

REPUBLIC OF PERU
SERVICIO DE AGUA POTABLE Y ALCANTARILLADO DE LIMA
(SEDAPAL)

THE FEASIBILITY STUDY
ON
THE IMPROVEMENT OF SEWERAGE SYSTEM
IN
SOUTHERN PART OF LIMA

FINAL REPORT

VOLUME III
APPENDICES

MARCH, 1990

JAPAN INTERNATIONAL COOPERATION AGENCY

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ORGANIZATION OF REPORTS

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APPENDIX 1

LIST OF PERSONNEL CONCERNED

APPENDIX 1 LIST OF PERSONNEL CONCERNED

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APPENDIX 2

BIBLIOGRAPHIES

APPENDIX 2 BIBLIOGRAPHIES

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APPENDIX 3

PRESENT CONDITION OF INDUSTRIAL WASTEWATER

APPENDIX 3 Present Condition of Industrial Wastewater

	Lima	Surco	Quantity (m ³ /day)
1A = Envasadoras de Pescados y Mariscos	14	1	1,380
1B = Envasadoras de Carne y Carnes	28	6	16,787
1C = Refinerías de Aceites Comestibles	7	-	-
1D = Pescados y Centros de Abasto	44	12	16,254
1E = Textiles con Lavado de Lana Cruda	5	-	-
1F = Curtiembres	35	6	49,992
1G = Grandes Chifas y Restaurantes	10	5	4,382
1H = Servicentros con Lavado y Engraso	36	7	5,463
3A = Envasadoras de Leche y Derivados	6	6	6,014
3B = Envasadoras de Frutas y Legumbres	7	3	8,738
3C = Pecas de Detergentes y Artículos de Tecedor	12	3	2,380
3D = Pecas de Locotas, Mosaicos y Derivados	39	7	8,489
3E = Galvanoplastías y Ensambladoras	97	20	40,079
3F = Pecas de Papel y Cartón	22	9	28,839
3G = Lavanderías	27	13	9,126
3H = Baños Públicos, Hoteles y Baños	67	14	78,162
3I = Hospitales y Clínicas	8	9	9,546
6A = Pecas de Cerveza y Bebidas Alcohólicas	10	3	56,714
6B = Pecas de Bebidas Gaseosas	4	-	-
6C = Textiles con Tejido y Acabado	116	62	329,963
6D = Pecas de Pinturas y Derivados	16	1	671
6E = Pecas de Vidrios y Espejos	9	5	4,056
6F = Industrias Químicas y Petroquímicas	98	24	88,767
6G = Laboratorios Farmacéuticos	47	23	42,380
6H = Pecas de Alimentos Deshidratados	54	10	28,490
Total	818	249	836,593. =

Present Industrial Wastewater: 0.323 m³/sec

APPENDIX 4

EXISTING TREATMENT PLANTS IN PERU

APPENDIX 4 EXISTING TREATMENT PLANTS IN PERU

A4.1 General

Relevant data on public sewage treatment plants in Peru are given in TABLE A4-1. Among these plants, only San Juan STP and Carapongo STP (aerated lagoon) which started operation only in 1988, are located in Metropolitan Lima. The plants are classified according to treatment method and scale in TABLE A4-2.

TABLE A4-2 CLASSIFICATION OF INFLOW AND TREATMENT METHOD
OF EXISTING SEWAGE TREATMENT PLANTS IN PERU

TREATMENT METHOD	INFLOW (m ³ /day)				Total
	below 5,000	5,000 10,000	10,000 50,000	Unknown	
Imhoff tank	-	1(1) *1	1(1)	4	6(2)
Waste stabilization pond	8	2(1)	3(1)	13	26(2)
Aerated lagoon	1	1	2(1)	-	4(1)
Trickling filter	-	-	1	1	2(1)
Total	9	4(2)	7(4)	18	38(6)

*1 Combination with two treatment methods in one plant.

According to the classifications given in TABLE A4-2 the treatment method most commonly used in Peru is the waste stabilization ponds method (especially facultative pond method), which constitutes 68% of the total. This is followed by methods using Imhoff tank, aerated lagoon and trickling filter. Scales of treatment of many plants are not known, but most are assumed to be small.

In most cases waste stabilization ponds and aerated lagoons are selected because of their low costs of construction and maintenance, and they do not require high-level treatment techniques. Also they are easily adaptable to natural conditions of warm climate and little rainfall.

San Juan and Carapongo treatment plants are described in Section 4.4 while the outline and operational conditions of Tacna and Arequipa treatment plants are given in the ensuing paragraphs.

TABLE A4-1 EXISTING SEWAGE TREATMENT PLANT IN PERU

Location	Planned Flow(lps)	Classification of Sewage	Treatment Method	Outline
1.San Juan, Lima	160	Domestic Sewage 50,000 persons	Primary and Secondary Facultative Ponds	21 Ponds, 2 Series Total 20 ha.
2.Ventanilla, Lima	-	Domestic Sewage	Primary and Secondary Facultative Ponds	8 Ponds, 2 Series Total 6.5 ha.
3.Los Recaudadores Lima	-	Domestic Sewage	Facultative Ponds	2,900 m ² x 1.23 mD x 1 Pond
4.Puente Piedra Lima	105	Domestic Sewage 40,000 persons	Aerated Lagoon	4,500 m ² x 2.1 mD x 2 Lagoons
5.Huaral, Lima	110	Domestic Sewage 50,000 persons	Facultative Ponds	16,000 m ² x 1.5 mD x 2 Ponds, 5 days
6.Carhuamayo, Junin	-	Domestic Sewage	Facultative Ponds	10,000 m ² x 0.9 mD x 2 Ponds
7.Pto.Chicama, La Libertad	-	Domestic Sewage 6,000 persons	Facultative Ponds	4 Ponds, 2 Series 1.2 m & 0.9 mD
8.Chocope, La Libertad	55	Domestic Sewage 10,000 persons	Aerated Lagoon	1,236 m ² x 3.0 mD x 2 Lagoons, 1.55 day
9.Moche, La Libertad	12	Domestic Sewage 5,000 persons	Facultative Pond	12,000 m ² x 1.1 mD x 1 Pond, 12.7 days
10.Viru, La Libertad	5	Domestic Sewage 4,000 persons	Facultative Ponds	4 Ponds, 2 Series
11.Illimo, Lambayeque	11	Domestic Sewage 5,500 persons	Facultative Ponds	6,000 m ² x 1.1 mD 2 Ponds, 13.7 days
12.Jayanca, Lambayeque	27	Domestic Sewage 6,000 persons	Facultative Ponds	12,000 m ² x 1.1 mD x 1 Pond, 5.5 days
13.Monsefu, Lambayeque	45	Domestic Sewage 18,000 persons	Facultative Ponds	11,000m ² x1.5m x 1Pond 25,000m ² x0.87m x 1Pond
14.Pacora, Lambayeque	-	Domestic Sewage	Facultative Pond	1 Pond
15.Sana, Lambayeque	12	Domestic Sewage 5,000 persons	Facultative Pond	9,000 m ² x 1.1 mD x 1 Pond, 9.5 days
16.Plura, Piura	-	Domestic Sewage	Facultative Ponds	4 Ponds, 2 Series Total 27,000 ha.
17.Tacna, Tacna	180	Domestic Sewage 76,000 persons	Aerated Lagoon + Facultative Ponds	Aer.Lagoon x 2 Fac.Pond x 2
18.Ayacucho	60	-	Imhoff tanks + F.P.	-
19.Ica	270	-	Facultative Ponds	-
20.Nazca	20	-	Facultative Ponds	-
21.Moquegua	30	-	Facultative Ponds	-
22.Lurin	-	-	Imhoff Tank	-
23.Olmos	-	-	Imhoff Tank	-
24.San Pedro de Lajas	-	-	Imhoff Tank	-
25.Chiquian	-	-	Imhoff Tank	-
26.Buenos Aires	-	-	Facultative Ponds	-
27.Arequipa	330	Domestic and Industrial	Imhoff Tank + Percolating Filter	I.T. x 4 Basin P.F. x 2 Basin
28.Sullana	-	-	Facultative Ponds	-
29.Paita	-	-	Facultative Ponds	-
30.Cajamarca	-	-	Facultative Ponds	-
31.Chicha	-	-	Facultative Ponds	-
32.Chopen	-	-	Facultative Ponds	-
33.Huanta	-	-	Percolating Filters	-
34.Juliaca	-	-	Facultative Ponds	-
35.Carapongo Chosica, Chaclacayo	139	Domestic Sewage 69,000 Persons	Aerated Lagoon	94m x 94m x 2.5mD x 4 Basin

Source : 1.Reuse of Waste Water at the San Juan Stabilization Ponds, CEPIS, 1984.
2.Lagunas de Estabilizacion en America Latina, CEPIS.

A4.2 Tacna Sewage Treatment Plant

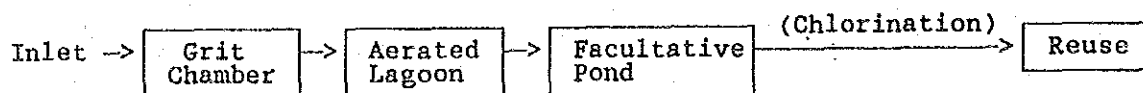
(1) Outline

Tacna STP is located in the Tacna on the southern end of Peru. The treatment plant began operation in 1975 as a facility for experiments on sewage reuse. All treated water is recycled as irrigation water for farm lands and other areas of vegetation. "Servicio de Agua Potable y Alcantarillado de TACNA (SEDATACNA)" is in charge of operation and maintenance work for this plant.

(2) Facilities

The design flow is 15,600 m³/day and the treatment method is aerated lagoon and facultative pond.

Flow diagram

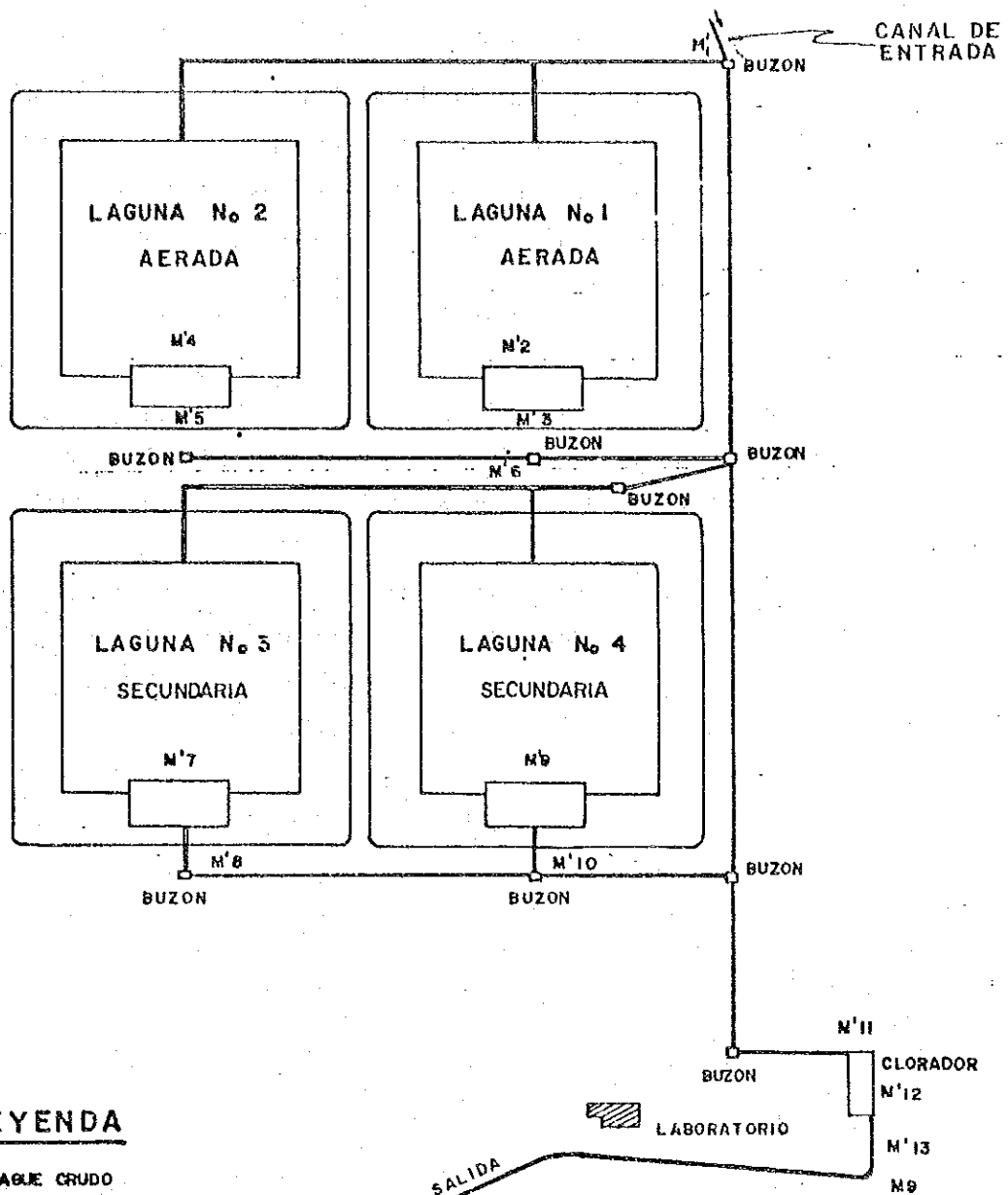


Detailed flow diagram is shown in FIGURE A4-1.

Facility

Aerated Lagoon: Embanked Rectangular Type
98m x 98m x Depth 4.5m x 2 basin
Detention Time 5.5 days
BOD Areal Loading 2,000 kg-BOD/ha.day
Aerator 8 units

Facultative Pond: Embanked Rectangular Type
(no aerator) 98m x 98m x Depth 4.5m x 2 basin
Detention Time: 5.5 days



LEYENDA

- M'1 EFLUENTE DESAGUE CRUDO
- M'2 LAGUNA PRIMARIA No. 1 DETRAS TALUD
- M'3 LAGUNA PRIMARIA No. 1 ZONA TALUD
- M'4 LAGUNA PRIMARIA No. 2 DETRAS TALUD
- M'5 LAGUNA PRIMARIA No. 2 ZONA TALUD
- M'6 CANAL MEZCLA EFLUENTES PRIMARIOS
- M'7 LAGUNA SECUNDARIA No. 3 DETRAS TALUD
- M'8 LAGUNA SECUNDARIA No. 3 ZONA TALUD
- M'9 LAGUNA SECUNDARIA No. 4 DETRAS TALUD
- M'10 LAGUNA SECUNDARIA No. 4 ZONA TALUD
- M'11 ENTRADA CAMARA CONTACTO
- M'12 CAMARA DE CONTACTO
- M'13 SALIDA CAMARA CONTACTO

FIGURE A4-1 FLOWDIAGRAM OF TACNA SEWAGE TREATMENT PLANT

SOURCE: Estudio de Factibilidad del Proyecto de Agua Potable y Alcantarillado de la ciudad de Tacna, Volúmen 2 SENAPA.

(3) Operation

The operational data of the Tacna STP for 1977 are given in TABLE A4-3. The flow was approximately 12,000 m³/day. At the inflow side, concentration of BOD was 278 mg/l and coliform bacteria 2.3×10^7 - 3.8×10^7 MPN/100ml at the discharge side, these values were 38mg/l and 3.1×10^4 - 5.4×10^4 MPN/100ml, respectively.

In April 1989, the average inflow was 17,300 m³/day, concentration of BOD in the inflow 260 mg/l and that in the outflow 45mg/l, indicating a removal rate of about 85%. Judging from the areal BOD load, operation at present is being carried out under conditions of slight overload.

The removal rate of coliform bacteria is high but that of pathogenic bacteria is low. For this reason, construction of maturation ponds has been planned, taking into consideration the reuse of the treated water on agricultural lands and other areas of vegetation.

