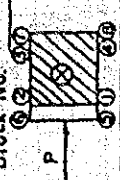




ROCK SHEAR TEST DATA SHEET (1)

Test Location DA-2, BS-4 Measuring Point TB-15-70M Date Measured 22.11.1988 Block No. 8  $\text{kg/cm}^2$   
 Geological Classification Opilolite Rock Grade \_\_\_\_\_  
 Vertical Jack Capacity 200 ton x 1 units, Max. Oil Pressure 3963  $\text{kg/cm}^2$  Ram Diameter 2585 cm  
 Diagonal Jack Capacity 200 ton x 2 units, Max. Oil Pressure 7017  $\text{kg/cm}^2$  Ram Diameter 1905 cm



Time Elapsed	Vertical Jack Pressure ( $\text{kg/cm}^2$ )		Diagonal Jack Pressure ( $\text{kg/cm}^2$ )		Vertical Displacement ( $\times 10^{-3}$ mm)		Horizontal Displacement ( $\times 10^{-3}$ mm)		Remarks
	①	②	③	④	⑤	⑥	⑦	⑧	
0	1	7	0	0	0	0	0	0	0
4	2	14	0	0	0	0	0	0	0
8	2	14	0	0	0	0	0	0	0
16	0	0	0	0	0	0	0	0	0.3
20	0	0	0	0	0	0	0	0	0.3
28	2	14	0	1	0	0	0	0	0.3
32	3	21	0	2	0	0	0	0	0.3
36	4	29	0	1	0	0	0	0	0.3
40	5	36	0	0	0	0	0	0	0.3
44	6	43	0	0	0	0	0	0	0.3
48	7	50	2	1	0	0	0	0	0.3
52	8	57	0	2	0	0	0	0	0.7
58	8	57	0	0	0	0	0	0	0.0

ROCK SHEAR TEST DATA SHEET (Z)

Measuring Point 7D 15.7 m Block No. PA-2, BS-4 N = 8 kg/cm<sup>2</sup>

Time	Time Elapsed	Vertical Jack		Diagonal Jack		Vertical Displacement (x 10 <sup>-3</sup> mm)				Horizontal Displacement (x 10 <sup>-3</sup> mm)				Remarks			
		Pressure (kg/cm <sup>2</sup> )	Stress (kg/cm <sup>2</sup> )	Pressure (kg/cm <sup>2</sup> )	Stress (kg/cm <sup>2</sup> )	①	②	①+② 2	③	④	③+④ 2	⑤	⑥		⑦	⑧	⑤-⑧ 4
	0	8	57	0.34	2.25	0	0	0	0	0	0	0	0	0	0	0	
	4	"	"	0.64	4.5	0	2	1.35	0	0	0	0	0	0	0	0	
	8	"	"	0.99	6.5	0	0	0.35	0	0	0	0	0	0	0	0	
	12	"	"	1.33	8.75	0	1	0.56	0	0	0	0	0	0	0	0	
	16	"	"	1.67	11.0	0	0	0.6	0	0	0	0	0	0	0	0	
	20	"	"	2.01	13.25	0	0	0.6	6	3.25	0	0	0	0	0	0	
	24	"	"	2.35	15.5	0	0	0.6	4	3.45	0	0	0	0	0	0	
	28	"	"	2.65	17.5	0	0	0.6	1	0.5.3	0	0	0	0	0	0	
	32	"	"	3.00	19.75	0	5	2.85	2	1.6	0	0	0	0	0	0	
	36	"	"	3.34	22.0	-6	5	0.5.8	0	0.6	0	0	0	0	0	0	
	40	"	"	3.68	24.25	0	0	0.8	1	0.5.25	1	0	0	0	0	0.3.13	
	44	"	"	4.02	26.5	0	0	0.8	0	0.65	2	0	0	0	0	0.5.18	
	48	"	"	4.32	28.5	0	0	0.8	0	0.65	1	0	0	0	0	0.3.21	
	52	"	"	4.66	30.75	0	0	0.8	0	0.65	0	0	0	0	0	0.3.21	
	56	"	"	5.00	33.0	0	0	0.8	0	0.65	1	0	0	0	0	0.3.24	
	1:00	"	"	5.35	35.25	0	0	0.8	0	0.65	1	0	0	0	0	0.3.27	
	4	"	"	5.69	37.5	0	0	0.8	0	0.65	1	0	0	0	0	0.8.25	
	8	"	"	5.99	39.5	0	0	0.8	0	0.65	1	0	0	0	0	0.3.28	
	12	"	"	6.33	41.75	0	0	0.8	0	0.65	1	2	1	0	0	1.4.8	
	16	"	"	6.67	44.0	0	0	0.8	0	0.65	3	0	0	0	0	0.8.56	
	20	"	"	7.01	46.25	0	0	0.8	0	0.65	4	0	0	0	0	1.2.6	
	24	"	"	7.36	48.5	0	-1	0.5.23	0	0.65	5	1	0	0	0	1.8.3.0	
	28	"	"	7.66	50.5	0	0	0.25	0	0.65	2	0	0	3	0	1.3.9.7	
	32	"	"	8.00	52.75	0	0	0.25	0	0.65	4	1	0	0	0	1.3.11	
	36	"	"	8.34	55.0	0	-1	0.5.7	0	0.65	5	0	0	0	0	2.3.33.3	
	40	"	"	8.68	57.25	1	-1	0.2	0	0.65	5	0	0	0	0	2.3.35.6	
	44	"	"	9.02	59.5	0	0	0.2	0	0.65	1	1	1	0	0	1.8.1.4	
	48	"	"	9.33	61.5	0	0	0.2	0	0.65	2	0	0	2	0	1.8.1.2	
	52	"	"	9.67	63.75	0	-2	-1.6	0	0.65	4	0	0	0	0	2.3.2.5	
	56	"	"	10.01	66.0	0	-1	0.5.55	0	0.65	4	1	2	0	0	2.8.2.4.3	
	2:00	"	"	10.35	68.25	0	-4	-2.2.5	0	0.65	10	0	0	0	0	5.3.2.6	
	4	"	"	10.69	70.5	0	-3	-1.5.2	0	0.65	10	0	0	0	0	5.8.2.6.4	

ROCK SHEAR TEST DATA SHEET (3)

Measuring Point TD-15.7m Block No. DA-2 BS-4 N = 2 kg/cm<sup>2</sup>

Time	Time Elapsed	Vertical Jack		Diagonal Jack		Vertical Displacement (x 10 <sup>-3</sup> mm)					Horizontal Displacement (x 10 <sup>-3</sup> mm)					Remarks
		Vertical Pressure (kg/cm <sup>2</sup> )	Stress (kg/cm <sup>2</sup> )	Vertical Pressure (kg/cm <sup>2</sup> )	Stress (kg/cm <sup>2</sup> )	①	②	$\frac{①+②}{2}$	③	④	$\frac{③+④}{2}$	⑤	⑥	⑦	⑧	
	8	8	57	11.00	72.5	-1	-4	$\frac{-2.5-0.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	4	0	6	7	$\frac{4.3-3.7}{4}$
	12	"	"	11.94	74.95	-1	-2	$\frac{2.5-2}{2}$	0	0	$\frac{0.65-0.65}{2}$	4	1	8	8	$\frac{5.3-4.5}{4}$
	16	"	"	11.68	77.0	-1	-2	$\frac{1.5-3.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	10	1	5	2	$\frac{4.5-4.5}{4}$
	20	"	"	12.02	79.25	0	-2	$\frac{1.5-5.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	6	1	5	2	$\frac{3.5-5.3}{4}$
	24	"	"	12.36	81.5	-3	-7	$\frac{3-9.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	3	4	6	10	$\frac{5.8-5.8}{4}$
	28	"	"	12.66	83.5	-3	-7	$\frac{3-9.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	3	4	4	8	$\frac{4.8-4.8}{4}$
	32	"	"	13.01	85.75	-1	-11	$\frac{6-20.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	10	11	14	8	$\frac{7.8-7.4}{4}$
	36	"	"	13.35	88.0	-3	-11	$\frac{7-20.5}{2}$	0	0	$\frac{0.65-0.65}{2}$	6	7	10	9	$\frac{8-8.7}{4}$
	40	"	"	13.69	90.25	-23	-13	$\frac{2.5-25.5}{2}$	0	-4	$\frac{-2.45-2.45}{2}$	26	10	16	11	$\frac{15.8-9.2}{4}$
	44	"	"	14.03	92.5	-21	-11	$\frac{2.5-25.5}{2}$	0	-8	$\frac{-4.05-4.05}{2}$	25	18	13	13	$\frac{7.3-10.3}{4}$
	48	"	"	14.37	94.5	-9	-13	$\frac{1-22.5}{2}$	-8	0	$\frac{-4.35-4.35}{2}$	1	10	20	10	$\frac{10.3-10.3}{4}$
	52	"	"	14.67	96.75	-10	-12	$\frac{1-22.5}{2}$	-6	0	$\frac{-3.65-3.65}{2}$	1	10	15	10	$\frac{7-23.8}{4}$
	56	"	"	15.01	99.0	-3	-10	$\frac{4.5-9.0}{2}$	-5	-2	$\frac{-3.3-1.0}{2}$	5	7	12	7	$\frac{2.8-11.6}{4}$
	3:00	"	"	15.36	101.25	-2	-9	$\frac{5.5-9.5}{2}$	-4	-1	$\frac{-2.5-12.5}{2}$	5	8	13	8	$\frac{8.5-10.1}{4}$
	4	"	"	15.70	103.5	-15	-28	$\frac{7.5-12}{2}$	-22	0	$\frac{-1-23.5}{2}$	17	27	42	17	$\frac{25.8-12}{4}$
	8	"	"	16.00	105.5	-16	-28	$\frac{2.2-19.8}{2}$	-18	-1	$\frac{-9.5-1.5}{2}$	18	28	42	15	$\frac{25.8-20.7}{4}$
	12	"	"	16.34	107.75	-6	-25	$\frac{15.3-14.7}{2}$	-12	0	$\frac{-6-3.9}{2}$	10	38	19	10	$\frac{19.3-22.1}{4}$
	16	"	"	16.58	110.0	-18	-25	$\frac{21.5-21}{2}$	-16	0	$\frac{-8-14.7}{2}$	7	16	30	7	$\frac{15-23.6}{4}$
	20	"	"	17.03	112.5	-2	-26	$\frac{14-18.5}{2}$	-15	-2	$\frac{-8.5-5.5}{2}$	5	18	20	18	$\frac{15.3-24.3}{4}$
	24	"	"	17.07	114.5	-18	-43	$\frac{20.5-11.5}{2}$	-27	0	$\frac{-13.5-6.9}{2}$	18	66	54	37	$\frac{43.8-29.5}{4}$
	28	"	"	17.67	116.5	-32	-18	$\frac{-25-10.5}{2}$	-13	-8	$\frac{-7.5-19.5}{2}$	40	6	12	6	$\frac{16-31.1}{4}$
	32	"	"	17.97	118.5	-6	-18	$\frac{12-15.2}{2}$	-14	-1	$\frac{-2.5-8.7}{2}$	0	16	32	12	$\frac{15-32.6}{4}$
	36	"	"	18.35	121.0	-16	-33	$\frac{21.5-27}{2}$	-26	0	$\frac{-2.5-10.0}{2}$	10	52	25	14	$\frac{25.3-37.4}{4}$
	40	"	"	18.65	123.0	-4	-18	$\frac{-1-28.6}{2}$	-7	0	$\frac{-2.5-10.5}{2}$	8	10	16	9	$\frac{10.5-34.2}{4}$
	44	"	"	18.96	125.0	-13	-28	$\frac{20.5-18.5}{2}$	-19	0	$\frac{-2.5-11.3}{2}$	17	42	37	3	$\frac{24-38.7}{4}$
	48	"	"	19.34	127.5	-15	-41	$\frac{28-23.5}{2}$	-22	-2	$\frac{2.5-15}{2}$	7	48	45	44	$\frac{36-42.3}{4}$
	52	"	"	19.68	129.75	-37	-31	$\frac{34-37.5}{2}$	-32	-5	$\frac{-13.5-19.5}{2}$	57	24	33	9	$\frac{30.8-25.8}{4}$
	56	"	"	20.02	132.0	-10	-26	$\frac{-18-38.5}{2}$	-19	-5	$\frac{-7.5-15.4}{2}$	7	46	23	5	$\frac{20.3-29.4}{4}$
	4:00	"	"	20.06	134.25	-7	-40	$\frac{23.5-42}{2}$	-25	0	$\frac{-2.5-18.5}{2}$	52	60	46	16	$\frac{43.5-27.6}{4}$
	4	"	"	20.70	136.5	-31	-57	$\frac{46-45.5}{2}$	-36	-1	$\frac{-7.5-18.5}{2}$	-17	48	76	31	$\frac{34-55.2}{4}$
	8	"	"	21.01	138.5	-22	-45	$\frac{33.5-37.5}{2}$	-38	-2	$\frac{-20-20.5}{2}$	5	47	31	12	$\frac{23.8-27.5}{4}$
	12	"	"	21.38	141.0	-33	-48	$\frac{-40-5.2}{2}$	-42	-3	$\frac{-22.5-22.5}{2}$	33	75	74	10	$\frac{45-23.9}{4}$

ROCK SHEAR TEST DATA SHEET (4) Measuring Point TD. 15.7 m Block No. DA-2, B5-4 N = 8 kg/cm<sup>2</sup>

Time	Vertical Jack Stress Pressure (kg/cm <sup>2</sup> )	Diagonal Jack Stress Pressure (kg/cm <sup>2</sup> )	Vertical Displacement (x 10 <sup>-3</sup> mm)				Horizontal Displacement (x 10 <sup>-3</sup> mm)				Remarks						
			①	②	③	④	⑤+④ 2	⑤	⑥	⑦		⑧	⑨-⑩ 4				
16	8	57	21.69	14.30	-16	-26	-21	-17	-7	-13	2.895	2	40	43	14	27.5	1987
20	"	"	22.03	14.525	-23	-27	-17	-7	-7	-13	2.85	20	55	47	18	35	1887
24	"	"	22.87	14.75	-35	-50	-58	-15	-15	-35	2.88	20	75	63	47	57.3	136
28	"	"	22.67	14.95	-40	-76	-22	-13	-13	-35	2.85	40	130	115	19	76.811	
32	"	"	23.05	15.20	-37	-61	43	-11	-11	-35	2.87	15	130	137	15	74.85	
36	"	"	23.36	15.40	-43	-53	-52	-26	-26	-35	2.85	21	120	115	48	76.763	
40	"	"	23.66	15.60	-26	-53	-51	-8	-8	-35	2.85	24	90	60	10	74.8073	
44	"	"	23.96	15.80	-39	-45	-52	-7	-7	-35	2.87	30	102	116	122	92.3	1097
48	"	"	24.34	16.05	-30	-37	-71	-26	-26	-35	2.86	50	118	129	46	80.8	100.6
52	"	"	24.65	16.25	-80	-53	-49	-44	-44	-35	2.82	90	80	91	87	87.867	
56	"	"	25.02	16.50	-100	-111	-156	-79	-79	-35	2.86	120	190	118	105	133.3	100.9
5:00	"	"	25.33	16.70	-228	-83	-447	-356	-356	-356	2.85	580	1180	974	455	77.3	100.5
4	"	"															
8	"	"															
12	"	"															
16	"	"															

### 3-12 Seismic Prospecting

**\* Hagiwara's analysis method:**

As shown in Fig. A, this method considers the ground to be a two layered structure, with velocity in the upper layer  $V_1$  and velocity in the lower layer,  $V_2$ .  $T_{AP}$  is travel time of refracted wave from shot point A, received at P;  $T_{BP}$  is travel time of the refracted wave from B to P; and  $T_{AB}$  is travel time of the refracted wave from A' to B (The white circles in the figure represent travel times of refracted waves received at P. The X marks represent travel times of direct waves—those waves received at P that are propagated in the first layer only.) Here,  $T_{AP}$ ,  $T_{BP}$  and  $T_{AB}$  are quantities obtainable through direct observation. The quantity  $t_0$ , where

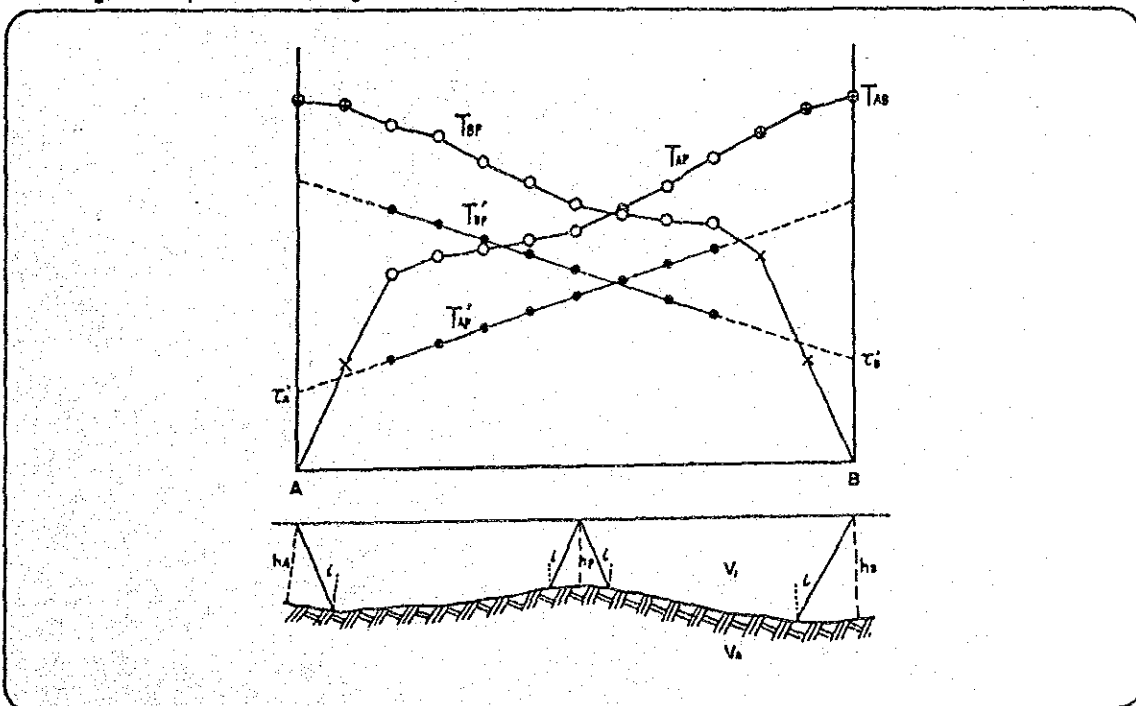
$$t_0 = T_{AP} + T_{BP} - T_{AB} \quad (a)$$

is called zero travel time. The quantities  $T_{AP}'$  and  $T_{BP}'$ , where

$$\left. \begin{aligned} T_{AP}' &= T_{AP} - t_0/2 = (T_{AP} - T_{BP} + T_{AB})/2 \\ T_{BP}' &= T_{BP} - t_0/2 = (T_{BP} - T_{AP} + T_{AB})/2 \end{aligned} \right\} (b)$$

are called velocity travel time (the black circles in the figure indicate velocity travel time). The curve that successively joins the velocity travel times determined for each receiving point is called the velocity travel time curve. Theoretically, this is a straight line, and its slope indicates velocity  $V_2$  of the lower layer. Velocity  $V_1$  of the upper layer is determined from the travel time of the direct wave mentioned above.

Fig. A Explanation of Hagiwara's analysis method



If we designate the length of a perpendicular line drawn from receiving point P to the surface of the lower layer (depth of the lower layer)  $h_P$ ,

$$h_P = \frac{V_1(T_{AP} + T_{BP} - T_{AB})}{2 \cos i} \quad (c)$$

where  $\sin i = V_1/V_2$ , meaning that  $h_P$  may be determined.

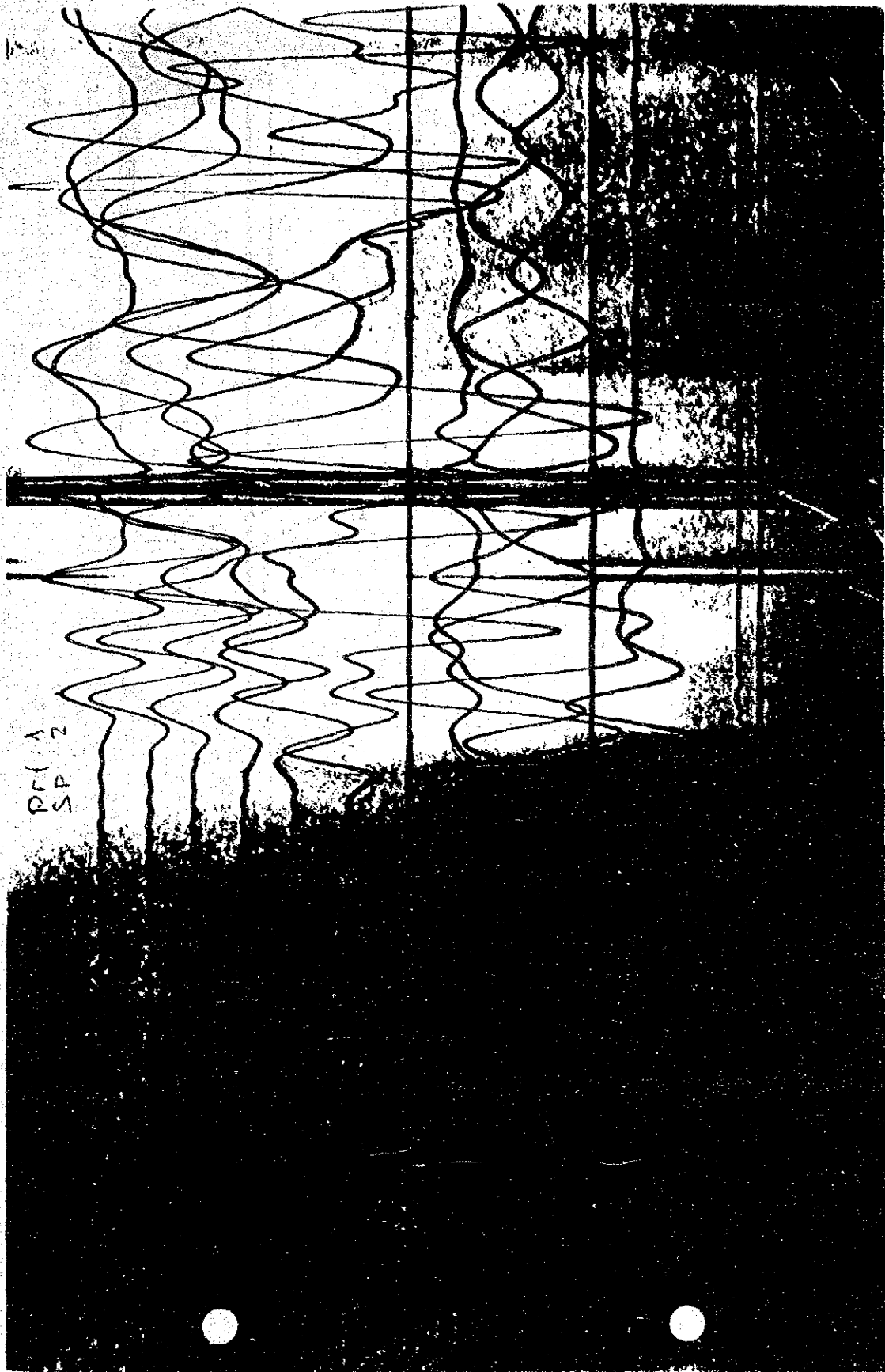
We have seen that where  $T_{AP}$  and  $T_{BP}$  are both known for the receiving point, depth of the lower layer can be determined using Formula (c). However, for the points marked  $\oplus$  in the figure, only one of the values,  $T_{AP}$  or  $T_{BP}$  is known. For these receiving points, Formula (b) is substituted into Formula (c), giving us:

$$\left. \begin{aligned} h_P &= \frac{V_1(T_{AP} - T_{AP}')}{\cos i} \\ h_P &= \frac{V_1(T_{BP} - T_{BP}')}{\cos i} \end{aligned} \right\}$$

Here, the values  $T_{AP}'$  or  $T_{BP}'$  extend the velocity travel time curve. The values at P read off from this extended curve may be used.

