 11.1 |
| | | | | | | | | : |
| 450 | 0-2 | 1 | 8 | | 1.11 | 1.1 | 8.9 | 0.0 |
| | 2-4 | 7 | 3 | | 2.22 | 15.5 | 6.7 | 0.0 |
| | 4-6 | 6 | 4 | | 2.77 | 16.6 | 11.1 | 0.0 |
| | . * | | | | | : | | |
| 500 | 0-2 | 7 | 2 2 | | 1.11 | 7.8 | 2.2 | 0.0 |
| 1 | 2-4 | 9 | 2 | | 2.22 | 20.0 | 4.4 | 0.0 |
| | 4-6 | 1 | 1 | | 2.77 | 2.8 | 2.8 | 0.0 |
| | ٠. | | | • [| | | | |
| 600 | 0-2 | 7 | 0 | | 1.11 | 7.8 | 0.0 | 0.0 |
| | 2-4 | 7 | 0 | | 2.22 | 15.5 | 0.0 | 0.0 |
| | 4-6 | 17 | 13 | | 2.77 | 47.1 | 36.0 | 0.0 |
| | · | | | | · | | | |
| 700 | 2-4 | 15 | 1 | | 4.10 | 61.5 | 4.1 | 0.0 |
| | 4-6 | 16 | 7 | | 5.10 | 81.6 | 35.7 | 0.0 |
| | 6-8 | l | 4 | | 6.10 | 0.0 | 24.4 | 0.0 |
| | | | : | | | | | _ |
| [800] | 4-6 | 7 | 4 | 0 | 5.10 | 35.7 | 20.4 | 0.0 |
| | 6-8 | 5 | 0 | 7 | 6.10 | 30.5 | 0.0 | 42.7 |
| | | | | | 0.4.0.5 | | | |
| Manhole w | vith Pump | 1 | | | 84.00 | 84.0 | | ' i |
| | | 3 | | 0.0 | | 501.0 | 157.4.4 | 0.7. (|
| Sub- | Total | 155 | 62 | 30 | | 521.3 | 174.4 | 87.6 |
| 200 | 4 - | ا ر | | | E 10 | | ارس | |
| 900 | 1 | 15 | 1 | 0 | 5.10 | 76.5 | 5.1 | 0.0 |
| | 6-8 | 1 | | 11 | 6.10 | 6.1 | 0.0 | 67.1 |
| C. h | Total | 16 | 1 | 11 | | 82.6 | 5.1 | 67.1 |
| Sub- | Total | | | | | | | 07.1 |
| To | otal | 171 | 63 | 41 | | 603.9 | 179,5 | 154.7 |

Table 6.10 (2) Manhole Construction Cost by Sub-zone, by Diameter and by Earth Covering Depth

| | | | | | | FET 5. 20 | · | <u> </u> | | |
|-----------|------------|--------|--------|-------|----------|-----------|-------|----------|-------------------|-------------|
| Sewer | Earth | | | | | Unit Cost | | Construc | | |
| Diameter | Covering | · . | | Manho | | (million | | | lion Rp.) | |
| (mm) | Depth | D | E | F | <u>G</u> | Rp./unit) | | E | F 3.3 | G 0.0 |
| 350 | 0-2 | 0 | 2 9 | 3 | | 1.11 | 0.0 | | $\frac{3.3}{2.2}$ | 0.0 |
| | 2-4 | 10 | i | 1 | | 2.22 | | Ī | | Ī |
| 400 | 0-2 | 2 5 | . 2 | - 2 | 4 | 1.11 | 2.2 | 2.2 | 2.2 | 4.4 |
| 1 | 2-4 | | . 32 | 4 | 13 | 2.22 | | 71.0 | 8.9 | 28.9 |
| | 4-6 | 8 | 5 | 0 | 2 | 2.77 | 22.2 | 13.9 | 0.0 | 5.5 |
| 450 | 0-2 | .0 | 0 | 2 | 2 | 1,11 | 0.0 | 0.0 | 2.2 | 2.2 |
| | 2-4 | 7 | 74 | . 9 | 1 | 2.22 | 15.5 | 164.3 | 20.0 | 2.2 |
| 1 | 4-6 | 1 | 1 | · | 7 | 2.77 | 2.8 | 2.8 | 0.0 | 19.4 |
| 500 | 0-2 | 0 | 3 | 3 | 0 | 1.11 | 0.0 | 3.3 | 3.3 | 0.0 |
| 300 | 2-4 | 2 | 26 | 4 | 5 | 2.22 | | • | 8.9 | 11.1 |
| | 4-6 | 4 | 15 | | 10 | | 11.1 | 41.6 | 0.0 | 27.7 |
| 600 | | | 1 | , , | | | l | | | |
| 600 | 2-4 | 2 4 | 17 | 18 | 7 | 2.22 | 4.4 | 37.7 | 40.0 | 15.5 |
| | 4-6 | 4 | 32 | . 12 | O | 2.77 | 11.1 | 88.6 | 33.2 | 16.6 0.0 |
| | 6-8 | | 2 | 1 | | 3.35 | 0.0 | 6.7 | 3.4 | |
| 700 | 2-4 | 0 | 0 | 4 | 3 | 4.10 | | | 16.4 | 12.3 |
| | 4-6 | 13 | 20 | 7 | 6 | 5.10 | | | | 30.6 |
| | 6-8 | 2 | 0 | | 10 | 6.10 | 12.2 | 0.0 | 0.0 | 61.0 |
| 800 | 4-6 | 4 | . 19 | 4 | 5 | 5.10 | 20.4 | 96.9 | 20.4 | 25.5 |
| | 6-8 | 14 | 14 | | 0 | 6.10 | 85.4 | 85.4 | 0.0 | 0.0 |
| | % < | 0 | | | 5 | 8.90 | 0.0 | 0.0 | 0.0 | 44.5 |
| Manhole w | ith Pump | • | | 1 | | 84.00 | 0.0 | 0.0 | 84.0 | 0.0 |
| <u></u> | Total | 78 | 273 | 75 | 86 | | 291.3 | 796.3 | 284.1 | 307.5 |
| 300 | TOTAL | | 213 | | 00 | | 271.5 | 170.3 | 204.1 | |
| 900 | 4-6 | 0 | 2 | | 2 | 5.10 | 0.0 | 10.2 | 0.0 | 10.2 |
| | 6-8 | 4 | 10 | 1 | 5 | 6.10 | 24.4 | 61.0 | 6.1 | 30.5 |
| | 8< | 5 | 4 | . [| 3 | 8.90 | 44.5 | 35.6 | 0.0 | 26.7 |
| 1000 | 6-8 | 0 | . 10 | - 1 | 3 | 6.10 | | 61.0 | 0.0 | 18.3 |
| 1000 | 0-6 8< | 1 | 3 | Į | . 5 | 8.90 | | 26.7 | 0.0 | 44.5 |
| | | 1 | | | | | | | | |
| 1100 | 6-8 | | 7 | | : | 6.10 | 0.0 | 42.7 | 0.0 | 0.0 |
| | 8< | | 4 | .] | | 8.90 | 0.0 | 35.6 | 0.0 | 0.0 |
| 1200 | 8< | | 2 | Ì | 7 | 8.90 | 0.0 | 17.8 | 0.0 | 62.3 |
| 1350 | 2-4 | | 3 | j | | 4.70 | 0.0 | 14.1 | 0.0 | 0.0 |
| | 4-6 | | 9 | ĺ | * | 5.70 | 0.0 | 51.3 | 0.0 | 0.0 |
| | 6-8 | | 4 | | | 6.80 | | 27.2 | 0.0 | 0.0 |
| | 8< | | 3 | - | 1 | 9.40 | 0.0 | 28.2 | 0.0 | 9.4 |
| 1500 | 6-8 | | 3 | | | 6.80 | 0.0 | 20.4 | 0.0 | 0.0 |
| <u> </u> | Total | 10 | 64 | 1 | 26 | - | 77.8 | 431.8 | 6.1 | 201.9 |
| | otal | 88 | 337 | 76 | 112 | | 369.1 | 1,228.1 | 290.2 | 509.4 |
| | 7141 | 001 | 321 | 7.01 | 116 | i | 202,1 | 1,220.1 | 270.2 | 207.7 |

Unit Construction Cost of Collection Sewer by Diameter and Earth Covering Depth Table 6.11

METHOD : OPENTRENCH
DEPTH : 150 - 10.00 m
MATERIAL: REINFORCED CONCRETE (R.C)

| Unit: Kp./m | B | м 1500 mm | 1,215.4 | 1,714.7 | 3,038.1 | 3,922.1 | 5,876.1 | 7 505 3 |
|-------------|----------|---|----------------------|----------------------|----------------------|----------------------|----------------------|------------------------|
| | ž5 | 8900 mm Ø 1000 mm Ø 1100 mm Ø 1200 mm Ø 1350 mm | 6766 | 1,478.4 | 2.768.7 | 3,643.5 | 5.574.4 | 50016 |
| | SS SS | Ø 1200 mm | 817.7 | 1,257.8 | 2,514.4 | 3.374.4 | 5.273.4 | 33125 |
| | Ŋ. | Ø 1100 mm | 724.4 | 1,176.9 | 2,402.1 | 3,254.7 | 5,137.8 | L |
| | 8 | Ø 1000 mm | 594.9 | 1,084.1 | 2,282.8 | 3,127.1 | 4,990.6 | L |
| | ည္မ | | \$20.1 | 993.8 | 2,164.8 | 2,996.9 | 4,837.8 | A 077 2 |
| | | Ø 800 mm | 438.5 | 898.0 | 2,045.1 | 2,869.2 | 4,691.8 | 5 124 4 |
| | æ | mm 007 00 | 403.3 | 850.8 | 1,992.5 | 2,794.4 | 4,601.0 | 5 (181.5 |
| | Q | Ø 500 mm Ø 600 mm Ø 700 mm Ø 800 mm | 362.5 | 472.6 | 1,903.4 | 2,712.4 | 4,502.4 | 8 060 7 |
| | Ŋ. | Ø 500 mm | 7.772 | 382.1 | 1,774.1 | 2,570.5 | 4,336.6 | 4 760 7 |
| | ä | Ø 450 mm | 252.7 | 354.3 | 1,729.3 | 2,519.6 | 4.274.6 | 4 696.6 |
| | ñ | Ø 400 mm | 224.2 | 323.0 | 1,679.9 | 2,463.6 | 4,206.8 | 4.626.6 |
| | æ | Ø 350 mm | 205.7 | 301.9 | 1,642.9 | 2,420.7 | 4,153.4 | 4.571.1 |
| | PVC | Ø 300 mm | 204.7 | 296.0 | 1,598.3 | 2,367.3 | 4,078.4 | 4 492 0 |
| | PVC | э 250 мш | 158.3 | 247.0 | 1,534.1 | 2,297.3 | 3,997.9 | 44113 |
| | bvc | в 150 mm в 200 mm | 131.6 | 218.4 | 1,491.3 | 2,250.9 | 3,943.2 | 5 556 7 |
| | PVC | р 150 mm | 104.4 | 188.0 | 1,445.4 | 2,196.8 | 3,876.6 | 4 285 8 |
| | | | 15 M | 3 M | 5 M | 7 M | . W 6 | M 01 |
| | | DESCRIPTION | EARTH COVERING DEPTH | EARTH COVERING DEPTH | EARTH COVERING DEPTH | EARTH COVERING DEPTH | HIABO DRIXBADO DENTA | 6 FARTH COVERING DEPTH |
| | | Š. | | 7 | ť. | 4 | 5 | 9 |
| | | | | | | | | |

Table 6.12 (1) Breakdown of Collection Sewer Unit Cost

| 009 | 1.5 | 0.774 | 2.474 | 1.152 | 3.9584 | 1.25 | 1.08 | 0.48 | 2.88 | 2.2 | | 21.5 | 39.8 | 2.9 | 6.1 | 6.0 | 93.4 | 185.6 | 7.1 | 362.5 |
|---------------|---------------------------------------|---|----------------------|--------------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|------------------------------------|--------------------|--------|------|--------------------------|---------------|----------|-------------|-------------------|-------|
| 500 | 1.5 | 0.642 | 2.342 | 1.008 | 3.2788 | 1.00 | 0.86 | 0.42 | 2.42 | 2 | | 17.8 | 31.8 | 2.3 | 5.4 | 5.1 | 85.0 | 125.1 | 5.4 | 277.7 |
| 450 | 1.5 | 0.584 | 2.284 | 0.936 | 2.9692 | 0.88 | 0.76 | 0.39 | 2.21 | 1.9 | | 16.1 | 28.1 | 2.0 | 5.0 | 4.6 | 80.7 | 111.1 | 5.0 | 252.7 |
| 400 | 1.5 | 0.52 | 2.22 | 0.864 | 2.664 | 0.77 | 0.67 | 0.36 | 2.00 | 1.8 | | 14.5 | 24.6 | 1.8 | 4.6 | 4.2 | 76.5 | 93.7 | 4. | 224.2 |
| 350 | 1.5 | 0.466 | 2.166 | 0.792 | 2.3826 | 0.67 | 0.59 | 0.33 | 1.79 | 1.7 | | 12.9 | 21.4 | 1.6 | 4.2 | 3.8 | 72.2 | 85.5 | 0.4 | 205.7 |
| 300 | 1.5 | 0.318 | 2.018 | 0.72 | 2.018 | 0.54 | 0.46 | 0.3 | 1.56 | 1.6 | | 11.0 | 17.2 | 1.2 | 3,00 | 3.3 | 68.0 | 96.3 | 4.0 | 204.7 |
| 250 | 1.5 | 0.265 | 1.965 | 0.648 | 1.7685 | 0.45 | 0.40 | 0.27 | 1.37 | 1.5 | | 9.6 | 14.5 | | 3.4 | 2.9 | 63.7 | 0.09 | 3.1 | 158.3 |
| 200 | 1.5 | 0.212 | 1.912 | 0.612 | 1.6252 | 0.40 | 0.36 | 0.255 | 1.27 | 1.45 | | 8.8 | 12.8 | 1.0 | 3.3 | 2.7 | 61.6 | 39.0 | 2.6 | 131.6 |
| 150 | 1.5 | 0.16 | 1.86 | 0.504 | 1.302 | 0.30 | 0.29 | 0.21 | 1.02 | 1.3 | | 7.1 | 9.6 | 0.8 | 2.7 | 2.1 | 55.2 | 24.9 | 2.0 | 104.4 |
| Diameter (mm) | (r | | | · (° | | | | | | | Rp./m) Unit Cost | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 1 ls | 1 18 | |
| Dian | (1) Quantity Earth Covering Depth (m) | Outside of Diameter (m) Width of Excavation (m) | Excavation Depth (m) | Volume of Pavement (m ³) | Excavation Backhoe (m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | (2) Construction Cost (1000 Rp./m) | Excavation Backhoe | | | Backfill (selected soil) | Residual Soil | Pavement | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (2) Breakdown of Collection Sewer Unit Cost

