

パキスタン回教共

ウェストワーフ火力発電建設調査

最終報告書 I (3/3)

1990年1月

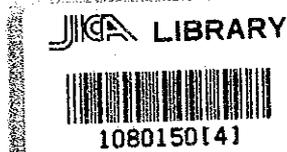
国際協力事業団

総 務 課
CR(C)
90-4 (3/3)

パキスタン回教共和国

ウェストワーフ火力発電所建設計画調査

最終報告書 I (3/3)



20632

1990年1月

国際協力事業団

国際協力事業団

20632

ウエスト・ワーフ火力発電所建設計画調査

最終報告書 I

第1巻 発電設備

第1節 序論

- I-1 まえがき
- I-2 プロジェクトの概要
- I-3 現地状況と設計条件

第2節 プラント全般

- GP-1 プラント性能
- GP-2 タービン・ヒート・バランス
- GP-3 最適蒸気条件
- GP-4 最適真空度
- GP-5 給水加熱器の段数

第3節 機械

- M-1 ボイラ
- M-2 ボイラ補機
- M-3 タービン
- M-4 タービン補機
- M-5 復水器及び循環水設備
- M-6 復水器細管の材質
- M-7 給水加熱装置
- M-8 ボイラ給水ポンプ
- M-9 軸受冷却設備
- M-10 主蒸気及び再熱蒸気配管ルート
- M-11 主要配管仕様
- M-12 屋外配管

第4節 共通設備

- C-1 燃料設備
- C-2 燃料加熱設備
- C-3 補助燃料（天然ガス）設備
- C-4 用水設備
- C-5 排水処理設備
- C-6 薬液注入設備
- C-7 塩素注入設備
- C-8 復水器保護設備
- C-9 循環水ポンプ、スクリーン保守設備
- C-10 防火設備

第5節 電気設備

E-1	所内変圧器容量
E-2	起動変圧器容量
E-3	所内変圧器, パーセント インピーダンス
E-4	起動変圧器, パーセント インピーダンス
E-5	バッテリー容量
E-6	非常用発電機容量
E-7	発電機変圧器容量
E-8	発電機サージアブソーバ容量
E-9	相分離母線
E-10	所内電気回路方式
E-11	接地抵抗値
E-12	220kV/132 開閉所配置
E-13	照明設備

第6節 計装・制御

PIC-1	プラント・インター・ロック
PIC-2	プラント制御システムの設計概要
PIC-3	タービン・バイパス・システム
PIC-4	コンピュータの適用
PIC-5	ボイラ制御
PIC-6	バーナー制御
PIC-7	タービン・ガバナーシステム

第7節 環境対策

EP-1	SO _x , NO _x 濃度の検討
EP-2	排水処理
EP-3	騒音対策

第8節 プロジェクト関連事項

PR-1	建設工程
PR-2	建設費

第2巻 送電線及び変電所設備

TLG-1	架空送電線
TLG-2	地中送電線
TLG-3	地中線洞道
TLG-4	開閉所/変電所設備

第3巻 建築・土木

第1節 建築設計

AR-1	発電所本館（上部構造）	（構造計算書）
AR-2	発電所本館（下部構造）	（ 〃 ）
AR-3	煙 突	（ 〃 ）
AR-4	事務所建屋	（ 〃 ）
AR-5	付属建屋	（ 〃 ）
AR-6	屋外機器の基礎	（ 〃 ）
AR-7	空調設備（本館）	
AR-8	空調設備（事務所建屋）	

第2節 土木設計

CV-1	土木設計条件	
CV-2	冷却水路の計算要項	
CV-3	水路の水理計算	
CV-4	取水路開渠の構造設計	
CV-5	取水スクリーン，ポンプ室の構造設計	
CV-6	放水路の構造設計	
CV-7	放水口の構造設計	
CV-8	護岸の構造設計	

添 付： 調査業務従事者及び分担業務
（第1巻 巻末に添付）

AR-1 發電所本館(上部構造)

構造計算書

目 次

I. 一般事項	1
1. 発電所本館	2
2. 構造	2
3. 構造設計法	3
4. 適用規格基準	3
5. 使用材料と許容応力	4
6. 建屋全体図	8
7. 設計荷重	17
II. 本館設計	29
1. 本館の荷重計算	29
1.1 柱軸力	29
1.2 地震荷重	64
2. クレーン・ガーダの設計	72
3. 構造解析	75
3.1 垂直荷重に対する垂直ブレースの計算	75
3.2 入力データ	83
3.3 応力	107
3.4 垂直ブレース, 柱, ガーダーの応力	131
4. 柱の設計	135
4.1 柱の設計	135
4.2 柱脚の設計	144
5. 梁の設計	161
5.1 小梁の設計	161
5.2 大梁の設計	176
5.3 ルーフ・トラスの設計	203

I. 一 般 事 項

1. Building Dimensions (発電所本館)

1) Building Area	4,181.6 m ²
2) Floor Area	
5th Floor	55.6 m ²
4th Floor	1,726.4 m ²
Operating Floor	4,181.6 m ²
Mezzanine Floor	4,181.6 m ²
Ground Floor	4,181.6 m ²
<hr/>	
Total	14,326.8 m ²

3) Building Height (T/S) GL+27.1 m

4) Building KVolume 102,057.6 m³

2. Structure (構造)

1) Main Structure : Steel

2) Foundation : Reinforced concrete mat foundation

3) Slab : Concrete slab and grating floor

4) Wall : Concrete Wall and precast concrete wall

3. Structural Design Method (構造設計法)

3.1 Superstructure

- 1) All frames in longitudinal direction are designed with vertical braced frames.
- 2) All frames in transverse direction are designed with vertical braced and open frames.
- 3) The bracing system will utilize x-braces or portal braces and the brace members are designed for compression and tension.
- 4) Every end joint of every girder is assumed to be a pin joint except that of a roof truss.
- 5) Every bottom of every column is assumed to be a pin support.
- 6) Every member including a roof truss is replaced to a line element.
- 7) Every structural analysis is based on elastic stiffness.

4. Design Criteria and Code (適用規格基準)

- 1) "Design Standard for Steel Structure", Architectural Institute of Japan(AIJ), 1970
- 2) "Standards for Calculation of Reinforced Concrete Structure", AIJ, 1982
- 3) "Standards for Structural Design of Building Foundation", AIJ, 1974
- 4) "Manual for Loads of Buildings", AIJ, 1981

5. Structural Materials and Allowable Stresses (使用材料と許容応力)

1) Qualities of materials

Structural steel : JIS G3101 SS41, JIS G3106 SM41 or equivalent — $F=2400\text{kg/cm}^2$

High Strength Bolt : JIS B1186 F10T or equivalent

Concrete : $F=210\text{kg/cm}^2$ (compressive strength of 28 days)

Reinforcement : Deformed bar, ASTM A615 Grade 40 or equivalent — $F=2800\text{kg/cm}^2$

2) Physical constants of structural steel

Modulus of elasticity $E=2.1 \times 10^5 \text{ kg/cm}^2$

3) Allowable stresses

Allowable stresses for each materials are shown in Table 5.1 to 5.7.

Table 5.1 Allowable Unit Stresses of Steel

	Allowable Unit Stress (t/cm ²)
Allowable Tensile Stress f_t	$f_t = \frac{F}{1.5} = 1.6$
Allowable Shear Stress f_s	$f_s = \frac{F}{1.5 \sqrt{3}} = 0.92$
Allowable Compressive Stress f_c	when $\lambda \leq \Lambda$ $f_c = \frac{(1-0.4(\lambda/\Lambda)^2)F}{\nu}$ when $\lambda > \Lambda$ $f_c = \frac{0.277F}{(\lambda/\Lambda)^2}$ where $\nu = 3/2 + 2/3 \cdot (\lambda/\Lambda)^2$ $\Lambda = \sqrt{\frac{\pi^2 E}{0.6F}}$: Critical slenderness ratio
Allowable Bending Stress f_b	$f_b = \text{Max}(f_{b1}, f_{b2})$, but not more than f_t $f_{b1} = (1-0.4 \frac{(l_b/i)^2}{C \cdot \Lambda^2}) f_t$ $f_{b2} = \frac{900}{(l_b \cdot h/\Lambda_r)}$ where $C = 1.75 - 1.05 \left(\frac{M_2}{M_1}\right)^2 + 0.3 \left(\frac{M_2}{M_1}\right)^2$, but not more than 2.3
Allowable Bending Stress of Bearing Plate f_{b3}	$f_{b3} = \frac{F}{1.3} = 1.85$

- Notes: 1. Each allowable stress indicated in this table is permanent.
 2. Temporary allowable stresses are 1.5 times as much as those in this table.
 3. Allowable bending stress f_b is that for a shape steel, a plate girder and another built-up member which are bent around the principal axis of maximum moment of inertia. This is not, however, applied to box section.
 4. Symbols in this table are defined as follows:
 λ : Slenderness ratio for compression member
 l_b : length of compressive side flange between supports
 h : Depth of beam(cm)
 A_f : Cross sectional area of compressive side flange(cm²)
 i : Radius of gyration of area of Tee section, comprising compressive side flange and one-sixth of depth of beam, around the axis of web(cm)

Table 5.2 Allowable Unit Stresses in Welded Joints (t/cm²)

Welding Position	Permanent Stresses					Temporary Stresses
	Groove Weld				Fillet Weld	
	Tension	Compress	Bending	shear		
(1)	1.44	1.44	1.44	0.83	0.83	Permanent stresses x 1.5
(2)	1.20	1.20	1.20	0.70	0.70	

Notes: (1) Flat or horizontal in the shop
 (2) Overhead or vertical in the shop and flat or horizontal in the field

Table 5.3 Allowable Strength per Medium Bolts (SS41)

Bolt Nominal Dia.	Dia. of Bolt Hole (mm)	Bolt Gross Area (cm ²)	Permanent Strength			Temporary Strength (t)
			Shear (t)		Tension (t)	
			Single shear	Double shear		
M12	12.5	1.13	1.02	2.03	1.36	Permanent Stresses x 1.5
M16	16.5	2.01	1.81	3.62	2.41	
M20	20.5	3.14	2.83	5.65	3.77	
M22	22.5	3.80	3.42	6.84	4.56	
M24	24.5	4.52	4.07	8.14	5.42	

Table 5.4 Allowable Strength per High Strength Bolts (F10T)

Bolt Nominal Dia.	Dia. of Bolt Hole (mm)	Bolt Effect. Area (cm ²)	Permanent Strength			Temporary Strength (t)
			Shear (t)		Tension (t)	
			Single shear	Double shear		
M16	17.0	1.57	3.02	6.03	6.23	Permanent Stresses x 1.5
M20	21.5	2.45	4.71	9.42	9.73	
M22	23.5	3.03	5.70	11.4	11.8	
M24	25.5	3.53	6.78	13.6	14.0	

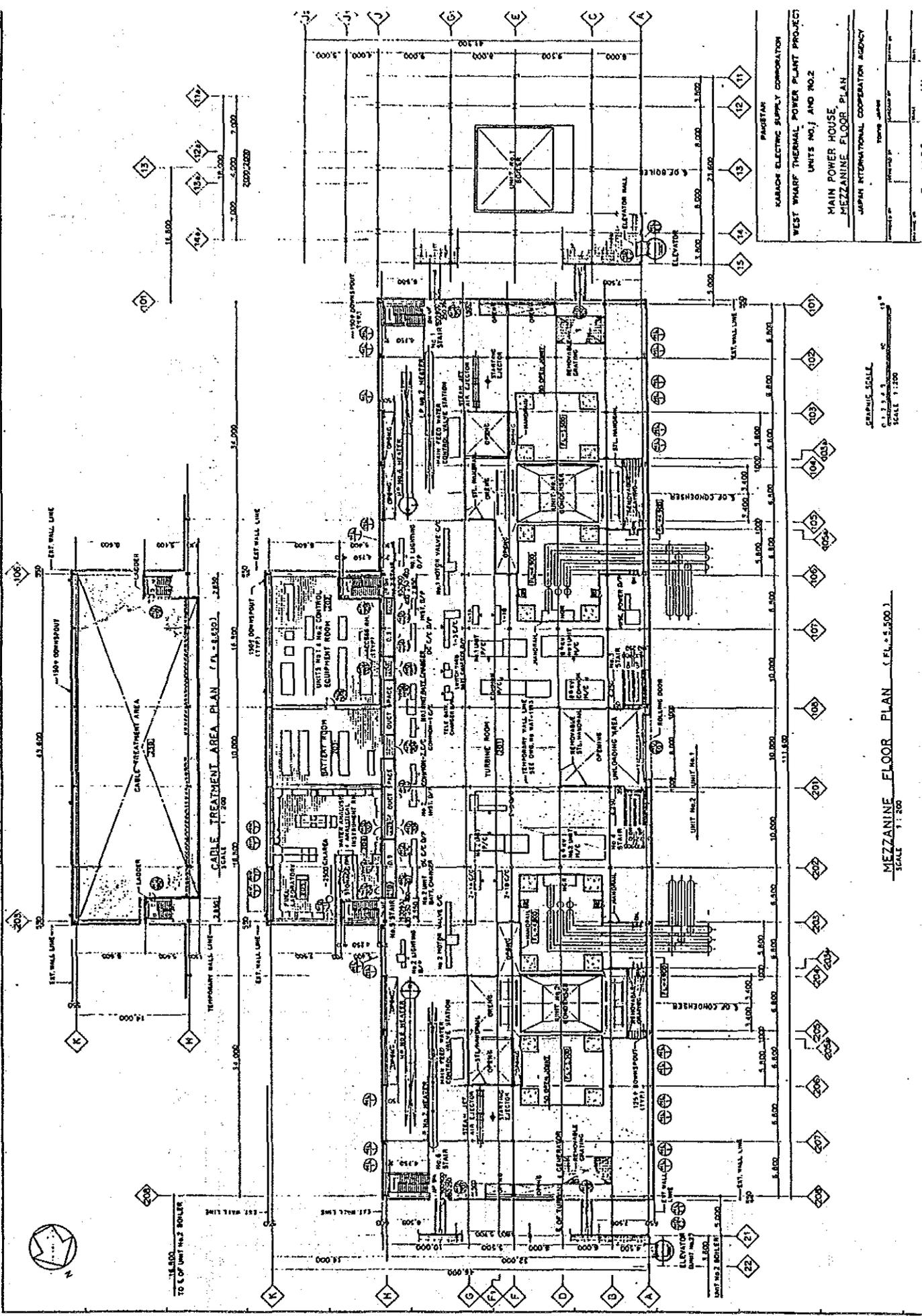
Table 5.5 Allowable Unit Stresses of Concrete (kg/cm^2)

Stresses Materials		Permanent Stresses					Temporary Stress	
		Compress	Shear	Bond			Compress	Shear Bond
				A	B	C		
Normal Concrete $F_c=210$	Plain bar Deformed bar	70	7.0	8.4	12.6	8.4	P.Stress	P.S.
				14.0	21.0	14.0		
Normal Concrete $F_c=250$	Plain bar Deformed bar	80	7.5	9.0	13.5	9.0	x 2.0	x 1.5
				15.6	23.5	15.6		

Remarks A : Top bar of flexural members
 B : Bar, except "Item A", of flexural members
 C : Anchors and lap splices.

