

**C.2.5. PERMEABILITY TEST**

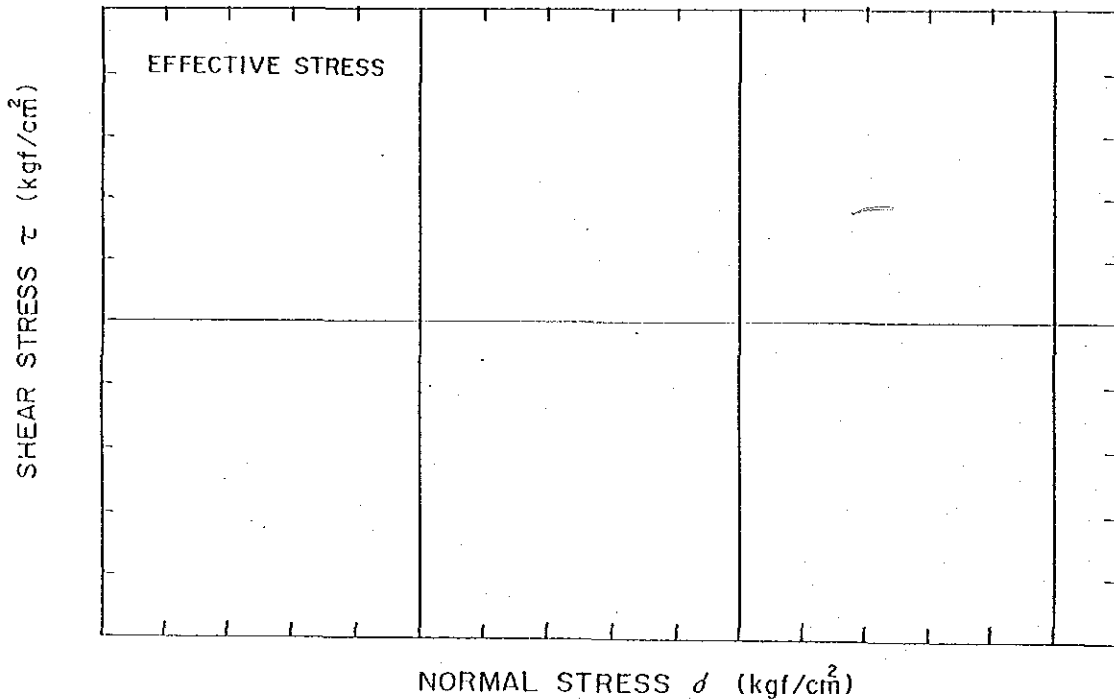
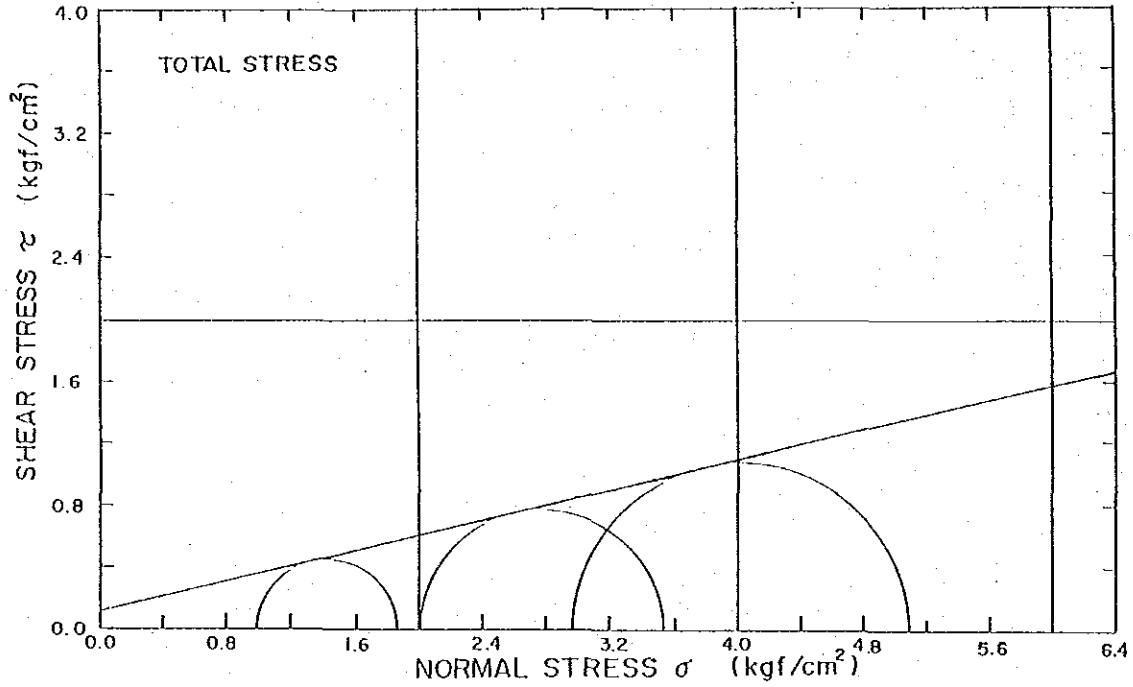


#### C.2.6. TRIAXIAL SHEAR TEST

## TRIAXIAL COMPRESSION TEST (UU-TEST)

SAMPLING SPOT & PLACE: KHLONG - SI - YAT DATE 4-4-1990  
 SAMPLE No. & DEPTH: No. TPB - 1 (1.90 - 5.50)

STRENGTH PARAMETER	SCOPE OF STRESS	C kgf/cm	$\phi$ DEGREE	$\tan \phi$	$C'$ kgf/cm	$\phi'$ DEGREE
	NORMALLY CONSOLIDATED REGION	0.105	13.723	0.244		
	OVER CONSOLIDATED REGION					

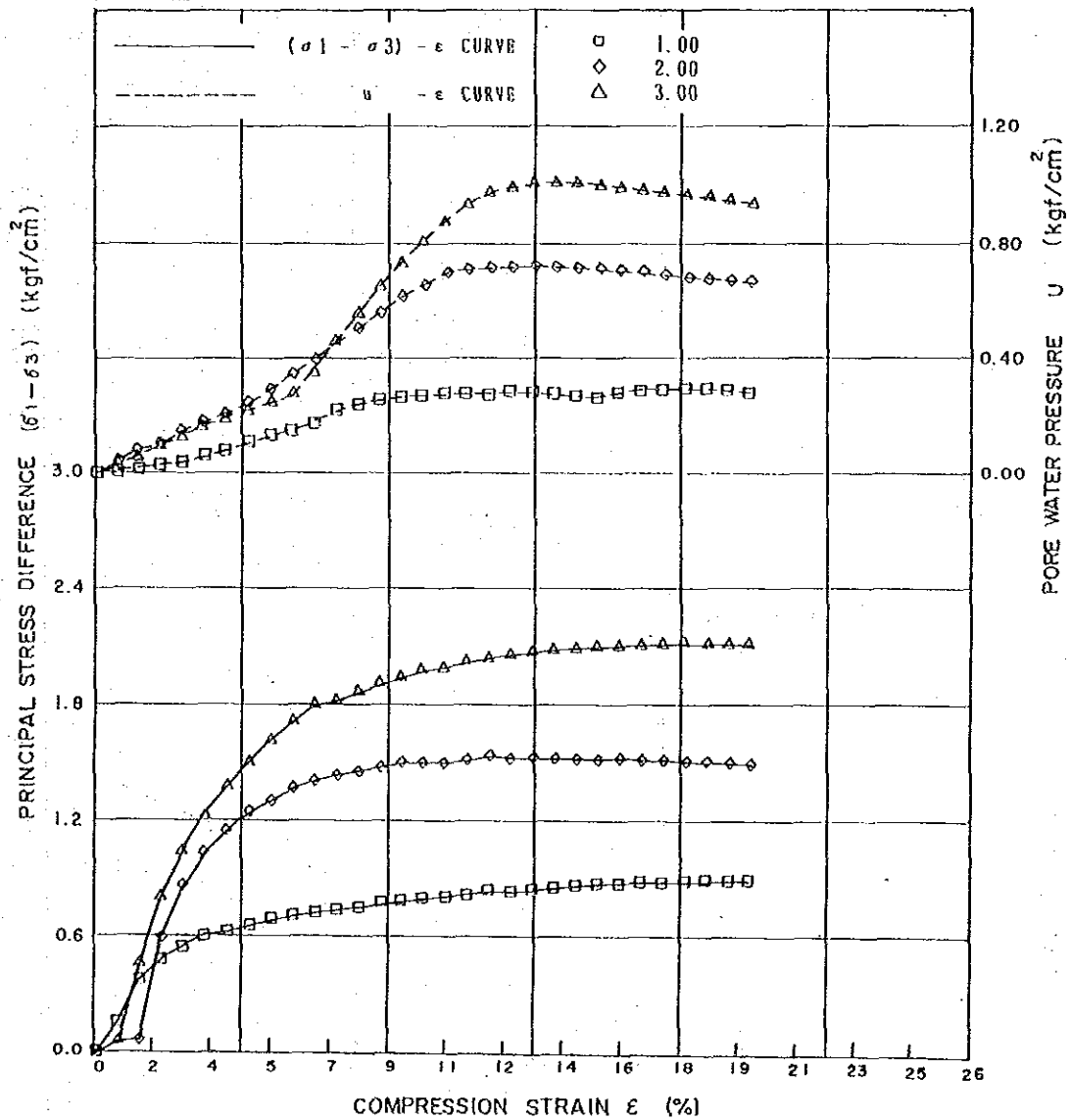


## TRIAXIAL COMPRESSION TEST (UU-TEST)

SAMPLING SPOT  
& PLACE: TPB-1 (1.90-5.50)

DATE 4-4-1990

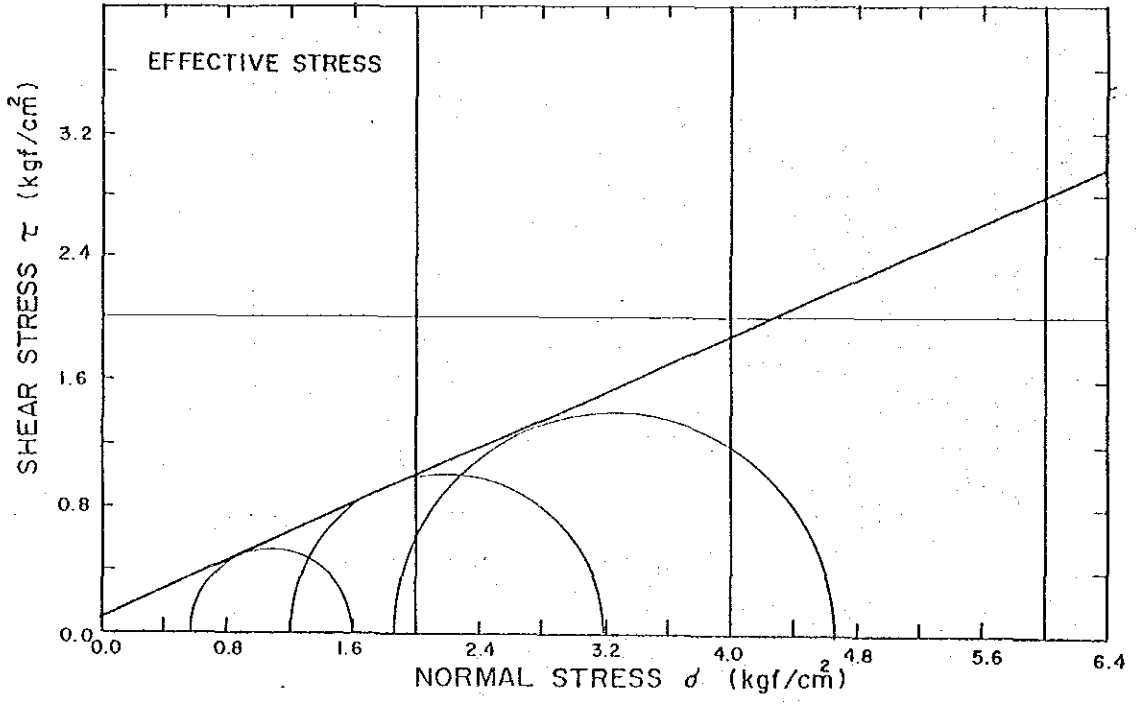
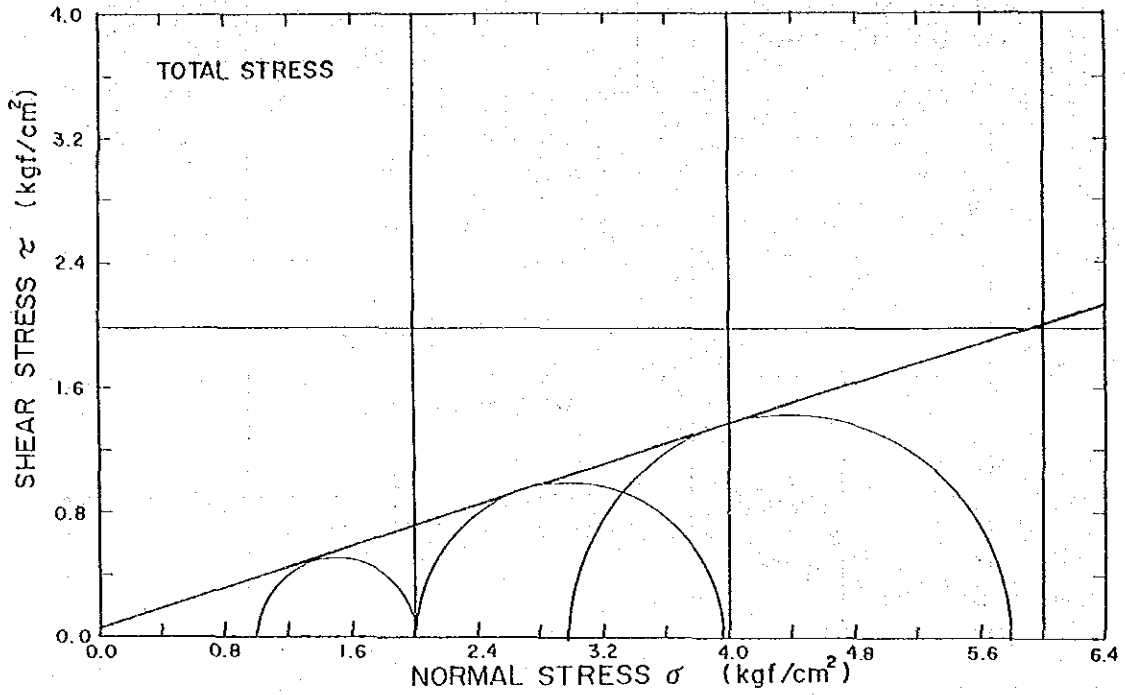
SPECIMEN No.		1 □	2 ◇	3 △
PRINCIPAL STRESS DIFFERENCE AT PEAK	MINOR PRINCIPAL STRESS $\sigma_3$ kgf/cm <sup>2</sup>	1.00	2.00	3.00
	PRINCIPAL STRESS DIFFERENCE $(\sigma_1 - \sigma_3)$ kgf/cm <sup>2</sup>	0.877	1.537	2.121
	COMPRESSION STRAIN $\epsilon_f$ %	15.072	11.842	17.880
	PORE WATER PRESSURE $U_f$ kgf/cm <sup>2</sup>			
	PORE PRESSURE COEFFICIENT $A_f$			



## TRIAxIAL COMPRESSION TEST (CU-TEST)

SAMPLING SPOT & PLACE: KHLONG - SI - YAT      DATE 29-3-1990  
 SAMPLE No. & DEPTH: No. TPB - 1 (1.90 - 5.50)

STRENGTH PARAMETER	SCOPE OF STRESS	C kgf/cm	$\phi$ DEGREE	$\tan \phi$	$c'$ kgf/cm	$\phi'$ DEGREE
NORMALLY CONSOLIDATED REGION OVER CONSOLIDATED REGION		0.049	18.051	0.326	0.084	24.194

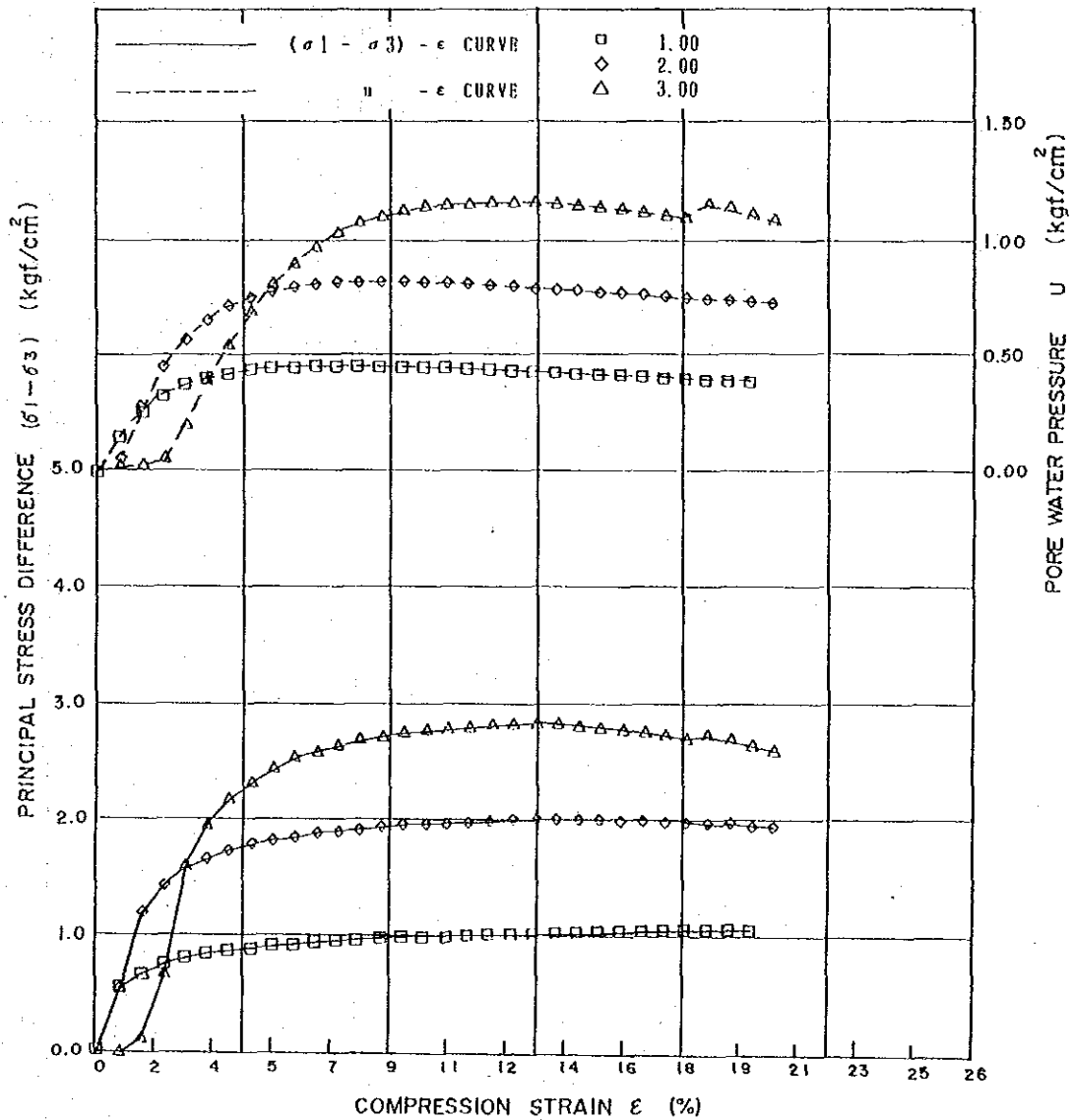


## TRIAxIAL COMPRESSION TEST (CU-TEST)

SAMPLING SPOT  
& PLACE: TPB-1 (1.90 - 5.50)

DATE 29-3-1990

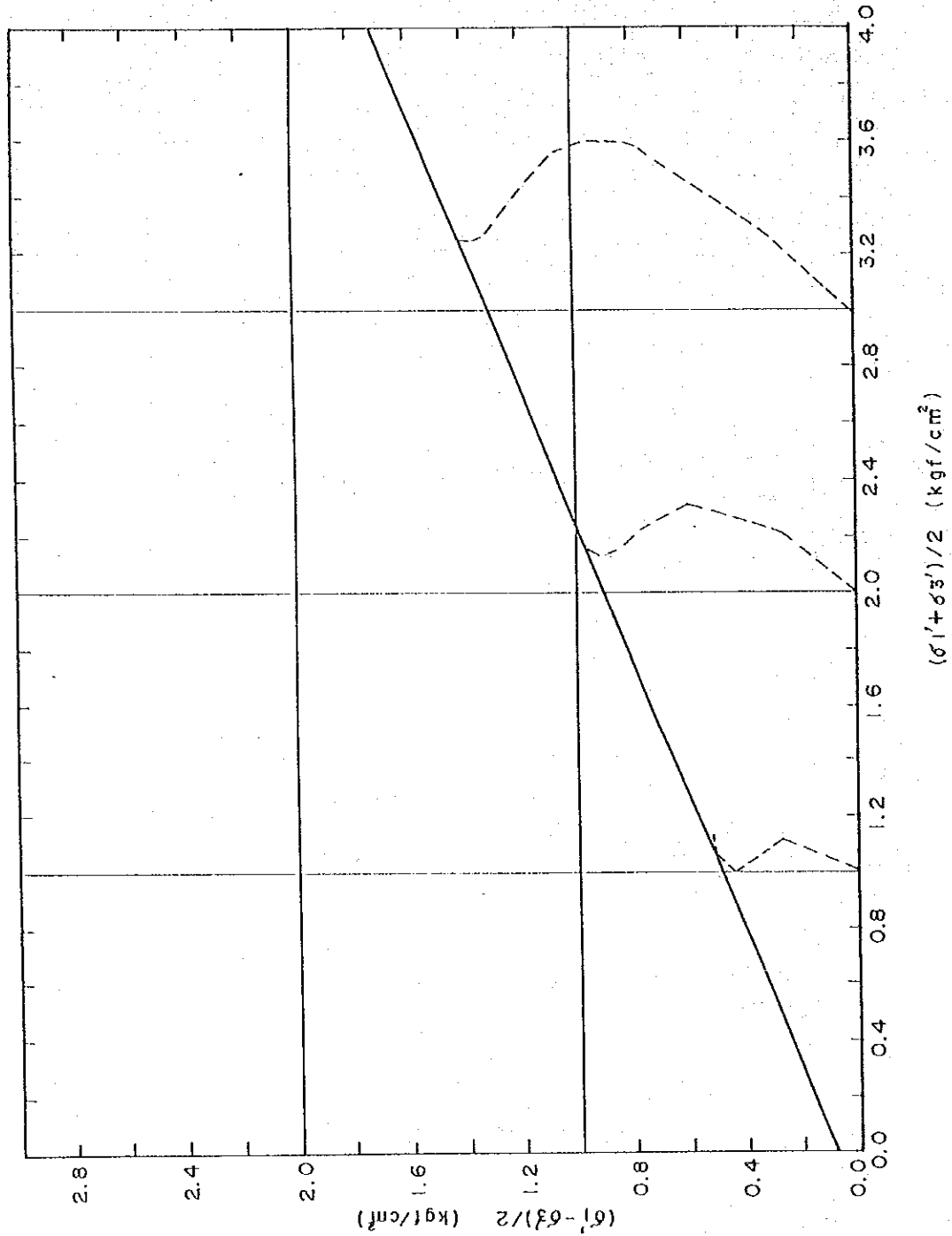
SPECIMEN No.		1 □	2 ◇	3 △
MINOR PRINCIPAL STRESS $\sigma_3$ kgf/cm <sup>2</sup>		1.00	2.00	3.00
PRINCIPAL STRESS DIFFERENCE AT PEAK	PRINCIPAL STRESS DIFFERENCE $(\sigma_1 - \sigma_3)$ kgf/cm <sup>2</sup>	1.009	1.983	2.805
	COMPRESSION STRAIN $\epsilon_f$ %	11.767	11.798	13.110
	PORE WATER PRESSURE $U_f$ kgf/cm <sup>2</sup>	0.441	0.797	1.148
	PORE PRESSURE COEFFICIENT $A_f$	0.437	0.402	0.409



TRIAXIAL COMPRESSION TEST (CU-TEST) (STRESS PATH)

SAMPLING SPOT & PLACE: KHLONG - SI - YAT DATE 29 - 3 - 1990  
 SAMPLE No. & DEPTH: No. TPB - 1 (1.90 - 5.50)

$P = 0.08 \text{ (kgf/cm}^2\text{)}$   $\alpha = 24.21 \text{ DEGREE}$

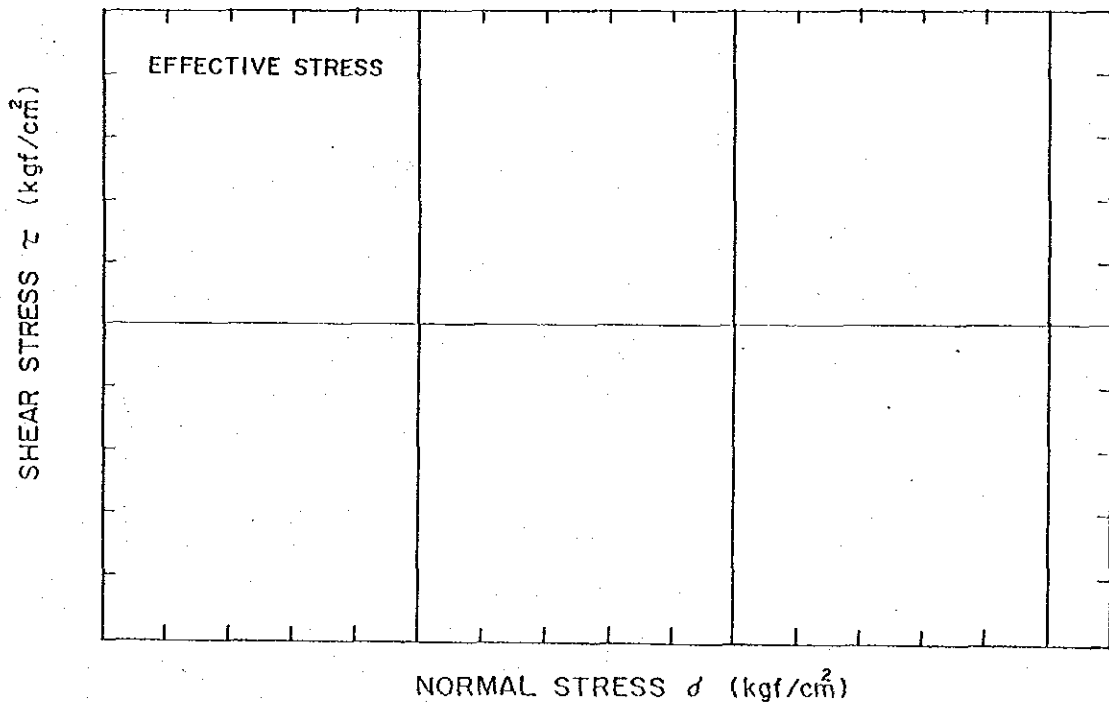
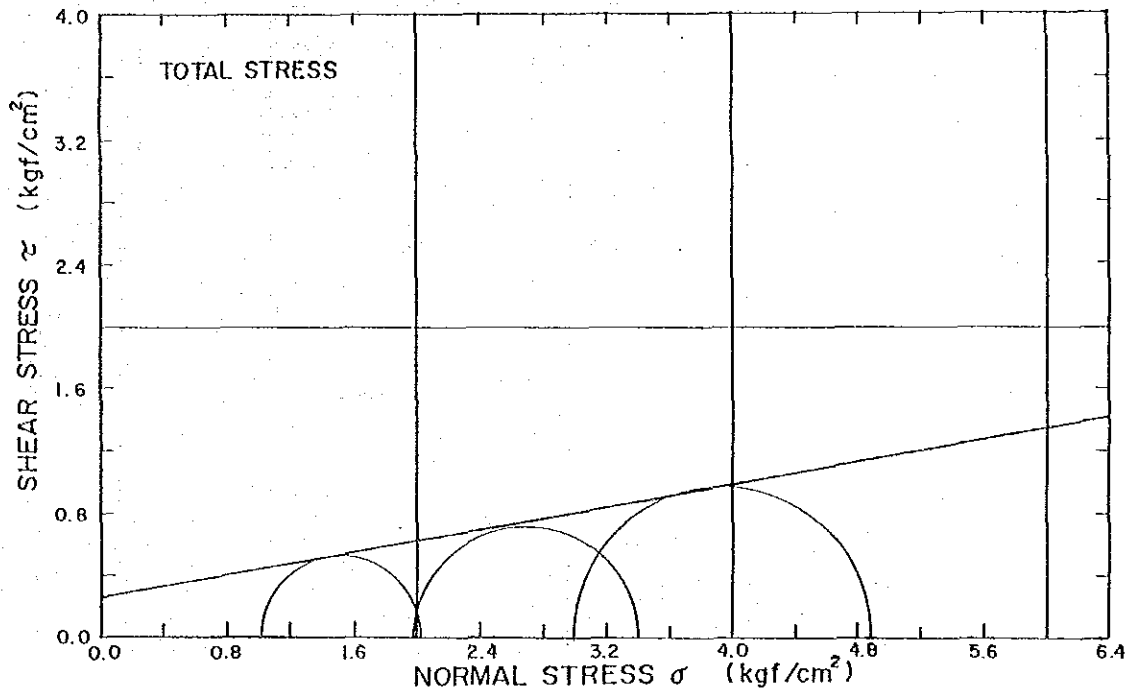