| , | | | | | | | | | | | · | | | | | | | | | | | | |
|---------------|---------------------------------------|---|----------------------|--------------------------------------|--------------------------------------|-------------------------------------|------------------------|--|---------------------------------|----------------------------|----------------------------------|------------------------------------|-----------|--------------------|---------------------|----------------------|------------------------|---------------|----------|----------------|-------------|-------------------|--------|
| 1500 | 1.5 | 1.81 | 3.51 | 1.872 | 9.126 | 2.91 | 3.56 | 0.78 | 5.56 | 3.2 | 7.02 | | • | 49.6 | 93.0 | 5.6 | 10.0 | 11.7 | 135.9 | 58.5 | 823.5 | 23.8 | 1215.4 |
| 1350 | 2.1 | 1.65 | 3.35 | 1.8 | 8.375 | 2.74 | 3.09 | 0.75 | 5.29 | 3.1 | 6.7 | | | 45.5 | 87.3 | 8.2 | 9.6 | | 131.7 | 55.8 | 626.1 | 19.5 | 994.9 |
| 1200 | 1.5 | 2.45 | 3.15 | 1.728 | 7.56 | 2.55 | 2.56 | 0.72 | 2.00 | 'n | 6.3 | | | 41.1 | 81.3 | 8.0 | 2.2 | 10.5 | 127.4 | 52.5 | 472.9 | 16.0 | 817.7 |
| 1100 | 1.5 | 1.35 | 3.05 | 1.584 | 6.71 | 2.20 | 2.27 | 99.0 | 4.44 | 2.8 | 6.1 | | ! | 36.5 | 70.7 | 0.9 | 4. | 9.3 | 118.9 | 50.8 | 410.0 | 14.2 | 724.4 |
| 1000 | 1.5 | 1.22 | 2.92 | 1.512 | 6.132 | 2.02 | 1.97 | 0.63 | 4.17 | 2.7 | 1 | | : | 33.3 | 04.0 | 5.2 | 2.0 | 8.7 | 114.7 | 0.0 | 348.7 | 11.7 | 594.9 |
| 006 | 1.5 | 1.1 | 2.8 | 1.368 | 5.32 | 1.71 | 1.67 | 0.57 | 3.65 | 2.5 | 1 | | | 28.9 | 0.4.0 | 4.0 | , 3 | 7.7 | 106.2 | 0.0 | 300.9 | 10.2 | 520.1 |
| 800 | 1.5 | 0.98 | 2.68 | 1.296 | 4.824 | 1.55 | 1.44 | 0.54 | | | | | | 26.2 | | | | | | | | | 438.5 |
| 700 | 1.5 | 0.88 | 2.58 | 1.224 | 4.386 | 1.40 | 1.25 | 0.51 | 3.13 | 2.3 | | | (| 23.8 | o. 4 | | 0.0 | 9.9 | 7.76 | 0.0 | 212.8 | 7.9 | 403.3 |
| Diameter (mm) | (m) | (E) | | (m ³) | (m ³) | 3) | | m ³) | | | | 000 Rp./m) | Unit Cost | 5.433 | 417.10 | 7.00 | 1) 12.7/4 | 2.1 | 42.475 | 8.334 | 1 is | l ls | |
| | (1) Quantity Earth Covering Depth (m) | Outside of Diameter (m) Width of Excavation (m) | Excavation Depth (m) | Volume of Pavement (m ³) | Excavation Backhoe (m ³) | Backfill (granular m ³) | Backfill (original m3) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Retaining Wall (m ²) | (2) Construction Cost (1000 Rp./m) | ; | Excavation Backhoe | Backilli (granular) | Backfilli (Originai) | Backilli (Selected Sol | Residual Soil | Pavement | Retaining Wall | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (3) Breakdown of Collection Sewer Unit Cost

| 909 | 3 | 1.8 | 3.974 | 1.296 | 7.1532 | 1.46 | 3.85 | 0.54 | 3.30 | 2.4 | 7.948 | | 38.9 | 46.7 | 10.3 | 6.9 | 6.9 | 101.9 | 66.2 | 185.6 | 9.3 | 472.6 |
|---------------|---|-------------------------|---------------------|--------------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|----------------------------------|------------------------------------|--------------------|---------------------|------|---------------|---------------|----------|----------------|-------------|-------------------|-------|
| 500 | 3 | 1.6 | 3.842 | 1.152 | 6.1472 | 1.18 | 3,33 | 0.48 | 2.82 | 2.2 | 7.684 | | 33.4 | 37.8 | 8.9 | 6.1 | 5.9 | 93.4 | 64.0 | 125.1 | 7.5 | 382.1 |
| 450 | 3 3 584 | 1.5 | 3.784 | 1.08 | 5.676 | 1.06 | 3.09 | 0.45 | 2.59 | 2.1 | 7.568 | | 30.8 | 33.8 | 8.2 | 5.7 | 5.4 | 89.2 | 63.1 | 111.1 | 6.9 | 354.3 |
| 400 | 6,43 | 4.4 | 3.72 | 1.008 | 5.208 | 0.94 | 2.84 | 0.42 | 2.36 | 7 | 7.44 | | 28.3 | 29.9 | 7.6 | 5.4 | 5.0 | 85.0 | 62.0 | 93.7 | 6.3 | 323.0 |
| 350 | 3 | 1.3 | 3.666 | 0.936 | 4.7658 | 0.83 | 2.61 | 0.39 | 2.15 | 1.9 | 7.332 | | 25.9 | 26.3 | 7.0 | 5.0 | 4.5 | 80.7 | 61.1 | 85.5 | 5.9 | 301.9 |
| 300 | | 1.2 | 3.518 | 0.864 | 4.2216 | 99:0 | 2.34 | 0.36 | 1.89 | 1.8 | 7.036 | | 22.9 | 21.1 | 6.2 | 4.6 | 4.0 | 76.5 | 58.6 | 96.3 | 5.8 | 296.0 |
| 250 | 3 | 1.1 | 3.465 | 0.792 | 3.8115 | 0.57 | 2.12 | 0.33 | 1.69 | 1.7 | 6.93 | | 20.7 | 18.1 | 5.6 | 4.2 | 3.5 | 72.2 | 57.8 | 60.0 | 8.8 | 247.0 |
| 200 | 3 | 1.05 | 3.412 | 0.756 | 3.5826 | 0.50 | 2.01 | 0.315 | 1.57 | 1.65 | 6.824 | | 19.5 | 16.0 | 5.3 | 4.0 | 3.3 | 70.1 | 56.9 | 39.0 | 4. 6. | 218.4 |
| 150 | 3 9 1 9 | 0.0 | 3.36 | 0.648 | 3.024 | 0.39 | 1.71 | 0.27 | 1.31 | 1.5 | 6.72 | | 16.4 | 12.6 | 9.4 | 3.4 | 2.8 | 63.7 | 56.0 | 24.9 | 3.7 | 188.0 |
| Diameter (mm) | th (m) | (II) | n) | t (m ³) | (m ³) | n3) | (3) | il m ³) | | | | (1000 Rp./m) Unit Cost | • | (L) | 2.66 | 12.774 | 2.1 | 42.475 | 8.334 | l Is | I Is | |
| | (1) Quantity Earth Covering Depth (m) Outside of Diameter (m) | Width of Excavation (m) | Excavation Depth (r | Volume of Pavement (m ³) | Excavation Backhoe (m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Retaining Wall (m ²) | (2) Construction Cost (1000 Rp./m) | Excavation Backhoe | Backfill (granular) | _ | $\overline{}$ | Residual Soil | Pavement | Retaining Wall | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (4) Breakdown of Collection Sewer Unit Cost

| Diameter | ter (mm) | 700 | 800 | 006 | 1000 | 1100 | 1200 | 1350 | 1500 |
|--|---------------------|--------|--------|-------|--------|--------|--------|--------|--------|
| (1) Quantity | 1 | | | | | | | | |
| Earth Covering Depth (m) | | 'n | en . | ന | m | m | 33 | m | m |
| Outer of Diameter (m) | | 0.88 | 0.98 | 1.1 | 1.22 | 1.35 | 1.45 | 1.65 | 1.81 |
| Width of Excavation (m) | | 2 | 2.1 | 2.2 | 2.4 | 2.5 | 2.6 | 2.8 | 2.9 |
| Excavation Depth (m) | • | 4.08 | 4.18 | 4.3 | 4.42 | 4.55 | 4.65 | 4.85 | 5.01 |
| Sheet Pile (m) | | 6.12 | 6.27 | 6.45 | 6.63 | 6.825 | 6.975 | 7.275 | 7.515 |
| Volume of Pavement (m ³) | | 1.44 | 1.512 | 1.584 | 1.728 | 1.8 | 1.872 | 2.016 | 2.088 |
| Excavation (Backhoe m ³) | | 8.16 | 8.778 | 9.46 | 10.608 | 11.375 | 12.09 | 13.58 | 14.529 |
| Backfill (granular m ³) | | 1.75 | 1.93 | 2.13 | 2.48 | 2.69 | 2.90 | 3.32 | 3.55 |
| Backfill (original m ³) | • | 4.37 | 4.70 | 5.09 | 5.68 | 6.13 | 6.54 | 7.40 | 8.03 |
| Backfill (selected soil m ³) | | 9.0 | 0.63 | 99.0 | 0.72 | 0.75 | 0.78 | 0.84 | 0.87 |
| Residual Soil (m ³) | • | 3.79 | 4.08 | 4.37 | 4.93 | 5.24 | 5.55 | 6.18 | 6.50 |
| Pavement (m ²) | · · · | 2.6 | 2.7 | 2.8 | m | 3.1 | 3.2 | 3.4 | 3.5 |
| Sheet Pile (m) | | 12.24 | 12.54 | 12.9 | 13.26 | 13.65 | 13.95 | 14.55 | 15.03 |
| Sheet Pile (kg) | | 587.52 | 601.92 | 619.2 | 636.48 | 655.2 | 9 699 | 698.4 | 721.44 |
| Bracing (kg) | | 146.88 | 150.48 | 154.8 | 159.12 | 163.8 | 167.4 | 174.6 | 180.36 |
| (2) Construction Cost (1000 Rp./m) | tp./m) Unit Cost | | - | | | | | | |
| Excavation Backhoe | 5.433 | 44.3 | 47.7 | 51.4 | 57.6 | 61.8 | 65.7 | 73.8 | 78.9 |
| | 31.914 | 55.9 | 61.7 | 68.0 | 79.1 | 86.0 | 92.5 | 106.0 | 113.2 |
| _: | 2.66 | 11.6 | 12.5 | 13.5 | 15.1 | 16.3 | 17.4 | 19.7 | 21.3 |
| ت | 12.774 | 7.7 | 8.0 | 4.8 | 9.5 | 9.6 | 10.0 | 10.7 | |
| Residual Soil | 2.1 | 8.0 | 8.6 | 9.5 | 10.3 | 11.0 | 11.7 | 13.0 | 13.7 |
| Pavement | 42.475 | 110.4 | 114.7 | 118.9 | 127.4 | 131.7 | 135.9 | 144.4 | 148.7 |
| Sheet Pile | 15 | 183.6 | 188.1 | 193.5 | 198.9 | 204.8 | 209.3 | 218.3 | 225.5 |
| Sheet Pile | 0.25 | 146.9 | 150.5 | 154.8 | 159.1 | 163.8 | 167.4 | 174.6 | 180.4 |
| Bracing | 0.36 | 52.9 | 54.2 | 55.7 | 57.3 | 59.0 | 60.3 | 62.9 | 64.9 |
| Pipe/Laying | 1 Is | 212.8 | 234.4 | 300.9 | 348.7 | 410.0 | 472.9 | 626.1 | 823.5 |
| Dewatering/Others | l Is | 16.7 | 17.6 | 19.5 | 21.3 | 23.1 | 24.9 | 29.0 | 33.6 |
| Total | | 850.8 | 0.868 | 993.8 | 1084.1 | 1176.9 | 1267.8 | 1478.4 | 1714.7 |
| | | | | | | | | | |

Table 6.12 (5) Breakdown of Collection Sewer Unit Cost

| 600 | ν. | 0.774 | 6.1 | 5.974 | 8.961 | 1.368 | 1.8506 | 9.5 | 1.57 | 7.84 | 0.57 | 3.51 | 2.5 | 44.805 | 2150.64 | 227.00 | | 34.4 | 51.6 | 50.1 | 20.9 | 7.3 | 7.4 | 106.2 | 672.1 | 537.7 | 193.6 | 185.6 | 36.7 | 1903.4 |
|---------------|---------------------------------------|-------------------------|-------------------------|----------------------|----------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|---------|---------|--------|------------------------------------|-----------------|--------------------|---------------------|---------------------|--------------------------|---------------|----------|-------------------|-------|---------|-------------|-------------------|--------|
| 500 | S | 0.642 | 1.7 | 5.842 | 8.763 | 1.224 | 1.4314 | 8.5 | 1.28 | 6.92 | 0.51 | 3.01 | 2.3 | 43.815 | 2103.12 | 323.10 | | 26.6 | 46.2 | 40.8 | 18.4 | 6.5 | 6.3 | 7.76 | 657.2 | 525.8 | 189.3 | 125.1 | 34.2 | 1774.1 |
| 450 | δ. | 0.584 | 1.6 | 5.784 | 8.676 | 1.152 | 1.2544 | ∞ | 1.15 | 6.48 | 0.48 | 2.78 | 2.2 | 43.38 | 2082.24 | 320.30 | | 23.3 | 43.5 | 36.6 | 17.2 | 6.1 | 5.8 | 93.4 | 650.7 | 520.6 | 187.4 | 111.1 | 33.5 | 1729.3 |
| 400 | 5 | 0.52 | 1.5 | 5.72 | 8.58 | 1.08 | 1.08 | 7.5 | 1.02 | 6.03 | 0.45 | 2.55 | 2.1 | 42.9 | 2059.2 | 0.44.0 | | 20.1 | 40.7 | 32.5 | 16.0 | 5.7 | 4.0 | 89.2 | 643.5 | 514.8 | 185.3 | 93.7 | 32.9 | 1679.9 |
| 350 | 5 | 0.456 | 1.4 | 5.666 | 8.499 | 1.008 | 0.9324 | 7 | 06.0 | 5.60 | 0.42 | 2.33 | 7 | 42.495 | 2039.76 | 302.74 | | 17.3 | 38.0 | 28.8 | 14.9 | 5.4 | 4.0 | 85.0 | 637.4 | 509.9 | 183.6 | 85.5 | 32.2 | 1642.9 |
| 300 | 5 | 0.318 | 1.3 | 5.518 | 8.277 | 0.936 | 0.6734 | 6.5 | 0.72 | 5.12 | 0.39 | 2.05 | 1.9 | 41.385 | 1986.48 | 470.07 | | 12.5 | 35.3 | 23.1 | 13.6 | 5.0 | 4.3 | 80.7 | 620.8 | 496.6 | 178.8 | 96.3 | 31.3 | 1598.3 |
| 250 | 5 | 0.265 | 1.2 | 5.465 | 8.1975 | 0.864 | 0.558 | 9 | 0.62 | 4.71 | 0.36 | 1.85 | 1.8 | 40.9875 | 1967.4 | 441.03 | | 10.4 | 32.6 | 19.9 | 12.5 | 4.6 | 3.9 | 76.5 | 614.8 | 491.9 | 177.1 | 0.09 | 30.1 | 1534.1 |
| 200 | 5 | 0.212 | 1.15 | 5.412 | 8.118 | 0.828 | 0.4738 | 5.75 | 0.55 | 4.50 | 0.345 | 1.73 | 1.75 | 40.59 | 1948.32 | 407.00 | | 00 00 | 31.2 | 17.7 | 12.0 | 4.4 | 3.6 | 74.3 | 6.809 | 487.1 | 175.3 | 39.0 | 29.0 | 1491.3 |
| 150 | 5 | 0.16 | | 5.36 | 8.04 | 0.72 | 0.36 | 5 | 0.44 | 3.90 | 0.3 | 1.46 | 1.6 | 40.2 | 1929.6 | 4.794 | | 6.7 | 27.2 | 14.0 | 10.4 | 9.8 8.0 | 3.1 | 68.0 | 603.0 | 482.4 | 173.7 | 24.9 | 28.3 | 1445.4 |
| Diameter (mm) | (u | | | | | 3) | | | | | ~ ~ | | | | | | Rp./m) Unit Cost | 18.6 | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | l Is | 1 is | |
| Diar | (1) Quantity Earth Covering Depth (m) | Outside of Diameter (m) | Width of Excavation (m) | Excavation Depth (m) | Sheetpile Length (m) | Volume of Pavement (m ³) | Excavation Crum (m ³) | Excavation Backhoe (m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | | \- | ł | (2) Construction Cost (1000 Rp./m) | Excavation Crum | Excavation Backhoe | Backfill (granular) | Backfill (original) | Backfill (selected soil) | Residual Soil | Pavement | Sheetpile Driving | | Bracing | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (6) Breakdown of Collection Sewer Unit Cost