Table 5.6 Allowable Unit Stresses of Reinforcing Bars (kg/cm^2)

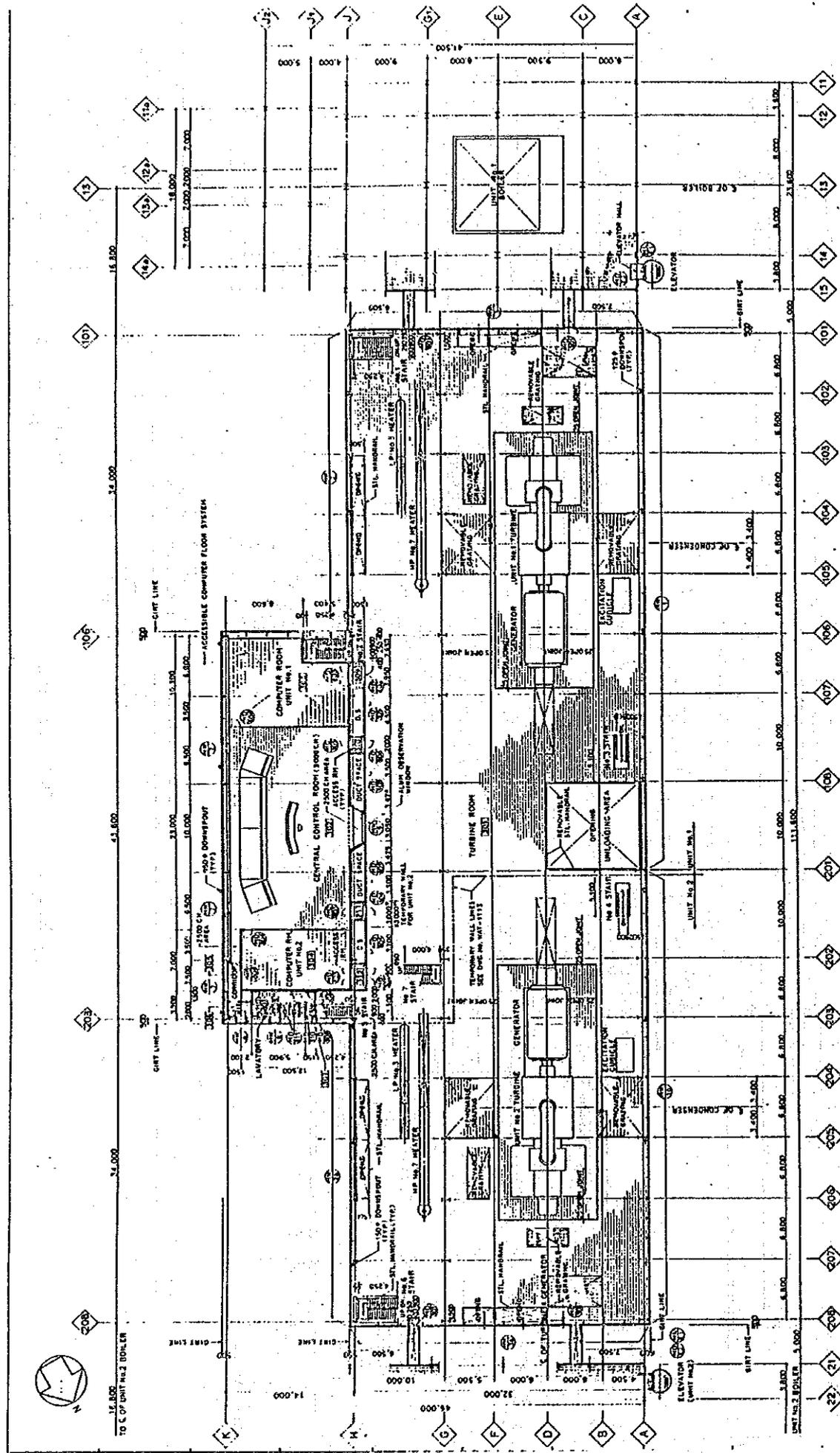
Stresses Material	Permanent Stresses		Temporary Stresses	
	Tension Compression	Shear Rein- forcement	Tension Compression	Shear Rein- forcement
Deformed bar ASTM A615 Grade40	1850	1850	2800	2800



PHOENIX
 KARACH ELECTRIC SUPPLY CORPORATION
 WEST WHARF THERMAL POWER PLANT PROJECT
 UNITS NO. 1 AND NO. 2
 MAIN POWER HOUSE
 MEZZANINE FLOOR PLAN
 JAPAN INTERNATIONAL COOPERATION AGENCY

GRAPHIC SCALE
 0 1 2 3 4 5 6 7 8 9
 SCALE 1:200

MEZZANINE FLOOR PLAN (F.L. 5.500)
 SCALE 1:200



KURASHI ELECTRIC SUPPLY CORPORATION
 WEST WHARF THERMAL POWER PLANT PROJECT
 UNITS NO. 1 AND NO. 2
 MAIN POWER HOUSE
 OPERATING FLOOR PLAN
 JAPAN INTERNATIONAL COOPERATION AGENCY

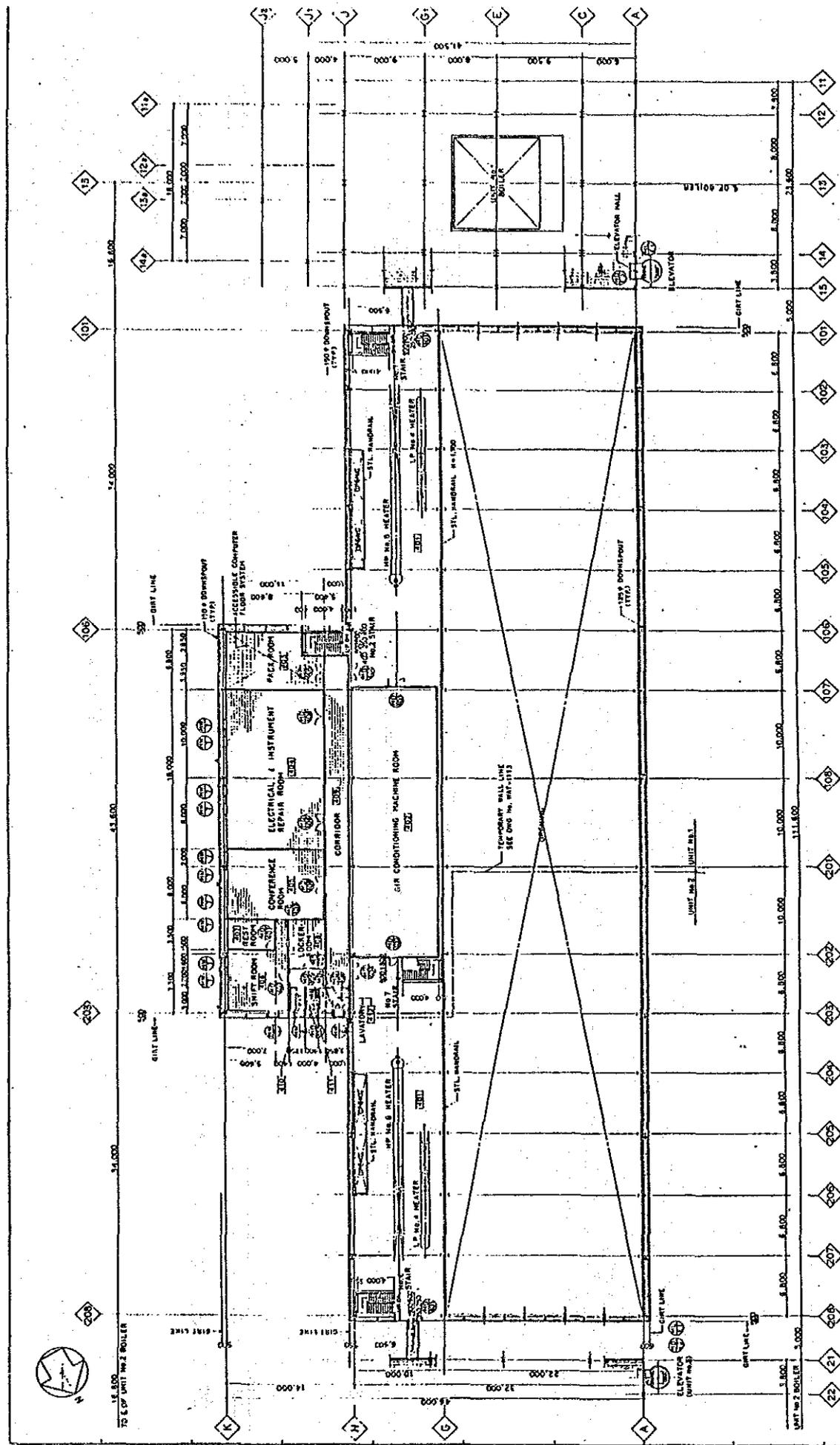
GRAPHIC SCALE
 0 10 20 30 40 50
 SCALE 1:100

OPERATING FLOOR PLAN (FL. #11,000)
 SCALE 1:200

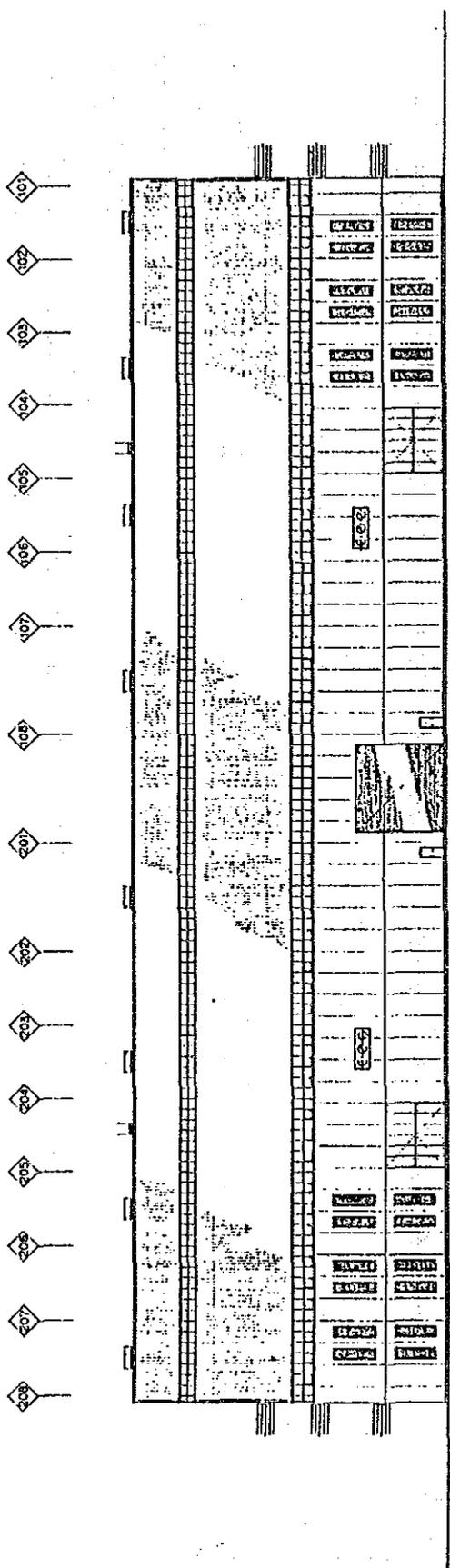
PAKISTAN
 KARACHI ELECTRIC SUPPLY CORPORATION
 WEST WHARF THERMAL POWER PLANT PROJECT
 UNITS NO. 1 AND NO. 2
 MAIN POWER HOUSE
 FORTH FLOOR PLAN
 JAPAN INTERNATIONAL COOPERATION AGENCY

GRAPHIC SCALE
 0 1 2 3 4 5 6 7 8 9 10
 SCALE 1 : 200

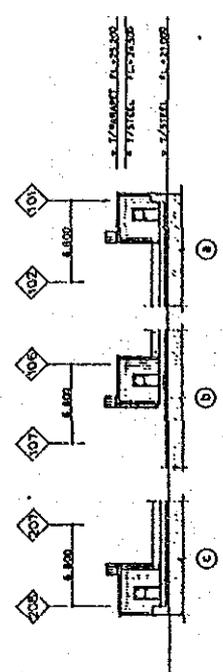
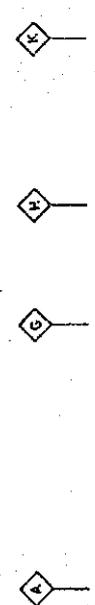
FORTH FLOOR PLAN (FL. 21.000)
 SCALE 1 : 200



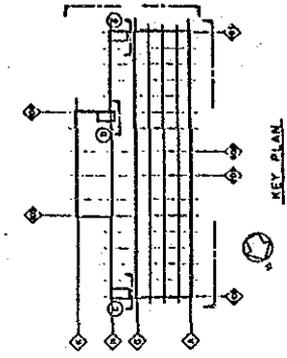
PAKISTAN
 KARACHI ELECTRIC SUPPLY CORPORATION
 WEST WHARF THERMAL POWER PLANT PROJECT
 UNITS NO.1 AND NO.2
 MAIN POWER HOUSE
 WEST & SOUTH ELEVATIONS
 JAPAN INTERNATIONAL COOPERATION AGENCY



WEST ELEVATION
 SCALE 1:200

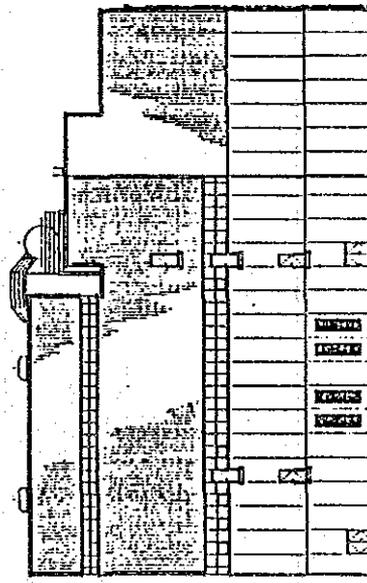


WEST ELEVATIONS (STAIR CASE)
 SCALE 1:200

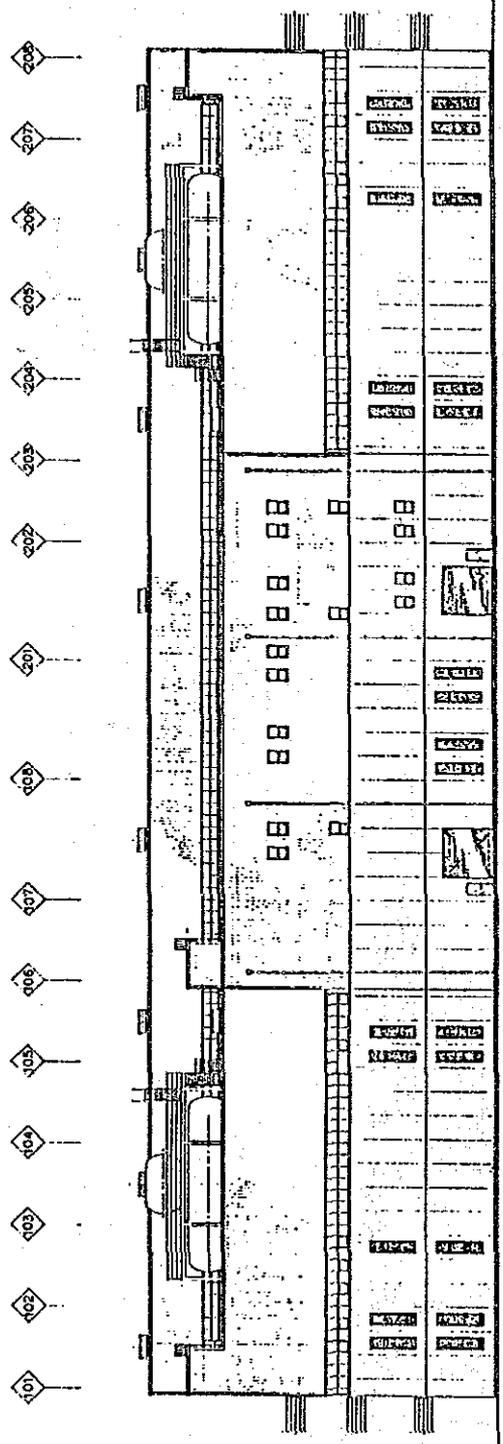


KEY PLAN

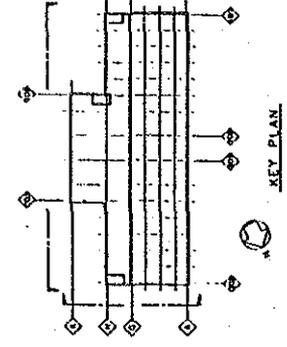
GRAPHIC SCALE
 0 5 10 15 M
 SCALE 1:200



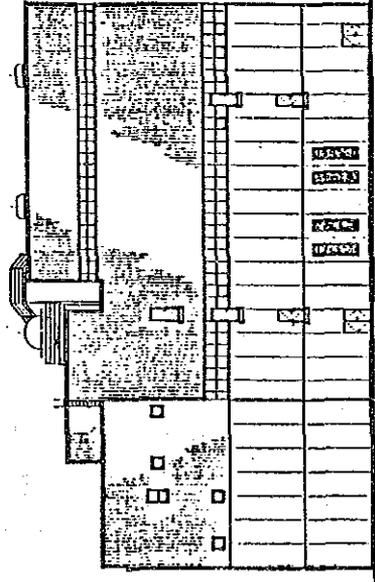
SOUTH ELEVATION
 SCALE 1:200



EAST ELEVATION
SCALE 1:200



KEY PLAN



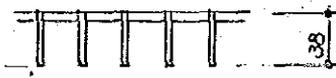
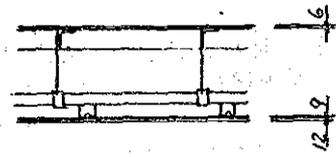
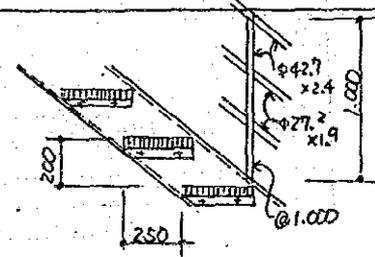
NORTH ELEVATION
SCALE 1:200



PAKISTAN
 KORACH ELECTRIC SUPPLY CORPORATION
 WEST WHARF THERMAL POWER PLANT PROJECT
 UNITS NO.1 AND NO.2
 MAIN POWER HOUSE
 EAST & NORTH ELEVATIONS
 JAPAN INTERNATIONAL COOPERATION AGENCY
 TOKYO JAPAN