## TRIAxIAL COMPRESSION TEST (UU-TEST)

SAMPLING SPOT & PLACE: KHLONG - SI - YAT DATE 6 - 4 - 1990  
 SAMPLE No. & DEPTH: No. TPB - 2 (0.00 - 4.00)

STRENGTH PARAMETER	SCOPE OF STRESS	C kgf/cm	$\phi$ DEGREE	$\tan \phi$	$C'$ kgf/cm	$\phi'$ DEGREE
NORMALLY CONSOLIDATED REGION		0.244	10.242	0.181		
OVER CONSOLIDATED REGION						



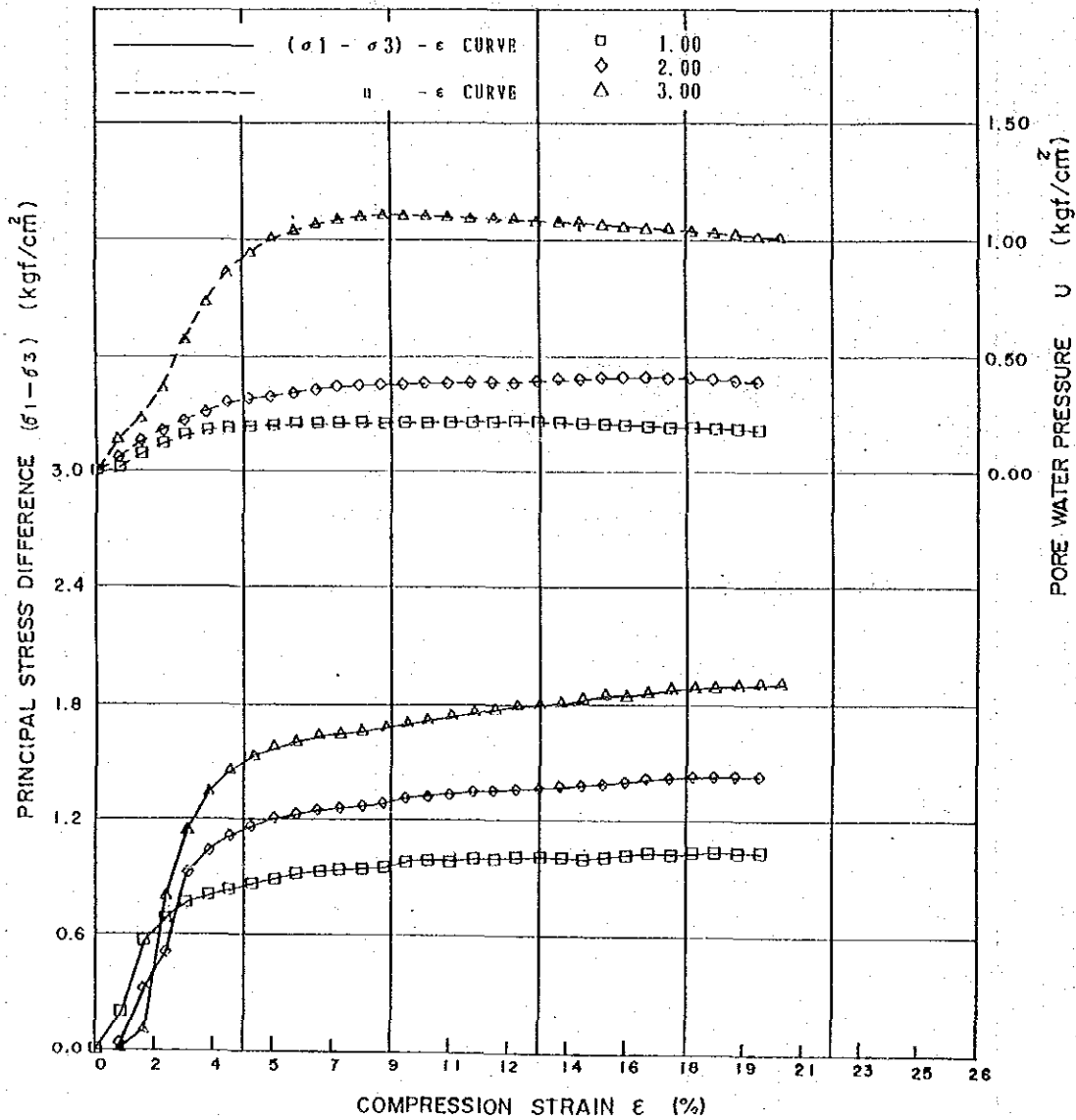
## TRIAXIAL COMPRESSION TEST (UU-TEST)

SAMPLING SPOT

B PLACE: TPB - 2 (0.00 - 4.00)

DATE 6-4-1990

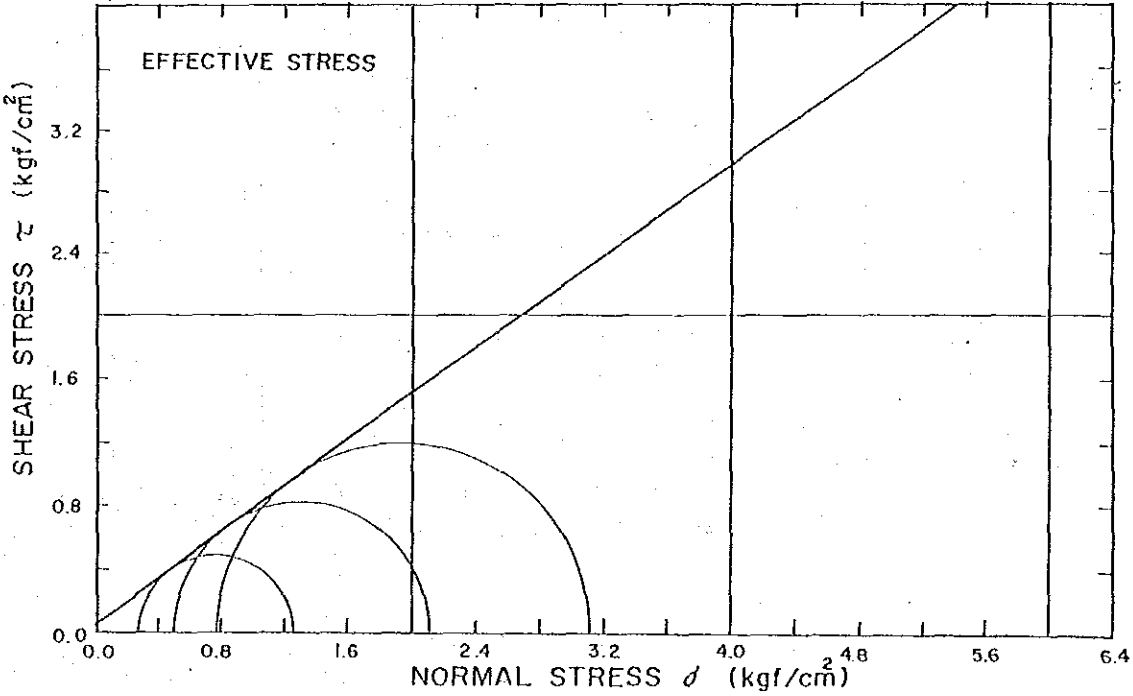
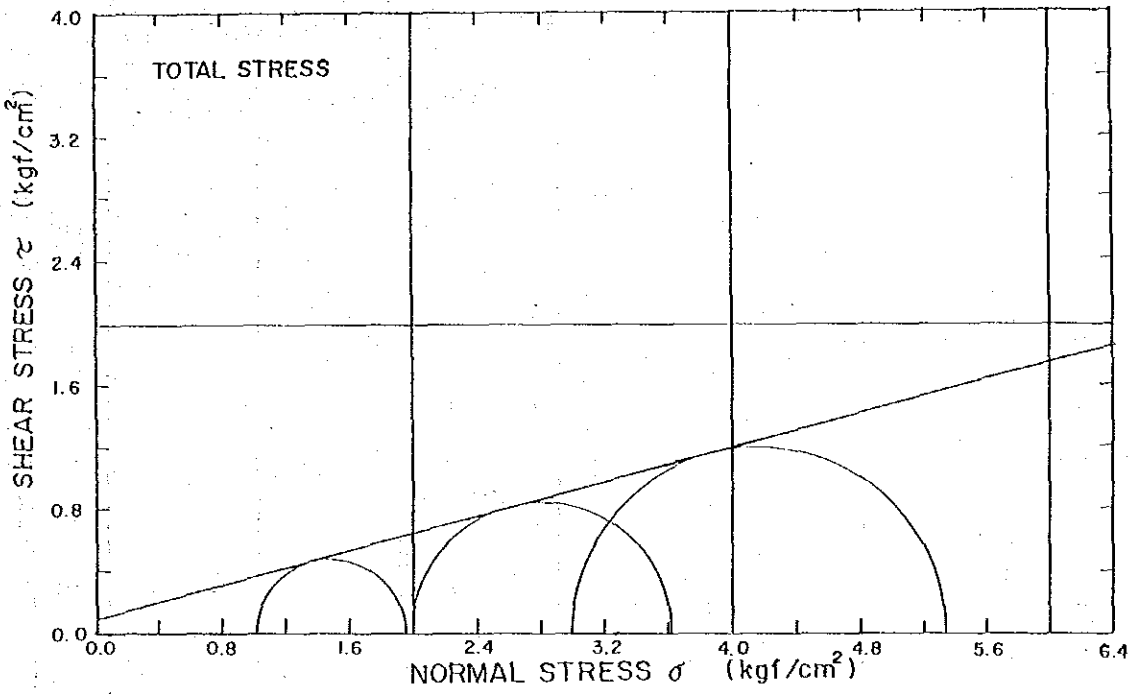
SPECIMEN No.	1 □	2 ◇	3 △	
MINOR PRINCIPAL STRESS $\sigma_3$ kgf/cm <sup>2</sup>	1.00	2.00	3.00	
PRINCIPAL STRESS DIFFERENCE AT PEAK	PRINCIPAL STRESS DIFFERENCE $(\sigma_1 - \sigma_3)$ kgf/cm <sup>2</sup>			
	1.047	1.391	1.912	
	COMPRESSION STRAIN $\epsilon_f$ %			
	18.408	15.092	19.898	
	PORE WATER PRESSURE $U_f$ kgf/cm <sup>2</sup>			
PORE PRESSURE COEFFICIENT $A_f$				



## TRIAxIAL COMPRESSION TEST (CU-TEST)

SAMPLING SPOT & PLACE: KHLONG - SI - YAT DATE 8 - 3 - 1990  
 SAMPLE No. & DEPTH: No. TPB - 2 (0.00 - 4.00)

STRENGTH PARAMETER	SCOPE OF STRESS	C kgf/cm	$\phi$ DEGREE	$\tan \phi$	$C'$ kgf/cm	$\phi'$ DEGREE
NORMALLY CONSOLIDATED REGION		0.098	15.010	0.268	0.049	35.965
OVER CONSOLIDATED REGION						

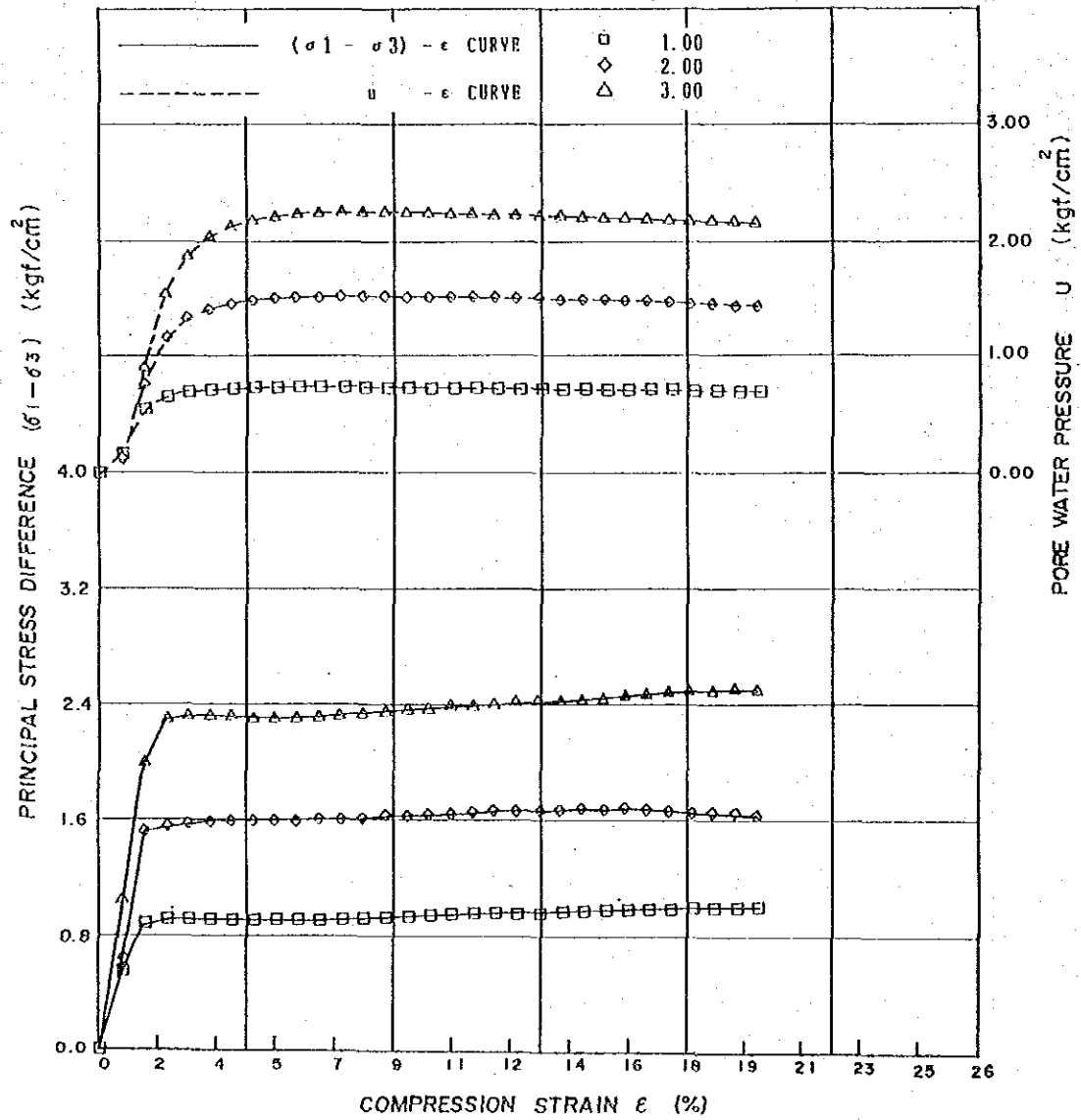


# TRIAxIAL COMPRESSION TEST (CU-TEST)

SAMPLING SPOT & PLACE: TPB-2 (0.00-4.00)

DATE B-3-1990

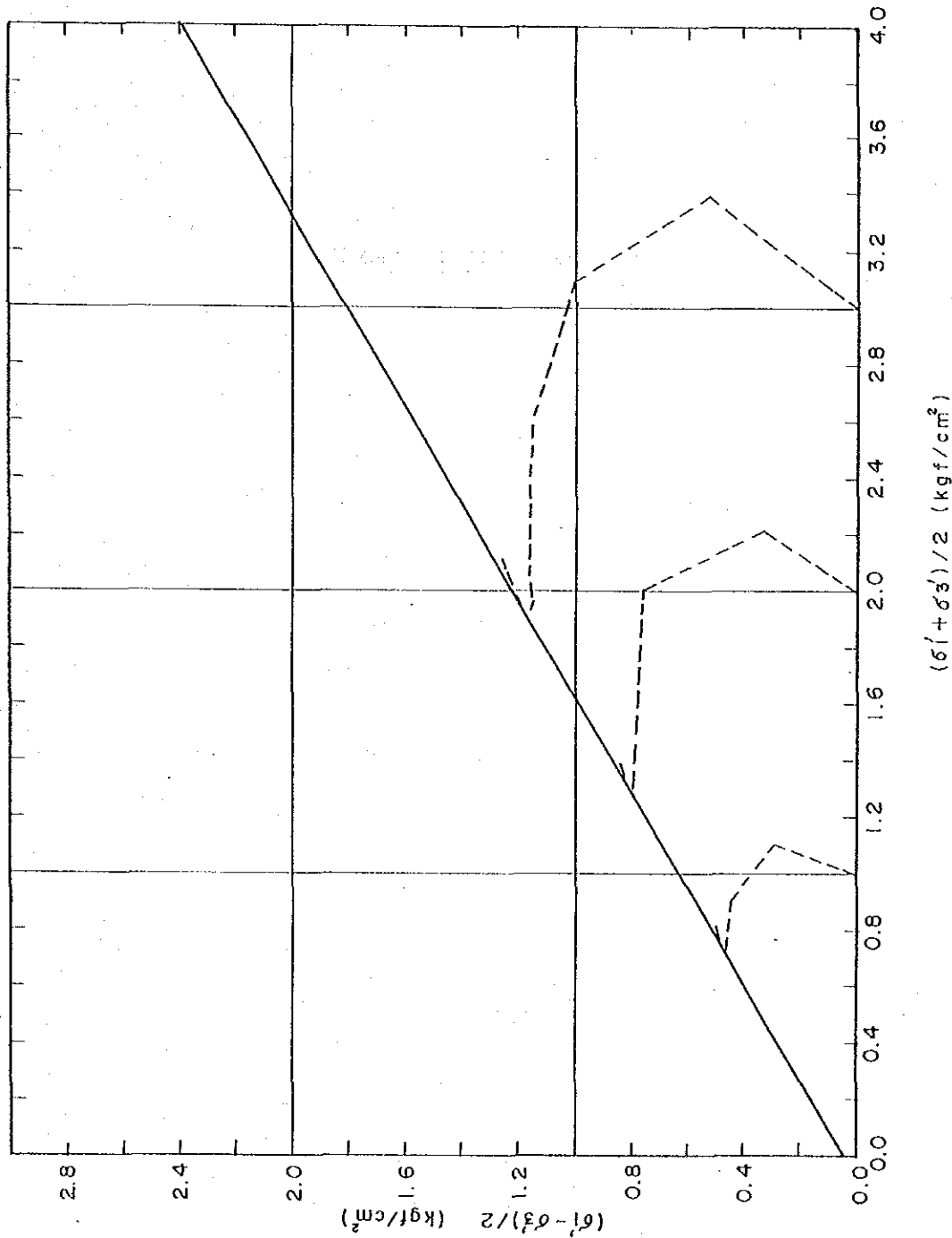
SPECIMEN No.		1 □	2 ◇	3 △
MINOR PRINCIPAL STRESS $\sigma_3$ kgf/cm <sup>2</sup>		1.00	2.00	3.00
PRINCIPAL STRESS DIFFERENCE AT PEAK	PRINCIPAL STRESS DIFFERENCE $(\sigma_1 - \sigma_3)$ kgf/cm <sup>2</sup>	0.985	1.830	2.363
	COMPRESSION STRAIN $\epsilon_f$ %	11.186	8.530	9.174
	PORE WATER PRESSURE $U_f$ kgf/cm <sup>2</sup>	0.722	1.505	2.232
	PORE PRESSURE COEFFICIENT $A_f$	0.748	0.923	0.945



# TRIAXIAL COMPRESSION TEST (CU-TEST) (STRESS PATH)

SAMPLING SPOT & PLACE: KHLONG - SI - YAT DATE 8 - 3 - 1990  
 SAMPLE No. & DEPTH: No. TPB - 2 (0.00 - 4.40)

$P = 0.04 \text{ (kgf/cm}^2\text{)} \quad \alpha = 35.98 \text{ DEGREE}$



C.2.7. ROCK TEST



คุณสมบัติของหินแยกและกรวด

ลำดับงานที่ CM. 113/2533

โครงการ ป้ายสำรวจธรณีวิทยา

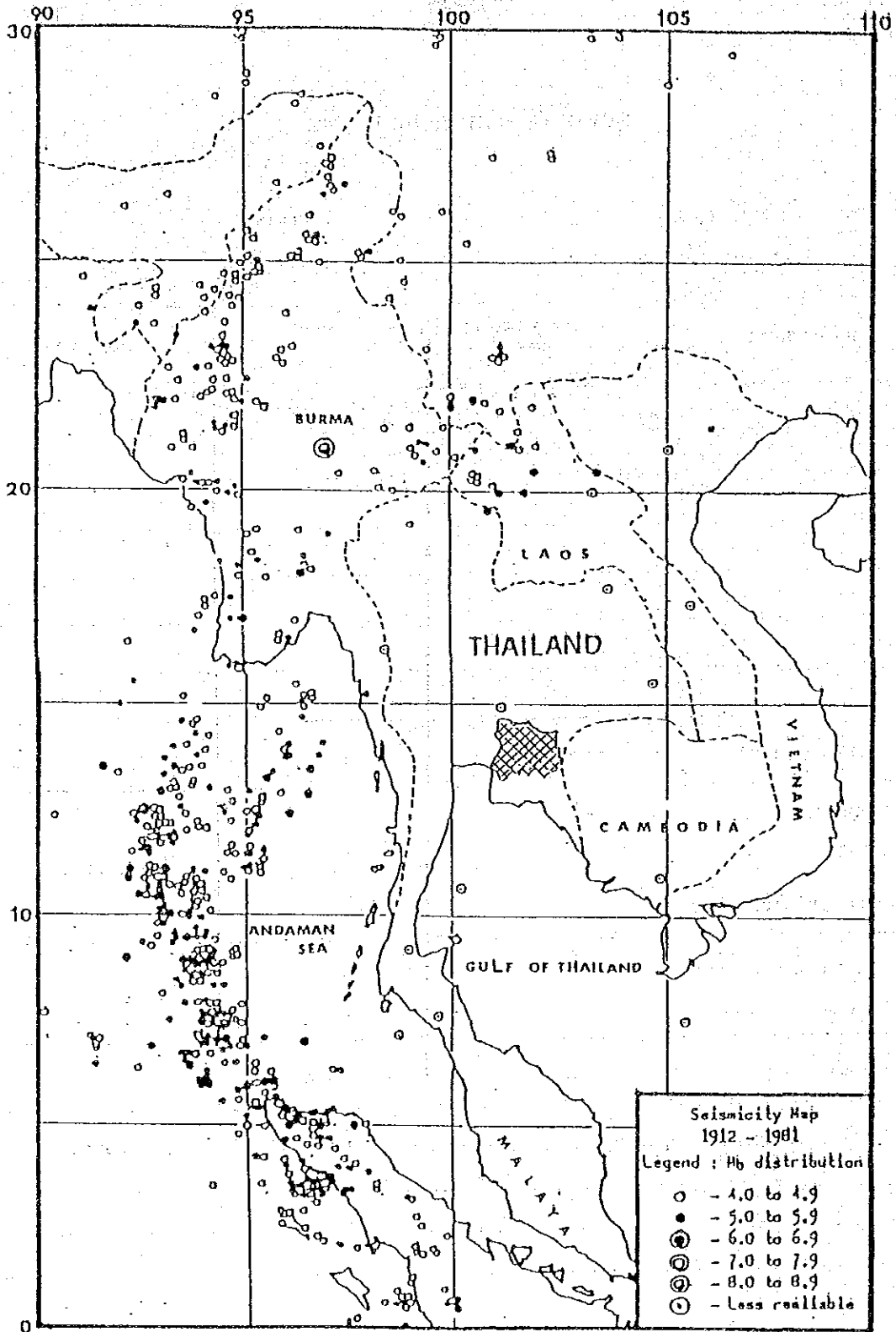
วันที่ 16 มกราคม 2533

ทดสอบโดย

ตรวจสอบโดย

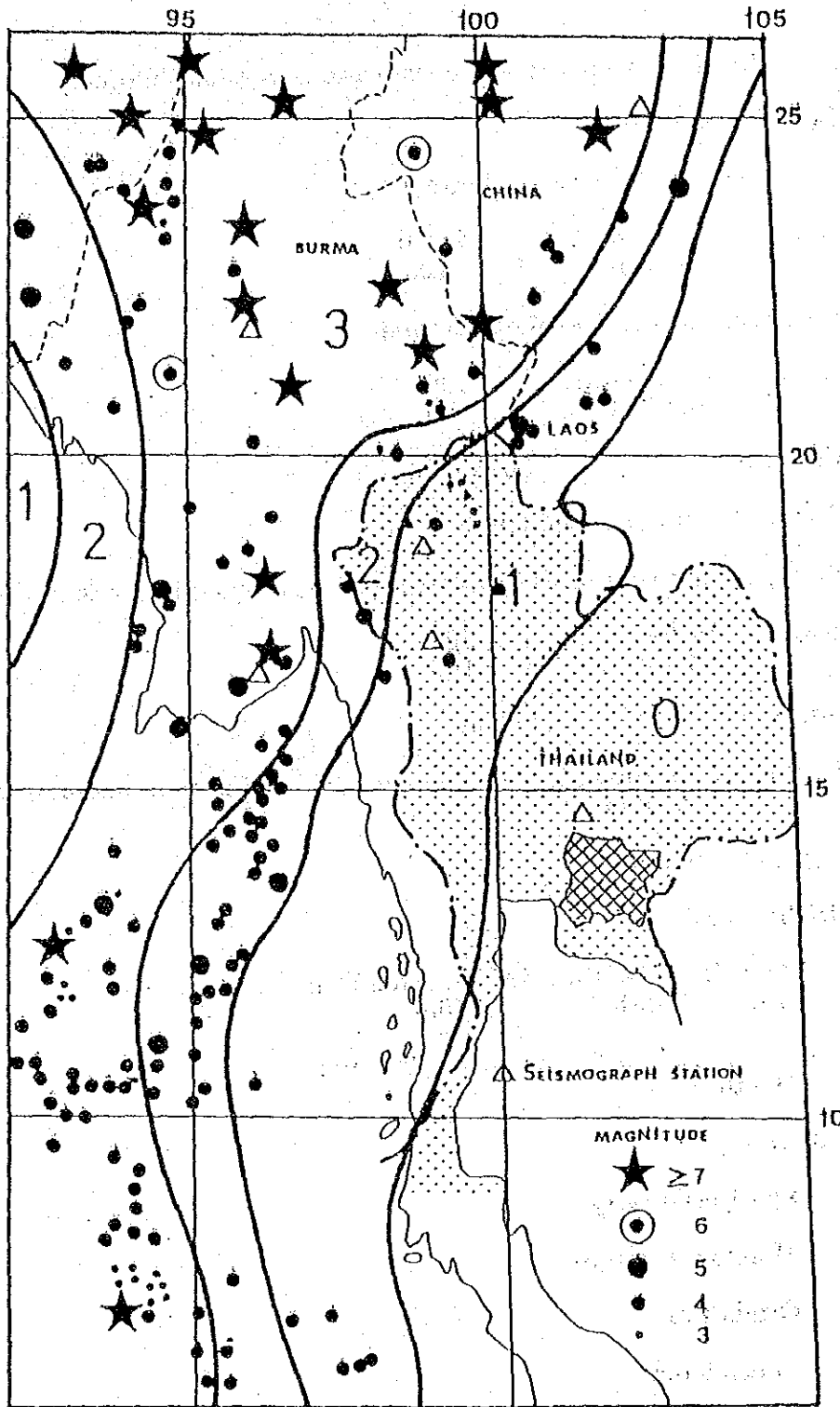
ขนาดตะแกรง	ส่วนสามตะแกรงโดยน้ำหนัก - ร้อยละ		เกณฑ์กำหนด
	Q 1	Q 2	
	Khao Ba Ra Rum Quarry (3 km far from dams site)	Khao Yai Ma Noi Quarry (8 km far from dams site)	
ความกว้างจำเพาะ	2.639	2.608	
ส่วนสูญหายเนื่องจาก การขัดสี โดย เครื่องลอสแอนเจลิส-ร้อยละ	-	-	ไม่มากกว่า 50
ส่วนสูญหายเมื่อทดลองด้วย ไซเคียมซิลเฟด-ร้อยละ	-	-	ไม่มากกว่า 12
การดูดน้ำ-ร้อยละ	0.575	2.059	
หมายเหตุ	ดินบริเวณ Quarry ของโครงการหลวง (อ.วังน้ำเขียว) จ. นครราชสีมา		
	จังหวัดเชียงใหม่		

