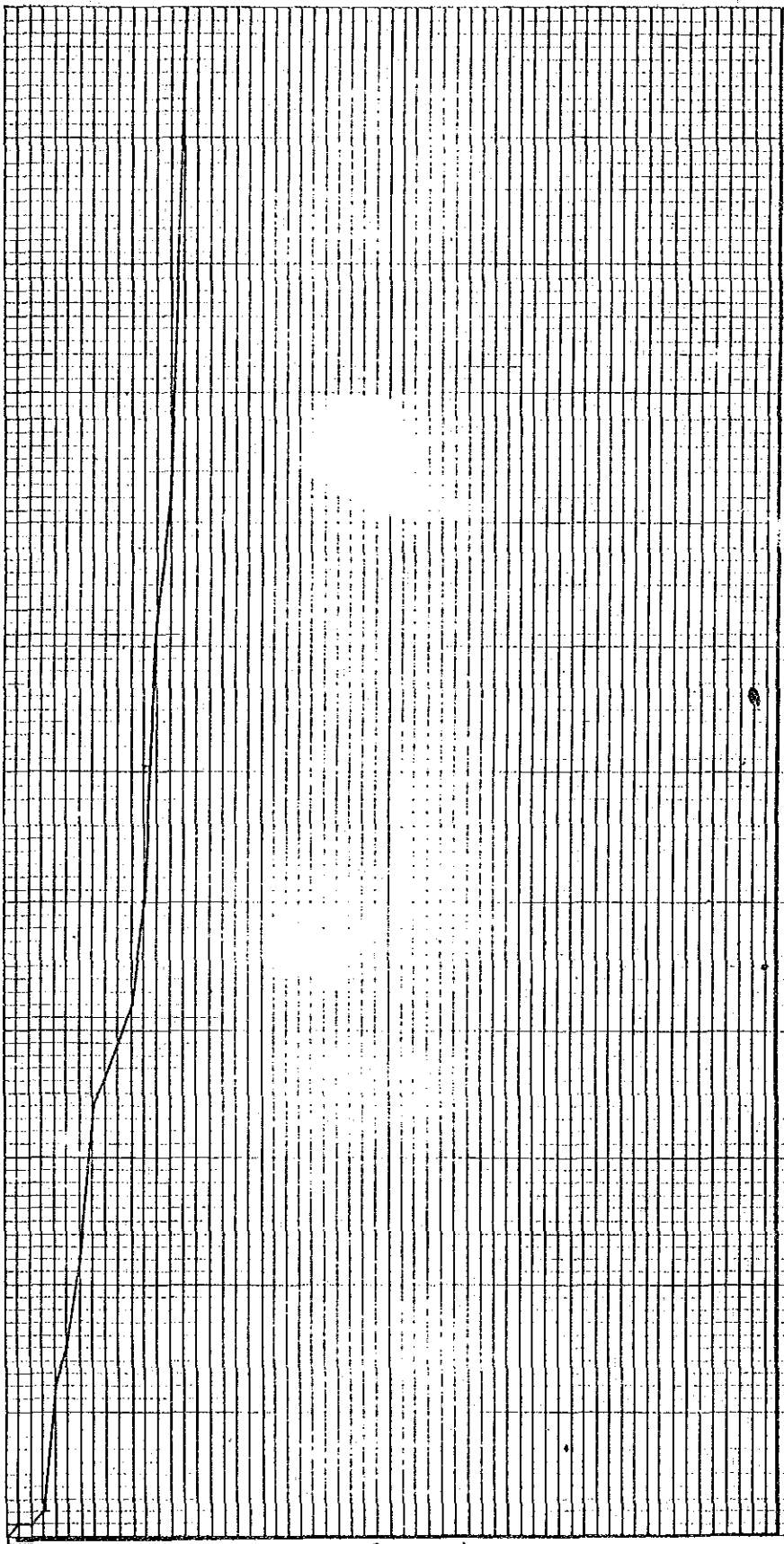


G.L. AP Ltd. L



304

G. 4

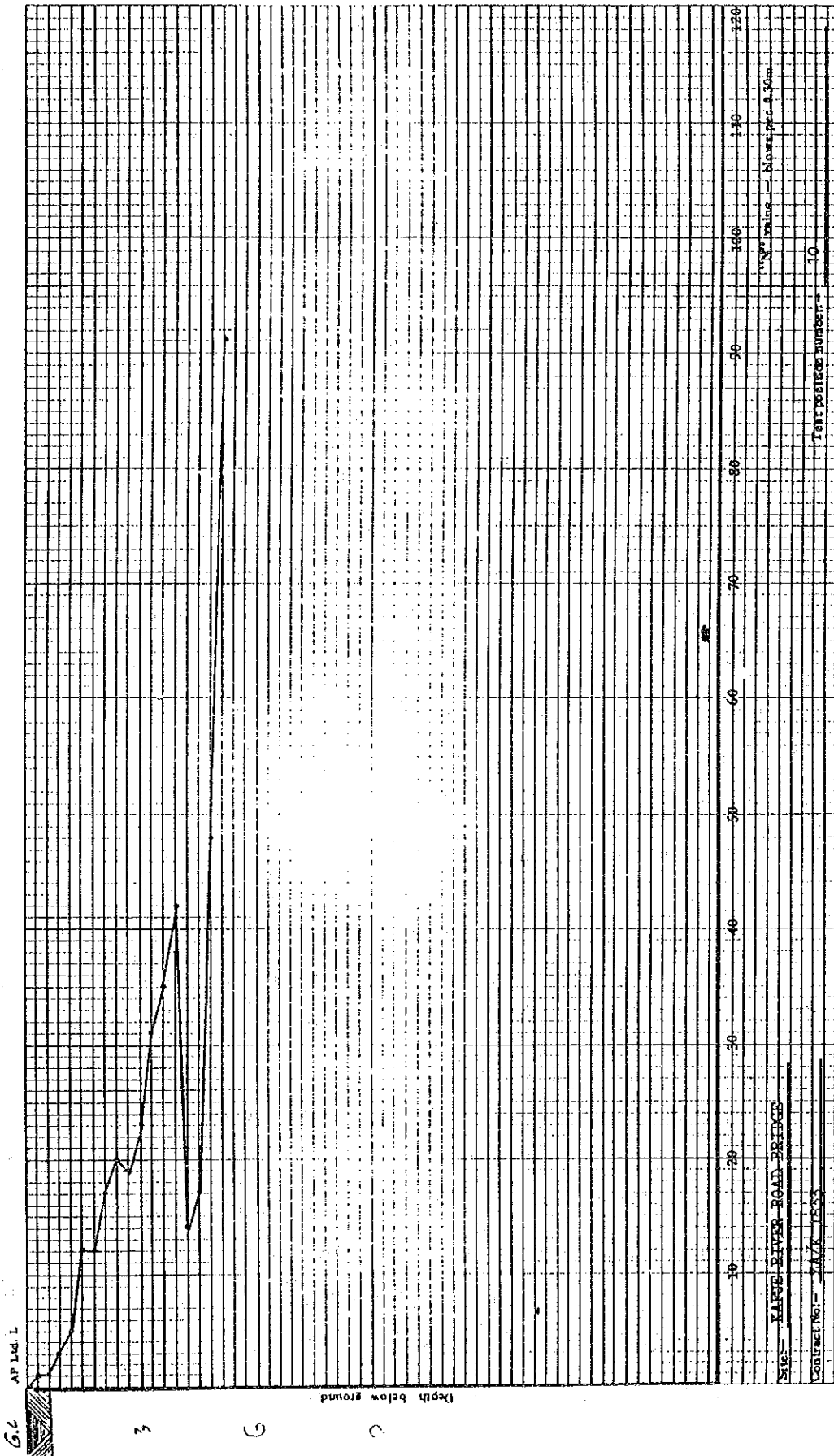
2.1 M

Site - SAFEBEY ROAD BRIDGE
 Test Position Number - 21
 Date 3RD MARCH 1990
 Blow count per 30cm - 100

WADE ADAMS PILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 3RD MARCH 1990



G.L AP Ltd. L

3

6

Site: KAROL RIVER ROAD BRIDGE
 Contract No: 207/1983
 Test position number: 10
 Test value - blows per 30cm

WADE ADAMS PILING AND FOUNDATIONS LTD. Date 3RD MARCH 1990

PENETROMETER TEST REPORT

G.C. AP LRL. L



SITE - EARLE RIVER ROAD BRIDGE

Contract No. - 2875-1877

Test position number -

Test value - blow per A.30m

WADE ADAMSPILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 3RD MARCH 1990

G.C.
AP 100' L



Site - ZARVE RIVER ROAD BRIDGE
 Contract No. - 2176 1855
 Test Position Number - 12

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 3RD MARCH 1990

G.L. AP Ltd. L

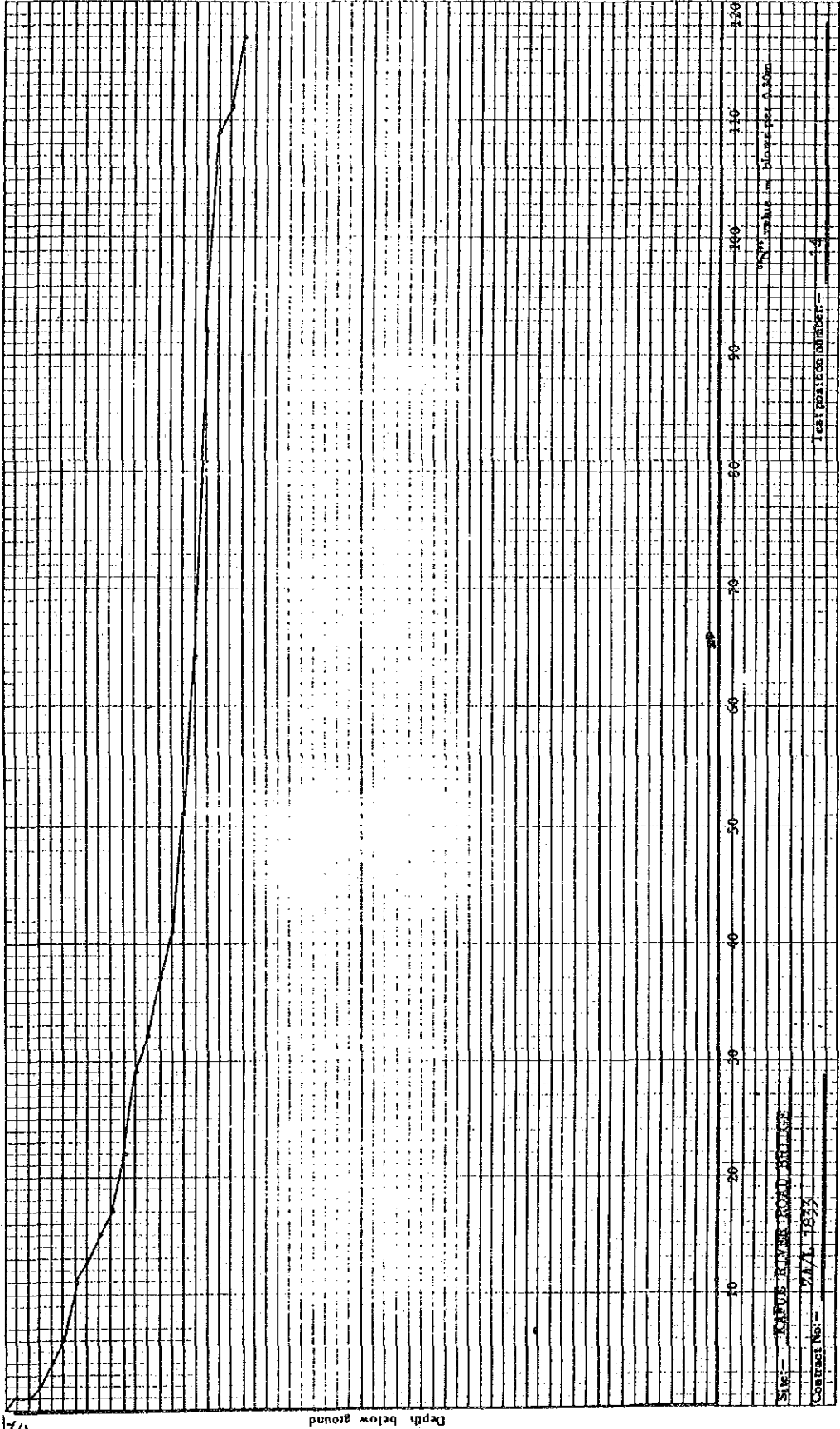


10 20 30 40 50 60 70 80 90 100 110 120
 KARUE ROAD BRIDGE
 Contract No. 72/1.18.55
 Test position number 15
 No. of tests 1
 Allow. test 0.30m

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 4TH MARCH 1990

G.C.

AP 114.1



Depth below ground

SITE - CAVEY RIVER ROAD BRIDGE
 Contract No. - WVA 1833

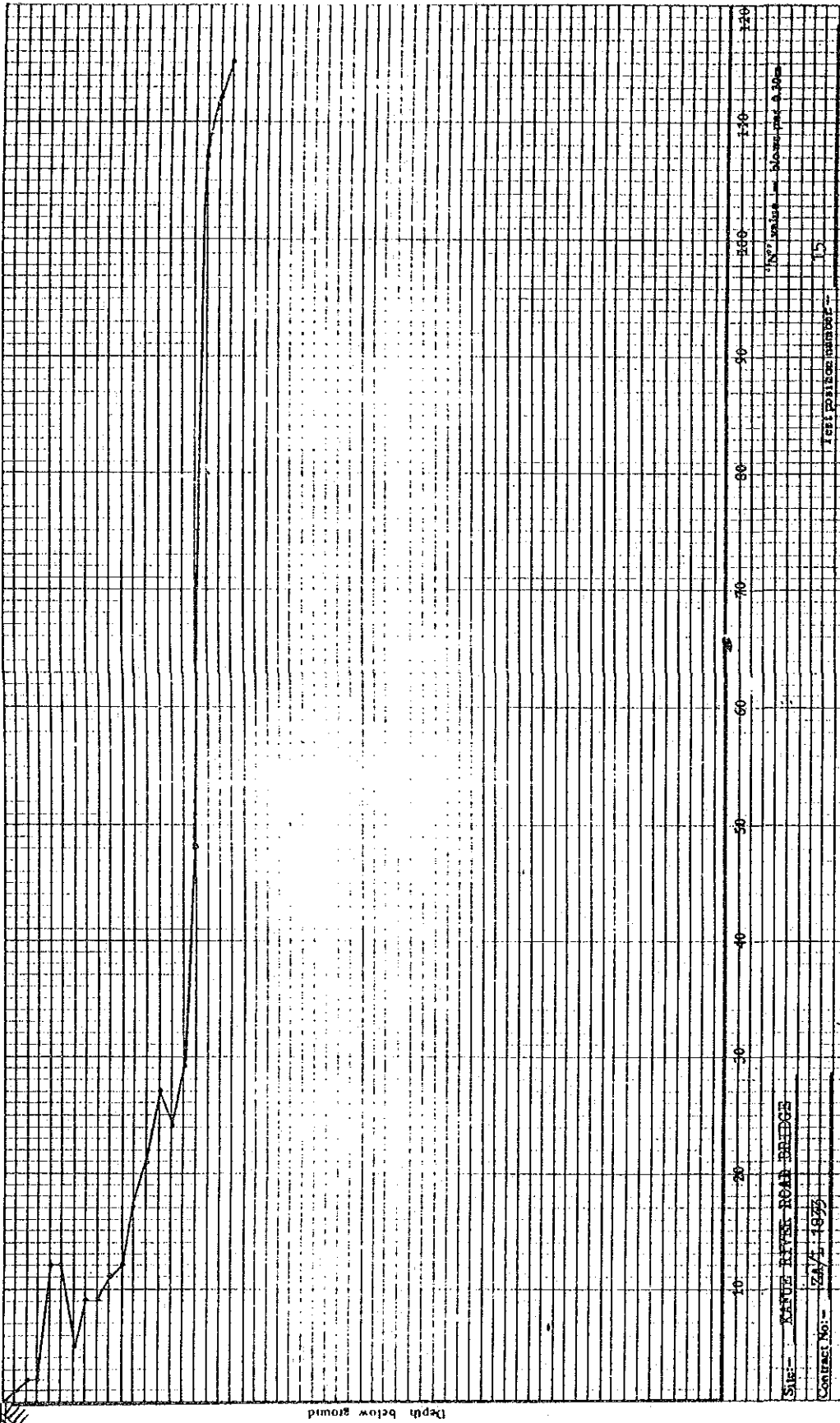
Number of blows per foot - 110
 Test position number - 14

WADE ADAMS PILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 4TH MARCH 1990