7. Design Load (設計荷重)				
7.1 DEAD LOAD (1)				
【固定荷重】				
ROOM NAME OR LOCATION	FIGURE (mm)	MATERIALS (THICKNESS-mm)	WEIGHT (kg/m ²)	TOTAL (kg/m ²)
ROOF TURBINE ROOM A/C MACHINE ROOM		CONCRETE BLOCK (THK=30) SAND (t=10) ASPHALT ROOFING (t=10) CONCRETE SLAB (t=100) DECK PLATE V-50 (t=1.2)	60 20 15 275 13.9	383.5 → 385
ROOF of ELECTORICAL & INST. REPAIR RM CONFERENCE ROOM		CONCRETE BLOCK SAND ASPHALT ROOFING CONCRETE SLAB DECK PLATE GLASS WOOL CEILING	383.5 2 25	410.5 → 415
4TH. FL. A/C MACHINE ROOM		CEMENT MORTAL (t=30) CINDER CONCRETE (t=60) ASPHALT WATER PROOF (t=10) CONCRETE SLAB (t=100) DECK PLATE (t=1.2)	60 120 15 275 13.9	483.9 → 485
4TH. FL. HP HEATER ROOM		CONCRETE SLAB (THK=100) DECK PLATE (t=1.2)	275 13.9	288.9 → 290
LABORATORY ROOM		VINYL ASBESTOS TILE (THK=2.0) CEMENT MORTAL (t=28) CONCRETE SLAB (t=100) DECK PLATE (t=1.2)	4 56 275 13.9	348.9 → 350
4TH. FL. ELECTORICAL & INST. REPAIR RM CORRIDOR		VINYL ASBESTOS TILE (THK=2.0) CEMENT MORTAL (t=28) CONCRETE SLAB (t=100) DECK PLATE (t=1.2) GLASS WOOL CEILING	398.9 2 25	375.9 → 380

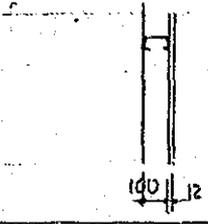
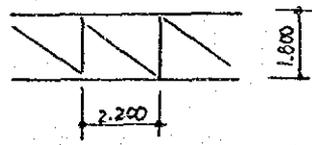
DEAD LOAD (3)
[固定荷重]

ROOM NAME OR LOCATION	FIGURE (mm)	MATERIALS (THICKNESS-mm)	WEIGHT (kg/m ²)	TOTAL (kg/m ²)
MEZ. FL. TURBINE RM	SAME AS OPE. FL. TURBINE FL.			425
MEZ. FL. CONTROL EQUIP. ROOM BATTERY ROOM LABORATORY ROOM	Ditto.			425
GRATING FLOOR		GRATING	44.4	→ 45
CHECKERED PLATE FLOOR		CHECKERED PLATE (THK=6) L-65x65x6 CEILING	48.8 20 25 93.8	→ 95
STAIR		L-250x90x9x13 x2 GRATING HAND RAIL 42.7 (1.0+1.9) x 2 27.2 1.4 x 2 x 2	97.8 33 18.7 18.6 163.1	→ 165 kg/m ²

DEAD LOAD (4)
[固定荷重]

ROOM NAME OR LOCATION	FIGURE (mm)	MATERIALS (THICKNESS-mm)	WEIGHT (kg/m ²)	TOTAL (kg/m ²)
PARAPET		WEATHER PROOF METAL (THK=10) L-30x30x2 @ 500 GIRT L-50x50x4 @ 500 VERTICAL GIRT L-100x50 @ 200 BEND. PLATE A=6.7 (THK=6.0) ASPHALT ROOFING (1" = 5.0) WEATHER PROOF METAL (1" = 10) CONCRETE BLOCK 150x120	5 3 6.1 4.7 34.2 7.5 7 43	110.5 → 120 kg/m
METAL SIDING		METAL SIDING (THK=1.0) FRAME (L-100x100x6, H=250)	18 25 43	→ 45
CONCRETE BLOCK WALL		CONCRETE BLOCK (THK=150)	300	→ 300
MORTAL FIN		MORTAL (THK=25)	50	→ 50
TILE FIN		MOSAIC TILE (THK=4) MORTAL (THK=25)	8 50 58	→ 60
PRECAST CONCRETE PANEL		PRECAST CONC. PANEL (THK=120)	288	→ 290

DEAD LOAD (5)
[固定荷重]

ROOM NAME OR LOCATION	FIGURE (mm)	MATERIALS (THICKNESS-mm)	WEIGHT (kg/m ²)	TOTAL (kg/m ²)
WALL (SINGLE SIDE)		FRAME... C-100 @ 450 PLASTER... BOAD. (THK=12)	15 10.2 25.2	→ 26
WALL (DOUBLE SIDE)		FRAME... C-100 @ 450 PLASTER... BOAD. (THK=12)	15 20.4 35.4	→ 36
GLASS WOOL		GLASS WOOL... (THK=50)	2	→ 2
TRUSS		H-350... 49.4 x 49.4 H-250... 32.4 x 49.4 POST. 2L-100 x 15... 57.6 x 1.8 x 2 BRACE... 57.6 x 2.25 x 2	243.6	→ 250 kg/m
FENCE				→ 110 kg/m

NOTE: DL --- DEAD LOAD PHL --- PIPE HANGER LOAD
 LL --- LIVE LOAD TL --- TOTAL FLOOR LOAD

Unit: kg/m²

FLOOR	ROOM NAME	SLAB		GIRDER			BEAM			COLUMN & FOUND.			SEISMIC			
		DL	LL	TL	DL	LL	PHL	TL	DL	LL	PHL	TL	DL	LL	PHL	TL
ROOF	T/G ROOM	385	90	475	535	60	595	535	60	595	535	60	595	535	60	595
5TH FL	COOLING TOWER ROOF	415	200	615	565	200	765	565	200	765	565	200	765	565	200	765
8-ROOF	A/C MACHINE ROOM ROOF	385	100	485	535	100	635	535	100	635	535	100	635	535	100	635
6L+21g	DEAERATOR ROOF	385	500	885	535	500	1,285	535	500	1,285	535	500	1,285	535	500	1,285
4TH FL	CONFERENCE ROOM	480	300	680	530	180	710	530	180	710	530	180	710	530	180	710
6L+16g	ELECTORICALS REPAIR ROOM	320	500	880	530	500	1,030	530	500	1,030	530	500	1,030	530	500	1,030
	H.P. HEATER ROOM	290	1000	1,290	440	1000	1,690	440	1000	1,690	440	1000	1,690	440	1000	1,690
	LAVATORY	510	130	640	660	130	790	660	130	790	660	130	790	660	130	790
	A/C MACHINE ROOM	485	500	985	635	500	1,135	635	500	1,135	635	500	1,135	635	500	1,135
07E FL	T/G ROOM (OVERHAUL)	425	2000	2,425	575	2000	2,825	575	2000	2,825	575	2000	2,825	575	2000	2,825
6L+11g	" (OTHER)	425	1000	1,425	575	1000	1,825	575	1000	1,825	575	1000	1,825	575	1000	1,825
	CENTRAL CONTROL ROOM-1	425	300	725	575	180	1,005	575	180	1,005	575	180	1,005	575	180	1,005
	COMPUTER ROOM	425	500	925	575	500	1,075	575	500	1,075	575	500	1,075	575	500	1,075
	LAVATORY	960	130	1,090	1,110	130	1,240	1,110	130	1,240	1,110	130	1,240	1,110	130	1,240
MEZ FL	T/G ROOM	425	800	1,225	575	800	1,625	575	800	1,625	575	800	1,625	575	800	1,625
6L+55	CONTROL EQUIPMENT ROOM	425	500	925	575	500	1,225	575	500	1,225	575	500	1,225	575	500	1,225
	BATTERY ROOM	425	500	925	575	500	1,225	575	500	1,225	575	500	1,225	575	500	1,225
	LABORATORY ROOM	425	300	725	575	180	905	575	180	905	575	180	905	575	180	905
COMMON	GRATING FLOOR	—	—	—	195	* 130	* 325	195	* 130	* 325	195	* 130	* 325	195	* 130	* 325
	STAIR	—	—	—	300	* 130	* 430	300	* 130	* 430	300	* 130	* 430	300	* 130	* 430
	CHECKERED PLATE FLOOR	95	100	195	95	100	195	95	100	195	95	100	195	95	100	195

7.3 Crane Load

1. Design Condition

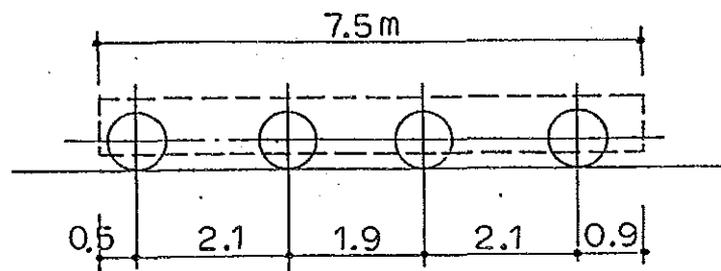
1.1 Overhead crane

Dead load : 45.0 ton (including 16.5 ton trolley)

Main hook : 60.0 ton

Auxiliary hook : 15.0 ton

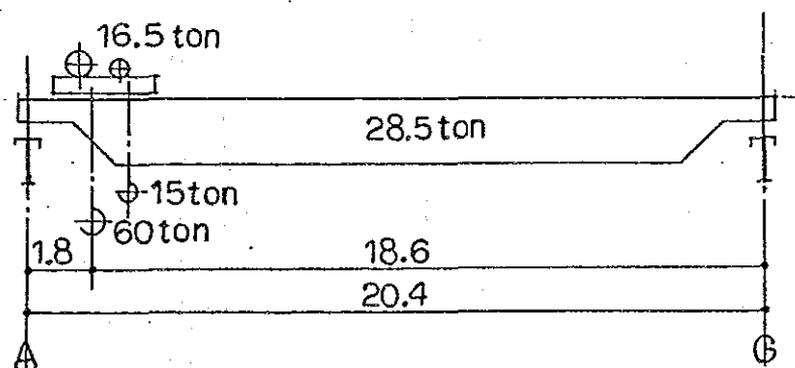
The wheel span is assumed to be as illustrated in the following diagram according to the experience.



1.2 The beam for the crane girder will be a simple beam each for 6.8 m span and 10.0 m span.

1.3 Maximum wheel load (P)

Crane span, $l = 20.4$ m



When

$$R_A = (60.0 + 16.5) \times \frac{18.6}{20.4} + \frac{1}{2} \times 28.5 = 84.0 \text{ ton}$$

$$R_B = (60.0 + 16.5) \times \frac{1.8}{20.4} + \frac{1}{2} \times 28.5 = 21.0 \text{ ton}$$

The maximum wheel load (A) per wheel becomes $84.0/4 = 21.0$ ton.

Since the impact factor is 120%, the design wheel load is

$$21 \times 1.2 = 25.2 \text{ ton / wheel.}$$

7.2 WIND LOAD

$$F_w = A \cdot C \cdot f$$

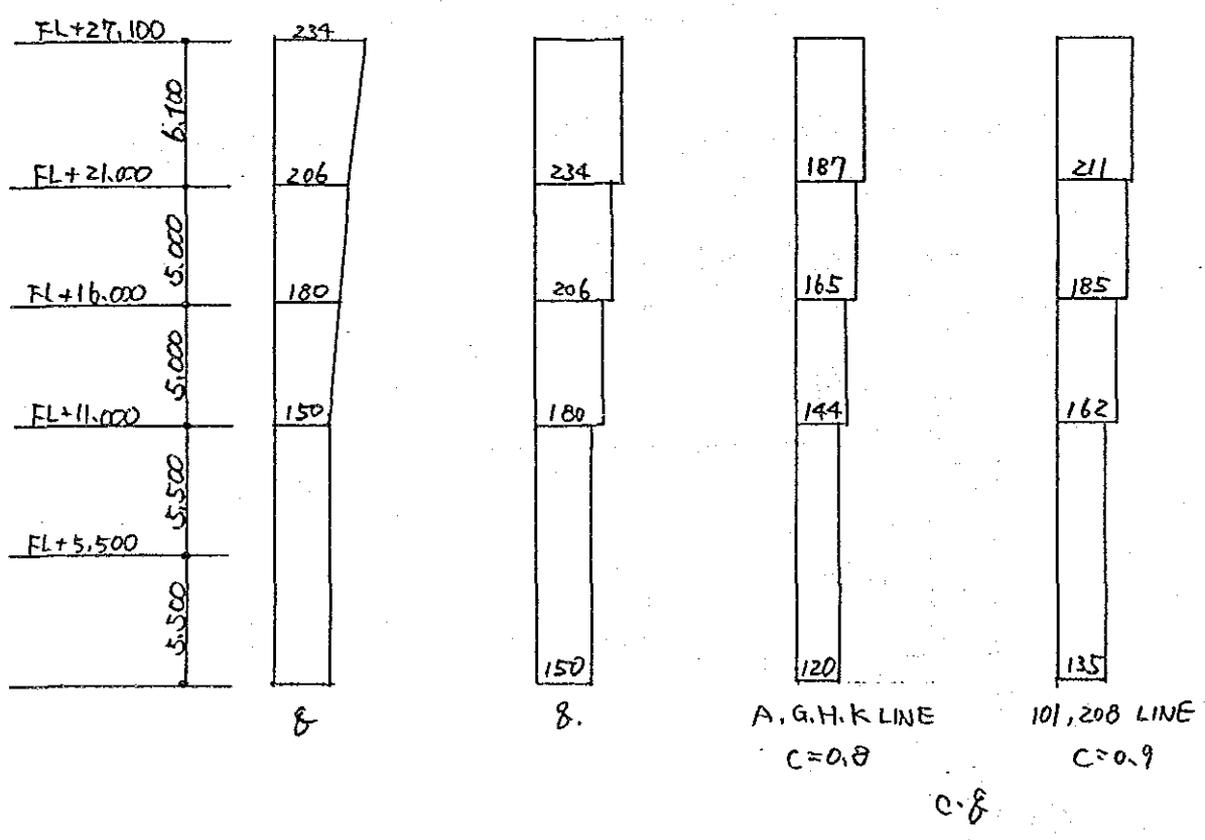
A: Projected area of structure (m²)

C: Shape factor

H: Height (m)

$$f = 150 \text{ kg/m}^2 \quad H \leq 10\text{m}$$

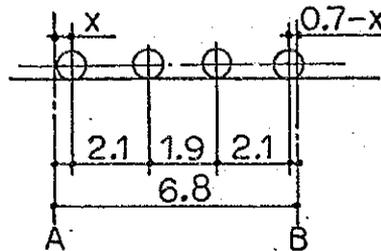
$$f = 45\sqrt{H} \quad H > 10\text{m}$$



2. Stress analysis

2.1 6.8m span

The stress becomes maximum in the case of the following diagram (four wheels are located on 6.8m span).



the stress of beam due to crane operation on simple beam "AB" becomes :

$$R_A = \frac{P}{6.8} (15.0 - 4x)$$

$$R_B = \frac{P}{6.8} (12.2 + 4x)$$

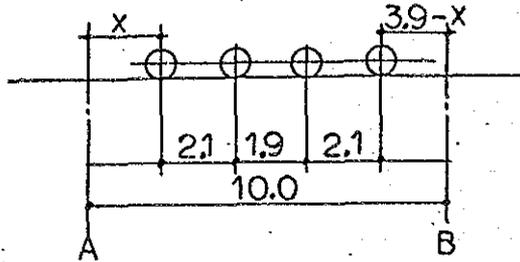
and the maximum bending moment becomes :

$$\begin{aligned} M_{\max} &= \left\{ \frac{15.0 - 4x}{6.8} (2.1 + x) - 2.1 \right\} P \\ &= -0.588 (x^2 - 0.97x - 4.31) P \end{aligned}$$

M becomes maximum when $x=0.485$ and P is 25.2 ton. Then, the maximum bending moment of beam becomes 67.3 t.m (vertical) and 5.6 t.m (horizontal), respectively.

2.2 10.0 m span

The stress becomes maximum in the case of the following diagram (four wheels are located on 10.0m span).



The stress of beam due to crane operation on simple beam "AB" is:

$$R_A = \frac{P}{10} (27.8 - 4x) \quad , \quad R_B = \frac{P}{10} (12.2 + 4x)$$

$$M_{max} = \left\{ \frac{27.8 - 4x}{10.0} (2.1 + x) - 2.1 \right\} P$$

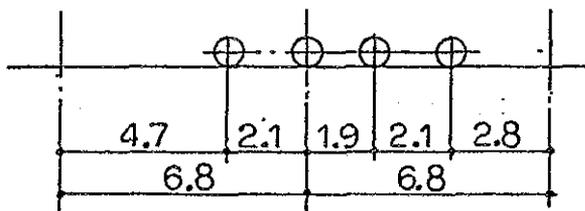
$$= -0.5(x^2 - 2.24x - 12.76) P$$

Therefore, the bending moment becomes maximum when $x = 2.425$, and the maximum vertical moment becomes 155.1 t.m (vertical) and 12.9 t.m (horizontal), respectively.

