

II. Design of Main Building

1. Calculation of Main Building.

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NOTE : UW --- UNIT WEIGHT [單位荷重]

PA --- FOR PERMANENT CONDITION [長期]

SE --- FOR SEISMIC CONDITION [地震時]

ML --- MACHINE LOAD [機械荷重]

1.1 COLUMN AXIAL LOAD (1)
[柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
A-101	ROOF	SLAB	$A=11.6 \times 4.0 =$	46.4	0.595	0.535	27.6	24.8	
	FL+27.1	PARAPET	$L=11.6+4.0 =$	15.6	0.12		1.9	1.9	
		SIDING	$A=(15.6 \times (1.8+0.9)) + 6.85 \times 4.3 =$	71.6	0.095		3.2	3.2	
	FL+21.0	COLUMN	$L =$	6.1	0.2		1.2	1.2	
		TRUSS	$L =$	11.0	0.25		2.8	2.8	
							36.7	33.9	
	FL+21.0	SIDING	$A=6.85 \times 10.0 =$	68.5	0.095		3.1	3.1	
		FL+11.0	COLUMN	$L =$	10.0	0.25		2.5	2.5
			CRANE GIRDER	$L =$	4.0	0.35		1.4	1.4
							7.0	7.0	
							(43.7)	(40.9)	
	FL+11.0	SLAB	$A=2.85 \times 4.0 =$	11.4	2.225	0.8	25.4	9.1	
COLUMN		$L =$	5.5	0.25		1.4	1.4		
						26.8	10.5		
						(70.5)	(51.4)		
FL+5.5	SLAB	$A =$	11.4	1.225	1.15	14.0	13.1		
	FL±0	P.C. PANEL	$A=6.85 \times 5.5 =$	37.7	0.29		10.9	10.9	
		COLUMN	$L =$	5.5	0.25		1.4	1.4	
						26.3	25.4		
						(96.8)	(76.8)		
A-102	ROOF	SLAB	$A=11.6 \times 6.8 =$	78.9	0.595	0.535	46.9	42.2	
	FL+27.1	PARAPET	$L =$	6.8	0.12		0.8	0.8	
		SIDING	$A=6.8 \times (6.1+0.9) =$	47.6	0.045		2.1	2.1	
	FL+21.0	COLUMN	$L =$	6.1	0.2		1.2	1.2	
		TRUSS	$L =$	11.0	0.25		2.8	2.8	
							53.8	49.1	
	FL+21.0	SIDING	$A=6.8 \times 10.0 =$	68.0	0.095		3.1	3.1	
		FL+11.0	COLUMN	$L =$	10.0	0.25		2.5	2.5
			CRANE GIRDER	$L =$	6.8	0.35		2.4	2.4
							8.0	8.0	
							(61.8)	(57.1)	
	FL+11.0	SLAB	$A=2.85 \times 6.8 =$	19.4	2.225	0.8	43.2	15.5	
COLUMN		$L =$	5.5	0.25		1.4	1.4		
						44.6	16.9		
						(106.4)	(79.0)		
FL+5.5	SLAB	$A =$	19.4	1.225	1.15	23.8	22.3		
	FL±0	P.C. PANEL	$A=6.8 \times 5.5 =$	37.4	0.29		10.8	10.8	
		COLUMN	$L =$	5.5	0.25		1.4	1.4	
						36.0	34.5		
						(142.4)	(108.5)		

NOTE : UW --- UNIT WEIGHT [單位荷重]								
COLUMN AXIAL LOAD (2) [柱軸力]								
PA --- FOR PERMANENT CONDITION [長期]								
SE --- FOR SEISMIC CONDITION [地震時]								
ML --- MACHINE LOAD [機械荷重]								
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
A-103	FL+27.1 S							
	FL+21.0		SAME AS A-102				53.8	49.1
	FL+21.0 S							
	FL+11.0		SAME AS A-102				8.0 (61.8)	8.0 (57.1)
	FL+11.0 S							
	FL+5.5		SAME AS A-102				44.6 (106.4)	16.9 (74.0)
	FL+5.5 S FL±0		SAME AS A-102				36.0 (142.4)	34.5 (108.5)
A-104	FL+27.1 S		SAME AS A-102				53.8	49.1
	FL+21.0							
	FL+21.0 S		SAME AS A-102				8.0 (61.8)	8.0 (57.1)
	FL+11.0	SLAB	A=2.85×3.4=	9.7	2.225	0.8	21.6	7.8
	FL+5.5	GRATING	A=	9.7	0.325	0.225	3.2	2.2
		COLUMN	L=	5.5	0.25		1.4	1.4
							26.2	11.9
							(88.0)	(68.5)
	FL+5.5	GRATING	A=2.85×6.8=	19.4	0.325	0.225	6.3	4.4
	FL±0	P.C. PANEL COLUMN	A=6.8×5.5=	37.4	0.29		10.8	10.8
		L=	5.5	0.25		1.4	1.4	
						18.5 (106.5)	16.6 (85.1)	
A-105	FL+27.1 S		SAME AS A-102				53.8	49.1
	FL+21.0							
	FL+21.0 S		SAME AS A-102				8.0 (61.8)	8.0 (57.1)
	FL+11.0							
	FL+11.0 S	SLAB	A=2.85×3.4=	9.7	2.225	0.8	21.6	7.8
	FL+5.5	GRATING	A=	9.7	0.325	0.225	3.2	2.2
		EXC. CUBICLE COLUMN	4×4.0				1.0	1.0
			L=	5.5	0.25		1.4	1.4
							27.2	12.9
							(89.0)	(69.5)
FL+5.5 S FL±0		SAME AS A-104				18.5 (107.5)	16.6 (86.1)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (3) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
A-106	FL+27.1 5		SAME AS A-102				53.8	49.1
	FL+21.0							
	FL+21.0 5		SAME AS A-102				8.0	8.0
	FL+11.0						(61.8)	(57.1)
	FL+11.0 5	SLAB	$A=2.85 \times 6.8 =$	19.4	2.225	0.8	43.2	15.5
	FL+5.5	EXC. CUBICLE	$1/4 \times 4.0$				1.0	1.0
		COLUMN	$L =$	5.5	0.25		1.4	1.4
							45.6	17.9
							(107.4)	(75.0)
	FL+5.5 5	SLAB	$A =$	19.4	2.225	1.15	23.8	22.3
FL±0	IPB	$L = 2.3 \times 2 =$	6.9	1.0		6.9	6.9	
	P.C. PANEL	$A = 6.8 \times 5.5 =$	37.4	0.29		10.8	10.8	
	COLUMN	$L =$	5.5	0.25		1.4	1.4	
						42.9	41.4	
						(150.3)	(116.4)	
A-107	FL+27.1 5	SLAB	$A = 11.6 \times 8.4 =$	97.4	0.595	0.535	58.0	52.1
	FL+21.0	PARAPET	$L =$	8.4	0.12		1.0	1.0
		SIDING	$A = 8.4 \times (6.1 + 0.9) =$	58.8	0.045		2.6	2.6
		COLUMN	$L =$	6.1	0.2		1.2	1.2
			TRUSS	$L =$	11.0	0.25		2.8
							65.6	59.7
	FL+21.0 5	SIDING	$A = 8.4 \times 10.0 =$	84.0	0.095		3.8	3.8
	FL+11.0	COLUMN	$L =$	10.0	0.25		2.5	2.5
		CRANE GIRDER	$L =$	8.4	0.35		2.9	2.9
							9.2	9.2
						(74.8)	(68.9)	
FL+11.0 5	SLAB	$A = 2.85 \times 8.4 =$	23.9	2.225	0.8	53.2	19.1	
FL+5.5	COLUMN	$L =$	5.5	0.25		1.4	1.4	
						54.6	20.5	
						(129.4)	(89.4)	
FL+5.5 5	SLAB	$A =$	23.9	2.225	1.15	29.3	27.5	
FL±0	P.C. PANEL	$A = 8.4 \times 5.5 =$	46.2	0.29		13.4	13.4	
	COLUMN	$L =$	5.5	0.25		1.4	1.4	
						44.1	42.3	
						(173.5)	(131.7)	

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 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (4)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
A-108	FL+27.1 s FL+21.0	SLAB	$A=11.6 \times 10.0 =$	116.0	0.595	0.535	69.0	62.1
		PARAPET	$L =$	10.0	0.12		1.2	1.2
		SIDING	$A=10.0 \times (6.1+0.9) =$	70.0	0.095		3.2	3.2
		COLUMN	$L =$	6.1	0.2		1.2	1.2
		TRUSS	$L =$	11.0	0.25		2.8	2.8
							77.4	70.5
	FL+21.0 s FL+11.0	SIDING	$A=10.0 \times 10.0 =$	100.0	0.045		4.5	4.5
		COLUMN	$L =$	10.0	0.25		2.5	2.5
		CRANE GIRDER	$L =$	10.0	0.35		3.5	3.5
							10.5	10.5
							(87.9)	(81.0)
	FL+11.0 s FL+5.5	SLAB	$A=2.85 \times 5.5 =$	15.7	1.925	1.35	22.9	21.2
HAND RAIL		$L =$	2.85	0.035		0.1	0.1	
COLUMN		$L =$	5.5	0.25		1.4	1.4	
SHUTTER		$A=5.0 \times 10.0 =$	50.0	0.1		5.0	5.0	
							28.9	27.7
						(116.8)	(108.7)	
FL+5.5 s FL±0	STAIR	$A =$	15.7	0.43	0.36	6.8	5.7	
	HAND RAIL	$L =$	2.85	0.035		0.1	0.1	
	COLUMN	$L =$	5.5	0.25		1.4	1.4	
	P.C. PANEL	$A=5.0 \times 5.5 =$	27.5	0.29		8.0	8.0	
							16.8	15.2
						(133.1)	(123.9)	
A-201			SAME AS A-108					
A-202			SAME AS A-107					
A-203			SAME AS A-106					
A-204			SAME AS A-105					
A-205			SAME AS A-104					
A-206			SAME AS A-103					
A-207			SAME AS A-102					
A-208			SAME AS A-101					

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NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (5) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
B-101	FL+11.0 § FL+5.5	SIDING	$A=5.25 \times 14.3 =$ 75.1	0.025			3.9	3.9
		SLAB	$A=5.25 \times 9.0 =$ 47.3	2.225	0.8		46.7	16.8
		COLUMN	$L=$ 5.5	0.2			1.1	1.1
							51.2	21.3
	FL+5.5 § FL±0	SLAB	$A=$ 21.0	1.225	1.15		25.7	24.2
		P.C. PANEL	$A=5.25 \times 5.5 =$ 28.9	0.29			8.4	8.4
COLUMN		$L=$ 5.5	0.2			1.1	1.1	
						35.2 (86.4)	33.7 (55.0)	
B-102	FL+11.0 § FL+5.5	SLAB	$A=5.25 \times 6.8 =$ 35.7	2.225	0.8		79.4	28.6
		COLUMN	$L=$ 5.5	0.2			1.1	1.1
							80.5	29.7
	FL+5.5 § FL±0	SLAB	$A=$ 35.7	1.225	1.15		43.7	41.1
		COLUMN	$L=$ 5.5	0.2			1.1	1.1
							44.8 (125.3)	42.2 (71.9)
B-103	FL+11.0 § FL+5.5	SLAB	$A=3.3 \times 6.8 =$ 22.4	2.225	0.8		49.8	17.9
		COLUMN					1.1	1.1
							50.9	19.0
	FL+5.5 § FL±0	SLAB	$A=$ 22.4	1.225	1.15		27.9	25.8
		COLUMN					1.1	1.1
							28.5 (79.4)	26.9 (45.9)
B-104	FL+11.0 § FL+5.5	SLAB	$A=3.3 \times 3.4 =$ 11.2	2.225	0.8		24.9	9.0
		GRATING	$A=$ 11.2	0.325	0.255		3.6	2.9
		COLUMN					1.1	1.1
							29.6	13.0
	FL+5.5 § FL±0	GRATING	$A=3.0 \times 6.8 =$ 20.4	0.325	0.255		4.9	3.5
		COLUMN					1.1	1.1
						5.5 (35.1)	4.6 (17.6)	
B-105	FL+11.0 § FL+5.5	SLAB	$A=$ 11.2	2.225	0.8		24.9	9.0
		GRATING	$A=$ 11.2	0.325	0.255		3.6	2.9
		EXC. CUBICLE COLUMN	$1/4 \times 4.0$				1.0	1.0
							1.1	1.1
							30.6	14.0
	FL+5.5 § FL±0		SAME AS B-104				5.5 (36.1)	4.6 (18.6)