G.C. AP Ltd. L



SITE - KATON RIVER ROAD BRIDGE

Contract No. - 7247-1873

TEST POSITION NUMBER - 15

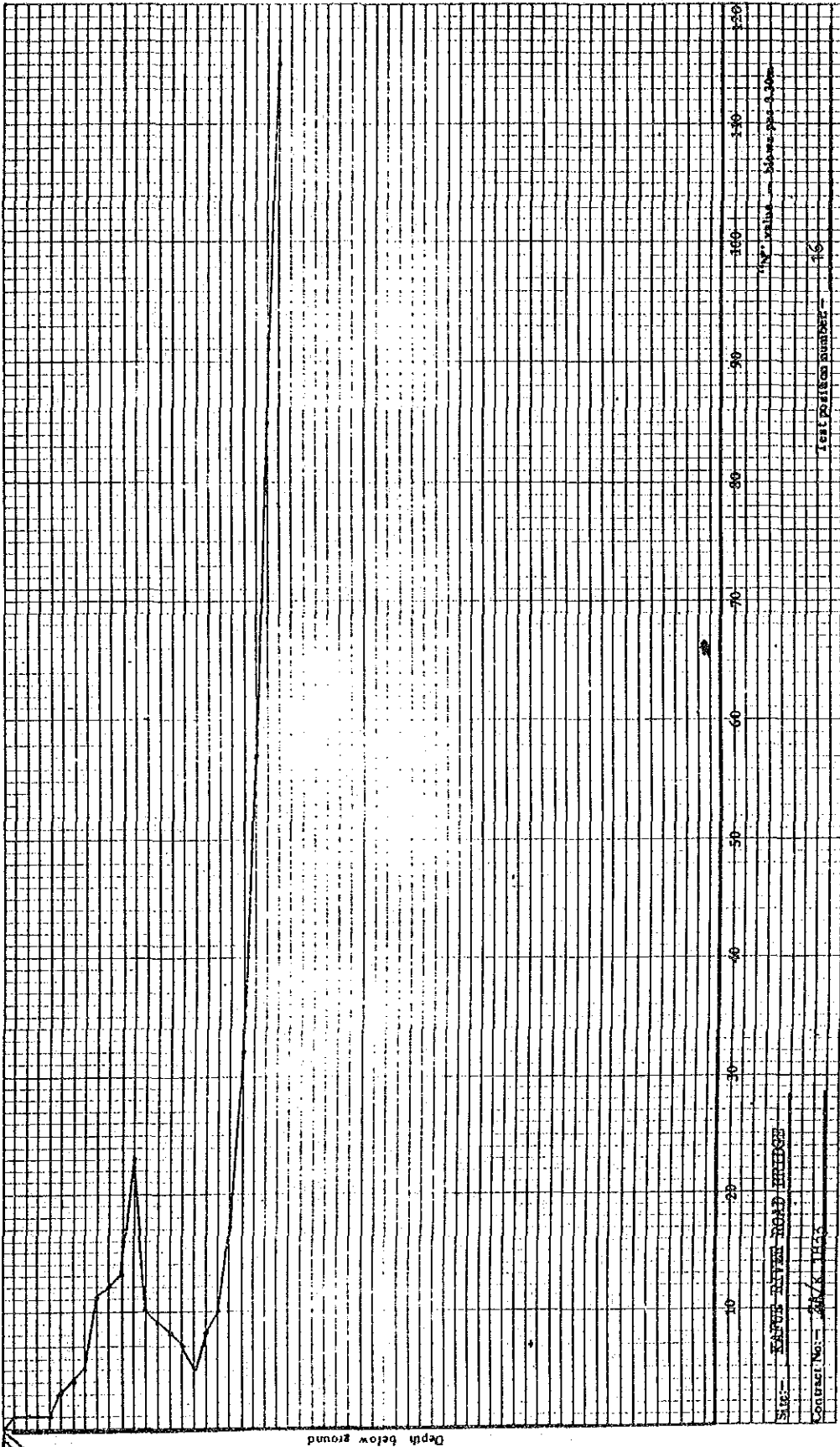
TEST VALUE - 1500 lbs/sq ft

WADE ADAMS PILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 4TH MARCH 1990

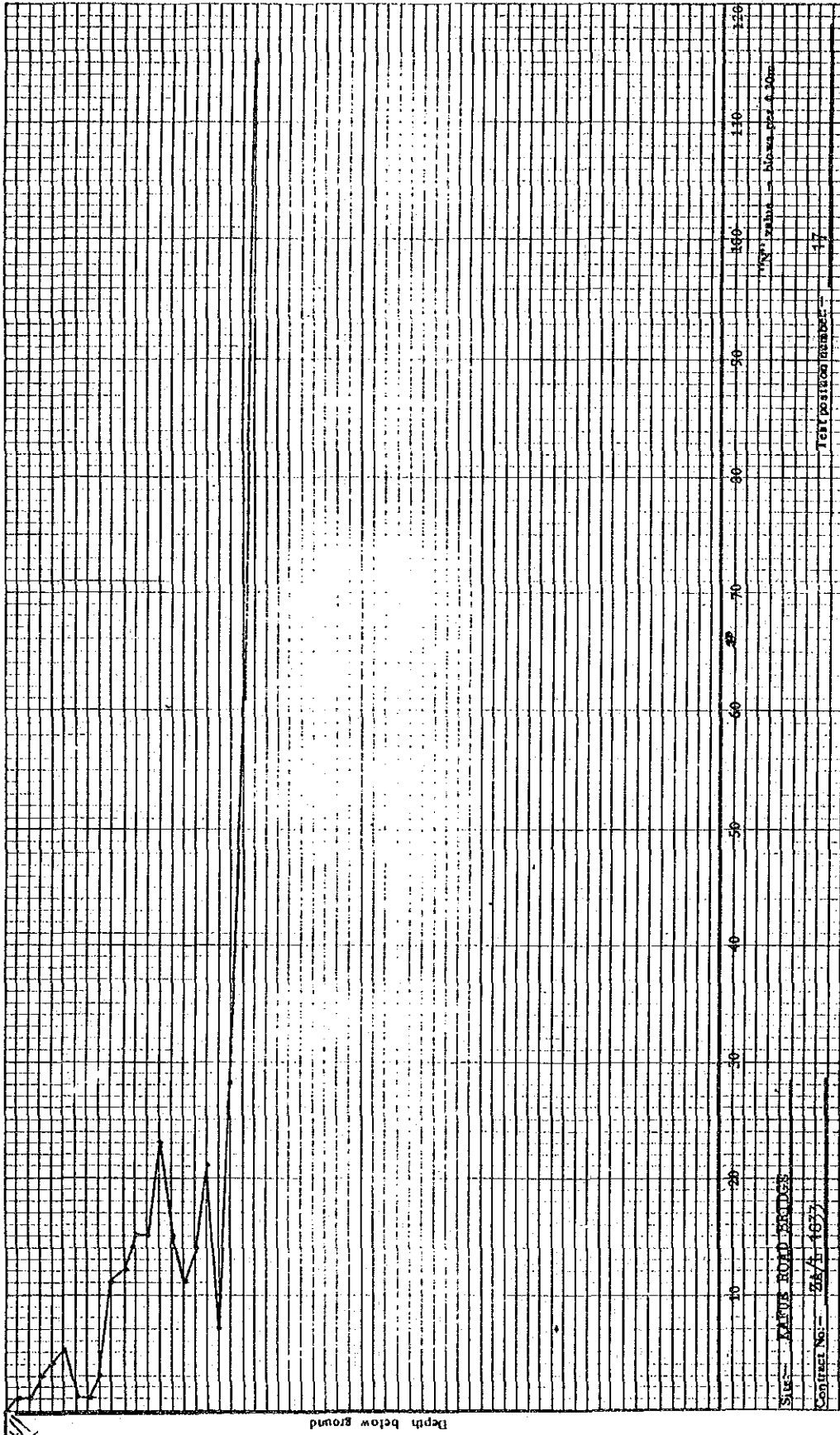
G.C. AP Ltd. L



SITE: FAIRFAX RIVER ROAD BRIDGE
 Contract No.: 2475 1143
 test position number: 16

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 4TH MARCH 1990

GC. AP LG. I.



SLUG - WHITE ROAD BRIDGES

CONTRACT NO. - 5A/5-1637

TEST POSITION NUMBER - 17

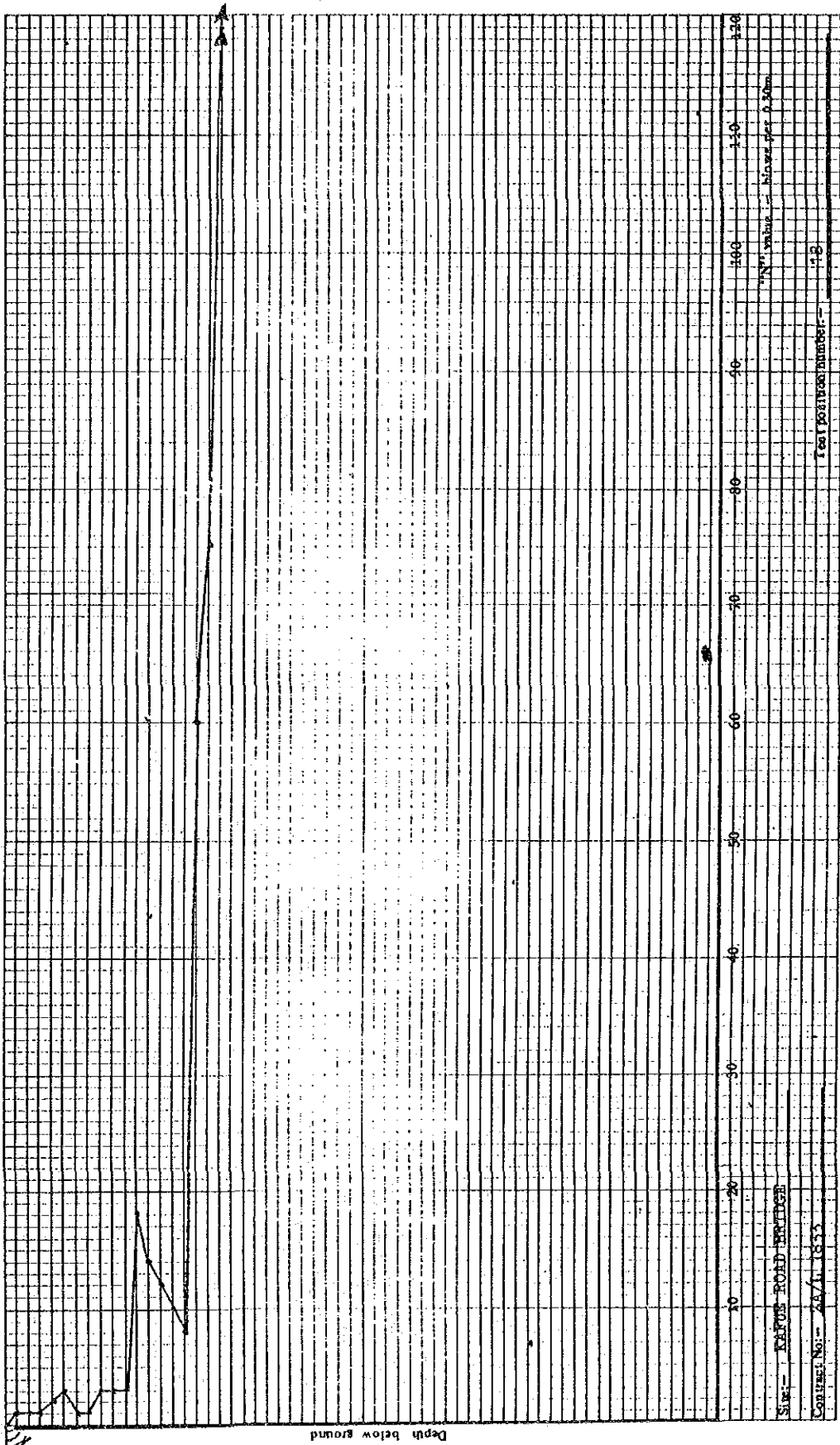
WATER VALUE - NONE PER 4.00m

WADE ADAMS PILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 5TH MARCH 1990

