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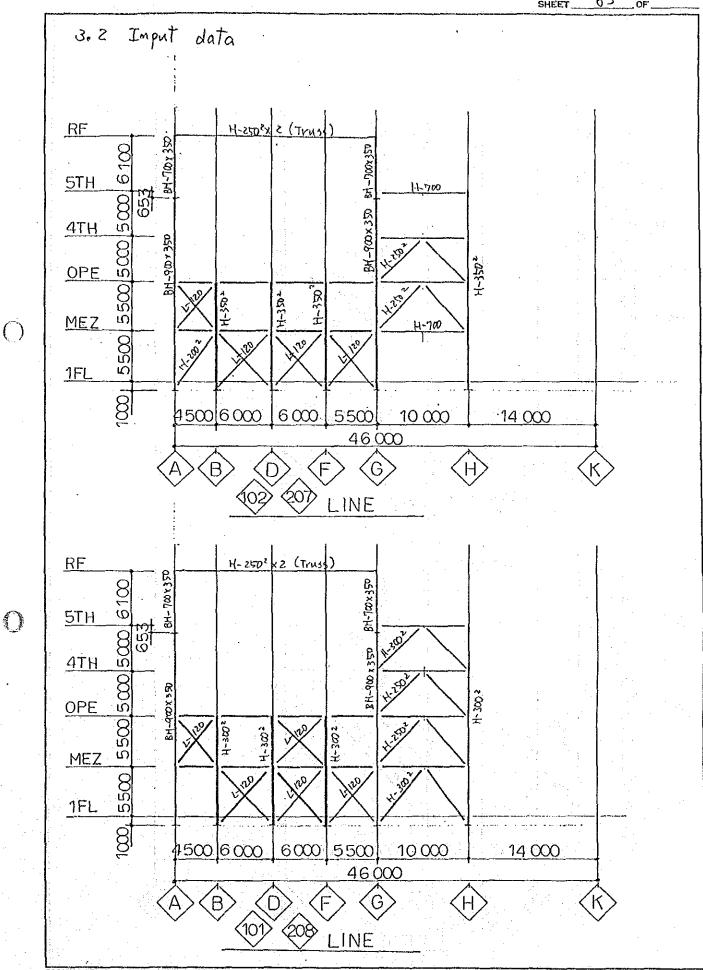
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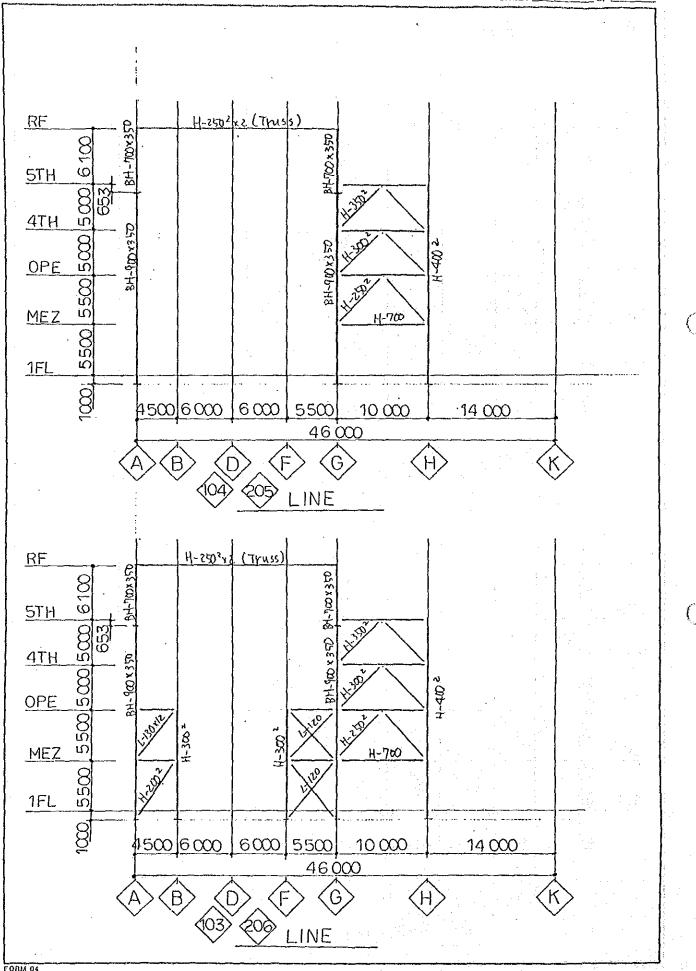
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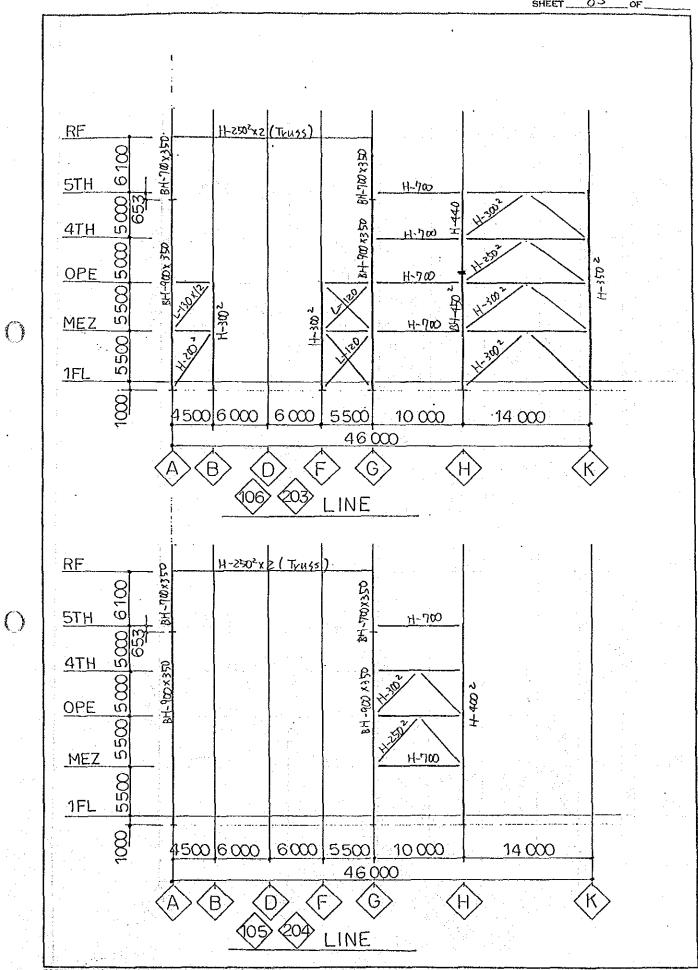
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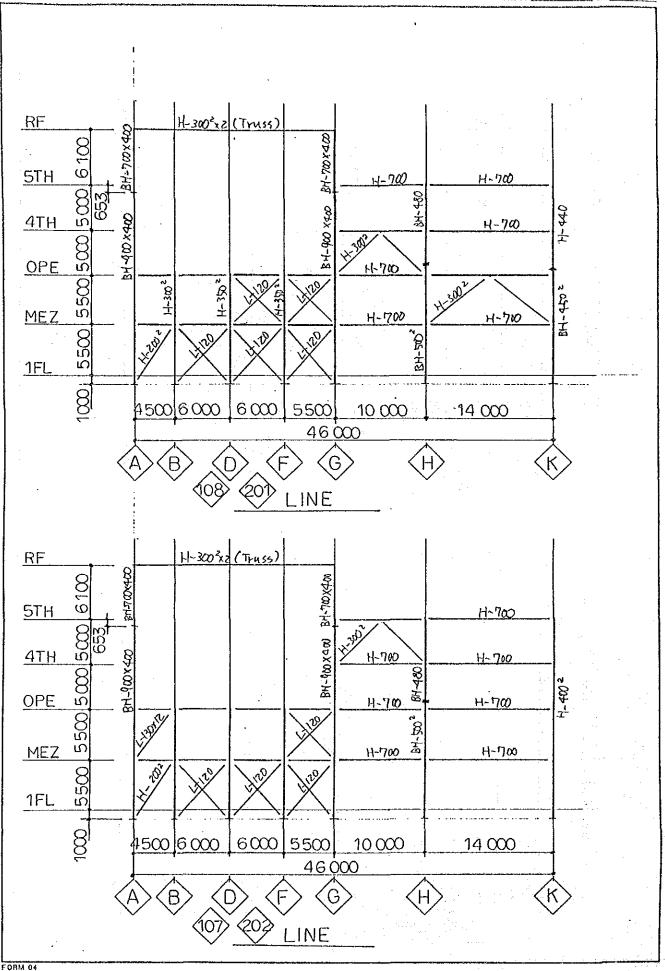
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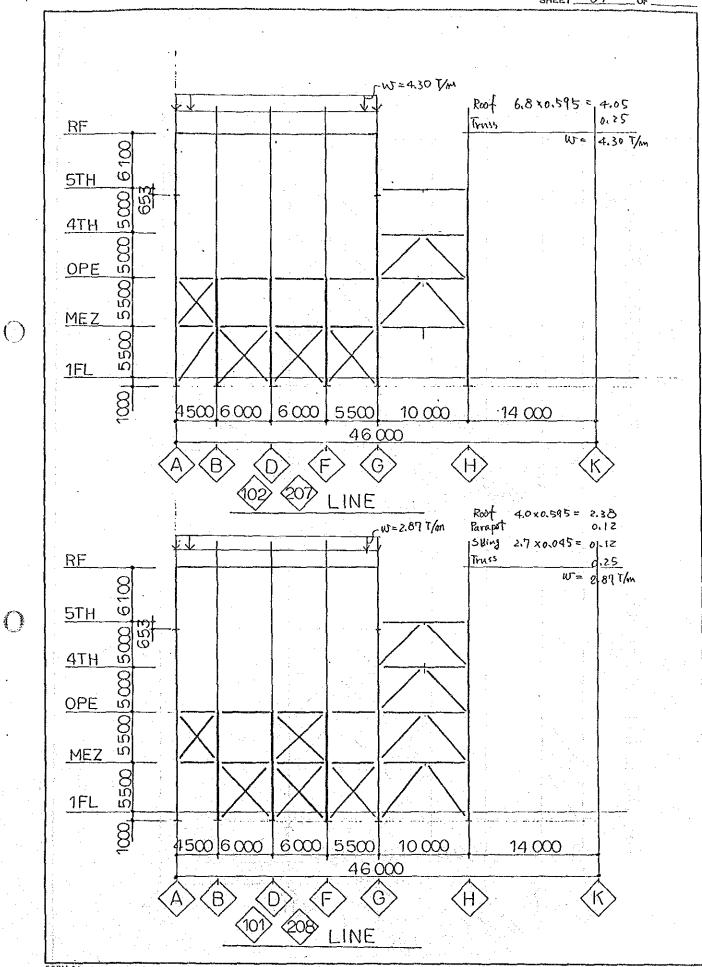
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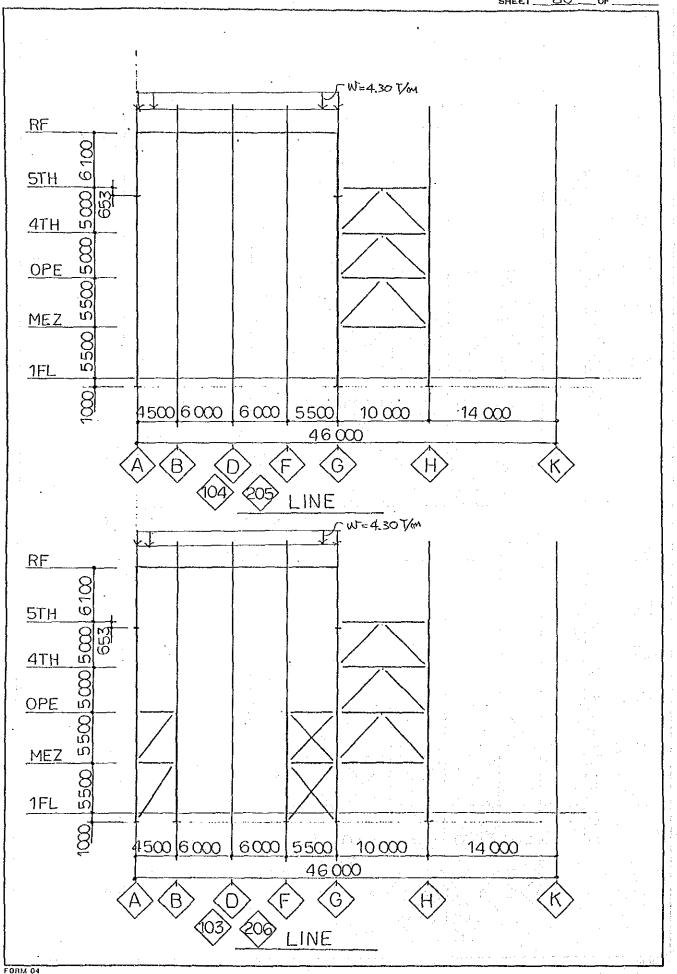


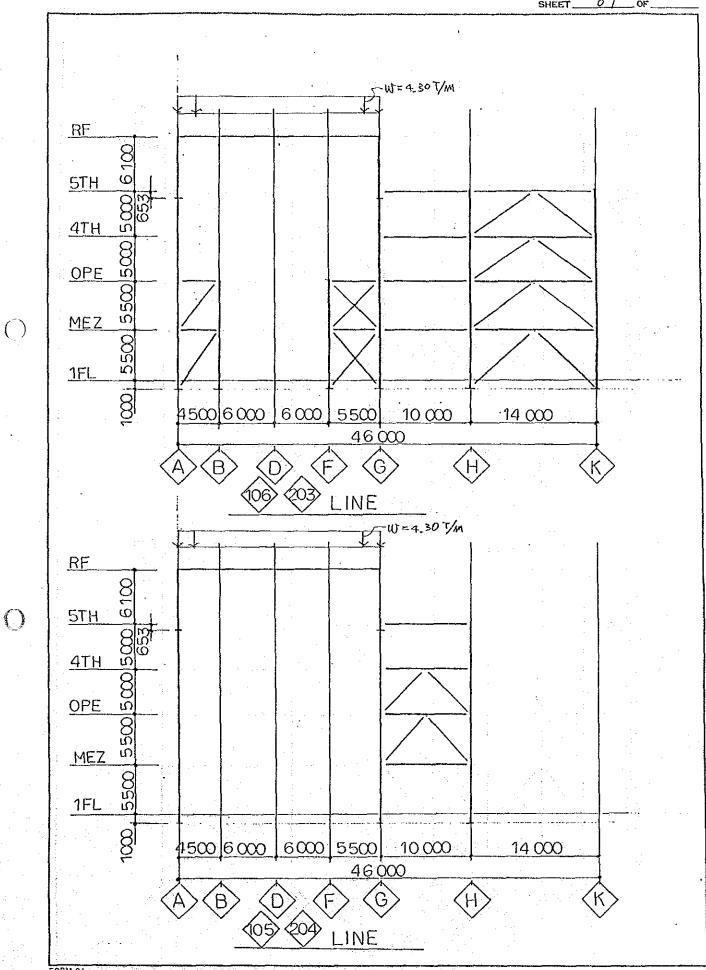


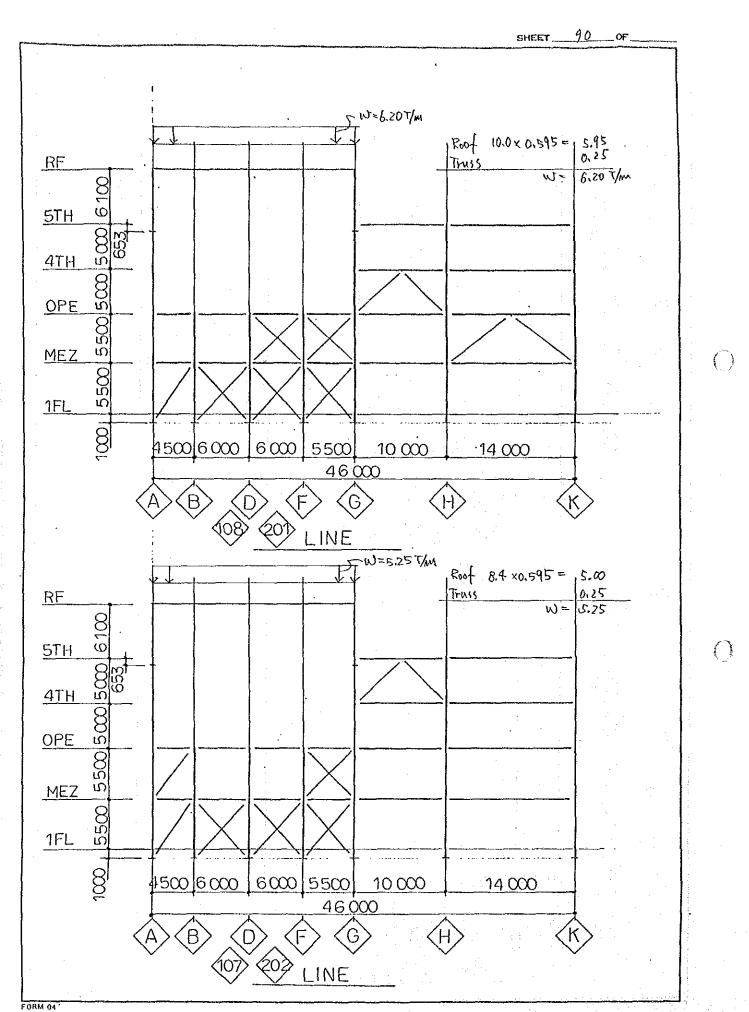












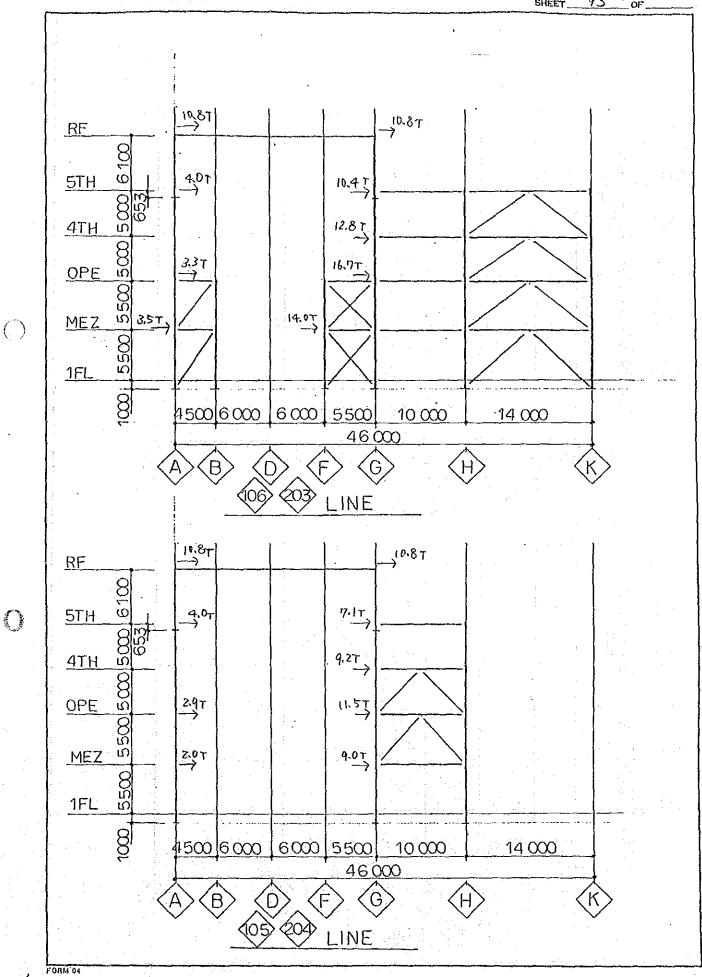
618 4,0T → <u>5TH</u> 5500 5500 5000 5000 4TH OPE &IT. <u>MEZ</u> 1FL 4500 6000 7.3 T RF () 2.97 <u>5TH</u> 1<u>00, 1550 | 550 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 | 500 |</u> <u>4TH</u> <u>OPE</u> 4.27 MEZ 1FL 5500 1 46000 LINE