(4) Field Investigation on the Tacna Sewage Treatment Plant

Field investigation for operation and maintenance condition of sewage treatment plant and sewage reuse project was conducted in October 1989. The result of survey is summarized below:

A. Outline of Tacna City

Population	: Approx. 170,000 persons
Percent of population served by public water supply system	: Approx. 70 % (out of this percentage, 30 % is indirectly supplied by tanker truck)
Percent of population served by sewerage system	: Approx. 50 %

B. Operation and maintenance condition for Sewage Treatment Plant

1) Inflow ;

Planned sewage inflow	: 180 l/s
Actual total sewage flow	: approx.280 l/s (Oct.1989)
Actual inflow into Plant	: approx.200 l/s (Oct.1989)

TABLE A4-3 CHARACTERISTICS OF RAW SEWAGE AND TREATED WATER INTACNA STP,
1977

Parameter	Range (mg/l)	
	Raw Sewage	Treated Sewage
Color	gray	green
pH	6.0 - 7.55	7.0 - 7.83
Electric Conductivity*	940 - 2400	936 - 1970
Alkalinity	92 - 234	104 - 196
Sulfates	325 - 530	315 - 678
Chloride	99 - 138	88 - 130
Sodium Chloride	163 - 219	145 - 215
BOD5	278	38
Total Nitrogen	48	18
Anmonium Nitrogen	No Investigation	12
Ortophosfate	No Investigation	13
Total Residue	1120 - 1390	910 - 1275
Disorved Residue	840 - 1140	850 - 1175
Suspended Residue	210 - 285	60 - 100
Sedimentable Residue**	2.8 - 9.5	0.23 - 0.46
Total Coliform***	2.3 x 10 ⁷ - 3.8 x 10 ⁷	3.1 x 10 ⁴ - 5.4 x 10 ⁴
Fecal Coliform***	4.3 x 10 ⁶ - 1.7 x 10 ⁷	2.7 x 10 ³ - 1.4 x 10 ³
Salmonella sp.***	47 - 666	6 - 58
Shigella sp.	---	---

* : $\mu\text{mho/cm}$ at 25°C

** : ml/l

*** : MNP/100 ml.

--- : Not Detected

Source: Estudio de Factibilidad del Proyecto de Agua Potable Y Alcantarillado de la Ciudad de Tacna, Volúmen 2, SENAPA.

Differential flow of 80 l/s of raw sewage is diverted before it reaches the Plant and is used for irrigation without treatment.

2) Water Quality ;

	Raw Sewage	Treated Water
BOD (mg/l)	: 197	--
SS (mg/l)	: 248	80
Total Residue (mg/l)	: 1,255	1,093
Total Coliforms (MPN/100 ml)	: $1.0 \times 10^7 - 2.6 \times 10^7$	$2.4 \times 10^5 - 5 \times 10^5$
Fecal Coliforms (MPN/100 ml)	: $3.6 \times 10^6 - 2.1 \times 10^7$	4.1×10^5

Source : Direction Regional de Agriculture

3) Operational Condition ;

- Aerators are of the vertical floating type. Each of the two lagoons are equipped with 4 sets (10HP) of aerators. Aerators have been running 24 hrs continuously and have been maintained in good condition with no trouble since start of operation.
- As for treatment condition, Aerated Lagoon is overloaded and is in anaerobic condition, which situation is observed to have been brought about by the following causes;
 - i. The sludge was never removed since start of operation, thus has presently accumulated to about half of the depth. Actual detention time, therefore, has been reduced to 2.5 days which is not sufficient for treatment. As a rule, periodic removal of accumulated sludge in the Lagoon should be carried out.
 - ii. Considering the inlet load, aerators lack about 40 % of necessary capacity.
 - iii. Water depth (4.5 m) is so great that mixing in the Lagoon is not sufficient.
- SEDATACNA has prepared the plans for a new plant to be constructed at another site in order to treat the actual total sewage flow quantity.

4) Operation and Maintenance Cost and Organization in Charge

- Power consumption per one month was about 20,000 kWh and power charge was estimated at US\$ 6,000 per annum.
- O/M cost was defrayed by the tariff collections from water supply and sewage charges of SEDATACNA.

C. Present Condition for Agriculture Area of Sewage Reuse

1) Total Irrigation Area by Treated Water Reuse:

Approx. 252 ha, at present

2) Category of Farm Products in the Irrigation Area:

Farm products are as follows:

Starchy corn

Yellow corn

Chili

Squash

Alfalfa

Sweet potato

Potato

3) Irrigation Method:

Flooding by ridge method.

Local water rate is about 0.8 l/s/ha.

4) Organization and Administration of the Area:

Agricultural Cooperation "Tupac Maru", consists of 52 families.

5) Water Tarif from Farmer:

In principle water tarif should be collected from farmer, however, it is not collected at present.

6) Marketing Area:

Mainly, the City of Tacna.

A4-3 Chilpina Sewage Treatment Plant, Arequipa

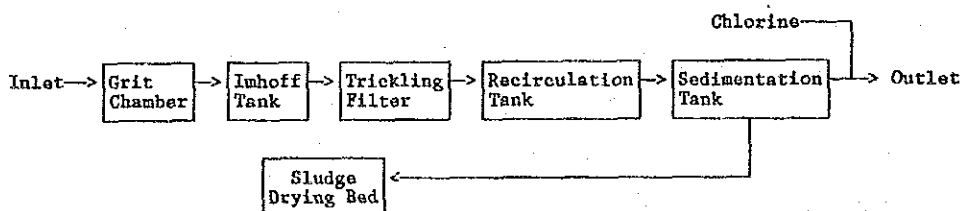
(1) Outline

Chilpina STP is located in Arequipa, a city south of Peru which is the second largest city in the country next to Lima. The treatment plant began operation in 1978. The inflow into the plant contains a high proportion of industrial waste water mainly from food industries. As in the Tacna STP treated water treated is returned to agricultural lands. Operation and maintenance of this plant is undertaken by SEDAPAR (Arequipa).

(2) Facilities

The treatment facilities consist of Imhoff tanks and trickling filters.

Flow diagram



Detailed flow diagram is shown in FIGURE A4-2.

Facility

Imhoff Tank : 4 basins
Trickling Filter : 600 m² x 1.8 mH x 2 basins
Sedimentation Tank : Circular Tank with clarifier
630 m³ x 1 basin, 300 m³ x 2 basins

(3) Operation

According to the records of April 1989, the average daily inflow was 28,330 m³/day. BOD concentration at the inflow side was 439.1 mg/l and that of coliform bacteria was 4 x 10⁶ MPN/100 ml. BOD concentration of the treated water was 221.6 mg/l and coliform bacteria was 8 x 10⁵ MPN/100ml. The removal rates of BOD and SS were approximately 50% and 87%, respectively. These data were obtained at average operational conditions. An aver-

age of 0.5 ppm of chlorine is injected for disinfection.

There has been no machinery breakdowns since it operation but the concentration of the inflow is high because of the influence of industrial wastewater and the plant as a whole is thought to be operating under over-load condition.

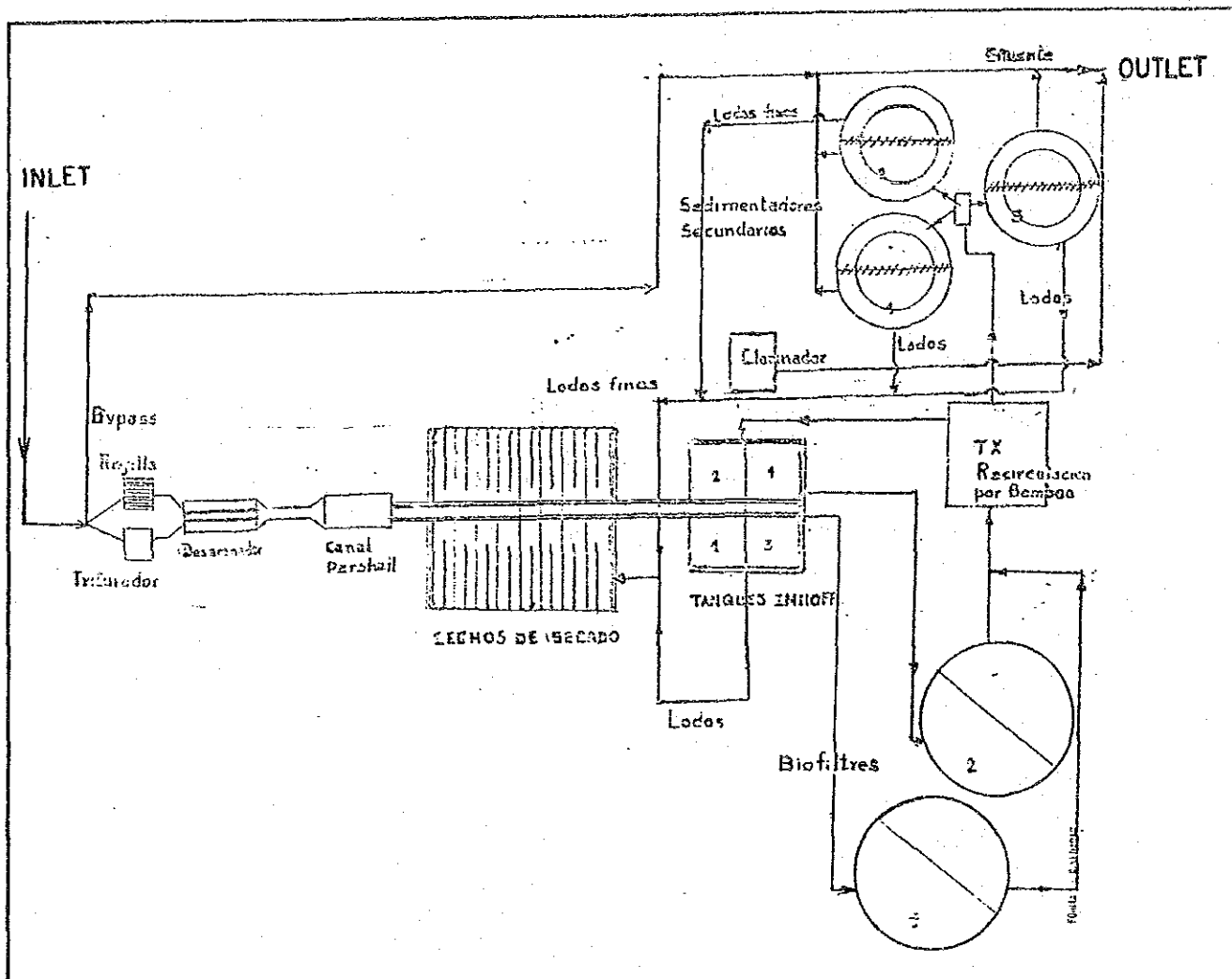


FIGURE A4-2 FLOWDIAGRAM OF CHILPINA SEWAGE TREATMENT PLANT

SOURCE: SEDAPAR.

APPENDIX 5

RESULTS OF SEWAGE QUALITY ANALYSIS

TABLE A5-1 QUALITY OF SEWAGE IN COLLECTORS
(June 19 to 20, 1989)

(1/2)

SAMPLE	Cir.	Surco.	B-Sur.	21	22	23
PARAMETER	11	12	13	19-6	19-6	19-6
DATE	19-6	19-6	19-6	16:00	16:00	16:00
TIME	10:00	10:00	10:00			
Conductivity Umhos/cm.	1,260	1,130	990	1,390	1,060	900
Total Residue mg/l.	736		950	1,182		868
Alcalinity (CaCO ₃) mg/l.	292	320	254	240	234	200
Total Hardness (CaCO ₃) mg/l.	400	345	315	380	390	300
Chloride (Cl.) mg/l.	116	78	65	164	84	58
Sulfates (SO ₄) mg/l.	350	150	150	200	188	175
Nitrites + Nitrates (N.) mg/l.	0.2	0.16	0.18	0.24	0.24	0.06
COD (Mn) mg/l.	54.4	47.6	23.1	25.8	28.5	21.7
BOD mg/l.	340	282	262	222	505	335
Total Coliform MPN/100 ml	4.6x10 ⁷	1.1x10 ⁸	1.1x10 ⁸	4.6x10 ⁷	2.4x10 ⁷	2.4x10 ⁷

(2/2)

SAMPLE	31	32	33	41	42	43
PARAMETER	19-6	19-6	19-6	20-6	20-6	20-6
DATE	22:00	22:00	22:00	4:00	4:00	4:00
TIME						
Conductivity	Umhos/cm.	1,350	1,050	910	1,540	800
Total Residue	mg/l.	1,072	840	818	1,096	634
Alcalinity	(CaCO ₃)	250	214	418	184	159
Total Hardness	(CaCO ₃)	380	350	330	440	330
Chloride	(Cl.)	131	93	58	259	64
Sulfates	(SO ₄)	150	200	163	138	350
Nitrites + Nitrates (N.)	mg/l.	0.12	0.09	0.04	1.4	0.5
COD (Mn)	mg/l.	8.9	8.1	6.5	4.9	2.4
BOD	mg/l.	110	175	145	23	50
Total Coliform	MPN/100ml	4.6x10 ⁷	2.4x10 ⁷	4.6x10 ⁷	9.3x10 ⁶	4.3x10 ⁶

TABLE A5-2 PHYSICAL CHEMISTRY ANALYSIS RESULTS ON COLLECTOR SURCO AND OTHERS
(Oct. 19 to 20, 1989)

SURCO SEWER AND OTHER		Civ.		Surco		B-Sur		(1/2)					1989
SAMPLE	DATE	21	22	23	31	32	33	41	42	43	51	52	53
PARAMETER	DATE	19-10	19-10	19-10	19-10	15-10	19-10	19-10	19-10	19-10	20-10	20-10	23-10
TIME		10:00	10:00	10:00	14:00	16:00	16:00	22:00	22:00	22:00	04:00	04:00	04:00
P.H.		7.2	6.95	7.05	6.95	6.15	6.45	6.75	6.60	6.65	6.75	6.60	5.75
CONDUCTIVITY	Umhos/cm.	1,200	1,100	925									
TOTAL RESIDUE	mg/l.	1,372	1,220	918	1,156	1,256	942	1,312	1,014	1,005	894	782	652
SUSPENDED SOLIDS	mg/l.	378	352	248	226	350	254	192	220	365	140	224	92
CHLORIDE	Cl.	162	96	58	164	127	55	165	100	55	136	53	36
NITRITE + NITRATES	N.	0.01	0.0	0.01	0.05	0.04	0.03	0.1	0.1	0.2	0.2	0.3	0.25
AMMONIA	as N.	28.6	27.1	25.7	16.0	16.2	15.6	15.1	16.2	16.5	10.0	7.5	9.0
ORGANIC NITROGEN	as N.	18.0	22.0	15.0									
TOTAL PHOSPHORUS	as P.	10.4	8.9	7.4	7.2	7.4	5.5	4.3	2.0	1.7	2.3	1.9	1.8
ORTHOPHOSPHATE	mg/l.	4.5	4.4	4.4	4.1	4.2	4.6	5.0	3.9	4.2	1.9	1.9	1.5
SO ₄	mg/l.	306	277	273	230	304	225	233	180	176	135	105	94
CO ₂ (Mn)	mg/l.	160	176	106	84	112	76	80	98	72	32	48	20

OFICINA DE CONTROL DE CALIDAD DE AGUA Y DESAGUE

PHYSICAL CHEMISTRY ANALYSIS RESULTS (3/2)

1989

SURCO SEWER AND OTHER

PARAMETER	DATE	11	12	13
TIME	18-10	18-10	18-10	18-10
	10:00	10:00	10:00	10:00
P.H.	7.05	6.95	6.95	6.95
CONDUCTIVITY	Umhos/cm	-	-	-
TOTAL RESIDUE	mg/l.	-	-	-
SUSPENDED SOLIDS	mg/l.	-	-	-
CHLORIDE	Cl. mg/l.	-	-	-
NITRITE + NITRATES	N. mg/l.	0.4	0.02	0.09
AMMONIA	mg/l.	26.0	28.0	23.5
ORGANIC NITROGEN	as P mg/l.	-	-	-
TOTAL PHOSPHORUS	as P mg/l.	9.0	10.2	9.2
ORTHOPHOSPHATE	mg/l.	4.6	4.8	4.5
BOD	mg/l.	378	334	254
COD (Mn)	mg/l.	120	163	114