		NOTE : UW --- UNIT WEIGHT [單位荷重] PA --- FOR PERMANENT CONDITION [長期] SE --- FOR SEISMIC CONDITION [地震時] ML --- MACHINE LOAD [機械荷重]						
COLUMN AXIAL LOAD (G) [柱軸力]								
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
B-106	FL+11.0 5 FL+5.5	SLAB	$A=3.3 \times 6.8=$ 22.4	2225	0.8		49.8	17.9
		Exc. CUBICLE	$1/4 \times 4.0$				1.0	1.0
		COLUMN					1.1	1.1
							51.9	20.0
	FL+5.5 5 FL±0	SLAB	$A=$ 22.4	1225	1.15		27.4	25.8
		T.P.B.	$L=5.3 \times 3=$ 15.9	1.0			15.9	15.9
		COLUMN					1.1	1.1
							49.4 (96.3)	42.8 (82.8)
B-107	FL+11.0 5 FL+5.5	SLAB	$A=3.3 \times 3.0 + 5.35 \times 5.4=$ 38.3	2225	0.8		85.2	30.6
		COLUMN					1.1	1.1
							86.3	31.7
		FL+5.5 5 FL±0	SLAB	$A=$ 38.3	1225	1.15		46.9
NO.1 M/C	$1/2 \times 18.5$					9.3	9.3	
COLUMN						1.1	1.1	
						57.3 (143.6)	54.4 (86.1)	
B-108	FL+11.0 5 FL+5.5	SLAB	$A=4.0 \times 5.4=$ 21.6	2225	0.8		48.1	17.3
		STAIR	$A=1.2 \times 5.4=$ 6.5	0.43	0.36		2.8	2.3
		HAND RAIL	$L=$ 5.25	0.035			0.2	0.2
			COLUMN				1.1	1.1
						52.2	20.9	
	FL+5.5 5 FL±0	SLAB	$A=$ 21.6	1225	1.15		26.5	24.8
		STAIR	$A=$ 6.5	0.43	0.36		2.8	2.3
		HANDRAIL	$L=$ 5.25	0.035			0.2	0.2
			COMMON MK COLUMN	$1/2 \times 16$			8.0	8.0
						1.1	1.1	
						38.6 (90.8)	36.4 (87.3)	
B-201	FL+11.0 5 FL+5.5		SAME AS B-108				52.2	20.9
	FL+5.5 5 FL±0	SLAB	$A=$ 21.6	1225	1.15		26.5	24.8
		STAIR	$A=$ 6.5	0.43	0.36		2.8	2.3
		HAND RAIL	$L=$ 5.25	0.035			0.2	0.2
		COLUMN					1.1	1.1
							30.6	28.4
							(82.8)	(89.3)
B-202			SAME AS B-107					
B-203			SAME AS B-106					
B-204			SAME AS B-105					

SHEET 36 OF

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (7) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
B-205			SAME AS B-109					
B-206			SAME AS B-103					
B-207			SAME AS B-102					
B-208			SAME AS B-101					
D-101	FL+11.0	SLIDING	$A=6.0 \times 14.3 =$	85.8	2.045		3.9	3.9
	FL+5.5	SLAB	$A=6.0 \times 4.0 =$	24.0	1.425	1.35	39.2	27.4
		M.S.P.	L=	5.0	1.0		5.0	5.0
		H.R.P.	L=	5.0	1.0		5.0	5.0
		COLUMN					1.1	1.1
							49.2	47.4
	FL+5.5	SLAB	A=	24.0	1.225	1.15	29.4	27.6
	FL±0	P.C. PANEL	$A=6.0 \times 5.5 =$	33.0	0.29		9.6	9.6
		COLUMN					1.1	1.1
							40.1	38.3
							(89.3)	(85.7)
D-102	FL+11.0	SLAB (O.H.)	$A=6.0 \times 2.3 =$	13.8	2.225	0.8	30.7	11.0
	FL+5.5	SLAB (O.H.)	$A=6.0 \times 3.9 =$	23.4	1.425	1.35	29.1	27.5
		GRATING	$A=6.0 \times 2.0 =$	12.0	0.325	0.255	3.9	3.1
		M.S.P.	L=	6.8	1.0		6.8	6.8
		H.R.P.	L=	6.8	1.0		6.8	6.8
		COLUMN					1.1	1.1
							78.4	56.3
	FL+5.5	SLAB	$A=6.0 \times 7.7 =$	46.2	1.225	1.15	56.6	53.1
	FL±0	COLUMN					1.1	1.1
							57.7	54.2
							(136.1)	(110.5)
D-107	FL+11.0	SLAB	$A=6.0 \times 6.0 =$	36.0	2.225	0.8	80.1	28.8
	FL+5.5	COLUMN					1.1	1.1
							81.2	27.9
	FL+5.5	SLAB	A=	36.0	1.225	1.15	44.1	41.4
	FL±0	No.1 M/C	$\frac{1}{2} \times 18.5$				9.3	9.3
		COLUMN					1.1	1.1
							54.5	51.8
							(135.7)	(81.7)

		NOTE : UW --- UNIT WEIGHT [單位荷重] PA --- FOR PERMANENT CONDITION [長期] SE --- FOR SEISMIC CONDITION [地震時] ML --- MACHINE LOAD [機械荷重]						
COLUMN AXIAL LOAD (8) [柱軸力]				UW		ML	COLUMN AXIAL LOAD	
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	PA	SE		PA (ton)	SE (ton)
D-108	FL+11.0 s FL+5.5	SLAB (O.H.)	$A = 6.0 \times 5.9 =$	32.9	2.225	0.8	72.1	25.9
		SLAB (OTHER)	$A = 6.0 \times 4.6 =$	27.6	1.425	1.35	39.3	37.3
		COLUMN					1.1	1.1
		HAND RAIL	$L = 3.0 \times 5.0 =$	8.0	0.035		0.3	0.3
							112.8	64.6
	FL+5.5 s FL±0	SLAB	$A = 32.9 + 27.6 =$	60.0	1.225	1.15	73.5	69.0
		COMMON M/C	$1/2 \times 16.0$				8.0	8.0
		COMMON P/C	$1/2 \times 5.5$				2.8	2.8
		HAND RAIL	$L =$	8.0	0.035		0.3	0.3
		COLUMN					1.1	1.1
						85.7 (192.5)	81.2 (195.8)	
D-201	FL+11.0 s FL+5.5	SAME AS D-108					112.8	64.6
FL+5.5 s FL±0	SLAB	$A =$	60.0	1.225	1.15	73.5	69.0	
	HAND RAIL	$L =$	8.0	0.035		0.3	0.3	
	COLUMN					1.1	1.1	
						74.9 (107.7)	70.4 (135.0)	
D-202		SAME AS D-107						
D-207		SAME AS D-102						
D-208		SAME AS D-101						
F-101	FL+11.0 s FL+5.5	SIDING	$A = 5.75 \times 14.3 =$	82.2	0.045		3.7	3.7
		SLAB	$A = 5.75 \times 4.0 =$	23.0	1.425	1.35	32.8	31.1
		C.R.P.	$L =$	5.0	1.0		5.0	5.0
		COLUMN					1.1	1.1
							42.6	40.9
	FL+5.5 s FL±0	SLAB	$A =$	23.0	1.225	1.15	28.2	26.5
		P.C. PANEL	$A = 5.75 \times 5.5 =$	31.6	0.29		9.2	9.2
		COLUMN					1.1	1.1
							38.5 (81.1)	36.8 (77.7)
	F-102	FL+11.0 s FL+5.5	SLAB (O.H.)	$A = 5.75 \times 3.4 =$	19.6	2.225	0.8	43.6
SLAB (OTHER)			$A =$	19.6	1.425	1.35	27.9	26.5
COLUMN							1.1	1.1
FL+5.5 s FL±0		C.R.P.	$L =$	6.8	1.0		6.8	6.8
		SLAB	$A = 5.75 \times 6.8 =$	39.1	1.225	1.15	47.9	45.0
		COLUMN					1.1	1.1
						49.0 (128.4)	56.1 (106.2)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (9) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
F-103	FL+11.0	SLAB	$A=5.75 \times 1.0 + 3.75 \times 5.8 = 27.5$	2.225	0.8		61.2	22.0
	FL+5.5	C.R.P. COLUMN	L=6.8	1.0			6.8	6.8
							1.1	1.1
							69.1	29.9
	FL+5.5	SLAB	A=27.5	1.225	1.15		33.7	31.6
	FL±0	COLUMN					1.1	1.1
							39.8	32.7
							(103.9)	(62.6)
F-104	FL+11.0	SLAB	$A=3.75 \times 3.4 = 12.8$	2.225	0.8		28.5	10.2
	FL+5.5	GRATING COLUMN	A=12.8	0.225	0.255		4.2	3.3
							1.1	1.1
							33.8	14.6
	FL+5.5	GRATING COLUMN	$A=2.75 \times 6.8 = 25.5$	0.225	0.255		8.3	6.5
	FL±0						1.1	1.1
							9.4	7.6
							(43.2)	(22.2)
F-105			SAME AS F-104					
F-106	FL+11.0	SLAB	$A=3.75 \times 3.4 + 1.25 \times 3.4 = 17.0$	2.225	0.8		37.8	13.6
	FL+5.5	GRATING COLUMN	$A=1.5 \times 3.4 = 5.1$	0.225	0.255		1.7	1.3
							1.1	1.1
							40.6	16.0
	FL+5.5						39.8	32.7
	FL±0		SAME AS F-103				(75.4)	(48.7)
F-107	FL+11.0	SLAB	$A=3.75 \times 3.4 + 5.75 \times 6.0 = 43.5$	2.225	0.8		96.8	34.8
	FL+5.5	COLUMN					1.1	1.1
							97.9	35.9
	FL+5.5	SLAB	A=43.5	1.225	1.15		53.3	50.0
	FL±0	NO.2 P/C COLUMN	$\frac{1}{2} \times 16.5$				8.3	8.3
							1.1	1.1
							62.7	59.4
							(160.6)	(95.3)
F-108	FL+11.0	SLAB (O.H.)	$A=5.75 \times 5.0 = 28.8$	2.225	0.8		64.1	23.0
	FL+5.5	SLAB (OTHER) COLUMN	A=28.8	1.225	1.35		41.0	38.9
							1.1	1.1
							106.2	63.0
	FL+5.5	SLAB	$A=5.75 \times 10.0 = 57.5$	1.225	1.15		70.4	66.1
	FL±0	COMMON P/C COLUMN	$\frac{1}{2} \times (5.5 + 18.5)$				12.0	12.0
							82.4	78.1
							(188.6)	(141.1)

		NOTE : UW --- UNIT WEIGHT [單位荷重] PA --- FOR PERMANENT CONDITION [長期] SE --- FOR SEISMIC CONDITION [地震時] ML --- MACHINE LOAD [機械荷重]						
COLUMN AXIAL LOAD (10) [柱軸力]								
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
F-201	FL+11.0							
	FL+5.5		SAME AS F-108				106.2	63.0
	FL+5.5	SLAB	$A=5.75 \times 10.0 = 57.5$	2.225	1.15		70.9	66.1
	FL±0	C/S COLUMN	$1/2 \times 4.0$				2.0 1.1 73.5 (179.7)	2.0 1.1 69.2 (132.2)
F-202			SAME AS F-107					
F-203	FL+11.0	SLAB	$A=3.75 \times 6.8 = 25.5$	2.225	0.8		56.7	20.4
	FL+5.5	COLUMN					1.1 57.8	1.1 21.5
	FL+5.5		SAME AS F-106				34.8 (92.6)	32.7 (54.2)
F-204			SAME AS F-109					
F-205			SAME AS F-105					
F-206	FL+11.0	SLAB	$A=5.75 \times (0.75 \times 2.4 + 2.25 \times 4) = 22.4$	2.225	0.8		49.8	17.9
	FL+5.5	GRATING	$A=1.5 \times 3.4 = 5.1$	2.225	0.255		1.7	1.3
		C.R.P.	$L=6.8$	1.0			6.8	6.8
		COLUMN					1.1 59.4	1.1 27.1
	FL+5.5		SAME AS F-103				34.8 (94.2)	32.7 (59.8)
F-207			SAME AS F-102					
F-208			SAME AS F-101					