| | | | | —– | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---------------|--------------|--------------------------|--------------------|-------------------------|----------------------|----------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|----------------------|----------------|--------------|--|-----------------|--------------------|---------------------|---------------------|--------------------------|---------------|----------|-------------------|----------------|---------|-------------|-------------------|--------|
| 1500 | | Ω <u>;</u> | 1.81 | 2.9 | 7.01 | 10.515 | 2.088 | 5.829 | 14.5 | 3.55 | 13.83 | 0.87 | 6.50 | 3.5 | 52.575 | 2523.6 | 630.9 | | 108.4 | 78.8 | 113.2 | 36.8 | | 13.7 | 148.7 | 788.6 | 630.9 | 227.1 | 823.5 | 57.4 | 3038.1 |
| 1350 | ١ | ς ; | 1.65 | 2.8 | 6.85 | 10.275 | 2.016 | 5.18 | 14 | 3.32 | 13.00 | 0.84 | 6.18 | 3.4 | 51.375 | 2466 | 616.5 | | 96.3 | 76.1 | 106.0 | 34.6 | 10.7 | 13.0 | 144.4 | 770.6 | 616.5 | 221.9 | 626.1 | 52.4 | 2768.7 |
| 1200 | | ^ | 1.45 | 2.6 | 6.65 | 9.975 | 1.872 | 4.29 | 13 | 2.90 | 11.74 | 0.78 | 5.55 | 3.2 | 49.875 | 2394 | 598.5 | | 79.8 | 70.6 | 92.5 | 31.2 | 10.0 | 11.7 | 135.9 | 748.1 | 598.5 | 215.5 | 472.9 | 47.7 | 2514.4 |
| 1100 | ١ | Λ <u>;</u> | 1.35 | 2.5 | 6.55 | 9.825 | 1.8 | 3.875 | 12.5 | 2.69 | 11.13 | 0.75 | 5.24 | 3.1 | 49.125 | 2358 | 589.5 | | 72.1 | 6.79 | 86.0 | 29.6 | 9.6 | 11.0 | 131.7 | 736.9 | 589.5 | 212.2 | 410.0 | 45.6 | 2402.1 |
| 1000 | ì | Λ ; | 1.22 | 2.4 | 6.42 | 9.63 | 1.728 | 3.408 | 12 | 2.48 | 10.48 | 0.72 | 4.93 | ന | 48.15 | 2311.2 | 577.8 | | 63.4 | 65.2 | 79.1 | 27.9 | 9.5 | 10.3 | 127.4 | 722.3 | 577.8 | 208.0 | 348.7 | 43.5 | 2282.8 |
| 006 | ı | 'n, | | 2.2 | 6.3 | 9.45 | 1.584 | 2.86 | 11 | 2.13 | 9.49 | 99.0 | 4.37 | 2.8 | 47.25 | 2268 | 567 | | 53.2 | 59.8 | 0.89 | 25.2 | 8.4 | 9.5 | 118.9 | 708.8 | 567.0 | 204.1 | 300.9 | 41.4 | 2164.8 |
| 800 | , l | n 6 | 0.98 | 2.1 | 6.18 | 9.27 | 1.512 | 2.478 | 10.5 | 1.93 | 8.90 | 0.63 | 4.08 | 2.7 | 46.35 | 2224.8 | 556.2 | | 46.1 | 57.0 | 61.7 | 23.7 | 8.0 | 8.6 | 114.7 | 695.3 | 556.2 | 200.2 | 234.4 | 39.2 | 2045.1 |
| 700 | 1, | n 6 | 2.88 | 7 | 6.08 | 9.12 | 1.44 | 2.16 | 10 | 1.75 | 8.37 | 9.0 | 3.79 | 2.6 | 45.6 | 2188.8 | 547.2 | | 40.2 | 54.3 | 55.9 | 22.3 | 7.7 | 8.0 | 110.4 | 684.0 | 547.2 | 197.0 | 212.8 | 38.8 | 1992.5 |
| Diameter (mm) | | e (ii | | | | | [3) | | 3) | | | (3) | | | | | | 0 Rp./m) Unit Cost | 18.6 | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | l Is | 1 ls | |
| Dia | (1) Quantity | Earth Covering Depth (m) | Outer Diameter (m) | Width of Excavation (m) | Excavation Depth (m) | Sheetpile Length (m) | Volume of Pavement (m ³) | Excavation (Crum m ³) | Excavation (Backhoe m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Sheetpile Length (m) | Sheetpile (kg) | Bracing (kg) | (2) Construction Cost (1000 Rp./m) Unit Cost | Excavation Crum | Excavation Backhoe | Backfill (granular) | Backfill (original) | Backfill (selected soil) | Residual Soil | Pavement | Sheetpile Driving | Sheetpile (kg) | Bracing | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (7) Breakdown of Collection Sewer Unit Cost