Also, if we designate the value of the point where velocity travel time curve  $T_{AP}'$  intersects the vertical axis at shot point A as  $\tau_A'$  and the point where  $T_{BP}'$  intersects the vertical axis at shot point B as  $\tau_B'$ , the following formulas are obtained:

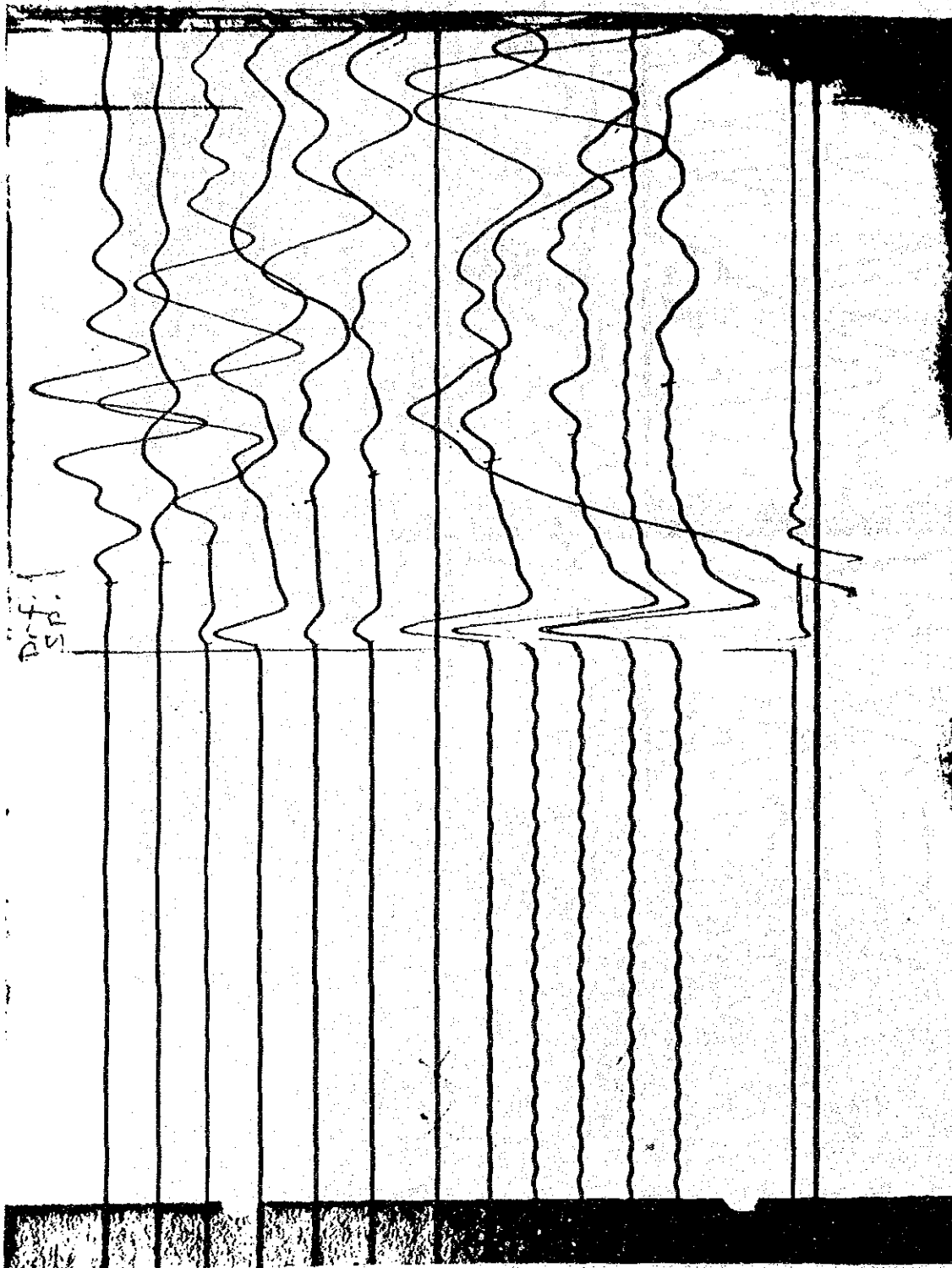
$$\left. \begin{aligned} h_A &= \frac{V_1 \tau_A'}{\cos i} \\ h_B &= \frac{V_1 \tau_B'}{\cos i} \end{aligned} \right\} \quad (d)$$



Seismic Prospecting Record

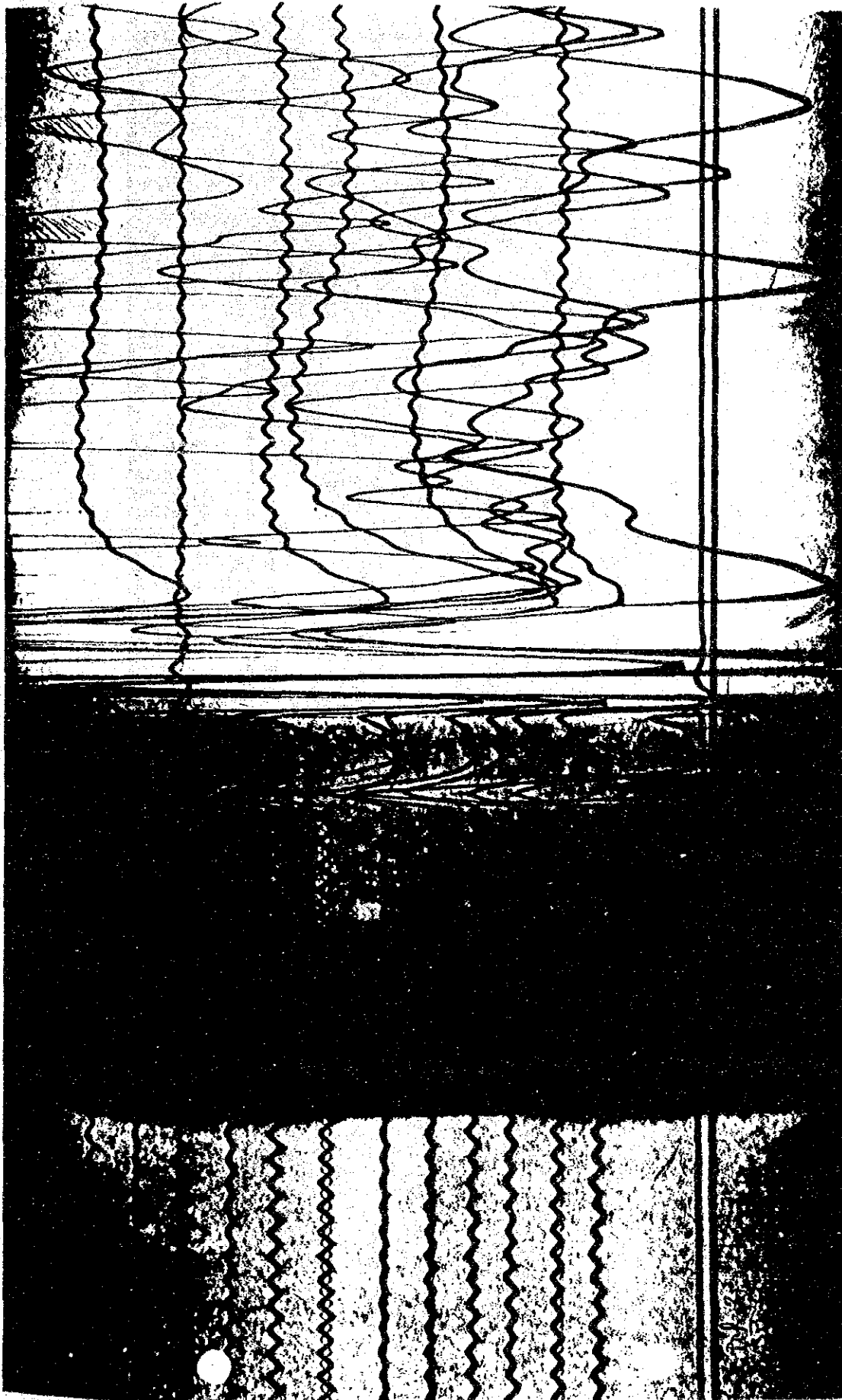
PS-1 Line Shot Point No. 1





Seismic Prospecting Record

PS-1 Line Shot Point No. II



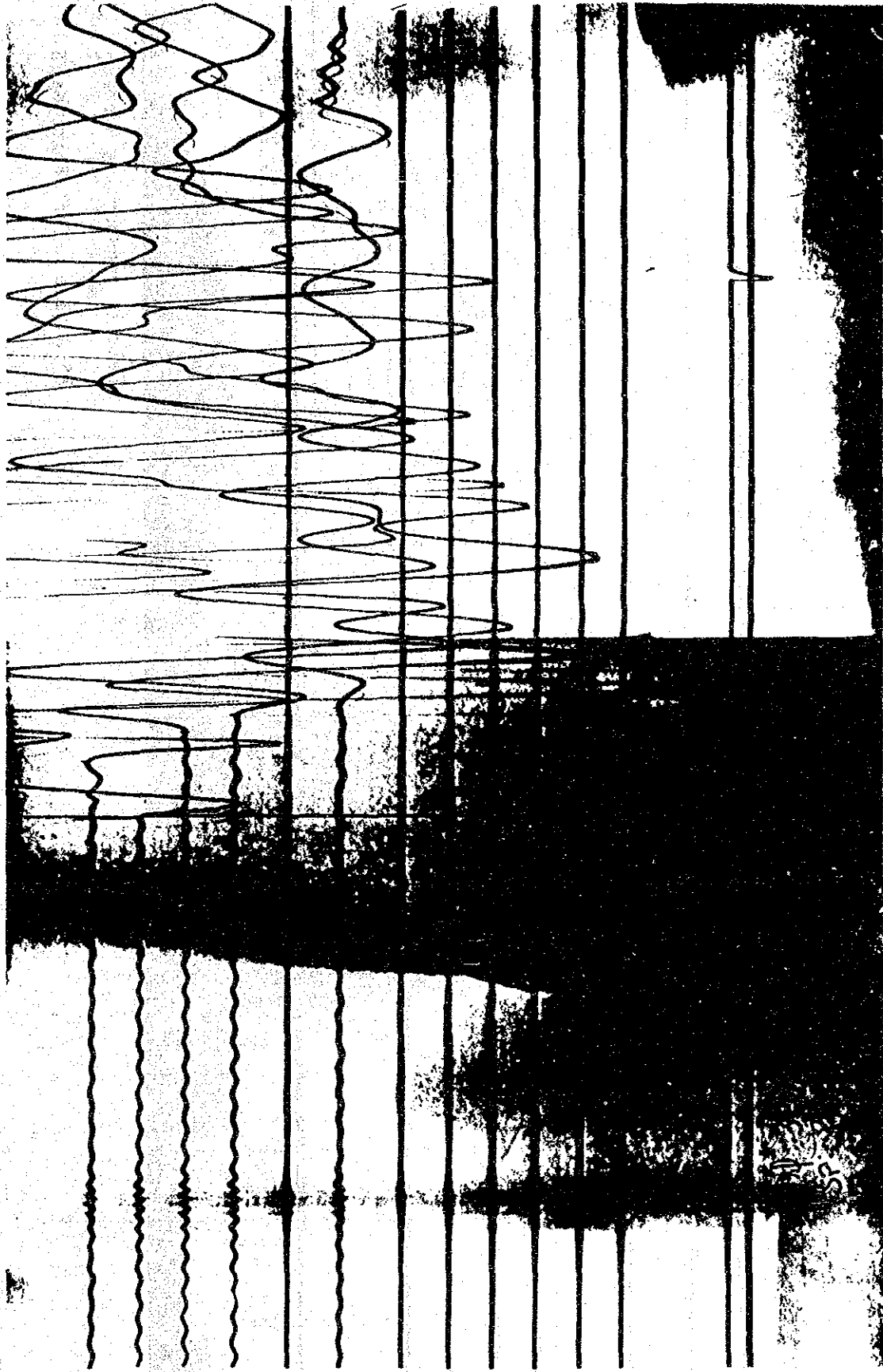
Seismic Prospecting Record

PS-2 Line Shot Point No. 1



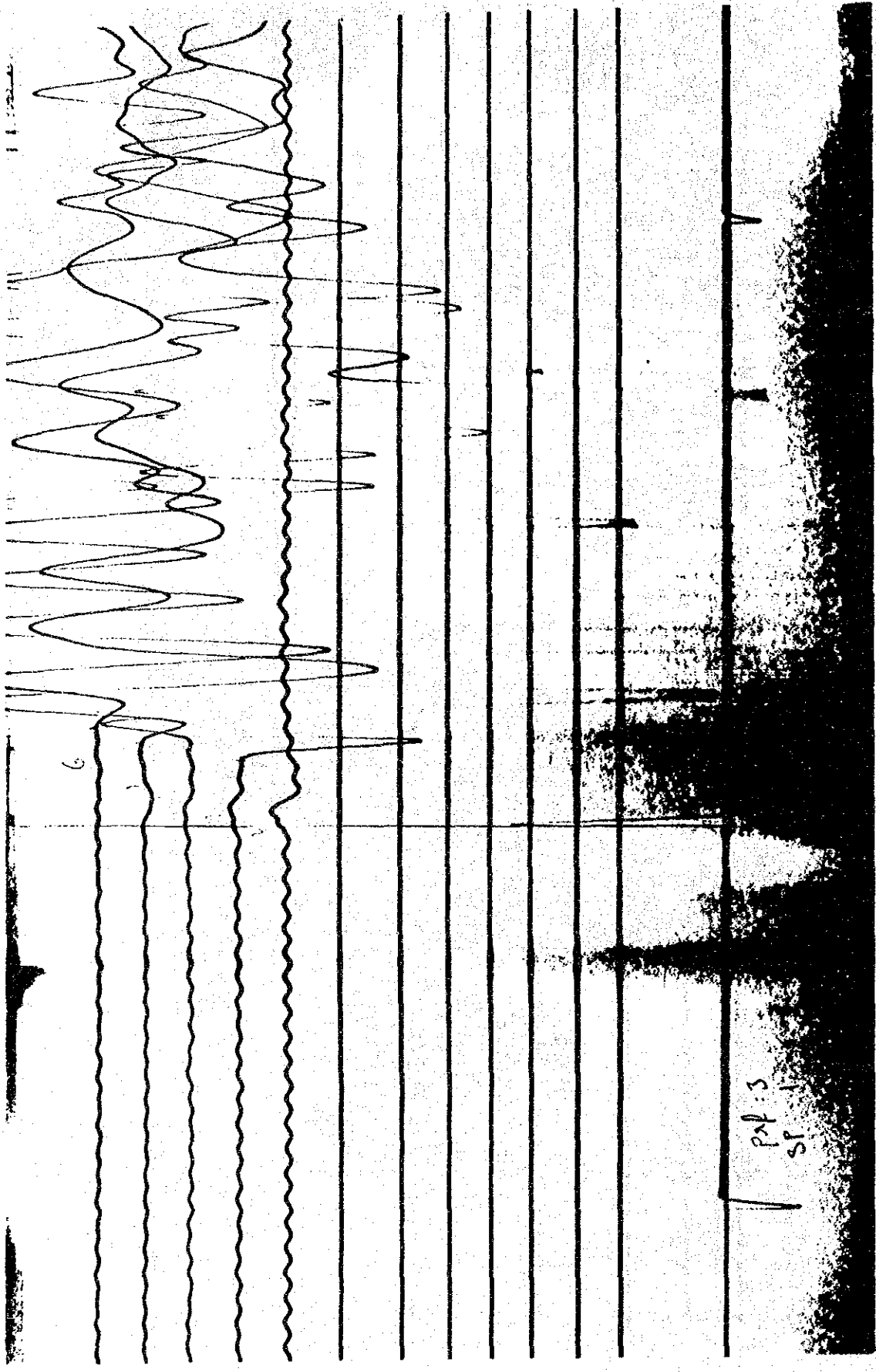
Seismic Prospecting Record

PS-2 Line Shot Point No. II

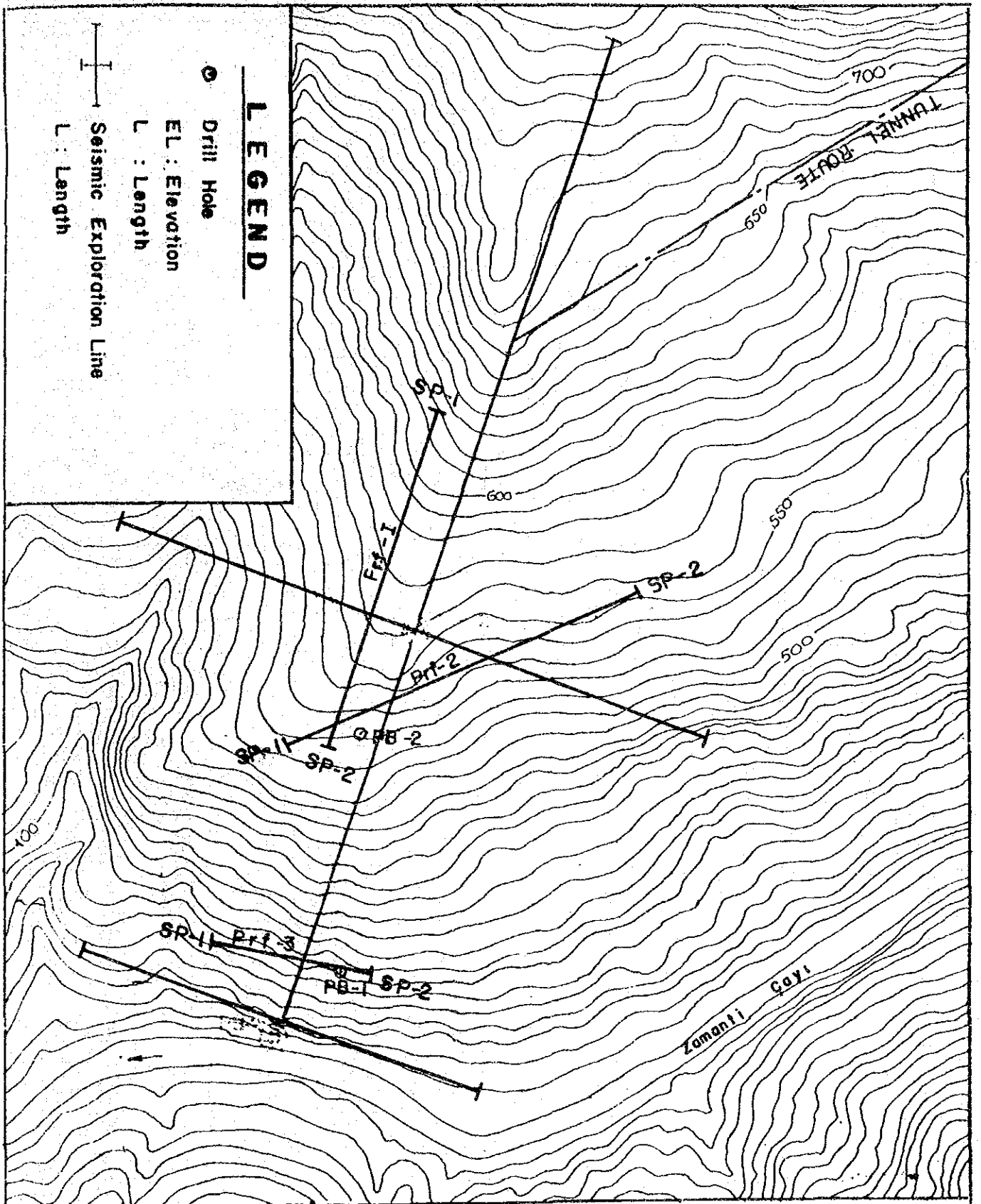


Seismic Prospecting Record

PS-3 Line Shot Point No. I

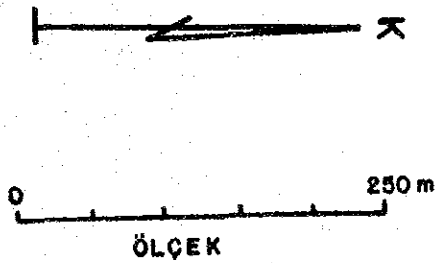


Seismic Prospecting Record  
PS-3 Line Shot Point No. II



DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ  
JEOTEKNİK HİZMETLER VE YERALTISULARI  
DAİRESİ BAŞKANLIĞI  
ANKARA

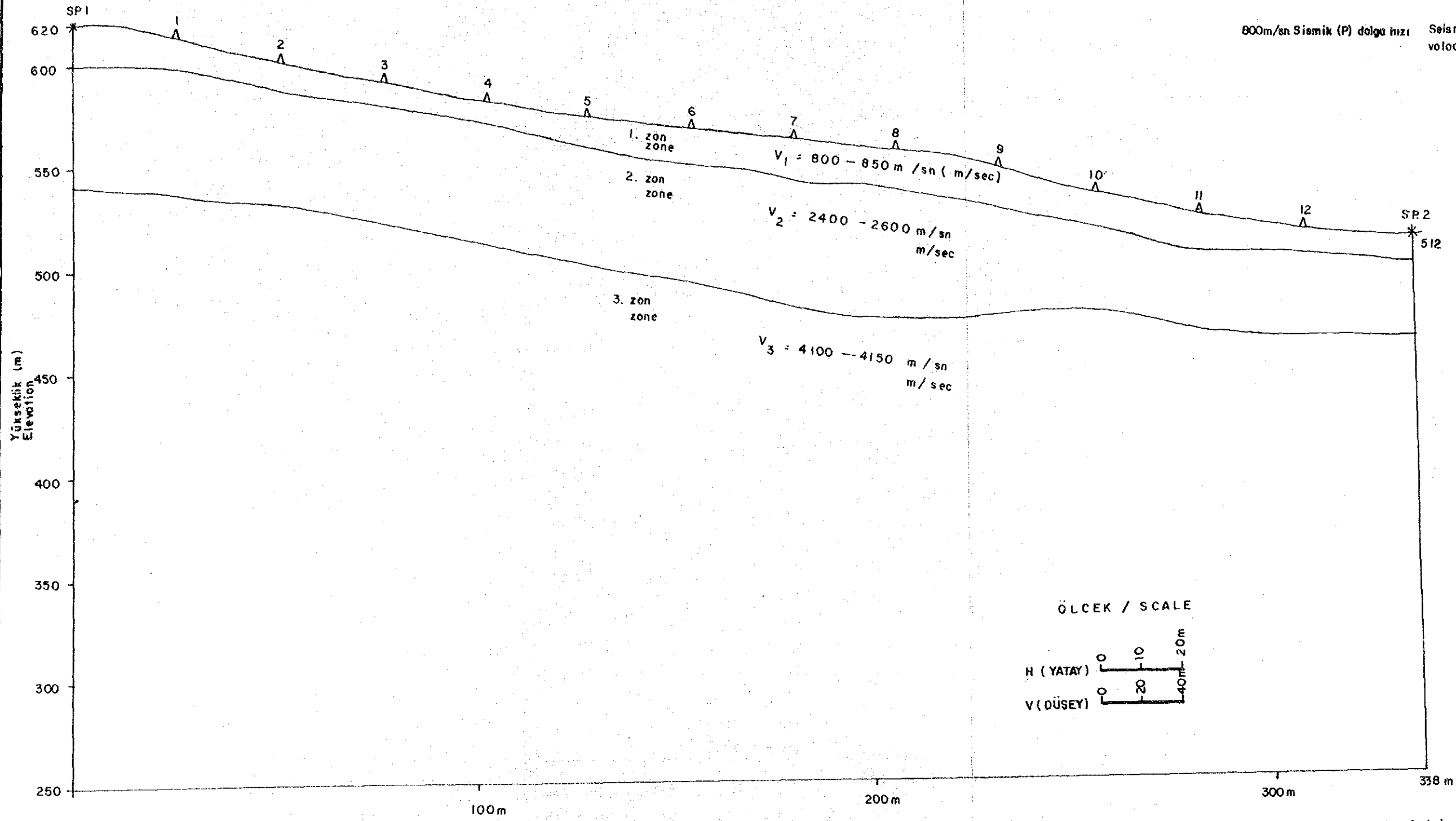
LOCATION OF GEOLOGICAL INVESTIGATION WORKS  
PENSTOCK AND POWERHOUSE-PLANE



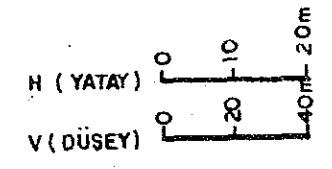
YAPAN : ÖL. UZUN	KONTROL :	TASDİK OLUNUR
ÇİZEN : SKBULUTİ	TASVİP :	
ÖLÇEK :	ARSİV. NO. :	Tarih / /