TABLE A5-3 BACTERIOLOGICAL ANALYSIS RESULTS ON COLLECTOR SURCO AND OTHERS
(Oct. 19 to 20, 1989)

1989

SURCO SEWER AND OTHERS Cir. Surco. B-Sur

SAMPLE	11	12	13	21	22	23	31	32	33	41
DATE	18-10	18-10	18-10	19-10	19-10	19-10	19-10	19-10	19-10	19-10
PARAMETER										
TIME	10:00	10:05	10:10	10:00	10:05	10:10	16:00	16:05	16:10	22:00
TOTAL COLIFORM. MPN/100 ml.	2.4×10^8	9.5×10^7	4.3×10^7	4.6×10^8	2.3×10^7	4.3×10^7	7.5×10^7	9.3×10^7	4.3×10^7	7.5×10^7
FECAL COLIFORM. MPN/100 ml.	9.3×10^7	4.3×10^7	2.3×10^7	2.4×10^3	2.3×10^7	4.3×10^7	4.5×10^7	2.3×10^7	2.3×10^7	4.3×10^7
SAMPLE	42	43	51	52	53					
DATE	19-10	19-10	20-10	20-10	20-10					
PARAMETER										
TIME	22:05	22:10	04:00	04:05	04:10					
TOTAL COLIFORM. MPN/100 ml.	4.5×10^8	4.3×10^7	9×10^6	9×10^6	4×10^6					
FECAL COLIFORM. MPN/100 ml.	2.4×10^8	4.3×10^7	4×10^6	9×10^6	4×10^6					

REMARK

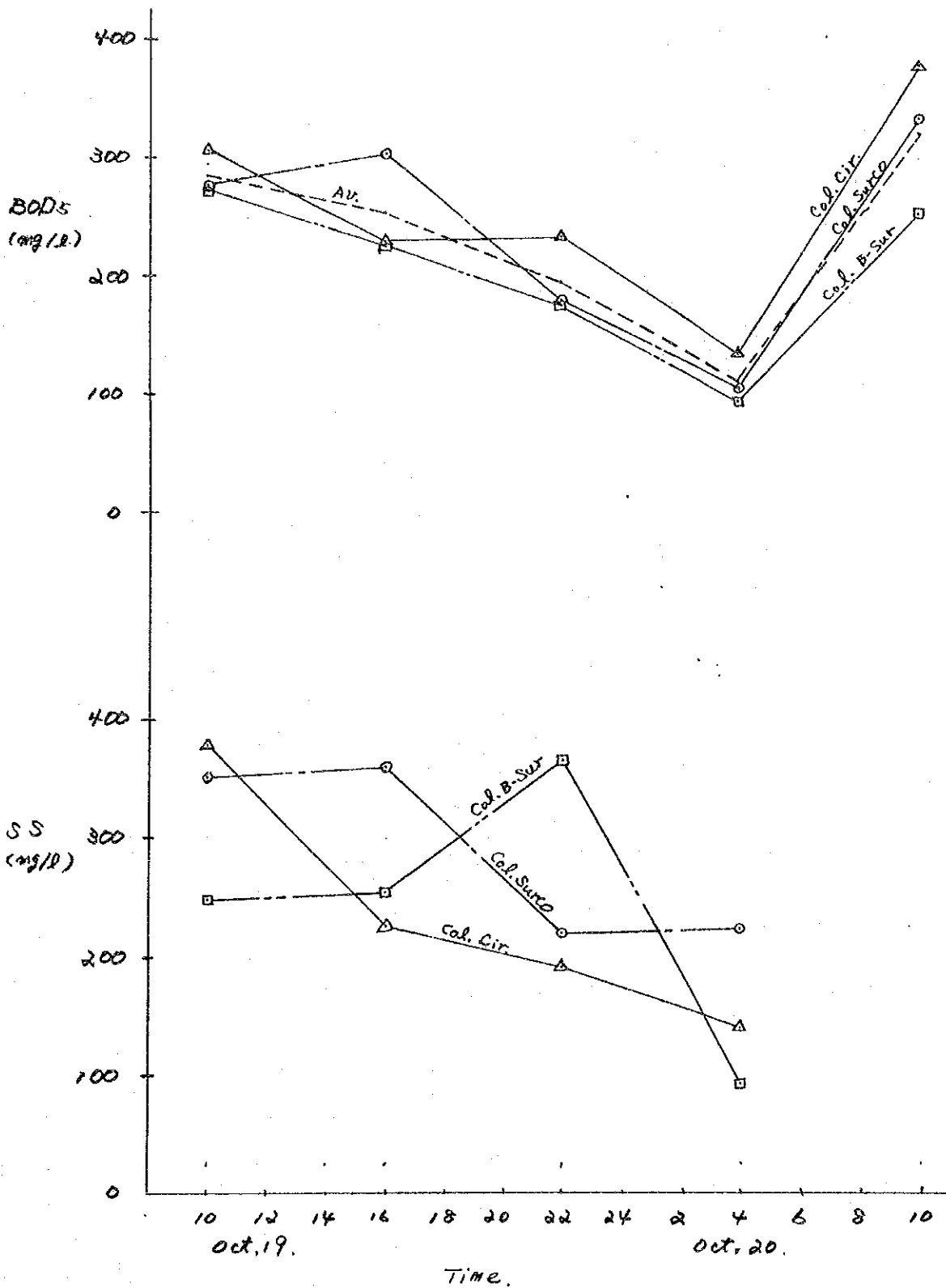


FIGURE A5-2 VARIATION OF RAW SEWAGE QUALITY ON COLLECTOR SURCO AND OTHERS
(BOD, SS Oct. 19 to 20, 1989)

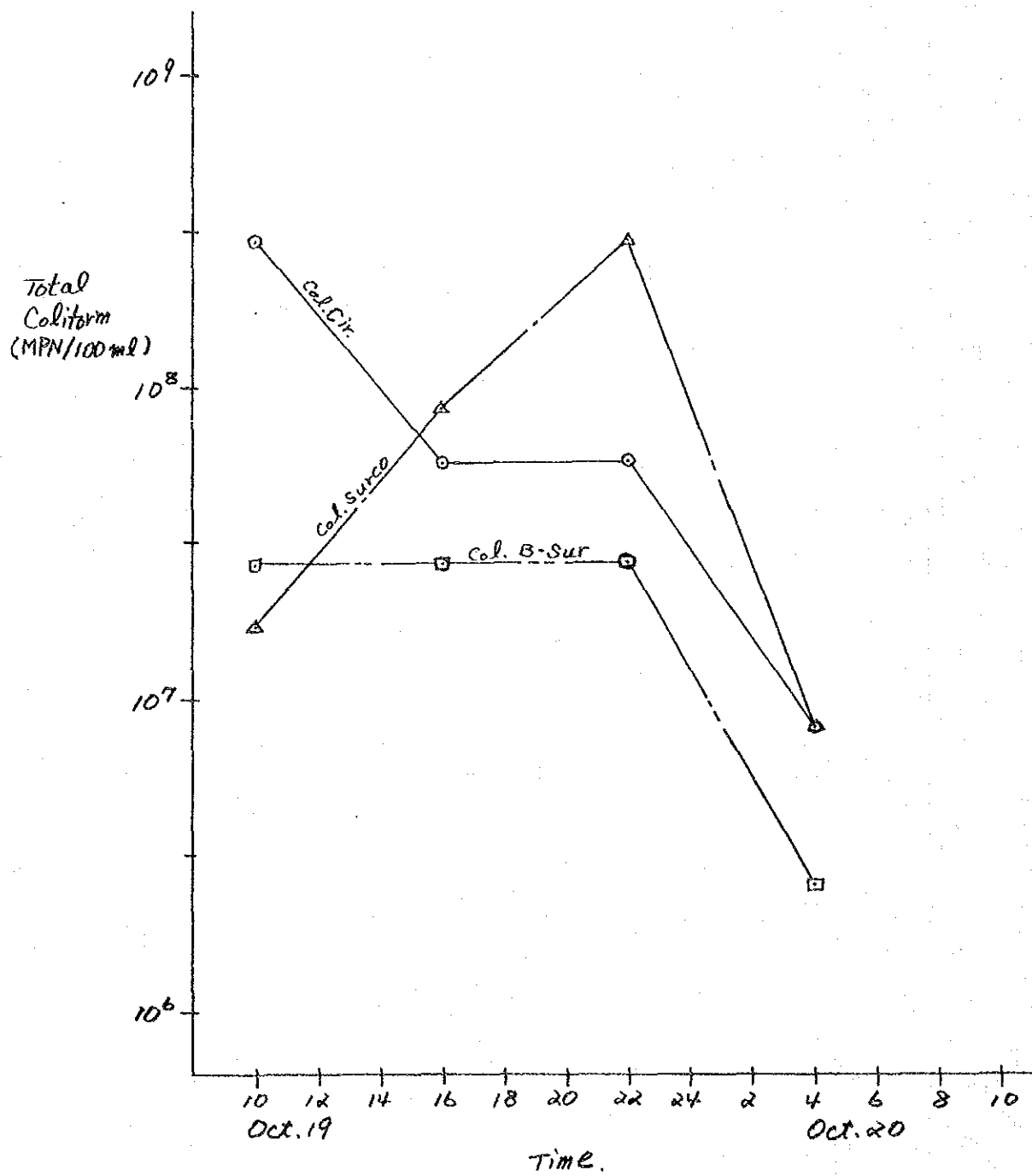


FIGURE A5-3 VARIATION OF RAW SEWAGE QUALITY ON COLLECTOR SURCO AND OTHERS (Total Coliforms, Oct. 19 to 20, 1989)

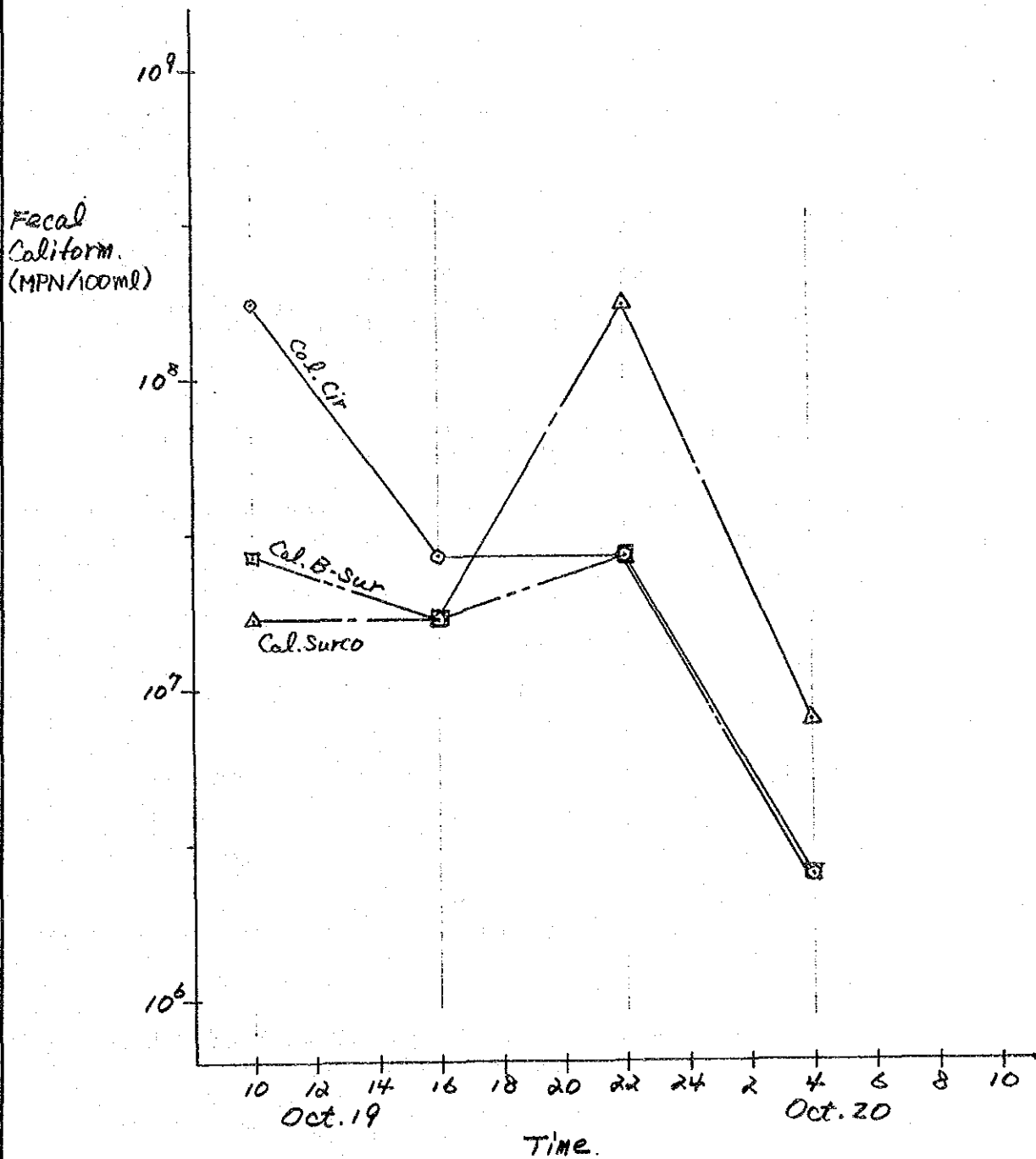


FIGURE A5-4 VARIATION OF RAW SEWAGE QUALITY ON COLLECTOR SURCO AND OTHERS (Fecal Coliforms, Oct.19 to 20, 1989)

TABLE A5-4 QUALITY OF SEWAGE IN THE CANAL IN PARQUE ZONAL
No.26 (1989)

Item	Time	June 1		
		May 31 14:15	10:25	13:15
NO3+NO2-N (mg/l)		1.5	0.3	0.15
NH4-N (")		59.0	53.2	59.0
Organic-N (")		--	36.4	--
Total - P (")		14.0	8.7	11.8
BOD (")		542	405	213
COD (Mn) (")		68	74	72
Total Coliform (MPN/100 ml)		1.1x10E9	4.6x10E8	4.6x10E8
Fecal Coliform (MPn/100ml)		2.4x10E8	2.4x10E8	2.4x10E8

TABLE A5-5 QUALITY OF SEWAGE AT CARAPONGO STP
(May 24, 1989)

Sample No.	1	2	3	4	1	2	3	4
Sampling Date	24-5	24-5	24-5	24-5	24-5	24-5	24-5	24-5
Sampling Time	9:05	9:22	9:40	9:45	14:20	14:25	14:40	14:45
pH	6.9	6.95	7.3	7.45	6.95	7.0	7.2	7.25
Nitrites+Nitrates as N mg/l.	1.5	1.1	1.4	1.6	0.08	0.07	0.5	1.1
Ammonia as N mg/l.	14.1	13.5	11.3	10.4	14.2	12.7	10.7	10.0
Organic Nitrogen as N mg/l.	6.0	3.0	2.8	3.6	NA	NA	NA	NA
Total Phosphorus as P mg/l.	2.7	3.1	3.1	2.9	4	2.8	3.0	2.9
Suspended Solids mg/l.	-	-	-	-	84	70	52	16
Total Residue mg/l.	-	-	-	-	846	646	666	668
BOD mg/l.	280	15	35	17	230	20	20	5
COD (Mn) mg/l.	37	36	10	7	34	32	9	6

TABLE A5-6 NUMBER OF TOTAL COLIFORM OF SEWAGE AT CARAPONGO STP
(May 24, 1989)

Sample	1	2	3	4	1	2	3	4
Date (day-month)	24-5	24-5	24-5	24-5	24-5	24-5	24-5	24-5
Time	9:05	9:22	9:40	9:45	14:20	14:25	14:40	14:45
Total Coliform (CFU/ 1 ml)	1.5x10E5	1x10E3	1x10E3	1x10E2	5x10E5	9x10E3	3x10E2	1x10E2

TABLE A5-7 MLSS VALUES IN CARAPONGO STP (June 2, 1989)

