	TABLE OF	COLUMN	LOAD FO	OR EACH	FLCOR	(SEIS MIC	CONDITIO	N)
FL+5	.5 ~ ±0				· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·		
	A	В	D	F	G	H.	К	TOTAL
				·				
101	26.8	55.0	85.7	77.7	159.2	72.2		526.6
102	108.5	71.9	110.5	106.2	234.1	169.2		800.4
103	108.5	45.9		62.6	302.8	238.2		258.0
104	85.1	17.6		22.2	306.7	262.7		694.3
105	86.1	18.6		<i>22</i> . Z	241.5	193.7		562.1
106	116.4	62,8		48.7	273.1	30/-2	143.1	895.3
107	131.7	86.1	81.7	95.3	295.2	495.4	276-3	1.461.7
108	123.9	57.3	145.8	141.1	337.1	557.6	282.0	1,644.8
201	123.9	49.3	135.0	132.2	327.8	525.9	259.4	1.553.5
202	131.7	86-1	81.7	95.3	279.7	487.8	224,2	1.386.5
203	116.4	62.8	,	54.2	226.5	334.6	166.0	960.5
204	86.1	18.6		22.2	227.1	197.2		551.2
205	85.1	17.6		22.2	306.7	262.7		694.3
206	108.5	45.9		59.8	321.9	235.2		171.3
207	108.5	71.9	110,5	106.Z	231,1	166.2		794.4
208	76.8	55.0	85.7	77.7	159.2	72.2		526.6
TOTAL	1.674.0	822.4	836.6	1.145.8	4.179.7	4,572.0	1,357.0	14,581.5
				,				
	Α	В	D	F	G	H	K	TOTAL
								<u> </u> .
101			-	<u></u>		· ·		
102							<u> </u>	
103						<u> </u>	ļ	
104								
105							<u> </u>	<u> </u>
106						<u> </u>	<u> </u>	ļ
107						ļ		
108					ļ		<del>                                     </del>	
201								<u> </u>
202					,	-		<u> </u>
203							ļ	<u> </u>
204						ļ	<u> </u>	1
205		<i>i</i>				<u> </u>		<u> </u>
206_								<u> </u>
207						<u> </u>		
208			1 1			ļ	ļ	
					<u> </u>	ļ		
TOTAL					<u></u>		<u> </u>	<u> </u>

SEISMIC	LOAD		÷	•			144				
[地震荷里 ITEM	<u> </u>			*··	CA	LCULA		NIT-	1 only	<u>/</u>	
ZONE FACTOR (Z	)					Z = 1	.0				
STANDARD SHEAR COEFFICIENT (C						Co =	0.1			: * .	
GROUND CONDITI	ON (Tc)					Tc = ,	o. 8				
	Hard					Tc =	0.4				
	Medium			·		Tc =		Δ	1 2 32		
	Soft					Tc =	0.8	<u> </u>	TDDOM	TON	
DIRECTION		,		IRECT LONG		PAN)	1	5H 0	IRECT		PAN)
NATURAL PERIOD OF BUILDING (T)				T = 0		2 743. 7			T = o.		
Heigh h= . T=0.03*h T=0.05*h/		Leng	th of	Span	D=	m	Leng = = 0.		Span	D=3≥	,0 m
T=h/70	T.W	= 0.3	9				=				
CHARACTERISTIC VIBRATION OF THE BUILDING (			1-0			Rt =	1.0	:			
			T		Rt			T	ļ	Rt	
Rt=1		Tc		. <del> </del>			Tc			<u>-</u>	
Rt=1-0.2*(T/Tc		2*Tc	<u>.                                 </u>	=	<u>-</u>		2*Tc	<u> </u>	<b></b>		
Rt=1.6*Tc/T	*******			=		· · · · · · · · · · · · · · · · · · ·			=		
2*T/(	1+3*T)	= 0.3	s59		•		= 0.	421			
SEISMIC LOAD FOR EACH FLOOR (Qi)				:							
	STORY	Wi	αi	Ai	Ci	Qi	Wi	αi	Ai	Ci	Qi
	RF	972,8	6.10	2.10	15.0	204,3		0.10	2.29	0.23	223.7
	574	2,353.2	0.23	1.67	0.17	400.0		0.23	478		423.6
	474	3.9 <u>58</u> ,2	0,39	1.43	0.14	554.1		0.39	1.51	0.15	593.7
	)PE	6.958.1	0.68	1.19	0.12	835.0		0.68	1.22	0.12	835,0
		10,193.5	1.00	1.00	0.1	1.019.4		1.00	1.00	0.1	1019.4
NOTE: $\alpha$ i =	Wi/∑ W 1 + (1	:			*T/(1	+ 3*	T)				
Ci =	Z*Rt*A	i*Co		41.11.1.11.11.11.11.11.11.11.11.11.11.11						·	

7	TABLE OF	COLUMN	LOAD FO	OR EACH	FLOOR (	SEISMIC (	CONDITION	)
FL+2	7.1 ~ 21.0	)			<del></del>			
	А	В	D	F	G	Н	К	TOTAL
101	ડેડે, 9				33.9			67.8
102	49.1	*			49.1			98.2
103	49.1				49.1	•		98.2
104	49.1				49.1			98.2
105	49.1				49.1			98.2
106	49.1				49.1			98.2
107	59.7				59.7			119.4
108	20.5				70.5			141.0
201	76.8		_		76.8			153.6
202								
203								
204			<u></u>					
205								
206								
207					[-			
208								
								1
TOTAL	486.4				486.4			972.8
	1.0 ~ 16.0	)	-					
	Α	В	D	F	G	Н	K	TOTAL
101	40.9				50-1	27.2		118.2
102	57.1				74.4	≥6.9		158.4
103	57.1	41			143.1	95.9		276.1
104	57.1				143.1	95.9		296.1
105	57.1		<u>:</u>		74.4	26.9		158.4
106	57.1	1 4 75			71.8	51.6	25.7	206.2
107	68.9			<u> </u>	87-4	63.5	48.2	268.0
108	81.0				103.3	75.5	45.6	305.4
201	84.9				110.9	15.5	45.6	3/6.9
202			3	<u> </u>	28.2	63.5	48.2	139-9
203					26.7	37.2	25.7	89.6
204								
205								
206								
207			:				· .	
208								
	an area y		·					
TOTAL.	561.2				913.4	639.6	239.0	2.353.2

E) ±/	6.0~11.0		· · · · · · · · · · · · · · · · · · ·			<del></del>		
<u> </u>	A A	В	D	F	G ·	Н	K.	TOTAL
					· · · · · · · · · · · · · · · · · · ·			<u> </u>
101	40.0		•		66.4	კგ.5		145.8
101	40.9				103.8	35.0		215.9
102	57.1				172.5	124.0		353. 6
103 104	57.1		<u> </u>		179.5	136.5		373.1
105	57.1				110.8	67.5		233:4
106	57.1				95.7	116.9	57.0	326.7
	57.1				143.7	188.4	127.1	ر.ور. 28.1ع
107	68.9				173.9	227.4	116.1	598.4
108	81.0				176.2	215.6	104.2	580.9
201	84.9				87.1	196.6	92.2	375.9
202		·			45.1	130.2	49.1	224.9
203					70.1	130.2	···· · <b>7·7 · [</b>	26 73 /
204							· · · · · · · · · · · · · · · · · · ·	
205				<b></b>				
206				<u> </u>				
207				<u>:</u>				2 - Va. 1
208							· · · · · · · · · · · · · · · · · · ·	
	· · · · · ·					1.06.6	NAT 2	3000 3
TOTAL	561.2	i			1354.7	1.496.6	545.7	3.958.2
FL+1	1.0 -5.5				G	Н	К	TOTAL
	A	В	D	F		<u> </u>		IOIAL
404		·		** 0	.50 D	40.0		8,4
101	51.4	21.3	47.4	40.9	109.7	47.7		489.7
102	74.0	29.7	<u>. 56.3</u>	50.1	169.1	110.5		540.2
103	74.0	19.0	<u> </u>	29.9	<u> 237.8</u>	179.5	<u> </u>	344.3
104	68.5	13.0		14.6	259.2	187.0		
105	69.5	14.0	· · · · · · · · · · · · · · · · · · ·	14.6	190.5	120.0	4.0	408.6
106	75.0	20.0		16.0	157.6	206-7	93.0	568.3
107	89.4	<u> </u>	29.9	35.9	206.6	348.0	198.9	940.4
108	108.7	20.9	64.6	63.0	237.3	396.6	188.8	1.079.9
201	92.0	4.5	27.9	49.6	265.6	365.6	176.4	981.6
202		<del></del>	: · · .		154.1	356.2	164.0	674.3
203					78.9	209.4	124.1	412.4
204							<u></u>	<u> </u>
205			<u> </u>					
206		:						
207					<u> </u>			ļ
208		-				<u> </u>		
				<u> </u>		-1		
TOTAL	702.5	- 1		314.6	2.066.4	2,529.2	945.2	6.958.1

'	TABLE OF	COLUMN	LOAD FO	OR EACH	FLOOR	( SEISMIC	CONDITIO	N)
FL+	5.5~±0							
	A	В	D	F	G .	Н	K	TOTAL
101	76.8	55.0	85.7	77.7	159.2	72.2		526.6
102	108.5	71.9	110.5	106.2	234.1	169.2		800.4
103_	108.5	45.9		62.6	302.8	238.2		758.0
104	85.1	17.6		22.2	306.7	262.7		694.3
105	86.1	18.6		22.2	241.5	193.7		562.1
106	116.4	62.8		48.7	223.1	301.2	143.1	<i>8</i> 95.3
107	131.7	86.1	81.7	95.3	295.2	495.4	276.3	1.461.7
108	123.9	57.3	145.8	141.1	<i>337.1</i>	<u>557.6</u>	282.0	1.644.8
201	94.1	6.9	52,4	89.1	342.2	525.9	259.4	1.370.0
202					211.7	487.8	224.2	923.7
203					108.5	282.1	166.0	556.6
204								
205								
206							ļ	
207								
208								
	_							ii us
TOTAL	931.1	422.1	476.1	665.1	2.762.1	3586.0	1.357.0	10.193.5
						<del>,</del>	· · · · · · · · · · · · · · · · · · ·	
	Α	В	D	F	G	Н	K	TOTAL
	Α	В	D	F	G	Н	К	TOTAL
101_	Α	В	D	F	G	Н	K	TOTAL
102	A	В	D		G	Н	К	TOTAL
	A	В	D	F	G	Н	К	TOTAL
102 103 104	A	В	D		G	H	K	TOTAL
102 103	A	В	D		G	H	K	TOTAL
102 103 104 105 106	A	В	D		G	H	K	TOTAL
102 103 104 105 106 107	A	В	D		G	H	K	TOTAL
102 103 104 105 106		В	D		G	H	K	TOTAL
102 103 104 105 106 107 108 201		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203 204		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203 204 205		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203 204		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203 204 205		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203 204 205 206		В			G	H	K	TOTAL
102 103 104 105 106 107 108 201 202 203 204 205 206 207					G	H	K	TOTAL

Γ	<del></del>		Ť	т		<u> </u>				:	<del></del>		Γ:	:	· · · · · · · · · · · · · · · · · · ·	-; 1	; 1	: 1	: 1	<u> </u>		
a		RKS																				
(ton)		REMARKS																				
FLOOR	ton)	7	4		4	<u>ئ</u>	Ā.	<u>ي</u>	201			9:550	256.0	295.9		31.0		65.7	113.4	13.3		
122			7/17/7		0,194	0.183	20.204	0.383	C V		3555	79.	2.8.1.	295.9 2		26.3	21/5		1 0.25	. M		
AT E	FLOOR		7			•			10			0	7	7.29		-+	\$ 8.23	47	ج. رج	8 11		1
FRAME AT EACH	RE		7 77		252.6	217.1	343.5	284.6	203		5888.	.298.0	7 52/3	1414		27.6	3	5,25	90/	150.8		
	AT EA	ASI'	7			•			105			355.8	7.32.7	= 1 1		21.6		65.	48.1			
OF EACH	FRAME JES AT	SHORT	17	4	0	2.1	3.6	3,2	104		5.915	355.8	235.7.			21.6	4.4	52	48.		7 7	
	R A	<b>ا</b>	7 7	4.090	613.0	830.1	1173.6	1458.2	206		5/6.5	3.55.6	407.5	.131.9.	:	27.6	74.4	65.7	28	50.5		
STIFFNESS AR FORCE	OF E	DIRECTION	-	1		9	- 0	~	102 1	:	3	273.6. 3	324.6	.295.9. 1		1.6		30.1	5,99	113.3		i.
1 121		DIRE	7, 17,		4263.8	4537.6	\$764.8	3807.2	101 10		355.8	273.8. 2	429.8 3	468.3.29		2.9.2	7.7	50.1 S	87.7	179.9		
ONTA	TION	>	_		_				2 2			27.	42	46			15	- 25	.90	7.		
HORIZONTAL SEISMIC SH	SEISMIC LOAD SUMMATION OF		V 7/17	0,283	0.252	0.289	0.304	0,387														,
	5 5		7	0	9	9	0	S	_ <b>×</b>		547.6	3-112	9:219	2/2.2			138.0	2as.7		82.1	l	
	Pi.	N.							I		1032.4	1299.2	1/40.6	9.980			260.2	375.5	346.7	420.5		
NOTE:		ХU	7						ণ্ড	578.4	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	<b>*</b>	<del></del>	1 8.606		163.7				352.1		•
NO	LOAD	र्भू	╁	_				·		40		<u>  :                                    </u>	662.0	453.6.9		"			1.2	75.5 3		
1	dic 1	FONG	7	327.7	576.9	830.1	1173.6	1458.2	u_				99	24.		:			201.	<i>'''</i>		:
	W W				-5	••0	7	7	Δ			<u>.</u>									<b>.</b>	
	0F 0	RECT		8.8	4.0	~ ∞.	2	5.4	102				0.299	453.6					2012	7.5.5		
	DISTRIBUTION OF SEI [地震力の架構への配	X X	2 N.	1156.8	2280.4	2871.8	3862.2	3765.4	∢	5915	210.4	0.198	185.0	9.64	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	163.7	0.641	248.8	238.6	251.4		
9	KRIBU 器力の	-1- X001-1	Ť	N N	STH	4TH	OPE	N III	NOI	R T				TA SA	NOIL	R T	5.T.H	4TH			1	
1010	· r		_	K		.	9	MER	LINE DIRECTION	₩	1/3	4.	0	2	LINE DIRECTION	œ	L <sub>2</sub>	4	0	Σ.		.1
	, in	E = = = = = = = = = = = = = = = = = = =				WHOLE	-		*			ĭ #			\$ <del>\</del>			‡ 5		* ;		

			Ī	Ī						m	:	Νį	9.	5	α,	T	$\exists$	0	[7]	0	~		
	1	ton)	REMARKS							203		8 387.3	248.0	344.5	282.8		10	.52	66.	76	(16.		
	` بدا	_	REM							202		35.55					22.	52.3					
	FLOOR	FLOOR (ton)		ΣK		7	~	0		108		:	355.8	0.955	.295.9		22.5		72.2	(72.3	121.6		
	江	~~~		01/		0.147	0,203	0.220	0.411	107		355.8		171.8	285.1		27.5	52.3		37.8	97		
	AT	AT EACH FLOOR OOR		3			•					1.3 34	0.0	7				5.6.95	60.5	7	.4		
	FRAME	FRAME AT AT EACH FL EACH FLOOR	SPAN	Pi			* 2		,	108		382.3	8.298,0	7.521.3	419.7		22.6	:25	$\dashv$	1,4	1,70.4		
- 1	144 P	HAFK AT EAC	11							501			45.5	.235.7			22.		72.2	51.9			
)	OF EA	SHEAR FORCE OF EACH LOAD OF EACH FRAME ! NO OF ALL FRAMES AT E	SHOR	0i	223.7	423.6	593.7	835.0	1.019.4	104		5.915	355.8	235.2			22.6	75.9	72.2	51.9		•	
	NESS	C FORCE O OF EACH ALL FRAM	-	Μ	22	4.2	3	8	1-0,	103		\$1.6.5.	355.8	407.5	131.9.		22.6	95.9	72,27	89.7	57.5		
	STIFFNESS	AK FO D OF 1	DIRECTION	Κí		2,875.0	2.6	7.9	2.3	201			.273.8	324.6	295.9.		22.6		55.6	71.4	121.6	• • •	.
i			IQ X	W		2,87	2,922.6	3.787.9	2,482.	101		\$55.8	273.8	3.23.8	469.3		15.6	52.3	555	94.6	192.9		
	HORIZONTAL	SEISMIC SEISMIC SUMMATI(		$\Sigma$ K	0.353	236	0.262	335	0.400														
	HC			2 01/E	0.0	0.2	2.0	0.3	9.	7		9-645	9/1/6	.6/2.6	2/2.2			2.62/	186.4	205.2	84.9	:	
,	Ki	9 % to 10 10 10 10 10 10 10 10 10 10 10 10 10	PAN	Pi	·					I		730.0	975.6	.824.8	874.4			186.4	255.6	276.3	349.8	. •	
7	NOTE:	<u> </u>								U	28912		3.		683.0		102.2				273.2		
	z	LOAD	ONG		n	0		0	4-	LL.				33/.0	.226.8.					10.9	90.0	·	
		· O	1	Σ Qi	204.3	400.0	554.1	835.0	1.019.4					\^3. 	<b>√</b> :								
		6	DIRECTION							0	:		:	0	80			:	:	e.			•
		6 <		ΣKi	570.4	1.692.8	2,117.7	2,491.9	2,548.0		<u> </u>	1	lo:	5 331.0	8. 226B.		2			110.9			
		SUTIO	٦.,			1.6	7.2	Ž	\\\\\	∢	2.89.2	.355.2.	430.5	3350	324,6		,50/	83.8	112.8	131.5	6767		
		DISTRIBUTION [古廊中包部蓝	FLOOR		R	STH	4 TT	OPE	MER	LINE DIRECTION	7	STH	414	305	MER	LINE DIRECTION	뀨	₹1H	411	Jd0	MF8		
		H -	TEM				WHOLE			X DIR			N C	<u> </u>	1	Y DIR			4.0				

INE	FLOOR	NUMBER	TYPE	MEMBER	AREA	L1	L2	L3	J
R				SIZE	(cm2)	(cm)	(cm)	(cm)	(t/cm)
<u> </u>	Pirectio	n	<u></u>						
	574		K	H-300 <sup>2</sup>	119.8	८००	500	707	<i>355.</i> 8
			_K	H-3502	173.9	500	500	707	516.5
			K	H-3002	119.8	700	500	860	387.3
<del> </del>	<u>4</u> TH		K	H-250 2	92.18	<i>300</i>	<u>حمی</u>	707	273.8
			K	H-300 <sup>2</sup>	119.8	500	500	707	355.8
			К	H-2502	92.18	760	500	860	298.0
	OPE		N	· 2L-130×12	39.52	450	<u>550</u>	711	70.5
			_ X	2L-120×8	37.52	450	550	711	88.9
			X	2L-120x 8	37.52	550	550	778	101.3
			X	21-12028	37.52	600	550	814	105.
			K	H-2502	92.18	500	550	743	235.
	-:-		K	H-3002	119.8	700	550	890	<i>349</i> .S
	MEZ		Ν	H-2002	63.53	450	650	791	54.7
			_ x	21-120×8	37.52	450	650	791	64.6
<u> </u> 			X	2L-120×8	37.52	030	650	851	77.2
			X	2L-120×8	37.52	600	650	885	82.0
			K	H-3002	119.8	500	650	820	228.1
			K	H-300 2	119.8	700	650	955	282.8
		- 1						-	
		į							
TE:					1	.3 = √	(Lengt	+L2*L2 h of B	race)
			2			J = 2*	A*E*L1	12/L313 : K or	3
	<u>/</u>					J = A*.	E*L1 <sup>2</sup> (TYPE	/L3 <sup>3</sup>	

n

		ZONTAL S ブレース		SS OF VERTICAL - 副 件 ]	BRACE				
LINE		NUMBER	TYPE	MEMBER SIZE	AREA (cm2)	L1 (cm)	L2 (cm)	L3 (cm)	J (t/cm)
X	Direction	m.		•					
A.G	RF_		X	2L-120x8	<i>ცე.5</i> 2	680	610	9/4	95.6
			<u> </u>	2L-120×8	37.52	1000	610	1171	98.0
A,H	STH		Χ	2L-120 X 8	37.52	_680_	500	844	121.2
Α			Χ	2L-120 x 8	37.52	1000	500	1118	112.8
H.K			K	H-2502	92.18	صي	500	707	273.8
Д,Н	474	1	X	2L-130×9	45.48	680	500	844	146.9
Α			χ .	2L~130 × 9	45,48	1000	500	1118	136.7
н, к			K	H-3005	119.8	500	500	707	<u> 355.8</u>
A, H	OPE		Χ	2L-130×9	45.48	680	550	875	132.0
A		·	X	2L-130X9	45,48	1000	550	1141	128.5
B.F			K	H-2502	92.18	<i>3</i> 40	550	647	165.5
н.К			K	14-300 2	119.8	500	550	143	<i>३</i> ०6-३
A.K.H	ME8		X	2L-130 X9	45.48	680	650	941	106.1
A			×	2L-130×9	45.48	1000	650	1193	112.6
B, F.G			K	H-5205	92.18	340	650	734	113.4
G	· 		K	H-3005	119.8	500	650	820	228.1
Н	l		K	H-3502	173.9	500	650	820	331.1
		:							
					Ì				
NOTE:						•	L1*L1 (Lengt A*E*L1	h of E ^2/L3^	race) 3
:			1.2			J = A*	(TYPE E*L1^2	: K or /L3^3	' X)
	<u>L1</u>	E : K	•	t1 TYPE : X or			(TYPE oung M (2100	odulus	(t/cm

- 2. Design of crane support girder (クレーン・ガーダの設計)
- (1) Design Condition

· Dead land = 45.0 ton (including 16.5 ton trolley)

Main hook: 60.0 ton

Aux hook: 15.0 tom

- . The beam for the crame girder will be a simple beam each for 6.8 m span and 10.0 m span.
- · Maximum wheel load (P)

Due to the previous calculation, the maximum wheel load is 25.2 tom/wheel

- · Stress of Girder
  - a. 6.8 mi span

 $M_{X2} = \frac{1}{8} \times 0.36 \times \overline{6.8} = 2.1 \text{ Tim}$  Girder D.L 306.9 Kg/m

Rail D.L 50

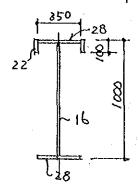
356.9 Kg/m

b. 10.0 on span

 $M_{X1} = 155.1 \text{ t.m.}, M_{Y} = 12.9 \text{ t.m.}, N = 10.1 \text{ T.M.}$   $M_{XZ} = \frac{1}{8} \times 0.522 \times 70^{2} = 6.5 \text{ T.m.} \qquad Girder D.L. 472 +8/m.$  Rail D.L. 50 522 +8/m.