SİMGELER	LEGEND	
SP1	Dinamit patlatma noktası	Shot Point
*	Geofon noktası	Geophone number
Δ	800m/sn Sismik (P) dalgı hızı	Seismik (P) wave velocity (m/sec)



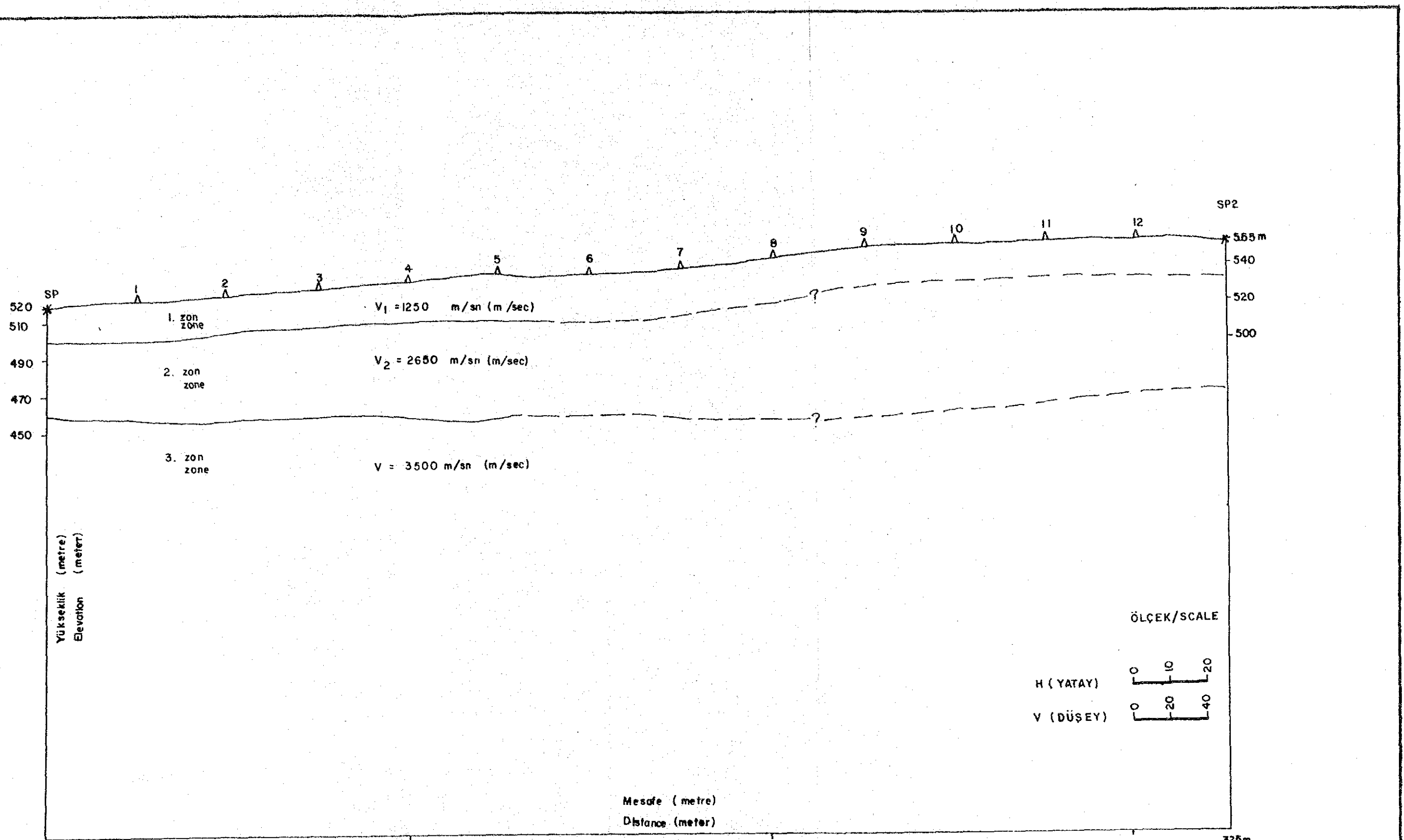
ÖLÇEK / SCALE



Sismik refraksiyon kesiti profili-1  
Seismic refraction cross section profile-1

C. KAPTAN  
Ö. L. UZUN

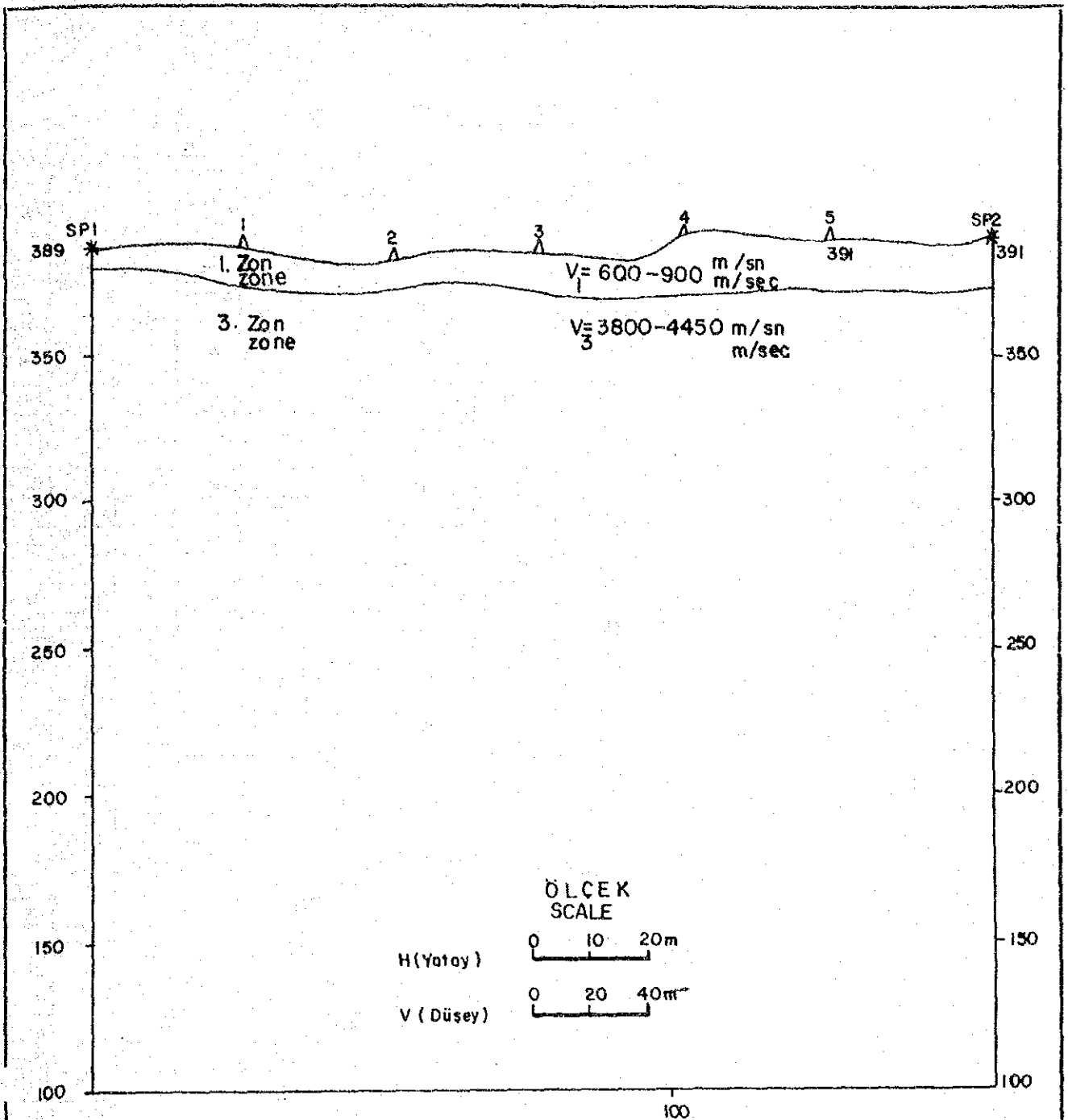




Sismik refraksiyon kesiti profil-2  
Seismic refraction cross section profile -2

C. KAPTAN  
Ö. L. UZUN





Sismik refraksiyon kesiti profil - 3  
Selsmic refraction cross section profile-3

Mesafe  
(Distance)

DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ  
JEOTEKNİK HİZMETLER VE YERALTISULARI  
DAİRESİ BAŞKANLIĞI  
ANKARA

GÖKTAŞ BARAJI  
SİSMİK REFRAKSİYON KESİTİ

YAPAN : C.KAPTAN	KONTROL :	TASDİK OLUNUR
ÇİZEN : N.TUĞ	TASVİP :	
ÖLÇEK :	ARŞİV NO. :	Tarih / /



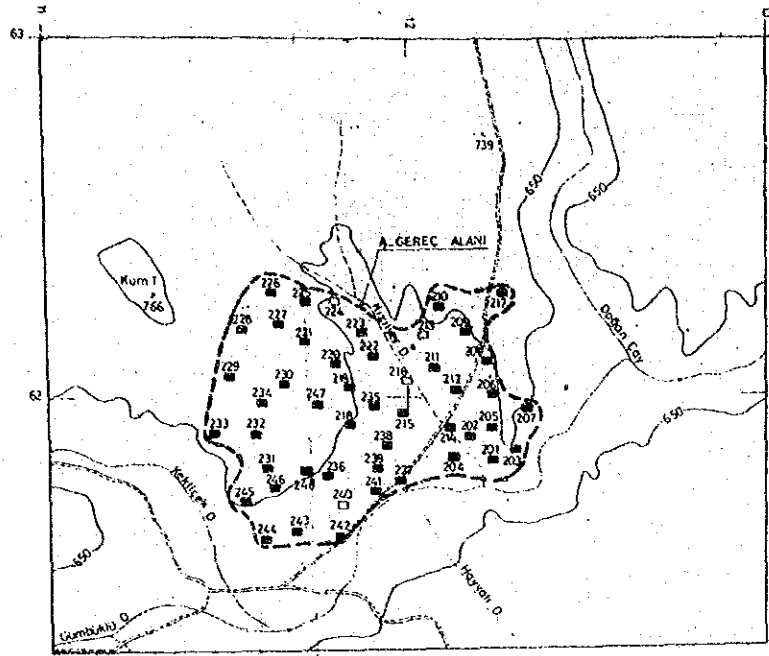
Result of Laboratory Test for Core Material(1/2)

Item	Grain size				Atterberg limits (%)			Soil classification	Specific gravity	(Optimum moisture content)					
										Compaction Test		Triaxial Shear strength		Coefficient of Permeability	
	Silt < clay under 0.075mm	Sand	gravel over 5mm	Max grain size (mm)	LL	PL	PI			Gs	ρd max (g/cm <sup>3</sup> )	wopt (%)	C (kg/cm <sup>2</sup> )		φ (°)
A-201	10	32	58	50	31.3	21.3	10.3	GP-GC	2.74	1.80	19.3	1.05	20	6.4 × 10 <sup>-7</sup>	
A-202	44.5	43.5	12	26	44.8	39.4	11.4	SH	2.69	1.58	19.5	-	-	-	
A-203	23	55	22	34	38.4	27.2	11.2	SH	2.65	1.60	10.2	-	-	-	
A-204	22	36.5	21.5	33	34.0	24.9	9.1	SH	2.76	1.75	17.0	-	-	-	
A-205	28.5	37.5	38	50	49.4	29.2	20.2	SH-SC	2.61	1.54	24.0	1.5	20	7.1 × 10 <sup>-7</sup>	
A-206	29	62	9	20	35.9	26.0	7.9	SH	2.68	1.64	20.3	-	-	-	
A-207	24	37	39	38	44.2	26.1	16.1	SH	2.66	1.67	20.1	-	-	-	
A-208	30	34	16	31	36.6	26.1	10.3	SH	2.69	1.66	20.0	1.3	20	4.5 × 10 <sup>-8</sup>	
A-209	24	59	13	34	-	-	-	SH	2.76	1.64	20.3	-	-	-	
A-210	34	53	10	31	37.4	28.0	11.4	SH	2.77	1.62	22.4	-	-	-	
A-211	32	55	13	31	33.1	25.5	7.6	SH	2.71	1.72	18.9	-	-	-	
A-212	48	46	6	19	40.6	28.3	12.3	SH	2.71	1.61	23.6	1.2	20	-	
A-214	28	60.5	13.5	38	-	-	-	SH	2.76	1.67	20.0	-	-	-	
A-215	64	33	3	30	52.4	28.7	23.7	CH-NH	2.73	1.57	23.0	2.0	13	6.6 × 10 <sup>-7</sup>	
A-217	19	58	24	34	32.3	25.3	7.0	SH	2.77	1.65	19.0	-	-	-	
A-218	22	30	48	56	55.5	26.6	26.9	GC	2.65	1.62	20.4	1.25	12	-	
A-219	57	39	4	19	45.7	30.0	15.1	NL	2.64	1.52	21.0	-	-	-	
A-220	40	46	14	32	41.5	30.4	11.1	SH	2.67	1.55	22.2	-	-	-	
A-221	54	25	21	50	58.0	39.9	18.1	NH	2.64	1.38	30.2	-	-	-	
A-222	60	19	1	19	51.4	30.4	20.8	NH	2.55	1.43	27.0	1.7	24	5.9 × 10 <sup>-7</sup>	
A-223	69	27	4	15	49.8	31.7	18.1	NL	2.70	1.43	25.0	1.7	21	3.3 × 10 <sup>-7</sup>	
A-225	46	54	-	5	46.0	24.2	20.8	SC	2.67	1.59	23.0	-	-	-	
A-226	57	43	-	13	43.6	28.1	19.7	CL-NL	2.59	1.6	21	1.15	24	-	
A-227	53	45	2	15	41.6	28.5	15.1	NL	2.64	1.57	22	-	-	-	
A-228	20	33	47	76	38.1	26.5	11.6	CH	2.71	1.62	20	-	-	-	
A-229	36	62.5	1.5	9.5	48.1	26.2	21.9	SC	2.67	1.61	23	1.18	18	2.4 × 10 <sup>-7</sup>	
A-230	53	26	21	38	49.2	24.9	24.3	CL	2.71	1.61	22	-	-	-	
A-231	59	21	21	19	51.9	35.6	26.3	NH	2.74	1.42	25	-	-	-	
A-232	38	34	28	38	45.3	29.0	16.3	CH-SH	2.71	1.50	27	-	-	-	
A-233	44	33.5	22.5	38	49.4	27.2	22.2	SC	2.68	1.66	13	2.0	16	-	
A-234	57	41	2	9.5	45.7	26.6	16.9	NL	2.67	1.53	23	-	-	-	
A-235	36.5	9.5	4	19	37.0	26.8	50.2	CH	2.71	1.46	25	-	-	-	
A-236	42	17	1	15	42.3	29.0	53.3	CH	2.66	1.34	28	1.55	10	4.2 × 10 <sup>-7</sup>	
A-237	68	26.5	5.5	38	37.2	23.9	13.3	CL-NL	2.64	1.65	19	-	-	-	

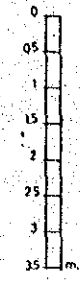
Result of Laboratory Test for Core Material(2/2)

Item	Grading				Atterberg limits (%)			Soil classification	Specific gravity	(Optimum moisture content)					
										Compaction Test		Triaxial Shear strength		Coefficient of Permeability	
	Slit clay under 0.075mm	Sand	Gravel over 5mm	Max grain size (mm)	LL	PL	PI			G <sub>s</sub> (1/m <sup>3</sup> )	ρ <sub>d</sub> max (g/cm <sup>3</sup> )	w <sub>opt</sub> (%)	C (ksc/cm <sup>2</sup> )		φ (°)
A-238	21	25	51	50	60.8	30.2	30.6	GC	2.75	1.63	21	-	-	-	
A-239	62	26	12	38	44.4	25.9	22.9	CL	2.63	1.64	18	2.35	19	2.1×10 <sup>-7</sup>	
A-241	41	52	1	9.5	34.8	19.7	15.1	SC	2.71	1.69	18	-	-	-	
A-242	47	44.5	8.5	19	51.0	29.2	21.8	SM	2.66	1.63	18	-	-	-	
A-243	42	32	26	38	32.2	17.3	14.9	SC	2.72	1.63	14	1.37	15	-	
A-244	65	28.3	6.3	28	42.0	24.1	17.9	CL	2.73	1.67	20	-	-	-	
A-245	16.5	42	41.5	50	-	-	-	SM	2.79	1.64	15	0.9	23	-	
A-246	53	37	10	19	51.3	30.6	20.7	MH	2.64	1.51	24	-	-	-	
A-247	29	43.5	27.5	38	49.0	24.0	25.0	SC	2.75	1.70	20	-	-	-	
A-248	67	12.5	20.5	38	75.2	30.1	45.1	CH	2.74	1.40	29	-	-	-	
B-301	56.5	31	12.5	38	58.2	30.8	27.4	MU	2.68	1.58	22.5	-	-	-	
B-302	38	51	11	38	45.4	25.1	20.3	SC	2.73	1.66	20.3	-	-	-	
B-303	53	30	17	38	61.3	29.9	31.4	CH-MH	2.73	1.49	25.8	-	-	-	
B-304	30	56	14	9.5	49.2	25.3	23.9	SC	2.71	1.63	19.0	-	-	-	
B-305	55	22	23	38	46.0	26.4	19.6	SC-SM	2.71	1.64	19.7	1.5	20	-	
B-306	44	39	17	38	60.0	29.8	30.2	CH	2.74	1.53	25.3	-	-	-	
B-307	65	13	22	38	80.8	29.5	51.3	CH	2.66	1.45	28.2	1.65	8	-	
B-308	54	36	10	19	38.3	23.2	15.1	CL	2.68	1.73	16.9	-	-	-	
B-309	51	11	38	76	78.4	30.6	48.0	CH	2.79	1.49	26.8	-	-	-	
B-310	43	52	5	9.5	56.0	36.7	25.3	SM	2.69	1.52	26.1	-	-	-	
B-311	29	24	47	53	64.4	34.1	30.3	CH	2.74	1.71	19.0	2.15	17	2.9×10 <sup>-7</sup>	
B-312	34	34	32	38	46.0	25.6	20.4	SC	2.73	1.60	22.3	-	-	-	
B-313	51	25	24	52	50.2	22.1	28.1	CH	2.73	1.61	22.4	-	-	-	
B-314	42	30	28	19	49.3	24.9	24.4	SC	2.70	1.71	18.0	-	-	-	
B-315	67	8	5	38	67.2	32.3	34.9	CH-MH	2.69	1.51	24.5	2.1	15	2.3×10 <sup>-7</sup>	
B-316	90.5	8.5	1	19	66.1	24.6	41.5	CH	2.70	1.52	23.2	-	-	-	
B-317	92	7	1	15	69.8	29.1	40.7	CH	2.59	1.56	20.6	2.1	17	-	
B-318	53.5	16.5	30	38	50.7	27.1	23.6	CH-MH	2.75	1.65	16.0	-	-	-	
B-319	36	44	20	19	49.6	26.3	23.3	SC	2.67	1.59	22.8	-	-	-	
B-320	52	38.5	9.5	19	56.7	30.2	26.5	CH-MH	2.69	1.52	25.4	-	-	-	
B-321	40	16	42	50	51.8	27.9	23.9	GC	2.68	1.59	23.0	-	-	-	

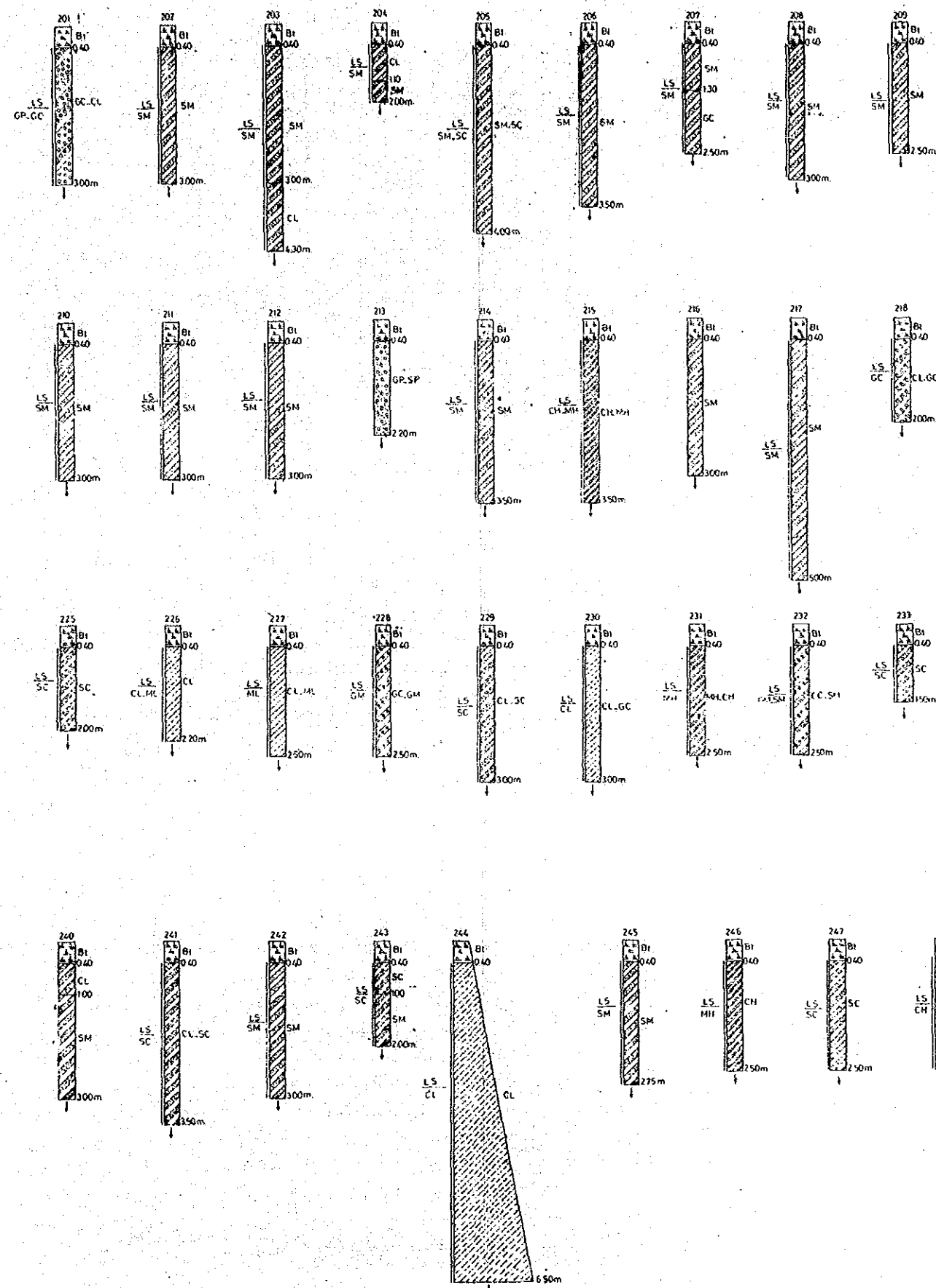
A - GEREÇ ALANI



KOZAN - M3A\_C3  
ÖLÇEK: 1/25000'ten 1/10000'lige papograf ile büyütülmüştür.