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (11)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-101	FL+27.1 5							
	FL+21.0		SAME AS A-101				36.7	33.9
	FL+21.0 5	SLAB	A=5.0x4.0= 20.0	0.635	0.535		12.7	10.7
	FL+16.0	PARAPET	L=5.0x4.0= 9.0	0.12			1.1	1.1
		SIDING	A=7.75x5.0= 38.8	0.095			1.7	1.7
		CRANE GIRDER	L= 9.0	0.35			1.4	1.4
		COLUMN	L= 5.0	0.25			1.3	1.3
							18.2	16.2
							(54.9)	(50.1)
	FL+16.0 5	SLAB	A= 20.0	1.29	0.665		25.8	12.3
	FL+11.0	SIDING	A=7.75x5.0= 38.8	0.095			1.7	1.7
		COLUMN					1.3	1.3
						28.8	16.3	
						(83.7)	(66.4)	
FL+11.0 5	SLAB	A=7.75x4.0= 31.0	1.925	1.35		44.2	41.9	
FL+5.5	COLUMN					1.4	1.4	
						45.6	43.3	
						(129.3)	(109.7)	
FL+5.5 5	SLAB	A= 31.0	1.225	1.15		38.0	35.7	
FL±0	P.C. PANEL	A=7.75x5.5= 42.6	0.29			12.4	12.4	
	COLUMN	L= 5.5	0.25			1.4	1.4	
						51.8	49.5	
						(181.1)	(159.2)	
G-102	FL+27.1 5							
	FL+21.0		SAME AS A-102				53.8	49.1
	FL+21.0 5	SLAB	A=5.0x2.4= 17.0	0.635	0.535		10.8	9.1
	FL+16.0	SLAB (DECK)	A= 17.0	1.035	0.69		17.6	11.7
		PARAPET	L= 6.8	0.12			0.8	0.8
		CRANE GIRDER	L= 6.8	0.35			2.4	2.4
		COLUMN					1.3	1.3
							22.9	25.3
							(86.7)	(74.4)
	FL+16.0 5	SLAB	A=5.0x6.8= 34.0	1.29	0.665		43.9	22.6
	FL+11.0	No. 4 LP	1/2 x 11				5.5	5.5
		COLUMN					1.3	1.3
						50.7	29.4	
						(137.4)	(103.8)	
FL+11.0 5	SLAB (OH)	A=7.75x6.8= 52.7	2.225	0.8		41.6	15.0	
FL+5.5	SLAB (OTHER)	A=5.0x6.8= 34.0	1.925	1.35		48.5	45.1	
	No. 3 LP	1/4 x 12.0				3.0	3.0	
	COLUMN					1.4	1.4	
						94.5	65.3	
						(231.9)	(169.1)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (12)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
(G-102)	FL+5.5	SLAB	A = 7.75 x 6.8 = 52.7	1.225	1.15		64.6	60.6
	5	No. 2 LP	1/4 x 12.0				3.0	3.0
	FL±0	COLUMN					1.4	1.4
							69.0	65.0
							(300.9)	(234.1)
G-103	FL+27.1		SAME AS A-103				53.8	49.1
	5							
	FL+21.0							
	FL+16.0	SLAB	A = 5.0 x 6.8 = 34.0	1.025	0.69		35.2	23.5
	5	PARAPET	L = 6.8	0.12			0.8	0.8
		CRANE GIRDER	L = 6.8	0.35		2.4	2.4	
		DEAERATOR	1/4 x 264.0			66.0	66.0	
		COLUMN				1.3	1.3	
						105.7	94.0	
						(159.5)	(143.1)	
G-102	FL+16.0		SAME AS G-102				50.7	29.4
	3						(210.2)	(172.5)
	FL+11.0							
	5						94.5	65.3
	FL+5.5		SAME AS G-102				(304.7)	(237.8)
G-102	FL+5.5		SAME AS G-102				69.0	65.0
	5						(373.7)	(302.8)
	FL±0							
G-104	FL+27.1		SAME AS A-104				53.8	49.1
	3							
	FL+21.0						105.7	94.0
	5		SAME AS G-103				(159.5)	(143.1)
	FL+16.0							
G-103	FL+16.0	SLAB	A = 5.0 x 6.8 = 34.0	1.29	0.665		43.9	22.6
	5	No. 8 HP	1/4 x 50.0				12.5	12.5
	FL+11.0	COLUMN					1.3	1.3
							57.7	26.4
							(217.2)	(179.5)
G-103	FL+11.0	SLAB (O.H.)	A = 2.75 x 3.9 = 9.4	2.225	0.8		20.9	7.5
	5	SLAB (OTHER)	A = 5.0 x 6.8 = 34.0	1.225	1.35		48.5	45.9
	FL+5.5	GRATING	A = 9.4	0.325	0.255		3.1	2.4
		No. 7 HP	1/2 x 45				22.5	22.5
		COLUMN					1.4	1.4
						96.4	19.7	
						(213.6)	(259.2)	
G-103	FL+5.5	SLAB	A = 5.0 x 6.8 + 2.75 x 0.9 = 36.5	1.225	1.15		44.7	42.0
	5	GRATING	A = 2.75 x 5.9 = 16.2	0.325	0.255		5.3	4.1
	FL±0	COLUMN					1.4	1.4
							51.4	47.5
							(365.0)	(306.7)

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NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (13) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-105	FL+27.1 s FL+21.0		SAME AS A-105				53.8	49.1	
	FL+21.0 s FL+16.0		SAME AS G-102				32.9 (86.7)	25.3 (74.4)	
	FL+16.0 s FL+11.0		SAME AS G-104				57.7 (144.4)	36.4 (110.8)	
	FL+11.0 s FL+5.5		SAME AS G-104				96.4 (240.8)	79.7 (190.5)	
	FL+5.5 s FL±0	No.1 C/C	1/2 x 7.0				51.4 3.5 54.9 (295.7)	47.5 3.5 51.0 (241.5)	
	G-106	FL+27.1 s FL+21.0		SAME AS A-106				53.8	49.1
		FL+21.0 s FL+16.0	SLAB PARAPET CRANE GIRDER COLUMN	A=5.0x6.8= 34.0 L= 6.8 L= 6.8	27.0 0.17 0.35	0.535 0.17 0.35		21.6 0.8 2.4 1.3 26.1 (79.9)	18.2 0.8 2.9 1.3 22.7 (71.8)
FL+16.0 s FL+11.0		SLAB COLUMN	A=5.0x6.8= 34.0	34.0	1.29 0.665		43.9 1.3 45.2 (125.1)	22.6 1.3 23.9 (95.7)	
FL+11.0 s FL+5.5		SLAB (O.H.) SLAB (OTHER) GRATING COLUMN	A=2.75x3.9= 9.9 A=5.0x6.8+1.25x3.9= 38.3 A=1.5x3.9= 5.7	9.9 38.3 5.7	2.225 1.925 0.325	0.8 1.35 0.255	20.9 54.6 1.7 6.4 78.6 (203.7)	7.5 51.7 1.3 6.4 61.9 (157.6)	
FL+5.5 s FL±0		SLAB No.1 C/C COLUMN	A=7.75x6.8= 52.7 1/2 x 7.0	52.7	1.225 1.15		64.6 3.5 6.4 69.5 (273.2)	60.6 3.5 6.4 65.5 (223.1)	

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NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (14)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-107	FL+27.1								
	FL+21.0		SAME AS A-107				65.6	59.7	
	FL+21.0	SLAB	A=5.0x8.4=	42.0	0.635	0.535		26.7	22.5
	FL+16.0	PARAPET	L=	8.4	0.12			1.0	1.0
		CRANE GIRDER	L=	8.4	0.35			2.9	2.9
		COLUMN						1.3	1.3
								31.9	27.7
								(97.5)	(87.4)
	FL+16.0	SLAB	A=5.0x3.9=	19.5	0.71	0.61		12.1	10.4
	FL+11.0	BLOCK WALL	A=5.0x(5.0x5.0)=	50.0	0.4			20.0	20.0
		COLUMN						1.3	1.3
		SLAB (M.C.)	A=5.0x5.0=	25.0	0.985			24.6	24.6
							58.0	56.3	
							(155.5)	(148.7)	
FL+11.0	SLAB (O.H.)	A=7.75x5.0+2.75x3.9=	48.1	2.225	0.8		107.0	48.5	
FL+5.5	SLAB (OTHER)	A=5.0x3.9=	19.5	1.425	1.35		24.2	23.0	
	COLUMN						1.4	1.4	
							132.6	62.9	
							(288.1)	(206.6)	
FL+5.5	SLAB	A=7.75x8.4=	65.1	1.225	1.15		79.7	79.9	
FL±0	1-3 C/C & N1,2/C	9.0+1/2x11.5					12.3	12.3	
	COLUMN						1.4	1.4	
							93.4	82.6	
							(381.5)	(295.2)	
G-108	FL+27.1								
	FL+21.0		SAME AS A-108				77.4	70.5	
	FL+21.0	SLAB	A=5.0x10.0=	50.0	0.635	0.535		31.8	26.8
	FL+16.0	PARAPET	L=	10.0	0.12			1.2	1.2
		CRANE GIRDER	L=	10.0	0.35			3.5	3.5
		COLUMN						1.3	1.3
								37.8	32.8
								(115.2)	(103.3)
	FL+16.0	SLAB	A=	50.0	0.985			49.3	49.3
	FL+11.0	BLOCK WALL	A=5.0x10.0=	50.0	0.4			20.0	20.0
		COLUMN						1.3	1.3
								70.6	70.6
							(185.8)	(173.9)	
FL+11.0	SLAB	A=7.75x10.0=	77.5	2.225	0.8		172.4	62.0	
FL+5.5	COLUMN						1.4	1.4	
							173.8	63.4	
							(359.6)	(237.3)	
FL+5.5	SLAB	A=	77.5	1.225	1.15		94.9	89.1	
FL±0	COMMON P/C	1 1/2 x 18.5					9.3	9.3	
	COLUMN						1.4	1.4	
							105.6	99.8	
							(465.2)	(337.1)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (15)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-201	FL+27.1							
	FL+21.0		SAME AS G-108				77.4	70.5
	FL+21.0						37.8	32.8
	FL+16.0		SAME AS G-108				(115.2)	(103.3)
	FL+16.0						70.6	70.6
	FL+11.0		SAME AS G-108				(185.8)	(173.9)
	FL+11.0						173.8	63.4
G-202	FL+27.1							
	FL+21.0		SAME AS G-107				65.6	59.7
	FL+21.0						31.9	27.7
	FL+16.0		SAME AS G-107				(97.5)	(87.4)
	FL+16.0	SLAB	$A=5.0 \times 5.4 = 27.0$	0.985			26.6	26.6
	FL+11.0	STAIR	$A=5.0 \times 3.0 = 15.0$	0.43	0.36		6.5	5.4
		BLOCK WALL	$A=5.0 \times (5.0 \times 2 + 5.0) = 75.0$	0.4			30.0	30.0
	COLUMN					1.3	1.3	
						64.4	63.3	
						(161.9)	(150.7)	
G-203	FL+27.1							
	FL+21.0		SAME AS G-106				53.8	49.1
	FL+21.0						26.1	22.7
	FL+16.0		SAME AS G-106				(79.9)	(71.8)
	FL+16.0						45.2	23.9
	FL+11.0		SAME AS G-106				(125.1)	(95.7)

		NOTE : UW --- UNIT WEIGHT [單位荷重]						
		PA --- FOR PERMANENT CONDITION [長期]						
		SE --- FOR SEISMIC CONDITION [地震時]						
		ML --- MACHINE LOAD [機械荷重]						
COLUMN AXIAL LOAD (16)		COLUMN AXIAL LOAD						
[柱軸力]		[柱軸力]						
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-203	FL+11.0						94.5	65.3
	FL+5.5		SAME AS G-102				(219.6)	(161.0)
	FL+5.5						69.5	65.5
G-204	FL+27.1							
	FL+21.0		SAME AS G-203				53.8	49.1
	FL+21.0						32.9	25.3
G-205	FL+16.0		SAME AS G-105				(86.7)	(74.4)
	FL+16.0						57.7	36.4
	FL+11.0		SAME AS G-105				(144.4)	(110.8)
G-206	FL+11.0						94.5	65.3
	FL+5.5		SAME AS G-103				(238.9)	(176.1)
	FL+5.5						54.9	51.0
G-207	FL+5.5		SAME AS G-105				(293.8)	(227.1)
	FL+10							
			SAME AS G-104					
G-208	FL+27.1							
	FL+21.0		SAME AS G-103				53.8	49.1
	FL+21.0						105.7	94.0
	FL+16.0		SAME AS G-103				(159.5)	(143.1)
	FL+16.0						50.7	29.4
	FL+11.0		SAME AS G-103				(210.2)	(172.5)
	FL+11.0						78.6	61.9
FL+5.5	No. 7 HP	1/2 x 45				22.5	22.5	
G-209	FL+5.5						101.1	84.4
	FL+10						(311.3)	(256.9)
	FL+10		SAME AS G-103				49.0	65.0
G-210	FL+10						(320.3)	(321.9)
	FL+10							
			SAME AS G-103					
G-211	FL+27.1							
	FL+21.0		SAME AS G-102				53.8	49.1
	FL+21.0						32.9	25.3
	FL+16.0		SAME AS G-102				(86.7)	(74.4)
G-212	FL+16.0						50.7	29.4
	FL+11.0		SAME AS G-102				(137.4)	(103.8)

SHEET 46 OF

NOTE : UW --- UNIT WEIGHT [單位荷重]

PA --- FOR PERMANENT CONDITION [長期]

SE --- FOR SEISMIC CONDITION [地震時]

COLUMN AXIAL LOAD (17)
[柱軸力]

ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-207	FL+11.0 5 FL+5.5	SLAB (O.H.)	$A=2.75 \times 6.8 =$	18.7	2.225	0.8	41.6	15.9	
		SLAB (OTHER)	$A=5.0 \times 6.8 =$	34.0	1.925	1.35	48.5	45.9	
	COLUMN					1.4	1.4		
							91.5	62.3	
							(228.7)	(166.1)	
	FL+5.5 5 FL±0		SAME AS G-102				69.0	65.0	
							(297.9)	(231.1)	
G-208			SAME AS G-101						
H-101	FL+21.0 5 FL+16.0	SLAB	$A=5.6 \times 4.0 =$	22.4	0.595	0.535	13.3	12.0	
		PARAPET	$L=5.6 \times 2 + 4.0 =$	15.2	0.12		1.8	1.8	
		SIDING	$L=15.2 \times 12.0 =$	45.6	0.045		2.1	2.1	
		STAIR	$A =$	22.4	0.43	0.36	9.6	8.1	
		SIDING	$A=5.0 \times (4.0+5.6) =$	48.0	0.045		2.2	2.2	
		COLUMN	$L =$	5.0	0.2		1.0	1.0	
							30.0	27.2	
	FL+16.0 5 FL+11.0	STAIR	$A =$	22.4	0.43	0.36	9.6	8.1	
		SIDING	$A =$	48.0	0.045		2.2	2.2	
		COLUMN					1.0	1.0	
								12.8	11.3
								(42.8)	(38.5)
FL+11.0 5 FL+5.5	STAIR	$A =$	22.4	0.43	0.36	9.6	8.1		
	COLUMN					1.1	1.1		
							10.7	9.2	
							(53.5)	(47.7)	
FL+5.5 5 FL±0	STAIR	$A =$	22.4	0.43	0.36	9.6	8.1		
	P.C. PANEL	$A=5.5 \times (4.0+5.6) =$	52.8	0.29		15.3	15.3		
	COLUMN					1.1	1.1		
							26.0	24.5	
							(79.5)	(72.2)	
H-102	FL+21.0 5 FL+16.0	SLAB	$A=5.6 \times 3.4 =$	19.0	0.635	0.535	12.1	10.2	
		SLAB (DEAF)	$A =$	19.0	1.035	0.69	19.7	13.1	
		PARAPET	$L =$	6.8	0.12		0.8	0.8	
		SIDING	$A=5.9 \times 6.8 =$	40.1	0.045		1.8	1.8	
	COLUMN					1.0	1.0		
							35.4	26.9	
FL+16.0 5 FL+11.0	SLAB	$A=5.6 \times 6.8 =$	38.1	1.29	0.665	49.1	25.3		
	SIDING	$A =$	40.1	0.045		1.8	1.8		
	COLUMN					1.0	1.0		
							51.9	28.1	
							(87.3)	(55.0)	
FL+11.0 5 FL+5.5	SLAB	$A =$	38.1	1.925	1.35	54.3	51.4		
	NA3 LP	$1/4 \times 12.0$				3.0	3.0		
	COLUMN					1.1	1.1		
							58.4	55.5	
							(145.7)	(110.5)	

		NOTE : UW --- UNIT WEIGHT [單位荷重] PA --- FOR PERMANENT CONDITION [長期] SE --- FOR SEISMIC CONDITION [地震時] ML --- MACHINE LOAD [機械荷重]							
COLUMN AXIAL LOAD (18) [柱軸力]				UW		ML	COLUMN AXIAL LOAD		
LOCA-TION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	PA	SE		PA (ton)	SE (ton)	
H-102	FL+5.5	SLAB	A= 38.1	1.225	1.15		46.7	43.8	
	FL±0	NO.2 LP	1/4 x 12.0				3.0	3.0	
		R.C. PANEL	A= 5.5 x 6.8 = 37.4	0.29			10.8	10.8	
		COLUMN					1.1	1.1	
						61.6 (207.3)	58.7 (169.2)		
H-103	FL+21.0	SLAB	A= 38.1	1.035	0.69		38.9	26.3	
	FL+16.0	PARAPET	L= 6.8	0.12			0.8	0.8	
		DEAERATOR	1/4 x 26.0				66.0	66.0	
	FL±0	SIDING	A= 5.9 x 6.8 = 40.1	0.095			1.8	1.8	
		COLUMN					1.0	1.0	
							109.0	95.9	
	FL+16.0		SAME AS H-102				51.9	28.1	
	FL+11.0						(160.9)	(129.0)	
	FL+11.0		SAME AS H-102				58.4	55.5	
	FL+5.5						(219.3)	(179.5)	
FL+5.5		SAME AS H-102				61.6	58.7		
FL±0						(280.9)	(238.2)		
H-104	FL+21.0		SAME AS H-103				109.0	95.9	
	FL+16.0								
	FL+16.0		SAME AS H-102				51.9	28.1	
	FL+11.0	NO.8 HP	1/4 x 50.0				12.5	12.5	
							69.4	40.6	
							(173.4)	(136.5)	
	FL+11.0	SLAB	A= 38.1	1.225	1.35		57.3	51.4	
	FL+5.5	COLUMN					1.1	1.1	
							55.4	52.5	
							(228.8)	(169.0)	
FL+5.5	SLAB	A= 38.1	1.225	1.15		46.7	43.8		
FL±0	NO.6 HP	1/2 x 36.0				18.0	18.0		
	R.C. PANEL	A= 37.4	0.29			10.8	10.8		
	COLUMN					1.1	1.1		
						76.6 (305.4)	73.7 (262.7)		
H-105	FL+21.0		SAME AS H-102				35.4	26.9	
	FL+16.0								
	FL+16.0		SAME AS H-104				69.4	40.6	
	FL+11.0						(99.8)	(67.5)	
	FL+11.0		SAME AS H-104				55.4	52.5	
	FL+5.5						(155.2)	(120.0)	
FL+5.5		SAME AS H-104				76.6	73.7		
FL±0						(231.8)	(193.7)		

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (19)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-106	FL+21.0 5 FL+16.0
	
	SLAB	$A = 7.0 \times 9.0 =$	28.0	0.575	0.535	16.7	15.0	
	PARAPET	$L = (7.0 + 9.0) \times 2 =$	22.0	0.12		2.6	2.6	
	SIDING	$A = 22.0 \times 3.0 =$	66.0	0.045		3.0	3.0	
	SLAB	$A = 5.0 \times 6.8 =$	34.0	0.635	0.535	21.6	18.2	
	STAIR	$A = 7.0 \times 3.0 =$	21.0	0.43	0.36	9.0	7.6	

	SIDING	$A = 5.0 \times (6.9 + 2.8) =$	46.0	0.095		2.1	2.1	
	PARAPET	$L = 3.4 \times 3 + 7.0 =$	17.2	0.12		2.1	2.1	
	COLUMN	1.0	1.0	
	58.1	51.6	
	
	FL+16.0 3 FL+11.0	SLAB	$A =$	34.0	1.29	0.665	43.9	22.6
	STAIR	$A =$	21.0	0.43	0.36	9.0	7.6
.....	BLOCK WALL	$A = 5.0 \times (8.0 + 9.0 \times 2) =$	80.0	0.4		32.0	32.0	
.....	SIDING	$A =$	46.0	0.045		2.1	2.1	
.....	COLUMN	1.0	1.0	
.....	88.0	65.3	
.....	(146.1)	(116.9)	
.....	
FL+11.0 3 FL+5.5	SLAB	$A =$	34.0	1.425	1.35	48.5	45.9	
.....	STAIR	$A =$	21.0	0.43	0.36	9.0	7.6	
.....	BLOCK WALL	$A = 5.5 \times (8.0 + 9.0 \times 2) =$	80.0	0.4		35.2	35.2	
.....	COLUMN	1.1	1.1	
.....	93.8	89.8	
.....	(229.9)	(206.7)	
.....	
FL+5.5 5 FL+0	SLAB	$A =$	39.0	1.225	1.15	41.7	39.1	
.....	STAIR	$A =$	21.0	0.43	0.36	9.0	7.6	
.....	BLOCK WALL	$A =$	80.0	0.4		32.0	32.0	
.....	P.C. PANEL	$A = 5.5 \times (6.9 + 2.8) =$	50.6	0.29		14.7	14.7	
.....	COLUMN	1.1	1.1	
.....	98.5	94.5	
.....	(339.9)	(301.2)	
.....	
H-107	FL+21.0 5 FL+16.0
	
		SLAB (MC)	$A = 5.0 \times 8.4 =$	42.0	0.635	0.535	26.7	22.5
		SLAB	$A = 7.0 \times 8.4 =$	58.8	0.765	0.665	45.0	39.1
		FENCE	$L =$	8.4	0.11		0.9	0.9
.....	COLUMN	1.0	1.0	
.....	73.6	63.5	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (20) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
H-107	FL+16.0	SLAB (CON)	$A = 5.0 \times (4 + 3.0 \times 8.4) = 42.2$	0.71	0.61		30.0	25.7	
		SLAB (CELY)	$A = 4.0 \times 8.4 = 33.6$	0.88			29.6	29.6	
	FL+11.0	BLOCK WALL	$A = 5.0 \times (0.9 \times 2 + 5.0) = 109.0$	0.4			43.6	43.6	
		WALL	$A = 0.0 \times 4.0 = 12.0$	0.036			0.4	0.4	
		COLUMN					1.0	1.0	
		SLAB (A/C)	$A = 5.0 \times 5.0 = 25.0$	0.985			24.6	24.6	
							129.2	129.9	
							(202.8)	(188.4)	
	FL+11.0	FL+5.5	SLAB	$A = 5.0 \times 8.4 = 42.0$	1.925	1.35		59.9	56.7
			COMPUTER	$A = 7.0 \times 6.9 = 48.3$	0.925			44.7	44.7
			CENT. CONT.	$A = 7.0 \times 1.5 = 10.5$	0.905	0.73		9.5	7.7
			BLOCK WALL	$A = 5.0 \times (8.4 \times 2 + 2.0 \times 2) = 104.0$	0.35			36.4	36.4
			WALL (SNG)	$A = 3.0 \times 8.4 = 25.2$	0.026			0.7	0.7
			WALL (W)	$A = 3.0 \times 7.0 = 21.0$	0.036			0.8	0.8
			COLUMN					1.1	1.1
			CHP. FLOOR	$A = 7.0 \times 8.4 = 58.8$	0.195			11.5	11.5
							164.6	159.6	
							(367.4)	(348.0)	
	FL+5.5	FL+0	SLAB	$A = 42.0$	1.225	1.15		51.5	48.3
			CENTRAL RM.	$A = 7.0 \times 8.4 = 58.8$	1.025	0.925		60.3	57.3
WALL			$A = 3.0 \times 8.4 = 25.2$	0.026			0.7	0.7	
BLOCK WALL			$A = 5.5 \times (8.4 \times 2 + 2.0 \times 2) = 114.4$	0.35			40.0	40.0	
COLUMN							1.1	1.1	
						152.6	147.4		
						(521.0)	(495.4)		
H-108	FL+21.0	FL+16.0							
			SLAB (A/C)	$A = 5.0 \times 10.0 = 50.0$	0.635	0.535		31.8	26.8
			SLAB	$A = 7.0 \times 10.0 = 70.0$	0.765	0.615		53.6	46.6
			FENCE	$L = 10.0$	0.11			1.1	1.1
			COLUMN				1.0	1.0	
							87.5	75.5	
	FL+16.0	FL+11.0	SLAB	$A = 7.0 \times 10.0 = 70.0$	0.88			61.6	61.6
			SLAB (A/C)	$A = 5.0 \times 10.0 = 50.0$	0.985			49.3	49.3
			BLOCK WALL	$A = 5.0 \times (10.0 \times 2) = 100.0$	0.4			40.0	40.0
			COLUMN					1.0	1.0
						151.9	151.9		
						(239.4)	(227.4)		
FL+11.0	FL+5.5	SLAB	$A = 5.0 \times 10.0 = 50.0$	1.925	1.35		71.3	67.5	
		CENT. CONT.	$A = 7.0 \times 10.0 = 70.0$	0.905	0.73		63.4	57.1	
		CHP. FLOOR	$A = 70.0$	0.195			13.7	13.7	
		BLOCK WALL	$A = 5.0 \times 10.0 \times 2 = 100.0$	0.35			35.0	35.0	
		WALL	$A = 3.0 \times 10.0 = 30.0$	0.028			0.8	0.8	
		COLUMN					1.1	1.1	
								185.3	169.2
						(424.7)	(396.6)		