| 7 0.318 0.466 1.3 1.4 7.518 7.666 11.277 11.499 0.936 1.008 3.2734 3.7324 6.5 0.90 7.72 8.40 0.39 0.42 2.05 2.33 1.9 2 56.385 57.495 3383.1 3449.7 845.385 57.495 3383.1 3449.7 56.385 57.495 3383.1 3449.7 845.385 57.495 386.445 862.4 863.6 8 | Diameter (mm) | 150 | 200 | 250 | 300 | 350 | 400 | 450 | 200 | 009 |
|--|---------------|---------|--------|--------|------------|---------|--------|--------|------------|---------|
| 2 0.265 0.318 0.466 1.2 1.3 7.666 1.1975 11.277 11.499 8 11.1975 11.277 11.499 8 2.958 3.2734 3.7324 9 0.864 0.936 0.90 10.62 0.72 0.90 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.87 2.05 2.24 2 35.0 60.9 69.4 32.6 35.3 38.0 11.89 2.05 2.24 2 32.6 60.9 69.4 3.9 4.3 4.3 4.9 4.6 5.0 5.4 <td>7</td> <td></td> <td>7</td> <td>7</td> <td>!~</td> <td>7</td> <td>7</td> <td>7</td> <td>!~</td> <td></td> | 7 | | 7 | 7 | ! ~ | 7 | 7 | 7 | ! ~ | |
| 5 1.2 1.3 1.4 5 7.465 7.518 7.666 8 7.465 7.518 7.666 9 0.864 0.936 1.008 11.1975 11.277 11.499 10.864 0.936 1.008 10.864 0.936 1.008 10.62 0.72 0.90 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.33 11.85 2.05 2.38 11.89 20.5 22.4 2 32.6 38.33 32.6 35.3 38.0 18.9 20.5 22.4 4.6 5.0 5.4 2 4.3 4.9 32.6 35.3 38.0 32.6 35.3 38.0 32.6 36.3 38.0 32.6 36.3 38.0 32.6 36.3 38.0 32.6 36.4 310.5 302.3 304.5 310.5 44.0 45.1 46.1 44.0 45.1 46.1 44.0 45.1 46.1 44.0 <t< td=""><td>0 9</td><td></td><td>-</td><td>0.265</td><td></td><td>0.466</td><td>0.52</td><td>0.584</td><td>0.642</td><td>0.774</td></t<> | 0 9 | | - | 0.265 | | 0.466 | 0.52 | 0.584 | 0.642 | 0.774 |
| 2 7.465 7.518 7.666 8 7.465 7.518 7.666 11.1975 11.277 11.499 0.864 0.936 1.008 0.864 0.936 1.008 1.085 3.2734 3.7324 0.62 0.72 0.90 7.11 7.72 8.40 0.36 0.39 0.42 1.85 2.05 2.33 1.85 2.05 2.33 1.85 2.05 2.33 1.85 3.83.1 3449.7 2 3359.25 3383.1 3449.7 32.6 383.1 3449.7 38.0 4.6 5.0 5.4 4.9 4.6 5.0 5.4 4.9 4.6 5.0 5.4 4.9 4.6 5.0 5.4 4.9 4.6 5.0 5.4 4.9 4.6 5.0 5.4 4.9 4.6 5.0 5.4 4.9 5.0 5.0 5.4 <td< td=""><td></td><td></td><td>1.15</td><td>1.2</td><td>1.3</td><td>1.4</td><td>1.5</td><td>1.6</td><td>1.7</td><td>1.9</td></td<> | | | 1.15 | 1.2 | 1.3 | 1.4 | 1.5 | 1.6 | 1.7 | 1.9 |
| 8 11.1975 11.277 11.499 0.864 0.936 1.008 0.864 0.936 1.008 2.958 3.2734 3.7324 6 6.5 0.72 0.90 7.11 7.72 8.40 0.36 0.39 0.42 1.85 2.05 2.33 1.85 2.05 2.33 1.85 2.05 2.33 1.85 2.05 2.33 1.85 2.05 2.33 1.85 2.05 2.33 1.89 2.05 2.33 2.05 2.34 2.05 2.34 2.07 2.34 2.07 2.34 2.07 2.34 2.07 2.34 2.08 2.4 2.09 2.34 2.09 2.09 2.00 2.00 2.00 2.00 2.00 2.00 | _ | <u></u> | 412 | | .51 | 7.666 | 7.72 | 7.784 | 7.842 | 7.974 |
| 8 0.864 0.936 1.008 2.958 3.2734 3.7324 6 6.5 0.72 0.90 7.11 7.72 8.40 0.36 0.39 0.42 1.85 2.05 2.33 1.85 2.05 2.33 1.85 2.05 2.33 2.95 3383.1 3449.7 8359.25 3383.1 3449.7 8359.25 32.6 50.9 69.4 55.0 60.9 69.4 4.6 5.0 5.4 5.0 5.4 4.9 60.9 85.0 839.8 845.8 862.4 839.8 845.8 862.4 839.8 845.8 862.4 840.0 96.3 85.5 60.0 96.3 2420.7 2 | | | .118 | , | .27 | 11.499 | 11.58 | 11.676 | 11.763 | 11.961 |
| 8 2.958 3.2734 3.7324 6 6.5 0.90 7.11 7.72 8.40 0.36 0.39 0.42 1.85 2.05 2.33 1.8 1.9 2 2 3359.25 3383.1 3449.7 3359.25 3383.1 3449.7 3359.25 3383.1 3449.7 4 335.3 38.0 5 45.775 862.425 4 5.0 60.9 69.4 5 45.6 5.0 5.4 4 4.6 5.0 5.4 4 5.0 5.4 4.9 5 885.0 862.4 862.4 8 839.8 845.8 862.4 8 839.8 845.8 862.4 8 304.5 86.5 46.1 6 60.0 96.3 88.5 6 60.0 96.3 85.5 6 60.0 96.3 85.5 6 60.0 9 | | | 0.828 | 0.864 | .93 | 1.008 | 1.08 | 1.152 | 1.224 | 1.368 |
| 5.75 6 6.5 7 0.55 0.62 0.72 0.90 6.80 7.11 7.72 8.40 0.345 0.36 0.39 0.42 1.73 1.85 2.05 2.33 1.73 1.8 1.9 2 1.73 1.8 1.9 2 1.73 1.8 1.9 2 1.73 1.8 1.9 2 1.73 1.8 1.9 2 25.9 25.9875 383.1 3449.7 33.6 33.3 383.1 3449.7 31.2 32.6 383.3 38.0 31.2 32.6 35.3 38.0 31.2 32.6 35.3 38.0 31.2 32.6 35.3 38.0 31.2 32.6 35.3 38.0 31.3 3.9 4.3 4.9 4.4 4.6 5.0 5.4 32.4 4.9 4.9 33.0 3.0 3.0 33.0 <td></td> <td></td> <td>2.7738</td> <td>2.958</td> <td>3.2734</td> <td>3.7324</td> <td>4.08</td> <td>4.4544</td> <td>4.8314</td> <td>5.6506</td> | | | 2.7738 | 2.958 | 3.2734 | 3.7324 | 4.08 | 4.4544 | 4.8314 | 5.6506 |
| 0.55 0.62 0.72 0.90 6.80 7.11 7.72 8.40 0.345 0.39 0.42 1.73 1.85 2.05 2.33 1.75 1.8 1.9 2 55.59 55.9875 56.385 57.495 33.85 835.25 3383.1 3449.7 33.85 839.8125 845.775 862.425 31.2 32.6 35.3 38.0 17.7 19.9 23.1 28.8 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 3.6 3.9 4.3 4.9 74.3 839.8 845.8 862.4 833.9 839.8 845.8 862.4 833.9 839.8 845.8 862.4 830.0 60.0 96.3 85.5 43.2 44.0 45.1 46.1 260.9 2297.3 2367.3 2420.7 2 | ς. | | 5.75 | 9 | 6.5 | 7 | 7.5 | ∞ | 8.5 | 9.5 |
| 6.80 7.11 7.72 8.40 0.345 0.35 0.35 0.35 0.39 0.42 1.75 1.85 2.05 2.33 1.75 1.8 1.9 2 2.05 3383.1 3449.7 3359.25 3383.1 3449.7 3359.25 3383.1 3449.7 33.85 839.8125 845.775 862.4 85.0 60.9 69.4 35.0 22.4 4.6 5.0 5.0 5.4 4.9 4.6 5.0 5.4 3.39.8 845.8 862.4 883.9 839.8 845.8 862.4 853.9 839.8 845.8 862.4 350.2 300.2 302.3 304.5 310.5 330.2 300.2 300.2 3367.3 2420.7 250.9 2297.3 2367.3 2420.7 250.9 | 0.44 | | 0.55 | 0.62 | 0.72 | 06.0 | 1.02 | 1.15 | 1.28 | 1.57 |
| 0.345 0.36 0.39 0.42 1.73 1.85 2.05 2.33 1.75 1.8 1.9 2 55.59 55.9875 56.385 57.495 335.4 3359.25 3383.1 3449.7 33.85 839.8125 845.775 862.425 31.2 32.6 35.3 38.0 17.7 19.9 23.1 28.8 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 4.3 4.3 4.9 74.3 845.8 862.4 833.9 839.8 845.8 862.4 833.9 839.8 845.8 862.4 833.9 60.0 96.3 85.5 43.2 44.0 45.1 46.1 250.9 2297.3 2367.3 2420.7 250.9 23.7 2367.3 2420.7 | 5.90 | | 6.80 | | 7.72 | 8.40 | 9.03 | 9.68 | 10.32 | 11.64 |
| 1.73 1.85 2.05 2.33 1.75 1.8 1.9 2 55.59 55.9875 56.385 57.495 33.54 3359.25 3383.1 3449.7 33.85 839.8125 845.775 862.425 31.2 32.6 35.3 38.0 17.7 19.9 23.1 28.8 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 3.6 3.9 4.3 4.9 74.3 839.8 845.8 862.4 833.9 839.8 845.8 862.4 833.9 60.0 96.3 310.5 250.9 2297.3 2367.3 2420.7 250.9 2297.3 2367.3 2420.7 | 0.3 | | 0.345 | 0.36 | 0.39 | 0.42 | 0.45 | 0.48 | 0.51 | 0.57 |
| 1.75 1.8 1.9 2 55.59 55.9875 56.385 57.495 335.4 3359.25 3383.1 3449.7 33.85 3359.25 3849.7 33.85 38.0 449.7 51.6 55.0 60.9 69.4 31.2 32.6 35.3 38.0 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 3.6 3.9 84.3 85.0 833.9 839.8 845.8 862.4 833.9 839.8 845.8 862.4 39.0 60.0 96.3 310.5 43.2 44.0 45.1 46.1 250.9 237.3 2367.3 2420.7 | 1.46 | | 7 | 1.85 | 2.05 | 2.33 | 2.55 | 2.78 | 3.01 | 3.51 |
| 55.59 55.9875 56.385 57.495 335.4 3359.25 3383.1 3449.7 33.85 839.8125 845.775 862.425 51.6 55.0 60.9 69.4 31.2 32.6 35.3 38.0 17.7 19.9 23.1 28.8 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 3.6 3.9 4.3 4.9 74.3 3.9 845.8 862.4 833.9 839.8 845.8 862.4 833.9 839.8 845.8 862.4 300.2 302.3 304.5 310.5 39.0 60.0 96.3 85.5 43.2 44.0 45.1 46.1 250.9 2297.3 2367.3 2420.7 | 1.6 | | 1.75 | 1.8 | 1.9 | 7 | 2.1 | 2.2 | 2.3 | 2.5 |
| 335.4 3359.25 3383.1 3449.7 33.85 839.8125 845.775 862.425 51.6 55.0 60.9 69.4 31.2 32.6 35.3 38.0 17.7 19.9 23.1 28.8 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 3.6 3.9 4.3 4.9 74.3 839.8 845.8 862.4 833.9 839.8 845.8 862.4 39.0 60.0 96.3 85.5 43.2 44.0 45.1 46.1 250.9 2297.3 2367.3 2420.7 | 55.2 | | 55.59 | 8 | 56.385 | 57.495 | 57.9 | 58.38 | | 59.805 |
| 33.85 839.8125 845.775 862.425 51.6 55.0 60.9 69.4 31.2 32.6 35.3 38.0 17.7 19.9 23.1 28.8 18.1 18.9 20.5 22.4 4.4 4.6 5.0 5.4 3.6 3.9 4.3 4.9 74.3 862.4 862.4 833.9 839.8 845.8 862.4 833.9 839.8 845.8 862.4 39.0 60.0 96.3 85.5 43.2 44.0 45.1 46.1 250.9 2297.3 2367.3 2420.7 | 312 | | (,, | 6 | 3383.1 | 3449.7 | 3474 | 3502.8 | 3528.9 | 3588.3 |
| 55.0 60.9 69.4 32.6 35.3 38.0 19.9 23.1 28.8 18.9 20.5 22.4 4.6 5.0 5.4 3.9 4.3 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 839.8 845.8 862.4 845.9 862.4 845.0 46.1 2297.3 2367.3 2420.7 | | \sim | 33.85 | .812 | 45.77 | 862.425 | 868.5 | 875.7 | | 897.075 |
| 55.0 60.9 69.4 32.6 35.3 38.0 19.9 23.1 28.8 18.9 20.5 22.4 4.6 5.0 5.4 3.9 4.3 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 839.8 845.8 862.4 845.9 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | | | | | | | | | | |
| 55.0 60.9 69.4 32.6 35.3 38.0 19.9 23.1 28.8 18.9 20.5 22.4 4.6 5.0 5.4 3.9 84.3 85.0 839.8 845.8 862.4 839.8 845.8 862.4 839.8 845.8 862.4 845.9 862.4 845.0 862.4 | | | ; | 1 | , | , | | | | |
| 32.6 35.3 38.0 19.9 23.1 28.8 18.9 20.5 22.4 4.6 5.0 5.0 5.4 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 839.8 845.8 862.4 850.0 96.3 85.5 60.0 96.3 85.5 44.0 45.1 46.1 2297.3 2367.3 2420.7 22 | 43.9 | | 51.6 | 55.0 | 6.09 | 69.4 | 75.9 | 82.9 | O. | 105.1 |
| 19.9 23.1 28.8 18.9 20.5 22.4 4.6 5.0 5.4 3.9 4.3 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | 27.2 | | 31.2 | 32.6 | 35.3 | 38.0 | 40.7 | 43.5 | S | 51.6 |
| 18.9 20.5 22.4 4.6 5.0 5.4 3.9 4.3 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | 14.0 | | 17.7 | 19.9 | 23.1 | 28.8 | 32.5 | 36.6 | 40.8 | 50.1 |
| 4.6 5.0 5.4 3.9 4.3 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | 15.7 | | 18.1 | 18.9 | 20.5 | 22.4 | 24.0 | 25.7 | _ | ₽-1 |
| 3.9 4.3 4.9 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | 3.8 | | 4.4 | 4.6 | 5.0 | 5.4 | 5.7 | 6.1 | 6.5 | 7.3 |
| 76.5 80.7 85.0 839.8 845.8 862.4 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | 3.1 | | 3.6 | 3.9 | 4.3 | 4.9 | 5.4 | 5.8 | 6.3 | 7.4 |
| 839.8 845.8 862.4 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | | | 74.3 | 76.5 | 80.7 | 85.0 | 89.2 | 93.4 | 97.7 | 106.2 |
| 839.8 845.8 862.4 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 | | ~ | 333.9 | 839.8 | 845.8 | 862.4 | 868.5 | 75 | 882.2 | 897.1 |
| 302.3 304.5 310.5 60.0 96.3 85.5 44.0 45.1 46.1 2297.3 2367.3 2420.7 | | | 833.9 | 839.8 | 845.8 | 862.4 | 868.5 | 875.7 | 882.2 | 897.1 |
| 60.0 96.3 85.5 44.0 45.1 46.1 2297.3 2367.3 2420.7 | 298.1 | | 300.2 | 302.3 | 304.5 | 310.5 | 312.7 | 1.5 | 317.6 | 322.9 |
| 2297.3 2367.3 2420.7 2 | 24.9 | | 39.0 | 0.09 | 96.3 | 85.5 | 93.7 | 111.1 | 125.1 | 185.6 |
| 2297.3 2367.3 2420.7 | 42.2 | - 1 | 43.2 | | 45.1 | 46.1 | 46.8 | 47.7 | 48.6 | 51.1 |
| | 2196.8 | C. | 2250.9 | 2297.3 | 2367.3 | 2420.7 | 2463.6 | 2519.6 | 2570.5 | 2712.4 |

Table 6.12 (8) Breakdown of Collection Sewer Unit Cost

| | | | | | | | | | | | | | | | | <u>·</u> | | | | | | | | | | | | | | | |
|------------|--------------|-------------------------|--------------------|-------------------------|----------------------|----------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|----------------------|----------------|--------------|----------------------------------|-----------------|--------------------|--------|-----------|----------|---------------|--------|-------------------|----------------|---------|-------------|-------------------|--------|
| 1500 | | 7 | 1.81 | 2.9 | 9.01 | 13.515 | 2.088 | 11.629 | 14.5 | 3.55 | 19.63 | 0.87 | 6.50 | 3.5 | 67.575 | 4054.5 | 1013.625 | · . | 216.3 | 78.8 | 113.2 | 52.2 | }~~\ | 13.7 | 148.7 | 1013.6 | 1013.6 | 364.9 | 823.5 | 72.6 | 3922.1 |
| 1350 | | _ | 1.65 | 2.8 | 8.85 | 13.275 | 2.016 | 10.78 | 4 | 3.32 | 18.60 | 0.84 | 6.18 | 3.4 | 66.375 | 3982.5 | 995.625 | • | 200.5 | 76.1 | 106.0 | 49.5 | 10.7 | 13.0 | 144.4 | 995.6 | 995.6 | 358.4 | 626.1 | 67.5 | 3643.5 |
| 1200 | l | <u></u> | 1.45 | 2.6 | 8.65 | 12.975 | 1.872 | 9.49 | 13 | 2.90 | 16.94 | 0.78 | 5.55 | 3.2 | 64.875 | 3892.5 | 973.125 | | 176.5 | 70.6 | 92.5 | 45.1 | 10.0 | 11.7 | 135.9 | 973.1 | 973.1 | 350.3 | 472.9 | 62.7 | 3374.4 |
| 1100 | t | _ | 1.35 | 2.5 | 8.55 | 12.825 | 1.8 | 8.875 | 12.5 | 5.69 | 16.13 | 0.75 | 5.24 | 3.1 | 64.125 | 3847.5 | 961.875 | | 165.1 | 6.19 | 86.0 | 42.9 | 9.6 | 11.0 | 131.7 | 961.9 | 961.9 | 346.3 | 410.0 | 9.09 | 3254.7 |
| 1000 | t | | 1.22 | 4.2 | 8.42 | 12.63 | 1.728 | 8.208 | 12 | 2.48 | 15.28 | 0.72 | 4.93 | co | 63.15 | 3789 | 947.25 | | 152.7 | 65.2 | 79.1 | 40.6 | 9.5 | 10.3 | 127.4 | 947.3 | 947.3 | 341.0 | 348.7 | 58.3 | 3127.1 |
| 006 | ţ | | 1.1 | 2.2 | 8.3 | 12.45 | 1.584 | 7.26 | prod prod | 2.13 | 13.89 | 99.0 | 4.37 | 2.8 | 62.25 | 3735 | 933.75 | | 135.0 | 59.8 | 68.0 | 36.9 | 4.8 | 9.5 | 118.9 | 933.8 | 933.8 | 336.2 | 300.9 | 56.1 | 2996.9 |
| 800 | 1 | | 0.98 | 2.1 | 8.18 | 12.27 | 1.512 | 8.678 | 10.5 | 1.93 | 13.10 | 0.63 | 4.08 | 2.7 | 61.35 | 3681 | 920.25 | | 124.2 | 57.0 | 61.7 | 34.9 | 8.0 | 9.8 | 114.7 | 920.3 | 920.3 | 331.3 | 234.4 | 53.9 | 2869.2 |
| 700 | • | | 0.88 | 2 | 8.08 | 12.12 | 1.44 | 6.16 | 10 | 1.75 | 12.37 | 9.0 | 3.79 | 2.6 | 9.09 | 3636 | 606 | | 114.6 | 54.3 | 55.9 | 32.9 | 7.7 | 8.0 | 110.4 | 0.606 | 0.606 | 327.2 | 212.8 | 52.5 | 2794.4 |
| teter (mm) | , | $\overline{}$ | | | | | | | | | , | | | | | • | | Rp./m) Unit_Cost | 18.6 | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | 1 ls | 1 ls | |
| Diameter | (1) Quantity | Earth Covering Depth (m | Outer Diameter (m) | Width of Excavation (m) | Excavation Depth (m) | Sheetpile Length (m) | Volume of Pavement (m ³) | Excavation (Crum m ³) | Excavation (Backhoe m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Sheetpile Length (m) | Sheetpile (kg) | Bracing (kg) | (2) Construction Cost (1000 Rp./ | Excavation Crum | Excavation Backhoe | | (original | ت | Residual Soil | | Sheetpile Driving | Sheetpile (kg) | Bracing | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (9) Breakdown of Collection Sewer Unit Cost