2.3 Maximum reaction against column due to wheel load

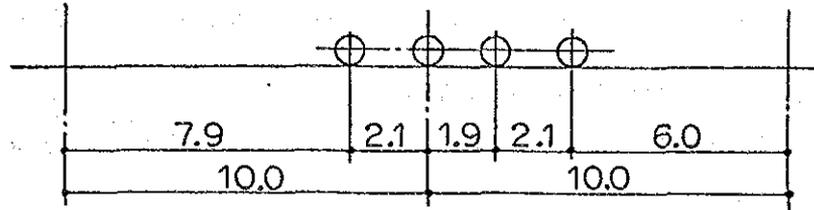
(1) In case of 6.8 m span

The reaction becomes maximum in the case of the following diagram:



$$\begin{aligned} N &= \frac{P}{6.8} \times (4.7 + 6.8 + 4.9 + 2.8) \\ &= 2.824 \cdot P \end{aligned}$$

- (2) In case of 10.0m span
The reaction becomes maximum in the case of the following diagram:



$$N = \frac{P}{10.0} (7.9 + 10.0 + 8.1 + 6.0)$$

$$= 3.2 \times P$$

2.4 Reaction against column due to dead load

$$R_A = 16.5 \times \frac{18.6}{20.4} + \frac{1}{2} \times 28.5 = 29.3 \text{ ton}$$

$$R_B = 16.5 \times \frac{1.8}{20.4} + \frac{1}{2} \times 28.5 = 15.7 \text{ ton}$$

(1) In case of 6.8m span

Reaction for foundation design:

$$P_A = 2.824 \times \frac{29.3}{4} = 20.7 \text{ ton}, \quad P_B = 2.824 \times \frac{15.7}{4} = 11.1 \text{ ton}$$

Reaction for column design (vertical):

$$20.7 \times 1.2 = 24.8 \text{ ton}, \quad 11.1 \times 1.2 = 13.3 \text{ ton}$$

Reaction for column design (horizontal):

$$20.7 \times 0.1 = 2.1 \text{ ton}, \quad 11.1 \times 0.1 = 1.1 \text{ ton}$$

Force in travelling direction:

$$20.7 \times 0.15 = 3.1 \text{ ton}, \quad 11.1 \times 0.15 = 1.7 \text{ ton}$$

(2) In case of 10.0m span

Reaction for foundation design:

$$P_A = 3.2 \times \frac{29.3}{4} = 23.4 \text{ ton}, \quad P_B = 3.2 \times \frac{15.7}{4} = 12.6 \text{ ton}$$

Reaction for column design (vertical) :

$$23.4 \times 1.2 = 28.1 \text{ ton} \quad , \quad 12.6 \times 1.2 = 15.1 \text{ ton}$$

Reaction for column design (horizontal) :

$$23.4 \times 0.1 = 2.3 \text{ ton} \quad , \quad 12.6 \times 0.1 = 1.3 \text{ ton}$$

Force in travelling direction :

$$23.4 \times 0.15 = 3.5 \text{ ton} \quad , \quad 12.6 \times 0.15 = 1.9 \text{ ton}$$

2.5 Reaction against column due to lifted load

(1) In case of 6.8 m span

Reaction for foundation design :

$$P_A = 2.824 \times \frac{84.0}{4} = 59.3 \text{ ton} \quad , \quad P_B = 2.824 \times \frac{21.0}{4} = 14.8 \text{ ton}$$

Reaction for column design (vertical) :

$$59.3 \times 1.2 = 71.2 \text{ ton} \quad , \quad 14.8 \times 1.2 = 17.8 \text{ ton}$$

Reaction for column design (horizontal) :

$$59.3 \times 0.1 = 5.9 \text{ ton} \quad , \quad 14.8 \times 0.1 = 1.5 \text{ ton}$$

Force in travelling direction :

$$59.3 \times 0.15 = 8.9 \text{ ton} \quad , \quad 14.8 \times 0.15 = 2.2 \text{ ton}$$

(2) In case of 10.0 m span

Reaction for foundation design :

$$P_A = 3.2 \times \frac{84.0}{4} = 67.2 \text{ ton} \quad , \quad P_B = 3.2 \times \frac{21.0}{4} = 16.8 \text{ ton}$$

Reaction for column design (vertical) :

$$67.2 \times 1.2 = 80.6 \text{ ton} \quad , \quad 16.8 \times 1.2 = 20.2 \text{ ton}$$

Reaction for column design (horizontal) :

$$67.2 \times 0.1 = 6.7 \text{ ton} \quad , \quad 16.8 \times 0.1 = 1.7 \text{ ton}$$

Force in travelling direction :

$$67.2 \times 0.15 = 10.1 \text{ ton} \quad , \quad 16.8 \times 0.15 = 2.5 \text{ ton}$$

II. Design of Main Building

本館設計

1. Calculation of Main Building (本館の荷重計算)

NOTE : UW --- UNIT WEIGHT [単位荷重]

PA --- FOR PERMANENT CONDITION [長期]

1.1 COLUMN AXIAL LOAD (1)
[柱軸力]

SE --- FOR SEISMIC CONDITION [地震時]

ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
A-101	ROOF	SLAB	$A=11.6 \times 4.0 =$	46.4	0.595	0.535	27.6	24.8
	FL+27.1	PARAPET	$L=11.6+4.0 =$	15.6	0.12		1.9	1.9
		SIDING	$A=15.6 \times (1.8+0.9) + 6.85 \times 3 =$	71.6	0.095		3.2	3.2
	FL+21.0	COLUMN	$L =$	6.1	0.2		1.2	1.2
		TRUSS	$L =$	11.0	0.25		2.8	2.8
							36.7	33.9
	FL+21.0	SIDING	$A=6.85 \times 10.0 =$	68.5	0.095		3.1	3.1
		COLUMN	$L =$	10.0	0.25		2.5	2.5
	FL+11.0	CRANE GIRDER	$L =$	4.0	0.35		1.4	1.4
							7.0	7.0
							(43.9)	(40.9)
	FL+11.0	SLAB	$A=2.85 \times 4.0 =$	11.4	2.225	0.8	25.4	9.1
COLUMN		$L =$	5.5	0.25		1.4	1.4	
FL+5.5						26.8	10.5	
						(70.5)	(51.4)	
FL+5.5	SLAB	$A =$	11.4	1.225	1.15	14.0	13.1	
	P.C. PANEL	$A=6.85 \times 5.5 =$	37.7	0.29		10.9	10.9	
	COLUMN	$L =$	5.5	0.25		1.4	1.4	
						26.3	25.4	
						(96.8)	(76.8)	
A-102	ROOF	SLAB	$A=11.6 \times 6.8 =$	78.9	0.595	0.535	46.9	42.2
	FL+27.1	PARAPET	$L =$	6.8	0.12		0.8	0.8
		SIDING	$A=6.8 \times (6.1+0.9) =$	47.6	0.095		2.1	2.1
	FL+21.0	COLUMN	$L =$	6.1	0.2		1.2	1.2
		TRUSS	$L =$	11.0	0.25		2.8	2.8
							53.8	49.1
	FL+21.0	SIDING	$A=6.8 \times 10.0 =$	68.0	0.095		3.1	3.1
		COLUMN	$L =$	10.0	0.25		2.5	2.5
	FL+11.0	CRANE GIRDER	$L =$	6.8	0.35		2.4	2.4
							8.0	8.0
							(61.8)	(57.1)
	FL+11.0	SLAB	$A=2.85 \times 6.8 =$	19.4	2.225	0.8	43.2	15.5
COLUMN		$L =$	5.5	0.25		1.4	1.4	
FL+5.5						44.6	16.9	
						(106.4)	(79.0)	
FL+5.5	SLAB	$A =$	19.4	1.225	1.15	23.8	22.3	
	P.C. PANEL	$A=6.8 \times 5.5 =$	37.4	0.29		10.8	10.3	
	COLUMN	$L =$	5.5	0.25		1.4	1.4	
						35.0	34.5	
						(142.4)	(108.5)	

NOTE : UW --- UNIT WEIGHT [單位荷重]															
PA --- FOR PERMANENT CONDITION [長期]															
SE --- FOR SEISMIC CONDITION [地震時]															
ML --- MACHINE LOAD [機械荷重]															
COLUMN AXIAL LOAD (2)															
[柱軸力]															
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD								
				PA	SE		PA (ton)	SE (ton)							
A-103	FL+27.1		SAME AS A-102				53.8	49.1							
	S														
	FL+21.0														
	S														
	FL+11.0														
	S														
	FL+5.5														
S															
A-104	FL+27.1		SAME AS A-102				53.8	49.1							
	S														
	FL+21.0														
	S														
	FL+11.0														
	S														
	FL+5.5														
	S														
	FL+5.5								SLAB	$A = 2.85 \times 3.4 =$	9.7	2.225	0.8	21.6	7.8
	S								GRATING	$A =$	9.7	0.225	0.225	3.2	2.2
	FL+5.5								COLUMN	$L =$	5.5	0.25		1.4	1.4
														26.2	11.4
														(88.0)	(68.5)
FL+5.5	GRATING	$A = 2.85 \times 6.8 =$	19.4	0.225	0.225	6.3	4.4								
S	P.C. PANEL	$A = 6.8 \times 5.5 =$	37.4	0.29		10.8	10.8								
FL±0	COLUMN	$L =$	5.5	0.25		1.4	1.4								
						18.5	16.6								
						(106.5)	(85.1)								
A-105	FL+27.1		SAME AS A-102				53.8	49.1							
	S														
	FL+21.0														
	S														
	FL+11.0														
	S														
	FL+5.5														
S															
FL+5.5	SLAB	$A = 2.85 \times 3.4 =$	9.7	2.225	0.8	21.6	7.8								
S	GRATING	$A =$	9.7	0.225	0.225	3.2	2.2								
FL+5.5	EXC. CUBICLE	$1/4 \times 4.0$				1.0	1.0								
	COLUMN	$L =$	5.5	0.25		1.4	1.4								
						27.2	12.9								
						(89.0)	(69.5)								
FL+5.5			SAME AS A-104				18.5	16.6							
S							(107.5)	(86.1)							
FL±0															

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (3)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
A-106	FL+27.1		SAME AS A-102				53.8	49.1
	FL+21.0		SAME AS A-102				8.0	8.0
	FL+11.0						(61.8)	(57.1)
	FL+11.0	SLAB	$A=2.85 \times 6.8 =$	19.4	2.225	0.8	43.2	15.5
	FL+5.5	EXC. CUBICLE	1.4×4.0				1.0	1.0
	FL+5.5	COLUMN	$L=$	5.5	0.25		1.4	1.4
							45.6	17.9
							(107.4)	(75.0)
	FL+5.5	SLAB	$A=$	19.4	1.225	1.15	23.0	22.3
	FL±0	IPB	$L=2.3 \times 3 =$	6.9	0.0		6.9	6.9
	P.C. PANEL	$A=6.8 \times 3.5 =$	23.8	0.29		10.8	10.8	
	COLUMN	$L=$	5.5	0.25		1.4	1.4	
						42.9	41.4	
						(150.3)	(116.4)	
A-107	FL+27.1	SLAB	$A=11.6 \times 8.4 =$	97.4	0.595	0.535	58.0	52.1
	FL+21.0	PARAPET	$L=$	8.4	0.12		1.0	1.0
		SIDING	$A=8.4 \times (6.1+0.9) =$	58.8	0.045		2.6	2.6
		COLUMN	$L=$	6.1	0.2		1.2	1.2
		TRUSS	$L=$	11.0	0.25		2.8	2.8
							65.6	59.7
	FL+21.0	SIDING	$A=8.4 \times 10.0 =$	84.0	0.045		3.8	3.8
	FL+11.0	COLUMN	$L=$	10.0	0.25		2.5	2.5
		CRANE GIRDER	$L=$	8.4	0.35		2.9	2.9
							9.2	9.2
							(74.0)	(68.9)
	FL+11.0	SLAB	$A=2.85 \times 8.4 =$	23.9	2.225	0.8	53.2	19.1
	FL+5.5	COLUMN	$L=$	5.5	0.25		1.4	1.4
							54.6	20.5
							(129.4)	(189.4)
FL+5.5	SLAB	$A=$	23.9	1.225	1.15	29.3	27.5	
FL±0	P.C. PANEL	$A=8.4 \times 5.5 =$	46.2	0.29		13.4	13.4	
	COLUMN	$L=$	5.5	0.25		1.4	1.4	
						44.1	42.3	
						(173.5)	(131.7)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (4) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
A-108	FL+27.1 s FL+21.0	SLAB	A=11.6x10.0=	116.0	0.595	0.535	69.0	62.1
		PARAPET	L=	10.0	0.12		1.2	1.2
		SIDING	A=10.0x(6.1+0.9)=	70.0	0.095		3.2	3.2
		COLUMN	L=	6.1	0.2		1.2	1.2
		TRUSS	L=	11.0	0.25		2.8	2.8
							17.4	70.5
	FL+21.0 s FL+11.0	SIDING	A=10.0x10.0=	100.0	0.045		4.5	4.5
		COLUMN	L=	10.0	0.25		2.5	2.5
		CRANE GIRDER	L=	10.0	0.35		3.5	3.5
							10.5	10.5
							(87.9)	(81.0)
	FL+11.0 s FL+5.5	SLAB	A=2.85x5.5=	15.7	1.925	1.35	22.4	21.2
		HAND RAIL	L=	2.85	0.035		0.1	0.1
		COLUMN	L=	5.5	0.25		1.4	1.4
		SHUTTER	A=5.0x10.0=	50.0	0.1		5.0	5.0
						28.9	27.7	
						(116.8)	(108.7)	
FL+5.5 s FL±0	STAIR	A=	15.7	0.43	0.36	6.8	5.7	
	HAND RAIL	L=	2.85	0.035		0.1	0.1	
	COLUMN	L=	5.5	0.25		1.4	1.4	
	P.C. PANEL	A=5.0x5.5=	27.5	0.29		8.0	8.0	
						16.8	15.2	
						(133.1)	(123.9)	
A-201			SAME AS A-108					
A-202			SAME AS A-107					
A-203			SAME AS A-106					
A-204			SAME AS A-105					
A-205			SAME AS A-104					
A-206			SAME AS A-103					
A-207			SAME AS A-102					
A-208			SAME AS A-101					

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (5)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
B-101	FL+11.0 s FL+5.5	SIDING	$A=5.25 \times 14.3 =$	75.1	0.025		3.4	3.4
		SLAB	$A=5.25 \times 4.0 =$	21.0	2.225	0.8	46.7	16.8
		COLUMN	$L =$	5.5	0.2		1.1	1.1
							51.2	21.3
	FL+5.5 s FL±0	SLAB	$A =$	21.0	1.225	1.15	25.7	24.2
		P.C. PANEL	$A=5.25 \times 5.5 =$	28.9	0.29		8.4	8.4
COLUMN		$L =$	5.5	0.2		1.1	1.1	
						35.2 (86.4)	33.7 (55.0)	
B-102	FL+11.0 s FL+5.5	SLAB	$A=5.25 \times 6.8 =$	35.7	2.225	0.8	79.4	28.6
		COLUMN	$L =$	5.5	0.2		1.1	1.1
								80.5
	FL+5.5 s FL±0	SLAB	$A =$	35.7	1.225	1.15	43.7	41.1
		COLUMN	$L =$	5.5	0.2		1.1	1.1
								44.8 (125.3)
B-103	FL+11.0 s FL+5.5	SLAB	$A=3.3 \times 6.8 =$	22.4	2.225	0.8	49.8	17.9
		COLUMN					1.1	1.1
								50.9
	FL+5.5 s FL±0	SLAB	$A =$	22.4	1.225	1.15	27.4	25.8
		COLUMN					1.1	1.1
								28.5 (79.4)
B-104	FL+11.0 s FL+5.5	SLAB	$A=3.3 \times 3.4 =$	11.2	2.225	0.8	24.9	9.0
		GRATING	$A =$	11.2	0.325	0.255	3.6	2.9
		COLUMN					1.1	1.1
							29.6	13.0
	FL+5.5 s FL±0	GRATING	$A=3.0 \times 6.8 =$	13.6	0.325	0.255	4.4	3.5
		COLUMN					1.1	1.1
							5.5 (35.1)	4.6 (17.6)
B-105	FL+11.0 s FL+5.5	SLAB	$A =$	11.2	2.225	0.8	24.9	9.0
		GRATING	$A =$	11.2	0.325	0.255	3.6	2.9
		EXC. CUBICLE COLUMN	$1/4 \times 4.0$				1.0	1.0
							1.1	1.1
							30.6	14.0
	FL+5.5 s FL±0		SAME AS B-104					5.5 (36.1)

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
B-106	FL+11.0	SLAB	$A=3.3 \times 6.8=$ 22.4	2.225	0.8		49.8	17.9
	FL+5.5	EXC. CUBICLE	$1/4 \times 4.0$				1.0	1.0
		COLUMN					1.1	1.1
							51.9	20.0
	FL+5.5	SLAB	$A=$ 22.4	1.225	1.15		27.4	25.8
	FL±0	I.P.B.	$L=5.3 \times 3=$ 15.9	1.0			15.9	15.9
		COLUMN					1.1	1.1
							44.4	42.8
						(96.3)	(62.8)	
B-107	FL+11.0	SLAB	$A=3.3 \times 3.0 + 5.25 \times 5.4=$ 38.3	2.225	0.8		85.2	30.6
	FL+5.5	COLUMN					1.1	1.1
							86.3	31.7
	FL+5.5	SLAB	$A=$ 38.3	1.225	1.15		46.9	44.0
FL±0	NO. 1 M/C	$1/2 \times 18.5$				9.3	9.3	
	COLUMN					1.1	1.1	
						57.3	54.4	
						(143.6)	(86.1)	
B-108	FL+11.0	SLAB	$A=4.0 \times 5.4=$ 21.6	2.225	0.8		48.1	17.3
	FL+5.5	STAIR	$A=1.2 \times 5.9=$ 6.5	0.43	0.36		2.8	2.3
		HAND RAIL	$L=$ 5.25		0.035		0.2	0.2
		COLUMN					1.1	1.1
						52.2	20.9	
	FL+5.5	SLAB	$A=$ 21.6	1.225	1.15		26.5	24.8
	FL±0	STAIR	$A=$ 6.5	0.43	0.36		2.8	2.3
		HANDRAIL	$L=$ 5.25		0.035		0.2	0.2
		COMMON M/C	$1/2 \times 16$				8.0	8.0
	COLUMN					1.1	1.1	
						38.6	36.4	
						(90.8)	(57.3)	
B-201	FL+11.0							
	FL+5.5		SAME AS B-108				52.2	20.9
	FL+5.5	SLAB	$A=$ 21.6	1.225	1.15		26.5	24.8
	FL±0	STAIR	$A=$ 6.5	0.43	0.36		2.8	2.3
		HAND RAIL	$L=$ 5.25		0.035		0.2	0.2
		COLUMN					1.1	1.1
						30.6	28.4	
						(82.8)	(49.3)	
B-202			SAME AS B-107					
B-203			SAME AS B-106					
B-204			SAME AS B-105					