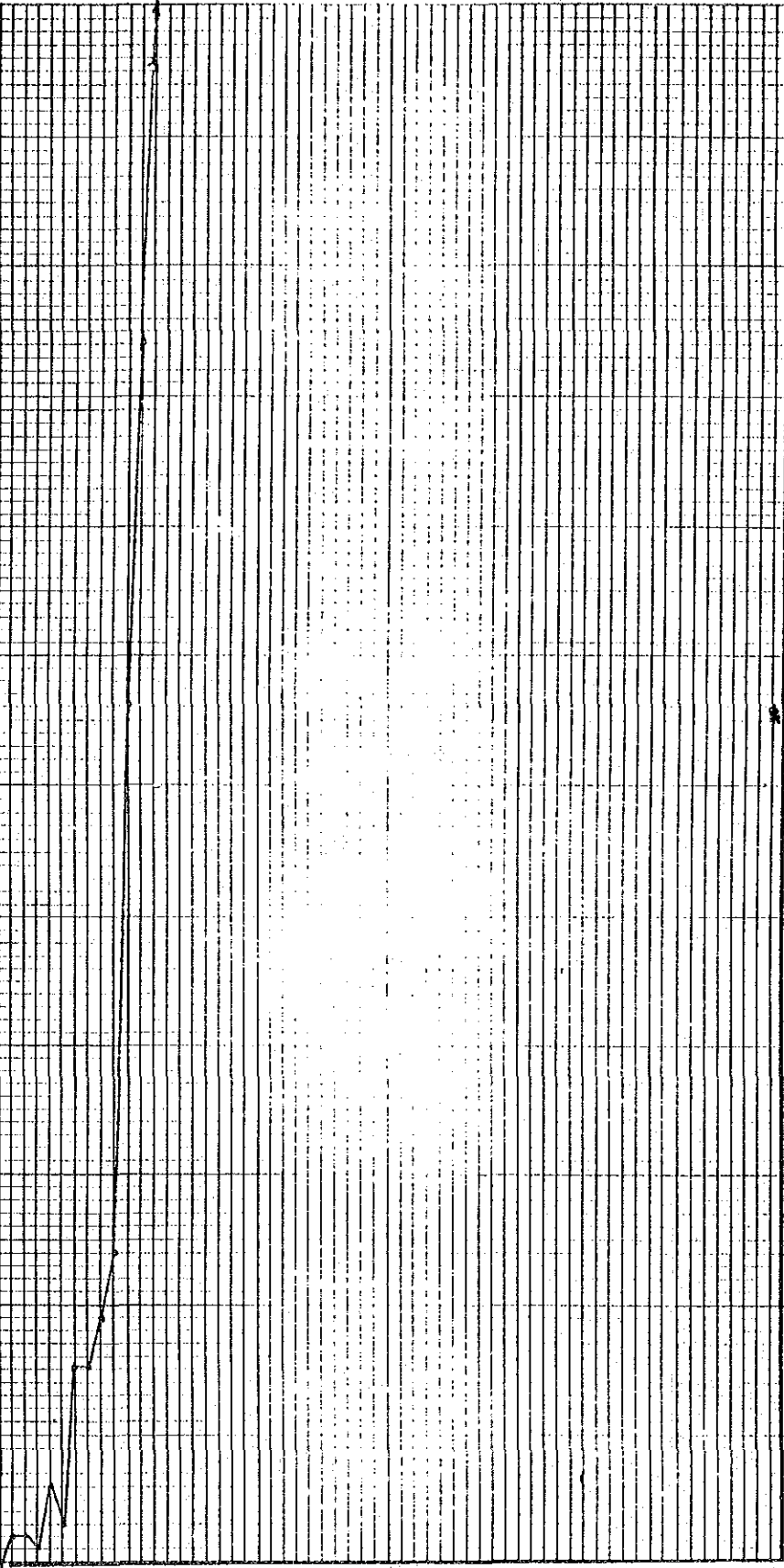
Ca AP Ltd. L



Site: KATON ROAD BRIDGE
 Contract No.: 24/11/1855
 Test position number: 113

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 5TH MARCH 1990

G.C. AP LMB. L



Blows per foot

10 20 30 40 50 60 70 80 90 100 110 120

10 20 30 40 50 60 70 80 90 100 110 120

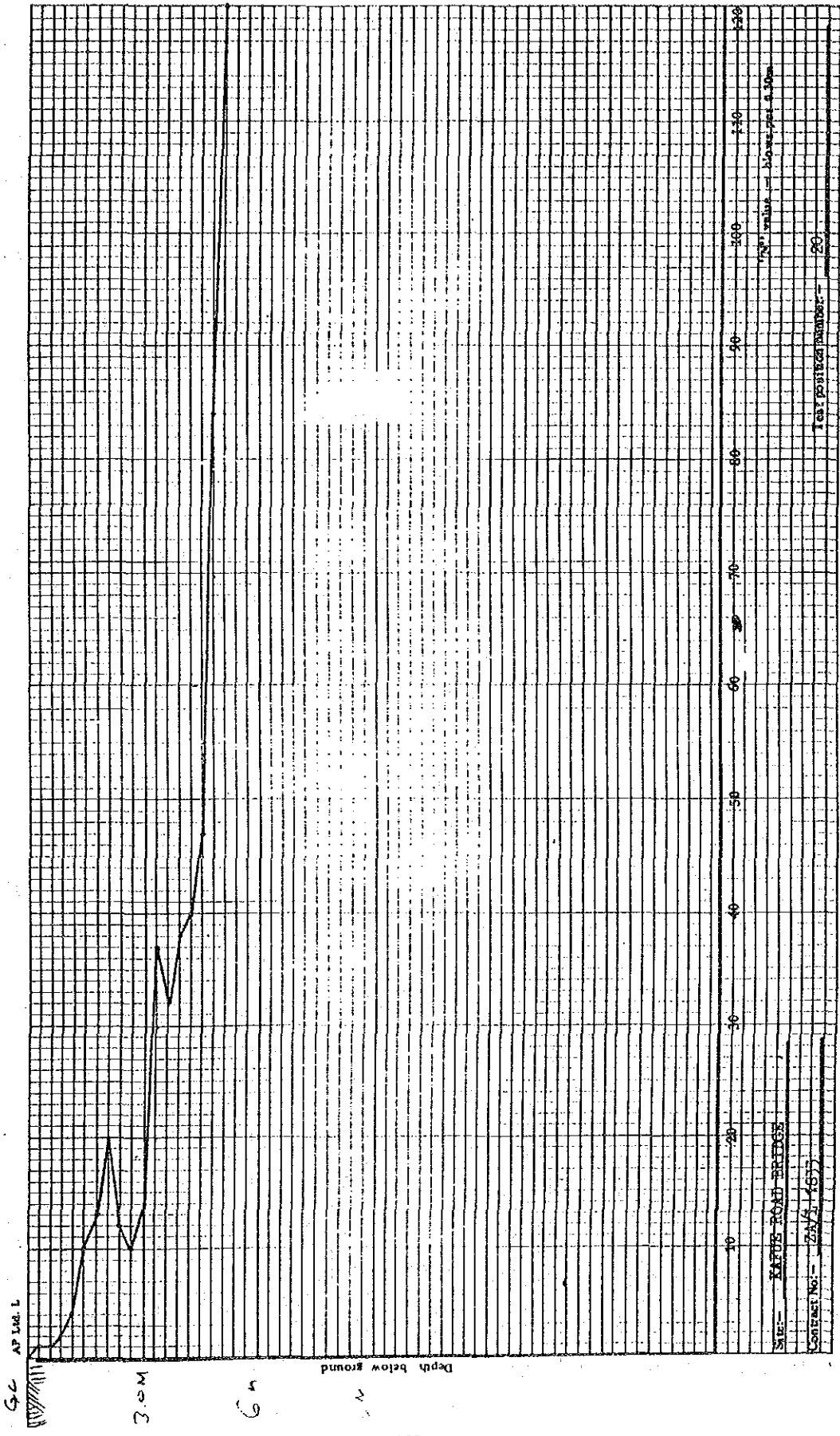
Site - KARVE ROAD BRIDGE

Contract No. - 24/1/855

Test position number - 19

Blows per foot

WADE ADAMS PILING AND FOUNDATIONS LTD. **PENETROMETER TEST REPORT** Date 5TH MARCH 1990



Date 1ST MARCH 1990

PENETROMETER TEST REPORT

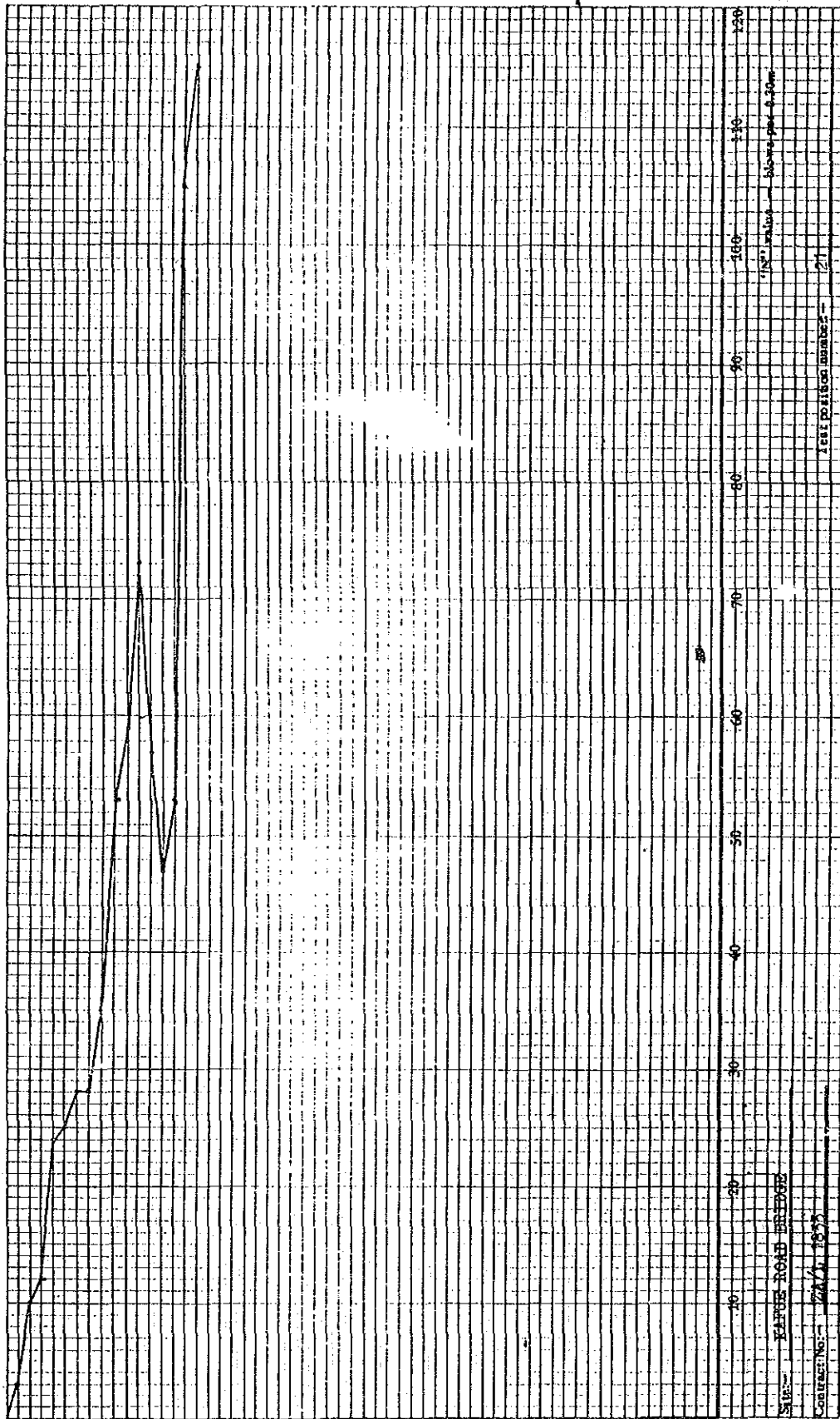
WADE ADAMS PILING AND FOUNDATIONS LTD.

G.C. AP Ltd. L

3.4

5.2

1/2" depth below ground



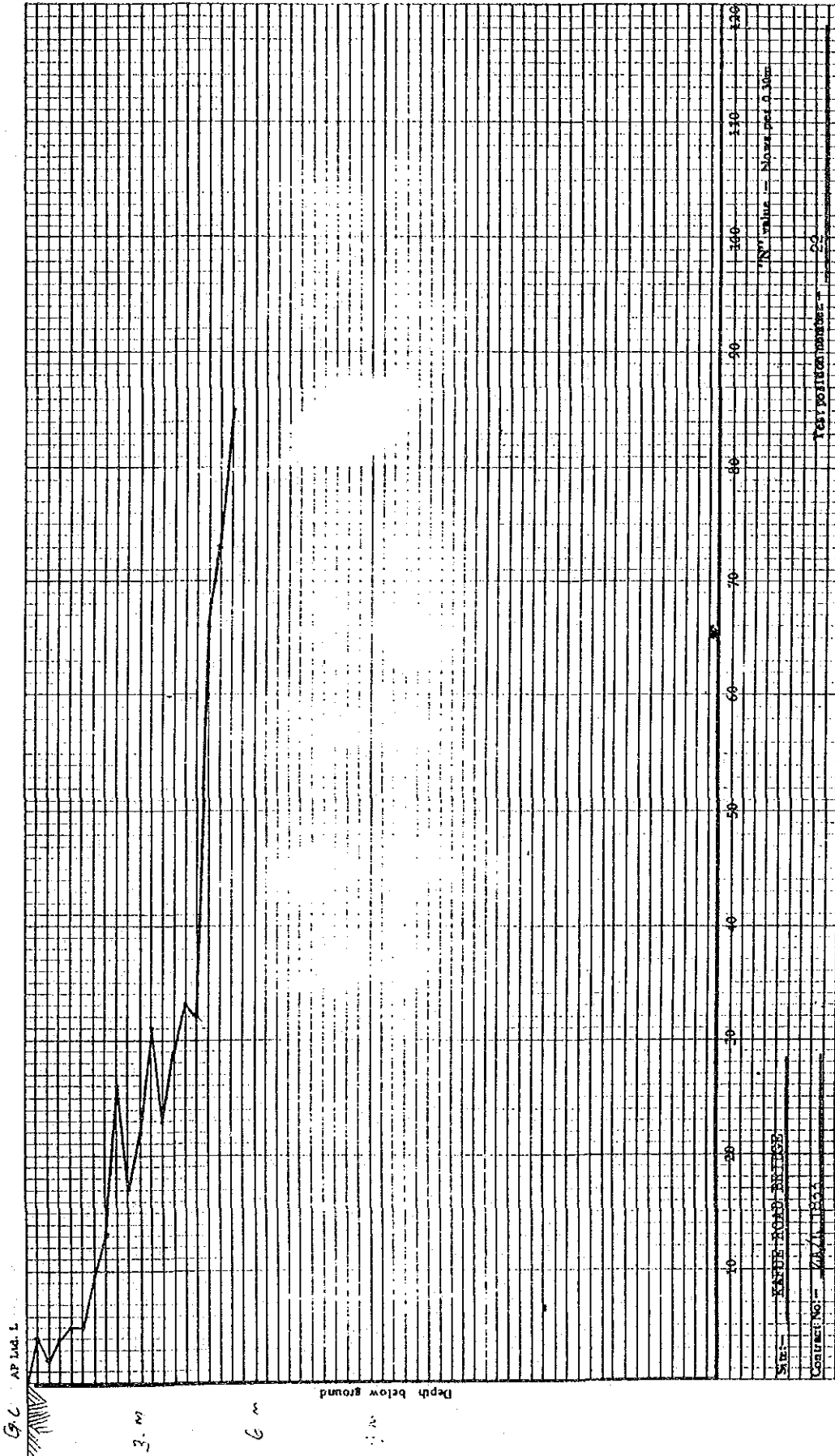
SUN - WATER ROAD BRIDGE

Contract No. 24/4 1873

Test position number - 21

1/2" values - blow per 30mm

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 1ST MARCH 1990



WADE ADAMS PILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 28TH FEBRUARY 1990

G.C. AP LID. L



SAW - ELITE ROAD BRIDGE

Contract No. - ZAE 1677

Test point number - 27

100% value of blow per 0.30m

Date 27th FEBRUARY 1990

PENETROMETER TEST REPORT

WADE ADAMS PILING AND FOUNDATIONS LTD.

