

The native structure of rock including bedded plane, joint plane and schistosity appears different characteristic between granite area and Paleozoic area. As mentioned above, the granite is generally recognized as a uniform rock mass with a few discontinuous plane. The zone of dense joint plane however develops sparsely at the margin of intrusive.

On the other hand, Paleozoic member is marked by distinct bedded plane with secondary schistosity. The bed commonly shows the ENE to WSW strike with high angle dip, and is noted by thin bed repeating in several centimeters. These directions are also reflected to the topography, depressions or cols on ridge arranging in similar direction to strike, and the structure also forms a linear weathering zone along softer facies example for slaty horizon. The schistosity of them commonly does not develop fully due to the low metamorphic grade.

The proposed dam axis situate on the hard sandstone horizon of Paleozoic member. The dam axis is in the same direction to the strike of them so that the most part of dam foundation are occupied by a similar rock facies as well as hard sandstone drilled by boring survey.

(4) Permeability and Bearing Capacity

Permeability and N value of site are summarized at Figures 5-4-8 and 5-4-9. The permeability test reveals low permeability condition less than 5 Lugeon in bedrock without near the saddle dam.

The CT-4 borehole was drilled in the slope stretching from the abutment of saddle dam. The permeability data of CT-4 shows a changeable value ranging 1 to 35 Lugeon with high permeable peak at 28 to 33 m depth. These permeable layer is also correlative with the upper horizon of bedrock which is observed as a slight sign of weathering such as an oxidation along cracks. The general tendency of permeability however is decreasing toward lower horizon below 2 Lugeon while the upper horizon of 15 m depth is in over 10 Lugeon.

Bearing capacity is obviously different between overburden and bedrock. The overburden usually shows very small bearing capacity which is indicated smaller than 20 blows as N value. The minimum value of 0 to 1 blow were observed at the center of dam axis, and is in the clayey layer where intercalates above the weathered bedrock. However, the N value abruptly increases toward the lower horizon made up of weathered bedrock to over 50 blows which is sufficient for the bearing capacity for dam foundation.

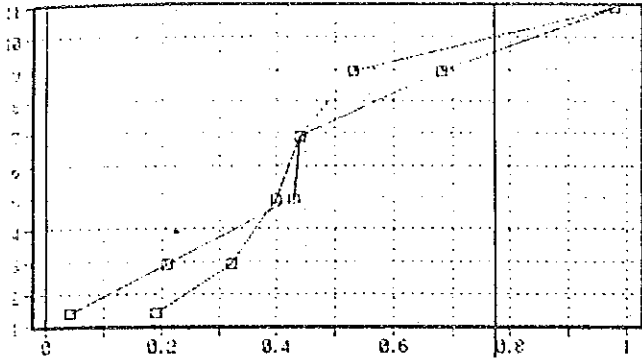
5.5 Khlong Katha

(1) Topography

The topography is characterized by two significant lineations which are expressed by two major valleys running through NW to SE and NE to SW. Furthermore, they are both noticeable for that convergence at the dam axis. The bird's eye views of Figures 5-5-1 and 5-5-2 distinctly shows these characteristics and that the proposed dam site is situated on a position at the intersected two valleys. Accordingly, the low hill extended from mountain remains at the center of dam site.

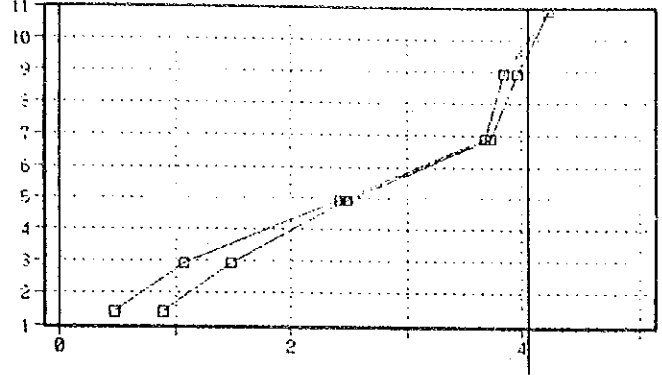
P-Q CURVE OF LUGION TEST OF CT1-1

$Lu = 0.77$



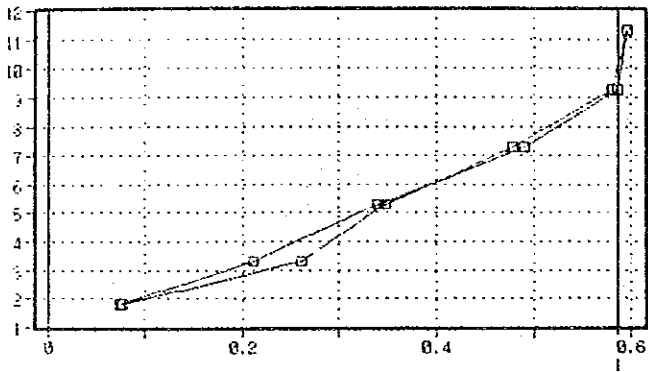
P-Q CURVE OF LUGION TEST OF CT1-4

$Lu = 4.1$



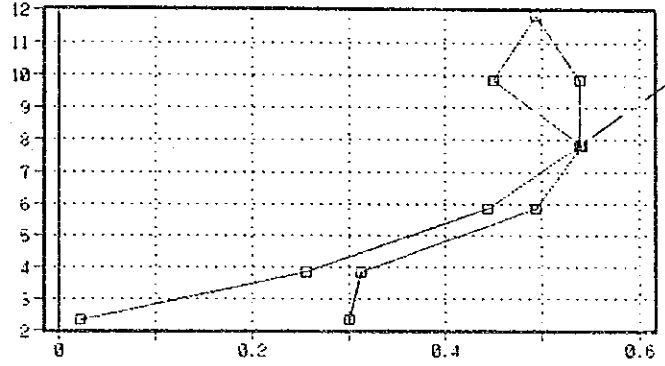
P-Q CURVE OF LUGION TEST OF CT1-2

$Lu = 0.58$



P-Q CURVE OF LUGION TEST OF CT1-5

$Lu = 0.69$



P-Q CURVE OF LUGION TEST OF CT1-3

$Lu = 1.7$

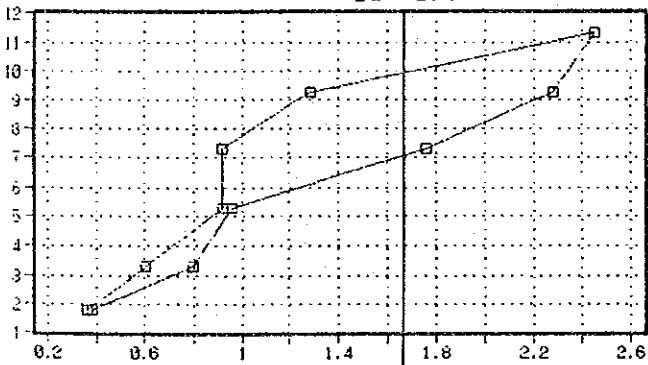
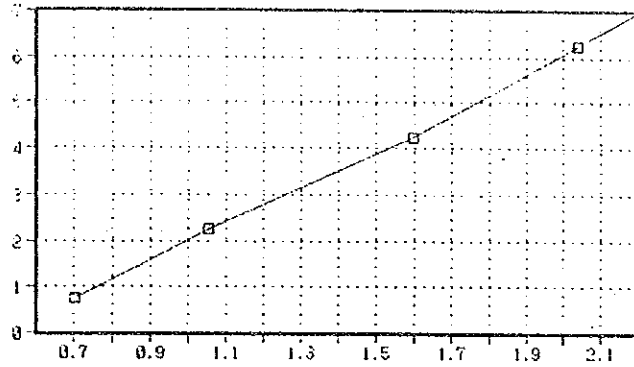
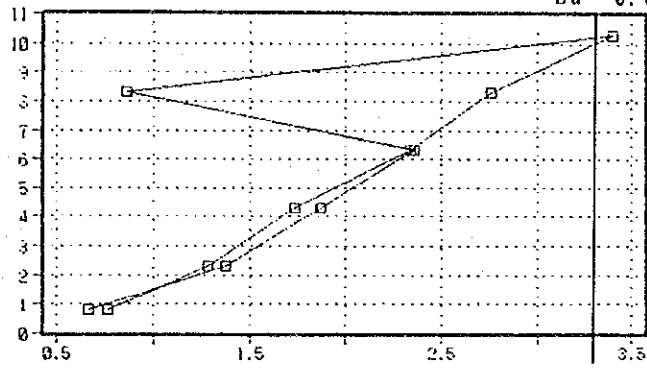


Fig. 5-4-9-1 Result of Lugion Test of CT-1

P-Q CURVE OF LUGION TEST OF CT2-1 $Lu' = 3.3$



P-Q CURVE OF LUGION TEST OF CT2-2 $Lu = 3.3$



P-Q CURVE OF LUGION TEST OF CT2-3 $Lu = 3.8$

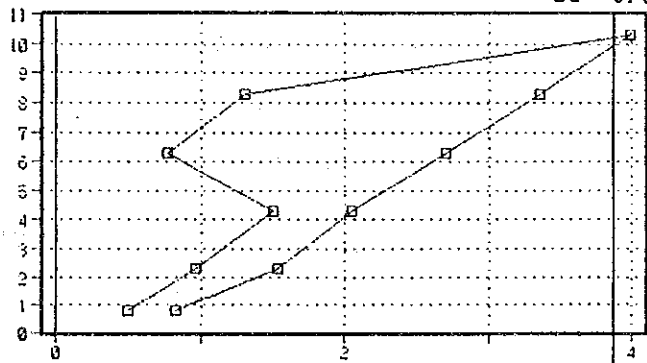
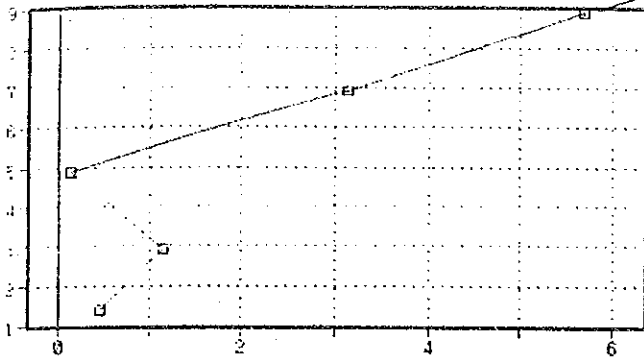


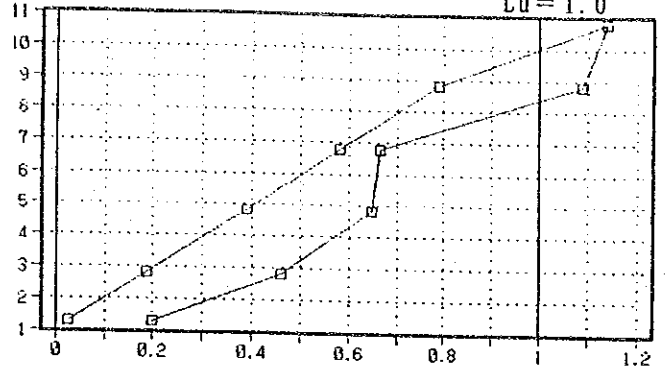
Fig. 5-4-9-2 Result of Lugion Test of CT-2

P-Q CURVE OF LUGION TEST OF CT3-1 $Lu' = 6.4$



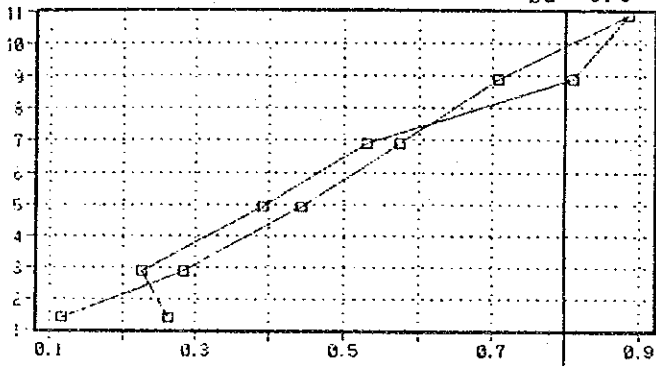
P-Q CURVE OF LUGION TEST OF CT3-4

$Lu = 1.0$



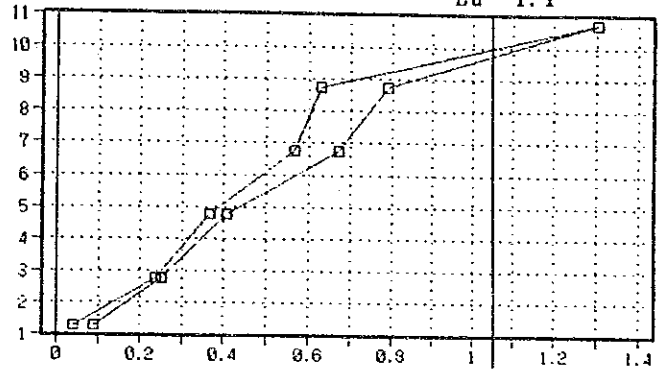
P-Q CURVE OF LUGION TEST OF CT3-2

$Lu = 0.8$



P-Q CURVE OF LUGION TEST OF CT3-5

$Lu = 1.1$



P-Q CURVE OF LUGION TEST OF CT3-3

$Lu' = 6.2$

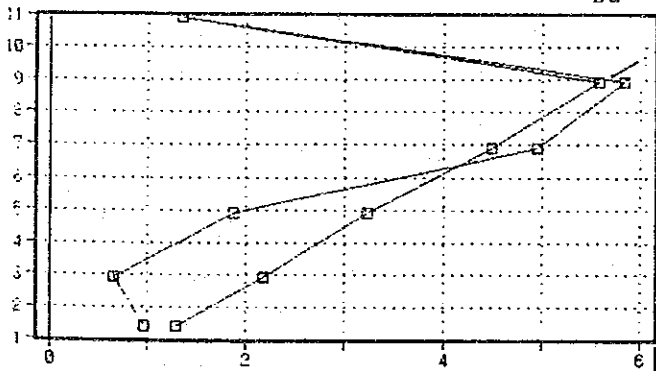
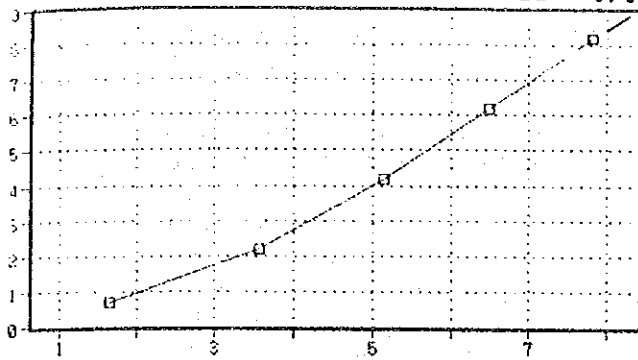


Fig. 5-4-9-3 Result of Lugion Test of CT-3

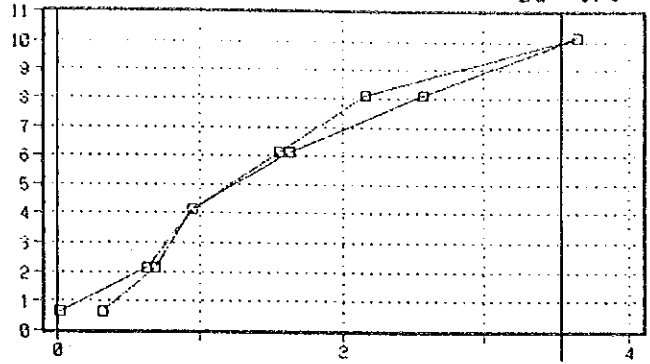
P-Q CURVE OF LUGION TEST OF CT4-1

$Lu' = 9.5$



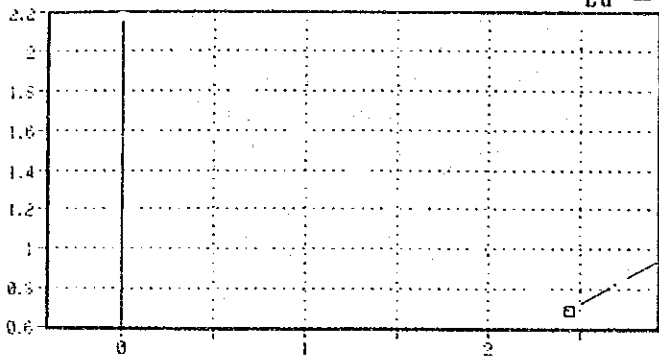
P-Q CURVE OF LUGION TEST OF CT4-4

$Lu = 3.5$



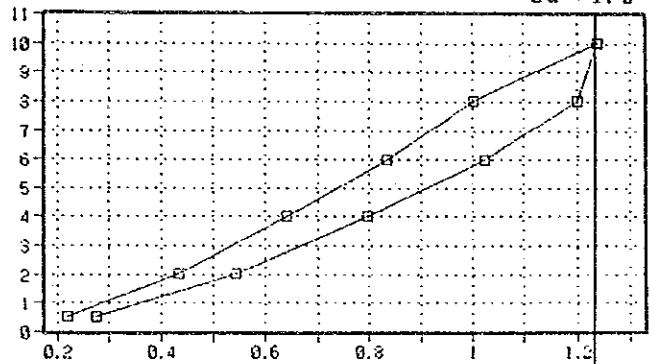
P-Q CURVE OF LUGION TEST OF CT4-2

$Lu' = 35$



P-Q CURVE OF LUGION TEST OF CT4-5

$Lu = 1.2$



P-Q CURVE OF LUGION TEST OF CT4-3

$Lu = 7.5$

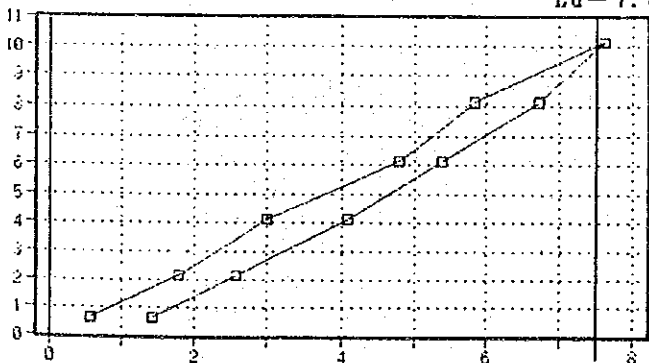


Fig. 5-4-9-4 Result of Lugion Test of CT-4

In accordance with above topographical feature, the lowest elevation at dam axis at about 24 m was seen at the both sides of river floor. The intermediate area of river floor is occupied by the low hilly mound with the relative height of about 5 m. As shown in Figure 5-5-5, the profile along dam axis also indicates the sharp boundary to sloping area from the river floor, so that the shape along dam axis is classified as a flat-floored to kastental valley.