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (7)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
B-205			SAME AS B-104					
B-206			SAME AS B-103					
B-207			SAME AS B-102					
B-208			SAME AS B-101					
D-101	FL+11.0	SIDING	A=6.0x14.3=	85.8	0.045		3.9	3.9
	FL+5.5	SLAB	A=6.0x4.0=	24.0	1.425	1.35	30.2	22.4
		M.S.P.	L=	5.0	1.0		5.0	5.0
		H.R.P.	L=	5.0	1.0		5.0	5.0
		COLUMN					1.1	1.1
							49.2	47.4
FL+5.5	FL+5.5	SLAB	A=	24.0	1.225	1.15	29.4	27.6
	FL±0	P.C. PANEL	A=6.0x5.5=	33.0	0.29		9.6	9.6
		COLUMN					1.1	1.1
							90.1	38.3
							(89.3)	(85.7)
D-102	FL+11.0	SLAB (O.H.)	A=6.0x2.3=	13.8	2.225	0.8	30.7	11.0
	FL+5.5	SLAB (OTHER)	A=6.0x3.9=	23.4	1.425	1.15	29.1	27.5
		GRATING	A=6.0x2.0=	12.0	0.325	0.255	3.9	3.1
		M.S.P.	L=	6.8	1.0		6.8	6.8
		H.R.P.	L=	6.8	1.0		6.8	6.8
		COLUMN					1.1	1.1
							78.4	56.3
FL+5.5	FL+5.5	SLAB	A=6.0x7.7=	46.2	1.225	1.15	56.6	53.1
	FL±0	COLUMN					1.1	1.1
							57.7	54.2
							(136.1)	(110.5)
D-107	FL+11.0	SLAB	A=6.0x6.0=	36.0	2.225	0.8	80.1	28.8
	FL+5.5	COLUMN					1.1	1.1
							81.2	29.9
FL+5.5	FL+5.5	SLAB	A=	36.0	1.225	1.15	44.1	41.4
	FL±0	No.1 W/C	1/2 x 18.5				9.3	9.3
		COLUMN					1.1	1.1
							54.5	51.8
							(135.7)	(81.7)

NOTE : UW --- UNIT WEIGHT [單位荷重]									
PA --- FOR PERMANENT CONDITION [長期]									
SE --- FOR SEISMIC CONDITION [地震時]									
ML --- MACHINE LOAD [機械荷重]									
COLUMN AXIAL LOAD (8) [柱軸力]									
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
D-108	FL+11.0 FL+5.5	SLAB (O.H.)	$A = 6.0 \times 5.9 =$	32.4	2.225	0.8	72.1	25.9	
		SLAB (OTHER)	$A = 6.0 \times 4.6 =$	27.6	1.425	1.35	39.3	37.3	
		COLUMN					1.1	1.1	
		HAND RAIL	$L = 3.0 \times 5.0 =$	8.0	0.035		0.3	0.3	
							112.8	64.6	
	FL+5.5 FL±0	SLAB	$A = 32.4 + 27.6 =$	60.0	1.225	1.15	73.5	69.0	
		COMMON M/C	$\frac{1}{2} \times 16.0$				8.0	8.0	
		COMMON P/C	$\frac{1}{2} \times 5.5$				2.8	2.8	
		HAND RAIL	$L =$	8.0	0.035		0.3	0.3	
		COLUMN					1.1	1.1	
						85.7	81.2		
						(198.5)	(195.8)		
D-201	FL+11.0 FL+5.5	SAME AS D-108						112.8	64.6
	FL+5.5 FL±0	SLAB	$A =$	60.0	1.225	1.15	73.5	69.0	
	HAND RAIL	$L =$	8.0	0.035		0.3	0.3		
	COLUMN					1.1	1.1		
						74.9	70.4		
						(187.7)	(135.0)		
D-202		SAME AS D-107							
D-207		SAME AS D-102							
D-208		SAME AS D-101							
F-101	FL+11.0 FL+5.5	SIDING	$A = 5.75 \times 14.3 =$	82.2	0.045		3.7	3.7	
		SLAB	$A = 5.75 \times 4.0 =$	23.0	1.425	1.35	32.8	31.1	
		C.R.P.	$L =$	5.0	1.0		5.0	5.0	
		COLUMN					1.1	1.1	
							42.6	40.9	
	FL+5.5 FL±0	SLAB	$A =$	23.0	1.225	1.15	28.2	26.5	
		P.C. PANEL	$A = 5.75 \times 5.5 =$	31.6	0.29		9.2	9.2	
		COLUMN					1.1	1.1	
							38.5	36.8	
							(81.1)	(77.7)	
F-102	FL+11.0 FL+5.5	SLAB (O.H.)	$A = 5.75 \times 3.9 =$	19.6	2.225	0.8	43.6	15.7	
		SLAB (OTHER)	$A =$	19.6	1.425	1.35	27.9	26.5	
		COLUMN					1.1	1.1	
		C.R.P.	$L =$	6.8	1.0		6.8	6.8	
							79.4	50.1	
FL+5.5 FL±0	SLAB	$A = 5.75 \times 6.8 =$	39.1	1.225	1.15	47.9	45.0		
	COLUMN					1.1	1.1		
						49.0	56.1		
						(128.4)	(106.2)		

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (9) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
F-103	FL+11.0	SLAB	$A=5.75 \times 10 + 3.75 \times 5.8 = 27.5$	2.225	0.8		61.2	22.0
	FL+5.5	G.R.P.	L=6.8	1.0			6.8	6.8
		COLUMN					1.1	1.1
							69.1	29.9
F-103	FL+5.5	SLAB	A=27.5	1.225	1.15		33.7	31.6
	FL±0	COLUMN					1.1	1.1
							39.8	32.7
							(103.9)	(62.6)
F-104	FL+11.0	SLAB	$A=3.75 \times 3.4 = 12.8$	2.225	0.8		28.5	10.2
	FL+5.5	GRATING	A=12.8	2.225	0.255		4.2	3.3
		COLUMN					1.1	1.1
							33.8	14.6
F-104	FL+5.5	GRATING	$A=3.75 \times 6.8 = 25.5$	0.325	0.255		8.3	6.5
	FL±0	COLUMN					1.1	1.1
							9.4	7.6
							(43.2)	(22.2)
F-105			SAME AS F-104					
F-106	FL+11.0	SLAB	$A=3.75 \times 3.4 + 1.25 \times 3.4 = 17.0$	2.225	0.8		37.8	12.6
	FL+5.5	GRATING	$A=1.5 \times 3.4 = 5.1$	0.325	0.255		1.7	1.3
		COLUMN					1.1	1.1
							40.6	16.0
F-106	FL+5.5		SAME AS F-103				39.8	32.7
	FL±0						(75.4)	(48.7)
F-107	FL+11.0	SLAB	$A=3.75 \times 3.4 + 5.75 \times 6.0 = 43.5$	2.225	0.8		96.8	39.8
	FL+5.5	COLUMN					1.1	1.1
							97.9	35.9
	F-107	FL+5.5	SLAB	A=43.5	1.225	1.15		53.3
FL±0		NO.2 P/C	$1/2 \times 16.5$				8.3	8.3
		COLUMN					1.1	1.1
							62.7	59.4
						(160.6)	(95.3)	
F-108	FL+11.0	SLAB (D.H.)	$A=5.75 \times 5.0 = 28.8$	2.225	0.8		64.1	22.0
	FL+5.5	SLAB (OTHER)	A=28.8	1.225	1.35		41.0	38.9
		COLUMN					1.1	1.1
							106.2	62.0
F-108	FL+5.5	SLAB	$A=5.75 \times 10.0 = 57.5$	1.225	1.15		70.4	66.1
	FL±0	COMMON P/C	$1/2 \times (5.5 + 18.5)$				12.0	12.0
		COLUMN					82.4	78.1
							(188.6)	(141.1)

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (10) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
F-201	FL+11.0							
	FL+5.5		SAME AS F-108				106.2	63.0
	FL+5.5	SLAB	A=5.75x10.0=	37.5	1.225	1.15	70.9	66.1
	FL±0	C/C COLUMN	1/2x4.0				2.0 1.1 73.5 (179.7)	2.0 1.1 69.2 (132.2)
F-202			SAME AS F-107					
F-203	FL+11.0	SLAB	A=3.75x6.8=	25.5	2.225	0.8	56.7	20.4
	FL+5.5	COLUMN					1.1	1.1
	FL±0		SAME AS F-106				57.8 (92.6)	21.5 (54.2)
F-204			SAME AS F-104					
F-205			SAME AS F-105					
F-206	FL+11.0	SLAB	A=5.75x(2.75x2.9+2.25x3.9)=	22.9	2.225	0.8	49.8	17.9
	FL+5.5	GRATING	A=1.5x3.4=	5.1	2.225	0.255	1.7	1.3
		C.R.P.	L=	6.8	1.0		6.8	6.8
	FL±0	COLUMN					1.1	1.1
	FL+5.5		SAME AS F-103				34.8 (99.2)	32.7 (59.8)
F-207			SAME AS F-102					
F-208			SAME AS F-101					

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (11)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-101	FL+27.1 5 FL+21.0		SAME AS A-101				36.7	33.9	
	FL+21.0 5 FL+16.0	SLAB	A=5.0x4.0=	20.0	0.635	0.535		12.7	10.7
		PARAPET	L=3.0x4.0=	9.0	0.12			1.1	1.1
		SIDING	A=7.75x5.0=	38.8	0.045			1.7	1.7
		CRANE GIRDER	L=	9.0	0.35			1.4	1.4
		COLUMN	L=	3.0	0.25			1.3	1.3
							18.2	16.2	
							(54.9)	(50.1)	
	FL+16.0 5 FL+11.0	SLAB	A=	20.0	1.29	0.665		25.8	13.3
		SIDING	A=7.75x5.0=	38.8	0.045			1.7	1.7
COLUMN							1.3	1.3	
						28.8	16.3		
						(83.7)	(66.4)		
FL+11.0 5 FL+5.5	SLAB	A=7.75x4.0=	31.0	1.25	1.35		44.2	41.9	
	COLUMN						1.4	1.4	
						45.6	43.3		
						(129.3)	(109.7)		
FL+5.5 5 FL±0	SLAB	A=	31.0	1.225	1.15		38.0	35.7	
	P.C. PANEL	A=7.75x5.5=	42.6	0.29			12.4	12.4	
	COLUMN	L=	5.5	0.25			1.4	1.4	
							51.0	49.5	
						(101.1)	(159.2)		
G-102	FL+27.1 5 FL+21.0		SAME AS A-102				53.8	49.1	
	FL+21.0 5 FL+16.0	SLAB	A=5.0x3.4=	17.0	0.635	0.535		10.8	9.1
		SLAB (DECK)	A=	17.0	1.035	0.69		17.6	11.7
		PARAPET	L=	6.8	0.12			0.8	0.8
		CRANE GIRDER	L=	6.8	0.35			2.4	2.4
		COLUMN						1.3	1.3
							32.9	25.3	
							(86.7)	(74.4)	
	FL+16.0 5 FL+11.0	SLAB	A=5.0x6.8=	34.0	1.29	0.665		43.9	22.6
		No. 4 LP COLUMN	1/2 x 11					5.5	5.5
						1.3	1.3		
						50.7	29.4		
						(137.4)	(103.8)		
FL+11.0 5 FL+5.5	SLAB (O.H.)	A=7.75x6.8=	18.7	2.225	0.8		41.6	15.0	
	SLAB (OTHER)	A=5.0x6.8=	34.0	1.225	1.35		48.5	45.9	
	No. 3 LP COLUMN	1/4 x 12.0					3.0	3.0	
							1.4	1.4	
						94.5	65.3		
						(231.9)	(169.1)		