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (Z)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-108	FL+5.5 FL±0	SLAB	$A=5.0 \times 11.0 =$	50.0	1.225	1.15	61.3	57.5
		CONTRL RM	$A=7.0 \times 5.0 =$	35.0	1.025	0.975	35.9	34.1
		BATTERY RM	$A=$	35.0	1.025	0.975	35.9	34.1
		BLOCK WALL	$A=5.5 \times 0.1 \times 1.5 =$	82.5	0.9		33.0	33.0
		WALL	$A=3.0 \times (5.0 \times 3) =$	45.0	0.026		1.2	1.2
		COLUMN					1.1	1.1
							168.4 (593.1)	161.0 (557.6)
H-201	FL+21.0 FL+16.0		SAME AS H-108				87.5	75.5
	FL+16.0 FL+11.0	SLAB (CON)	$A=7.0 \times 6.5 =$	45.5	0.71	0.61	27.3	27.8
		SLAB (A/C)	$A=5.0 \times 10.0 =$	50.0	0.985		49.3	49.3
		SLAB	$A=7.0 \times 3.5 =$	24.5	0.88		21.6	21.6
		BLOCK WALL	$A=5.0 \times 10.0 \times 2 =$	100.0	0.9		40.0	40.0
		WALL	$A=3.0 \times 4.0 =$	12.0	0.036		0.4	0.4
		COLUMN					1.0	1.0
						144.6 (232.1)	140.1 (215.6)	
	FL+11.0 FL+5.5		SAME AS H-108				157.4 (389.5)	150.0 (365.6)
FL+5.5 FL±0	LABO	SLAB	$A=5.0 \times 10.0 =$	50.0	1.225	1.15	61.3	57.5
		LABO	$A=7.0 \times 5.0 =$	35.0	0.855	0.705	29.9	24.7
		BATTERY RM	$A=$	35.0	1.025	0.975	35.9	34.1
		BLOCK WALL	$A=5.5 \times 10.0 \times (1.5 + 2.0 \times 2) =$	104.5	0.9		41.8	41.8
		WALL	$A=3.0 \times 2.0 \times 2 =$	12.0	0.026		1.1	1.1
		COLUMN					1.1	1.1
					171.1 (560.6)	160.3 (525.9)		
H-202	FL+21.0 FL+16.0		SAME AS H-107				13.6	63.5
	FL+16.0 FL+11.0	SLAB (A/C)	$A=5.0 \times 5.0 =$	25.0	0.985		24.6	24.6
		SLAB (CON)	$A=5.0 \times 3.4 + 7.0 \times 8.4 =$	75.8	0.71	0.61	53.8	46.2
		BLOCK WALL	$A=5.0 \times (0.4 \times 2 + 4.0 + 5.0) =$	129.0	0.9		51.6	51.6
WALL	$A=3.0 \times (5.0 + 4.0) =$	27.0	0.36		9.7	9.7		
COLUMN					1.0	1.0		
					140.7 (214.3)	133.1 (196.6)		

		NOTE : UW --- UNIT WEIGHT [單位荷重] PA --- FOR PERMANENT CONDITION [長期] SE --- FOR SEISMIC CONDITION [地震時] ML --- MACHINE LOAD [機械荷重]						
COLUMN AXIAL LOAD (22) [柱軸力]								
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-202	FL+11.0						164.6	159.6
	FL+5.5		SAME AS H-107				(378.9)	(356.2)
	FL+5.5	SLAB	$A=5.0 \times 8.9 =$	42.0	1.225	1.15	57.5	48.3
	FL±0	LABO	$A=7.0 \times 8.9 =$	58.8	0.855	0.705	50.3	41.5
		WALL	$A=3.0 \times 8.9 =$	25.2	0.026		0.7	0.7
		BLOCK WALL	$A=5.5 \times (0.4 \times 2 + 2.0 \times 2) =$	114.4	0.35		40.0	40.0
		COLUMN					1.1	1.1
						143.6	131.6	
						(522.5)	(487.8)	
H-203	FL+21.0	SLAB (ROOF)	$A=5.0 \times 6.8 =$	34.0	0.635	0.535	21.6	18.2
		SLAB (TOWER)	$A=7.0 \times 7.0 =$	48.0	0.765	0.665	21.4	18.6
	FL+16.0	FENCE	$L=3.9 \times 7.6 =$	11.0	0.11		1.2	1.2
		PARAPET	$L=2.8 \times 6.9 =$	9.2	0.12		1.1	1.1
		SIDING	$A=5.9 \times 9.2 =$	54.3	0.045		2.4	2.4
		COLUMN					1.0	1.0
							48.7	42.5
	FL+16.0	SLAB (LAV)	$A=7.0 \times 3.9 =$	23.8	0.79	0.72	18.8	17.1
	FL+11.0	SLAB	$A=5.0 \times 6.8 =$	34.0	1.29	0.665	43.9	22.6
		BLOCK WALL	$A=5.0 \times (7.0 \times 2 + 3.9 \times 5) =$	155.0	0.4		62.0	62.0
		COLUMN					1.0	1.0
		SIDING	$A=5.0 \times 9.2 =$	46.0	0.045		2.1	2.1
							127.8	109.8
							(176.5)	(149.3)
FL+11.0		SAME AS H-106				93.8	89.8	
FL+5.5	No.3 LP	$1/4 \times 12.0$				3.0	3.0	
						96.8	92.8	
						(273.3)	(240.1)	
FL+5.5		SAME AS H-106				98.5	94.5	
FL±0						(371.8)	(339.6)	
H-204	FL+21.0		SAME AS H-105				35.9	27.4
	FL+16.0						64.4	40.6
	FL+16.0		SAME AS H-105				(100.3)	(68.0)
	FL+11.0						55.4	32.5
	FL+5.5	No.3 LP	$1/4 \times 12.0$				3.0	3.0
							58.4	55.5
						(158.7)	(123.5)	
FL+5.5		SAME AS H-105				76.6	73.7	
FL±0						(235.3)	(197.2)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (23)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-205			SAME AS H-104					
H-206	FL+21.0		SAME AS H-103				109.0	95.9
	FL+16.0						51.9	28.1
	FL+11.0		SAME AS H-103				(160.9)	(129.0)
	FL+5.5		SAME AS H-104				55.4	52.5
	FL±0		SAME AS H-103				(227.9)	(235.2)
H-207	FL+21.0		SAME AS H-102				35.4	26.9
	FL+16.0						56.8	28.1
	FL+11.0		SAME AS H-102				(87.3)	(55.0)
	FL+5.5	SLAB	A=5.6x6.8=	38.1	1.925	1.35	54.2	51.4
	FL±0	COLUMN					61.6	58.7
H-208			SAME AS H-101					
K-106	FL+21.0	SLAB	A=2.6x4.0=	30.4	0.765	0.665	23.3	20.2
	FL+16.0	PARAPET SIDING COLUMN	L=2.6+7.0= A=5.9x11.6=	11.6 68.4	0.12 0.045		1.4 4.1 1.0	1.4 3.1 1.0
K-106	FL+16.0	SLAB	A=	30.4	0.08		26.8	26.8
	FL+11.0	WALL	A=3.0x(3.7+7.0)=	31.2	0.028		0.9	0.9
		SIDING	A=5.0x11.6=	58.0	0.045		2.6	2.6
		COLUMN					1.0	1.0
						31.3	31.3	
						(60.1)	(57.0)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
K-106	FL+11.0 5 FL+5.5	COMPUTER	A= 30.7	0.925			28.1	28.1
		WALL	A= 31.2	0.028		0.9	0.9	
		COLUMN				1.1	1.1	
		CHR. FLOOR	A= 30.7	0.195		5.9	5.9	
							26.0	26.0
							(96.1)	(93.0)
	FL+5.5 5 FL±0	SLAB	A= 30.7	1.025	0.975		31.2	29.6
		WALL	A= 31.2	0.028		0.9	0.9	
		P.C. PANEL	A= 5.5 × (4.0 + 2.6) = 63.8	0.29		18.5	18.5	
		COLUMN				1.1	1.1	
							51.7	50.1
							(147.8)	(143.1)
K-107	FL+21.0 5 FL+16.0	SLAB	A= 7.6 × 8.4 = 66.1	0.765	0.665		50.6	44.0
		PARAPET	L= 8.4	0.12		1.0	1.0	
		SIDING	A= 5.9 × 8.4 = 49.6	0.095		2.2	2.2	
		COLUMN				1.0	1.0	
							54.8	48.2
	FL+16.0 5 FL+11.0	SLAB	A= 7.6 × 3.4 = 25.8	0.88			22.7	22.7
		SLAB	A= 2.8 × 5.0 = 14.0	0.88			33.4	33.4
		BLOCK WALL	A= 5.0 × 7.4 = 37.0	0.35			18.6	18.6
		WALL	A= 3.0 × (8.4 + 7.6) = 48.0	0.028			1.3	1.3
		SIDING	A= 5.0 × 8.4 = 42.0	0.095			1.9	1.9
		COLUMN					1.0	1.0
							78.9	78.9
						(133.7)	(127.1)	
FL+11.0 5 FL+5.5	COMPUTER	A= 2.6 × 6.9 = 17.9	0.925			48.5	48.5	
	CENT. CONT.	A= 2.0 × 1.5 = 3.0	0.905	0.73		10.3	8.3	
	WALL (S)	A= 3.0 × 8.4 = 25.2	0.028			0.7	0.7	
	WALL (W)	A= 3.0 × 7.6 = 22.8	0.038			0.8	0.8	
	CHR. FLOOR	A= 7.6 × 8.4 = 63.8	0.195			12.4	12.4	
	COLUMN					1.1	1.1	
						73.8	71.0	
						(207.5)	(198.9)	
FL+5.5 5 FL±0	SLAB	A= 63.8	1.025	0.975		65.4	63.2	
	WALL	A= 3.0 × 8.4 = 25.2	0.028			0.7	0.7	
	P.C. PANEL	A= 5.5 × 8.4 = 46.2	0.29			13.4	13.4	
	COLUMN					1.1	1.1	
						80.6	77.4	
						(288.1)	(276.3)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (25)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
K-108	FL+21.0	SLAB	$A=7.6 \times 10.0 =$	76.0	0.515	0.535	45.2	40.7
		PARAPET	$L=$	10.0	0.12		1.2	1.2
	FL+16.0	SIDING	$A=5.9 \times 10.0 =$	59.0	0.045		2.7	2.7
		COLUMN					1.0	1.0
							50.1	45.6
K-201	FL+16.0	SLAB	$A=$	76.0	0.88		66.9	66.9
		WALL	$A=3.0 \times 10.0 =$	30.0	0.028		0.8	0.8
	FL+11.0	SIDING	$A=5.0 \times 10.0 =$	50.0	0.045		2.3	2.3
		COLUMN					1.0	1.0
							71.0	71.0
							(121.7)	(116.6)
K-201	FL+11.0	SLAB	$A=$	76.0	0.705	0.73	68.8	55.5
		WALL	$A=$	30.0	0.028		0.8	0.8
	FL+5.5	CH. R. FLOOR	$A=$	76.0	0.195		14.8	14.8
		COLUMN					1.1	1.1
							85.5	72.2
							(206.6)	(188.8)
K-201	FL+21.0	SLAB	$A=$	76.0	1.025	0.975	77.9	74.1
		WALL	$A=3.0 \times (7.0 \times 2 + 10.0) =$	72.0	0.028		2.0	2.0
	FL+20	P.C. PANEL	$A=5.5 \times 10.0 =$	55.0	0.29		16.0	16.0
		COLUMN					1.1	1.1
							97.0	93.2
							(303.6)	(282.0)
K-201	FL+21.0	SLAB (CON)	$A=7.6 \times 6.5 =$	49.4	0.71	0.61	35.1	30.1
		SLAB	$A=2.6 \times 3.5 =$	26.6	0.88		23.4	23.4
	FL+16.0	WALL	$A=3.9 \times (16.0 + 7.0) =$	51.0	0.016		1.8	1.8
		SIDING	$A=5.0 \times 10.0 =$	50.0	0.045		2.3	2.3
	FL+11.0	COLUMN					1.0	1.0
							63.6	58.6
							(113.7)	(104.2)

NOTE : UW --- UNIT WEIGHT [單位荷重]								
COLUMN AXIAL LOAD (26)								
[柱軸力]								
PA --- FOR PERMANENT CONDITION [長期]								
SE --- FOR SEISMIC CONDITION [地震時]								
ML --- MACHINE LOAD [機械荷重]								
LOCA-TION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
(K-201)	FL+5.5	SLAB	$A=2.6 \times 5.0 =$	38.0	0.025	0.975	29.0	37.1
	FL±0	LAB	$A=$	58.9	0.855	0.705	32.5	26.8
		WALL	$A=3.0 \times (7.0 \times 2 + 11.0) =$	72.0	0.028		2.0	2.0
		P.C. PANEL	$A=5.5 \times 10.0 =$	55.0	0.29		16.0	16.0
		COLUMN					1.1	1.1
						90.6	83.0	
						(289.8)	(259.4)	
K-202	FL+21.0		SAME AS K-107				59.8	48.2
	FL+16.0	SLAB	$A=2.6 \times 8.9 =$	63.8	0.71	0.61	45.3	38.9
		WALL (S)	$A=3.0 \times 8.9 =$	25.2	0.028		0.7	0.7
		WALL (W)	$A=3.0 \times (7.0 \times 2 + 2) =$	41.4	0.036		1.5	1.5
		SIDING	$A=5.0 \times 8.9 =$	42.0	0.045		1.9	1.9
		COLUMN					1.0	1.0
							50.4	44.0
							(105.2)	(97.2)
	FL+11.0		SAME AS K-107				73.8	71.8
							(179.0)	(164.0)
	FL+5.5		SAME AS K-107					
	FL±0	SLAB	$A=2.6 \times 8.9 =$	63.8	0.855	0.705	54.5	45.0
		WALL	$A=3.0 \times 8.9 =$	25.2	0.028		0.7	0.7
		P.C. PANEL	$A=5.5 \times 8.9 =$	46.2	0.29		13.4	13.4
		COLUMN					1.1	1.1
							69.7	60.2
						(248.7)	(224.2)	
K-203	FL+21.0		SAME AS K-107				28.8	25.7
	FL+16.0		SAME AS K-107					
	FL+11.0	SLAB	$A=$	30.9	0.71	0.61	21.6	18.5
		WALL (S)	$A=3.0 \times (3.9 + 7.0) =$	31.2	0.028		0.9	0.9
		WALL (W)	$A=3.0 \times 3.9 =$	10.2	0.036		0.9	0.9
		SIDING	$A=5.0 \times 11.6 =$	58.0	0.045		2.6	2.6
		COLUMN					1.0	1.0
							26.5	23.9
							(55.3)	(49.1)
	FL+11.0	SLAB	$A=2.6 \times 9.0 =$	30.9	1.24	1.17	37.7	35.6
	FL+5.5	BLOCK WALL	$A=5.0 \times (7.0 \times 2 + 3.5 \times 2) =$	105.0	0.36		37.8	37.8
		WALL	$A=3.0 \times 7.0 =$	21.0	0.026		0.5	0.5
		COLUMN					1.1	1.1
							77.1	75.0
							(132.4)	(124.1)
FL±0	SLAB	$A=2.6 \times 9.0 =$	30.9	0.855	0.705	26.0	21.4	
	WALL	$A=3.0 \times (3.9 + 7.0) =$	31.2	0.028		0.9	0.9	
	P.C. PANEL	$A=5.5 \times (4.0 + 7.0) =$	63.8	0.29		18.5	18.5	
	COLUMN					1.1	1.1	
						46.5	41.9	
						(178.9)	(166.0)	