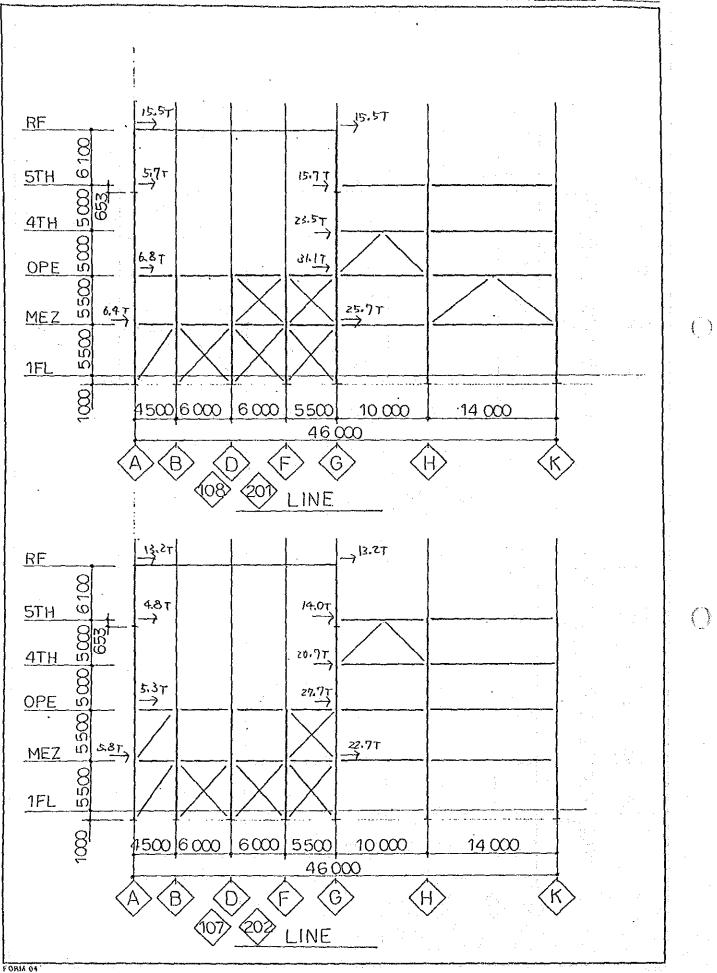
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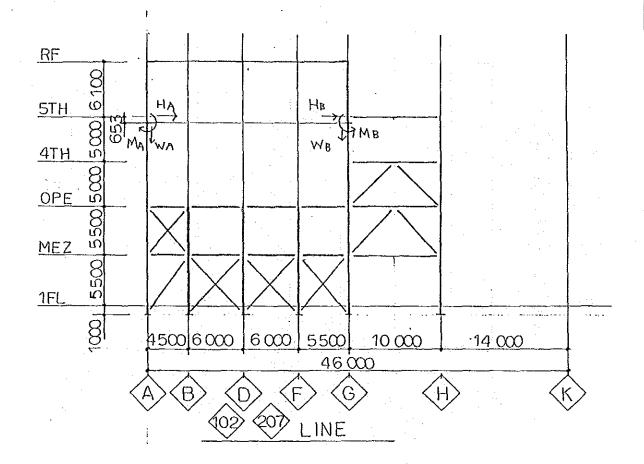
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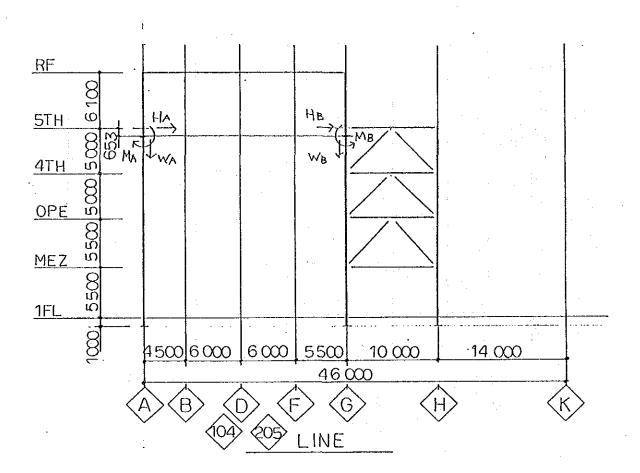
C.D.L. 
$$WA = 24.8T$$
  $WB = 13.3T$   $HA = 2.1T$   $HB = 1.1T$   $MA = 19.8T$   $MB = 10.6T$   $MB = 10.6T$   $MB = 17.8T$   $MA = 5.9T$   $MB = 1.5T$   $MA = 57.0T$   $MB = 14.2T$   $MB = 14.2T$   $MB = 14.2T$   $MB = 14.2T$   $MB = 14.2T$ 

FORM 04

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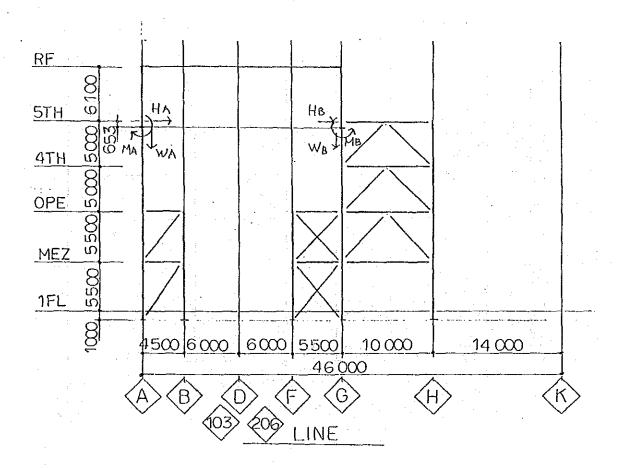
C.D.L. WA= 24.8T WB= 13.3T  

$$HA=$$
 2.1T  $HB=$  1.1T  
 $MA=$  19.8T·m  $MB=$  10.6T·m

C.L.L. WA = 71.2 T 
$$W_B = 17.8 \text{ T}$$
  
 $H_A = 5.9 \text{ T}$   $H_B = 1.5 \text{ T}$   
 $M_A = 57.0 \text{ TM}$   $M_B = 14.2 \text{ Tm}$ 

$$C.H.L.$$
  $HA = 4.27$   $HB = 8.37$ 

FURM 04

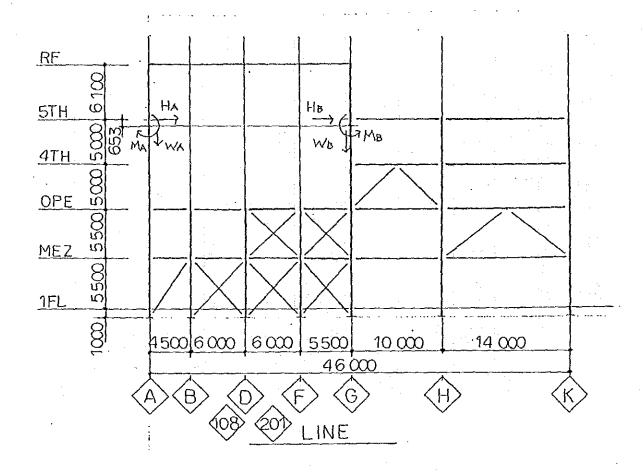


C.D.L. 
$$WA = 24.8 \text{ T}$$
  $WB = 13.3 \text{ T}$ 
 $HA = 2.1 \text{ T}$   $HB = 1.1 \text{ T}$ 
 $MA = 19.8 \text{ T/M}$   $MB = 10.6 \text{ T}$ 

C.L.L.  $WA = 71.2 \text{ T}$   $WB = 17.8 \text{ T}$ 
 $HA = 5.9 \text{ T}$   $HB = 1.5 \text{ T}$ 
 $MA = 57.0 \text{ T/M}$   $MB = 14.2 \text{ T/M}$ 

C.H.L.  $HA = 4.2 \text{ T}$   $HB = 2.3 \text{ T}$ 

FORM 04



C.D.L. WA = 
$$28.1 \text{ T}$$
  
HA =  $2.3 \text{ T}$ 

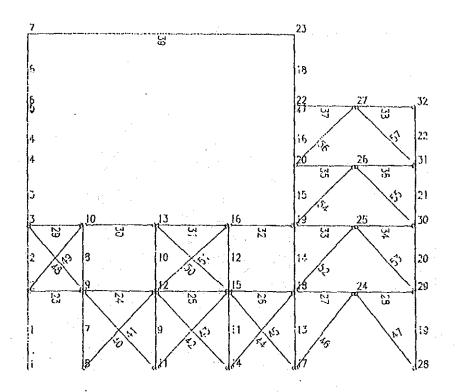
$$W_B = 15.1 T$$
 $H_B = 1.3 T$ 

$$MA = 22.5 \text{ Tim}$$

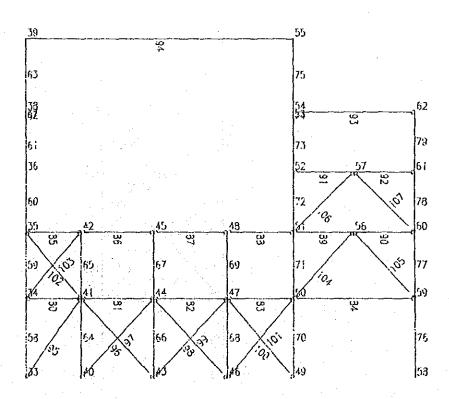
C.L.L. WA = 
$$80.6 \text{ T}$$
  
HA =  $6.7 \text{ T}$ 

$$MA = 64.5 \text{ T.m.}$$

EORM 04



フレーム モデル 図 ( 101 - LINE SCALE 1/300 )

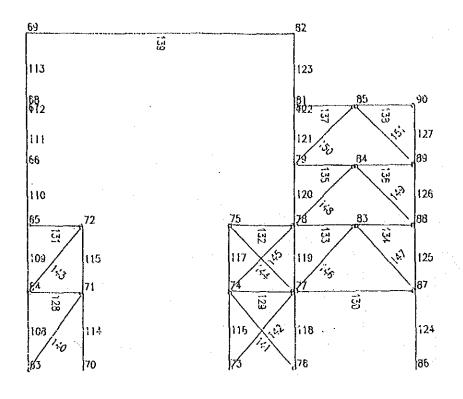


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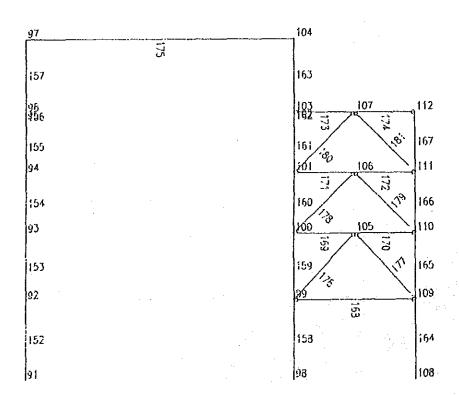
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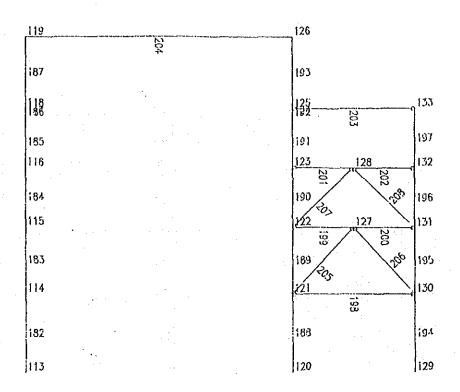
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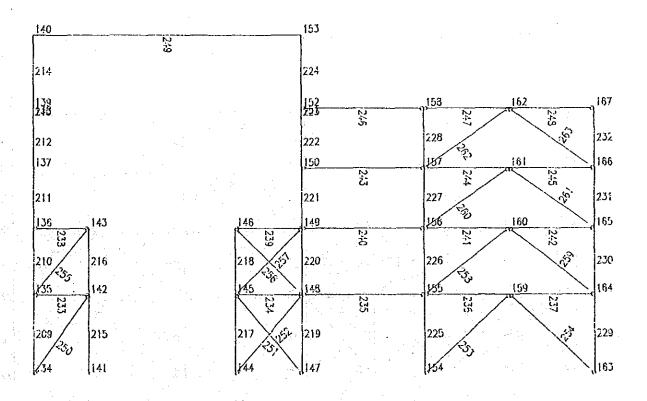


フレーム モデル 図 ( 103 -- LINE SCALE 1/300 )

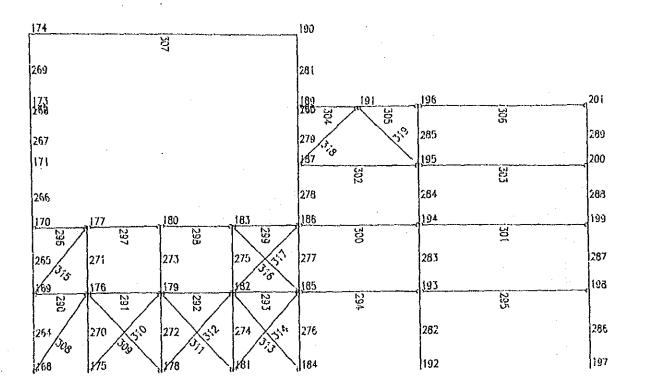


フレーム モデル 図 ( 104 - LINE SCALE 1/300 )

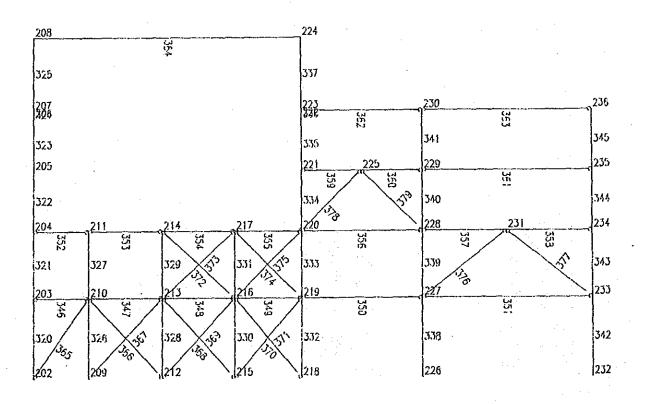




フレーム モデル 図 ( 106 - LINE SCALE 1/300 )



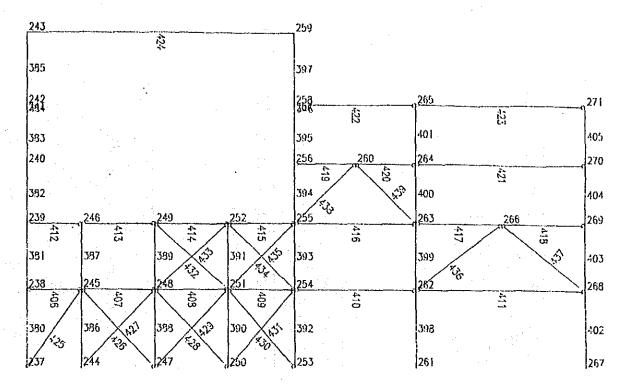
フレーム モデル 図 ( 107 - LINE SCALE 1/300 )



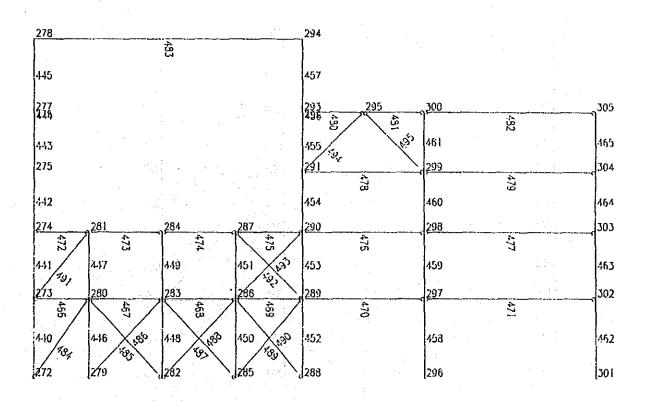
フレーム モデル 図 ( 108 - LINE SCALE 1/300 )

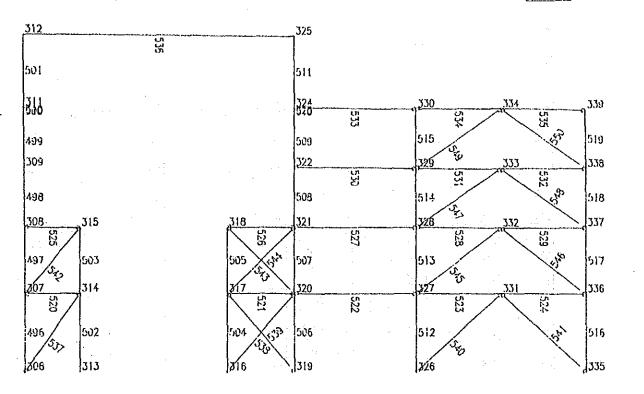
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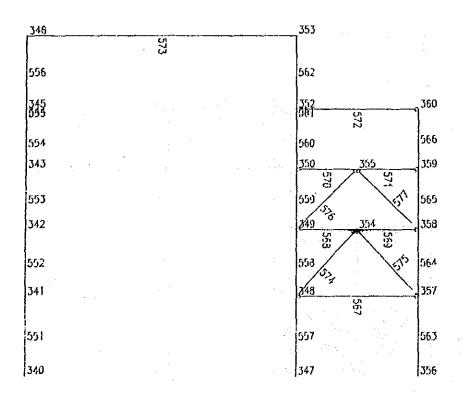


フレーム モデル 図 ( 201 - LINE SCALE 1/300 )

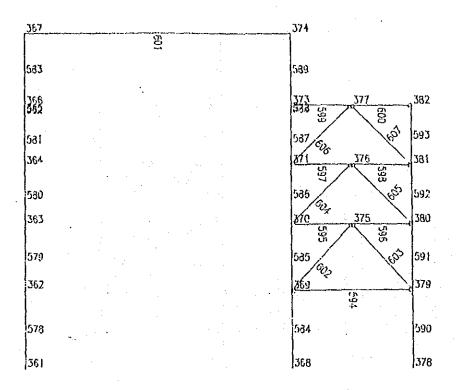




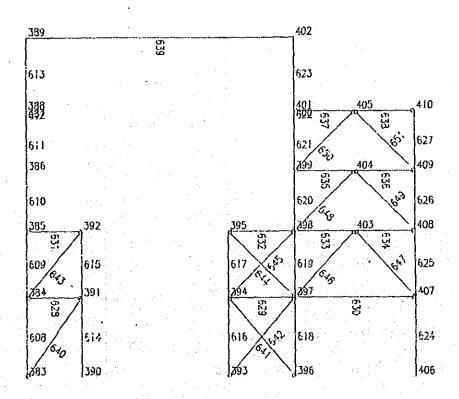
フレーム モデル 図 ( 203 - LINE SCALE 1/300 )



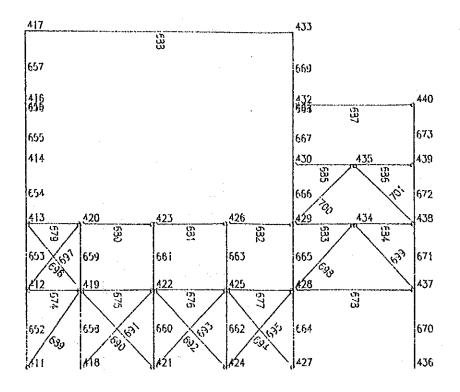
フレーム モデル 図 ( 204 - LINE SCALE 1/300 )



