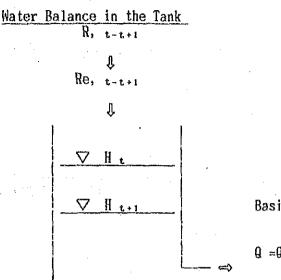
2.2 Runoff Analysis by Tank Model (X-57)

(1) Tank Model

(a) General



Basic Equation

Q ≈Qo x e^{-Kt}

The water balance in the tank is given by the following differential equation :

$$\begin{array}{rcl} H_{t+1}=&H_{t}+(\operatorname{Re},_{t-t+1}-(Q_{t}+Q_{t+1})/2) \ dt \\ \\ \text{where: } H_{t+1}&=& \text{water stage in the tank at }t=t+1 \\ \\ H_{t}&=& -do-& \text{at }t=t \\ \\ \operatorname{Re}, t_{t+1}&=& \operatorname{effective rain poured into tank during time }t \\ \\ & & \operatorname{and }t+1 \\ \\ Q_{t}&=& \operatorname{outflow from tank at }t=t \\ \\ Q_{t+1}&=& -do-& \operatorname{at }t=t+1 \\ \\ & & \operatorname{dt }&=& \operatorname{time interval for calculation} \end{array}$$

Giving that dt = 1.0 day, $Q_{t} = KH_{t}$ and $Q_{t+1} = KH_{t+1}$, then ; $H_{t+1} = H_{t} + Re, t_{-t+1} - KH_{t+1}/2$ $H_{t+1} = (1-K/2)/(1+K/2)H_{t} + 1/(1+K/2)Re, t_{-t+1}$ $= aH_{t} + bRe, t_{-t+1}$

RUNOFF ANALYSIS BY TANK MODEL (DAILY BASE)• LOCATION: P H A N G• ANALYSED POINT: X - 5 7• ANALYSED POINT: P H A N O M + K A P O N G + T A K U A P A• CA: 312.000 (km)

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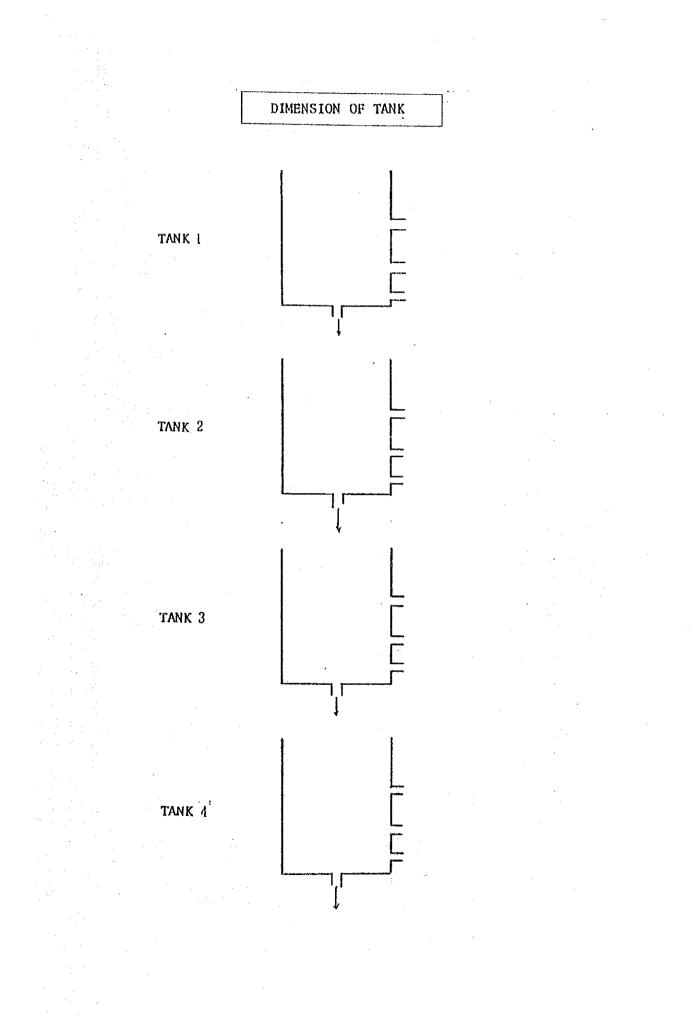
DIMENSION OF TANK

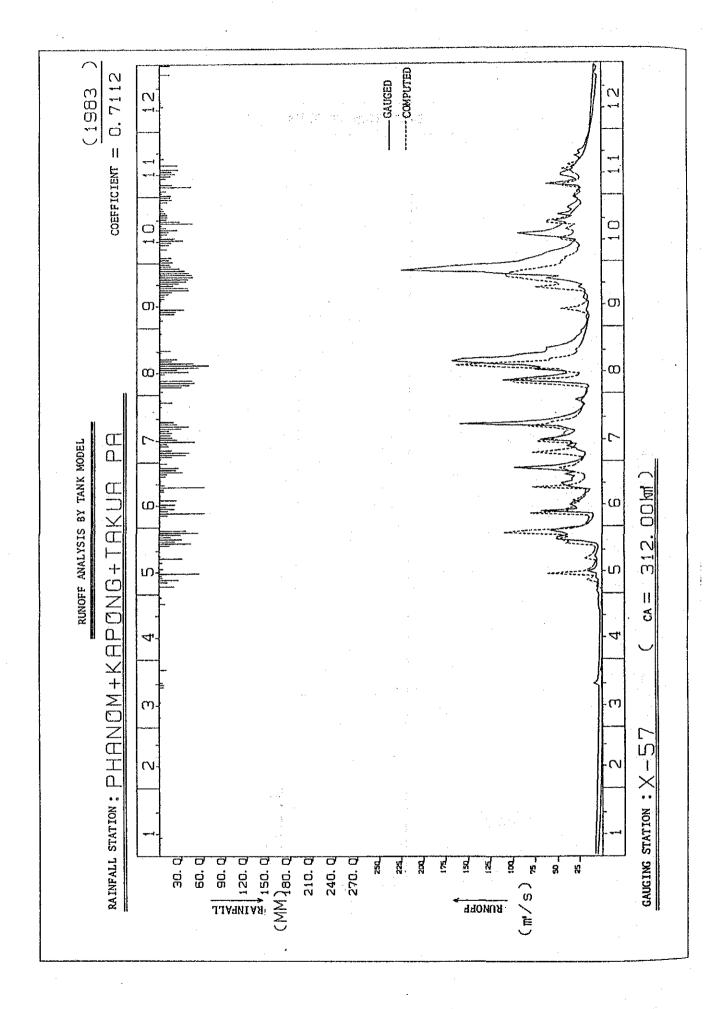
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		UPPER HOLE	MIDDLE HOLE	LOWER HULE	PENETRATION	EVAPORATION
	COEFFI	0.300000	0.100000	0.050000	0.450000	1.000
TANK 1	HE IGHT	40.0	20.0	0.0	0.450000	1.000
	COEFFI	0.200000	0.150000	0.100000	0.300000	0.0
tank 2	HE IGHT	40.0	20.0	0.0	0.300000	0.0
· _	COEFFI	0.020000	0.010000	0.005000	0.000100	0.0
TANK 3	HE IGHT	10.0	5.0	0.0	0.000100	0.0
	COEFFI	0.0	0.0	0.0	0.0	0.0
TANK 4	HE IGHT	0.0	0.0	0.0	0.0	0.0

[MONTHLY EVAPORATION]

(mm/D)

[1	2	3	4	5	6	. 7	8	9	10	11	12
	RATE .	3.70	3.90	4.50	3.70	3.10	2.90	3.10	3.80	3.30	3.10	3.10	3.20





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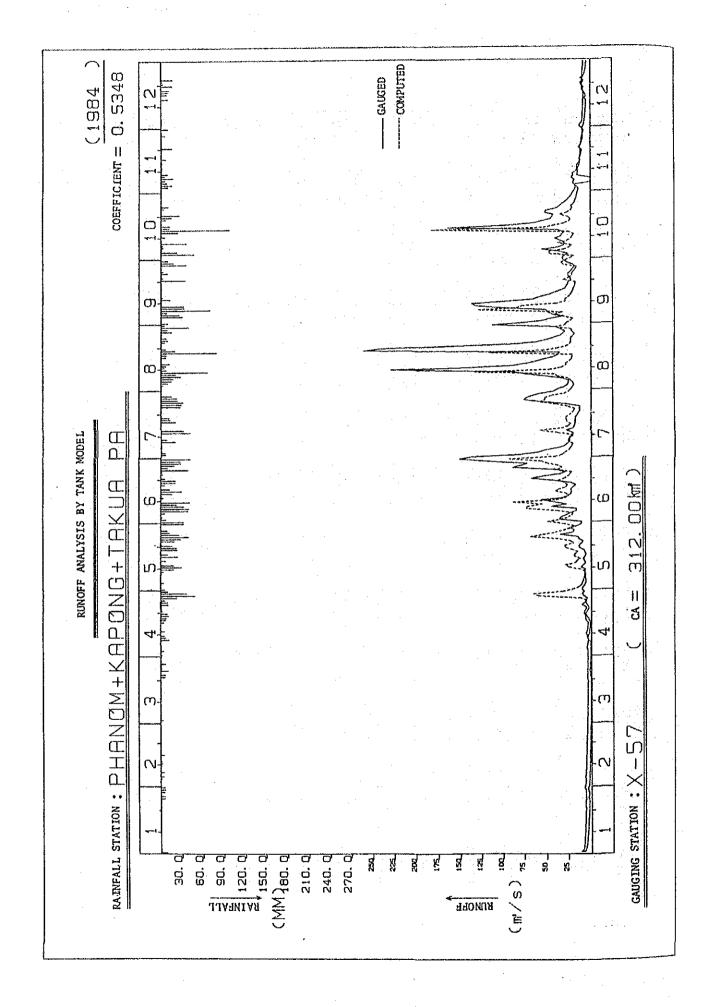
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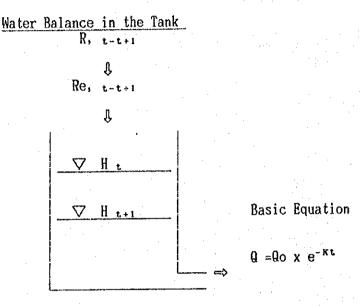
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	8 AUG	37.380 29.510 26.400 25.280 24.720 143.290)(28.658><	32.610 50.050 73.000 89.800 229.800 229.800 475.250)(142.360 71.800 71.800 53.200 53.250)(83.252><	41.670 36.720 35.090 241.200 241.200 241.200 241.200 241.200 241.200 163.050 81.600 50.050 601.5500 601.5500 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3102 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3100 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.3000 80.30000 80.30000000000	34.160 31.060 29.200 33.540 40.350 112.720** 281.030)(*	333.669 1 646. 75.280 261.600 24.720
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	6 JUN	10.800 35.730 33.540 28.350 28.350 28.350 28.254	16.250 16.750 44.800 27.800 42.350 29.5900(29.590>(56.000 31.990 25.000 22.200 21.920 21.920 31.422><	22.760 19.960 17.000 23.040 23.040 24.886 67.400 54.400 34.780 34.780 28.920 34.780 28.920 28.920 28.920 28.920 28.920 28.920 28.920	88.900 79.000 72.200 117.600 150.950 150.950 558.650 508.650 508.650 508.650 508.650 508.650 508.650 508.650 508.650 508.650 508.650 508.550 508.550 508.550 508.550 508.550 508.550 508.550 508.550 509.550 509.550 509.550 509.550 500.550 500.550 500.550 500.550 500.550 500.550 500.550 500.550 500 50	294.340 1 358. 43.145 150.950 10.800
	5 MAY	10-200 7-200 6-200 8-200 8-200 7-800 39-200	6.600 6.600 6.600 6.600 6.600 7.600 8.4000 8.6800 8.6800	11.000 8.600 6.400 7.600 8.800 8.400 8.4800 8.4800	6.800 6.000 7.200 7.200 7.200 413.250 8.2500 113.250 8.800 113.500 113.500 113.500 113.500 113.500	34.470 20.800 18.560 15.250 11.200 23.680 123.250	341.280 11.955 34.470 6.000
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Runoff Analysis by Tank Model (X-58) 2.3

- Tank Model (1)
 - (a) General

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The water balance in the tank is given by the following differential equation :

$$\begin{array}{rcl} H_{t+1}=&H_t+(\operatorname{Re},t_{t-t+1}-(\operatorname{Q}_t+\operatorname{Q}_{t+1})/2) \ dt\\\\ \text{where:} H_{t+1}&=&\operatorname{water stage in the tank at }t=t+1\\\\ H_t&=&-\operatorname{do-}& \text{at }t=t\\\\ \operatorname{Re},t_{t-t+1}&=&\operatorname{effective rain poured into tank during time }t\\\\ & & \operatorname{and }t+1\\\\ \operatorname{Q}_t&=&\operatorname{outflow from tank at }t=t\\\\ \operatorname{Q}_{t+1}&=&-\operatorname{do-}& \text{at }t=t+1 \end{array}$$

= time interval for calculation dt

Giving that dt = 1.0 day, $Q_{t} = KH_t$ and $Q_{t+1} = KH_{t+1}$, then ; H t+1 = H t + Re, t-t+1 - KH t+1/2 $H_{t+1} = (1-K/2)/(1+K/2)H_t + 1/(1+K/2)Re, t-t+1$

= all t + bRe, t-t+1

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RUNOFF ANALYSIS BY TANK MODEL (UAILY BASE);)

• LOCATION • ANALYSED POINT

: PHANG X - 58 PHANOM : 8.000 (km)

♦ CA

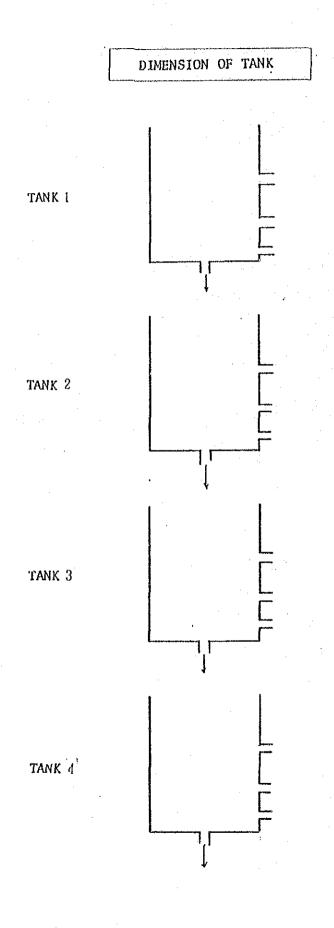
DIMENSION OF TANK

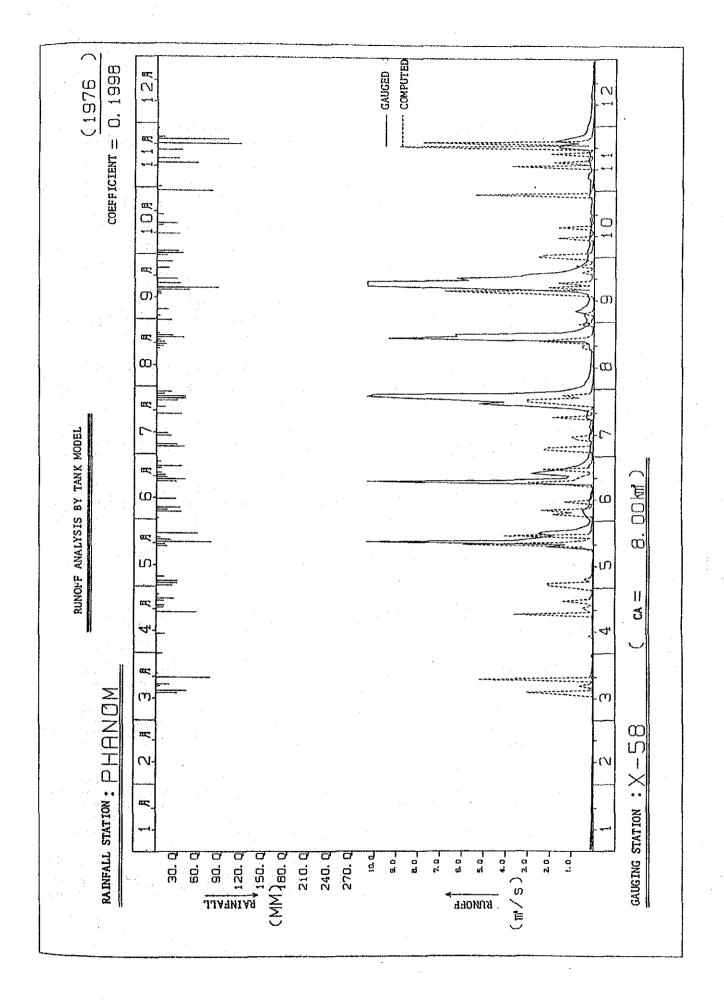
		UPPER HOLE	MIDDLE HOLE	LOWER HOLE	PENETRATION	EVAPORAT ION
	COEFFI	0.400000	0.300000	0.200000		
TANK I	HE IGHT	15.0	5.0	0.0	0.100000	1.000
	COEFF I	0.300000	0.250000	0.150000		
TANK 2	HE IGHT	30.0	10.0	0.0	0.050000	0.0
	COEFFI	0.020000	0.010000	0.005000		
tank 3	HE IGHT	10.0	5.0	0.0	0.000100	0.0
	COEFFI	0.0	0.0	0.0		
TANK 4	HE IGHT	0.0	0.0	0.0	0.0	0.0