Pond No. 1								
Sample Number	Depth (m)	MLSS (mg/l)	Sample Number	Depth (m)	MLSS (mg/l)	Sample Number	Depth (m)	MLSS (mg/l)
101	0.5	22	111	0.5	60	122	0.5	76
102	1.2	56	112	1.2	130	121	1.2	1,576
103	2.3	108	113	2.3	2,056	123	2.3	6,856

Pond No. 2			Sedimentation Tank No. 4		
Sample Number	Depth (m)	MLSS (mg/l)	Sample Number	Depth (m)	MLSS (mg/l)
201	0.5	4	301	0.5	80
202	1.2	24	302	1.2	108
203	2.3	664	303	2.3	120

TABLE A5-8 WATER QUALITY AT CARAPONGO STP (June 8, 1989)

Item	Point No.*	1	2	3	4
NO3 + NO2-N (mg/l)		1.6	0.4	1.1	1.2
NH4-N (")		19.3	12.6	10.9	10.6
Organic-N (")		12.7	11.2	8.4	6.4
Total - P (")		3.8	3.2	1.2	1.0
Suspended Solid (")		251	55	26	10
Total Residue (")		935	692	650	608

* Sampling Point : 1. Raw Sewage
 2. After Pond No. 3
 3. Before Sedimentation
 4. Treated Sewage

TABLE A5-9 WATER QUALITY OF SAN JUAN STABILIZATION POND
(Upper Battery, April 12, 1989)

Item	Sampling Point	Entrance	Exit
Temp (C)		24	26
E.C (uS/cm)		790	680
pH		6.8	7.4
Cl- (mg/l)		56	42
SO4 (")		150	150
NO3-N (")		0.1	0.8
NH4-N (")		22	16
Total-P (")		6.4	4.6
SS (")		306	114
TR (")		830	494
BOD (")		263	100 *(34)
TOC (")		135	48
IOC (")		86	38
Total Coliform (MPN/100ml)		1.1x10E8	4.6x10E5
Fecal Coliform (MPN/100ml)		2.4x10E7	2.4x10E5

* Dissolved BOD

TABLE A5-10 WATER QUALITY OF SAN JUAN STABILIZATION POND
AT EACH POND (June 7, 1989)

ITEM	Sampling Point					
	1	2	3	1	2	3
NO3+NO2-N (mg/l)	0.14	0.41	0.50	0.08	0.36	0.32
NH4-N (")	17.4	22.6	17.9	13.4	20.7	19.0
Organic-N (")	12.8	10.7	10.1	-	-	-
Total - P (")	6.7	5.2	5.0	4.6	4.2	5.1
SS (")	206	31	23	88	29	22
TR (")	926	748	731	802	744	727
BOD (")	300	90	120	230	120	120
COD (Mn) (")	55.0	14.6	11.7	70.4	17.6	13.2
Total Coliform (MPN/100ml)	8 2.4x10	7 1.1x10	6 2.4x10	7 4.3x10	6 4.3x10	5 2.4x10
Fecal Coliform (MPN/100ml)	7 2.4x10	6 2.4x10	5 2.4x10	7 2.4x10	5 2. x10	4 2.4x10

TABLE A5--11 RESULTS OF PREVIOUS WATER QUALITY ANALYSIS
AT SAN JUAN STABILIZATION POND

(mg/l)

Sampling Point	Date	As	Cd	Cu	Fe	Pb	Zn	Na	K
Effluent of Lower Battery	4.20.87	4.0	<0.005	14.0	124.0	<10	66.0	52,000.0	15,220.0
	8. 3.87	0.0	<0.005	17.0	105.0	<10	52.0	35,840.0	9,460.0
	8.10.87	10.0	<0.005	17.0	140.0	<10	110.0	40,285.0	2,030.0

(mg/l)

Sampling Point	Date	pH	Temp (C)	Pb	Cd	Cu	Fe	Zn
Lower Battery	11.11.86	6.8	22.0	10	<0.005	6	300	32
Upper Battery	11.17.86	7.1	22.5	<10	<0.005	12	300	63

Sampling Point	Date	Water Temp	Amb. Temp	pH	DO mg/l	Total Coli MPN/100ml	Fe.Coli MPN/100ml	Salmonella MPN/100ml	NH4+ mg/l	COD mg/l	BOD mg/l
Eff.of Lower Batt.	11.11.86	22.0	20.0	6.8	4.7	5.0x10E5	5.0x10E5	7.5x10	21.5	170.7	36.0
-do-	3.30.87	26.0	24.0	6.2	4.2	9.0x10E5	3.0x10E5	4.3x10	14.0	196.0	38.0
-do-	8.10.87	19.0	17.5	7.6	8.4	1.3x10E5	5.0x10E4	2.3x10	22.0	141.0	49.0
Eff.of Upper Batt.	11.17.86	22.5	23.0	7.1	2.9	5.0x10E6	3.0x10E6	9.3x10	29.0	170.7	49.0
-do-	4.20.87	25.0	23.0	7.2	2.2	9.0x10E6	5.0x10E6	1.1x10E3	28.5	151.0	98.0
-do-	8. 3.87	19.5	17.0	7.5	4.1	3.0x10E5	1.7x10E5	2.3x10	25.5	110.0	51.0

TABLE A5-12 ANALYSIS RESULTS FOR HEAVY METALS ON COLLECTOR SURCO AND OTHERS
(Oct. 9 AND 18, 1989)

SURCO SEWER AND OTHERS		1989			
SAMPLE		SURCO ⑤ SEWER	CIRCUNVALACION ④ SEWER	FIR. ① 11	SURCO ② 12
PARAMETER	DATE	9-10	9-10	18-10	18-10
TIME IN HOUR		10:30	13:30	10:00	10:00
pH.		6.95	8.00	7.05	6.95
NITRITES + NITRATES	as N mg/l.	0.04	1.15	0.4	0.02
AMMONIA	as N mg/l.	20.7	30.2	26.0	28.0
ORGANIC NITROGEN	as N mg/l.	16.8	26.3	ND	ND
TOTAL PHOSPHORUS	as P mg/l.	7.8	9.1	9.0	10.2
ORTHOPHOSPHATE	as P mg/l.	3.6	0.9	4.6	4.8
SUSPENDED SOLIDS	mg/l.	268	278	ND	ND
TOTAL RESIDUE	mg/l.	1072	1480	ND	ND
BOD	mg/l.	233	169	378	334
COD (Mn)	mg/l.	50	28	120	163
MERCURY	Hg ug/l.	1.3	0.3	0.9	1.3
CADMIUM	Cd. mg/l.	0.010	0.020	0.005	0.008
LEAD	Pb. mg/l.	0.02	0.27	0.08	0.12
CHROMIUM	Cr. mg/l.	0.00	0.00	0.00	0.00
IRON	Fe. mg/l.	1.27	1.20	1.26	1.44
MANGANESE	Mn mg/l.	0.06	0.08	0.06	0.06
COPPER	Cu mg/l.	0.10	0.06	0.22	0.08
ZINC	Zn mg/l.	0.53	0.40	0.32	0.41
TOTAL COLIFORM	MPN/100 ml	4.3x10 ⁷	4.3x10 ⁷	2.4x10 ⁸	9.3x10 ⁷
FECAL COLIFORM	MPN/100 ml	2.3x10 ⁷	2.3x10 ⁷	9.3x10 ⁷	4.3x10 ⁷

SURCO SEWER IS NEAR TOMAS MARZANO AVENUE...
 CIRCUNVALACION SEWER IS TAKED AT PANAMERICAN HIGH WAY
 ST 11 (SURCO) IS AT MEXICO STREET
 ST 12 (CIRCUNVALACION) IS NEAR MEXICO STRET
 ND : NOT DETERMINED

TABLE A5-13 COMPARISON OF HEAVY METALS IN COLLECTORS RAW SEWAGE AND STANDARD

Parameter	Past Data	Analysis Result in Field Work	Water Quality Standard *
	Collector Surco	Collector Surco and Cir.	CLASS - III
Date	Nov. 1984	Oct. & Nov. 1989	-
Mercury Hg μ g/l	-	0.3 - 1.3	10
Cadmium Cd mg/l	0.01 - 0.03	0.005 - 0.02	0.05
Lead Pb mg/l	0.15 - 0.35	0.02 - 0.27	0.1
Chromium Cr mg/l	-	0.00	1.0
Iron Fe mg/l	3.2 - 6.25	1.2 - 1.44	1.0
Manganese Mn mg/l	0.05 - 0.12	0.06 - 0.08	0.5
Copper Cu mg/l	0.1 - 0.55	0.06 - 0.22	0.5
Zinc Zn mg/l	0.16 - 0.34	0.32 - 0.53	25
Arsenic As mg/l	0.02 - 0.04	-	0.2

Remark ; * : Ley General de Aguas, Decreto Ley No.17752, Nov. 1983
 Government of Peru.
 Based upon the Standards of EPA, United States.

APPENDIX 6

POPULATION PROJECTION BY THREE METHODS

APPENDIX 6 POPULATION PROJECTION BY THREE METHODS

Based on the estimated population by district in each census year shown in TABLE 4-5 in the Section 4.1 of Main Report, population in the future can be projected by the following three methods:

(1) Logistic Curve Method

This curve shows the population change with respect to time which is assumed to start from zero in the distant past, initially increases slowly, rises abruptly at about midpoint, gradually decreases in growth rate as it passes the point of inflection, and reaches saturation point in the distant future.

Its formula is expressed as:

$$P_x = \frac{K}{1 + e^{(a-bx)}} \quad (1)$$

where: P_x : population in year x
 x : time in years from the base year
 e : natural logarithm base
 k : saturation population
 a, b : constants

The population of 1961, 1971* and 1981 were used as a base to calculate the saturated population (theoretical) through the logistical curve formula. Population in 1971* is calculated by interpolating the figures from the 1961 and 1981 census.

TABLE A6-2 shows the saturation populations calculated mathematically. Of these values, saturation populations of some districts which do not have theoretical solution were calculated using estimated saturation population densities based on the past population densities or those compared with other districts (TABLE A6-1), and areas measured from the map.

Using the saturation population and census figures, the future population was calculated through the logistic curve method.

TABLES A6-3 and A6-4 show the calculated values and the parameters for calculation, respectively.

As a result of estimates made by this method, the following population projections were obtained:

<u>YEAR</u>	<u>POPULATION</u>
1989	5,896,600
1990	6,035,200
1995	6,673,600
2000	7,239,000

TABLE A6-1

POPULATION DENSITY (as of July 30, 1961, 1972, 1981)

(unit: pop./ha)

DISTRICT	AREA (ha)	1961	1972	1981
LIMA	2,121	123.72	172.80	184.12
ANCON	28,883	0.14	0.20	0.31
ATE	9,822	8.24	6.44	14.13
BARRANCO	273	160.07	185.88	179.15
BREÑA	320	321.25	362.60	369.60
CARABAYLLO	35,557	1.22	0.82	1.56
CHACLACAYO	4,405	2.18	5.04	7.55
CHORRILLOS	3,681	9.05	25.56	40.56
CIENEGUILLA	24,470	0.05	0.11	0.20
COMAS	4,621	21.08	38.91	64.46
EL AGUSTINO	1,836	41.99	66.15	96.15
INDEPENDENCIA	1,336	63.70	85.20	108.47
JESUS MARIA	425	203.76	204.68	205.94
LA MOLINA	4,169	0.50	1.49	7.14
LA VICTORIA	909	222.00	302.24	313.45
LINCE	282	300.71	304.53	300.21
LURIGANCHO-CHOSICA	24,667	1.36	2.16	2.78
LURIN	33,251	0.19	0.40	0.54
MAGDALENA DEL MAR	318	180.50	184.96	183.76
PUEBLO LIBRE	470	150.21	172.05	188.03
MIRAFLORES	912	99.89	113.20	119.36
PACHACAMAC	15,364	0.79	0.31	0.46
PUCUSANA	3,027	0.59	0.97	1.43
PUENTE PIEDRA	6,267	1.37	3.13	5.70
PUNTA HERMOSA	293	1.02	3.21	3.63
PUNTA NEGRA	278	1.44	2.76	2.09
RIMAC	1,215	122.30	146.94	159.77
SAN BARTOLO	15,134	0.07	0.10	0.20
SAN BORJA	1,046	51.24	65.83	56.66
SAN ISIDRO	1,007	38.73	65.06	72.20
S. J. DE LURIGANCHO	14,034	1.66	6.44	19.45
S. J. DE MIRAFLORES	2,351	27.22	47.14	74.19
SAN LUIS	356	24.16	70.43	149.74
SAN MARTIN DE PORRES	5,791	17.25	41.44	73.56
SAN MIGUEL	1,018	23.48	64.40	102.56
SANTA MARIA DEL MAR	755	0.13	0.06	0.13
SANTA ROSA	1,982	0.05	0.11	0.26
SANTIAGO DE SURCO	3,493	13.34	19.99	42.11
SURQUILLO	413	56.90	72.14	237.94
VILLA EL SALVADOR	3,368	-	-	42.33
V.M. DEL TRIUNFO	7,149	13.26	26.31	26.28
TOTAL (PROV. LIMA)	267,069	7.96	11.58	16.41
CALLAO	4,565	27.29	45.05	59.25
BELLAVISTA	456	97.15	90.44	151.64
CARMEN DE LA LEGUA	212	86.32	127.25	186.31
LA PERLA	275	80.00	125.92	175.86
LA PUNTA	75	81.33	92.21	85.55
VENTANILLA	7,352	2.34	2.36	2.74
TOTAL (PROV. CALLAO)	14,698	15.82	22.64	30.90
TOTAL (METRO. LIMA)	281,767	8.37	12.15	17.16

NOTE: 1/ Total area of Provincia de Constitucional de Callao include the area of island (1,763 ha).