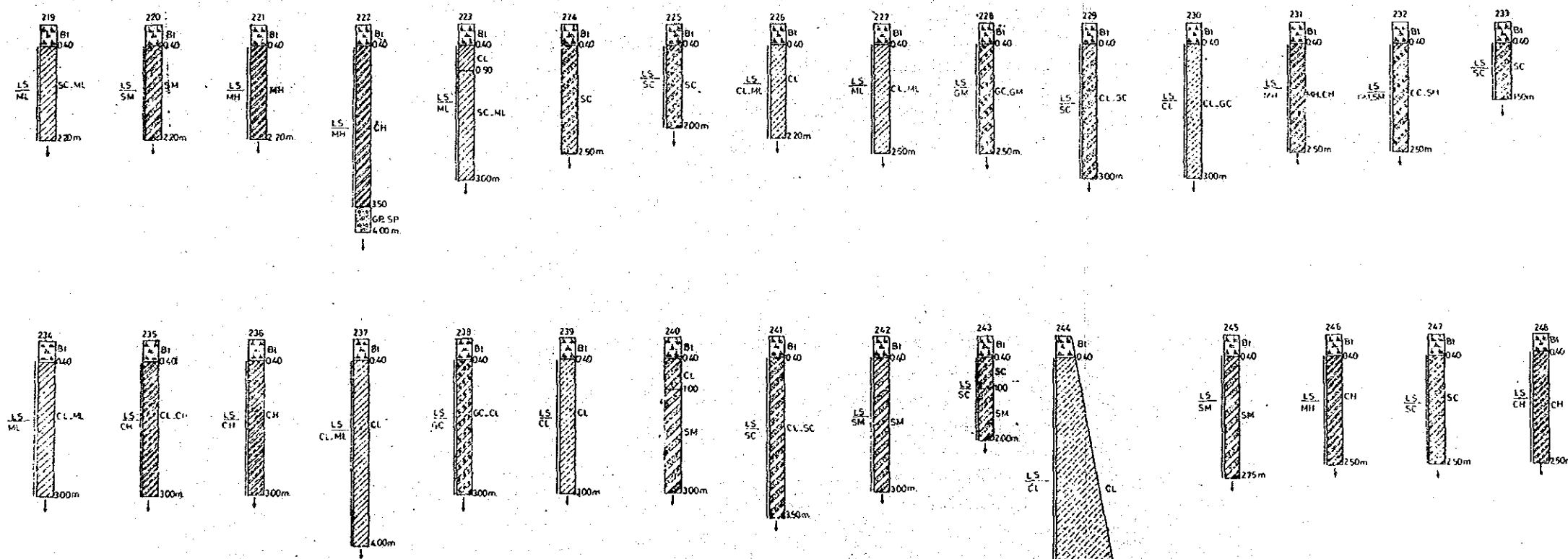


1/10000



SİMGELER

- Bi Bişetli toprak
- SM Siltli kum, kölü dereceli kum, silt karışımı
- GC Kili çakıl, kölü dereceli çakıl, kum, kıl karışımı
- ML İnorganik silt ve çok ince kum az plastik
- MH İnorganik silt, kumlu silt Elastik siltler
- SC Kili kum, kölü dereceli kum, kıl karışımı
- GM Siltli çakıl, kölü dereceli çakıl, kum, silt karışımı
- CL İnorganik kil, çakıllı, kumlu kil, siltli kil, az-orta plastik
- CH İnorganik kil, çok plastik (yağlı kiler)
- GP.GC Kölü dereceli çakıl, kili çakıl karışımı
- SM.SC Kili kum, siltli kum, kölü dereceli kum, silt-kil karışımı
- GP.SP Kölü dereceli çakıl, kum karışımı
- CL.MH Kili çakıl ile inorganik silt, kumlu silt, elastik siltler
- CL.ML İnorganik kil ile siltli, ince kumlu az-orta plastik, kil karışımı
- GM.SM Siltli çakıl ile kölü dereceli kum, silt karışımı
- Gereç araştırma kuyusu (örnek alınmamış)
- Gereç araştırma kuyusu (örnek alınmış)
- Gereç kuyusu kesiti (örnek alınmamış)
- Gereç kuyusu kesiti Gereç devam ediyor (örnek alınmış)
- Yarma kesiti (örnek alınmış)
- Gereç alanı sınırı



GEREÇ ALANLARI ÖZELLİKLERİNİ GÖSTERİR ÇİZELGE

A - GEREÇ ALANI	
YAPIYA İRAKLİĞİ (m)	29800 - 30000
YOL DURUMU	Var. Onarımına gerekiz.
ACILAN KUYU VE YARMA SAYISI	47 Kuyu, 1 Yarma
ORTALAMA SİYİRMİ (cm)	0.40
ÖNERİLEN KAZI DERİNLİĞİ (m)	3
GEREÇ NİCELİĞİ (m <sup>3</sup> )	1 x 10 <sup>6</sup>

DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ  
VI BÖLGE MÜDÜRLÜĞÜ  
PLANLAMA ŞUBE MÜDÜRLÜĞÜ  
A D A N A

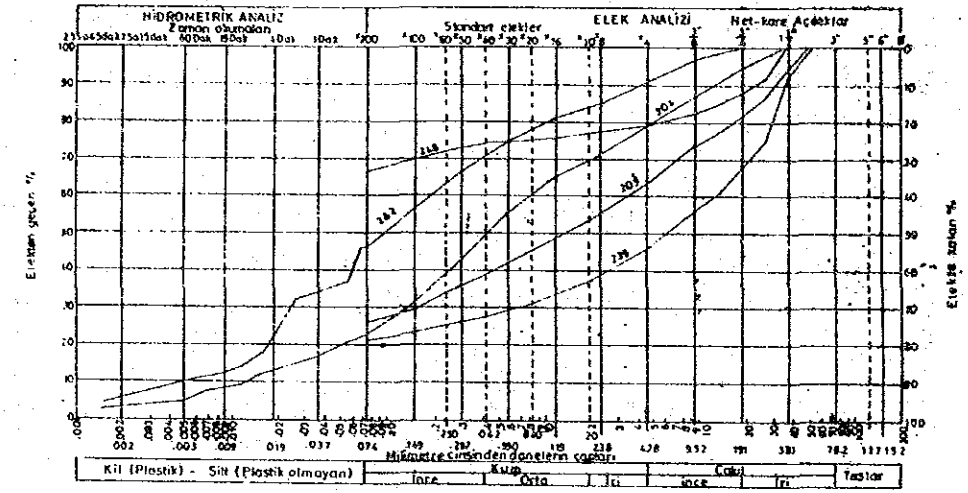
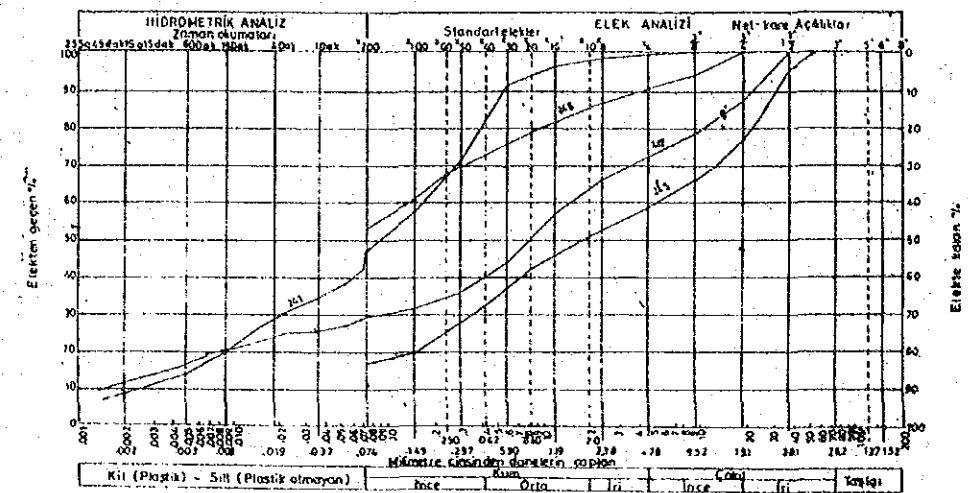
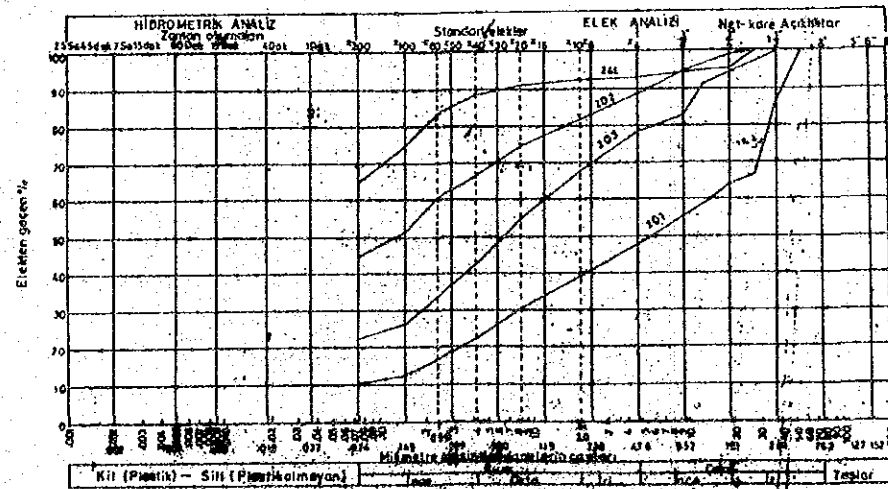
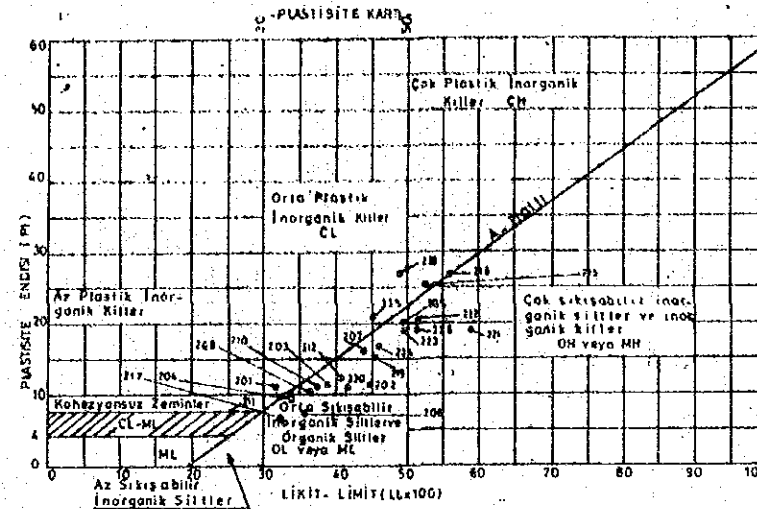
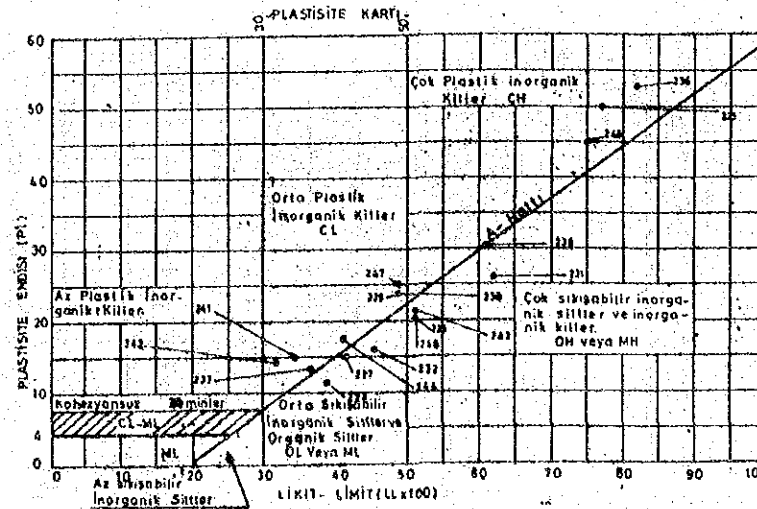
GÖKTAS BARAJI ve HES PROJESİ

**A - GEÇİRİMSİZ GEREÇ ALANI HARİTASI KUYU KESİTLERİ ve LABORATUVAR SONUÇLARI**

YAPAN Y. KARAOĞLU ARINDAN	DENETİM: CANNAN YORUK	
ÇİZEN: BEKİR S. UĞUR	ONAMA: HASAN METEL	
TARİH: 1988	PAFTA NO: 1	
ÖLÇEK: 1/10000	ARŞİV NO:	

GEÇİRİMSİZ GEREÇ TANIMLAMA VE MÜHENDİSLİK DENEY SONUÇLARI

Kuyu No	Kuyu Derinliği (m)	Özgül Ağırlık (g/cm <sup>3</sup> )	Sıkıştırma		Kıvam Limitleri			Dene Boyutları		Maksimum sıklıkta ve optimum su içeriğinde							Grup Sınıfı
			Y <sub>R</sub> max (%)	W <sub>opt</sub> (%)	LL	PL	PI	Kil Kum (%)	Cakıl (%)	Jamul (mm)	m. opt. (%)	0.075 mm'ye kadar	0.075 mm ile 0.425 mm arası	0.425 mm ile 0.850 mm arası	Geçirimsizlik Oranı (%)	Başka Ölçümler	
201	2762	1.880	19.30	31.0	21.3	10.5	10	32	5.0	1.80	19	1.05	20	6.4x10 <sup>-3</sup>	GP-GC		
202	2693	1.582	19.60	44.0	33.4	11.4	14.5	4.35	12	4.50	19				SH		
203	2659	1.804	10.23	38.4	27.2	11.2	2.9	5.5	2.2	1.90	10				SH		
204	2768	1.754	17.00	34.0	24.9	9.1	2.2	5.5	2.5	1.75	17				SH		
205	2615	1.540	26.00	49.5	29.2	20.2	26.5	37.5	2.4	1.54	24	1.6	20		SH-SC		
206	2664	1.680	20.30	35.9	28.0	7.9	2.9	6.2	9	1.80	20				SH		
207	2662	1.670	20.10	44.2	28.1	14.1	2.4	3.7	3.9	1.67	20				SH		
208	2608	1.660	20.00	34.6	26.1	10.5	3.0	5.4	1.4	1.66	20	1.3	20	4.5x10 <sup>-3</sup>	SH		
209	2764	1.681	20.50	X	X	X	2.8	5.9	1.3	1.68	20				SH		
210	2771	1.622	22.40	37.4	26.0	11.4	3.0	5.2	1.0	1.62	22				SH		
211	2717	1.722	18.90	33.1	25.5	7.6	3.2	5.5	1.3	1.72	18				SH		
212	2718	1.618	23.00	40.6	28.3	12.3	4.8	4.6	6	1.61	23	1.26	20		SH		
214	2768	1.670	20.00	X	X	X	2.6	60.5	13.5	1.67	20				SH		
215	2739	1.573	23.60	52.4	24.7	25.7	6.4	3.3	3	1.57	23	2.0	13	8.6x10 <sup>-3</sup>	CH-MH		
217	2774	1.850	15.00	32.3	25.3	7.0	1.0	5.8	2.4	1.85	15				SH		
218	2654	1.627	28.40	55.5	28.6	26.9	2.2	3.0	4.8	1.62	20	1.25	12		GC		
219	2642	1.520	27.00	45.1	30.0	15.1	5.7	3.9	4	1.52	27				ML		
220	2676	1.556	22.20	43.5	30.4	13.7	4.0	4.6	1.4	1.55	22				SH		
221	2642	1.383	30.20	58.0	39.9	18.1	5.4	2.5	2.1	1.38	30				MH		
222	2553	1.430	27.00	51.4	30.6	20.6	8.0	1.0	1	1.43	27	1.7	24	5.9x10 <sup>-3</sup>	MH		
223	2704	1.480	25.00	49.8	31.7	18.1	6.9	2.7	4	1.48	27	1.7	21	3.3x10 <sup>-3</sup>	ML		
225	2671	1.591	23.00	45.0	24.2	20.6	4.6	5.4	4	1.59	23				SC		
226	2598	1.680	21.00	45.8	28.1	10.7	5.7	4.3	-	1.68	21	1.15	24		CL-ML		
227	2649	1.570	22.60	41.6	26.5	15.1	5.3	4.5	2	1.57	22				ML		
228	2714	1.627	24.50	38.1	26.5	11.6	2.0	3.3	4.7	1.62	20				GM		
229	2675	1.616	23.20	48.1	26.2	21.9	3.8	6.25	1.8	1.61	23	1.10	18	2.1x10 <sup>-3</sup>	SC		
230	2717	1.612	22.20	49.2	24.9	24.3	5.3	2.6	2.1	1.61	22				CL		
231	2748	1.426	25.80	61.9	35.8	26.3	5.0	2.1	2.1	1.42	25				MA		
232	2719	1.503	27.50	45.3	29.0	18.3	3.0	3.4	2.8	1.50	27				GM-SH		
233	2807	1.665	18.00	49.4	27.2	22.7	4.4	33.5	22.5	1.66	18	2.0	16		SC		
234	2672	1.538	23.00	45.7	28.8	18.9	5.7	4.1	2	1.53	23				ML		
235	2713	1.466	25.60	77.0	26.8	50.2	8.5	6.3	4	1.46	25				CH		
236	2665	1.388	28.40	82.3	29.0	53.3	8.2	1.7	1	1.38	28	1.85	10	4.2x10 <sup>-3</sup>	CH		
237	2648	1.656	19.40	37.2	23.9	13.3	8.0	26.5	3.5	1.65	19				CL-ML		
238	2755	1.635	21.40	68.8	30.2	58.6	2.1	2.5	5.4	1.63	21				GC		
239	2688	1.644	18.70	48.0	25.9	22.9	6.2	2.6	1.2	1.64	18	2.35	19	2.1x10 <sup>-3</sup>	CL		
241	2717	1.692	18.00	76.0	19.2	15.1	4.7	3.2	3	1.69	18				SC		
242	2664	1.638	18.00	51.0	28.2	21.8	4.7	44.5	8.5	1.63	18				SH		
243	2718	1.836	16.80	32.2	17.3	14.9	4.2	3.2	2.6	1.83	16	1.37	15		SC		
244	2739	1.678	20.40	42.0	24.3	17.9	6.5	28.5	6.9	1.67	20				CL		
245	2787	1.849	15.50	X	X	X	16.5	4.2	41.5	1.84	15	0.9	23		SH		
246	2648	1.510	24.20	51.3	30.8	20.7	5.3	3.7	3.0	1.51	24				MH		
247	2753	1.780	20.30	49.8	24.0	25.0	8.0	43.5	27.5	1.78	20				GC		
248	2716	1.400	29.80	75.2	30.1	45.1	6.7	12.5	20.5	1.40	29				CH		