## (2) Design of cross section

a. 6.8 m spam.



$$A=391.0 \text{ cm}^2$$
,  $ix=40.9 \text{ cm}$ ,  $iy=13.3 \text{ cm}$ 
 $Ec=14,571 \text{ cm}^3$   $Ix=654.677 \text{ cm}^4$ 
 $Et=11.888 \text{ cm}^3$   $ib=12.4 \text{ cm}$ 
 $EY=1.282 \text{ cm}^3$   $Iy=25,267 \text{ cm}^4$ 
 $A=\frac{lb}{\lambda b}=55$   $f_b=1.6 \text{ t/cm}^2$ 
 $A=\frac{lR}{\lambda y}=51.1$   $f_c=1.37 \text{ t/cm}^2$ 

$$\begin{aligned}
& \nabla_{bx} t = \frac{67.3 + 2.1}{11,888} \times 10^{2} = 0.58 \text{ t/cm}^{2} \\
& \nabla_{bx} c = \frac{67.3 + 2.1}{14571} \times 10^{2} = 0.48 \text{ y} \\
& \nabla_{by} c = \frac{5.6 \times 10^{2}}{1282} = 0.44 \text{ H} \\
& \nabla_{by} c = \frac{8.9/2}{391.0} = 0.01 \text{ H}
\end{aligned}$$

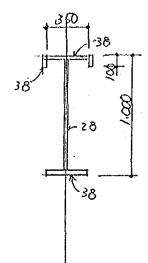
Compression side: 
$$\frac{0.48}{1.6} + \frac{0.44}{1.6} + \frac{0.01}{1.37} = 0.58$$
 < 1.0

Tension side = 
$$\frac{0.58}{1.6} + \frac{0.01}{1.6} = 0.37$$
 (1.0

Deflection : 
$$S = \frac{5 \times 0.15 \times 680^{4}}{364 \times 2/00 \times 654.677} = 0.3 \text{ cm}$$

$$\frac{5}{\ell} = \frac{0.3}{680} = \frac{1}{2266} < \frac{1}{100}$$

## b. 10 m span



$$A=600.7 \text{ cm}^2$$
,  $ix=39.4 \text{ cm}$ ,  $iy=14.2 \text{ cm}$ 
 $Z_c=21091 \text{ cm}^3$ ,  $Ix=934.560 \text{ cm}4$ 
 $Z_t=16.781 \text{ cm}^3$ ,  $ib=13.1 \text{ cm}$ 
 $Z_T=1985 \text{ cm}^3$ ,  $I_T=42.295 \text{ cm}4$ 
 $\lambda_b=\frac{1b}{\lambda b}=76$ .  $f_b=1.6 \text{ t/cm}^2$ 
 $\lambda_c=\frac{1k}{\lambda \gamma}=70.4$   $f_c=1.2 \text{ t/cm}^2$ 

$$\frac{d_{b}x}{d_{b}x} = \frac{155.1 + 6.5}{16.781} \times 10^{2} = 0.96 \text{ T/cm}^{2}$$

$$\frac{d_{b}x}{d_{b}x} = \frac{155.1 + 6.5}{21.091} \times 10^{2} = 0.77 \text{ "}$$

$$\frac{d_{b}x}{d_{b}x} = \frac{129 \times 10^{2}}{1985} = 0.65 \text{ "}$$

$$\frac{d_{b}x}{d_{b}x} = \frac{129 \times 10^{2}}{1985} = 0.65 \text{ "}$$

$$\frac{d_{b}x}{d_{b}x} = \frac{10.1/2}{600.7} = 0.008 \text{ "}$$

Compression side: 
$$\frac{0.77}{1.6} + \frac{0.65}{1.6} + \frac{0.008}{1.2} = 0.89 < 1.0$$

Tension side : 
$$\frac{0.96}{1.6} + \frac{0.008}{1.6} = 0.61 < 1.0$$

Deflection 
$$S = \frac{5 \times 0.1 \times 1000}{384 \times 2100} \times 934500$$

$$\frac{S}{l} = \frac{0.66}{1000} = \frac{1}{1515} < \frac{1}{1000}$$

3. Structural Analysis of Frames (構造解析) 75 of 3.1 Calculation of vertical bracing by vertical load SHEET ... (垂直プレースの計算) MEZ 5FL 1FL ळा,व ळ्ळदेळ्ळटळटचे व्या. <u>१००० इच्छाड्ड्य इच्छाड्</u> 6,800 92.7  $[6,8\infty]$ **@** FORM 04

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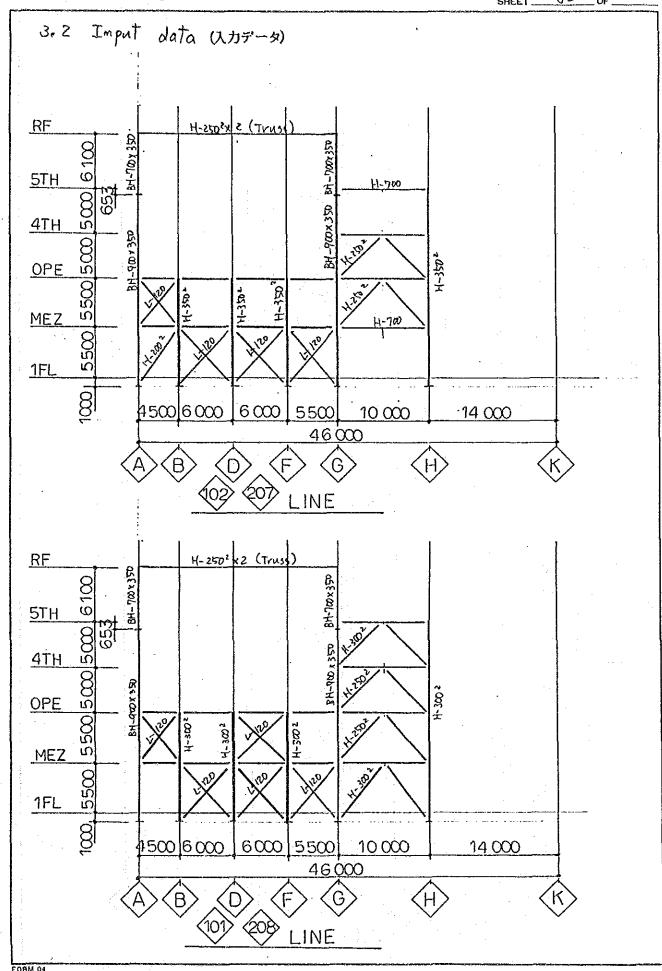
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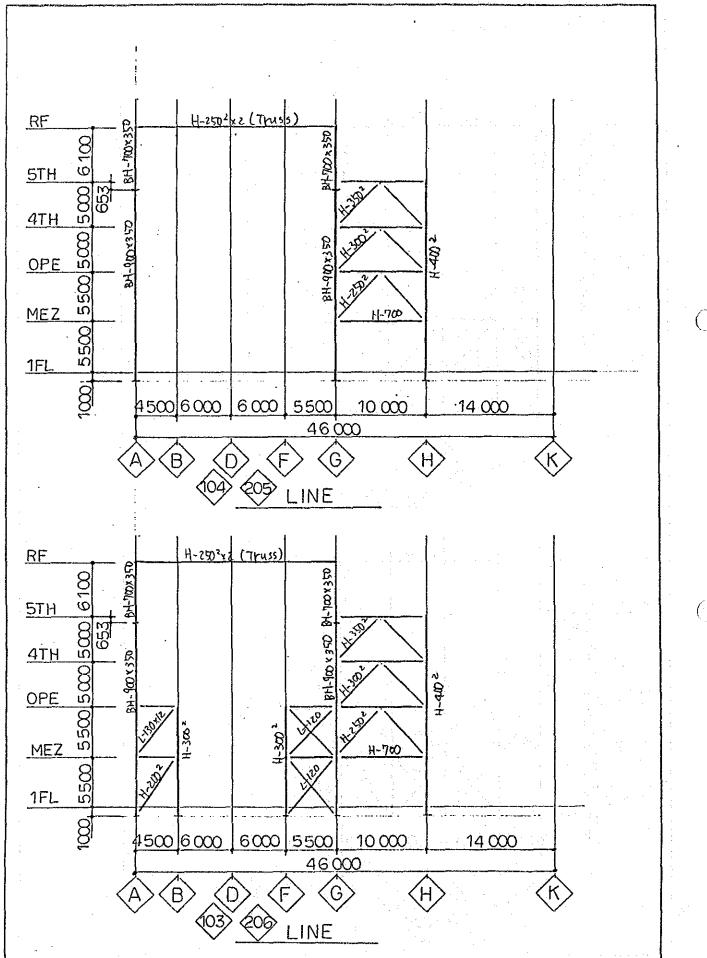
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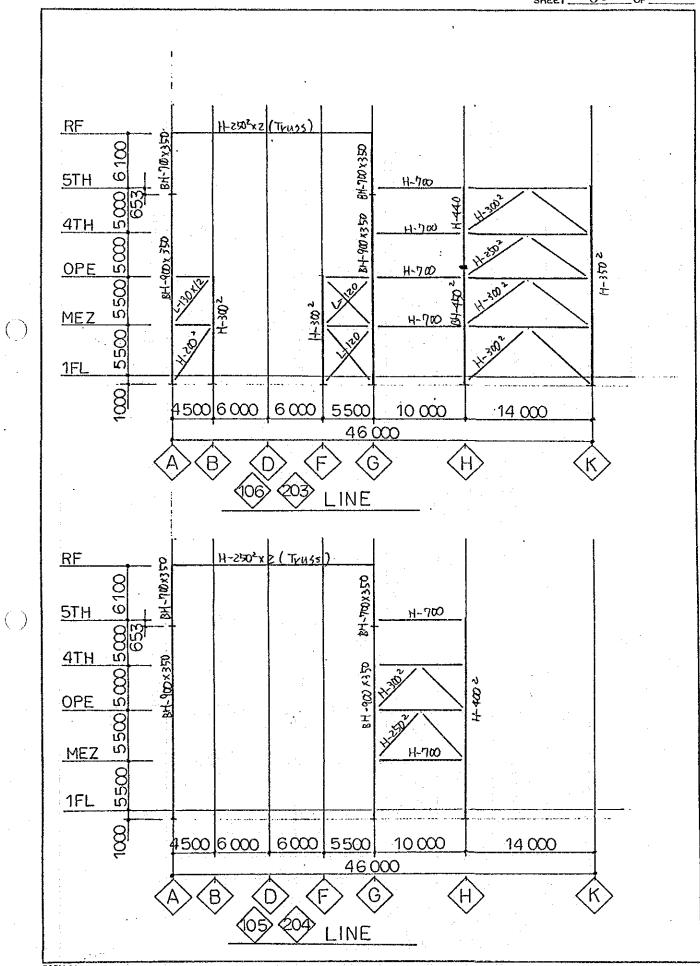
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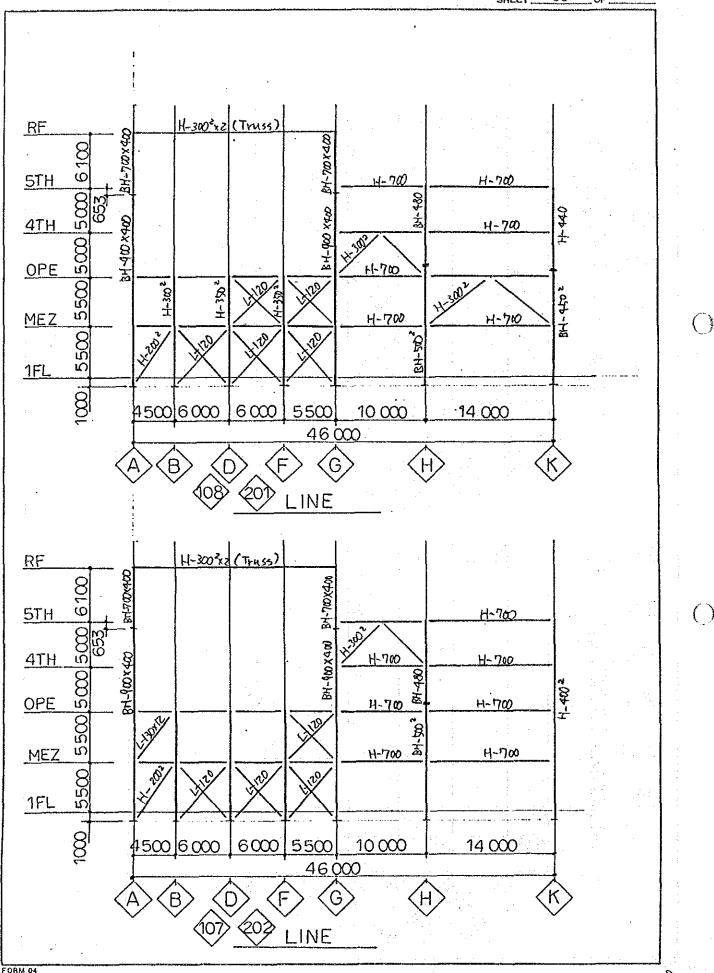
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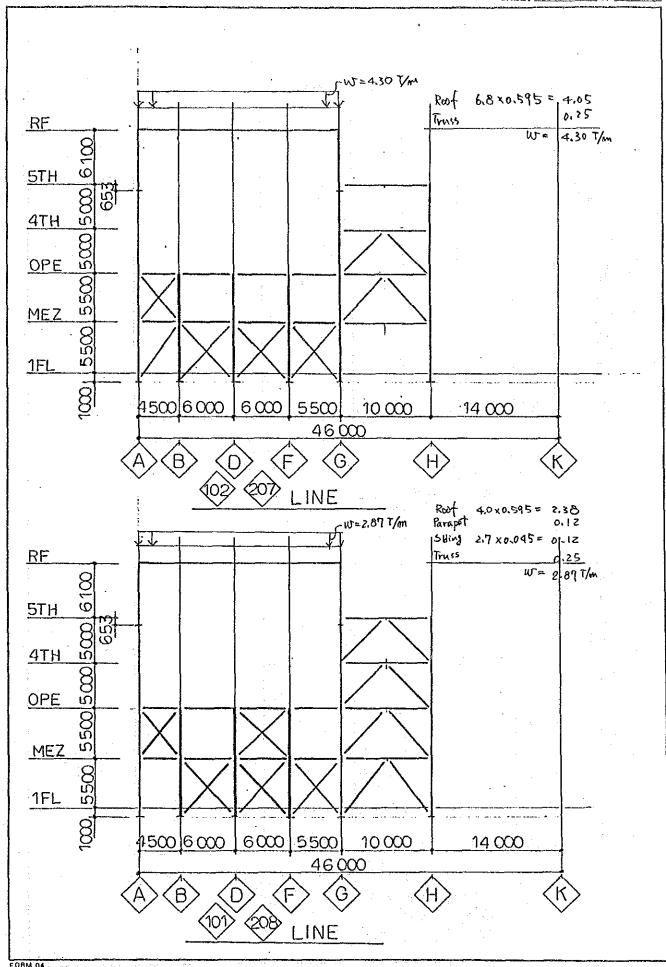
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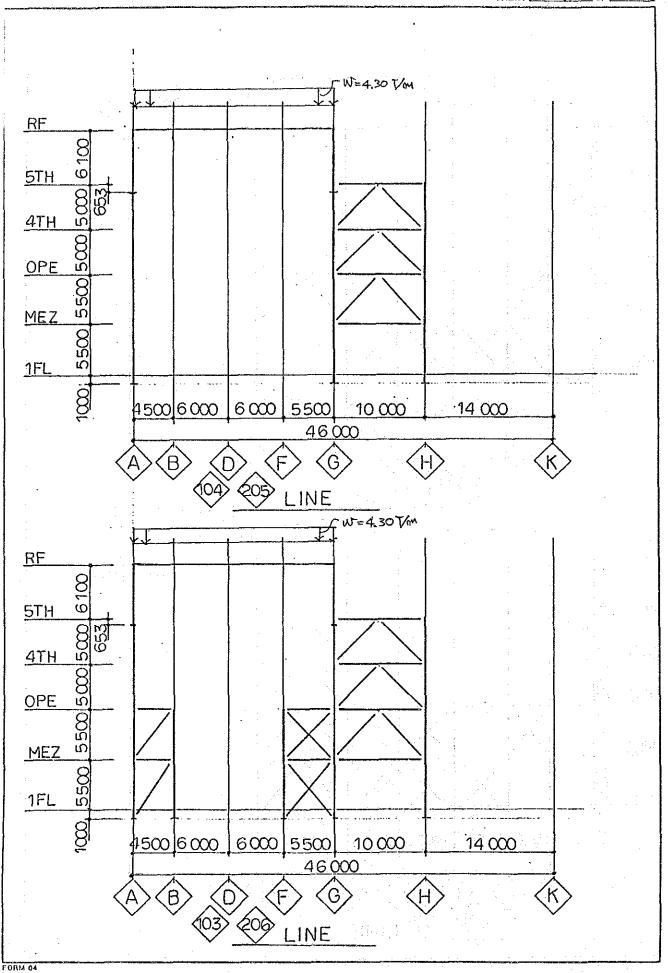
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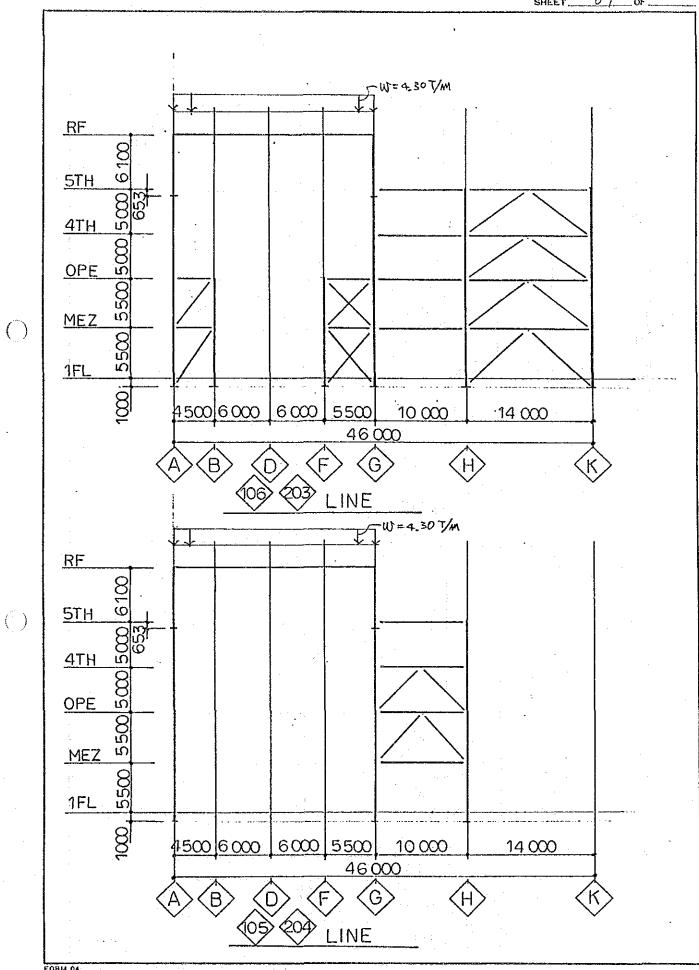


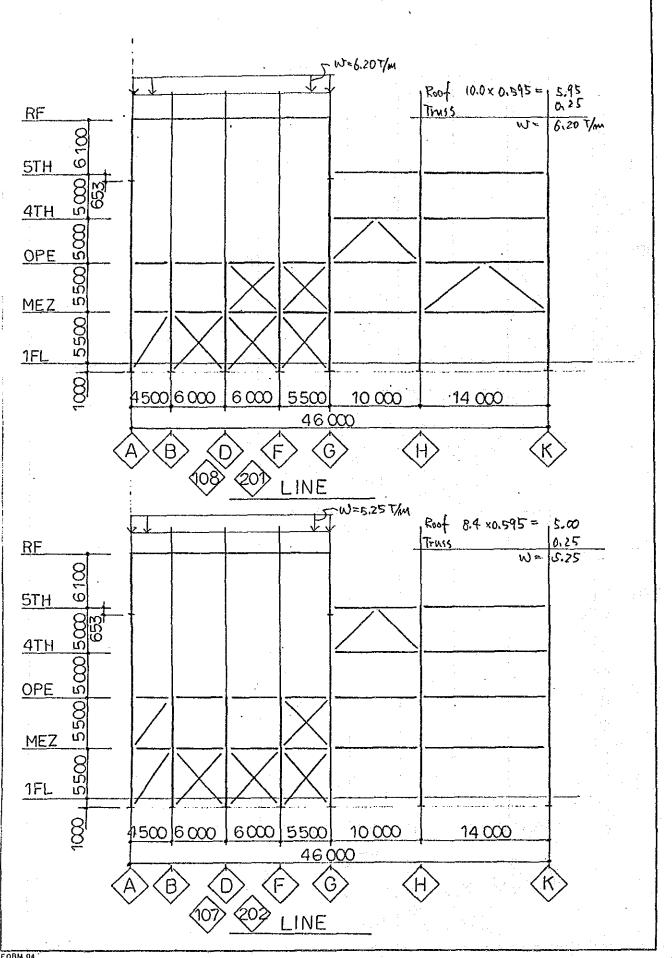
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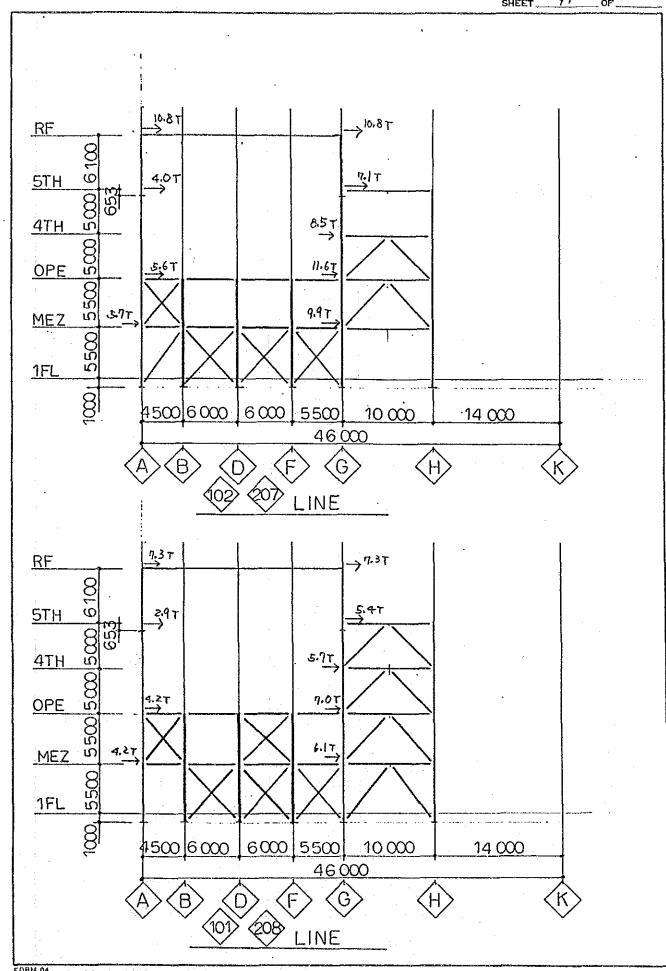