NOTE : UW --- UNIT WEIGHT [単位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (12)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-102	FL+5.5	SLAB	$A = 2.75 \times 6.8 = 18.7$	1.225	1.15		69.6	60.6
	s	No. 2 LP	$1/4 \times 12.0$				3.0	3.0
	FL±0	COLUMN					1.4	1.4
							69.0	65.0
							(300.9)	(239.1)
G-103	FL+27.1		SAME AS A-103				53.8	49.1
	s							
	FL+21.0							
	FL+21.0	SLAB	$A = 5.0 \times 6.8 = 34.0$	1.035	0.69		35.2	23.5
	s	PARAPET	$L = 6.8$	0.12			0.8	0.8
	FL+16.0	CRANE GIRDER	$L = 6.8$	0.35			2.4	2.4
		DEAERATOR	$1/4 \times 269.0$				66.0	66.0
		COLUMN					1.3	1.3
							105.7	99.0
							(159.5)	(193.1)
G-104	FL+16.0		SAME AS G-102				50.7	29.4
	s						(210.2)	(172.5)
	FL+11.0							
	FL+11.0		SAME AS G-102				94.5	65.3
	s						(304.7)	(237.8)
	FL+5.5							
FL+5.5		SAME AS G-102				89.0	65.0	
s						(373.7)	(307.8)	
FL±0								
G-104	FL+27.1		SAME AS A-104				53.8	49.1
	s							
	FL+21.0							
	FL+21.0		SAME AS G-103				105.7	99.0
	s						(159.5)	(193.1)
	FL+16.0							
	FL+16.0	SLAB	$A = 5.0 \times 6.8 = 34.0$	1.29	0.665		43.9	22.6
	s	No. 8 HP	$1/4 \times 50.0$				12.5	12.5
	FL+11.0	COLUMN					1.3	1.3
							57.7	26.4
							(217.2)	(179.5)
	FL+11.0	SLAB (O.H.)	$A = 2.75 \times 3.9 = 10.7$	2.225	0.8		20.9	7.5
	s	SLAB (OTHER)	$A = 5.0 \times 6.8 = 34.0$	1.925	1.35		18.5	45.9
FL+5.5	GRATING	$A = 9.9$	0.325	0.255		3.1	2.4	
	No. 7 HP	$1/2 \times 45$				22.5	22.5	
	COLUMN					1.4	1.4	
						96.4	19.7	
						(313.6)	(259.2)	
FL+5.5	SLAB	$A = 5.0 \times 6.8 + 2.75 \times 0.9 = 36.5$	1.225	1.15		44.7	42.0	
s	GRATING	$A = 2.75 \times 5.9 = 16.2$	0.325	0.255		5.3	4.1	
FL±0	COLUMN					1.4	1.4	
						51.4	47.5	
						(365.0)	(306.7)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (13) [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-105	FL+27.1		SAME AS A-105				53.8	49.1	
	FL+21.0								
	FL+21.0		SAME AS G-102				32.9 (86.7)	25.3 (74.4)	
	FL+16.0								
	FL+16.0		SAME AS G-104				57.7 (144.4)	36.4 (110.8)	
	FL+11.0								
	FL+11.0		SAME AS G-104				96.4 (240.8)	79.7 (190.5)	
FL+5.5									
FL+5.5		No.1. C/C	SAME AS G-104 1/2 x 7.0				51.4 3.5 54.9 (295.7)	47.5 3.5 51.0 (291.5)	
FL±0									
G-106	FL+27.1		SAME AS A-106				53.8	49.1	
	FL+21.0								
	FL+21.0	SLAB	A=5.0x6.8=	34.0	0.635	0.535		21.6	18.2
	FL+16.0	PARAPET	L=	6.8	0.12			0.8	0.8
		CRANE GIRDER	L=	6.8	0.35			2.4	2.4
		COLUMN						1.3	1.3
								26.1	22.7
								(79.9)	(71.8)
	FL+16.0	SLAB	A=5.0x6.8=	34.0	1.29	0.665		43.9	22.6
	FL+11.0	COLUMN						1.3	1.3
								45.2	23.9
								(125.1)	(95.7)
	FL+11.0	SLAB (O.H.)	A=2.75x3.4=	9.4	2.225	0.8		20.9	7.5
FL+5.5	SLAB (OTHER)	A=5.0x6.8+1.25x3.4=	38.3	1.925	1.35		59.6	51.7	
	GRATING	A=1.5x3.4=	5.1	0.325	0.255		1.7	1.3	
	COLUMN						1.4	1.4	
							78.6	61.9	
							(203.7)	(157.6)	
FL+5.5	SLAB	A=7.75x6.8=	52.7	1.225	1.15		64.6	60.6	
FL±0	No.1. C/C	1/2 x 7.0					3.5	3.5	
	COLUMN						1.4	1.4	
							69.5	65.5	
							(273.2)	(223.1)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (14)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-107	FL+27.1							
	FL+21.0		SAME AS A-107				65.6	59.7
	FL+21.0	SLAB	A=5.0x8.4= 42.0	0.635	0.535		26.7	22.5
	FL+16.0	PARAPET	L= 8.4	0.12			1.0	1.0
		CRANE GIRDER	L= 8.4	0.35			2.9	2.9
		COLUMN					1.3	1.3
							31.9	27.7
							(97.5)	(87.4)
	FL+16.0	SLAB	A=5.0x3.9= 19.5	0.71	0.61		12.1	10.4
		BLOCK WALL	A=5.0x(5.0+5.0)= 50.0	0.4			20.0	20.0
		COLUMN					1.3	1.3
	FL+11.0	SLAB (Y/C)	A=5.0x5.0= 25.0	0.985			24.6	24.6
						58.0	56.3	
						(155.5)	(143.7)	
FL+11.0	SLAB (O.H.)	A=7.75x5.0x2.75x3.9= 40.1	2.225	0.8		107.0	48.5	
	SLAB (OTHER)	A=5.0x3.9= 19.5	1.925	1.35		29.2	23.0	
FL+5.5	COLUMN					1.4	1.4	
						132.6	62.9	
						(288.1)	(206.6)	
FL+5.5	SLAB	A=7.75x8.4= 65.1	1.225	1.15		79.7	79.9	
	1/3 C/S & 1/2 P/C	9.0x1/2x16.5				12.3	12.3	
	COLUMN					1.4	1.4	
						93.4	82.6	
						(281.5)	(295.2)	
G-108	FL+27.1							
	FL+21.0		SAME AS A-108				77.4	70.5
	FL+21.0	SLAB	A=5.0x10.0= 50.0	0.635	0.535		31.8	26.8
	FL+16.0	PARAPET	L= 10.0	0.12			1.2	1.2
		CRANE GIRDER	L= 10.0	0.35			3.5	3.5
		COLUMN					1.3	1.3
							37.8	32.8
							(115.2)	(103.3)
	FL+16.0	SLAB	A=	0.985			49.3	49.3
		BLOCK WALL	A=5.0x10.0= 50.0	0.4			20.0	20.0
	FL+11.0	COLUMN					1.3	1.3
							70.6	70.6
						(185.8)	(173.9)	
FL+11.0	SLAB	A=7.75x10.0= 77.5	2.225	0.8		172.4	62.0	
	COLUMN					1.4	1.4	
FL+5.5						173.8	63.4	
						(359.6)	(237.3)	
FL+5.5	SLAB	A=	1.225	1.15		94.9	89.1	
	COMMON P/C	1/2 x 18.5				9.3	9.3	
	COLUMN					1.4	1.4	
						105.6	99.8	
						(465.2)	(337.1)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (1/5)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-201	FL+27.1		SAME AS G-108				77.4	70.5
	FL+21.0						37.8	32.8
	FL+16.0		SAME AS G-108				(115.2)	(103.3)
	FL+16.0						70.6	70.6
	FL+11.0		SAME AS G-108				(185.8)	(173.9)
	FL+5.5		SAME AS G-108				173.8	63.4
G-202	FL+27.1		SAME AS G-107				65.6	59.7
	FL+21.0						31.9	27.7
	FL+16.0		SAME AS G-107				(97.5)	(87.4)
	FL+16.0	SLAB	A=5.0x5.4=	27.0	0.985		26.6	26.6
	FL+11.0	STAIR	A=5.0x3.0=	15.0	0.43	0.36	6.5	5.4
	FL+11.0	BLOCK WALL	A=5.0x(5.0x2x15.0)=	75.0	0.4		30.0	30.0
G-203	FL+11.0	COLUMN					1.3	1.3
	FL+5.5						64.4	63.3
	FL+5.5						(161.9)	(150.7)
	FL+11.0	SLAB (O.H)	A=5.0x2.0+2.25x0.4=	23.1	2.225	0.8	73.6	26.5
	FL+5.5	SLAB (OTHER)	A=5.0x3.9=	19.5	1.925	1.35	24.2	23.0
	FL+5.5	STAIR	A=5.0x3.0=	15.0	0.43	0.36	6.5	5.4
G-203	FL+5.5	COLUMN					1.4	1.4
	FL+5.5						105.7	56.3
	FL+5.5						(267.6)	(207.0)
	FL+5.5	SLAB	A=7.75x0.9-5.0x3.0=	50.1	1.225	1.15	61.9	57.6
	FL+0	STAIR	A=5.0x3.0=	15.0	0.43	0.36	6.5	5.4
	FL+0	NO.2 P/C	1/2 x 16.5				8.3	8.3
G-203	FL+0	COLUMN					1.4	1.4
	FL+0						77.6	72.7
	FL+0						(345.2)	(279.7)
	FL+0							
G-203	FL+27.1		SAME AS G-106				53.8	49.1
	FL+21.0						26.1	22.7
	FL+16.0		SAME AS G-106				(79.9)	(71.8)
	FL+11.0		SAME AS G-106				45.2	23.9
FL+11.0						(125.1)	(95.7)	

NOTE : UW --- UNIT WEIGHT [單位荷重]								
COLUMN AXIAL LOAD (t6) [柱軸力]								
PA --- FOR PERMANENT CONDITION [長期]								
SE --- FOR SEISMIC CONDITION [地震時]								
ML --- MACHINE LOAD [機械荷重]								
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-203	FL+11.0						94.5	65.3
	FL+5.5		SAME AS G-102				(219.6)	(161.0)
	FL+5.5						69.5	65.5
	FL±0		SAME AS G-106				(289.1)	(226.5)
G-204	FL+27.1							
	FL+21.0		SAME AS G-203				53.8	49.1
	FL+21.0						32.9	25.3
	FL+16.0		SAME AS G-105				(86.7)	(74.4)
	FL+16.0						57.7	36.4
	FL+11.0		SAME AS G-105				(144.4)	(110.8)
G-205	FL+11.0						94.5	65.3
	FL+5.5		SAME AS G-103				(238.9)	(176.1)
	FL±0		SAME AS G-105				54.9	51.0
			SAME AS G-105				(293.8)	(227.1)
G-205			SAME AS G-104					
G-206	FL+27.1							
	FL+21.0		SAME AS G-103				53.8	49.1
	FL+21.0						105.7	94.0
	FL+16.0		SAME AS G-103				(159.5)	(143.1)
	FL+16.0						50.7	29.4
	FL+11.0		SAME AS G-103				(210.2)	(172.5)
	FL+11.0						78.6	61.9
FL+5.5	No.7 HP	1/2 x 45	SAME AS G-106			22.5	22.5	
						101.1	84.4	
						(311.3)	(256.9)	
	FL+5.5						69.0	65.0
	FL±0		SAME AS G-103				(320.3)	(221.9)
G-207	FL+27.1							
	FL+21.0		SAME AS G-102				53.8	49.1
	FL+21.0						32.9	25.3
	FL+16.0		SAME AS G-102				(86.7)	(74.4)
	FL+16.0						50.7	29.4
	FL+11.0		SAME AS G-102				(137.4)	(103.8)

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (17) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
G-207	FL+11.0 S FL+5.5	SLAB (O.H.)	$A=2.75 \times 6.8 = 18.7$	2.235	0.8		41.6	15.0	
		SLAB (OTHER)	$A=5.0 \times 6.8 = 34.0$	1.925	1.35		98.5	45.9	
	COLUMN						1.4	1.4	
							91.5	62.3	
							(228.9)	(166.1)	
	FL+5.5 S FL±0		SAME AS G-102				69.0	65.0	
							(297.9)	(231.1)	
G-208			SAME AS G-101						
H-101	FL+21.0 S FL+16.0	SLAB	$A=5.6 \times 4.0 = 22.4$	0.595	0.535		13.3	12.0	
		PARAPET	$L=5.6 \times 2 \times 4.0 = 45.2$	0.12			1.8	1.8	
		SIDING	$L=15.2 \times 2.0 = 30.4$	0.045			2.1	2.1	
		STAIR	$A = 22.4$	0.43	0.36		9.6	8.1	
		SIDING	$A=5.0 \times (4.0+5.6) = 48.0$	0.045			2.2	2.2	
		COLUMN	$L = 5.0$	0.2			1.0	1.0	
							30.0	27.2	
		FL+16.0 S FL+11.0	STAIR	$A = 22.4$	0.43	0.36		9.6	8.1
			SIDING	$A = 48.0$	0.045			2.2	2.2
			COLUMN					1.0	1.0
								12.8	11.3
								(42.8)	(38.5)
	FL+11.0 S FL+5.5	STAIR	$A = 22.4$	0.43	0.36		9.6	8.1	
		COLUMN					1.1	1.1	
							10.7	9.2	
							(53.5)	(47.7)	
	FL+5.5 S FL±0	STAIR	$A = 22.4$	0.43	0.36		9.6	8.1	
		P.C. PANEL	$A=5.5 \times (4.0+5.6) = 52.8$	0.29			15.8	15.3	
		COLUMN					1.1	1.1	
							26.0	24.5	
							(79.5)	(72.2)	
H-102	FL+21.0 S FL+16.0	SLAB	$A=5.6 \times 3.4 = 19.0$	0.635	0.535		12.1	10.2	
		SLAB (DEAF)	$A = 19.0$	1.035	0.69		19.7	13.1	
		PARAPET	$L = 6.8$	0.12			0.8	0.8	
		SIDING	$A=5.9 \times 6.8 = 40.1$	0.045			1.8	1.8	
		COLUMN					1.0	1.0	
							35.4	26.9	
		FL+16.0 S FL+11.0	SLAB	$A=5.6 \times 6.8 = 38.1$	1.29	0.665		49.1	25.3
			SIDING	$A = 40.1$	0.045			1.8	1.8
			COLUMN					1.0	1.0
								51.9	28.1
								(87.3)	(55.0)
		FL+11.0 S FL+5.5	SLAB	$A = 38.1$	1.925	1.35		54.3	51.4
		No. 3 LP	$1/4 \times 12.0$				3.0	3.0	
		COLUMN					1.1	1.1	
							58.9	55.5	
							(145.7)	(110.5)	

NOTE : UW --- UNIT WEIGHT [單位荷重]								
COLUMN AXIAL LOAD (18) [柱軸力]								
PA --- FOR PARMANENT CONDITION [長期]								
SE --- FOR SEISMIC CONDITION [地震時]								
ML --- MACHINE LOAD [機械荷重]								
LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-102	FL+5.5	SLAB	A=	38.1	1.225	1.15	96.7	93.8
	FL±0	No.2 LP	1/4 x 12.0				3.0	3.0
		R.C. PANEL	A=5.5 x 6.8=	37.4	0.29		10.8	10.8
		COLUMN					1.1	1.1
						51.6	58.7	
						(207.3)	(169.2)	
H-103	FL+21.0	SLAB	A=	38.1	1.035	0.69	38.9	26.3
	FL+16.0	PARAPET	L=	6.8	0.12		0.8	0.8
		DEAERATOR	1/4 x 269.0				66.0	66.0
	FL+11.0	SIDING	A=5.9 x 6.8=	40.1	0.095		1.8	1.8
		COLUMN					1.0	1.0
							109.0	95.9
	FL+16.0						51.9	28.1
	FL+11.0		SAME AS H-102				(160.9)	(129.0)
	FL+11.0						58.4	55.5
	FL+5.5		SAME AS H-102				(219.3)	(179.5)
FL+5.5						61.6	58.7	
FL±0		SAME AS H-102				(280.9)	(238.2)	
H-104	FL+21.0							
	FL+16.0		SAME AS H-103				109.0	95.9
	FL+16.0		SAME AS H-102				51.9	28.1
	FL+11.0	No.8 HP	1/4 x 50.0				12.5	12.5
							69.4	40.6
							(173.4)	(136.5)
	FL+11.0	SLAB	A=	48.1	1.225	1.35	59.3	51.4
		COLUMN					1.1	1.1
	FL+5.5						55.4	52.5
							(228.8)	(189.0)
FL+5.5	SLAB	A=	38.1	1.225	1.15	46.7	43.8	
	FL±0	No.6 HP	1/2 x 36.0				18.0	18.0
		R.C. PANEL	A=	37.4	0.29		10.8	10.8
		COLUMN					1.1	1.1
						76.6	73.7	
						(305.4)	(262.7)	
H-105	FL+21.0							
	FL+16.0		SAME AS H-102				35.4	26.9
	FL+16.0						69.4	40.6
	FL+11.0		SAME AS H-104				(99.8)	(67.5)
	FL+11.0						55.4	52.5
	FL+5.5		SAME AS H-104				(155.2)	(120.0)
FL+5.5						76.6	73.7	
FL±0		SAME AS H-104				(231.8)	(193.9)	

60

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (19)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-106	FL+21.0 5 FL+16.0	SLAB	$A = 7.0 \times 9.0 =$	28.0	0.575	0.535	16.7	15.0
		PARAPET	$L = (7.0 + 9.0) \times 2 =$	22.0	0.12		2.6	2.6
	SIDING	$A = 22.0 \times 3.0 =$	66.0	0.045		3.0	3.0	
	SLAB	$A = 5.0 \times 6.8 =$	34.0	0.635	0.535	21.6	18.2	
	STAIR	$A = 7.0 \times 3.0 =$	21.0	0.93	0.36	9.0	7.6	
	SIDING	$A = 5.0 \times (6.9 + 2.8) =$	46.0	0.045		2.1	2.1	
	PARAPET	$L = 3.9 \times 3 + 7.0 =$	17.2	0.12		2.1	2.1	
	COLUMN					1.0	1.0	
						58.1	51.6	
	FL+16.0 3 FL+11.0	SLAB	$A =$	39.0	1.29	0.665	43.9	22.6
	STAIR	$A =$	21.0	0.93	0.36	9.0	7.6	
	BLOCK WALL	$A = 5.0 \times (8.0 + 9.0 \times 2) =$	80.0	0.9		32.0	32.0	
	SIDING	$A =$	46.0	0.045		2.1	2.1	
	COLUMN					1.0	1.0	
						88.0	65.3	
					(146.1)	(116.9)		
FL+11.0 3 FL+5.5	SLAB	$A =$	39.0	1.225	1.35	48.5	45.9	
STAIR	$A =$	21.0	0.93	0.36	9.0	7.6		
BLOCK WALL	$A = 5.5 \times (8.0 + 9.0 \times 2) =$	88.0	0.9		35.2	35.2		
COLUMN					1.1	1.1		
					93.8	89.8		
					(129.9)	(206.7)		
FL+5.5 3 FL±0	SLAB	$A =$	39.0	1.225	1.15	41.7	39.1	
STAIR	$A =$	21.0	0.93	0.36	9.0	7.6		
BLOCK WALL	$A =$	80.0	0.9		32.0	32.0		
P.C. PANEL	$A = 5.5 \times (6.9 + 2.8) =$	50.6	0.29		19.7	19.7		
COLUMN					1.1	1.1		
					98.5	94.5		
					(138.4)	(201.2)		
H-107	FL+21.0 5 FL+16.0	SLAB (AC)	$A = 5.0 \times 8.4 =$	42.0	0.635	0.535	26.7	22.5
		SLAB	$A = 7.0 \times 8.4 =$	58.8	0.765	0.665	45.0	39.1
		FENCE	$L =$	8.4	0.11		0.9	0.9
		COLUMN					1.0	1.0
							73.6	63.5

15

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (20)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-107	FL+16.0 5 FL+11.0	SLAB (CON)	$A = 5.0 \times (4.7 + 3.0 \times 2) = 42.2$	0.71	0.61		30.0	25.7
		SLAB (CEILING)	$A = 4.0 \times 8.9 = 35.6$	0.88			29.6	29.6
	BLOCK WALL	$A = 5.0 \times (0.4 \times 2 + 5.0) = 109.0$	0.4			43.6	43.6	
	WALL	$A = 0.0 \times 4.0 = 12.0$	0.036			0.4	0.4	
	COLUMN					1.0	1.0	
	SLAB (A/C)	$A = 5.0 \times 5.0 = 25.0$	0.985			24.6	24.6	
						129.2	129.2	
						(202.8)	(188.4)	
	FL+11.0 5 FL+5.5	SLAB	$A = 5.0 \times 8.9 = 44.5$	1.925	1.35		59.9	56.7
		COMPUTER	$A = 7.0 \times 6.9 = 48.3$	0.925			44.7	44.7
		CENT. CONT.	$A = 7.0 \times 1.5 = 10.5$	0.925	0.73		9.5	7.7
		BLOCK WALL	$A = 5.0 \times (8.4 \times 2 + 2.0 \times 2) = 104.0$	0.35			36.4	36.4
WALL (SING)		$A = 3.0 \times 8.9 = 26.7$	0.026			0.7	0.7	
WALL (W)		$A = 3.0 \times 7.0 = 21.0$	0.036			0.8	0.8	
COLUMN						1.1	1.1	
FL+5.5 5 FL±0	CHR. FLOR.	$A = 7.0 \times 8.9 = 62.3$	0.925			11.5	11.5	
						164.6	159.6	
					(367.4)	(348.0)		
FL+5.5 5 FL±0	SLAB	$A = 42.0$	1.225	1.15		51.5	48.3	
	CENTRAL RM.	$A = 7.0 \times 8.4 = 58.8$	1.025	0.925		60.3	57.3	
	WALL	$A = 3.0 \times 8.9 = 26.7$	0.026			0.7	0.7	
	BLOCK WALL	$A = 5.5 \times (8.4 \times 2 + 2.0 \times 2) = 119.4$	0.35			40.0	40.0	
	COLUMN					1.1	1.1	
					153.6	147.4		
					(521.0)	(495.4)		
H-108	FL+21.0 5 FL+16.0							
FL+16.0 5 FL+11.0	SLAB (A/C)	$A = 5.0 \times 10.0 = 50.0$	0.635	0.535		31.8	26.8	
	SLAB	$A = 7.0 \times 10.0 = 70.0$	0.765	0.665		53.6	46.6	
	FENCE	$L = 10.0$	0.11			1.1	1.1	
	COLUMN					1.0	1.0	
					87.5	75.5		
FL+16.0 5 FL+11.0	SLAB	$A = 7.0 \times 10.0 = 70.0$	0.88			61.6	61.6	
	SLAB (A/C)	$A = 5.0 \times 10.0 = 50.0$	0.985			49.3	49.3	
	BLOCK WALL	$A = 5.0 \times (10.0 \times 2) = 100.0$	0.4			40.0	40.0	
	COLUMN					1.0	1.0	
					151.9	151.9		
					(239.4)	(227.4)		
FL+11.0 5 FL+5.5	SLAB	$A = 5.0 \times 10.0 = 50.0$	1.925	1.35		71.3	67.5	
	CENT. CONT.	$A = 7.0 \times 10.0 = 70.0$	0.925	0.73		63.4	51.1	
	CHR. FLOOR	$A = 7.0$	0.195			13.7	13.7	
	BLOCK WALL	$A = 5.0 \times 10.0 \times 2 = 100.0$	0.35			35.0	35.0	
	WALL	$A = 3.0 \times 10.0 = 30.0$	0.028			0.8	0.8	
	COLUMN					1.1	1.1	
					185.3	169.2		
					(424.7)	(396.6)		