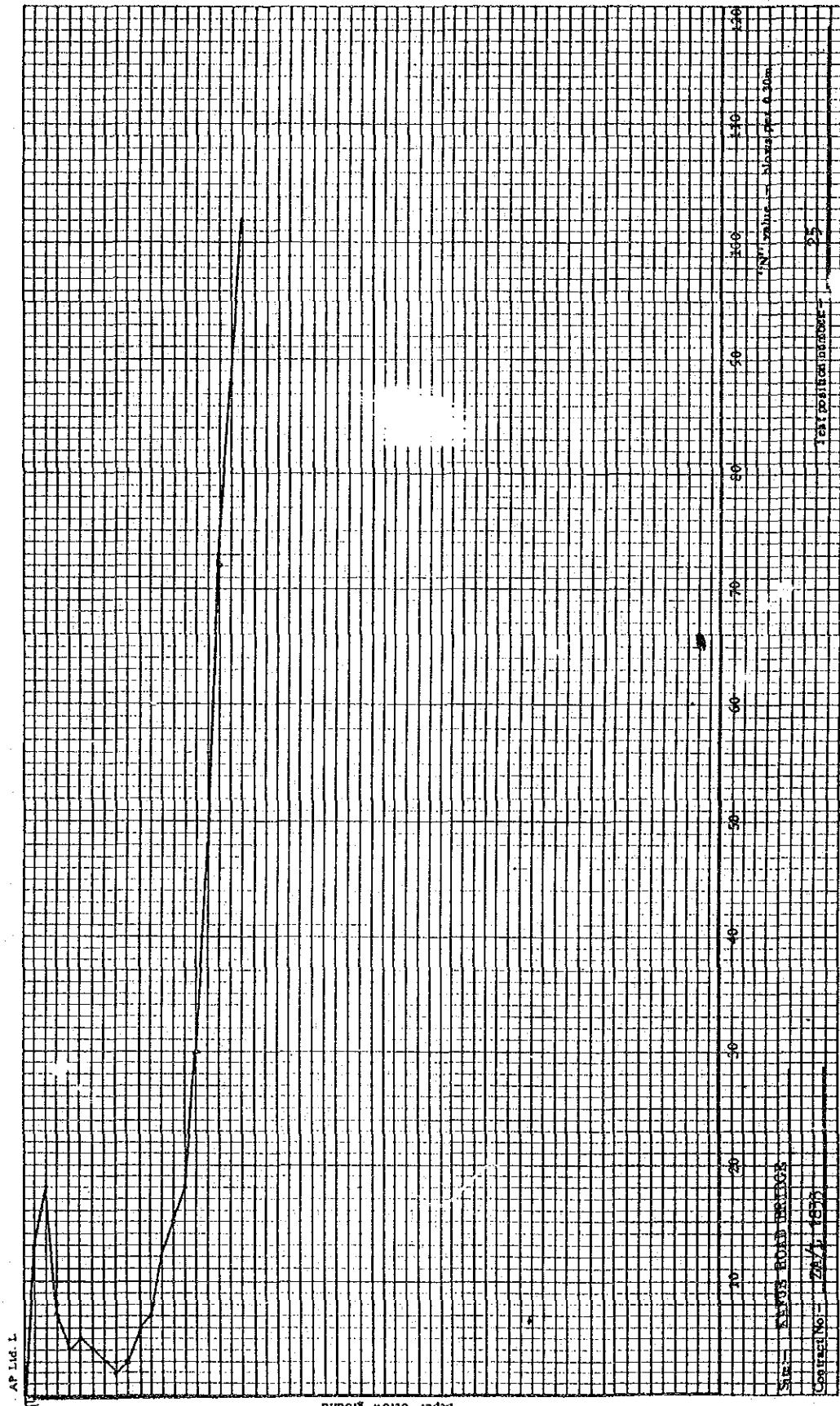
G-1 AP 144.1



SKIN - EXPOSED ROAD SURFACE
 CONFIRM NO. - ZM/S-1077

Test position number - 24
 (N) units - blow per A.30m

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 27th FEBRUARY 1990

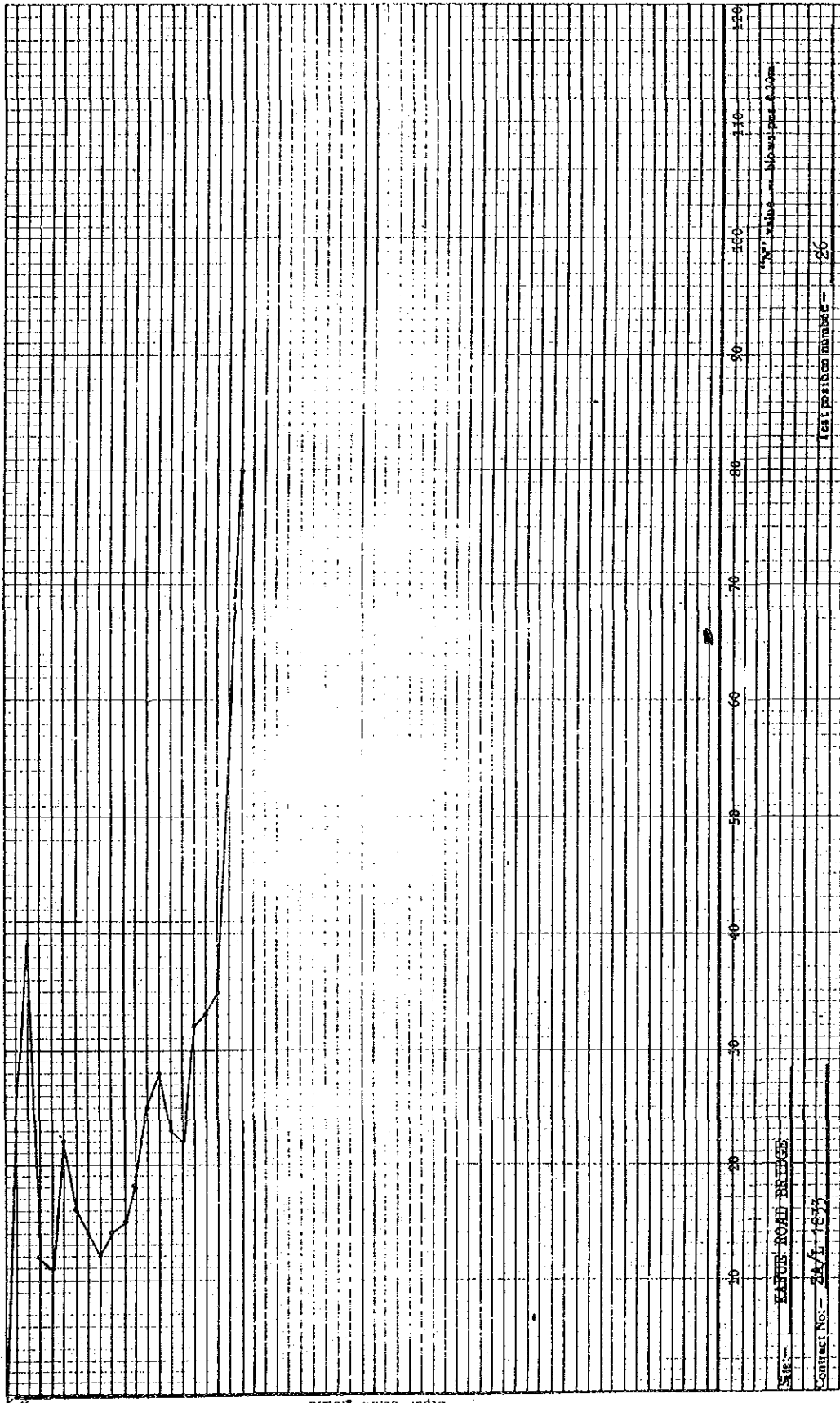


WADE ADAMS PILING AND FOUNDATIONS LTD.

PENETROMETER TEST REPORT

Date 27TH FEBRUARY 1990

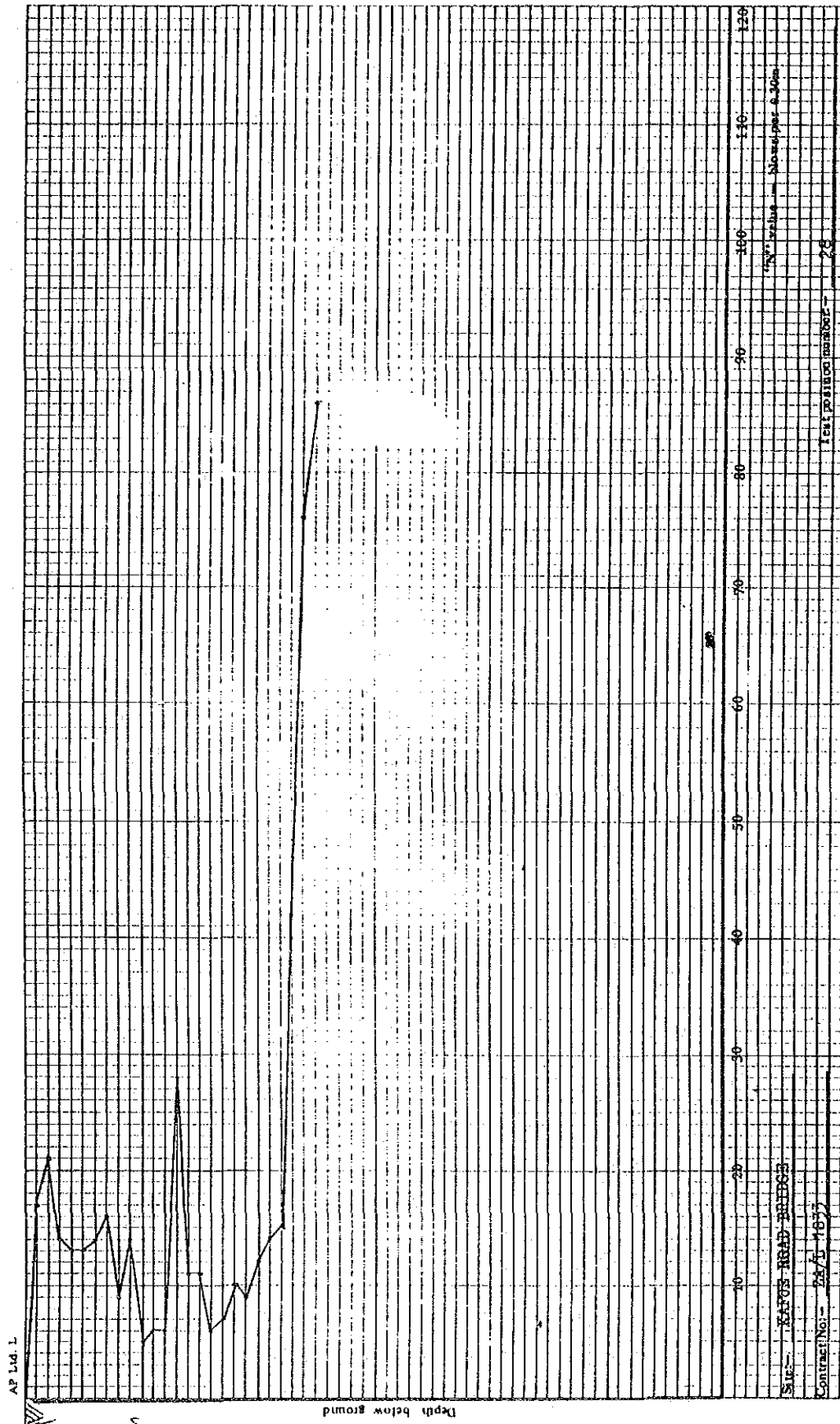
G.C. AP Ltd. L



Depth below ground

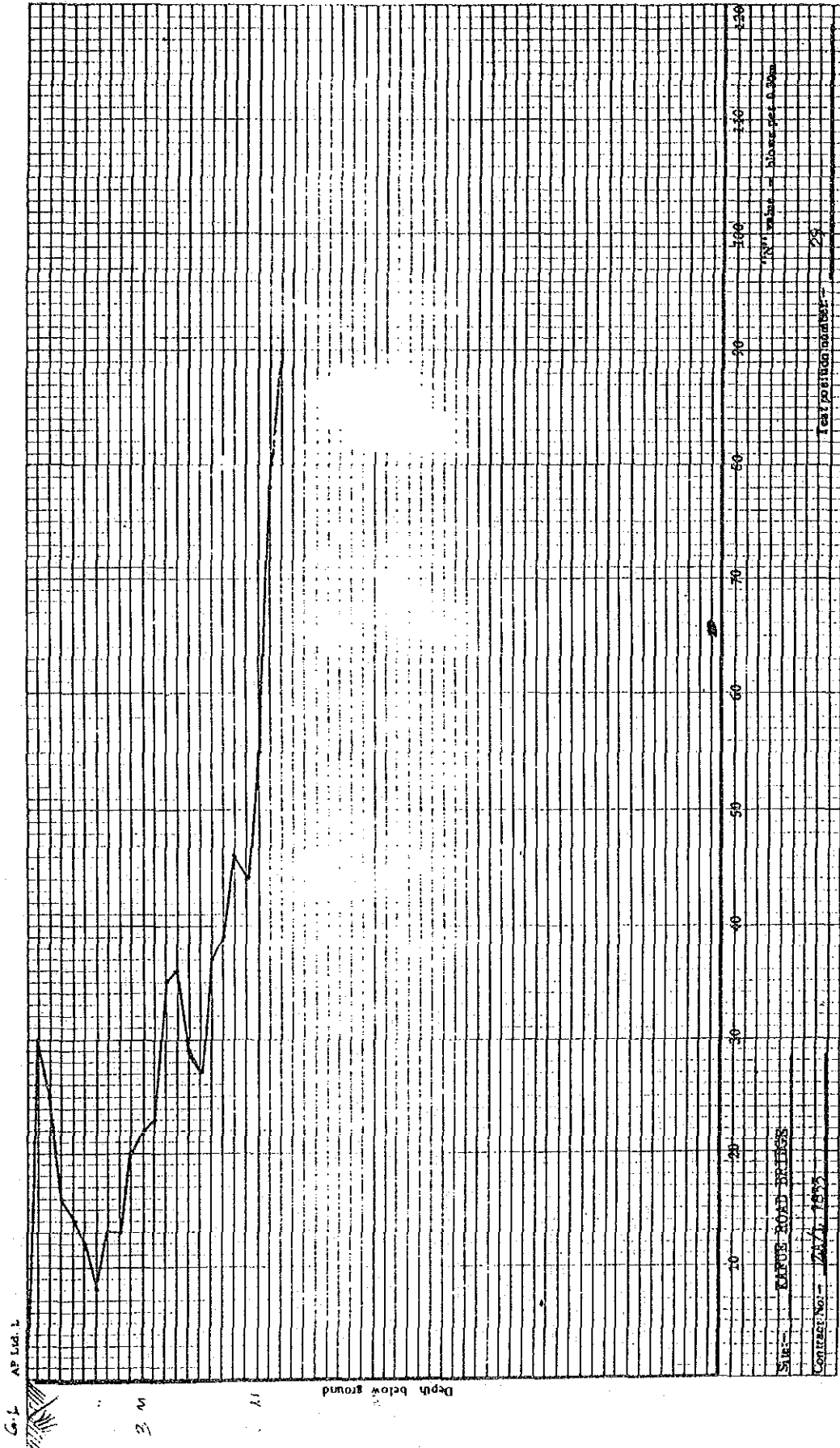
10 20 30 40 50 60 70 80 90 100 110 120
 0 10 20 30 40 50 60 70 80 90 100 110 120
 SITE: KATFOE ROAD BRIDGE
 Contract No: 244/L-1673
 Test position number: 26
 Blow pipe diameter: 30mm

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 6TH MARCH 1990



WADE ADAMS PILING AND FOUNDATIONS LTD. Date 6TH MARCH 1990

PENETROMETER TEST REPORT



WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 6TH MARCH 1990

AP Ltd. L

3



Site: KARFOS BOND BRIDGE

Contract No.: 25/1/1955

Test position number = 30

Blow value = blow per 6.00m

WADE ADAMS PILING AND FOUNDATIONS LTD. PENETROMETER TEST REPORT Date 6TH MARCH 1990

付属資料 4-6 水平積荷試験結果

Project: Kafue Road Bridge

$P_w = 0.05 \text{ (kg/cm}^2\text{)}$

Boring Hole

Water Level

Location : 3 Depth: GL. -0.50m upon Surveyed: GL ± 0.00m

$E_p = 2(1+\nu)(V_c + V_m) dp/dv$

$= 2(1+0.3)(535 + 71) \times 0.011$

$= 17.33 \text{ (kg/cm}^2\text{)}$

$P_0 = 0.73 \text{ (kg/cm}^2\text{)}$ $P_f = 2.29 \text{ (kg/cm}^2\text{)}$ $P_i = \text{ (kg/cm}^2\text{)}$

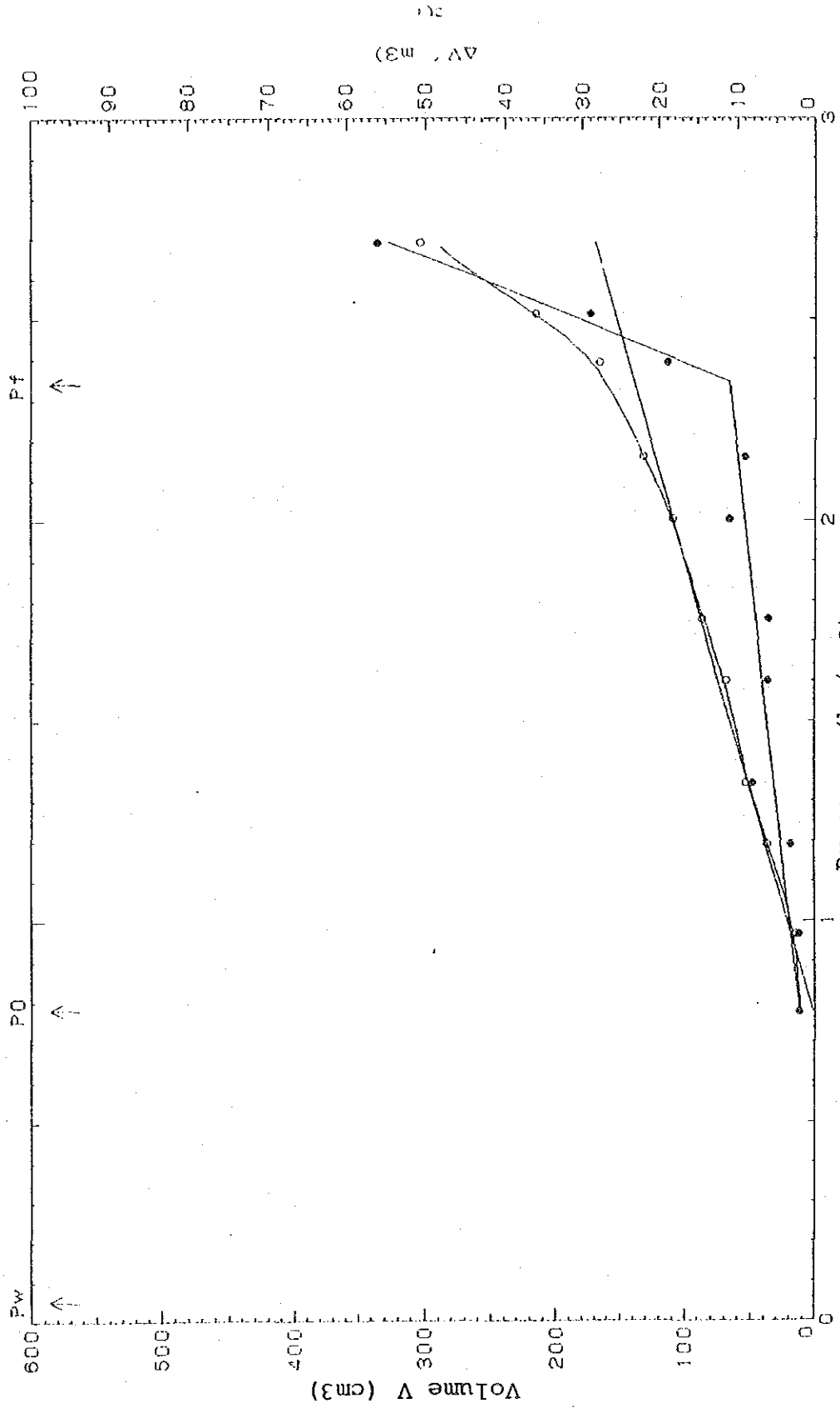


Fig. 1 Records of Horizontal Loading Test

Project: Kafue Road Bridge

Boring Hole

Location : S

Depth: GL -2.50m

Water Level

upon Surveyed : GL -2.50m

Pw = 0.00 (kg/cm²)

$E_p = 2(1+\nu)(V_c + V_m) dp/dv$
 $= 2(1+0.3)(535+309) \times 0.022$
 $= 48.28 \text{ (kg/cm}^2\text{)}$

P0 = 2.44 (kg/cm²) Pf = 4.25 (kg/cm²) P1 = (kg/cm²)