The highest point in the watershed is shown at the northern summit of divide with the elevation of 400 m. On the other hand, the distance from dam axis to this summit is only 2.5 km. Therefore, the average gradient of slope is calculated to be 16/100. As shown in the longitudinal section of Figure 5-5-6, the detail profile along stream reveals the steeper slope than average gradient on the mountain area locally. The steepest slope is seen on the higher part above 100 m. The gradient is over 30/100 in contrast to lower part consisting of the flat river floor and terrace plane.

The terrace plane formed at several elevations are also observed in the watershed. The lowest terrace is situated beside the river bed and have a uniform plane flattened by sandy deposit at the elevation of 30 to 35 m. The higher terrace is on the side slope ranging the elevation of 50 to 75 m. Though the height of terrace position is variable individually, their topographical feature seems to be similar that the terrace deposit is not resting on plane and the eroded flat plane is only reserved on steep slopes. These terrace plane may be referred to as that the lower terrace and sandy deposit is corresponded to last transgression and that higher terrace is identical with an event of pleistocene eustatic changes.

(2) Overburden

The thickness of overburden is relatively thin in comparison with the other site as shown in Fig 5-5-8. The boring survey clarifies that its maximum depth is 13 m at the KK-2. The member of overburden however is same to the other site which is composed by top soil, sandy river deposit, sand and gravel terrace deposit and clayey deposit in order from the surface.

Top soil is marked by facies which is of the loose sand and gravel. It is underlain by a remarkable reddish layer. The layer mainly consist of lateritic clay, furthermore, the under horizon of this layer is made up of the sandy terrace deposit. Terrace deposit correlating with lowest terrace is marked by the coarse sand with granule gravel and contains the tin minerals which has been dug for mining by several decades. The clayey facies was locally observed just on the bedrock. Its distribution shows an aspect as the filling up the relief of bedrock up to the 23 m in elevation. The facies are mainly of very soft greyish clay with high plasticity, but the thickness is less than 5 m resulting from KK-3 located at the center of its distribution.

(3) Bedrock

The geological members exposed on the site are shown in the Geological map of Khlong Katha Dam Site of Figure 5-5-7. The members without above mentioned overburden are grouped into two lithological types of granite which are of the coarse grained biotite porphyry or granite and the fine grained biotite-muscovite granite.

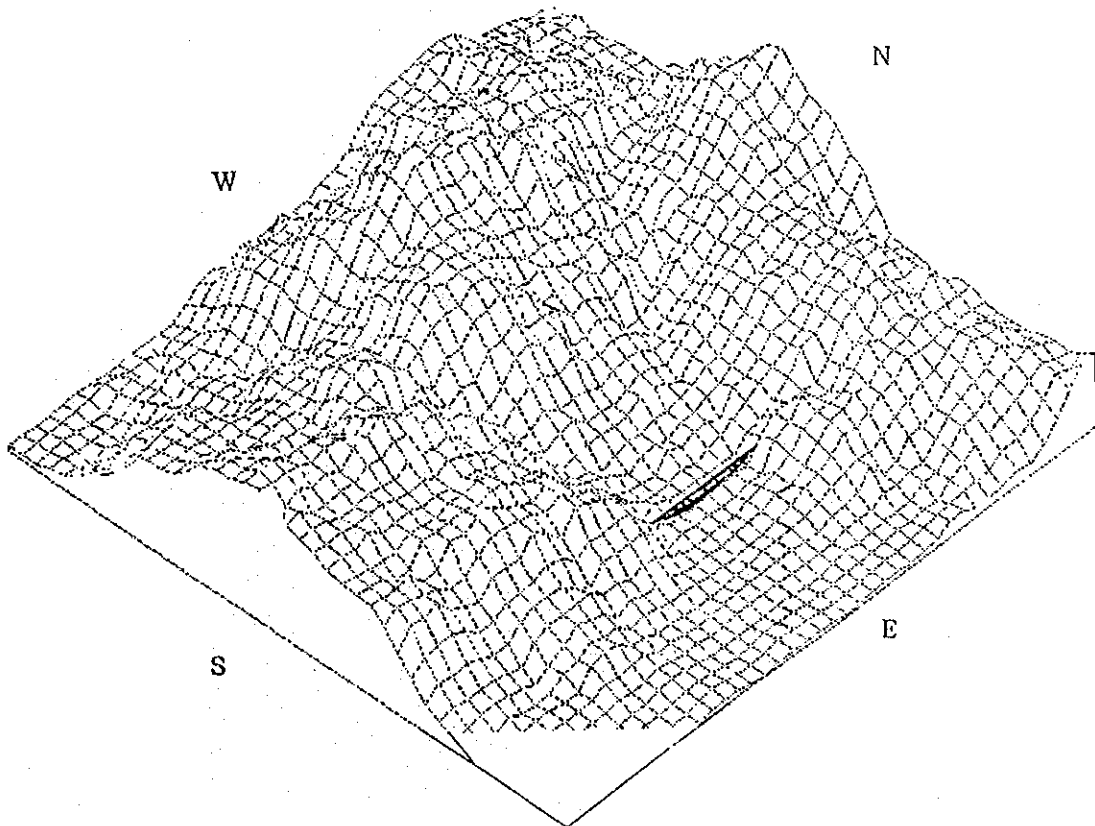
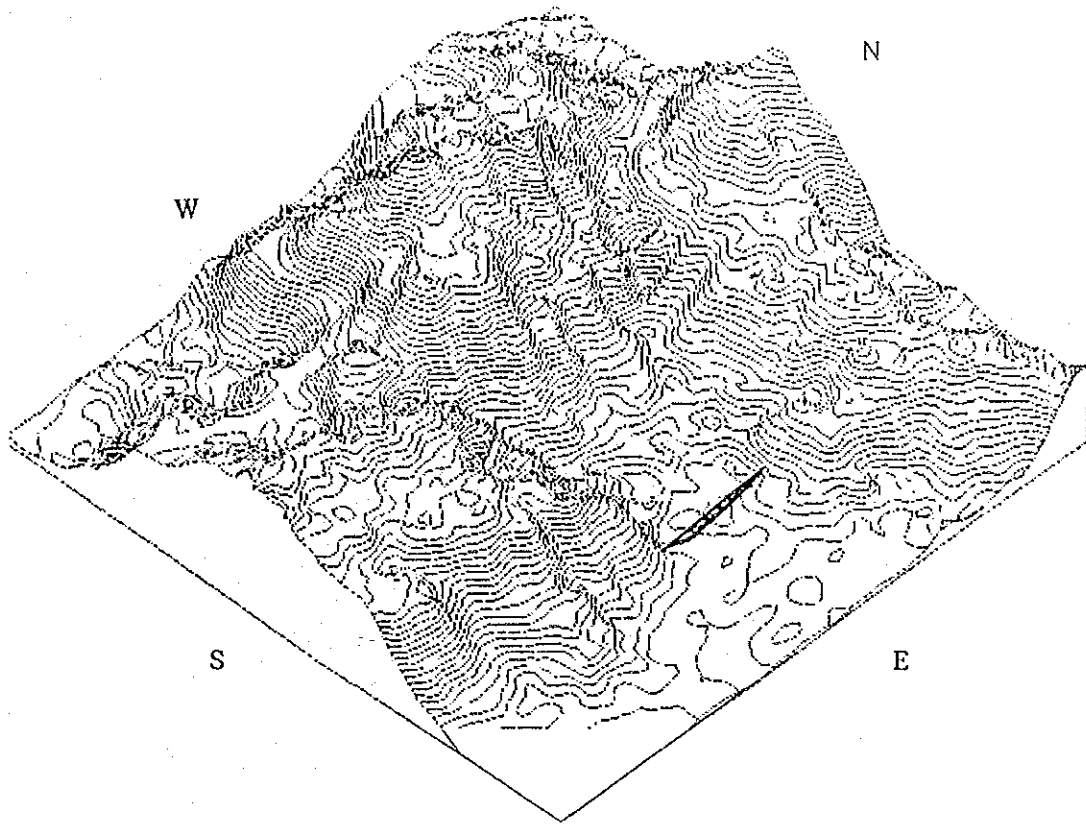


Fig. 5-5-1 Bird's Eye View of Khlong Katha Dam Site(1)

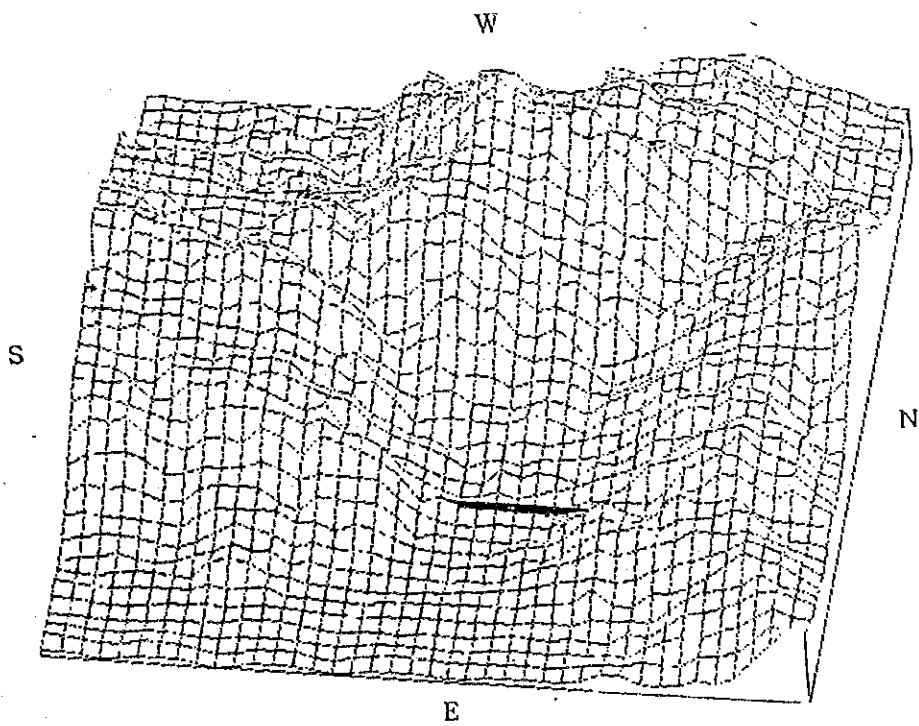
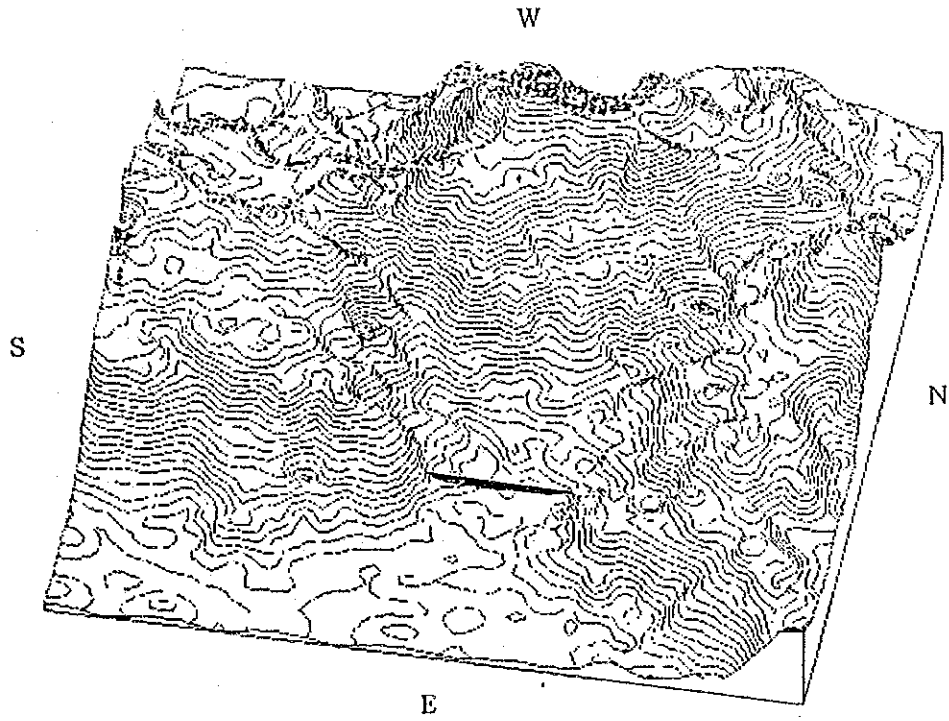