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (2/)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
(H-108)	FL+5.5 5 FL±0	SLAB	$A=5.0 \times 11.0 =$	55.0	1.225	1.15	61.3	57.5
		CONTRL. RM.	$A=7.0 \times 5.0 =$	35.0	1.025	0.975	35.9	34.1
		BATTERY RM.	$A=$	35.0	1.025	0.975	35.9	34.1
		BLOCK WALL	$A=5.5 \times 10.1 \times 1.5 =$	82.5	0.9		33.0	33.0
		WALL	$A=3.0 \times (5.0 \times 3) =$	45.0	0.026		1.2	1.2
		COLUMN					1.1	1.1
							168.4 (593.1)	161.0 (557.6)
H-201	FL+21.0 7 FL+16.0		SAME AS H-108				87.5	75.5
	FL+16.0 5 FL+11.0	SLAB (CON)	$A=7.0 \times 6.5 =$	45.5	0.71	0.61	32.3	27.8
		SLAB (AVC)	$A=5.0 \times 10.0 =$	50.0	0.985		49.3	49.3
		SLAB	$A=7.0 \times 3.5 =$	24.5	0.88		21.6	21.6
		BLOCK WALL	$A=5.0 \times 10.0 \times 2 =$	100.0	0.9		40.0	40.0
		WALL	$A=3.0 \times 4.0 =$	12.0	0.026		0.9	0.9
	COLUMN					1.0	1.0	
						144.6 (232.1)	140.1 (215.6)	
	FL+11.0 5 FL+5.5		SAME AS H-108				157.4 (289.5)	150.0 (265.6)
FL+5.5 5 FL±0	SLAB	$A=5.0 \times 10.0 =$	50.0	1.225	1.15	61.3	57.5	
	LABO	$A=7.0 \times 5.0 =$	35.0	0.855	0.705	29.9	24.7	
	BATTERY RM.	$A=$	35.0	1.025	0.975	35.9	34.1	
	BLOCK WALL	$A=5.5 \times (10.0 \times 1.5 + 2.0 \times 2) =$	104.5	0.9		41.8	41.8	
	WALL	$A=3.0 \times 2.0 \times 2 =$	12.0	0.026		1.1	1.1	
	COLUMN					1.1	1.1	
					171.1 (560.6)	160.3 (525.9)		
H-202	FL+21.0 7 FL+16.0		SAME AS H-107				73.6	63.5
	FL+16.0 5 FL+11.0	SLAB (AVC)	$A=5.0 \times 5.0 =$	25.0	0.855		24.6	24.6
		SLAB (CON)	$A=5.0 \times 3.4 + 7.0 \times 3.4 =$	25.8	0.71	0.61	53.8	46.2
		BLOCK WALL	$A=5.0 \times (8.4 \times 2 + 4.0 \times 5.0) =$	129.0	0.9		51.6	51.6
WALL	$A=3.0 \times (5.0 + 4.0) =$	27.0	0.36		9.7	9.7		
COLUMN					1.0	1.0		
					140.7 (219.3)	133.1 (196.6)		

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (22) [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD		
				PA	SE		PA (ton)	SE (ton)	
H-202	FL+11.0						164.6	159.6	
	FL+5.5		SAME AS H-107				(378.9)	(356.2)	
	FL+5.5 FL±0	SLAB	$A=5.0 \times 8.9 =$	42.0	1.225	1.15		51.5	48.3
		LADO	$A=7.0 \times 8.9 =$	58.8	0.855	0.705		50.3	41.5
		WALL	$A=2.0 \times 8.9 =$	25.2	0.026			0.7	0.7
		BLOCK WALL COLUMN	$A=5.5 \times (0.4 \times 2 + 2.0 \times 2) =$	114.4	0.35			40.0	40.0
						1.1	1.1		
						143.6	131.6		
						(522.5)	(487.8)		
H-203	FL+21.0	SLAB (CROF)	$A=5.0 \times 6.8 =$	34.0	0.625	0.535	21.6	18.2	
		SLAB (DWER)	$A=7.0 \times 9.0 =$	28.0	0.165	0.665	21.4	10.6	
	FL+16.0	FENCE	$L=3.4+7.6 =$	11.0	0.11		1.2	1.2	
		PARAPET	$L=2.8+6.4 =$	9.2	0.12		1.1	1.1	
		SIDING	$A=5.9 \times 9.2 =$	54.3	0.045		2.4	2.4	
		COLUMN					1.0	1.0	
							48.7	42.5	
	FL+16.0	SLAB (CLAY)	$A=7.0 \times 2.9 =$	23.8	0.79	0.72	18.8	17.1	
	FL+11.0	SLAB	$A=5.0 \times 6.8 =$	34.0	1.29	0.665	43.9	27.6	
		BLOCK WALL	$A=5.0 \times (7.0 \times 2 + 3.4 \times 5) =$	155.0	0.4		62.0	62.0	
		COLUMN					1.0	1.0	
		SIDING	$A=5.9 \times 9.2 =$	46.0	0.045		2.1	2.1	
							127.8	109.8	
							(176.5)	(147.3)	
FL+11.0		SAME AS H-106				93.8	89.8		
FL+5.5	NO.3 LP	$1/4 \times 12.0$				3.0	3.0		
						96.8	92.8		
						(273.3)	(240.12)		
FL+5.5		SAME AS H-106				98.5	94.5		
FL±0						(371.8)	(334.6)		
H-204	FL+21.0						35.9	27.9	
	FL+16.0		SAME AS H-105				64.7	40.6	
							(140.3)	(68.9)	
	FL+16.0		SAME AS H-105				55.4	32.5	
	FL+11.0	NO.3 LP	$1/4 \times 12.0$				3.0	3.0	
							38.4	55.5	
						(158.7)	(123.5)		
FL+5.5		SAME AS H-105				76.6	73.7		
FL±0						(235.3)	(197.2)		

fy

NOTE : UW --- UNIT WEIGHT [単位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (23)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
H-205			SAME AS H-104					
H-206	FL+21.0		SAME AS H-103				109.0	95.9
	FL+16.0							
	FL+16.0		SAME AS H-103				51.9	28.1
	FL+11.0						(160.9)	(124.0)
	FL+11.0		SAME AS H-104				55.4	52.5
	FL+5.5						(216.3)	(176.5)
	FL+5.5		SAME AS H-103				61.6	58.7
	FL±0						(277.9)	(235.2)
H-207	FL+21.0		SAME AS H-102				35.4	26.9
	FL+16.0							
	FL+16.0		SAME AS H-102				51.9	28.1
	FL+11.0						(87.3)	(55.0)
	FL+11.0	SLAB	$A=5.6 \times 6.8 =$	38.1	1.925	1.35	54.3	51.4
	COLUMN					1.1	1.1	
	FL+5.5					55.4	52.5	
						(142.7)	(107.5)	
	FL+5.5		SAME AS H-102				61.6	58.7
	FL±0						(209.3)	(166.2)
H-208			SAME AS H-101					
K-106	FL+21.0	SLAB	$A=7.6 \times 4.0 =$	30.4	0.765	0.665	23.3	20.2
		PARAPET	$L=7.6 \times 7.0 =$	11.6	0.12		1.4	1.4
	FL+16.0	SIDING	$A=5.9 \times 11.6 =$	68.4	0.095		3.1	3.1
		COLUMN					1.0	1.0
							28.8	25.7
	FL+16.0	SLAB	$A=$	30.4	0.08		26.8	26.8
		WALL	$A=3.0 \times (3.9+7.0) =$	31.2	0.028		0.9	0.9
FL+11.0	SIDING	$A=5.0 \times 11.6 =$	58.0	0.045		2.6	2.6	
	COLUMN					1.0	1.0	
						31.3	31.3	
						(60.1)	(57.0)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (24) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
(K-106)	FL+11.0 FL+5.5	COMPUTER	A=.....30.4	0.925			28.1	28.1
		WALL	A=.....31.2	0.028		0.9	0.9	
		COLUMN				1.1	1.1	
		CHR. FLOOR	A=.....30.4	0.195		5.9	5.9	
							26.0	26.0
							(96.1)	(93.0)
	FL+5.5 FL±0	SLAB	A=.....30.4	1.025	0.975		31.2	29.6
		WALL	A=.....31.2	0.028		0.9	0.9	
		R.C. PANEL	A=5.5×(8.0+7.6)=	63.8	0.29		18.5	18.5
		COLUMN				1.1	1.1	
							51.7	50.1
							(147.8)	(143.1)
K-107	FL+21.0 FL+16.0	SLAB	A=7.6×8.4=	66.1	0.765	0.665	50.6	44.0
		PARAPET	L=.....8.4	0.12		1.0	1.0	
		SIDING	A=5.9×8.4=	49.6	0.045		2.2	2.2
		COLUMN				1.0	1.0	
							54.8	48.2
	FL+16.0 FL+11.0	SLAB	A=7.6×3.4=	25.8	0.88		22.7	22.7
		SCAB	A=7.6×5.0=	38.0	0.88		33.4	33.4
		BLOCK WALL	A=5.0×7.6=	53.2	0.35		18.6	18.6
		WALL	A=3.0×(8.4+7.6)=	48.0	0.028		1.3	1.3
		SIDING	A=5.0×8.4=	42.0	0.045		1.9	1.9
		COLUMN				1.0	1.0	
							78.9	78.9
						(133.2)	(127.1)	
FL+11.0 FL+5.5	COMPUTER	A=3.6×6.9=	52.9	0.925		48.5	48.5	
	CENT. CONT.	A=2.0×1.5=	11.9	0.925	0.73	10.3	8.3	
	WALL (S)	A=3.0×8.4=	25.2	0.028		0.7	0.7	
	WALL (N)	A=3.0×7.0=	21.0	0.038		0.8	0.8	
	CHR. FLOOR	A=4.6×8.4=	63.8	0.195		12.4	12.4	
	COLUMN					1.1	1.1	
						73.8	71.0	
						(207.5)	(198.9)	
FL+5.5 FL±0	SLAB	A=.....63.8	1.025	0.975		65.4	62.2	
	WALL	A=3.0×8.4=	25.2	0.028		0.7	0.7	
	R.C. PANEL	A=5.5×8.4=	46.2	0.29		13.4	13.4	
	COLUMN					1.1	1.1	
						80.6	77.4	
						(288.1)	(276.3)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (25)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
K-108	FL+21.0 s FL+16.0	SLAB	$A=7.6 \times 10.0 =$	76.0	0.525	0.535	45.2	40.7
		PARAPET	$L =$	10.0	0.12		1.2	1.2
	SIDING	$A=5.9 \times 10.0 =$	59.0	0.045		2.7	2.7	
		COLUMN				1.0	1.0	
						50.1	45.6	
K-201	FL+16.0 s FL+11.0	SLAB	$A =$	76.0	0.088		66.9	66.9
		WALL	$A=3.0 \times 10.0 =$	30.0	0.028		0.8	0.8
	SIDING	$A=5.0 \times 10.0 =$	50.0	0.045		2.3	2.3	
		COLUMN				1.0	1.0	
						71.0	71.0	
						(121.7)	(116.6)	
K-201	FL+11.0 s FL+5.5	SLAB	$A =$	76.0	0.925	0.73	68.8	55.5
		WALL	$A =$	30.0	0.028		0.8	0.8
	CH. R. FLOOR	$A =$	76.0	0.195		14.8	14.8	
		COLUMN				1.1	1.1	
						85.5	72.2	
						(206.6)	(188.8)	
K-201	FL+5.5 s FL+0	SLAB	$A =$	76.0	1.025	0.975	77.9	74.1
		WALL	$A=3.0 \times (7.0 \times 2 + 10.0) =$	72.0	0.028		2.0	2.0
	P.C. PANEL	$A=5.5 \times 10.0 =$	55.0	0.29		16.0	16.0	
		COLUMN				1.1	1.1	
						97.0	93.2	
						(303.6)	(282.0)	
K-201	FL+21.0 s FL+16.0	SAME AS K-108					50.1	45.6
	FL+16.0 s FL+11.0	SLAB (CON.)	$A=7.6 \times 6.5 =$	49.4	0.71	0.61	35.1	30.1
		SLAB	$A=7.6 \times 3.5 =$	26.6	0.88		23.4	23.4
	WALL	$A=3.0 \times (16.0 + 7.0) =$	51.0	0.036		1.8	1.8	
		SIDING	$A=5.0 \times 10.0 =$	50.0	0.045		2.3	2.3
	COLUMN					1.0	1.0	
						63.6	58.6	
						(113.7)	(104.2)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (26)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)		UW		ML	COLUMN AXIAL LOAD	
					PA	SE		PA (ton)	SE (ton)
K-201	FL+5.5 ↓ FL±0	SLAB	$A=7.6 \times 5.0 =$	38.0	1.025	0.975		39.0	37.1
		SLAB	$A=$	38.0	0.855	0.705		32.5	26.8
		WALL	$A=3.0 \times (7.0 \times 2 + 11.0) =$	72.0	0.028			2.0	2.0
		P.C. PANEL	$A=5.5 \times 10.0 =$	55.0	0.29			16.0	16.0
		COLUMN						1.1	1.1
							90.6	83.0	
							(289.8)	(259.4)	
K-202	FL+21.0 ↓ FL+16.0		SAME AS K-107					57.8	48.2
	FL+16.0 ↓ FL+11.0	SLAB	$A=7.6 \times 8.9 =$	63.8	0.71	0.61		45.3	38.9
		WALL (S)	$A=3.0 \times 8.9 =$	25.2	0.028			0.7	0.7
		WALL (W)	$A=3.0 \times (7.0 + 3.9 \times 2) =$	41.4	0.036			1.5	1.5
		SIDING	$A=5.0 \times 8.9 =$	42.0	0.045			1.9	1.9
		COLUMN						1.0	1.0
								50.4	44.0
								(105.2)	(92.2)
	FL+11.0 ↓ FL+5.5		SAME AS K-107					73.8	71.8
							(179.0)	(169.0)	
FL+5.5 ↓ FL±0	SLAB	$A=7.6 \times 8.9 =$	63.8	0.855	0.705		54.5	45.0	
	WALL	$A=3.0 \times 8.9 =$	25.2	0.028			0.7	0.7	
	P.C. PANEL	$A=5.5 \times 8.9 =$	46.2	0.29			13.4	13.4	
	COLUMN						1.1	1.1	
							69.7	60.2	
							(248.7)	(229.2)	
K-203	FL+21.0 ↓ FL+16.0		SAME AS K-106					28.8	25.7
	FL+16.0 ↓ FL+11.0	SLAB	$A=$	30.9	0.71	0.61		21.6	18.5
		WALL (S)	$A=3.0 \times (5.9 + 7.0) =$	31.2	0.028			0.9	0.9
		WALL (W)	$A=3.0 \times 3.9 =$	10.2	0.036			0.9	0.9
		SIDING	$A=5.0 \times 11.6 =$	58.0	0.045			2.6	2.6
		COLUMN						1.0	1.0
							26.5	23.9	
							(55.3)	(49.1)	
	FL+11.0 ↓ FL+5.5	SLAB	$A=7.6 \times 9.0 =$	30.9	1.24	1.17		37.7	35.6
BLOCK WALL		$A=5.0 \times (7.0 \times 2 + 3.5 \times 2) =$	105.0	0.36			37.8	37.8	
WALL		$A=3.0 \times 7.0 =$	21.0	0.026			0.5	0.5	
COLUMN							1.1	1.1	
						77.1	75.0		
						(132.4)	(124.1)		
FL+5.5 ↓ FL±0	SLAB	$A=7.6 \times 9.0 =$	30.9	0.855	0.705		26.0	21.4	
	WALL	$A=3.0 \times (3.9 + 7.0) =$	31.2	0.028			0.9	0.9	
	P.C. PANEL	$A=5.5 \times (4.0 + 7.6) =$	63.8	0.29			18.5	18.5	
	COLUMN						1.1	1.1	
						46.5	41.9		
						(178.9)	(166.0)		