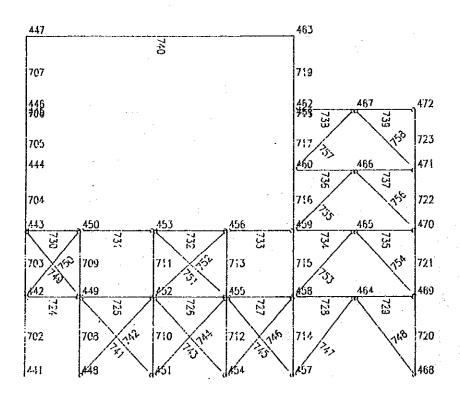
フレーム モデル 図 ( 205 - LINE SCALE 1/300 )



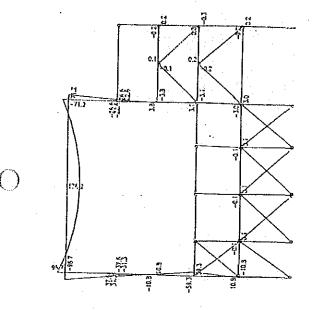
フレーム モデル 図 ( 206 - LINE SCALE 1/300 )

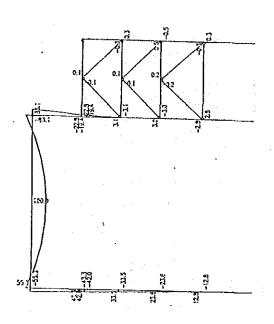


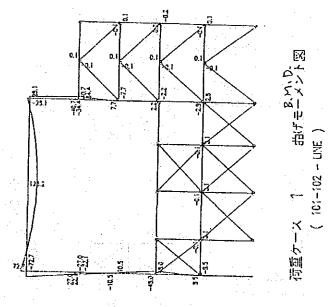
フレーム モデル 図 ( 207 - LINE SCALE 1/300 )

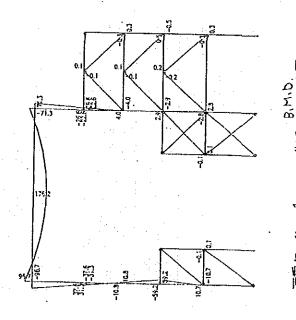


フレーム モデル 図 ( 208 - LINE SCALE 1/300 )

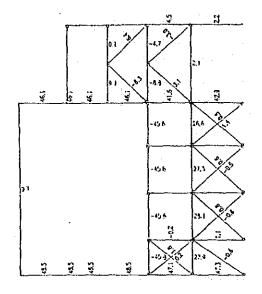


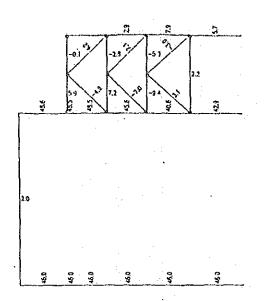


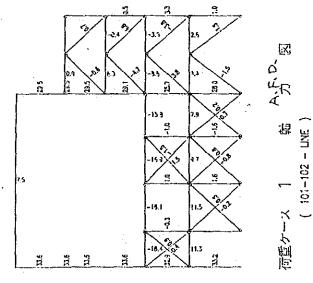


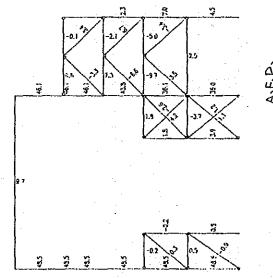


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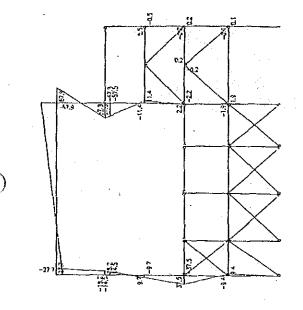


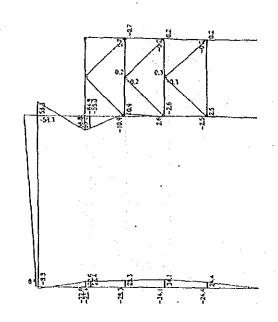


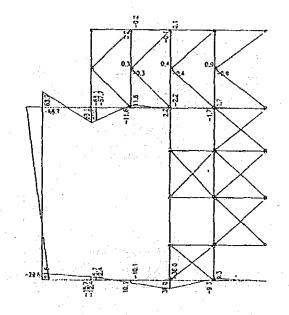


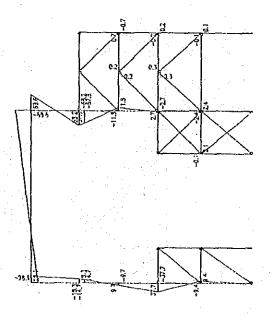
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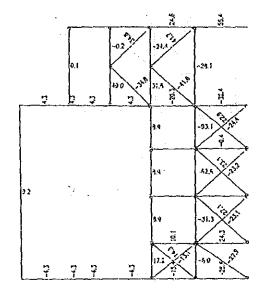


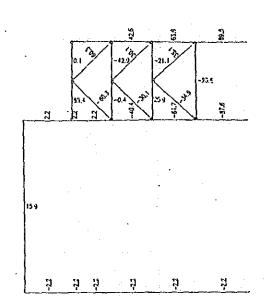


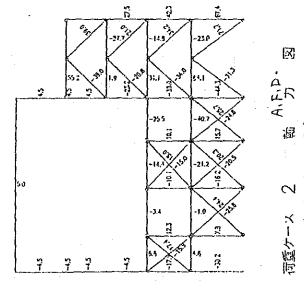


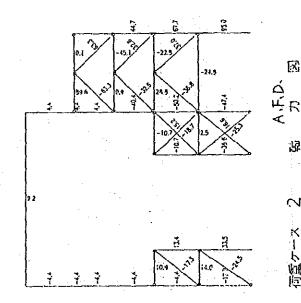


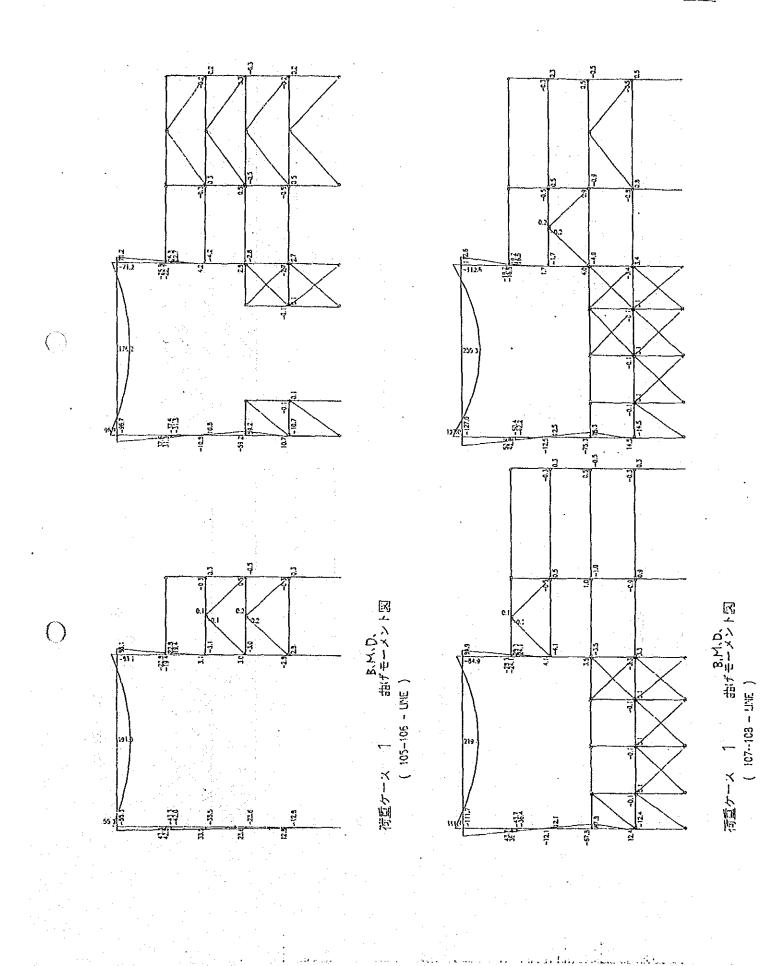
B,A,D, 出げホーメント図 (103-104 - UNE)

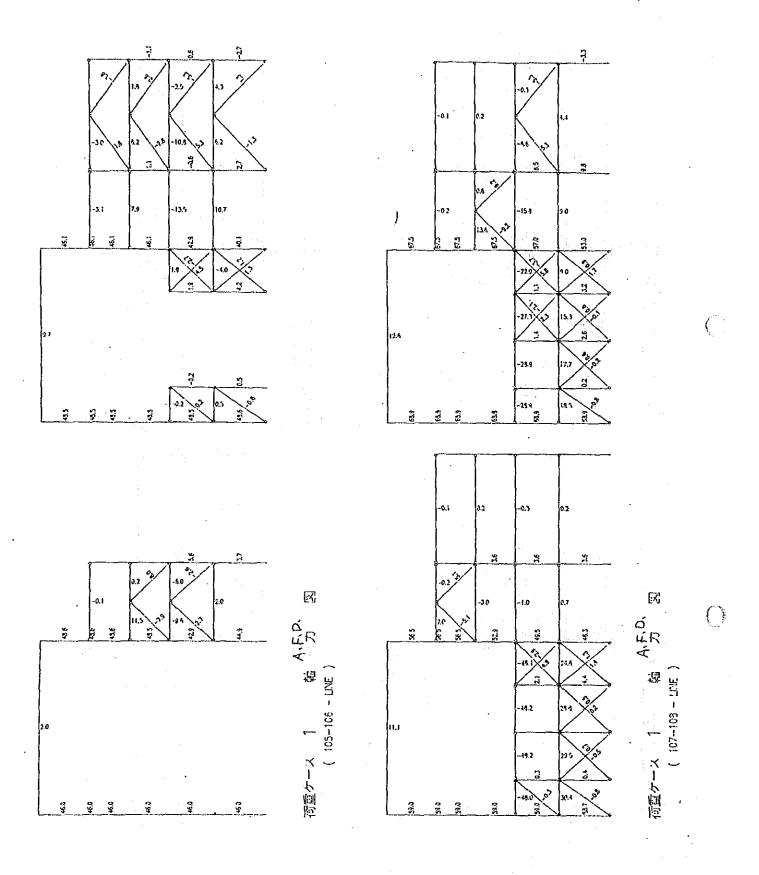




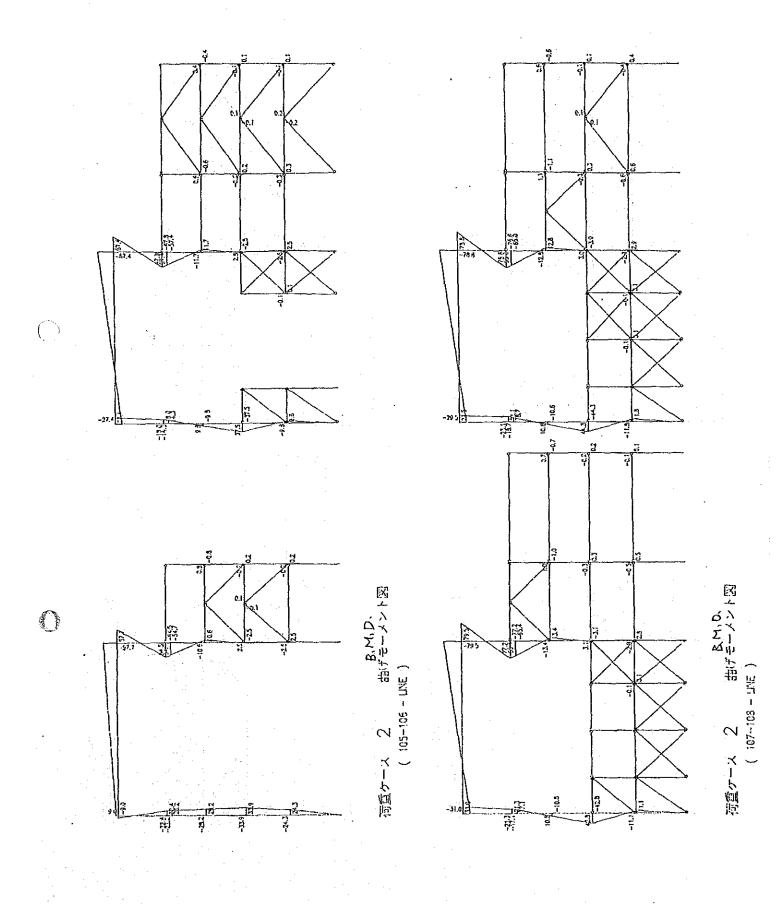




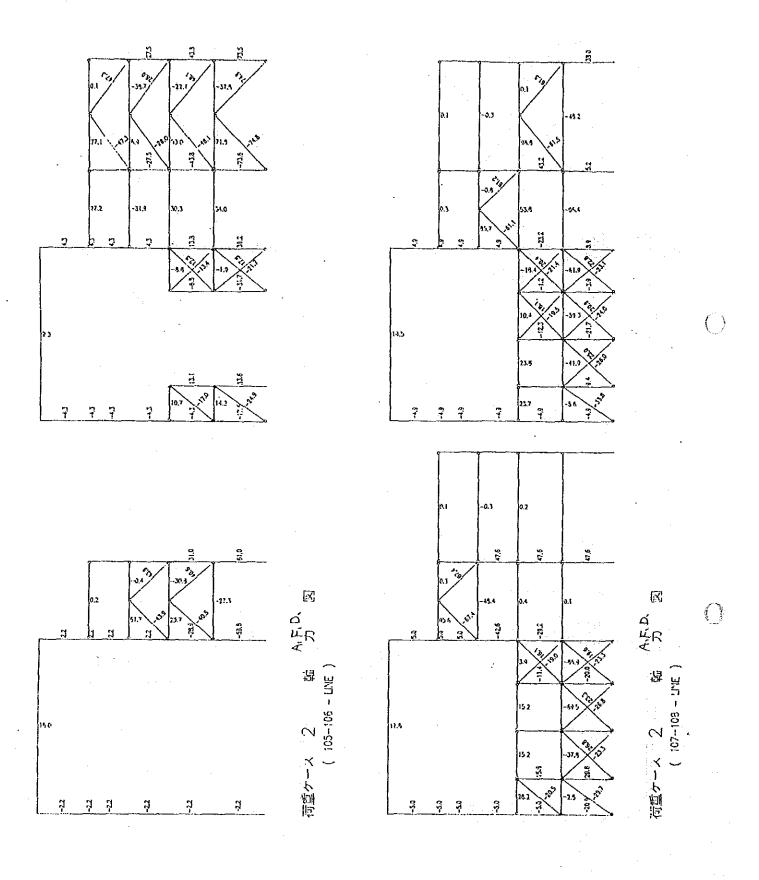


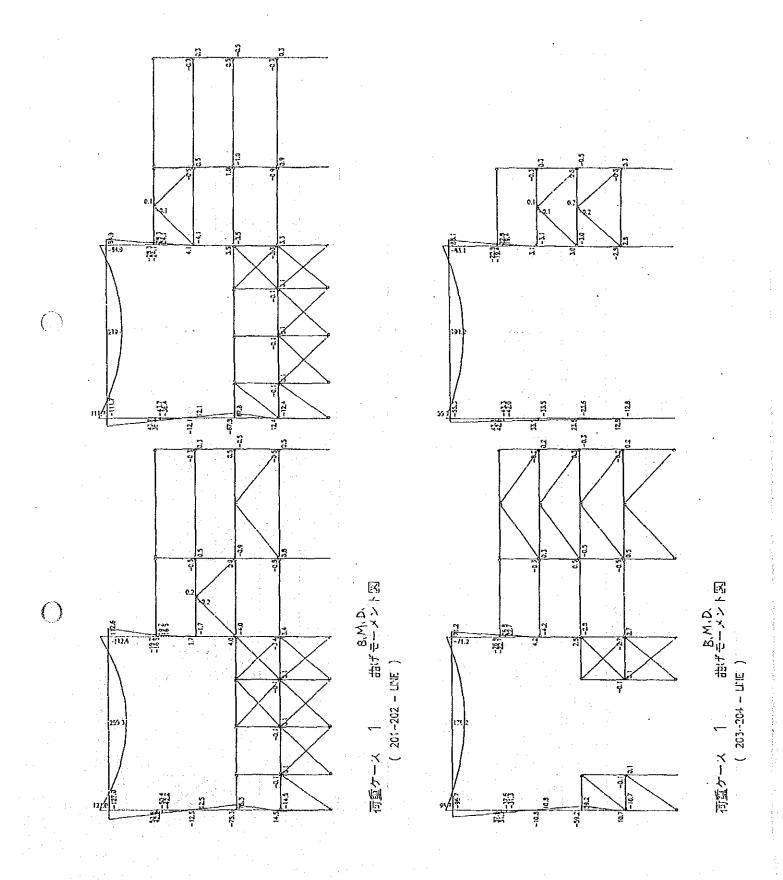


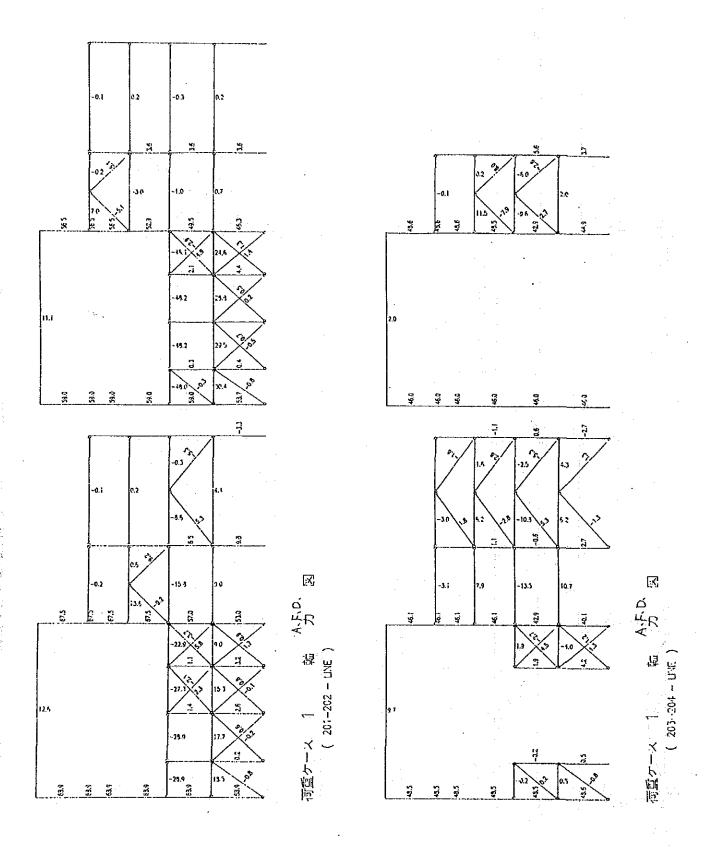
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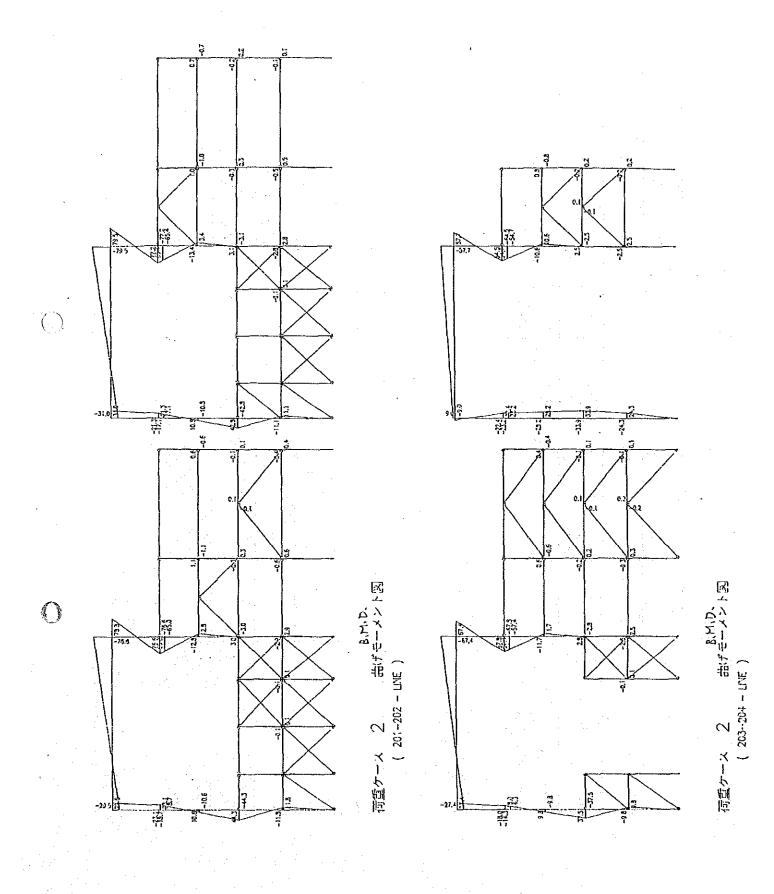