TABLE A6-2 ESTIMATED SATURATION POPULATION

DISTRICT	TOTAL AREA (ha)		POPULATION		AREA TO BE INHABITED		POPULATION DENSITY (pop./ha.)		
	TOTAL AREA	1981	SATURAT'N	%	ha	1981	SATURAT'N	TOTAL AREA	INHABITED
LIMA	2,121	390,513	396,800 *	100	2,121	184.12	187.08	187.08	
ANCON	28,883	8,865	288,800	10	2,888	0.31	10.00	100.00	
ATE	9,822	138,746	392,800	20	1,964	14.13	39.99	200.00	
BARRANCO	3,273	48,907	68,300	100	273	179.15	230.00	250.00	
BREÑA	320	118,271	118,800 *	100	320	369.60	371.25	371.25	
CARABAYLLO	35,557	55,558	355,600	10	3,556	1.56	10.00	100.00	
CHACLAAYO	4,405	33,243	45,300 *	10	441	7.55	10.28	102.72	
CHORRILLOS	3,681	149,294	194,600 *	50	1,841	40.56	52.87	105.70	
CIENEGUILLA	24,470	4,783	122,400	5	1,224	0.20	5.00	100.00	
COMAS	4,621	297,870	462,200	2,311	2,311	64.46	100.02	200.00	
EL AGUSTINO	1,836	176,537	257,000	1,285	1,285	96.15	139.98	200.00	
INDEPENDENCIA	1,336	144,918	187,000	1,935	1,935	108.47	139.97	200.00	
JESUS MARIA	4,425	87,525	106,300	425	425	205.94	250.00	250.00	
LA MOLINA	4,169	29,786	417,000 *	2,085	2,085	7.14	100.02	200.00	
LA VICTORIA	909	284,922	286,400 *	909	909	313.45	315.07	315.07	
LINCE	282	84,660	86,000	282	282	306.21	305.00	305.00	
LURIGANCHO-CHOSICA	24,667	68,542	246,700	2,467	2,467	2.78	10.00	100.00	
LURIN	33,251	18,104	332,500	3,325	3,325	0.54	10.00	100.00	
MAGDALENA DEL MAR	318	58,437	79,500	318	318	183.76	290.00	250.00	
POEBLO LIBRE	470	88,374	118,700 *	470	470	188.03	251.28	251.28	
MIRAFLORES	912	108,859	114,700 *	912	912	119.36	125.77	125.77	
PACHACAMAC	15,364	7,134	153,600	1,536	1,536	0.46	10.00	100.00	
FUCUSANA	3,027	4,319	15,100	151	151	1.43	4.99	100.00	
PUNTA HERMOSA	6,267	35,694	286,500 *	1,253	1,253	5.70	45.72	228.65	
PUNTA NEGRA	293	1,063	2,900	29	29	3.63	9.90	100.00	
RIMAC	278	1,582	1,400	14	14	2.09	5.04	100.00	
SAN BARTOLO	1,215	194,123	212,600 *	1,215	1,215	159.77	174.98	174.98	
SAN BORJA	15,134	3,067	151,300	1,513	1,513	0.20	10.00	100.00	
SAN ISIDRO	1,007	59,270	209,200	1,048	1,048	56.68	200.00	200.00	
S. J. DE LURIGANCHO	14,034	72,706	74,500 *	1,007	1,007	72.20	73.98	73.98	
S. J. DE MIRAFLORES	2,351	272,943	561,400	2,807	2,807	19.45	40.00	200.00	
SAN LOUIS	336	174,426	329,200	1,846	1,846	74.19	140.03	200.00	
SAN MARTIN DE PORRES	5,791	53,306	71,200	356	356	149.74	200.00	200.00	
SAN MIGUEL	1,018	426,010	1,197,200 *	4,054	4,054	73.56	206.73	295.31	
STA. MARIA DEL MAR	1,755	104,405	139,800 *	1,018	1,018	102.56	137.43	137.43	
SANTA ROSA	1,982	101	7,600	176	176	0.13	10.07	100.00	
SANTIAGO DE SURCO	3,493	518	9,900	99	99	0.26	4.99	100.00	
SURQUILLO	413	147,105	698,600	3,493	3,493	42.11	200.00	200.00	
VILLA EL SALVADOR	3,368	98,289	103,300	413	413	237.94	250.00	250.00	
V. M. DEL TRIUNFO	7,149	267,039	421,000	1,684	1,684	79.29	125.00	250.00	
TOTAL (PROV. LIMA)	267,069	4,641,771	10,039,200	56,622	21.2	17.38	37.59	177.23	
CALLAO	4,565	270,499	384,700 *	2,283	50	59.25	84.27	168.51	
BELLAVISTA	456	69,148	114,000	456	100	151.64	250.00	250.00	
CARMEN DE LA LEGUA	212	39,498	53,000	212	100	186.31	250.00	250.00	
LA PERLA	275	48,362	68,800	275	100	175.86	250.00	250.00	
LA PUNTA	75	6,416	7,500	75	100	85.55	100.00	100.00	
VENTANILLA	7,352	20,177	367,500	3,676	50	2.74	50.00	100.00	
TOTAL (PROV. CALLAO)	14,698	454,100	995,600	6,977	47.5	30.90	67.74	142.70	
TOTAL (METRO. LIMA)	281,767	5,095,871	11,033,800	63,599	22.6	18.09	39.16	173.49	

Note: 1. Total area of Prov. Callao includes the area of islands (1,763 ha).
 2. Saturation population with an asterisk is calculated as a theoretical solution.

TABLE A6-4

PARAMETERS OF LOGISTIC CURVES

DISTRICT	K		a	b
LIMA	396,800	*	(2.545460)	0.172734
ANCON	288,800		3.842034	0.040417
ATE	392,800		1.178518	0.034866
BARRANCO	68,300		(0.866052)	0.018498
BRENA	118,800	*	(3.786585)	0.177221
CARABAYLLO	355,600		2.018281	0.012184
CHACLACAYO	45,300	*	0.035553	0.116356
CHORRILLOS	194,600	*	0.058113	0.138470
CIENEGUILLA	122,400		3.807638	0.068536
COMAS	462,200		0.328914	0.095153
EL AGUSTINO	257,000		0.003082	0.081117
INDEPENDENCIA	187,000		(0.546288)	0.070313
JESUS MARIA	106,300		(1.510378)	0.002910
LA MOLINA	417,000		3.924067	0.134725
LA VICTORIA	286,400	*	(3.242821)	0.219191
LINCE	86,000		(4.988364)	0.002241
LURIGANCHO-CHOSICA	246,700		1.335552	0.044997
LURIN	332,500		3.286003	0.054343
MAGDALENA DEL MAR	79,500		(1.008970)	0.003479
PUEBLO LIBRE	118,100	*	(0.776887)	0.034656
MIRAFLORES	114,700	*	(2.210296)	0.078659
PACHACAMAC	153,600		3.296488	0.020844
PUCUSANA	15,100		1.408548	0.054207
PUENTE PIEDRA	286,500	*	2.627637	0.076376
PUNTA HERMOSA	2,900		1.092175	0.082409
PUNTA NEGRA	1,400		0.332971	0.031434
RIMAC	212,600	*	(1.667271)	0.075426
SAN BARTOLO	151,300		4.456800	0.056052
SAN BORJA	209,200		0.896713	0.007808
SAN ISIDRO	74,500	*	(2.047553)	0.180092
S.J. DE LURIGANCHO	561,400		1.512843	0.153526
S.J. DE MIRAFLORES	329,200		0.608933	0.076711
SAN LUIS	71,200		0.399178	0.152777
S.MARTIN DE PORRES	1,197,200	*	1.397662	0.090219
SAN MIGUEL	139,900	*	0.120244	0.132903
STA.MARIA DEL MAR	7,600		4.576773	(0.002118)
SANTA ROSA	9,900		3.690010	0.084086
SANTIAGO DE SURCO	698,600		2.009343	0.064905
SURQUILLO	103,300		(0.417556)	0.203127
VILLA EL SALVADOR **	421,000		(0.403829)	0.134345
V.M. DEL TRIUNFO **	715,000		0.287653	0.093012
TOTAL (PROV. LIMA)	10,217,000			
CALLAO	384,700	*	(0.141486)	0.079897
BELLAVISTA	114,000		0.167840	0.042305
CARMEN DE LA LEGUA	53,000		(0.213044)	0.084777
LA PERLA	68,800		(0.093496)	0.080406
LA PUNTA	7,500		(1.919267)	0.018078
VENTANILLA	367,600		2.949835	0.008129
TOTAL (PROV. CALLAO)	995,600			
TOTAL (METRO. LIMA)	11,212,600			

Note: 1. Saturated population with an asterisk is calculated as a theoretical solution.
 2. Formula used for projection is as follows:

$$P_i = K / (1 + (e^{-(a - b * x)})$$

where ;

- P_i ; population in the year i .
 x ; number of years after the basic year.
 basic year ; 1972 (**;1989)
 e ; base of natural logarithm
 K ; constant (saturation population)
 a, b ; constants

(2) Exponential Curve Method

This method can be applied to calculate future population under various increase or decrease conditions.

It is expressed by the formula:

$$P_x = P_o + A * x^a \quad (2)$$

where: P_x : population in x years from the base year

P_o : population in the base year

x : time in years from the base year

A, a : constants

Using the population data shown in TABLE 4-5 as a base, calculate the parameters through the exponential curve formula. The results are shown in TABLE A6-5 while the parameters are in TABLE A6-6.

As a result of estimates made by this method, the following population projections were obtained:

<u>YEAR</u>	<u>POPULATION</u>
1989	6,592,800
1990	6,858,600
1995	8,360,200
2000	10,206,400

TABLE A6-6

PARAMETERS OF EXPONENTIAL CURVE

DISTRICT	1961	A	a
LIMA	262,400	45280.847	0.347170
ANCON	4,000	32.629352	1.670530
ATE	80,900	2892.1633	1.000016
BARRANCO	43,700	23702.897	(0.505918)
BRENA	102,800	7065.5489	0.261618
CARABAYLLO	43,500	602.94555	0.999975
CHACLACAYO	9,600	1007.3612	1.053410
CHORRILLOS	33,300	4552.7516	1.080806
CIENEGUILLA	1,300	27.770930	1.612848
COMAS	97,400	2331.9089	1.486774
EL AGUSTINO	77,100	1738.5631	1.350743
INDEPENDENCIA	85,100	1515.7768	1.226871
JESUS MARIA	86,600	12.365775	1.440336
LA MOLINA	2,100	1.9739111	3.187423
LA VICTORIA	201,800	43171.204	0.218690
LINCE 1/	84,800	0.9984140	
LURIGANCHO-CHOSICA	33,500	1965.3712	0.961657
LURIN	6,400	804.31318	0.893837
MAGDALENA DEL MAR	57,400	4939.4980	(0.521052)
PUEBLO LIBRE	70,600	1134.5689	0.918468
MIRAFLORES	91,100	2634.5940	0.636961
PACHACAMAC 1/	12,100	1.0473357	
PUCUSANA	1,800	47.615552	1.324704
PUENTE PIEDRA	8,600	298.08951	1.505366
PUNTA HERMOSA	300	316.20273	0.294043
PUNTA NEGRA	400	6198.7237	(1.177706)
RIMAC	148,600	5574.3873	0.701009
SAN BARTOLO	1,000	2.0123562	2.314809
SAN BORJA	53,600	809905.79	(1.656266)
SAN ISIDRO	39,000	10123.339	0.401515
S.J. DE LURIGANCHO	23,300	345.03173	2.197844
S.J. DE MIRAFLORES	64,000	1501.0489	1.434768
SAN LUIS	8,600	300.27528	1.670097
SAN MARTIN DE PORRES	99,900	4723.8526	1.413548
SAN MIGUEL	23,900	2965.6464	1.101976
SANTA MARIA DEL MAR 1/	100	1	0.000000
SANTA ROSA	100	1.0267009	2.005897
SANTIAGO DE SURCO	46,600	65.059458	2.451041
SURQUILLO	23,500	0.3071037	4.140132
VILLA EL SALVADOR 2/	142,567	9508.8619	1.170502
V.M. DEL TRIUNFO 2/	187,878	11487.242	1.121766
CALLAO	124,600	7660.3688	0.983684
BELLAVISTA	44,300	1242.2177	1.000049
CARMEN DE LA LEGUA	18,300	241.22373	1.494105
LA PERLA	22,000	659.30518	1.231249
LA PUNTA	6,100	36661.865	(1.586841)
VENTANILLA	17,200	0.0006862	5.101577

Note: Formula used for projection is as follows:

$$P_i = P_{1961} + A * (x^a)$$

$$(1/ P_i = P_{1961} * A^x)$$

where ;

P_i ; population in the year i .

P_{1961} ; population in 1961 (2/ 1981).

x ; number of years after the basic year.

$$x = (\text{calender year}) - 1961$$

A, a ; constants

(3) Geometrical Method

This method calculates the future population assuming that the rate of increase or decrease as obtained from past data remains constant.

It is expressed by the formula:

$$P_x = P_o * (1 + r)^x \quad (3)$$

where: P_x : the population x years from the base year
 P_o : the population in the base year
 r : average annual rate of increase

The rate is calculated from the population data for 1972 and 1981 shown in TABLE 4-5.

Formula (3) is used to calculate the future population using the calculated increase in rate. TABLE A6-7 shows the calculated values.

As a result of estimates made by this method, the following population projections were obtained:

<u>YEAR</u>	<u>POPULATION</u>
1989	6,858,600
1990	7,200,400
1995	9,371,100
2000	12,655,700

APPENDIX 7

RESULTS OF SEWAGE FLOW MEASUREMENT

TABLE A7-1

RESULTS OF SEWAGE FLOW MEASUREMENT (SURCO OUTFALL)

unit : cu.m./sec.

MEASURING TIME hh : mm	May 31 - June 1, 1989					October 19-20, 1989				
	SURCO	CIRCUN.	B.SUR	TOTAL Q	RATE	SURCO	CIRCUN.	B.SUR	TOTAL Q	RATE
10 : 00	4.744	1.358	0.284	6.386	1.189	4.395	1.562	0.289	6.246	1.258
10 : 15	4.791	1.358	0.280	6.428	1.197	4.413	1.578	0.289	6.281	1.265
10 : 30	4.726	1.454	0.276	6.456	1.202	4.413	1.443	0.286	6.141	1.237
10 : 45	4.929	1.345	0.295	6.569	1.223	4.432	1.462	0.286	6.180	1.245
11 : 00	4.855	1.382	0.305	6.542	1.218	4.413	1.457	0.289	6.159	1.240
11 : 15	4.827	1.321	0.304	6.452	1.201	4.422	1.440	0.296	6.159	1.240
11 : 30	4.754	1.269	0.289	6.312	1.175	4.477	1.380	0.288	6.145	1.238
11 : 45	4.901	1.269	0.280	6.450	1.201	4.349	1.372	0.280	6.002	1.209
12 : 00	4.837	1.338	0.273	6.447	1.200	4.331	1.380	0.283	5.993	1.207
12 : 15	4.763	1.294	0.276	6.332	1.179	4.358	1.365	0.278	6.001	1.209
12 : 30	4.837	1.276	0.275	6.387	1.189	4.331	1.360	0.282	5.973	1.203
12 : 45	4.869	1.276	0.271	6.357	1.193	4.340	1.343	0.280	5.963	1.201
13 : 00	4.609	1.251	0.280	6.340	1.180	4.303	1.331	0.278	5.912	1.191
13 : 15	4.618	1.251	0.275	6.344	1.181	4.349	1.321	0.280	5.949	1.198
13 : 30	4.763	1.246	0.271	6.281	1.169	4.340	1.303	0.282	5.925	1.193
13 : 45	4.781	1.271	0.269	6.322	1.177	4.303	1.289	0.277	5.869	1.182
14 : 00	4.661	1.219	0.268	6.149	1.145	4.322	1.276	0.276	5.874	1.183
14 : 15	4.625	1.219	0.265	6.108	1.137	4.303	1.299	0.272	5.874	1.183
14 : 30	4.763	1.222	0.260	6.245	1.162	4.258	1.331	0.263	5.851	1.178
14 : 45	4.680	1.227	0.256	6.163	1.147	4.267	1.274	0.262	5.803	1.169
15 : 00	4.717	1.227	0.253	6.197	1.154	4.312	1.256	0.262	5.831	1.174
15 : 15	4.698	1.246	0.248	6.193	1.153	4.276	1.254	0.262	5.791	1.166
15 : 30	4.689	1.246	0.241	6.177	1.150	4.276	1.234	0.260	5.770	1.162
15 : 45	4.689	1.271	0.251	6.212	1.156	4.267	1.202	0.260	5.729	1.154
16 : 00	4.708	1.271	0.260	6.238	1.161	4.230	1.189	0.254	5.673	1.143
16 : 15	4.579	1.214	0.248	6.041	1.124	4.249	1.177	0.251	5.677	1.143
16 : 30	4.597	1.214	0.242	6.053	1.127	4.212	1.169	0.249	5.631	1.134
16 : 45	4.597	1.209	0.234	6.040	1.124	4.212	1.179	0.242	5.634	1.135
17 : 00	4.597	1.209	0.228	6.034	1.123	4.139	1.172	0.249	5.560	1.120
17 : 15	4.615	1.174	0.225	6.014	1.120	4.185	1.187	0.240	5.611	1.130
17 : 30	4.615	1.179	0.221	6.016	1.120	4.137	1.199	0.233	5.590	1.126
17 : 45	4.579	1.202	0.207	5.938	1.115	4.076	1.189	0.233	5.498	1.107
18 : 00	4.533	1.202	0.209	5.943	1.106	4.166	1.187	0.218	5.571	1.122
18 : 15	4.487	1.259	0.205	5.951	1.108	4.176	1.182	0.210	5.568	1.121
18 : 30	4.432	1.259	0.200	5.891	1.097	4.203	1.119	0.209	5.531	1.114
18 : 45	4.413	1.174	0.195	5.782	1.076	4.103	1.132	0.209	5.443	1.096
19 : 00	4.349	1.174	0.186	5.709	1.063	4.076	1.149	0.210	5.434	1.094
19 : 15	4.358	1.129	0.176	5.664	1.054	4.076	1.147	0.211	5.433	1.094
19 : 30	4.377	1.129	0.178	5.684	1.058	4.030	1.137	0.199	5.366	1.081
19 : 45	4.377	1.134	0.173	5.683	1.058	4.012	1.092	0.186	5.290	1.065
20 : 00	4.386	1.147	0.167	5.699	1.061	3.958	1.104	0.176	5.238	1.055
20 : 15	4.230	1.149	0.162	5.541	1.031	3.877	1.099	0.174	5.150	1.037
20 : 30	4.267	1.149	0.160	5.576	1.038	3.716	1.109	0.157	4.982	1.003
20 : 45	4.258	1.134	0.155	5.546	1.032	3.609	1.104	0.153	4.867	0.980
21 : 00	4.249	1.134	0.151	5.534	1.030	3.689	1.082	0.157	4.928	0.992
21 : 15	4.285	1.139	0.142	5.566	1.036	3.645	1.089	0.144	4.878	0.982
21 : 30	4.285	1.142	0.140	5.567	1.036	3.600	1.092	0.142	4.834	0.973
21 : 45	4.139	1.137	0.138	5.414	1.008	3.662	1.077	0.144	4.883	0.983