[MONTHLY EVAPURATION]

(mm/D)

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RATE	3.70	3.90	4.50	3.70	3.10	2.90	3.10	3.80	3.30	3.10	3.10	3.20





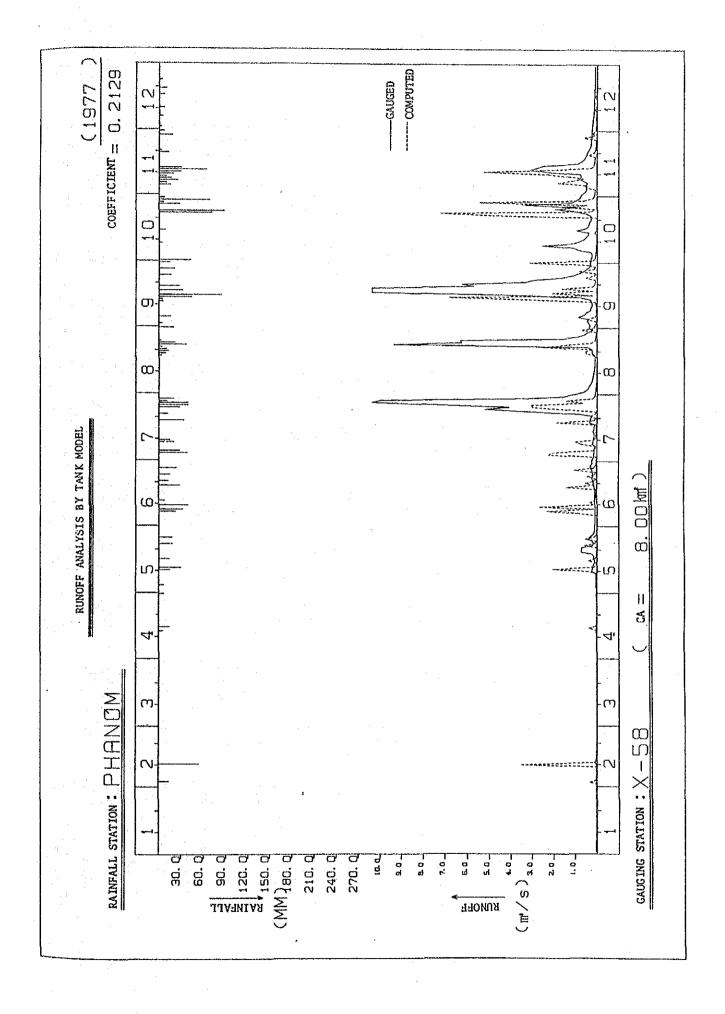
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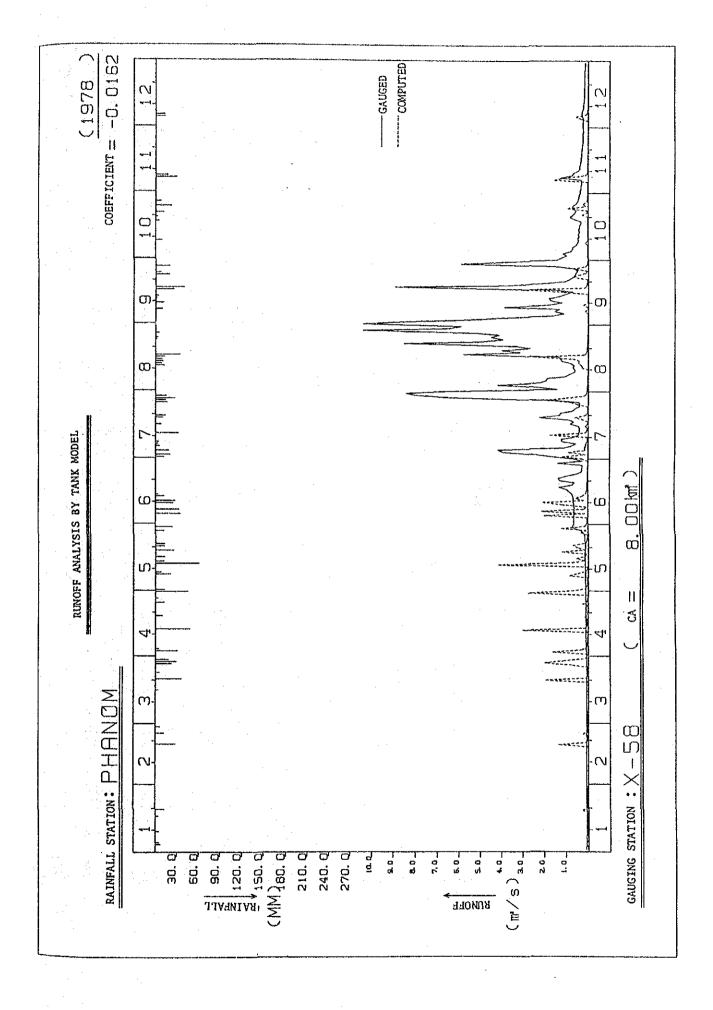
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(cuw/b)	I NON I I	0.760 0.670 0.490 0.490 0.490 0.490 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.590 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.540 0.5400 0.540000000000	0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.440 0.4400 0.4400 0.4400 0.4400 0.4400 0.4400 0.4400000000	0.770 2.080 3.120 2.820 2.820 11.290)(2.258><	0.900 0.710 0.590 0.540 0.5470 0.5470 0.5423 0.6423 0.6423	0.450 0.450 0.320 0.270 0.270 0.270 0.270 0.270 0.346V(0.260 0.250 0.240 0.240 0.230 1.220)(1.220)(0.244><	23.510 254- 0.784 3.120 0.230
CO TIND	I 0 OCT	0.330 0.330 0.280 0.230 0.230 1.380)< 1.380)< 0.2765<	0.320 0.870 1.100 2.580 1.100 1.100 1.194>	0.840 0.610 0.520 0.420 0.420 0.420 2.780)<	0.960 0.420 0.420 0.480 0.480 0.5260 0.5260 0.5260	0.880 0.840 0.840 0.840 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.5580 0.55800 0.55800 0.55800 0.55800 0.55800 0.55800 0.55800 0.55800 0.5580000000000	1-960 3.770 3.5500 1-180 0.960 9.1700***	24.430 264. 0.788 3.500 0.230
	9 SEP	0.390 0.330 0.390 0.460 0.460 1.970) 0.3942<	0.870 0.590 0.510 0.330 0.330 2.590)(2.590)(0.240 0.220 0.280 0.290 0.290 0.290 0.290 0.264><	3.650 20.000 18.000 5.760 5.410)<	6.260 3.410 2.930 1.670 1.140 3.082><	0.840 0.610 0.500 0.350 0.350 0.350 0.310 2.610)(.522><	89.810 970- 2.994 20.000 0.220
	8 AUG	1.060 0.470 0.470 0.420 0.420 2.950) 2.950) 2.950) 2.590><	0.280 0.230 0.230 0.220 0.180 0.180 1.110) 0.222><	0.160 0.140 0.130 0.120 0.120 0.120 0.134>	0.1110 0.1100 0.1100 0.1200 0.2200 0.1265	0.220 0.220 0.460 9.460 9.460 8.280 3.316V 5.316V	6.340 1.550 0.880 0.570 0.410 0.190*** 1.6570	31.880 344. 1.028 9.400 0.100
	7nr 2	0.230 0.210 0.170 0.150 0.150 0.150 0.150 0.180><	0.130 0.130 0.080 0.220 0.180 0.180 0.148>	0.180 0.150 0.150 0.150 0.150 0.150 0.150 0.158 0.1585 0.1585	0.1140 0.1140 0.1140 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.1460 0.14600000000000000000000000000000000000	0.300 0.870 0.870 2.980 2.980 2.980 2.980	4.140 7.140 12.000 3.350 38.250) (6.375><	51.390 555. 1.658 12.000 0.080
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A-2-2-40

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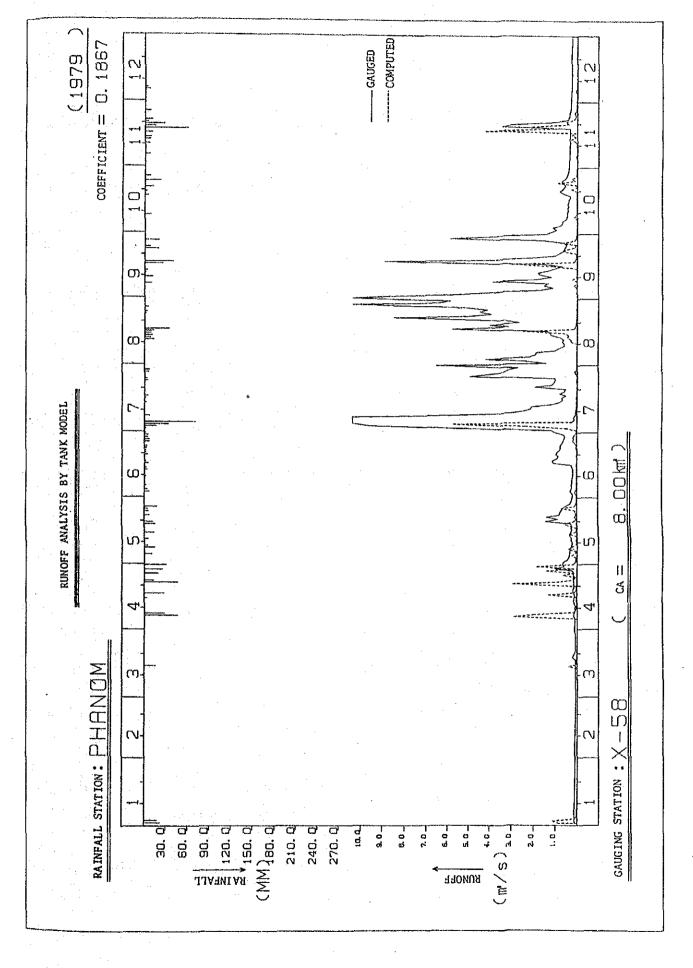
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C S S D		13.630 8.540 5.570 3.890 1.770 8.6905 6.6905	7.120 1.520 1.520 3.860 7.700) 4.700) 4.9200	1.41840 7.0900 7.0900 7.0900	0.710 1.500 8.920 3.800 1.960 3.378><	1.040 0.840 0.680 0.590 0.590 3.710)<	0.490 0.640 1.550 5.910 5.860 3.860 12.450)(2.490>A	83.290 900- 2.776 13.6530 0.490
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A MA	IYM C	0.120 0.120 0.120 0.120 0.120 0.120 0.130 0.130 0.130 0.130 0.122 0.122 0.1220	0.1130	0.150 0.150 0.150 0.150 0.150 0.150 0.150	0.160 0.160 0.150 0.150 0.150 0.150	0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140 0.140	0.170 0.180 0.240 0.330 0.530 0.530 0.7503*	5,780 62. 0,186 0,120
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ເດີ	NAR 5	0.070 0.070 0.070 0.070 0.070 0.070	0.070 0.070 0.080 0.080 0.080 0.080 0.080	0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080 0.080	0.080 0.080 0.080 0.070 0.380 0.376 0.376 0.076	0.070 0.070 0.070 0.070 0.070 0.350)<	0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070 0.070	2.280 25. 0.074 0.070
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71.325 0.010 0.009 0.008 0.008 0.008 0.008 0.007 0.006 0.006 0.006 0.006 0.005 0.005 0.005 0.005 0.005 0.005 0.005 230 0.071) 0.007 0.007 0.006 0.321 0.327) 0.347) 0.736) 0 018 0 016 0 0.551 0.109 0.030 14. 0.041 0.551 0.551 01 ----0.043 0.036 0.030 0.026 0.022 0.022 0.022 0.019 0.017 0.015 0.015 0.013 0.015 0.015 0.015 0.015 0.015 0.015 0.015 0.015 0.015 0.015 0.015 0.015 0.019 0.011 0.010 0.010 0.009 0.048) 0.010>< 0.037)(1.100 0.144 0.051 2.830)(0.566>< NON 0.008 0.008 0.007 0.007 8.4 7 3.228 35. 0.108 1.524 0.007 1.524 ---6.123 0.227 0.060 0.032 0.023 *** 0.493) C 0.014 0.138 0.031 0.216)(0.043>< 0.017 0.015 0.014 0.014 0.013 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.012 0.017 0.011 0.010 0.034 0.011 0.010 0.076 0.015 0.015 0 0.009 0.266 0.052 0.052 0.052 0.052 0.252 0.251 2.2 0.034 0.029 0.025 0.025 0.025 0.025 0.131)(0.131)(0.026>< ក្ត 0.017 0.072 0 0.014 0.013 0.013 0.011 0.011 0.011 0.012 0.012 0.043 0.020 0.021 0.016 0.113)< 0.237< 1.143 3.012 0.163 0.070 0.058 4.446)(0.889>< 0.973)(0.010 0.149 0.027 0.015 0.015 0.221 0.221 0.221 0.048 0.646 0.646 0.440 0.440 0.105 0.105 0.256>< 0,047 0,100 0,130 1,048 1,048 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 1,148 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4.158 0.735 0.735 0.735 0.735 1.158 1.031>< 0.531 0.148 0.071 1.207 0.162 2.1203 0.42424 0.424 4.158 0.029 3.145 MAY ທ 0.052 0.043 0.036 0.036 0.036 0.035 0.1875 0.1875 0.022 3.019 3.014 0.139 0.056 3.250)(3.250)(0.047 0.039 0.159 0.159 0.333 0.159 0.333 0.06276 0.025 0.025 0.019 0.016 0.249 0.0567 0.0567 2.872)(0.064 0.052 1.607 0.291 0.291 2.094)(2.094)(2.094)(2.094)(0.301 3.014 0.016 APK 1.938 0.035 0.035 0.040 2.136) 2.136) 2.136) 0.427>< 0.008 0.007 0.007 0.006 0.034) ശ 0.005 0.005 0.005 0.005 0.005 0.005 0.005 0.015 0.013 0.011 0.010 0.010 0.058) 0.012>< Z∞ 0.204 MAR ŝ ≺ഗ 江 I ന 0.005 0.005 0.005 0.005 0.103 1.466) 0.103 0.030 0.025 0.021 0.237 0.237 0.237 0.236 0.236 0.036 0.036 0.070>6 0.102)(0.006 0.006 0.006 0.005 0.005 0.005 0.005 0.007 0.007 0.006 0.006 0.032) (0.0067< 0.005 0.005 0.005 0.005 0.005 0.005 0.005 0.010 0.021 0.009 0.064 0.008********* 0.008********** 0.007********* 0.021 0.064 0.018 ***** 0.072 2.003 DAILY RUNOFF (1978 0.008 0.008 0.008 0.008 0.264 0.0296)(0.011 0.010 0.010 0.009 0.009 0.019)(0.010>< 0.039 0.016 0.0114 0.012 0.012 0.012 0.010 0.010 0.009 0.081 0.088 0.198) 0.040>< 0.016 0.014 0.013 0.013 0.013 0.013 0.023 0.023 0.026 0.016 0.016 0.016 0.016 0.016 0.016 0.016 0.016 0.016 0.765 1 JAN GRAND TOTAL AVERAGE MAX MEN AVERAGE AVERAGE AVERAGE AVERAGE AVERAGE NNNNNN NNNNNN 500010