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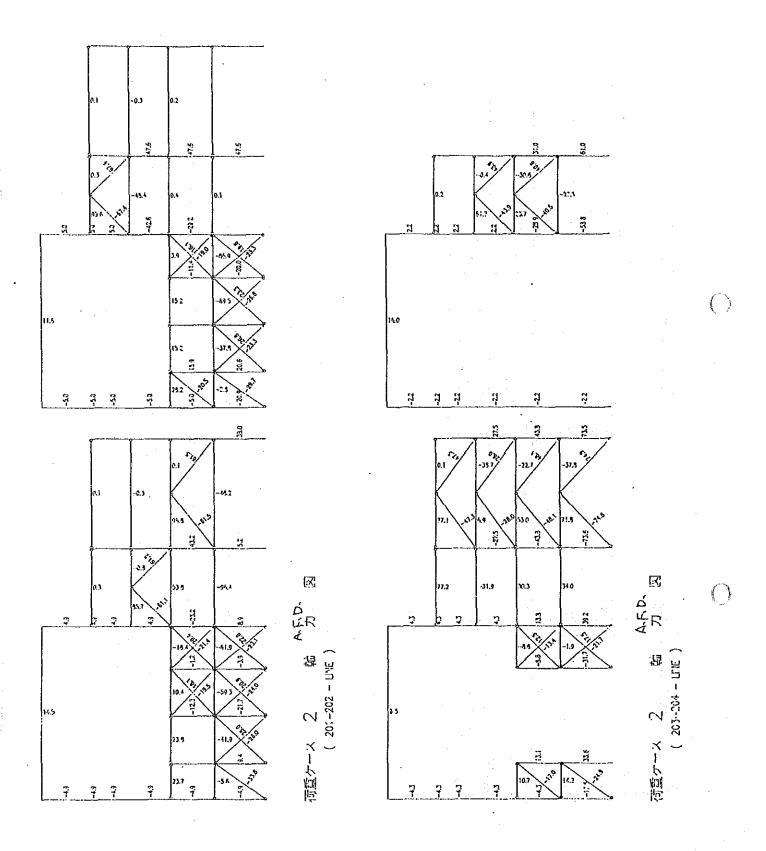


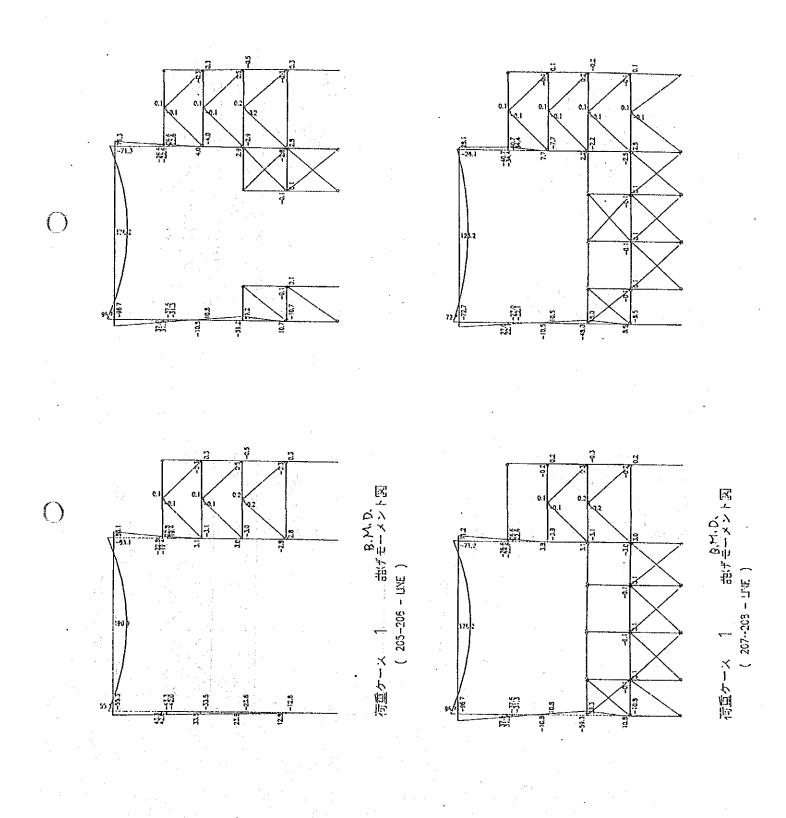


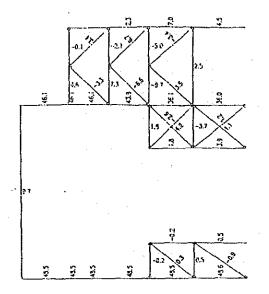


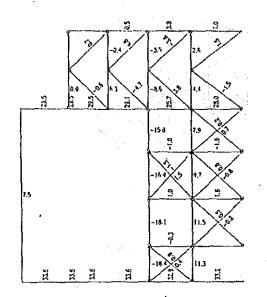


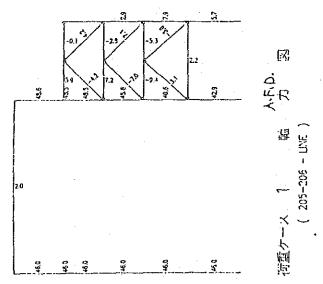
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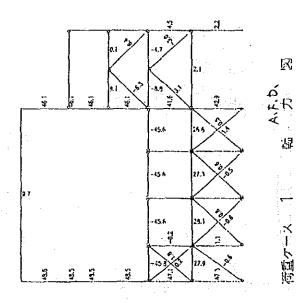




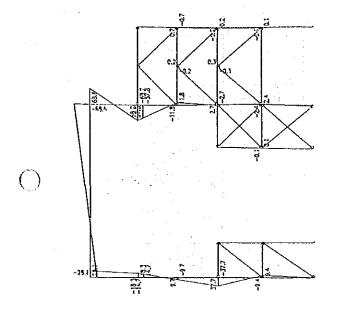


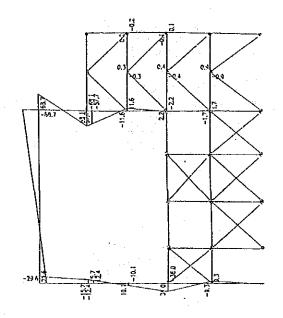


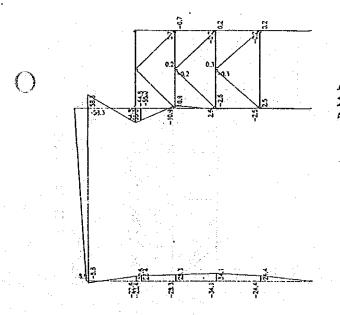


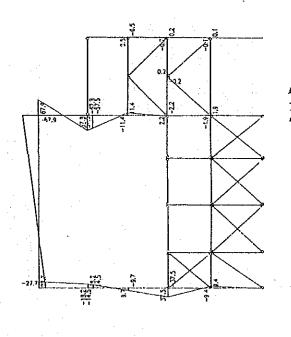


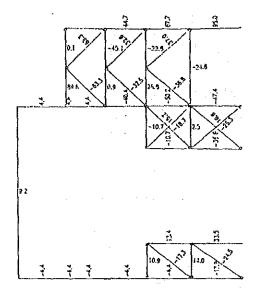
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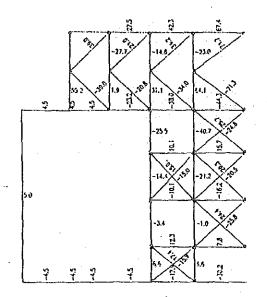


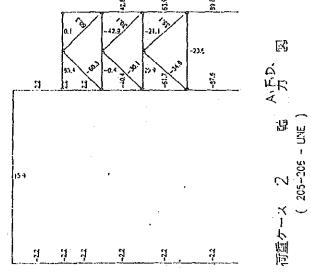


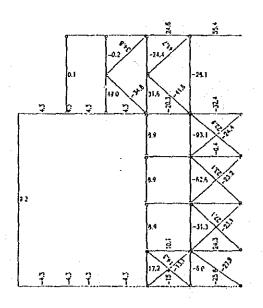


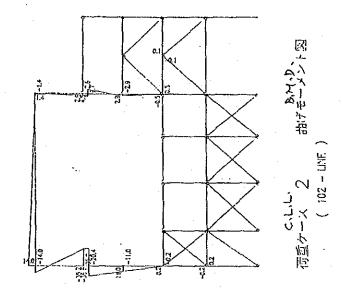


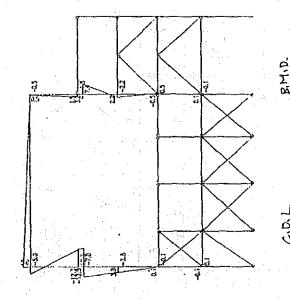


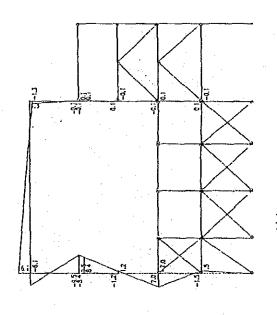






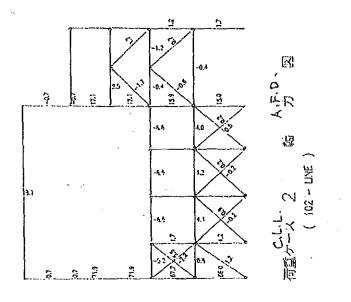


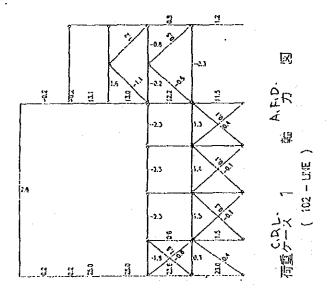


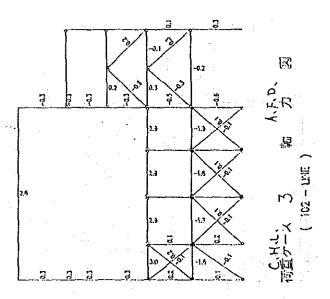


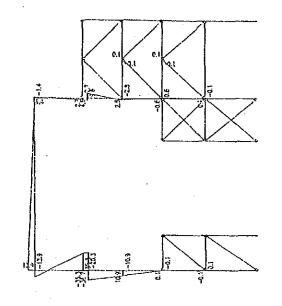
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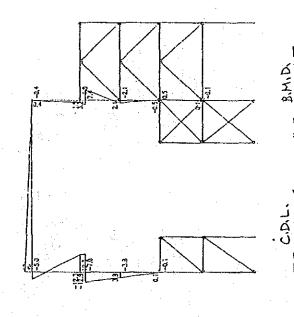


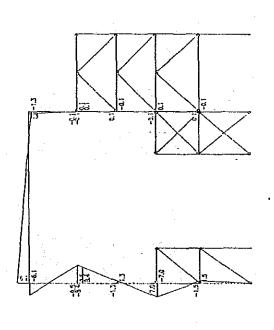


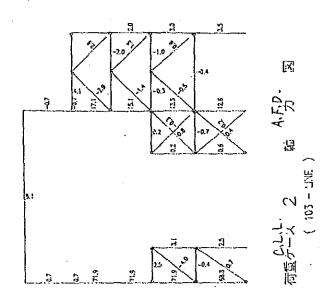


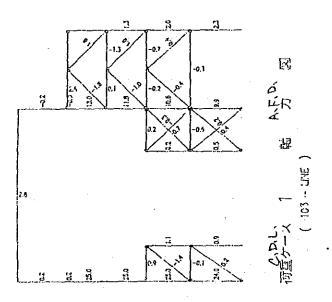


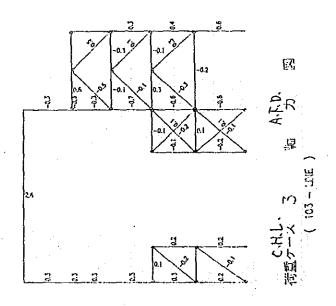
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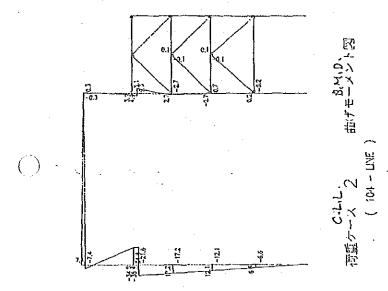


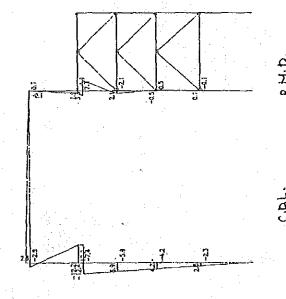


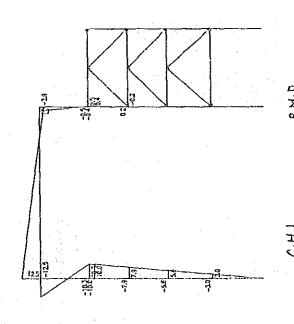




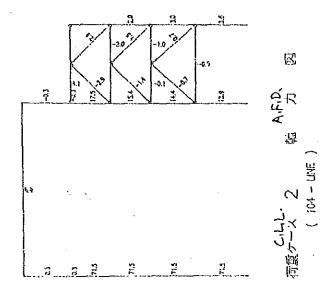
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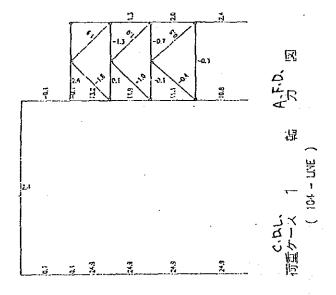


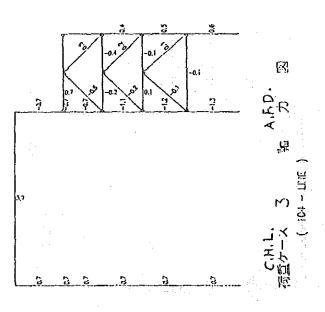


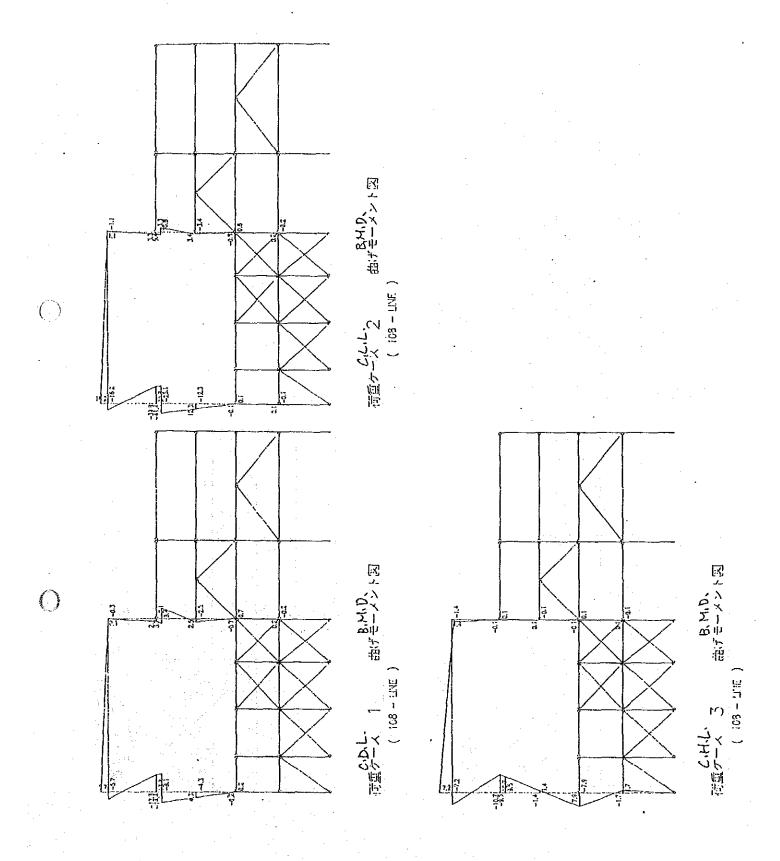


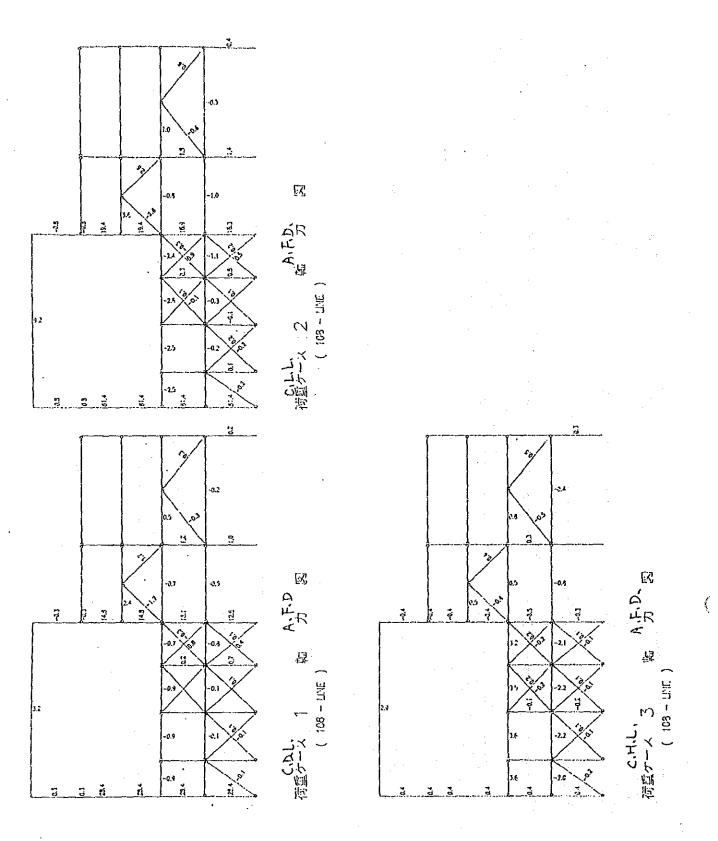
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[ [ [ ] [ ] [ ]	1:0 0:82 1:0 0:82 23:6 -0.1	25.0 -0.1 24.0 0.0 24.9 -7.4 24.9 -7.3	24.9 -7.4	25.0 -0.1
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TABL [24] [2600A-DIRLEV TION A-[0] RF A-208	A-207 A-207 A-103	A-206 A-206 A-205	A-204 A-204 A-706 A-705	