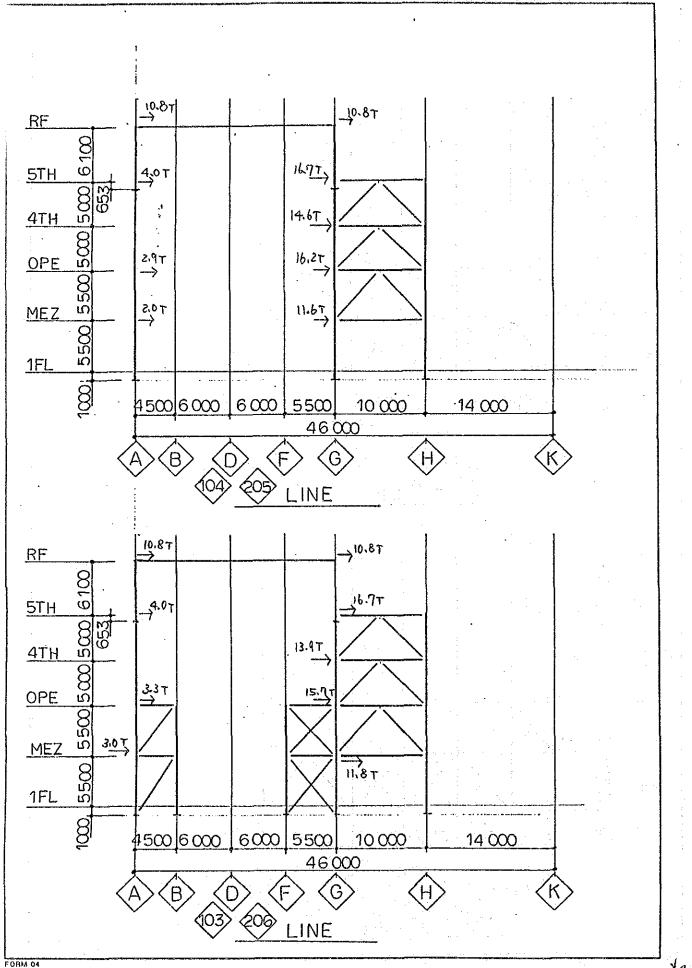
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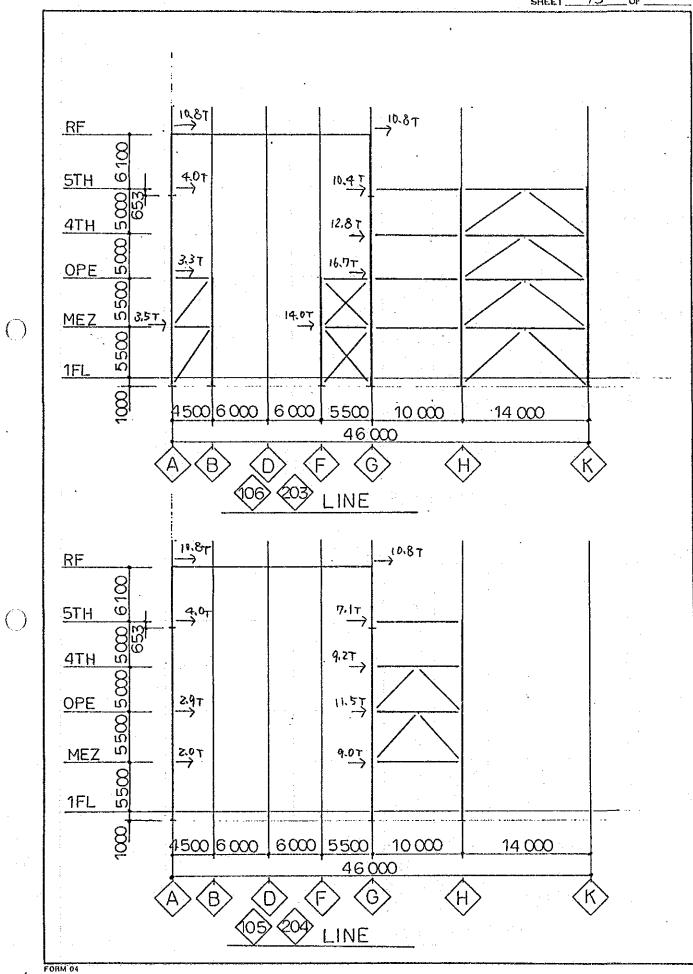


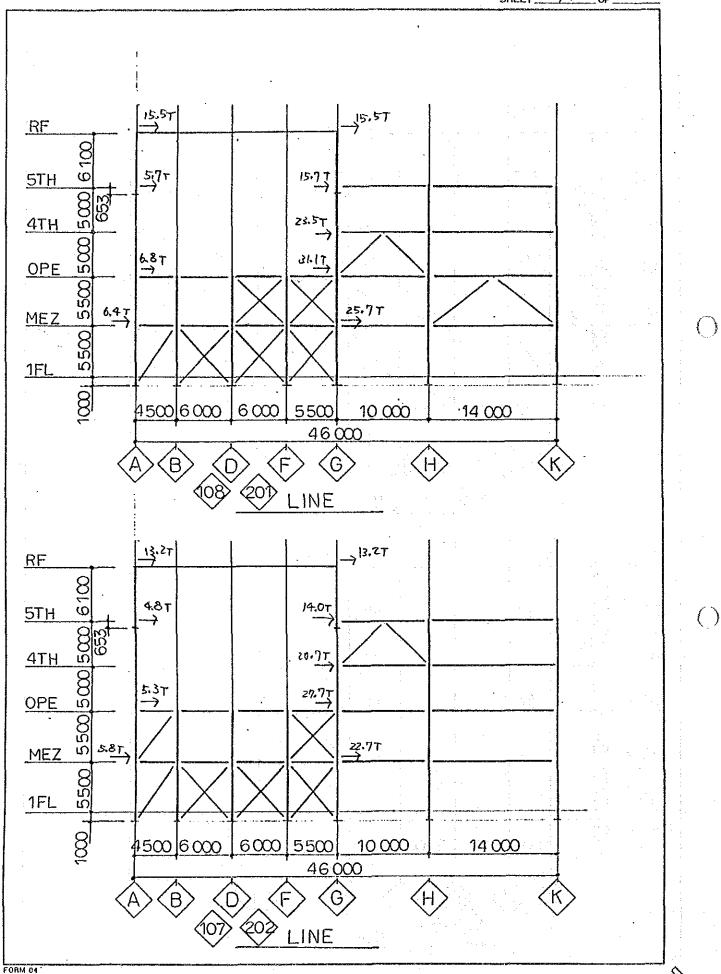


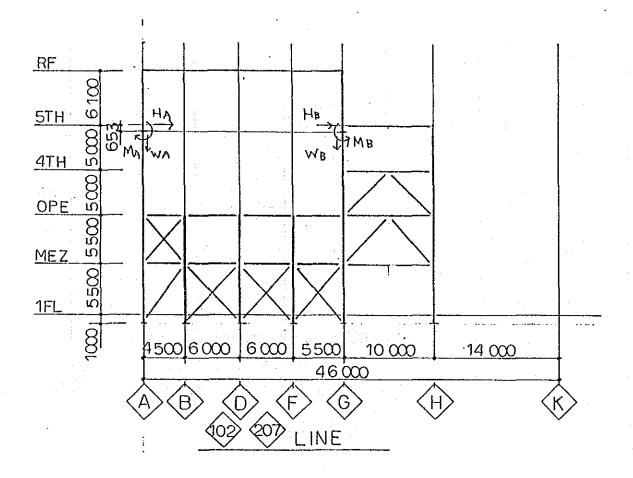




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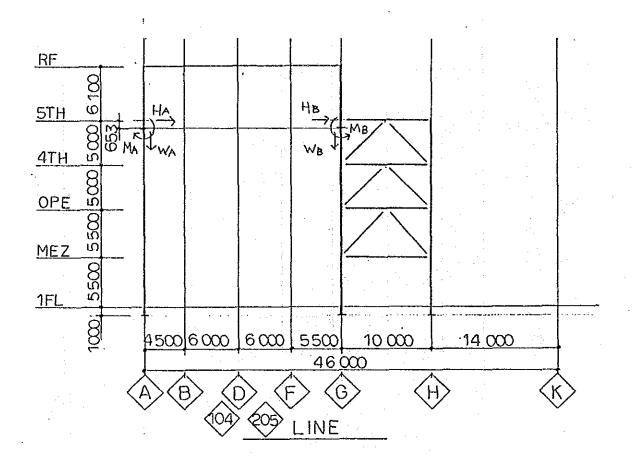


C.D.L. 
$$WA = 24.8 \text{ T}$$
  $WB = 13.3 \text{ T}$   $HA = 2.1 \text{ T}$   $HB = 1.1 \text{ T}$   $MA = 19.8 \text{ Tm}$   $MB = 10.6 \text{ Tm}$   $MB = 10.6$ 

FORM 04

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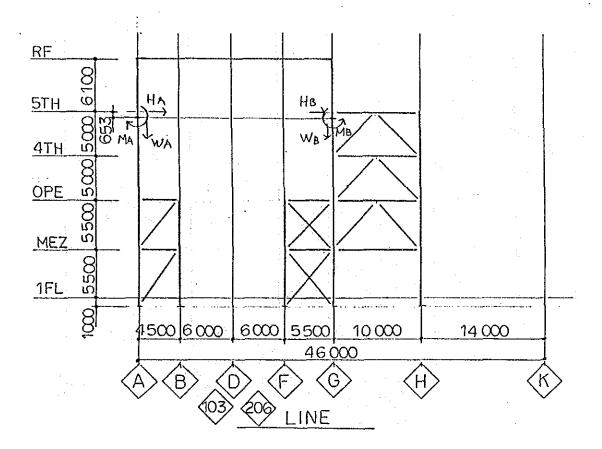
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C.D.L. WA= 24.8T WB= 
$$13.3T$$
HA= 2.1T HB= 1.1T
MA= 19.8Tm MB= 10.6Tm

C.L.L. WA= 71.2T WB= 17.8T
HA= 5.9T HB= 1.5T
MA= 57.0TM MB= 14.2Tm

FORM 04



C.D.L. WA= 24.8 T WB= 13.3 T

$$HA = 2.1 \text{ T}$$
  $HB = 1.1 \text{ T}$ 
 $MA = 19.8 \text{ FM}$   $MB = 10.6 \text{ T}$ 

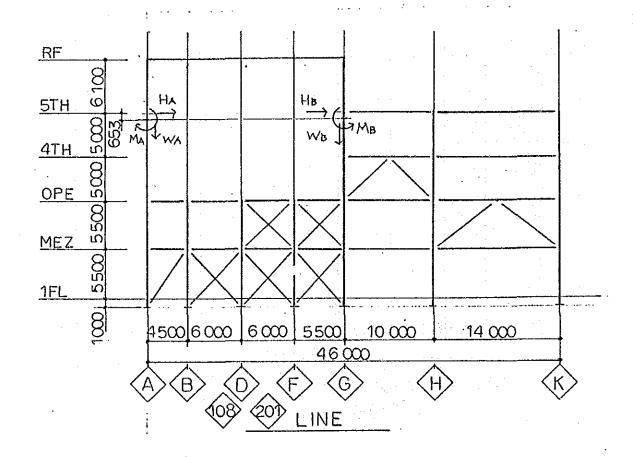
C.L.L. WA= 71.2 T  $HB = 1.5 \text{ T}$ 
 $HA = 5.9 \text{ T}$   $HB = 1.5 \text{ T}$ 
 $MA = 57.0 \text{ T}$   $MB = 14.2 \text{ T}$   $MB = 2.3 \text{ T}$ 

C.H.L.  $HA = 4.2 \text{ T}$   $HB = 2.3 \text{ T}$ 

FORM 04

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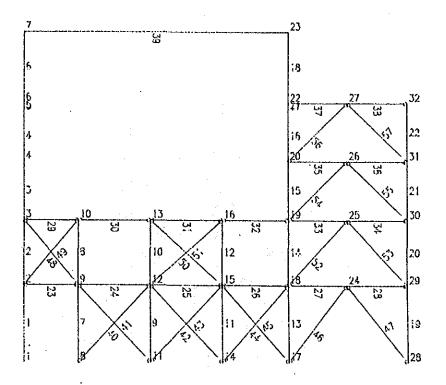
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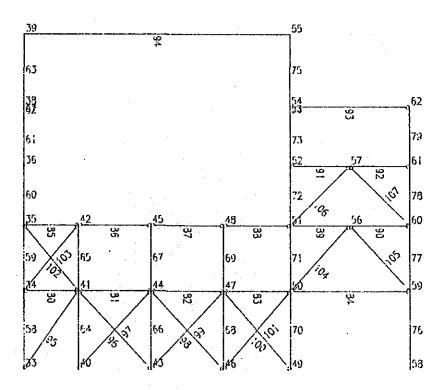
C.D.L. 
$$WA = 28.1 \text{ T}$$
  $WB = 15.1 \text{ T}$   
 $HA = 2.3 \text{ T}$   $HB = 1.3 \text{ T}$   
 $MA = 22.5 \text{ T.m}$   $MB = 12.1 \text{ T.m}$ 

C.L.L. 
$$WA = 80.6T$$
  $WB = 20.2T$   
 $HA = 6.7T$   $HB = 1.7T$   
 $MA = 64.5Tm$   $MB = 16.2Tm$ 

FORM 04

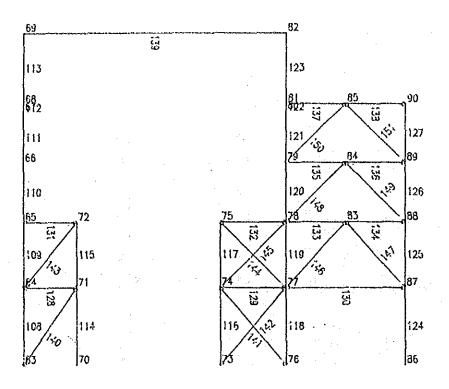


フレーム モデル 図 ( 101 - LINE SCALE 1/300 )

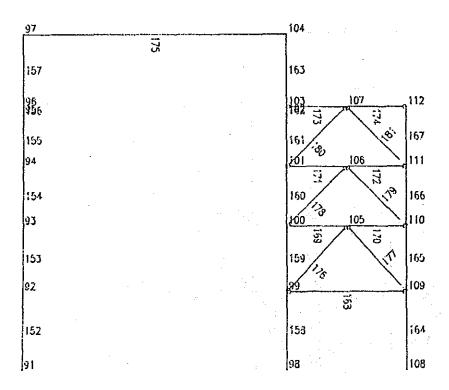


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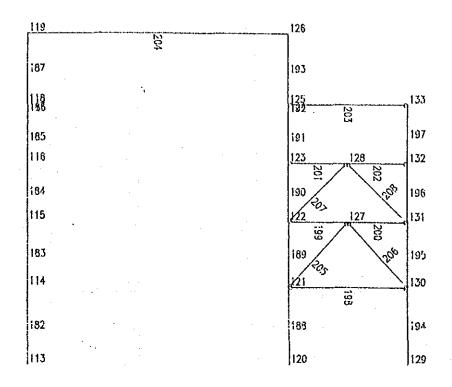
フレーム モデル 図 ( 103 -- LINE SCALE 1/300 )

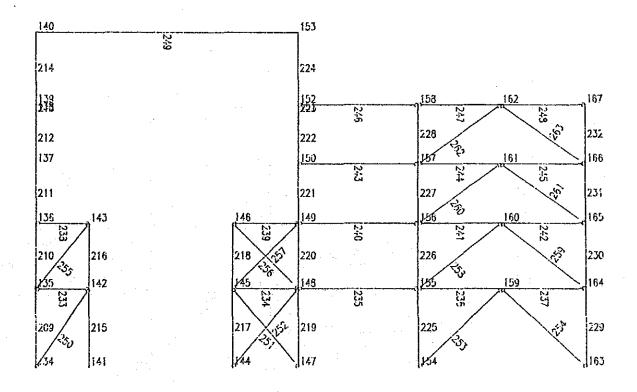


フレーム モデル 図 ( 104 - LINE SCALE 1/300 )

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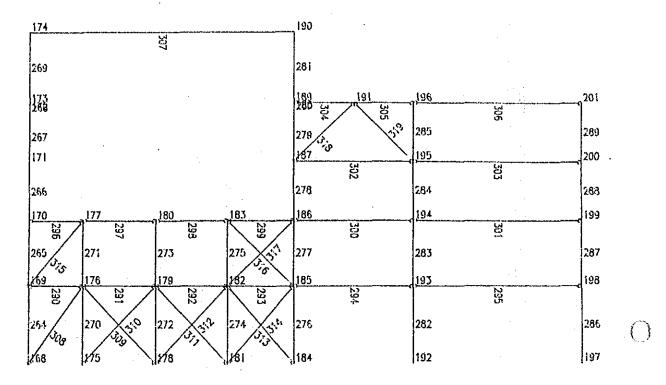
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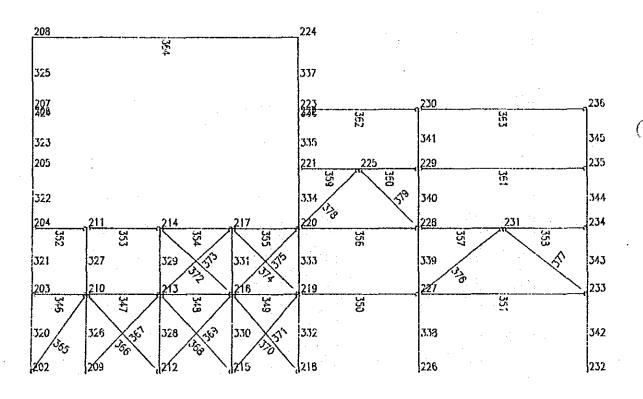


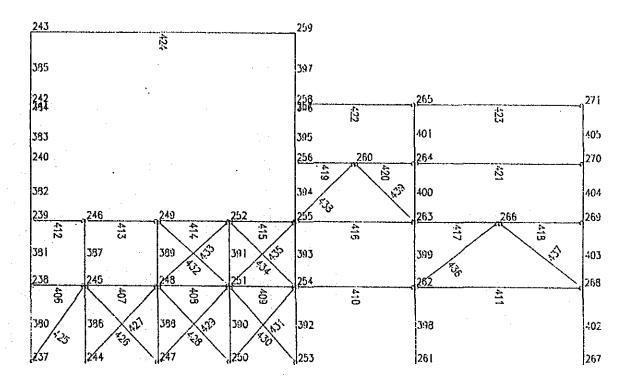
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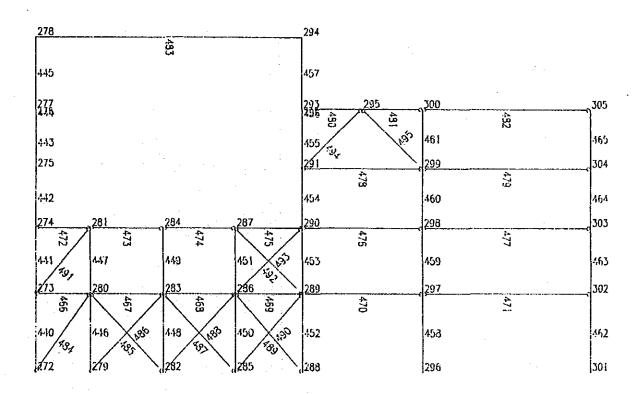


フレーム モデル 図 ( 107 -- LINE SCALE 1/300 )

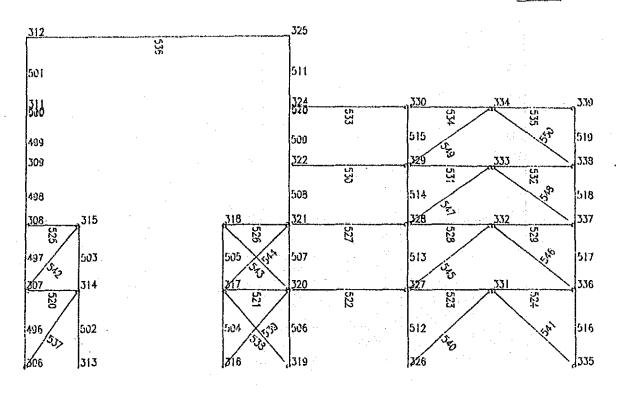




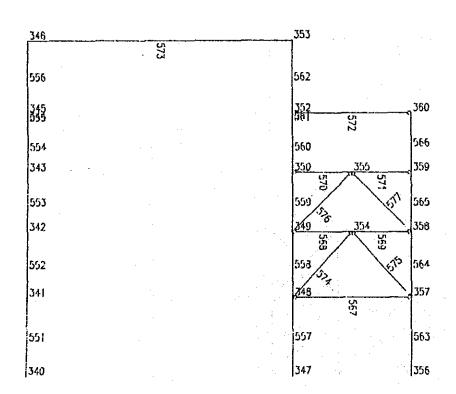
フレーム モデル 図 ( 201 - LINE SCALE 1/300 )



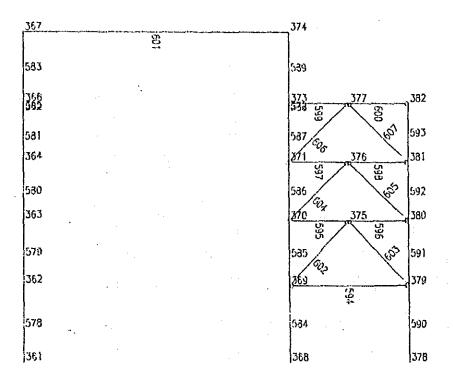
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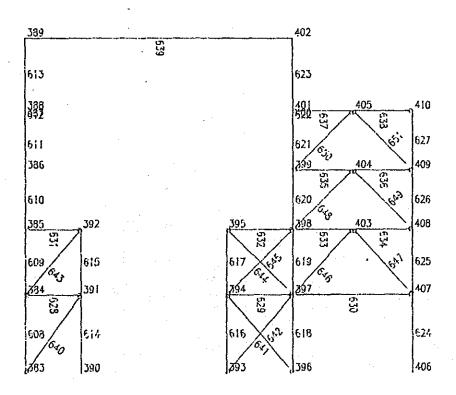


フレーム モデル 図 ( 203 - LINE SCALE 1/300 )

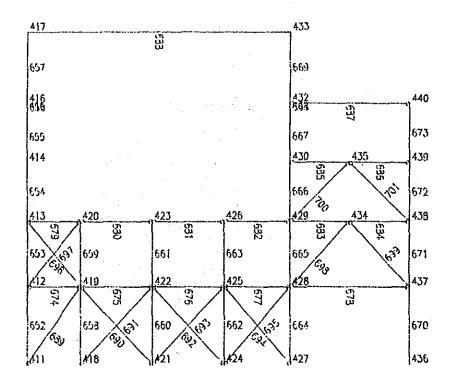


フレーム モデル 図 ( 204 - LINE SCALE 1/300 )

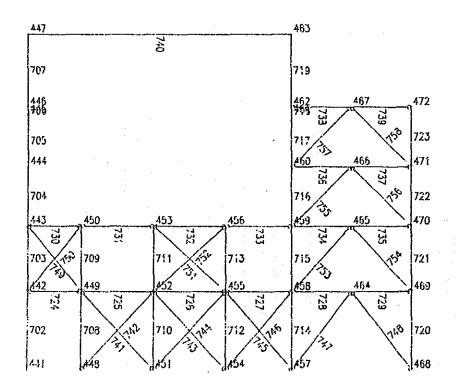




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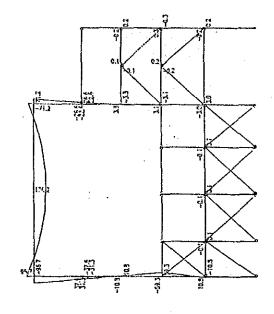


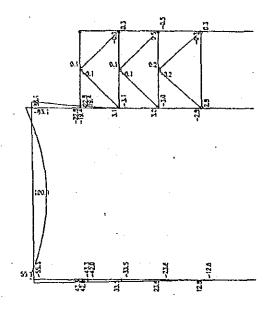
フレーム モデル 図 ( 207 - LINE SCALE 1/300 )