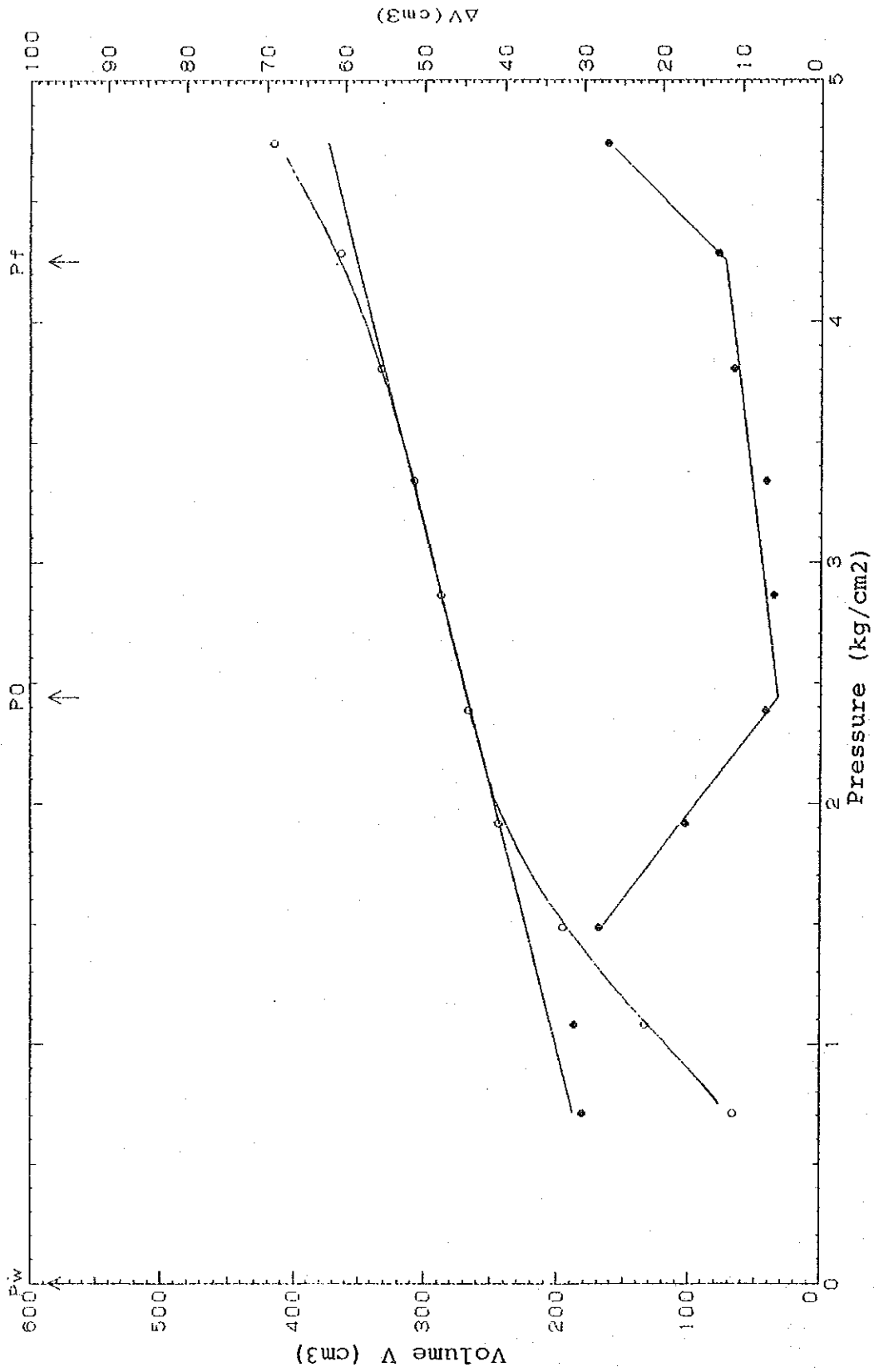


Fig. 2 Records of Horizontal Loading Test

Project: Kafue Road Bridge

$P_w = 0.15 \text{ (kg/cm}^2\text{)}$

Boring Hole Location: 8
Depth: GL -3.50m
Water Level upon Surveyed: GL -2.00m

$E_p = 2(1+\nu)(V_c + V_m) dp/dv$

$= 2(1+0.3)(535+130) \times 0.055$

$= 95.09 \text{ (kg/cm}^2\text{)}$

$P_0 = 1.99 \text{ (kg/cm}^2\text{)}$ $P_f = 6.50 \text{ (kg/cm}^2\text{)}$ $P_1 = \text{ (kg/cm}^2\text{)}$

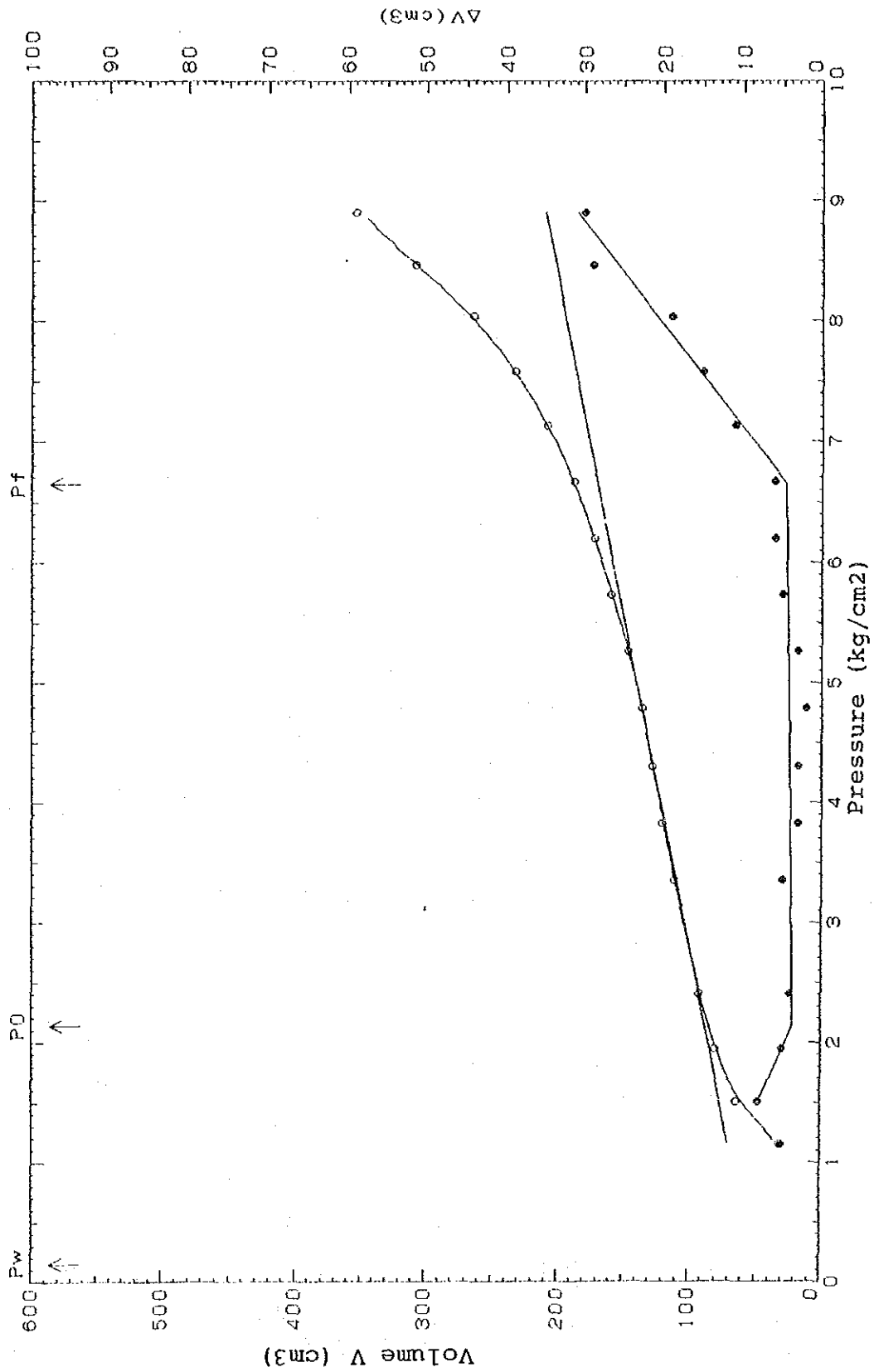


Fig. 3 Records of Horizontal Loading Test

Project: Kafue Road Bridge

Boring Hole : 8
Location

Water Level
upon Surveyed: GL

Depth: GL -1.50m -2.00m

$P_w = -0. \quad (\text{kg/cm}^2)$

$E_p = 2(1+\nu)(V_c + V_m) dp/dv$
 $= 2(1+0.3)(535+26i) \times 0.018$
 $= 37.25 (\text{kg/cm}^2)$

$P_0 = 2.01 (\text{kg/cm}^2) \quad P_f = 4.00 (\text{kg/cm}^2) \quad P_1 = \quad (\text{kg/cm}^2)$

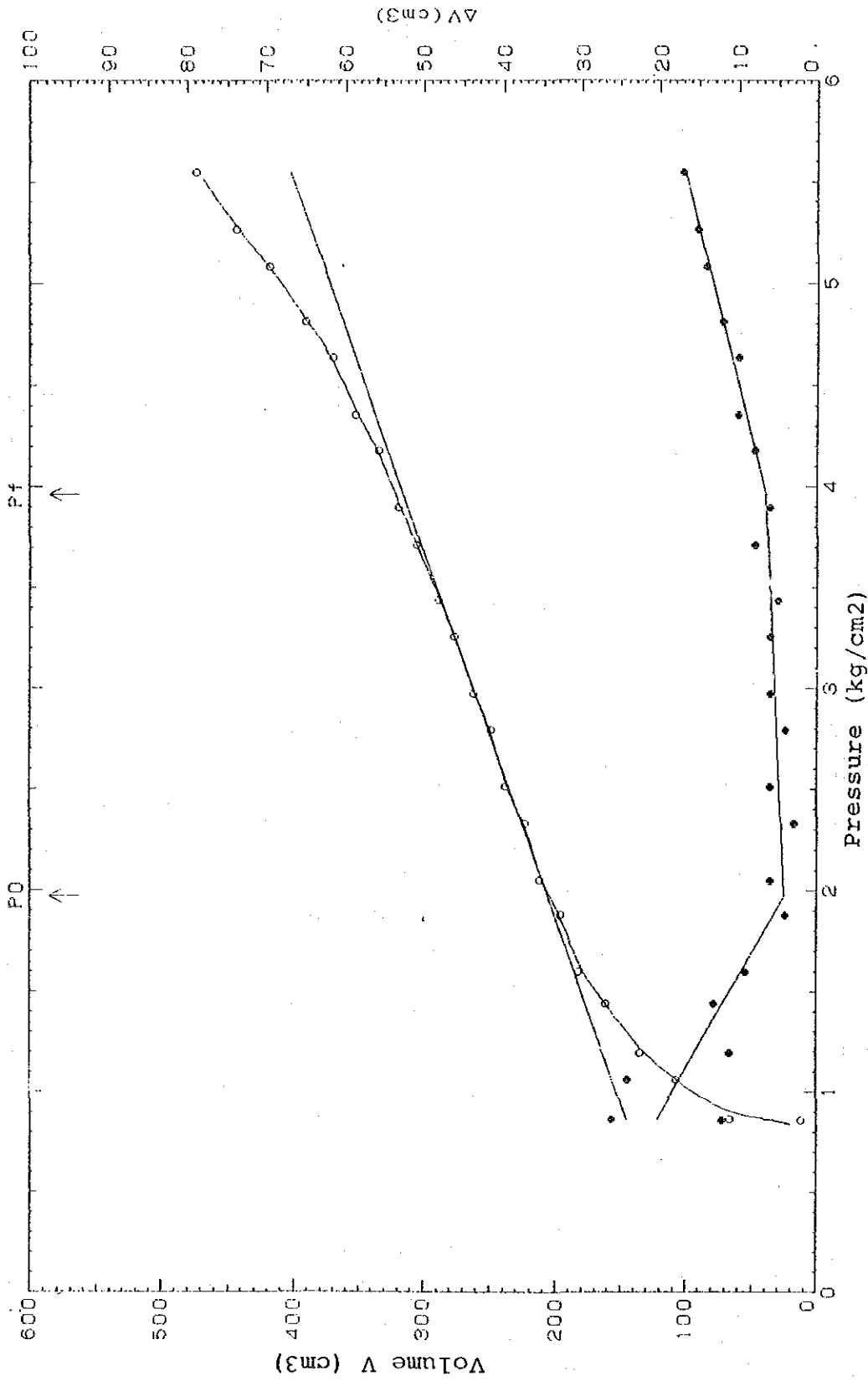


Fig. 4 Records of Horizontal Loading Test

Project: Kafue Road Bridge

$P_w = 0.75 \text{ (kg/cm}^2\text{)}$

Boring Hole : 11
Location :

Depth : GL - 3.30m
upon Surveyed: GL + 4.20m

Water Level

$P_0 = 0.31 \text{ (kg/cm}^2\text{)}$

$P_f = 7.93 \text{ (kg/cm}^2\text{)}$

$P_1 = \text{ (kg/cm}^2\text{)}$

$E_p = 2(1+\nu) (V_c + V_m) dp/dv$
 $= 2(1+0.3) (535+126) \times 0.033$
 $= 56.71 \text{ (kg/cm}^2\text{)}$