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TABLE	LOCA-DIRLEV.	- 1	P.F.	200	MEE	A T	906	7.00	₹ Fæ	72.2	A15 &	MEZ	43	Ž Š	ا ا ا ا	M W W	MEZ	M E	MER	<u>بر</u> پر	MEZ	MES	
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	LOCA	7	A-107 A-202	<u>:</u> _		A-108	• •		8-101	201-8	8-103	B-104	8-105	8-106	8-109 202	8-10	8-201	1 91-0	0-102	101-0	200	02-G	ļ

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<u>}</u>								•											_13	7	
(VL+CLL)	REMARKS	-												- 20			-;_		•		
CONDITIONS CONDITIONS HL+SL)	ζ t										22.	29.7	0	3 29.5	24.0	0.2	29.7	24.2	0.0		
10 11 11 11 11 11 11 11 11 11 11 11 11 1	M, tmQ										107.6	\$.99.€	17	139.3	80.4		138.4	82.7	4.0		
15.55	R,	9.8	1,69.1	[45.1 141.2	43.2	116,1.	201,3	193.2	184.3	133,3	40.7	199	237.5	57.6	98.5	345,4	50.0	168,4	431.6		:
PERMANENT CONDI TEMPORARY CONDI (VL+CDL+CHL+SL)	t Ö										1.2-	6.21	4,0	7.3	9.0	ó	7.3	0.6	.e		
TEMPOI (VL+C	M, tmO,										40.7	4 4	\$ 2	21.2	32.3	ە. ق	71.3	3.2.2	7.7		
1	N, t	81:1	128.4	104.5	43.2	76.0		189.4	180.5	93.2	36.7	72.0	196.1	53.8	10308	3/5.9	53.8	176.6	386.3		
0 H	ζ, <del>†</del>										210~	0.1	0.0	200-	0	0.0	-0.2	0.1	0.0		
LOAD	M. timo										-1.3	9.1	-6-	-1-3	- 6		-1-3	0.1	9~		
OAD OI D	Ñ, t	0	00	-0.2	0.0	210-	0,0	0.0	0.0	-0,2	-0.3	6,3	9.0	26,3	.0	~0.b	-0.3	-6.3	-0.6		
DEAD LOAD LIFTING L LOAD	1							2			22.4	-(5.9	က ဝ်	22.2	15,8	013	22.4	-16.0	4,0		
Olo	οι M, tmQ,			3							68.7	57.7	1.9	67.9	57.5	) 4	68.6	-57.0	24	:	
CRANE CRANE EISMI	N, t	.0.0. 15.7	40.7.	9.98-	0.0	40.7	-40.7	3.00	0 0	-90.7	9.0	9.0.	-44.3	2.21 4 3	24.3	24.3	-12. 2.	-24.3	-24.2		
)	1							:			2.0	5.9	•	20	2.9	00	2 6	. 6	0.0		
CLL	M, tho,										4-	d d	0.0	4	á.ŋ.	0.0	41-	9-6	-0.		
	4	٥	0,0	ء ه	0,0	9	00	9,0	ص ة	9.0	10-		15.0	0.0	1.7.1	5.0		-2	.9'2!		
(m	Z t									:	~·o	2.2	0,0	5.0		0.0	<del>- :</del> -	<del>                                     </del>	0,0		-
	M, tmO, t				:					i	4	3.4		5 0	. 4	7	4,0	4.4	- ဂု	:	-
TRES	4	0.0	0		00	5,0	có	6.0	6.0	0.5		3. [	5 /1	5.0	13.1	بر =	2 10	3.0	9		
COLUMN STRESS	γ.										-2.1	0.0	4-0	7.3		و ب	7.3	1.6	4.0	-	
COLU	M, tmQ, t							:			40.7		3	71.2	22.6	3.0	71.3		2.8		-
FOR 力報]	, t	8/-/	28.4	10.29	43.2	15.4	9.091	188-6	1.911	9.26	36.7		1.1/81	53.8					373.7	:	-
- Car 1 No.	لحا	MEZ		MER	-		-				P.F.	7.1-1 7.1-1 5.1-1			ETH 8	MES 30	η η Έν	•			-
TA		5	MEZ	Σ	Ī	<u>Σ</u>	<u> </u> <u>≾</u>	Σ	Σ	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	Δ.	<u>                                     </u>	<u> </u>	α α	<u>بر</u> ا	1 <u> </u>		1 0	1	L	L.
TABLE (神 近	TION	F-101	F-102	2000	55 % 670 %	F- 106	F-107	F-108	F-20 /	F-203	G-101	}		201-P	 -	· · · · · · · · · · · · · · · · · · ·	6-103	902			
<u></u> 1	1 F.1				<u> </u>	1				17_	<u>.                                     </u>			~	<u> </u>	·	1.0	<del></del>		<del></del>	

	1	<del></del>	·			<sub>1</sub>	<del>, , , , , ,</del>	τ	1	<del></del>	1	······································	<del></del>		<del></del>	<del></del>		<del>- 1</del>	· · ·	·	
(VL+CLL)	REMARKS						. "														
	رړ	29.3	22.3	0.0	29.6	27.2	0.0	21.5	24.2	9,0	35.1	27.1	6	40.9	24,6	5 6					
ZONTAL LOAD CONDITIONS CONDITIONS	TC M, tmO,	138.6	81.3	2.0	(38.0	91.0	0.2	136.9	87,4	5.0	L'291	97.6	80	1.89.7	89.8	6.0					
120N 00N 00N H +S	Z,	1 :3	191.2	464.7	53.2	104.17	366.6	57.6	8 80 80	321.7	5-69	116.3	393.1	9 18	125.55	486.3	70.3	148.4	125,3	313.0	
HOR NENT RARY	0 · 0	∀	8.0	ó		8.0	4.0	7.3	0	Ó	9.3	.0	0.0	15.3	7.5	0.7					
CRANE HORIZON PERMANENT CON TEMPORARY CON	PC .	23.4	28.7	2.6	```	28.7	2.6	71.2	32.8	2.5	9. 4.	39.9	*>	9 211	27.3	3.2					
OR H	+ Z	53.5	107.0	376.9	53.5	104.2	30 9.6	53.8	97.0	285.8	65.6	116.9	397.8	77.4	134.6	487.5	9.25		87.3	209.0	
H SE	0, t	1	0.(	0.0	-0.6	0, (	0.0	-0,2	0.1	0.0	-0,2		0,0	210-	0,0	0.0	:				
,	CHL THO	-2.9	0,4	0.0		4,6	0,0	-1.3	9.1	7	, <u>,</u> ,	6	-0,1	サデ	0,1	-0.1					
LOAD	2	7.0-	.o.7	-1.3	-0.7			20,3	-0,3	9.6~	4	-0,4	က ဝ	4.0	-04	. 0, 3	00	0,3	0.0	6,3	
L LOAD DEAD LO LIFTING LOAD	4	ا^	1.5.1	2,6	11	0.51-	<b>∀</b>	22.2	15.9	0,4		-18-1	0,4	25.5	-19.9	5.4					
1-6	내겠던	58.3	-55.0	3 2	50.7			67.4	-57.4	5.5	79.5	-65.4	2,8	78.8	-65.0	2.9	:				
ERTICAL CRANE D CRANE L	, Z	2.5	2.2	0.0	2.5).	24.3	24,3, -58,8,	-12.2 ★3	-79.3 4.3	5.45.5	5.4	.16.9.	1,45	4.9	-16.9	-16.9	27.5	.0.0.	38.0	55.4	
V	بد  ار	l v	2,8	0.0	510	2,8	0.0	0.2	2.9	0.0	4.0	ر ئ	2 0	9	3.3	2 0					
VL CDL CLL	CLL M, tmO	0,3	9.3	9.2	9 3	9 3	210-	4	9.6	167	-1-1	10.8	2.0-	1.1	10.8	-0.2					,!
	Z.	8,0	17.5	. 9		17.5	13.9	-0.4	[22]	9721	6.0-		16.3	8,0-	4,61		~	<u> </u>			
NOTE 4)	Ď, t	^^ • 6	2.1	0	0.3	2.1	0 %	0.2	2.2	0.0	6.3	1	0.0	8 0	2.5	. 0			7. 7.		
	CDI. M, tmO,	ó	2.5	<u> </u>	0.1	9.3	ģ	-0,4	<u>с</u> 4	-0.	-0.3	8.4	-0.2	~0,3	. ₩ ∞						
STRESS	14 14	<b>6</b>	13.2	10.0	100	(3.2	€,01	-0.2	13.0	9.9	-0.3	14,8	7.5	-6,3	\$ 5		0.0	2.7	0.0	21/	
согами	φ 4	0.0	5,2	6,4	9.9	, <u>r</u> ,	<del>ψ</del>	7,7	5.9	4,0	9,3	6.5	0.5	15.3	2,2						
· ·	VI. M, tmb,	83.1	19.4	2.8	83.1	4.6	2.8	71.2	22.7	2,7	84.9	24.1		112.6	15,51	4%					
. FOR 比	ير بد	53.8	159.5	365.0	53.8	36.7	295.7	53,8	99.9	273.2	65.6	97.5	381.5	77.4	115.2			<del>!</del> -	4rH 89.3	207.3	
TABLE [林 蔚	J • ;	RF	51H	MER	다	T.	MEZ	R	STH	MER	RF.	STH	MER	19 T	Ŧ	Z.	£	MER	4 1.7.4	HE&	;
F1	COCA-DIRLEV TION	٠ ـــ		.													0	3			
	LOCA	6-104	8		8-105 204			G-106			6-107 202		· .	801-5	3		191-14	<u> </u>	4-11 2	S	

AD CHL CRANE HORIZONTAL LOAD  TING LOAD  TO TO TO TO TO TEMPORARY CONDITIONS  AD CHL  (VL+CDL+CHL+5L)  (VL+CL+CHL+5L)  (VL+CDL+CHL+5L)  (VL+CL+CHL+5L)  (VL+CL+CHL+5L)  (VL-CL+CHL+5L)  (VL-CL+CHL+5L)  (VL-CL+CHL+5L)  (VL-CL+CHL+5L)  (VL-CL+CHL	
ADD CHL CRANE HORIZONTAL LOAD TING LOAD TC PERMANENT CONDITIONS AD (VL+CDL+CHL+5L)  (VL+CDL+CHL+5L) (VL+C	
DAD CHL—— CRANE HOR FING LOAD PC —— PERMANENT ING LOAD TC —— TEMPORARY (VL+CDL+C)  AD CHL PC  (VL+CDL+C)  (VL+CDL+	
DAD CHL—— CRANE HOR FING LOAD PC —— PERMANENT ING LOAD TC —— TEMPORARY (VL+CDL+C)  AD CHL PC  (VL+CDL+C)  (VL+CDL+	
DEDAD CHL  NING LOAD TC  ND CHL  O. A. CHL  O. CHL  O. A. C	389.6
DEDAD CHL  NING LOAD TC  ND CHL  O. A. CHL  O. CHL  O. A. C	
D. LOAD CHL.  D. LOAD PC.  D. LOAD TC.  CHL.  CH	304.0
DAD O LOAD PO LOAD PO LOAD PO CHILL AN, t M, t	/2/ 08
DAD DO LOAD HING LOAD HING LOAD W. t N. t O. 0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
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ERTICAL L CRANE DEAJ CRANE LIF EISMIC LO. SL. M. tmQ 	38.0
	0000
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L COLUMN VL WL tmo. t	
23.6 Go.1 (23.7) (23.7	
	3.6
	7H 121.1
EOCA-D EION H-103 H-105 H-105 H-105 H-105 K-105 K-105 K-105	47H 121.1

(vr+cLL)	ARKS			<del></del> :						:							·	· · · · · · · · · · · · · · · · · · ·		. 11 . 2			
(VL+	REMARKS										÷					٠,						- 1	
ZONITAL LOAD   CONDITIONS (CONDITIONS   CONDITIONS	1 1	ζ, L																					
TAL	∦ԲՎ.	7, TMC																					
ZON COON		N, T	5/01/2	3.25.8	(38.2)	334.9	82.8	272.9															
CRANE HORI PERMANENT TEMPORARY	11	اري																					
SANE SRMAJ	PC	3, tm - 0,									1												
111	-	- L	113.7	2.067	105.2	25		199.3	<del></del>		:								:				7
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AT THE THE PER PER PER PER PER PER PER PER PER PE		t n		<u>:</u>														:					
CRANE D		ין מ	0,0	0.6.0	0,0	2.98.5	0,0	73.5															
	'l	Ų			1	:		<u> </u>															
VE	CLL	, E		<u>:</u> :									:					:					
>000		ب ال	0.0	4	0	4	0.0	4,0	:														New York
NOTE		۲. حا		:		:																	
	CDL			- <u>:</u> -		:		•															
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COLUMN	VI	<u> </u>				•		: .			:			:		<u>:</u>   <u>:</u>		:	<u>:                                    </u>		<u>;</u>		
1 ~	1  2	- -	113.7	289.8	105.2	24.8.7	f	178.9					:	:		:		:			:		
. ~ . <del>-</del>	√ I.		4TH 1:	MEK 28	4TH 10	ME2 24		ME8 17	:	:	<del>-</del>	:				i	<u> </u>	:	<u>;                                    </u>		9 E	]:	
TAB	TRUE	+	4	Σ	4	ž	- ₩	I.		<u> </u>		<u> </u>		<u> </u>	l	L	<u> </u>	<u> </u>	<u></u>	L		l	
	LOCA-DIRLEV.	10/-/	) 2		K-202		K-203			· ·												•	