A-2-2-44

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	ר אור 1	0.025 0.022 0.487 2.408 5.733 5.733 5.733	220 1113 079 079 079	0100282 0100282 0100282	0.024 0.024 0.027 0.017 0.017 0.013 0.013	N00 00	0.063 0.129 0.028 0.016 0.260) 0.043><	9.993 1085 1282 5.733 0.011
·	NUL 3	0.024 0.021 0.082 0.083 0.231)< 0.231)<	111111	0.011 0.010 0.023 0.012 0.012 0.012 0.013 0.013 0.013 0.013 0.013 0.013 0.013 0.013 0.013 0.013 0.013 0.011	00000000000000000000000000000000000000	0000	0.150 0.171 0.093 0.6697 0.1347 0.1347	1,235 135 0,041 0,175 0,009
	5 MAY	0.172 0.074 0.061 0.250 0.597) 0.597) 0.597)	1000 1000 1000 1000 1000 1000 1000 100	0.035 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.028 0.020000000000	00000000000000000000000000000000000000	78 777	0.128 0.039 0.033 0.028*** 1.024)(0.171><	4 441 484 0 143 0 716 0 0233
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APPENDIX A-3-1

Study on Flow and Pressure Measurement in Distribution System

APPENDIX STUDY ON FLOW AND PRESSURE MEASUREMENTS IN DISTRIBUTION SYSTEM

(1) Introduction

To evaluate the characteristics of the distribution system, pressure and flow measurements were made from August 29 and 31, 1988.

(2) Method and results

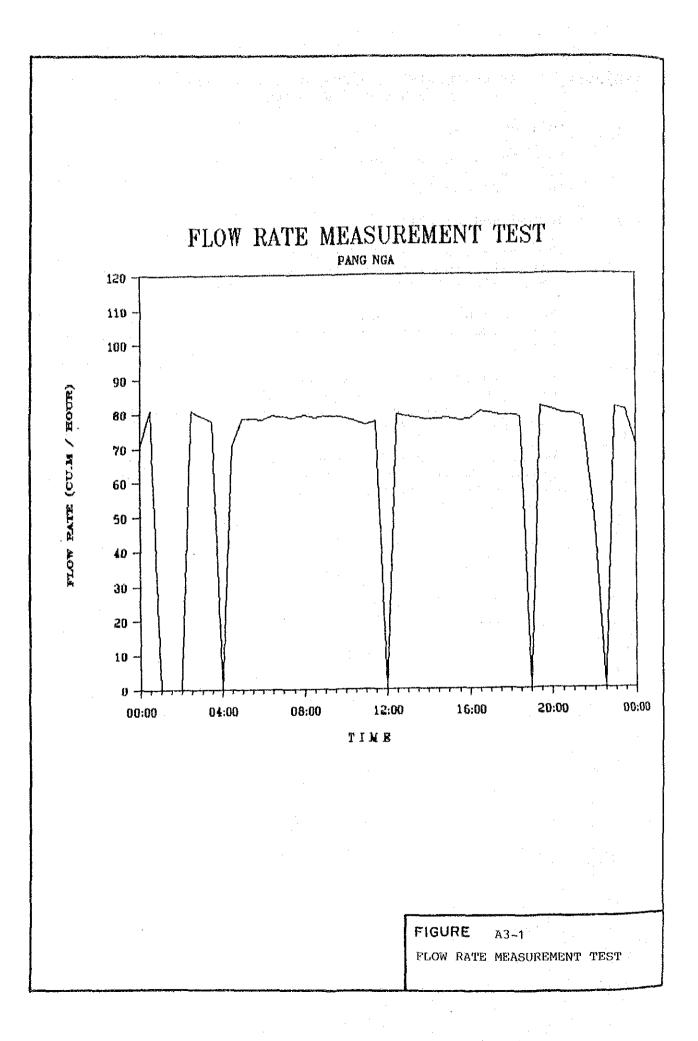
The flow measurements of 24-hours were conducted at the main distribution pipe in the treatment plant using the ultrasonic flow meter with pen recorder.

The pressure measurements were made by installing pressure gage at 6 house connections in the distribution system.

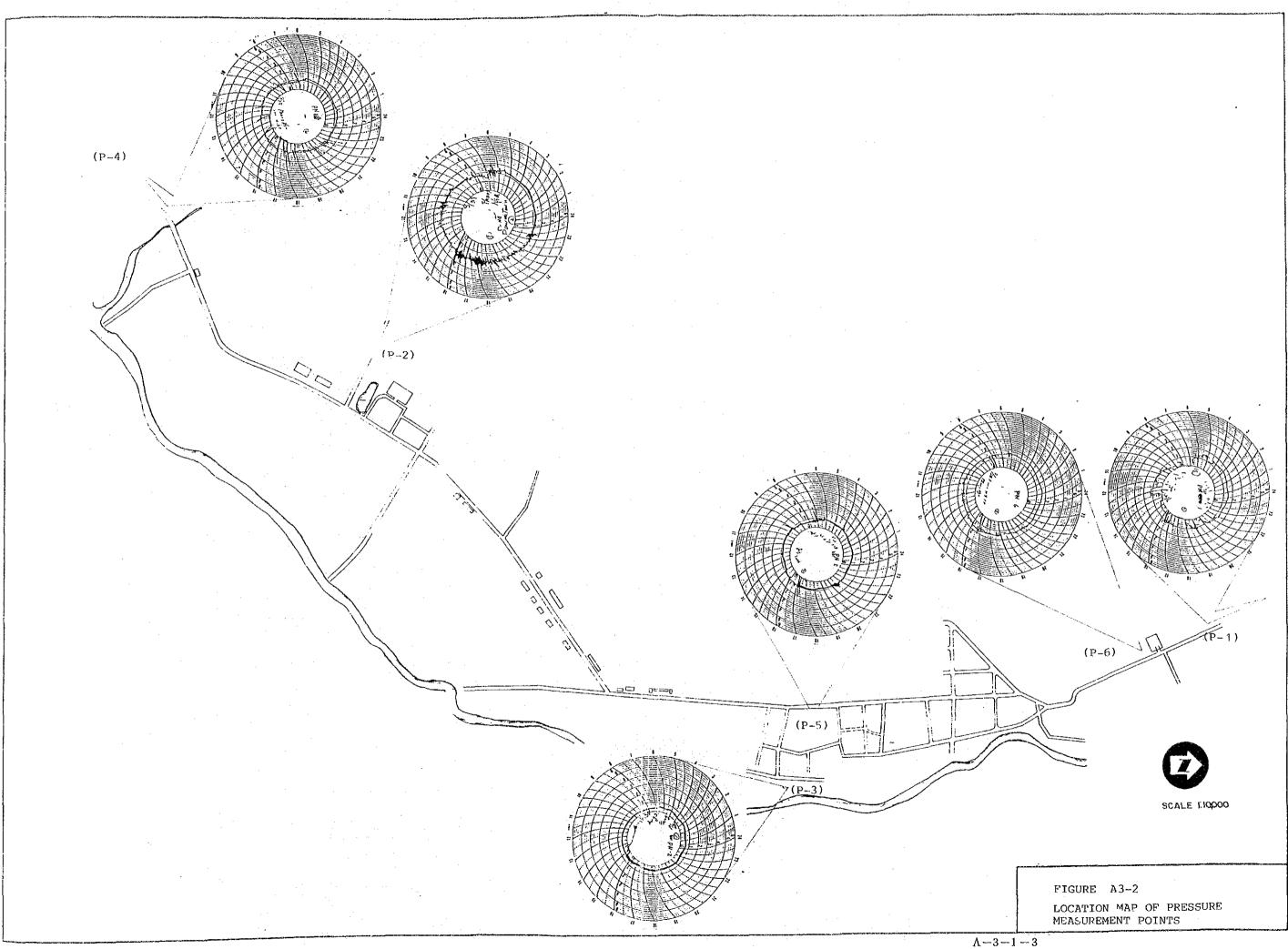
The results of flow measurement at the Phang Nga waterworks, Location of pressure measurement points and the results of pressure measurement are shown in Figure A3-1, A3-2 and A3-3 to A3-9, respectively.

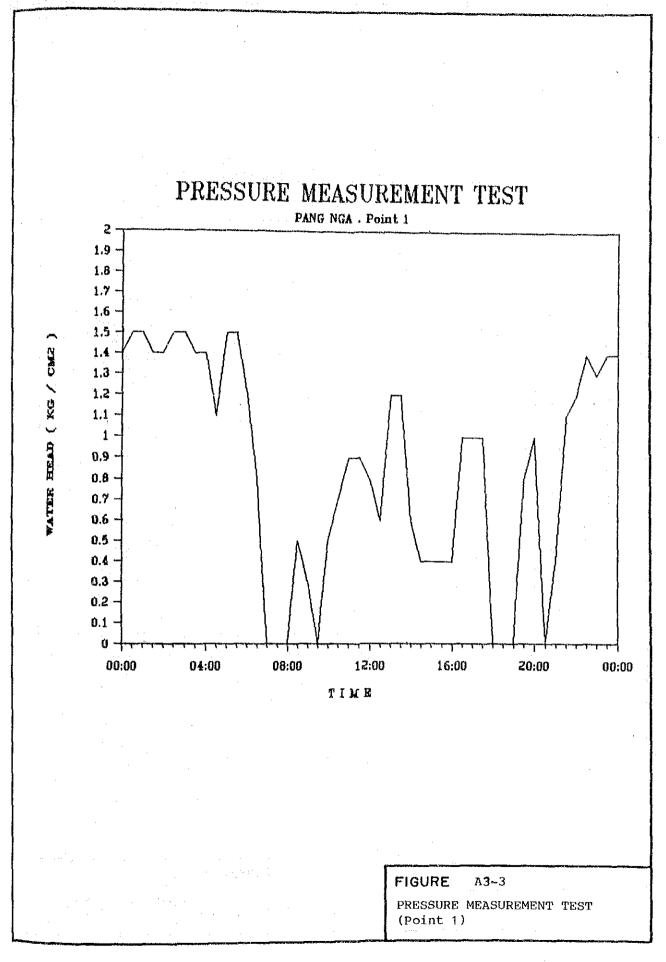
The results of pressure measurements in the distribution system show simular conditions with distribution network analysis (refer to section 3.1.3).

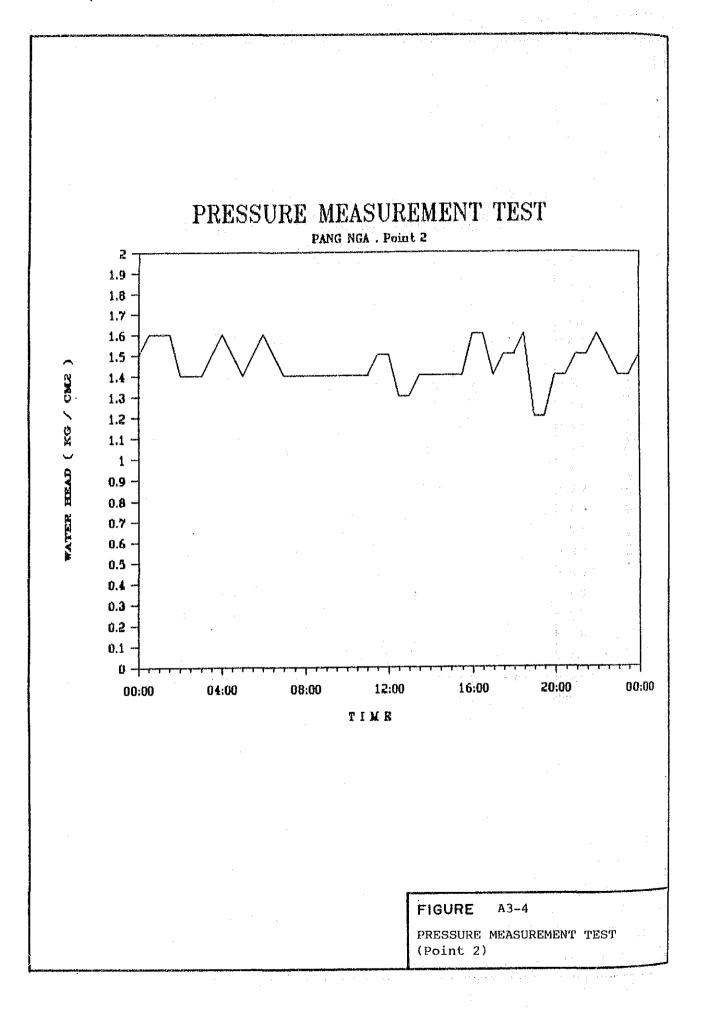
A-3-1-1

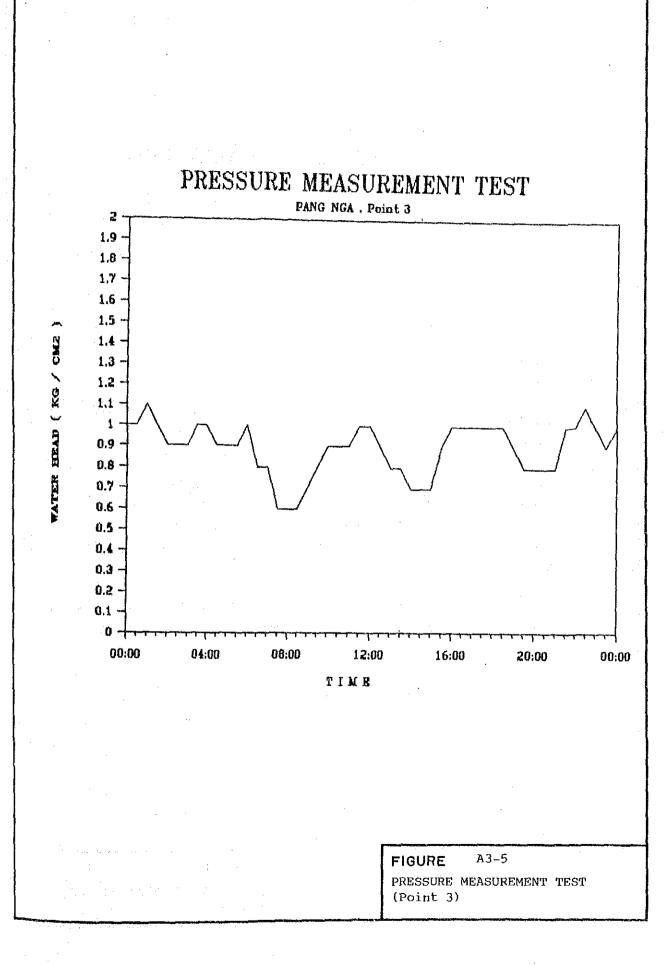


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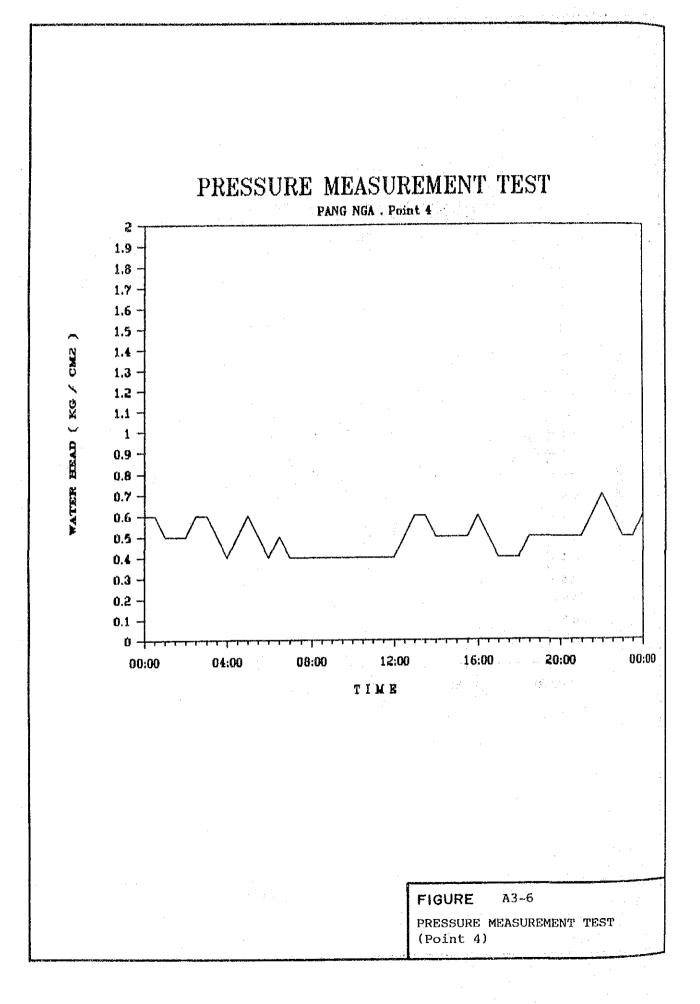