### C.3.1. SEISMICITY MAP





C.3.2. SEISMIC PROBABILITY MAP



**LEGEND**

★ ≥ 7  
 ○ 6  
 ● 5  
 ● 4  
 ● 3

△ Seismograph station

ZONE 0 : No damage  
 ZONE 1 : Minor damage  
 ZONE 2 : Moderate damage  
 ZONE 3 : Major damage

#### **C.4.1. Bang Pakong Diversion Dam**

**(1) Topographic Survey for Diversion Dam and Diversion Channel**

Scale 1 : 2,000

**(2) Profile Survey for Diversion Channel**

Scale Horizontal 1 : 2,000  
Vertical 1 : 100

**(3) Sectional Survey for Diversion Channel**

Scale Horizontal 1 : 500  
Vertical 1 : 100

Interval 100 m

**(4) Additional River Cross Section Survey**

- to clarify the dike of the river upstream of the proposed Diversion Dam

Scale Horizontal 1 : 500  
Vertical 1 : 100

Interval 200 m

Section from the proposed diversion dam to the conjunction of the Tha Lat river

**(5) Geological Investigation**

**1) Drilling Test**

- Additional Drilling for Diversion Dam  
2 hole, Depth : 50m
- At least 3 hole for Diversion Channel  
Depth : 20m

**2) Laboratory Test**

- Specific Gravity
- Moisture Content
- Gradation
- Consistency
- Standard Proctor Compaction
- Permeability

**(6) Survey for navigation frequency**

-to clarify the transportation on the river

Station : at the Chachoengsao bridge

at the proposed diversion dam site

at Bang Khla

**(7) Transportation Survey**

-to clarify the traffic on the Chachoengsao bridge and Route 304 and 315,

Station : at the Chachoengsao bridge

at the junction of route 304 and 315



### C.4.3. Khlong Si Yat Dam

(1) Plan Map Survey

<u>Location</u>	<u>Quantity</u>
Quarry	40 ha
Spillway	60 ha
<b>Total</b>	<b>100 ha</b>

(2) Strip Topography Survey

<u>Location</u>	<u>Quantity</u>
Road	23 km
Others	2 km
<b>Total</b>	<b>25 km</b>

(3) Seismic Prospecting

<u>Location</u>	<u>Quantity</u>
Quarry	2.0 km
Main Dam : dam axis	3.2 km
Main Dam : cross section	0.8 km
Saddle Dam	1.0 km
Outlet Works	1.0 km
Spillway	1.0 km
<b>Total</b>	<b>9.0 km</b>

(0.4 km × 2)

(4) Core Drilling

<u>Location</u>	<u>Quantity</u>	
Dam	20 m × 60 Nos.	1,200 m
Spillway & Others	20 m × 30 Nos.	600 m
Quarry	50 m × 4 Nos.	200 m
<b>Total</b>		<b>2,000 m</b>

(5) Test Pit & Auger Drilling

<u>Location</u>	<u>Test Pit</u>	<u>Auger Drilling</u>
Damsite	20 Nos.	300 m
Borrow Area	40 Nos.	600 m
Spillway	30 Nos.	500 m
<b>Total</b>	<b>90 Nos.</b>	<b>1,400 m</b>

(6) Laboratory Test

Location	Physical Test	Mechanical Test
Damsite	20 Nos.	10 Nos.
Borrow Area	40 Nos.	20 Nos.
Spillway	30 Nos.	20 Nos.
Quarry	10 Nos.	10 Nos.
Total	100 Nos.	60 Nos.

**APPENDIX-D. IRRIGATION, DRAINAGE AND  
WATER BALANCE**





## APPENDIX-D IRRIGATION, DRAINAGE AND WATER BALANCE

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## D.1 IRRIGATION WATER REQUIREMENT

The following tables and figures present procedures employed in estimating irrigation water requirement:

Table D-1-1 Crop Water Requirement by Modified Penman (Prachinburi)

Table D-1-2 Crop Water Requirement by Modified Penman (Chonburi)

Figure D-1-1 Kc and Kp values by crop

Figure D-1-2 Field Water Requirement (Wet Season Paddy:Broadcasted)

Figure D-1-3 Field Water Requirement (Wet Season Paddy:Transplanted)

Figure D-1-4 Field Water Requirement (Dry Season Paddy:Broadcasted)

Figure D-1-5 Field Water Requirement (Dry Season Paddy:Transplanted)

Figure D-1-6 Field Water Requirement (Soybean:Dry Season)

Figure D-1-7 Field Water Requirement (Groundnuts:Dry Season)

Figure D-1-8 Field Water Requirement (Mungbean:Dry Season)

Figure D-1-9 Rainfall - Effective Rainfall Relationship

Figure D-1-10 Proposed Cropping Calendar

Table D-1-1 Crop Water Requirement by Modified Penman (Prachinburi)

STATION: PRACHINBURI

Station Index: 48430

Latitude: 14° 03' N

Longitude: 101° 22' E

Elevation of Station above MSL : 5m

Height of Barometer above MSL : 6m

Height of Thermometer above Ground: 1.20m

Height of Wind Vane above Ground : 11.00m

ITEM	(Unit)	JAN.	FEB.	MAR.	APR.	MAY	JUNE	JULY	AUG.	SEP.	OCT.	NOV.	DEC.
1 Tmax	( °C )	32.4	34.1	35.7	36.0	34.4	33.0	32.2	31.8	31.6	31.8	31.8	31.6
2 Tmin	( °C )	19.4	22.1	23.9	24.9	25.1	24.9	24.6	24.6	24.5	24.3	22.4	19.9
3 Tmean	( °C )	25.9	28.1	29.8	30.5	29.8	29.0	28.4	28.2	28.1	28.1	27.1	25.8
4 ea	(mbar)	33.4	38.0	41.9	43.7	41.9	40.1	38.7	38.3	38.0	38.0	35.9	33.2
5 RHmax	(%)	83.1	86.2	87.9	89.4	91.4	92.7	93.5	93.9	94.1	90.0	84.5	81.9
6 RHmin	(%)	40.7	43.3	44.5	49.9	59.2	64.5	67.1	68.6	69.1	63.5	52.9	44.7
7 RHmean	(%)	61.9	64.8	66.2	69.7	75.3	78.6	80.3	81.3	81.6	76.8	68.7	63.3
8 ed=ea*RHmean/100	(mbar)	20.7	24.6	27.7	30.5	31.6	31.5	31.1	31.1	31.0	29.2	24.7	21.0
9 (ea-ed)	(mbar)	12.7	13.4	14.2	13.2	10.3	8.6	7.6	7.2	7.0	8.8	11.2	12.2
10 U (h=11.00m)	(knots)	3.6	3.3	2.9	2.4	2.4	2.1	2.1	2.3	2.2	3.0	4.1	4.3
11 U (h=2m)	(km/day)	112	103	90	75	75	65	65	72	68	93	128	134
12 f(u)=0.27(1+U/100)		0.57	0.55	0.51	0.47	0.47	0.45	0.45	0.46	0.45	0.52	0.62	0.63
13 1-w (Elevation=5m)		0.25	0.23	0.22	0.21	0.22	0.22	0.23	0.23	0.23	0.23	0.24	0.25
14 (1-w)f(u)(ea-ed)	(mm/day)	1.81	1.70	1.59	1.30	1.07	0.85	0.79	0.76	0.72	1.05	1.67	1.92
15 Ra (14.03'N)	(mm/day)	12.4	13.6	14.9	15.7	15.8	15.7	15.7	15.7	15.1	14.1	12.8	12.0
16 Cloudiness (0-10)		3.7	4.7	5.3	6.3	7.6	8.3	8.4	8.7	8.2	6.6	4.7	3.8
17 n/N		0.68	0.58	0.54	0.47	0.34	0.26	0.24	0.20	0.27	0.44	0.58	0.67
18 0.25+0.5n/N		0.59	0.54	0.52	0.49	0.42	0.38	0.37	0.35	0.39	0.47	0.54	0.59
19 Rs=15*18	(mm/day)	7.32	7.34	7.75	7.69	6.64	5.97	5.81	5.50	5.89	6.63	6.91	7.08
20 Rns=0.75Rs	(mm/day)	5.49	5.51	5.81	5.77	4.98	4.48	4.36	4.13	4.42	4.97	5.18	5.31
21 f(T)		15.9	16.3	16.7	16.8	16.7	16.5	16.4	16.3	16.3	16.3	16.1	15.9
22 f(ed)=0.34-0.044sqrt(ed)		0.14	0.12	0.11	0.10	0.09	0.09	0.09	0.09	0.10	0.10	0.12	0.14
23 f(n/N)=0.1+0.9n/N		0.71	0.62	0.59	0.52	0.41	0.33	0.32	0.28	0.34	0.50	0.62	0.70
24 Rn1=21*22*23	(mm/day)	1.58	1.21	1.08	0.87	0.62	0.49	0.47	0.41	0.55	0.82	1.20	1.56
25 Rn=Rns-Rn1	(mm/day)	3.91	4.30	4.73	4.90	4.36	3.99	3.89	3.72	3.87	4.15	3.98	3.75
26 W		0.75	0.77	0.78	0.79	0.78	0.78	0.77	0.77	0.77	0.77	0.76	0.75
27 W*Rn	(mm/day)	2.93	3.31	3.69	3.87	3.40	3.11	3.00	2.86	2.98	3.20	3.02	2.81
28 14+27	(mm/day)	4.74	5.01	5.28	5.17	4.47	3.96	3.79	3.62	3.70	4.25	4.69	4.73
29 c		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
30 ETO	(mm/day)	4.74	5.01	5.28	5.17	4.47	3.96	3.79	3.62	3.70	4.25	4.69	4.73

Table D-1-2 Crop Water Requirement by Modified Penman (Chonburi)

STATION: CHONBURI  
 Station Index: 48459  
 Latitude: 13° 22' N  
 Longitude: 100° 59' E  
 Elevation of Station above MSL : 1m  
 Height of Barometer above MSL : 2m  
 Height of Thermometer above Ground: 1.50m  
 Height of Wind Vane above Ground : 13.45m

ITEM	(Unit)	JAN.	FEB.	MAR.	APR.	MAY	JUNE	JULY	AUG.	SEP.	OCT.	NOV.	DEC.
1 Tmax	( °C)	31.7	32.4	33.5	34.3	33.6	32.7	32.2	31.9	31.6	31.7	31.6	31.7
2 Tmin	( °C)	20.1	22.5	24.3	25.4	25.5	25.5	25.1	24.9	24.3	23.6	22.1	20.4
3 Tmean	( °C)	25.9	27.5	28.9	29.9	29.6	29.1	28.7	28.4	28.0	27.7	26.9	26.1
4 ea	(mbar)	33.4	36.8	39.9	42.2	41.5	40.3	39.4	38.7	37.8	37.2	35.5	33.8
5 RHmax	(%)	84.4	87.4	86.8	87.0	88.0	87.2	88.3	89.2	92.2	92.3	88.0	83.3
6 RHmin	(%)	48.8	54.8	55.2	55.3	59.3	60.4	61.4	62.4	65.8	64.3	55.0	47.5
7 RHmean	(%)	66.6	71.1	71.0	71.2	73.7	73.8	74.9	75.8	79.0	78.3	71.5	65.4
8 ed=ea*RHmean/100	(mbar)	22.2	26.2	28.3	30.0	30.6	29.7	29.5	29.3	29.9	29.1	25.4	22.1
9 (ea-ed)	(mbar)	11.2	10.6	11.6	12.2	10.9	10.6	9.9	9.4	7.9	8.1	10.1	11.7
10 U (h=13.45m)	(knots)	4.7	5.2	5.3	4.8	4.2	4.9	4.6	4.6	3.5	3.5	4.9	5.1
11 U (h=2m)	(km/day)	146	162	165	149	131	152	143	143	109	109	152	159
12 f(u)=0.27(1+U/100)		0.66	0.71	0.72	0.67	0.62	0.68	0.66	0.66	0.56	0.56	0.68	0.70
13 1-w (Elevation=1m)		0.25	0.23	0.23	0.22	0.22	0.22	0.23	0.23	0.23	0.23	0.24	0.25
14 (1-w)f(u)(ea-ed)	(mm/day)	1.85	1.73	1.92	1.80	1.49	1.59	1.50	1.43	1.02	1.04	1.65	2.05
15 Ra (13.22° N)	(mm/day)	12.5	13.7	15.0	15.7	15.8	15.6	15.6	15.7	15.1	14.2	13.0	12.2
16 Cloudiness (0-10)		4.0	4.3	4.5	5.4	7.3	8.0	8.1	8.4	8.2	7.0	5.2	4.1
17 n/N		0.65	0.62	0.60	0.53	0.37	0.30	0.29	0.24	0.27	0.40	0.54	0.64
18 0.25+0.5n/N		0.58	0.56	0.55	0.52	0.44	0.40	0.40	0.37	0.39	0.45	0.52	0.57
19 Rs=15*18	(mm/day)	7.25	7.67	8.25	8.16	6.95	6.24	6.24	5.81	5.89	6.39	6.76	6.95
20 Rns=0.75Rs	(mm/day)	5.44	5.75	6.19	6.12	5.21	4.68	4.68	4.36	4.42	4.79	5.07	5.21
21 f(T)		15.9	16.2	16.5	16.7	16.6	16.5	16.4	16.4	16.3	16.2	16.1	15.9
22 f(ed)=0.34-0.044sqrt(ed)		0.13	0.11	0.11	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.12	0.13
23 f(n/N)=0.1+0.9n/N		0.69	0.66	0.64	0.58	0.43	0.37	0.36	0.32	0.34	0.46	0.59	0.68
24 Rn1=21*22*23	(mm/day)	1.43	1.18	1.16	0.97	0.71	0.61	0.59	0.52	0.55	0.75	1.14	1.41
25 Rn=Rns-Rn1	(mm/day)	4.01	4.57	5.03	5.15	4.50	4.07	4.09	3.84	3.87	4.04	3.93	3.80
26 W		0.75	0.77	0.77	0.78	0.78	0.78	0.77	0.77	0.77	0.77	0.76	0.75
27 W*Rn	(mm/day)	3.01	3.52	3.87	4.02	3.51	3.17	3.15	2.96	2.98	3.11	2.99	2.85
28 14+27	(mm/day)	4.86	5.25	5.79	5.82	5.00	4.76	4.65	4.39	4.00	4.15	4.64	4.90
29 c		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
30 ETO	(mm/day)	4.86	5.25	5.79	5.82	5.00	4.76	4.65	4.39	4.00	4.15	4.64	4.90

Figure D-1-1 Kc and Kp values by crop

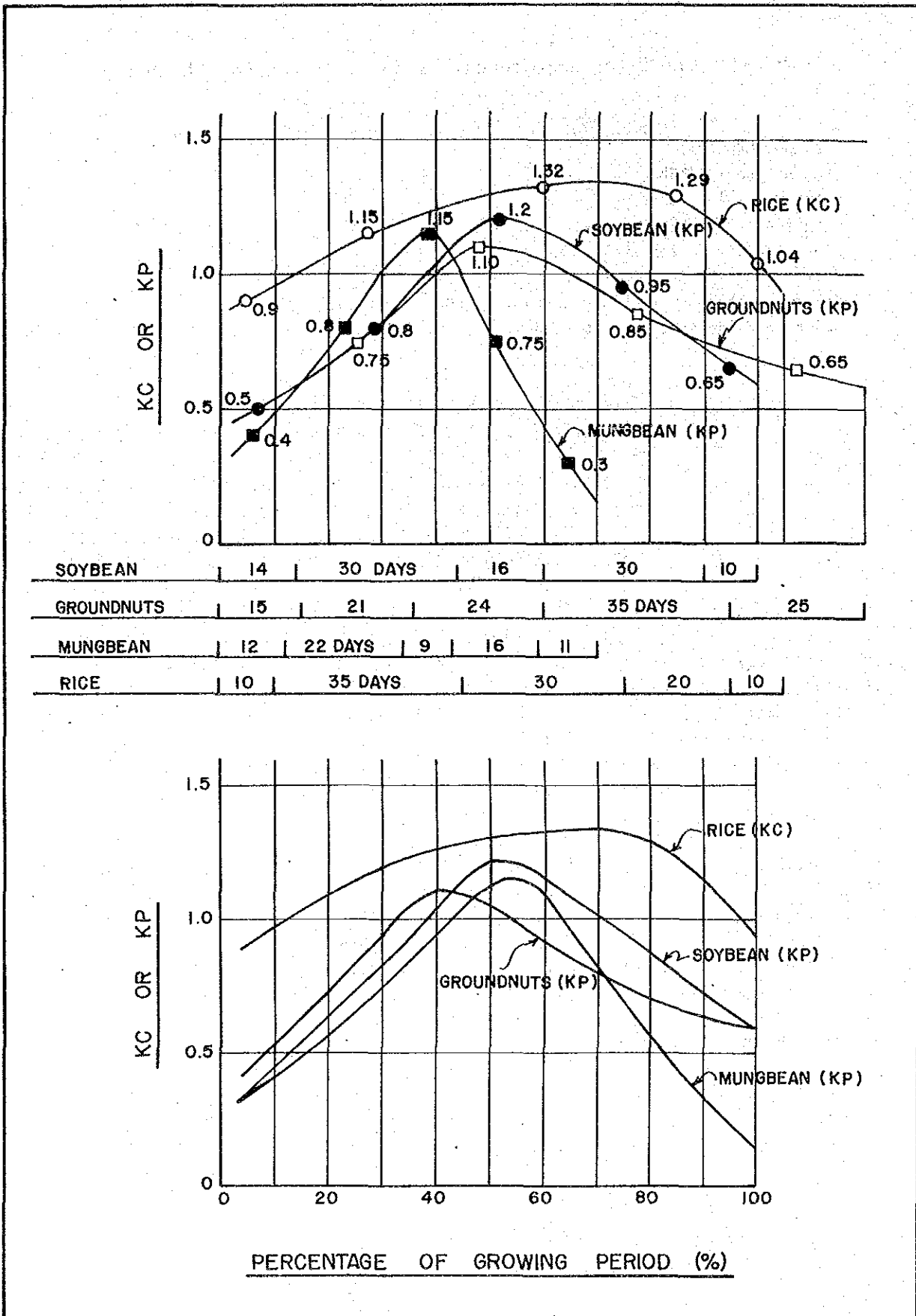


Figure D-1-2 Field Water Requirement (Wet Season Paddy:Broadcasted)

Month	July			August			September			October			November			December				
	10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	
Cropping Pattern																				
1. ELEMENT Z of Growing Season	10	19	29	38	48	57	67	76	86	95	100									
Crop Coefficient (Kc)	0.96	1.07 0.96	1.17 1.07 0.96	1.25 1.17 1.07 0.96	1.29 1.17 1.07 0.96	1.32 1.25 1.17 1.07	1.33 1.29 1.25 1.17	1.32 1.33 1.32 1.29	1.20 1.33 1.32 1.29	1.05 1.20 1.32 1.33	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32				
Kc Average	0.96	1.02	1.07	1.11	1.15	1.22	1.27	1.30	1.29	1.24	1.17	1.13	1.06	0.99	0.93					
ETD by Penman (mm/day)		4.22			4.01			3.85			4.20			4.67						
ETc (mm/day)	4.05	4.30	4.52	4.45	4.61	4.89	4.89	5.01	4.97	5.21	4.91	4.75	4.95	4.62	4.34					
Percolation (P, mm/day)		2.00			2.00			2.00			2.00			2.00						
ETc + P (mm/day)	6.05	6.30	6.52	6.45	6.61	6.89	6.89	7.01	6.97	7.21	6.91	6.75	6.95	6.62	6.34					
Initial Leaching (mm)		50																		
Land Preparation (mm)		150																		
2. EQUATION Initial Leaching	2/9	2/9	2/9	2/9	1/9															
Land Preparation	2/9	2/9	2/9	2/9	1/9															
Normal Irrigation	1/9	3/9	5/9	7/9	35/36	1/1	1/1	1/1	1/1	1/1	35/36	7/9	5/9	3/9	1/9					
3. WATER REQUIREMENT Initial Leaching (mm)	11.1	11.1	11.1	11.1	5.6															
Land Preparation (mm)	33.3	33.3	33.3	33.3	16.8															
Normal Irrigation (mm)	6.7	21.0	36.2	50.2	64.3	68.9	68.9	70.1	69.7	72.1	67.2	52.5	38.6	22.1	7.0					
Requirement (mm/month)	197.1			250.2			208.7			191.8			67.7							

Figure D-1-3 Field Water Requirement (Wet Season Paddy:Transplanted)

Month	July			August			September			October			November			December				
	10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	
Cropping Pattern																				
1. ELEMENT Z of Growing Season	10	19	29	38	48	57	67	76	86	95	100									
Crop Coefficient (Kc)	0.96	1.07 0.96	1.17 1.07 0.96	1.25 1.17 1.07 0.96	1.29 1.17 1.07 0.96	1.32 1.25 1.17 1.07	1.33 1.29 1.25 1.17	1.32 1.33 1.32 1.29	1.20 1.33 1.32 1.29	1.05 1.20 1.32 1.33	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32	0.93 1.05 1.20 1.32				
Kc Average	0.96	1.02	1.07	1.11	1.15	1.22	1.27	1.30	1.29	1.24	1.17	1.13	1.06	0.99	0.93					
ETD by Penman (mm/day)		4.22			4.01			3.85			4.20			4.67						
ETc (mm/day)	4.05	4.30	4.52	4.45	4.61	4.89	4.89	5.01	4.97	5.21	4.91	4.75	4.95	4.62	4.34					
Percolation (P, mm/day)		2.00			2.00			2.00			2.00			2.00						
ETc + P (mm/day)	6.05	6.30	6.52	6.45	6.61	6.89	6.89	7.01	6.97	7.21	6.91	6.75	6.95	6.62	6.34					
Initial Leaching (mm)		50																		
Land Preparation (mm)		150																		
2. EQUATION Initial Leaching			2/9	2/9	2/9	2/9	1/9													
Land Preparation			1/9	2/9	2/9	2/9	7/36	1/36												
Normal Irrigation			1/9	3/9	5/9	7/9	35/36	1/1	1/1	35/36	7/9	5/9	3/9	1/9						
3. WATER REQUIREMENT Initial Leaching (mm)			11.1	11.1	11.1	11.1	5.6													
Land Preparation (mm)			16.7	33.3	33.3	33.3	29.2	4.2												
Normal Irrigation (mm)				7.2	22.0	38.3	53.6	68.2	69.7	72.1	67.2	52.5	38.6	22.1	7.0					
Requirement (mm/month)	27.8			200.7			230.5			191.8			67.7							

Figure D-1-4 Field Water Requirement (Dry Season Paddy:Broadcasted)

Month	December			January			February			March			April			May		
10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3
Cropping Pattern																		
1. ELEMENT I of Growing Season	10	19	29	38	48	57	67	76	86	95	100							
Crop Coefficient (Kc)	0.96	1.07 0.96	1.17 1.07 0.96	1.25 1.17 0.96	1.29 1.25 1.07	1.32 1.29 1.17	1.33 1.32 1.25	1.32 1.33 1.29	1.20 1.32 1.25	1.05 1.20 1.32	0.93 1.05 1.32			0.93				
Kc Average	0.96	1.02	1.07	1.11	1.15	1.22	1.27	1.30	1.29	1.24	1.17	1.13	1.06	0.99	0.93			
ETD by Penman (mm/day)	4.82			4.80			5.13			5.54			5.50			4.74		
ETc (mm/day)	4.63	4.92	5.16	5.33	5.52	5.86	6.52	6.67	6.62	6.87	6.48	6.26	5.83	5.45	5.12			
Percolation (P, mm/day)	2.00			2.00			2.00			2.00			2.00					
ETc + P (mm/day)	6.63	6.92	7.16	7.33	7.52	7.86	8.52	8.67	8.62	8.87	8.48	8.26	7.83	7.45	7.12			
Initial Leaching (mm)	50																	
Land Preparation (mm)	150																	
2. EQUATION																		
Initial Leaching	2/9	2/9	2/9	2/9	1/9													
Land Preparation	2/9	2/9	2/9	2/9	1/9													
Normal Irrigation	1/9	3/9	5/9	7/9	35/36	1/1	1/1	1/1	1/1	1/1	35/36	7/9	5/9	3/9	1/9			
3. WATER REQUIREMENT																		
Initial Leaching (mm)	11.1	11.1	11.1	11.1	5.6													
Land Preparation (mm)	33.3	33.3	33.3	33.3	16.8													
Normal Irrigation (mm)	7.4	23.1	39.8	57.0	73.1	78.0	85.2	86.7	86.2	88.7	82.4	64.2	43.5	24.8	7.9			
Requirement (mm/month)	203.5			275.5			258.1			235.3			76.2					

Figure D-1-5 Field Water Requirement (Dry Season Paddy:Transplanted)