Fig. 5-5-2 Bird's Eye View of Khlong Katha Dam Site(2)
5-38

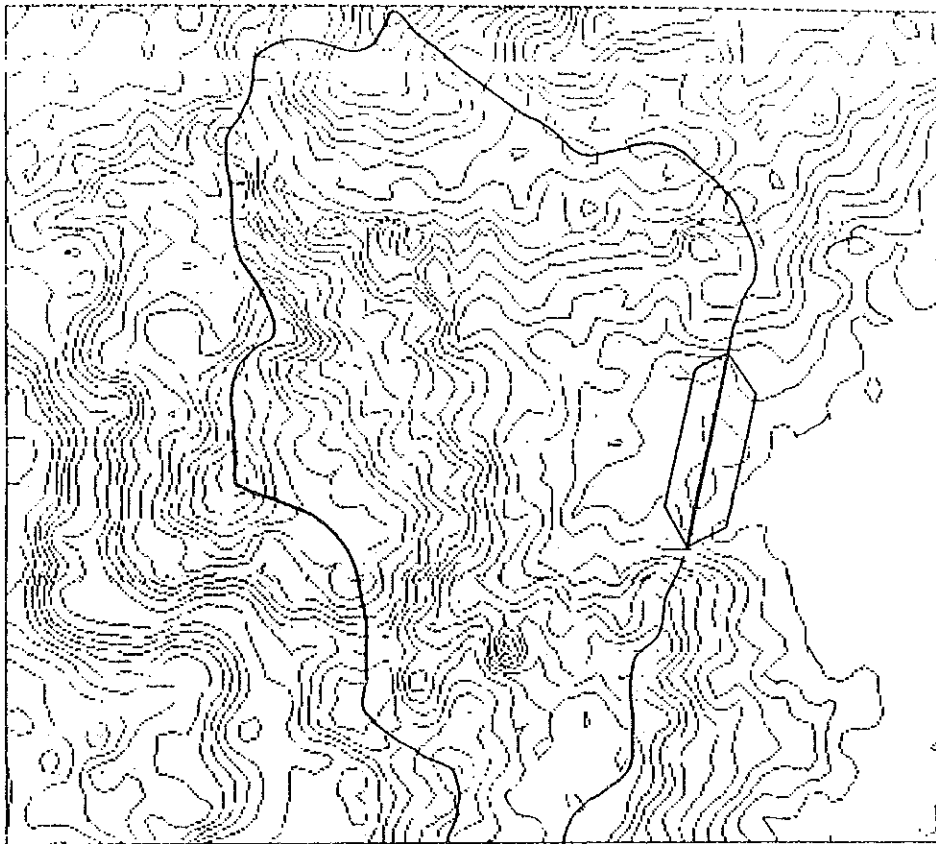


Fig. 5-5-3 Catchment Area of Khlong Katha Dam Site

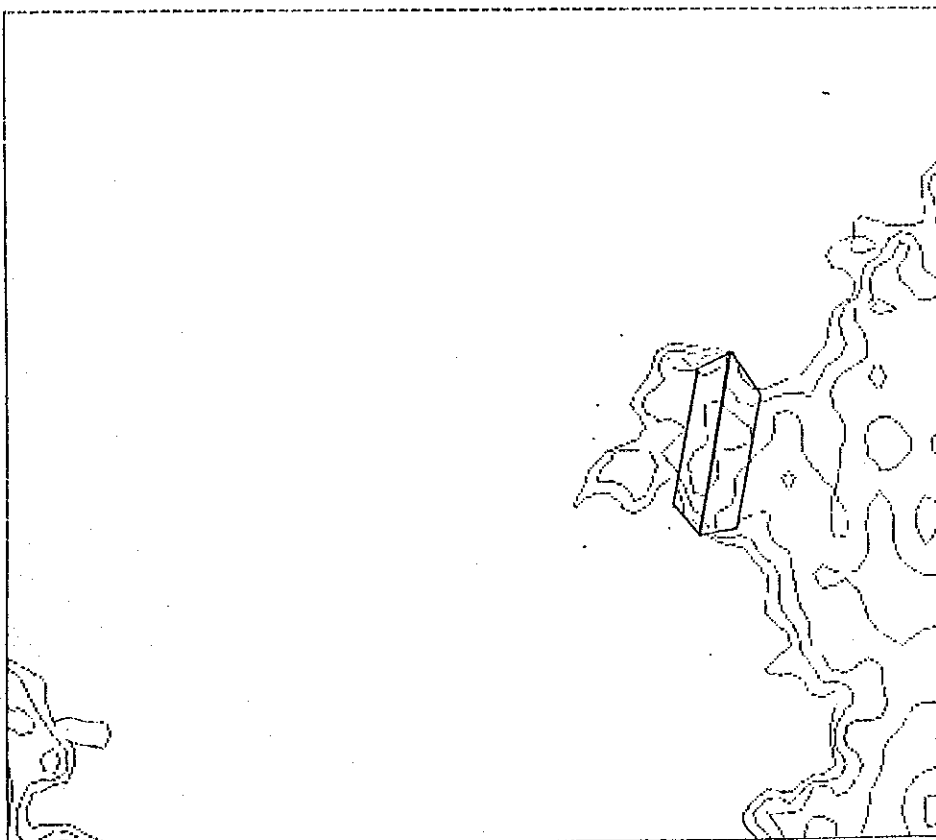


Fig. 5-5-4 Reservoir Area of Khlong Katha Dam Site

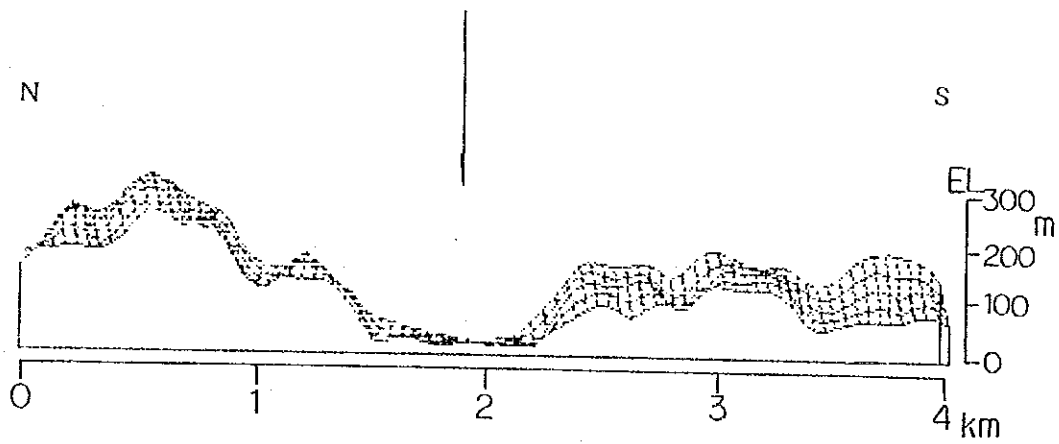


Fig. 5-5-5 Cross Section along Dam Axis of Khlong Katha Dam Site

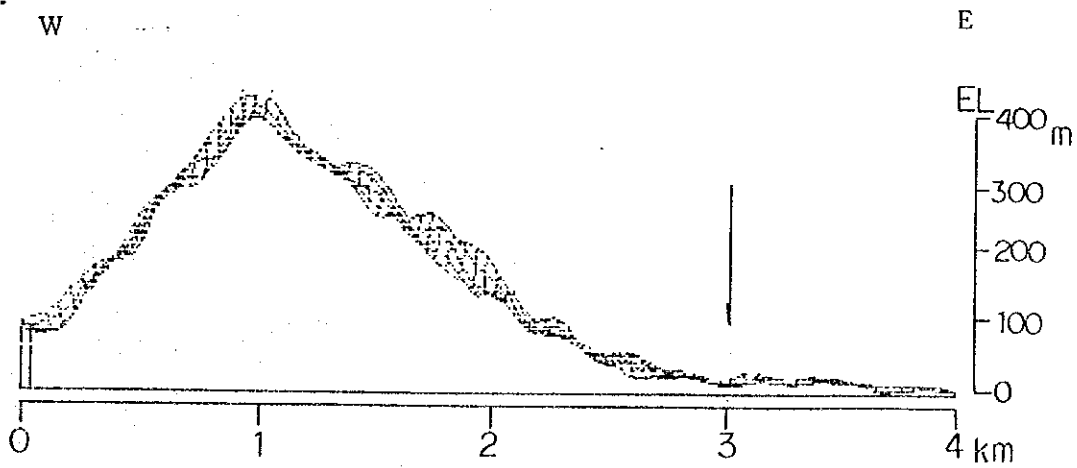
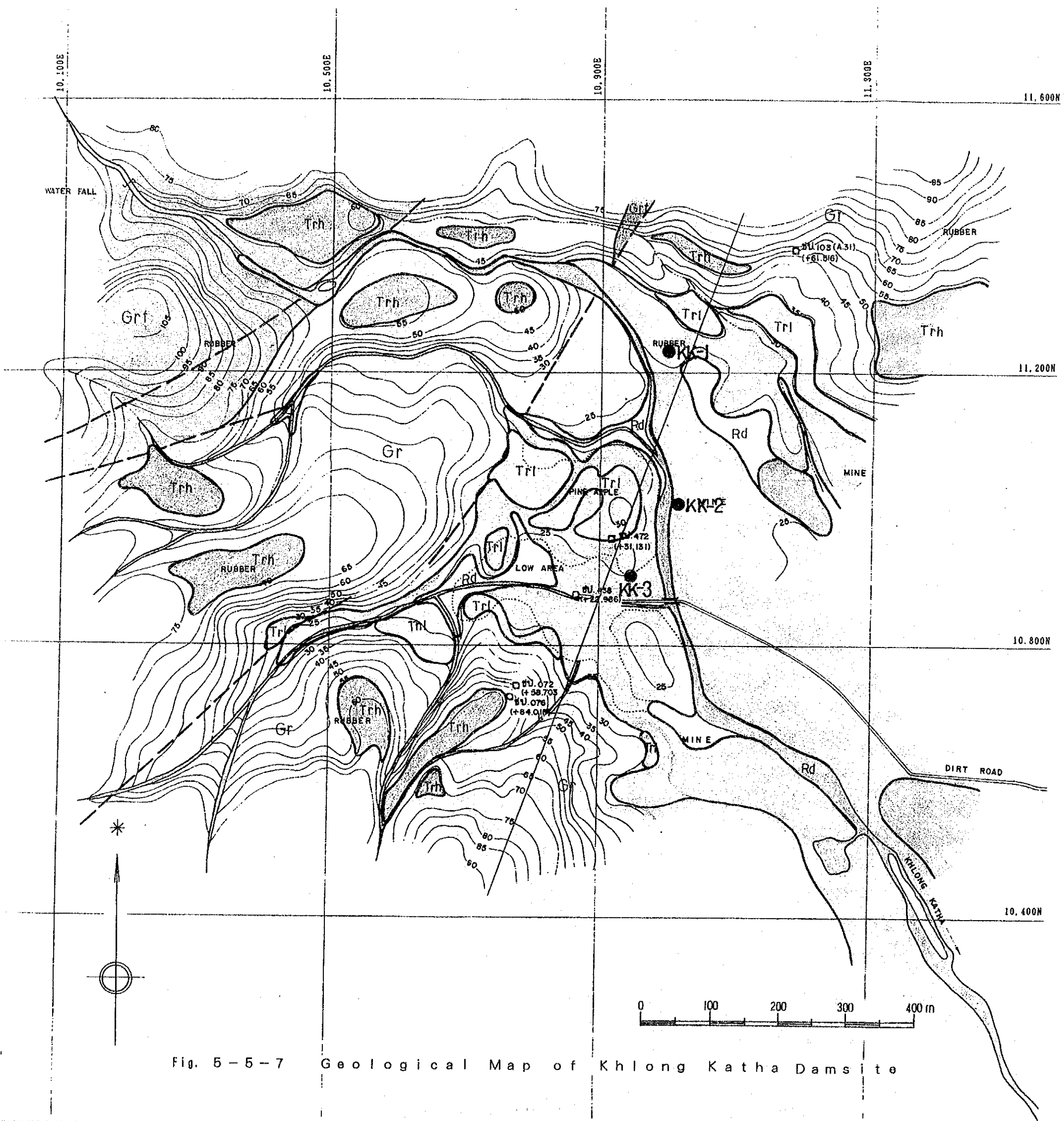


Fig. 5-5-6 Longitudinal Section along River Stream of Khlong Katha Dam Site



LEGEND

Rd	Alluvium	: Recent river deposit consisting of sand, gravel and silt. Swamp sediment of sand to clay
Tre	Erosional Terrace	: Recent river terrace made up of gravel, sand and silt
Trl	Lower Terrace	: Terrace deposit mainly composed of sand and gravel with thin clay layer
Trm	Middle Terrace	: Flat plain covering thin weathering material and sandy layer locally
Trh	Higher Terrace I	: Erosional plain on the sideslope at the elevation of 100-150 m
Trp	Higher Terrace II	: Highest erosional plain at the elevation of 200 m
Ta	Talus	: Talus and colluvial deposit consisting of various size materials
Ss	Sandstone	: Palaeozoic sandstone, bedded metamorphosed sandstone with chert and slate layer
Sl	Slate	: Slate with metamorphosed sand and chert layer
Gr	Coarse Granite	: Coarse grained biotite-porphyrritic granite
Gf	Fine Granite	: Fine grained biotite-muscovite granite

Fig. 5-5-7 Geological Map of Khlong Katha Damsite

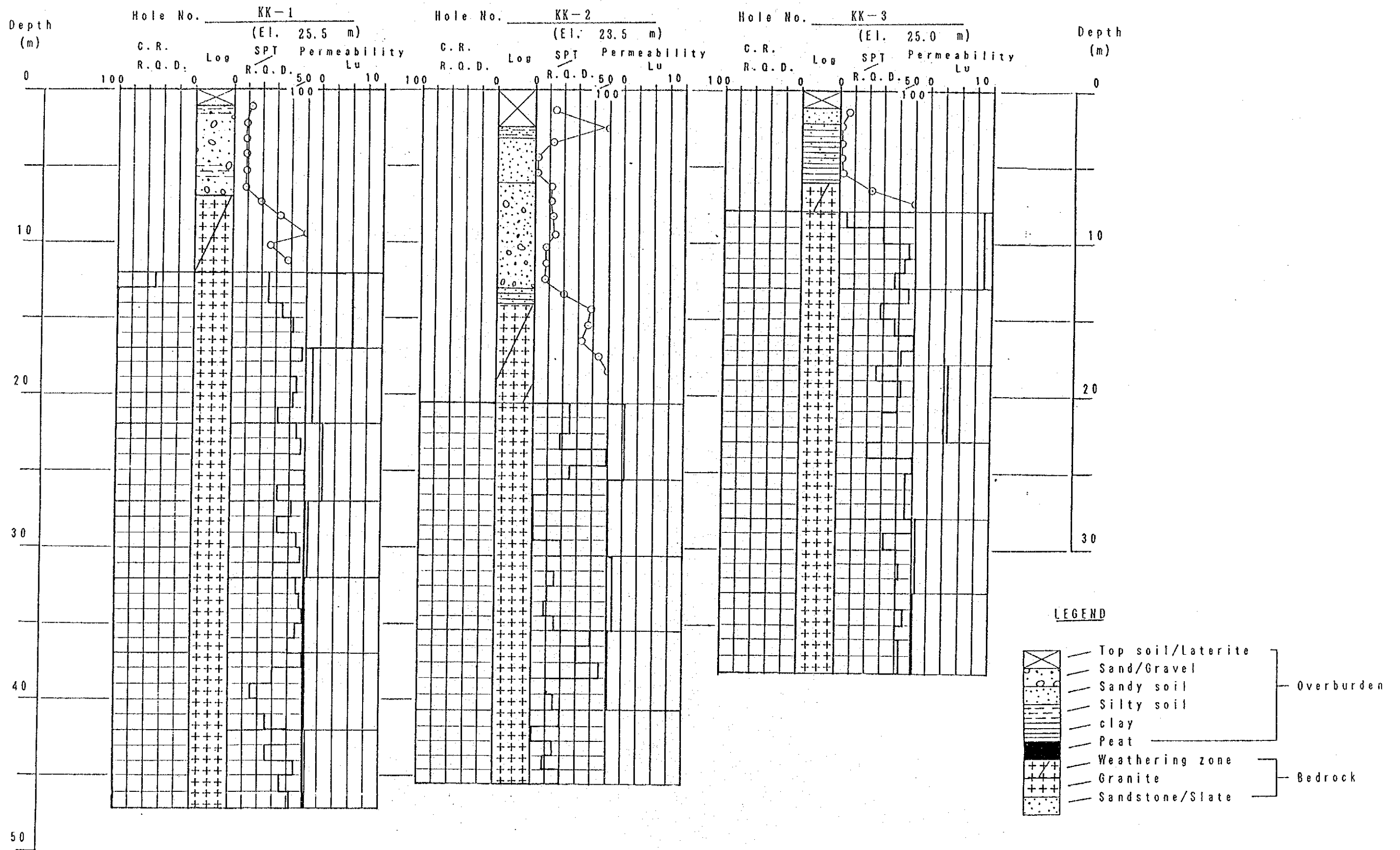


Fig. 5-5-8 Result of Boring Survey

The coarse granite forms the most part of the reservoir and south-eastern area of watershed. The rock mass is pronounced by the massive facies with minor intrusion of the granite-phophyry and quartz vein. According to the progress of boring survey, quartz vein is enable to readily distinguished from others since these facies are fairly harder for drilling than other layers. The facies' change occurs at the several meters in boreholes repeatedly.

The fine granite is marked by not only fine grained phonocryst but also high frequency of intrusive vein and remaining the bedded plane and joint plane. At the many outcrop these discontinuous planes well develop; consequently, the scarp or free face is formed in direction of these planes.

The weathering zone is also observed at both of boring core and outcrop. The weathering zone is less than 10 m and is of the light brown gravely facies with much content of clay. From the result of KK-2 which is located on the center of dam axis and have the thickest weathering zone, the sign of weathering was found out up to 24 m depth.

(4) Geological Structure

Two zones of faults are recognized in the field reconnaissance, one of which is running through the upstream of reservoir area and another is along the dam axis. Both roughly show the same trend of NE-SW to NNE-SSW and can be trace out in the watershed from the right slope to the left slope which does not across to the dam axis. Along these fault zone, the scarp or differential weathering is observed, Slight alterative layer is also interbedded along the contacted plane.