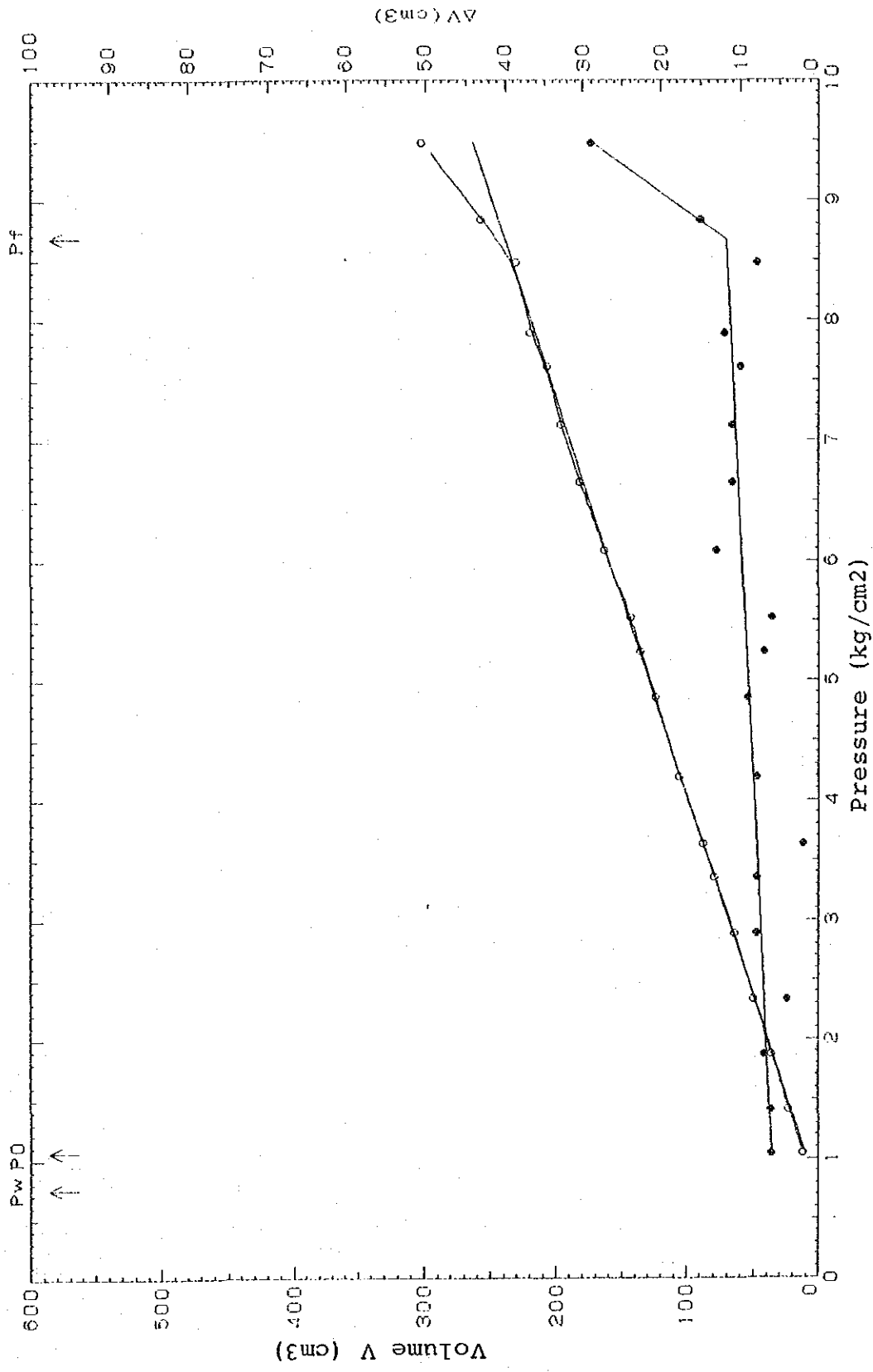


Fig. 5 Records of Horizontal Loading Test

付属資料 5-1 カフェの水位

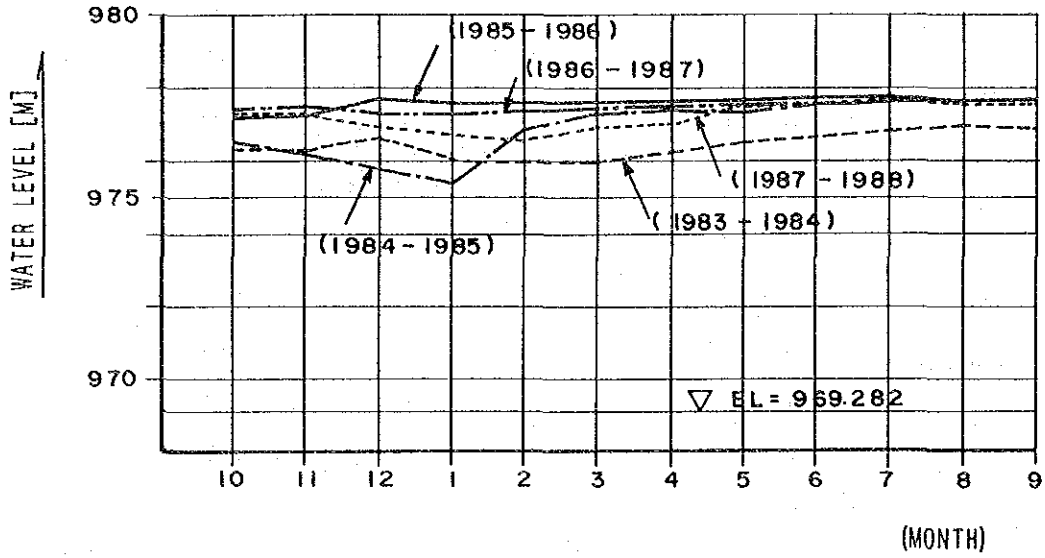


FIG. 1. MAXIMUM MONTHLY WATER LEVEL

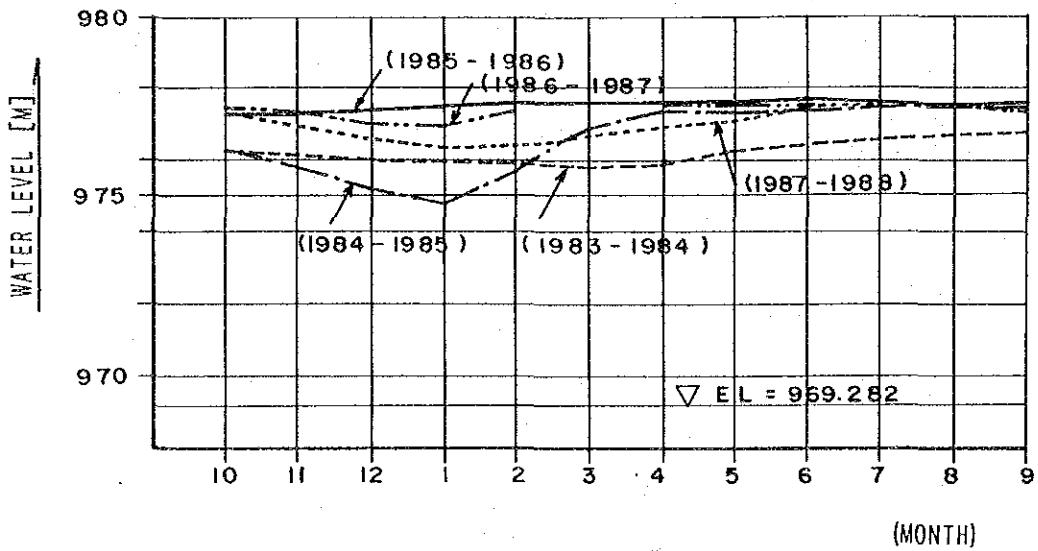
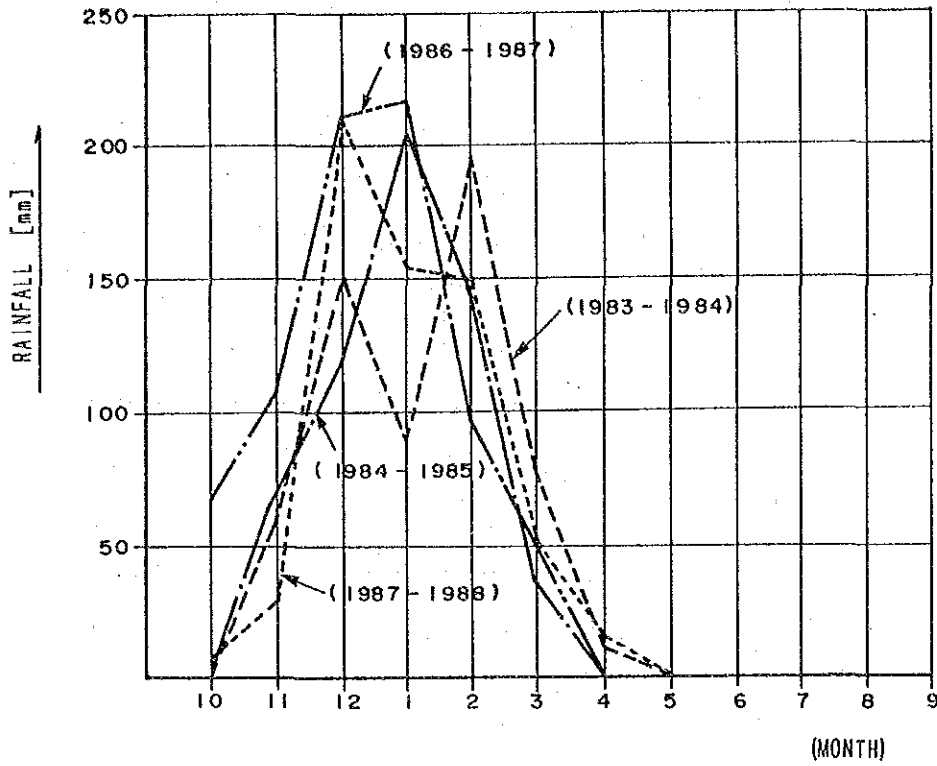
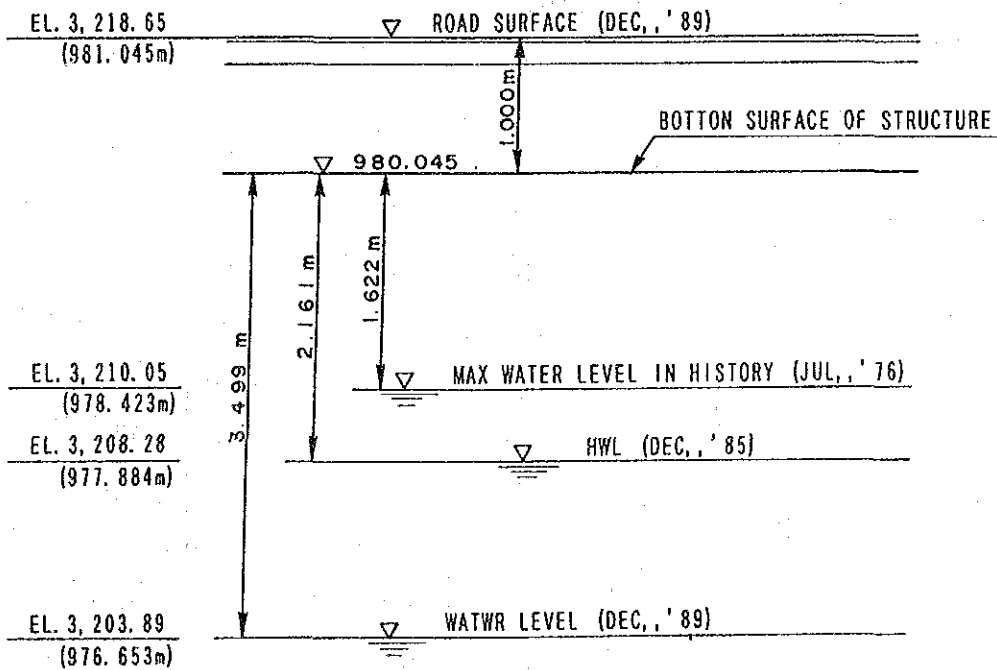


FIG. 2. MINIMUM MONTHLY WATER LEVEL

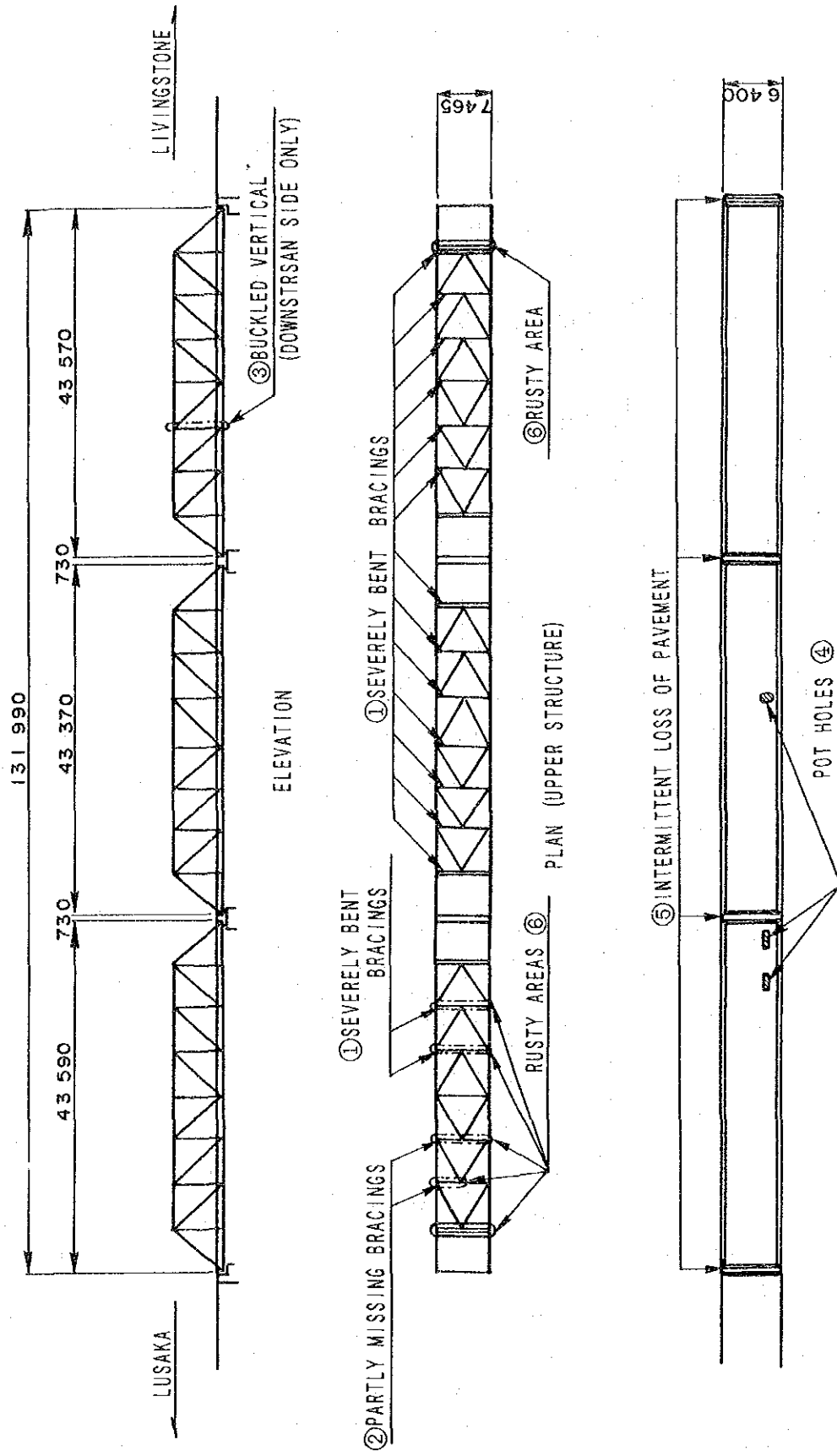
付属資料 5-2 カフェの降雨記録

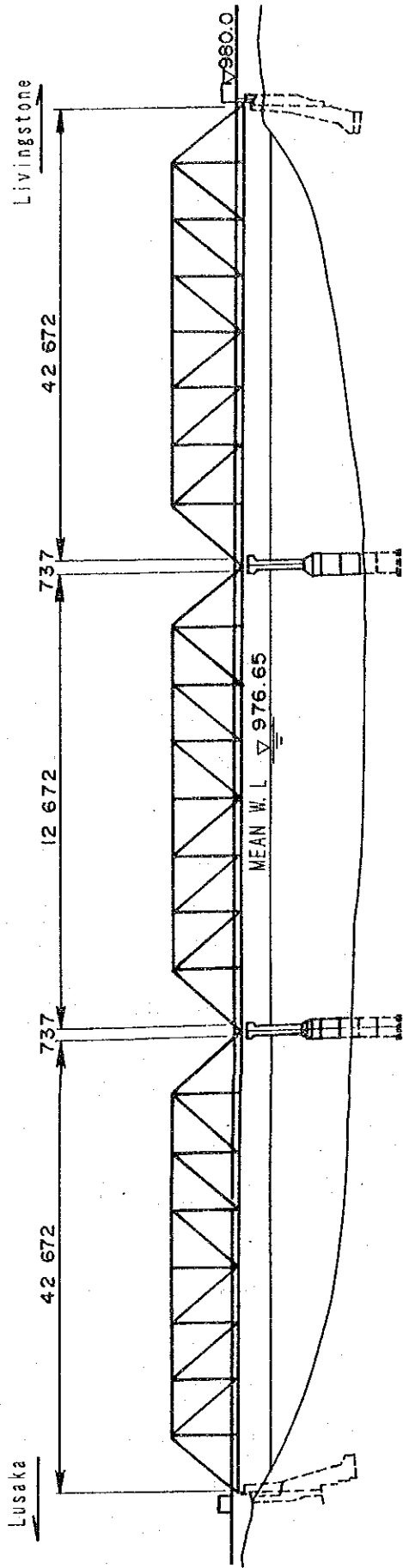


付属資料 5-3 カフェ橋桁下余裕高

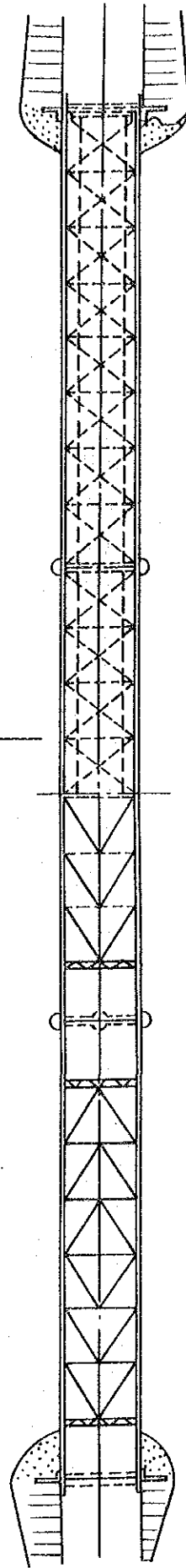


付属資料 5-4 損傷部位位置図





Kafue River



付属資料 5-5 既存橋梁一般図

付属資料 6-1 道路区分

1. Summary of Road System

T	International Main Roads
M	Main Roads
D, RD	District Roads
B	Branch Roads
R	Rural Roads
E	Estate Roads

2. Road Classification

Class	Formation Width at Finished Surface Level (Meters)	Carriageway	
		Width (Meters)	Type of Surface
Class 1A	13.30	7.30	Bituminous
Class 1B	10.10 to 12.70 According to Traffic Needs	6.70	Bituminous
Class 1C	10.10	6.10	Bituminous
Class II	10.10	Minimum 6.10	Gravel
Class III	7.50	Minimum 5.50	Gravel where Necessary for All Weather Standard
Unclassified	Cleared and Stumped Track of 5.50 minimum Width and Skeleton Drainage		Earth with 3.50 Gravel Surface Where Essential

付屬資料 6-2 幾何学的設計条件

Road Classification		Class I A	Bituminous B C	Class II Gravel	Class III Gravel
Average daily traffic estimated for nth year after construction (ADT)		1,500 to 5,000	500 to 1,500	Up to 500	20 to 50
Width of surfaced carriageway (metres)		7.30	6.70 6.10	Min. 6.10	Min. 5.50
Shoulder widths (metres)		3	2-3 2	2	1
Flat	Design Speed (km/h)	100	100 100	80-100	60-80
Topo- graphy	Limiting Grade (%)	4	5 6	6	8
Rolling to Hilly Topo- graphy	Design Speed (km/h)	100	80-100 80	60-80	50-60
	Limiting Grade (%)	6	6 7	8	10
	Maximum length of limiting grade (metres)	220	220 180	150	N.A.
	Design Speed (km/h)	80	60-80 60	50-60	30-50
Mountain- us Topo- graphy	Limiting Grade (%)	7	8 8	10	12
	Maximum length of limiting grade (metres)	180	150 150	150	N.A.