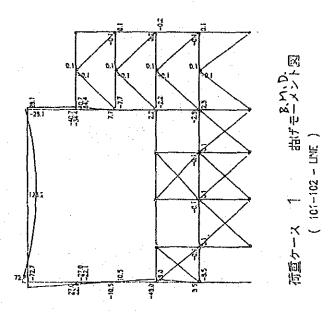


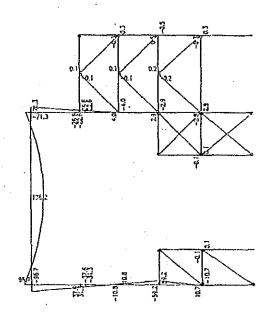
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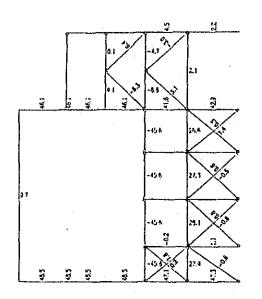
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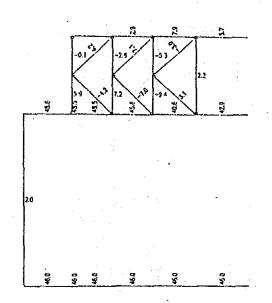


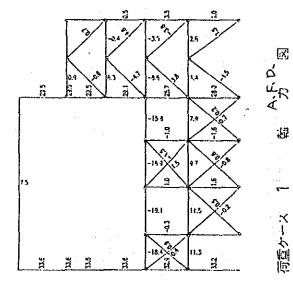




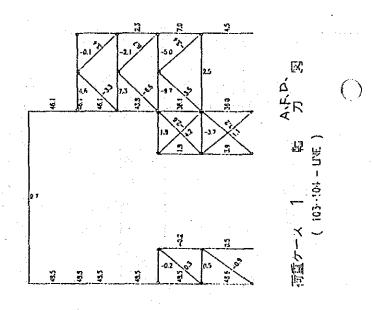


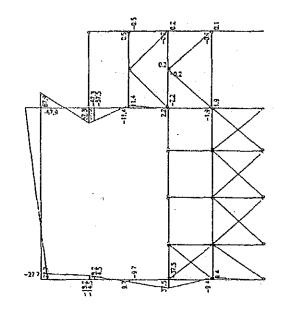


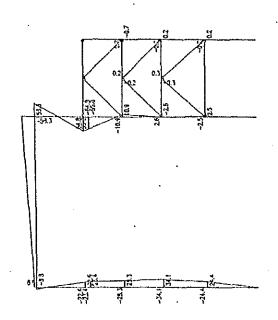


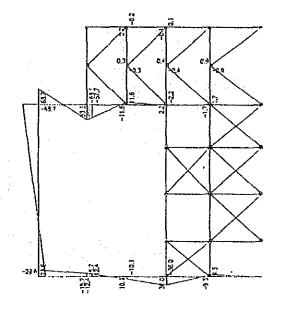


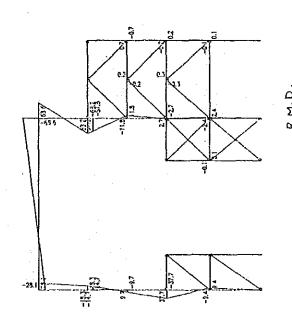
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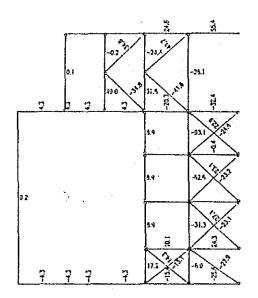


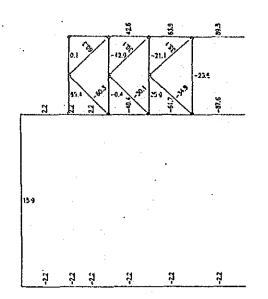


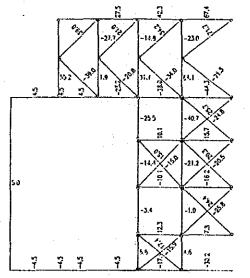


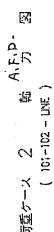


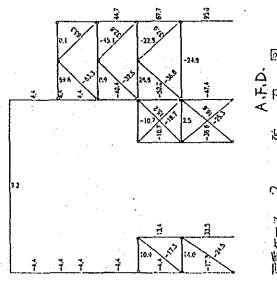




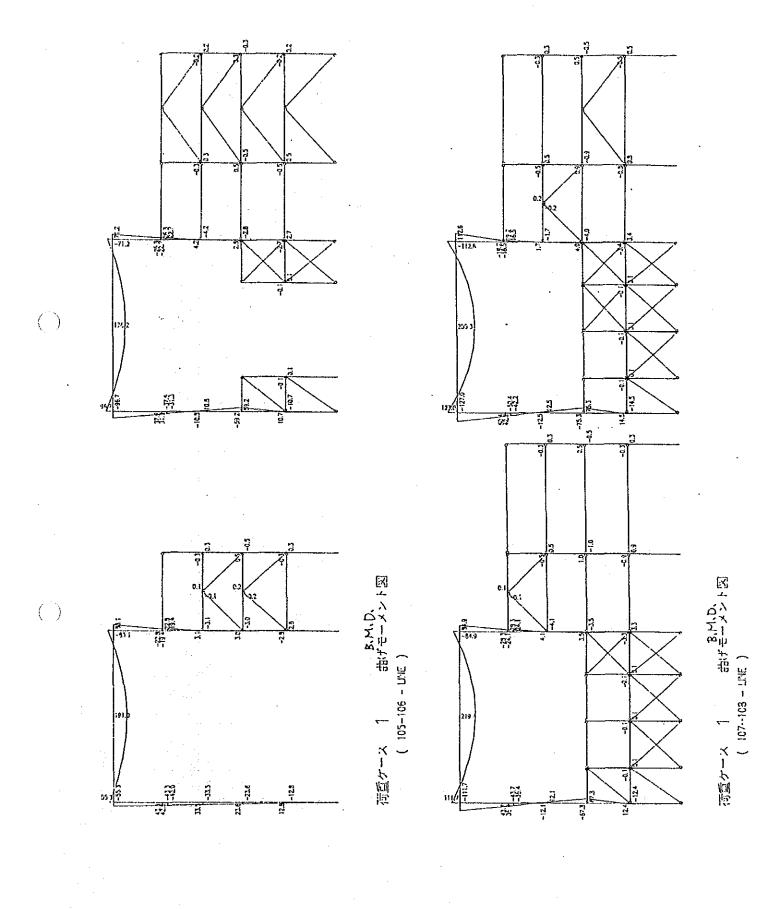




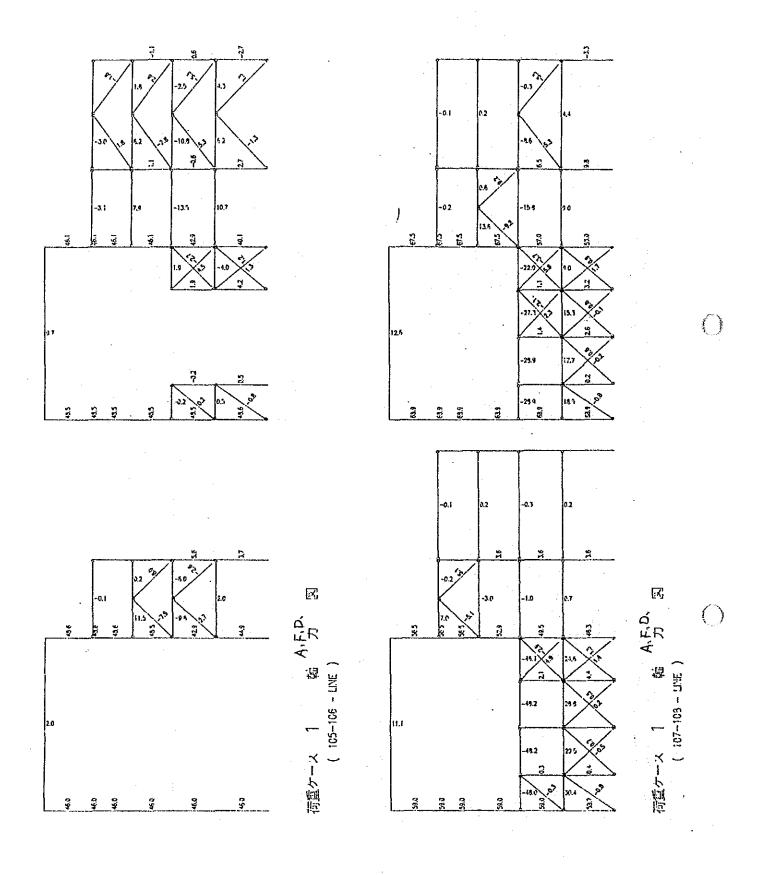




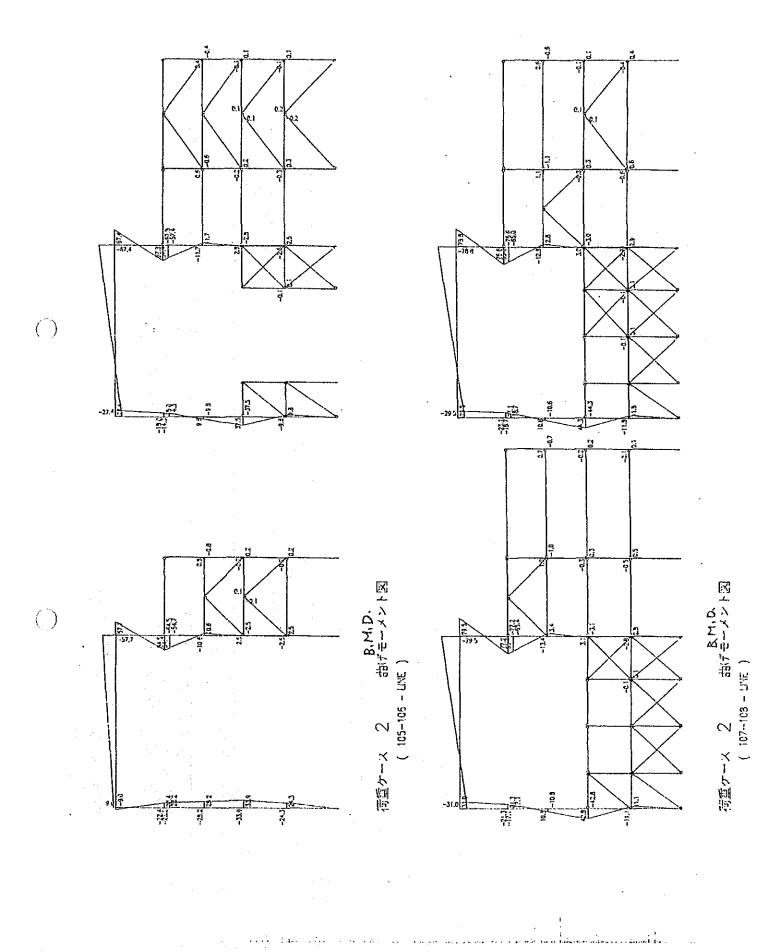
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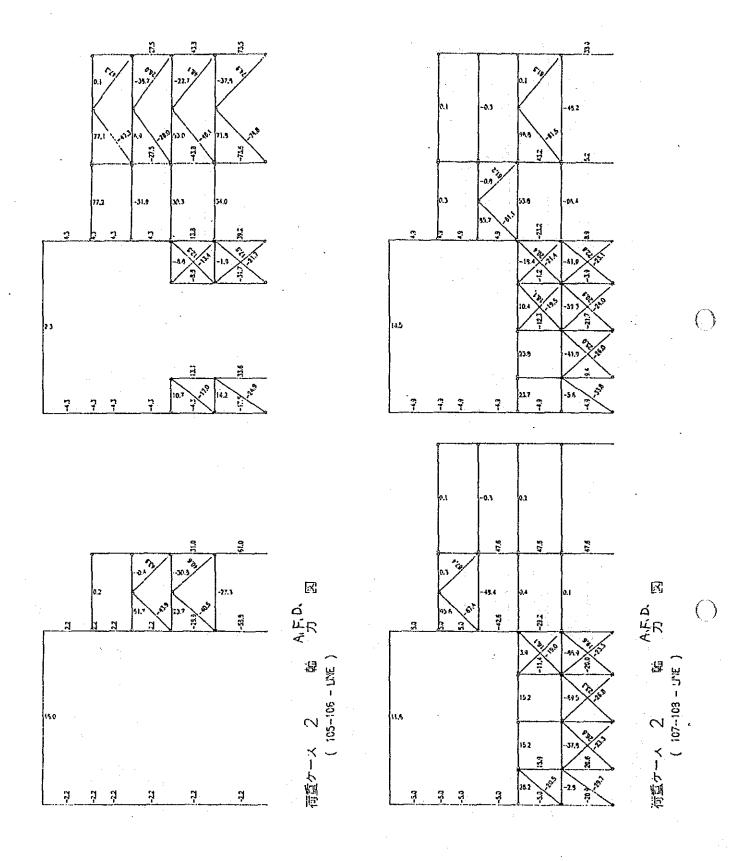


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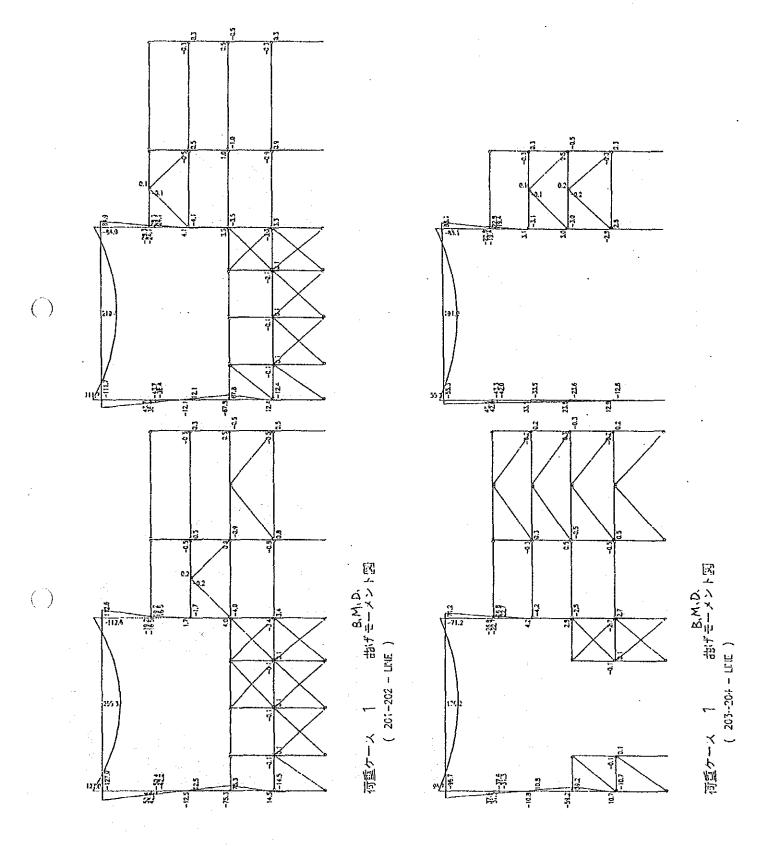


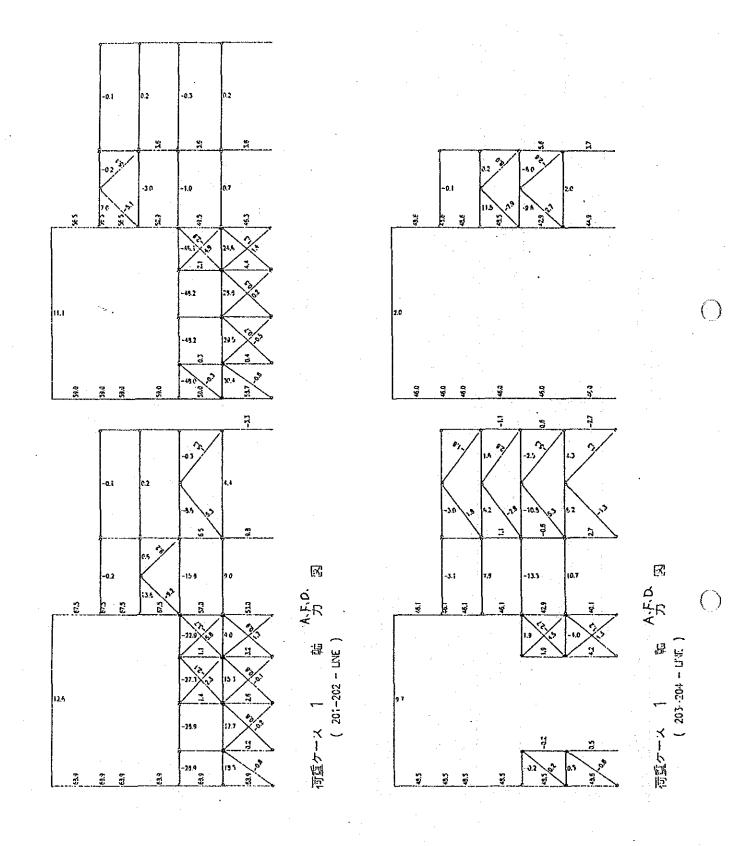
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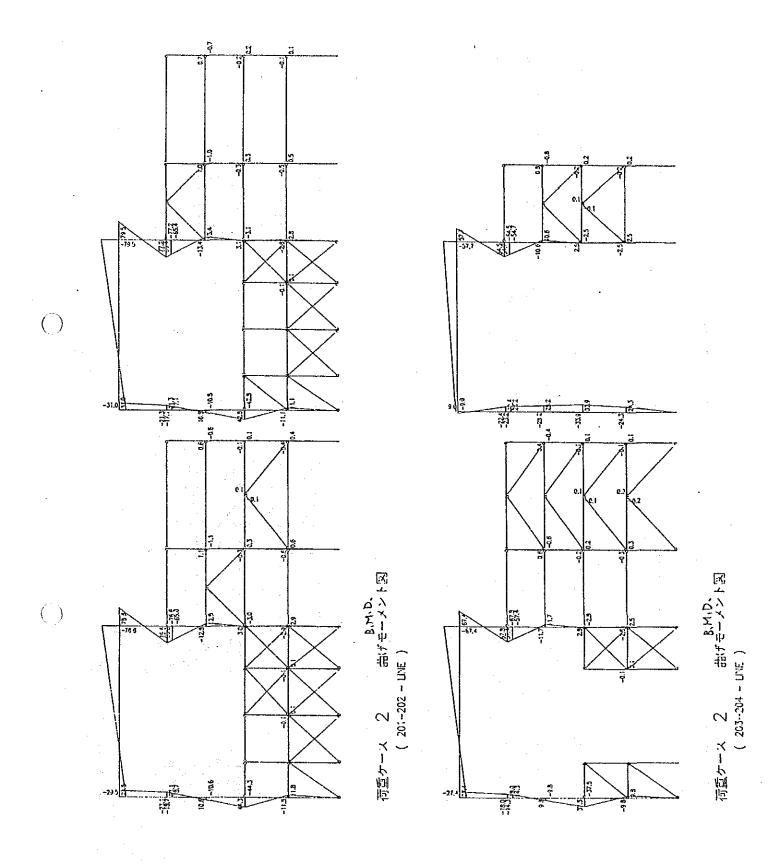
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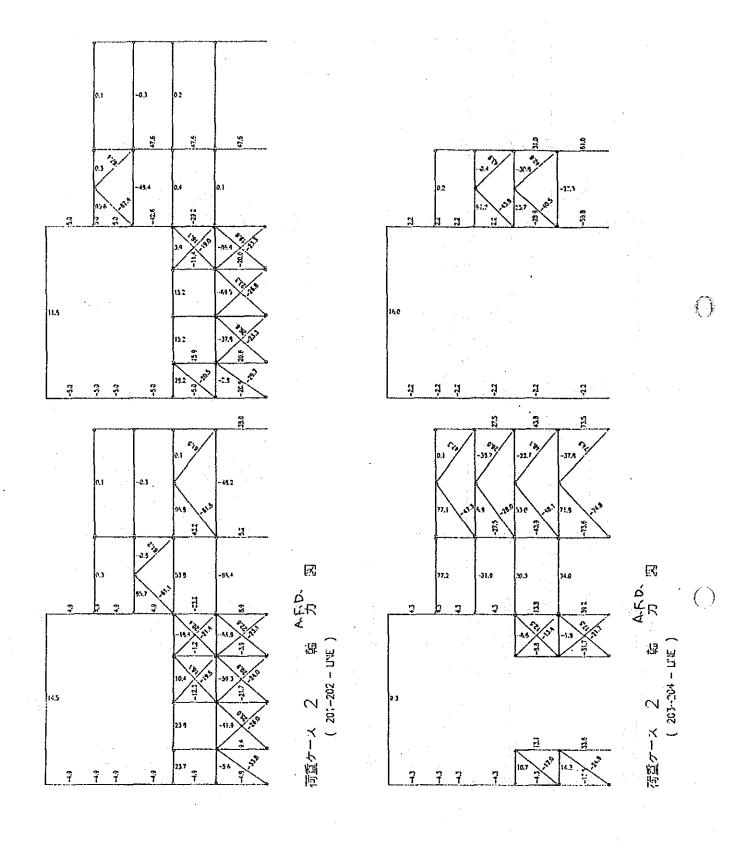


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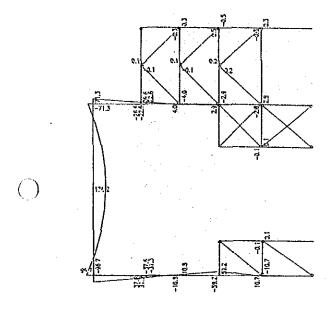


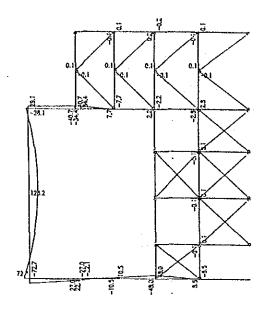
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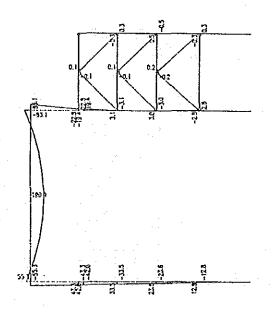


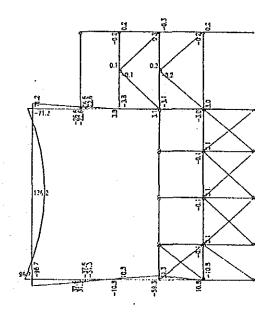
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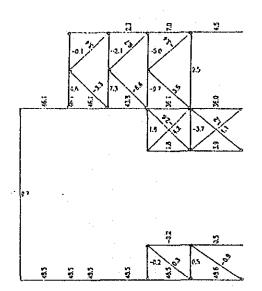


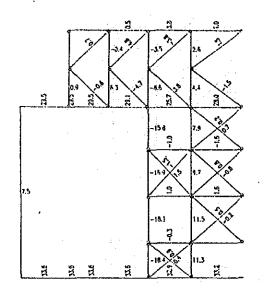


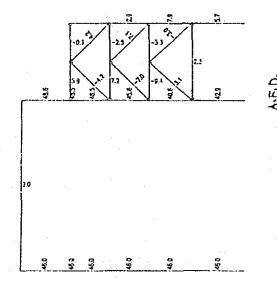


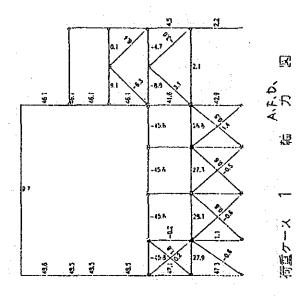
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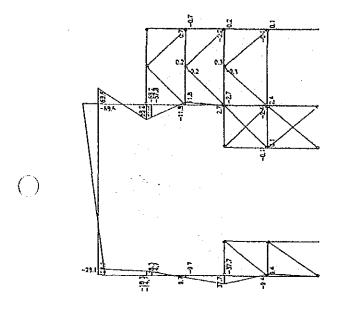
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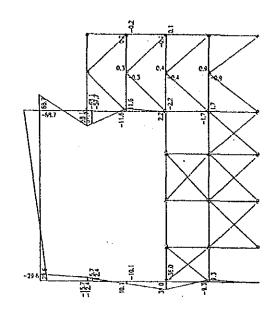