| 009 | | 6 | 0.774 | 1.9 | 9.974 | 15.0 | 1.368 | 9.4506 | 9.5 | 1.57 | 15.44 | 0.57 | 3.51 | 2.5 | 74.805 | 7854.525 | 1963.631 | | 175.8 | 7-4 | 50.1 | 41.1 | 7.3 | 7.4 | 106.2 | 1122.1 | 1963.6 | 706.9 | 185.6 | 84.8 | 4502.4 |
|---------------|--------------|--------------------------|-------------------------|-------------------------|----------------------|--------------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|----------------------|--------------------------|--------------|---|-----------------|--------------------|---------------------|---------------------|--------------------------|---------------|----------|-------------------|--------------------|---------|-------------|-------------------|--------|
| 500 | | 0, | 0.642 | 1.7 | 9.842 | 14.8 | 1.224 | 8.2314 | 8.5 | 1.28 | 13.72 | 0.51 | 3.01 | 2.3 | 73.815 | 7750.575 | 1937.643 | | 153.1 | vo | 40.8 | 36.5 | 6.5 | 6.3 | 97.7 | 1107.2 | 1937.6 | 697.6 | 125.1 | 82.1 | 4336.6 |
| 450 | | o, | 0.584 | 1.6 | 9.784 | 14.7 | 1.152 | 7.6544 | 00 | 1.15 | 12.88 | 0.48 | 2.78 | 2.2 | 73.38 | 7704.9 | 1926.225 | | 142.4 | 43.5 | 36.6 | 34.2 | 6.1 | 5.8 | 93.4 | 1100.7 | 1926.2 | 693.4 | 111.1 | 81.0 | 4274.6 |
| 400 | | 6 | 0.52 | 1.5 | 9.72 | 14.6 | 1.08 | 7.08 | 7.5 | 1.02 | 12.03 | 0.45 | 2.55 | 2.1 | 72.9 | 7654.5 | 1913.625 | | 131.7 | 40.7 | 32.5 | 32.0 | 5.7 | 4.0 | 89.2 | 1093.5 | 1913.6 | 688.9 | 93.7 | 79.8 | 4206.8 |
| 350 | | 0, | 0.466 | 1.4 | 9.666 | 14.5 | 1.008 | 6.5324 | 7 | 06:0 | 11.20 | 0.42 | 2.33 | 7 | 72.495 | 7611.975 | 1902.993 | | 121.5 | 38.0 | 28.8 | 29.8 | 5.4 | 4.9 | 85.0 | 1087.4 | 1903.0 | 685.1 | 85.5 | 79.1 | 4153.4 |
| 300 | | 6 | 0.318 | 1.3 | 9.518 | 14.3 | 0.936 | 5.8734 | 6.5 | 0.72 | 10.32 | 0.39 | 2.05 | 1.9 | 71.385 | 7495.425 | 1873.856 | | 109.2 | 35.3 | 23.1 | 27.5 | 5.0 | 4.3 | 80.7 | 1070.8 | 1873.9 | 674.6 | 96.3 | 77.8 | 4078.4 |
| 250 | | 6 | 0.265 | 1.2 | 9.465 | 14.2 | 0.864 | 5.358 | 9 | 0.62 | 9.51 | 0.36 | 1.85 | 1.8 | 70.9875 | 3 | 1863.421 | | 66.7 | 32.6 | 19.9 | 25.3 | 4.6 | 3.9 | 76.5 | 1064.8 | 1863.4 | 670.8 | 0.09 | 76.4 | 3997.9 |
| 200 | | 6 | 0.212 | 1.15 | 9.412 | 14.1 | 0.828 | 5.0738 | 5.75 | 0.55 | 9.10 | 0.345 | 1.73 | 1.75 | 70.59 | Ċν. | 1852.987 | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | 94.4 | 31.2 | 17.7 | 24.2 | 4.4 | 3.6 | 74.3 | 1058.9 | 1853.0 | 667.1 | 39.0 | 75.5 | 3943.2 |
| 150 | | 6 | 0.16 | _ | 9:36 | 14.0 | 0.72 | 4.36 | S | 0.44 | 7.90 | 0.3 | 1.46 | 1.6 | 70.2 | 7371 | 1842.75 | | 81.1 | 27.2 | 14.0 | 21.0 | 3.8 | 3.1 | 68.0 | 1053.0 | 1842.8 | 663.4 | 24.9 | 74.4 | 3876.6 |
| Diameter (mm) | | | (m) | | | (s) | <u> </u> | | | | | <u> </u> | | | | | | Rp./m) Unit Cost | 18.6 | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | l Is | 1 ls | |
| Dian | (1) Quantity | Earth Covering Depth (m) | Outer Diameter of sewer | Width of Excavation (m) | Excavation Depth (m) | Sheetpile Length (m/pcs) | Volume of Pavement (m ³) | Excavation Crum (m ³) | Excavation Backhoe (m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Sheetpile Length (m) | Weight of Sheetpile (kg) | Bracing (kg) | (2) Construction Cost (1000 Rp./m) | Excavation Crum | Excavation Backhoe | Backfill (granular) | Backfill (original) | Backfill (selected soil) | Residual Soil | Pavement | Sheetpile Driving | Lease of Sheetpile | Bracing | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (10) Breakdown of Collection Sewer Unit Cost

| | | | | | | | | | | | | | | | | | | <u> </u> | - | | | | | | | | | | |
|-----------|--|--------------------|-------------------------|----------------------|--------------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|----------------------|---------------------------------------|------------------------------------|-----------------|--------------------|---------------------|---------------------|--------------------------|---------------|----------|-------------------|--------------------|---------|-------------|-------------------|--------|
| 1500 | o | 1.81 | 2.9 | 11.01 | 16.5 | 2.088 | 17.429 | 14.5 | 3.55 | 25.43 | 0.87 | 6.50 | 3.5 | 82.575 | 8670.375 2167.593 | | 324.2 | 78.8 | 113.2 | 9.79 | 7 | 13.7 | 148.7 | 1238.6 | 2167.8 | 780.3 | 823.5 | 108.9 | 5876.1 |
| 1350 | o | 1.65 | 2.8 | 10.85 | 16.3 | 2.016 | 16.38 | 14 | 3.32 | 24.20 | 0.84 | 6.18 | 3.4 | | 8544.375 2136.093 | | 304.7 | 76.1 | 106.0 | 64.4 | 10.7 | 13.0 | 144.4 | 1220.6 | 2136.1 | 769.0 | 626.1 | 103.3 | 5574.4 |
| 1200 | σ | 1 45 | 2.6 | 10.65 | 16.0 | 1.872 | 14.69 | 13 | 2.90 | 22.14 | 0.78 | 5.55 | 3.2 | 79.875 | 8386.875 2096.718 | | 273.2 | 70.6 | 92.5 | 58.9 | 10.0 | 11.7 | 135.9 | 1198.1 | 2096.7 | 754.8 | 472.9 | 98.1 | 5273,4 |
| 1100 | o | 1.35 | 2.5 | 10.55 | 15.8 | 1.8 | 13.875 | 12.5 | 2.69 | 21.13 | 0.75 | 5.24 | 3.1 | 79.125 | 8308.125 2077.031 | | 258.1 | 62.9 | 86.0 | 56.2 | 9.6 | 11.0 | 131.7 | 1186.9 | 2077.0 | 747.7 | 410.0 | 95.7 | 5137.8 |
| 1000 | σ | 1.22 | 2.4 | 10.42 | 15.6 | 1.728 | 13.008 | 12 | 2.48 | 20.08 | 0.72 | 4.93 | 'n | 78.15 | 8205.75 2051.437 | | 241.9 | 65.2 | 79.1 | 53.4 | 9.5 | 10.3 | 127.4 | 1172.3 | 2051.4 | 738.5 | 348.7 | 93.1 | 4990.6 |
| 006 | | , | 2.2 | 10.3 | 15.5 | 1.584 | 11.66 | Ï | 2.13 | 18.29 | 0.66 | 4.37 | | M | ~ | | 216.9 | 59.8 | 68.0 | 48.6 | 8.4 | 9.5 | 118.9 | 1158.8 | 2027.8 | 730.0 | 300.9 | 90.5 | 4837.8 |
| 800 | σ | 0.98 | તાં | 10.18 | 15.3 | 1.512 | 10.878 | 10.5 | 1.93 | 17.30 | 0.63 | 4.08 | 2.7 | 76.35 | 8016.75 2004.187 | | 202.3 | 57.0 | 61.7 | 46.0 | 8.0 | 8.6 | 114.7 | 1145.3 | 2004.2 | 721.5 | 234.4 | 88.0 | 4691.8 |
| 700 | σ | 0.88 | 2 | 10.08 | 15.1 | 1.44 | 10.16 | 10 | 1.75 | 16.37 | 9.0 | 3.79 | 2.6 | 75.6 | 7938 1984.5 | | 189.0 | 54.3 | 55.9 | 43.5 | 7.7 | 8.0 | 110.4 | 1134.0 | 1984.5 | 714.4 | 212.8 | 86.5 | 4601.0 |
| eter (mm) | | | | | | | | | | | | | | | | Rp./m) Unit Cost | 18.6 | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | I Is | l Is | |
| 1 1 | (1) Quantity Farth Covering Denth (m) | Outer Diameter (m) | Width of Excavation (m) | Excavation Depth (m) | Sheetpile Length (m/pcs) | Volume of Pavement (m ³) | Excavation (Crum m ³) | Excavation (Backhoe m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Sheetpile Length (m) | Weight of Sheetpile (kg) Bracing (kg) | (2) Construction Cost (1000 Rp./m) | Excavation Crum | Excavation Backhoe | Backfill (granular) | Backfill (original) | Backfill (selected soil) | Residual Soil | Pavement | Sheetpile Driving | Lease of Sheetpile | Bracing | Pipe/Laying | Dewatering/Others | Total |

Table 6.12 (11) Breakdown of Collection Sewer Unit Cost

| 500 600 | 10 | 0.642 0.774 | 1.7 | 10.842 | | 1.224 | 9.9314 11.3506 | | 1.28 | | 0.51 | | 2.3 2.5 | | 8538.075 8642.025 | 777 | | | 184.7 | 46.2 | 40.8 | 41.0 | 6.5 | 6.3 | 7.76 | <u>``</u> | 2134.5 | 768.4 | 125.1 | 7 22 |
|---------------|---------------------------------------|--------------------|-------------------------|----------------------|--------------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|------------------------|-------|---------------------------------|----------------------------|---------|-----------------------------|----------|------------------------------------|-----------|-------|-------|--------|---------------------|--------------------------|---------------|----------|-------------------|--------|---------|-------------|-------------------|
| 450 | Ç | 0.584 | 1.6 | 10.784 | 16.2 | 1.152 | 9.2544 | ∞ | 1.15 | 14.48 | 0.48 | 2.78 | 2.2 | 80.88 | 8492.4 | 4143.1 | | | 172.1 | 43.5 | 36.6 | 38.5 | 6.1 | 5.8 | 93.4 | 1213.2 | 2123.1 | 764.3 | 111.1 | 000 |
| 400 | 10 | 0.52 | 1.5 | 10.72 | 16.1 | 1.08 | 8.58 | 7.5 | 1.02 | 13.53 | 0.45 | 2.55 | 2.1 | 80.4 | 8442 | | | | 159.6 | 40.7 | 32.5 | 36.0 | 5.7 | 5.4 | 89.2 | 1206.0 | 2110.5 | 759.8 | 93.7 | 8 O6 |
| 350 | C | 0.466 | 1.4 | 10.666 | 16.0 | 1.008 | 7.9324 | 7 | 06.0 | 12.60 | 0.42 | 2.33 | .2 | 79.995 | 8399.475 | 022.00 | | | 147.5 | 38.0 | 28.8 | 33.5 | 5.4 | 4.9 | 85.0 | 1199.9 | 2099.9 | 756.0 | 85.5 | 89.7 |
| 300 | Ç | 0.318 | 1.3 | 10.518 | 15.8 | 0.936 | 7.1734 | 6.5 | 0.72 | 11.62 | 0.39 | 2.05 | 1.9 | 78.885 | 8282.925 | 40/0./21 | | | 133.4 | 35.3 | 23.1 | 30.9 | 5.0 | 4.3 | 80.7 | 1183.3 | 2070.7 | 745.5 | 96.3 | 88.2 |
| 250 | 10 | 0.265 | 1.2 | 10.465 | 15.7 | 0.864 | 6.558 | 9 | 0.62 | 10.71 | 0.36 | 1.85 | 8.7 | 78.4875 | 8241.187 | યા | | | 122.0 | 32.6 | 19.9 | 28.5 | 4.6 | 3.9 | 76.5 | 1177.3 | 2060.3 | 741.7 | 0.09 | 86.5 |
| 200 | 1.0 | 0.212 | 1.15 | 10.412 | 15.6 | 0.828 | 6.2238 | 5.75 | 0.55 | 10.25 | 0.345 | 1.73 | 1.75 | 78.09 | 8199.45 | 700.4407 | | • | 115.8 | 31.2 | 17.7 | 27.3 | 4.4 | 3.6 | 74.3 | 1171.4 | 2049.9 | 738.0 | 39.0 | 85.4 |
| 150 | 10 | 0.16 | | 10.36 | 15.5 | 0.72 | 5.36 | 32 | 0.44 | 8.90 | 0.3 | 1.46 | 1.6 | 7.77 | 8158.5 | 7.07 | | | 99.7 | 27.2 | 14.0 | 23.7 | 3.8 | 3.1 | 68.0 | , | 2039.6 | 734.3 | 24.9 | 84.1 |
| Diameter (mm) | | | | | | | | | | | | | | | | | Rp./m) | Unit Cost | 18.6 | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | l is |]]s |
| Diam | (1) Quantity Farth Covering Denth (m) | Outer Diameter (m) | Width of Excavation (m) | Excavation Depth (m) | Sheetpile Length (m/pcs) | Volume of Pavement (m ³) | Excavation Crum (m ³) | Excavation Backhoe (m ³) | Backfill (granular m ³) | Backfill (original m3) | | Residual Soil (m ³) | Pavement (m ²) | | Sheetpile (kg) Bracing (kg) | | (2) Construction Cost (1000 Rp./m) | | | Ö | | Backfill (original) | Backfill (selected soil) | Residual Soil | Pavement | Sheetpile Driving | | Bracing | Pipe/Laying | Dewatering/Others |