Notes: 1. Where difficulty is encountered in obtaining lengths of limiting grade less than the maximum length stated in Table 3.2., reference should be made to the Director of Works (Roads).

2. Shoulder widths for Class IB roads should be chosen according to traffic needs and economics.

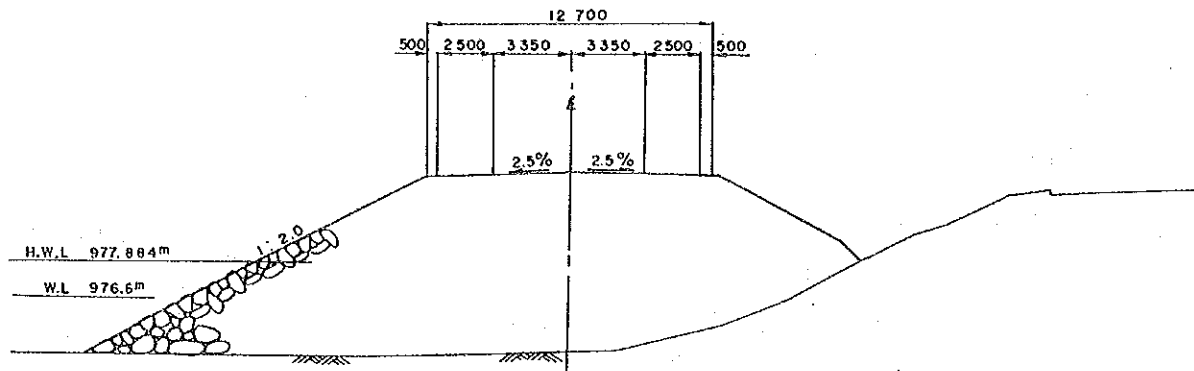
付属資料 6-3 設計車両寸法

Design Vehicle		Dimensions in Metres					
Symbol		Wheel- base	Front Overhang	Rear Overhang	Overall Length	Overall Width	Height
Passenger P		3.3	1.0	1.5	5.8	2.1	-
Single Unit Trucks	SU	6.1	1.2	1.8	9.1	2.5	4.1
Trailer WB-40		4.0+6.2 =12.2	1.2	1.8	15.2	2.5	4.1
Trailer WB-50		6.2+9.0 =15.2	1.0	0.6	16.8	2.5	4.1

Item	Unit	Design Standard
Design Classification	-	Class-1B
Design Speed	km/hr	100
Cross Section Elements		
Lane Width	m	3.35
Left Shoulder Width	m	2.00
Marginal Strip Width	m	0.30
Crossfall of Tavelled Way	%	2.5
Crossfall of Outer Shoulder	%	2.5
Vertical Clearance	m	5.0
Min. Stopping Sight Distance	m	160
Horizontal Alignment		
Min. Radius		
Absolute Min.	m	340
Desirable	m	700
Min. Radius Without Superelevation	m	5,000
(*) Min. Curve Length	m	1,200/a or 170
Max. Superelevation	%	10
Min. Transition curve Length	m	50
Min. Radius Without Transition Curve	m	1,500
Vertical Alignment		
Max. Grade		
Standard	%	3
Absolute Max.	%	6
Min. Vertical Curve Radius		
Crest, Standard	m	6,500
Crest, Desirable	m	10,000
Sag, Standard	m	3,000
Sag, Desirable	m	4,500
Min. Vertical Curve Length	m	50

Note:

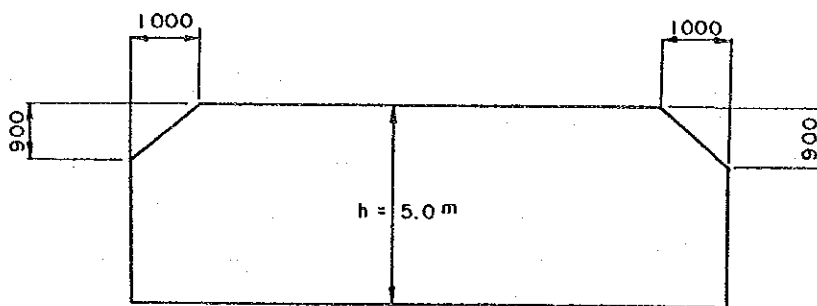
- 1) The above values are based on the "Standard Specifications for Geometric Design of Kafue Road Bridge."
- (*) 2) The value of "a" in minimum horizontal curve length shows an interesting angle in degrees (min. 2 degrees), when the angle is less than 7 degrees.