From the view point of general topographical property, two large lineations across the dam axis and may be inferred as a structural valley as previously stated. However, neither boring investigation nor field reconnaissance survey revealed any evidence of a fracture zone. The quantity of investigation however has not been enough until the present for leading a conclusion that these lineations are not derived from a fault structure.

The primary structure such as the bedding or joint plane is not so many in the boring core while the watershed area locally presents a high frequency of discontinuous plane as already stated. Accordingly, it may be concluded that the bedrock making up of dam foundation does not involve any fracture and is built up of the massive rock facies.

(5) Permeability and Bearing Capacity

Permeability observed in borehole was very low and less than 9 Lugeon. Especially below the 18 m depth, the permeability was further lower around 1 Lugeon. The bearing capacity in overburden deposit normally indicates small value which is less than 1 converting into N value.

The result of permeability test is shown in Figure 5-5-9, the permeability of bedrock varies from 10 to 0.1 Lugeon which is differed in degree between the upper horizon and the lower one. The upper horizon, generally above 20 m depth, finding out a sign of weathering is

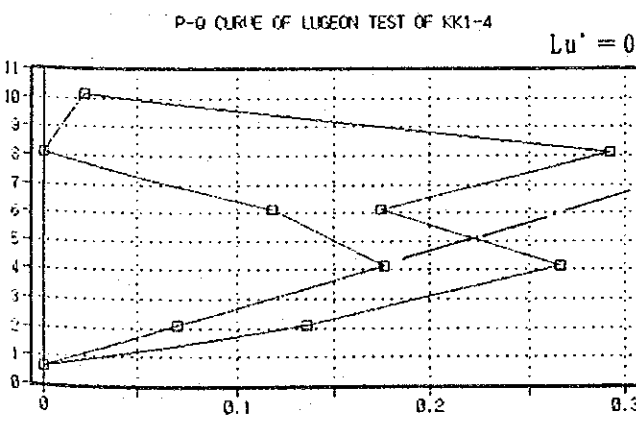
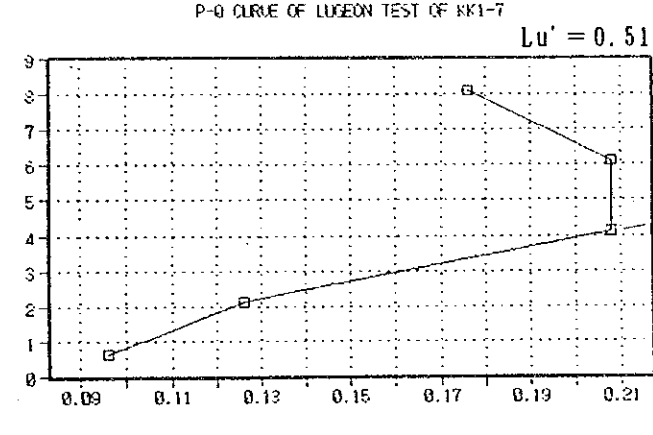
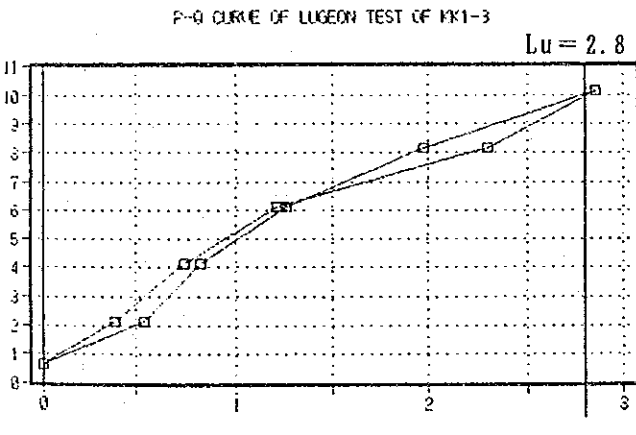
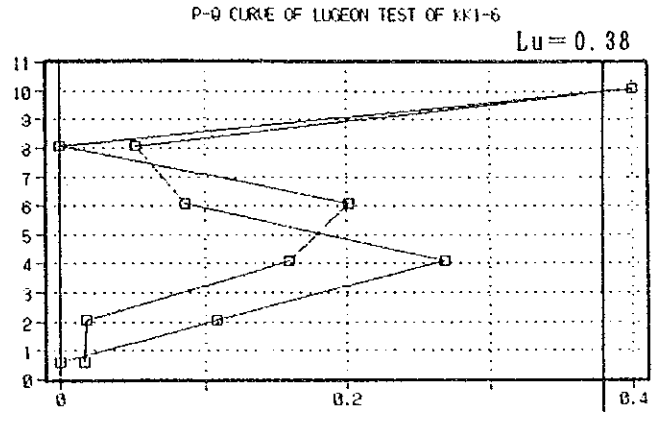
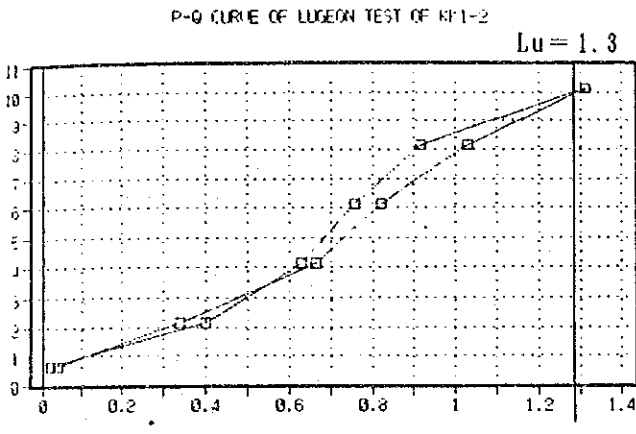
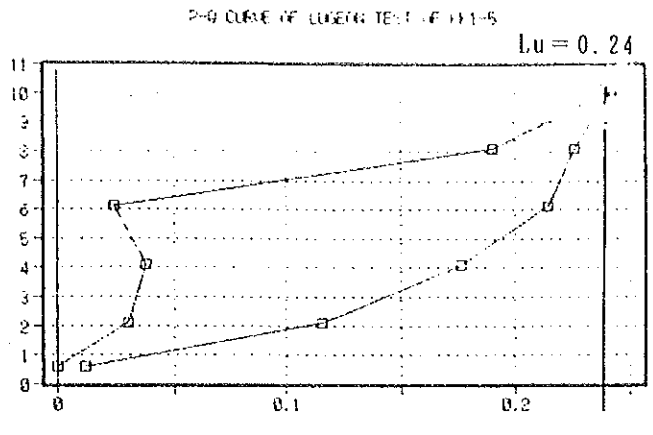
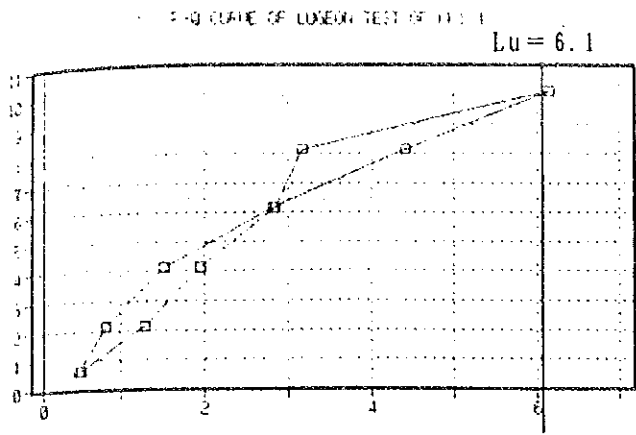
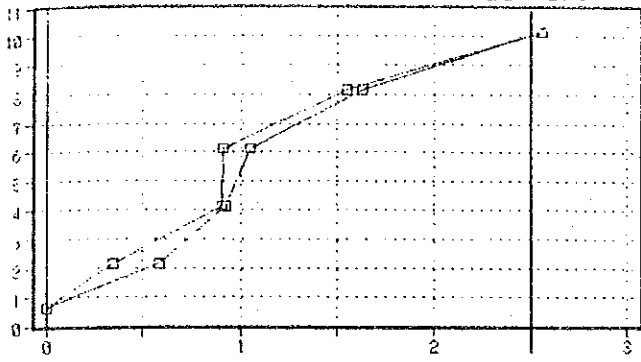


Fig. 5-5-9-1 Result of Lugeon Test of KK-1

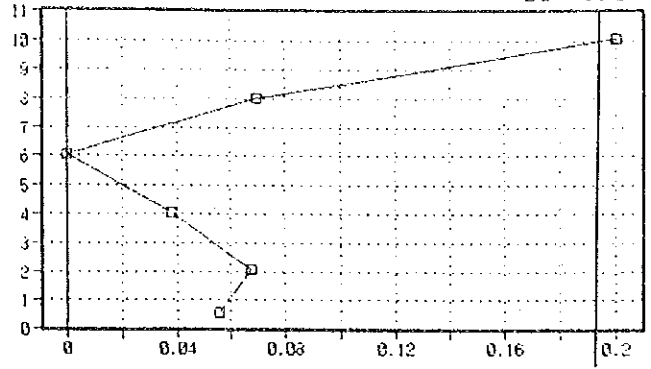
P-Q CURVE OF LUGION TEST OF 112-1

Lu = 2.5



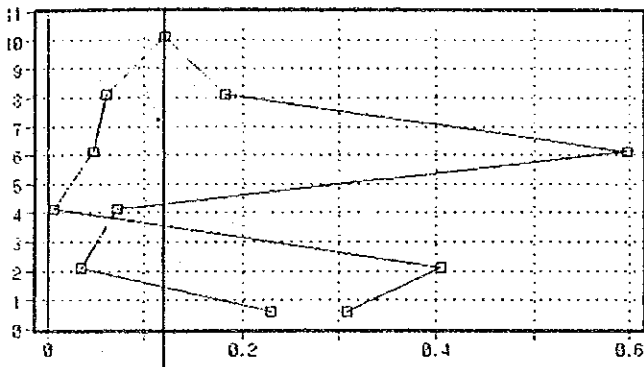
P-Q CURVE OF LUGION TEST OF 112-4

Lu = 0.19



P-Q CURVE OF LUGION TEST OF 112-2

Lu = 0.12



P-Q CURVE OF LUGION TEST OF 112-3

Lu = 0.86

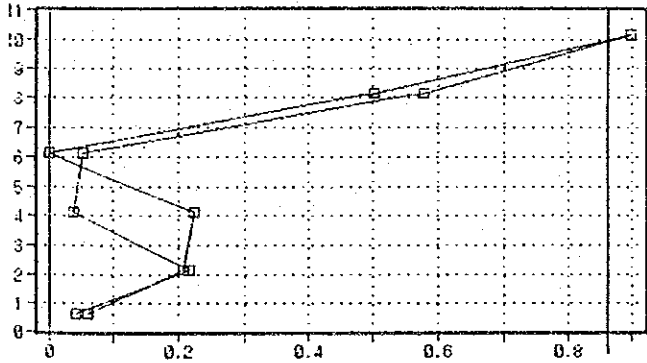
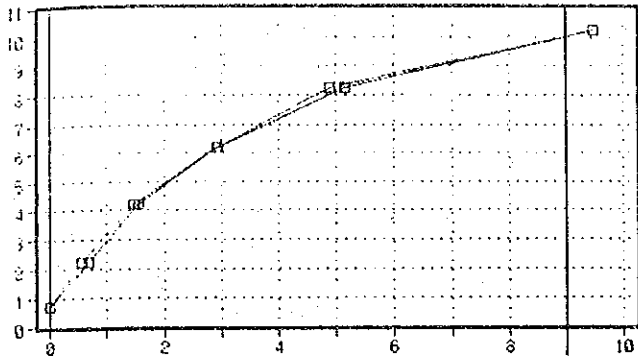


Fig. 5-5-9-2 Result of Lugion Test of KK-2

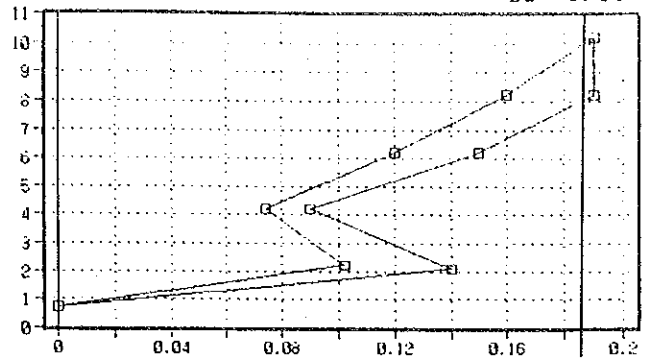
P-Q CURVE OF LUGION TEST OF KK3-1

Lu = 9.0



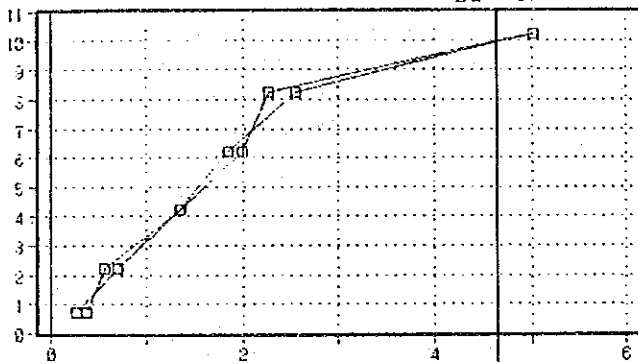
P-Q CURVE OF LUGION TEST OF KK3-4

Lu = 0.18



P-Q CURVE OF LUGION TEST OF KK3-2

Lu = 4.6



P-Q CURVE OF LUGION TEST OF KK3-3

Lu = 0.61

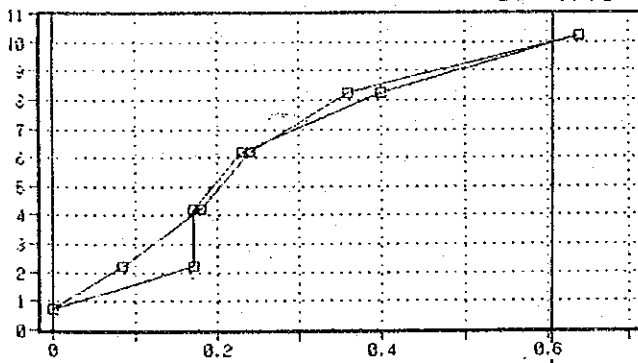


Fig. 5-5-9-3 Result of Lugion Test of KK-3

correlative with high permeability zone over the 5 Lugeon. In contrast to these, the lower horizon shows very low value indicating smaller than 1 Lugeon. Especially below the 25 m depth where any oxidative pollution by weathering was not found out in the boring core, the layer looks like a impervious rock mass.

Although the bearing capacity in overburden is very low, the layer showing low N value is so thin as usually presenting about 10 to 15 m. The minimum value observed in the site is obtained from the clayey horizon situated between weathering granite and sandy terrace deposit. Only 0 to 1 blow was observed there. It is increasing to both directions of the upper and lower horizon. Toward the lower facies of weathering granite, the bearing capacity becomes increasingly a large value more than 50 blows while it only increases up to 15 blows even in maximum value toward the upper facies of the terrace deposit and top soil.

5-6 Bang Tho Sung

(1) Topography

The Bang Tho Sung site is situated beside the Bangwad Reservoir. Its topographical aspect pronounced by the narrower river bed than the other proposed sites. As visualized from the bird's eye view of Figure 5-6-1, the site represents a trough shaped valley which involves the flat riverbed having longer axis along the stream and contrastively narrow width of riverbed. The planned dam length is less than 600 m which is the shortest among all sites in this study. However the short dam length, the reservoir occupies an area stretching out over 800 m to the upstream.

The lowest elevation is seen near the center of dam axis and is represented with the elevation about 30 m. The wide flat spreads out with similar elevation of dam axis to the down area and also the flat extends to 500 m to the upstream. The abutment slope have considerably steep gradient in contrast with these of low flat and continues to the the ridges forming the divide of watershed. The highest summit making the divide is presented with the elevation over the 450 m. As illustrated longitudinal and cross profile in Figures 5-6-5 and 5-6-6, the slope forming the ridge to low flat shows a peculiar form of the convex in the upper and contrastively concave in the lower slope ,and is marked by a modification of the several terrace plane resting on it. The average gradient of slope is estimated about 30/100. The terrace planes can be classified into three groups depending on their elevation respectively.