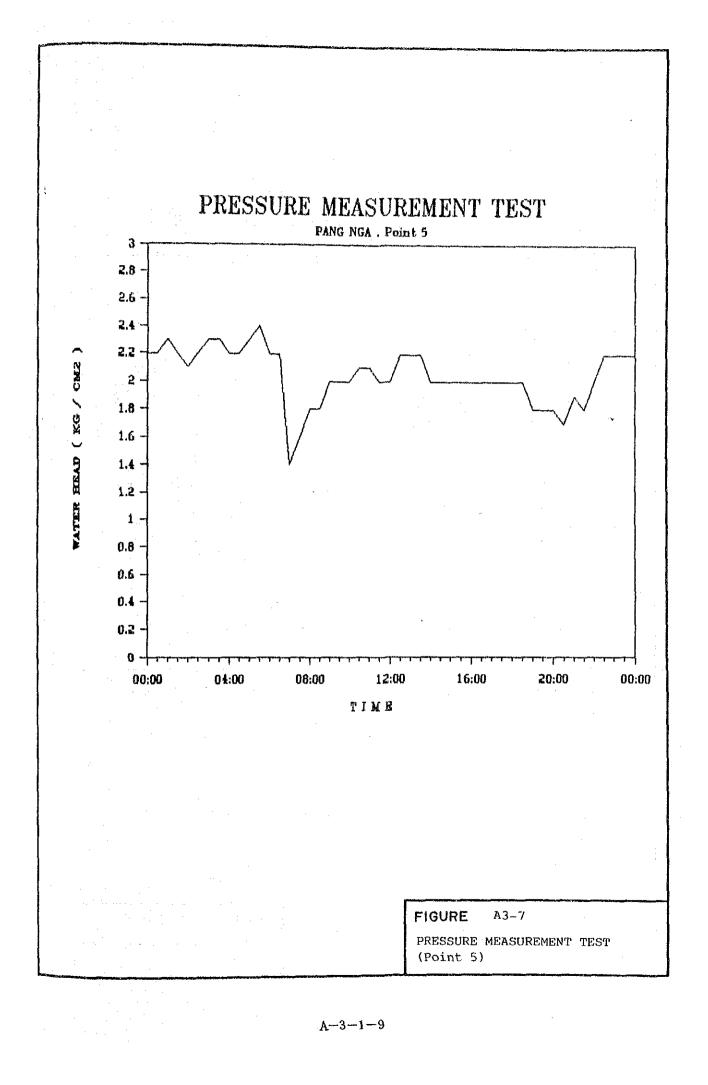


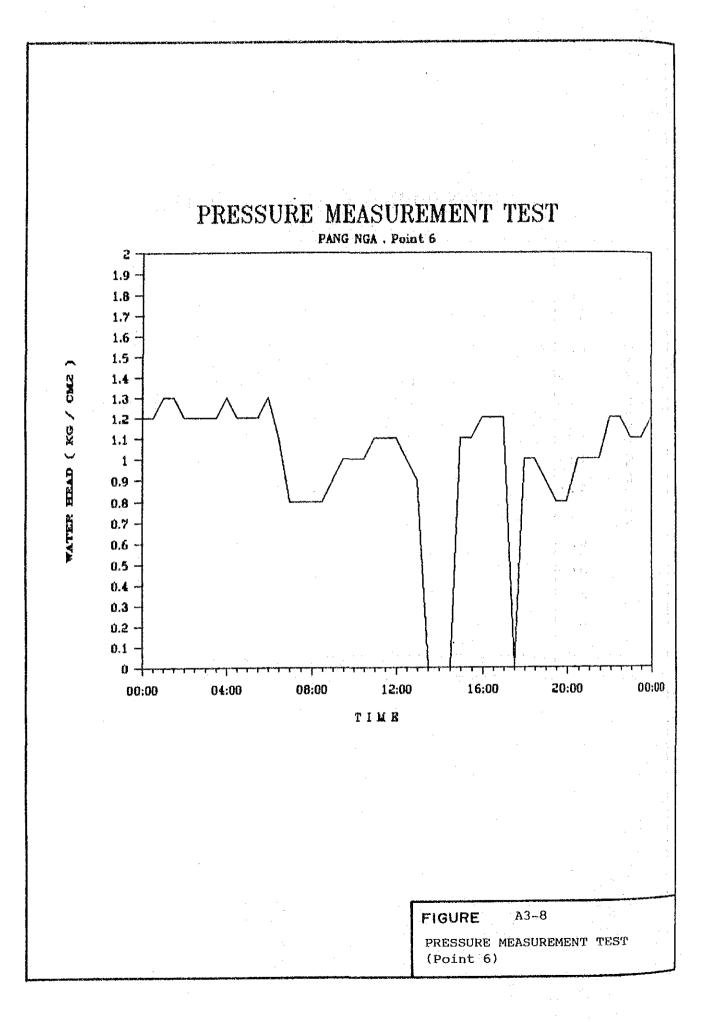


A---3----



A - 3 - 1 - 8





APPENDIX A-3-2

Study on Water Quality on Distribution Network

APPENDIX WATER SUPPLY ON DISTRIBUTION NETWORK

1. General

Water quality analysis was conducted along the existing distribution mains with a portable water quality analyzer. Parameters of the analysis are pH, conductivity and temperature.

The results of the analysis are shown in Table A4-1, and sampling points are presented in Figure A4-1.

2. Causes of high pH

Results show that pH values are 9.0 at No.1, 8.7 at No.4 and 8.6 at No.7, respectively.

Others are relatively low within the water quality standard.

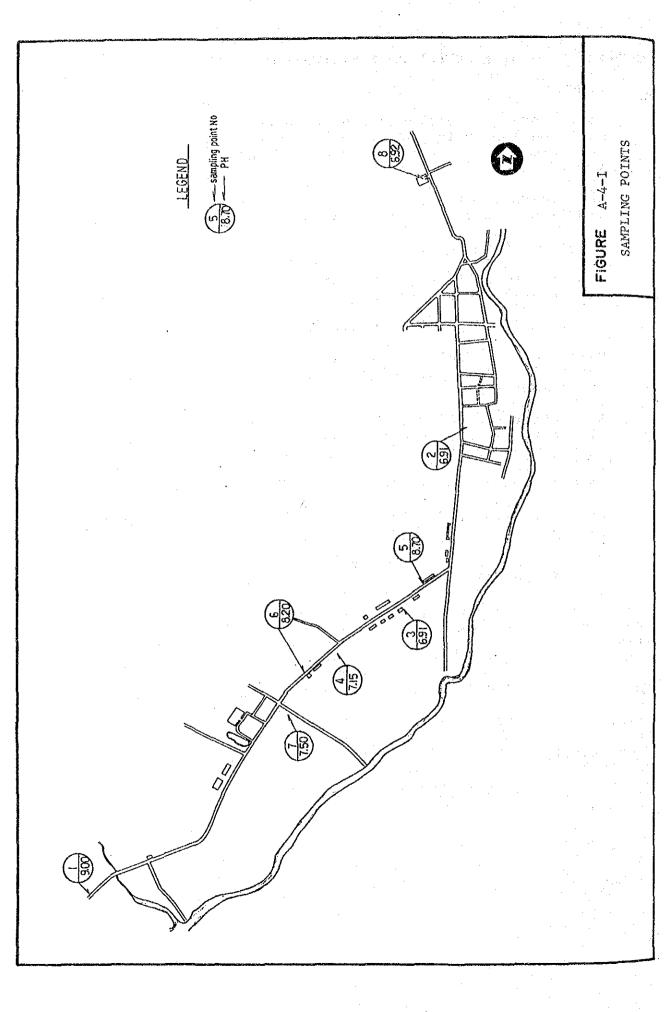
Based on the field investigation, the following causes may result in calcium dissolution from the inner wall of asbestos cement pipe.

- 1) Pipes are newly installed about two year age;
- 2) Water is retained for long time inside the pipe due to low water flow.

3. Counter measure

Drain-off from hydrants or blow-off pipes should be periodically carried out in these area.

Table A4-1 Results of Water Quality Analysis Sampling $1 \quad 2 \quad 3 \quad 4 \quad 5 \quad 6$ 7 Point 8 ----9.00 6.91 6.91 7.15 8.70 8.20 7.50 6.92 ЪН 27.4 27.8 27.4 27.0 28.8 28.3 28.3 Temp. 27.8 (C) Conductivity 6.4 4.6 4.8 4.8 5.9 5.8 5.0 4.5 (x5x10)



APPENDIX A-3-3

Jar Test on Raw Water of the Water Treatment Plant

문화

APPENDIX JAR TEST

1 General

Jar fast was conducted to evaluate the present dosage rate of coagulant and to verify the appropriate dosage rate. The test was conducted on september 1988 for the raw water presently used by the waterworks.

2 Coagulant Used

Aluminum Sulfate is being used as coagulant at Phang Nga Waterworks as well as the other waterworks. The chemical is a solid type in a package of 25 kg bad, which is dissolved in the coagulant solution tank with an effective volume of about 0.6 cu.m.

According to the operator, they are consumind 12.5 kg of alumnium suitate a day. From this amount of consumption. dosage rate is calculated as below:

bosade rate (R) for daily average flow rate:

R = 12.000 g/1,560 cu.m/day =8.0 mg/1

Concentration of the coagulant in the solution tank is calculated from the amount of chemical dissolved and the volume of the tank as follows:

Concentration of coagulant solution (C)

C = 12,500 g/0.6 cu m = 20,800 mg/1

This solution was diluted 10 times for use of Jar Fest; therefore, solution had the concentration of:

 $20.800 \times (1/10) = 2.080 \text{ mg/l}$

Test Procedure

3

Test procedure followed the PWA's regulation for Jar Test. Sequence and time are shown as follows:

a) Coagulant dosed

b) Rapid Mixing, 60 rpm - 7.5 min

c) Flocculation, 40 rpm - 7.5 min

d) Flocculation, 25 rpm - 5.0 min

e) Sedimentation, about 5 min

4 Condition and Results

Jar Test was conducted with a series of six different dosage rates. The condition and results are as shown in Table A5-1.

		1	2	3	4	5	6
	ant again garantee war war fa fan gagantee war sen wet wet wet wet an an gagantee of the same wet gave					**************************************	
	Coagulant Solution (ml)	1.25	2.5	5.0	7.5	10.0	12.5
2	Dosage Rate (mg/l)	2.5	5.0	10	15	20	25
,	Turbidity after settling	1.0	1.0	1.0	1.5	1.5	2.0
•	На	7.05	7.01	6.59	6.50	6.65	6.55
*	Conductivity (micro ohm/cm)	4.2	4.5	4.6	4.6	4.5	4.4
• •	Characteristics of floc		large floc	largest floc		fine floc	fine floc

Table A5-1 Jar Test Condition and Result

Dosage rate of 5 and 10 mg/l showed the better result than others in terms of floc formation, and turbidity removal. Actual dosage rate (8.0 mg/l) falls in this range.

APPENDIX A-4-1

Study on Water Consumption

APPENDIX

STUDY ON WATER CONSUMPTION

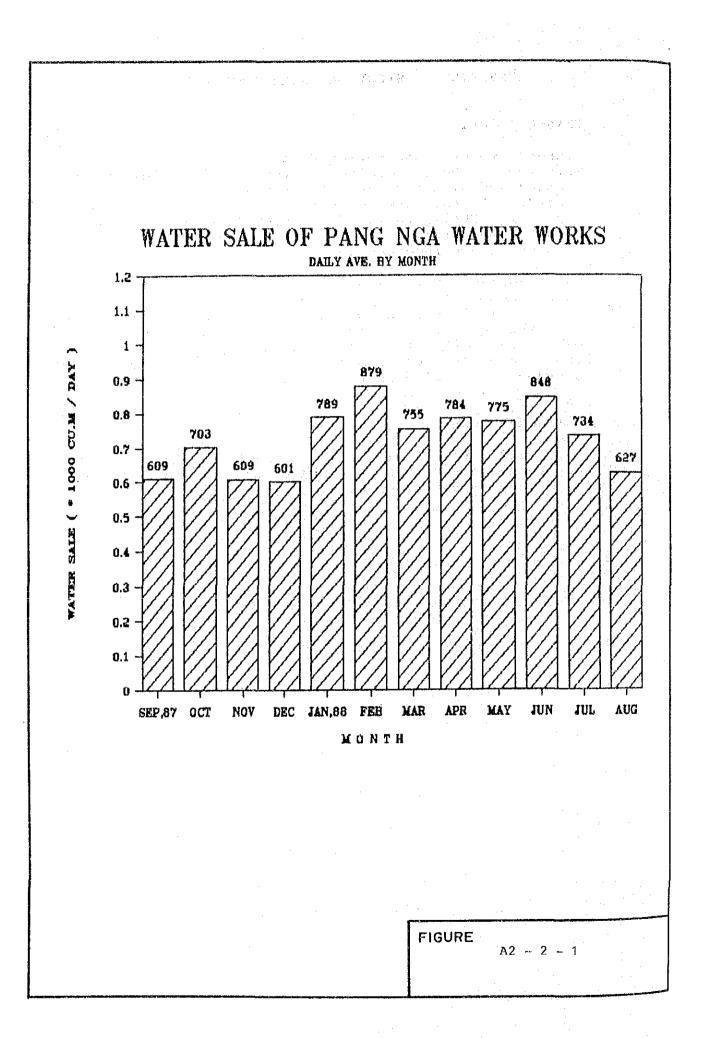
1 Data Collection

Present water consumption data was collected from the waterworks' meter reading records for the study of water demand and distribution network analysis. Meter reading records at the waterworks office consist of volumes of cards in PWA's format for each connection. Monthly consumptions from September 1987 to August 1988 of each connection are recorded on this card.