TABLE A7-1 (Cont'd)

22 : 00	4.194	1.137	0.133	5.464	1.017	3.689	1.072	0.138	4.899	0.986
22 : 15	4.112	1.112	0.130	5.354	0.997	3.609	1.062	0.136	4.807	0.968
22 : 30	4.103	1.114	0.128	5.345	0.995	3.494	1.054	0.122	4.671	0.941
22 : 45	4.003	1.087	0.125	5.215	0.971	3.512	1.015	0.114	4.641	0.935
23 : 00	3.967	1.089	0.120	5.176	0.963	3.530	1.027	0.113	4.670	0.940
23 : 15	3.943	0.923	0.116	4.979	0.927	3.468	1.012	0.133	4.613	0.929
23 : 30	3.958	0.911	0.107	4.976	0.926	3.433	0.965	0.124	4.522	0.911
23 : 45	3.850	1.034	0.108	4.992	0.929	3.416	0.938	0.116	4.470	0.900
0 : 09	3.850	1.034	0.107	4.991	0.929	3.225	0.946	0.109	4.279	0.862
0 : 15	3.571	1.027	0.113	4.811	0.896	3.199	0.919	0.103	4.221	0.856
0 : 30	3.707	1.030	0.109	4.845	0.902	3.096	0.921	0.093	4.110	0.828
0 : 45	3.680	0.865	0.107	4.652	0.866	2.994	0.916	0.093	4.003	0.806
1 : 00	3.320	0.868	0.105	4.292	0.799	2.894	0.950	0.086	3.930	0.791
1 : 15	3.096	0.906	0.101	4.103	0.764	2.827	0.938	0.085	3.850	0.775
1 : 30	3.045	0.909	0.098	4.052	0.754	2.671	0.955	0.086	3.712	0.747
1 : 45	3.011	0.911	0.095	4.018	0.748	2.638	0.978	0.080	3.696	0.744
2 : 00	3.020	0.911	0.091	4.022	0.749	2.566	0.985	0.080	3.631	0.731
2 : 15	2.894	0.909	0.088	3.890	0.724	2.518	0.987	0.079	3.584	0.722
2 : 30	2.827	0.909	0.087	3.823	0.711	2.518	1.000	0.076	3.594	0.724
2 : 45	2.794	0.911	0.086	3.792	0.706	2.486	0.997	0.078	3.561	0.717
3 : 00	2.769	0.911	0.084	3.765	0.701	2.446	0.955	0.077	3.478	0.700
3 : 15	2.635	0.916	0.082	3.833	0.713	2.534	0.894	0.078	3.506	0.706
3 : 30	2.844	0.921	0.083	3.848	0.716	2.438	0.841	0.078	3.357	0.676
3 : 45	2.827	0.916	0.086	3.830	0.713	2.391	0.846	0.087	3.324	0.669
4 : 00	2.794	0.914	0.088	3.795	0.706	2.367	0.846	0.085	3.298	0.664
4 : 15	2.827	0.839	0.091	3.756	0.699	2.391	0.841	0.082	3.314	0.667
4 : 30	2.852	0.839	0.088	3.778	0.703	2.313	0.848	0.079	3.240	0.652
4 : 45	2.860	0.884	0.089	3.834	0.713	2.383	0.843	0.076	3.302	0.665
5 : 00	2.978	0.887	0.091	3.955	0.736	2.367	0.875	0.079	3.321	0.669
5 : 15	2.877	0.887	0.088	3.852	0.717	2.415	0.877	0.078	3.370	0.678
5 : 30	2.860	0.889	0.091	3.841	0.715	2.462	0.872	0.077	3.411	0.687
5 : 45	2.894	0.923	0.094	3.911	0.728	2.478	0.904	0.083	3.465	0.698
6 : 00	2.919	0.923	0.097	3.939	0.733	2.526	0.911	0.082	3.519	0.708
6 : 15	2.994	0.941	0.094	4.029	0.750	2.510	0.990	0.086	3.586	0.722
6 : 30	3.020	0.941	0.094	4.054	0.755	2.502	1.005	0.083	3.590	0.723
6 : 45	3.164	1.005	0.095	4.264	0.794	2.728	1.097	0.087	3.912	0.788
7 : 00	3.302	1.097	0.099	4.498	0.837	2.844	1.132	0.086	4.063	0.818
7 : 15	3.451	1.092	0.107	4.649	0.865	3.096	1.194	0.109	4.399	0.886
7 : 30	3.716	1.236	0.120	5.072	0.944	3.294	1.259	0.129	4.682	0.943
7 : 45	4.048	1.239	0.133	5.421	1.009	3.662	1.316	0.163	5.141	1.035
8 : 00	4.285	1.271	0.158	5.714	1.064	3.877	1.353	0.185	5.415	1.090
8 : 15	4.413	1.345	0.176	5.935	1.105	4.121	1.411	0.228	5.761	1.160
8 : 30	4.523	1.353	0.192	6.068	1.130	4.249	1.474	0.244	5.966	1.201
8 : 45	4.579	1.387	0.222	6.187	1.152	4.249	1.562	0.275	6.085	1.226
9 : 00	4.837	1.454	0.248	6.539	1.217	4.258	1.581	0.280	6.118	1.232
9 : 15	4.883	1.387	0.287	6.557	1.221	4.276	1.583	0.288	6.147	1.238
9 : 30	4.652	1.384	0.293	6.330	1.178	4.331	1.608	0.289	6.227	1.254
9 : 45	4.744	1.392	0.290	6.426	1.196	4.422	1.612	0.290	6.324	1.274
10 : 00	4.744	1.358	0.284	6.386	1.189	4.395	1.562	0.289	6.246	1.258
MAXIMUM Qmax	4.929	1.454	0.305	6.569	1.223	4.477	1.612	0.296	6.324	1.274
AVERAGE Qave	4.058	1.134	0.178	5.370	1	3.625	1.157	0.181	4.963	1
MINIMUM Qmin	2.769	0.839	0.082	3.756	0.699	2.313	0.841	0.076	3.240	0.652

Location of Measuring Point, and Measuring Date and Time

1. Surco : Colector Surco, Diameter 1.54 meters

TABLE A7-1 (Cont'd)

- Av. JR Mexico 270, Surquillo
1st : from 10:00, May 31 to 10:00, June 1, 1989
2nd : from 8:45, October 19 to 8:30, October 20, 1989
2. Circun. : Colector Circunvalacion, Diameter 1.31 meters
Av. Julio Calero 140, Surquillo
1st : from 10:30, May 31 to 10:30, June 1, 1989
2nd : from 8:45, October 19 to 8:30, October 20, 1989
3. B. Sur : Colector Baños del Sur, Diameter 0.75 meters
Av. Daniel Portocarrero 264, Surquillo
1st : from 10:30, May 31 to 10:30, June 1, 1989
2nd : from 8:45, October 19 to 8:30, October 20, 1989

TABLE A7-2 RESULTS OF SEWAGE FLOW MEASUREMENT

unit : cu.m/sec.

MEASURING TIME	Intake Point No.1 Circunvalacion				Intake Point No.2 Villa Maria				Intake Point No.3 Surco			
	6/06-07		10/24-25		6/06-07		10/24-25		6/06-07		10/24-25	
	FLOW	RATE	FLOW	RATE	FLOW	RATE	FLOW	RATE	FLOW	RATE	FLOW	RATE
10 : 00	0.4865	1.33	0.5373	1.26	0.1367	1.13	0.0684	1.40	3.1817	1.15	3.1026	1.20
10 : 15	0.4668	1.28	0.5478	1.29	0.1379	1.14	0.0670	1.37	3.2643	1.16	3.0919	1.20
10 : 30	0.4194	1.15	0.5270	1.24	0.1356	1.12	0.0725	1.49	3.3469	1.21	3.0919	1.20
10 : 45	0.4865	1.36	0.5373	1.26	0.1312	1.08	0.0670	1.37	3.3163	1.20	3.1026	1.20
11 : 00	0.4476	1.23	0.5116	1.20	0.1323	1.09	0.0725	1.49	3.0973	1.12	3.1026	1.20
11 : 15	0.5690	1.56	0.5270	1.24	0.1142	0.94	0.0591	1.21	3.0973	1.12	2.9848	1.15
11 : 30	0.2443	0.67	0.5116	1.20	0.0983	0.81	0.0617	1.26	3.2027	1.15	2.9361	1.14
11 : 45	0.3488	0.96	0.5531	1.30	0.1152	0.95	0.0591	1.21	3.2027	1.15	3.0439	1.18
12 : 00	0.4287	1.18	0.5270	1.24	0.1163	0.96	0.0554	1.13	3.2079	1.16	3.1185	1.21
12 : 15	0.4381	1.20	0.5116	1.20	0.1183	0.97	0.0630	1.29	3.2443	1.17	3.0919	1.20
12 : 30	0.4865	1.33	0.5531	1.30	0.1081	0.89	0.0542	1.11	3.1765	1.15	3.0439	1.18
12 : 45	0.4766	1.31	0.5637	1.32	0.1091	0.90	0.0604	1.24	3.2702	1.18	3.0385	1.17
13 : 00	0.4523	1.24	0.5270	1.24	0.1215	1.00	0.0554	1.13	3.1660	1.14	2.9686	1.15
13 : 15	0.4381	1.20	0.5531	1.30	0.0983	0.81	0.0518	1.06	3.2753	1.18	2.9902	1.16
13 : 30	0.4865	1.33	0.5531	1.30	0.1031	0.85	0.0460	0.94	3.1660	1.14	3.0385	1.17
13 : 45	0.5167	1.42	0.5270	1.24	0.1022	0.84	0.0405	0.83	3.2443	1.17	3.0706	1.19
14 : 00	0.4381	1.20	0.5690	1.34	0.1071	0.88	0.0427	0.87	3.1660	1.14	3.0813	1.19
14 : 15	0.4103	1.12	0.5690	1.34	0.0983	0.81	0.0416	0.85	3.1555	1.14	3.0063	1.16
14 : 30	0.3745	1.03	0.5637	1.32	0.1002	0.82	0.0405	0.83	3.1396	1.13	3.0063	1.16
14 : 45	0.3404	0.93	0.5321	1.25	0.1012	0.83	0.0482	0.99	3.1449	1.13	3.0171	1.17
15 : 00	0.4381	1.20	0.5321	1.25	0.1031	0.85	0.0344	0.70	3.2027	1.15	2.9848	1.15
15 : 15	0.5167	1.42	0.5373	1.26	0.1041	0.86	0.0374	0.77	3.1555	1.14	3.0063	1.16
15 : 30	0.4766	1.31	0.5116	1.20	0.0983	0.81	0.0335	0.68	3.1291	1.13	3.0919	1.20
15 : 45	0.4766	1.31	0.4815	1.13	0.1012	0.83	0.0354	0.72	2.9794	1.07	3.0332	1.17
16 : 00	0.4572	1.25	0.4815	1.13	0.1002	0.82	0.0364	0.74	3.0439	1.10	2.9577	1.14
16 : 15	0.4381	1.20	0.5270	1.24	0.1091	0.90	0.0316	0.64	3.0706	1.11	2.9469	1.14
16 : 30	0.4766	1.31	0.5270	1.24	0.1142	0.94	0.0364	0.74	2.8817	1.04	2.9577	1.14
16 : 45	0.4915	1.35	0.5065	1.19	0.1101	0.91	0.0364	0.74	2.7721	1.00	2.9523	1.14
17 : 00	0.4287	1.18	0.5116	1.20	0.1091	0.90	0.0354	0.72	2.9361	1.06	2.9577	1.14
17 : 15	0.3922	1.07	0.5270	1.24	0.1061	0.87	0.0271	0.55	3.0706	1.11	2.8980	1.12
17 : 30	0.4381	1.20	0.5270	1.24	0.1051	0.86	0.0316	0.64	3.1079	1.12	2.7446	1.06
17 : 45	0.4668	1.28	0.5116	1.20	0.1031	0.85	0.0471	0.96	3.0760	1.11	2.7391	1.06
18 : 00	0.4381	1.20	0.5116	1.20	0.1081	0.89	0.0604	1.24	3.0385	1.10	2.7335	1.06
18 : 15	0.4287	1.18	0.4523	1.06	0.1132	0.93	0.0591	1.21	3.0332	1.09	2.7115	1.05
18 : 30	0.4012	1.10	0.4572	1.07	0.1041	0.86	0.0542	1.11	2.9955	1.08	2.6949	1.04
18 : 45	0.3404	0.93	0.4766	1.12	0.1121	0.92	0.0596	1.04	3.0063	1.08	2.7391	1.06
19 : 00	0.3702	1.01	0.4815	1.13	0.1204	0.99	0.0542	1.11	2.7721	1.00	2.8489	1.10
19 : 15	0.3745	1.03	0.4334	1.02	0.1081	0.89	0.0579	1.19	2.7721	1.00	2.8215	1.09
19 : 30	0.3789	1.04	0.4381	1.03	0.1121	0.92	0.0566	1.16	2.8434	1.02	2.7391	1.06
19 : 45	0.3833	1.05	0.4334	1.02	0.1121	0.92	0.0542	1.11	2.8434	1.02	2.6894	1.04
20 : 00	0.3922	1.07	0.4334	1.02	0.1132	0.93	0.0617	1.26	2.8380	1.02	2.7004	1.04
20 : 15	0.3615	0.99	0.4381	1.03	0.1163	0.96	0.0657	1.35	2.8980	1.04	2.6673	1.03
20 : 30	0.3404	0.93	0.4523	1.06	0.1204	0.99	0.0630	1.29	2.8980	1.04	2.4841	0.96
20 : 45	0.3001	0.82	0.4334	1.02	0.1236	1.02	0.0697	1.43	2.8106	1.01	2.5008	0.97
21 : 00	0.2696	0.74	0.4668	1.10	0.1226	1.01	0.0670	1.37	2.7996	1.01	2.5119	0.97
21 : 15	0.2270	0.62	0.4194	0.98	0.1183	0.97	0.0670	1.37	2.7721	1.00	2.4953	0.96

TABLE A7-2 (Cont'd)