Table 6.12 (12) Breakdown of Collection Sewer Unit Cost

| 1500 | | 2; | 1.81 | 2.9 | 12.01 | 18.0 | 2.088 | 20.329 | 14.5 | 3.55 | 28.33 | 0.87 | 6.50 | 3.5 | 90.075 | 9457.875 2364.468 | | 378.1 | 78.8 | 113.2 | 75.3 | 11.1 | 13.7 | 148.7 | 1351.1 | 2364.5 | 851.2 | 823.5 | 116.6 | 6325.7 |
|---------------|--------------|--------------------------|------------------------------|-------------------------|----------------------|-----------------------|--------------------------------------|-----------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--|---------------------------------|----------------------------|----------------------|---|----------------------------------|-----------------|--------------------|---------------------|---------------------|--------------------------|---------------|----------|-------------------|--------------------|---------|-------------|-------------------|--------|
| 1350 | | 2, | 20.7 | 7. X | 11.85 | 17.8 | 2.016 | 19.18 | 14 | 3.32 | 27.00 | 0.84 | 6.18 | 3.4 | | 9331.875 2332.968 | | 356.7 | 76.1 | 106.0 | 71.8 | 10.7 | 13.0 | 144.4 | 1333.1 | 2333.0 | 839.9 | 626.1 | 111.0 | 6021.9 |
| 1200 | | 10 | 24.1 7 c | 7.0 | 11.65 | 17.5 | 1.872 | 17.29 | 13 | 2.90 | 24.74 | 0.78 | 5.55 | 3.2 | 87.375 | 9174.375 | | 321.6 | 70.6 | 92.5 | 65.8 | 10.0 | 11.7 | 135.9 | 1310.6 | 2293.6 | 825.7 | 472.9 | 105.7 | 5716.6 |
| 1100 | | 10 | 1.55 7.5 | 5.7 | 11.55 | 17.3 | 7.8 | 16.375 | 12.5 | 2.69 | 23.63 | 0.75 | 5.24 | 3.1 | 86.625 | 9095.625 2273.906 | | 304.6 | 67.9 | 86.0 | 62.9 | 9.6 | 11.0 | 131.7 | 1299.4 | 2273.9 | 818.6 | 410.0 | 103.4 | 5578.9 |
| 1000 | | 100 | 77.1 | 4:7 | 11.42 | 17.1 | 1.728 | 15.408 | 12 | 2.48 | 22.48 | 0.72 | 4.93 | m | 85.65 | 8993.25 2248.312 | | 286.6 | 65.2 | 79.1 | 59.8 | 9.2 | 10.3 | 127.4 | 1284.7 | 2248.3 | 809.4 | 348.7 | 100.9 | 5429.7 |
| 006 | , | 2 - | | 7.7 | 11.3 | 17.0 | 1.584 | 13.86 | | 2.13 | 20.49 | 99.0 | 4.37 | 2.8 | 84.75 | 8898.75 2224.687 | | 257.8 | 59.8 | 68.0 | 54.5 | 8.4 | 9.2 | 118.9 | 1271 3 | 2224.7 | 800.9 | 300.9 | 98.4 | 5272.6 |
| 800 | , | 0.0 | 67.0 | 7.7 | 11.18 | 16.8 | 1.512 | 12.978 | 10.5 | 1.93 | 19.40 | 0.63 | 4.08 | 2.7 | 83.85 | 8804.25 2201.062 | · | 241.4 | 57.0 | 61.7 | 51.6 | 8.0 | 8.6 | 114.7 | 1257.8 | 2201.1 | 792.4 | 234.4 | 95.8 | 5124.4 |
| 700 | , | 10 | 0.88 0.88 | 7 | 11.08 | 16.6 | 1.44 | 12.16 | 10 | 1.75 | 18.37 | 9.0 | 3.79 | 2.6 | 83.1 | 8725.5 2181.375 | | 226.2 | 54.3 | 55.9 | 48.9 | 7.7 | 8.0 | 110.4 | 1246.5 | 2181.4 | 785.3 | 212.8 | 94.1 | 5031.5 |
| Diameter (mm) | | (m) | (EE) | | | cs) | 13) | | 3) | | | (3) | | | | (3) | 0 Rp./m) Unit Cost | - | 5.433 | 31.914 | 2.66 | 12.774 | 2.1 | 42.475 | 15 | 0.25 | 0.36 | 1 ls | 1 1s | |
| Dia | (1) Quantity | Earth Covering Depth (m) | United Diameter of Sewer (m) | widen of excavation (m) | Excavation Depth (m) | Sheetpile Length (m/p | Volume of Pavement (m ³) | Excavation (Crum m ³) | Excavation (Backhoe m ³) | Backfill (granular m ³) | Backfill (original m ³) | Backfill (selected soil m ³) | Residual Soil (m ³) | Pavement (m ²) | Sheetpile Length (m) | Weight of Sheetpile (kg Bracing (kg) | (2) Construction Cost (1000 Rp./ | Excavation Crum | Excavation Backhoe | Backfill (granular) | Backfill (original) | Backfill (selected soil) | Residual Soil | Pavement | Sheetpile Driving | Lease of Sheetpile | Bracing | Pipe/Laying | Dewatering/Others | Total |

Table 6.13 (1) Unit Construction Cost of Manhole by Diameter and by Manhole Height

| Manhole | Height | Unit Price (x1,000 Rp.) | Remarks |
|---------|--------|-------------------------|-------------------------------------|
| 2.0 m | | 1,110 | Type 1 Sewer Diameter: 150 - 600 mm |
| 2.5 m | | 1,257 | " Manhole Height : 2.0 - 7.5 m |
| 3.0 m | | 1,378 | u |
| 3.5 m | | 2,220 | Type 2 Manhole type is as shown in |
| 4.0 m | | 2,339 | " Drawing Book |
| 4.5 m | | 2,483 | n . |
| 5.0 m | | 2,627 | D D |
| 5.5 m | | 2,770 | п |
| 6.0 m | | 2,914 | И |
| 6.5 m | 1 | 3,058 | м |
| 7.0 m | | 3,202 | ef |
| 7.5 m | | 3,345 | н |

Table 6.13 (2) Unit Construction Cost of Manhole by Diameter and by Manhole Height

| Manhole Height | 700 m | 800 m | 900 m | 1,000 m | 1,100 m | 1,200 m |
|----------------|-------|-------|-------|---------|---------|---------|
| 4.0 m | 4,085 | 4,099 | 4,121 | 4,143 | 4,198 | 4,465 |
| 6.0 m | 5,080 | 5,100 | 5,122 | 5,145 | 5,199 | 5,458 |
| 8.0 m | 6,105 | 6,126 | 6,148 | 6,170 | 6,224 | 6,483 |
| 10.0 m | 8,798 | 8,824 | 8,852 | 8,880 | 8,949 | 9,291 |

Table 6.13 (3) Unit Construction Cost of Manhole by Diameter and by Manhole Height

| Manhole | Height | Unit Price (x1,000 Rp.) | Remarks |
|---------|--------|-----------------------------|-----------------------------------|
| 3.0 m | | 4,027 | Sewer Diameter : 1,350 - 1,500 mm |
| 4.0 m | | 4,512 | Manhole Height: 3.0 - 10.0 m |
| 4.5 m | | 4,740 | Į |
| 6.0 m | | 5,505 | İ |
| 6.5 m | | 5,741 | |
| 8.0 m | | 6,530 | · · |
| 8.5 m | | 6,767 | |
| 10.0 m | | 9,350 | <u> </u> |

Table 6.14 Labour Wages

| Item | Description | Unit | Unit Cost |
|------|-----------------------|---------|-----------|
| No | | | (Rp.) |
| 1 | Common labor | Man-day | 3,500 |
| 2 | Semi skilled labor | Man-day | 4,000 |
| 3 | Skilled labor | Man-day | 5,000 |
| 4 | Mason | Man-day | 5,000 |
| 5 | Plasterer | Man-day | 5,500 |
| 6 | Concrete worker | Man-day | 5,500 |
| 7 | Steel worker | Man-day | 5,500 |
| 8 | Carpenter | Man-day | 5,500 |
| 9 | Foreman | Man-day | 8,000 |
| 10 | Welder | Man-day | 6,000 |
| 11 | Electrician | Man-day | 6,000 |
| 12 | Plumber | Man-day | 6,000 |
| 13 | Operator | Man-day | 10,000 |
| 14 | Assistent Operator | Man-day | 6,500 |
| 15 | Driver (dump truck) | Man-day | 6,000 |
| 16 | Mechanic | Man-day | 7,000 |
| 17 | Surveyor | Man-day | 8,000 |

Table 6.15 Rental Cost of Equipment

| Item | Description | Capacity | Unit Price |
|------|---------------------|-------------|-------------|
| No | | | (Rp./day) |
| 1 | Concrete mixer | 0.1 m 3 | 32,000 |
| 2 | Concrete vibrator | dia. 40 m m | 25,000 |
| 3 | Water pump | dia. 75 m m | 32,800 |
| 4 | Excavator / backhoe | 0.6 m 3 | 360,000 |
| 5 | Bulldozer | 11 ton | 465,000 |
| 6 | Bulldozer | 15 ton | 520,000 |
| 7 | Crawler crane | 16 ton | 480,000 |
| 8 | Dump truck | 2 ton | 122,000 |
| 9 | Dump truck | 8 ton | 164,000 |
| 10 | Vibro hammer | 2.4 ton | 710,000 |
| 11 | Tamping rammer | 80 k g | 34,000 |
| 12 | Compressor | 3 m3/min. | 425,000 |
| 13 | Vibratory compactor | 23 ton | 320,000 |
| 14 | Generator set | - | 95,000 |

Table 6.16 Fuel and Materials Cost

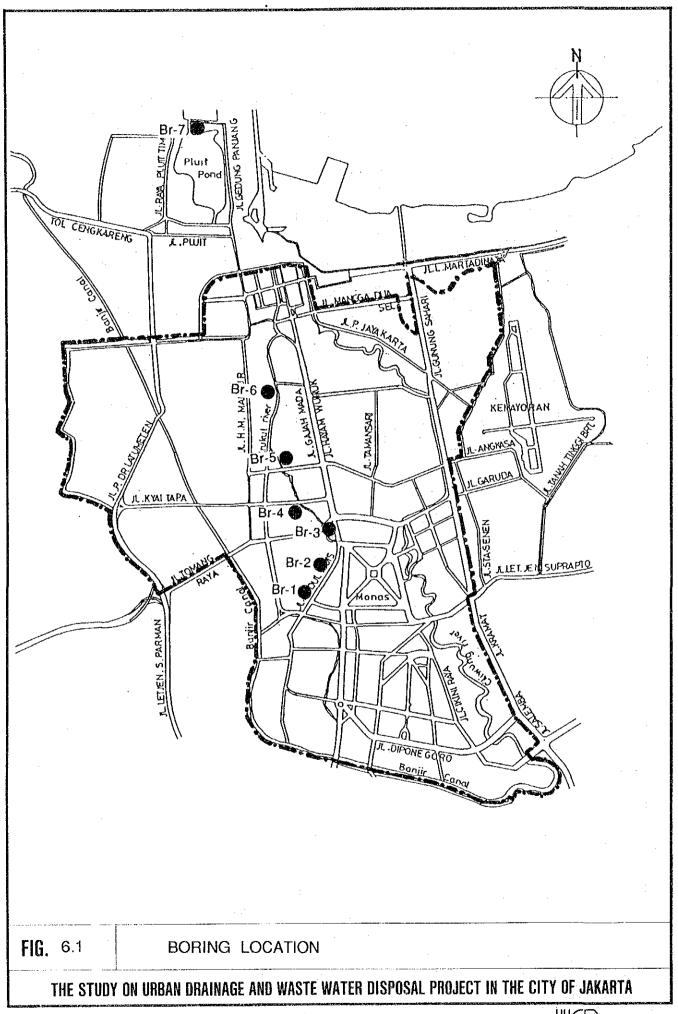
| Item No. | Description | Unit | Unit Cost (Rp.) |
|----------|--------------------------------------|-------|-----------------|
| 1 | Gasoline | lit. | 450 |
| 2 | Diesel oil | lit. | 245 |
| 3 | Hydraulic oil | lit. | 5,200 |
| 4 | Lubricant oil | lit. | 6,500 |
| 5 | Grease | kg | 7,750 |
| 5 6 | Portland cement | bag | 5,400 |
| 7 | Sand for concrete | m 3 | 24,000 |
| 8 | Sand for others | m 3 | 20,000 |
| 9 | Sand gravels | m 3 | 21,000 |
| 10 | Crushed stone for concrete | m 3 | 24,000 |
| 11 | Broken stone | m 3 | 21,000 |
| 12 | Brick | рс | 80 |
| 13 | Selected soil | m 3 | 2,750 |
| 14 | Meranti Wood (class III) : | | |
| | a. Plank | m 3 | 230,000 |
| | b. Square | m 3 | 210,000 |
| 15 | Plywood 4 x 8 t 9 mm | sheet | 16,000 |
| 16 | Plywlld 4 x 8 t 12 mm | sheet | 21,000 |
| 17 | Dolken wood dia. 80 mm | рс | 1,000 |
| 18 | Reinforced steel bar | ton | 800,000 |
| 19 | Steel materials : | | |
| ` . | a. Sheet pile type II (48 kg/m) | k g | 1,250 |
| | b. Sheet pile type III (60 kg/m) | k g | 1,250 |
| | c. Sheet pile type VL (105 kg/m) | k g | 1,250 |
| | d. H Shape steel | k g | 1,250 |
| 20 | Concrete wire | k g | 1,500 |
| 21 | Nails | k g | 1,300 |
| 22 | Polyvinyl Chloride (pvc) Pipes : | | |
| | a. Diametre 150 mm | m | 19,850 |
| | b. Diametre 200 mm | m | 31,350 |
| | c. Diametre 250 mm | m | 48,350 |
| | d. Diametre 300 mm | m | 78,750 |
| 23 | Reinforced Concrete (RC) Pipes | | |
| | (including rubber joint) | | |
| | a. Diameter 350 mm | m | 68,100 |
| | b. Diameter 400 mm | m | 75,100 |
| | c. Diameter 450 mm | m | 91,100 |
| | d. Diameter 500 mm | m | 100,850 |
| | e. Diameter 600 mm | m | 146,100 |
| | f. Diameter 700 mm | m | 170,000 |
| | g. Diameter 800 mm | m | 188,750 |
| | h. Diameter 900 mm | m | 247,900 |
| | i. Diameter 1000 mm | m | 292,000 |
| | j. Diameter 1100 mm | m | 343,300 |
| | k. Diameter 1200 mm | m | 399,500 |
| | Diameter 1350 mm | m | 521,250 |
| | m. Diameter 1500 mm | m | 686,200 |

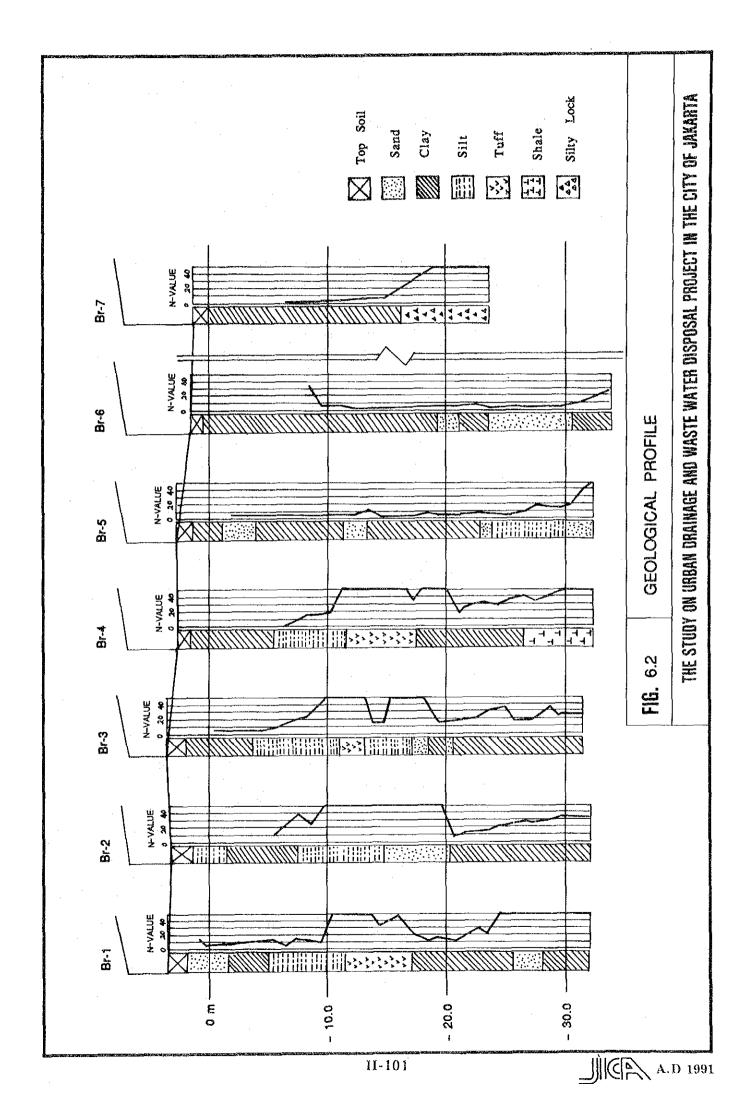
Table 6.17 O&M Cost for Central Sewerage Zone

| | (Unit : millio | on Rp./annum) |
|--|---------------------|---------------------|
| Year Item | 2000 | 2010 |
| 1) F/S Area Sewer line Lift Pump Station Treatment Plant | 164 114 3,311 | 164 126 6,612 |
| Sub Total | 3,589 | 6,902 |
| 2) JSSP Project Area Sewer line | 2 4 | 76 |
| Total | 3,589 | 6,978 |