Data collection was made in a manner of copying figures water consumption of each consumer for every month. For distribution network analysis, each consumer was located on the map by interviewing meter readers of the waterworks. When the exact locations were not identified, they were located in some extent of the pipeline. Big consumers were also identified for further analysis.

2 Collected Data

Raw data copied from meter reading books was then summed up by month and by area. The attached sheets hereafter show the summary of water consumption.



A-4-1-2

Table A2 - 2 - 1

Water Sale of Pang Mga Waterworks

Book No. 1 2 3-1 3-2 4 5	SEP,87 8,664 3,127 1,039 1,691 2,566 1,190	OCT 8,221 4,525 1,475 2,336 3,607 1,616	NOV 7,371 3,403 1,281 1,971 3,104 1,129	DEC 7,309 3,456 1,124 1,928 3,357 1,462	JAN, 88 11, 954 3, 975 1, 444 2, 497 3, 001 1, 584	PEB 10,559 4,664 1,592 2,781 3,938 1,957	MAB 9,360 5,176 1,688 2,130 3,506 1,536	APR 10,257 4,998 2,010 1,856 2,519 1,880	NAY 8,021 4,465 1,593 3,666 4,636 1,639	JUN 11,162 3,935 1,333 3,092 4,619 1,307	JUL 7,384 4,771 1,657 3,268 3,907 1,770	: : AUG : Fotal : Day Ave. 7,034 : 107,296 : 293.96 3,645 : 50,141 : 137.37 1,499 : 17,735 : 48.59 2,509 : 29,725 : 81.44 3,135 : 41,915 : 114.84 1,609 : 18,679 : \$1.18
Total by Nonth (cu.u/mo)	18,277	21,781	18,259	18,636	24,455	25,491	23,395	23,520	24,010	25,448	22,757	19,431 : 265,491 : 727.37
Total by Day (cu.m/ day)	609	703	609	601	789	879	755	784	775	848	734	627

.

APPENDIX A-4-2

Questionnaire Survey for Residents

APPENDIX QUESTIONNAIRE SURVEY IN PHANG NGA (RESIDENTIAL)

Objective A1.1

The door-to-door questionnaire survey was conducted to obtain the basic information on the resident's living conditions, water 1195 patterns, responses to the municipal system and/or their อเงก water sources and willingness for house-connection supply, and covered the area served or unserved by the municipal water supply system.

A1.2 Survey Area

The survey area was divided into 5 blocks traversing the national road Route 4 as shown in Figure A1-2-1, All blocks were at present, partially served by the municipal system.

A1.3 Survey Item

1.

The form used for the questionnaire survey was originally written by Thai and included the following items.

- General
 - 1.1 Address
 - 1.2 Type of House
 - No. of Persons in Family 1.3
 - No. of Employees 1.4
 - 1.5 Average Monthly Income
 - 1.6 Average Monthly Medical Expense
- Type of Water Supply 2.

Conditions in case of Municipal System 3.

- 3.1 Pressure
- 3.2 Quantity

Other Sources than Municipal System 4.

> 4.1 Type of Source 4.2 Conditions in case of Groundwater

5. Potability

7.

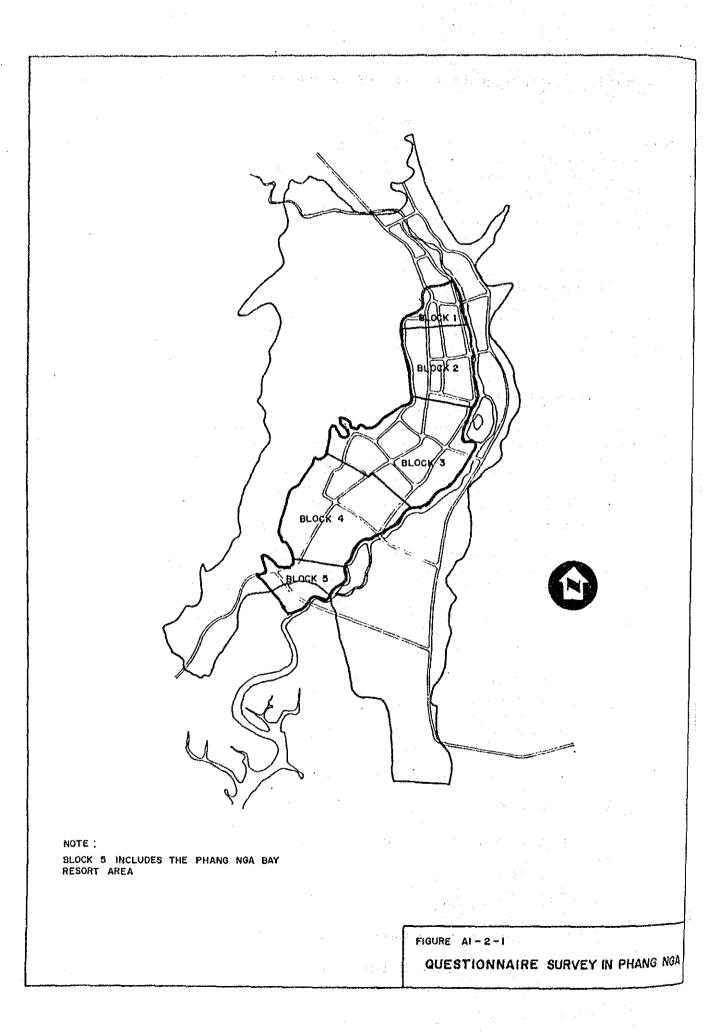
Water Quality in case of Municipal System 6.

- Color
- 6.1 Smell
- 6.2
- Turbidity 6.3

Average Monthly Water Consumption

Average Monthly Water Charge 8.

A - 4 - 2 - 1



9. Willingness to Pay for Water Charge

10. Water Quality in case of Other Source

10.1 Color 10.2 Smell 10.3 Turbidity

11. Willingness to Connect to the Municipal System

12. Willingness to Pay for Connection Fee

13. Willingness to Pay for Water Charge

4 Survey Method

College students were employed as interviewers and were engaged in the questionnaire survey with the guidance of the PWA Head Office staff. The survey was conducted to 122 residents on August 30, 1988.

5 Survey Results

The results of the questionnaire survey are summarized in Table A1-5-1.

1) General

64.8% of the respondents lived in residential houses while 33.6% in commercial buildings and the remaining 1.6% was unknown due to the omission of confirmation by the interviewers.

The total numbers of persons in families and employees were 556 and 274, respectively. Accordingly, one household is composed of 4.56 family members and 2.25 employees on an average or a total of 6.81 persons.

Regarding the average monthly income, 70.5% respondents were in the up-to-4,500 Baht bracket, or 19.7% in the up-to-2,000 Baht, 32.0% in the 2,001-3,000 Baht and 18.8% in the 3,001-4,500 Baht brackets, respectively. The average in respondents weighted by the number of persons and the median in each income bracket was approximately 3,840 Baht, but the number of persons was biggest in the 2,001-3,000 Baht bracket.

As to the average monthly medical expense, 27.9% was in the up-to-50 Baht bracket and 19.7%, 15.6% and 27.9% were in the 51-100, 101-200 and 201-500 Baht brackets, respectively. The average in respondents calculated by the same method as the above is 300 Baht, but the number of persons was biggest in the up-to-50 and 201-500 Baht brackets.

A-4-2-3

2) Type of Water Supply

26.2% of the respondents used the municipal system only, 50.0% the other source than the municipal system and 23.8% the combined system of the municipal system and other source(s).

91.1% or 82 out of 90 other sources was groundwater as shown below.

ا میں است ایس						
Block No.	1	2	3	4	5	Total
	*******	*****			*****	
				an der		
Municipal System Only	7	8	6	8	3	32
plus Rain/River		1	. 1	1	· · ·	· · 3
plus Water Vender		 .	1		. 1	2
plus Well	8	3	4	6	2	23
plus Others	1*	- ·	· -	.	- We r	1
Well Only	7	18	13	11	2	51
plus Rain/River	3	~~	****		2	5
plus Pond/Reservoir	2	-			-	2
				· · · 1		
Paid Source Only					3	3
Total	28	30	25	26	13	122
计算机计算机 化化化合金 化化合金 化化合金 化化合金 化化合金 化化合金 化化合金 化化	*******			=====		*******

* Rain/River and Well

3) Response to Municipal System

The reputation of the PWA waterworks among 61 respondents using the municipal system was not so good, that is to say, 45.9% complained of low pressure, 24.6% of insufficient water, 59.0% of color, 55.7% of smell and 68.9% turbidity. However, there were big gaps in response by the block. Though the low pressure took place in all blocks, the respondents in Block 2 have sufficient water. The complaint of color was conspicious in Blocks 1, 3 and 5, while no complaint in Block 2. Smell was mostly detected in Blocks 1 and 3. Turbidity was found in other area than Block 2.

4) Potability

This question was originally intended to know the potability of tap water, but the answer seemed to be made not only for the tap water but also for other source water, since the question followed that on other sources.

Accordingly, the evaluation was made extracting the data from respondents using tap water or well water only.

	the range plane have not made that the start start was been been been been been that the start start start star	
Drinking	12 (37.5%)	32 (62.7%)
Not Drinking	14 (43.8%)	6 (11.8%)
Both	6 (18.7%)	12 (23.5%)
Unknown	- (-)	1 (2.0%)

Total 32 (100%) 51 (100%)

37.5% used tap water for drinking and 18.7% for drinking and not-drinking in spite of their complaints of its water quality, while 62.7% used well water for drinking and 23.5% for drinking and not-drinking.

The doubt as to the kind of water the respondents (who answered that they didn't use only one source for drinking) used for drinking is remained. They may use the water vendor, although this is not expressed clearly in the survey.

5) Water Quality of Other Sources

As mentioned above, the main water source was the groundwater. 34.4% complained of color, 31.1% of smell and 40.0% of turbidity. Scrutinizing the data block by block, such complaints mostly took place in Blocks 3 and 5, while Block 4 was blessed with water quality. Compared with those in tap water, the complaint of water quaily was less in well water.

6) Conditions of Wells

The well depth distribution is shown below. Between 2 and 40 m and 82.9% wells had depths of not more than 10 m. The deep wells with depths of more than 30 m were located along the Petchkaseam Road and Soi Dokya in Blocks 2 and 3.

A-4-2-5

i.

Block No.	 5m	>5m <10m	>10m <15m		>20m <30m	>30m	Un- known	Total
			· ·					
1	3	15	***	2	1		***	21
2	7	6	2	1	1	3	1	21
3	9	6	***	**	1	1 1 -	5 - **	17
4	13	4			ر میں د	ند. ا	.**	17
6	1	4	1	5				6
Total	33	35	3	3	3	4	1	82
Well Dep.	4.0	7.8	14.3	20.0	28.3	38.8		
(m)	(33)	(35)	(3)	(3)	(3)	(4)		
Water Dep.	1.9	3.8	2.7	11.7	18.3	15.0		
(m)	(33)	(35)	(3)	(3)				
Operation	1.7	2.8	0.5	10.3	1.0	18.1		
Time (h/d)			(1)	(3)	(2)	(4)		
No.of								
Fetching	5.4	8.7	6.0	1 <u></u>	7.0	4.0		
Times	(10)	(10)			(1)	(1)	· · ·	
(1/d)	(12)	(18)	(2)		(1)	(1)	·· •	
							, en en en en	

The figures in parentheses show the number of wells used for the average calculation.

7) Average Monthly Water Consumption, Water Charge and Willingness-to-Pay

Regarding the average monthly water consumption, 54.1% belonged to the up-to-15 cu m bracket and 24.6% to the 16-30 cu m bracket.

42.6% paid for the water charge in the up-to-50 Baht bracket and 27.9% in the 51-100 Baht bracket, while, according to the result on the willingness-to-pay for water charge, 57.4% wanted that the water charge would be in the up-to-50 Baht bracket and 32.8 % in the 51-100 Baht bracket. The expectant amount was less than the actual payment.

8) Willingness-to-Connect

Out of 122 respondents, 61 didn't use the municipal system at present. However, 42.6% was willing to connect to the municipal system. Such people mainly lived in Blocks 1, 2 and 5. They wanted that the connection fee would be less than 2,500 Baht (100%) and the water charge less than 100 Baht (84.6%). The response to the water charge of the possible consumers was almost equal to that of the existing consumers.

A-4-2-6

Table A1-5-1 SUNNARY OF QUESTIONNAIRE SURVEY IN PHANG NGA

Block	KO.	111	2 }	3 }	4	5 }	lotal	Rate (%)
		28	30	25	26	13	122	••••••••••••••••••••••••••••••••••••••
1. Ge	neral		1		1	ا ا ب ر		4
	1 Address		¥ ا	เ 1	۱ ۶	1		1 1.
	2 Type of House		1 1	i i	1 1	· · · · ·		4 - 1 1
•	Residential	23	20	11 ¦	21	ء (ز	79	1
	Connercial	10 5	9	14	5	4 8	41	
	Residential/Comercial	1 V I	, i	_ i	2 I 2 I	- 1	+1	1 00.0
	Unknown	· · ·	1	۱ ۲		1	2	1 1
1.	3 No.of Persons in Family	123		100	122	-70	556	
	Unknown (No.of Samples)	110 ~	147 1	- !	- 1		- 100	1
1 h.	4 No.of Employees	61	57	58 ;	53			1
	Unknown (No.of Samples)	1 1	J/ 1 	1 OL	ມຈູ 1	45 {	274	۹ ۱
1.	5 Ave. Nonthly Income	• •	1	1	- I I	· · · · ·	-	ł r
1,	Baht	i i	1	1	1	1		1
	up to 2,000	6	8	7 [1	2	24	19,1
	2,001-3,000	13	1	12	. 1	3 4	- 39	
	3,001-4,500	3	. 51	4	6	5 !	23	-
	4,501-6,000		3	1	9	2 I : • 1	- 18	
	6,001-7,500		2	11	1	1	10	1 2 14.3
	7,501-10,000	1 _1	3	2 I - I	3	1 a - !	6	4.
	10,001-15,000	1 I 1 I	2	. 1	2 1	1	ş	1 1. (.)
	15,001-50,000	t 1	L 1	1	4 1	1 L	J	⊦ %∎. 1
	Over 50,000		_ 1	1			1	เ เ
	Unknown	([] 1 _]	1	ິ 1 ໂ	- 1 _ 1	1 I - I	1	0.8
1	6 Ave. Konthly Nedical Expense	1 1 1 1	1	ł	1	1		4
1.	Baht		1	1		ł		4
	up to SO	4 1 	12	12	5	1 1	- 34	27.9
	51-100	1 1	8	2	6	4	24	-
	101-200	 	5 ;	4 4	5	2	19	-
	201-500	1 17	. 1 I	4	-1	5	34	
	501-1,000	1 <u>1</u>	· · · · · · · · · · · · · · · · · · ·	1	1	11	6	4.5
	1,001-2,000		11	1 I I I	11	- 1	3	2.
	2,001-5,000					- 1	-	1 I.
	Over 5,000	4 <u>1</u>	1 _ l		1	- 1	3	1.1
	Unknown	1 I 1 I	- 1	- !	- !	_ ! _ !		· · ·
2. Ty	pe of Water Supply		1	1	1	1		1
en ly	Hunicipal System	1 7	8	6	8	3	32	26.1
	Combined	9	4 1	61	7	3	29	
	Other Sources		18	13	11	7	61	50.1
		1 76 1	10 1	i		· · ·	-	i .
3. Hu	Unknown gioinal System	F 7 F	1 1	1	ł	ł		
	nicipal System	8 - F 1 - F	! 1	ł		ļ		1
٥.	1 Pressure	1 1 1 1	6 5	8	4 1	i A l	28	45.9
	LON	6	61	4	11	2	25	41.1
	High		_ I	• • •	- 11 + - 1	_ F	8	13.
-	Unknown	, Ŭ į I I	- ;	- I I	- 1	1	v	i 191
3.	2 Quantity	i i	1	1	11	4	46	75.
	Sufficient Not Sufficient		12	5 1	4 L	2	15	

A-4-2-7

Reasons for unwillingness-	to-co	onnect		summar	ized k	below.
ESERECTER NO.	===== 1 =====	2 2 2 2 2 2	*===== 3 ======	======= 4 =======	====== 5 ======	Total
There is a well		4	5	1	·.	10
Well water is enough	1	1	3	3	2. <mark>19</mark> 10 - 1917	8
Vell water is clean	1	5	51.5 4	2	. ***	8
Vell water is convenient			**.	1	1	2
ack of money			1	1		2
ess stay in the house		ines.	1		. .	1
lot my own house	· 1			2	·••	3
Jnknown			****		. 1	1
Total	3	10	10	10	2	35
	====					

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Most people who were unwilling to connect to the municipal system thought that they already had wells and those were or clean. The wells were very close and enough indispensable to their living.