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									NOTE:	σ b/					
			~~~		COT	****	1 1			σ c/:					_
7						UMN	(1)			σ b/.	fb +	σC	/fc ·	< 1.	0 .
			の斯												
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ION	I O VI	1	N	М	Q .	И	M	Q	(A, I, Z)	imin		fc	fb	fc	$\sigma c/f$
A-los	٠, ١							ا ا	BH-700x350x28×40	1	.46.		1.07	0.12	<u></u>
106	_3		34.3	110.6	17.0		123.0	11:4	453.6 10,311 BH-900x350x28×4		59	1.35	0.67	0.09	0.76
20.3	2		ing 3	59.1	127	201.5		ا ع د د	509.6 14,372	1 3000	. 60	1.6			_ ~
203	-=-		111-5	\$ 10.1	12:1		103.0	<<.D	501.0 14,372		73		0.26	0.30	0.5
208	7	÷	219.1	10.6		191.6		2 4	Ditto	. ]. 9./.5.]	.7.L. 87	1.6.			0.4
			21 1.1	10:0	<del>-'`</del> -	74 1-0	27.0	3,7	BH-700X 400 X 28 X C	0 10.8	90		1.24	0.16	<u> </u>
A-107	3	111	20.2	1/12 >	218	ככם	1550	ا و در	493.6 11.557	1	46	l.b.	0.78		0.89
ς <u>ι</u>			70-61	743, 6	. 27.0	06.0	755.0		BH-900x900 x28 x 4			1.6		0.38	<u> </u>
202	2		210 2	47.9	102	1624	וו פוו		549.6 16.016	12.7.1.	63		0.50		0,5
ł	-		210.0	5 /-	7 0	1021	1,70.7		0.17.0	16.2.	62		0.08		
. }	4		254 9	12.5	, 0	227.4	25. 2	39	Ditte	8.8	74	1.16		1	0.4
		-	30	77.3					BH-70x350 x28x4		4.5		18.0		
G-101	3		£\$. 51	83.4	ma	55.2	138.6	20.8	453.6 10.311	1	54		0.51	0.09	0.60
106				<u>(, , , , , , , , , , , , , , , , , , ,</u>	1122	1	1 10010	- 1:5	BH-9083508 2889		.55	16.	0.20	0.35	<u> </u>
203	2		177.6	20.7	80	101.2	   81. ₹	22.3	509.6 14.372		67	1.23		1	0.41
			1 2	-00	0.0	1	1	-	00,00 (1,10,10	.2.15	7.1	1.6.	0.02	0.76	<u> </u>
208	4		3,88	2.7	0.4	437.6	0.4	0.0	Ditto	2.5	87	7.02	!	0.75	0.76
G-107	-				<u> </u>		<u> </u>		BH-700×900 X28X4		40	1,6.	0.70	0.16	i
4-101	3		17.4	112.6	15.3	87.6	189.7	40.9	493.6 16.016		46		0.04	0.11	0.5
202		14. Tu							BH-90x90x2879		48	.16.	0.17	0.24	
i	2	1	134.6	27.3	7.5	125.5	89.8	24.6	549.6 16.016		57	1.32	0.11	0.18	0.20
1									,	10.5	6.≥	16.	0.02	0.88	
	1		481.5	3.2	0.7	486.3	6.0	0.9	Ditto	8.8	74	1.16	5.01	0.76	0.77
B-101								1							1
103							<u> </u>	ļ							ļ
106								-	'						
201					<u> </u>		<u> </u>	ļ	. \.			<u> </u>			<b>}</b>
20/							ļ		H-3002			.,		0.82	
388			98.8	0.0	0.0		<u> </u>	ļ	114.8	7.51	87	1.02	ļ	0.80	0.80
8-102	. !				{		\ ·	\ ^		<b></b> .			}	1	
202						<del> </del>	<del> </del>	ļ	<b></b>						<del> </del>
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}	1 1			<del> </del> -		<del> </del>		<u> </u>	H-3503		<del></del>	<b> </b>	<del>                                     </del>	0.83	-
	1	. 9.	143.6			ļ			173.9	است م	D4			0.72	0.73
			123.0	0.0	0.0	<del> </del>		<u> </u>	1.17 - 1	8.84	74	1-16	<del>}</del>	10.12	1 0 /4
D-101		* 				 			1						
100	<del>                                     </del>			342	<del> </del>	<del>                                     </del>	<del>                                     </del>	<del> </del> -				<del> </del>	<del> </del> -		
}						. :	1		}			\			1
				<b></b>		-	<del> </del>	<del> </del>	H-3005	<del></del>			<del> </del>	0175	<b> </b>
1	7		89.3	0.0	0.0	Maria d			119.8	7.51	80	1.02		0.74	0,74
j	1 1 1 1 1	L	· · · · ·		1010		<u> </u>	J	MBER OF CO	1.01	101	11.7 -	<del></del>	1 7 - / -	

A,I,Z --- SECTION AREA (cm2), GEOMETRY MOMENT OF INERTIA (cm4), SECTION MODULUS (cm3)

A.STRESS --- ALLOWABLE STRESS

i,η --- CONSTANTS FOR DECISION OF ALLOWABLE BENDING STRESS imin --- MINIMUM RADIUS OF SECTION (cm)
λ b,λ c --- BUCKLING COEFFICIENT FOR BENDING MOMENT AND AXIAL FORCE fb,fc --- ALLOWABLE STRESS FOR BENDING AND AXIAL FORCE (t/cm2)

SHEET 142 OF

NOTE:  $\sigma$  b/fb < 1.0  $\sigma$  c/fc < 1.0 DECISION OF COLUMN (2)  $\sigma$  b/fb +  $\sigma$  c/fc < 1.0 [柱の断面算定] o b COCADIVDIR PERMANENT oc ob/fb TEMPORARY MEMBER A.STRESS i,η λb fb X CONDITIONS CONDITIONS o b Q C fc fb гіоимо. N Μ. (A,I,Z) $imin \lambda c$ fc σ c/fc D-103 20 7 1.14 H-3502 8.84 74 0.98 198.5 0.0 0.98 0.0 173.9 F-101 103 563 ঠo ৮ H-3002 0.89 805 0.0 0.85 104.5 0.0 119.8 7.57 87 234 E- 105 10°7 207 H-350? 1.09 0.94 189.4 0.0 1 0.0 173.9 8.24 74 1.16 0.94 4-101 208 H-300 2 0.36 70.3 7.51 67 1623 0.29 0.29 47.8 119.8 1.24 7.51 87 1.02 0.81 148,4 Ditto 81.2 0.81 H-105 209 0.64 H-390 69 87.3 7.28 0,53 0.53 H-4002 0.96 0.76 1.26 0.76 209.0 64 14-103 Clobx H-440 1.03 204 7.18 70 157.4 0,86 0,86 162.9 206 BH- 4502 614 11.46 289.0 57 <u>هری ۱</u> 0,89 341.9 104 4-107 -108 BH-480 X350 x 16 x 22 1,07 - 202 223.76 239.4 8.38 60 0.82 34.516x500 203 1.20 205 1594.5 495.92 12.96 0.87 NOTATION: DIV NO. --- DIVISION NUMBER OF COLUMN DIR --- DIRECTION, X OR Y N,M,Q --- AXIAL FORCE(t), BENDING MOMENT(tm), SHEAR FORCE(t) A,I,Z --- SECTION AREA (cm2), GEOMETRY MOMENT OF INERTIA (cm4), SECTION MODULUS (cm3) 3045 (30 A.STRESS --- ALLOWABLE STRESS i, 7 --- CONSTANTS FOR DECISION OF ALLOWABLE BENDING STRESS imin --- MINIMUM RADIUS OF SECTION (cm) λ b, λ c --- BUCKLING COEFFICIENT FOR BENDING MOMENT AND AXIAL FORCE fb,fc --- ALLOWABLE STRESS FOR BENDING AND AXIAL FORCE (t/cm2)

SHEET 143 OF

[									NOTE:	σb/í		1.0		OF-	
									MOID.	σ c/1				:	
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LOCA	DIV			MANE			PORA		MEMBER	A. S	TRE	SS	σb	σс	o b/fb
		X			ONS					i, n	\ b	fb	<u>o b</u>	o c	+
TION	NO.	Y	N	М	Q	N	М	Q	(A,I,Z)	imin/	\ C	fc	fb	fc	σc/fc
K-106 203															
									H-3502				7	0,35	
	2		60.1			[ 	ļ	<u> </u>	103.9	8.84	[2	1.35		0.27	0,27
	1		179.3		. : i				Ditto	8,89	74	1.16		1.03	0.89
K~10J								1							
202				<u> </u>				<u> </u>	H-440	1				0,85	<b></b>
	7		133.7	<u> </u>			L	<u>.                                    </u>	157.4	7-18	70_	1.20		0.71	0.91
		Ĭ		· _					H-4502					1.05	
			304.0	<u> </u>	<u> </u>				289.0	11.46	57	1.32		0.80	0.80
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N,M,Q --- AXIAL FORCE(t), BENDING MOMENT(tm), SHEAR FORCE(t)

A,I,Z --- SECTION AREA (cm2), GEOMETRY MOMENT OF INERTIA (cm4), SECTION MODULUS (cm3)

A.STRESS --- ALLOWABLE STRESS

i,η --- CONSTANTS FOR DECISION OF ALLOWABLE BENDING STRESS imin --- MINIMUM RADIUS OF SECTION (cm)
λ b,λ c --- BUCKLING COEFFICIENT FOR BENDING MOMENT AND AXIAL FORCE fb,fc --- ALLOWABLE STRESS FOR BENDING AND AXIAL FORCE (t/cm2)

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	4,	2	De.	سهواد	10	Colu	(MM)	Ва	se.												-	
(VE+CLL)		REMARKS		X pirectrom	Ditto			X piraction	Γ	Ditto			X Direction	Ditto			X Directions	0:70				: .
OAD	SNS	1		20.6	3.05.	3.6	<b>Ы</b>	20.65	47.4	2.8		17.4	33.2	33-9.			1,83.9	3.0	17.8	11:8		
ALL	) III	TC M.tmD																				
CONDITIONS	CONE	<b>↓</b>	2	57.5	3/6.1	133.0	/33.9	4, 5° €	250.7	1.292		98.9	4,564	163.0			13.6	56.9	60.6	9.26.		
HORI		Z	, ;	1:5		-7:0:	-7.0	4	/			0.7	16.9	-11.9			0.0	0.7	0.0	0,7		
CRANE HORI	TEMPORARY CONDI (VL+CDL+CHL+SL)	. PG +			-:-			<u> </u>					•									
11 1	1	1		209.4	211.8	178.0	179.0	2/9.7		2,4		5.09	129.4	81.9	1:33	36.1	98.8	43.5	90.7	82.7		
CHL	i D		'n		<del></del>	2,0		· η 0		4	1	0.7	0,0	0.0			0.0	0 /	0.1	0		
		ξ 1 1 1 2 1 1 2 1 1 2 1 3 1 3 1 3 1 3 1 3						:														
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S - C	コ 3 3 5 7	4	1 5	· -		01-		9	1			170	0.0	0.0			0.0		0.1	6		
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> O	ပေ ທ	U ∑	1:~	67.0	5 69	11.5	2/2	4.64	ŝ	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		4	7.7	2.5	_ <u>-</u> -		5.	1.0-	1.0-	9.1		
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۲, ۱	/ }	CDE M +mD				<u>'</u>			<del> </del>				- <del>:-</del> -			.> •						
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	COLUMN	VI.				:	:			-	<del></del> -	\- <u>:</u> -	:	1:	<u>:</u>			1:			2	
0	ž K	τ. Σ	1	142.4	7251	106.5	107.5	750.3	2 2 2			86.4	125.3	79.4	35.1	36.1	96.3	143.6	8.06	<u>   </u>		
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L		7 E						<u></u>				S)								L		

	(עב+כרר)	REMARKS		Y Direction						YDirection	X Divection	0;40	$\times$	V Divection				×	Y Direction				
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	200 XX		ار اح	76.0		1321	2/9.8	208.7		93.1	45.7	187.4			243.3	(82.5		-	.44.3				
i	CRANE HOR (ZON) PERMANENT CON) TEMPORARY CON) (VL+COL+CHL+SI		다 나	0.7	0.7	/ 0	ò	ó		-6.2	11.9	-0.3		-6.3	9:21:9	N.O	0.2	-0.3					
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			الا ح	89.8	3.98.6			<del> </del>		978	<del> </del> -	1	43.2	76.3	6.797	189.9	(81.0	93.5					
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	VERTICAL CRANE D CRANE L SEISMIC		z	7 -13,	2.9	-:-	1		<u> </u>	1 1	82.7.	1	· !	3 -48 3	582.7	-6	• 4	3 -48.3	•				
			다 일	ó	0, 2		6	6	1	20.2	2.01	.0.		6.3	2.0	2 6	0.2	9	-:				
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	COLUMN		Z tigo,			:				\	11.9	.:-11.9.		11.9.	::11:3								
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	TABI	COCA-DIREEV.		6												;			ļ	<u> </u>	1		
		G-AC	NO NO	208	201	207	80/	102		101	102	20,00	4505	\$	707	803	201	203				<del></del> -	
	<u></u>	ğ	LION	۵		<u> </u>				F-101	<u> </u>											<u> </u>	· ]

(VL+CLL)		Direction			-						Y Direction .	Divection	Ditto		Direction	Y Direction	Direction	D: #0	Y Direction		
IZONTAL LOAD CONDITIONS (V CONDITIONS HL+SL)	<u>ئ</u> ل	70.1	15.1	32.0	4.0	36.9	.36.9. 11.7	74.5	74.5		53.4.	20.6. 20.6 X	20.6		20.6 X	711.3. 38.3 Y	114.9. 13.3 X	3,3	.69.8 39.8		
JAL L	E				•				:												
PERMANE HORIZONI PERMANENT CONI TEMPORARY CONI (VL+CDL+CHL+SI	Į.	2.5 A.0.	378.8 285.9	339.0	464.7	372 8	349.5 220.6	4.8.4	4.65.9		205. 44.8	33.50 20.50	408.5	398.2	356.5	245.7	9.00.	372.7	747.4		
	t)	9.6	0.50	0.7	0.0	9.49	1.0	20	30.3		9.7	0.0	0	0	0	-16.5	80	8:00	-15.0		
CRANE HOR PERMANENT TEMPORARY (VL+CDL+C	M, tmO										:										
유 대 대 대 대 대 ( ( ( ( ( ( ( ( ( ( ( ( ( (	, th	196.2	316.0	386.4	378.9	300€	285.9	1980	481.7		81.2	209.0	284.4	309.0	235.4	3469	522.4	594.5	375.3		
CHL	t,	1.0	0.7	0.0	90	٥			- 0		000			0.0		0.0	0.0	0.0	0.0		
	M, tmO,																				
д 107	1	5'0-	-0.5	-0.6	, w	~	9.0	~	20-		3,3		9.0	9.0	9,0	3,0	0.0	00	9.0		
16 H ();	4	6.3	12.0	-:-	4	22.0	1	24.2	<del></del>	•	43.7	20.6	20.5	0.0	20.5	20.05	17:59	69.7	20.6 77.8		
	t mO																				
ERTICAL CRANE D CRANE L EISMIC	τ	2.18-	45.3	347	-87.6	.66.3	166.3	74.3	-19.3		174.3	175.3	0.29	80	125.3	-125.3	220.4	2.2	125.3		
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	t M,	(5.1	1 5	12.7	6.5	· · ·		:10	16.7		1.7	1.7	5,5	3.6	7	~	4	4	3.5		
NOTE	τ, Σ		0,0				<del> </del> -	<u> </u>	1		0.0	0.0		000	<del> </del> -		0.0	000	0.0		
NC NC S) 3	M, tmo, t																				
BASE C	t)	11.6	11.6	0,0	a0 6	8	0.6	2	12.6		2/				4	~	9	0	2 3		
1 1	Z V	6.6-	6.51	0'5/-		.6.9	6.67-	30.3	2.05.		6.0					277	_	3.08	0.57		
COLUMN	M, tmb	: 1		1:		···	<u>                                    </u>	3									;				
YOR W	Σ Ψ	181.1	300.9		365.0		27.5	77.74	465.2		79.5	207.3	280.9	2054	2 2	) Dacc	0 10	7,59			
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TAE					<u> </u>	<u> </u>					<u> </u>										1
TABLE [推応] LOCA-DIRLEV.	TON	G-101 209,	102	103 206	104 205	201	706	167	801		H-101	702	200	\$ 5	3 4	90/	107	80/			•

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												, <u>-</u>				-			······	-	
(אד+כרר)	REMARKS	Y Divection	X Divertion	0, #0	Ditto	0:40	Y Direction														
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CONTAL LOAD CONDITIONS CONDITIONS HI +SI	\\Z	14	386.2	189.8	2.5.5 7.9.5	29.56	1,26.0							<u> </u>				:	•		
HORI ENT ARY +CH	) - <del> </del>	, 4		7		0.0	15.0									<u>.</u>			:		
CRANE HORIZ PERMANENT C TEMPORARY C	T + N + Cm+ X					i										<u> </u>			-		
H P P P P P P P P P P P P P P P P P P P	}  -  -	1 · N	0.986	304.0	290.2	249.1	199.1							<u>:</u>   <u>:</u>   :		<del></del>					
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AD LOAD				× 0		9,3	5 10	:	-			<u> </u>				<u>:</u>   <u>:</u>					
LOAD AD LOAD FING L	} +	20.62		•	<del> </del> -	30.0	54.9	:									\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \				
DEA)	3.1. + m5					7	1V: 3	:									<del> </del>				
VERTICAL LOAD - CRANE DEAD LOAI - CRANE LIFTING I	∑ ⊥	29.3	27.9	286.0	38.0	-46.7	-39:3					-				<u></u>	<del>                                   </del>	 	-		<u>:</u>
	1 17	1 : 3	1		1						:	<del>                                     </del>					<del> </del>		<del>                                     </del>		
VL CDE	Ş	<del>}</del>				: 0		:	:						: 			<u>:</u>   <u>:</u>	╁	:	
	5 5 z +	┧÷	4	4	) 0	40	4	:	<u>                                    </u>		-		<u>                                   </u>		<u> </u>						
NOTE +	1		1	<del>                                     </del>	1	4	<del> </del>	:	-	<u>                                     </u>	<u>                                   </u>		1				<u> </u>	<u>                                   </u>			
or 4	CDE			3 : 6	0.0		- 0	:										<u>                                   </u>		<u>                                   </u>	
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	-{  ∟	9: 1	786	303.6	287.8	X8.0	177			!	:	-	1:	:	1:			<u>                                     </u>	<del>                                     </del>		!
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	COCA-DIRLEY.	4-106	601	108	102	202	203					<u> </u>			•			• •	·		
	Ď.	15			12	~	~											· ·			

			<del>annela (O) de proprieta (o) p</del>	;				
	DECISION	OF COLUM	N BASE ( 1 )					
		面算定儿	i j zead n					
LOCAT			A-105 (101.104,1	05, 204, 205, 208)	A-103 /102, 106,20.	3,206,207)		
COLUM	N SIZE		BH-900 x 350 x 28	X40	BH-900 X 350 X 2	0.p.k.?		
DIREC	LION		X	Y	X	Y		
PERMA		M (tm)		0.5	0.2			
CONDI	TIONS	N (t)		119.0	211.8	<b></b>		
TEMPO	RARY	M (tm)		1.8.	10.3			
CONDI	TIONS	N (t) Q (t)	/33.9 3.5		-7.2 20.6			
		· · · · · · · · · · · · · · · · · · ·		0	same as A-	105		
			+	居	Dama wy W.			
FIGUR	FIGURE		RE		+	用コー		
•			≥∞0 900	Rate Es				
			1300					
BASE		(LxBxt)	1300 × 700	x.29		7@x29		
• • • • • • • • • <i>•</i> • • •	AP PLATE (THICK.)		*		: .≥≥		1	Z
RIB P		(HxBxt)	/6:					
	ING PLATE (HxBxt)		2 <u>Z</u>			2		
· · · <i>· ·</i> · · · · · · · ·	NCHOR BOLT $(n-D_{\phi})$ HEAR KEY $(H \times B \times t)$		8-309		8÷ 3οφ			
CONC.	e=M/N	(cm)	0.5x102 = 0.28 <	$\frac{130}{4} = 21.7$				
t +	σ c <fc< td=""><td>(kg/cm2)</td><td>179.0x103 (1+ 6x0.</td><td></td><td><math display="block">\frac{211,8\times10^{3}}{65\times130} = 25.1</math></td><td>&lt; 70</td></fc<>	(kg/cm2)	179.0x103 (1+ 6x0.		$\frac{211,8\times10^{3}}{65\times130} = 25.1$	< 70		