Month	December			January			February			March			April			May		
10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3
Cropping Pattern																		
1. ELEMENT I of Growing Season	10	19	29	38	48	57	67	76	86	95	100							
Crop Coefficient (Kc)	0.96	1.07 0.96	1.17 1.07 0.96	1.25 1.17 0.96	1.29 1.25 1.07	1.32 1.29 1.17	1.33 1.32 1.25	1.32 1.33 1.29	1.20 1.32 1.25	1.05 1.20 1.32	0.93 1.05 1.32			0.93				
Kc Average	0.96	1.02	1.07	1.11	1.15	1.22	1.27	1.30	1.29	1.24	1.17	1.13	1.06	0.99	0.93			
ETD by Penman (mm/day)	4.82			4.80			5.13			5.54			5.50			4.74		
ETc (mm/day)	4.63	4.92	5.16	5.33	5.52	5.86	6.52	6.67	6.62	6.87	6.48	6.26	5.83	5.45	5.12			
Percolation (P, mm/day)	2.00			2.00			2.00			2.00			2.00					
ETc + P (mm/day)	6.63	6.92	7.16	7.33	7.52	7.86	8.52	8.67	8.62	8.87	8.48	8.26	7.83	7.45	7.12			
Initial Leaching (mm)	50																	
Land Preparation (mm)	150																	
2. EQUATION																		
Initial Leaching			2/9	2/9	2/9	2/9	1/9											
Land Preparation			1/9	2/9	2/9	2/9	7/36	1/36										
Normal Irrigation				1/9	3/9	5/9	7/9	35/36	1/1	1/1	35/36	7/9	5/9	3/9	1/9			
3. WATER REQUIREMENT																		
Initial Leaching (mm)			11.1	11.1	11.1	11.1	5.6											
Land Preparation (mm)			16.7	33.3	33.3	33.3	29.2	4.2										
Normal Irrigation (mm)				8.1	25.1	43.7	66.3	84.3	86.2	88.7	83.9	64.2	43.5	24.8	7.9			
Requirement (mm/month)	27.8			210.1			275.8			236.8			76.2					



Figure D-1-6 Field Water Requirement (Soybean: Dry Season)

Month	December			January			February			March			April			May		
10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3
Cropping Pattern																		
1. ELEMENT I of Growing Season	5 15 25 35 45 55 65 75 85 95 100																	
Crop Coefficient (Kp)	0.36	0.55	0.73	0.93	1.14	1.21	1.09	0.95	0.79	0.65	0.59		0.59	0.65	0.59			
Kp Average	0.36	0.46	0.55	0.64	0.84	1.00	1.09	1.10	1.01	0.87	0.75	0.68	0.67	0.59				
E (Pan E) (mm/day)	4.56			4.45			4.70			5.50			5.53			5.14		
Etc (mm/day)	1.64	2.10	2.45	2.85	3.74	4.70	5.12	5.17	5.56	4.79	4.13	3.76	3.71	3.26				
2. EQUATION Normal Irrigation	1/24	1/3	2/3	23/24	1/1	1/1	1/1	1/1	1/1	5/6	1/2	1/6						
3. WATER REQUIREMENT Normal Irrigation (mm)	0.7	7.0	16.3	27.3	37.4	47.0	51.2	51.7	55.6	39.9	20.7	6.3						
Requirement (mm/month)	7.7			81.0			149.9			116.2			6.3					

Figure D-1-7 Field Water Requirement (Groundnuts: Dry Season)

Month	December			January			February			March			April			May		
10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3
Cropping Pattern																		
1. ELEMENT I of Growing Season	4 13 21 29 38 46 54 63 71 79 88 96 100																	
Crop Coefficient (Kp)	0.42	0.58	0.75	0.91	1.09	1.08	1.00	0.88	0.78	0.70	0.65	0.61	0.58	0.61	0.58	0.58	0.61	0.58
Kp Average	0.42	0.50	0.58	0.67	0.83	0.96	1.02	1.01	0.94	0.84	0.75	0.69	0.64	0.61	0.60	0.60	0.58	0.58
E (Pan E) (mm/day)	4.56			4.45			4.70			5.50			5.53			5.14		
Etc (mm/day)	1.92	2.28	2.58	2.98	3.69	4.51	4.79	4.75	5.17	4.62	4.13	3.82	3.54	3.37	3.08	2.98		
2. EQUATION Normal Irrigation	1/24	1/3	2/3	23/24	1/1	1/1	1/1	1/1	1/1	1/1	1/1	1/1	5/6	1/2	1/6			
3. WATER REQUIREMENT Normal Irrigation (mm)	0.8	7.6	17.2	28.6	36.9	45.1	47.9	47.5	51.7	46.2	41.3	30.8	17.7	5.6				
Requirement (mm/month)	8.4			82.7			140.5			139.2			54.1					

Figure D-1-8 Field Water Requirement (Mungbean: Dry Season)

Month	March			April			May			June			July			August		
10-day	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3	1	2	3
Cropping Pattern																		
1. ELEMENT I of Growing Season	14 29 43 57 71 86 100																	
Crop Coefficient (Kp)				0.48	0.73	1.00	1.14	0.80	0.42	0.15	0.15	0.15						
Kp Average				0.48	0.61	0.74	0.96	0.98	0.79	0.46	0.29	0.15						
E (Pan E) (mm/day)	5.50			5.53			5.14			4.50			4.81			4.30		
Etc (mm/day)				2.65	3.37	4.09	4.93	5.04	4.06	2.07	1.31	0.68						
2. EQUATION Normal Irrigation				1/6	1/2	5/6	1/1	1/1	1/1	5/6	1/2	1/6						
3. WATER REQUIREMENT Normal Irrigation (mm)				4.4	16.9	34.1	49.3	50.4	40.6	17.3	6.6	1.1						
Requirement (mm/month)				55.4			140.3			25.0								

Figure D-1-8 Rainfall - Effective Rainfall Relationship

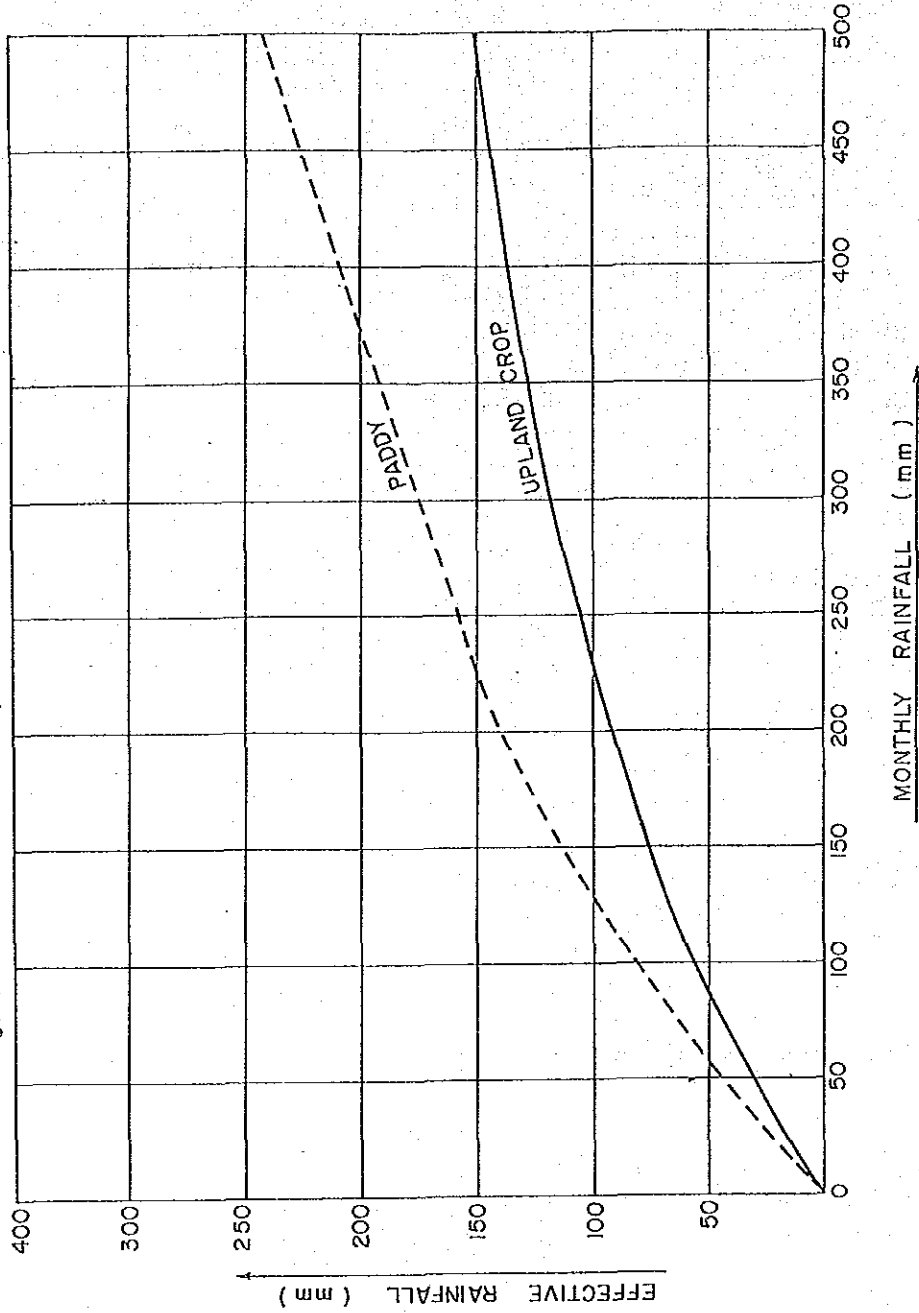

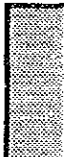


TABLE D-I-10 PROPOSED CROPPING CALENDAR

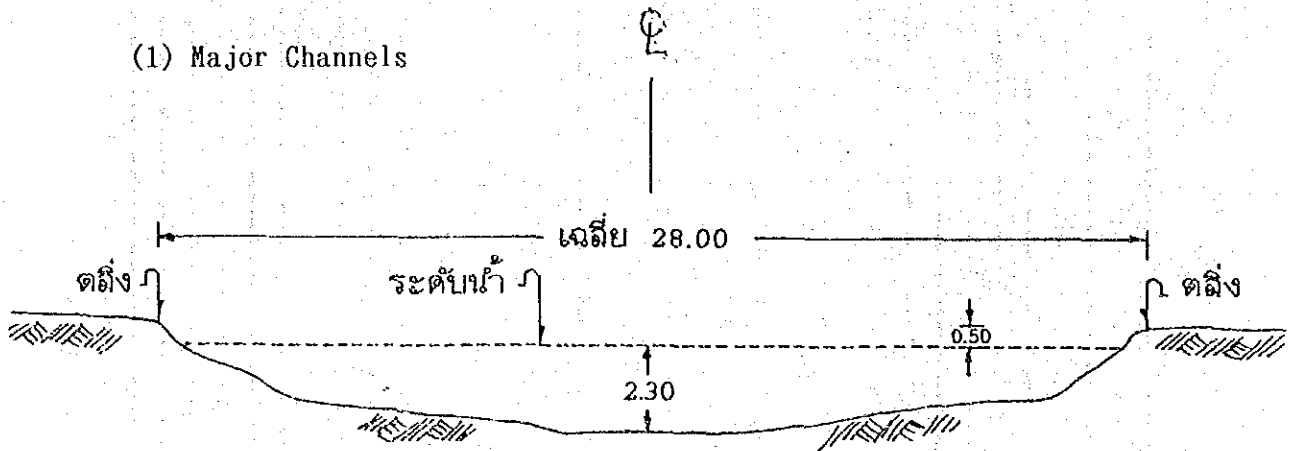
Crops	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.
Wet Season Paddy -Transplanted												
-Broadcasting												
Dry Season Paddy -Transplanted												
-Broadcasting												
Soybean												
Groundnuts												
Mungbean												
Maize												
Orchard (Mango)												
Vegetable												

Remark: for Fruit trees  irrigated,  not irrigated.

D.2 CHANNEL STORAGE VOLUME IN WATER CONSERVATION AREA  
(EXPERIENCE IN PRA-ONG CHAIYANUCHIT PROJECT AREA)

(PRA-ONG CHAIYANUCHIT AREA)

(1) Major Channels

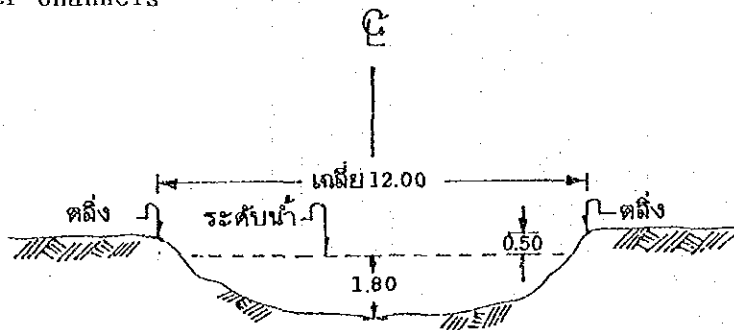


รูปตัดตามขวางคลองชลประทาน

มาตราส่วน 1:200

$$46.5 \text{ sq.m} \times 452.2 \text{ km} = 21.027 \text{ MCM}$$

(2) Other Channels



รูปตามขวางคลองธรรมชาติ

มาตราส่วน 1:200

$$14.8 \text{ sq.m} \times 470.75 \text{ km} = 6.967 \text{ MCM}$$

(3) Unit Area Storage Volume

$$(21.027 + 6.967) \text{ MCM} / 81,600 \text{ ha} = 3.43 \times 10^{-4} \text{ MCM/ha}$$

### D.3 WATER BALANCE SIMULATION

The following tables present the basic data and procedures employed in the water balance simulation study:

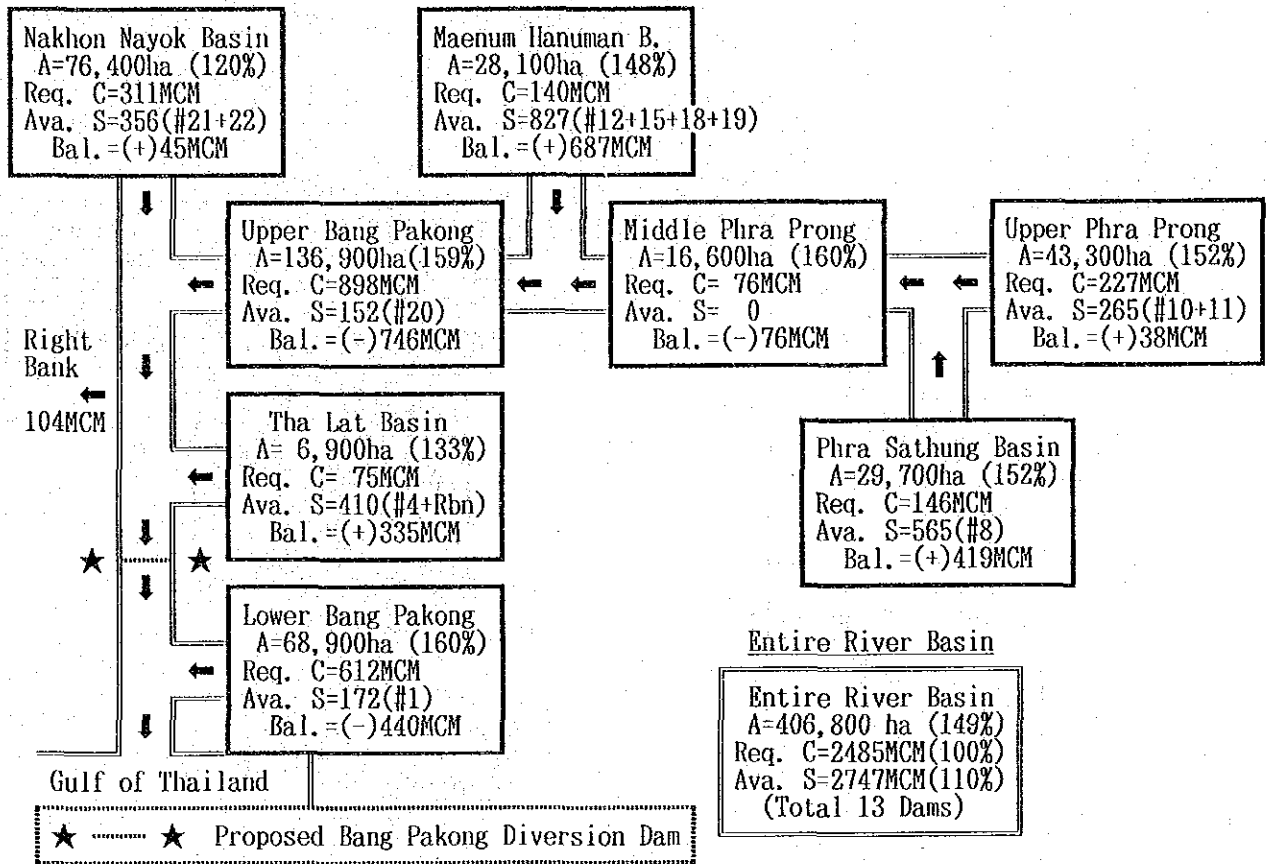
- Table D-3-1 List of Data Given in Water Balance Computation
- Figure D-3-1 Overall Basin Development Plan (Alternative Plan-1)
- Figure D-3-2 Overall Basin Development Plan (Alternative Plan-2)
- Figure D-3-3 Overall Basin Development Plan (Alternative Plan-3)
- Figure D-3-4 Overall Basin Development Plan (Alternative Plan-4)
- Table D-3-2 Simulated Results by Sub-Basin (1)
- Table D-3-3 Simulated Results by Sub-Basin (2)
- Table D-3-4 Simulated Results by Sub-Basin (3)
- Table D-3-5 Summary of Water Demand Computation  
(average in 20 years 1968 - 1987)
- Table D-3-6 Summary of Water Balance Computation  
(average in 20 years 1968 - 1987)
- Table D-3-7 Summary of Water Balance Computation  
(specific year of 1979)
- Table D-3-7 Summary of Water Balance Computation  
(specific year of 1983)

Table D-3-1 List of Data Given in Water Balance Computation

\* LIST OF INPUT DATA \*

BLOCK NO.	CATCHMENT	FARM LAND	OTHER LAND	R.O. COEF	CHANNEL STOTAGE	WATER SUPPLY	INDUS-TRY	FISH-ERY	<WET PADDY> T.P.	<DRY PADDY> B.C.	<SOYBERN> DRY WET	<GRNDNUT> DRY WET	<MNGBEAN> DRY WET	FRUIT CROP	VEGE-TABLE
UPP-3	26600	0	26600	0.24	0.241	1.249	0.250	0	0	0	0	0	0	0	0
UPP-2	77400	31840	45560	0.24	0.741	5.795	1.160	0	7670	230	0	3200	0	1000	0
UPP-1	58800	52250	6550	0.24	0.915	3.200	0.640	0	18740	5960	0	9500	0	2900	0
SUB-T	162800	84090	78710		1.897	10.244	2.050	0	26410	6190	0	12700	0	3900	0
KPS-4	61400	25090	36310	0.21	0.855	0.859	0.252	0	0	0	0	0	0	0	0
KPS-3	83800	33040	50760	0.21	0.995	1.718	0.504	0	0	0	0	0	0	0	0
KPS-2	80100	19570	60530	0.21	1.311	1.718	0.504	0	360	1440	0	1000	0	600	0
KPS-1	39000	32450	6550	0.21	0.555	2.400	0.480	0	15070	4830	0	7000	0	2600	0
SUB-T	264300	110150	154150		3.716	6.695	1.740	0	15430	6270	0	8000	0	3200	0
MPP-1	97000	41340	55660	0.30	1.071	2.436	2.302	0	10320	5180	0	5500	0	1500	0
SUB-T	97000	41340	55660		1.071	2.436	2.302	0	10320	5180	0	5500	0	1500	0
MHM-5	6800	0	6800	0.27	0.036	0.000	0.000	0	0	0	0	0	0	0	0
MHM-6	9600	5650	3950	0.27	0.036	0.000	0.000	0	0	0	0	0	0	0	0
MHM-4	17400	4270	13130	0.27	0.126	0.000	0.000	0	3400	0	0	1100	0	300	0
MHM-8	6400	0	6400	0.28	0.048	0.000	0.000	0	0	0	0	0	0	0	0
MHM-9	14700	0	14700	0.30	0.105	0.800	0.160	0	0	0	0	0	0	0	0
MHM-7	23200	0	23200	0.35	0.288	0.800	0.160	0	0	0	0	0	0	0	0
MHM-3	27300	0	27300	0.40	0.198	0.000	0.000	0	0	0	0	0	0	0	0
MHM-2	15900	0	15900	0.45	0.165	0.000	0.000	0	0	0	0	0	0	0	0
MHM-1	91700	54700	37000	0.36	1.137	3.495	6.101	0	18200	0	0	9600	0	1400	0
SUB-T	213000	64620	148380		2.139	5.095	6.421	0	21600	0	0	10700	0	1700	0
URP-3	75000	72140	2860	0.28	1.646	4.420	6.746	0	11860	11840	0	3300	0	4800	0
URP-5	10700	0	10700	0.70	0.096	0.207	0.491	0	0	0	0	0	0	0	0
URP-4	39400	33390	6010	0.50	0.720	3.606	6.518	0	3400	5400	0	1700	0	2100	0
URP-2	44600	44000	600	0.40	5.879	5.245	8.147	0	1400	5700	0	1100	0	1400	0
URP-1	106000	105420	580	0.22	17.890	4.443	6.244	0	36730	36770	0	10300	0	15200	0
SUB-T	275700	254950	20750		26.231	17.921	28.146	0	55390	59710	0	16400	0	23500	0
MNN-6	15100	0	15100	0.70	0.087	0.000	0.000	0	0	0	0	0	0	0	0
MNN-5	45600	11640	33960	0.65	0.630	1.566	2.505	0	6030	2470	0	1300	0	0	0
MNN-4	11400	0	11400	0.60	0.126	0.000	0.000	0	0	0	0	0	0	0	0
MNN-3	34500	22350	12150	0.55	0.381	1.958	3.480	0	7310	2990	0	1600	0	0	0
MNN-2	36900	30650	6250	0.30	5.247	1.864	3.970	0	0	27300	0	4100	0	0	0
MNN-1	49800	35080	14720	0.28	20.240	7.266	12.522	0	21510	8790	0	4500	0	0	0
SUB-T	193300	99720	93580		26.711	12.654	22.477	0	36850	41550	0	11500	0	0	0
KTL-9	58500	2580	55920	0.22	0.975	0.000	0.000	0	0	0	0	0	0	0	0
KTL-8	39100	4020	35080	0.22	0.543	0.000	0.000	0	0	0	0	0	0	0	0
KTL-7	39500	10120	29380	0.21	0.696	1.483	0.296	0	1100	600	0	300	0	1100	0
KTL-4	13700	10870	2830	0.16	0.240	2.966	0.592	0	710	390	0	300	0	400	0
KTL-6	18100	8410	9690	0.22	0.273	0.742	0.148	0	0	0	0	0	0	0	0
KTL-5	61700	12880	48820	0.21	1.134	1.483	0.296	0	0	0	0	0	0	0	0
KTL-3	6900	6270	630	0.17	0.102	0.381	0.530	0	230	270	0	100	0	100	0
KTL-2	8600	7860	740	0.18	0.180	0.762	1.060	0	0	0	0	0	0	0	0
KTL-1	3200	2890	310	0.16	0.234	0.285	1.022	0	0	0	0	0	0	0	0
SUB-T	249300	65900	183400		4.377	8.102	3.944	0	2040	1260	0	700	0	1600	0
LBP-17	2000	1550	450	0.15	0.042	0.762	1.060	0	410	490	0	100	0	200	0
LBP-16	19900	18280	1620	0.15	0.345	1.504	1.208	0	710	590	0	100	0	400	0
LBP-15	6500	6030	470	0.15	0.078	1.380	5.676	0	0	0	0	0	0	0	0
LBP-14	20100	18380	1720	0.15	0.315	0.690	2.837	0	0	0	0	0	0	0	0
LBP-13	5600	4050	1550	0.15	0.030	1.963	6.704	0	0	0	0	0	0	0	0
LBP-12	34400	25140	9260	0.22	0.346	1.035	0.200	0	0	0	0	0	0	0	0
LBP-11	18400	14430	3970	0.20	0.150	0.621	0.120	0	0	0	0	0	0	0	0
LBP-10	8500	7920	580	0.16	0.108	0.000	0.000	0	1700	0	0	0	0	0	0
LBP-9	8800	8080	720	0.18	0.084	0.414	0.080	0	500	0	0	300	0	0	0
LBP-8	18200	17730	470	0.15	0.318	1.166	2.056	0	5600	0	0	700	0	300	0
LBP-7	50100	45520	4580	0.20	0.450	1.615	3.005	0	300	0	0	100	0	400	0
LBP-6	8000	7260	740	0.15	0.129	2.915	5.140	0	1800	0	0	100	0	400	0
LBP-3	52600	52110	490	0.15	33.567	14.967	54.958	981	6460	25840	0	4800	0	6400	0
LBP-2	11900	6100	5800	0.15	1.577	5.958	24.168	0	0	0	0	0	0	0	0
LBP-5	27900	27700	200	0.15	0.099	7.319	11.147	0	0	0	0	0	0	0	0
LBP-4	14200	12550	1650	0.15	2.223	4.078	9.224	350	9000	0	0	3000	0	700	0
LBP-1	3500	160	3340	0.15	0.073	6.460	20.495	0	0	0	0	0	0	0	0
SUB-T	310600	272990	37610		39.934	52.847	148.078	1331	26480	26920	0	9600	0	8000	0
TOTAL	1766000	993760	772240		106.076	115.994	215.158	1331	192520	147080	0	38200	0	70000	0

FIGURE D-3-1 OVERALL BASIN DEVELOPMENT PLAN (ALTERNATIVE PLAN-1)

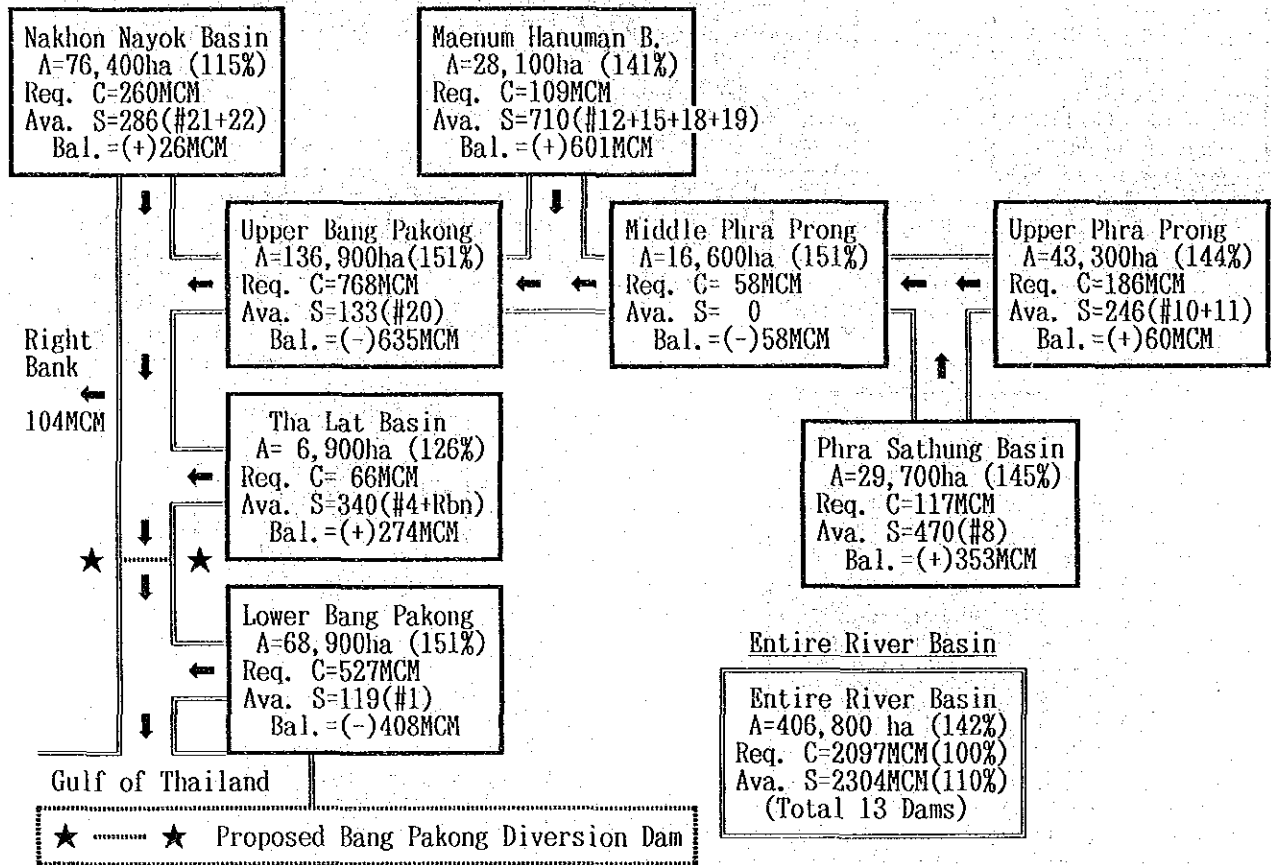


CROPPING AND IRRIGATION PLAN (ALTERNATIVE PLAN-1)

Wet Season	Dry Season	Net Irrigable Area = 406,800ha
Paddy (339,600ha)	Paddy (67,200ha)	
	Upland Crops (105,000ha)	
Maize (15,000ha)	Upland Crops (15,000ha)	
Vegetable (28,000ha)	Vegetable (28,000ha)	
Orchard (24,200ha)	Orchard (24,200ha)	

- Notes:
- 1) Req. C stands for water shortage analyzed in terms of the required live storage of reservoirs.
  - 2) Ava. S stands for available storage capacity at the proposed damsites.
  - 3) Effective live storage of freshwater at the proposed Bang Pakong diversion dam is estimated at 30 MCM and is treated as the available channel storage in the water balance simulation study.