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Table A1-5-1 SUMMARY OF QUESTIONNAIRE SURVEY IN PHANG NGA (CONT'D)

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1	2 ;	3	4	15	e lorat	Rate 18
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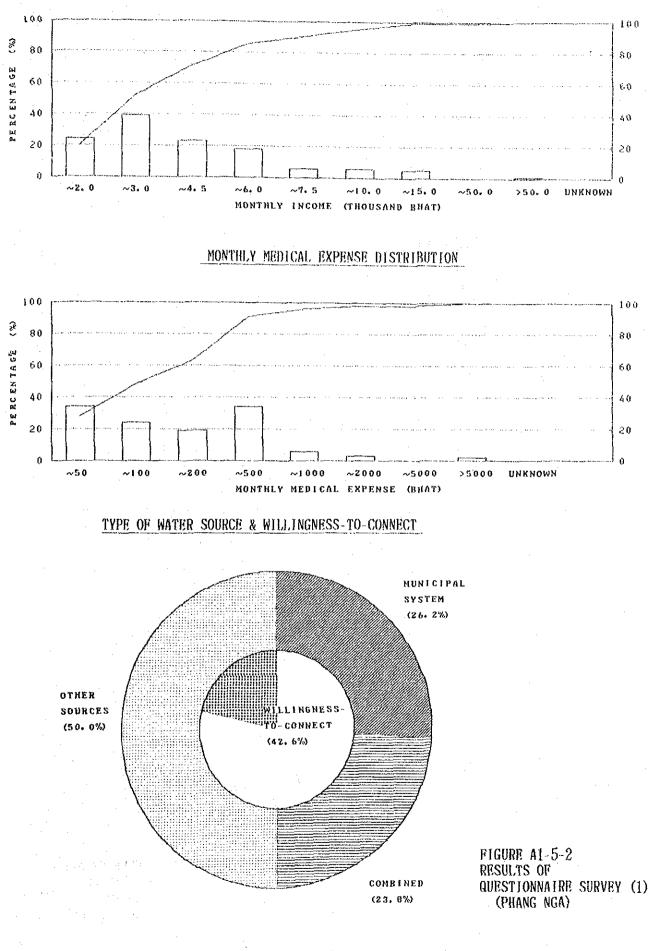
Table A1-5-1 SUMMARY OF QUESTICHMAIRE SURVEY IN PHANG NGA (CONT'D)

Block No.	1	2 .	3	1.4	5.	Total	Rate (%)
9. Willingness to Pay		1		 : !		1	
Baht			1	ł.		1. 1	
Up to 50	. 9	6	; 9	9	2	35	57.4
51-100	6	5	1 3	5	3.1	20	32.8
101-200	+	1	- 1	3		4	6.6
201-500	1	-	L -	-		1 ;	1.6
501-1,000		-	(+		- 1	- 1	-
Over 1,000	-	-		1 -	- 1	~ 1	. -
Unknown	-	1 -	i	i -	1	. 11	1.6
0. Water Quality (Other Source)		1	1	1			· .
10.1 Color) 1	5 1	3 . 4 ·		3	5 5 F
Yes	5	5	15	1 1	5	31	- 34,4
No	16	16	3		4	54	60.0
Unknown	-	1	1	2	1	5	5.6
10.2 Seell		L B	i F	L I - I		1	
Yes	5	1	15	<u> </u> i	6	28	31.1
Ko	16	20	3	16	3	58	64.5
Unknowa	-	1	¦ 1	; 1	. 1	4 1	
10.3 Turbidity		1	1 1 -	t .		1	
Yes	6	1	15	3	5	36	40.0
Ho	15	14	; 3	15	41	51	56.7
Unknown	-	1	l . 11			3	3.3
1. Willingness to Connect			L 1 -	£ 1		. 1	
Yes	9	1 8	1 3	1	5	26	. 42.6
Но	3	10	10	10	2;	35	51.4
Unknown	-	-	i	- 1		. .]	· -
2. Willingness to Pay for Connection Feel		1 }	1 . I	É I		. 1	
Baht			e e e	1 I	1 •	1	
Up to 1,000	5	1	1	-	1	8.1	30.8
1,001-2,000	3	1 3	1	t _	1	. 8	30.8
2,001-2,500	· · 1	4	1	· 1	3	10	28.4
2,501-3,000	•	-	-	-		-	-
3,001-4,000	-	-	-	· ·	-	 	-
4,001-5,000	•	-	¦ -		- }	•.i	- <u>-</u>
5,001-6,000		1	-		-	-	-
Over 6,000	-	1 -	i	¦ · -	¦ - ¦		
Unknown			-	-	- 1		. '
3. Willingness to Pay for Water Charge		i t	1 t	; 1			
Baht		1	1 . T	1		ł	
Up to 50	2	. 5	2	¦. •:	-	. 9	34.6
51-100	1	2	1	1	2	13	50.0
101-200	-	1		-	2	3	11.5
201-500	· -	-	-	- '		- 1	
501-1,0D0	-	-			1	1]	3.9
(iver 1,000	-	•	-	-	-	-	-
Unknown		·	-		-	-	-
		i			i i	i	

A-4-2-10

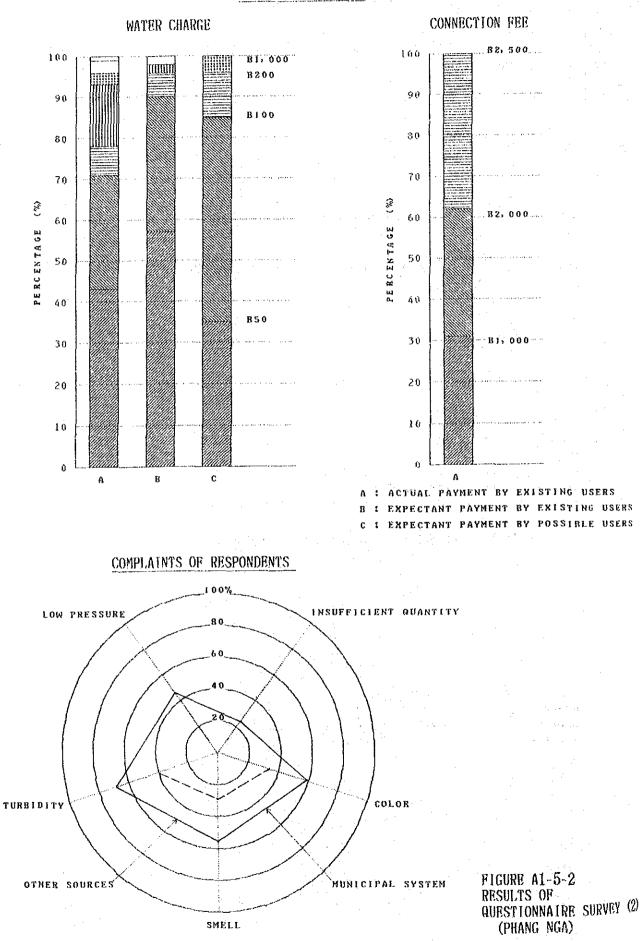
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MONTHLY INCOME DISTRIBUTION



A-4-2-11





A-4-2-12

APPENDIX A-6-1

Construction Unit Cost

Unit Cost

· ·	Ite	1		¥8	iterial		Labor	SubTotal	Transprt	Profit	Total 1 1 (w/10%cont)		Total 2	
Pi	pel	iae			******		*****					*		
				. (*****	*******		PKA's	llait Ral	La [1989]	*******	*******	****	
	â.	N/C	Pipe(lor ta	l fytpe)	(25%)				. (1901)				
		100			85	21	56	162	8	35	224	140	364	
		150			142	38	11	255			353	154	507	
						64	90		19	90	569	166	735	
		250			352	88	126			125	792	179	971	
					507	127	167			177	1119	223	1342	
		100			970	243	248			324	2050	248	2298	
			11		1362	341	278				2812	283	3095	
•		0UU	62		1761	440	354	2555	161	570	3815	319	3934	
	Ь.	Stee	el Pip	e		(35X)								
			-		545	191	99	835	12	178	1127	140	1267	
		200	12		720	252	111			232	1471	154	1625	
	•	250	TT.		1080	378	153	1611	38	345	2195	166	2361	
			11		1330	465	202	1998	- 58	432	2736	179	2915	
÷			11			497	250			472	2991	223	3214	
:			-		1104	625	361				3901	248	4149	
			IL		2140	749	468					283	5103	
۰.		300	88		2495	873	582	3950	322	891	5686	319	6005	
••								*****	*******				********	
•														
	÷.,													
•														
						-								

lten	Material	Fitting (10X)		SubTotal				otal 1 Pa 10%cont)	venent	Total 2			Adopter (1988)
		Unit Rate	Based on	Pipe Nat	terial Con	st as of	Decei	uber,1988		\$ # #####	PWA Price	Batio	
a. A/C Pipe (CIASS ZV NOI	(10 %) (10 %)		-	i ata		-		: 	· · ·	(1987)	÷ .	
100 mm	115	12	63	190	1	4	1 :	281	153	414	364	-1.14	41
150 🖬	189	19	87	295	12	64	ţ	408	168	577	507	1.14	58(
200 💵	328	-33	101	462	21	10	i 🦷	643	181	824	735	1.12	82
250 mm	454	45	142	641	32	14	l	895	196	1091	971	- 1.12	1,09
300 mm	643	64	188	895	44	- 19	7	1249	244	1493	1342	1.11	1,49
400 💵	1217	122	279	1618	87	351	3	2270	271	2541	2298	1.11	8,54
500 au	1699	170	313	2182	144	484	}-, -, -, -, -, -, -, -, -, -, -, -, -, -	3096	309	3405	3095	1.10	3,41
600 mm	2187	219	398	2804	176	621	5 	3967	349	4315	3934	1:10	4,32
. Steel Pipe		(15 X)											:
150 📷	550	83	111	744	13	159	}.	1008	168	1176	1267	0.93	1,27
200	908	136	125	1168	24	25()	1587	181	1769	1625	1.09	1,17
250 mm	1210	182	172	1564	42	331	1.	2136	196	2332	2361	0.99	2,36
300 mm	1507	226	227	1960	63	42	5	2693	244	2937	2915	1.01	2,94
400 mm	1887	283	281	2451	87	53:	} . •	3378	271	3649	3214	1.14	3,65
500	2261	339	406	3006	175	668	} .	4233	309	4542	4149	1.09	4,54
600 m	2723	408	526	3657	288	829	}	5252	349	\$800	\$103	: 1,10	5,60
700 BB	3179	477	655	4311	352	979	}	6206	407	6612	6005	1.10	6,61
800 m	4527	679	932	6138	460	138	5	8781	485	9246			9,25
900 ##	5104	765	1051	\$921	582	1575	;	9986	523	10508			10,51
1000 ##	6804	1021	1401	9225	718	2088	}	13234	581	13815			13,82
1100 mm	7926	1189	1632	10746		2439	}	15460	639	16099			16,10
1200	9048	1357	1863	12268	1034	2793	}	17705	691	18402			18,40
1350 mm	11000	1650	2265	14915	1309	3401		21594	784	22378			22,38
1500 ##	12953	1943	2687	17563	1616	4021	-	25528	871	26398			26,40

For Transmission Pipeline (Transportation < 800 km)

*** Note: Pipe material prices are estimated from the contractor's purchasing price

as of Dec.1988

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For Distribution Pipeline (Transportation < 800 km)

	ter				natefi	al Fitti	ng	Lador	SUDTOLAI	Trans ((800	prt ku) e	Profit tc.(21%	Tot)(v/10	al 1 - Pa Xcont)	venent	fotal 2		-	Adopteo (1988)
8	•	Ą/C	Pipe	e (Cl	(\$\$\$ ase 20	** Unit B Normal ty (25	pe)	Based on	ı Pipe Ka	terial	Cost	as of	Decenb	er,1988		* * ****	PWA Price (1987)	Ratio	
		100	23		1		29	63	207		1	45		284	153	437	364	1.20	44(
		150					{ 7	87	323		12	70		446	168	614	507	1.20	510
	1	200	ER -				82	101	\$11		21	112		708	181	890	735	1.21	890
	1	250	۲		4	54 1	13	142	709		32	155		986	196	1181	971	1.22	1,180
	. ;	300	38		6	43 1	61	188	991		44	217		1378	244	1621	1342	1.21	1,620
	4	100	11		12	17 3	04	279			87	397		2513	271	2784	2298	1,21	2,780
	ļ	500	Ħ		16	99 4	25	313	2437		144	542		3435	309	3744	3095	1.21	3,740
	6	500	81		21	87 5	47	398	3132		176	695		4403	349	4752	3934	1.21	4,750
Ь	. (Ster	el Pi	ъA		(35	¥ 1											·	
		150			5		93	111	854		13	182		1154	188	1322	1267	1.01	1 200
		200		·			18	125	1350		24	289		1829	181	2010	1625	1.04	1,320
		250			12		24	172	1806		42	388		2459	196	2654	1025 2361	1.24	2,010
		300			15		27	227	2262		63	488		3095	244	3338	2915	1.12 1.15	2,650 3,340
		100			18		60	281	2828		87	612		3880	271	4151	3214	1.15	4,150
		100			22		91	406	3458		175	763		4835	309	5144	4149	1.24	5,140
		500			27		53	526	4202		2 88 -	943		5977	349	6325	5103	1.24	6,33(
		700		-	31			655	4946		352	1113		7052	407	7459	6005	1.24	7,460
		300			45		84	932	7043		460	1576		9986	465	10451	9444	1167	10,450
		100			51			1051	1941		582	1790		11344	523	11867			11,870
		100			68			1401	10586		718	2374		15045	581	15625			15,630
		100			79			1632	12332		869	2772		17570	639	18209			18,210
		200			90	1.1		1863	14077		034	3173		20113	697	20810		. *	20,810
		150			110		50	2255	17115		309	3869		24522	784	25307		1.4	25,310
· .		500			129			2667	20153		616	4571		28974	871	29846			29,850