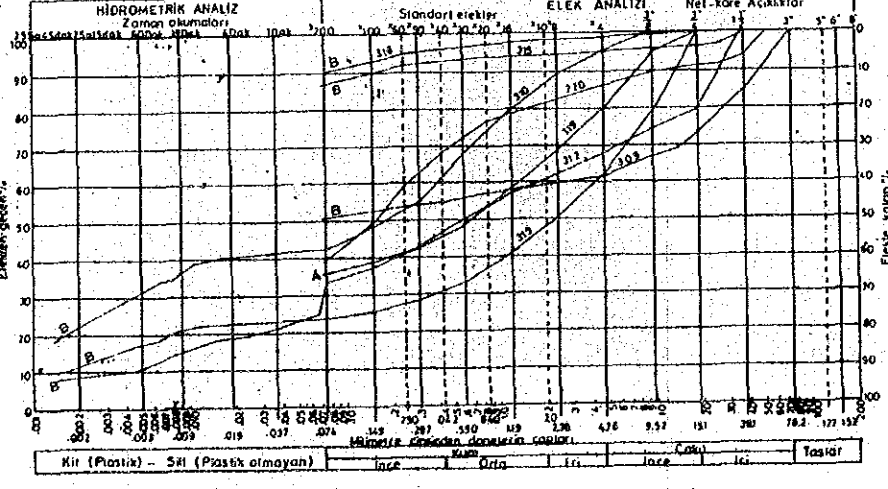
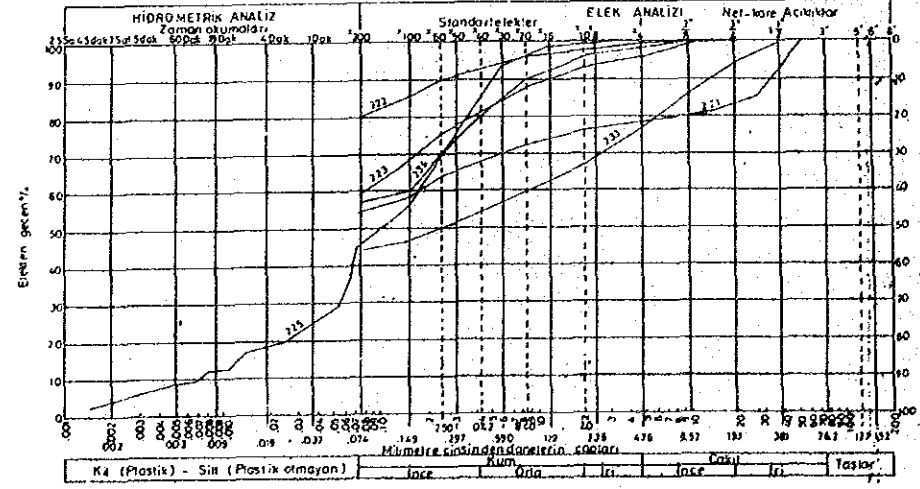
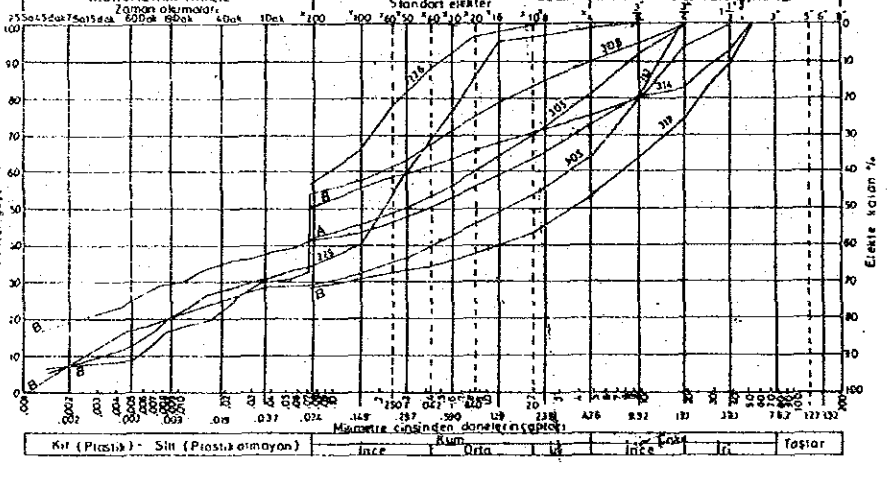
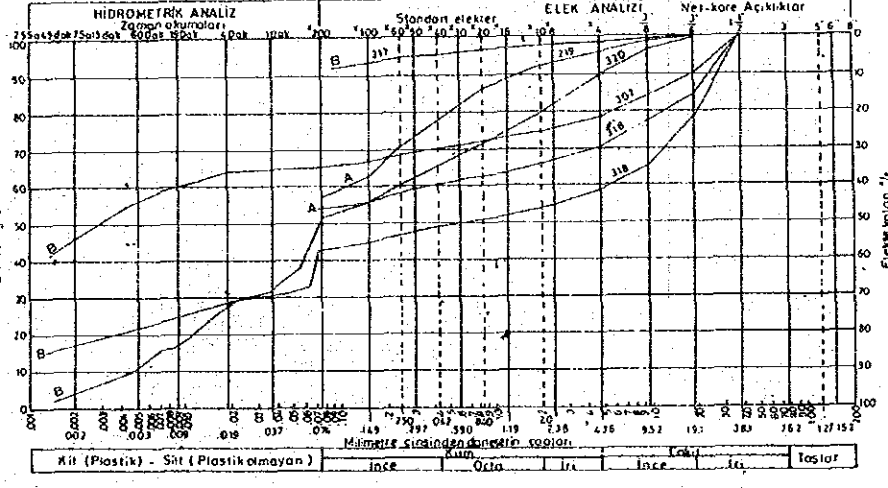
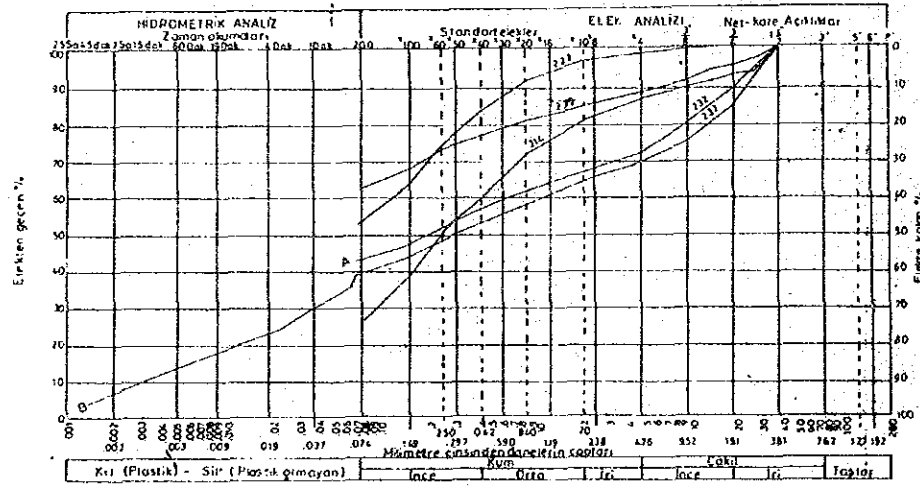
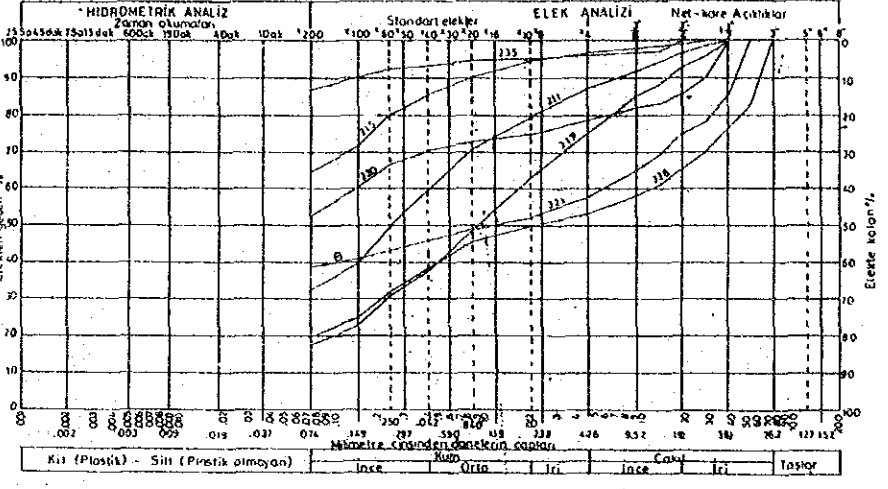
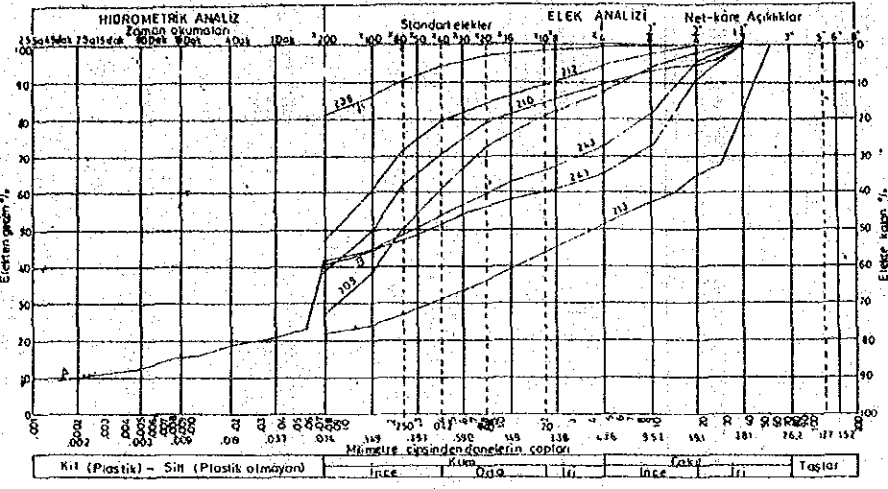
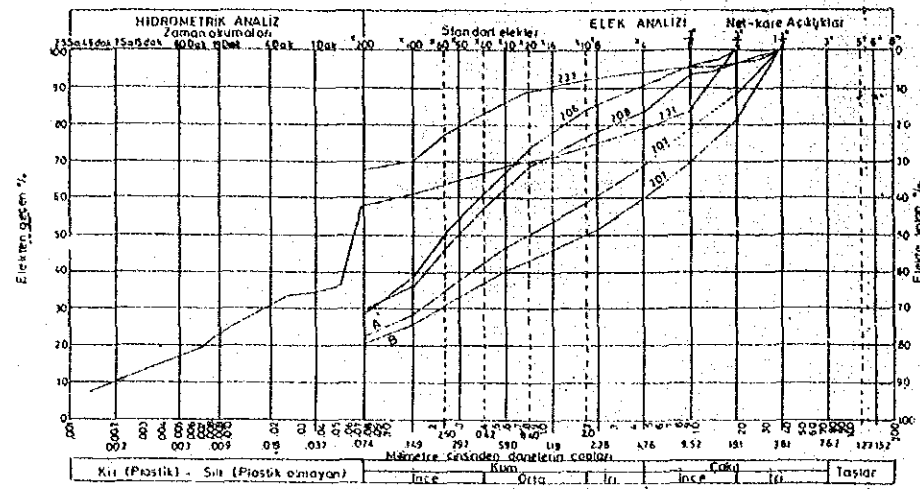


DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ  
VI. BÖLGE MÜDÜRLÜĞÜ  
PLANLAMA ŞUBE MÜDÜRLÜĞÜ  
A. B. İ. N. A.  
GÖRTAŞ BAĞAÇI ve HES. PROJESİ  
A-GEÇİRİMSİZ GEREÇ ALANI LABORATUVAR  
SONUÇLARI

YAPAN: KARAGÖZLER  
ÇİZEN: Emine TOKSOZ  
TARİH: 1588  
ÖLÇEK: ---

ONAY: [Signature]  
ONAY: [Signature]  
PAFİA NO: 7  
ARŞİN NO: ---





DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ  
VI. BÖLGE MÜDÜRLÜĞÜ  
PLANLAMA VE İZLENİM MÜDÜRLÜĞÜ  
B. O. A. N. A.

GÖRTAS BARAJI VE HES PROJESİ  
A-B GEÇİRİMSİZ GEREÇ ALANLARI  
ELEK ANALİZLERİ

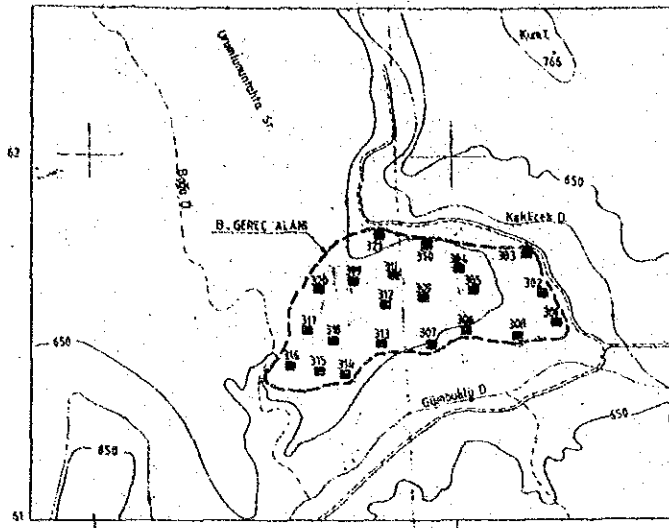
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İNŞAAT MÜHÜRLEME / ÇİZİM: M. HANCI / TARİH: 1968 / DİŞEK: -

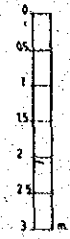
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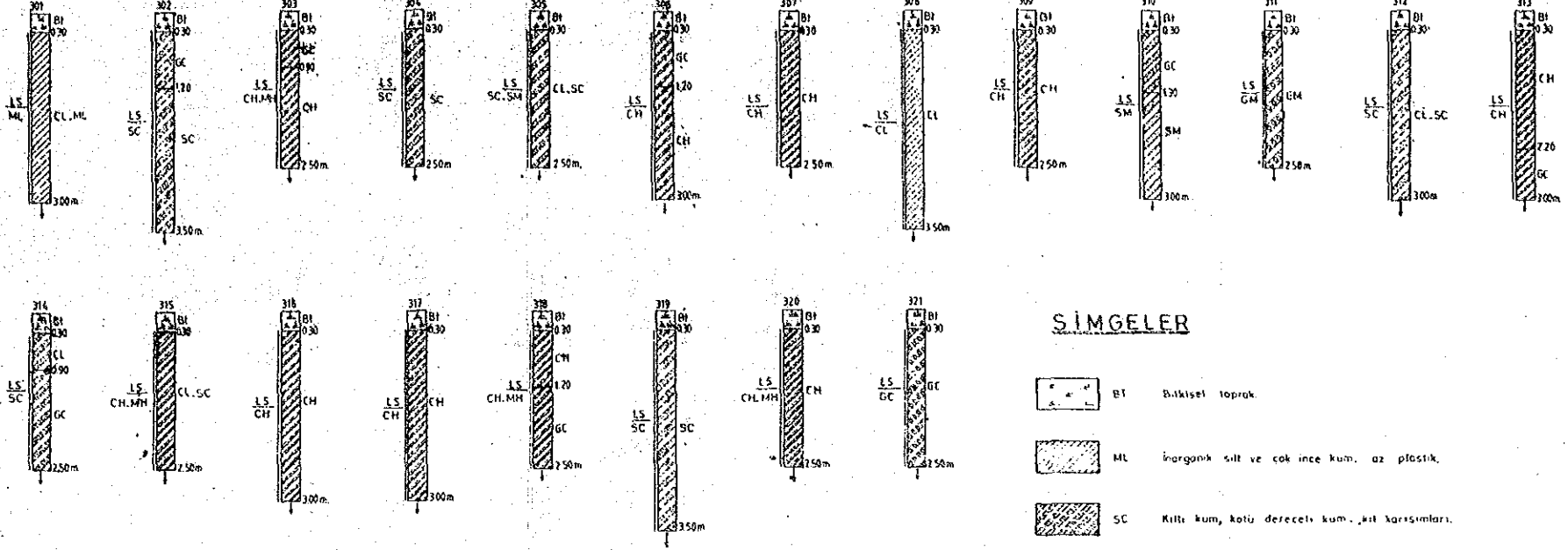
## B GEREC ALANI



ÖLÇEK: M 24 - C3  
1:25000'likten 1:10000'lüğe pantografya büyütülmüştür.



1/10000



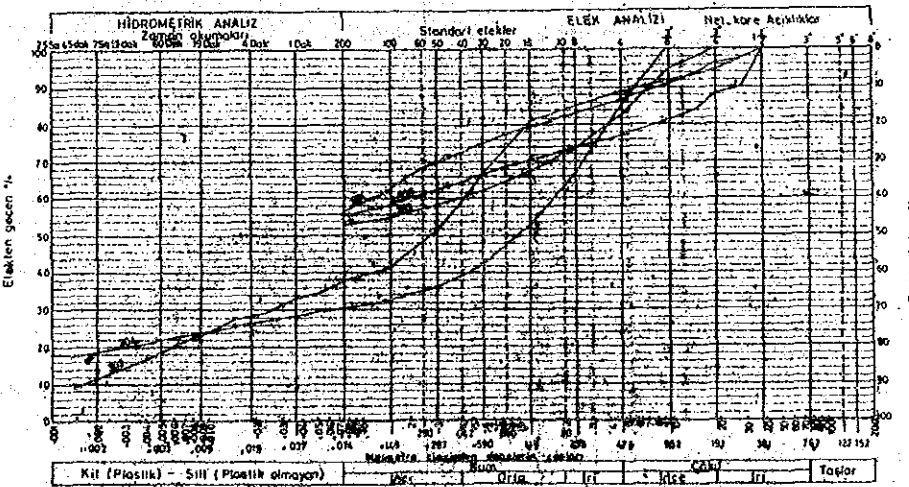
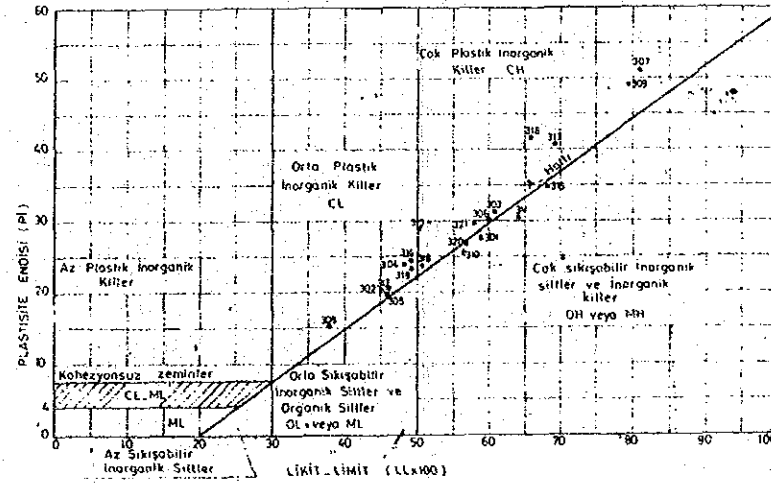
### SİMGELER

- BT Silty toprak
- ML İnorganik silt ve çok ince kum, az plastik
- SC Kaba kum, katı dereceli kum-silt karışımları
- CH İnorganik kil çok plastik (yağlı killler)
- CL İnorganik kil, çakıllı, kumlu kil, siltli kil, orta plastik
- SM Siltli kum, katı dereceli kum-silt karışımları
- GM Siltli çakıl, katı dereceli çakıl-kum-silt karışımları
- GC Kaba çakıl, katı dereceli çakıl-kum-silt karışımları
- CH, MH Kaba çakıl ile inorganik sil, kumlu silt, plastik siltler
- SC, SM Kaba kum, siltli kum, katı dereceli kum-silt karışımları
- Gerec araştırma kuyusu (örnek alınmamış)
- Gerec araştırma kuyusu (örnek alınmış)
- Gerec kuyusu kesili (örnek alınmış) - gerec devam ediyor
- Gerec kuyusu kesili (örnek alınmamış)
- Gerec alanı sınırı

### GEÇİRİMSİZ GEREC TANIMLAMA ve MÜHENDİSLİK DENEY SONUÇLARI

B Nüye No	Kuyu Derinliği (cm)	Sıkıştırma Örgütü (gr/cm <sup>3</sup> )	Sıkıştırma			Kıvam Limitleri			Dene Boyut Özellikleri			Maksimum sıklıkta ve optimum su içeriğinde			Grup Sembolü
			γ <sub>K</sub> max (gr/cm <sup>3</sup> )	w <sub>opt</sub> (%)	LL (%)	PL (%)	PI (%)	Kil (%)	Kum (%)	Çakıl (%)	Plastisite İndeksi (PI)	Dayanım (g/cm <sup>2</sup> )	Geçirimsizlik Oranı (%)	Bastırma Oranı (%)	
301	2685	1583	22.50	58.2	30.8	27.4	64.5	31	12.5	15.8	22			ML	
302	2724	1664	20.50	45.4	25.1	20.3	28	51	11	16.6	20			SC	
303	2726	1495	25.80	61.3	29.9	24.4	53	30	13	14.9	25			CH, MH	
304	2712	1635	19.00	49.2	25.3	23.9	30	56	14	16.3	19			SC	
305	2713	1647	19.70	45.0	26.4	19.6	55	22	23	16.4	19			SC, SM	
306	2744	1530	25.30	60.0	29.8	20.7	44	39	17	15.2	25			CH	
307	2662	1454	28.20	80.8	29.5	51.3	65	13	22	14.5	28	1.85	8	CH	
308	2694	1730	18.90	38.3	22.2	15.1	54	26	30	17.3	18			CL	
309	2791	1492	26.80	78.8	30.8	46.0	51	11	38	14.9	26			CH	
310	2697	1522	26.30	56.0	30.7	25.3	43	52	5	15.2	26			SM	
311	2745	1710	19.00	54.4	24.1	30.3	29	24	47	17.1	19	2.9x10 <sup>-7</sup>		GM	
312	2737	1504	22.30	46.0	25.6	20.4	34	34	32	16.0	22			SC	
313	2735	1690	22.40	50.2	22.1	28.1	51	25	24	16.1	22			CH	
314	2709	1715	18.00	49.2	24.9	24.4	42	30	28	17.1	18			SC	
315	2699	1510	24.50	62.2	32.3	34.9	87	8	5	15.1	24	2.1x10 <sup>-7</sup>		CH, MH	
316	2705	1528	23.20	66.1	24.6	43.3	92.5	8.5	1	15.2	23			CH	
317	2798	1565	20.60	69.8	29.1	40.7	92	7	1	15.6	20	24	17	CH	
318	2756	1667	16.00	50.7	22.1	23.8	53.5	16.5	30	16.6	16			CH, MH	
319	2673	1598	22.80	49.6	26.3	23.2	36	44	20	15.9	22			SC	
320	2697	1522	25.40	56.7	30.7	26.5	52	38.5	9.5	15	26			CH, MH	
321	2687	1590	23.00	57.8	27.9	23.5	40	18	42	15.9	23			GC	

### PLASTİSİTE KARTI



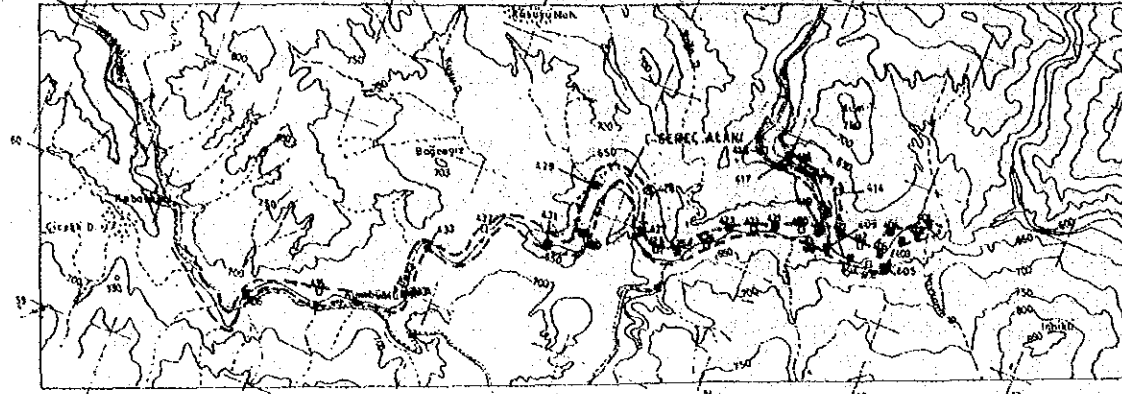
### B - GEREC ALANLARI ÖZELLİKLERİNİ GÖSTEREN ÇİZELGE

YAPMA İRANLIĞI (m)	5000-31000
YOL DURUMU	Yol çalışmaları gerektirir
AÇILAN KUYU ve YARMA SAYISI	21 kuyu
ORTALAMA SİYRİRA (cm)	0.49
DENİZDEN KAZI DERİNLİĞİ (m)	25
GEREC MİCELİĞİ (m <sup>2</sup> )	15x10 <sup>7</sup>

DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ VI BÖLGE MÜDÜRLÜĞÜ PLANLAMA SUBE MÜDÜRLÜĞÜ A D A N A	
GÖKTAS BARAJI ve HES PROJESİ	
B - GEÇİRİMSİZ GEREC ALANI HARİTASI KUYU KESİTLERİ ve LABORATUVAR SONUÇLARI	
MÜHÜR YERİ KAYDEDİLMİŞTİR	DEĞİTİM YERİ
ÇİZEN: BEKİR SİTİ LÖLÜR	ONAYLA: HAYRİ
TARİH: 1988	PAPYA NO: 4
ÖLÇEK: 1/10000 ve 1/5000	ARŞİV NO:

Ç-GEREÇ ALANI

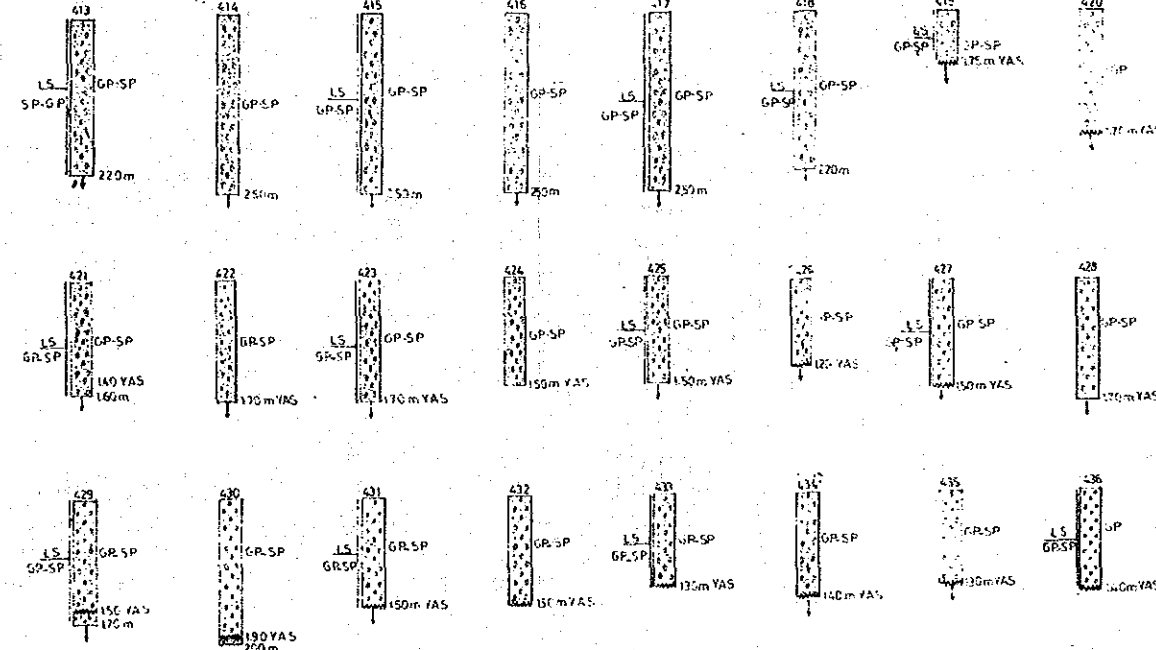
NOZAN M34-C4-C3



ÖLÇEK: 1/25000

GEÇİRİMLİ GEREÇ FİZİKİ DENEY SONUÇLARI

Örnek No	Birim Ağırlık (gr/cm <sup>3</sup> )		Özgül Ağırlık (gr/cm <sup>3</sup> )		200 Mesh elekten geçen (%)		Kil topaktarı (%)		N <sub>2</sub> O <sub>2</sub> dan kayıp (%)		Los Angeles Sınıma kayıp (%)	
	Kum	Cakil	Kum	Cakil	Kum	Cakil	Kum	Cakil	Kum	Cakil	100 mesh	200 mesh
401	1840	1748	2,558	2,728	50,4	11,3	320	0,31	44,8	4,3	5,22	28,6
402	1853	1745	2,547	2,668	59,9	0,92	571	0,76	45,9	31,9	5,80	26,20
403	1705	1800	2,523	2,656	8,13	1,58	798	1,25	41,4	42,9	7,20	26,6
404	1831		2,558	2,679	7,20	0,55	711	0,47	39,4	31,4	7,20	29,50
405	1786	1856	2,584	2,678	7,50	0,81	599	0,51	42,9	33,5	5,00	28,00
406	1753		2,555	2,570	7,33	1,52	644	1,11	40,2	37,4		
407	1761	1870	2,526	2,637	7,07	1,26	552	0,68	45,3	46,3	5,30	28,2
408	1774		2,525	2,525	7,06	1,60	598	1,46	44,7	44,4		
411	1855	1807	2,579	2,671	8,87	1,15	555	0,90	48,6	42,5	5,76	24,14
413	1808	1782	2,579	2,579	7,14	0,70	565	0,22	41,6	32,1	5,9	25,2
415	1855		2,544	2,735	6,50	1,25	508	0,82	42,3	39,6	7,50	29,80
417	1839	1832	2,546	2,798	6,10	0,83	442	0,12	44,0	41,6	6,24	27,9
418	1825	1829	2,575	2,713	6,83	1,72	642	1,47	38,5	28,4	6,00	23,90
419	1754	1756	2,551	2,551	7,25	1,38	531	0,71	47,1	32,5	8,80	31,2
421	1749	1844	2,585	2,685	10,37	0,78	27	0,62	12,1	37,2	6,30	25,4
422	1777	1719	2,529	2,665	5,64	0,90	536	0,86	52,1	38,9	7,32	36,5
425	1874	1715	2,557	2,714	4,95	0,58	379	0,23	38,3	40,4		
427	1845		2,594	2,691	4,20	1,50	596	0,66	21,4	30,9	6,00	22,80
429	1852	1823	2,609	2,709	10,5	1,25	979	1,04	39,2	25,2	5,70	25,5
431	1829	1865	2,589	2,721	9,21	0,96	492	0,54	44,9	25,1	7,14	27,4
433	1751	1900	2,598	2,657	9,09	0,88	903	0,21	45,5	24,3	9,4	26,0
436	1781	1828	2,584	2,759	9,11	0,26	843	0,20	47,1	33,6	6,50	21,90

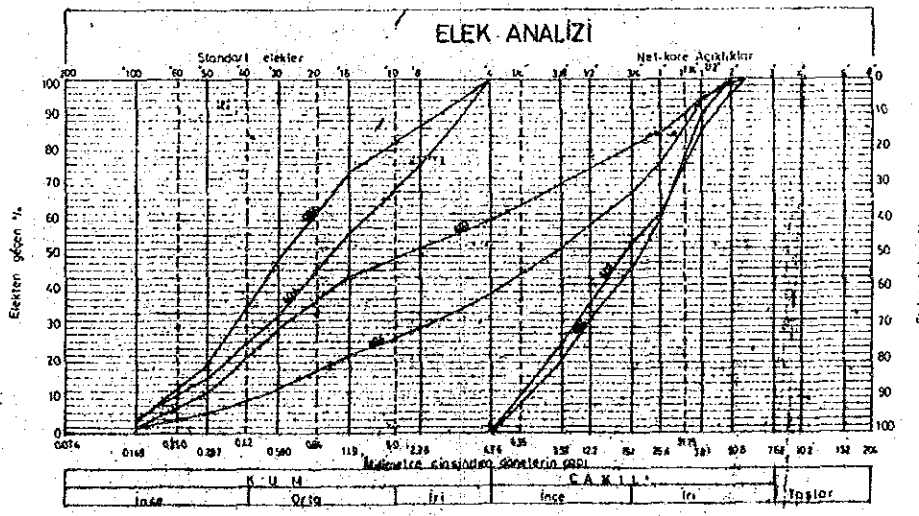
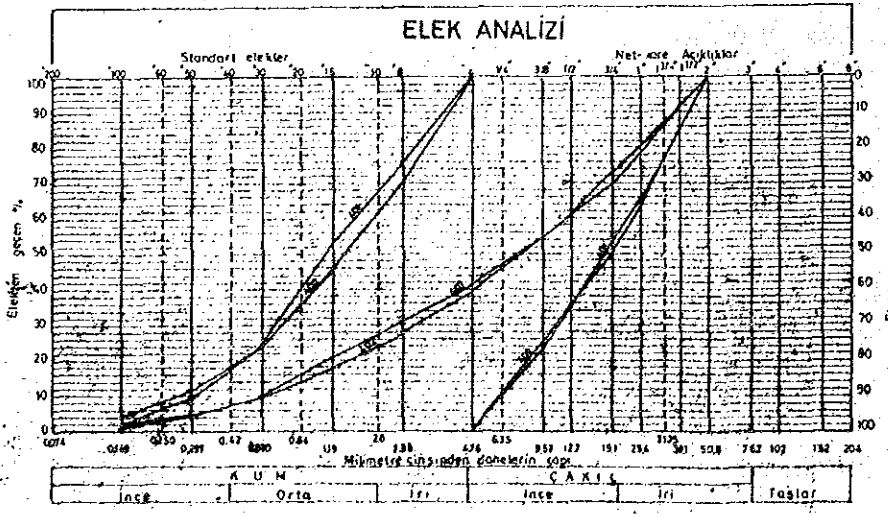


SİMGELER

- GP-SP Kötü dereceli çakıl-kum karışımları.
- SP-GP Kötü dereceli kum-çakıl karışımları.
- SP Kötü derecelenmiş kumlar, faklı faklı fraksiyonları az veya hiç olmayan gereçler.
- GP Kötü derecelenmiş çakıl-kum çakıl karışımları, ince daneleri az veya hiç olmayan gereçler.
- YAS Yatay su düzeyi: Gerek değeri ediyor.
- Gerek alanı sınırı.

GEREÇ ALANLARI ÖZELLİKLERİNİ GÖSTERİR ÇİZELGE

A-GEREÇ ALANI	
YAPILMA İRAKLİĞİ (m)	30000 - 37000
YOL DURUMU	Yol. Onarılmaması gerekir.
AÇILAN KUYU VE YARMA SAYISI	36 Kuyu
ORTALAMA SIVIRMA (cm)	—
İNCELEME KAZI DERİNLİĞİ	3
GEREÇ NİCELİĞİ (A)	2-10



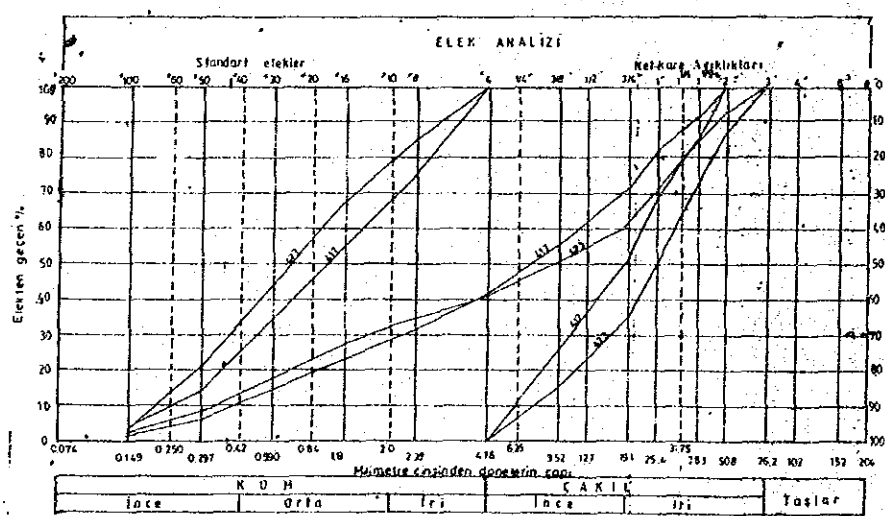
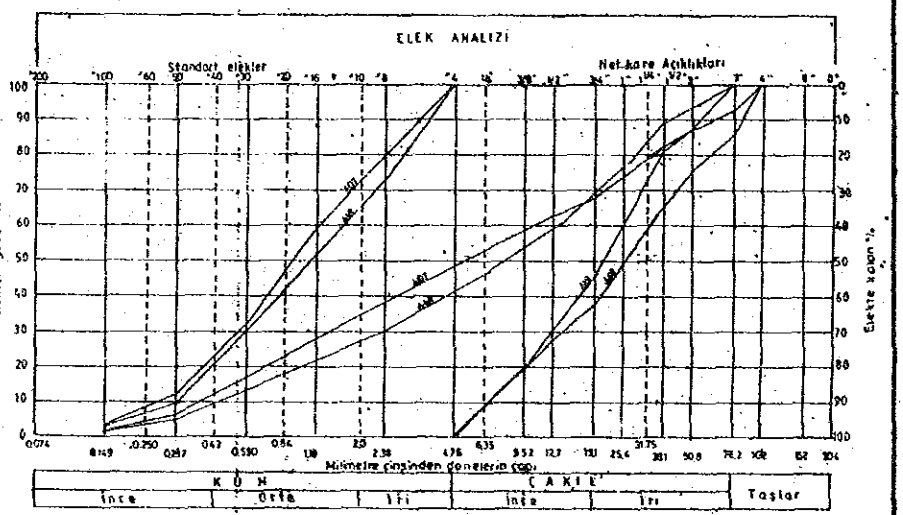
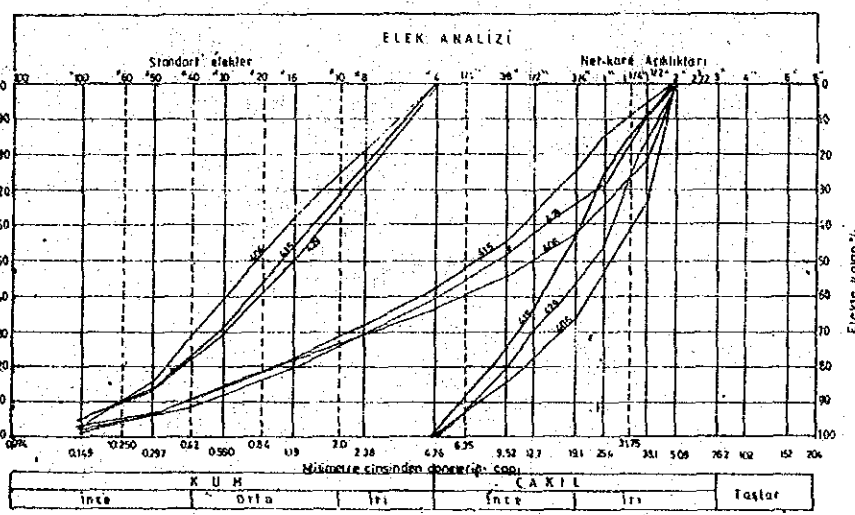
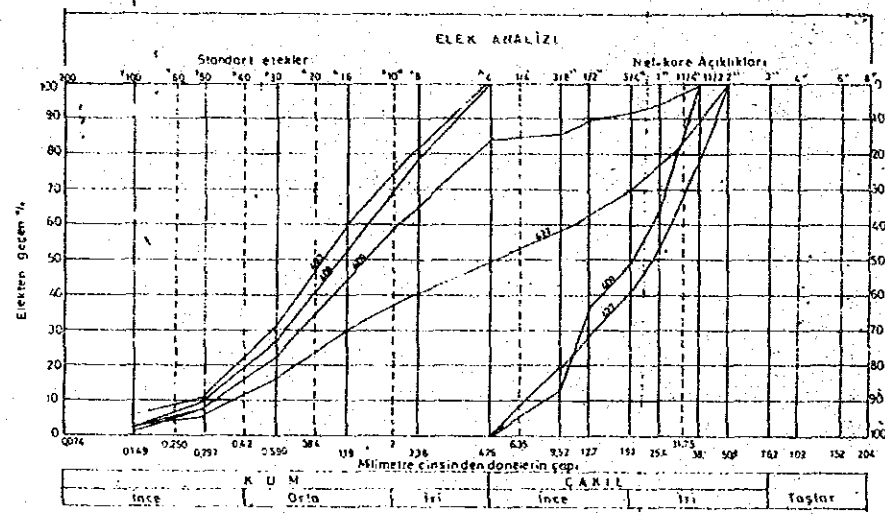
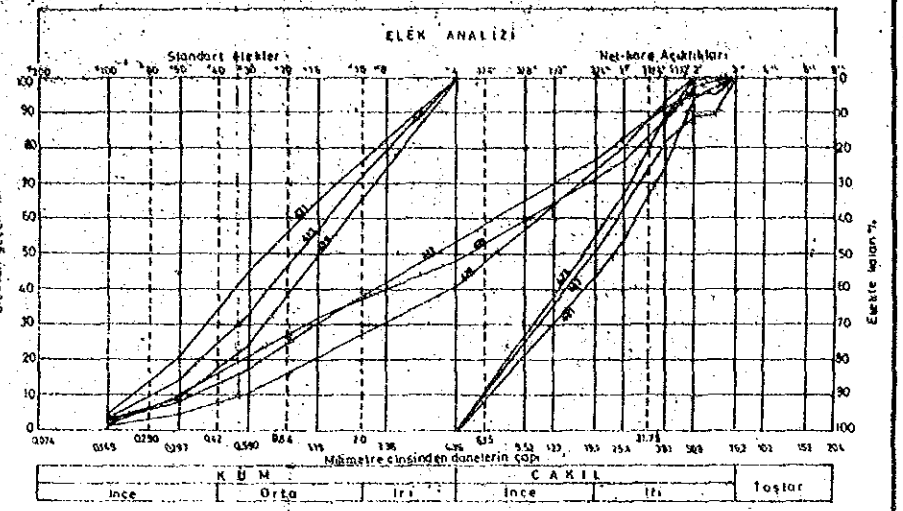
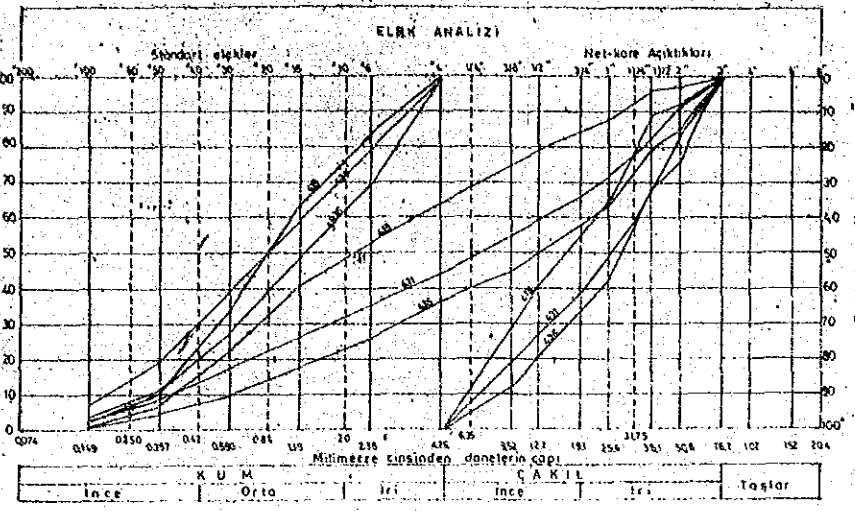
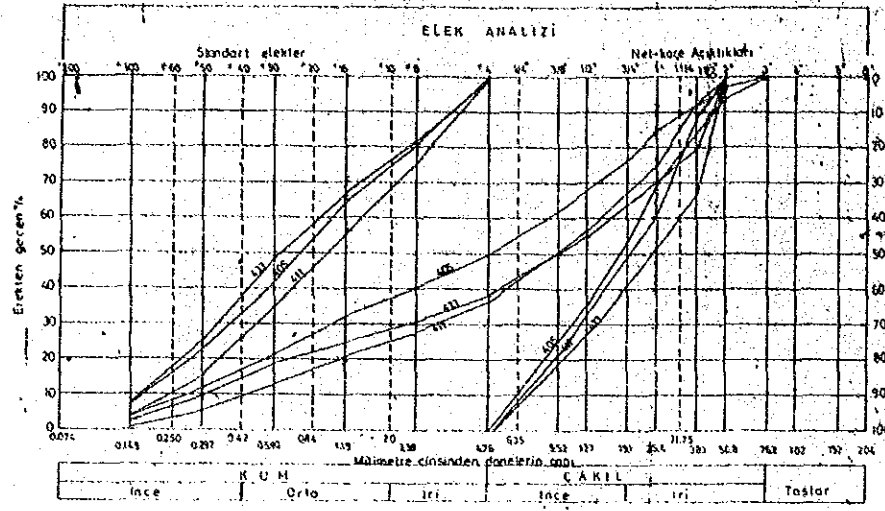
DEVLET SU İŞLERİ GENEL MÜDÜRLÜĞÜ  
VI. BÖLGE MÜDÜRLÜĞÜ  
PLANAŞA ŞUBE MÜDÜRLÜĞÜ  
A. O. A. N. A.

GÖKTAŞ BARAJI VE HES PROJESİ  
Ç-GEÇİRİMLİ VE AĞREGA GERECİ ALANI  
HARİTASI.

KUYU KESİTLERİ VE LABORATUVAR SONUÇLARI

YAPANI: Karadutlu İnşaat DENETİMİ: CABBAR YÖRÜK  
ÇİZENİ: DEĞİRMENÇİOĞLU ONAMA: HASAN İMRAK

TARİH: 1988 PAPA NO: 3  
ÖLÇEK: 1/25000 ARSİY NO:



DEVLET SU İŞLER GENEL MÜDÜRLÜĞÜ  
VI. BÖLGE MÜDÜRLÜĞÜ  
PLANLAMA ŞUBE MÜDÜRLÜĞÜ  
A.D.A.M.A.

**GÖRTAŞ BARAJI VE HES PROJESİ  
C-GEÇİRİMLİ GEREÇ ALANI  
LABARATUVAR SONUÇLARI**

YAPAN: Y. KARA	DEĞERLENDİREN: DEMETİM CEMAL YARDEM	ORJEN: [Stamp]
ÇİZEN: M. DEĞİRHENÇİOĞLU	OKUYAN: HASAN HESRİ	
TARİH: 1988	PASTA NO: 6	
ÖLÇEK:	ARŞİV NO:	