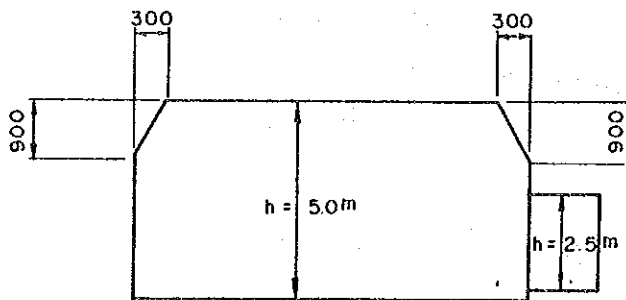
付属資料 6-5 取付道路部標準断面

All demensions are in mm. otherwise indicated

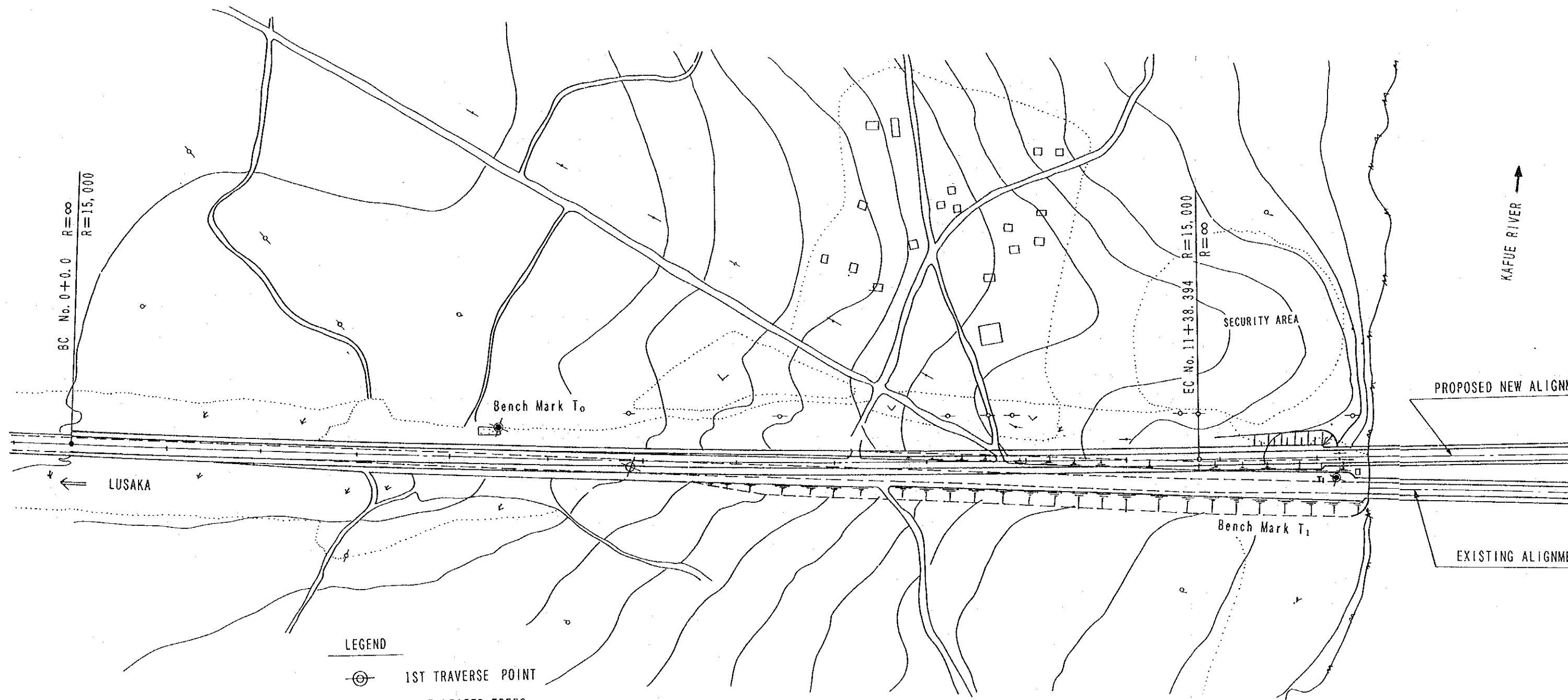
For Roadway Clearance on Embankment



For Roadway and Footpath no Bridge

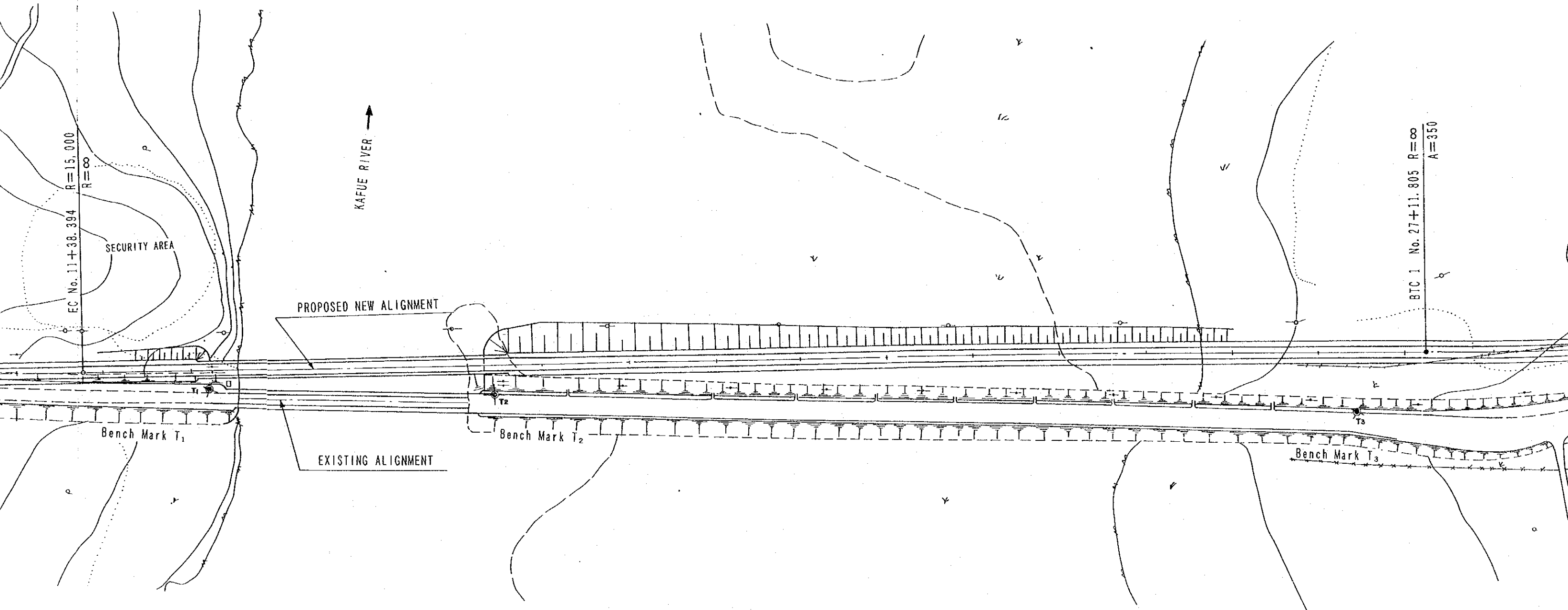


付属資料 6-6 建築限界

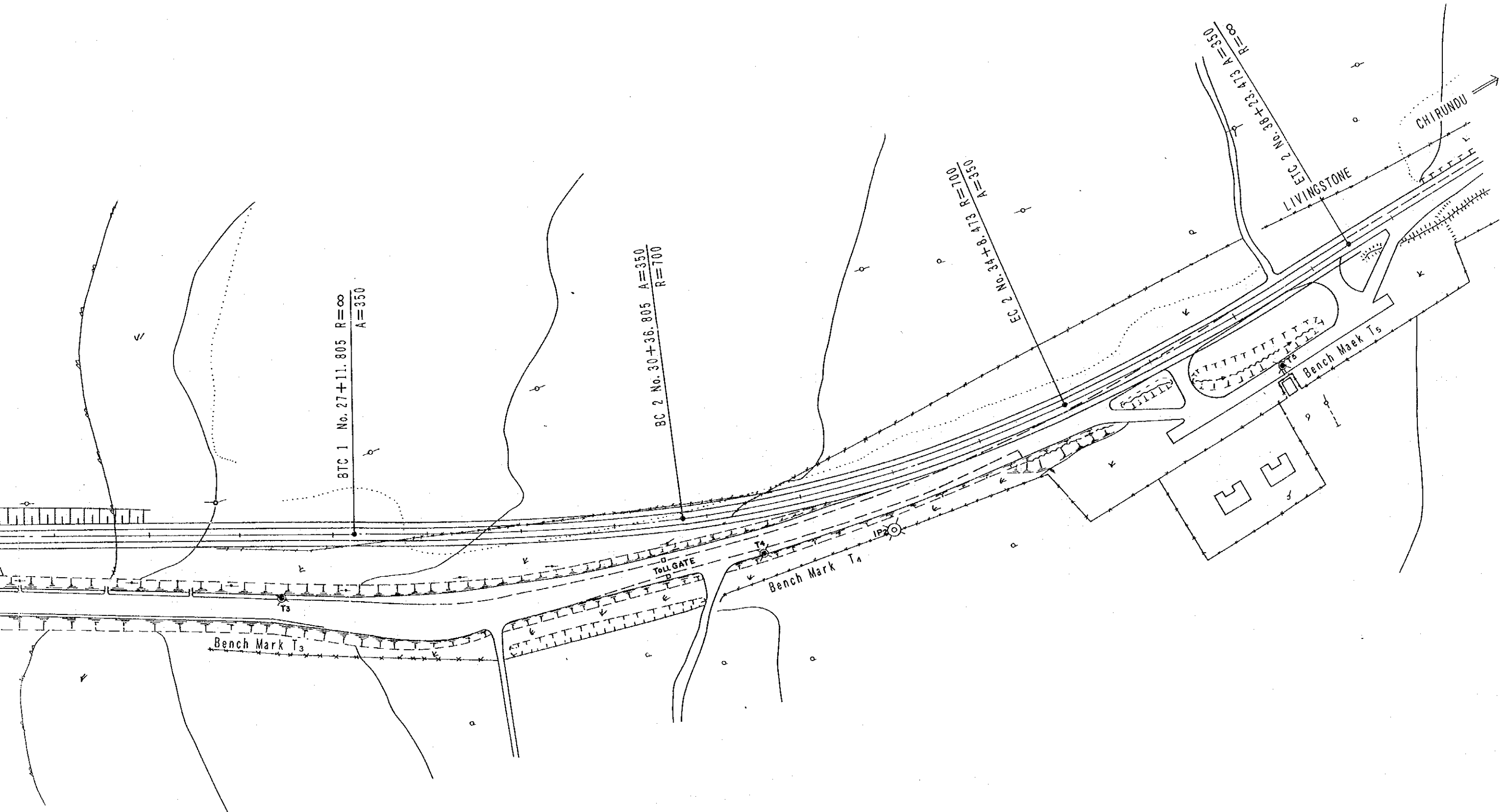


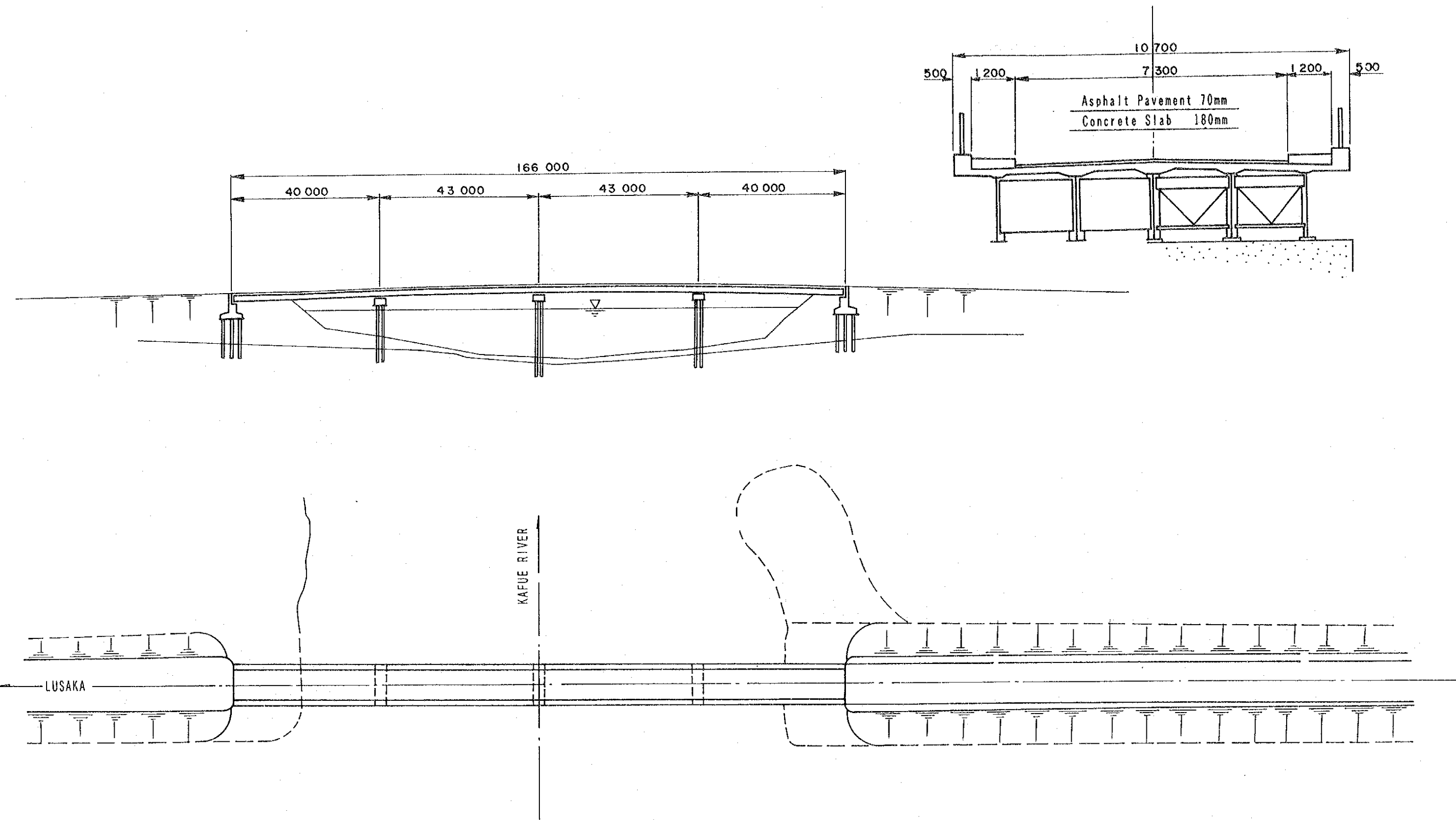
LEGEND

- ⊙ 1ST TRAVERSE POINT
- WIDE LEAFED TREES
- ∨ GRASSLAND
- POWERLINE
- TELEPHONE LINE
- BOUNDARY OF SAVANNAH
- ⌌ EMBARKMENT, SLOPE

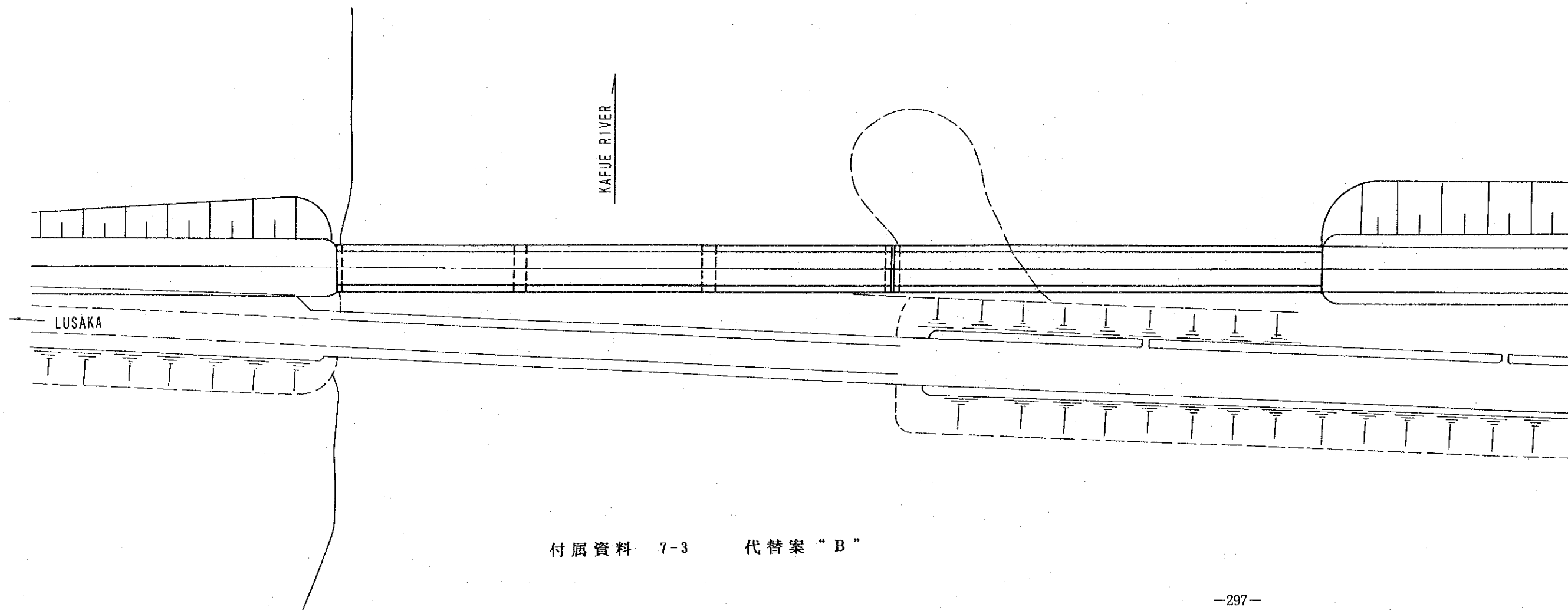
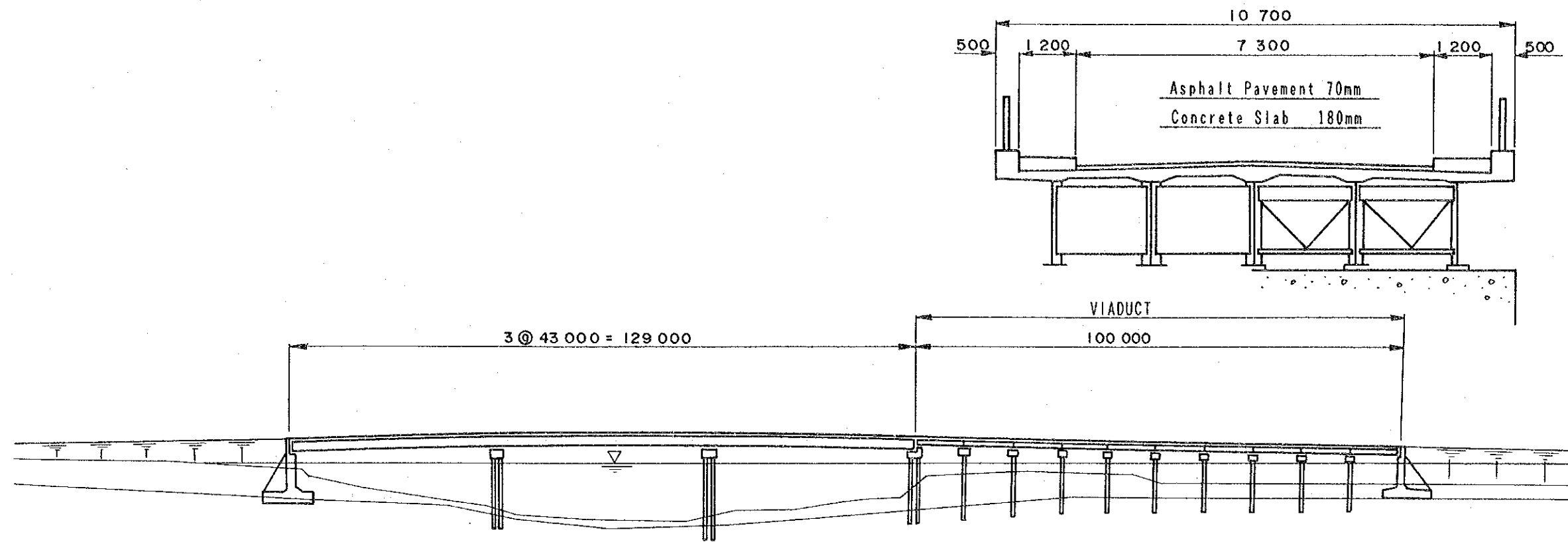


付属資料 7-1 提案線形

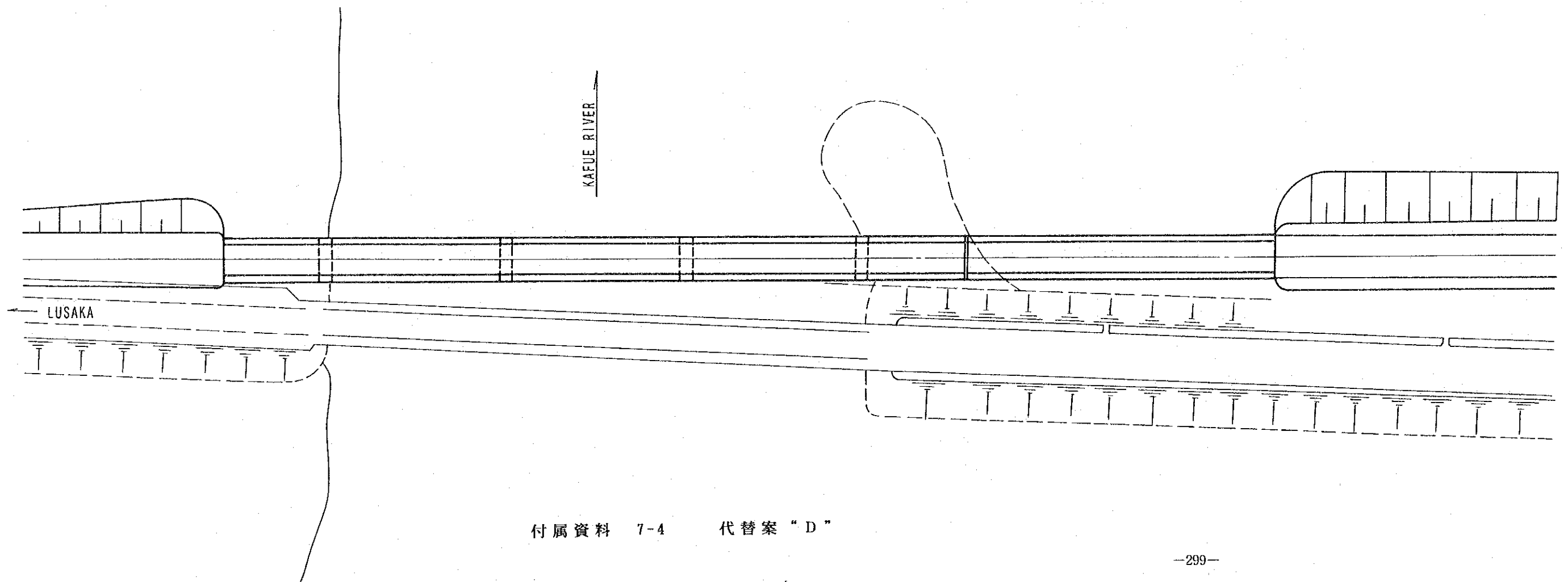
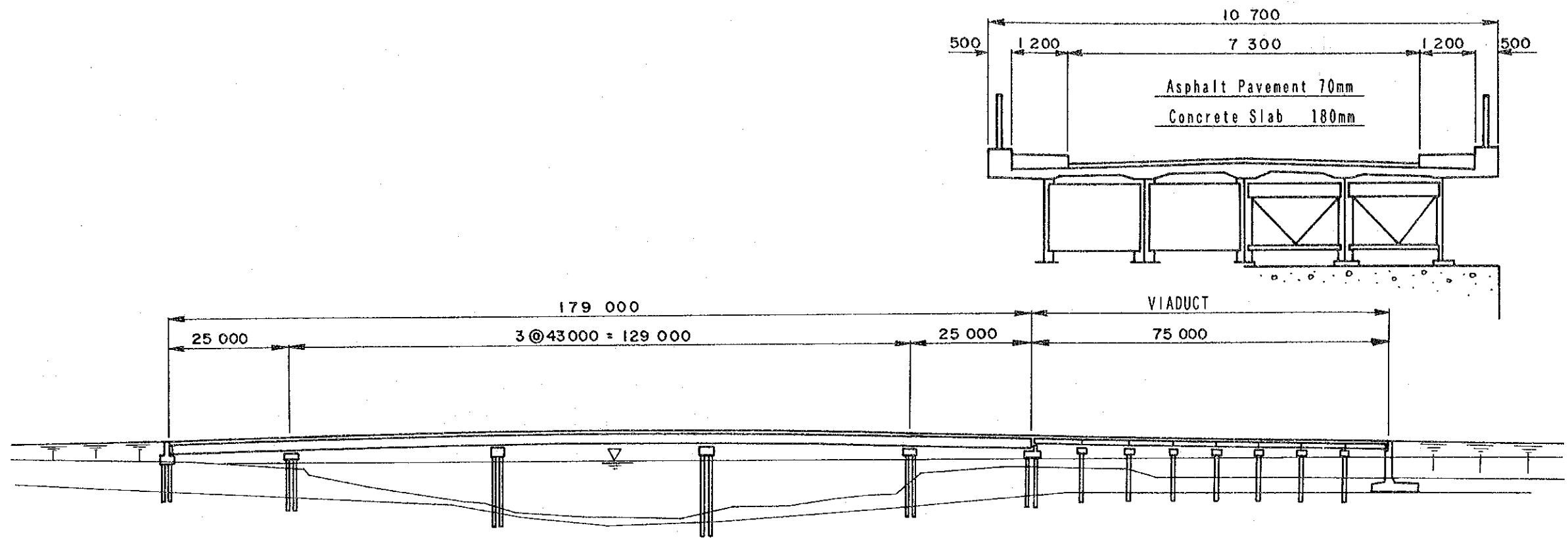




付属資料 7-2 代替案 "A"



付属資料 7-3 代替案“B”



付属資料 7-4 代替案 "D"

			(Unit: Kwacha)	
(Type of Vehicle)			Passenger Car	Large Truck
(Representative Vehicle)			Toyota Cressida	Leyland 3-Axle Draw-Bar-Trailer
(1) Vehicle Price			598,800	1,356,000
Usage	Hour Running	(/Year) (1000Km)	3000 36	3650 42
(2) Duration	Year Distance	(1000Km)	12 432	15 630
	Salvage Value		20%	10%
(3) Depreciation	Time	(Ratio/Year)	3%	3%
	Distance	(Ratio/100000Km)	10%	7%
			44%	45%
(4) Tyre	Duration	(1000Km)	60	60
	Unit Price		6,000	8,500
	Number of Tyre		4	18
	Total Price		24,000	153,000
(5) Fuel	Price	(/l)	7.6	4.7
	Consumption	(l/Km)	0.083	0.11
(6) Oil	Percentage for Fuel		6%	7%
(7) Maintenance		(/1000Km)	694	4638
(8) Insurance		(/Year)	80,000	22,300
(9) Personnel Cost			4,300	4,300
(10) Overhead			10%	20%
(11) Interest Rate			15%	15%
(A) Time		(/Year)		
	Capital Cost		44,910	101,700
	Personnel Cost		4,300	4,300
	Insurance		80,000	22,300
	Depreciation		17,964	40,680
	(Subtotal)		147,174	168,980
	(Subtotal Converted to /1000Km)		4,088	4,023
(B) Distance		(/1000Km)		
	Fuel		631	517
	Oil		38	36
	Tyre		400	2,550
	Maintenance		694	4,638
	Depreciation		610	969
	(Subtotal)		2,373	8,710
(C) Total			6,461	12,733
(D) Overhead			646	2,547
(E) Grand Total		(/1000Km)	7,107	15,280

Note : 1) Basic data are interviewed from car-dealers and trucking company.
 2) Data used are as of January, 1990.
 3) Exchange rate: US\$ 1 = Kwacha 40

JICA