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as of Dec.1988

A-6-1-3

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	10001 241	(101)	PERAL	98610191					otal 1 P 10%cont)		Fotal 2			Adopte (1988)
								***				· · · · · · · · · · · · · · · · · · ·		
	(*****	Unit Rate	Based on	Pipe Na	teria	l Cos	t as of I	lecer			******	e La sere		
a. A/C Pipe (Class 20 Nor	mal type)												
		(10 %)											1 - <u>1</u>	
100 na	115	12	63	190		13	43		270	153	423	364	1.16	. 42
150 mm	189	19	87	295		24	67		424	168	593	507	1.19	59
200	328	33	101	462		42	106		670	181	852	735	1.16	85
250 un	454	45	142	641		63	148		937	196	1133	971		1,13
300 me	643	64	188	895	÷	87	206		1308	244	1551	1342	1.16	1,53
400 mm	1217	122	279	1618		175	377	. : .	2387	271	2658	2298	1.16	2,6(
500 mm	1699	170	313	2182	•	288	519		3288	309	3597	3095	1.18	3,6(
600 mm	2187	219	398	2804		352	663		4201	349	4549	3934	1.18	4,58
b. Steel Pipe		(15 X)		·						÷ .				e Dana
150 an	550	83	- 111	144		26	162		1025	168	1193	1267	0.94	1,21
200 mm	908	136	125	1168		48	255		1619	181	1801	1625	1.11	1,80
250 mm	1210	182	172	1564		83	346		2192	196	2387	2361	1,01	2,3
300 mm	1507	226	227	1960		127	438		2778	244	3022	2915	1.04	3,0
400	1887	283	281	2451		175	551		3495	- 271	3766	3214	1.17	3,11
500 mm	2261	339	406	3006		350	705	. •	4466	309	4775	4149	1.15	4,18
600 mm	2723	408	526	3657		577	889	· · .	5636	349	5984	5103	1.17	5,98
700 mm	3179	477	655	4311		704	1053		6674	407	7081	6005	: 1,18.	7,0
800 mm	4527	679	932	6138		919	1482		9393	465	9857			9,81
900 m m	5104	766	1051	6921		1163	1698		10760	523	11283			11,28
1000 ##	6804	1021	1401	9225		1436	2239		14190	- 581	14771			14,71
1100	7926	1189	1632	10746		1738	2622	• .	16616	639	17256		1.1	17,20
1200 ##	9048	1357	1863	12268	· ·	2088	3011		19081	697	19778		. ii.	19,18
1350 mm	11000	1650	2265	14915		2617	3682		23336	784	24120		te a l'	24,12
1500 mm	12953	1943	2857	17563		3231	4367		27677	871	28548			28,5
						1	× .			1.4	1.11		$A_{i+1} = A_{i}$	

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Iten			Naterial	Pitting			()=800ke);	etc.(21%	Total 1 Pa (w/10%cont)		Total 2		•	Adopte (1988)
												*******		*******
	a'nti.	101.	(######	Unit Kate	based on	Pipe Nat	erial Cos	t as of	December,1988		******			
8. A/	o ripe	(619	65-6V KQ1	real type)										
10	0 44			(25 X)	6.6									
	1 A		115	29	63	207	13	46		153		364	1.23	45
	0 11	•	189	47	87	323	24	73		168	630	507	1.24	63
	0 88		328	82	101	511	42	116		181		135	1.25	92
	0 aa .		454	113	142 -	709	63	162		196	1223	971	1.26	1,22
	6 au		643	161	188	991	87	227		244		1342	1.25	1,68
	() en		1217	304	279	1801	175	415		271	2901	2298	1.26	2,90
	0		1699	425	313	2437	. 288	572		309	3336	3095	1.27	3,94
60	() HH		2187	547	398	3132	352	732	4637	349	4986	3934	1.27	4,99
b. St	eel Pij	be		(35 X)			•							
15	0 💼	· ·	550	193	111	854	28	185	1171	168	1340	1267	1.06	1,34
20	0 BH		908	318	125	1350	48	294		181	2042	1625	1.26	2,04
25	0 ee		1210	424	172	1808	83	397		198	2709	2361	1.15	2,71
- 30	0 mm		1507	527	227	2262	127	502		244	3423	2915	1.17	3,42
40	0 se		1887	660	281	2828	175	631		271		3214	1.33	4,27
58	0 DR		2281	791	408	3458	350	800		309	5377	4149	1.30	5,38
60	0 10		2723	953	528	4202	577	1004		349	6709	5103	1.31	6,71
70	0 00		3179	1113	655	4946	704	1187		407	7927	6005	1.32	7,93
80	0 88		4527	1584	932	7043	919	1672		465	11062			11,06
90) se		5104	1786	1051	7941	1163	1912		523	12641			12,64
) . .		6804	2381	1401	10586	1436	2525		581	16582			16,58
			7926	2774	1632	12332	1738	2955		639	19365			19,37
	0		9048	3167	1863	14077	2068	3391		697	22187			22,19
	0 88		11000	3850	2265	17115	2617	4144		784	27049			27,05
) ne		12953	4533	2667	20153	3231	4911		871	31998			32,00

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Unit Cost

For Distribution Pipeline (Transportation >= 800 km)

Note: Pipe material prices are estimated from the contractor's purchasing price
 as of Dec.1988

Onit Cost

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Construction Works		Price in Tenders (A)	3 Lovest (1988)		(A)*1.35	t PWA's Unit Cost (for 1987)	Cost (1988)	
Concrete Work (incl.Porm Work,Scafolding)		2,200		Babt	2,970 /cn 1	•		2
Re-Bar	Baht	18	/kg	8aht	24 /kg	•		
Unit Concrete Cost (incl.Porm Work,Scafolding, Re-Bar(100kg/cummcon))		Baht	5,370 /cu 1	1 ~	5,400	
Barth Work Bxcavation (with Backfill)		55	/cu #		79 /cu 1	1	80	: : :
Soil Fill			/cu n		76		120 (Prob PKA	Cost)
Architectural Works Administration Bldg. Eead Quarter Bldg.			/sq 1	• .	6,451 ∕sq ∓ 5,180		5,000	
Chlorination House	Baht	2,830	/sq n	Babt	4,043 /sq m	3610 - 4309	3,800	
Pump House (ercl.pump pit)	Baht	1,860	/sq ∎	Baht	2,657 /8q m	3540 - 4200	3,600	

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antruction Rock	mark a co		*************************	*************
Construction Works	PWA's Cost	Unit Cost	Estimated Cost	Adopted
di seconda d	(foc 1987) (Baht 1000)	(Baht/cu s/h)	(for 1989)	Cost
	(ballt TUVV)	(A)	(A)*1.30	(1988)
reatment Pacilities			Unit Cost	Unit Cost
Ballastatian Basta	:		(Baht/cu m/h)	(Baht/cu m/h)
Sedimentation Basin	1 910	A.C. A.D.C.		
50 cu s/hr	1,310	26,200	34,100	34,000
100 cu s/br	1,533	16,330	21,200	21,000
200 cu ø/hr	3,136	15,680	20,400	20,000
250 cu u/hr	5,133	20,532	26,700	27,000
500 cu #/hr	7,708	15,416	20,000	20,000
1000 ca s/hr	17,723	17,723	23,000	23,000
Filters				
50 cu n/hr	.588	11,760	15,300	15,000
100 cu ø/hr	1,044	10,440	13,800	14,000
200 cu u/hr	2,227	11,135	14,500	15,000
250 cu #/hr	2,337	9,348	12,200	12,000
500 cu n/hr	4,674	9,348	12,200	12,000
1000 cu ø/hr	11,356	11,356	14,800	15,000
Clear Water Reservoir			Unit Cost	Vait Cost
			(Baht/cu m)	(Baht/cu m)
500 cu m	887	1,774	2,300	2,300
1000 cu a	1,628	1,628	2,100	2,100
1500 cu u	2,699	1,799	2,300	2,300
2000 cu a	2,803	1,402	1,800	1,800
2250 cu 🖬	3,282	1,459	1,900	1,900
3000 cu m	6,633	2,211	2,900	2,900
3300 cu m	6,603	2,001	2,600	2,600
4000 cu e	7,730	1,933	2,500	2,500
5800 ca m	10,809	1,864	2,400	2,400
levated Tank			Cost	Cost
			(Baht 1000)	(Baht 1000)
50 cu m	722		940	900
120 cu #	1,145		1,490	1,500
250 cu m	1,334		1,810	1,800

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APPENDIX A-8-1

Distribution Network Analysis

1.1.1.1.1.1.1.1

				· .		e e e			
									•
TIT NO DI	L E F PIPES	:	Pang Nga 109	(Pro	posed)			• .	
NO. OI	NODES	:	S0 1.3			. *			
MAX HI	ADLOSS,		100 .008				• •		
PIPE	FROM	TO	LENGTH	DIA	HWC	FLOW	VELOCITY	HEADL	055
NO.	Node	Node	(M)	(MM)		(LPS)	(MPS)	(M/KM)	(M)
1 2	1 2	2 3	800.00 3200.00	200 200	100 100	6.33	0.20L0	0.46	0.37
3	1	4	550.00	200	100	0.69	0.02L0 0.72	0.01 4.80	0.02 2.64
4	4	.5	240.00	200	100	13.76	0.44	1.94	2.84
5	5	6	90.00	100	100	2.46	0.31	2.35	0,21
5	5	7	160.00	200	100	11.18	0.36	1.32	0.21
7	7	8	515.00	200	100	8.48	0.2710	0.79	0.41
8	8	9	990.00	200	100	8,64	0.28L0	0.82	0.81
9	9	10	390.00	150	110	4.00	0.23L0	0.67	0.26
10	10	11	220.00	150	110	2.87	0.16LO	0.36	0.08
11	11	12	200.00	150	110	2.03	0.11LO	0.19	0.04
12	12	13	230.00	100	100	0.16	0.02LO	0.02	0.00
13	13	15	600.00	100	110	0.60	0.08L0	0.14	0.09
14	15	14	200.00 130.00	100 100	110	0.26	0.0310	0.03	0.01
15 16	14 16	16 17	240.00	100	100 100	1.18 0.70	0.15L0	0.61	0.08
17	17	18	135.00	100	110	0.47	0.09L0 0.06L0	0.23 0.09	0.06 0.01
18	19	18	405.00	100	100	0.18	0.0210	0.02	0.01
19	19	20	170.00	100	100	0.04	0.01L0	0.00	0.00
20	- 9	66	410.00	150	100	3.25	0.18L0	0.55	0.22
21	21	22	710.00	150	100	1.83	0.10L0	0.19	0.13
22	22	23	650.00	100	100	0.16	0.02L0	0.02	0.01
23	16	24	290.00	100	100	0.29	0.04L0	0.04	0.01
24	24	25	240.00	130	100	1.88	0.11L0	0.20	0.05
- 25	25	20	640.00	150	110	1.50	0.08L0	0.11	0.07
26	27	26	400.00	100	100	1.07	0.14L0	0.51	0.20
27	26	28	550.00	150	110	2.05	0.12L0	0.20	0.11
28	29	30	260.00	100	100	0.01	0:00L0 0.47	0.00 3.15	0.00 0.19
29	4	29	60.00	150 100	100 100	8.39 1.62	0.2110	1.09	0.19
30	29	31 31	250.00 130.00	100	100	0.34	0.04L0	0.06	0.01
31 32	32 31	33	180.00	100	100	1.71	0.22L0		0.22
33	34	33	165.00	100	100	0.15	0.02L0	0.01	0.00
34	33	35	115.00	150	100	4.08	0,23L0	0.83	0.10
35	36	35	185.00	100	100	0.16	0.0210	0.01	0.00
36	35	37	180.00	150	100	4.20	0.2400	0.88	0.16
37	37	38	215.00	100	100	0.34	0.04LO	0.05	0.01
38	37	39	90.00	150	100	3.84	0.22LO	0.74	0.07
39	40	39	205.00	100	100	0.10	0.01L0	0.01	0.00
40	42	41	145.00	150 .		0.95	0.05L0	0.06 0.51	0.01
41	40	42	155.00	150	100	3.12	0.18L0	0.51	6.08

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PIPE NO.	FROM Node	TO Node	LENGTH (M)	DIA (MM)	НМС	FLOW (LPS)	VELOCITY (MPS)	HEADLOSS (M/KM) (M)
42	41	43	230.00	150	100 150	4.32	0.24L0 0.55	0.92 0.21 3.15 0.25
43 44	43 44	44 61	80.00 125.00	100 150	100	4.32 3.62	0.55	0.67 0.08
45	45	46	160.00	150	100	0.15	0.0110	0.00 0.00
46	100	1	30.00	200 150	100 100	28.85	0.92 0.30	7.63 0.23 1.39 0.26
47 48	29 32	32 34	200.00	150	100	4.76	0.27L0	1.10 0.22
49	34	36	100.00	150	100	4.38	0.2510	0.95 0.09
50 51	36 38	38 40	215.00 60.00	150 150	100 100	4.01 4.21	0.23L0 0.24L0	0.80 0.17 0.88 0.05
52	39	41	110.00	150	100	3.93	0.2210	0.78 0.09
53	7	33	15.00 20.00	200 150	100	2.51 0.56	0.08L0 0.03L0	0.08 0.00 0.02 0.00
54 55	41 45	8	20.00	100	100	1.38	0.1810	0.80 0.02
56	12	14	520.00	150	100	1.82	0.1010	0.19 0.10
57 59	14 11	22 21	20.00	100 100	100 100	0.55	0.07L0 0.10L0	0.15 0.00 0.28 0.01
-60	25	19	20.00	100	100	0.27	0.0310	0.04 0.00
61	17 22	19 24	270.00 380.00	100 150	100 100	0.18 2.06	0.02L0 0.12L0	0.02 0.01 0.24 0.09
62 63	10	13	950.00	100	100	0.51	0.0610	0.13 0.12
64	61	45	550.00	150	100	2.94 5.50	0.17L0 0.31	0.45 0.25 1.21 0.24
65 66	2 51	51 52	200.00 250.00	150 100	110 ° 110	2.54	0.32	2.09 0.52
67	51	53	650.00	100	110	2.77	0.35	2.44 1.59
68 69	52 56	56 55	670.00 850.00	100 100	110 110	2.17	0.28L0 0.22L0	1.55 1.04 1.01 0.86
70	58 53	. 54	750.00	100	110	1.76	0.22LO	1.06 0.80
71	29	54	300.00 310.00	100 100	110 110	1.23	0.16L0 0.10L0	0.55 0.16 0.25 0.03
72 73	40 60	57 59	400.00	100	110	0.41	0.05L0	0.07 0.03
74	42	60	420.00	100	110	2.00 0.17	0:25L0 0.02L0	1.34 0.56 0.01 0.01
75 76	61 57	60 58	400.00 250.00	100 100	110 110	0.17	0.0200	
77	58	62	700.00	100	110	1.60	0.20L0	0.89 0.62
78 79	62 63	64 64	850.00 630.00	$\frac{100}{100}$	110 110	1.13 0.23	0.14L0 0.03L0	0.47 0.40 0.02 0.02
80	60 66	21	200.00	100	100	1.21	0.15L0	0.63 0.13
81	64	67	720.00	100 100	110 110	$1.06 \\ 1.03$	0.13L0 0.13L0	0.41 0.30 0.40 0.18
82 83	66 67	65 68	450.00 740.00	100	110	0.65	0.08L0	0.17 0.12
84	23	63	890.00	100	110	0.57	0.0710	0.13 0.12 0.00 0.00
ଟର ଅନ୍ତ	69 27	70 70	950.00 1070.00	100 100	110 110	0.01 0.73	0.00L0 0.09L0	0.21 0.23
87	18	71	320.00	100	110	0.18	0.0210	0.02 0.00
88 89	9 45	27 63	400.00 320.00	100	110 110	$\begin{array}{c} 1.95 \\ 1.33 \end{array}$	0.25L0 0.17L0	1.28 0.51 0.63 0.20
30	40 54	55	420.00	100	110	0.47	V.ValO	
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PIPE	FROM	то	LENGTH	DIA	HWC	FLOW			
NO.	Node		(M)	(MM)	ELM/C	(LPS)	VELOCITY (MPS)	HEAD (M/KM)	LUSS (M)
91	10	73	330.00	100	110	0.47	0.06L0	0.09	
92	72	73	430.00	100	110	0.17	0.0210		0.01
93	74	72	400.00	- 100	110	1.05	0.13L0		0.16
94	75	74	600.00	100	110	1,48	0.1910	0.76	0.46
95	76	75	400.00	100	110	2.00	0.25L0	1.34	0.53
96	6	76	200.00	100	110	2.30		1.74	0.35
97	. 77	23	400.00	100	110	0.48	0.0610	0.09	0.04
98	65	77	350.00	100	110	0.65	0.08L0	0.17	0.06
. 99	78	65	370.00	100	110	0.78	0.10L0	0.23	0.09
100	63	78	350.00	100	110	0.99	0.13L0	0.37	0.13
101	79	63	350.00	100	110	1.32	0.17L0	0.63	0.22
102	59	79	270.00	100	110	1,45	0.18L0	0.74	0.20
103	.80	59	320.00	100	110	1.56	0.20L0	0.84	0.27
104	57	80	340.00	100	110	1.65	0.21LO	0.94	0.32
105	81	57	350.00	100	- 110	1.69	0.22L0	0.98	0.34
106	54	81	350.00	100	110	1.77	0.22L0	1.07	0.37
107	55	82	350.00	100	110	1.76	0.22L0	1:06	0.37
108	82	58	350.00	100	110	1.61	0.21LO	0.90	0.31
109	· 68	83	490.00	100	110	0.70	0.0910	0.19	0.09
110	83	69	580.00	100	110	0.26	0.03LO	0.03	0,02