FIGURE D-3-2 OVERALL BASIN DEVELOPMENT PLAN (ALTERNATIVE PLAN-2)



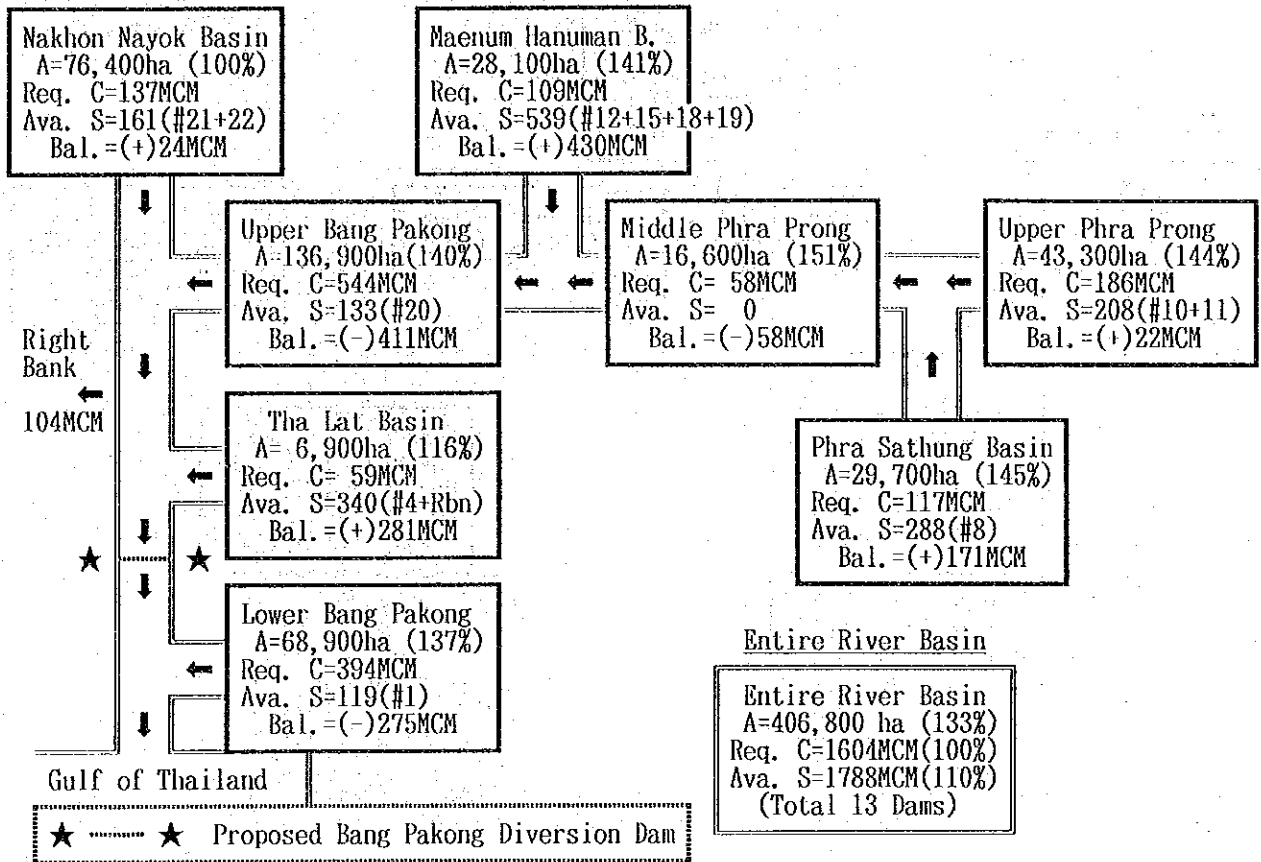
CROPPING AND IRRIGATION PLAN (ALTERNATIVE PLAN-2)

Wet Season	Dry Season	Net Irrigable Area = 406,800ha
Paddy (339,600ha)	Paddy (38,200ha)	
	Upland Crops (105,000ha)	
Maize (15,000ha)	Upland Crops (15,000ha)	
Vegetable (28,000ha)	Vegetable (28,000ha)	
Orchard (24,200ha)	Orchard (24,200ha)	

- Notes : 1) Req. C stands for water shortage analyzed in terms of the required live storage of reservoirs.  
 2) Ava. S stands for available storage capacity at the proposed damsites.  
 3) Effective live storage of freshwater at the proposed Bang Pakong diversion dam is estimated at 30 MCM and is treated as the available channel storage in the water balance simulation study.



FIGURE D-3-3 OVERALL BASIN DEVELOPMENT PLAN (ALTERNATIVE PLAN-3)

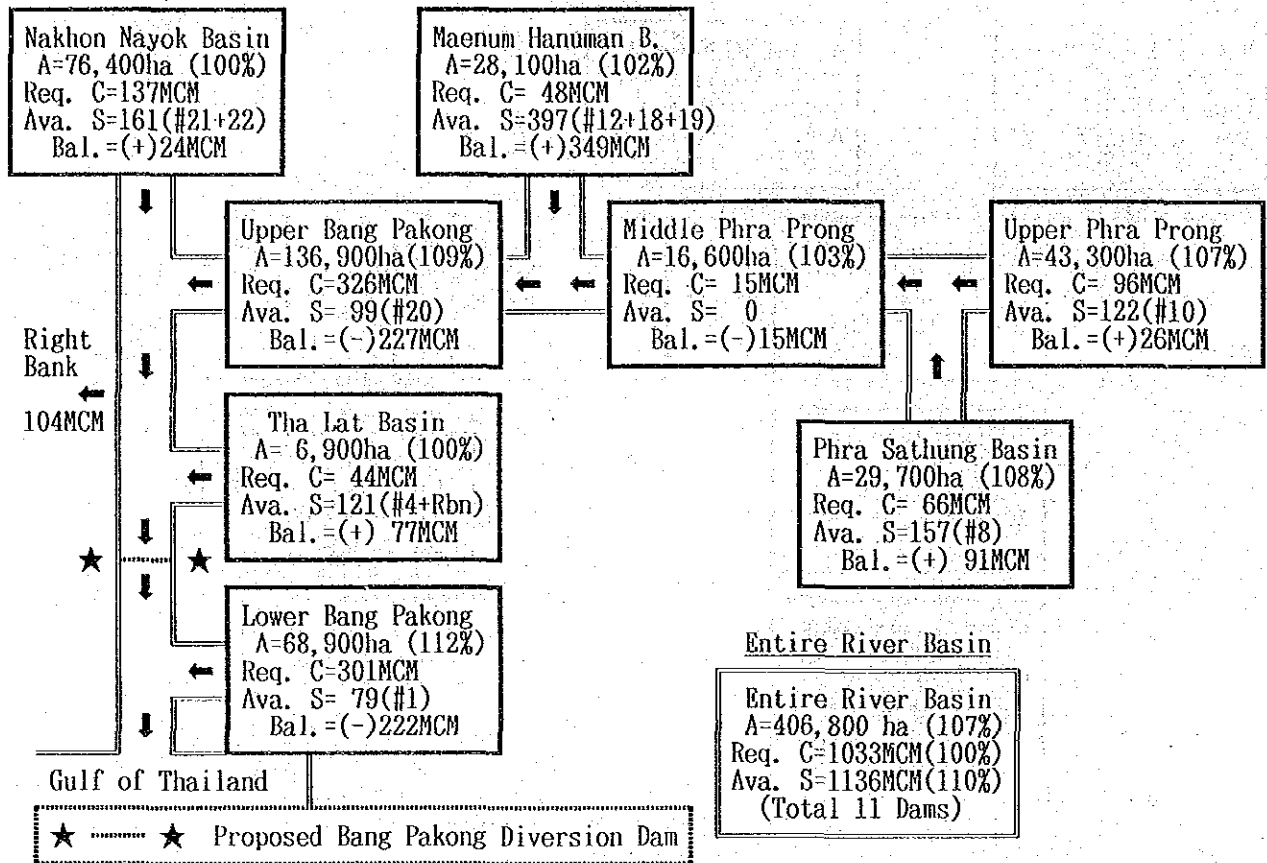


CROPPING AND IRRIGATION PLAN (ALTERNATIVE PLAN-3)

Wet Season	Dry Season	Net Irrigable Area =406,800ha
Paddy(339,600ha)	Upland Crops(105,000ha)	
Maize (15,000ha)	Upland Crops(15,000ha)	
Vegetable(28,000ha)	Vegetable(28,000ha)	
Orchard (24,200ha)	Orchard (24,200ha)	

- Notes : 1) Req.C stands for water shortage analyzed in terms of the required live storage of reservoirs.  
 2) Ava.S stands for available storage capacity at the proposed damsites.  
 3) Effective live storage of freshwater at the proposed Bang Pakong diversion dam is estimated at 30 MCM and is treated as the available channel storage in the water balance simulation study.

FIGURE D-3-4 OVERALL BASIN DEVELOPMENT PLAN (ALTERNATIVE PLAN-4)



CROPPING AND IRRIGATION PLAN (ALTERNATIVE PLAN-4)

Wet Season		Dry Season		Net Irrigable Area =406,800ha
Paddy(339,600ha)				
Maize (15,000ha)		Upland Crops(15,000ha)		
Vegetable(28,000ha)		Vegetable(28,000ha)		
Orchard (24,200ha)		Orchard (24,200ha)		

- Notes :
- 1) Req. C stands for water shortage analyzed in terms of the required live storage of reservoirs.
  - 2) Ava. S stands for available storage capacity at the proposed damsites.
  - 3) Effective live storage of freshwater at the proposed Bang Pakong diversion dam is estimated at 30 MCM and is treated as the available channel storage in the water balance simulation study.

Table D-3-2 Simulated Results by Sub-Basin (1)

UPPER PHRA PRONG

BLOCK NO. = 1 + 2 + 3 +  
 TCA = 162800. (HA) QMA = 1.628 (CUM) TCV = 1.897 (MCM)

YEAR	WATER DEMAND (MCM)	BASIN RUNOFF (MCM)	E. RAIN RETURN (MCM)	TAIL-END<-- OUTFLOW (MCM)	WATER RIVER (MCM)	SUPPLY FROM<--> CHANNEL (MCM)	DAM (MCM)	REQUIRED CAPACITY (MCM)
1968	361.774	429.070	114.365	406.541	140.877	4.693	225.123	190.606
1969	334.579	530.624	147.555	535.810	173.217	2.869	192.211	182.628
1970	325.667	540.371	126.105	500.384	185.139	5.196	159.577	145.693
1971	335.813	449.605	116.293	398.589	189.814	4.709	169.246	157.082
1972	295.467	524.215	147.591	532.849	158.242	4.768	157.425	144.572
1973	336.728	471.771	132.832	430.285	195.107	5.383	162.410	143.273
1974	304.014	475.689	118.584	421.552	193.284	4.908	130.157	113.537
1975	327.570	448.878	120.017	399.491	196.415	5.161	158.389	136.894
1976	319.199	601.733	170.911	647.004	153.629	2.944	192.658	189.621
1977	302.404	483.903	151.513	494.774	165.069	3.957	162.436	153.933
1978	332.409	477.403	147.776	471.288	185.235	3.727	177.945	162.948
1979	375.294	531.631	165.111	522.165	201.102	5.589	200.782	153.108
1980	305.678	593.643	170.664	609.139	177.592	5.383	150.511	133.182
1981	273.643	543.419	148.638	558.543	154.387	5.335	141.065	111.988
1982	317.974	552.729	156.616	567.226	165.455	4.482	174.739	162.202
1983	299.968	636.844	190.905	699.298	157.419	3.176	173.377	152.452
1984	313.371	538.626	153.385	569.188	146.946	3.469	190.550	185.669
1985	292.909	567.887	149.131	564.971	173.942	4.448	140.729	129.540
1986	315.470	555.262	153.409	531.721	200.258	2.195	137.741	136.587
1987	379.729	444.165	115.846	404.791	178.496	6.324	223.770	161.100
TOTAL	6449.645	10397.453	2897.244	10265.586	3511.622	88.916	3420.817	3046.612
MEAN	322.482	519.873	144.862	513.279	175.581	4.446	171.041	152.331

PROBABILITY OF REQUIRED CAPACITY (MCM)  
 1/2 = 152.1 1/5 = 174.0 1/7 = 179.9 1/10 = 185.6

KHLONG PHRA SATHUNG

BLOCK NO. = 4 + 5 + 6 + 7 +  
 TCA = 264300. (HA) QMA = 2.643 (CUM) TCV = 3.716 (MCM)

YEAR	WATER DEMAND (MCM)	BASIN RUNOFF (MCM)	E. RAIN RETURN (MCM)	TAIL-END<-- OUTFLOW (MCM)	WATER RIVER (MCM)	SUPPLY FROM<--> CHANNEL (MCM)	DAM (MCM)	REQUIRED CAPACITY (MCM)
1968	254.917	706.989	71.961	653.384	178.942	7.215	131.204	118.709
1969	230.267	793.577	89.489	765.921	167.301	6.520	114.991	101.946
1970	221.162	836.396	80.852	801.178	152.157	5.603	103.265	94.378
1971	233.720	698.005	73.245	634.941	173.884	8.235	98.689	95.500
1972	210.280	857.720	90.045	842.248	144.876	4.298	105.320	101.379
1973	234.149	774.857	82.840	709.773	187.569	4.960	86.227	78.950
1974	216.040	769.738	73.290	696.588	181.765	4.430	68.451	67.279
1975	227.738	773.405	74.469	722.107	173.909	4.827	102.572	93.028
1976	218.379	845.741	100.987	866.826	139.073	4.168	136.717	128.999
1977	206.436	684.720	88.466	655.293	155.949	5.496	90.309	86.604
1978	235.354	605.848	86.006	587.090	169.129	4.035	128.837	116.229
1979	262.072	737.715	96.662	704.546	189.200	4.844	134.001	112.136
1980	210.727	809.387	98.506	798.927	154.890	4.835	101.201	86.963
1981	195.532	902.009	90.266	887.185	143.998	7.563	88.721	80.715
1982	222.948	771.226	92.471	749.628	158.129	6.848	108.317	97.206
1983	206.955	947.354	112.257	962.468	144.301	5.790	113.220	97.237
1984	220.586	731.521	90.042	718.463	145.756	5.703	117.423	110.490
1985	208.374	824.228	88.904	807.937	154.450	4.412	100.525	93.470
1986	219.977	846.174	91.707	804.171	171.594	5.421	88.670	83.957
1987	268.032	724.405	72.112	622.278	212.824	6.483	92.153	81.397
TOTAL	4503.641	15640.988	1744.575	14990.922	3299.694	111.684	2110.812	1926.571
MEAN	225.182	782.049	87.229	749.546	164.985	5.584	105.541	96.329

PROBABILITY OF REQUIRED CAPACITY (MCM)  
 1/2 = 95.3 1/5 = 108.9 1/7 = 112.8 1/10 = 116.6

MIDDLE PHRA PRONG

BLOCK NO. = 8 +  
 TCA = 97000. (HA) QMA = 0.970 (CUM) TCV = 1.071 (MCM)

YEAR	WATER DEMAND (MCM)	BASIN RUNOFF (MCM)	E. RAIN RETURN (MCM)	TAIL-END<-- OUTFLOW (MCM)	WATER RIVER (MCM)	SUPPLY FROM<--> CHANNEL (MCM)	DAM (MCM)	REQUIRED CAPACITY (MCM)
1968	145.905	354.283	56.424	1379.176	137.507	2.142	54.449	37.072
1969	134.818	429.241	67.306	1723.136	120.470	1.318	60.746	60.278
1970	127.947	458.783	61.625	1747.009	116.565	2.142	51.913	49.364
1971	125.083	416.768	59.214	1428.711	118.625	1.071	45.354	45.354
1972	118.136	429.801	69.146	1799.805	106.623	1.071	43.897	43.897
1973	130.998	395.176	61.894	1500.926	128.067	2.142	34.795	30.996
1974	122.203	424.830	60.956	1514.899	115.700	1.581	33.175	33.175
1975	124.593	413.648	59.656	1501.780	121.476	2.069	31.269	31.269
1976	117.259	572.639	88.231	2118.118	99.667	1.071	59.605	59.605
1977	108.690	488.483	74.393	1641.912	101.295	2.142	37.658	36.795
1978	128.329	513.346	75.093	1561.082	120.761	1.741	42.593	42.593
1979	148.895	519.910	76.148	1715.676	141.604	2.065	41.800	41.536
1980	109.610	606.151	87.532	2031.387	100.439	1.071	60.319	40.319
1981	108.068	539.508	75.421	1986.838	103.120	2.142	34.251	22.707
1982	121.449	516.830	73.803	1847.269	101.840	1.990	60.159	60.159
1983	110.089	607.864	96.915	2295.905	105.776	1.071	40.520	65.447
1984	123.068	501.276	68.980	1785.404	114.005	2.142	50.564	48.913
1985	117.930	502.113	66.727	1863.045	108.645	1.071	39.227	39.227
1986	119.070	509.273	72.681	1839.998	111.914	1.071	40.149	40.149
1987	132.082	396.129	56.118	1381.583	126.766	2.142	34.349	29.207
TOTAL	2474.021	9596.027	1408.259	34663.633	2300.862	33.236	876.791	858.061
MEAN	123.701	479.801	70.413	1733.182	115.043	1.662	43.840	42.903

PROBABILITY OF REQUIRED CAPACITY (MCM)  
 1/2 = 41.9 1/5 = 52.2 1/7 = 55.3 1/10 = 58.3

Table D-3-3 Simulated Results by Sub-Basin (2)

MAENUM NANUMAN SUB-BASIN

BLOCK NO. = 9 + 10 + 11 + 12 + 13 + 14 + 15 + 16 + 17 +  
 TCA = 213000. (HA) QMA = 2.130(CUM) TCV = 2.139(MCM)

YEAR	WATER DEMAND	BASIN RUNOFF	E. RAIN RETURN	TAIL-END<-----	WATER SUPPLY FROM----->	REQUIRED CAPACITY		
	(MCM)	(MCM)	(MCM)	OUTFLOW RIVER	CHANNEL DAM	(MCM)		
1968	220.930	993.190	91.365	933.115	169.248	3.459	71.631	46.118
1969	193.363	1359.310	127.290	1410.913	104.091	2.648	117.677	111.208
1970	203.509	1164.884	93.025	1135.753	141.755	2.342	80.198	78.067
1971	213.963	1115.632	98.325	1091.393	146.650	2.675	92.555	86.656
1972	191.970	1260.967	125.322	1303.609	104.660	2.617	109.165	101.553
1973	210.029	1081.619	117.177	1059.106	157.717	3.833	70.340	59.798
1974	204.166	1257.609	111.362	1236.313	150.533	2.818	71.636	68.606
1975	169.502	1466.837	152.495	1523.851	117.676	2.604	73.860	69.266
1976	185.185	1353.187	123.179	1390.707	112.223	2.368	98.398	92.164
1977	197.013	1172.852	112.761	1180.701	126.794	3.080	93.169	86.818
1978	204.199	1412.236	132.895	1451.991	119.716	2.690	109.651	106.178
1979	261.455	1406.468	128.194	1363.923	196.337	4.756	91.163	80.363
1980	162.286	1489.140	137.104	1549.784	100.216	2.337	86.816	80.187
1981	192.247	1460.575	121.350	1476.081	123.747	3.644	86.602	79.628
1982	200.321	1202.721	107.202	1238.013	100.497	2.649	126.273	119.922
1983	208.963	1360.917	131.666	1377.397	145.474	2.654	95.918	75.803
1984	207.466	1369.610	122.438	1398.102	121.069	2.767	113.324	106.809
1985	209.052	1245.842	100.272	1228.473	139.478	2.635	91.610	81.186
1986	221.714	1139.028	95.023	1103.780	155.186	3.689	91.318	84.779
1987	206.405	1107.474	99.363	1082.798	142.939	4.550	81.148	54.778
TOTAL	4063.737	25420.066	2327.785	25535.773	2676.002	60.817	1852.451	1669.886
MEAN	203.187	1271.003	116.389	1276.789	133.800	3.041	92.623	83.494

----- PROBABILITY OF REQUIRED CAPACITY (MCM) -----  
 1/2 = 82.9 1/5 = 99.7 1/7 = 104.3 1/10 = 108.8

UPPER BANG PAKONG SUB-BASIN

BLOCK NO. = 18 + 19 + 20 + 21 + 22 +  
 TCA = 275700. (HA) QMA = 2.757(CUM) TCV = 26.231(MCM)

YEAR	WATER DEMAND	BASIN RUNOFF	E. RAIN RETURN	TAIL-END<-----	WATER SUPPLY FROM----->	REQUIRED CAPACITY		
	(MCM)	(MCM)	(MCM)	OUTFLOW RIVER	CHANNEL DAM	(MCM)		
1968	1477.558	919.948	503.269	2991.430	911.790	57.447	733.585	665.862
1969	1323.624	1031.289	524.214	3964.426	896.594	27.029	624.446	622.543
1970	1320.959	1155.714	557.152	3867.957	892.653	51.250	567.163	509.896
1971	1363.887	996.761	495.484	3339.593	852.307	62.109	695.206	651.714
1972	1247.638	1011.558	512.230	3896.590	861.862	47.431	520.492	517.220
1973	1480.174	783.270	446.026	3020.708	925.802	57.677	725.619	629.628
1974	1369.339	920.723	512.266	3463.072	917.737	46.604	626.645	590.684
1975	1389.501	849.638	483.920	3611.993	921.949	51.618	646.263	593.558
1976	1308.806	933.217	553.740	4377.367	832.187	29.047	687.186	679.785
1977	1378.124	890.751	479.464	3381.842	972.032	48.162	567.427	545.305
1978	1502.359	900.816	486.445	3662.443	934.362	46.351	764.226	729.743
1979	1718.399	817.646	459.598	3623.588	951.815	51.945	984.435	906.709
1980	1343.315	1078.714	561.998	4461.387	935.477	42.404	590.375	588.061
1981	1409.996	1180.879	566.793	4382.648	976.698	47.207	576.194	544.469
1982	1443.929	920.152	491.350	3721.757	929.102	61.278	667.172	641.757
1983	1211.509	1130.852	631.439	4756.406	855.513	39.708	557.869	500.867
1984	1356.048	900.768	498.944	3945.563	830.564	40.096	715.663	696.317
1985	1425.993	938.101	518.521	3839.861	912.652	45.513	695.052	657.618
1986	1396.982	982.130	541.057	3863.335	830.573	26.997	793.362	788.312
1987	1454.466	881.066	471.079	3004.456	939.656	76.895	644.745	458.251
TOTAL	27922.574	19223.957	10294.969	75176.375	18081.285	956.766	13383.297	12518.258
MEAN	1396.129	961.198	514.748	3758.819	904.064	47.838	669.165	625.913

----- PROBABILITY OF REQUIRED CAPACITY (MCM) -----  
 1/2 = 612.3 1/5 = 707.7 1/7 = 737.3 1/10 = 767.2

MAENUM NAKHON HAYOK SUB-BASIN

BLOCK NO. = 23 + 24 + 25 + 26 + 27 + 28 +  
 TCA = 193300. (HA) QMA = 1.933(CUM) TCV = 26.711(MCM)

YEAR	WATER DEMAND	BASIN RUNOFF	E. RAIN RETURN	TAIL-END<-----	WATER SUPPLY FROM----->	REQUIRED CAPACITY		
	(MCM)	(MCM)	(MCM)	OUTFLOW RIVER	CHANNEL DAM	(MCM)		
1968	677.444	1011.944	313.230	864.848	390.292	58.980	243.831	223.042
1969	586.315	1318.528	379.088	1307.969	353.962	62.381	186.835	162.623
1970	540.482	1489.544	382.115	1477.600	360.372	53.969	136.916	114.544
1971	595.114	1209.438	358.766	1176.410	326.082	71.785	215.299	185.562
1972	481.163	1350.879	394.902	1378.324	357.405	32.332	105.397	99.385
1973	689.673	1017.213	294.831	895.854	380.856	52.125	273.485	250.516
1974	554.380	1119.865	329.871	993.857	435.820	42.689	91.314	84.162
1975	585.941	1323.782	381.014	1280.621	359.454	83.247	158.362	118.737
1976	526.394	1305.378	445.251	1406.110	337.650	28.595	180.283	178.545
1977	614.681	970.908	311.425	816.581	403.783	66.210	160.460	144.540
1978	679.570	1061.064	340.250	1074.433	317.556	44.068	340.405	332.585
1979	758.058	980.445	304.345	658.059	386.446	54.893	336.001	300.436
1980	539.220	1297.450	394.260	1316.116	362.696	35.291	159.712	153.199
1981	575.223	1315.833	364.795	1277.376	347.533	67.632	177.601	128.913
1982	602.049	1320.244	372.122	1291.140	369.242	55.544	194.358	118.929
1983	506.298	1540.531	485.899	1653.078	308.969	73.308	147.839	109.419
1984	615.767	1155.201	342.821	1114.289	339.898	48.919	243.850	240.858
1985	649.826	1227.306	394.834	1187.323	404.267	77.599	188.301	160.055
1986	607.808	1301.444	397.958	1302.401	361.397	46.639	217.280	213.387
1987	818.458	874.758	264.589	624.600	440.974	95.482	298.085	164.841
TOTAL	12203.836	24191.727	7252.352	23294.953	7344.641	1151.682	4055.614	3484.275
MEAN	610.192	1209.586	362.617	1164.748	367.232	57.584	202.781	174.214

----- PROBABILITY OF REQUIRED CAPACITY (MCM) -----  
 1/2 = 160.3 1/5 = 222.4 1/7 = 243.6 1/10 = 265.9

Table D-3-4 Simulated Results by Sub-Basin (3)