LD

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]
 COLUMN AXIAL LOAD () [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
		In case of	UNIT. I. only					
A-201	FL+27.1 S FL+21.0	SLAB	$A=11.6 \times 10.6 = 123.0$	0.595	0.525		73.2	65.8
		PARAPET	$L=11.6 \times 10.6 = 22.2$	0.12			2.7	2.7
		SIDING	$A=22.2 \times (1.8+0.9) + 8.5 \times 2.3 = 96.5$	0.095			4.3	4.3
		COLUMN	$L=6.1$	0.2			1.2	1.2
		TRUSS	$L=11.0$	0.15			2.8	2.8
							84.2	76.8
	FL+21.0 S FL+11.0	SIDING	$A=8.5 \times 10.0 = 85.0$	0.095			3.8	3.8
		COLUMN	$L=10.0$	0.25			2.5	2.5
		CRANE GIRDER	$L=5.0$	0.25			1.8	1.8
							8.1	8.1
							(92.3)	(84.9)
	FL+11.0 S FL+5.5	COLUMN	$L=5.5$	0.25			1.4	1.4
		SHUTTER	$A=5.0 \times 10.0 = 50.0$	0.1			5.0	5.0
		SIDING	$A=2.9 \times 5.5 = 16.0$	0.095			0.7	0.7
							7.1	7.1
						(99.4)	(92.0)	
FL+5.5 S FL±0	SIDING	$A=2.9 \times 5.5 = 16.0$	0.095			0.7	0.7	
	COLUMN					1.4	1.4	
						2.1	2.1	
						(101.5)	(94.1)	
B-201	FL+11.0 S FL+5.5	SIDING	$A=5.25 \times 14.3 = 75.1$	0.095			3.4	3.4
		COLUMN					1.1	1.1
							4.5	4.5
FL+5.5 S FL±0	SIDING	$A=5.25 \times 5.5 = 28.9$	0.095			1.3	1.3	
	COLUMN					1.1	1.1	
						2.4	2.4	
						(6.9)	(6.9)	
D-201	FL+11.0 S FL+5.5	SIDING	$A=6.0 \times 14.3 = 85.8$	0.095			3.9	3.9
		SLAB	$A=3.0 \times 5.6 = 16.8$	1.925	1.35		23.9	22.7
		HAND RAIL	$L=5.0$	0.035			0.2	0.2
		COLUMN	$L=5.5$	0.2			1.1	1.1
							29.1	27.9
FL+5.5 S FL±0	SLAB	$A=5.6 \times 3.6 = 20.2$	1.225	1.15		24.7	23.2	
	HAND RAIL	$L=5.0$	0.035			0.2	0.2	
	COLUMN					1.1	1.1	
						26.0	24.5	
						(55.1)	(52.4)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 [柱軸力] ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
F-201	FL+11.0	SIDING	$A = 5.75 \times 19.3 =$	111.0	0.045		5.0	5.0
	FL+5.5	SLAB	$A = 5.75 \times 5.6 =$	32.2	1.425	1.35	45.9	43.5
							1.1	1.1
							52.0	49.6
F-201	FL+5.5	SLAB	$A =$	32.2	1.225	1.15	39.4	37.0
	FL±0	SIDING	$A = 5.75 \times 5.5 =$	31.6	0.045		1.4	1.4
		COLUMN					1.1	1.1
							41.9	39.5
							(93.9)	(89.1)
G-201	FL+27.1		SAME AS A-201				87.2	76.8
	FL±0							
G-201	FL+21.0	SLAB	$A = 5.0 \times 10.0 =$	50.0	0.665	0.565	33.3	28.3
	FL+16.0	PARAPET	$L =$	10.0	0.12		1.2	1.2
		SIDING	$A = 5.0 \times (2.2 + 9.9) =$	33.0	0.045		1.5	1.5
		CRANE GIRDER	$L =$	5.0	0.35		1.8	1.8
		COLUMN	$L =$	5.0	0.25		1.3	1.3
						39.1	34.1	
						(123.3)	(110.9)	
G-201	FL+16.0	SLAB	$A = 5.0 \times 10.0 =$	50.0	0.85		42.5	42.5
	FL+11.0	BLOCK WALL	$A = 5.0 \times 10.0 =$	50.0	0.4		20.0	20.0
		COLUMN					1.3	1.3
	FL+11.0	SIDING	$A =$	33.0	0.045		1.5	1.5
							65.3	65.3
							(188.6)	(176.2)
G-201	FL+11.0	SLAB	$A = 5.0 \times 10.0 + 2.8 \times 5.0 =$	69.0	1.425	1.35	91.2	86.9
	FL+5.5	SIDING	$A = 5.5 \times (2.2 + 9.9) =$	36.3	0.045		1.6	1.6
		COLUMN	$L =$	5.5	0.25		1.4	1.4
						99.2	89.9	
							(282.8)	(265.6)
G-201	FL+5.5	SLAB	$A =$	69.0	1.225	1.15	70.9	73.6
	FL±0	SIDING					1.6	1.6
		COLUMN					1.4	1.4
						81.4	76.6	
							(369.2)	(342.2)

NOTE : UW --- UNIT WEIGHT [單位荷重]

PA --- FOR PERMANENT CONDITION [長期]

SE --- FOR SEISMIC CONDITION [地震時]

ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (3)

[柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-20Z	FL+21.0	SLAB	$A=5.0 \times 8.4 = 42.0$	0.665	0.565		27.9	23.7
		PARAPET	$L=8.4$	0.12			1.0	1.0
	FL+16.0	SIDING	$A=5.9 \times 8.4 = 49.6$	0.095			2.2	2.2
		COLUMN	$L=5.0$	0.25			1.3	1.3
						32.4	28.2	
FL+16.0	S	SLAB	$A=5.0 \times 8.4 = 42.0$	0.85			35.7	35.7
		BLOCK WALL	$A=5.0 \times (5.0 + 5.0) = 50.0$	0.4			20.0	20.0
	FL+11.0	COLUMN					1.3	1.3
		SIDING	$A=5.0 \times 8.4 = 42.0$	0.095			1.9	1.9
						58.9	58.9	
						(91.3)	(87.1)	
FL+11.0	S	SLAB	$A=5.6 \times 8.4 = 47.0$	1.925	1.25		67.0	63.5
		COLUMN	$L=5.5$	0.25			1.4	1.4
	FL+5.5	SIDING	$A=5.5 \times 8.4 = 46.2$	0.095			2.1	2.1
							70.5	67.0
						(161.8)	(154.1)	
FL+5.5	S	SLAB	$A=$	1.225	1.15		57.6	54.1
		COLUMN					1.4	1.4
	FL±0	SIDING					2.1	2.1
						61.1	57.6	
						(222.9)	(211.7)	

		NOTE : UW --- UNIT WEIGHT [單位荷重]							
		PA --- FOR PERMANENT CONDITION [長期]							
		SE --- FOR SEISMIC CONDITION [地震時]							
		ML --- MACHINE LOAD [機械荷重]							
COLUMN AXIAL LOAD (4)		[柱軸力]							
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-203	FL+21.0 s FL+16.0	SLAB	$A=5.6 \times 9.0 =$	22.4	0.665	0.565	14.9	12.7	
		PARAPET	$L=5.6 \times 4.0 =$	9.6	0.12		9.7	9.7	
		SIDING	$A=9.6 \times (5.0 + 0.9) =$	56.6	0.045		2.5	2.5	
		COLUMN	$L=$	7.0	0.25		1.8	1.8	
							28.9	26.7	
		FL+16.0 s FL+11.0	SLAB	$A=$	22.4	1.29	0.665	28.9	19.9
			SIDING	$A=9.6 \times 5.0 =$	48.0	0.045		2.2	2.2
			COLUMN	$L=$	5.0	0.25		1.3	1.3
							32.4	18.4	
							(61.3)	(45.1)	
		FL+11.0 s FL+5.5	SLAB	$A=$	22.4	1.425	1.35	31.9	30.2
			SIDING	$A=$	48.0	0.045		2.2	2.2
		COLUMN	$L=$				1.4	1.4	
						35.5	33.8		
						(96.8)	(78.9)		
	FL+5.5 s FL±0	SLAB	$A=$	22.4	1.225	1.15	27.4	25.8	
		SIDING	$A=9.6 \times 5.5 =$	52.8	0.045		2.4	2.4	
		COLUMN	$L=$	5.5	0.25		1.4	1.4	
						31.2	29.6		
						(128.0)	(108.5)		
H-203	FL+21.0 s FL+16.0	SLAB (ROOF)	$A=5.0 \times 9.0 =$	20.0	0.635	0.535	12.7	10.7	
		SLAB (TONER)	$A=9.6 \times 4.0 =$	30.4	0.765	0.665	23.3	20.2	
		FENCE	$L=3.9 \times 7.6 =$	11.0	0.11		1.2	1.2	
		PARAPET	$L=$	12.0	0.12		1.4	1.4	
		SIDING	$A=5.0 \times 12.0 =$	60.0	0.045		2.7	2.7	
		COLUMN	$L=$	5.0	0.2		1.0	1.0	
							42.3	37.2	
		FL+16.0 s FL+11.0	SLAB (CON)	$A=5.0 \times 3.4 =$	17.0	0.71	0.61	12.1	10.4
			SLAB (CLAV)	$A=7.0 \times 3.4 =$	23.8	0.79	0.72	18.8	17.1
			Block WALL	$A=5.0 \times (7.0 \times 2 + 3.0 \times 5) =$	155.0	0.4		62.0	62.0
			SIDING	$A=5.0 \times 12.0 =$	60.0	0.045		2.7	2.7
			COLUMN	$L=$				1.0	1.0
							96.6	93.2	
							(138.9)	(130.2)	
		FL+11.0 s FL+5.5	SLAB	$A=7.0 \times 4.0 =$	28.0	0.905	0.73	25.3	20.4
			SLAB	$A=5.0 \times 4.0 =$	20.0	1.425	1.35	28.5	27.0
			Block WALL	$A=5.0 \times (3.4 \times 2 + 9.0) =$	79.0	0.35		27.7	27.7
			COLUMN	$L=$				1.1	1.1
			SIDING	$A=5.5 \times 12.0 =$	66.0	0.045		3.0	3.0
							85.6	79.2	
							(224.5)	(209.4)	
		FL+5.5 s FL±0	SLAB	$A=5.0 \times 4.0 =$	20.0	1.225	1.15	24.5	23.0
			STAIR	$A=2.0 \times 7.0 =$	14.0	0.43	0.36	6.0	5.0
			Block WALL	$A=5.0 \times (8.0 + 4.0 \times 2) =$	30.0	0.4		32.0	32.0
		SIDING	$A=5.5 \times 5.5 =$	30.3	0.045		1.4	1.4	
		COLUMN	$L=$	5.5	0.2		1.1	1.1	
		P.C PANEL	$A=6.4 \times 5.5 =$	35.2	0.29		10.2	10.2	
						75.2	72.7		
						(299.7)	(282.1)		