Table 6.18 Breakdown of O&M Cost for Central Sewerage Zone

| | : | 1 | ······································ | | | Unit: million | Rp./annum) |
|----|-----------------------|--------------------------|--|-------|----------------------------|---------------|------------|
| | | | 2,000 | | | 2,010 | m . 1 |
| | Year Item | Quantity | Unit Cost | Total | Quantity | Unit Cost | Total |
| Α. | F/S Area | | | | | | |
| | (1) Sewer line | | | | | | |
| | Secondary/tertiary | 460,000 m | 300 Rp./m | 138 | 460,000 m | 300 Rp./m | 138 |
| | Main pipe | 59,955 m | 300 Rp./m | 18 | 59,955 m | 300 Rp./m | 18 |
| | Trunk/force main | 18,030 m | 300 Rp./m | 5 | 18,030 m | 300 Rp./m | 5 |
| | Conveyance | 10,340 m | 300 Rp./m | 3 | 10,340 m | 300 Rp./m | 3 |
| | Sub total | | | 164 | | | 164 |
| | (2) Lift Pump Station | | | | | | |
| | Electricity | 792,000 k w h | 100 Rp./kwh | 79 | 905,000 | 100 Rp./kwh | 91 |
| | Repairing | 1 ls | - | 26 | 1 ls | - | 26 |
| | Personnel expenditure | 1 ls | - | 9 | 1 ls | - | 9 |
| | Sub total | - | | 114 | | | 126 |
| | (3) Treatment Plant | | | | | | |
| | Electricity | 30.5x10 ⁶ kwh | 100 Rp./kwh | 3,050 | 56.0x10 ⁶ k w h | 100 Rp./kwh | 5,600 |
| | Chemicals | 1 ls | - | - | 1 ls | - | 614 |
| | Repairing | 1 ls | - (| 171 | 1 ls | - | 288 |
| | Personnel expenditure | 1 ls | - | 90 | 1 1s | - | 110 |
| | Sub total | | | 3,311 | | | 6,612 |
| | Total of A | | | 3,589 | | | 6,902 |
| В. | ISSP Area | | | | | | |
| | Secondary/tertiary | - | - | | 126,000 m | 300 Rp./m | 38 |
| | Main/trunk | - | - | | 59,000 m | 300 Rp./m | 18 |
| | Pump Station | - | - | | 1 ls | | 20 |
| | Sub total | | | | | | 76 |
| | TOTAL | | | 3,589 | | | 6,978 |





Chapter 7 IMPLEMENTATION PROGRAMME

7.1 Project Phasing

7.1.1 General

The proposed sewerage system is planned to not only serve 1,659,000 people in the Project Area but also receive the wastewater of 663,900 people in the JSSP area in 2000. It is because the capacity of the existing Setia Budi aerated Iagoon treatment plant is only 34,000m³/d and will be overloaded after 1993. Hence, the wastewater of the JSSP area will be introduced to Pluit Pond aerated Iagoon treatment plant soon after completion of the whole conveyance sewer.

The design wastewater discharge of the Project Area in the year 2000 is estimated at $316,200 \text{ m}^3/\text{d}$. While, that of the JSSP area in 2000 is estimated to be $124,800 \text{ m}^3/\text{d}$.

The proposed sewerage development project will be implemented in two (2) phases since it requires a large cost of Rp. 445.3 billion at 1990 price and a long construction period of eight (8) years. The first phase will be completed in 1996. The second phase will subsequently be implemented to complete in 2000.

The following two (2) alternative plans are considered for phasing the Project.

- (1) The whole distance of the conveyance sewer will be completed along with the collection sewers of some areas located along the conveyance sewer, in the first phase. This plan is in expectation of a high cost recovery of the conveyance sewer construction cost by potential connection to high rise buildings located along the route of conveyance sewer. Moreover, this plan is of advantage to early settlement of the overload of the Setia Budi treatment plant.
- (2) The proposed project will be developed from the lowermost area towards upstream in accordance with normal implementation

method. Sewerage system of the lower part area will be completed in the first phase.

Based on the above considerations, two (2) alternative plans for the first phase are compared in the following sections.

7.1.2 Alternative Plan A

The whole conveyance sewer of 10.3 km with a diameter ranging from 1,900 mm to 2,900 mm will be completed.

In the Project Area, 920 ha or 24% of the total area located along the conveyance sewer is covered by sewerage system. As a result, 350,100 person and 112 high rise buildings (building with more than four (4) floors) will receive sewerage service in 1997. Among the 112 high rise buildings, 62 are existing and the remaining 50 are expected to be built until 1997. The service area according to the first phase program is shown in Fig. 7.1.

The wastewater discharge of the first phase of this Alternative Plan A, that enter the sewer system from the Project Area is estimated to be 63,990 m³/d. This wastewater discharge is 20% of the design discharge (including groundwater infiltration) in the year 2000.

While in the JSSP Area, wastewater of 645,600 person and 64 high rise buildings will be collected by the JSSP sewerage system in 1997. Among the 64 high rise buildings, 55 are existing and the remaining 9 are future ones.

The collected wastewater discharge of 121,400 m³/d will be introduced to the Pluit Pond treatment plant through the completed conveyance sewer in 1997 when the first phase project will start operation.

Hence, the first phase system of this Alternative Plan A will serve 995,700 person and treat the wastewater of 185,390 m³/d in total in 1997. As a result, pollution load of 30,170 kg/d as BOD will be reduced.

Half of the total inflow pump capacity, whole aerated lagoon structures and 10 units of aerator will be installed in the first phase.

Total direct construction cost of the first phase plan is estimated at Rp. 204.5 billion with its break-down of collection system of Rp. 64.7 billion, conveyance sewer of Rp. 117.0 billion and treatment plant of Rp. 22.8 billion.

Construction cost per population served, wastewater discharge treated and the construction cost per unit pollution load reduction are respectively Rp. 205,000/person, Rp. 1.10 million m³/d and Rp. 6.8 million/kg·BOD/d.

Construction cost recovery by Alternative Plan A of first phase project by high rise buildings is estimated based on the following assumptions.

- The maximum affordable cost of a high rise building is equivalent to the construction cost of substitutional individual treatment facility.
- All high rise buildings which will be built in the future (59 buildings) bear the cost equivalent to substitutional individual facility.
- 70% of the existing high rise building which are equipped with only toilet waste treatment units (117 x 0.7 = 82 buildings) bear the cost under the same condition on that of future buildings.
- 25% of the existing high rise building which are not equipped with proper individual treatment system (117 x 0.25 = 29 buildings) bear 50% of the cost of that of substitutional facility.

The total construction cost recovery by high rise buildings by Alternative Plan A (first phase) is estimated at Rp. 22.0 billion at 1990 Price.

7.1.3 Alternative Plan B

The conveyance sewer will be extended over 6.4 km towards Pluit Pond from upstream. The downstream project area of 1,824 ha, which can be connected to this conveyance sewer or 47% of the total area, will be covered by sewerage system in the first phase. As a result, 910,700 person and 59 high rise buildings will be served in 1997. Among the 59 high rise buildings, the existing ones account for 33 and future ones 26. The

sewerage service area according to the first phase project of alternative Plan B is shown in Fig. 7.2.

Wastewater of 172,360 m³/d or 55% of the total design wastewater in the year 2000 will be collected and treated by the first phase project, resulting in reduction of BOD load of 27,590 kg/d.

In case of Alternative Plan B, no wastewater is introduced from the JSSP area before completion of the second phase in 2000.

In the first phase, half of the total pump capacity, whole aerated lagoon structures and 10 units of aerator will be constructed.

Total direct construction cost of the first phase Alternative Plan B is estimated to be Rp. 206.1 billion. It is broken down into collection system of Rp. 106.6 billion, conveyance sewer of Rp. 76.7 billion and treatment plant of 22.8 billion.

Construction cost per population served, wastewater discharge treated and the construction cost per unit pollution load reduction is estimated to be respectively, Rp. 226,000/person, Rp. 1.20 million/m³/d and Rp. 7.5 million/kg·BOD/d.

The total construction cost recovery by high rise buildings for this Alternative Plan B (first phase) is estimated to be Rp. 7.4 billion at 1990 price in the same manner as Alternative Plan A.

7.1.4 Comparative Evaluation

The above two (2) alternative plans are comparatively evaluated as follows. For details, refer to Table 7.1.

| | Alternative A | Alternative B |
|--|---------------|---------------|
| Construction Cost (Rp. billion) | 204.5 | 206.1 |
| Served Area (ha) | 2,758 | 1,824 |
| Served Population | 995,700 | 910,700 |
| Treated Wastewater Discharge (m ³ /d) | 185,390 | 172,360 |
| BOD Load Reduction (kg/d) | 30,170 | 27,590 |
| Const. Cost per Served | 205,000 | 226,000 |
| Population (Rp./person) | | |
| Const. Cost per Treated Wastewater | 1.10 | 1.20 |
| Discharge (Rp. million/m ³ /d) | | |
| Construction Cost per Unit | 6.8 | 7.5 |
| BOD Load Reduction (Rp. million/kg/d) | | |
| Recovery Cost of High Rise Building | 22.0 | 7.4 |
| (Rp. billion) | | |

As evident from the above table, Alternative Plan A is more economically efficient than Alternative Plan B.

Furthermore, Alternative Plan A has the following noticeable advantages.

(1) Pollution load reduction in the upper reaches contributed to river water quality improvement more than that in the lower reaches even when the amount of reduction remains the same.

Both alternative plans treat almost the same amount of wastewater. The sewage collection area of Alternative Plan A extends from the lowermost reaches of the Project Area to JSSP area, while that of Alternative Plan B is limited to the lower reaches of the Project Area.

Hence, contribution to river water quality improvement of Alternative Plan A is higher than that of Alternative Plan B.

(2) The whole conveyance sewer will be completed in 1996 in case of Alternative A, four (4) years earlier than in case of Alternative B. Hence, the overload of the existing Setia Budi aerated lagoon treatment plant will be settled earlier.

The following volume of wastewater in the JSSP area will be discharged into the Banjir Canal with no treatment before full completion of the conveyance sewer.

Alternative Plan A: discharge pollution load of 18,637 ton as BOD during 1994 - 1996

Alternative Plan B: discharge pollution load of 44,355 ton as BOD during 1994 - 2000

- (3) The first phase project of Alternative Plan A covers the most important institutional and commercial areas of Jakarta city.
- (4) Once the whole conveyance sewer is completed, collection system can be optionally developed areawise and timewise.

7.2 Implementation Programme

The proposed sewerage development project will be completed within nine (9) years from 1992 to 2000. The implementation programme of the project is prepared based on the phasing policy discussed in the previous Section 7.1 and conforms to Alternative Plan A, as follows.

The first phase project will be completed within five (5) years from 1992 to 1996. The detailed design will be completed in 1992. The construction works will be commenced in 1993 and be accomplished in 1996. The included major construction works are:

- whole conveyance sewer of 10.34 km
- sewerage collection system of 920 ha in Sub-zones A, B, C, D and E along the area of conveyance sewer
- connection pipe between JSSP area and conveyance sewer (0.5 km)
- half capacity of inflow pump station (218m³/min.)
- open ditch connecting inflow pump station and aerated lagoon
- aerated lagoon structure including embankment, excavation, etc.
- 10 units of aerator

The second phase project will be completed within five (5) years from 1996 to 2000. The detailed design will be accomplished in 1996. The construction works will be commenced in 1997 and be completed in 2000. The included major construction works are:

- sewerage collection system of 2,927 ha in Sub-zone A, B, C, D, E, F and G
- lift pump station (63.1 m³/min.)
- remaining capacity of inflow pump (256m³/min.)
- remaining aerators (14 units)
- dying bcd $(3,000m^2)$

The proposed implementation programme is shown in Table 7.2.

7.3 Disbursement Schedule

The proposed disbursement schedule of the project cost is shown in Table 7.3.