21 : 30	0.2103	0.57	0.4057	0.95	0.1091	0.90	0.0697	1.43	2.7721	1.00	2.5342	0.98
21 : 45	0.2622	0.72	0.3877	0.91	0.1132	0.93	0.0670	1.37	2.7115	0.98	2.5119	0.97
22 : 00	0.2808	0.77	0.4194	0.98	0.1121	0.92	0.0617	1.26	2.7060	0.98	2.4841	0.96
22 : 15	0.2622	0.72	0.3922	0.92	0.1121	0.92	0.0566	1.16	2.7666	1.00	2.5175	0.97
22 : 30	0.2808	0.77	0.3922	0.92	0.1132	0.93	0.0579	1.19	2.7611	1.00	2.3285	0.90
22 : 45	0.2006	0.55	0.3745	0.88	0.1121	0.92	0.0518	1.06	2.7060	0.98	2.2120	0.85
23 : 00	0.1501	0.41	0.3789	0.89	0.1101	0.91	0.0449	0.92	2.7170	0.98	2.2287	0.86
23 : 15	0.1529	0.42	0.3573	0.84	0.1111	0.91	0.0374	0.77	2.7721	1.00	2.2952	0.89
23 : 30	0.1789	0.49	0.3615	0.85	0.1121	0.92	0.0344	0.70	2.7115	0.98	2.2730	0.88
23 : 45	0.2006	0.55	0.3199	0.75	0.1091	0.90	0.0325	0.66	2.6063	0.94	2.4063	0.93
0 : 00	0.2771	0.76	0.3199	0.75	0.1111	0.91	0.0307	0.63	2.6063	0.94	2.4230	0.94
0 : 15	0.2550	0.70	0.2733	0.64	0.1091	0.90	0.0344	0.70	2.5952	0.93	2.2896	0.88
0 : 30	0.2443	0.67	0.2923	0.68	0.1081	0.89	0.0325	0.66	2.5397	0.91	2.1512	0.83
0 : 45	0.2478	0.68	0.2923	0.68	0.1111	0.91	0.0307	0.63	2.4563	0.88	2.1347	0.82
1 : 00	0.2550	0.70	0.3404	0.80	0.1132	0.93	0.0289	0.59	2.4063	0.87	2.1954	0.85
1 : 15	0.1943	0.53	0.3488	0.82	0.1142	0.94	0.0254	0.52	2.3285	0.84	2.2176	0.86
1 : 30	0.2006	0.55	0.3446	0.81	0.0983	0.81	0.0280	0.57	2.3285	0.84	2.1789	0.84
1 : 45	0.1881	0.51	0.2622	0.61	0.1002	0.82	0.0230	0.47	2.3452	0.84	2.2120	0.85
2 : 00	0.1789	0.49	0.2622	0.61	0.1022	0.84	0.0246	0.50	2.2065	0.79	2.2231	0.86
2 : 15	0.2550	0.70	0.2659	0.62	0.1002	0.82	0.0230	0.47	2.2176	0.80	2.2508	0.87
2 : 30	0.3363	0.92	0.2478	0.58	0.1031	0.85	0.0246	0.50	2.1678	0.78	2.2674	0.88
2 : 45	0.2443	0.67	0.2478	0.58	0.1002	0.82	0.0222	0.45	2.1512	0.77	2.2342	0.86
3 : 00	0.1912	0.52	0.2338	0.55	0.1002	0.82	0.0222	0.45	2.1678	0.78	2.1016	0.81
3 : 15	0.2846	0.78	0.2338	0.55	0.1031	0.85	0.0222	0.45	2.0961	0.75	1.7972	0.69
3 : 30	0.2622	0.72	0.2478	0.58	0.1081	0.89	0.0271	0.55	2.1071	0.76	1.7120	0.66
3 : 45	0.2696	0.74	0.2338	0.55	0.1142	0.94	0.0246	0.50	2.1071	0.76	1.5138	0.58
4 : 00	0.3615	0.99	0.2270	0.53	0.0983	0.81	0.0271	0.55	1.5292	0.55	1.3292	0.59
4 : 15	0.2771	0.76	0.2304	0.54	0.1061	0.87	0.0238	0.48	1.5189	0.55	1.5757	0.61
4 : 30	0.2202	0.60	0.2338	0.55	0.0993	0.82	0.0230	0.47	1.6330	0.59	1.6017	0.62
4 : 45	0.2103	0.57	0.2478	0.58	0.0936	0.77	0.0271	0.55	2.1512	0.77	1.6697	0.64
5 : 00	0.2270	0.62	0.2478	0.58	0.0964	0.79	0.0254	0.52	2.0851	0.75	1.7067	0.66
5 : 15	0.2478	0.68	0.2270	0.53	0.0945	0.78	0.0280	0.57	2.1071	0.76	1.7651	0.68
5 : 30	0.2622	0.72	0.2006	0.47	0.0964	0.79	0.0298	0.61	2.1512	0.77	1.7332	0.67
5 : 45	0.2923	0.80	0.2103	0.49	0.1081	0.89	0.0335	0.68	2.1999	0.79	1.8671	0.72
6 : 00	0.3001	0.82	0.2550	0.60	0.1101	0.91	0.0374	0.77	2.1899	0.79	1.9593	0.76
6 : 15	0.3079	0.84	0.2733	0.64	0.1194	0.98	0.0591	1.21	2.2065	0.79	1.9811	0.76
6 : 30	0.3159	0.86	0.2696	0.63	0.1290	1.06	0.0657	1.35	2.2508	0.81	2.1182	0.82
6 : 45	0.3001	0.82	0.3001	0.70	0.1952	1.61	0.0643	1.32	2.2896	0.82	2.1182	0.82
7 : 00	0.3001	0.82	0.3530	0.83	0.1952	1.61	0.0684	1.40	2.3618	0.85	2.2342	0.86
7 : 15	0.3922	1.07	0.3833	0.90	0.2018	1.66	0.0711	1.46	2.5730	0.93	2.4508	0.95
7 : 30	0.4766	1.31	0.4766	1.12	0.2085	1.72	0.0754	1.55	2.6617	0.96	2.6396	1.02
7 : 45	0.5015	1.38	0.4865	1.14	0.2153	1.78	0.0812	1.67	2.8917	1.04	2.7225	1.05
8 : 00	0.5270	1.45	0.5270	1.24	0.2181	1.80	0.0827	1.70	2.9577	1.07	2.8215	1.09
8 : 15	0.5531	1.52	0.5531	1.30	0.2222	1.83	0.0827	1.70	3.0760	1.11	2.9686	1.15
8 : 30	0.5961	1.64	0.5961	1.40	0.2045	1.69	0.0827	1.70	3.2339	1.17	2.9849	1.15
8 : 45	0.5798	1.59	0.5373	1.26	0.2085	1.72	0.0711	1.46	3.2339	1.17	3.0278	1.17
9 : 00	0.5744	1.58	0.6693	1.57	0.1822	1.50	0.0711	1.46	3.2391	1.17	3.1344	1.21
9 : 15	0.5906	1.62	0.5798	1.36	0.1599	1.32	0.0697	1.43	3.2443	1.17	3.1132	1.20
9 : 30	0.6071	1.67	0.5426	1.27	0.1345	1.11	0.0697	1.43	3.2235	1.16	3.1291	1.21
9 : 45	0.5167	1.42	0.5584	1.31	0.1447	1.19	0.0670	1.37	3.2026	1.15	3.1238	1.21
10 : 00	0.4865	1.33	0.5373	1.26	0.1367	1.13	0.0684	1.40	3.1817	1.15	3.1026	1.20
MAXIMUM Q _{max}	0.6071	1.67	0.6693	1.57	0.2222	1.83	0.0827	1.70	3.3469	1.21	3.1344	1.21
AVERAGE Q _{ave}	0.3632	1	0.4243	1	0.1209	1	0.0486	1	2.7611	1	2.5751	1
MINIMUM Q _{min}	0.1501	0.41	0.2006	0.47	0.0936	0.77	0.0222	0.45	1.5189	0.55	1.5138	0.58

TABLE A7-2 (Cont'd)

- Location of Measuring Point, and Measuring Date and Time
1. Diversion Pt. No.1 : Colector Circunvalacion, Diameter 1.3 meters
Parque Fundadores, Av. J. de Aliaga, Santiago de Surco
1st : from 10:45, June 6 to 10:30, June 7, 1989
2nd : from 9:45, October 24 to 9:30, October 25, 1989
 2. Diversion Pt. No.2 : Colector Emisor-Villa Maria del Triunfo
1st : Av. Pachacutec 828, Diameter 1.2 meters
from 11:15, June 6 to 11:00, June 7, 1989
2nd : Av. Pachacutec/Jose Carlos Mariategui, Dia. 0.632 meters
from 9:30, October 24 to 9:15, October 25, 1989
 3. Diversion Pt. No.3 : Colector Surco, Diameter 1.25 meters
Av. Nueva Tomas Marsano/Jorge Chavez CDA 38
1st : from 11:00, June 6 to 10:45, June 7, 1989
(Data at 9:30, 9:45 and 10:15 are interpolated.)
2nd : from 9:15, October 24 to 9:00, October 25, 1989

TABLE A7-3

RESULTS OF FLOW MEASUREMENT (1988)

CUADRO RESUMEN AFORO DE COLECTORES

ENTOR LA CHIRA

FEBRERO/MARZO 1988

COLECTORES: CIRCUNVALACION-BALNEARIOS DEL SUR-SURCO

DIA	CIRCUNVALACION			BALNEARIOS DEL SUR			SURCO			TOTAL Q prom. m ³ /s.
	Q prom. m ³ /s.	Q max. m ³ /s.	Q min. m ³ /s.	Q prom. m ³ /s.	Q max. m ³ /s.	Q min. m ³ /s.	Q prom. m ³ /s.	Q max. m ³ /s.	Q min. m ³ /s.	
Thu 25	1.097	1.471	0.711	0.161	0.226	0.070	3.406	4.103	2.407	4.664
Fri 25	1.086	1.328	0.771	0.149	0.203	0.102	3.261	4.012	2.391	4.496
Sat 27	1.155	1.461	0.749	0.156	0.227	0.095	3.420	4.606	2.379	4.732
Sun 28	1.020	1.194	0.834	0.150	0.182	0.104	3.346	4.074	2.311	4.525
Mon 29	1.105	1.407	0.839	0.218	0.309	0.121	3.591	4.551	2.508	4.914
Tue 1	1.104	1.502	0.844	0.209	0.245	0.101	3.597	4.358	2.605	4.910
Wed 7	1.125	1.464	0.844	0.234	0.357	0.105	3.839	4.772	2.728	5.168

GASTO PROMEDIO :

CIRCUNVALACION	1.100 m ³ /s.
BALNEARIOS DEL SUR	0.182 m ³ /s.
SURCO	3.490 m ³ /s.
GASTO PROMEDIO ENTOR LA CHIRA	4.772 m ³ /s.

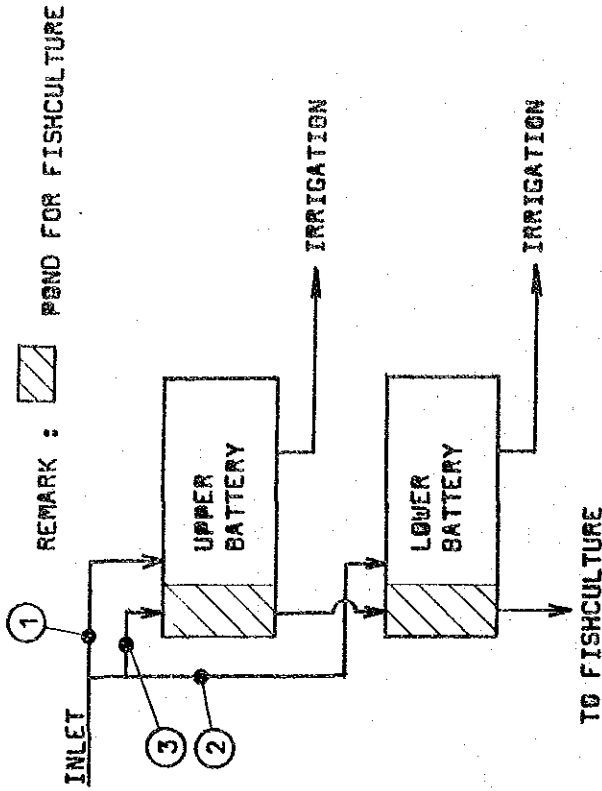
ANALISIS DE GASTOS MAXIMOS Y MINIMOS CON GASTOS RECIPROCOS
EN HORAS COINCIDENTES :

	CIRCUNVALACION		BALNEARIOS DEL SUR		SURCO	
	DIA/HORA	Q m ³ /s	DIA/HORA	Q m ³ /s	DIA/HORA	Q m ³ /s
MAXIMO	1/0.521	1.273	1/0.521	0.355	1/0.521	4.772
MINIMO	28/0.167	0.843	1	0.126	28/0.167	2.344

GASTO MAXIMO DIARIO ASUMIDO 6.400 m³/sGASTO MINIMO DIARIO ASUMIDO 3.313 m³/sGASTO PROMEDIO DIARIO 4.772 m³/s

TABLE A7-4 VARIATION OF INFLOW AMOUNT AT SAN JUAN STABILIZATION POND, JUNE, 1989.

HOUR	QUANTITY (cu.m/h)		TOTAL
	① UPPER	② LOWER	
6/20 11:00	590.4	579.6	1,170.0
12:00	572.4	558.0	1,130.4
13:00	576.0	507.6	1,083.6
14:00	568.8	514.8	1,083.6
15:00	561.6	529.2	1,090.8
16:00	604.8	604.8	1,209.6
17:00	684.0	583.2	1,267.2
18:00	669.6	482.4	1,152.0
19:00	662.4	478.8	1,141.2
20:00	655.2	496.8	1,152.0
21:00	637.2	489.6	1,126.8
22:00	633.6	424.8	1,058.4
23:00	615.6	435.6	1,051.2
24:00	601.2	432.0	1,033.2
6/21 1:00	550.8	367.2	918.0
2:00	525.6	352.8	878.4
3:00	536.4	356.4	892.8
4:00	522.0	345.6	867.6
5:00	615.6	349.2	964.8
6:00	716.4	363.6	1,080.0
7:00	828.0	381.6	1,209.6
8:00	871.2	381.6	1,252.8
9:00	763.2	378.0	1,141.2
10:00	640.8	374.4	1,015.2
MEAN VALUE	633.5	448.7	1,082.2



TOTAL INFLOW ;

① UPPER	15,204 cu.m/day
② LOWER	10,769 cu.m/day
SUB TOTAL	25,973 cu.m/day
③ FOR FISHCULTURE (AV.0.025 cu.m/s)	2,160 cu.m/day
TOTAL	28,133 cu.m/day