1.2 SEISMIC LOAD
[地震荷重]

ITEM		CALCULATION									
ZONE FACTOR (Z)		Z = 1.0									
STANDARD SHEAR COEFFICIENT (Co)		Co = 0.1									
GROUND CONDITION (Tc)		Tc = 0.8									
		Hard		Tc = 0.4		<input type="checkbox"/>					
		Medium		Tc = 0.6		<input type="checkbox"/>					
		Soft		Tc = 0.8		<input checked="" type="checkbox"/>					
DIRECTION		X DIRECTION (LONG SPAN)					Y DIRECTION (SHORT SPAN)				
NATURAL PERIOD OF BUILDING (T)		T = 0.39					T = 0.57				
Heigh h= 27.1 m		Length of Span D= m					Length of Span D= 32.0 m				
T=0.03*h		=					=				
T=0.05*h/4√D		=					= 0.57				
T=h/70		= 0.39					=				
CHARACTERISTICS OF VIBRATION OF THE BUILDING (Rt)		Rt = 1.0					Rt = 1.0				
		T		Rt		T		Rt			
Rt=1		-		=		-		=			
Rt=1-0.2*(T/Tc-1) ²		Tc		-		-		-			
		2*Tc		-		-		-			
Rt=1.6*Tc/T		=		=		=		=			
2*T/(1+3*T)		= 0.359					= 0.421				
SEISMIC LOAD FOR EACH FLOOR (Qi)											
STORY		Wi	α i	Ai	Ci	Qi	Wi	α i	Ai	Ci	Qi
RF		1.638.4	0.11	2.04	0.20	327.7		0.11	2.22	0.22	360.4
5TH		2625.6	0.25	1.63	0.16	576.9		0.25	1.74	0.17	613.0
4TH		5537.1	0.38	1.45	0.15	830.1		0.38	1.52	0.15	830.1
OPE		9779.7	0.67	1.20	0.12	1173.6		0.67	1.23	0.12	1173.6
MEZ		14581.5	1.00	1.00	0.1	1458.2		1.00	1.00	0.1	1458.2
NOTE:		α i = Wi/Σ W									
		Ai = 1 + (1/√ α i - α i)*2*T/(1 + 3*T)									
		Ci = Z*Rt*Ai*Co									

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+27.1~21.0								
	A	B	D	F	G	H	K	TOTAL
101	33.9				33.9			67.8
102	49.1				49.1			98.2
103	49.1				49.1			98.2
104	49.1				49.1			98.2
105	49.1				49.1			98.2
106	49.1				49.1			98.2
107	59.7				59.7			119.4
108	70.5				70.5			141.0
201	70.5				70.5			141.0
202	59.7				59.7			119.4
203	49.1				49.1			98.2
204	49.1				49.1			98.2
205	49.1				49.1			98.2
206	49.1				49.1			98.2
207	49.1				49.1			98.2
208	33.9				33.9			67.8
TOTAL	819.2				819.2			1,638.4
FL+21.0~16.0								
	A	B	D	F	G	H	K	TOTAL
101	40.9				50.1	27.2		118.2
102	57.1				74.4	26.9		158.4
103	57.1				143.1	95.9		296.1
104	57.1				143.1	95.9		296.1
105	57.1				74.4	26.9		158.4
106	57.1				71.8	51.6	25.7	206.2
107	68.9				87.4	63.5	48.2	268.0
108	81.0				103.3	75.5	45.6	305.4
201	81.0				103.3	75.5	45.6	305.4
202	68.9				87.4	63.5	48.2	268.0
203	57.1				71.8	42.5	25.7	197.1
204	57.1				74.4	27.4		158.9
205	57.1				143.1	95.9		296.1
206	57.1				143.1	95.9		296.1
207	57.1				74.4	26.9		158.4
208	40.9				50.7	27.2		118.8
TOTAL	952.6				1,495.8	918.2	239.0	3,605.6

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+16.0~11.0								
	A	B	D	F	G	H	K	TOTAL
101	40.9				66.4	38.5		145.8
102	57.1				103.8	55.0		215.9
103	57.1				172.5	124.0		353.6
104	57.1				179.5	136.5		373.1
105	57.1				110.8	67.5		235.4
106	57.1				95.7	116.9	57.0	326.7
107	68.9				143.7	188.4	127.1	528.1
108	81.0				173.9	227.4	116.6	598.9
201	81.0				173.9	215.6	104.2	574.7
202	68.9				150.7	196.6	92.2	508.4
203	57.1				95.7	147.3	49.1	349.2
204	57.1				110.8	68.0		235.9
205	57.1				179.5	136.5		373.1
206	57.1				172.5	124.0		353.6
207	57.1				103.8	55.0		215.9
208	40.9				66.4	38.5		145.8
TOTAL	952.6				2,099.6	1,935.7	546.2	5,534.1
FL+11.0~5.5								
	A	B	D	F	G	H	K	TOTAL
101	51.4	21.3	47.4	40.9	109.7	47.7		318.4
102	74.0	29.7	56.3	50.1	169.1	110.5		489.7
103	74.0	19.0		29.9	237.8	179.5		540.2
104	68.5	13.0		14.6	259.2	189.0		544.3
105	69.5	14.0		14.6	190.5	120.0		408.6
106	75.0	20.0		16.0	157.6	206.7	93.0	568.3
107	89.4	31.7	29.9	35.9	206.6	348.0	198.9	940.4
108	108.7	20.9	64.6	63.0	237.3	396.6	188.8	1,079.9
201	108.7	20.9	64.6	63.0	237.3	365.6	176.4	1,036.5
202	89.4	31.7	29.9	35.9	207.0	356.2	164.0	914.1
203	75.0	20.0		21.5	161.0	240.1	124.1	641.7
204	69.5	14.0		14.6	176.1	123.5		397.7
205	68.5	13.0		14.6	259.2	189.0		544.3
206	74.0	19.0		27.1	256.9	176.5		553.5
207	74.0	29.7	56.3	50.1	166.1	107.5		483.7
208	51.4	21.3	47.4	40.9	109.7	47.7		318.4
TOTAL	1,221.0	339.2	396.4	532.7	3,141.1	3,204.1	945.2	9,779.7

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+5.5 ~ ±0

	A	B	D	F	G	H	K	TOTAL
101	76.8	55.0	85.7	77.7	159.2	72.2		526.6
102	108.5	71.9	110.5	106.2	234.1	169.2		800.4
103	108.5	45.9		62.6	302.8	238.2		758.0
104	85.1	17.6		22.2	306.7	262.7		694.3
105	86.1	18.6		22.2	241.5	193.7		562.1
106	116.4	62.8		48.7	223.1	301.2	143.1	895.3
107	131.7	86.1	81.7	95.3	295.2	495.4	276.3	1,461.7
108	123.9	57.3	145.8	141.1	337.1	557.6	282.0	1,644.8
201	123.9	49.3	135.0	132.2	327.8	525.9	259.4	1,553.5
202	131.7	86.1	81.7	95.3	277.7	487.8	224.2	1,386.5
203	116.4	62.8		54.2	226.5	334.6	166.0	960.5
204	86.1	18.6		22.2	227.1	197.2		551.2
205	85.1	17.6		22.2	306.7	262.7		694.3
206	108.5	45.9		59.8	321.9	235.2		771.3
207	108.5	71.9	110.5	106.2	231.1	166.2		794.4
208	76.8	55.0	85.7	77.7	159.2	72.2		526.6
TOTAL	1,674.0	822.4	836.6	1,145.8	4,179.7	4,572.0	1,357.0	14,581.5

	A	B	D	F	G	H	K	TOTAL
101								
102								
103								
104								
105								
106								
107								
108								
201								
202								
203								
204								
205								
206								
207								
208								
TOTAL								

SEISMIC LOAD
[地震荷重]

UNIT-1 only.

ITEM		CALCULATION									
ZONE FACTOR (Z)		Z = 1.0									
STANDARD SHEAR COEFFICIENT (Co)		Co = 0.1									
GROUND CONDITION (Tc)		Tc = 0.8									
		Hard		Tc = 0.4						<input type="checkbox"/>	
		Medium		Tc = 0.6						<input type="checkbox"/>	
		Soft		Tc = 0.8						<input checked="" type="checkbox"/>	
DIRECTION		X DIRECTION (LONG SPAN)					Y DIRECTION (SHORT SPAN)				
NATURAL PERIOD OF BUILDING (T)		T = 0.39					T = 0.57				
Heigh h= 27.1 m		Length of Span D= m					Length of Span D= 32.0 m				
T=0.03*h		=					=				
T=0.05*h/4√D		=					= 0.57				
T=h/70		= 0.39					=				
CHARACTERISTICS OF VIBRATION OF THE BUILDING (Rt)		Rt = 1.0					Rt = 1.0				
		T		Rt			T		Rt		
Rt=1		-		=			-		=		
Rt=1-0.2*(T/Tc-1) ²		Tc		-			Tc		-		
Rt=1.6*Tc/T		2*Tc		-			2*Tc		-		
2*T/(1+3*T)		= 0.359					= 0.421				
SEISMIC LOAD FOR EACH FLOOR (Qi)											
	STORY	Wi	α i	Ai	Ci	Qi	Wi	α i	Ai	Ci	Qi
	RF	972.8	0.10	2.10	0.21	204.3		0.10	2.29	0.23	223.7
	5TH	2,353.2	0.23	1.67	0.17	400.0		0.23	1.78	0.18	423.6
	4TH	3,958.2	0.39	1.43	0.14	554.1		0.39	1.51	0.15	593.7
	0PE	6,958.1	0.68	1.19	0.12	835.0		0.68	1.22	0.12	835.0
	MEZ	10,193.5	1.00	1.00	0.1	1,019.4		1.00	1.00	0.1	1,019.4
NOTE:		α i = Wi/Σ W									
		Ai = 1 + (1/√ α i - α i)*2*T/(1 + 3*T)									
		Ci = Z*Rt*Ai*Co									

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+27.1~21.0								
	A	B	D	F	G	H	K	TOTAL
101	33.9				33.9			67.8
102	49.1				49.1			98.2
103	49.1				49.1			98.2
104	49.1				49.1			98.2
105	49.1				49.1			98.2
106	49.1				49.1			98.2
107	59.7				59.7			119.4
108	70.5				70.5			141.0
201	76.8				76.8			153.6
202								
203								
204								
205								
206								
207								
208								
TOTAL	486.4				486.4			972.8
FL+21.0~16.0								
	A	B	D	F	G	H	K	TOTAL
101	40.9				50.1	27.2		118.2
102	57.1				74.4	26.9		158.4
103	57.1				143.1	95.9		296.1
104	57.1				143.1	95.9		296.1
105	57.1				74.4	26.9		158.4
106	57.1				71.8	51.6	25.7	206.2
107	68.9				87.4	63.5	48.2	268.0
108	81.0				103.3	75.5	45.6	305.4
201	84.9				110.9	75.5	45.6	316.9
202					28.2	63.5	48.2	139.9
203					26.7	37.2	25.7	89.6
204								
205								
206								
207								
208								
TOTAL	561.2				913.4	639.6	239.0	2353.2

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+16.0~11.0								
	A	B	D	F	G	H	K	TOTAL
101	40.9				66.4	38.5		145.8
102	57.1				103.8	55.0		215.9
103	57.1				172.5	124.0		353.6
104	57.1				179.5	136.5		373.1
105	57.1				110.8	67.5		235.4
106	57.1				95.7	116.9	57.0	326.7
107	68.9				143.7	188.4	127.1	528.1
108	81.0				173.9	227.4	116.1	598.4
201	84.9				176.2	215.6	104.2	580.9
202					87.1	196.6	92.2	375.9
203					45.1	130.2	49.1	224.4
204								
205								
206								
207								
208								
TOTAL	561.2				1,354.7	1,496.6	545.7	3,958.2
FL+11.0~5.5								
	A	B	D	F	G	H	K	TOTAL
101	51.4	21.3	47.4	40.9	109.7	47.7		318.4
102	74.0	29.7	56.3	50.1	169.1	110.5		489.7
103	74.0	19.0		29.9	237.8	179.5		540.2
104	68.5	13.0		19.6	259.2	189.0		544.3
105	69.5	14.0		19.6	190.5	120.0		408.6
106	75.0	20.0		16.0	157.6	206.7	93.0	568.3
107	89.4	31.7	29.9	35.9	206.6	348.0	198.9	940.4
108	108.7	20.9	69.6	63.0	237.3	396.6	188.8	1,079.9
201	92.0	4.5	27.9	49.6	265.6	365.6	176.4	981.6
202					154.1	356.2	164.0	674.3
203					78.9	209.4	124.1	412.4
204								
205								
206								
207								
208								
TOTAL	702.5	174.1	226.1	314.6	2,066.4	2,529.2	945.2	6,958.1

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+5.5~10

	A	B	D	F	G	H	K	TOTAL
101	76.8	55.0	85.7	77.7	159.2	72.2		526.6
102	108.5	71.9	110.5	106.2	234.1	169.2		800.4
103	108.5	45.9		62.6	302.8	238.2		758.0
104	85.1	17.6		22.2	306.7	262.7		694.3
105	86.1	18.6		22.2	241.5	193.7		562.1
106	116.4	62.8		48.7	223.1	301.2	143.1	895.3
107	131.7	86.1	81.7	95.3	295.2	495.4	276.3	1461.7
108	123.9	57.3	145.8	141.1	337.1	557.6	282.0	1644.8
201	94.1	6.9	52.4	89.1	342.2	525.9	259.4	1370.0
202					211.7	487.8	224.2	923.7
203					108.5	282.1	166.0	556.6
204								
205								
206								
207								
208								
TOTAL	931.1	422.1	476.1	665.1	2,762.1	3,586.0	1,357.0	10,193.5
	A	B	D	F	G	H	K	TOTAL
101								
102								
103								
104								
105								
106								
107								
108								
201								
202								
203								
204								
205								
206								
207								
208								
TOTAL								