The lower terrace is at the elevation below 45 m and is located at the surroundings of the river bed. The middle terrace is at the elevation of about 50 m; the higher terrace is at 75 m. The deposit making the terrace surface is only shown on the lower one. The others are formed with an erosional plane as well as that of other dam site. Their origin is same to other site. Therefore, they are resulted from the Pleistocene sea level changes.

(2) Overburden

Since the boring survey was not conducted until the present, the overburden in the site only observed at the surface by the field

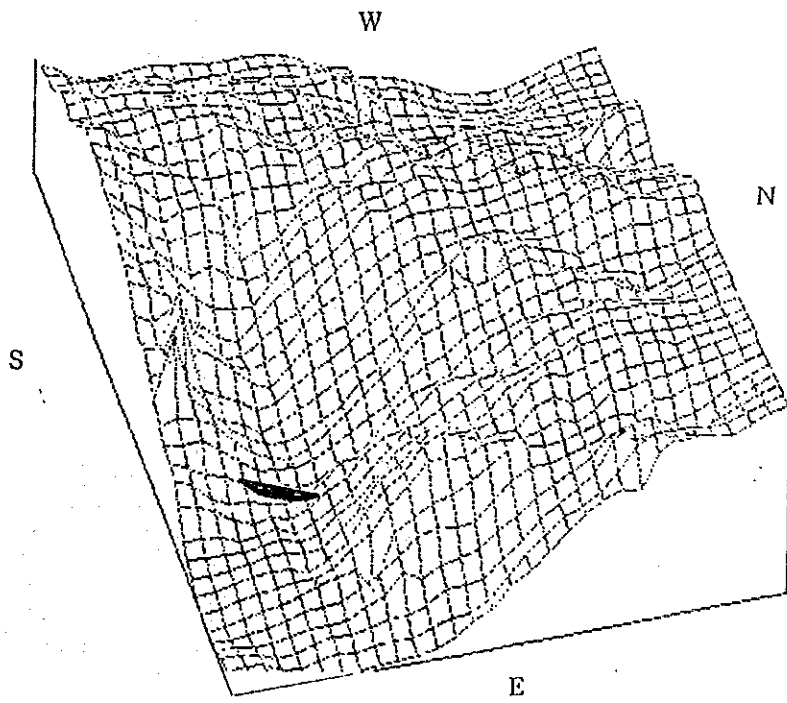
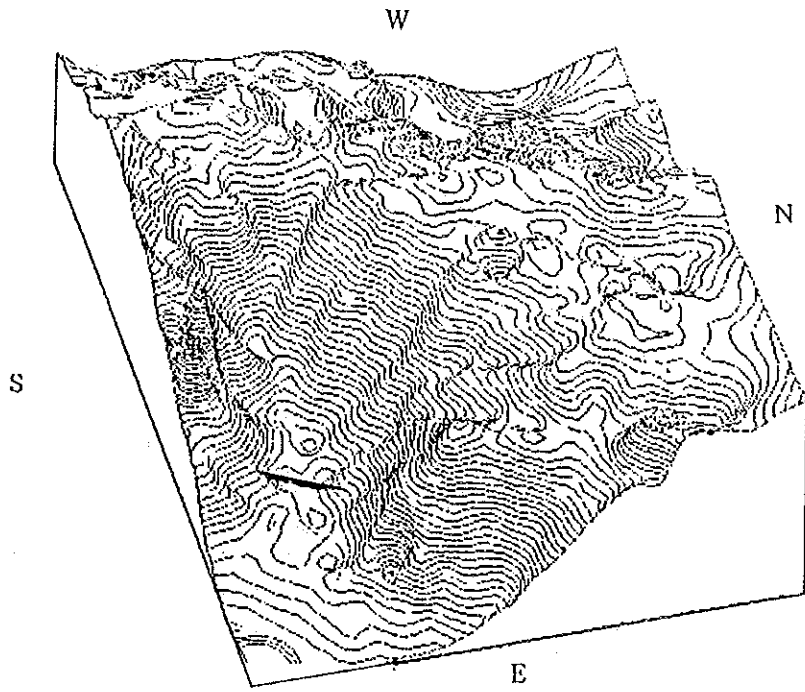


Fig. 5-6-1 Bird's Eye View of Bang The Sung Dam Site(1)

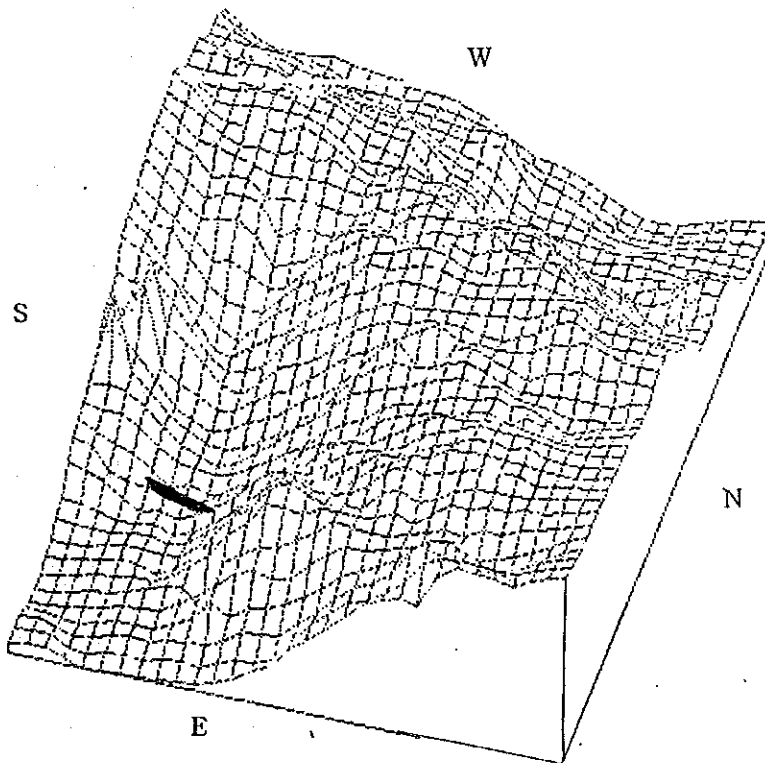
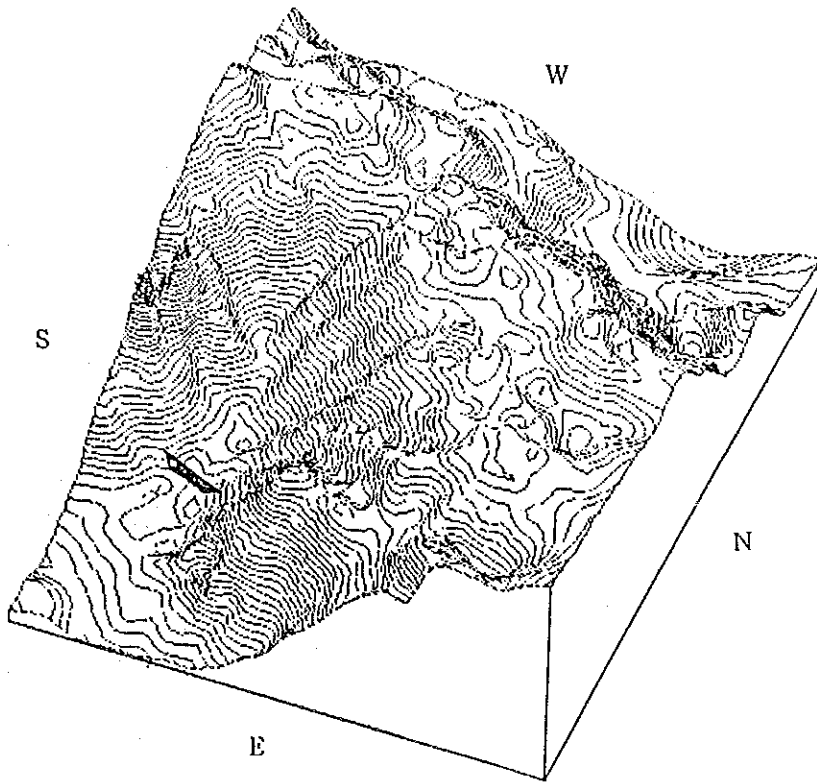


Fig. 5-6-2 Bird's Eye View of Bang The Sung Dam Site(2)

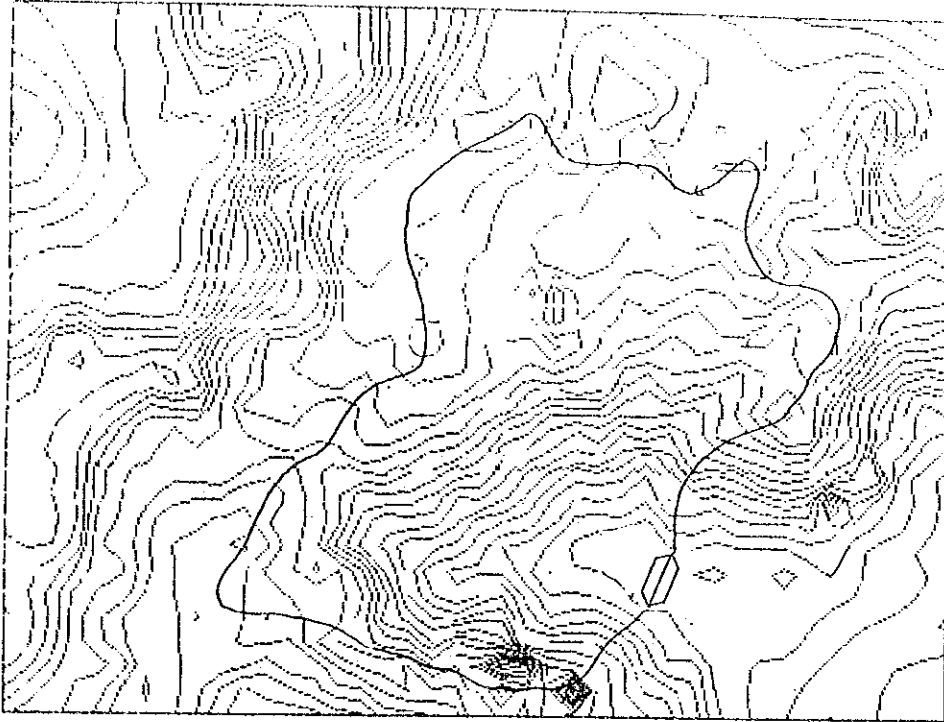


Fig. 5-6-3 Catchment Area of Bang The Sung Dam Site

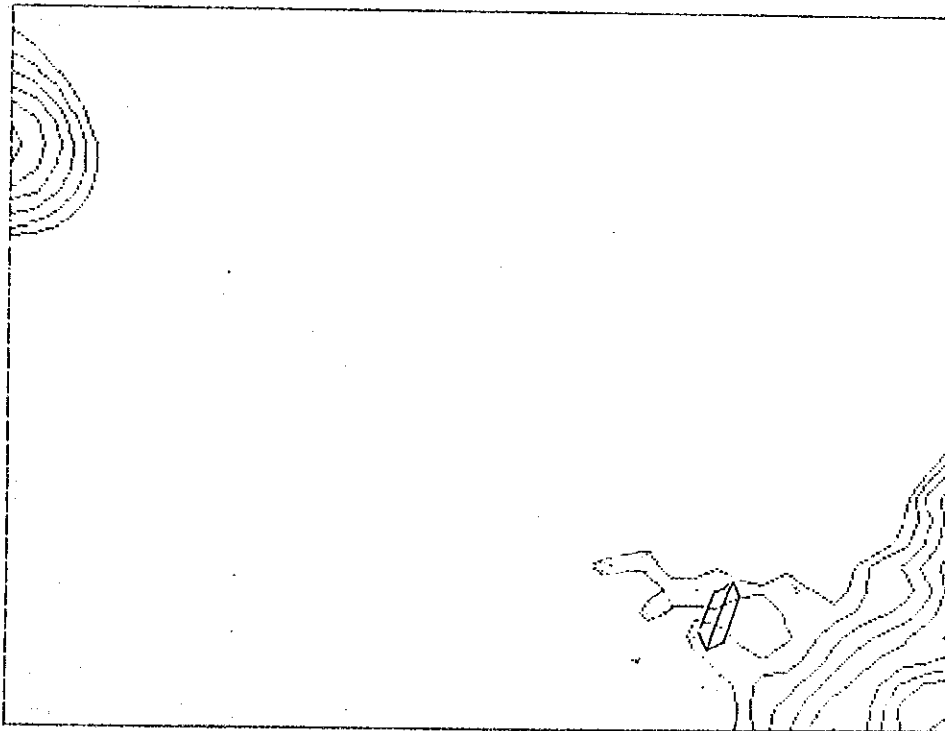


Fig. 5-6-4 Reservoir Area of Bang The Sung Dam Site

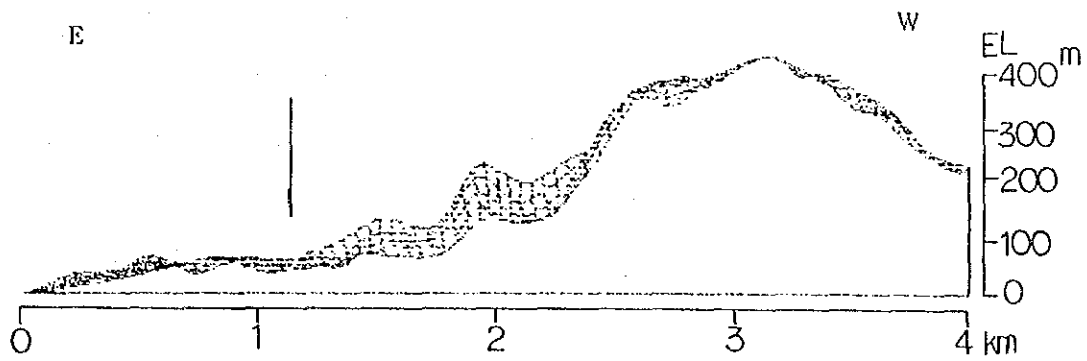


Fig. 5-6-5 Longitudinal Section along River Stream of Bang The Sung Dam Site

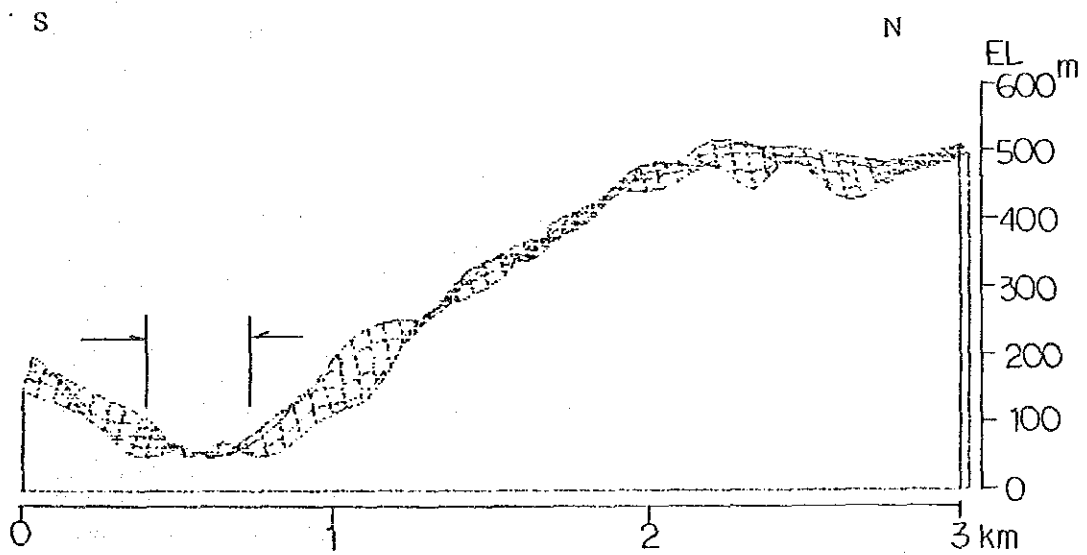


Fig. 5-6-6 Cross Section along Dam Axis of Bang The Sung Dam Site

reconnaissance. The member recognized in the surface is describable as three layers inclusive of Recent river deposit, lower terrace deposit and talus deposit in the order from the upper horizon.

Recent river deposit and lower deposit mainly consist of the loose sand and gravel facies, and spreads widely out the down of dam axis. The talus deposit is tractable to the foot slopes, especially between the constant slope and waning slope. It covers thickly on the surface. And the component of tulus deposit is mostly formed of the lateritic clay with gravel. Although the stratigraphical order for the overburden is still unknown, the layer of tulus facies plunges beneath the overlaying terrace deposit at the center of valley.