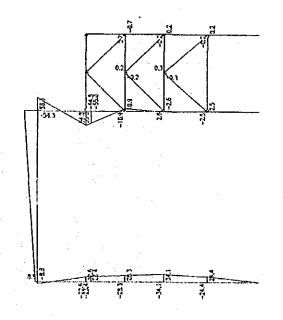


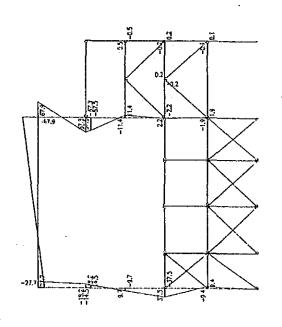


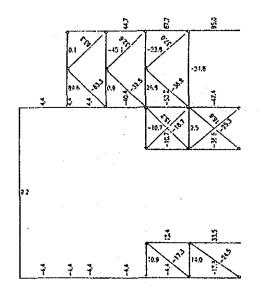


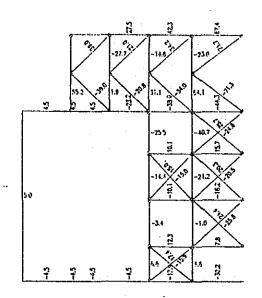


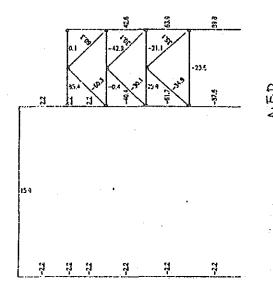




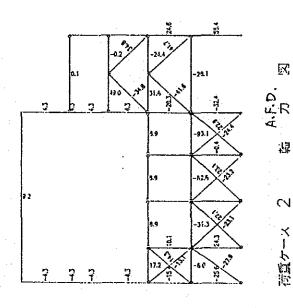


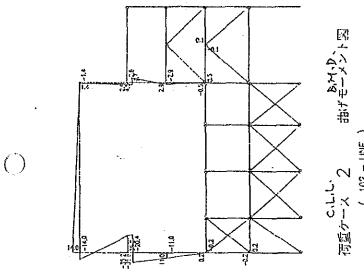




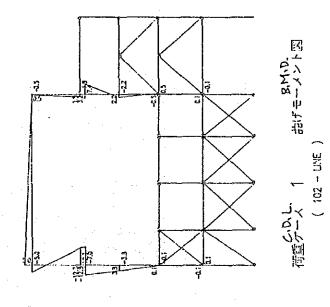


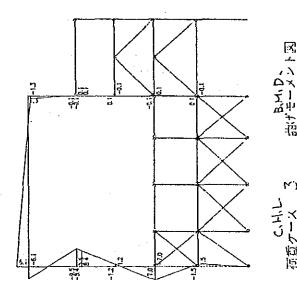
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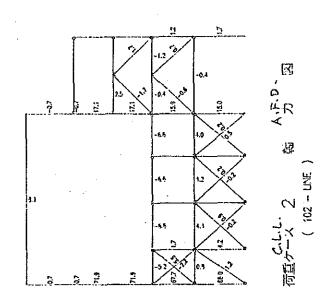


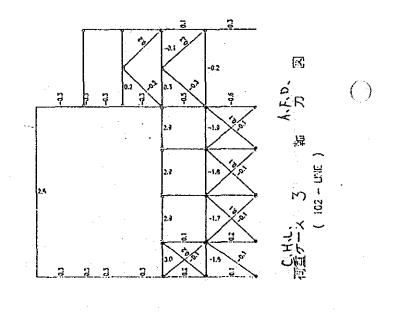


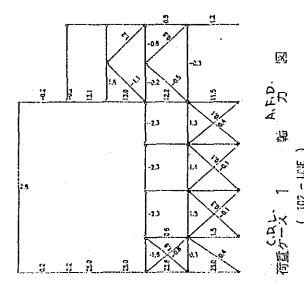


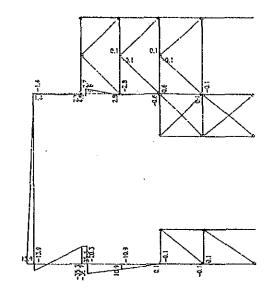




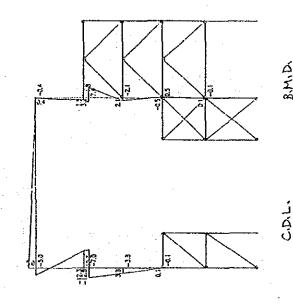


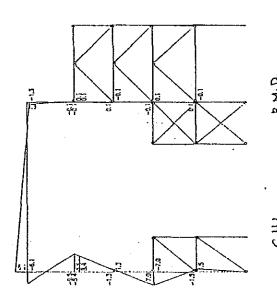




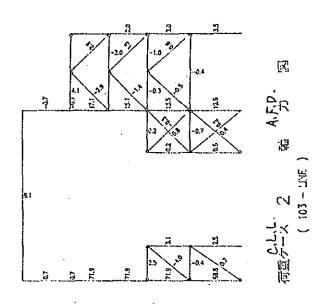


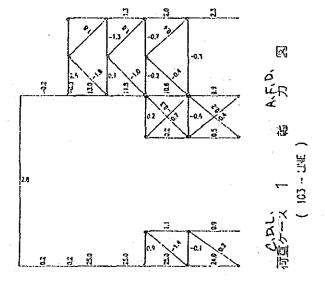
OCLC、2 出げモーメント図 (103・17/E)

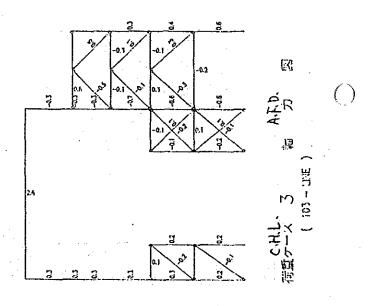


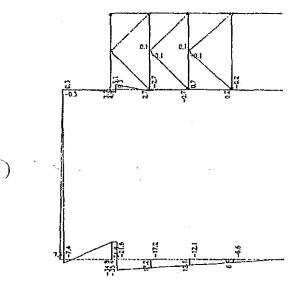


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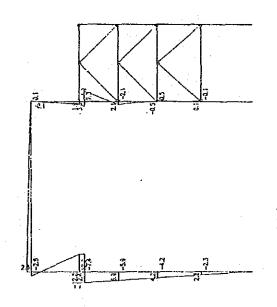


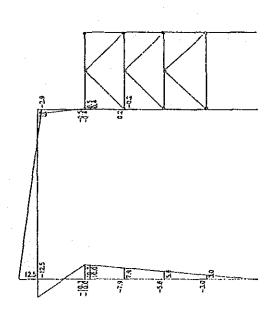


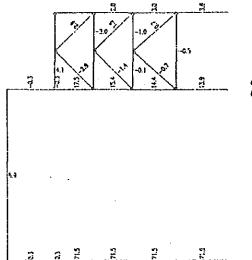


C-1-1・ 指型ケース 2 超げモーメント図 (104 - LNE)

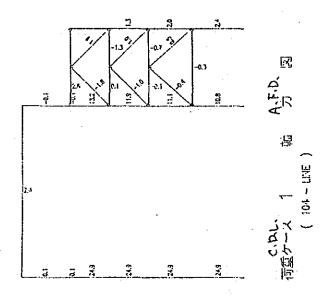
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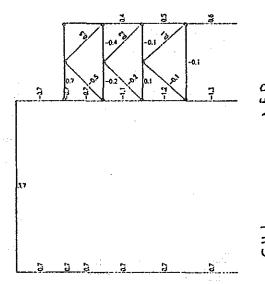




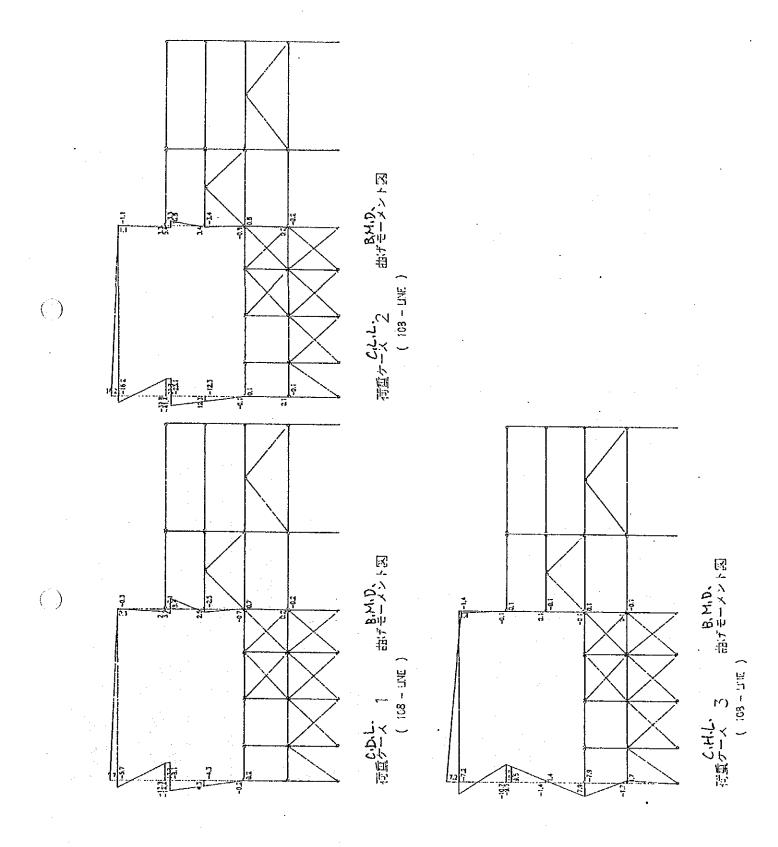


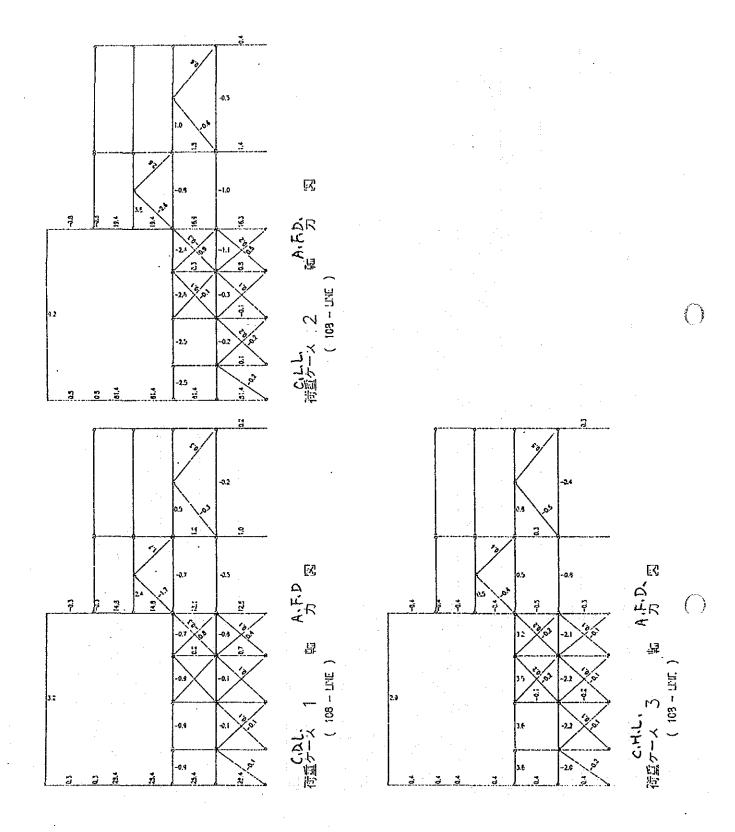






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LLOATIONS	TC	2	ξ.		==	127.7 28,	4		<u>:</u>			-			:						
ZONTA CONDI CONDI	)["] <sub>&gt;</sub>	70.5				1.98.5 149.7 12	219.7 166.0 24.	95.4	L-:	14.0	35.1	36.	131.0	189.3	100.2	2.16	105.5	136.7	135.7	2.022	209.4
HORI VENT	1 2				-21.8	16.3	-2.2					- : · · ·					•				
CRANE HORIZONTAL LOAD PERMANENT CONDITIONS TEMPORARY CONDITIONS (MI+CDI+CRI+SI)	A POLY	6.121-	67.9	-12,5	-143.2	75.4	1 4														
OWH		30	7 210.8	•		7 198.2	3/4.5		179.5	6.0	35.	1 72	9.8.8	143.6	90.6	87.8	69.3		135.7	198.5	189.7
T ST	CHIL A + mD +				5 -2.9	9 -1.7	1.9 0.3								<i>'</i>						
LOAD	E E	1 : .		4 1.7	0.4 -7.2	0,4	4.0		~	~	0.0	9	2'0	: 0	0 0	000	<del></del>	80	90	۵0.	
L LOAD DEAD LOAD LIFTING L	-	ه ا	-9.8 8.	0 61	0 0.7	-10.2	0.0		0	0											
CAL E	31–Cl	31.0	-42.B	1:17	29.5	4.4															
VERTICAL CRANE D CRANE L CRANE L	+	13.7			<u> </u>		- 86.6	9.6	7. 5. 7. 7. 7. 7.	33.5	0.0	9 9	13.6	20.6	9.6	0.0	-16.2	0.0	000	0.0	0.0
		, 9	0.0	0.0	<del>  </del>	1 000	0.0										<u>;</u>				
VI. CDI. CLI.	NET COL	1 : 7	81.4	8/14		81.4 0.	1.6- 4.18	~	2.4	2.5	: ~	50	<i>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</i>	0.0	0	0.0	0		C é	3.0	0.0
NOTE:		<b>'</b>		ļ	<del> -:`</del>	0.0	8 0.0	4	4	~							: 0				0
1	CDI.	12.7		000	5.7	9,2	0.0		:												
STRESS		, °	····.	28.4		28.4	28.4			9	<del> </del>	2	6	) 6	0.0	-	0.0	9.0	•	0.0	0.0
COLUMN	VI TED +	1-11-1	8 14.5	4-109	7	3 16.3	5 -2.2														
A. R	<b>(1)</b>		129.9 67.8		17.4 -127.0	116.8 75.3	133.1 -14.5	86.4	125.3	00.0	35.7		'n	7 7 7	90.8	82.8	89.3	/36.1	135.7		189.7
TABLE F	DIRLEV	RF 65	ų i			OPE	MEE 13.		1	1	1	1		7		1	Ţ			ME & 1.9	11
TA TA											·	ļ									
	EOCA-	A-107	one W		A-108 A-201	•		8-101 208	201-8	8-103	B-104	13-105	8-106	B-107	8-100	8-701	D-101-0	207	(o)-()	₹01-Q	102-0

	(柱 応 力 获 ).coca-pircev.	۲ ۲ ۲	MEK 81.1	MEE 128.4		43.2	15.4	1,60.6			9.26	RF 36.7	5TH 59.9	MEZ 18/1	53.8			53.8		373.7	
FOR COLUMN		M, tmO, t										40.7 -2.1	34.4 9.1	2.8 0.4	8.0 2.14	22.6 6.1	3.0 0.5	71.3 7.3	22.6 6.1	2.8 0.4	
STRESS (	CDL	N, t M, tm	0.0	0.0	14	040	ر د د	5.0	6.0	0.7	0.5	\$'0- 2.0-	13.1 7.4	11.5 -0.1	-0,2 -0,5	13.1 7.4	1,5 - 2,1	-0.2 -0.4	13.0 9.4	10 66	
NOTE:		10, t N, t	Š	0.0	e e	200			6.0	8.6	0,6	7.0- 5.0	2.2	0.0 15.0	0.2 0.1	2.2 19.1	0.0 15.0	0.2 -0.7	2.2	0.0	
VIL - 100 CDIL -	SL	1	•									-1-4	4.7	0.0	-1.4	9.0	0.0	-1.4	9-6	9.	
VERTICAL CRANE DI CRANE L	- SEISMIC	ر د د	0.00		-40.7	0.0	40.7	-40.7	-3.9	0 6 %	-36.7	0.0 5.0	2.9 4.5	0.0 -44.3	0,2 4.3	2,9 4.3	0.0 -32-4	0, 2 4.4	2.9 4.4	24.3	- <del>-</del> -
1	IC LOAD St	۲, tmb,										68.7 22.4		1.7 0,3	67.3	-57.5-15.8	1.9 0.3	68.6 22.4	-57.8 -16.	2.4 O.4	
L LOAD DEAD LOAD LIFTING LOAD		t N, t	00	0 %	^	0 6	20	9	0,6	0.0		-0.3	-6,3	-0.6	22,2	. 6.3	-0.6	-0.3	20,3	9.0~	
L SE	Ŧ	M, tmQ, t	*									-1.3 -0.8	0.1	00 10	-1-3 -0.2	1.0 1.0	00 100	-1.3 -0.2	0.1 0.1	-0.0	<u>:</u>
CRA PER	(WL	N, t M,	8/-1	178.4	7. 7.	43,2	76.0	<del>}                                    </del>	189.4			36.7	72.0 44.	196.1	53.3	103.8 32.	375.8	183.8	176.6 32.	386,3 2.	
NE HORI MANENT PORARY	(VL+CDL+CHL+SL)	tmO, t N										1.2- 1.04	9.21	2.8 0.4	71.2 7.3	3 9.0	3.0 0.5	91.3 7.3	2.2 9.0	7. 0.4	
CRANE HORIZONTAL LOAD PERMANENT CONDITIONS TEMPORARY CONDITIONS	L+SL) TC	μ	8.96	128.8	(45.1	43.2	116.1	181.3	193.2	184.3	1332.3	40.7 107.6 22.6	66.5 99.4 21.7	237.5 1.1 0.	57.6 139.3 29.5	98.5 89.4 24.0	345,4 1,1 0.2	51.7 138.4 29.7	168,4 82.7 24.2	431.6 0.4 0.0	<u>:</u>

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CLL	RKS										:										T	
(vr+cll	REMARKS	_	:00				:2		:15	7:		-:-	:	:0	:0	و:					-	
LOAI	1 1	*! 앝	6 29.8	3 22.3	0.0	0 29 6	22 0	0.0	.6 2)	F 24.2	0 0 0	135.	8 27.	0	1.740.9	0 24	0, 9					
NTAL		M, tmg	138	2	7 6.2		œ	5.0 3.	13.6	97.4	0.5 6	F 16Z.1	3 47.8	7 5.8	:87	.5 89.6	<u> </u>	M	4	m:	0.7	
CRANE HORIZONTAL LOAD PERMANENT CONDITIONS TEMPORARY CONDITIONS		۲, ک	\$ 55.2	0 1912	464.1	\$ 55.2	7 104.1	4 366.6	3 59.6	88.9			8 1163	393	3 81.6	5 125.5	7 486.3	70.	148.4	12513	313.0	
IANEN ORAB	5	tmO, t	4 10.4		9	4 10.4	7 8.0	4 0.4		3 9.	500 9	.9 9.3	9.0	1.0.7	6 15.	3.7	7.0 5	:				$\dashv$
CRAN	S	Σ	5 23.4	0.28.7	9 2.6	5 83. 4	.2 28.7		2-12 8	0 32.	5.8 2.6	6 84.9	9 39	: S	4 //2 6	6 23	.5	6	2.18		0.602	
		Ψ', Z	:23	0.601	378.9		1 104.2		2 53.8		8382 6	2 65-6	0 116.9	297	2 77	0 134.6	0 481.5	42.8	20	87.	2	
금없임		tmD, t	9 6	1 2	0,0	9.0- 6	0	000	3 -0,	101	0.0	2,0- 4,5	-	0,0	4:	0,0	0.0			:		$\dashv$
LOAD	동	Σ	7 -2.9	_	3 0,0			3 0.0	3 -1.3	9	1:4-	: 1		3 -0:)		. 6	3 ~0,	0	-i-m			277
1 4		R, t	2 -0.7			0-0.9	.0 -0.7		į	.9- 9.	4 -0.6	7 -0.4	1 -0.4	্র	7 10.4	40-	, o -	0.0	6	0.0		
LOJ TRAD		, tmO, t	. 2	0-15.	7 0,4		1-15.0	5 0.4	67.4 22.2	4-15.9	5.0,4	79.5 25.7	4-18-1	•	18.8 25.	p.0-17.9	9.0.4					:
C	11-41	Σ	2 58.3	2 -55.0	3.2 9.		5 -54.1	3.5		3 -57.4	5'2   ? ;		965.4	2.0		9 - 65.0	9 2.9	5	6.4	0	9	
VERTIC CRANE CRANE		۲ ح	0,0	3.2.2	2 -87.6		3.2.2	24.3	2 43	9 4.3	5.95.	5.4	3 5.0	2. 0.0	18.4	3 9.9	2 8.9	27.5		38.0	105.9	
		tmo, t	3 0.5	3 2.8	2 0.0		3 2,8	2 0.0	4 0.2	. 2	1 0.0		8 3.3	5.0.5	1.0.4	3.3	2 0 2					
VL CDL CLL	CLL	Σ	్ ర	5	9		5	21- 6		9.6	10- 9	8 [-1-]	4 10.8	3 -0.2	. 7	4 10.8	3 -0.	. 0		o	7	
E _		S,	3 -6.3	Ξ	13.9	98	<u> </u>	W	2 -0.4		0 12.	3 ~0.8		, ý 1	3 -0.8	19	5 76.	0.0		: 0	1	
NOTE (+)	CDL	를 다 다	ó	3.2.1	0.6			0.0	0.2	4 2.2	0.0	3 0.3	2 2	3 0.0	м 6	₹ 2.5						
STRESS			0		9	0.1	2 7.3	٩ ٩	2 -0.4	0 7.4	9 -0.1	, é	8 8 4	12.5 -0.2	3 - 0,	\$ 60 €	12.5 -0.2		:			:
1	1 1	7	6	2 13.2	. 6 6 6		2	f 10.8	3 -0.2		4 9.9	\$	7. 4.0	ائ_ا	\$ -0,3	2 14.8		•		0.0	717	:
COLUMN		M, 4m0, t	1 9.9	4 5.2	* _	9.9	4 5.2	9.0.4	2 9,3	7.6.2	P. 0.4			.3 0.5	.6 15.3	5 4.2	4.6.5					
		1	83	5 19.4	50 Z.B	83.1	7 19.4	9.2 6:	8 41.2	9 22.7	2 2.7	6 84 4		: 'n	4 112.6	115.2 16.5	φ. 			: :m	2	
	ļ	r S	\$3.0	1 (5) 5		53.8	1 06.7	8 295.7	53.8	1 79.9	8 273.2	9:59	4 97.5		97.4		2.596 8		i	1 87.3		:
TABLE	LOCA-DIRLEV.		정	길	MEZ	a	STH	MEZ	RE	15	MER	R	HLS	MEE	S T	T Z	7	£	7 73 79	4 I	15. 15.	
	A-PI		4	<u></u>	<u> </u>	-105	• ;		901-			-107			106	<u>.</u> }	-	(01-	5	-11 2		<u> </u>
	COC		401-6	٧.		A-105 204	.:		6-106			69-107	, 		901-5	*		H-10)	<b>.</b>	1-11 2		