ITEM	NOTE: K1 --- HORIZONTAL STIFFNESS OF EACH FRAME AT EACH FLOOR Q1 --- SEISMIC SHEAR FORCE OF EACH FRAME AT EACH FLOOR (ton) P1 --- SEISMIC LOAD OF EACH FRAME AT EACH FLOOR (ton) Σ --- SUMMATION OF ALL FRAMES AT EACH FLOOR										REMARKS		
	FLOOR X DIRECTION (LONG SPAN)			Y DIRECTION (SHORT SPAN)			Σ Qi/Σ K			Σ Qi/Σ K			
	Σ Ki	Σ Qi	Σ Pi	Σ Ki	Σ Qi	Σ Pi	Σ Ki	Σ Qi	Σ Pi	Σ Qi/Σ K	Σ Pi/Σ K	Σ Qi/Σ K	Σ Pi/Σ K
DISTRIBUTION OF SEISMIC LOAD [地震力の架組への配分]													
WHOLE FRAME													
RF	1156.8	327.7			0.283			360.4					
5TH	2290.4	576.9			0.252		4263.8	613.0		252.6		0.194	
4TH	2871.8	830.1			0.289		4537.6	830.1		217.1		0.183	
OPE	3862.2	1173.6			0.304		5764.8	1173.6		343.5		0.204	
MEZ	3765.4	1458.2			0.387		3807.2	1458.2		284.6		0.383	
LINE DIRECTION	A	B	D	F	G	H	K						
RF	578.4				578.4								
5TH	710.4					1022.4	577.6			516.5		355.8	
4TH	861.0					1299.2	711.6			273.8		355.8	355.8
OPE	785.0	662.0		662.0		1140.6	612.6			429.8	324.6	235.7	171.8
MEZ	649.6	453.6		453.6		1086.6	212.2			469.3	295.9	181.9	295.9
LINE DIRECTION													
RF	163.7				163.7					14.9	21.6	21.6	21.6
5TH	179.0					260.2	138.0			51.2		74.4	55.8
4TH	248.8					375.5	205.7			50.1	50.1	65.1	65.1
OPE	238.6	201.2		201.2		346.7	186.2			87.7	66.2	83.1	48.1
MEZ	257.4	175.5		175.5		420.5	82.1			179.7	113.3	50.5	158.8
RF													
5TH													
4TH													
OPE													
MEZ													

ITEM	FLOOR	X DIRECTION (LONG SPAN)										Y DIRECTION (SHORT SPAN)										REMARKS	
		Σ Ki	Σ Qi	Σ Pi	Σ Qi/Σ K	Σ Ki	Σ Qi	Σ Pi	Σ Qi/Σ K	Σ Ki	Σ Qi	Σ Pi	Σ Qi/Σ K	Σ Ki	Σ Qi	Σ Pi	Σ Qi/Σ K						
NOTE: Ki --- HORIZONTAL STIFFNESS OF EACH FRAME AT EACH FLOOR Qi --- SEISMIC SHEAR FORCE OF EACH FRAME AT EACH FLOOR (ton) Pi --- SEISMIC LOAD OF EACH FRAME AT EACH FLOOR (ton) Σ --- SUMMATION OF ALL FRAMES AT EACH FLOOR																							
DISTRIBUTION OF SEISMIC LOAD [地震力の架構への配分] UNIT-1cm/y																							
WHOLE FRAME	RF	578.4	204.3		0.353							223.7											
	5TH	1,692.8	400.0		0.236							423.6				0.147							
	4TH	2,117.7	554.1		0.262							593.7				0.203							
	OPE	2,491.9	835.0		0.335							835.0				0.220							
	MEZ	2,548.0	1,019.4		0.400							1,019.4				0.411							
LINE																							
X DIRECTION		A	B	D	F	G	H	K				101	102	103	104	105	106	107	108	201	202	203	
Ki	RF	289.2				289.2																	
	5TH	355.2					790.0	547.6				516.5						302.3	355.8			355.8	387.3
	4TH	430.5					975.6	711.6				355.8						298.0				355.8	298.0
	OPE	392.5	331.0				824.8	612.6				407.5	235.7					521.3	171.8			556.0	349.5
	MEZ	324.8	226.8				603.0	212.2				131.9						414.7	295.9			245.9	283.8
LINE																							
Y DIRECTION																							
Qi	RF	102.2				102.2																	
	5TH	83.8					186.4	129.2				75.9						56.9	52.3			52.3	56.9
	4TH	112.8					255.6	186.4				72.2						60.5					60.5
	OPE	131.5	110.9				276.3	205.2				87.7	51.9					114.7	37.8				76.9
	MEZ	129.9	90.7				349.8	84.9				192.9	121.6					170.4	121.6				116.2

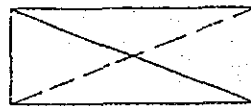
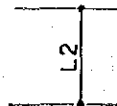
HORIZONTAL STIFFNESS OF VERTICAL BRACE
 [鉛直ブレースの水平剛性]

LINE	FLOOR NUMBER	TYPE	MEMBER SIZE	AREA (cm ²)	L1 (cm)	L2 (cm)	L3 (cm)	J (t/cm)
Y Direction								
	5TH	K	H-300 ²	119.8	500	500	707	355.8
		K	H-350 ²	173.9	500	500	707	516.5
		K	H-300 ²	119.8	700	500	860	387.3
	4TH	K	H-250 ²	92.18	500	500	707	273.8
		K	H-300 ²	119.8	500	500	707	355.8
		K	H-250 ²	92.18	700	500	860	298.0
	OPE	N	2L-130x12	59.52	450	550	711	70.5
		X	2L-120x8	37.52	450	550	711	88.9
		X	2L-120x8	37.52	550	550	778	101.3
		X	2L-120x8	37.52	600	550	814	105.2
		K	H-250 ²	92.18	500	550	743	235.7
		K	H-300 ²	119.8	700	550	890	349.5
	MEZ	N	H-200 ²	63.53	450	650	791	54.7
		X	2L-120x8	37.52	450	650	791	64.6
		X	2L-120x8	37.52	550	650	851	77.2
		X	2L-120x8	37.52	600	650	885	82.0
		K	H-300 ²	119.8	500	650	820	228.1
		K	H-300 ²	119.8	700	650	955	282.8

NOTE:



TYPE : K



TYPE : X or N

$$L3 = \sqrt{L1^2 + L2^2}$$

(Length of Brace)

$$J = 2 * A * E * L1^2 / L3^3$$

(TYPE : K or X)

$$J = A * E * L1^2 / L3^3$$

(TYPE : N)

E -- Young Modulus (t/cm²)
(2100 for steel)

2. Design of crane support girder

(1) Design Condition

• Dead load : 45.0 ton (including 16.5 ton trolley)

Main hook : 60.0 ton

Aux hook : 15.0 ton

• The beam for the crane girder will be a simple beam each for 6.8 m span and 10.0 m span.

• Maximum wheel load (P)

Due to the previous calculation, the maximum wheel load is 25.2 ton/wheel

• Stress of Girder

a. 6.8 m span

$$M_{x1} = 67.3 \text{ t}\cdot\text{m}, \quad M_y = 5.6 \text{ t}\cdot\text{m}, \quad N = 8.9 \text{ t}$$

$$M_{x2} = \frac{1}{8} \times 0.36 \times 6.8^2 = 2.1 \text{ T}\cdot\text{m}$$

$$\text{Girder D.L } 306.9 \text{ kg/m}$$

$$\text{Rail D.L } 50$$

$$\hline 356.9 \text{ kg/m}$$

b. 10.0 m span

$$M_{x1} = 155.1 \text{ t}\cdot\text{m}, \quad M_y = 12.9 \text{ t}\cdot\text{m}, \quad N = 10.1 \text{ t}$$

$$M_{x2} = \frac{1}{8} \times 0.522 \times 10^2 = 6.5 \text{ T}\cdot\text{m}$$

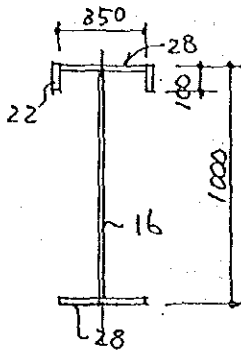
$$\text{Girder D.L } 472 \text{ kg/m}$$

$$\text{Rail D.L } 50$$

$$\hline 522 \text{ kg/m}$$

(2) Design of cross section

a. 6.8 m span



$$A = 391.0 \text{ cm}^2, \quad i_x = 40.9 \text{ cm}, \quad i_y = 13.3 \text{ cm}$$

$$Z_c = 14,571 \text{ cm}^3, \quad I_x = 654,677 \text{ cm}^4$$

$$Z_t = 11,888 \text{ cm}^3, \quad i_b = 12.4 \text{ cm}$$

$$Z_y = 1,282 \text{ cm}^3, \quad I_y = 25,267 \text{ cm}^4$$

$$\lambda_b = \frac{l_b}{i_b} = 55, \quad f_b = 1.6 \text{ t/cm}^2$$

$$\lambda_c = \frac{l_c}{i_y} = 51.1, \quad f_c = 1.37 \text{ t/cm}^2$$

$$\sigma_{bxt} = \frac{67.3 + 2.1}{11,888} \times 10^2 = 0.58 \text{ t/cm}^2$$

$$\sigma_{bxc} = \frac{67.3 + 2.1}{14,571} \times 10^2 = 0.48 \text{ t/cm}^2$$

$$\sigma_{by} = \frac{5.6 \times 10^2}{1,282} = 0.44 \text{ t/cm}^2$$

$$\sigma_t = \sigma_c = \frac{8.9/2}{391.0} = 0.01 \text{ t/cm}^2$$

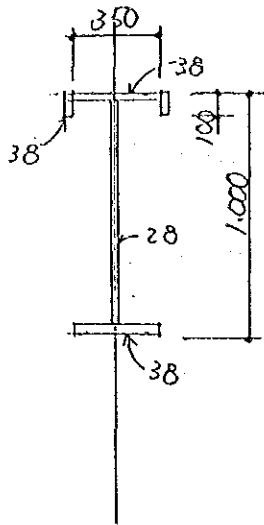
$$\text{Compression side} = \frac{0.48}{1.6} + \frac{0.44}{1.6} + \frac{0.01}{1.37} = 0.58 < 1.0$$

$$\text{Tension side} = \frac{0.58}{1.6} + \frac{0.01}{1.6} = 0.37 < 1.0$$

$$\text{Deflection} = \delta = \frac{5 \times 0.15 \times 680^4}{384 \times 2100 \times 654,677} = 0.3 \text{ cm}$$

$$\frac{\delta}{l} = \frac{0.3}{680} = \frac{1}{2266} < \frac{1}{1000}$$

b. 10 m span



$$A = 600.7 \text{ cm}^2, \quad i_x = 39.4 \text{ cm}, \quad i_y = 14.2 \text{ cm}$$

$$Z_c = 21091 \text{ cm}^3, \quad I_x = 934.560 \text{ cm}^4$$

$$Z_t = 16781 \text{ cm}^3, \quad i_b = 13.1 \text{ cm}$$

$$Z_y = 1985 \text{ cm}^3, \quad I_y = 42.295 \text{ cm}^4$$

$$\lambda_b = \frac{l_b}{i_b} = 76, \quad f_b = 1.6 \text{ t/cm}^2$$

$$\lambda_c = \frac{l_c}{i_y} = 70.4, \quad f_c = 1.2 \text{ t/cm}^2$$

$$\sigma_{bxt} = \frac{155.1 + 6.5}{16781} \times 10^2 = 0.96 \text{ t/cm}^2$$

$$\sigma_{bxc} = \frac{155.1 + 6.5}{21091} \times 10^2 = 0.77 \text{ ''}$$

$$\sigma_{by} = \frac{12.9 \times 10^2}{1985} = 0.65 \text{ ''}$$

$$\sigma_z = \sigma_c = \frac{10.1/2}{600.7} = 0.008 \text{ ''}$$

$$\text{Compression side} : \frac{0.77}{1.6} + \frac{0.65}{1.6} + \frac{0.008}{1.2} = 0.89 < 1.0$$

$$\text{Tension side} : \frac{0.96}{1.6} + \frac{0.008}{1.6} = 0.61 < 1.0$$

$$\text{Deflection} : \delta = \frac{5 \times 0.1 \times 1000^4}{384 \times 2100 \times 934.560} = 0.66 \text{ cm}$$

$$\frac{\delta}{l} = \frac{0.66}{1000} = \frac{1}{1515} < \frac{1}{1000}$$