When the whole geological condition is supposed to be similar to Bangwad reservoir or Bang Nio Dam site, the facies of overburden can be classified into above stated three members a total thickness of which may be inferred to be reached up to 20 m locally ranging from top soil to weathering granite.

(3) Bedrock

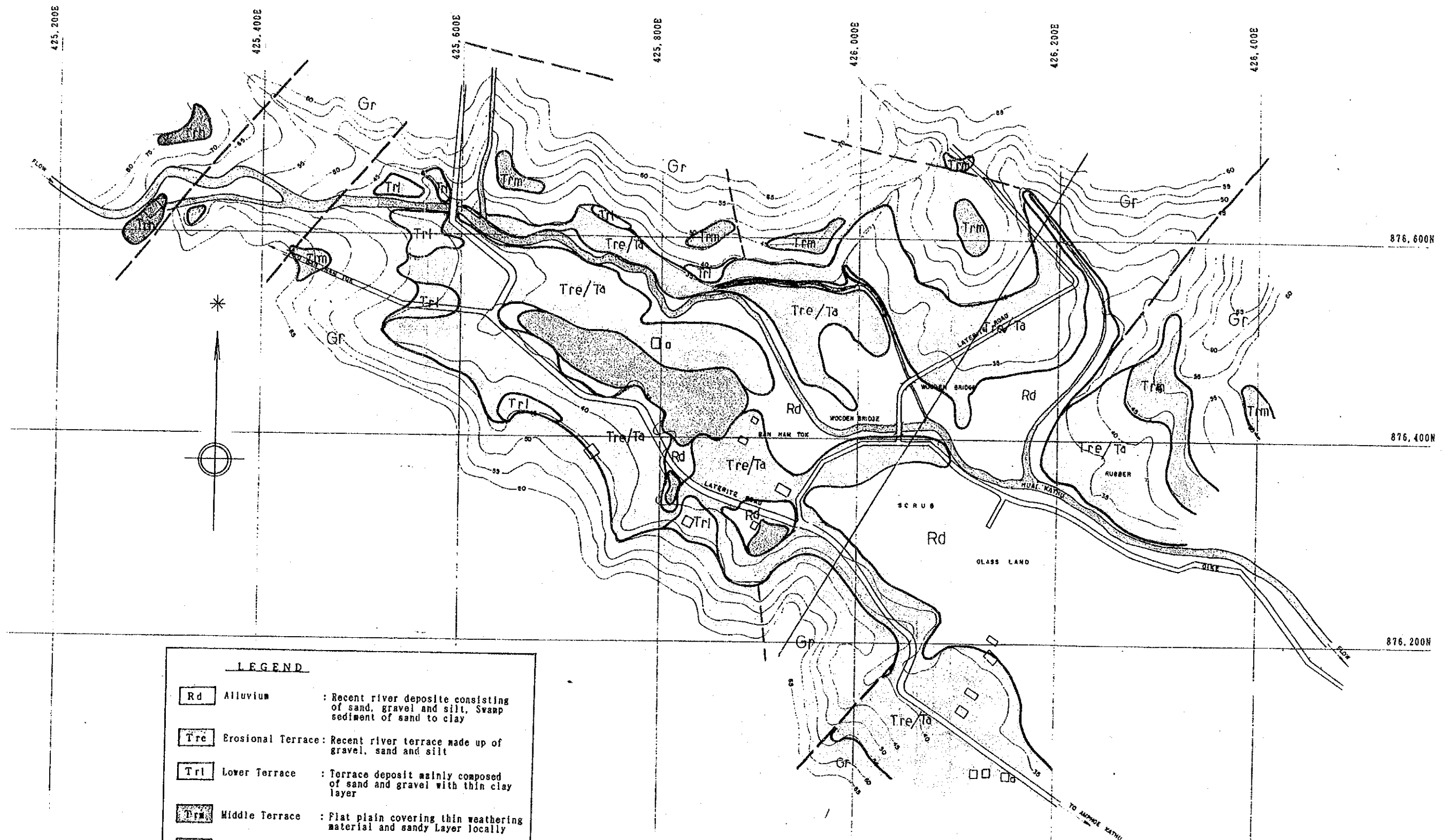
The bedrock of site is of granite which is classified as coarse grained biotite-phophyrific granite. Whole watershed area is formed of same type of granite; the rock is noted by uniformed mass with a few joint plane. The joint well develops locally for example, along a distributary at north of watershed. The distinct joints arranging in direction of ENE with 0.5 to 1 m interval is observed along a stream. The huge exposures are scattered on slope and fresh rock was observed there. The frequent quartz veins run through these outcrops on the southern slope of the reservoir.

However, the weathered zone in which the rock completely alter to sapolite is also found out at many places. Its thickness can hardly be grasped obviously since there is no boring survey. In case that the weathering condition seems to be equivalent to the Bang Nio Dam site, the weathering zone is inferred to be at least 5 m on fresh granite facies.

(4) Geological Structure

The fault trending to the NE-SW is expressed as the main structure in the site which is the same faulting system as the regional lineation. In consequence of practical survey within the watershed, two faults in accordance with NE-SW trend are traceable downstream and the upstream respectively. Both faults are also supported by topographical characteristics such as cols on ridge, linear valley and fault scarp. They are correlative with next lineations recognized in the surrounding area at the site.

The secondary structure is shown along the direction of WNW-ESE and N-S which are interrupted by the main structure above. From the result of field reconnaissance, the lineations along them are dominant on the northern slope of reservoir and are topographically marked by a continuous fault scarp in many places, while most of these lineations are intercepted by the main structure. In view of geological structure, it may be assumed that a part of them passes the dam foundation, nevertheless the secondary faults are not continuously crossing by a main fault.



LEGEND

Rd	Alluvium	: Recent river deposit consisting of sand, gravel and silt. Swamp sediment of sand to clay
Tre	Erosional Terrace	: Recent river terrace made up of gravel, sand and silt
Trl	Lower Terrace	: Terrace deposit mainly composed of sand and gravel with thin clay layer
Trm	Middle Terrace	: Flat plain covering thin weathering material and sandy layer locally
Trp	Higher Terrace I	: Erosional plain on the sideslope at the elevation of 100-150 m
Ta	Higher Terrace II	: Highest erosional plain at the elevation of 200 m
Ta	Talus	: Talus and colluvial deposit consisting of various size materials
Ss	Sandstone	: Palaeozoic sandstone, bedded metamorphosed sandstone with chert and slate layer
Sl	Slate	: Slate with metamorphosed sand and chert layer
Gr	Coarse Granite	: Coarse grained biotite-porphyritic granite
Gf	Fine Granite	: Fine grained biotite-muscovite granite

Fig. 5-6-7 Geological Map of Bang The Sung Damsite

(5) Permeability and Bearing Capacity

Neither permeability nor bearing capacity was grasped through the in-situ data not for carrying out the boring survey in this site. In order to be in these circumstance, the permeability and bearing capacity for this site have to be presumed on the basis of the data in other site. From the view point of surface condition of geology, the Ban Nieu Dam site appears to be similar to this site, so that it is expected that the permeability is probably low in the bed rock and that the bearing capacity is also low in the overburden.

6. GEOLOGICAL ASSESSMENT

The engineering geological assessment for dam construction is described in the following chapter with respect to the geological members of overburden and bedrock respectively.

6.1 Overburden

At the thought of dam construction, the most severe problem in all dam sites are very deep Recent river bed and terrace deposit.

The overburden, in general, can be classified into three facies as shown in the following Table 6-1-1 from the engineering properties inclusive of topographical situation, lithological change, permeability and bearing capacity.

Table 6-1-1 Classification of Overburden

Practical Classification	Generalized Lithology	Average of N-value	Maximum Thickness (m)		
			BN	CT	KK
Top soil/River bed	Lateritic soil, Sand and gravel with clay	10	2	2	2
Terrace Deposit	Sand and gravel	15	10	7	10
Clay/Peat	Clayey deposit with sandy intercalations.	1	7	7	3
Weathering zone of Bedrock	Lateritization, sapolite	50	17	15	6

BN : Bang Nieo Dam CT : Khao Che Tra KK : Khlong Katha

Based on the engineering geological characteristics, at least overburden including the lowest member of clayey deposit shall be excavated off throughout the dam foundation, because it is too loose to be used for dam foundation. Even if the dam scale is smaller than 30 m high, the bearing capacity of clayey deposit is so weak that all of overburden members have to be taken away. When the higher dam is planned, the excavation line shall be put on the deeper horizon which is correlative with the weathering zone of bedrock. For both abutment of all site and the low terrace hill seeing at Che tra site, the overburden are so thin that the stripping off can be reduced to only for top soil.

While at an impervious core trench, the overburden should be excavated completely, because it has high permeability in contrast to the bedrock especially at sandy and gravel layer. Moreover there is a doubt for a toughness as a foundation of dam, even small scale dam, in a part of loose clayey deposit.

6.2 Bedrock

The bed rock in the site consists of granite and sandstone. Among five proposed dam site, four sites of Khlong Lo Young, The Bang Nieo Dam, Khlong

Katha and Bang Tho Sung are observed granite. Only Che Tra site is formed of sandstone. Although they are lithologically quite different, the engineering characteristic is similar in parts; for example, the bearing capacity and permeability is indicated as severally strong and low for both rock facies. Consequently, there is no problem with a bearing capacity as a foundation of fill type dam if the overburden and highly weathered rock have been excavated. In the case of a large scaled dam more than 50 m in height such as Khlong Katha, Bang Tho Sung and Khlong Lo Young, the main part of foundation should be based on the fresh bedrock.

Permeability of foundation is generally low; however, the value shows the slightly higher than 2 Lugeon which is a target value for grouting work. Accordingly, the grouting work shall be required up to impervious zone of bedrock. The depth reaching out the impervious horizon is supposed to be 10 to 15 m from the surface of foundation so that the grouting curtain shall be provided to this level. The high permeability of grouting zone is mainly due to the crack and/or joints opened under the influence of weathering. A cement grouting is recommended for an impervious curtain treatment.

At any rate, grouting work is required to all of proposed sites even though the quantity of work is not so much.

6.3 Embankment Materials

There is no information for embankment materials except for that of field reconnaissance and existing study at Khlong Katha.

All of sites are composed of hard rock which is locally crop out on the side slope in the vicinity of dam site. In view of these geological condition, the riprap and rock zone materials are available to be readily obtained from the surrounding area.

The weathering zone is thick especially in the Paleozoic layer which is spread out in the Che Tra site, the down stream of Khlong Lo Young, Bang Nieo Dam and Bang Tho Sung sites. These weathering facies show favorable properties for core zone or random materials. The quantities of materials are also enough for embankment volume of proposed dam at above mentioned four sites but Khlong Katha. Since the bedrock of site is formed of granite at all, furthermore, the adjacent area of site is not also underlain by Paleozoic facies, the Khlong Katha site may be pointed out a difficulty in procurement of the core zone materials. However, it would be expected that a coming study for a wider area than this time or a careful consideration for dam types will solve the problem in the detailed design.

APPENDIX A-1

Boring Log

BOREHOLE LOG

Bang Nie

PROJECT	PROVINCIAL WATER SUPPLY						SITE		BANG NIEO DAM, A.THA LANG, PHU KET				
HOLE NO.	BN-1	ELEVATION	32.0 MSL	ANGLE	0°		MACHINE	ROTARY ACKER		BEOUH	10/10/89	SITE ENGINEER	TEERAYUT
		DEPTH	50.0 m	BIT	NMLC	BIT	PUMP	TP 1800	COMPLETED	24/10/89	FOREMAN	-	
		DIAMETER	74 mm.	GV LEVEL	-8.0 m		ENGINE	GOLD FIELDS	DAYS REQUIRED	15	DRILLER	NIYOM	
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	RQD 10 20 30 40	SPT-N 10 20 30 40	DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC			DESCRIPTIONS
							NO. OF CORE	RECOVERY (%)		10 ⁻³	10 ⁻²	PERMEABILITY, K, LUGEON	
							10 20 30 40	20 40 60 80	1 2	0.10		1.00	
	32.0	1											0.00
	30.5	2	1.5		(GC)			12					1.00
		3						33					
		4						41					
	26.5	5	4.0					24					
		6			(SM)			18					5.50
		7						12					
	23.8	8	2.7					41					
		9			(CH)			26					6.20
	22.5	10	1.3					26					9.50
		11						27					
		12						40					
		13						18					
		14			(SL)			25				1.5 x 10 ⁻²	
		15						50					
		16						35					
		17						19					
		18						31				5.2 x 10 ⁻³	
		19						50/9					
	12.0	20	10.5					50/7					20.00
		21						15				CL	
		22						6				CL	
		23						50					
		24			GRANITE			40					
		25						30					
		26						50					
		27						80					
		28						50					
		29						70					
		30											

BOREHOLE LOG

Bang Nie

PROJECT		PROVINCIAL WATER SUPPLY						SITE					
HOLE NO.	BN-1	ELEVATION	32.0 MSL	ANGLE	0° <th>MACHINE</th> <td>ROTARY ACKER <th>BEGUN</th> <td>10/10/89</td> <th>SITE ENGINEER</th> <td>TEERAYUT</td> </td>	MACHINE	ROTARY ACKER <th>BEGUN</th> <td>10/10/89</td> <th>SITE ENGINEER</th> <td>TEERAYUT</td>	BEGUN	10/10/89	SITE ENGINEER	TEERAYUT		
		DEPTH	50.0 m	BIT	NMLC BIT	PUMP	TP 1800	COMPLETED	24/10/89	FOREMAN	-		
		DIAMETER	74 mm.	GV LEVEL	-8.0 m	ENGINE	GOLD FIELDS	DAYS REQUIRED	15	DRILLER	NIYOM		
DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINO'Y	COLOR	ROD	SPT-N	DRILL SPEED	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS	
	32.0						10 20 30 40	10 20 30 40	(h/m)	10 ⁻³	10 ⁻²		
							NO. OF CORE	RECOVERY(%)		0.10	1.00		
							10 20 30 40	20 40 60 80					
20/10/89	31												
	32												
	33												
	34												
	35												
	36												
	37												
	38												
	39												
	40				GRANITE								
	41												
	42												
	43												
	44												
	45												
	46												
	47												
	48												
	49												
24/10/89	50	300											50.00
													END OF CORING

BOREHOLE LOG

Bang Nue

PROJECT		PROVINCIAL WATER SUPPLY					SITE					
PROJECT	HOLE NO.	ELEVATION	23.0 MSL	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	25/10/89	SITE ENGINEER	TEERAYUT	
		DEPTH	52.5 m	BIT	IMPREGNATED	PUMP	TP 1800	COMPLETED	8/11/89	FOREMAN	---	
		DIAMETER	74 mm.	GV LEVEL	0.0	ENGINE	GOLD FIELDS	DAYS REQUIRED	19	DRILLER	PYSAN	
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	NO. OF CORE	RECOVERY (%)	DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS
25/11/89	23.0											
	22.5	1	0.5									0.00
	21.5	2	1.0									0.50
		3										1.50
		4			(SP-SM)							
		5										
		6										
		7										
		8										
	13.5	9	8.0									9.50
27/11/89	12.5	10	1.0		(SC)							9.50
		11			(CL-SC)							10.50
		12										
		13										
		14			(SC)							12.50
	8.5	15	4.0									14.50
		16			(SC)							17.50
		17										
	5.5	18	3.0									
		19										
		20			(SC)							
		21										
		22										
	0.5	23	5.0									22.50
		24										
		25			GRANITE							
		26										
		27										
		28										
		29										
31/11/89		30										
		31										

BOREHOLE LOG

Bang Nie

PROJECT		PROVINCIAL WATER SUPPLY						SITE		BANG NIEO DAM, A-THA-LANG, PHU KEY			
HOLE NO.	BN-2	ELEVATION	23.0 MSL	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	25/10/89	SITE ENGINEER	TEERAYUT		
		DEPTH	52.5 m	BIT	IMPREGNATED	PUMP	TP 1800	COMPLETED	8/11/89	FOREMAN	-		
		DIAMETER	74 mm.	GV LEVEL	0.0	ENGINE	GOLD FIELDS	DAYS REQUIRED	19	DRILLER	PYSAN		
DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINO'Y	COLOR	ROD	SPT-N	DRILL SPEED	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS	
							10 20 30 40	10 20 30 40	(n/m)	10 ⁻⁴ 10 ⁻³	0.10 1.00		
							NO. OF CORE	RECOVERY(%)					
							10 20 30 40	20 40 60 80					
31/10/89	31												
3/11/89	32												
	33												
	34												
	35												
	36												
	37												
	38												
	39												
	40												
	5/11/89	41				GRANITE	GRAYISH WHITE						
42													
43													
44													
45													
46													
47													
48													
49													
50													
6/11/89	51												
	52												
7/11/89	29.5	30.0											
												END OF CORING	
												52.5	