KHLONG THA LAT SUB-BASIN

BLOCK NO. = 29 + 30 + 31 + 32 + 33 + 34 + 35 + 36 + 37 +  
 TCA = 249300. (HA) QMA = 2.493(CUM) TCV = 4.377(MCM)

YEAR	WATER DEMAND	BASIN RUNOFF	E. RAIN RETURN	TAIL-END<-----	WATER RIVER	SUPPLY FROM----->	REQUIRED CAPACITY	
	(MCM)	(MCM)	(MCM)	OUTFLOW	(MCM)	CHANNEL	DAM	
1968	83.631	586.191	11.839	581.327	65.966	4.614	66.930	49.421
1969	69.462	638.222	11.927	618.978	57.854	5.577	42.671	51.206
1970	70.750	995.801	13.618	979.139	58.327	6.488	38.025	34.200
1971	78.432	647.510	12.329	638.879	62.230	5.509	59.685	43.844
1972	82.541	554.388	11.927	535.528	66.587	6.204	51.996	39.595
1973	79.737	524.992	10.889	503.773	66.863	6.612	47.391	36.736
1974	79.405	487.495	12.195	480.194	63.880	4.489	55.969	43.515
1975	75.394	672.195	12.622	644.315	66.013	5.442	39.077	36.617
1976	74.188	680.627	12.888	667.520	59.516	6.807	46.530	36.211
1977	71.504	719.477	12.236	675.451	65.398	6.949	16.911	23.578
1978	75.449	744.328	12.710	724.736	63.056	6.245	42.911	38.581
1979	84.514	724.634	12.634	718.799	66.789	7.122	63.775	45.434
1980	76.711	785.773	12.496	761.175	64.729	5.072	42.132	38.350
1981	76.342	1089.290	15.390	1083.560	60.252	4.405	50.927	40.956
1982	82.054	586.383	11.155	568.846	64.902	4.654	53.286	41.661
1983	70.567	877.295	14.573	856.236	60.450	7.041	39.052	27.441
1984	73.424	590.213	11.458	554.771	66.982	5.438	26.299	13.871
1985	78.820	604.058	12.070	595.543	62.100	4.776	54.456	44.599
1986	74.000	689.100	13.341	691.849	56.240	4.377	63.307	45.908
1987	75.513	559.185	11.129	519.241	67.024	6.289	27.665	5.668
TOTAL	1532.437	13757.133	249.424	13399.832	1265.155	114.113	928.993	737.389
MEAN	76.622	687.856	12.471	669.991	63.258	5.706	46.450	36.869

----- PROBABILITY OF REQUIRED CAPACITY (MCM) -----  
 1/2 = 33.8    1/5 = 52.1    1/7 = 58.5    1/10 = 65.3

LOWER BANG PAKONG SUB-BASIN

BLOCK NO. = 38 + 39 + 40 + 41 + 42 + 43 + 44 + 45 + 46 + 47 + 48 + 49 + 50 + 51 + 52 + 53 + 54 +  
 TCA = 310600. (HA) QMA = 3.106(CUM) TCV = 39.934(MCM)

YEAR	WATER DEMAND	BASIN RUNOFF	E. RAIN RETURN	TAIL-END<-----	WATER RIVER	SUPPLY FROM----->	REQUIRED CAPACITY	
	(MCM)	(MCM)	(MCM)	OUTFLOW	(MCM)	CHANNEL	DAM	
1968	1124.968	597.754	166.907	4573.270	818.693	52.137	498.042	436.371
1969	1073.045	536.618	176.116	6007.594	745.824	46.075	512.181	444.232
1970	1101.788	632.798	173.279	6483.055	813.712	67.275	418.206	371.661
1971	1064.335	607.613	187.397	5440.117	717.125	67.439	554.673	461.364
1972	1126.173	525.039	175.974	5853.113	795.050	47.801	468.292	395.333
1973	1095.015	539.252	172.184	4530.695	772.637	86.685	496.530	407.490
1974	1035.604	650.847	197.119	5211.707	752.617	50.708	457.796	397.206
1975	1109.873	533.885	168.162	5626.141	824.174	46.271	498.786	408.629
1976	1063.538	563.216	178.791	6660.801	715.715	69.600	531.723	449.260
1977	1119.612	473.669	165.548	4830.645	842.018	47.857	442.563	397.479
1978	1125.317	521.425	179.275	5634.465	752.193	50.784	592.892	512.140
1979	1229.848	383.351	158.450	5187.242	807.293	44.935	675.812	588.144
1980	1084.232	495.527	173.956	6645.410	769.590	52.399	520.145	434.932
1981	1101.396	568.823	180.833	6860.527	784.346	48.671	469.692	405.387
1982	1094.307	506.231	175.167	5679.469	739.327	73.102	509.079	450.076
1983	979.778	695.531	210.318	7650.164	704.022	49.197	479.144	338.458
1984	1152.975	469.223	167.383	5692.418	806.457	45.941	574.492	514.416
1985	1111.525	416.291	172.937	5643.148	750.895	50.648	541.825	472.735
1986	1117.381	550.172	188.391	6054.488	752.206	46.303	576.838	502.833
1987	1108.732	568.275	174.175	4161.891	856.894	75.693	379.421	282.201
TOTAL	22019.410	10835.520	3542.360	114426.062	15520.758	1119.541	10198.117	8670.328
MEAN	1100.970	541.776	177.118	5721.301	776.038	55.977	509.906	433.516

----- PROBABILITY OF REQUIRED CAPACITY (MCM) -----  
 1/2 = 428.4    1/5 = 490.5    1/7 = 508.6    1/10 = 526.4

ENTIRE BANG PAKONG RIVER BASIN

BLOCK NO. = 1 + 2 + 3 + 4 + 5 + 6 + 7 + 8 + 9 + 10 + 11 + 12 + 13 + 14 + 15 + 16 + 17 + 18 + 19 + 20 +  
 21 + 22 + 23 + 24 + 25 + 26 + 27 + 28 + 29 + 30 + 31 + 32 + 33 + 34 + 35 + 36 + 37 + 38 + 39 + 40 +  
 41 + 42 + 43 + 44 + 45 + 46 + 47 + 48 + 49 + 50 + 51 + 52 + 53 + 54 +  
 TCA = 1766000. (HA) QMA = 17.660(CUM) TCV = 106.076(MCM)

YEAR	WATER DEMAND	BASIN RUNOFF	E. RAIN RETURN	TAIL-END<-----	WATER RIVER	SUPPLY FROM----->	REQUIRED CAPACITY	
	(MCM)	(MCM)	(MCM)	OUTFLOW	(MCM)	CHANNEL	DAM	
1968	4347.113	5599.316	1329.358	4573.270	2833.312	190.687	2024.789	1767.192
1969	3945.469	6637.352	1522.983	6007.594	2619.309	154.417	1851.952	1736.662
1970	3912.260	7274.230	1487.769	6483.055	2720.677	194.264	1555.260	1397.800
1971	4010.345	6141.273	1401.050	5440.117	2586.713	223.532	1930.703	1727.075
1972	3753.365	6514.508	1527.135	5853.113	2595.300	146.523	1561.982	1442.931
1973	4256.496	5588.082	1318.671	4530.695	2814.614	219.417	1896.793	1637.385
1974	3885.148	6106.734	1415.642	5211.707	2811.333	158.226	1535.138	1398.162
1975	4009.908	6482.199	1452.352	5626.141	2781.062	201.239	1708.573	1487.996
1976	3812.944	6855.668	1673.975	6660.801	2449.655	144.598	1933.095	1814.187
1977	3998.460	5884.707	1395.802	4830.645	2832.334	183.852	1570.928	1475.049
1978	4282.977	6236.402	1460.447	5634.465	2662.003	159.641	2199.455	2040.994
1979	4838.523	6101.750	1401.140	5187.242	2940.581	176.150	2527.765	2227.863
1980	3831.776	7155.723	1636.513	6645.410	2665.620	148.792	1691.206	1555.191
1981	3932.445	7600.270	1563.484	6860.527	2694.079	186.599	1625.029	1414.762
1982	4085.028	6376.453	1479.884	5679.469	2628.490	210.547	1893.378	1691.911
1983	3594.122	7797.113	1873.969	7650.164	2481.919	181.944	1644.935	1367.121
1984	4062.702	6256.371	1455.448	5692.418	2571.675	154.476	2032.161	1917.361
1985	4094.425	6325.762	1503.398	5643.148	2706.426	191.302	1851.723	1678.427
1986	4072.399	6572.508	1553.565	6054.488	2639.366	136.891	2008.661	1895.910
1987	4443.402	5555.398	1264.388	4161.891	2965.569	273.658	1781.331	1237.421
TOTAL	81169.125	129061.562	29716.926	114426.062	53999.992	3636.753	36826.828	32911.359
MEAN	4058.456	6453.078	1485.846	5721.301	2700.000	181.838	1841.341	1645.568

----- PROBABILITY OF REQUIRED CAPACITY (MCM) -----  
 1/2 = 1613.6    1/5 = 1844.6    1/7 = 1916.2    1/10 = 1988.2

Table D-3-5 Summary of Water Demand Computation  
(average in 20 years 1968 - 1987)

\*\* SUMMARY OF WATER DEMAND COMPUTATION (AVERAGE IN 20 YEARS, 1968 - 1987) \*\*

BLOCK AND BASIN	CROP CONSUMPTION	EFFECTIVE RAINFALL	ON-FARM REQUIREMENT	DIVERSION REQUIREMENT	DOMESTIC WATER SUPPLY	OTHER WATER SUPPLY INDUSTRIAL	FRESH WATER TO FISHERY	TOTAL WATER DEMAND
(1) UPPER PHRA PRONG								
UPP-3	0.000	0.000	0.000	0.000	1.250	0.250	0.000	1.500
UPP-2	96.818	50.036	46.782	87.927	5.800	1.161	0.000	94.888
UPP-1	281.012	161.896	119.116	222.252	3.203	0.660	0.000	226.095
SUB-TOTAL	377.830	211.933	165.897	310.179	10.252	2.051	0.000	322.482
(2) KHLONG PHRA SATHUNG								
KPS-4	0.000	0.000	0.000	0.000	0.860	0.252	0.000	1.112
KPS-3	0.000	0.000	0.000	0.000	1.719	0.504	0.000	2.224
KPS-2	46.932	21.822	25.110	48.938	1.719	0.504	0.000	51.162
KPS-1	215.019	124.255	90.764	167.803	2.402	0.480	0.000	170.685
SUB-TOTAL	261.950	146.077	115.873	216.740	6.700	1.741	0.000	225.182
(3) MIDDLE PHRA PRONG								
MPP-1	159.574	94.342	65.232	118.959	2.438	2.304	0.000	123.701
SUB-TOTAL	159.574	94.342	65.232	118.959	2.438	2.304	0.000	123.701
(4) MAENUM HANUMAN SUB-BASIN								
MHM-5	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MHM-6	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MHM-4	35.695	16.914	16.781	30.721	0.000	0.000	0.000	30.721
MHM-8	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MHM-9	0.000	0.000	0.000	0.000	0.801	0.160	0.000	0.961
MHM-7	0.000	0.000	0.000	0.000	0.801	0.160	0.000	0.961
MHM-3	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MHM-2	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MHM-1	196.709	111.278	85.431	160.942	3.498	6.105	0.000	170.545
SUB-TOTAL	230.404	128.192	102.212	191.662	5.099	6.425	0.000	203.187
(5) UPPER BANG PAKONG SUB-BASIN								
UBP-3	328.448	163.317	165.131	299.979	4.424	6.751	0.000	311.153
UBP-5	0.000	0.000	0.000	0.000	0.207	0.491	0.000	0.699
UBP-4	146.289	70.605	75.684	136.548	3.609	6.522	0.000	146.679
UBP-2	109.821	53.096	56.725	104.168	5.249	8.153	0.000	117.570
UBP-1	900.838	445.845	454.995	809.334	4.447	6.248	0.000	820.029
SUB-TOTAL	1485.396	732.863	752.533	1350.028	17.936	28.165	0.000	1396.128
(6) MAENUM NAKHON NAYOK SUB-BASIN								
MNN-6	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MNN-5	81.052	46.007	35.045	58.899	1.567	2.507	0.000	62.973
MNN-4	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
MNN-3	98.471	55.305	43.166	72.548	1.960	3.482	0.000	77.990
MNN-2	299.108	161.151	137.957	251.861	1.866	3.975	0.000	257.699
MNN-1	287.478	161.500	125.978	211.729	7.272	12.531	0.000	231.531
SUB-TOTAL	766.108	423.962	342.146	575.036	12.664	22.492	0.000	610.192
(7) XHLONG THA LAT SUB-BASIN								
XTL-9	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
XTL-8	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
XTL-7	22.101	8.690	13.411	24.110	1.484	0.296	0.000	25.891
XTL-4	26.541	8.668	17.873	34.055	2.968	0.592	0.000	37.616
XTL-6	0.000	0.000	0.000	0.000	0.743	0.148	0.000	0.891
XTL-5	0.000	0.000	0.000	0.000	1.484	0.296	0.000	1.780
XTL-3	6.280	2.636	3.644	6.401	0.381	0.530	0.000	7.313
XTL-2	0.000	0.000	0.000	0.000	0.763	1.061	0.000	1.823
XTL-1	0.000	0.000	0.000	0.000	0.285	1.023	0.000	1.308
SUB-TOTAL	54.923	19.995	34.928	64.567	8.109	3.947	0.000	76.622
(8) LOWER BANG PAKONG SUB-BASIN								
LBP-17	10.019	4.517	5.502	9.637	0.763	1.061	0.000	11.460
LBP-16	13.872	6.498	7.375	12.997	1.505	1.209	0.000	15.711
LBP-15	0.000	0.000	0.000	0.000	1.381	5.680	0.000	7.061
LBP-14	0.000	0.000	0.000	0.000	0.691	2.839	0.000	3.529
LBP-13	0.000	0.000	0.000	0.000	1.965	6.709	0.000	8.673
LBP-12	0.000	0.000	0.000	0.000	1.036	0.200	0.000	1.236
LBP-11	0.000	0.000	0.000	0.000	0.622	0.120	0.000	0.742
LBP-10	25.375	10.401	14.974	27.161	0.000	0.000	0.000	27.161
LBP-9	6.833	2.402	4.431	7.447	0.414	0.080	0.000	7.942
LBP-8	73.363	32.296	41.068	74.745	1.167	2.057	0.000	77.970
LBP-7	3.254	1.378	1.877	3.154	1.616	3.007	0.000	7.777
LBP-6	16.979	7.966	9.012	15.892	2.917	5.144	0.000	23.953
LBP-3	457.040	186.602	270.439	484.594	14.979	54.995	10.682	565.251
LBP-2	0.000	0.000	0.000	0.000	5.963	24.184	0.000	30.147
LBP-5	0.000	0.000	0.000	0.000	7.325	11.155	0.000	18.480
LBP-4	135.450	54.695	80.755	144.075	4.081	9.230	3.811	161.198
LBP-1	0.000	0.000	0.000	0.000	6.465	20.509	0.000	26.974
SUB-TOTAL	742.186	306.755	435.431	779.703	52.890	148.179	14.494	995.266
(9) ENTIRE BANG PAKONG RIVER BASIN								
SUB-TOTAL	4078.369	2064.116	2014.252	3606.873	116.089	215.304	14.494	3952.757









#### D.4 OPTIMUM SCALE OF WATER RESOURCES DEVELOPMENT

The following tables summarize the procedures employed to determine the optimum scale of water resources development:

Table D-4-1 Irrigable Area by Alternative Plan

Table D-4-2 Required Dam Storage by Alternative Development Plan

Table D-4-3 Net Production Value by Alternative Development Plan

Table D-4-4 Amount of Water Resources to be Allocated to Sectors

Table D-4-5 Construction Cost (Agricultural Sector) by Alternative Plan

Table D-4-6 B/C Ratio by Alternative Development Plan

TABLE D-4-1

## IRRIGABLE AREA BY ALTERNATIVE PLAN

(Unit = ha)

	Wet Season		Dry Season				Net Irrigation Area
	Paddy	Veget.	Paddy	Upland C	Veget.	Orchrd	
Alt. Plan-1	339,600	28,000	67,200	120,000	28,000	24,200	406,800
Alt. Plan-2	339,600	28,000	38,200	120,000	28,000	24,200	406,800
Alt. Plan-3	339,600	28,000	0	120,000	28,000	24,200	406,800
Alt. Plan-4	339,600	28,000	0	15,000	28,000	24,200	406,800

Note: Net irrigation area = 339,600 + 28,000 + 15,000 + 24,200 = 406,800 ha

TABLE D-4-3

## NET PRODUCTION VALUE BY ALTERNATIVE DEVELOPMENT PLAN

(Unit = million baht)

Sub-Basin	Alt. Plan-1	Alt. Plan-2	Alt. Plan-3	Alt. Plan-4
Lower Bang Pakong	648	624	547	501
Tha Lat	103	100	95	92
Upper Bang Pakong	1,365	1,300	1,194	382
Nakhon Nayok	160	137	62	62
Middle Phra Prong	96	85	85	59
Maenum Hanuman	254	240	240	198
Phra Sathung	254	239	239	205
Upper Phra Prong	412	390	390	338
Total	3,292	3,115	2,852	1,837

TABLE D-4-4

## AMOUNT OF WATER RESOURCES TO BE ALLOCATED TO SECTORS

(Unit = MCM &amp; %)

Sector	Alt. Plan-1	Alt. Plan-2	Alt. Plan-3	Alt. Plan-4
Irrigation	4,066(92%)	3,607(92%)	2,997(91%)	2,412(87%)
Industrial Suupply	215( 5%)	215( 5%)	215( 6%)	215( 8%)
Water Supply	116( 3%)	116( 3%)	116( 3%)	116( 4%)
Fishery	14( 0%)	14( 0%)	14( 0%)	14( 1%)
Total	4,412	3,953	3,343	2,758

Note: Irrigation and fishery are counted as agricultural sector.

TABLE D-4-5 CONSTRUCTION COST (AGRICULTURAL SECTOR) BY ALTERNATIVE PLAN  
(Unit = million baht)

Item	Dam No.	Alt. Plan-1	Alt. Plan-2	Alt. Plan-3	Alt. Plan-4
Dam	1	814	685	685	615
	4	961	820	820	-
	5	-	-	-	170
	Rabom	97	97	97	97
	8	1,185	1,056	896	786
	10	738	738	696	696
	11	247	223	223	-
	12	885	754	754	577
	15	174	150	139	-
	18+19	1,943	1,939	1,951	1,419
	20	1,820	1,548	1,548	1,222
	21	169	152	111	111
	22	1,048	837	616	616
	Total	9,957	8,902	7,839	6,212
Agri.	92%= 9,160	92%= 8,190	91%= 7,133	88%= 5,466	
Diversion Dam		595	595	595	595
Main Facilities		12,302	12,302	12,302	12,302
On-farm Facilities		4,037	4,037	4,037	4,037
Total		26,094	25,124	24,067	22,400

Note: (1) The Rabom dam is at present under construction.  
(2) The proposed Khlong Luang dam is assigned as the No.1 dam.  
(3) About 1,500 million bahts of construction cost of the Bang Pakong diversion dam is allocated fiftyfifty between agriculture and other sectors. The cost for agricultural sector is then allocated between both banks of the Bang Pakong river, as;  
 $1,500 \times 50\% \times 384.6\text{MCM}/484.6\text{MCM} = 595$  million bahts (Left bank)  
and  $1,500 \times 50\% - 595 = 155$  (Right bank).

TABLE D-4-6 B/C RATIO BY ALTERNATIVE DEVELOPMENT PLAN

Item	Alt. Plan-1	Alt. Plan-2	Alt. Plan-3	Alt. Plan-4
Benefit				
- Benefit (1)	3,292	3,115	2,852	1,837
- O/M Cost (2)	302	301	300	299
- ((1)-(2))/0.12	24,917	23,450	21,267	12,817
Cost				
- Financial Cost	26,094	25,124	24,067	22,400
- Economic(0.9)	23,484	22,611	21,660	20,160
B/C Ratio	1.06	1.04	0.98	0.64

Note: Discount rate was taken at 12% and project life was considered to be 60 years as an average.

TABLE D-4-2 REQUIRED DAM STORAGE BY ALTERNATIVE DEVELOPMENT PLAN (Unit = MCM)

Dam No.	Alt. Plan-1	Alt. Plan-2	Alt. Plan-3	Alt. Plan-4
1	172	119	119	79
4	370	300	300	-
5	-	-	-	81
Rabom	(40)	(40)	(40)	(40)
8	565	470	288	157
10	160	160	122	122
11	105	86	86	-
12	350	290	290	193
15	150	98	45	-
18 + 19	327	322	204	204
20	152	133	133	99
21	230	188	90	90
22	126	98	71	71
Total	2,747	2,304*	1,788	1,136

Note: (1) Rabom dam is under construction.  
 (2) 2,304 MCM(\*) includes losses due to evaporation and seepage, that correspond to 10% of the required capacity.

## D.5 SELECTION OF PRIORITY PROJECT

The following tables summarized the procedures employed to select the priority sub-project:

Table D-5-1 Evaluation from National Economic Point of View

Table D-5-2 Evaluation from Technical Point of View

Table D-5-3 Evaluation from Social Point of View

Table D-5-4 Evaluation from Farm Economic Point of View

TABLE D-5-1 EVALUATION FROM NATIONAL ECONOMIC POINT OF VIEW (B/C RATIO)

Sub-Basin	Incremental Benefit			Feasible Investment		Cost		B/C Ratio	
	Incre. Benefit	O/M Cost	Benefit	Discount Rate		Financial	Economic	Discount Rate	
				10 %	12 %			10 %	12 %
				Lower Bang Pakong	624			52.7	571.3
Tha Lat	100	6.9	93.1	931	776	707	636	1.46	1.22
Upper Bang Pakong	1,300	97.7	1202.3	12,023	10,019	8,118	7,306	1.65	1.37
Nakhon Nayok	137	54.4	82.6	826	688	3,379	3,041	0.27	0.23
Middle Phra Prong	85	13.5	71.5	715	596	822	740	0.97	0.80
Maenum Hanuman	240	21.4	218.6	2,186	1,822	3,060	2,754	0.79	0.66
Phra Sathung	239	22.4	216.6	2,166	1,805	1,957	1,761	1.23	1.02
Upper Phra Prong	390	32.0	358.0	3,580	2,983	1,808	1,627	2.20	1.83
Total	3,115	301	2,814	28,140	23,450	25,019	22,516	1.25	1.04

Note: Benefit, feasible investment and cost are given in million bahts.

TABLE D-5-2 EVALUATION FROM TECHNICAL POINT OF VIEW (DAM CONSTRUCTION AND COMPENSATION)

Sub-Basin	Dam No.	Height (m)	Dam Volume 1000m <sup>3</sup>	Compensation (ha)	Reliability of Dam Foundation (Max=5) Quality of Embankment Material (3) Workability (Max=2) Total EVALUATION Compensation, N/P Houses, G/L EVALUATION							
					3	3	2	8	B	P	G	C
Lower Bang Pakong	1	23.5	3,340	5,500	3	3	2	8	B	P	G	C
Tha Lat	4	33.5	3,740	7,370	3	3	2	8	C	P	G	C
Phra Sathung	8	30.3	4,270	9,710	3	2	2	7	C	P	G	C
Upper Phra prong	10	23.5	2,750	8,580	1	3	2	6	E	P	G	C
	11	28.3	1,060	1,720	4	3	2	9	A	N	0	A
Maenum Hanuman	12	35.2	3,450	2,600	3	2	1	6	D	P	L	B
	15	24.2	260	3,010	5	1	2	8	B	P	G	C
	18	85.6	10,800	1,130	4	1	1	6	E	N	0	A
	19	23.0	480	430	4	2	1	7	C	N	0	A
Upper Bang Pakong	20	86.6	11,150	450	5	2	1	8	B	N	0	B
Nakhon Nayok	21	47.7	920	2,480	1	3	1	5	E	N	0	A
	22	43.2	6,600	700	4	2	1	7	C	P	L	B

- Note: (1) No proposed dams site in the Middle Phra prong sub-basin.  
 (2) Decimal point denotes 0.5 point (ex. 1.=1.5). Three(3) point is given for the grade "A" (excellent or no problem), 2 point is given for "B", while 1 point for "C" and "D", and 0 point for "E" (difficult).  
 (3) For compensation of the reservoir area, 3 point is given for "A" where no or less impact is evaluated, 2 point for "B" (moderate) and 1 point for "C" where a considerable impact is presumed.



TABLE D-5-3 EVALUATION FROM SOCIAL POINT OF VIEW (URGENCY AND INHABITANT'S NEEDS)

Sub-Basin	Inhabitant's Needs					Supply Urgency	
	Irrigat.	Domestic	Road	Elec	Evaluat.	Industry	Domes.
Lower Bang Pakong	2	1	(3)	(1)	3	3	3
Tha Lat	3	1	(2)	(1)	4	3	3
Upper Bang Pakong	3	1	(2)	(1)	4	2	2
Nakhon Nayok	2	1	(3)	(1)	3	2	2
Middle Phra Prong	2	1	(3)	(1)	3	1	1
Maenum Hanuman	2	1	(3)	(1)	3	1	1
Phra Sathung	1	1	(3)	(2)	2	1	1
Upper Phra prong	1	1	(3)	(1)	2	1	1

Note: Needs for road and electricity were excluded from overall evaluation.  
 Inhabitants' s needs: 3=eager, 2=moderate and 3=modest.  
 Urgency: 3=very urgent, 2=moderate and 1=not urgent.

TABLE D-5-4 EVALUATION FROM FARM ECONOMIC POINT OF VIEW (PER HA BENEFIT)

Sub-Basin	Annual Benefit (Million B)	Beneficial Area (ha)	Unit Area Benefit (1000baht/ha)	Evaluation
Lower Bang Pakong	624	68,900	9.1	2
Tha Lat	100	6,900	11.8	3
Upper Bang Pakong	1,300	136,900	9.4	2
Nakhon Nayok	137	76,400	1.8	1
Middle Phra Prong	85	16,600	5.1	1
Maenum Hanuman	240	28,100	8.2	2
Phra Sathung	239	29,700	8.0	2
Upper Phra Prong	390	43,300	9.2	2
Total	3,115	406,800	7.7	

Notes: Evaluation; 3 points for benefit >10.0, 2 points for  $10.0 \leq \text{benefit} \leq 7.5$  and 1 point for benefit <7.5.

## D.6 FLOOD RUNOFF ANALYSIS

### D.6.1 Flood Rainfall

#### Probable 3 Day Consecutive Rainfall

As is seen in Figure D-6-1, probable 3 day consecutive rainfall has a close relation with the average annual rainfall at selected rain stations, where storm rainfall data are available. The following equation was developed to estimate probable consecutive rainfall in each irrigation block (see Tables D-6-1 and D-6-2).

$$X = 0.0865 \times Y + 65.1$$

where, X : 3 day consecutive rainfall which would occur once in 10 years, in mm

Y : Average annual rainfall (mm)

#### Hourly Distribution of Storm Rainfall

Hourly distribution of storm rainfall was determined in proportion to the actual pattern of the storm rainfall measured at stations in September 1972 (cf. Appendix A-2-10).