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:	(VL+CLL)	REMARKS				-									-							
٠.	LOAD	tmo, t																				
	CRANE HORIZONTAL LOAD FERMANENT CONDITIONS TEMPORARY CONDITIONS (VL+CDL+CH+SL)	Ε Σ	7.2	3.86.6 378.8	219.9	398.2	137.8	337.5	184-1	2:901	250.4	658.1	5.13.9	730.2	214.5	4.19.5	87.6	221.8	7-59)	1.50>	9:55)	323.6
	CRANE HORIZON PERMANENT CON TEMPORARY CON	t t																	<u>:</u>			
:	111	ν + Σ	162.9	284.4	175.4	309.0	8.101	235.⊄	148.1	341.9	202.8	\$252	235.4	294.5	78.5	375.3	66.1	151.18	183.7	288.5		0.64
)	CHL PC T DT	t.30, t																				
	L LOAD DEAD LOAD LIFTING LOAD LOAD	Z CH		9.6	2.4	0.6	4.0	9.6		-0 0	0.0	0.0			0.3	9.6	0.0		0.0	0.3	00	<u> </u>
	L LOAD DEAD L LIFTIN	ᄴ														*****						
	VERTICAL CRANE D CRANE L SEISMIC	Z S		-105.7	42.6		380	67.0		-13.4	:	47.6		-133:1	.38.0	105.7	27.5	73.5	74.5	9.717	34.5	0 : 0 0 : 0
	1 1 1	CLL M. tmo, t											-									
.)	E : VL CD CL SL	₹ K	2.0		1	3.6		: Q	20	30		4	<del></del>	4	0,5	د د د	00	Ą		4	0 0	
•	( 'Z)	CDI.																				
-	STRESS	1 1	1	, , , , , , , , , , , , , , , , , , ,	/. 3	2,4	۶,	2.4	, m	1:2	0.0			<u>-</u>	m	2,3	0.0	2.0	0.0		1	
	FOR COLUMN 九 桒 1	VE M. tmb, t N. t																				
		بر ح	160.3			305.4		231.8	<del>}</del>					593.	1	37/.8		ļ	-	<del></del>		15/5/
	TABLE (件 応	LOCA-DIRLEV.	41H	MBR	47.4	MER	47TH	#E8	4T4	M. M.	4 KT X	t t	47.1	7	4+1+	MER		MFR	7	ָּרְנָי עני		7
		TOCAL	402 206		H-104 205		H-105		901-H		11-107		H-108	,	4-203		K-106		K-107		K-108	
φι														÷						-		

٦	KKS.						T .			<u> </u>				:		*					
(vi+cll	REMARKS													1		<b>.</b>		•			
CONDITIONS (CONDITIONS HL+SL)	+	. 1 .																•••••			
TAL	· 🔍	-																	•		
NO2 CONICONI	į.	18,2	3.75.8	(3).9	334.9	82.8	22.5														
ARY C+Q	+	, :														******					
CRANE HORIZONI PERMANENT CONI TEMPORARY CONI (VL+COL+CHL+SI	PC +		1														:				
	 +	:-	2.062		<u> </u>	<del> </del>	174.3													<u>:</u>	
	- Z	1:	: ^	12	<u>                                     </u>							:			- <u>-</u> -	:	<u> </u>			:	
공원	2 1 1 1 1			1			1							:			-			:	
L LOAD DEAD LOAD LIFTING LOAD LOAD	E F	<b>!!!</b>	<del> </del> -			0	:	<del>                                     </del>	1	:											
LOAD	≥	: d	<u>ا</u> خ	0,0		0,0	ರ				-							<u>;</u>   <u>;</u>	<u>:</u>	:	
L LOAD DEAD L LIFTIN	ST.				<u> </u> 		-	-		<u> </u>	:				:		i.				
NE DE	SL			lo:	8:	0:40	20:10													:	
VERTICAL CRANE D CRANE L SEISMIC	+ 2		0.98	2.60	-86.0	27.5	73.5								:						
	‡ <u>ç</u>				1												<u> </u>				
VL CDL CLL	CLL + #D																				
••	‡ 2	-1 6	6	0	4	9 9	4														
NOTE ( 4 )	ţ																				
ł ·	CDI A +mD		:																		
STRESS	+	9 6	20	0	7.0	0	5.0		i												
	+	<u>.                                    </u>												1 1						•	
согоми	VL +mo			-										:	<del>                                     </del>	1 2	li				
ror 表	  -  -	1	<u> </u>	105.2		55.3	178.9	H										╁			
TABLE I	_ ≥  -	4TH 11				+	HES "		<u>  : </u>		<u>:</u>		:	:	-	-	<u>                                     </u>	<u>                                     </u>	1 : 1 : 1	:	
TAB	I RUE	4	\ <u>\\ \forall \\ \forall \\ \\ \equiv \   \( \forall \)</u>	4	[ ]	4	I	<u> </u>	<u> </u>	1		<u>]                                     </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	1	<u> </u>		<u> </u>	L
TABLE 1 [柱 応力	FOCA-D	162-X	· <del>  </del>	K-202	:	K-203			:	• • • • • • • • • • • • • • • • • • •											·

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		-	•	£					МО		σb/					İ
1		חקר	ፐሮፐስ	M OF	COT.	UMN	/ 1 ×				o c/			160	< 1.	, 1
	- ;			面質	4.5	Ollin	( 1 )				O D/	דט יר	συ,	/ I C	, +.	۱ ۱
LOCA	DIV			MANE		TEM	PORA	RY	MEM	BER	Α.	STRE	SS I	σb	σC	o b/fb
		Х		DITI		4.00	DITI				i, n	λb			σC	+ -
пои	NO.	Y	N	M	Q		M	Q	: (A,		imin		fc	fb		o c/fc
A-101			1			100			BH-700x350	0.9×85×	9.4	46.	!.6	1.07	0.12	
106	_3_		54.5	110.6	17.8	58.0	123.0	11.4	453.6	10,311	7.95	59	1,35	0.67	0.09	0.26
1,00			[			201.5			BH-900x35		9.15	. 60	.1.6.	0.41	0.35	
203	2		179.3	39.1	12.7		103.6	<u> ₹₹</u> 8	509.6	14,372	7.5	73		0.26		0.56
208	,					280.3					.9./.5.	7./	1.6.	0.07	0.43	
-	_/_	ļ	214.1	10.6	1.7	1916	22.0	<u> ታ∙</u> ዊ	Ditt BH-700x94	D 28 2 4 0	7.5	87				0.46
A-107	9					03.3	, 50				10.8.	70	. l.b.	1.24	0.16	- 50
5	_ ري_		78.<	143.2	21.0	06.6	755.0	13.1	493.6 BH-900x90		10.5	46	1.41	0.78		0.89
202	2		2m. 8	47.9	14.6	11.7.4	118.7	26.1	549.6		8.8	.પ્રસ	1.27		0,30	0.56
		<del></del>	200	<u> </u>	1	102:1	1, 4. /			10,0.0	18-2	.62.	3.6	0.08	0.46	
	1		254.9	12.5	1.9	222.4	25.2	3.9	Dì t	t n	8.6	74	1.16	0.05	1 .	0.45
6		ĺ								50 X28 X40		.46.		0.8/	0./2	
G-101	<u>_</u>		33.5	83.4	10.9	55.2	138.6	29.8	453.6	10,311	2.95	54	1.35	0.51	0.09	0.60
106									BH-900×3	50x28x40	915	. 5.5.	/.6	0150	0.35	
203	2	<u> </u>	177.0	28.7	8.0	171.2	81.3	22, 3	509.6	14.372	7.5	67	1.23	013	0.28	0.41
208	<b>.</b>									•	2.15	7.1	. Js.b	0.02	0.76	
	1_		386.3	2.7	0.4	431.6	0.4	0.8	Ditt		1.5	87	1.05		0.75	0.76
G-107			:				50.5			0 X 2 8 X 4 O	10.8	.40	.:(1,6,	0.70	0.16	[
1	3	<del> </del> -	77.4	112.6	15.3	8/-6	184.1	40.4	493.6	16.016	9.3	4.6		0.04		0.55
202			120 8	22.2	.0.0		60 B	.,,		04×8×40	.19.5.	<del>4</del> .8	4.6	0.17	0.24	
	2		134.6	< /- 3	7.5	125.5	87.0	24.1	549.6	16,016	8.8	57		0.11	0.18	0,29
	1		281 5	3.2	0.7	486.3	6.0	0.9	Ditt	_	8.8	6.≥ 14	1.16	0.05	0.28	0.77
B-10]	-		.7011.2	314	0.7	700.3	0.0	1 0.7			0.0	7-	1110	0001	0.78	0177
103												• • • • • • • • • • • • • • • • • • • •	•••••			
106									1		1					
108																
563									H-30	10 ž	ļ				0.82	
200	1	<u> </u>	98.8	0.0	0.0				119.8		7.51	87	1.02		0.80	0.80
18-102											. <i>.</i>	<b></b>			{	
202	<b> </b> -	ļ- <u></u> -			·····				ļ	<del></del>	<del> </del>	<del></del>	<del></del> -		<del> </del>	
207	1							ļ			ļ	ļ	<b></b>			
'		<del> </del> -	<del>-</del>				<del></del>	ļ <u>-</u>	H-35	0 2		<del></del>	<u> </u>		0.83	ļ
	1		143.6	0.13	0.0				193.9		8.84	74	1.16		0.72	0.72
10.101	<del>}</del> 	<del>                                     </del>	, , - , 0		V.V.	<del> </del>		! 	1 1 2 1		1 <u>01 0 7</u> .	1 · · · ·	<u> </u>	<del> </del>	14.7.	
D-101		/							1			[	······			
		T-							1			·	[		T	
						1 . 1 .	· 					L			<u>                                      </u>	
		1: 🝈		:	* 1			]	.H-3	00.5		ļ	ļ		0175	
	1_	Ĺ	89.3	0.0	0.0				119.8		7.51	87	1.05	<u> </u>	0.74	0,74
								N NU	MBER C	F COLU	IMN					
					-	X O		D-11-1-	T.100	arearen f		0.775	<b>,</b>			
										MENT (t						
	Α,Ι	, 4								'RY MOM	TALI	OF. I	MERT	IA (	cm4)	,
	A C	का का				MODU		•	,							
1						ABLE			NT O'ER P	LLOWAE	יים אינו מינו או	י רוזאיז	MC C	ተውውረ	20	
1									TION (		מ מוני	TUNDI	טאר פ	TUES	Ç	
1										BENDI	NG M	IOMEN	T AN	ID AS	KTAT.	FORCE
1 .										NG ANI						
<i>\$</i>															- *** 65	

			<u> </u>	·					NOTE:	σ b/	fb ≺	1.0	***	- 01	
			-							σ c/	fc <	1.0			1
						UMN	(2)	÷		σ b/	fb +	σc	/fc	< 1.	0
COCA	DIV		の断 PER			TEM	PORA	RY	MEMBER	Α.	STRE	SS	σb	σC	σ b/fb
]		Х		DITI	ONS		DITI				λ.b.		o b	OC	+
TION	мо.	Y	И	M	Q	N	M	Q	(A,I,Z)	imin	λс	fс	fb	fc	σ c/fc
D-105													•		
20 7															
									H-3502			:		1.14	
	1		198.5	0.0	0.0				173.9	8.84	74	1.16		0.98	0.98
F- 101 F- 103			N 1										* .I.s	<b>.</b>	
100				<u> </u>			<del></del>	3.5					 !		
563							-					3 7 7			
805		 							H~300 2				:	0.87	- , _
	1_		104.5	0.0	0.0				119.8	7.51	87	1-05		V82	0.85
F-102	l		·											<u> </u>	
1971 202 207															• •
						· · · · · · · ·	·		H-350°		,			1.09	
	1		189.4	0.0	0.0	:	1		173.9	8.34	74	1.16		0.94	0.94
4-101														<b>.</b>	1
208					<u>-</u>				H-300 E				_ <del></del>	0.36	
	۷		42.8			70.3			119.8	7.51	67	1.23		0.29	0.29
			0.4.3			148,4			Ditto	7,51	 87	1.02		1.24	0.81
H-105	<u> </u>		81.2			14011		]	DINB	1,21		1.00		0,0,	2101
201									H-390					0.64	
	Z		87.3			!			136.0	7 28	69	1.21		0.53	0.53
	<u> </u>		1, 1,	- <del></del>					H-4002					0.96	
			209.0						₹19.7	10,1	64	1.26		0.76	0.76
H-103					<u> </u> 								Ì .	1	
(001) Pos									H-440					103	
506	2_		162.9		ļ		<u> </u>		157, 4 BH. 4502	7.18	70	1.50	ļ	1.18	0.86
	1		341.9			, .			289.0	11.46	57	[-35			0.89
104									: :						
H-107					<b> </b>	<del> </del>		<u> </u>	BH-480 X350 x 16x 2 L		<del> </del>	<del> </del>	<b> </b>	1.07	
202	2	<u> </u>	239.4	}					223.76	8,38	60	1.30		0.82	0.82
205									44. 219x300					1-20	
i			394.5			DIV	ISTO	N NII	495.92 MBER OF COLU	12,96 MN	1 20	1.38		10.87	0.87
	DIR		- DI	RECT	ION,	ХО	RY								
									ING MOMENT (t						
,	н, Т	, 4						(2), (cm3	GEOMETRY MOM	ENT	Ot I	MEKT.	TW (	cm4)	
			SS -	A	LLOW	ABLE	STR	ESS			and the second				
									N OF ALLOWAB	LE E	ENDI	NG S	TRES	SS	
									TION (cm) NT FOR BENDI	NG M	OMEN	T AN	D AS	ZIAT.	FORCE
									BENDING AND						
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SHEET 143 OF

			<u> </u>										T 14:	OF	
									NOTE:	σ b/f					
		ከድሮ	ISIO	እን ሰፑ	COL	IIMN	151			σ c/f σ b/f				2 1	,
			の断			Othi	۱ ی			0 0/1	. D	0 0	/10	` 1.	
LOCA	pīv	DIR	PER	MANE	NT		PORA		MEMBER	A.S	TRE	SS	σb	σC	σ b/fb
		Х		DITI			DITI	ONS		1,7 /		fb	σb	σc	+
TION		Y	N	М	Q	N	M	Q	(A,I,Z)	imin	( C	fc	fb	fc	σ c/fc
K-106												• • • • • •			
	2	].	١, .				)		H-3502					0,15	
	-		60.1					<b> </b>	173,9	8.84	57	1,30	ļ	1.03	0.22
	1		179.3				:		Ditto	8.84	74	1.16		0.89	0.89
K-10J													<u> </u>	<u> </u>	
505	. 1	)	1	]					H-440					0.85	
	Z		133.7					<u> </u>	157.4	2.18	70	750		0.71	0.71
	,		304.0						157.4 11-4502 289.0	11.46				1.05	0.80
ļ —	 	<del> </del>	304.0	! 			<del>                                     </del>	<del> </del>	20/2	1/14	<del>-2</del> [	10.15		0.80	0.00
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NOT	ATI	ON:	DIV	NO.		DIV	ISIC	N NU	MBER OF COLU	NMU	<del></del>	<del>'</del>	<u></u>	<del></del>	
			- DI									<u>:.                                    </u>			
									ING MOMENT (						
	α, ±	, Z.					. (cm LUS		GEOMETRY MOI	PENT (	JE J	MEKI	. LA (	CIR4)	•
	A.S	TRE	ss -												
									N OF ALLOWA	BLE BI	ENDI	NG S	TRES	SS	
	imi	n -	M	MINI	UM R	ADIU	S OF	SEC	TION (cm)						
									NT FOR BEND						
<u> </u>	fb,	fc		ALLO	WABI.	e st	'RESS	FOR	BENDING AND	D AXI	AL E	ORCI	<u> (t,</u>	/cm2	)
,															

N. T. T. C.	TABLE TABLE	NOIE: FOR COLUMN BASE (/)	VL VEKIICAL LUAD CDL CRANE DEAD LOAD CLL CRANE LIFTING LOAD ST SFISMIC LOAD	CHL CKANE HOKIZONIAL LOAD PC PERMANENT CONDITIONS (VL+CLL) TC TEMPORARY CONDITIONS
Y         966         23.3         26.5         17.0         27.0         17.0         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.2         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         27.1         18.6         18.	LOCA-DIRLEV	VE VE CDE L. T.	St. St. N. t. M. t.	PC PC N. t M. tmO, t N. t
Y         162.4         19.0         1		0.79 2.8.3	-0.7 -30 1.4 0.1	3 163.8 -0.7 150
X         X	ļ <u>.</u>	23.3	149,6	209.4
Y         100         110	<u> </u>	24.(	-379 50.4	211.8 -0.4 -7.2 20.6
Y         150.3         74.5         1.0         -2.5         3.7         6.7         6.5         79.0         -1.0         32.8           Y         150.3         24.1         -0.1         6.4         4.84.6         4.84.6         6.2         6.7         6.7         7.92.7         7.0		24,9	-2.3	178.0 -1.0 133.0 3.6
X         159.3         221,1         -0.1         674         -2.64<	×>	74.9	-2.5	179.0 -1.0 133.9
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		24.)	169.6	219.7
X         [33.1]         258.3         0.0         4/2.2         2.1         0.3         0.4         27.4         0.1         27.4           86.4         1.4         0.0         4/2         0.1         1.1.2         0.1         96.3         0.1         18.4           86.4         1.4         0.0         4/1         0.1         1.1.2         0.1         1.2         0.2         1.2         0.1         1.2         0.2         1.2         0.1         1.2         0.1         1.2         0.1	1	28.3	42.4	250.7 42.4
10   10   10   10   10   10   10   10	1	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	-1907	26.13 26.1 21.8
10		2002		
10   10   10   10   10   10   10   10			1.6 2.1.	0.1 00.5
19,4	102	10.9	82.7	129.4 0.1 42,6
36.1   11.9	103	0.0 6.0 6.1)-	0.0 33.5 0.0 0.2	0.0 81.9 0.0 -3.3 10.1 5
36.7	104			j
96.3     11.9     0.0     2.5     0.0<	785			34.1
143.6	70/	111.9	33.6 0.0 0.2	0.0 98.8 0.0 13.6
828	202	-(1.9	0.1 3.4 52.0	143.5
828 . 0.0 0.0 -0.1 -0.7 17.7 -0.1 82.7 0.1 92.6	801	0,0	-4.7	90.7 00.1 100.6
	102	0.0	-9.7	1.0 128

	(VI+CLL)	REMARKS	Y Direction						YDirection	X Divoction	0:40	X Directton .	V. Divection				1	y Direction				
			0 0 V 4	34.6	36.4	33.5	33.3		29.7.	33.9	23.5			33:9	29.7	29.1		0.4.0				
:	CRANE HORIZONTAL LOAD PERMANENT CONDITIONS TEMPORARY CONDITIONS (VL+CDL+CHL+SL)	πc t⊞D																				
	CON CON HC+SI	<u>ج</u> بر	00.0	139.2	135-1	219,8	208.7		93.1	1.54	187.4		27.1	243.3	(82.5	73.6	9.9	44.3				
:	HOR NENT RARY DL+C			0.7	0.1	ŏ	0.7		-6.2	11.9	:-(1.9. -0.3		.(e.9,	-11.9	0.2	2.2	-0.3					
	CRANE HORIZONTA PERMANENT CONDI TEMPORARY CONDI (VL+CDL+CHL+SL)	PC + HD									i											
	O H H	+ Z	1 0	<u></u>	35.		187.7		978	128.9	104.3	23.2	76.3	761.9	187.9	(8/.0	93.5					
-(`) :	TOH TOH	+ 		6	20	0,2	5.0		· · ·	ò	0.1		0.1	0.1	0	0,0	ó					
• • •	D LOAD	된 된	: <u>  :</u> _			2							-:- -:-	/								
	D LOAD NG L(	_ Z		00		0	-0,	<u> </u>	0.0		3-0.3		-0	.0-	1-0-1	1.0,	20.3					
:	L LOAD DEAD LO LIFTING LOAD	+ Cm	30.4	34.2	36.4	33.3	33.3		862	22.0	22.0		.22.0.	30.9	29.1	29.1	22.0					
:	ৰ ত	S.L Cm + M					0	<u> </u>		7.	7	-	3.	- Jo		<u></u>	3.					
:	VERTIC CRANE CRANE SEISMI	Z		2.9		0.12-	1 - 21.0		100	82.7			3 -48.3	2 -20.6	1.9-	-6.1	3 -48,3	:				
:		tmO.t	- I - Z	6,9	ó	1.0	0.1		-0.2	2.0	6-		. 0	0.5	26	0.2	-0.3	:				
: •	VL CDL SL	ပြု		<u>ا</u>			2	:	n					M	m	3:						
	TE (	Z-	<i>`</i>	6	0	0.0	0.40	<u> </u>	2	2 6 5	6		6.9		2 /	7 2	0	<u> </u>	<u>                                     </u>			
:	( S)	Cm	0		Ó	ò			0,	9			9		9	9	ို	-		-		
:	BASE	CDL T M + mD					1		1				40 6	0		0	92					
	<b>1</b> 00	_ ≥	<u>:                                   </u>	0.2	9	9	۱۵		4,0	4					01	· · ·	% Ć	:		:	-:	
:	OLUM	VL M.tmD		:				:					11.9.	-11.9					<u>                                   </u>	<u> </u>	:	
· 1	FOR COLUMN	Σ Ψ	89.3	:-		5,891	1.7		2.58	<u> </u>	103.9	,	4	. 9	9	7	. 10					
i	LE F	_ z >	89	1.36.1	(35.	195	182.7	<u> </u>	<u> </u>	128.4	:00	43.2	25.4	760.6	9-881	1.641	9.26	<u>:</u>		:	:	
:	TAB	I 자.					•					-							<u></u>			
:	TABLE FOR (柱底力赛)	oca-b Ion	D-101	201	109 202	801	201	10 to 1	101-	102	103	55% 42% 42%	406	107 202	801	201	503					
	IL	16	<u></u>	لــــا	<u></u> -		<u> </u>	<u></u>	I.L.	<u> </u>	<b></b>		<u> </u>	I	<u> </u>	L		!			<del></del>	