B O R E H O L E L O G

Bang Nie

PROJECT		PROVINCIAL WATER SUPPLY						SITE				
BN-3		ELEVATION	50.0 msl	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	25/10/89	SITE ENGINEER	TEERAYUT	
HOLE NO.		DEPTH	50.0 m	BIT	IMPREGNATED	PUMP	TP 1800	COMPLETED	9/11/89	FOREMAN	—	
DATE		DIAMETER	74 mm.	GW LEVEL	-9.10 m	ENGINE	GOLD FIELDS	DAYS REQUIRED	15	DRILLER	SERM	
ELEVATION	DEPTH	THICK'S	LOG	TERMINO'Y	COLOR	<input checked="" type="checkbox"/> ROD 10 20 30 40 NO. OF CORE	<input type="checkbox"/> SPT-N 10 20 30 40 RECOVERY(%)	DRILL SPEED (h/m) 1 2	<input checked="" type="checkbox"/> PERMEABILITY, K, CM/SEC 10^{-3} 10^{-2}	PERMEABILITY, K, LUGEON 1.00 10.00	DESCRIPTIONS	
						<input type="checkbox"/> NO. OF CORE 10 20 30 40	<input type="checkbox"/> RECOVERY(%) 20 40 60 80		<input type="checkbox"/> PERMEABILITY, K, LUGEON			
50.0	1			(SC)							0.00	
	2											
47.0	3	3.0					11	0.7			MEDIUM TO STIFF CLAYEY SAND, REDDISH BROWN	3.00
	4											
	5											
	6			(SM)								
	7											
	8											
41.1	9	5.9						16			COMPLETELY WEATHERED GRANITE, LI-GREY REDDISH BROWN AND YELLOWISH BROWN	8.90
	10							15				
	11	1.6						19			GRANITE BOULDER	10.50
39.5	12			(SM)				24			COMPLETELY WEATHERED GRANITE	13.50
	13	3.0						37				
36.5	14			(SP-SM)							MEDIUM DENSE, SILTY TO MEDIUM SAND, YELLOWISH BROWN	14.50
35.5	15	1.0						21				
	16							50/5"				
	17							50/8"				
	18							58"				
	19			(SC, SH)				50/2"			COMPLETELY WEATHERED GRANITE, LI-GREY AND BROWN	21.80
	20							50"				
	21							50/10"				
28.20	22	8.3										
	23											
	24											
	25											
	26			GRANITE							LIGHT COLOR, PHANERITIC TEXTURE, MODERATELY HARD TO HARD FRESH ROCK TO SLIGHTLY WEATHERED, SLIGHTLY JOINTING JOINT PLANE ABOUT 10°-40° DIP COMPOSED OF QUARTZ, NA-FELDSPAR, BIOTITE AND HORNBLende	
	27											
	28											
	29											
	30											

BOREHOLE LOG

Bang Nie

PROJECT		PROVINCIAL WATER SUPPLY					SITE							
BN-3		ELEVATION	50.0 MSL	ANGLE	0	MACHINE	ROTARY ACKER	BEGUN	25/10/89	SITE ENGINEER	TEERAYUT			
		DEPTH	50.0 m	BIT	BIT	PUMP	TP 1800	COMPLETED	9/11/89	FOREMAN	-			
		DIAMETER	74 mm.	GV LEVEL	- 9.10	ENGINE	GOLD FIELDS	DAYS REQUIRED	15	DRILLER	SERM			
DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINO'Y	COLOR	ROD		SPT-N		DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC		DESCRIPTIONS
							NO. OF CORE	RECOVERY(%)	NO. OF CORE	RECOVERY(%)		10 ⁻³	10 ⁻²	
1/11/89	31						10 20 30 40	10 20 30 40				0.10		
2/11/89	32						10 20 30 40	10 20 30 40				0.27		
3/11/89	33						10 20 30 40	10 20 30 40				0.79		LIGHT COLOR, PHANERITIC TEXTURE, MODERATELY HARD TO HARD FRESH TO SLIGHTLY WEATHERED, JOINT PLANE ABOUT 60°-90° (WEATHERED SURFACE, CLEAN) COMPOSED OF QUARTZ, NA-FELDSPAR, BIOTITE AND HORNBLende
4/11/89	34				GRANITE		10 20 30 40	10 20 30 40				0.27		
5/11/89	35						10 20 30 40	10 20 30 40				0.79		
6/11/89	36						10 20 30 40	10 20 30 40				0.27		
7/11/89	37						10 20 30 40	10 20 30 40				0.27		
8/11/89	38						10 20 30 40	10 20 30 40				0.27		
9/11/89	39						10 20 30 40	10 20 30 40				0.27		
10/11/89	40						10 20 30 40	10 20 30 40				0.27		
11/11/89	41						10 20 30 40	10 20 30 40				0.27		
12/11/89	42						10 20 30 40	10 20 30 40				0.27		
1/12/89	43						10 20 30 40	10 20 30 40				0.27		
2/12/89	44						10 20 30 40	10 20 30 40				0.27		
3/12/89	45						10 20 30 40	10 20 30 40				0.27		
4/12/89	46						10 20 30 40	10 20 30 40				0.27		
5/12/89	47						10 20 30 40	10 20 30 40				0.27		
6/12/89	48						10 20 30 40	10 20 30 40				0.27		
7/12/89	49						10 20 30 40	10 20 30 40				0.27		
8/12/89	50	0.20	28.00				10 20 30 40	10 20 30 40				0.27		END OF CORING 50.0

BOREHOLE LOG

Khao Che Tra

PROJECT		PROVINCIAL WATER SUPPLY					SITE		KHAO CHE TRA, A. THA-LANG, PHU KET			
HOLE NO.	DATE	ELEVATION	36.2	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	11/10/89	SITE ENGINEER	TEERAYUT	
		DEPTH	50.00	BIT	NO DIAMOND BIT	PUMP	TP 1800	COMPLETED	22/10/89	FOREMAN	-	
		DIAMETER	74 mm.	GW LEVEL	-10.20	ENGINE	GOLD FIELDS	DAYS REQUIRED	12	DRILLER	PYSAN	
DATE	SLEEVAT *N	DEPTH	THICK *S	LOG	TERMINO *Y	COLOR	ROD NO. OF CORE	SPT-N RECOVERY (%)	DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS
11/10/89	36.2	1	1.0									0.00 CLAY, TRACE OF SAND BROWN
		2			(SC/CL)							1.00 LOOSE TO MEDIUM DENSE CLAYEY FINE TO COARSE SAND, BROWN, LI-BROWN AND REDDISH BROWN (INTERMEDIATE SOIL)
	30.7	3	4.5									5.50 VERY STIFF TO HARD CLAY, LI-GREY, BROWN AND REDDISH BROWN
	28.7	4	2.0		(CL)							7.50 VERY DENSE SILTY-GRAVELLY FINE TO COARSE SAND, WHITE AND REDDISH BROWN
	27.7	5	1.0		(SH)							8.50 VERY STIFF TO HARD CLAY AND SANDY CLAY, REDDISH BROWN AND YELLOW
		6			(CL, CL/SC)							
	21.7	7	6.0									14.50 VERY DENSE CLAYEY-GRAVELLY SAND, REDDISH BROWN AND YELLOWISH BROWN
	20.7	8	1.0		(SC)							15.50 HARD SANDY CLAY, YELLOWISH BROWN (INTERMEDIATE SOIL)
		9			(CL/SC)							
	16.7	10	4.0									19.50 HARD FINE SANDY CALY, BROWN
		11			(CL)							
	13.7	12	3.0									22.50 GREENISH GRAY, BROWN, PEBBLY, JOINT PLANE ABOUT 40°, 70° dip QUARTZ VEIN AT 22.55-22.58 M. 23.70-23.73 M.
		13			SANDSTONE (METAKOR-PROSED)							
		14										
		15										
		16										
		17										
		18										
		19										
		20										
		21										
		22										
		23										
		24										
		25										
		26										
		27										
		28										
		29										
		30										

BOREHOLE LOG

Khao Che Tra

PROJECT		PROVINCIAL WATER SUPPLY				SITE		KHAO CHETRA, A. THA-LANG, PHU KET	
HOLE NO.	CT-1	ELEVATION	36.2	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	11/10/89
		DEPTH	50.00	BIT	NO DIAMOND BIT	PUMP	TP 1800	COMPLETED	22/10/89
		DIAMETER	74 mm.	GW LEVEL	10.20	ENGINE	GOLD FIELDS	DAYS REQUIRED	12
								FOREMAN	-
								DRILLER	PYSAN

DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	ROD		SPT-N		DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC		DESCRIPTORS		
							NO. OF CORE		RECOVERY (%)			10 ⁻³			10 ⁻²	
							10	20	30	40		10	20		30	40
19/10/89		31														
		32														
		33														
		34														
		35														
		36			SANDSTONE											
		37			(METAMORPHOSIS)											
		38														
		39														
		40														
		41														
		42														
		43														
		44														
		45														
		46														
		47														
-11.3		25.0												47.50		
														END OF CORING.		

B O R E H O L E L O G

Khao Che Tra

PROJECT		PROVINCIAL WATER SUPPLY						SITE			
ROLE NO.	CT-2	ELEVATION	31.0	ANGLE	VERTICAL <th>MACHINE</th> <td>ROTARY ACKER <th colspan="2">KHAO CHE TRA, A. THA-LANG, PHU KET</th> <th colspan="2"></th> </td>	MACHINE	ROTARY ACKER <th colspan="2">KHAO CHE TRA, A. THA-LANG, PHU KET</th> <th colspan="2"></th>	KHAO CHE TRA, A. THA-LANG, PHU KET			
		DEPTH	50.00	BIT	DIAMONDBIT	PUMP	TP 1800	BEGUN	19/9/89	SITE ENGINEER	TEERAYUT
		DIAMETER	74 mm.	QV LEVEL	-2.60 M	ENGINE	GOLD FIELDS	COMPLETED	6/10/89	FOREMAN	
DATE	19/9/89	TRICK'S		LOG		TERMINOLOGY		PERMEABILITY, K, CM/SEC		PERMEABILITY, K, LUGEON	
ELEVATION	31.0	DEPTH		NO. OF CORE		DRILL SPEED (h/m)		ROCK CLASS		DESCRIPTIONS	
DEPTH		TRICK'S		NO. OF CORE		DRILL SPEED (h/m)		ROCK CLASS		DESCRIPTIONS	
19/9/89	31.0									0.00	
	24.5	6.5	(SH)							6.50	LOOSE TO MEDIUM DENSE SILTY FINE TO COARSE SAND, TRACE OF GRAVEL, BROWN AND GREY
21/9/89	23.6	0.9	(CL)							7.40	MEDIUM CLAY, LI-GREY
	22.4	1.2	(SH)							8.60	LOOSE SILTY FINE TO MEDIUM SAND, LI-BROWN
	17.0	5.8	(CL)							14.00	VERY SOFT CLAY, BROWN AND GREY
	16.0	1.0	(CL)							15.00	VERY STIFF CLAY, WITH FINE SAND, YELLOWISH BROWN AND YELLOWISH BROWN
22/9/89	9.8	6.2	(CL/SC)							21.15	HARD FINE SANDY CLAY, BROWN YELLOWISH BROWN AND YELLOW
	4.5	5.3	(SM)							26.50	
23/9/89	1.0		PEBBLE AND GRAVEL								PEBBLE AND GRAVEL, SAND

BOREHOLE LOG

Khao Che Tra

PROJECT	PROVINCIAL WATER SUPPLY					SITE		KHAO CHE TRA, A.THA-LANG, PHU KET		
	ELEVATION	31.0	ANGLE	VERTICAL	MACHINE	ROTARY ACKER	BEGUN	19/9/89	SITE ENGINEER	TEERAVUT
	DEPTH	50.00	BIT	DIAMOND BIT	PUMP	TP 1800	COMPLETED	6/10/89	FOREMAN	
DIAMETER	74 mm.	GV LEVEL	-26.0M	ENGINE	GOLD FIELDS	DAYS REQUIRED	18	DRILLER	PYSAN	

DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	NO. OF CORE		RECOVERY (%)		DRILL SPEED (h/m)	DRILL TIME	PERMEABILITY, K, CM/SEC		DESCRIPTIONS
							10	20	30	40			10	20	
27/9/89		31					10								
		32													
28/9/89		33													
		34													
		35													
		36					71								
		37			SANDSTONE		70								
29/9/89		38					60							2.75	
		39													
		40													
		41					80								
		42					70								
4/10/89		43					9							3.2	
		44					80								
		45					80								
		46					8								
		47					9								
		48					80							3.6	
5/10/89		49													
		50					20								

50.00
END OF CORING.

BOREHOLE LOG

Khao Che Tra

PROJECT		PROVINCIAL WATER SUPPLY						SITE		KHAO CHE TRA, A. THA-LANG, PHU KET		
HOLE NO.	CT-3	ELEVATION	45.0 M.	ANGLE	0° <th>MACHINE</th> <td>ROTARY ACKER</td> <th>BEGUN</th> <td>19/9/89</td> <th>SITE ENGINEER</th> <td>TEERAYUT</td>	MACHINE	ROTARY ACKER	BEGUN	19/9/89	SITE ENGINEER	TEERAYUT	
		DEPTH	48.50 M.	BIT	DIAMOND BIT	PUMP	TP 1800	COMPLETED	5/10/89	FOREMAN	-	
		DIAMETER	74 mm.	GW LEVEL	-7.60 M.	ENGINE	GOLD FIELDS	DAYS REQUIRED	17	DRILLER	SERM	
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	RQD	SPT-N	DRILL SPEED	PERMEABILITY	DESCRIPTIONS	
19/9/89	45.0	0.0					10 20 30 40	10 20 30 40	(n/m)	10 ⁻³ 10 ⁻²		
1					(CL)				9		0.00	STIFF TO VERY STIFF CLAY WITH FINE SAND, YELLOWISH BROWN AND REDDISH BROWN
2									6			
3									12			
4									16			
5									13			
6		39.0	6.0						33		6.00	
7									55			HARD FINE SANDY CLAY, YELLOWISH BROWN AND REDDISH BROWN
8					(CL)				43			
9									78			
10									65/11			
11									50/6			
12									50/8			
13									55			
14									38			
15									50/8			
16									6		18.00	
17									68			
18		27.0	12.0		(SM)				50/8		18.50	VERY DENSE SILTY-GRAVELLY SAND, BROWN AND GREENISH GREY
19		26.5										SANDSTONE, GREEN, SLIGHTLY TO MODERATELY METAMORPHOSED HIGHLY CRACKS AND FILL WITH CALCITE AND QUARTZ, FRESH ROCKS JOINT PLANE IS VARY DIPPING (5°-65°) WITH CALCITE AND QUARTZ FILL, HIGHLY WEATHERED AT 24.00-24.30 M. DEPTH, QUARTZ VEIN AT 19.25-19.35 M., 21.50-21.72 M. AND ABOUT 40° DIPPING
20												
21												
22												
23					SANDSTONE							
24												
25												
26												
27												
28												
29												

BOREHOLE LOG

Khao Che Tra

PROJECT		PROVINCIAL WATER SUPPLY						SITE							
HOLE NO.	CT-3	ELEVATION	45.0 M.		ANGLE	0°		MACHINE	ROTARY ACKER		BEGUN	19/9/89		SITE ENGINEER	TEERAYUT
		DEPTH	48.50 M.		BIT	DIAMOND BIT		PUMP	TP 1000		COMPLETED	5/10/89		FOREMAN	-
		DIAMETER	74 mm.		GV LEVEL	-7.60 M.		ENGINE	GOLD FIELDS		DAYS REQUIRED	17		DRILLER	SIRH
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	ROD	SPT-N	DRILL SPEED	PERMEABILITY, K, CM/SEC	DESCRIPTIONS				
27/9/89	31	31					51				SANDSTONE, GREEN, SLIGHTLY TO MODERATELY METAMORPHOSED, HIGHLY CRACKED AND FILL WITH CALCITE AND QUARTZ, WIDE TO CLOSE SPACING JOINTING, JOINT PLANE 40°-70° DIP QUARTZ VEIN AT 38.50-38.74 M. 39.32-39.50 M.				
28/9/89	32	32					70								
2/10/89	33	33					65								
3/10	34	34					50								
4/10/89	35	35					55								
5/10/89	36	36					50								
	37	37					70								
	38	38			SANDSTONE		75								
	39	39					70								
	40	40					70								
	41	41					60								
	42	42					50								
	43	43					80								
	44	44					50								
	45	45					50								
	46	46					50								
	47	47					50								
	48	48					50								
	48.50	48.50													