#### Runoff Coefficient during Flood

Amount of flood runoff was plotted against the amount of flood rainfall at stations. An envelope curve was drawn mainly from the data obtained at the station kgt.12, as shown in Figure D-6-1. This envelope curve was then converted into an equation showing relation between the accumulated storm rainfall and rainfall loss as under:

$$SL = 0.5 \times SR, \text{ when } SR < 100 \text{ mm}$$

$$SL = 0.375 \times SR + 12.5, \text{ when } SR > 100 \text{ mm}$$

where, SL : Cumulative loss of rainfall (mm)

SR : Cumulative rainfall (mm)

### Effective Rainfall

Effective rainfall is, then, calculated as follows:

$$SRE_t = SR_t - SL_t$$

$$RE_t = SRE_t - SRE_{t-1}$$

where, SRE : Cumulative effective rainfall (mm)

t, t-1: time

### D.6.2 Flood Runoff Model

The mechanism of surface runoff may fall generally into two parts; namely (1) the behaviour of rain water which flows down a sloping surface and pours directly into river channel and (2) the behaviour of lateral inflow which pours into such a stream. As a simplified stream condition, the behaviour of unsteady flow in an open channel with distributed lateral inflow along a channel is studied hydraulically to establish the basic relationship between the rate of inflow and runoff in a stream or on a sloping surface. Hydrographs under this simplified condition are easily computed for both laminar and turbulent flows, and the hydraulic character of hydrographs resulting from simulated inflow at a given rate are investigated. The method of characteristics was employed to express this phenomenon. Brief explanation is as below:

If the law of resistance of Manning's type is used, unsteady flow in an open channel with a given rate of lateral inflow would be expressed for the practical purposes by the equations;

$$A = n \times I^{-1/2} \times R^{2/3} \times Q = kQ^n \quad \text{and}$$

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial X} = q$$

The method of characteristics is applied in order to solve the above equations and the characteristic curves are given as follows:

$$\frac{dX}{I} = \frac{dt}{dA/dQ} = \frac{dt}{pkQ^{p-1}} = \frac{dQ}{q}$$

where, A : cross-sectional area of flow (sq.m)

n : Manning's roughness coefficient

I : water surface slope of flow

R : hydraulic radius (m)

Q : discharge (cu.m/sec)

k, p : constants

t : time

X : distance along channel (m)

q : lateral inflow per unit length of channel (cu.m/sec/m)

This means that to solve the former equations is to solve the following two equations on a characteristic curve, which is expressed as  $dX/dt = Q^{1-p}/pk$ . Thus;

$$qdt = pkQ^{p-1} \text{ or } qt = kQ^p + \text{constant, and}$$

$$qdX = dQ \text{ or } qX = Q + \text{constant}$$

Taking that constant = 0, the flow condition is expressed for a given magnitude of lateral inflow q, as;

$$t = kQ^p/q \text{ and}$$

$$t = kXQ^{p-1}$$

When q=0, it is expressed on a characteristic curve given above that A = constant and  $Q = \text{constant} = (A/k)^{1/p}$ . The flow condition is so given as follows:

$$X = (Q^{1-p}/pk)t.$$

### Some Consideration on Effect of Storage on a Paddy Plot

The time lag of concentration of runoff is generally recognized to be remarkable for drainage area mainly composed of low flat paddy because of storage capacity on a paddy plot. A paddy plot surrounded by levees with certain depth of flooding water can be regarded as a small reservoir and, therefore, the conception of simplified reservoir operation could be introduced to take into account the effect of rain water deposit on a paddy plot.

A storage function is introduced to calculate the specific runoff capacity from a paddy plot by the following equation:

$$\frac{dV}{dt} = I - O$$

where  $V$  denotes storage on a paddy plot,  $I$  and  $O$ , inflow into and outflow from a paddy plot respectively, and  $t$  time. The above equation can be divided by the water surface area on a plot,  $A$ , and then transformed:

$$\frac{dH}{dt} = i - o$$

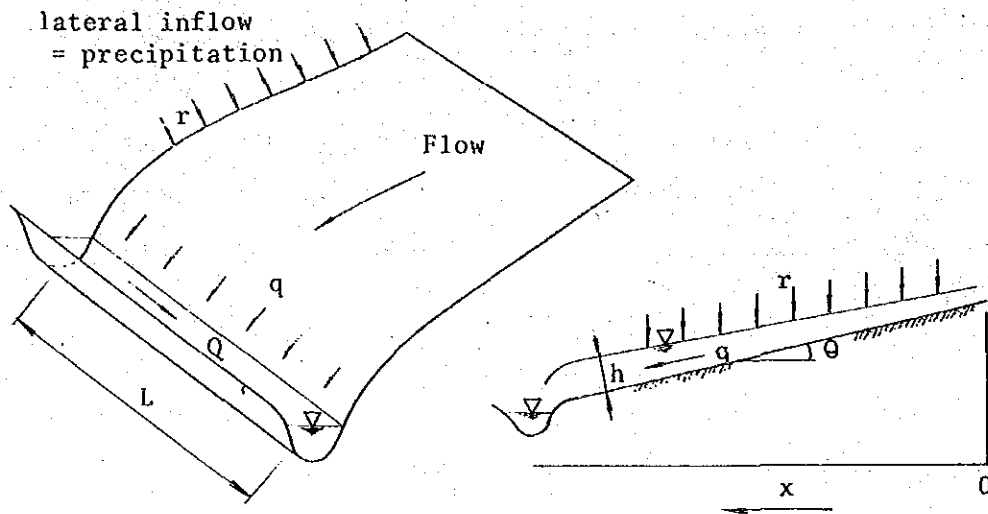
where  $H$  shows ponding depth on a plot,  $i$  specific inflow corresponding to effective rainfall on a plot, and  $o$  specific outflow corresponding to the specific runoff capacity from a paddy plot. A differential equation is constructed to solve the above equation by a computer as;

$$H_{t+1} = H_t + (RE_{t, t+1} - \frac{o_t + o_{t+1}}{2}) \Delta t$$

where  $RE_{t, t+1}$  represents effective rainfall between time  $t$  and  $t+1$  and  $\Delta t$  is a time interval given for computation. The specific runoff capacity from paddy fields is thus computed at corresponding time  $t$ , and then this is considered as a lateral inflow of drainage canal or stream.

Application of the Characteristic Method to a Real Problem

(1) For a Slope



- In the case when  $r \neq 0$

$$t = kq^p / \delta r$$

$$t = kXq^{p-1}$$

- When  $r = 0$

$$t = pkX/q^{1-p} = 0.6q^{-0.4}(N/I^{1/2})^{0.6}X$$

where,  $\delta$  : conversion rate from mm/hr to  $m^3/sec = 0.2778 \times 10^{-6}$

$r$  : effective rainfall (mm/hr)

$q$  : discharge per unit width of slope ( $m^3/sec/m$ )

$N$  : equivalent roughness coefficient of slope

$I$  : slope =  $\sin \theta$

$X$  : flow distance

(2) For River or Channel : As stated previously with theoretical conception.

(3) For Paddy Field

- for ditch

$$A_m = kQ_m^p$$

$$\frac{\partial A_m}{\partial t} + \frac{\partial Q_m}{\partial X} = (2bO)\alpha, \text{ and}$$

- for lateral drainage canal

$$A_b = kQ_b^p$$

$$\frac{\partial A_b}{\partial t} + \frac{\partial Q_b}{\partial X} = \frac{Q_m}{2b}$$

where,  $A_m, Q_m$  : flow area and discharge in a ditch

$A_b, Q_b$  : flow area and discharge in a lateral canal

$k, p$  : constants

$\alpha$  : =  $0.2778 \times 10^{-6}$

$b$  : see Figure

$O$  : runoff capacity per unit area (mm/hr)

Table D-6-3 presents basin characteristics given for flood runoff analysis, and Figure D-6-2 shows the concept of runoff capacity from the paddy field.

TABLE D-6-1 DAILY RAINFALL IN OCTOBER, 1983

Daily Rainfall in (1983 October) (1)

Day	0304	0308	0321	0322	0606	0901	0916	0917	2204	2207
6	-	-	-	6.2	7.4	-	-	2.6	-	0.9
7	-	13.0	-	-	8.1	-	-	-	-	-
8	-	12.4	-	9.1	17.6	3.2	7.4	-	14.5	-
9	-	24.0	-	1.0	16.0	13.0	4.5	63.1	22.5	43.2
10	35.1	38.6	30.4	44.1	12.7	13.2	18.7	21.8	62.5	70.5
11	16.2	45.6	40.4	49.3	6.4	15.6	7.8	25.6	26.6	92.3
12	8.2	5.7	-	2.8	43.8	18.8	-	0.3	35.8	9.5
13	0.0	12.7	40.0	36.7	2.3	40.1	-	10.0	22.6	56.2
14	14.5	10.0	15.5	21.0	14.7	-	36.8	0.7	10.2	-
15	0.0	8.4	-	-	7.4	10.3	7.6	-	16.2	27.5
16	40.1	-	6.5	2.7	-	-	-	-	12.1	-
17	0.0	11.5	-	-	48.4	6.0	17.6	26.1	5.2	28.2
18	25.3	40.0	45.5	41.1	39.4	50.1	132.0	37.6	22.3	73.9
19	56.6	3.3	40.4	44.9	28.7	25.8	6.9	1.5	8.6	-
20	0.0	-	8.5	-	25.1	0.1	2.6	7.8	29.5	4.2
21	-	-	0.5	-	-	14.5	0.9	-	6.2	-
22	-	-	-	-	-	0.9	9.6	11.0	15.8	-
23	-	-	-	3.3	-	4.2	27.9	51.5	8.5	-
24	-	-	-	-	-	-	-	-	-	-
25	-	-	-	-	-	-	-	-	51.2	-
Total	196.3	231.2	277.7	262.2	283.0	215.8	280.3	259.6	324.3	406.4

Daily Rainfall in October, 1983 (2)

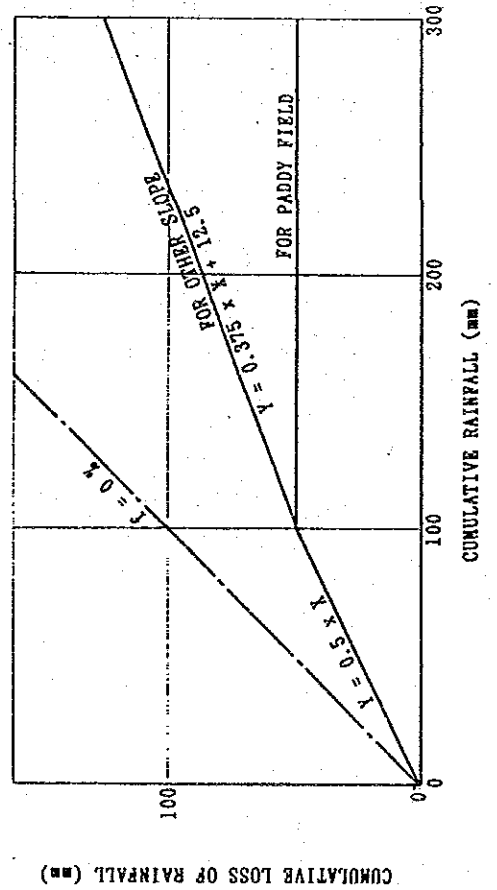
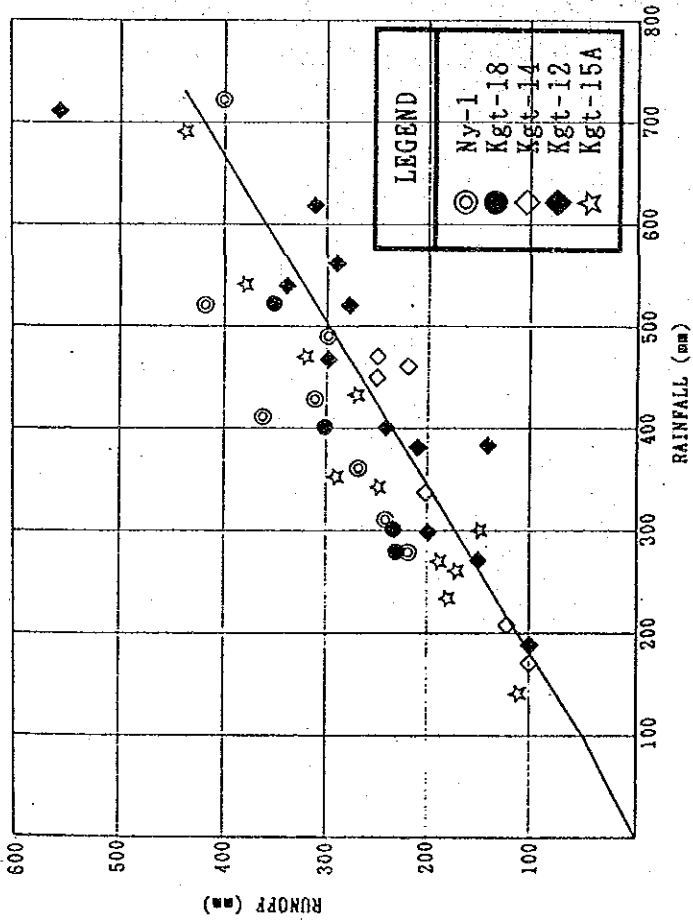
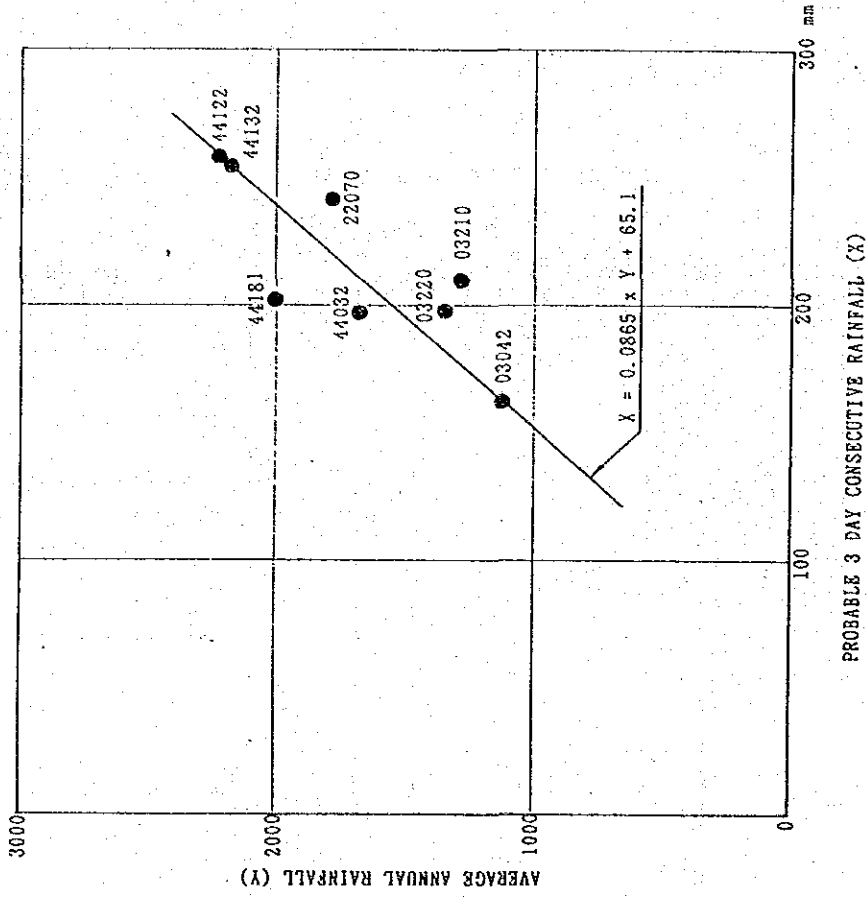
Day	2215	2517	2553	4402	4404	4406	4408	4412	4413	2515
6	-	-	0.8	-	-	-	-	-	-	1.0
7	5.7	-	-	10.0	-	-	-	-	-	-
8	2.4	7.9	4.8	-	6.0	5.8	6.7	26.0	25.3	6.0
9	22.8	3.9	2.3	35.5	2.9	11.2	13.1	31.8	4.0	2.9
10	83.2	111.0	55.1	40.0	51.3	60.2	49.5	97.5	88.0	69.4
11	16.7	59.0	2.4	60.0	1.0	-	2.8	4.9	-	3.0
12	5.6	110.0	41.0	15.0	8.2	-	48.5	1.8	51.0	51.7
13	22.4	136.7	5.7	20.5	5.2	-	33.3	43.9	26.3	7.2
14	22.6	8.3	0.4	-	2.9	15.0	-	-	52.6	0.5
15	167.4	33.5	21.3	15.0	21.8	16.5	-	24.5	3.7	26.8
16	-	-	-	-	2.6	-	8.0	-	3.0	-
17	35.5	-	27.7	30.5	55.3	-	65.9	95.3	37.6	34.9
18	31.4	49.6	21.7	-	34.1	23.0	5.0	18.8	9.0	27.3
19	42.3	-	-	-	0.7	30.5	-	-	1.3	-
20	-	-	5.7	-	6.7	25.0	6.8	25.6	7.2	7.2
21	-	1.3	-	-	-	-	-	-	7.1	-
22	-	3.2	7.0	-	0.8	-	-	66.0	0.4	8.8
23	-	-	-	-	-	-	-	-	-	-
24	-	-	-	-	-	-	-	8.3	-	-
25	-	-	-	-	-	-	-	-	-	-
Total	398.0	524.4	195.9	226.5	199.5	187.2	239.6	434.4	316.5	246.7



TABLE D-6-2 AREAL DISTRIBUTION OF DAILY RAINFALL IN OCTOBER, 1983

		OCTOBER 1983																							TOTAL
NO.	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23							
LBP	1	0.0	0.0	3.2	13.0	13.2	15.6	18.8	40.1	0.0	10.3	0.0	6.0	50.1	25.8	0.1	14.5	0.9	4.2	215.8					
	2	0.0	1.3	1.3	2.8	34.8	19.1	8.3	3.1	13.6	1.1	34.9	1.3	27.8	49.8	0.0	0.4	0.0	0.1	199.9					
	3	0.0	0.0	0.0	0.0	49.6	23.9	5.6	12.8	14.8	0.0	29.3	0.0	32.0	51.0	2.7	0.2	0.0	0.0	221.9					
	4	0.0	0.0	2.0	8.2	21.3	15.8	14.9	25.3	5.4	6.5	14.8	3.8	41.0	37.0	0.1	9.1	0.6	2.6	208.4					
	5	0.4	0.0	2.9	20.1	14.8	16.7	15.1	33.6	1.9	8.6	0.0	9.6	52.3	21.2	1.4	11.6	2.8	12.5	225.6					
	6	1.5	0.0	0.0	35.3	27.7	21.5	3.8	5.6	6.8	0.0	17.6	14.6	32.3	25.5	4.4	0.0	6.2	28.8	231.5					
	7	1.8	0.0	2.1	46.1	20.9	20.4	0.2	7.1	11.2	2.2	0.0	23.6	65.0	3.1	6.3	0.3	10.6	44.7	265.6					
	8	1.7	0.0	0.0	40.4	26.6	22.2	3.1	6.4	5.7	0.0	14.4	16.7	33.3	21.1	5.0	0.0	7.0	33.0	236.6					
	9	2.6	0.0	0.0	63.1	21.8	25.6	0.3	10.0	0.7	0.0	0.0	26.1	37.6	1.5	7.3	0.0	11.0	51.5	259.6					
	10	2.6	0.0	0.0	63.1	21.8	25.6	0.3	10.0	0.7	0.0	0.0	26.1	37.6	1.5	7.8	0.0	11.0	51.5	259.6					
	11	2.8	0.0	0.5	60.0	22.9	26.8	0.4	11.3	1.7	0.0	0.1	24.8	37.8	3.7	7.1	0.0	10.4	49.1	259.7					
	12	3.1	0.0	1.2	55.0	24.7	28.7	0.6	13.5	3.3	0.0	0.4	22.7	38.1	7.1	6.8	0.0	9.6	45.2	259.9					
	13	0.0	0.0	0.0	0.6	43.3	20.6	6.7	7.2	14.7	0.0	34.1	0.0	29.2	53.2	1.5	0.1	0.0	0.0	210.5					
	14	2.5	0.0	0.0	59.9	24.7	26.3	0.3	11.5	1.4	0.0	0.3	24.8	38.0	3.4	7.8	0.0	10.4	48.9	260.5					
	15	0.3	0.0	0.0	6.9	74.0	38.8	0.0	36.7	13.9	0.0	5.8	2.9	44.6	36.1	8.4	0.4	1.2	5.7	275.7					
	16	1.0	0.0	0.9	9.6	68.0	39.1	0.3	35.2	13.8	0.0	5.1	3.9	43.9	35.0	7.5	0.4	1.6	8.1	273.4					
	17	0.0	0.0	0.0	0.0	80.4	40.4	0.0	40.0	15.5	0.0	6.5	0.0	45.5	40.4	8.5	0.5	0.0	0.0	277.7					
KTL	1	0.0	0.0	0.0	0.0	80.4	40.4	0.0	40.4	15.5	0.0	6.5	0.0	45.5	40.4	8.5	0.5	0.0	0.0	277.7					
	2	0.0	0.0	0.0	0.0	80.4	40.4	0.0	40.4	15.5	0.0	6.5	0.0	45.5	40.4	8.5	0.5	0.0	0.0	277.7					
	3	0.0	0.0	0.0	0.0	80.4	40.4	0.0	40.4	15.5	0.0	6.5	0.0	45.5	40.4	8.5	0.5	0.0	0.0	277.7					
	4	3.6	0.0	5.3	0.6	59.3	45.6	1.6	38.1	18.7	0.0	4.3	0.0	42.9	43.0	3.6	0.2	0.0	1.9	268.7					
	5	5.4	0.0	7.9	1.2	44.9	48.7	2.5	35.5	19.8	0.1	4.0	0.1	42.2	44.0	0.7	0.4	0.0	2.9	260.3					
	6	4.5	0.0	6.6	2.4	32.6	49.9	2.3	28.7	16.6	0.8	8.0	0.7	45.1	41.9	0.2	1.9	0.0	2.6	244.6					
	7	5.8	0.0	8.0	8.5	41.4	46.5	2.5	33.5	18.6	0.0	2.4	3.1	40.7	39.7	0.9	0.0	1.3	9.1	251.9					
	8	6.2	0.0	9.1	1.0	44.1	49.3	2.8	38.7	21.0	0.0	2.7	0.0	41.1	44.9	0.0	0.0	0.0	3.3	262.2					
	9	6.2	0.0	9.1	1.0	44.1	49.3	2.8	38.7	21.0	0.0	2.7	0.0	41.1	44.9	0.0	0.0	0.0	3.3	262.2					
UBP	1	0.0	4.2	2.8	13.9	61.2	38.6	4.4	25.1	10.9	7.7	2.9	8.9	29.8	23.7	8.2	0.2	0.0	0.0	242.6					
	2	0.2	3.4	3.4	25.9	56.1	40.3	6.4	19.9	7.3	18.2	0.0	15.2	30.7	13.7	12.0	0.0	0.0	0.0	252.7					
	3	0.0	0.0	2.4	6.9	28.5	34.4	2.1	9.3	10.3	6.5	13.8	2.7	44.0	32.7	8.4	4.3	0.0	0.5	206.6					
	4	0.0	0.0	6.8	10.8	61.6	0.0	2.5	1.3	16.9	15.9	0.1	1.9	22.3	29.0	24.1	0.4	0.0	0.0	193.7					
	5	0.0	0.0	9.3	9.9	65.2	0.0	9.2	4.7	21.8	14.2	0.5	6.8	20.5	25.2	21.8	1.3	0.1	0.0	210.5					
MNV	1	0.5	4.2	1.4	31.9	57.3	60.5	20.4	31.2	0.3	21.9	0.2	30.0	33.0	0.2	3.5	0.1	2.0	0.2	298.8					
	2	0.6	1.0	6.6	14.8	62.1	23.2	37.9	16.8	2.7	22.1	2.9	25.3	26.9	2.1	11.2	1.5	8.4	2.0	268.2					
	3	0.3	0.0	9.6	29.5	65.2	48.9	26.9	34.0	6.7	20.0	8.0	13.0	39.8	5.7	20.9	4.1	10.4	5.6	348.8					
	4	0.0	0.0	11.9	15.2	81.4	39.2	64.7	67.1	9.5	22.9	7.4	3.2	32.9	5.2	18.0	4.3	10.9	5.2	399.1					
	5	0.7	0.0	2.3	33.7	79.1	82.3	32.7	72.6	2.2	28.4	0.5	21.1	66.5	9.3	4.3	0.5	1.3	0.3	428.9					
	6	0.3	0.0	3.7	22.9	65.0	35.3	5.7	23.5	9.3	20.9	0.0	10.4	42.4	18.6	16.8	0.0	0.1	0.0	275.0					
MPP	1	0.4	0.0	13.4	18.3	52.1	28.4	1.5	27.7	3.8	13.3	9.8	42.9	36.4	18.1	12.9	2.9	32.3	0.6	314.9					
MHM	1	0.0	0.2	22.7	8.6	82.4	5.3	38.7	27.2	40.5	9.7	4.0	41.4	14.7	5.0	9.0	5.9	9.5	0.1	324.9					
	2	0.0	0.0	21.0	5.6	81.9	0.0	39.8	20.5	44.3	6.5	2.3	29.3	12.1	7.7	11.1	5.5	0.3	0.0	288.1					
	3	0.0	2.4	7.0	13.5	85.6	21.8	35.5	46.5	20.7	57.5	0.3	19.0	31.6	24.6	6.3	1.1	0.8	0.0	374.3					
	4	0.0	4.7	6.3	19.6	84.0	13.9	13.3	23.1	27.7	89.8	0.5	35.9	27.6	35.3	1.2	1.2	0.1	0.0	384.1					
	5	0.0	5.7	2.4	22.8	83.2	16.7	5.6	22.4	22.6	107.4	0.0	35.5	31.4	42.3	0.0	0.0	0.0	0.0	398.0					
	6	0.0	5.7	2.4	22.8	83.2	16.7	5.6	22.4	22.6	107.4	0.0	35.5	31.4	42.3	0.0	0.0	0.0	0.0	398.0					
	7	0.0	0.0	26.0	31.8	97.5	4.9	1.8	43.9	0.0	24.5	0.0	85.3	18.8	0.0	25.6	0.0	66.0	0.0	426.1					
	8	0.0	3.1	13.2	24.7	89.0	10.9	7.8	30.9	15.4	67.6	0.2	54.6	24.8	22.9	10.3	0.6	25.1	0.0	402.1					
	9	0.3	0.0	19.0	22.1	83.5	4.1	14.7	31.3	0.1	23.4	0.0	66.3	19.8	0.0	19.0	0.0	46.5	0.0	350.1					
KPS	1	0.0	0.0	19.0	25.6	81.7	4.2	17.2	40.4	0.0	16.4	2.6	78.9	14.2	0.0	19.4	0.0	44.2	0.0	370.1					
	2	1.8	0.0	8.9	11.1	51.8	16.5	31.5	35.1	6.1	2.0	5.8	48.3	16.6	13.0	6.3	0.0	5.3	1.0	364.5					
	3	3.3	2.8	10.8	12.8	36.0	9.2	43.6	22.8	7.5	2.6	4.6	52.5	21.0	15.0	12.5	0.0	0.4	257.3						
	4	5.7	4.7	13.6	12.0	26.9	15.6	38.2	16.1	13.4	4.3	2.1	40.6	33.3	27.0	15.8	0.0	0.0	0.8	270.0					
UPP	1	0.0	0.0	23.9	29.7	92.2	4.7	6.9	42.7	0.0	21.8	0.9	83.2	17.3	0.0	23.5	0.0	53.7	0.0	405.6					
	2	0.0	0.0	11.1	17.4	60.5	3.3	37.8	35.7	0.0	5.6	6.2	70.4	8.2	0.0	11.1	0.0	15.2	0.0	282.5					
	3	0.4	0.0	7.9	10.2	57.4	2.8	39.9	21.8	0.2	12.5	3.4	50.5	14.2	0.0	8.4	0.0	10.5	0.0	240.9					
MEAN		1.2	0.8	6.5	18.9	55.7	26.3	12.7	26.9	11.3	14.8	5.3	22.6	34.0	22.3	8.4	1.4	8.0	7.9	285.2					

FIGURE D-6-1 STORM RAINFALL ANALYSIS



### D.6.3 Flood Discharge

The following peak discharge and total amount of the design flood were obtained for (1) existing basin condition and (2) anticipated condition in future, as the result of the preliminary study of flood analysis. For the computation, topographic condition of irrigation block including drainage area and surface slope by land use and length and slope of river channel was read on 1/50,000 topo-maps (Table D-6-3).