## **A-4 TRANSMISSION LINE PLAN AND SYSTEM ANALYSIS**



#### A-4 TRANSMISSION LINE PLAN AND SYSTEM ANALYSIS

Fig. A-4-1 (1) - (5) Load Flow Study

Fig. A-4-2 (1) - (3) Short Circuit Study

Fig. A-4-3 (1) - (9) Stability Study





Fig. A-4-1 (1) Load Flow Study

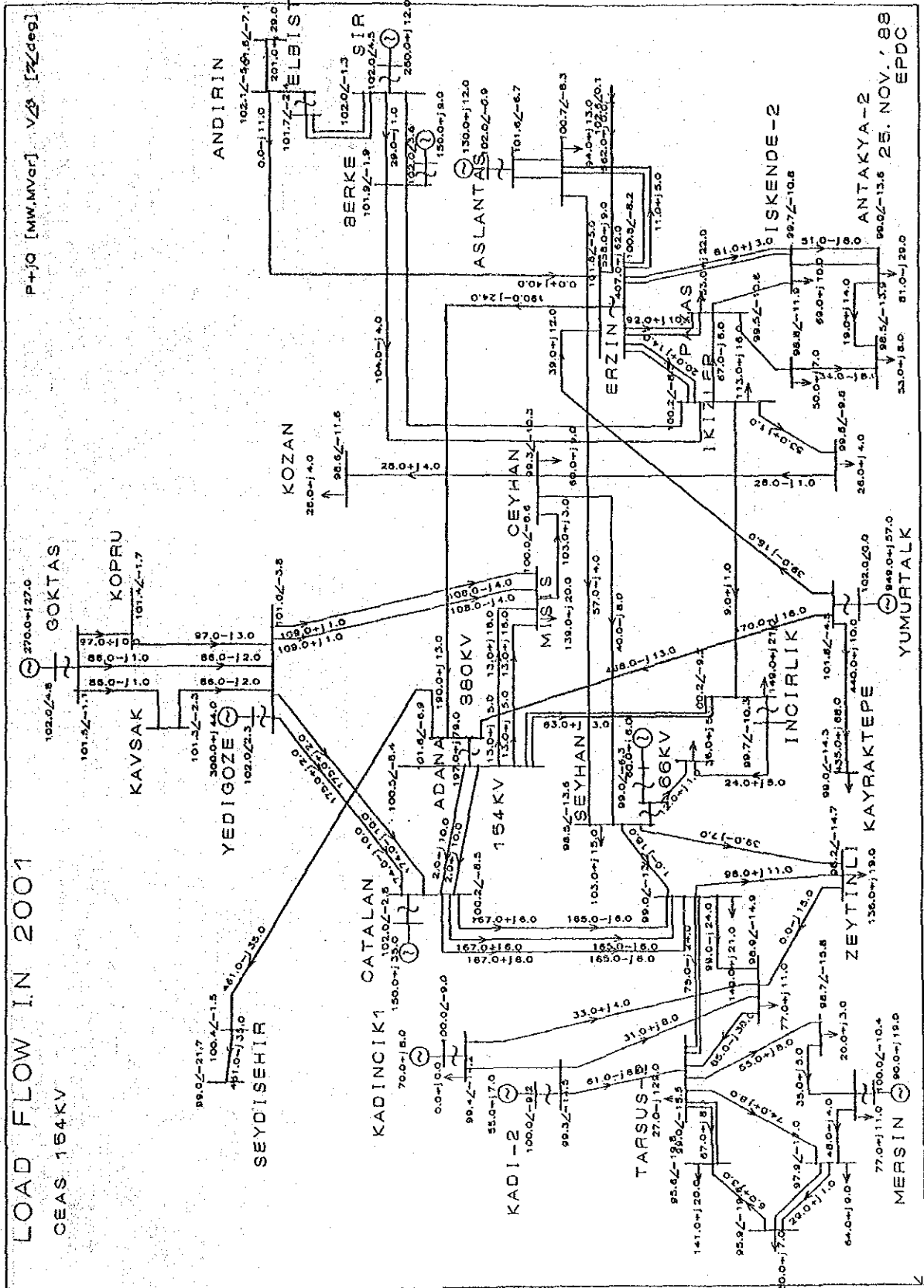






Fig. A-4-1 (4) Load Flow Study

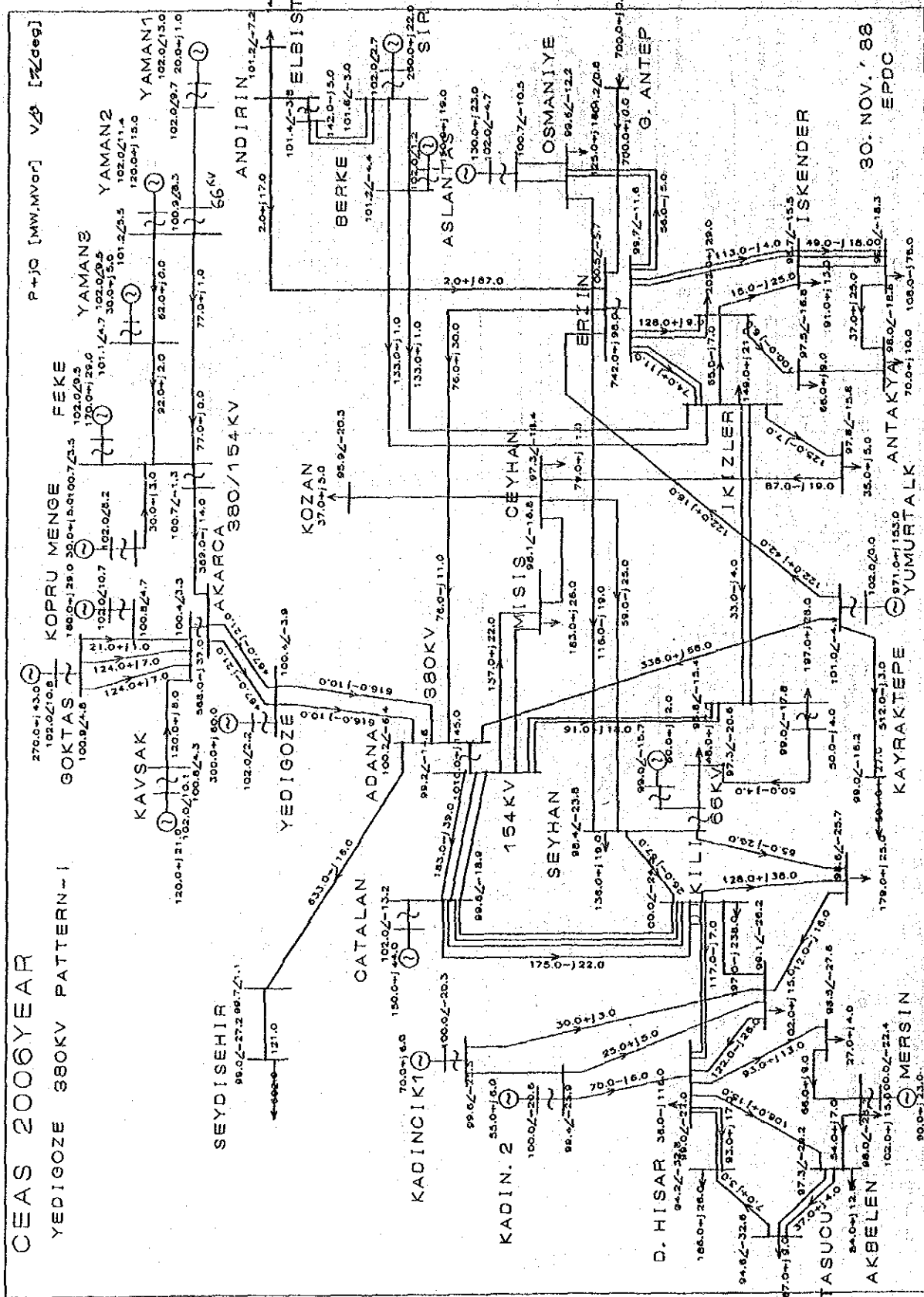


Fig. A-4-1 (5) Load Flow Study

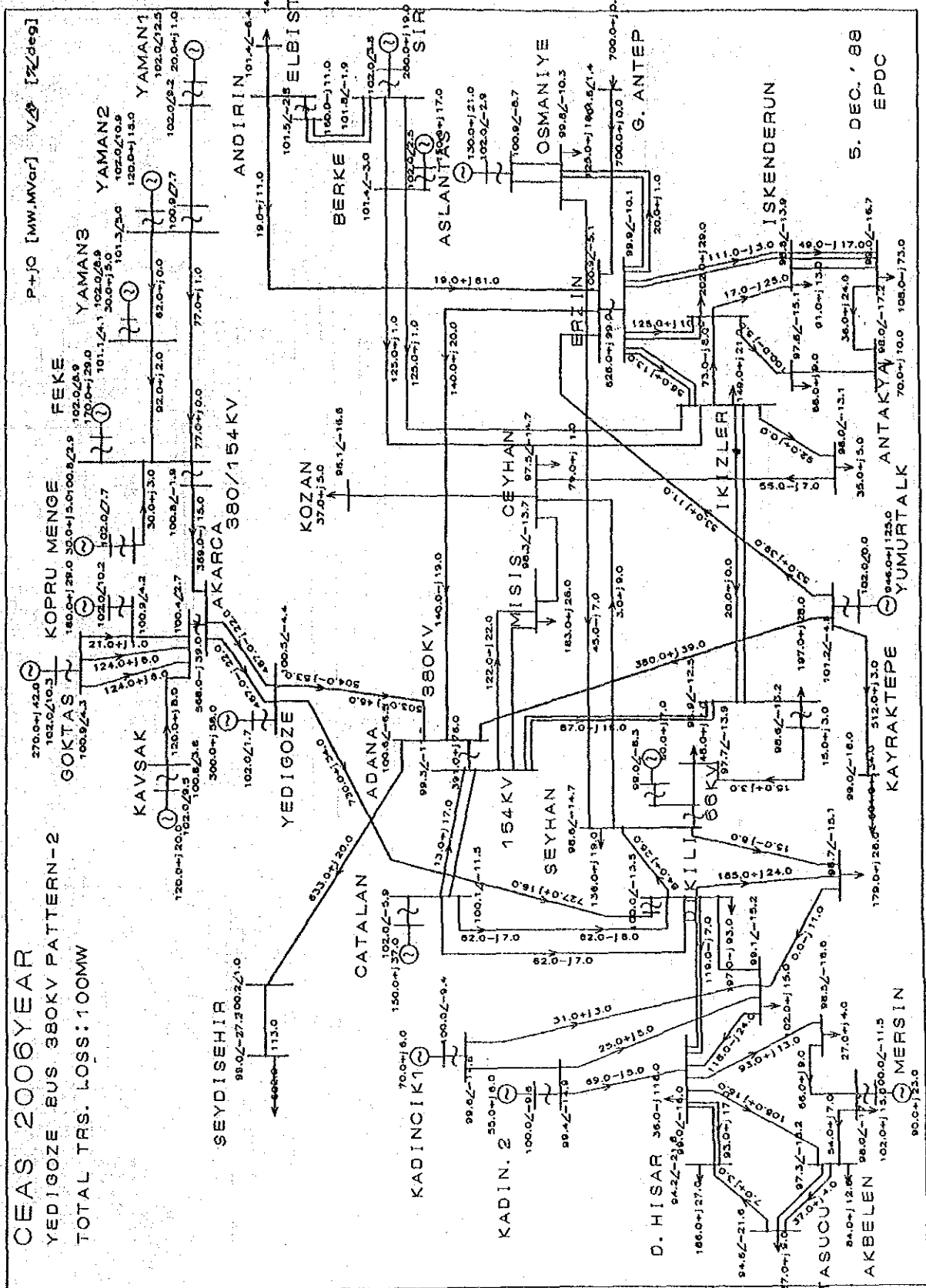


Fig. A-4-2 (1) Short Circuit Study

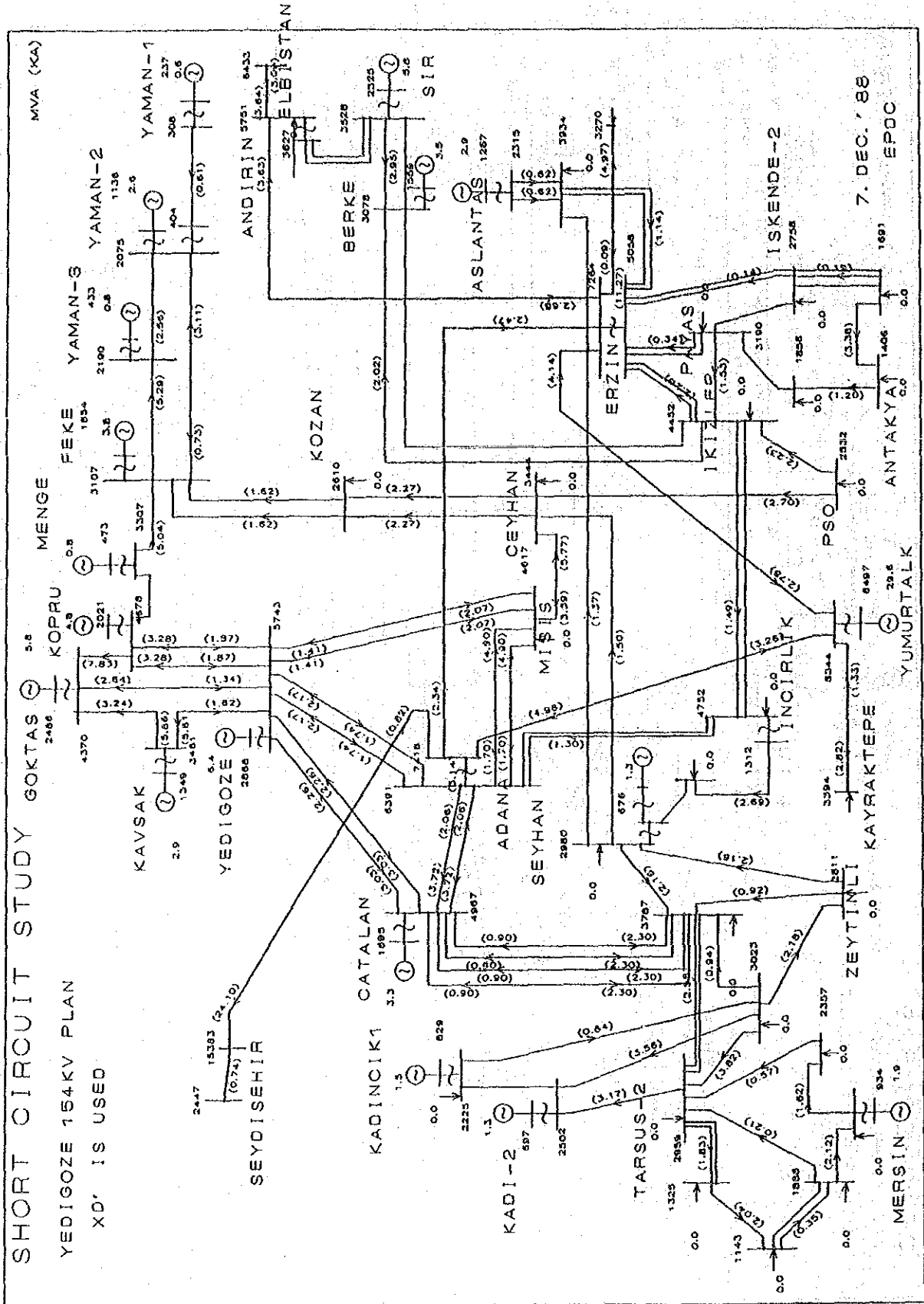


Fig. A-4-2 (2) Short Circuit Study

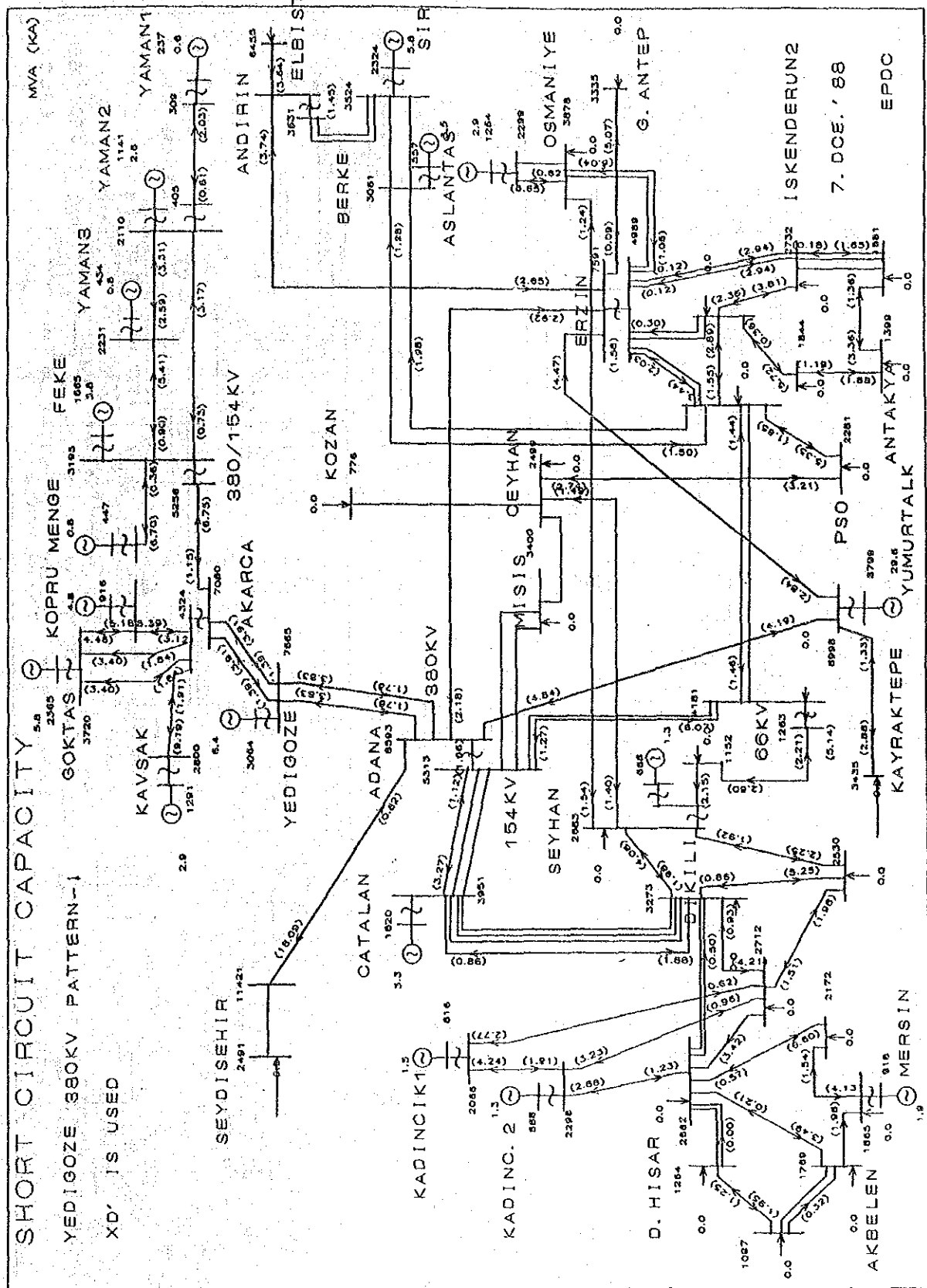
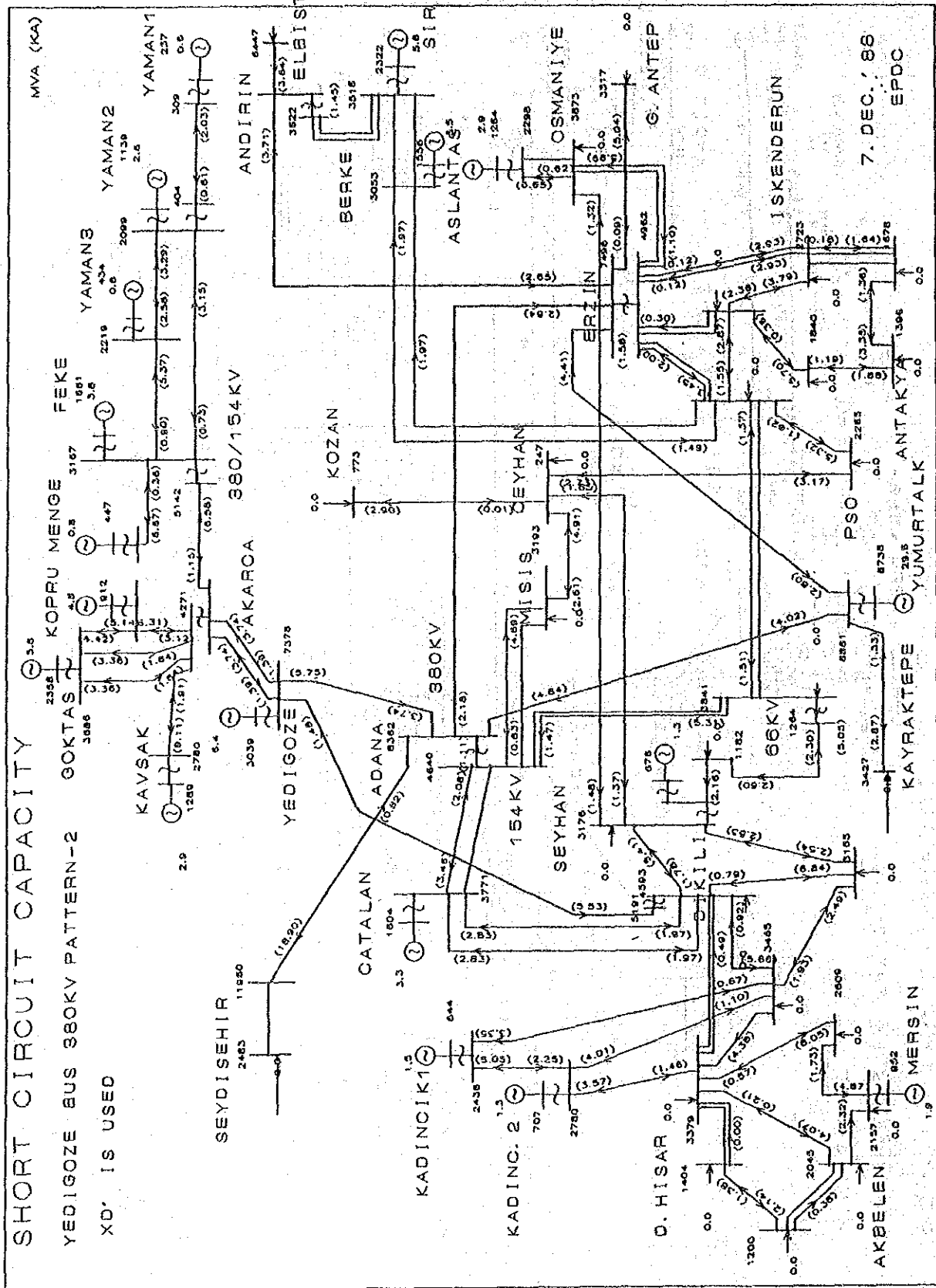




Fig. A-4-2 (3) Short Circuit Study



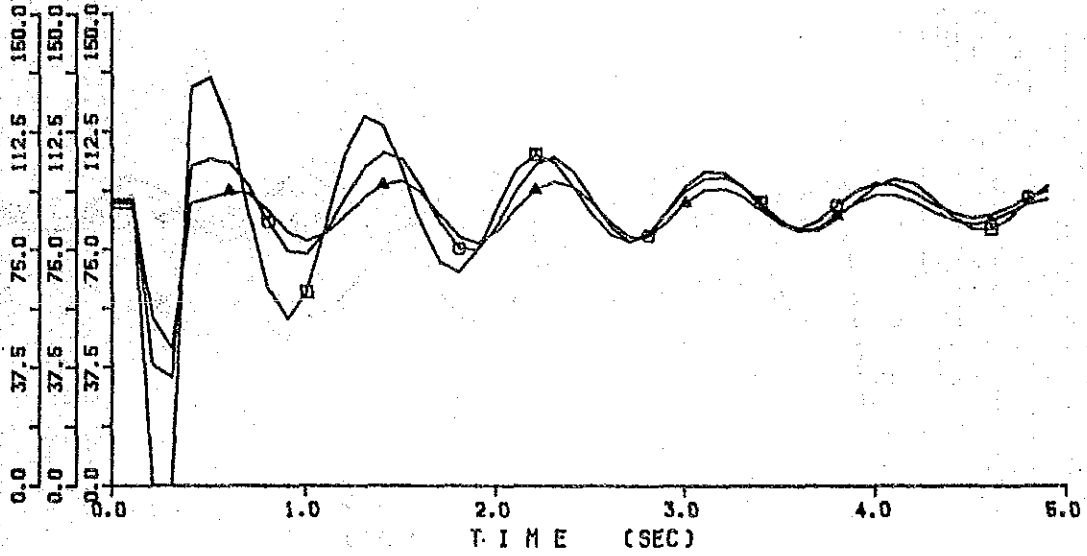
7. DEC. '88  
EPDC

SHORT CIRCUIT CAPACITY  
YEDIGOZE BUS 380KV PATTERN-2  
XD' IS USED

Fig. A-4-3 (1) Stability Study

STABILITY CEAS154KV 2001YEAR 3LG at Goktas

CASE	NAME	項目名	MAX	MIN
□	G-GOKTAS	PG	129.6	0.0
○	G-YEDIGO	PG	106.6	34.2
△	G-CATALA	PG	96.9	43.6



IN 2001

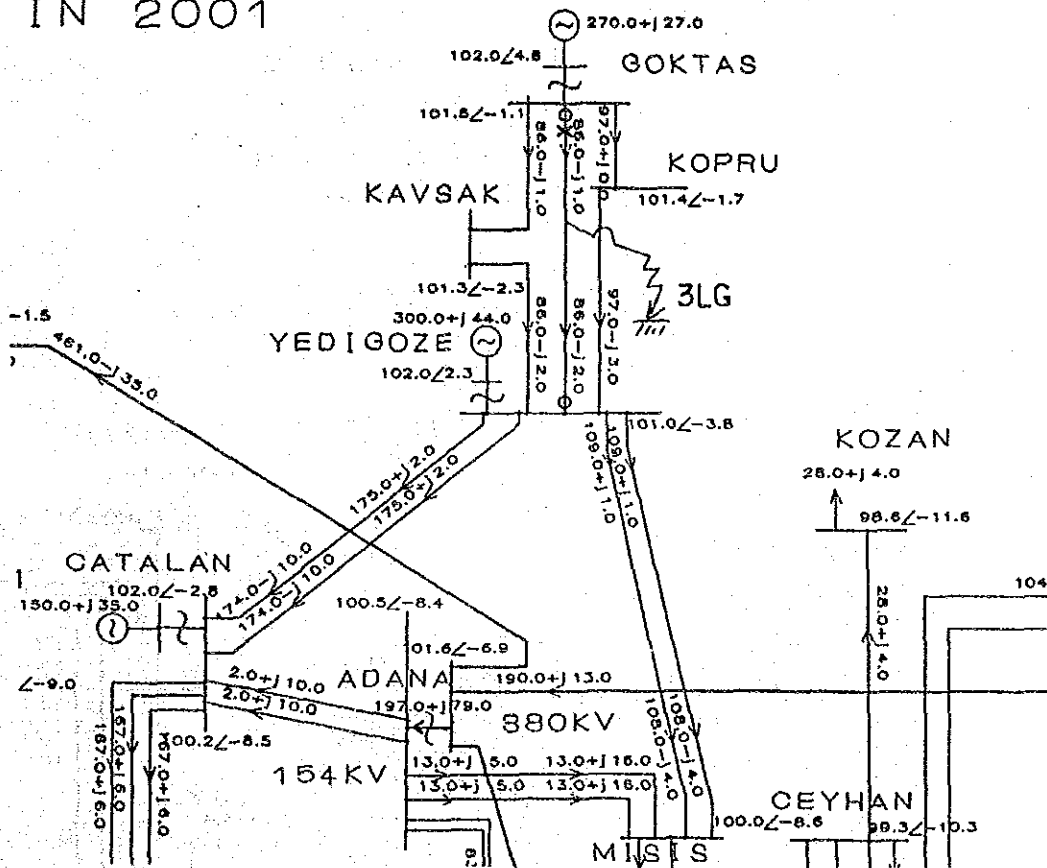
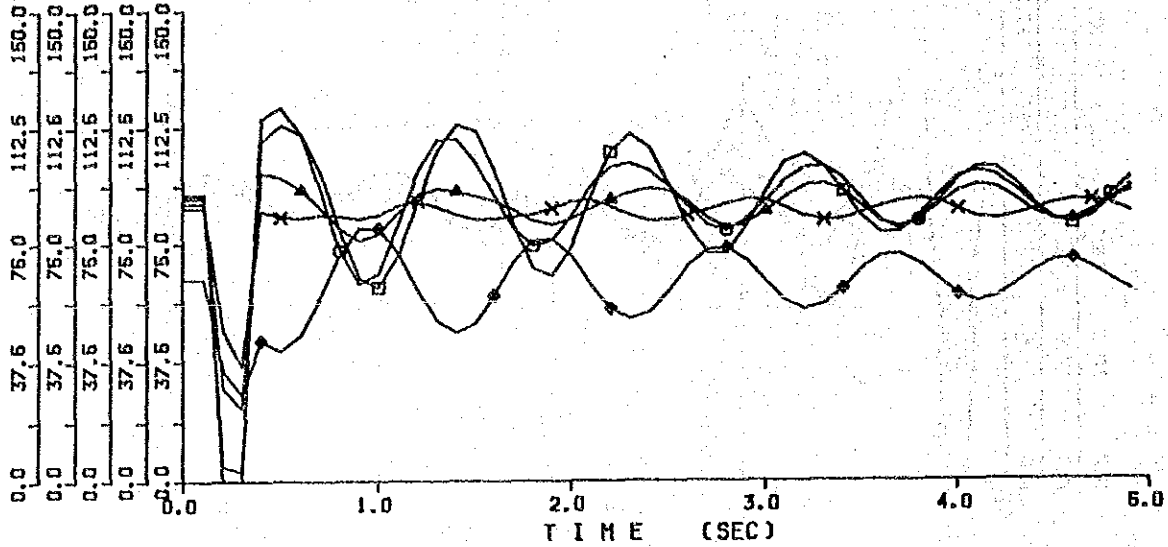