		NODE	FLOW	ELEVATION	HGL	PRESSURE	
		NO.	(LPS)	(M)	(M)	(M)	
		100 R	28.853	100.00	128.00	28.00	
		1	-0.052	100.00	127.77	27.77	
		2.	-0.138	102.00	127.40	25.40	
	•	3	-0.690	106.50	127.38	20.88	
		4	-0.324	104.00	125.13	21.13	
		5	-0.113	105.00	124.66	19.66	
		6	-0.159	107.00	124.45	17.45	
		7	-0.191	106.00	124.45	18.45	
		8	-0,400	108.00	124.05	16.05	
		9	-0.820	100.00	123.23	23.23	
		10	-0.153	100.00	122.97	22.97	
		11	-0.056	99.50	122.89	23.39	
		12	-0.049	99.50	122.85	23.35	
		13	-0.074	102.20	122.85	20.65	
		14	-0.339	99.50	122.76	23.26	
		15.	-0.342	104.40	122.76	18.36	
		16	-0.190	99.50	122.68	23.18	
		17	-0.053	99.50	122.59	23.09	
		18	-0.464	100.00	122.58	22.58	
		19	-0.239	99.50	122.59	23.09	
		20	-0.040	99.00	122.59	23.59	
		21	-0.161	99.50	122.88	23.38	
		22	-0.156	99.50	122.75	23.25	
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	NODE	• • •	FLOW	ELEVATION	HGL	PRESSURE		
	NO.	· · ·	(LPS)	(M)	(M)	(M)		
	23	tan bak gip yan tah v	-0.070	96.50	122.74	26.24		
•	24		-0.476	99.50	122.66	23.16		1. A
	25		-0.109	99.50	122.59	23.09		
	26 07		-0.516 -0.142	99,00 94,00	122.52 122.72	23.52	- <u>-</u>	
	27 28	н. 1	-2.054	104.00	122.41	18.41		· .
	29	· · ·	-0.129	104.00	124.94	20.94	1	54 -
	30		-0.013	103.80	124.94	21.14		
	31		-0.256	105.00	124.67	19.67 20.68	÷ .	· · ·
	32 33		-0.291 -0.298	104.00	124.68 124.45	18.45		
	34		-0.222	104.00	124.46	20.46	a de la composition de la comp	
	35		-0.029	106.00	124.36	18.36		
	36		-0.217	104.00	124.36	20.36	1	. · ·
	37 39		-0.022 -0.140	107.00 105.00	124.20 124.19	17.20	N	
· · · · · · · · · · · · · · · · · · ·	38 39		-0.013	107.00	124.13	17 13		
	40		-0.177	105.00	124 14	19 14		
	41		0.000	108.00	124.05	16.05	i to a	
	42		-0.176	106.00	124.05	18.05	n de la composition de la comp	
	43 44		0.000 -0.705	109.40	123.83 123.58	14.43		
	44		-0.074	100.00	123.25	23.25		
	46		-0.155	100.00	123.25	23.25		
	51		-0.195	100.00	127.16	27.16		
	52		-0.377	102.00	126.64	24.64 25.57		
	53 54		-1.002 -0.764	100.00	125.57 124.78	23.37		
	54 55		-0.422	100.00	124.74	24.74		
	56		-0.452	102.00		23.60	i e	
	57 -		-0.641	104.00	124.06	20.06		
	58		-0.209	104.00	124.05 123.47	20.05 20.47		
	59 60		-0.519 -1.758	103.00 107.00	123.47	16.49		
	61		-0.511	108.00	123.50	15.50		
	62		-0.467	103.00	123.43	20.43		, i
	63		-1.430	100.00	123.05	23.05		
	<u>6</u> 4		-0.307	100.00	123.03 122.83	23.03 23.83		
	65 68		-1.161 -1.006	99.00 79.30	123.01	23.65		
	67 .		-0.406	100.00		22.73		
	68		-0.519	99.50	122.62	23.12		•
	64		-0.254	99.00	122.50	23.50		
	70		-0.740	99.00	122.50 122.58	23,50 22,58		
	71 72		-0.190 -0.680	100.00		22.95		
	73		-0.642	100.00	122.94	22.94		
	74		-0.422	106.00	123.11	17.11		

	• . •			. *	
NODE NO.	FLOW (LPS)	ELEVATION (M)	H G L (M)	PRESSURE (M)	
75 76 77 78 79 80 81 82 83	$\begin{array}{r} -0.521 \\ -0.307 \\ -0.178 \\ -0.214 \\ -0.121 \\ -0.098 \\ -0.074 \\ -0.146 \\ -0.439 \end{array}$	107.00 107.00 99.50 99.50 101.50 103.50 102.00 102.00 99.00	123.57 124.10 122.77 122.92 123.27 123.74 124.40 124.37 122.51	$16.57 \\ 17.10 \\ 23.27 \\ 23.42 \\ 21.77 \\ 20.24 \\ 22.40 \\ 22.37 \\ 23.51 $	
		·			
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· · · · · · · · ·			·		·

A-8-1-5

APPENDIX A-11-1

Details of Operation Cost

Phing New

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RATER TRANSMISSION AND DISTRIBUTION COST STUDY (Phang Rga)	AFD DIST.	HOLLOGI	COST STUI	DY (Phan	(283)							:										
	0561	1661 1661 1661	1992	1993	1991	r 2001	3661	1997	E 1998	1999	2000	A 2001	2002	2003	8 2004	2005	2006	2007	2008	2638	2610	28:1
[. Planned Daily Average Water Demand (cu m/d)	erage fate	r Desad	(cn =/d	-	T 4 3 4 4 4 4 4 4										· .		1 I					
	I,639	1,639 1,683 1,728 1,775	1,728	1,775	1,824	1,875	1,928	2,048	2,170	2,232	2,415	015'2	2,612	2,567	2,764	2,844	2,927	3,006	3,885	101	3.245	
 Planned Baily Maximum Water Demand: QDM {cu s/d Peak Factor = 1.30 Planned Total 2,131 2,188 2,246 2,308 	zinun ¥at< 1.30 2,131	um Water Demand: QDN (cn s/d 1.30 2,131 2,188 2,246 2,308	: 90% (cu s/d} 2,308	2,371	2,438	2,506	2,652	2,821	2,980	3,140	3,302	3,396	3,493	3+593	3, 697	3, 805	3,905	f, 008	4,115	1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1 (2) (***) 1 (***) 1 (***) 1 (***)
3. Freatment Plant Eristing Plant After Modified	*****	***************************************	*****	*****	tattstettettettettstat) far. (apacity	****		Кал. Саре	scity	1,440 cu #/d	1	Set Capacity	ity	1,313 c	1,333 cu #/da7	1 1 1 1 1 1	1,330 cu e /đ	1	*****			(#)###################################
fier freathent Unit	it.				·	*****	*******	******		Kar. Capacity	acity	3,300 cu e/d		Set Capacity	ity .	3,056 01	cu, ø/day	4	z#######	**********************	******	:****
Treatment Capacity 1,440 Ret Treatment Capa 1,333	ty 1,440	1,440	1,440 1,333	1,440 1,333	4,740	4,740 4,386	4,740 4,386	4,740 4,385	4,740 4,386	4,740 4,386	4,740 4,386	986,4 4,740	4,740	4,740 4,385	4,740 4,386	4,740	4,740 4,385	4,740 4,35ô	4,740 5,005	4, 740 4, 386	فرزز میڈ سرم وروز کاس وروز سرم میڈ	120 623 1491 615 611 615
4. Water Amount for Intake Design : (cu m/d)	r Intake I	esign :	(cu =/d)	_															5 2 5 1 1			
Raw waler for Treatment Plant (Daily Kax)*1.08#1.1		2,531 2,599	2,669	2,741	2,917	2,896	2,978	3, 183	1,351	3,540	1,730	3,923	£,03£	1,150	4,259	4,392	4,520	f. 539		1,835	10.5	5,151
5 Daily Average Transmission Asount	ranszissic	a Asount																				
Raw water for Treatment Plant		1,639 1,683	1,728	I,775	1,824	1, 875	1,928	2,048	2,170	2,292	2,415	2,540	2,612	2,687	2+764	2,844	2,927	3,004	3,083	°, 155	55 - 57 55 - 5	19 19 10
6. Purp Characteristics Raw Water Purp	stics			Dia-	100 .	а А А	10	Li Dat Dat	10.0 =	H Of M	8	cu a/ain,		Ho.of Pumps =	ц гч	units (excluding I vait staad-by)	l gaida:	i uait si	taad-09)			
Clear Rater P un p at New MTP at Exis.WTP		Eris.Punp	<u>م</u>	Dia= Dia=	200 mm. 100 mm.	ំ ម ម ស ស គឺ ឆឺ	26 KW 10 KW	2 0 22 2 22 22	40.0 z, 40.0 z,	он сто-	_	1.6 cu m/miu, 0.7 cu m/miu,	No.of No.of	a.of Pueps = No.of Pueps =	51 64 1 1 1	2 units (excluding 1 unit stand-by) 2 units (excluding 1 unit stand-by)	l galbulc I galbulc	l unit si unit st	tand-by) tand-by}			

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rby jarad

5 te	1990	1991	1992	[993	1991	Y 1995	1996	1997	.E 1998	5661	2000	A 2001 -	2002	2003	R 2004	2005	2006	2002	2008	6807	3010	2011
8. Ko.of Operating Pumps Ray Water Pump Max.Capa.of Pump	2	1 2,575	2,575	1 2,575	1 2,575	1 2,575	1 2,575	1 2,575	1 2,575	1 2,575	1 2,575	2 5,151	5,15I	2 5,151	2 5,151	2 5,151	2 5,151	5,151	2 5,151			4
r Pump Pasp a.of Pump a.of Pump apacity (cu e)	2,016	2,016 2,016	2,016 2,016	2,016 2,016	1 2,310 2,016 4,325	2,310 2,310 2,016 4,326	2,310 2,310 2,016 4,326	2,016 4,326 4,326	2,310 2,310 4,326	2 4,620 2,016 6,636	4,628 4,628 2,016 6,635	4,620 2,016 5,635	2,620 2,620 2,016 6,536	6,620 6,620 6,635	4,620 2,615 5,536 5,536	4,620 2,016 6,636	4,620 4,620 2,016 6,636	4,520 2,015 5,536	4, 620 5,916 536			າ) ເມື່າ ເມື່າ ເປັນ ເມື່ອງ ເປັນເຜັ້ ເພື່ອງ ເປັນເຜັ້
9. Notor Output (Iw) Raw Eater Pump Clear Water Pump	20	10 20	20	20	0.0	0	91 91	10	9.4	01	2.6	20 60	20 60	20 60	50 50	20 60	29	20	50 50	03	98 98	「「「「「「」」」
 Energy Consumption (Ewh/day) Ray Eater Pump Clear Rater Pump 317 	a (Evh/d	14y) 242 317	249	255 317	592 592	270 168	277 181	295 511	312	330 161	348 526	366 551	316	387 583	338 600	409 617	121	432	539 117	456 687	105 105	100 100 100
11. Purp Operation Cost (Baht x1,000/year) Deemod Charge 55 82 83 Eacrgy Charge 143 251 25	st (Baht 55 143	t x1,000/ 82 251	year) 82 254	82	111	131	137 341	137 362	383	192 371	165 391	220	220 423	228 +35	220 448	191 191	11	226	200	515	କାର କାର୍ଯ୍ୟ ମଧ୍ୟ	1 7 1 941 7 1 941 7 1 942
Total Cost	197	134	337	340	191	469		1 36	22	564	584	159	643	655	668	681	69	101	719 otal (11	719 733 Total (1990-2011)	55: 1 11	18.419
Chemical Cost			, 9 9 1 1			- 			E 6 2 3 3 4 5 4 5 4 5 4 5 7								7 2 2 2 2 2 2 2 3					1 · ·
Alum (ave 10.0 mg/l) Chemical (kg/y) Cost (Baht 1000)	5,982 24.2	5,143 24.9	6, 307 25.5	6, 1 79 26. 2	6,658 27.0	6,844 27.7	7,037 28.5	7,475	7,921	8,366	35.7	9,271 37.3	9.534 38.6	9, 808 39.5	10,389 40.3	16.381	10,624	19,965 4,4	52°5	61.3 005 645 - 645 - 645 - 645 - 845	11,250	6-1 (9) 6-1 (9) 6-1 (9) 6-1 (9)
<pre>Lime [ave 3.9 mg/1] Chemical [kg/y] Cost (Baht 1000)</pre>	1,795	1,843	1,892	1,946	1,997	2,053	2,111	2,243	2,376	2,510	2,64	2,781	2,850	2,942	3,027	142 CD 177 CD 177	3,205	3,289	3,376	99 4	1,552 4.4	3,852
Chlorine (ave 2.0 mg/l) Chemical (kg/y) 1, Cost (Baht 1000) 1	1,196 1,196 18.7	1,229	1,261	1,296 20.2	1,332	1,369	1,407	23.3	1,584	1,673	1,763	50 02 50 02 50 02	1,907	1,962	2,018	2,075	2,137 33.3	2,193	35.1	2,310 35.8	37.5	2.435
Total cost(Baht 1000	45.1	16.3	41.6	f8.9	50.2	51.6	53.1	56.4	59.8	63.1	66.5	6.9	71.9	14.0	76.1	78.3	80.6	82.7	84.3	81.2	5. 5. 5.	

A-11-1-2

[Phang Hen]	
STORY	
100	
EATER TRANSMISSION AND DISTRIBUTION COST STODY (Phang New)	
(BY)	
TRANSMISSION	
EATER	

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0.6	
	1992 1993 1994 1995 1996 1997 1998 1998 2000 2001 2007 2002 2003 2004 2052 2059 2073 2093 2005

Bote: Peaps is designed for Gda (Daily Arerage Demand)
8. Energy Consumption (KMh) = Ho.of Pumps x Notor Output(KM) x 24 h/day x (actual daily demand(gla//max.capacity of pump(8))
9. Demand Charge = Baht 223 /KW/arx Energy Consumption (7) KW
Energy Charge = Baht 1.23 /KWh x Energy Consumption (6) KWh/day x 365 days/year
Besign Pump Head-[Head Loss of Fipeline)+(Actual Read for M.E.1)+(Fump Head 1.5 m)

DA : Daily Average DR : Deily Marigum

Electricity Pee = Mate of Provincial Electricity Authority(PEA) for Warathised as of January, 1969.

Chemical Cost

4.05 /kg 1.25 /kg 15.60 /kg Alus :Baht Line :Baht Cl gas :Baht

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