Table 7.1 Alternatives of Project Phasing

Alternative A : Sewerage Development by Conveyance Sewer Precedence

| disch | 11/.1 | 22.8 | 02.4 | 1 1 1 | | | 230 1,838 2,758 |
|-------|-------|-------------|-------------------------------------|------------|-------------------------------|-------------------|-----------------------|
| Cons | 117.1 | 22.8 | 62.4 | 31.9 | 33,760 | 179,800 | 330 |
| | 25.9 | ı | 14.3 | 11.6 | 10,080 | 50,000 | 170 |
| | 24.6 | ı | 15.5 | 9.1 | 3,560 | 16,900 | 150 |
| Cons | 14.3 | ı | 11.4 | | 5,410 | 45,000 | 9 |
| | 22.6 | . 1 | 13.4 | 2.6 | 11,180 | 58,400 | 210 |
| | | Plant | sewer | | (m3/d) System | (person) | |
| | Total | Freatment | Collection Conveyance Treatment | Collection | Discharge | Area (ha) in 1997 | rea (ha) |
| | | o. billion) | Construction Cost (Rp. billion) | | Covered Population Wastewater | Population | overed |

Construction cost per served population:

Rp.205,000/person

Construction cost per wastewater
discharge: Rp. 1.10 million

Alternative B : Sewerage Development from Downstream Most

| | Covered | Population | Covered Population Wastewater | | Construction Cost (Rp. billion) | (p. billion) | | |
|-------|-----------|------------|-------------------------------|------------|---------------------------------------|--------------|-------|-------|
| ···· | Area (ha) | in 1997 | Area (ha) in 1997 Discharge | Collection | Collection Conveyance Treatment Total | Treatment | Total | |
| | | (person) | (m3/d) System | System | sewer | Plant | | |
| Ą | - | - | - | ŀ | • | • | • | |
| ф | 1 | 1 | 1 | 1 | • | 1 | ı | Cons |
| ບ | 1 | 1 | 1 | ı | ı | • | ı | |
| Ω | 331 | 97,300 | 19,630 | 22.5 | 14.3 | 1 | 36.8 | |
| m | 1,493 | 813,400 | 152,730 | 84.1 | 62.4 | 22.8 | 169.3 | Cons |
| 阡 | 1 | ı | • | t | • | 1 | 1 | discl |
| Ö | ı | ı | ı | 1 | , | ţ | ı | |
| JSSP | _ | • | ŀ | | 1 | - | - | ٠ |
| Total | 1,824 | 910,700 | 172,360 | 106.6 | 76.7 | 22.8 | 206.1 | |

Construction cost per served population: Rp.226,000/person

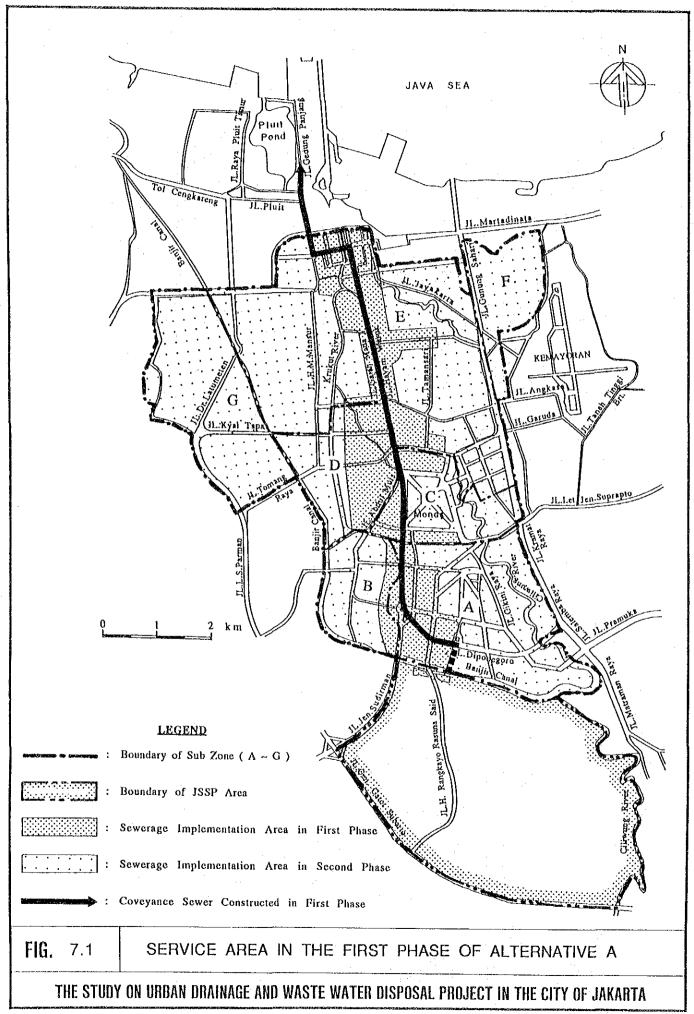
Construction cost per wastewater ischarge : Rp.1.20 million

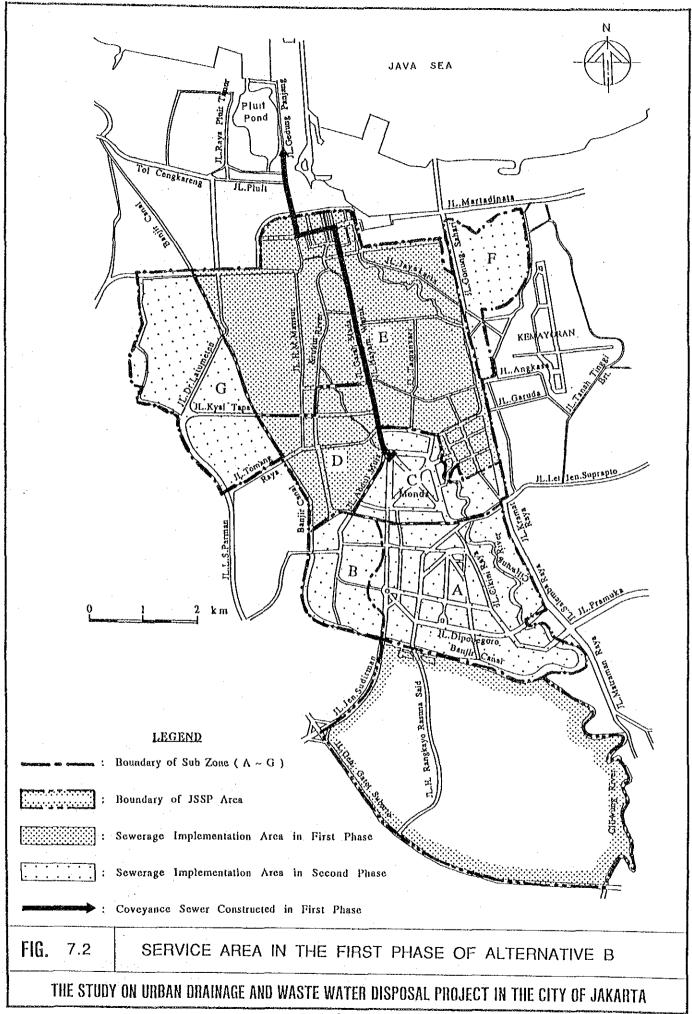
Table 7.2 Implementation Programme of Sewerage Development

| Year | 1992 | 1993 | 1994 | 1995 | 1996 | 1997 | 1998 | 1999 | 2000 |
|-------------------------------|------|---------------------------------------|---------------------------------------|---------------------------------------|----------------------------|-------|----------------------------|----------------------------|--|
| Construction | | | <u>.</u> | | | | | | |
| (1) Collection System | | | | • | | | | | |
| Sub-Zone A | | | | , . | ***** | | | ***** | **** |
| Sub-Zone B | | | | | * * * | | | ***** | ***** |
| Sub-Zone C | | | | * * * * | * * * | | | ***** | |
| Sub-Zone D | | | | **** | | | **** | | |
| Sub-Zone E | | | **** | ***** | | ***** | **** | ***** | |
| Sub-Zone F | | | | | | | | *** | **** |
| Sub-Zone G | | | | | | | | * * * | **** |
| (2) Conveyance Sewer S1-S9 | | * * * * * * | ************** | * * * * * * * * * * * * * * * * * * * | * * * * * * | | | | A THE STREET AND A |
| (3) Treatment Plant | | • | | | | | | | |
| Inflow Pump Aerated Lagoon | | * * * * * * * * * * * * * * * * * * * | * * * * * * * * * * * * * * * * * * * | | | | * * * * * * | * * * * * * | * * * * * * * |
| Others | | | | | *** | | | | |
| (4) Detailed Design | | | | | | | | | |
| (5) Supervision | | * * | * * | * * | - * * * | * | * | * | * * |

Table 7.3 Disbursement Schedule

| | | | | | | | | צ | Unit: Ro. | billion) |
|-------------------------|---|------|------|------|------|------|------|------|-----------|----------|
| Year | r 1992 | 1993 | 1994 | 1995 | 1996 | 1997 | 1998 | 1999 | 2000 | Total |
| 1. Direct Cost | | 29.2 | 49.1 | 8.99 | 55.3 | 19.0 | 30.3 | 65.0 | 9.09 | 375.3 |
| (1) Collection System | | | | | - | | | | | |
| Collection Sewer | | | 15.0 | 32.6 | 17.1 | 17.0 | 27.9 | 62.0 | 46.2 | 217.8 |
| Lift Pump Station | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | | | | | | | | 4.1 | 4.1 |
| (2) Conveyance Sewer | | 29.2 | 29.2 | 29.2 | 29.4 | | | | | 117.0 |
| (3) Treatment Plant | | | 4.9 | 5.0 | 8.8 | 2.0 | 2.4 | 3.0 | 10.3 | 36.4 |
| (4) Connection Pipe | | | | | | | | | | |
| 2. Land Acquisition & | 9.0 | | , | | , | | | | | 9.0 |
| Compensation | | | | | | | | | | |
| 3. Administration Cost | 0.6 | 9.0 | 9.0 | 9.0 | 0.0 | 9.0 | 0.6 | 0.7 | 0.7 | 5.6 |
| 4. Engineering Services | 8.3 | 2.0 | 2.0 | 2.0 | 6.0 | 1.5 | 1.5 | 1.5 | 1.5 | 26.3 |
| 5. Physical Contingency | | 2.0 | 4 | 6.7 | ν. | 1.9 | 3.0 | 7 | 6.1 | 37.5 |
| 1 | | | | | | | | | | |
| 6. Total | 9.5 | 34.7 | 56.6 | 76.1 | 67.4 | 23.0 | 35.4 | 73.7 | 68.9 | 445.3 |





Chapter 8. ECONOMIC, SOCIAL AND ENVIRONMENTAL EVALUATION

8.1 Nos. of Beneficiaries

Beneficiaries of sewerage services were divided into 11 categories of houses, shops, factories, hotels, restaurants, hospitals, offices, schools, religious institutions, others and high rise buildings.

Shops, factories, hotels, restaurants, private hospitals, private offices and some of "others" and "high rise buildings" can be classified as commercial establishments, while public hospitals, government offices, schools, religious institutions and some of "others" and "high rise buildings" can be categorized as social institutions.

A high rise building is defined as a building having more than four (4) stories, the categorization of which became necessary as it would be a potential source of revenue for sewerage enterprise.

The total number of beneficiaries of sewerage across the entire categories in the Project Area works out to 203,818 for 1988 and 225,773 for 2000, of which houses account for respectively 95% and 91.5% in 1988 and 2000. Shops and offices occupy the second and third places respectively with the share of 2.0% and 1.0% for 1988, and 3.7% and 1.7% for 2000. High rise buildings account for 0.04% (89 buildings) for 1988, and 0.08% (264 buildings) for 2000.

The number of commercial establishments is expected to grow by 95.2% from 6,752 in 1988 to 13,180 in 2000. Likewise, the number of social institutions is expected to increase by 68.9% from 3,494 to 5,000 and that of houses by 12.4% from 193,572 in 1988 to 217,627 in 2000.

However by the year 2000 as the JSSP Area would also be integrated into the Project Area, the total number of beneficiaries of all categories would expand to 312,147 by the year 2000 and 338,605 by the year 2010. Out of these, houses account for 93% in 2000 and 91% in 2010, shops for 3.2% in 2000 and 4.1% in 2010, and offices for 1.3% in 2000 and 1.6% in 2010.

The number of properties in the Poject Area and in the JSSP Area on a Kelurahan basis is shown in Table 8.1.

8.2 Reduction of Pollution Load

8.2.1 Existing and Future Pollution Load without Project

(1) Existing Pollution Load

Existing pollution load as BOD in the Project Area is estimated to be 44,572 kg/d, and the breakdown is shown in Table 8.2.

The share of pollution load from resident is 71.3%, from commerce and institution is 24.8% and from industry is 3.9%. The total pollution load from resident is 31,762 kg·BOD/d, of which toilet waste accounts for 4,690 kg·BOD/day and gray water the remaining 27,072 kg·BOD/d.

(2) Future Pollution Load

Future pollution load discharge of the Project Area without project in the year 2000 is estimated to be 59,145 kg·BOD/d under the following assumptions.

- The ratio of sanitary disposal of toilet waste by households in septic tank/leaching systems remains the same as existing conditions with a 74%.
- Gray water, commercial and institutional wastewater and industrial wastewater are discharged to the public water bodies under the same conditions as existing.

Future pollution load as BOD, discharged from each pollution sources are as follows:

| · · · · · · · · · · · · · · · · · · · | <u> </u> | · | (Unit: k | g·BOD/d) |
|---------------------------------------|------------|---------------|----------|----------|
| Domestic | Waste | Commercial & | | |
| Toilet Waste | Gray Water | Institutional | Industry | Total |
| 4,852 | 33,663 | 19,016 | 1,614 | 59,145 |

8.2.2 Reduction of Pollution Load by Sewerage Development

Reduction of pollution load by sewerage development is estimated assuming the following conditions.

- All domestic and commercial and institutional wastewater in conventional sewerage areas are collected, treated and discharged to the rivers and canals nearby treatment plant with a BOD of 30 mg/l.
- All gray water discharged by domestic, commercial and institutional sources in interceptor area is collected, treated and discharged with a BOD of 30 mg/l.
- All toilet wastewater in interceptor area is treated by on-site sanitation facilities.

Accordingly, it is estimated that the pollution load of 59,145 kg/d as BOD discharged from the Project Area will be treated to 9,486 kg/d by the proposed wastewater treatment plant at Pluit Pond with a net pollution load reduction of 49,659 kg/d. This implies that sewerage development would contribute toward a BOD removal efficiency of 84% in the year 2000. Furthermore, the pollution load of 24,960 kg/d as BOD discharged from JSSP Area in the year 2000 will be also treated to 3,750 kg/d with a reduction of 21,210 kg/d by the proposed Pluit Pond treatment plant. Consequently, the total pollution load reduction by the proposed sewerage system comes to 70,869 kg/d as BOD in the year 2000.

8.3 Reduction of Waterborne Disease

8.3.1 Disease Contraction Ratio

Based on field investigation, questionnaire survey and statistical data of disease contraction, waterborne disease contraction ratio in the project area and JSSP Area is determined to be 72.34 and 40.55 per 1000 person respectively. Moreover, the correlation between water color/smell of river/canal and disease contraction ratio for each area is represented by the following equations:

 $Y_1 = 32.9602 + 0.1907X_1 + 0.82926X_2$ (Project Area) $Y_2 = 27.3599 + 0.1907X_1 + 0.82926X_2$ (JSSP Area)

Where

X₁: Percentage of respondents who replied that water color of near-by rivers/canals was black (%)

X₂: Percentage of respondents who replied that water smell of near-by rivers/canals was strong (%)

Y: No. of those who contracted major water-borne diseases in the last three years (cases/1,000 population)

After the completion of sewerage development, disease contraction ratio of the project area and JSSP area will be reduced to 32.96 and 27.36 per 1000 person respectively.

8.3.2 Medical Costs

Medical costs consist of medication cost and economic losses resulting from unworkable days and death of patient, on the basis of each patient. Medical cost of waterborne disease per patient ranges from Rp. 60,000 for Cholera to Rp. 460,000 for Tuberculosis, with an average of Rp. 229,000 (Ref. Table J.20 of Appendix J, Supporting Report of Master Plan).

Reduction of medical costs is estimated from the difference in the total medical costs between the "without" and "with" project cases.