(VL+CLL)	REMARKS		VI 16C 110M								Y Direction .		Ditto		X Direction	Y Direction	X Direction	Δ; ‡	Yourstian	
ONS	1	√×.	32.0	32.0.	9.0	36.9	36.9.	13.1	74.5		53.4	20.6	20.6		20.6.	38.3	13.3	1.15.9	39.8	
ZONTAL LOAD CONDITIONS CONDITIONS L+SL)	TC																			
120N CON CON CON	+	XS	378.8	339.0	469.7		349.5	468.4 366.6	438.9		2053	333.8	408.5 (35.6		359.5 106.5		300.6	814.5 372.9	499.1	
HORI NENT RARY DL+CH		.  <u> </u>	0.50	-15.0	0.0	64.9	-14.9	.30.3	-30.3		9.7	0.0	0.0	0	0	2.91-	800	5.05: 0.0	0.51-	
CRANE HORIZONT PERMANENT COND TEMPORARY COND (VL+CDL+CHL+SL	PC +															*****				
5665 111	+	رِّ ا	3/60	386.4	378.9	3.605	285.9	398.0	181.7		81.2	209.0	284.4	309.0	235.4	341.9	\$22.4	594.5	375.3	
12 22 1	4	,	ó	00	0,0	٥	0.0	1.0	1.0		00	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
	CHI																			
LOAD NG LOAD	1	:L	20.5	9.0-		7	9.0-	200-	-0.2		0, 3		9.0	9.0	9.0	9.0		0 0	9.0	
L LOAD DEAD LOA LIFTING LOAD	:    -  -	· ·	2002	22.0	4.0	22.0	22.9.	44.2	44.2		43.7	20.6	20.6	0.0	20.6	20.6 59.8	64	1.69.1	20.5 8.72	
SAL LOA E DEAD E LIFTI	SL							••••							••••••					
ERTICAL CRANE D CRANE L	+		d:&: ⊻	-663	87.5	56.33	52.6	74.3	-19.3	:	124.3	5553	0:58 6:57-	89.0	6.23	-125.3	220.9	250.4	125.3	
(S	1	, i.	0		T-:-	0	:	1-0	1:0	:-	0.0	0.0	00	0.0	0.0	:0	:0	0.0	0 0	
VL CDL CLL	CLL																			
	1		1.5	7 2/	6.53	6.87	12.9	(6, 5	5.91		7.7	67	3.5	3.6	3.6	45	4,	4	3.5	
NOTE	3 1	1	9 9	7.0	00	0 0	0.7	- 6	0		0	0.0	00	0	00	0.0	0.0	0	0.0	4 1
	CDL						:				1 2						1			 1
BASE	1	1	7:1	0.01	φ 4	8,0	10.0	12.6	12.6		27	11.		2.4	4,7	2,3		9		 1, 1
Ĭ	1 1		3,5	-/5,0		.6.9	-19.9	J.	-39.3		0.0			:		-16.5	8.00	8	-/5.0	
COLUMN	VI.																1			
FOR 力 来 J			<del></del>	372.7	965.0	246.7	273.2		445.2		2 64	6.44	780.0	42.0	8 7 8	2002	627.0	ź	377.8	
			<b> </b>	<u> </u>					1	<u> </u>									1	
TAB	H Q		<u> </u>																1 1	
	LOCA-	4-101	207	205	104 205	§ \$	205 205	107	801		H-101	102	3 %	\$ 5	34	106	107	80 2	3	• Fi

	(VL+CLL)	REMARKS	Y Divection	_ ~1	Ditto	Ditto	0;40	\ <u>}</u>				,									
	LOAD ONS ONS	τ \$	11.4	9.02			3,02.	39.9													
	CRANE HORIZONTAL LOAD PERMANENT CONDITIONS (TEMPORARY CONDITIONS (VL+CDL+CHL+SL)	M, tho																			
	CON CON CON	S.	272.1	t _	1389.8			25.5													
	HOR NENT RARY DL+C	t t	76.5	0.0	0.0	0.0	0	15.0													
	RANE ERMA EMPO	N. C. E.											1	:	٠			***	:		
		, Z	/48.2	288.5	304.0	290.2	740.1				1						•				
$\bigcirc$	HO L L L L	Q Q	6	ó	ó	0	0,0	0 0													:
	D	조 건 1 1 1 1 1																			
	် လို ဗ္ဗ	Z t	مر ق	ـــــــا	۵ ۳	o vi	6	. •													
	VERTICAL LOAD CRANE DEAD LO CRANE LIFTING	t g	50.6	20.6.	ě	ė	20.6	20.6													:
	VERTICAL CRANE DE CRANE LI SEISMIC L	SI. M, tmD		- :0	~~																:
	VERTICA CRANE CRANE SEISMI	Z 47	2. 8. 8. A.		38.0	38.0		1.29.3													
		\ \f	0.0	0	0,0	0 6	0	0.0	<u> </u>												
	VL CDL CLL	M, tho					<u> </u>			<u> </u>											
( )	 ພ	\t	0.4	9			1. *	0													
•.•	NOTE	, t		0.0	0.0	0,0	0.0	0													
	ш	CDI.																			
	BASE	¥,	6.5	5.6	, 0	9, 2	ļ	-نــــــــــــــــــــــــــــــــــــ	{— <u>:</u> -												
	COLUMN	VI. M, tmO, t	16.5					12.0	1 :		<u>                                     </u>			<u>                                     </u>		<u> </u>				*:	
٠.		f. i				:													<u> </u>		
	TABLE FOR [井 庇 力 崧	با ا	147.8	288.1	303.6	287.8	78.7	6 841													
	TABLE 1 (特 码 力	品で									<u> </u>										
		LOCA-DIRLEV	901-X	LOI	801	loz	202	203					<del></del>			•	 <del></del>	٠.	·····	<del></del> .	<del></del> .

	DECISION OF COLUM	N BASE ( 1 )			
	「柱脚の断面算定」				`
LOCAT	ION	A-105 (101,104,10	5,204,205,208)	A-103 (102,106,20	3,206,207)
COLUM	N SIZE	BM-900 x 350 x 281		BH-900x350 x26	
DIREC		Х	Y	X	Y
PERMA	[		0.5	5.0	
CONDI	TIONS N (t)		179.0 1.0	211.8	
TEMPO			1.8	10.3	
CONDI	TIONS N (t)		/33.9 3.5_	-7.2 20.6	
				same as A-	185
		1-1 / 1	700 14+H+	20WG 42 4-	
	· ·		± 58 8		
FIGUR	E		E 4 7		
		ا مره اصلاد	E E		
		1300			
BASE	PLATE (LxBxt)	1300 × 700 X	29	1300 × 0	200 x 29
CAP P		1300 1700 1	<i> J.</i>	1390.5.2	
RIB P				/	• • • • • • • • • • • • • • • • • • • •
WING		/		2	
	R BOLT (n-Dø)	8-309	· · · · · · · · · · · · · · · · · · ·	8- 30	
SHEAR			••••••••••••••••••••••••••••••••••••••		
	e=M/N (cm)	$\frac{0.5 \times 10^2}{179} = 0.28 < \frac{9}{2}$	30 = 21.7		
	σc <fc (kg="" cm2)<="" td=""><td>179.0x103 (1+ 6x0.28</td><td>)= 21.5 &lt; 70</td><td><math>\frac{211,8\times10^3}{65\times130} = 25.1</math></td><td>&lt; 70</td></fc>	179.0x103 (1+ 6x0.28	)= 21.5 < 70	$\frac{211,8\times10^3}{65\times130} = 25.1$	< 70
ANCHOR	P/(n*A) <ft< td=""><td>85 X130 \ 130</td><td><del></del></td><td>N/PP=-00002/b</td><td></td></ft<>	85 X130 \ 130	<del></del>	N/PP=-00002/b	
BOLT	(t/cm2)				=130×65×Pt/2=5,5cm2
BASE	$M=\alpha$ wlx <sup>2</sup> (tcm)	0,09 x 0.022 × 32,5 2 =	= 2.1 T.Cm	0.09 x 0.025 x 32.	E = 2.4 T. CAM
PLATE	t>./ 6xM/fb (cm)	16x2.1/1.85 =	2.61 →29		.79 -12.9
CAP	$M=\alpha$ wlx <sup>2</sup> (tcm)				
PLATE	t>/ 6xM/fb (cm)			16x1.51/2.77 = 1	18 ->2.2
	$\tau = \sigma \text{ cA/(txH)}$ $(t/\text{cm2})$	$\frac{0.022 \times 32^5 \times 35}{1.6 \times 50} = 0$	31 <0.9	$\frac{1.90 \times 2.06}{4.6 \times 50} = 0$	15 (1.35
RIB	H/t	1			
PLATE	*************	<u>50</u> = 31,3	< 71		
	WELDING			[	
<u> </u>	$\tau < fs (t/cm2)$		· · · ·		
	$\tau = \sigma \text{ cA/(txH)} $ $(t/\text{cm2})$	0.022×37.5×15.0 = 0	.15 < 0.9		
WING	H/t	1		1	
PLATE	************************	$\frac{50}{1.6} = 31.3$	<u>&lt; 11</u>	<u>                                     </u>	
	WELDING	1			
L	τ < fs (t/cm2)				
	CONC. :fc(t/cm2)			<b></b>	
ABLE	A.BOLT :ft(t/cm2)			ļ	
CCANIC	PLATE : fb(t/cm2)	<b></b>		<b></b>	,
REMAR	PLATE :fs(t/cm2) KS			(150×55-130×1.5)×7×1.5	11m = 9527
	ION: L,H,B,t,D	LENGTH HEIG	HT. WIDTH.	THICKNESS.	IAMETER (mm)
	e ECCENTR			and the second s	
	oc COMPRE		N/(B*L) OR	OTHER EQUAT	ION DUE TO e
	P UP-LIFT	FORCE FOR CO	LUMN BASE (	t)	
	a COEFFIC	IENT FOR BENI	ING MOMENT	OF SLAB	

 $(\dot{\ })$ 

,							
	DECISION	OF COLUM	N BASE (2)				
	[柱脚の削	「面算定」					
LOCAT	ION		A-107. 202		A-108,201		
	N SIZE		BH-900x400x28	3×40	BH-900x400 x	04X85	
DIREC			X	Y	X	Y	
PERMA		M (tm) N (t) Q (t)	0.1 254,7 0.1		214,3		
TEMPO CONDI		M (tm) N (t) Q (t)	21.2 124.6 42.4		10.9 32.4 21.8		
			Same as G-10	8	Same as	6-108	
FIGUR	F.			•		· '	
1 100%						•	
BASE	PLATE	(LxBxt)	13@x 70	0 × 35	1300 ×	700 x 35	
CAP P		(THICK.)	2≥		1	22	
RIB P		(HxBxt)	16			16	
WING		(HxBxt)	22			22	
	R BOLT	$(n-D\phi)$	8-35	<del>p</del>	8-	35 <b>¢</b>	
SHEAR		(HxBxt)					
соис.	e=M/N	(cm)				•	
	σ c <fc< td=""><td>(kg/cm2)</td><td>254.7×103 65 ×130</td><td><!--70</td--><td></td><td></td></td></fc<>	(kg/cm2)	254.7×103 65 ×130	70</td <td></td> <td></td>			
ANCHOR	P/(n*A)<	ft	N/bDf=0008) Pt=0		N/bDf = 0.00217	2=0.02do	
BOLT	, (,	(t/cm2)	M/6024 = 0002) at=1	30x154/2 = 8cm2	1 '	1=130x65Pt/z=0.81m2	
BASE	$M=\alpha$ wlx <sup>2</sup>	2 (tcm)	0.09 × 0.03 ×32.5 ==			<u> </u>	
PLATE	t>√6xM7		J6x2.85/1.85 = 3.	1 ->3-5			
CAP	$M=\alpha$ wlx		0.085x{2×1.8×8.0/20×14				
PLATE	t>,/ 6xM/		16×1.71/2.77 = 1	92 ->2.2	ļ		
	$\tau = \sigma \text{ cA}/$		0.03 x32 5x35 20.5	7 <0.9			
RIB	H/t	(t/cm2)	1,2 × 50				
PLATE	WELDING	*************			·		
	τ < fs	(t/cm2)	<u> </u>			· · · · · · · · · · · · · · · · · · ·	
	$\tau = \sigma  cA/$	• •	0.03×37.5×17.5	018 704		•	
WING	H/t	(t/cm2)	1.6 × 50			·····	
PLATE	WELDING		:	,			
	τ < fs				<u> </u>		
		fc(t/cm2)		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
ABLE		ft(t/cm2)					
STRESS		fb(t/cm2)					
77.77.22		fs(t/cm2)	<u> </u>	· · · · · · · · · · · · · · · · · · ·			
REMAR		D 4 D	T ENCOUNT TIPE	CHW CITTONI	TUTOVNECO	DIAMERICA ( '	
NOTAT		B, C, D - ECCENTR		dni, widin,	TUTCVNESS'	DIAMETER (mm)	
:			and the second s	и//R*r) о	R OTHER FOIL	ATION DUE TO e	
	P	- UP-LIFT	FORCE FOR C	OLUMN BASE	(t)	WITON DOE TO 6	
L	α	- COEFFIC	IENT FOR BEN	DING MOMENT	OF SLAB		

	appearing the supplement year. On the state the section of the state o				
	DECISION OF COLUM [柱脚の断面算定]	N BASE (3)			
LOCAT		B-108 (101, 201, 2	08)	B-107 (102, 202, Z	(07)
COLUM	N SIZE	H-3002 X10 X 1	5	H-350 2 x 12 X 1	19
DIREC		Х	Y	X	Y
PERMA	NENT M (tm)		0.1	3.6	
CONDI	TIONS N (t)		90,7	143.5	
TEMPO	`		5.4	3.0	
CONDI	TIONS N (t)		100.6 17-8	60.9	
		1/4		1	<u></u>
					<u>8</u> 8
FIGUR	E		<u> </u>		1, 4
<b>y</b>	•	200 300 200 h	:	200 350 20D	51
DACE	PLATE (LxBxt)	7 1	3 V > 2	750×4α	\ \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
CAP P		700 x 350	(	/>vx4c	
RIB P		12.		14	
1	PLATE (HxBxt)			<u></u>	
1	R BOLT $(n-D\phi)$	4-259	)	4-25	5Φ
SHEAR					
CONC.	e=M/N (cm)				
	σc <fc (kg="" cm2)<="" td=""><td><math display="block">\frac{90.7 \times 10^{3}}{90 \times 35} = 37.0</math></td><td><b>〈</b>70</td><td></td><td></td></fc>	$\frac{90.7 \times 10^{3}}{90 \times 35} = 37.0$	<b>〈</b> 70		
ANCHOR BOLT	P/(n*A) <ft (t/cm2)</ft 	N/bof = 0.027 ) Pt M/box = 0.0024)			
BASE	$M=\alpha$ wlx <sup>2</sup> (tcm)	0.085×0.037 × 77.52	= 0.49		
PLATE	t>√ 6xM/fb (cm)	16x0.49 /1.85	= 1.26 -> 2.2		
CAP PLATE	$M = \alpha Wlx^2 (tcm)$ t>\sqrt{6xM/fb} (cm)	a de la fili			
	$\tau = \sigma \text{ cA/(txH)}$ $(t/\text{cm2})$	0.037 × 12.5 × 20 =	0.26 <0.9		
RIB	H/t				
PLATE	*****************				
	WELDING				
·	$\tau < fs (t/cm2)$				
	$\tau = \sigma  \text{cA/(txH)}$				
WING	(t/cm2) H/t				7.1 48
PLATE	WELDING		, 		
	τ < fs (t/cm2)				
ALLOW~ ABLE	CONC. :fc(t/cm2) A.BOLT :ft(t/cm2)				
STRESS	PLATE :fb(t/cm2) PLATE :fs(t/cm2)				
REMAR	KS			<u> </u>	
	ION: L,H,B,t,D	LENGTH, HEI	GHT, WIDTH,	THICKNESS, I	DIAMETER (mm)
	e ECCENTR		المراجع المراجع المراجع المراجع المراجع المراجع	OMUNN TOTAL	NTON DATE TO
	σ c COMPRE P UP-LIFT	BUBUR BUB G PSTAF SIKERS	, NY(B*L) OF	C OIDER EQUAT	TION DOR TO e
	COERTO	IENT FOR BEN			
	a COEFFIC	THE TOTAL STATE	TATE LIGHTIAL	ve vunu	

LOCATION	<u> 脚の間</u>		B-103 (106, 203,	206)	8-	105 (104,204	,205)	
COLUMN S	IZE		H-3002 X10X	15	1	H-3002 ×10 ×15		
DIRECTIO			X	Y		X Y		
PERMANEN		M (tm)				0.0		
CONDITIO	NS ·	N (t)	18.8	[		36./		
TEMPORAR	**	Q (t)	11.9			0.0	ļ	
CONDITIO		M (tm) N (t)	-3.3			*********		
COMPARAC		Q (t)	10.1		• • • • • • • • • • • • • • • • • • • •			
		<u> </u>		<del>,,,l </del>				
÷				<u> </u>	So	ime as B-	-108	
7 7 6 1 1 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7				<u> </u>	ĺ			
FIGURE			7	B.	- 1			
			व्या व्याच्या	-3-		-		
			700					
BASE PLA		(LxBxt)	700 x 3	50 x2Z		<b>100 х</b> .	350×22	
CAP PLAT		(THICK.)	.]					
RIB PLAT		(HxBxt)	. ]				2	
WING PLA		(HxBxt)						
ANCHOR E		$(n-D\phi)$	. 9.	.≥54		4	-524	
SHEAR KE		(HxBxt)	1	13-		<del></del>		
J., J.	7-1/ IX	( Cm)	$\frac{3.6 \times 10^2}{98.8} = 3.$	6 < 35	:		•	
σ	c <fc< td=""><td>(kg/cm2)</td><td></td><td></td><td></td><td>***************</td><td>********</td></fc<>	(kg/cm2)				***************	********	
	<u> </u>			5.20 = 65.2	< 70			
NCHOR P/	(n*A)<		N/bof = -0.0007	Perond Ho				
OLT ASE M=	·	(t/cm2)	M/bD24 = 0.0019)	10+ 20x15Pt/2 =	3.70m2	<del></del>	<del> </del>	
1 '		2 (tcm) Tb (cm)	0.085×0.065×15		,			
		2 (tcm)	0.085×(2x3·7x1.8/	$\frac{(15.5 \times 50)}{3.5 \times 5.0}$	= 0.71	-		
1		<u>fb</u> (cm)	J6x 2.71/2.77 :	= 1,24 -> 1,6				
	= o cA/		0.065 x17.5 x15			<del></del>		
<b></b>		(t/cm2)	1,5, 4,30	0.47 (	0.9	*******		
IB H/	t							
LATE	LDING		.					
1		(t/cm2)			}			
	= g cA/					<del></del>	· · · · · · · · · · · · · · · · · · ·	
		(t/cm2)			1	•	•	
ING H/	t			************				
LATE	<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>					***************************************		
	LDING	1 + 1		•		i de la companya de la companya de la companya de la companya de la companya de la companya de la companya de La companya de la co		
TIOU CO	< IS	(t/cm2) fc(t/cm2	<del>,   · · · · · · · · · · · · · · · · · · </del>					
BLE A.	BOT T	ft(t/cm2	{  <u>-</u>			• • • • • • • • • • • • • • • • • • • •	••••	
		fb(t/cm2				••••••	•••••	
		fs(t/cm2		*************		***************************************		
REMARKS								
NOTATION		and the second of	- LENGTH, HE	EIGHT, WI	DTH, THI	CKNESS,	DIAMETE	
		- ECCENTI		· · · · · · · · · · · · · · · · · · ·	T 1 OD 00	·		
			ESSIVE STRES F FORCE FOR			nek EQUA	ттои ро	
•			CIENT FOR BE			STAD		

<u> </u>	<del></del>				Ondina				
·			•						
		「OF COLUM 所簡算定]	N BASE (5)						
LOCAT		川川县水丁	h lal		D 1006 - 0	201 202 202)			
LOCAT	TOM		D-101 . 208		D-108(102,107,201,202,207)				
COLUM	N SIZE		H-3002 X10 X15		H-3502 X 12 X 19				
DIREC	TION		X	Y	X				
PERMA	NENT	M (tm)		0.2		0,1			
CONDI	TIONS	M (tm) N (t)		89.8	198.5				
		Q (t)		0.7	0.1				
TEMPO		M (tm)		9.1		/0.1			
CONDI	TIONS	N (t)		76.0	<b> </b>	2/9.8			
) }		1 Q (t)		30.4		33.5			
		,	Same as B-10	\R	Same as B-	107			
	19 July 19		JAPIC US O IC						
FIGUR	כו	ļ				6.5 E.			
FIGOR.	E.					<b>、</b>			
		1			<u> </u>				
				,					
BASE	ים של אינו	(LxBxt)	Day	N ( )	2	F			
CAP P			192X	\$50×72	750.x4	DX 2.5			
RIB P		(THICK.)			76				
<i></i>				2					
WING		(HxBxt)							
	R BOLT	$(n-D\phi)$		25. <b>9</b>	4-25\$				
SHEAR		(HxBxt)		<del></del>					
CONC.	e=M/N	(cm)							
	σ c <fc< td=""><td>(kg/cm2)</td><td></td><td>•••••</td><td>1-0 Page 3</td><td>• • • • • • • • • • • • • • • • • • • •</td></fc<>	(kg/cm2)		•••••	1-0 Page 3	• • • • • • • • • • • • • • • • • • • •			
	9 C/10	(Kg/Cm2)	**		198,5x103 = 6	5.2 <70			
ANCHOR	P/(n*A)<	ft			12740				
BOLT		(t/cm2)	•		2.554				
BASE	$M=\alpha$ wlx		<del></del>		0.085×0.066×17	12 = 1.15			
PLATE	t>√ 6xM7				J6x1.72/1.85 =				
CAP	M=a wlx		·						
PLATE	t>√ 6xM/								
	$\tau = \sigma  cA/$				0.016 x17.5 x20				
[		(t/cm2)			1.6 × 30	= 0.48 <0.9			
RIB ·	H/t		**************************************						
PLATE									
}	WELDING								
	$\tau$ < fs	(t/cm2)			<u> </u>	·			
	τ = σ cA/	(txH)		, .					
		(t/cm2)	•						
WING	H/t		**************						
PLATE									
	WELDING			,					
	τ < fs		•						
, . I	CONC. :	fc(t/cm2)							
ABLE	A.BOLT :	ft(t/cm2)							
STRESS	PLATE:	fb(t/cm2)	***************************************						
	PLATE :	fs(t/cm2)							
REMAR	KS								
NOTAT:				GHT, WIDTH,	THICKNESS,	DIAMETER (mm)			
		- ECCENTR			to the state of th	- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1			
						TION DUE TO e			
			FORCE FOR C						
	α	- COEFFIC	IENT FOR BEN	DING MOMENT	OF SLAB				
		· =							

LOCAT	【柱脚の街		in base (6)					
1	【柱脚の街		IN RASE (6)					
1	「住脚の世	「田算疋」						
			lis 162 (		T			
	KON.	* * * * * * * * * * * * * * * * * * * *	F-103 (101, 206, 20	8)	F-108 (102, 107, 201, 202, 207)			
I COLUM	N SIZE		H-3002 X10 X15		H-3502 X12 X19			
DIREC'			X X	Y	X X	Y Y		
PERMAI		M (tm)		3.6		0.1		
CONDI	rions	N (t)		104.8		189.9		
(7)		Q (t)		11.9		5.0		
TEMPOI CONDI		M (tm)	{	<u></u> 8.£		8.2.		
CONDI	TTONS	N (t) Q (t)	21.2	69. <u>5</u>	.	182.5		
		1-3-7-1-	1	≥9.8		29.1		
			same as B-10	8	Same as	B-107		
		•						
FIGURI	3							
BASE	DT ለጥሮ	(LxBxt)	Deal	V 5 3		4.50		
CAP P		(THICK.)	700 x 350		.	0x400 <i>x</i> 25		
RIB P		(HxBxt)	12	***********	-	16		
WING		(HxBxt)	<u> </u>	*****************	-	···		
	R BOLT	$(n-D\phi)$	4-25	<b></b>		4-254		
SHEAR		(HxBxt)				*******************		
CONC.	e=M/N	(cm)	$\frac{3.6 \times 10^2}{104.8} = 3.4$	70		•		
		Wali yawa w			.			
} :	σ c <fc< td=""><td>(kg/cm2)</td><td>70x35 x (1+ bx</td><td>5,00 = (20</td><td></td><td></td></fc<>	(kg/cm2)	70x35 x (1+ bx	5,00 = (20				
	P/(n*A)<	ft				· · · · · · · · · · · · · · · · · · ·		
BOLT		(t/cm2)						
BASE	$M=\alpha Wlx^{-}$		0.085×0.055×12.5		İ			
CAP	t > 6xM/6xM/6xM/6xM/6xM/6xM/6xM/6xM/6xM/6xM/		V6×0.73/1.85 =	1.2 - 2.2				
PLATE	$t > \sqrt{6xM}$					1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
	$\tau = \sigma \text{ cA}/$							
		(t/cm2)	<u>                                     </u>					
RIB	H/t							
PLATE	THE TENED	************		***************************************				
	WELDING	and the second s			}			
	$\frac{\tau}{\tau} = \sigma \frac{\text{cA}}{\text{cA}}$		<del>                                     </del>		<del></del>			
	-	(t/cm2)		• •		•		
WING	H/t							
PLATE								
	WELDING							
		(t/cm2)						
ALLOW-	CONC.	fc(t/cm2)						
ABLE	A.BOLT :	ft(t/cm2)						
Paness		fs(t/cm2)						
REMARI								
	ION: L,H,	B,t,D	LENGTH, HEI	GHT, WIDTH,	THICKNESS	, DIAMETER (		
NOTAT.	the state of the s		YOTMIY			•		
NOTAT.		- ECCENTR			the second second			
NOTAT	, ø c, –	COMPRE	CICITY ESSIVE STRESS F FORCE FOR C			UATION DUE T		