FIGURE A7-1 VARIATION OF SEWAGE INFLOW AT SAN JUAN STABILIZATION POND
JUNE, 1989

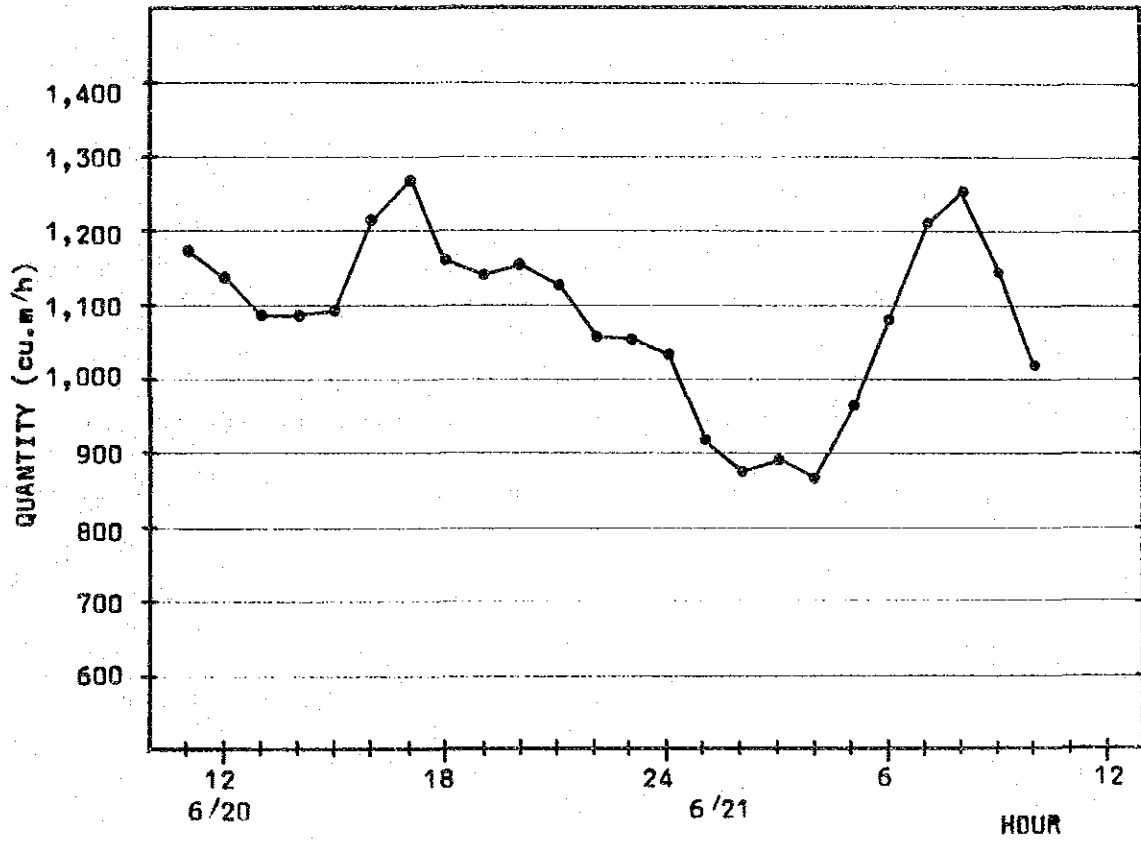
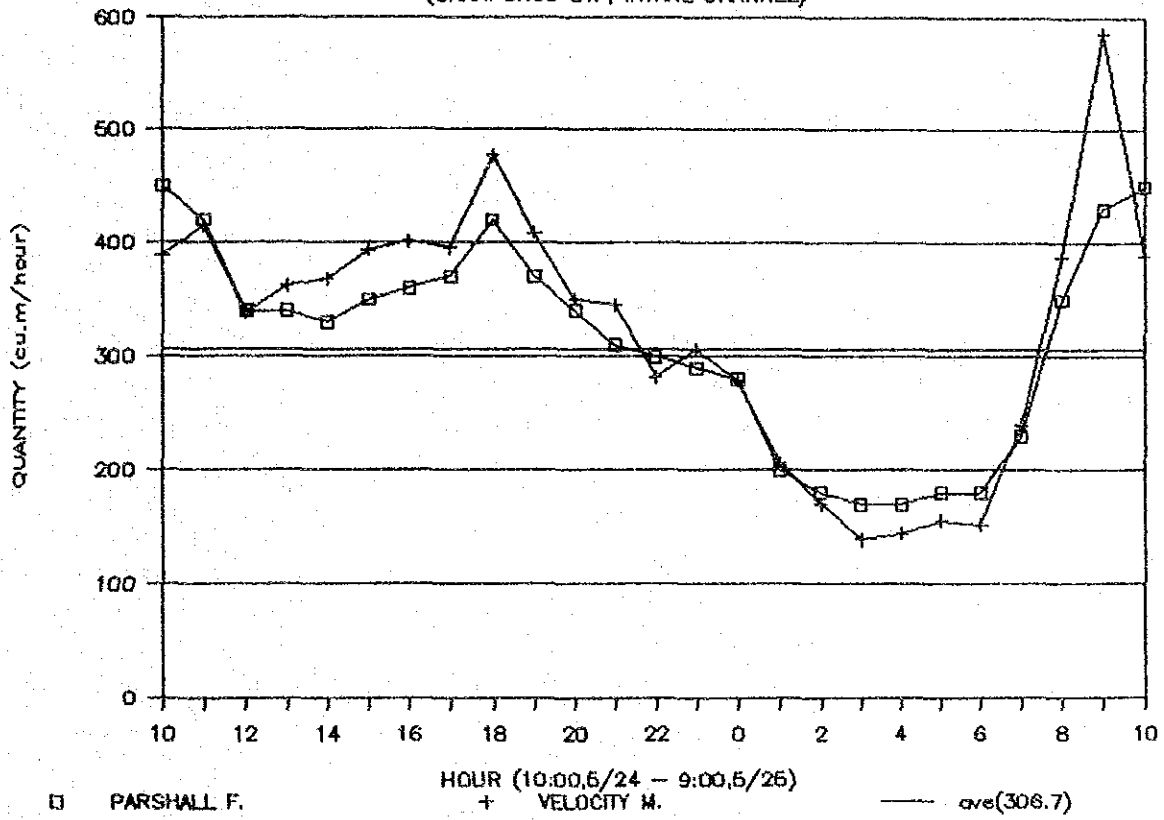


TABLE A7-5 VARIATION OF INFLOW AMOUNT AT CARAPENDO STP

Upper width of channel (W) = 1.310 m
 Bottom width of channel (B) = 500 mm
 Height of channel (H) = 820 mm

HOUR	PARSHALL FLUME			VELOCITY METER		
	INDICATION (x 10 cu.m/hour)	QUANTITY (cu.m/hour)	SLOPE LENGTH TO W.S.	VELOCITY (m/sec.)	QUANTITY (cu.m/h)	VELOCITY (m/sec.)
5/24 10:00	45	450	585	0.566	388.9	
11:00	42	420	600	0.640	415.4	
12:00	34	340	640	0.615	338.8	
13:00	34	340	630	0.630	362.2	
14:00	33	330	640	0.668	368.0	
15:00	35	350	630	0.685	393.9	
16:00	36	360	620	0.670	401.6	
17:00	37	370	620	0.660	395.6	
18:00	42	420	590	0.708	477.5	
19:00	37	370	620	0.682	408.8	
20:00	34	340	640	0.635	349.8	
21:00	31	310	650	0.655	345.2	
22:00	30	300	665	0.573	281.8	
23:00	29	290	655	0.594	306.0	
5/25 0:00	28	280	662	0.559	278.8	
1:00	20	200	700	0.500	206.0	
2:00	18	180	730	0.490	169.8	
3:00	17	170	735	0.414	139.1	
4:00	17	170	717	0.385	144.2	
5:00	18	180	715	0.411	155.8	
6:00	18	180	712	0.395	152.3	
7:00	23	230	678	0.519	239.7	
8:00	35	350	615	0.634	387.8	
9:00	43	430	556	0.769	585.3	
MEAN VALUE.	30.7	306.7	650	0.586	320.6	
TOTAL (cu.m/day)		7,360.0			7,693.4	

FIGURE A7-2 VARIATION OF SEWAGE INFLOW
(CARAPONGO STP, INTAKE CHANNEL)



APPENDIX 8

STUDY ON PER CAPITA SEWAGE QUANTITY

APPENDIX 8 STUDY ON PER CAPITA SEWAGE QUANTITY

The average per capita sewage discharge is estimated based on the area classification by use, and the projected population for the target year. The following data were considered for this calculation:

- (1) Per capita sewage discharge based on design standard.
- (2) Per capita sewage discharge based on supplied volume of drinking water.
- (3) Per capita sewage discharge based on flow measurement results.

(1) Per capita sewage discharge based on Design Standard

The sewage discharge is set at 90% of the average consumption of supplied drinking water administered by SEDAPAL according to the design standard of SEDAPAL which is shown in TABLE A8-1.

TABLE A8-1 Per Capita Sewage Discharge Based on Design Standard

CATEGORY OF WATER CONSUMPTION TYPE	SUPPLIED WATER VOLUME/ CAPITA/DAY (lpcd)	PERCENTAGE OF WATER DISCHARGE	BASIC SEWAGE DISCHARGE/ CAPITA/DAY (lpcd)
D/S.H.	300	90 %	270
D/S.L.	250	90 %	225
I.D.	150	90 %	135

Note: The volume of supplied water and the rate of sewage discharge is based on the Design Standard of SEDAPAL.

(2) Per capita sewage discharge based on Volume of Supplied Water

Per capita sewage discharge can be determined from the actual volume of supplied drinking water. According to statistics of the Project Department of SEDAPAL, the actual volume of supplied water during the period from January to December of 1988 is as shown in TABLE A8-2.

TABLE A8-2 Volume of Actual Supplied Water

(m ³ /year)	(m ³ /year)	
645,440,000	346,777,000	53.73%

Source: Project Dept, SEDAPAL, 1988.

Given the amount of supplied water, the percentage of sold drinking water, and the percentage of discharge to the sewer system (90% of consumed drinking water), the per capita sewage discharge is determined as shown in TABLE A8-3.

TABLE A8-3 Per Capita Sewage Discharge Based on Supplied Volume

CATEGORY OF WATER CONSUMPTION	SUPPLIED WATER/ CAPITA/DAY (lpcd)	SOLD PERCENTAGE	PERCENTAGE OF SEWAGE DISCHARGE	SEWAGE DISCHARGE/ CAPITA/DAY (lpcd)
D/S.H.	300	53.73 %	90 %	145
D/S.L.	250	53.73	90	120
I.D.	150	53.73	90	72

(3) Per Capita Sewage Discharge Based on Flow Measurement Results

Per capita sewage discharge based on flow measurement results will be calculated taking into account the following points in determining the actual discharge volume of domestic sewage.

- a) Analysis of measured flow
- b) Management of water of unknown origin

a) Analysis of measured flow

To calculate per capita sewage discharge from the analysis of measured flow, all factors in the production of sewage in the project area are con-

sidered. Measurement results are shown in TABLE A8-4.

TABLE A8-4 Flow Measurement

DATE OF MEASUREMENT	MEASURED FLOW (m ³ /sec.)	REMARKS
May 31 - Jun. 1, 1989	5.370	JICA
Oct.19 - Oct.20, 1989	4.963	JICA
Feb.25 - Mar. 2, 1988	4.773	SEDAPAL
average	around 5.0	

(refer to APPENDIX 7)

The sewage quantity estimated from water supply amount is shown in TABLE A8-5.

TABLE A8-5 Volume of Sewage Discharge into Colector Surco

PRODUCTION VOLUME m ³ /sec	% ADMIN. SOUTHERN DISTRICT	% ADMIN. SURCO DRAINAGE ZONE	% SOLD	% DIS-CHARGED	DRAINAGE m ³ /sec
1)	2)	3)	4)	5)	6)
20.467	53.13	55.68	53.73	90	2.928

- 1) Annual Volume of Water Produced
645,440,000 m³/year = 20.467 m³/sec
- 2) Percentage supplied to Southern District
Southern Dist. 9.17 m³/sec.
Total Admin. = 17.26 m³/sec. = 53.13 %
- 3) Percentage supplied to Surco drainage area
Surco Drainage Area 5.106 m³/sec.
Vol. supplied to Surco Area = 9.17 m³/sec. = 55.68 %
- 4) Percentage sold (see TABLE A8-2)
- 5) Discharge: (referred to the Regulation and Design Standard of SEDAPAL)

The sewage in Colector Surco includes domestic sewage and industrial wastewater and that from unknown origin.

- * Volume of domestic sewage: $2.928 - 0.323 \text{ m}^3/\text{sec} = 2.605 \text{ m}^3/\text{sec}$.
- * Industrial wastewater: $0.323 \text{ m}^3/\text{sec}$ (see APPENDIX 3)
- * Unknown water volume: $5.0 \text{ m}^3/\text{sec} - 2.928 \text{ m}^3/\text{sec} = 2.072 \text{ m}^3/\text{sec}$

From the aforementioned results, $2.072 \text{ m}^3/\text{sec}$ or 41% of the total volume of sewage in Colector Surco is of unknown origin.

b) Management of Water of Unknown Origin

Investigation on the source of unknown water is possible up to a certain extent by analyzing the water quality, but because of lack of information and the peculiarity of land in large areas, it is very difficult to determine exactly the source of the wastewater. However, from the little information available, the following conclusions related to unknown water can be drawn:

- 1) Inaccuracy of the amount supplied because of defective water meter.
- 2) Direct flow into sewers from breaks in drinking water supply pipes.
- 3) Exact data on domestic consumption of well water are not available.
- 4) Intrusion of water from irrigation canal at many points.

For the above reasons and because of the fact that the percentage of water sold increased from 42.7 % in 1986 to 53.73 % in 1988, it is deemed appropriate to consider that a large part of the unknown water originates from domestic sewage.

In the future, water intrusion from irrigation canals would be eliminated and is therefore excluded from the subject of treatment. The volume categorized "Others" is considered as "volume produced by industrial activities of unknown sources", and given the same treatment as that for industrial wastewater.

Per capita domestic sewage discharge can be obtained by multiplying per capita domestic water supply by the percentages in sold water and discharged sewage. Therefore, it is necessary to use an adjusted value of the percentage of water sold based on the latest figure of 53.73%. According to Information from the Planning and Budget Administration 1988, of SEDAPAL, the percentage of sold water for the month of April, 1986 was 42.7%, in-

creasing to 53.73% in 1988. This means an improvement of 25% in a span of two years, which is expected to further increase to 30% in the future. Accordingly, the percentage in sold of 53.73% is readjusted in the following manner:

$$\text{Readjusted Percentage of Water Sold} = 53.73\% \times 1.30 = 69.8 = 70\%$$

TABLE A8-6 shows the per capita sewage discharge obtained by applying the readjusted percentage in water sold.

TABLE A8-6 Per Capita Sewage Discharge Based on Sewage Flow Measurement

CATEGORY OF WATER CONSUMPTION	BASIC UNIT OF VOLUME ADMIN. l/capita/day	REVISED % OF WATER SOLD	PER CAPITA SEWAGE DISCG. l/capita/day
D/S.H.	300	70	210
D/S.H.	250	70	175
I.D.	150	70	105

To determine the basic unit of volume of sewage, three points, namely, (1) Design Standard, (2) Volume of Supplied Water, and (3) Measurement of Sewage Flow are considered.

From the foregoing analysis, it can be concluded that the value obtained by measuring the sewage flow proved to be closest to the actual value, and is therefore adopted with specific adjustments as shown in TABLE A8-7.

TABLE A8-7 Per Capita Domestic Sewage Unit l/capita/day

WATER CONSUM.	DESIGN STANDARD	VOLUME ADMIN.	SEWAGE MEASUREMENT	BASIC UNIT DETERMINED
D/S.H.	270	145	210	210
D/S.L.	225	120	175	180*
I.D.	135	72	105	110*

* With application of specific readjustments

(4) Planned Sewage Quantity

Since the planned sewage quantity is a basic value in determining the cost and volume, it will be calculated by considering the SEDAPAL Project

Regulations and Standards, and the "Guidelines for Design of Sewerage Facilities" edited by the Japan Sewage Works Association.

1) Daily Maximum Quantity

Because the project area is large, the daily maximum quantity is taken as 120% of the average daily quantity.

$$D/S.H. = 210 \times 1.2 = 252 = 250 \text{ l/capita/day}$$

$$D/S.L. = 180 \times 1.2 = 216 = 210 \text{ l/capita/day}$$

$$I.D. = 110 \times 1.2 = 132 = 132 \text{ l/capita/day}$$

2) Daily Average Quantity

$$D/S.H. = 210 \text{ l/capita/day}$$

$$D/S.L. = 180 \text{ l/capita/day}$$

$$I.D. = 110 \text{ l/capita/day}$$

3) Hourly Maximum Quantity

Considering the future increase in water supply, the average value given in the above reference was used, thus:

$$(2.6 + 1.2 \times 1.8) \times 1/2 = 2.38 = 2.4 \text{ times}$$

Therefore, 2.4 times of the daily average quantity is adopted as hourly maximum quantity.

$$D/S.H. = 210 \times 2.4 = 504 = 500 \text{ l/capita/day}$$

$$D/S.L. = 180 \times 2.4 = 432 = 430 \text{ l/capita/day}$$

$$I.D. = 110 \times 2.4 = 264 = 260 \text{ l/capita/day}$$

The following TABLE A8-8 is a summary of the above calculations:

TABLE A8-8 Programmed Per Capita Sewage Discharge

ITEM	D/S.H. l/capita/day	D/S.L. l/capita/day	I.D. l/capita/day
Daily Maximum of sewage	250	210	130
Daily Average of sewage	210	180	110
Hourly Maximum of sewage	500	430	260

"SEDAPAL: Project Regulations and Standards" and the "Guidelines for Design of Sewerage Facilities" give the following values:

REFERENCE	(1) DAILY MAXIMUM	(2) DAILY AVERAGE	(3) HOURLY MAXIMUM
SEDAPAL	1.3 times of (2)	90% of admin. drinking water	2.6 times of (2)
JSWA	Total drinking water admin. plus other water	70 - 80% of (1)	1.3 to 1.8 times of (1)