NCHOR	P/(n*A)<	ft	85 X130 \ 130		N/bbf=-0005/A			
BOLT	- , (,	(t/cm2)	•	-				
BASE	M=a wlx		0.09 x 0.022 × 32,5 2	= 2,1 T.CM	0.09 x 0.025 x 32.	t=130x65x14/2=5,5042		
LATE	t>√_6xM7	Tb (cm)	$\sqrt{6 \times 2.1 / 1.85} =$	2.61 -> 29	$\sqrt{6x2.4/1.85} = $	2,79 -12,9 20×14)> ×142 = 1.51		
AP	M=α wlx~	2 (tcm)			·			
LATE	t>√ 6xM/		· · · · · · · · · · · · · · · · · · ·		16×1.51/2.77=			
	$\tau = \sigma  \text{cA}/$	(txH) (t/cm2)	0.022×32,5×35 1.6×50	0.31 < 0.9	1.90 x 9.06 1.5 x 50	2.15 (1.35		
RIB	H/t		_50	4 5)				
LATE	WELDING		<u>-50</u> = 31.3					
	r < fs							
	$\tau = \sigma  cA/$	(txH) (t/cm2)	0.022×37,5×15.0=	0.15 < 0.9				
ING	H/t				1			
LATE			$\frac{50}{1.6} = 31.3$	< 71				
· .	WELDING	14/240)						
TTON	$\tau < fs$		·		<del> </del>			
BLE		fc(t/cm2)						
	PLATE :	ft(t/cm2) fb(t/cm2)						
מינים מיני		fs(t/cm2)				<del></del>		
REMAR		D + D	TEMORIT TET	CUP DITORII	(150x55-130x(5)x7×4			
NOTAT				ont, WIDIH,	THICKNESS,	DIAMETER (mm)		
		- ECCENTR		N//p*r.1 O	י מיוובף ארווזאי	TION DUE TO e		
			FORCE FOR C					
			IENT FOR BEN					
		0000000	TONT HOW DEAL	22 2 1 C 2 2 C 1 2 L 2 L 2 L 2 L 2 L 2 L 2 L 2 L 2 L 2				

0

' 			N BASE (2)					-
	「柱脚のじ	「面算定」	<del>,</del>					·
LOCAT			A-107, 202			A-108,201		
COLUM	N SIZE		BH-900×400 x 28	3X40		BH-900x40	4X 85K 0	D
DIRECT	<del></del>		Х	Y		X		Ϋ́
PERMAI		M (tm)	0.1			0.1		
CONDI	rions	N (t) Q (t)	25.4.7	*****************		214.3		
TEMPO	RARY	M (tm)	31.2			10.9		
CONDI	rions	N (t)	124.6	**************		2.4		
	· :	Q (t)	42.4			21. (	3	
			Same as G-108			Same a	6-106	8
			1.00		Ì			
FIGUR	Ξ			* .	].			
					1			•
	•			* ************************************		,		
BASE	PLATE	(LxBxt)		2× 2/-			w	₹€-
CAP P		(THICK.)	1300×700×35				0×700×	·····
RIB P		(HxBxt)	<b>,</b>		•••••	••••••	22	
WING		(HxBxt)	1b.				16	************
*****	R BOLT	$(n-D\phi)$				27 8-35 <del>9</del>		
SHEAR		(HxBxt)	8=35 <b>?</b>					
CONC.		(cm)		<del></del>	}-	<del></del>	<u> </u>	<del></del> -
Joho.	G137 M	(Cm)			1		,	
	o c <fc< td=""><td>(kg/cm2)</td><td>254.7×103 65×130</td><td>&lt; 70</td><td></td><td></td><td></td><td></td></fc<>	(kg/cm2)	254.7×103 65×130	< 70				
ANCHOR	P/(n*A)	(ft	N/bDf=00008 ) Pt=0			N/bDf = 0.002	1 182=0.020	do
30LT		(t/cm2)	M/602f=0002) at=1					58t/z=0.8cm
BASE	M=a wlx		0.09 × 0.03 × 32.5 ==	2,85		7 7 00 1 0000	12 12 10	micre oron
PLATE	t>√ 6xM7		16x2.85/1.85 = 3.	1>3-5				
CAP	M=α wlx		0.085x(2×1.8×8.0/20×14					
PLATE	t>/ 6xM/		V6x1.71 /2.77 = 1	92 -> 2.2	1			
	$\tau = \sigma  cA$		0.03 x325 x35 = 0.5				<del></del>	
1		(t/cm2)	1,2 ×50 =0.3	7 <0.9		Sp. 1		
RIB (	H/t		:					
PLATE			<b>.</b>					
	WELDING							
	τ < fs				l			
	$\tau = \sigma cA$	/(txH) (t/cm2)	0.03×37.5×17.5	0.18 <0.0	1	in the section		•
WING	н/t			· · · · · · · · · · · · · · · · · · ·				• • • • • • • • • • • • • • • • • • • •
PLATE	11/ C	1.7	}		1		-	
DAIL	WELDING							
	τ < fs							
		fc(t/cm2)	].		]			
ABLE	A.BOLT	ft(t/cm2)		*******	, . , }	*******		
STRESS		fb(t/cm2) fs(t/cm2)		************	•••••	***********	**************************************	********
REMAR	KS	ing the second s						
NOTAT	ION: L,H	B, t, D	LENGTH, HEI	GHT, WID	TH,	THICKNES	S, DIA	METER (mn
	Att. March 1997 April 1997	ECCENTR	to the second					•
			SSIVE STRESS				OITAU	N DUE TO
			FORCE FOR C				٠	-
	α		IENT FOR BEN					

,	DECISION OF COLU	AN BASE (, ? )		3113141	730
LOCAT	【柱脚の断而复定】	8-108 (101,201,2	28)	B-107 (102, 202, 20	7)
COLUMI	N SIZE	H-3002 X10 X 1		H-3502 x 12 x 15	
DIREC'		X	Ÿ	X	Y
PERMA			0.1	3.6	
CONDI:			90.7	143.5	
TEMPO	Q (t) RARY M (tm		0.1 5.4	11.9	
CONDI	17.7		100.6	60.9	
 	Q (t)		17-8	10.1	
FIGUR		200 200 200 7000		350 250 200 S	000
	PLATE (LxBxt)	700 x 350	7×2Z	750x4∞	X5.2
CAP P					
RIB P WING				.  <u></u> b.	
	R BOLT $(n-D\phi)$	4-25	) 	9-25	₽,
	e=M/N (cm)			1	
	σ c <fc (kg="" cm2)<="" td=""><td>90.7 ×10 3 = 37.0</td><td> ⟨७०</td><td></td><td></td></fc>	90.7 ×10 3 = 37.0	 ⟨७०		
ANCHOR	P/(n*A) <ft< td=""><td>N/bof = 0.027 \Pt</td><td>500</td><td><del> </del></td><td></td></ft<>	N/bof = 0.027 \Pt	500	<del> </del>	
BOLT	(t/cm2)	M/b0f = 0.0024)			
BASE	$M=\alpha$ wlx <sup>2</sup> (tcm)	0.085 x 0.037 x 77.52			
PLATE CAP	$t > \sqrt{6xM/fb}$ (cm) $M = \alpha$ wlx <sup>2</sup> (tcm)	16x0.49 /1.85	= 1.20 72.2		
PLATE	t> \( \int \text{6xM/fb} \text{(cm)}				
	$\tau = \sigma  cA/(txH)$	0.037 × 12.5 × 20	0.26		
RIB	H/t (t/cm2)	/s,2x 30	0150 <013		
PLATE	WELDING				
ļ	$\tau$ < fs (t/cm2)				
}	$\tau = \sigma  \text{cA/(txH)}$				
WING PLATE	(t/cm2)		• • • • • • • • • • • • • • • • • • • •		
E PUTE	WELDING				
<u>                                      </u>	τ < fs (t/cm2)				
1 1	CONC. :fc(t/cm2				
ABLE	A.BOLT :ft(t/cm2				
PTRESS	PLATE :fb(t/cm2 PLATE :fs(t/cm2				
REMAR	KS				
	ION: L,H,B,t,D		GHT, WIDTH,	THICKNESS, D	IAMETER (mm)
(	e ECCENTI		M//P*T\ A	פ אינואים פאנואים	דרוא חזוב שיין ב
1		SSIVE STRESS FORCE FOR C			TOW DOE TO S
1		CIENT FOR BEN		Fig. 5 is a second of the first of the contract of the contrac	
L				<del> </del>	

		DECISION	OF COLUM	N BASE (4)			1.0	4	
			[面算定]	H DROB (4-)					1
	LOCAT	ION	1.14 37 10 3	B-103 (106, 203, 20	06)		8-105 (104,20	4,205)	
	COLUMI	V SIZE		H-3002 X10X1			H-3002 x10.	.,.	
	DIREC			X X	, Y		X X	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	
	PERMAI		M (tm)	3.6			0.0		
	CONDI	rions	N (t) Q (t)	98.8 11.9			36.1		
-	TEMPO		M (tm)	3.0					
	CONDI	rions	N (t) Q (t)	-3.3					
			1 0 10	10.1	<u></u>		<del></del>		
					81		same as B	-108	1
	BICUDI	<b>.</b>	. ]	+ 7 =	<u>\$</u>	. [		٠.	
<i>(</i> )	FIGURI	L.	i	200 300 200	e,				•
( )	-	•		760		1	•		
		· · · · · · · · · · · · · · · · · · ·						<u> </u>	
	BASE		(LxBxt)	7¢ò x 351			7∞;	(350×22	
	CAP PI		(THICK.) (HxBxt)	(6. 12	************************************				
	WING	•••••••••	(HxBxt)	! <del>6.</del> .		••••••	<u></u>		
	ANCHO	R BOLT	(n-Dø )	4- :	25₽		• • • • • • • • • • • • • • • • • • • •	1~25·P	
	SHEAR		(HxBxt)						
	CONC.	e=M/N	(cm)	$\frac{3.6 \times 10^2}{98.8} = 3.6$	6 32			•	
		σ c <fc< td=""><td>(kg/cm2)</td><td>- 98.8×10<sup>3</sup> (1+ 6)</td><td>13.6) = 65.2</td><td>&lt; 70</td><td>•••••••</td><td></td><td></td></fc<>	(kg/cm2)	- 98.8×10 <sup>3</sup> (1+ 6)	13.6) = 65.2	< 70	•••••••		
		P/(n*A)<		N/OP = - a0007, Pe	=0131/0				
	BOLT		(t/cm2)	M/bort = 0.0019 )a.	1=10x35Pe/2=.	3.7 cm2	<del></del>		<u># </u>
	BASE PLATE	$M=\alpha Wlx^2$ t> $\sqrt{6xM}$		0.085×0.065×15	*= 1,24  >0    \  \  \  \  \  \				
	CAP	$M = \alpha \text{ wlx}^{2}$		0.085×(2×3.7×1.8/(12	2,5/1/X ((02X5)	0.71			
	PLATE	t>√ <u>6xM</u> /		J6x011/2177 =	1,24 -> 1,6				
		$\tau = \sigma \text{ cA}/$		0.065 x17.5 x15 = 0.	<i>ልባ</i> ረዕ	,,9			
	RIB	н/t	(t/cm2)	1.5 x 30			• • • • • • • • • • • • • • • • • • • •		
	PLATE	, .	· ·			1			
		WELDING							
	<b> </b>	<u>τ &lt; fs</u>			<del>_</del>				
		$\tau = \sigma \text{ cA}/$	(txn) (t/cm2)		• •	- (			
	WING	H/t	.1.57.9.112.7						••••••
	PLATE								
1		WELDING	(+ ( 0)		* •				
· ·	ALLOW-	τ < fs	fc(t/cm2)		<u></u>				
	1	A.BOLT	ft(t/cm2)				•••••••••••••••••••••••••••••••••••••••		
		PLATE :	fb(t/cm2)					,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
	DEMAR		fs(t/cm2)						
* * * * * * * * * * * * * * * * * * * *	REMARI		B.t.D	LENGTH, HEI	GHT WIT	HTC	THICKNESS	DIAMETER	R (mm)
		e	- ECCENTR	ICITY					
	ni chi a			SSIVE STRESS				ATION DU	Е ТО е
				FORCE FOR C					
	1 .	α	_ CAUSTIC	IENT FOR BEN	INTIAC MOU	TITILITY (	AT PLAR		

		· · · · · · · · · · · · · · · · · · ·	and the second s		Lanc	/32 Og	
	DECISION	Γονισοτημ	N BASE (5)			in the	
		<b>近面算定</b> ]	( C) dent n				
LOCAT	ION		D-101 . 208		D-108(102,107,	201,202,207)	
	N SIZE		H-3002 X10 X15	<u> </u>	H-3502 X 12 X	19	
DIREC			X	Y	X	Y	
PERMA		M (tm)	********	0.2		0,1	
CONDI	TIONS	N (t)	• • • • • • • • • • • • • • • • • • • •	89.8		198.5	
TEMPO	RARY	Q (t) M (tm)		9.1		0.1	
CONDI		N (t)		76.0		219.8	
ļ	·	Q (t)	30.4			33.5	
÷			Same as 8-108		Same as B-	107	
FIGUR	r						
LIGON	IGURE			•		•	
	PLATE	(LxBxt)	7.90.x	350×72	750.x44	0x25	
CAP P		(THICK.)			[ <del></del>		
RIB P		(HxBxt)		12	/6		
	NOTION PLATE (HxBxt)		4~	25. <del>P</del>	4-25Ф		
	HEAR KEY (HxBxt)			** ** *********************************			
CONC.	e=M/N	(cm)			Lett.		
	σ c≺fc	(kg/cm2)			$\frac{198.5 \times 10^3}{75 \times 40} = 60$	5,2 <70	
ANCHOR	P/(n*A)<	ft			15x40		
BOLT		(t/cm2)					
BASE	$M=\alpha$ wlx				0.085×0.066×17.		
PLATE CAP	t>√ 6xM/				J6x1.72/1.85 -	2.36 → 2.5	
PLATE	$M=\alpha Wlx^{2}$ t> $\sqrt{6xM}$						
	τ = σ cA/			<del></del>	0.068x17.5x20		
		(t/cm2)	***********		1.6 ¥ 30	- 0.48 <0.9	
RIB PLATE	H/t						
PLAIL	WELDING			*****************			
	$\tau < fs$	(t/cm2)	:				
	$\tau = \sigma  \text{CA}/$	(txH)		• •		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
	<u></u> .,,,	(t/cm2)	·				
WING	H/t						
PLATE	WELDING	•••••					
	τ < fs	(t/cm2)					
ALLOW-		fc(t/cm2)			formalis of the		
ABLE	A BOLT :	ft(t/cm2)	*******************				
STRESS		fb(t/cm2)	************	*******************			
REMAR		fs(t/cm2)	 	<del></del>	<u> </u>		
		B, t, D	LENGTH, HEI	GHT, WIDTH.	THICKNESS.	DIAMETER (mm)	
	e	- ECCENTR	ICITY		April 1985		
	σ <sub>C</sub> -	COMPRES	SSIVE STRESS	, N/(B*L) OR	OTHER EQUA	TION DUE TO e	
				OLUMN BASE (			
L	α	- COEFFIC	LENT FOR BEN	DING MOMENT	OF SLAR		

					211	EET /2 OF		
4						}		
1	DECISION	OF COLUM	N BASE (6)					
Į	[柱脚の断							
TOOAT		四字 15 1						
LOCAT	TOM	er jaron i	F-103 (101, 206 iz	ø8 )	F-108 (102,1	107,201,202,207)		
1					· ·			
COLUM	N SIZE		H-3002 X10 X1		H-3502 XI	> > 19		
DIREC			X	Y	X Y			
PERMA		N. Chan	A					
		M (tm)		3.6	<b></b>	9./		
CONDI	TIONS	N (t)	<b> </b>	104.8	[	189.9		
		Q (t)		11.9		5.0		
TEMPO	RARY	M (tm)	ಚಿ.ಂ			8.7		
CONDI	TIONS	N (t)	21.2	8.9 69.5		102 5		
100112	220113					182.5		
<u></u>		Q (t)	10.1	≥9.8	<b></b> .	29.1		
	•		same as B-1	n Q	Same as	B-107		
	4.0		200-5 W2 D-1	VO	JUME U.S	B 10 1		
			!		<b>}</b> .	•		
FIGUR	E	* .			<b>[</b> .			
1 ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~					<b>l</b> .	•		
	4	i i			<u>{</u>			
					Programme and the second			
BASE	PLATE	(LxBxt)	700×350	) X 2 7	7	50x400X25		
CAP P		(THICK.)	برد د ۲۰۰۸	1915 Marian	<u> </u>	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		
					<b> </b>			
RIB P		(HxBxt)	12	1 '. '		<u> </u>		
WING	PLATE	(HxBxt)	<del></del>	<del>-</del>				
	R BOLT	(n-Dø)	4-2	r-ib		4-254		
SHEAR				3.18				
J————		(HxBxt)			ļ			
роис.	e=M/N	(cm)	3.6×102	. 10	1	· -		
1			$\frac{3.6 \times 10^2}{104.8} = 3.4$	1 < 6				
	σ c <fc< td=""><td>(kg/cm2)</td><td></td><td></td><td></td><td></td></fc<>	(kg/cm2)						
		(0.9)	104.0x10 ×/1+ 0x	(3,4) = SS. 2 (70				
ANCHOR	P/(n*A)<	£ 4	70 X 35 ( 7		<del> </del>			
•	F/(II*A)							
BOLT		(t/cm2)						
BASE	M=α wlx^	2 (tcm)	0.085 x 0.055 x 12.5	$^{2} = 0.73$		The second second		
PLATE	t>./ 6xM/	Tb (cm)	V6x0.73/1.85	= 1.5 -> 22				
CAP	M=a wlx							
PLATE					l l			
PRIL	t>√ 6xM/				<del> </del>			
<u> </u>	r = o  cA/							
		(t/cm2)			L			
RIB	H/t			****************	1			
PLATE					]			
	MET DEMO	••••			ļ			
	WELDING		;					
1	τ < fs							
	$\tau = \sigma  cA/$	(txH)						
1:	]	(t/cm2)						
WING	U/+	117677774		******************	······			
1	H/t		· · ·	•	•	•		
PLATE					1			
	WELDING							
	τ < fs	(t/cm2)						
ATT OIL	CONC. :				{ <del>-</del>	<del></del>		
					]			
ABLE		ft(t/cm2)		· · · · · · · · · · · · · · · · · · ·	<b></b>			
STRESS	PLATE :	fb(t/cm2)						
	PLATE :		*****	**************				
REMAR		<del></del>		<del></del>	·  <del>-</del>			
	NOTATION: L, H, B, t, D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)							
MOTAT			The state of the s	oni, WIDIR,	THICKNES	S, DIAMETER (MM)		
	the state of the s	<ul><li>ECCENTR</li></ul>		international design of the second				
	oc COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e							
The part for a	P	- UP-LIFT	FORCE FOR C	COLUMN BASE	't)			
1 - (				DING MOMENT				
	u		THAT TOW DEL	DTMO BORENT	OR SUND			

	·				<del>, , , , , , , , , , , , , , , , , , , </del>		
	DECISION [柱 脚 の世			N BASE (7)			
LOCAT		1	<del></del>	F-104 (105,204,2	205)	F-203, 106	
COLUM	N SIZE	<del></del>	<u>`</u>	H-300 2 X 10 X 15		H-300 2 XID XIS	
DIREC'	<del></del>		. : : .	X X	Y	X X	Y
PERMA		М (	tm)		<del></del>	3.6	0-1
CONDI	TIONS	N (	t) t)	43.Z 0.0		?6.3 /1.9	9.3- <i>5</i>
TEMPO		M (	tm)				4.2 27.1 14.0
	·			Same as 8-108		Same as B	
DICUD	r						· · · · · · · · · · · · · · · · · · ·
FIGUR	ىز					:	
				·			
BASE	PLATE	(LxBxt	- )	700 x 3	57) x ?/	700 x 35	70 y 27
	BASE PLATE (LxBxt) CAP PLATE (THICK.				- -		WV # 2
1	RIB PLATE (HXBxt)			12			2
1	PLATE	(HxBx1			`		<del></del>
1	R BOLT	(n-Dφ		4-	<u></u>	4-	·25ф
	HEAR KEY (HxBxt)				8.5.5		
CONC.	e=M/N	(cm)					
	σ c <fc< td=""><td>(kg/cm</td><td>12)</td><td></td><td></td><td></td><td></td></fc<>	(kg/cm	12)				
ANCHOR	P/(n*A)<	ft					
BOLT		(t/cm2	?)		·		
BASE	M=a wlx	2 (tcm	n )				
PLATE	t>√ 6xM/						
CAP	$M=\alpha$ wlx			1	•		
PLATE	t>√ 6xM/		n )	. 			
	$\tau = \sigma CA/$	(txH) (t/cm2	2.)				'Agraph'
RIB PLATE	H/t						<i>2</i> 0 (2)
בנתנה	WELDING				****************		
<u></u>	τ < fs		:}				
	$\tau = \sigma \text{ cA}/$	-	. 1				•
WING PLATE	H/t	(t/cm2	'	· · · · · · · · · · · · · · · · · · ·			
	WELDING		•••••		*****************		
	$\tau$ < fs			<u>:</u>			
ALLOW- ABLE	A.BOLT		m2)				
STRESS	PLATE						
REMAR		fs(t/c	-m2)				
		B.t.D		LENGTH HET	GHT. WIDTH	, THICKNESS, I	JAMETER /mm\
HOTAL		- ECCE				, manufathor, r	
					, N/(B*L)	OR OTHER EQUAT	ION DUE TO e
				FORCE FOR CO			
				IENT FOR BENI			San
	<u></u>						

		<del></del>						
,				•				
			N BASE (8)					
	[柱脚の町	[直算定]						
LOCAT	TON		G-101, 208		A-103 (102, 105, 204, 204,	206.207)		
COLUM	N SIZE		BH~900x350 x28	X & U	5 x 078 x009-HB			
DIREC	TION		X.Y	Y	X	Y		
PERMA		M (tm)	4.8		9.5			
CONDI	TIONS	N (t) Q (t)	19b.2 9-6		386.4 -15.0			
TEMPO	RARY	M (tm)	35.1	30.2	16.0	5.6		
CONDI	TIONS	N (t) Q (t)	274,4 70.1	99.9 60.3	450.0 339.0 32.0 11-1			
		·	Same as A-1	·		0		
		$\mathcal{P}^{\prime}$	Same No M		# 7	ESOS		
FIGUR	E	A Section 1	·		1 1	\$1.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50 \$2.50		
					100 900	200 8		
					1300	<u> </u>		
	PLATE	(LxBxt)	13co×.7		1300×700×	. 45		