	DECISION	OF COLUM	n base (7)				
		<b>近面算定</b> ]		"·			
LOCAT	ION	7. Jul -42 AC 2	F-104 (105,204.	205)	F-203, 106		
COLUM	N SIZE		H-300 2 X10 X15		H-3002 X10 X15		
DIREC'		7	X	Y	X	Y	
PERMA		M (tm)			3.6	0-1	
CONDI	TIONS	N (t)	43.2 0.0			93. <i>S</i> _ 0,3	
TEMPO	RARY	M (tm)	) — — <del>— — — — —</del>		6,6	4.2	
CONDI	TIONS	N (t) Q (t)			27.1		
		1	Same as 8-108		Same as B-11		
		. "					
FIGUR	E						
		-			1		
		•	·				
BASE	PLATE	(LxBxt)	700 x 3	50×22	700,x350	x 27	
CAP P		(THICK.)	-	19¥10;11 1 000 000 000 000 000 000 000 000	-	4	
RIB P		(HxBxt)		ž .	/2		
WING	<i></i>	(HxBxt)	<u></u>	<del>,</del>		***************************************	
	R BOLT	$(n-D\phi)$ (HxBxt)	4	-250	<del>9</del> 2	5.7	
	e=M/N	(cm)			<del> </del>		
	σ c <fc< td=""><td>(kg/cm2)</td><td></td><td>••••</td><td></td><td></td></fc<>	(kg/cm2)		••••			
NCHOR	P/(n*A)<						
OLT	2 / (24/	(t/cm2)					
ASE	M=a wlx						
LATE	t>√ 6xM/						
AP LATE	$M=\alpha Wlx$ t> $\sqrt{6xM}$					•	
	$\tau = \sigma  cA$			<u> </u>			
IB	H/t	(t/cm2)	*********************	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
LATE	WELDING		] 		-		
	$\tau$ < fs						
	$\tau = \sigma  cA$	/(txH) (t/cm2)					
ING LATE	H/t		4.				
	WELDING						
	τ < fs	(t/cm2)	·				
LLOW- BLE	A.BOLT	fc(t/cm2) ft(t/cm2)					
TRESS		fb(t/cm2) fs(t/cm2)					
REMAR	KS						
NOTAT	e	ECCENTR	ICITY		THICKNESS, DI	1	
	σc -	COMPRE			R OTHER EQUATI	ON DUE TO e	
			IENT FOR BEN				

					laanc	137 04
		**	•			
	DEGTOTOR					
			n base (8)			
		面算定〕		·		
LOCAT	ION		G-101, 208		6-103/102, 105,	106
					(203,204,	206,201)
COLUM	N SIZE		BH~900x350 x 28	7X⊈U	BH-900x350 x 2	
DIREC	TION		**	Y	X	Y
PERMA	NENT	M (tm)	4.8		2.5	
CONDI		n (t)	5.49(	*****	386.4	
		Q (t)	9-6		-/5.0	
TEMPO	RARY	M (tm)	35.1	30.2	16.0	5.6
CONDI		N (t)	274.4	99,9		
10000		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	1		450.0	339.0
	<del></del>	( ) c/	70.1	60.3	32√0	11-1
			Same as A-	105	·	2
		:			+, /	4 4 6 150 4 4 6 150 1 4 6 150 1 4 6 150 1 4 6 150 1 4 6 150 1 4 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1
D.T. O.Y.	-				▎ <del>▗▗</del> ┤╀	4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
FIGUR	r				1	性 4 1 1 ( )
					= -	200 8
1					l +	45 3
			l		1300	<del></del>
BASE		(LxBxt)	1300×7	/∞ x25	1300 x 700 x	45
CAP P	LATE	(THICK.)		2	22	
RIB P	LATE	(HxBxt)		16	16	
WING	PLATE	(HxBxt)		2	≥≥	
	R BOLT	(n-Dφ )		-3 <b>e</b> ∳	8-30	. ds
SHEAR		(HxBxt)	Ω	· Y	0.739	Z.Y
	e=M/N	(cm)	<del></del>		D C. A	<del></del>
50.10.	C-11/11	Comp			$\frac{7.5 \times 10^2}{386.4} = 1.94$	< <del>20</del>
	σ c <fc< td=""><td>(kg/cm2)</td><td></td><td></td><td>306.4</td><td></td></fc<>	(kg/cm2)			306.4	
	0 0/10	(Rg/Cm2)	•		386.4 ×103 × (1+ 6	×1.94 00)=49.5 <70
ANCHOR	P/(n*A)<	f t-	N/LNA - OUD		130 X 70	ייי ייי סף
BOLT	E) (II.K) <		N/60f = 0.017 ) Pe	=0.0 %		<del>-</del>
BASE	M	(t/cm2)	M/bD4 = 0.0016)			
	$M=\alpha \text{ wlx}^{2}$				0.085×0.0495×352	
PLATE	t>√ 6xM/			<del> </del>	16x5.15/117 ·	<i>4,86 → 4.</i> 5
CAP	$M=\alpha wlx^{-}$		ł		and the second	•
PLATE	t>√ 6xM7					
	$\tau = \sigma \text{ cA}/$				$\frac{0.0495 \times 35 \times 35}{1.0 \times 67} =$	0.16 <0.9
	***************************************	(t/cm2)		******************	1.6 X 50	v. / v
RIB	H/t				50 - 21	₹ /⊡1
PLATE	. <u></u>				$\frac{50}{l,b} = 3l.$	<sup>3</sup> <71
1 1	WELDING					
	$\tau$ < fs					
1 []	$\tau = \sigma \text{ cA}/$	(txH)	1		0.0495x37.5×17.5	- 4 4 4
		(t/cm2)	<b>†</b>		0.0495x37.5 x17.5	0.54 (0.9
WING	H/t				1	
PLATE			1		- 50 1.2 = 41.7	(71
	WELDING	• • • • • • • • • • • • • • • • • • •				
1	τ < fs		4.		· ·	•
		fc(t/cm2)				
ABLE		ft(t/cm2)				
Janeso	PLATE :	fb(t/cm2) fs(t/cm2)	· · · · · · · · · · · · · · · · · · ·	• • • • • • • • • • • • • • • • • • • •		
REMAR		15(1/0112)				
		P + N -	TENCOU TEST	CUT DED	TUTCVNECO 1	DIAMETER (mm)
MOTAL		The state of the s		.Gni, Willin,	THICKNESS, 1	TAMETER (MM)
		- ECCENTR		, XT / / m & F l . ~ ~	Ontino moves	BTOW DIE MO
						TION DUE TO e
				COLUMN BASE (		• •
L	α	- COEFFIC	LENT FOR BEN	IDING MOMENT	OF SLAB	
			the second control of the control of			

	DECISION	OF COLUM	n base (9)			
1		而算定】		,		
LOCAT			G-104,205		G-108 (107,201,	202)
COLUM	N SIZE		BH-900x350 x28	XQO	BH-900 x400 x	2840
DIREC	TION		X	Y	X	Y
PERMA	NENT	M (tm)		0.0	15.2	14.1.4
CONDI	TIONS	N (t) Q (t)	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	378.9	481.7 30.3	
TEMPO	RARY	M (tm)		0.2	₹7.3	7.6
CONDI	TIONS	N (t) Q (t)		46 <i>4.7</i> 0.4	557.1 74.5	438.9 15.2
FIGUR	E		Same as G-1	<b>D3</b>		200 000 000 000 000 000 000 000 000 000
	•			·	200 900	***
BASE	PLATE	(LxBxt)	1300 x 7	00 X 4 5	1300×700×	50
CAP P		(THICK.)			.22	
RIB P		(HxBxt)			16	***************************************
WING		(HxBxt)	?2.		22	
1	R BOLT	$(n-D\phi)$	.6-30	<u>, †</u>	8-35	ф
SHEAR		(HxBxt)				
CONC.	e=M/N	(cm)		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	15,2 ×102 481,7 = 3.16	
·		(kg/cm2)	:		481.7 XID3 x (1+ 6x	5.16)=69.3 (70
ANCHOR BOLT	P/(n*A)<	ft (t/cm2)			_	_
BASE	$M=\alpha$ wlx	2 (tcm)			0.085 x0.067 x 35 2	
PLATE	t>√ 6xM7				16x7.0/1,70 =	4.95 → 5.0
CAP PLATE	M=α wlx^ t>√ 6xM/	fb (cm)	:			•
	$\tau = \sigma  cA/$	(txH) (t/cm2)		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.067x35x35 = 0	.75 (0.9
RIB PLATE	H/t				35 = 15.9	<16
	WELDING $\tau$ < fs	(t/cm2)	:			
	$\tau = \sigma \text{ cA}/$				0.067x15x37.5 1.2 x 50	
WING PLATE	H/t		*****************	*****************	$\frac{15}{1.2} = 12.5$	<16
	WELDING	/+/cm2)	******************	***********		
ALLOW-		fc(t/cm2)				
ABLE		ft(t/cm2)				
	PLATE :	fb(t/cm2)	••••••			
REMAR		fs(t/cm2)				
		B.t.D	LENGTH, HEI	GHT. WIDTH	THICKNESS, I	IAMETER (mm)
	е σс -	<ul><li>ECCENTR</li><li>COMPRE</li></ul>	ICITY SSIVE STRESS		OTHER EQUAT	TION DUE TO e
	α	- COEFFIC	IENT FOR BEN	IDING MOMENT	OF SLAB	

		dellar, marcare della regge planta Consume di Consume		error de la company de la comp		
	DECISION	OF COLUM	N BASE (10)			
	[柱脚の町	面算定〕				
LOCAT	ГОИ		H-101, 208		H-102, 207	
COLUM	N SIZE		H-3002 X 10 X 1	5	H-4002 x13 x2	21
DIREC	rion		<u>* Y</u>	Y	X	<b>₹</b> ×
PERMA	the state of the s	(.tm)	2.9		0.0	]
CONDI,	rions	N (t) Q (t)	81. Z 9. 7		209.0	
TEMPO:	The second secon	M (tm) N (t)	16:0 205.3	13.1	6.2 333.8	6.2 87.0
		Q (t)	53.4	43.7	20.6	20.6
FIGUR	€		260 300 200 700	89	[ˈ <del>ˈ</del>	8
BASE	PLATE	(LxBxt)	700 x 600	x 2Z	700 x	700 x 25
CAP P	LATE	(THICK.)	19	<u> </u>	~	
RIB P		(HxBxt)	16			9
WING	PLATE	(HxBxt)	16.			9
ANCHO	R BOLT KEY	(n-Dø ) (HxBxt)	8-30	4	8-	>\$Φ
	e=M/N	(cm)	1L n v10 2			
			$\frac{16.0 \times 10^2}{205.3} = 17.8$	$\cdot < \frac{70}{6}$		
	σ c <fc< td=""><td>(kg/cm2)</td><td></td><td>1.8)=81.6 &lt;140</td><td><math display="block">\frac{209.0\times10^3}{70\times70}=4</math></td><td>2.ባ &lt; 70</td></fc<>	(kg/cm2)		1.8)=81.6 <140	$\frac{209.0\times10^3}{70\times70}=4$	2.ባ < 70
ANCHOR	P/(n*A)<	ft	N/bof=0.0039, Pt=	0.64.		· · · · · · · · · · · · · · · · · · ·
BOLT		(t/cm2)	M/802+ = 0.0025) A+=	70x60 ft/4=6.3cm=		
BASE	$M=\alpha$ wlx		0.085 40.082 x 12.5	= 1.09	0.085 x 0.043 Y 2	
PLATE	t>√ 6xM7		16x1.09/2.77 =1.	5 -> 19	V6x1.46/1.85	= 2.18 -> 2.5
CAP	$M=\alpha \text{ wlx}^{-}$		0.085x (2x6.3x1.8412.			
PLATE	t>√6xM7		1 = PF.5/5,1×6			
	$\tau = \sigma  CA/$		0.082X20 x12.5 =	0.57 <1.35	0.043×20×35	0.53 (0.9
RIB	H/t	(t/cm2)	1.2 × 30		J.9.530	
PLATE	WELDING				<b>[</b>	
	$\tau$ < fs	1+/cm21	,			•
	$\frac{\tau}{\tau} = \frac{15}{\sigma} \frac{\text{CA}}{\text{CA}}$		10.153.45.20.5		135	<u></u>
1		(t/cm2)	13 A SV 700 CV 17 45 19 =	0.94 <1.35	0,043×12.5×25, 1,9 × 30	0,24 < 0.9
WING PLATE	H/t	.1.5%.Y##4		*************************	J	
FINITE	WELDING	• • • • • • • • • • • • • • • • • • • •				*******************
	τ < fs	(t/cm2)	} •••			
ALLOW-	CONC.	fc(t/cm2)			<del> </del>	<u> </u>
ABLE	A.BOLT	ft(t/cm2)		· · · · · · · · · · · · · · · · · · ·	}	******************
	PLATE :	fb(t/cm2)				
		fs(t/cm2)				
REMAR		8-1-2				
TATON				GHT, WIDTH,	THICKNESS,	DIAMETER (mm)
		- ECCENTR		? M//D#7\ 07	. Variable 15/114	ማፐብዝ ከመ ጣጣ
				COLUMN BASE (		TION DUE TO e
				IDING MOMENT	•	•
L	α	CORREIC	TENT BOY DEL	ANTING MOMERAT	OF SHAD	

 $\bigcirc$ 

WING PLATE (HxBxt) /2 ANCHOR BOLT (n-D¢) & 2259 & 6-259 SHEAR KEY (HxBxt)  ONC. e=M/N (cm)							
H-03 (105. 204. 206)				N BASE (1)			
DIRECTION	LOCAT		<u> </u>	H-103 (105, 204, 206)		H-203 (104, 106, 205)	
DIRECTION	COLUMN STZE			H-4502 V/A X 2.5		H-516x 500 x 77 X 40	
PERMANENT				X X X			
TEMPORARY M (tm) 6.2. 6.2 20.9				0.0		4.5	
CONDITIONS						A. Newstern	
FIGURE	conditions N (t)		408.5 155.6		499.1	247.4	
CAP FLATE (THICK) RIB PLATE (HXBXt) /9 22  ANCHOR BOLT (n-D\$) 8-25\$  ANCHOR BOLT (n-D\$) 8-25\$  ANCHOR BOLT (n-D\$) 8-25\$  SHEAR KEY (HXBXt)  CONC. e=M/N (cm)  G cfc (kg/cm²)  ANCHOR P/(n*A) <ft (t="" (tcm)="" anse="" cm²)="" late="" m="a" solt="" t="" wlx²="">/ GXM/TE (cm)  LATE t&gt;/ GXM/TE (cm)  LATE t&gt;/ GXM/TE (cm)  LATE t&gt;/ GXM/TE (cm)  LATE t&gt;/ GXM/TE (cm)  LATE t=G cA/(txH)  (t/cm²)  RIB  H/t  LATE  WELDING  T = G cA/(txH)  (t/cm²)  ANCHOR P/(n*A) = 0.52 (0.9  RIB  H/t  LATE  WELDING  T = G cA/(txH)  (t/cm²)  LLOW- CONC. :fc(t/cm²)  TRESS PLATE :fb(t/cm²)  RIBLE A. BOLT: ft(t/cm²)  RIBLE A. BOLT: ft(t/cm²)  RIBLE A. BOLT: ft(t/cm²)  RIBLE A. BOLT: ft(t/cm²)  PLATE :fs(t/cm²)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  G c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e  P UP-LIFT FORCE FOR COLUMN BASE (t)</ft>	FIGURE			1-11-11-11-11-11-11-11-11-11-11-11-11-1		500 219 500 ++ ++ ++ ++ ++ ++ ++ ++ ++ ++ ++ ++ ++	
CAP PLATE (HXBXt) (9 22 WING PLATE (HXBXt) (9 22 WING PLATE (HXBXt) (9 22 WING PLATE (HXBXt) (9 22 WING PLATE (HXBXt) (9 22 WING PLATE (HXBXt) (10 20 WING PLATE (HXBXt) (10 20 WING PLATE (HXBXt) (10 20 WING PLATE (HXBXt) (10 20 WING PLATE (HXBXt) (10 20 WING PLATE (HXBXt) (HXBXT) (HXB	BASE PLATE (LxBxt)			750 × 700 × 30		916×700×40	
WING PLATE (HxBxt) /9							
ANCHOR BOLT (n-D d) SHEAR KEY (HxBxt)  SONC. e=M/N (cm)			19		27		
SHEAR KEY (HxBxt)  CONC. e=M/N (cm)  Signal = 1,2			19				
CONC. $e=M/N$ (cm) $\frac{45 \times 10^2}{3(5,3)} = \frac{1}{1} \times \frac{49b}{4}$ $\sigma < fc (kg/cm2)$ $\frac{375,3 \times 10^3}{3(5,3)} = \frac{1}{1} \times \frac{49b}{4}$ $\sigma < fc (kg/cm2)$ $\frac{375,3 \times 10^3}{3(b,3)} \times (\frac{b-1,2}{3(b,b)} = \frac{1}{4}.1) < 70$ ANCHOR P/(n*A) ft (t/cm2)  CASE $M = \alpha \text{ wlx } ^2 \text{ (tcm)}$ $0.085 \times 0.063 \times 25.8^3 = 3.56$ $0.085 \times 0$		**************************************		8-259		8-25.9	
$\sigma < fc  (kg/cm2)$ $\frac{375,3 \times 10^3}{9(b,x)0} \times (1 + \frac{b \times 1,2}{9(b,x)0}) = \{3.\}  (70)$ $10CHOR P/(n*A) < ft \qquad (t/cm2)$ $20ASE                                    $							
NICHOR   P/(n*A) < ft	CONC.	-			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
SOLT (t/cm2)  ASE $M=\alpha$ w1x^2 (tcm) $CATE t>\sqrt{6xM/fb}$ (cm)  AP $M=\alpha$ w1x^2 (tcm) $CATE t>\sqrt{6xM/fb}$ (cm) $CATE t>6$						375,3 × 103 × (1+ 6x 916 × 70	1.2 b)=13.1 <70
PLATE t> $\sqrt{6xM/fb}$ (cm)  DAP $M = \alpha \text{ wlx}^2 \text{ (tcm)}$ PLATE t> $\sqrt{6xM/fb}$ (cm) $ \tau = \sigma \text{ CA/(txH)} $ $ (t/cm2) $ $ \tau = \sigma \text{ CA/(txH)} $ $ \tau = \sigma \text{ CA/(txH)} $ $ (t/cm2) $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < \text{fs (t/cm2)} $ $ \frac{\pi}{\epsilon} < fs (t$	ANCHOR BOLT	P/(n*A)<		· 			
CAP	BASE			:			
Description   Description	PLATE				<u> </u>	16x3.56 /1.85 = J	1.40 → 4.0
(t/cm2)    (t/cm2)	CAP PLATE	t>√ 6xM/	fb (cm)				
(t/cm2)		$\tau = \sigma  \text{cA}/$					50.52. (0.9
WELDING  \( \tau < \text{fs} \) (t/cm2)  \( \tau = \text{c} \text{cA}/(txH) \)  \( (t/cm2) \)  \( \text{VING} \)  \( \text{VING} \)  \( \text{VING} \)  \( \text{VELDING} \)  \( \tau < \text{fs} \) (t/cm2)  \( \text{LLOW-} \)  \( \text{CONC.} \) : fc(t/cm2)  \( \text{BLE} \)  \( \text{A.BOLT : ft(t/cm2)} \)  \( \text{TRESS} \)  \( \text{PLATE : fb(t/cm2)} \)  \( \text{PLATE : fs(t/cm2)} \)  \( \text{REMARKS} \)  \( \text{NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER \)  \( \text{mm} \)  \( \text{e ECCENTRICITY} \)  \( \text{g c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e \)  \( \text{P UP-LIFT FORCE FOR COLUMN BASE (t) } \)	RIB PLATE	H/t	(t/cm2)	***************************************	• • • • • • • • • • • • • • • • • • • •		
T = G CA/(txH) (t/cm2)  OING H/t  OLATE  WELDING T < fs (t/cm2)  OLLOW- CONC. :fc(t/cm2)  OBLE A.BOLT :ft(t/cm2)  OTRESS PLATE :fb(t/cm2)  PLATE :fs(t/cm2)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY G C COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e  P UP-LIFT FORCE FOR COLUMN BASE (t)		WELDING					
WELDING  \( \tau < \tau \) (t/cm2)  \( \tau < \tau \) (t/cm2)  \( \tau < \tau \) (t/cm2)  \( \tau < \tau \) (t/cm2)  \( \tau \		τ < fs	(t/cm2)	·	· ·		
WELDING  \( \tau < \tau \) (t/cm2)  \( \tau < \tau \) (t/cm2)  \( \tau < \tau \) (t/cm2)  \( \tau < \tau \) (t/cm2)  \( \tau \						0.043 x12,5 x32.9	5
WELDING	WING PLATE	H/t	(t/cm2)				~0.23
LLOW- CONC. :fc(t/cm2)  BLE A.BOLT :ft(t/cm2)  TRESS PLATE :fb(t/cm2)  PLATE :fs(t/cm2)  REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e  P UP-LIFT FORCE FOR COLUMN BASE (t)		_	/#/~~^^	·····	,,,,,,		
REMARKS NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm) e ECCENTRICITY o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e P UP-LIFT FORCE FOR COLUMN BASE (t)	Δ T.T.ΩW.—				<u> </u>		
REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e  P UP-LIFT FORCE FOR COLUMN BASE (t)							
REMARKS  NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm)  e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e  P UP-LIFT FORCE FOR COLUMN BASE (t)		PLATE:	fb(t/cm2)	• • • • • • • • • • • • • • • • • • • •			
NOTATION: L,H,B,t,D LENGTH, HEIGHT, WIDTH, THICKNESS, DIAMETER (mm) e ECCENTRICITY o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e P UP-LIFT FORCE FOR COLUMN BASE (t)	BEMAR		rs(t/cm2)		<u></u>		
e ECCENTRICITY  o c COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e  P UP-LIFT FORCE FOR COLUMN BASE (t)			B. t D	LENGTH HET	GHT. WIDTH	THICKNESS T	IAMETER /mml
O C COMPRESSIVE STRESS, N/(B*L) OR OTHER EQUATION DUE TO e P UP-LIFT FORCE FOR COLUMN BASE (t)	HULEL.		1 1 1		CALL F STADER		manage and (mm)
P UP-LIFT FORCE FOR COLUMN BASE (t)		α C : −	COMPRE	SSIVE STRESS	, N/(B*L) O	R OTHER EOUAT	ION DUE TO e
// COEFFICIENT FOR BENDING MOMENT OF SLAB		P	- UP-LIFT	FORCE FOR C	OLUMN BASE	(t)	
		α	- COEFFIC	IENT FOR BEN	DING MOMENT	OF SLAB	