B O R E H O L E L O G

Khao Che Tra

PROJECT		PROVINCIAL WATER SUPPLY					SITE					
HOLE NO. CT-4	ELEVATION	39.0 M.	ANGLE	0°	MACHINE	ROTARY ACKER	SITE		KHAO CHE TRA, A. THALANG, PHU KEY			
	DEPTH	48.00M.	BIT	HMLL BIT	PUMP	TP 1800	BEGUN	7/10/89	SITE ENGINEER	TEERAYUT		
	DIAMETER	74 mm.	GW LEVEL	- 0.70	ENGINE	GOLD FIELDS	COMPLETED	21/10/89	FOREMAN			
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	ROD	SPT-N	DRILL SPEED	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS
							NO. OF CORE	RECOVERY (%)	(h/m)	10 ⁻³	10 ⁻²	
							10 20 30 40	10 20 30 40	1 2	100	10.00	
	39.0											0.00
		1			(CL, CR)							STIFF TO VERY STIFF AND HARD FINE SANDY CLAY AND CLAY WITH FINE SAND, YELLOWISH BROWN AND REDDISH BROWN
		2										
		3										
		4										
		5										
		6										
	32.0	7	7.0									7.00
		8			(CL, CL/SC)							
		9										
		10										
		11										
		12										
		13										
		14										
		15										
		16										
		17										
	21.0	18	11.0									18.00
		19	0.50		QUARTZ	WHITE						HIGHLY WEATHERED, WHITE
		20				BROWN TO DARK BROWN						18.50
		21										MODERATELY TO COMPLETELY WEATHERED
		22			SANDSTONE (SLIGHTLY METAMORPHOSIZED)							SLIGHTLY TO HIGHLY WEATHERED. JOINT PLANE ABOUT 50 TO 70° DIPPING AND OPEN STAINED
		23										
		24				GREENISH GREY						QUARTZ VEIN AT 24.85-24.95 M
		25										25.40-25.60 M
		26										
		27										
		28										
		29										
		30										

BOREHOLE LOG

Khao Che Tra

PROJECT	PROVINCIAL WATER SUPPLY				SITE	KHAO CHE TRA, A. THALANG, PHU KEY				
HOLE NO. CT-4	ELEVATION	39.0 M	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	7/10/89	SITE ENGINEER	TEERAYUT
	DEPTH	48.00 M	BIT	NMLL BIT	PUMP	TP 1800	COMPLETED	21/10/89	FOREMAN	
	DIAMETER	74 mm.	QV LEVEL	- 0.70	ENGINE	GOLD FIELDS	DAYS REQUIRED	15	DRILLER	SERM

DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	ROD		SPT-N		DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC		DESCRIPTORS
							NO. OF CORE	RECOVERY (%)	NO. OF CORE	RECOVERY (%)		10 ⁻³	10 ⁻²	
14/10/89	31.0	31			SANDSTONE (SLIGHTLY METAMORPHOSED)		10	10	10	10				
	32.0	32					15	15	15	15				
	33.0	33					20	20	20					
	34.0	34					30	30	30					
	35.0	35					40	40	40					
	36.0	36					50	50	50					
	37.0	37					20	20	20					
	38.0	38					50	50	50					
	39.0	39					15	15	15					
	40.0	40					50	50	50					
	41.0	41					50	50	50					
	42.0	42					15	15	15					
	43.0	43					50	50	50					
	44.0	44					50	50	50					
	45.0	45					50	50	50					
	46.0	46					15	15	15					
	47.0	47					50	50	50					
	48.0	48					50	50	50					

FRESH ROCK, JOINT PLANE
30-70 DIP MAY BE FILL
WITH CLAY

ZZZ VEIN AT 34.80 m(DIP 40)
41.00-41.10 m.
46.50-46.60 m.

CORE NOT RECOVERY AT
30.00-30.50 m
39.15-40.00 m

END OF CORING

B O R E H O L E L O G

Khlong Katha

PROJECT		PROVINCIAL WATER SUPPLY					SITE		BANG NIEO DAM, A.THA LANG, PHU KEY			
HOLE NO.	DATE	ELEVATION	25.5	ANGLE	0° <th>MACHINE</th> <td>ROTARY ACKER <th>BEGUN</th> <td>17/11/89 <th>SITE ENGINEER</th> <td>TEERAYUT</td> </td></td>	MACHINE	ROTARY ACKER <th>BEGUN</th> <td>17/11/89 <th>SITE ENGINEER</th> <td>TEERAYUT</td> </td>	BEGUN	17/11/89 <th>SITE ENGINEER</th> <td>TEERAYUT</td>	SITE ENGINEER	TEERAYUT	
		DEPTH	42.0	BIT	IMPREGNATED	PUMP	TP 1800	COMPLETED	26/1/89	FOREMAN	-	
		DIAMETER	0.37	GV LEVEL	-1.03	ENGINE	GOLD FIELDS	DAYS REQUIRED	15	DRILLER	-	
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	ROD NO. OF CORE	SPT-N RECOVERY (%)	DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS
12/11/89	25.5	0										
	25.0	1	0.5		(SH)							0.00 SAND, LI-BROWN
	24.0	2	1.0									0.50 MEDIUM DENSE SILTY FINE SAND
	19.0	6	3.0		(SH)							1.50 LOOSE SILTY FINE SAND, LI-BROWN & GREY
15/11/89	14.5	11	4.5		(SH)							6.50 MEDIUM DENSE TO DENSE SILTY FINE SAND, LI-GRY AND LI-YELLOWISH BROWN
16/11/89	13.5	12	1.0									11.00 COMPLETELY WEATHERED GRANITE
17/11/89		13			GRANITE							12.00
18/11/89		14										
		15										
		16										
		17										
		18										
		19										
		20										
		21										
		22										
		23										
		24										
		25										
		26										
		27										
		28										
		29										
		30										

BOREHOLE LOG

Khlung Katha

PROJECT		PROVINCIAL WATER SUPPLY					SITE			BANG NIEO DAM, A.THA LANG, PHU KET				
HOLE NO.	KK-1	ELEVATION	25.5 NSL		ANGLE	0°		MACHINE	ROTARY ACKER		BEGUN	17/1/89	SITE ENGINEER	TEERAYUT
		DEPTH	42.0 m		BIT	INTEGRATED		PUMP	TP 1800		COMPLETED	26/1/89	FOREMAN	-
		DIAMETER	0.37m.		GV LEVEL	-1.03 m		ENGINE	GOLD FIELDS		DAYS REQUIRED	15	DRILLER	SEIM

DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINO'T	COLOR	<input type="checkbox"/> ROD 10 20 30 40	<input type="checkbox"/> SPT-H 10 20 30 40	DRILL SPEED (h/m)	ROCK	<input type="checkbox"/> PERMEABILITY, K, CM/SEC 10 ⁻³	DESCRIPTIONS
							<input type="checkbox"/> NO. OF CORE 10 20 30 40	<input type="checkbox"/> RECOVERY(%) 20 40 60 80			<input type="checkbox"/> PERMEABILITY, K, LUGEON 0.10 1.00	

DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINO'T	COLOR	<input type="checkbox"/> ROD 10 20 30 40	<input type="checkbox"/> SPT-H 10 20 30 40	DRILL SPEED (h/m)	ROCK	<input type="checkbox"/> PERMEABILITY, K, CM/SEC 10 ⁻³	DESCRIPTIONS
							<input type="checkbox"/> NO. OF CORE 10 20 30 40	<input type="checkbox"/> RECOVERY(%) 20 40 60 80			<input type="checkbox"/> PERMEABILITY, K, LUGEON 0.10 1.00	
19/11/89		31			GRANITE	GREEN LI-ORANGE				CH		PORPHYRITIC-TEXTURE, SLIGHTLY WEATHERED, JOINT PLANE ABOUT 40° TO 60° COMPOSED OF QUARTZ, K-FELDSPAR, HORNBLENDE AND BIOTITE
		32										
20/11/89		33										PORPHYRITIC-PHANCRITIC TEXTURE, FRESH, JOINT PLANE ABOUT 40° TO 80° COMPOSED OF QUARTZ, CA-FELDSPAR, BIOTITE AND HORNBLENDE
		34										
21/11/89		35										END OF CORING
		36										
22/11		37			GRANO - DIORITE	GREYISH WHITE						
23/11/89		38										
25/11/89		39										
26/11/89		40										
		41										
		42										
		43										
		44										
		45										
		46										
		47										

BOREHOLE LOG

Khlong Katha

PROJECT		PROVINCIAL WATER SUPPLY					SITE		NILONG KRA THA, A. KRA THU, PHU KET		
HOLE NO.	DATE	ELEVATION		ANGLE	MACHINE	ROTARY ACKER	BEGUN	SITE ENGINEER	TEERAYUT		
		DEPTH		BIT	PUMP	TP 1800	COMPLETED	FOREMAN		-	
		DIAMETER		GW LEVEL		ENGINE	GOLD FIELDS	DAYS REQUIRED	13		DRILLER

DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINOLOGY	COLOR	ROD		SPT-N		DRILL SPEED (h/a)	PERMEABILITY, K, CM/SEC		DESCRIPTIONS
							NO. OF CORE	RECOVERY (%)	NO. OF CORE	RECOVERY (%)		10 ⁻³	10 ⁻²	
	235													
15/11/89	230	1	0.5										0.00	SAND, LI-BROWN
		2			(SH)								0.50	MEDIUM DENSE TO VERY DENSE SILTY FINE SAND, LI-YELLOWISH BROWN, REDDISH BROWN AND LI-GREY
	200	3	3.0										3.50	VERY LOOSE SILTY FINE SAND, LI-BROWN AND LI-GREY
	180	4	2.0		(SH)								5.50	MEDIUM DENSE SILTY FINE SAND, LI-GREY AND LI-YELLOWISH BROWN
16/11/89	140	9	4.0		(SH)								9.50	LOOSE SILTY FINE SAND, LI-GREY
	110	12	3.0		(SH)								12.50	MEDIUM DENSE TO DENSE SILTY FINE SAND, LI-GREY
17/11/89	85	15	2.5		(SH)								15.00	COMPLETELY WEATHERED GRANITE LI-GREENISH GREY
18/11/89		16												
		17			(SH)									
		18												
		19												
	35	20	5.5										20.05	PORPHYRYTIC-PHANERITIC TEXTURE, FRESH ROCK, JOINT PLANE ABOUT 40° TO 70°, SOME JOINT IS SLICKEN SIDE COMPOSED OF QUARTZ, NA-FELDSPAR, BIOTITE AND HORNELENDE
19/11/89		21			PORPHYRYTIC DIORITE									
		22												
		23												
		24												
		25												
		26												
		27												
		28												
		29												
23/11/89		30												

B O R E H O L E L O G

Khlung Katha

PROJECT		PROVINCIAL WATER SUPPLY						SITE			
K-K-2		ELEVATION	23.5 m.	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	15/11/89	SITE ENGINEER	TEERAYUT
HOLE NO.		DEPTH		BIT	IMPREGNATED	PUMP	TP 1800	COMPLETED	27/11/89	FOREMAN	
DATE		DIAHETER	74 mm.	GV LEVEL		ENGINE	GOLD FIELDS	DAYS REQUIRED	13	DRILLER	PYSAN
ELEVATION	DEPTH	THICK'S	LOG	TERMINO'Y	COLOR	RQD 10 20 30 40 NO. OF CORE	SPT-N 10 20 30 40 RECOVERY(%)	DRILL SPEED (h/m)	PERMEABILITY, K, CM/SEC 10 ⁻³ 10 ⁻² PERMEABILITY, K, LUGEON 0.10 1.00	DESCRIPTIONS	
31											
32											
33											
34											
35				GRANITE							
36											
37											
38											
39											
40											
41					GREYISH WHITE						
42											
43											
44											
45											
220		250									45.50
END OF CORING											

FORPHYBITIC-PHANERITIC TEXTURE, FRESH ROCK, JOINT PLANE ABOUT 40° TO 60° DIPPING (SLICKEN SIDE) COMPOSE OF QUARTZ, NA-FELDSPAR, BIOTITE AND HORNBLENDE

B O R E H O L E L O G

Khlong Katha

PROJECT		PROVINCIAL WATER SUPPLY						SITE		KHLONG KATHA, A KRA THU, PHU KET			
HOLE NO.	KK-3	ELEVATION	25.0 M.	ANGLE	0° <th>MACHINE</th> <td>ROTARY ACKER <th>BEGUN</th> <td>20/10/89 <th>SITE ENGINEER</th> <td>TEERAYUT</td> </td></td>	MACHINE	ROTARY ACKER <th>BEGUN</th> <td>20/10/89 <th>SITE ENGINEER</th> <td>TEERAYUT</td> </td>	BEGUN	20/10/89 <th>SITE ENGINEER</th> <td>TEERAYUT</td>	SITE ENGINEER	TEERAYUT		
		DEPTH	38.0 M.	BIT	IMPREGVATED	PUMP	BEAN	COMPLETED	12/11/89 <th>FOREMAN</th> <td></td>	FOREMAN			
		DIAMETER	74 mm.	GW LEVEL	-2.00 M.	ENGINE	VK	DAYS REQUIRED	16	DRILLER	SIYOH		
DATE	ELEVATION	DEPTH	THICKNESS	LOG	TERMINOLOGY	COLOR	NO. OF CORE	RECOVERY (%)	DRILL SPEED (RPM)	PERMEABILITY, K, CM/SEC	PERMEABILITY, K, LUGEON	DESCRIPTIONS	
							10 20 30 40	10 20 30 40		10 ⁻³ 10 ⁻²	1.00 10.00		
28/10/89	250	1			(SM)				0.6			0.00	VERY LOOSE TO LOOSE SILTY FINE SAND, LI-BROWN
	225	2			(SC)				0.2			2.50	VERY LOOSE CLAYEY FINE SAND LI-BROWN
	195	3			(SM)				0.2			5.50	COMPLETELY WEATHERED GRANITE LI-BROWN AND YELLOWISH BROWN
29/10/89	17.0	4							0.2			7.02	PORPHYRITIC TEXTURE, COMPOSED OF QUARTZ, K-FELDSPAR, HORNBLende
30/10/89		5				GREY PINK			0.2			9.00	PORPHYRITIC PHANESITIC TEXTURE, FRESH ROCK, JOINT PLANE ABOUT 30°-70° DIPPING, COMPOSED OF QUARTZ, CA-FELDSPAR, BIOTITE AND HORNBLende
31/10/89		6			GRANITE				0.2				
6/11/89		7							0.2				
10/11/89		8							0.2				
		9							0.2				
		10							0.2				
		11							0.2				
		12							0.2				
		13							0.2				
		14							0.2				
		15							0.2				
		16							0.2				
		17							0.2				
		18							0.2				
		19							0.2				
		20							0.2				
		21							0.2				
		22							0.2				
		23							0.2				
		24							0.2				
		25							0.2				
		26							0.2				
		27							0.2				
		28							0.2				
		29							0.2				
		30							0.2				

B O R E H O L E L O G

Khlong Katha

PROJECT	PROVINCIAL WATER SUPPLY				SITE	KHLONG KATHA, A. KRA THU, PHU KET				
HOLE NO. KK-3	ELEVATION	25.0 M.	ANGLE	0°	MACHINE	ROTARY ACKER	BEGUN	28/10/89	SITE ENGINEER	TEERAYUT
	DEPTH	38.0 M.	BIT	IMPREGNATED	PUMP	BEAN	COMPLETED	12/11/89	FOREMAN	-
	DIAMETER	74 mm.	GV LEVEL	-2.00 M.	ENGINE	VM	DAYS REQUIRED	16	DRILLER	NIYON

DATE	ELEVATION	DEPTH	THICK'S	LOG	TERMINO'Y	COLOR	ROD		SPT-N		DRILL SPEED (h/a)	PERMEABILITY, K, CM/SEC		LUGEON	DESCRIPTIONS		
							NO. OF CORE	RECOVERY(%)	10	20		30	40			10 ⁻³	10 ⁻²
11/21/89		31															
12/11/89		32			GRANITE	GREY											
		33															
		34															
		35															
		36															
		37															
		38	30.0														
	-13.0															END OF CORING	