FD

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD () ML --- MACHINE LOAD [機械荷重]
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
		Im. case of	UNIT I only					
A-201	FL+27.1 s FL+21.0	SLAB	$A=11.6 \times 10.6 =$	123.0	0.595	0.535	73.2	65.8
		PARAPET	$L=11.6+10.6 =$	22.2	0.12		2.7	2.7
		SIDING	$A=22.2 \times (1.0+0.9) + 8.5 \times 4.3 =$	96.5	0.095		4.3	4.3
		COLUMN	$L =$	6.1	0.2		1.2	1.2
		TRUSS	$L =$	11.0	0.25		2.8	2.8
							84.2	76.8
	FL+21.0 s FL+11.0	SIDING	$A=8.5 \times 12.0 =$	85.0	0.095		3.8	3.8
		COLUMN	$L =$	10.0	0.25		2.5	2.5
		CRANE GIRDER	$L =$	5.0	0.35		1.8	1.8
							8.1	8.1
							(92.3)	(84.9)
	FL+11.0 s FL+5.5	COLUMN	$L =$	5.5	0.25		1.4	1.4
SHUTTER		$A=5.0 \times 10.0 =$	50.0	0.1		5.0	5.0	
SIDING		$A=2.9 \times 5.5 =$	16.0	0.095		0.7	0.7	
						7.1	7.1	
						(99.4)	(92.0)	
FL+5.5 s FL+0	SIDING	$A=2.9 \times 5.5 =$	16.0	0.095		0.7	0.7	
	COLUMN					1.4	1.4	
						2.1	2.1	
						(101.5)	(94.1)	
B-201	FL+11.0 s FL+5.5	SIDING	$A=5.25 \times 14.3 =$	75.1	0.095		3.4	3.4
		COLUMN					1.1	1.1
							4.5	4.5
FL+5.5 s FL+0	SIDING	$A=5.25 \times 5.5 =$	28.9	0.095		1.3	1.3	
	COLUMN					1.1	1.1	
						2.4	2.4	
						(6.9)	(6.9)	
D-201	FL+11.0 s FL+5.5	SIDING	$A=6.0 \times 14.3 =$	85.8	0.095		3.9	3.9
		SLAB	$A=3.0 \times 5.6 =$	16.8	1.925	1.35	23.9	22.7
		HAND RAIL	$L =$	5.0	0.035		0.2	0.2
		COLUMN	$L =$	5.5	0.2		1.1	1.1
							29.1	27.9
FL+5.5 s FL+0	SLAB	$A=5.6 \times 3.6 =$	20.2	1.225	1.15	24.7	23.2	
	HAND RAIL	$L =$	5.0	0.035		0.2	0.2	
	COLUMN					1.1	1.1	
						26.0	24.5	
						(55.1)	(52.4)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 COLUMN AXIAL LOAD (2) [柱軸力]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m ² or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
F-201	FL+11.0 S FL+5.5	SIDING	$A=5.75 \times 19.2 = 111.0$	0.095			5.0	5.0
		SLAB	$A=5.75 \times 5.6 = 32.2$	1.925	1.35		25.9	43.5
							1.1	1.1
							52.0	49.6
	FL+5.5 S FL±0	SLAB	$A = 32.2$	1.225	1.15		39.4	37.0
		SIDING	$A=5.75 \times 5.5 = 31.6$	0.095			1.4	1.4
		COLUMN					1.1	1.1
							41.9	39.5
							(93.9)	(89.1)
	G-201	FL+27.0 S FL+21.0		SAME AS A-201				87.2
FL+21.0 S FL+16.0		SLAB	$A=5.0 \times 10.0 = 50.0$	0.665	0.565		23.3	28.3
		PARAPET	$L=10.0$	0.12			1.2	1.2
		SIDING	$A=5.0 \times (2.2 + 3.9) = 33.0$	0.095			1.5	1.5
		CRANE GIRDER	$L=5.0$	0.35			1.8	1.8
		COLUMN	$L=5.0$	0.25			1.3	1.3
						39.1	34.1	
							(123.3)	(110.9)
FL+16.0 S FL+11.0		SLAB	$A=5.0 \times 10.0 = 50.0$	0.85			42.5	42.5
		BLOCK WALL	$A=5.0 \times 10.0 = 50.0$	0.4			20.0	20.0
		COLUMN					1.3	1.3
		SIDING	$A = 33.0$	0.095			1.5	1.5
							65.3	65.3
							(188.6)	(176.2)
FL+11.0 S FL+5.5	SLAB	$A=5.0 \times 10.0 + 2.8 \times 5.0 = 69.0$	1.925	1.35		91.2	86.4	
	SIDING	$A=5.5 \times (2.2 + 4.4) = 36.3$	0.095			1.6	1.6	
	COLUMN	$L=5.5$	0.25			1.4	1.4	
						94.2	89.4	
						(282.8)	(265.6)	
FL+5.5 S FL±0	SLAB	$A = 69.0$	1.225	1.15		70.4	73.6	
	SIDING					1.6	1.6	
	COLUMN					1.4	1.4	
						81.4	76.6	
						(369.2)	(392.2)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

COLUMN AXIAL LOAD (3)
 [柱軸力]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-202	FL+21.0 S	SLAB	A=5.0x8.4= 42.0	0.665	0.565		27.9	23.7
		PARAPET	L= 8.4	0.12			1.0	1.0
		SIDING	A=5.9x8.4= 49.6	0.095			2.2	2.2
	FL+16.0	COLUMN	L= 5.0	0.25			1.3	1.3
							32.4	28.2
	FL+16.0 S	SLAB	A=5.0x8.4= 42.0	0.85			35.7	35.7
		BLOCK WALL	A=5.0x(5.0+5.0)= 50.0	0.4			20.0	20.0
		COLUMN					1.3	1.3
		SIDING	A=5.0x8.4= 42.0	0.095			1.9	1.9
	FL+11.0						58.9	58.9
							(191.3)	(187.1)
FL+11.0 S	SLAB	A=5.6x8.4= 47.0	1.425	1.35		67.0	63.5	
	COLUMN	L= 5.5	0.25			1.4	1.4	
	SIDING	A=5.5x8.4= 46.2	0.095			2.1	2.1	
FL+5.5						70.5	67.0	
						(161.8)	(159.1)	
FL+5.5 S	SLAB	A= 47.0	1.225	1.15		57.6	54.1	
	COLUMN					1.4	1.4	
	SIDING					2.1	2.1	
FL±0						61.1	57.6	
						(222.9)	(211.7)	

NOTE : UW --- UNIT WEIGHT [單位荷重]
 PA --- FOR PERMANENT CONDITION [長期]
 SE --- FOR SEISMIC CONDITION [地震時]
 ML --- MACHINE LOAD [機械荷重]

LOCATION	LEVEL	NAME OF LOAD	AREA OR LENGTH (Unit:m2 or m)	UW		ML	COLUMN AXIAL LOAD	
				PA	SE		PA (ton)	SE (ton)
G-203	FL+21.0	SLAB	$A=5.6 \times 9.0 =$	22.9	0.665	0.565	14.9	12.7
		PARAPET	$L=5.0 \times 1.9 =$	9.6	0.12		9.7	9.7
	FL+16.0	SIDING	$A=9.6 \times (5.0 + 0.9) =$	56.6	0.045		2.5	2.5
		COLUMN	$L=$	7.0	0.25		1.8	1.8
							28.9	26.7
	FL+16.0	SLAB	$A=$	22.9	1.27	0.665	28.9	19.9
		SIDING	$A=9.6 \times 5.0 =$	48.0	0.045		2.2	2.2
		COLUMN	$L=$	5.0	0.25		1.3	1.3
							32.4	18.4
							(61.3)	(45.1)
	FL+11.0	SLAB	$A=$	22.4	1.425	1.35	31.9	30.2
		SIDING	$A=$	48.0	0.045		2.2	2.2
COLUMN		$L=$				1.4	1.4	
						35.5	33.8	
						(96.8)	(72.9)	
FL+5.5	SLAB	$A=$	22.4	1.225	1.15	27.4	25.8	
	SIDING	$A=9.6 \times 5.5 =$	52.8	0.045		2.4	2.4	
	COLUMN	$L=$	5.5	0.25		1.4	1.4	
						31.2	29.6	
						(122.0)	(108.5)	
H-203	FL+21.0	SLAB (ROOF)	$A=5.0 \times 9.0 =$	20.0	0.635	0.535	12.7	10.7
		SLAB (TOWER)	$A=9.6 \times 9.0 =$	30.9	0.765	0.665	23.3	20.2
	FL+16.0	FENCE	$L=3.4 \times 7.6 =$	11.0	0.11		1.2	1.2
		PARAPET	$L=$	12.0	0.12		1.4	1.4
	FL+16.0	SIDING	$A=5.0 \times 12.0 =$	60.0	0.045		2.7	2.7
		COLUMN	$L=$	5.0	0.2		1.0	1.0
							42.3	37.2
	FL+16.0	SLAB (CON)	$A=3.0 \times 3.4 =$	17.0	0.71	0.61	12.1	10.4
		SLAB (IAU)	$A=7.0 \times 3.4 =$	23.8	0.77	0.72	18.8	17.1
		Block WALL	$A=5.0 \times (7.0 \times 2 + 3.0 \times 5) =$	155.0	0.4		62.0	62.0
	FL+11.0	SIDING	$A=5.0 \times 12.0 =$	60.0	0.045		2.7	2.7
		COLUMN	$L=$				1.0	1.0
							96.6	93.2
						(138.9)	(130.2)	
FL+11.0	SLAB	$A=7.0 \times 4.0 =$	28.0	0.905	0.73	25.3	20.4	
	SLAB	$A=5.0 \times 4.0 =$	20.0	1.425	1.35	28.5	27.0	
FL+5.5	Block WALL	$A=5.0 \times (3.4 \times 2 + 9.0) =$	79.0	0.35		27.7	27.7	
	COLUMN	$L=$				1.1	1.1	
FL+5.5	SIDING	$A=5.5 \times 12.0 =$	66.0	0.045		3.0	3.0	
						85.6	79.2	
						(224.5)	(209.4)	
FL+5.5	SLAB	$A=5.0 \times 4.0 =$	20.0	1.225	1.15	24.5	23.0	
	STAIR	$A=2.0 \times 7.0 =$	14.0	0.43	0.36	6.0	5.0	
FL±0	Block WALL	$A=5.0 \times (8.0 + 4.0 \times 2) =$	30.0	0.4		32.0	32.0	
	SIDING	$A=5.5 \times 5.5 =$	30.3	0.045		1.4	1.4	
	COLUMN	$L=$	5.5	0.2		1.1	1.1	
	P.C PANEL	$A=6.4 \times 5.5 =$	35.2	0.29		10.2	10.2	
						95.2	72.7	
						(229.7)	(222.1)	

1.2 SEISMIC LOAD
[地震荷重]

ITEM		CALCULATION										
ZONE FACTOR (Z)		Z = 1.0										
STANDARD SHEAR COEFFICIENT (Co)		Co = 0.1										
GROUND CONDITION (Tc)		Tc = 0.8										
		Hard	Tc = 0.4									<input type="checkbox"/>
		Medium	Tc = 0.6									<input type="checkbox"/>
		Soft	Tc = 0.8									<input checked="" type="checkbox"/>
DIRECTION		X DIRECTION (LONG SPAN)					Y DIRECTION (SHORT SPAN)					
NATURAL PERIOD OF BUILDING (T)		T = 0.39					T = 0.57					
Heigh h = 27.1 m		Length of Span D =					m Length of Span D = 32.0 m					
T = 0.03 * h		=					=					
T = 0.05 * h / 4 * sqrt(D)		=					= 0.57					
T = h / 10		= 0.39					=					
CHARACTERISTICS OF VIBRATION OF THE BUILDING (Rt)		Rt = 1.0					Rt = 1.0					
			T	Rt				T	Rt			
Rt=1			-	=				-	=			
Rt=1-0.2*(T/Tc-1)^2		Tc	-	=			Tc	-	=			
Rt=1.6*Tc/T		2*Tc	-	=			2*Tc	-	=			
2*T/(1+3*T)		= 0.359					= 0.421					
SEISMIC LOAD FOR EACH FLOOR (Qi)												
	STORY	Wi	α i	Ai	Ci	Qi	Wi	α i	Ai	Ci	Qi	
	RF	1638.4	0.11	2.04	0.20	327.7		0.11	2.22	0.22	360.4	
	5TH	3625.6	0.25	1.63	0.16	576.9		0.25	1.74	0.17	612.0	
	4TH	5539.1	0.38	1.45	0.15	830.1		0.38	1.52	0.15	830.1	
	0PE	9779.7	0.67	1.20	0.12	1173.6		0.67	1.23	0.12	1173.6	
	MEZ	14581.5	1.00	1.00	0.1	1458.2		1.00	1.00	0.1	1458.2	
NOTE:		α i = Wi / Σ W										
		Ai = 1 + (1/√ α i - α i) * 2 * T / (1 + 3 * T)										
		Ci = Z * Rt * Ai * Co										

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+27.1~21.0								
	A	B	D	F	G	H	K	TOTAL
101	33.9				33.9			67.8
102	49.1				49.1			98.2
103	49.1				49.1			98.2
104	49.1				49.1			98.2
105	49.1				49.1			98.2
106	49.1				49.1			98.2
107	59.7				59.7			119.4
108	70.5				70.5			141.0
201	70.5				70.5			141.0
202	59.7				59.7			119.4
203	49.1				49.1			98.2
204	49.1				49.1			98.2
205	49.1				49.1			98.2
206	49.1				49.1			98.2
207	49.1				49.1			98.2
208	33.9				33.9			67.8
TOTAL	819.2				819.2			1,638.4
FL+21.0~16.0								
	A	B	D	F	G	H	K	TOTAL
101	40.9				50.1	27.2		118.2
102	57.1				74.4	26.9		158.4
103	57.1				143.1	95.9		296.1
104	57.1				143.1	95.9		296.1
105	57.1				74.4	26.9		158.4
106	57.1				71.8	51.6	25.7	206.2
107	68.9				87.4	63.5	48.2	268.0
108	81.0				103.3	75.5	45.6	305.4
201	81.0				103.3	75.5	45.6	305.4
202	68.9				87.4	63.5	48.2	268.0
203	57.1				71.8	42.5	25.7	197.1
204	57.1				74.4	27.4		158.9
205	57.1				143.1	95.9		296.1
206	57.1				143.1	95.9		296.1
207	57.1				74.4	26.9		158.4
208	40.9				50.7	27.2		118.8
TOTAL	952.6				1,495.8	918.2	239.0	3,605.6

TABLE OF COLUMN LOAD FOR EACH FLOOR (SEISMIC CONDITION)

FL+16.0-11.0								
	A	B	D	F	G	H	K	TOTAL
101	40.9				66.4	38.5		145.8
102	57.1				103.8	55.0		215.9
103	57.1				172.5	124.0		353.6
104	57.1				179.5	136.5		373.1
105	57.1				110.8	67.5		235.4
106	57.1				95.7	116.9	57.0	326.7
107	68.9				143.7	188.4	127.1	528.1
108	81.0				173.9	227.4	116.6	598.9
201	81.0				173.9	215.6	104.2	574.7
202	68.9				150.7	196.6	92.2	508.4
203	57.1				95.7	147.3	49.1	349.2
204	57.1				110.8	68.0		235.9
205	57.1				179.5	136.5		373.1
206	57.1				172.5	124.0		353.6
207	57.1				103.8	55.0		215.9
208	40.9				66.4	38.5		145.8
TOTAL	952.6				2099.6	1935.7	546.2	5534.1
FL+11.0-5.5								
	A	B	D	F	G	H	K	TOTAL
101	51.4	21.3	47.4	40.9	109.7	47.7		318.4
102	74.0	29.7	56.3	50.1	169.1	110.5		489.7
103	74.0	19.0		29.9	237.8	179.5		540.2
104	68.5	13.0		14.6	259.2	189.0		544.3
105	69.5	14.0		14.6	190.5	120.0		408.6
106	75.0	20.0		16.0	157.6	206.7	93.0	568.3
107	89.4	31.7	29.9	35.9	206.6	348.0	198.9	940.4
108	108.7	20.9	64.6	63.0	237.3	396.6	188.8	1079.9
201	108.7	20.9	64.6	63.0	237.3	365.6	176.4	1036.5
202	89.4	31.7	29.9	35.9	207.0	356.2	164.0	914.1
203	75.0	20.0		21.5	161.0	240.1	124.1	641.7
204	69.5	14.0		14.6	176.1	123.5		397.7
205	68.5	13.0		14.6	259.2	189.0		544.3
206	74.0	19.0		27.1	256.9	176.5		553.5
207	74.0	29.7	56.3	50.1	166.1	107.5		483.7
208	51.4	21.3	47.4	40.9	109.7	47.7		318.4
TOTAL	1221.0	339.2	396.4	532.7	3141.1	3204.1	945.2	9779.7