Peak Discharge and Flood Volume Analyzed

<u>Station</u>	<u>Drainage Area</u> (sq. km)	<u>Basin Rainfall</u> (mm)	<u>Existing</u>		<u>Anticipated</u>	
			<u>Peak Discharge</u> (cum/sec)	<u>Specific Volume</u> (mm)	<u>Peak Discharge</u> (cum/sec)	<u>Specific Volume</u> (mm)
801	1,628	247.6	1500.6	115.2	2056.0	131.8
701	2,643	227.9	1203.3	82.8	1408.6	88.6
601	5,241	223.3	3079.0	91.4	3941.0	102.3
501	2,130	247.6	1492.2	118.5	1700.6	125.1
401	10,128	171.2	5750.9	85.8	7063.8	102.8
301	1,933	199.0	1085.4	97.0	2129.1	118.4
201	2,493	153.9	1042.7	68.8	1060.0	70.2
101	17,660	145.8	8,361.8	72.3	10181.3	89.8

FIGURE D-6-2 RUNOFF CAPACITY FROM PADDY FIELD

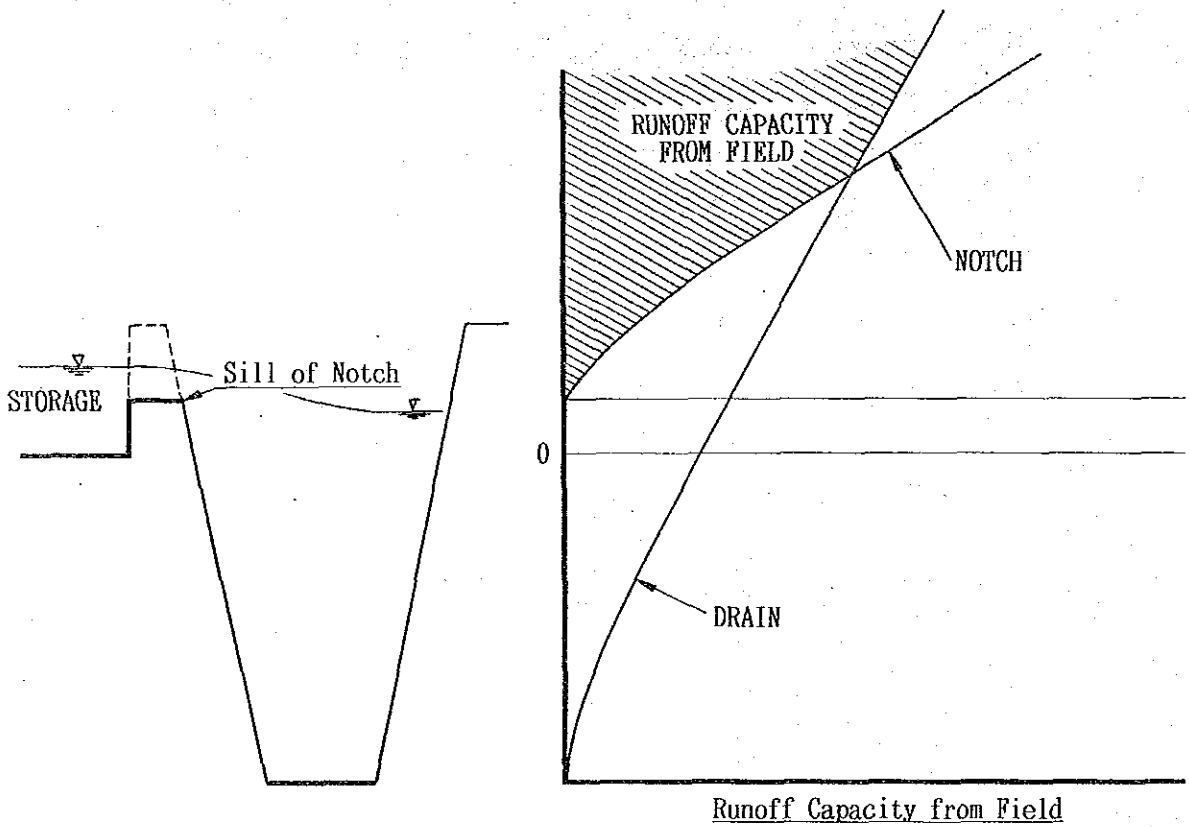
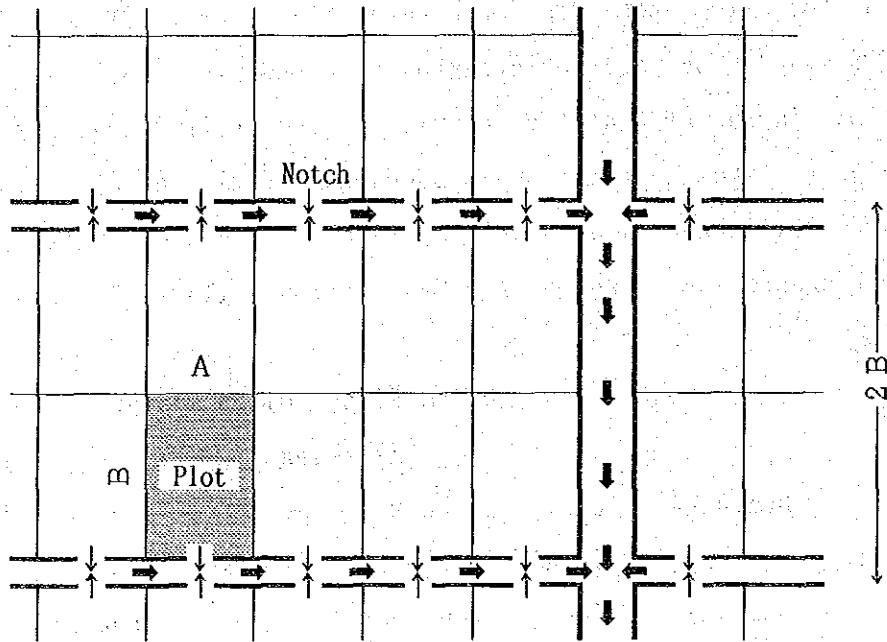


TABLE D-6-3 BASIN CHARACTERISTICS GIVEN FOR FLOOD RUNOFF ANALYSIS

Sub Basin	Site No	Watershed Area (sq. km)				Slope Gradient			River Chan.	
		Paddy	Upland	Others	Total	Pad.	Upld	Othr	Lngt	Slope
UPP-3	803	0.0	0.0	266.0	266.0	300	200	30	30	90
-2	802	215.2	103.0	455.6	774.0	200	170	10	58	1450
-1	801	250.4	272.1	65.5	588.0	200	160	10	58	2900
KPS-4	704	19.7	231.2	363.1	614.0	200	150	40	48	170
-3	703	26.2	304.2	507.6	838.0	200	120	80	45	1100
-2	702	92.4	103.3	605.3	801.0	200	150	120	38	1900
-1	701	161.3	163.2	65.5	390.0	300	200	100	50	5000
MPP-1	601	195.2	218.2	556.6	970.0	500	450	200	63	6300
MHM-5	505	0.0	0.0	68.0	68.0	400	300	25	8	90
-6	506	0.0	56.5	39.5	96.0	500	400	15	7	35
-4	504	38.8	3.9	131.3	174.0	800	500	60	18	450
-8	508	0.0	0.0	64.0	64.0	200	100	60	10	250
-9	509	0.0	0.0	147.0	147.0	1000	500	50	20	80
-7	507	0.0	0.0	232.0	232.0	1000	800	100	28	350
-3	503	0.0	0.0	273.0	273.0	200	150	110	37	300
-2	502	0.0	0.0	159.0	159.0	500	300	170	28	40
-1	501	206.4	340.6	370.0	917.0	2000	800	80	53	1300
UBP-3	403	346.9	374.5	28.6	750.0	1000	400	300	40	8000
-5	405	0.0	0.0	107.0	107.0	300	200	130	15	35
-4	404	230.3	103.6	60.1	394.0	500	250	110	35	1200
-2	402	177.2	262.8	6.0	446.0	1000	250	60	38	8000
-1	401	919.4	134.8	5.8	1060.0	5000	3000	400	43	8000
MNN-6	306	0.0	0.0	151.0	151.0	200	100	10	18	450
-5	305	34.3	82.1	339.6	456.0	3000	500	300	40	110
-4	304	0.0	0.0	114.0	114.0	200	100	10	8	60
-3	303	179.4	44.1	121.5	345.0	1000	300	60	25	1700
-2	302	260.3	46.2	62.5	369.0	5000	5000	400	15	1900
-1	301	342.8	8.0	147.2	498.0	5000	5000	400	35	3500
KTL-9	209	1.3	24.5	559.2	585.0	200	100	70	38	420
-8	208	3.8	36.4	350.8	391.0	200	100	20	38	380
-7	207	2.5	98.7	293.8	395.0	200	140	70	33	1700
-4	204	7.9	100.8	28.3	137.0	200	100	100	25	2500
-6	206	1.8	82.3	96.9	181.0	200	180	100	18	1800
-5	205	7.6	121.2	488.2	617.0	200	200	100	43	1100
-3	203	0.0	62.7	6.3	69.0	5000	3000	200	18	1000
-2	202	1.3	77.3	7.4	86.0	5000	3000	200	15	7500
-1	201	13.7	15.2	3.1	32.0	5000	3000	400	22	9000
LBP-17	117	11.9	3.6	4.5	20.0	5000	500	400	7	3500
-16	116	22.0	160.8	16.2	199.0	5000	400	400	30	1250
-15	115	18.5	41.8	4.7	65.0	5000	300	300	12	500
-14	114	12.5	171.3	17.2	201.0	5000	250	300	25	830
-13	113	27.8	12.7	15.5	56.0	5000	400	400	8	800
-12	112	1.4	250.0	92.6	344.0	2000	100	70	15	500
-11	111	6.4	137.9	39.7	184.0	2000	200	70	15	1000
-10	110	25.3	53.9	5.8	85.0	2000	200	300	13	1300
-9	109	26.2	54.6	7.2	88.0	2000	200	70	15	300
-8	108	99.5	77.8	4.7	182.0	5000	500	300	28	2800
-7	107	39.7	415.5	45.8	501.0	2000	100	70	25	190
-6	106	59.6	13.0	7.4	80.0	2000	200	200	25	1050
-3	103	438.9	82.2	4.9	526.0	5000	3000	400	45	15000
-2	102	48.4	12.6	58.0	119.0	5000	3000	400	8	15000
-5	105	104.8	172.2	2.0	279.0	2000	100	70	20	670
-4	104	83.9	41.6	16.5	142.0	5000	800	400	15	5000
-1	101	0.0	1.6	33.4	35.0	5000	500	400	10	1700

## D.7 INTAKE-RATE TEST

Intake-rate tests were conducted at the selected five (5) sites in the proposed Tha Lat Expansion irrigation area. Figure D-7-1 shows the selected sites, and the measurements are summarized in Table D-7-1. They were further analyzed in order to obtain the basic intake-rates and others as given in Figure D-7-2.

FIGURE D-7-1 LOCATION OF INTAKE-RATE TEST SITES

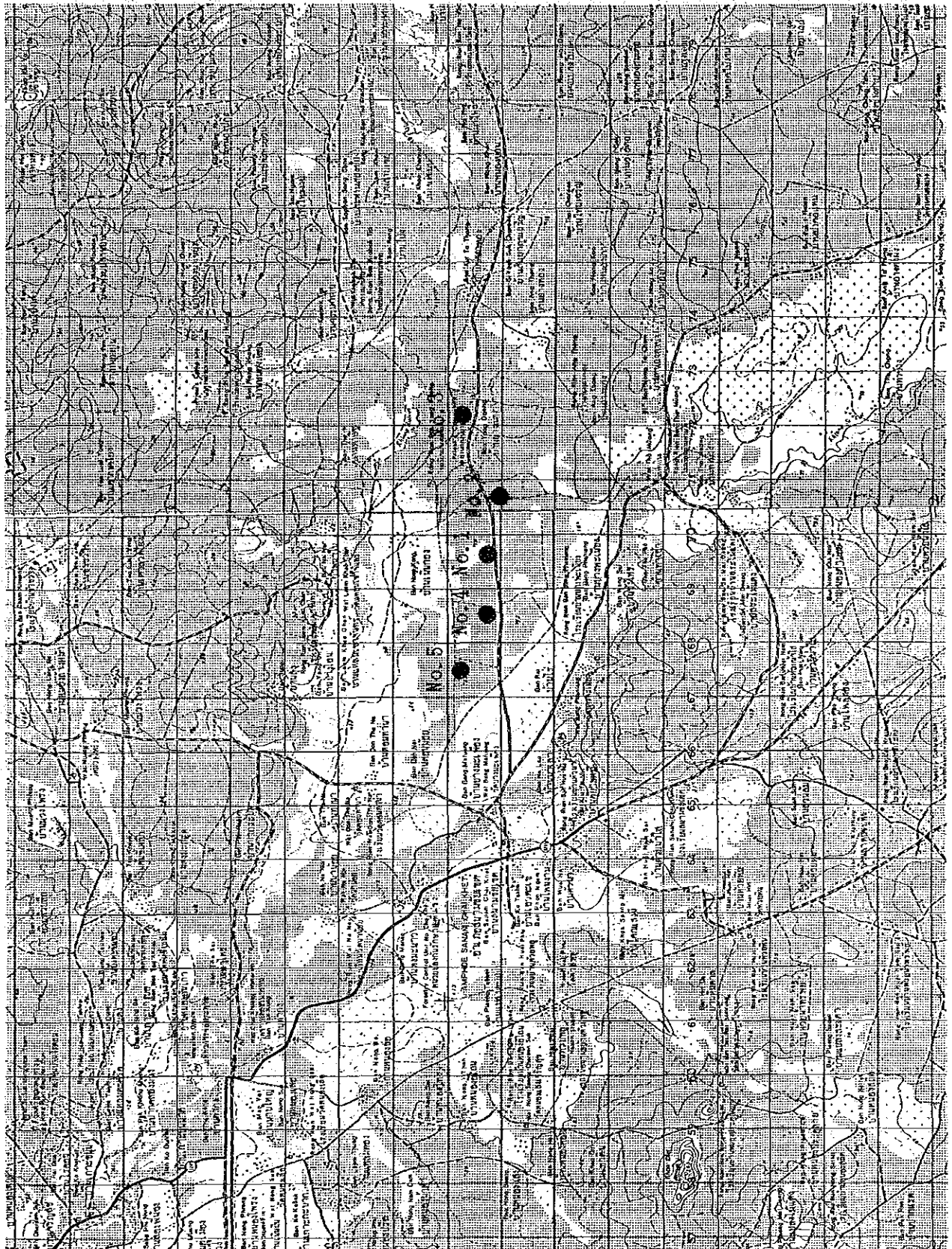
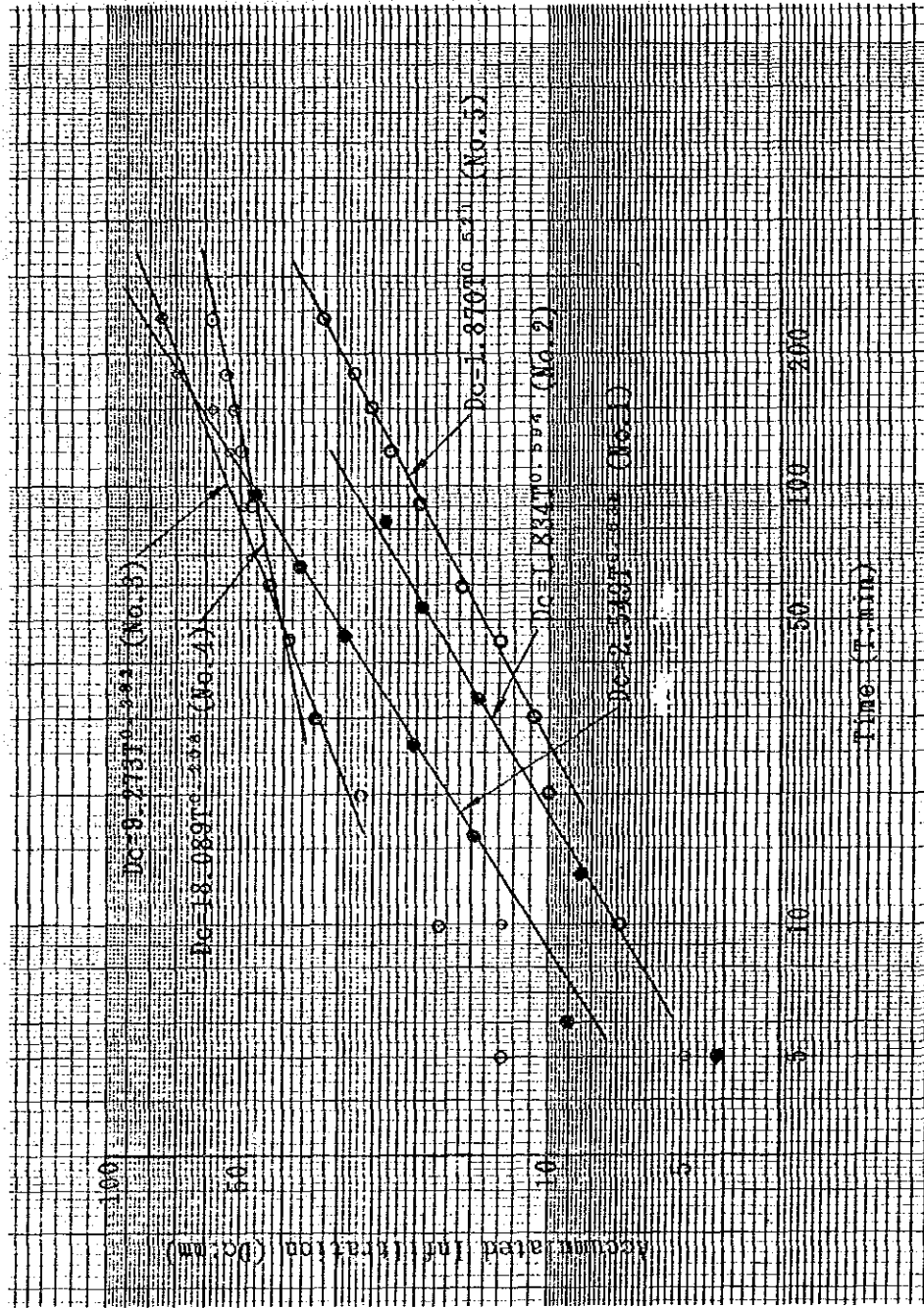


TABLE D-7-1 Data Sheet for Intake Rate Test

Reading Time (Hr)	Time between Readings (min.)	Water Level (cm.)	Difference (mm)	Accumulation (mm)
(1) No. 1 site				
11:24	0	24.33	0.0	0.0
30	6	23.42	9.1	9.1
40	16	22.83	5.9	15.0
50	26	22.30	5.3	20.3
12:10	46	21.40	9.0	29.3
30	66	20.67	7.3	36.6
13:00	96	19.72	9.5	46.1
(2) No. 2 site				
11:32	0	19.50	0.0	0.0
37	5	19.08	4.2	4.2
45	13	18.65	4.3	8.5
12:05	33	18.03	6.2	14.7
25	53	17.57	4.6	19.3
55	83	17.14	4.3	23.6
(3) No. 3 site				
13:30	0	20.0	0.0	0.0
35	5	19.5	5.0	5.0
40	10	18.7	8.0	13.0
50	20	17.3	14.0	27.0
14:00	30	16.6	7.0	34.0
15	45	16.1	5.0	39.0
30	60	15.7	4.0	43.0
15:00	90	15.1	6.0	49.0
30	120	14.6	5.0	54.0
16:00	150	14.2	4.0	58.0
30	180	14.0	2.0	60.0
17:30	240	13.5	5.0	65.0
(4) No. 4 site				
10:45	0	20.0	0.0	0.0
50	5	18.7	13.0	13.0
55	10	18.2	5.0	18.0
11:05	20	17.3	9.0	27.0
15	30	16.6	7.0	34.0
30	45	16.0	6.0	40.0
45	60	15.7	3.0	43.0
12:15	90	15.3	4.0	47.0
45	120	15.0	3.0	50.0
13:15	150	14.8	2.0	52.0
45	180	14.6	2.0	54.0
14:45	240	14.2	4.0	58.0
(5) No. 5 site				
9:50	0	20.0	0.0	0.0
55	5	19.5	5.0	5.0
10:00	10	19.3	2.0	7.0
10	20	19.0	3.0	10.0
20	30	18.9	1.0	11.0
35	45	18.7	2.0	13.0
50	60	18.4	3.0	16.0
11:20	90	18.0	4.0	20.0
50	120	17.7	3.0	23.0
12:20	150	17.5	2.0	25.0
50	180	17.2	3.0	28.0
13:50	240	16.8	4.0	32.0



FIGURE D-7-2 INTAKE-RATES AT VARIOUS TEST SITES



## D.8 PEAK IRRIGATION REQUIREMENT

### Peak Irrigation Requirements

(Unit: liter/sec/ha)

Year	Existing		Existing & Proposed		Proposed	
	Tha	Lat	Bang Pakong		Tha	Lat
	First	Second	First	Second	First	Second
1968	1.188	0.961	1.360	1.162	1.072	1.051
69	0.970	0.754	1.049	0.759	0.866	0.818
1970	1.042	0.909	1.277	1.209	0.903	0.884
71	1.035	0.958	1.101	1.036	1.105	1.016
72	1.060	1.031	1.137	1.040	1.206	1.087
73	1.188	0.997	1.231	1.112	1.208	1.031
74	1.056	0.952	1.081	1.035	1.146	1.132
1975	1.084	0.878	1.068	0.968	1.176	0.961
76	1.050	0.909	1.149	0.934	0.971	0.933
77	0.988	0.978	1.126	1.122	0.970	0.962
78	1.192	0.801	1.302	0.877	1.155	0.859
79	1.199	1.188	1.273	1.231	1.208	1.208
1980	1.209	0.797	1.284	0.831	1.246	0.838
81	1.197	0.944	1.296	1.100	1.189	0.962
82	1.109	0.901	1.143	0.907	1.171	1.053
83	0.976	0.896	1.022	0.973	1.188	0.912
84	1.188	0.801	1.231	1.057	1.208	0.785
1985	1.101	0.877	1.189	0.834	1.125	1.088
86	1.170	0.787	1.324	0.904	1.118	0.915
87	0.946	0.882	1.037	1.015	1.054	0.743
Probable(*)	1.2	1.0	1.3	1.2	1.2	1.1

Note(\*): Probable value during 5-year drought period.

## D.9 RESULTS OF WATER BALANCE SIMULATION

The following tables summarize the simulated results of water balance for the subject feasibility study area:

<u>Tables</u>	<u>Case of Computation</u>	<u>Remarks</u>
D-9-1	1	Water demand computation
D-9-2	1	Runoff and water diversion
D-9-3	1	Water balance at the water sources
D-9-4	2	Water demand computation
D-9-5	2	Runoff and water diversion
D-9-6	2	Water balance at the water sources

TABLE D-9-1 WATER DEMAND COMPUTATION (COMPUTATION CASE-1)

YEAR	WERT	IRRIG.	BANG FISH.	PAKONG OTHER	TOTAL	EXISTING FISH.	THA LAT OTHER	EXPANSION OTHER	TOTAL	IRRIG.	ENTIRE FISH.	PROJECT OTHER	AREA TOTAL
1968	1	1	1	1	1	1	1	1	1	1	1	1	1
1969	1	1	1	1	1	1	1	1	1	1	1	1	1
1970	1	1	1	1	1	1	1	1	1	1	1	1	1
1971	1	1	1	1	1	1	1	1	1	1	1	1	1
1972	1	1	1	1	1	1	1	1	1	1	1	1	1
1973	1	1	1	1	1	1	1	1	1	1	1	1	1
1974	1	1	1	1	1	1	1	1	1	1	1	1	1
1975	1	1	1	1	1	1	1	1	1	1	1	1	1
1976	1	1	1	1	1	1	1	1	1	1	1	1	1
1977	1	1	1	1	1	1	1	1	1	1	1	1	1
1978	1	1	1	1	1	1	1	1	1	1	1	1	1
1979	1	1	1	1	1	1	1	1	1	1	1	1	1
1980	1	1	1	1	1	1	1	1	1	1	1	1	1
1981	1	1	1	1	1	1	1	1	1	1	1	1	1
1982	1	1	1	1	1	1	1	1	1	1	1	1	1
1983	1	1	1	1	1	1	1	1	1	1	1	1	1
1984	1	1	1	1	1	1	1	1	1	1	1	1	1
1985	1	1	1	1	1	1	1	1	1	1	1	1	1
1986	1	1	1	1	1	1	1	1	1	1	1	1	1
1987	1	1	1	1	1	1	1	1	1	1	1	1	1
ANNUM		4870.	255.	3296.	8421.	4522.	128.	1078.	5728.	1414.	382.	457.	15745.



TABLE D-9-3 WATER BALANCE AT WATER SOURCES (COMPUTATION CASE-1)

3) WATER SOURCES

YEAR	ENTIRE INFLOW	THA INFLOW	EXISTING THA INFLOW	THA LAT EXPANSION INFLOW	ENTIRE OUTFLOW	THA LAT EXPANSION OUTFLOW	EXISTING THA OUTFLOW	THA LAT EXPANSION OUTFLOW	PROJECT UPSTREAM LOSSES	AREAS LOSSES	OUTFLOW
1968	34287	27840	11560	4000	3934	292	11560	292	408	12	4069
1969	33901	38930	12291	30	43267	1147	12291	1147	372	11	4573
1970	55903	31028	12291	310	55903	1417	12291	1417	1	11	55903
1971	54376	50232	12291	316	55055	1169	12291	1169	1	11	55055
1972	44326	31617	12291	316	47488	1422	12291	1422	1	11	47488
1973	53308	22955	12291	229	55581	1422	12291	1422	1	11	55581
1974	43308	22955	12291	229	47488	1422	12291	1422	1	11	47488
1975	43308	22955	12291	229	47488	1422	12291	1422	1	11	47488
1976	44326	31617	12291	316	47488	1422	12291	1422	1	11	47488
1977	55489	32235	12291	322	55581	1422	12291	1422	1	11	55581
1978	44326	31617	12291	316	47488	1422	12291	1422	1	11	47488
1979	44326	31617	12291	316	47488	1422	12291	1422	1	11	47488
1980	45489	32235	12291	322	47488	1422	12291	1422	1	11	47488
1981	45489	32235	12291	322	47488	1422	12291	1422	1	11	47488
1982	65489	45228	12291	452	65489	1422	12291	1422	1	11	65489
1983	54326	31617	12291	316	55055	1422	12291	1422	1	11	55055
1984	46326	31617	12291	316	47488	1422	12291	1422	1	11	47488
1985	54326	31617	12291	316	55055	1422	12291	1422	1	11	55055
1986	54326	31617	12291	316	55055	1422	12291	1422	1	11	55055
1987	43308	22955	12291	229	47488	1422	12291	1422	1	11	47488
ANNUM	101188	6843	5807	0	114271	9355	5807	9355	15162	359	119341