CAP P		(THICK.)		Σ	≥z			
RIB P		(HxBxt)		'b	[			
WING		(HxBxt)		2	>2			
	R BOLT	$(n-D\varphi)$	8. <del>-</del>	3e\$	8-30\$			
SHEAR		(HxBxt)						
CONC.	e=M/N	(cm)				< 20		
	σc≺fc	(kg/cm2)		•••••		$\frac{\times 1.94}{20}$ = 49.5 <70		
	P/(n*A)<		N/bbt = 0.017 } Pt	=0.0%		_		
BOLT		(t/cm2)	M/bD7 = 0.0016)					
BASE	M=α wlx				0.085×0.0495×35			
PLATE	t>√6xM/			<del> </del>	16x5.15/117 °	: 4,26 → 45		
CAP	$M=\alpha Wlx^{-1}$							
PLATE	t>√ 6xM/		<del></del>					
	$\tau = o \text{ cA}/$				0.0495x35x35 =	0.76 (0.9		
RIB	H/t	(t/cm2)		******	1:6 X 50			
PLATE	11/ (		1		-50 = 31.	3 <71		
	WELDING		}					
	$\tau$ < fs	(t/cm2)						
	$\tau = \sigma  cA/$				0.0495437.5 x17.5 1,2 X SO			
[	<b></b>	(t/cm2)			1,2 X S0	0.54 (1.7		
WING PLATE	H/t				<u> 50</u> = 41.7			
	WELDING							
	I .	(t/cm2)						
ALLOW-		fc(t/cm2)			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			
ABLE	A BOTT	ft(t/cm2)		.,				
1 1	PLATE :	fb(t/cm2)						
	PLATE :	fs(t/cm2)						
	REMARKS							
NOTAT	NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)							
e ECCENTRICITY  oc COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e								
P UP-LIFT FORCE FOR COLUMN BASE (t)								
L				IDING MOMENT		· · · · · · · · · · · · · · · · · · ·		
			***					

(<u>`</u>)

	<u> </u>	<del></del>	<u> </u>		, radiic	
	DECISION 【柱 脚 の D	OF COLUM 前面質点 1	n base (9)			
LOCAT	ION	1 101 24 15 1	G-104,205		G-108 (107, 201,	202)
COLUM	N SIZE		BH-900x350 x28	Y Q D	BH-900 x400 XZ	8290
DIREC	TION		X	Y	X	Y
PERMA		M (tm)		0.0	15,2	
CONDI	TIONS	N (t) Q (t)		378.9	481-7 30.3	
TEMPO	RARY	M (tm)	<u>                                     </u>	0.2	37.3 7.6	
CONDI	TIONS	N (t)		46 <i>4</i> ,7	5\$?.1 74.5	438.9 15.2
10.00			Same as G-11			8
			June 30 9-71			E 88 8
FIGUR	E					国 4 7
					200 q00 2	₩ 8 ×
BASE CAP P	PLATE	(LxBxt)	1300 x 7		1300 x 700 x 5	0
RIB P		(THICK.)	{ <u>}≥</u>		2 <u>2</u>	
	PLATE	(HxBxt)			ļ/6	
	R BOLT	$(n-D\phi)$	<u>408-6</u>		22 8-35 <del>P</del>	
SHEAR		(HxBxt)	0.304			
CONC.	e=M/N	(cm)			15.2 ×10 <sup>2</sup> 481.7 = 3.16	< 70 6
	σ c <fc< td=""><td>(kg/cm2)</td><td colspan="2"></td><td>481.7 × 103 × (1+ 6x3</td><td></td></fc<>	(kg/cm2)			481.7 × 103 × (1+ 6x3	
ANCHOR BOLT	P/(n*A)<	ft (t/cm2)			-	-
BASE	M=a wlx		<u></u>	<del></del>	0.085 × 0.067 × 35 ===	2.0
PLATE	t>√ 6xM7		:		16x20/1,70 =	
CAP	M=a wlx			· · · · · · · · · · · · · · · · · · ·		
PLATE	t>√ 6xM/	fb (cm)				
	τ = σ cA/	(txH) (t/cm2)			$\frac{0.067x35x35}{2.2\times50}=0.$	75 (0.9
RIB	H/t					
PLATE	WELDING	************		·····	$\frac{35}{2.2} = 15.4$	<16
	$\tau < fs$	(t/cm2)	;			
	$\tau = \sigma  cA/$	(txH)	1		0.067x15x37.5	
		(t/cm2)			1.2 x 50	
WING PLATE	H/t				$\frac{-15}{1.2} = 12.5$	<16
	WELDING					
ATT OUT	τ < fs					
ABLE PPLOM-	CONC. :	fc(t/cm2) ft(t/cm2)	***************************************			
, ,	PLATE :		***************************************			
<u> </u>	PLATE :	fs(t/cm2)				44-14
REMARI		<u> </u>	T TING MET TIME	CITIES TATEMAT	WILLIAM SEC. 2	TAMPORED ()
MOTAT.		B, C, D		GHT, WIDTH,	THICKNESS, D	TAMETER (MM)
]				, N/(B*L) OF	OTHER EQUAT	ION DUE TO e
	P	- UP-LIFT	FORCE FOR C	OLUMN BASE	(t)	
<u> </u>	α	- COEFFIC	IENT FOR BEN	DING MOMENT	OF SLAB	

	DECISION	OF COLUM	N BASE (10)							
<b>-</b>	【柱脚の間	[面算定]	_ (()							
LOCAT	ION		H-101, 208		H-102, 207					
COLUM	NSIZE		H-3002 × 10 × 15	,	H-4002 x13x2	<del>, ,</del>				
DIREC			* `	Y	X ¥×					
PERMAI		M (tm)	2.9	*********	0.0					
CONDI	rions	N (t)	81.2 9.7	***************	209.0					
TEMPO		M (tm)	16.0		6.2 6.2					
CONDI	rions	N (t) Q (t)	205.3 53.4	-44.8	333.8	82.0 20.6				
<del> </del>		1 6 7 7 7	33.4	<del>43.7</del>	20.6	20.0				
			8	<del></del>		1				
			1 1 / 直 🖺 8	8	1 II 8	8 8				
FIGUR	E		#N E #	1 1		47 .1				
		eran di sa	260 300 209	<b>←</b>	150 400 150	8				
			700		700	<b>~</b> )				
BASE	PLATE	(LxBxt)	700 x 600	x 22	700 x	700 x 2.5				
CAP P		(THICK.)	19							
RIB P	LATE	(HxBxt)	16		(	9				
WING		(HxBxt)	lb.			9				
ANCHO	R BOLT	(n-Dφ ) (HxBxt)	8-30	£	8-254					
	e=M/N	(cm)	16.0×102	10						
			$\frac{16.0 \times 10^2}{205.3} = 17.8$							
	σ c <fc< td=""><td>(kg/cm2)</td><td>205,3×103×(1+ 6×1</td><td>1.8)=81.6 &lt;140</td><td><math display="block">\frac{209.0 \times 10^3}{90 \times 90} = 4</math></td><td>2.7 &lt;70</td></fc<>	(kg/cm2)	205,3×103×(1+ 6×1	1.8)=81.6 <140	$\frac{209.0 \times 10^3}{90 \times 90} = 4$	2.7 <70				
ANCHOR	P/(n*A)<	<b>f</b> <del>f</del>	N/60 f=0.0059 R=1	740	70 2 70					
BOLT	E/(II.A)	(t/cm2)	M/602+= 6,0025/A+=1		r exit					
BASE	M=α wlx^		0.085 40.082 x 12.5		0.085 × 0.043 × 202 = 1.46					
PLATE	t>./ 6xM/		16x1.09/2.77 =1.	5 -> 19.	√6×1.46 / 1.85 = 2.18 → 2.5					
CAP	$M=\alpha$ wlx	2 (tcm)	0.085x (2x6,3x1,8/17	2×50)×1512 + 1'50	V 6 XI, TO / 1.83 - 2.10 - 2.3					
PLATE	t>√ 6xM/		16x 42/2,77 = 1							
	$\tau = \sigma  cA/$		0.087×20 ×12.5	0.57 (1.35	0.043×20×35 - 0.53 (0.9					
RIB	H/t	(t/cm2)	1.2 x 30		1.9×30					
PLATE	11/ 6		i.		1.					
	WELDING									
	τ < fs	(t/cm2)								
	$\tau = \sigma  cA/$		- 1.5×21×230.0	- 04 - 135	0,043x12,5x25,=					
		(t/cm2)	1,2 × 30	0.94 <1.35	1.4 × 30	0.24 < 0.9				
WING	H/t				•					
PLATE	Expert to the second	•				• • • • • • • • • • • • • • • • • • • •				
	WELDING	/+/~mn1				s in the second				
A T. T. OW	$\tau < fs$	fc(t/cm2)								
ABLE	A BOLT	ft(t/cm2)								
		fb(t/cm2)	••••••••							
		fs(t/cm2)								
REMARKS										
NOTAT	NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)									
I Partie	e ECCENTRICITY									
	oc COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e P UP-LIFT FORCE FOR COLUMN BASE (t)									
1										
L	<u>α</u>	COERFIC	IENT FOR BEN	IDING MOMENT	OL PLAR					

	DECISION	OF COLUM	N BASE (1)	•			
LOCAT	17 脚 (7) 围	[面算定]	111 102 (1 50 200	206)	1112		
HOOM	TON	11	H-103 (105, 204,	200)	H-203 (104, 106	1 205)	
COLUM	N SIZE		H-4502 X/6 x 2		H-516×500 XZZ	×40	
DIREC			X	¥Χ	* Y	Y	
PERMA		M (tm)	0.0	T	4.5		
CONDI	TIONS	N (t) Q (t)	≥84.4 0-0	*****************	37.5.3 /5.0		
TEMPO	RARY	M (tm)	6.2	6.2	20.9	11.9	
CONDI		N (t)	408.5 155.6 20.6 20.6		499.1 247.4 69.8 39.8		
<u> </u>		1		<u> </u>	, VIII	<u> </u>	
FIGUR	FIGURE		140 450 1150 BY		200 516 200 916		
BASE	PLATE	(LxBxt)	750 X	7.00 × 3.0	91b x 7	0×40	
CAP P		(THICK.)		/:sv.,n =:v.,		w.n	
RIB P	LATE	(HxBxt)	1	9			
	PLATE	(HxBxt)	1	9	5.5		
1	R BOLT	$(n-D\phi)$	8:	-259	~8	25.\$	
SHEAR CONC.		(HxBxt) (cm)	<u>                                     </u>		4F v 102		
LONG.					$\frac{4.5 \times 10^2}{375.3} = 1.2$		
ļ	σ c <fc< td=""><td>(kg/cm2)</td><td colspan="2"></td><td>375,3 × 103 × (1+ 62</td><td>112 = 63.1 &lt;70</td></fc<>	(kg/cm2)			375,3 × 103 × (1+ 62	112 = 63.1 <70	
	P/(n*A)<						
BOLT	M=a wlx	(t/cm2)			0.085×0.063×25.	6°2 - 3 b-h	
PLATE	t>√6xM7				16x3.56 /1.85 = 0		
CAP	M=α wlx^				1 4 4 75 - 20 1 1 1 0 3 - 5	5.70 - 7 4.0	
PLATE	t>√ 6xM/			*			
	$\tau = \sigma  cA/$				0.063×25,8×35		
		(t/cm2)		• • • • • • • • • • • • • • • • • • • •	05 x 22	= 0.52 (0.9	
RIB	H/t				:		
PLATE							
	WELDING	· · · · · · · · · · · · · · · · · · ·					
	$\tau < fs$		· · · · · · · · · · · · · · · · · · ·				
}	τ =σ CA/	(t/cm2)			0.06) x12,5 x32.9	=0.23 20.9	
WING	H/t	( ) 0     2	•		2,2× 50		
PLATE	WELDING	• • • • • • • • • • • • • • • • • • • •				······	
	$\tau$ < fs	(t/cm2)					
ALLOW-		fc(t/cm2)			<del> </del>		
ABLE	A. BOT.T	ft(t/cm2)				,	
	PLATE :	fb(t/cm2)			1		
		fs(t/cm2)	:	••••••••••••••••••••••••••••••••••••••			
REMAR	KS		-				
NOTAT		The state of the s		GHT, WIDTH,	THICKNESS, I	DIAMETER (mm)	
1		- ECCENTR		A DA	gorgania (1980-1981) Berginda (1980-1981)		
]						CION DUE TO e	
}			FORCE FOR C				
L	α	- COFF.F.T.C	IENT FOR BEN	DING MOMENT	OF SLAB		

[柱脚の断面算定] LOCATION				H-108 (107, 201	, 202)	K-106, 203	K-106, 203	
COLUMN SIZE			H-516x500 x22	X40	H-3502 X	H-3502 XIZ X19		
DIRECTION			X	₹Χ	* Y			
PERMAI		М	(tm)	15.2		4,5		
CONDI	LIONS	N = N	( <u>t</u> )	599.5	.,,,,			
EMPO	RARY	Q M	(t) (tm)	50.B 34.5	4.0	15.0 21.4		.5
	TIONS	N	(t)	8/4.5	372.7	272.7		
. <u></u>		Q	(t)	114.9	13-3	71.4	******	3.4
			٠.		- <del>                                     </del>	Same as	8-107	
	. • •			+	1 00 = 00 = 00 = 00 = 00 = 00 = 00 = 00	ì		
IGUR	E			+ + +	4			
					- <u>E</u>			
•				200   516   2005 110	43		•	
ASE	PLATE	(LxBx	<del>t</del> )	9162110	 ก v ⊰ 8	72	0x 400 X25	
	LATE	(THIC			V.V.X.V			• • • • • • • • • • • • • • • • • • • •
IB P	LATE	(нхвх		22.			16 Land Control	
	PLATE	(HxBx		22				,,
	R BOLT	$\{n-D\phi\}$		8-2	₹		4-25\$	
HEAR		(HxBx		2.2	<del></del>			
) INC.	e=M/N	(cm)		$\frac{15.2 \times 10^2}{594.5} = 2.$	56 < 110			
	σ c≺fc	(kg/c	m2)		2.56)= 67,2 < 7	0		******
CHOR	P/(n*A)<	ft	<del></del>	41-6 × 110 11 1	10 / - 111 - < 7	N/60f= 0.004	- DL-V500	
LT	17 (11 12)	(t/cm	2)			M/hp+ = 0.002	8/21=75×40Pt/2	= 3144)
SE	M=a wlx^			0.085 x 0.067 x 280.0	B.8 = 1	72,733	<u> </u>	
ATE	t>,/ 6xM/			16x3.8/1.85	≈ 3,5 → 3.8			····
P	$M=\alpha Wlx^{\gamma}$			to the second				
ATE	$t > \sqrt{6xM}/$ $\tau = \sigma cA/$		m )	-10 25 C. FF				
·	L -O CM	(t/cm	2)	22 x 50	= 0.86 < 0.9			
В	H/t	. 3 . 22	7.5		*********			
ATE				· · · · · · · · · · · · · · · · · · ·			· · · · · · · · · · · · · · · · · · ·	
1	WELDING							
	τ < fs				<del> </del>	<del></del>	<del></del>	
	$\tau = \sigma \text{ cA}/$	(t/cm		6.067.x 30x 32.9	= 0.6 < 5.9			
NG	H/t	3.52.54	÷./	2.2 X 50				
ATE				· 1.1				
	WELDING							
	τ < fs			<u> </u>				
LOW-	CONC.							
	A.BOLT :				,,			
ادرويي		fs( <u>t</u> /			***************************************	10 100		
EMAR	KS							
OTAT				LENGTH, HEI	GHT, WIDTH	, THICKNESS	, DIAMETE	R (m
				ICITY SSIVE STRESS	N //D*T 1	ህል ህወይል ልህ	יים זוא שראו	ድ ም스
A 1-				FORCE FOR C			OWITON DO	E IO
1				IENT FOR BEN			•	
				**				

 $(\cdot)$ 

DECISION OF COLUMN BASE (15)	•							
DOCATION				N BASE (13)				
DIRECTION	LOCAT:	ION		K-107, 202		K-108, 201		
DIRRECTION   X   \$\frac{\pmathcal{X}}{\pmathcal{X}} \   X   Y   PERMANENT   M   (tm)   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00   0.00	COLUMN SIZE			H-4502 X16X	52	H-4502 X16 X25		
CONDITIONS				Х	大学		Ä	
TEMPORARY			N (t)	≥88.5				
CONDITIONS	TEMPO	RARY			6.2	7.0		
BASE PLATE (LxBxt)	CONDI'	TIONS	N (t)	5.886	190.2	■ Pr		
BASE PLATE		: .		Same as H-1	03	Same as H-10	3	
CAP PLATE (THICK)  RIB PLATE (HXBXt) 19 19  WING PLATE (HXBXt) 19 19  ANCHOR BOLT (n-D.6) 8-25.\$ 5-25.\$  SHEAR KEY (HXBXt)  CONC. e=M/N (cm)  G c <fc (kg="" (n*a)<ft="" (t="" (tcm)="" anchor="" base="" bolt="" cm2)="" m="a" p="" plate="" t="" wlx^2="">/ 6xM/FD (cm)  CAP M=a wlx^2 (tcm)  PLATE t&gt;/ 6xM/FD (cm)  CAP M=a wlx^2 (tcm)  PLATE to fix (t/cm2)  CAP M=a wlx^2 (tcm)  CAP M=a w</fc>	FIGUR	E	!					,
CAP PLATE			·					( )
CAP PLATE	:				.1.	grant to		
RIB PLATE				750×7	00×30	759×.7.	00 x 30	
WING PLATE								
ANCHOR BOLT								
SHEAR KEY						ا	@_ >F\$	
CONC. e=M/N (cm)  σ c <fc (kg="" (n*a)<ft="" (t="" anchor="" base<="" cm2)="" p="" td=""><td></td><td></td><td></td><td>Ω</td><td>&lt;&gt;.γ</td><td></td><td>5.3l</td><td></td></fc>				Ω	<>.γ		5.3l	
ANCHOR P/(n*A) <ft (t="" (tcm)="" 2="" 3olt="" base="" cm2)="" m="a" plate="" t="" wlx=""> \( \sigma \) 6XM/FD (cm)  PLATE t&gt; \( \sigma \) 6XM/FD (cm)  \[ \text{T} \times \circ \ci</ft>								
ANCHOR P/(n*A) <ft (t="" (tcm)="" base="" bolt="" cm2)="" m="q" plate="" t="" wlx^2="">√ 5xM/f5 (cm)  CAP M=q wlx^2 (tcm) PLATE t&gt;√ 5xM/f5 (cm)   \[ \begin{align*} \text{CAP} &amp; \text{M=q wlx^2 (tcm)} &amp; \text{(bx25/1.85 \circ 2.85 \rightarrow 3.0} \\  \text{CAP} &amp; \text{M=q wlx^2 (tcm)} &amp; \text{(bx25/1.85 \circ 2.85 \rightarrow 3.0} \\  \text{PLATE} &amp; \text{(t/cm2)} &amp; \text{(t/cm2)} \\  \text{RIB} &amp; \text{H/t} &amp; \text{(t/cm2)} \\  \text{V=\sigma cA/(txH)} &amp; \text{V=\sigma cA/(txH)} \\  \text{V=\sigma cA/(txH)} &amp; V</ft>	-	ø c <fc< td=""><td>(kg/cm2)</td><td></td><td></td><td><math display="block">\frac{304 \times 10^3}{76 \times 70} = 57.</math></td><td>9 &lt; 70.0</td><td></td></fc<>	(kg/cm2)			$\frac{304 \times 10^3}{76 \times 70} = 57.$	9 < 70.0	
### BASE   M=\alpha wlx^2 (tcm)								
ELATE						55 X R R D , O X 280.0	<u>'2, = 5'20</u>	
CAP						·		
T = G CA/(txH)	CAP							
RIB	PLATE							1
RIB PLATE  WELDING  \$\tau < fs (t/cm2)\$  \$\tau = \sigma cA/(txH)\$  \$\tau (t/cm2)\$  WING  H/t  PLATE  WELDING  \$\tau < fs (t/cm2)\$  ALLOW-CONC. ':fc(t/cm2)  ABLE A.BOLT :ft(t/cm2)  BTRESS PLATE :fb(t/cm2)  PLATE :fb(t/cm2)  PLATE :fs(t/cm2)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  \$\text{e} ECCENTRICITY  \$\sigma c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e		τ ≃σ cA/		·		0.058x22.5x35	0.8 (0.9	(_)
WELDING  \( \tau < fs \ (t/cm2) \)  \( \tau = \sigma cA/(txH) \) \( (t/cm2) \)  WING  H/t  PLATE  WELDING  \( \tau < fs \ (t/cm2) \)  ALLOW- CONC. \( :fc(t/cm2) \)  ABLE A.BOLT \( :ft(t/cm2) \)  STRESS PLATE \( :fb(t/cm2) \)  PLATE \( :fs(t/cm2) \)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  \( e \text{ ECCENTRICITY} \) \( \sigma c  COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO \)  \( e \)		H/t			· · · · · · · · · · · · · · · · · · ·			
T = G CA/(txH)		WELDING						
WING H/t  PLATE  WELDING				<u> </u>				
WING H/t  PLATE  WELDING	[	$\tau = \sigma  cA/$			• •	0.058x 12.5x 26.3	= 0.33 /0.9	
### To the content of		ਮ/t	(t/cm2)					
ALLOW- CONC. :fc(t/cm2)  ABLE A.BOLT :ft(t/cm2)  STRESS PLATE :fb(t/cm2)  PLATE :fs(t/cm2)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e	1 1	l .			.,,			:
ABLE A.BOLT :ft(t/cm2)  STRESS PLATE :fb(t/cm2)  PLATE :fs(t/cm2)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e	L T T CO.			·				
REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e	1 1	,					•••••	!
PLATE :fs(t/cm2)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e	t 1			{	••••••			•
REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e	211123							1
NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm) e ECCENTRICITY o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e		KS						`` ``
σ c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e			B, t, D	LENGTH, HE	GHT, WIDTH,	, THICKNESS, I	DIAMETER (mm)	*
	{							1.7.5
TO DEED LONG CONDING DECEM (C)							TION DUE TO e	
α COEFFICIENT FOR BENDING MOMENT OF SLAB								