Fig. A-4-3 (2) Stability Study

164KV PLAN(2001)3LG-0 120MS AT YEDIGOZE 164KV BUS

CASE	NAME	#	MAX	MIN
LINE BTW	G-GOKTAS	PG	113.3	3.2
YEDIGOZE	G-YEDIGOZE	PG	119.0	0.0
CATALAN	G-CATALA	PG	97.9	23.3
1-CCT	G-SIR	PG	89.4	36.4
TRIPPED	G-YUMURT	PG	80.0	27.3



IN 2001

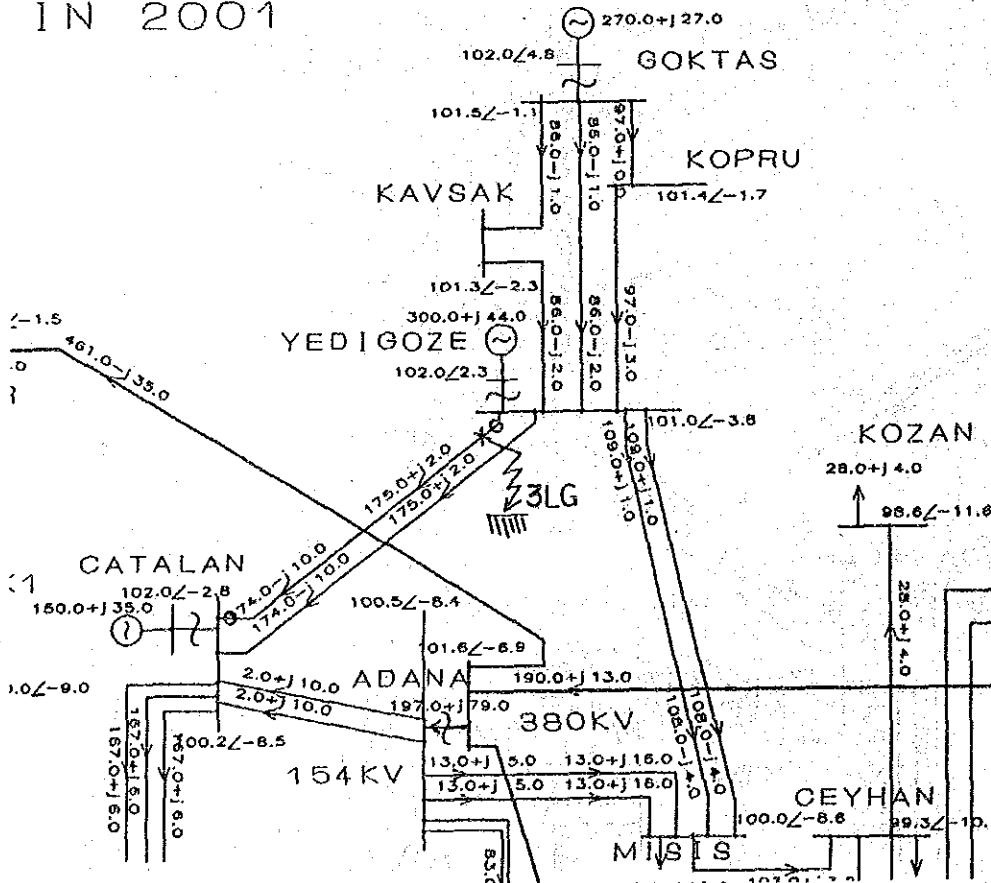
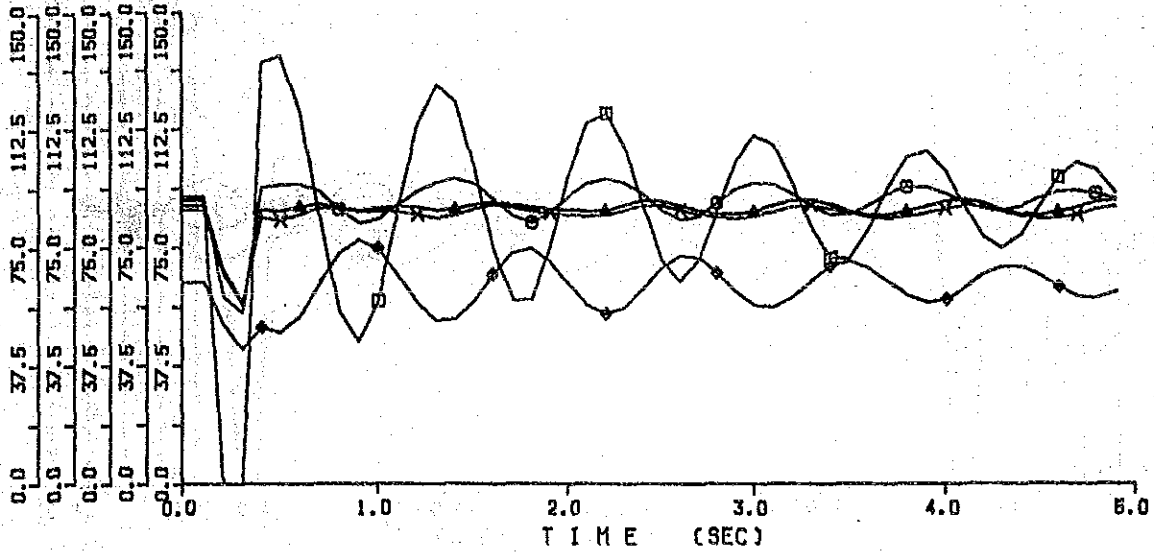


Fig. A-4-3 (3) Stability Study

3LG AT GOKTAS 164KV BUS IN 2001YEAR YEDIGOZE 380KV

	CASE	NAME	項目名	MAX	MIN
—	3LG-0	G-GOKTAS	PG	136.8	0.0
△	120MS AT	G-YEDIGO	PG	96.7	54.2
□	GOKTAS	G-CATALA	PG	90.7	57.1
X		G-SIR	PG	88.7	56.8
◇		G-YUMURT	PG	77.4	42.6



2001

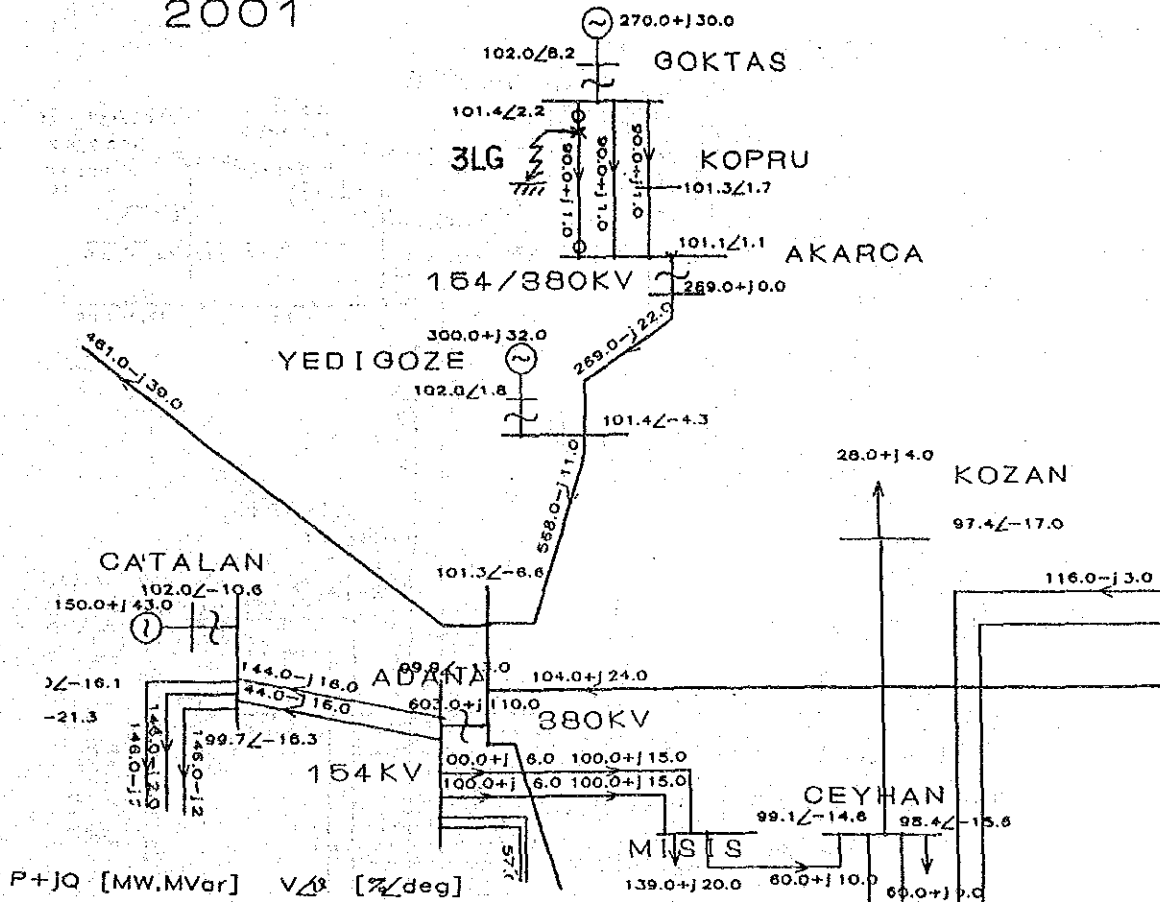
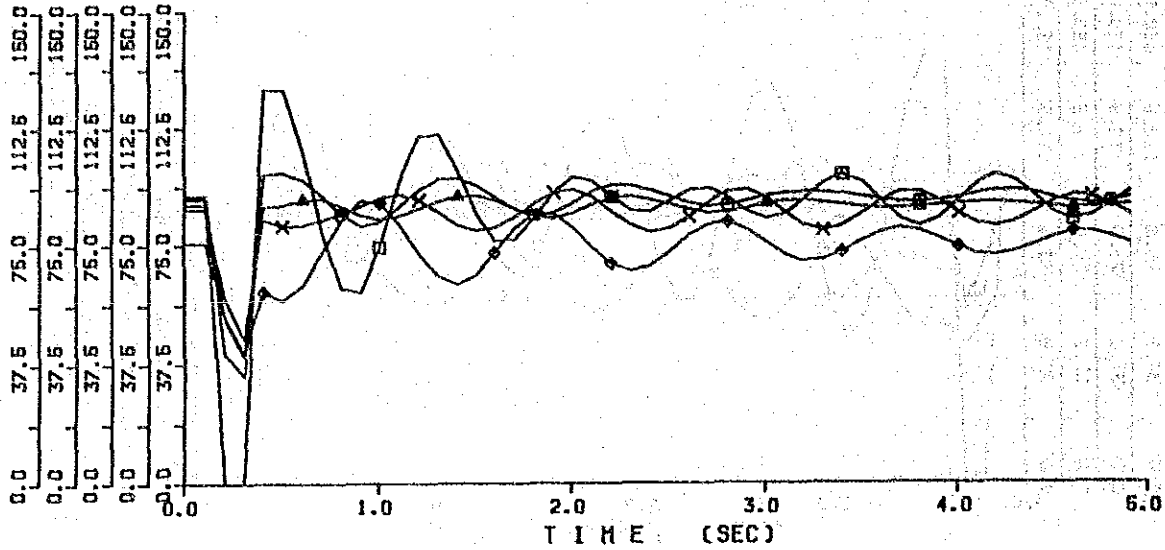


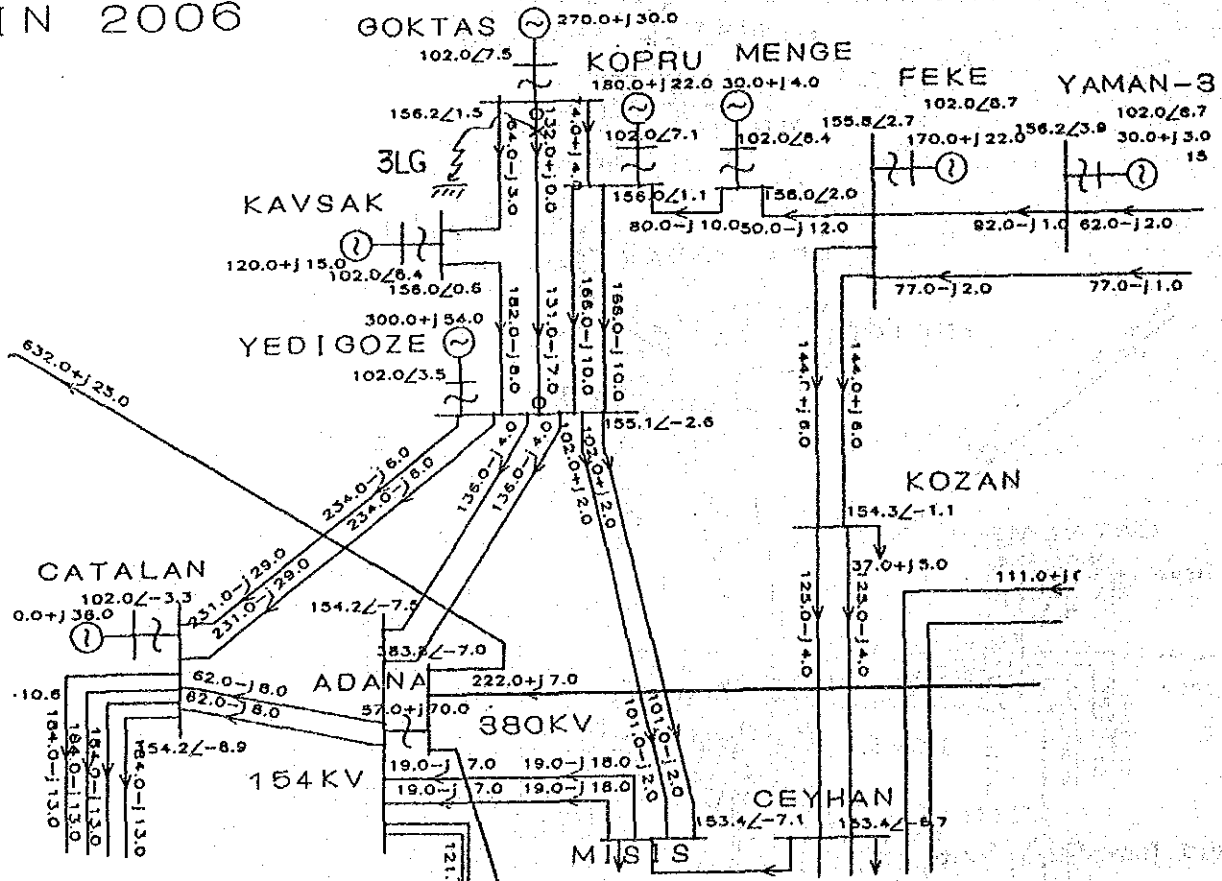
Fig. A-4-3 (4) Stability Study

164KV PLAN(2006) 3LG-0 120MS AT GOKTAS 164KV BUS

CASE	NAME	TYPE	MAX	MIN
LINE BTW	G-GOKTAS	PG	124.6	0.0
GOKTAS	G-YEDIGO	PG	98.1	33.9
YEDIGOZE	G-CATALA	PG	90.9	39.8
I-CCT	G-SIR	PG	93.0	46.6
TRIPPED	G-YUNURT	PG	89.4	41.2



IN 2006

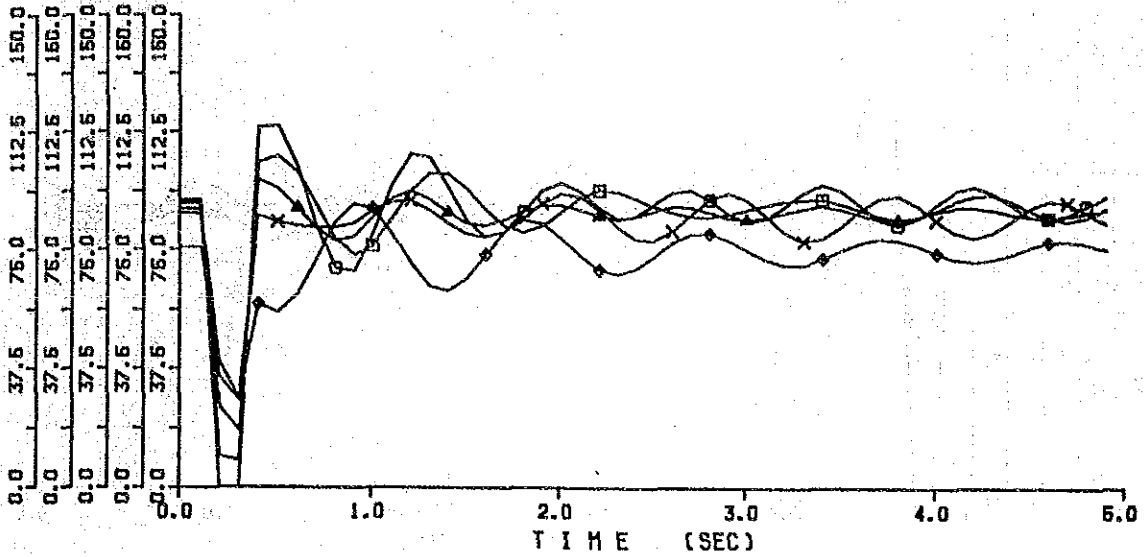


P+jQ [MW,MVar]  $\angle^{\circ}$  [KV/deg]

Fig. A-4-3 (5) Stability Study

154KV PLAN(2006) 3LG-0 120MS AT YEDIGOZE 154KV BUS

CASE	NAME	項目名	MAX	MIN
—	LINE BTW	G-GOKTAS	PG 104.9	8.6
—	YEDIGOZE	G-YEDIGO	PG 114.6	0.0
—	CATALAN	G-CATALA	PG 97.4	17.9
X	I-CCT	G-SIR	PG 93.9	28.5
◇	TRIPPED	G-YUMURT	PG 90.0	27.3



IN 2006

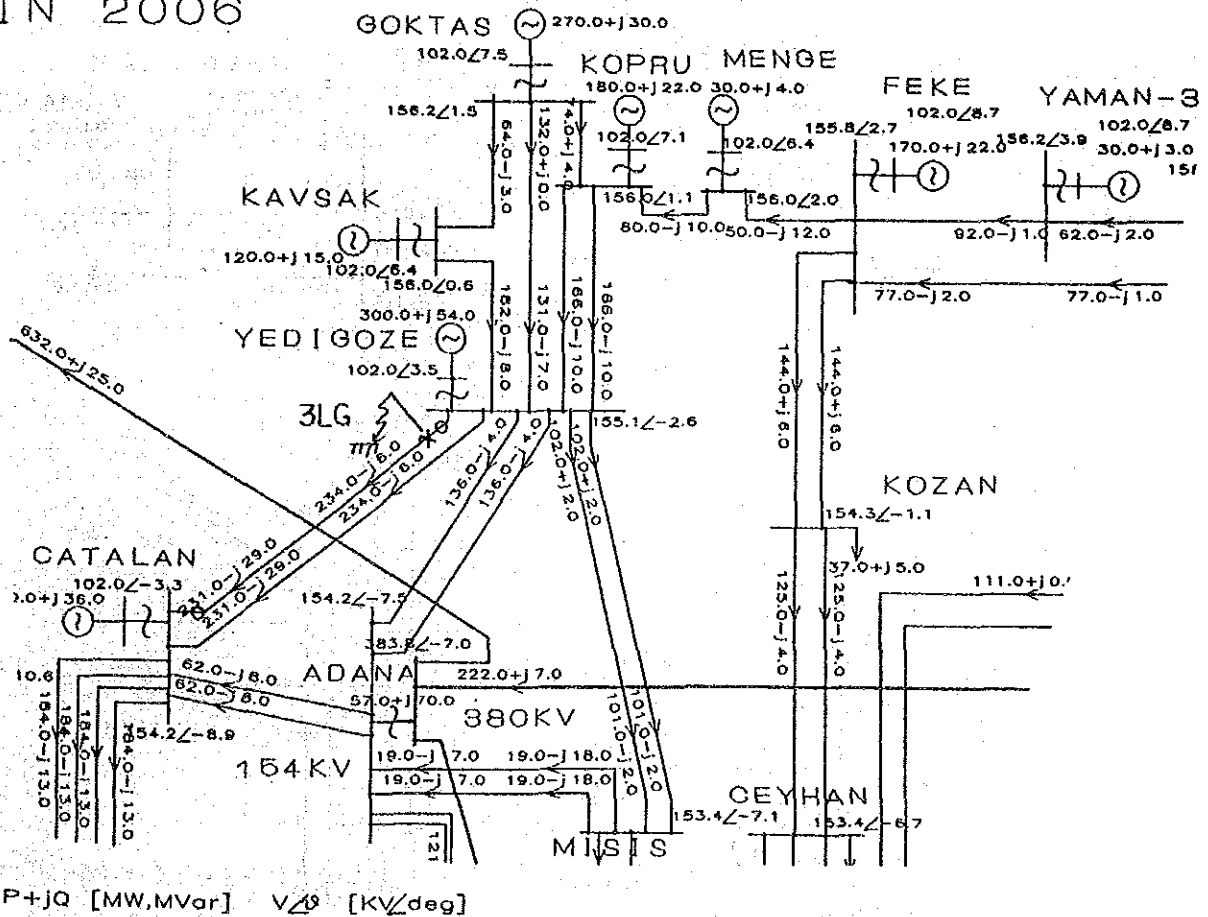


Fig. A-4-3 (6) Stability Study

YEDIGOZE, 380KV (IN 2006)

	CASE	NAME	#	MAX	HIN
—	3LG-0	G-GOKTAS	PG	131.6	0.0
—	120MS AT	G-YEDIGO	PG	91.9	52.2
—	GOKTAS	G-CATALA	PG	91.3	54.6
—	164KV	G-FEKE	PG	89.0	46.6
—	BUS	G-YUMURT	PG	89.5	

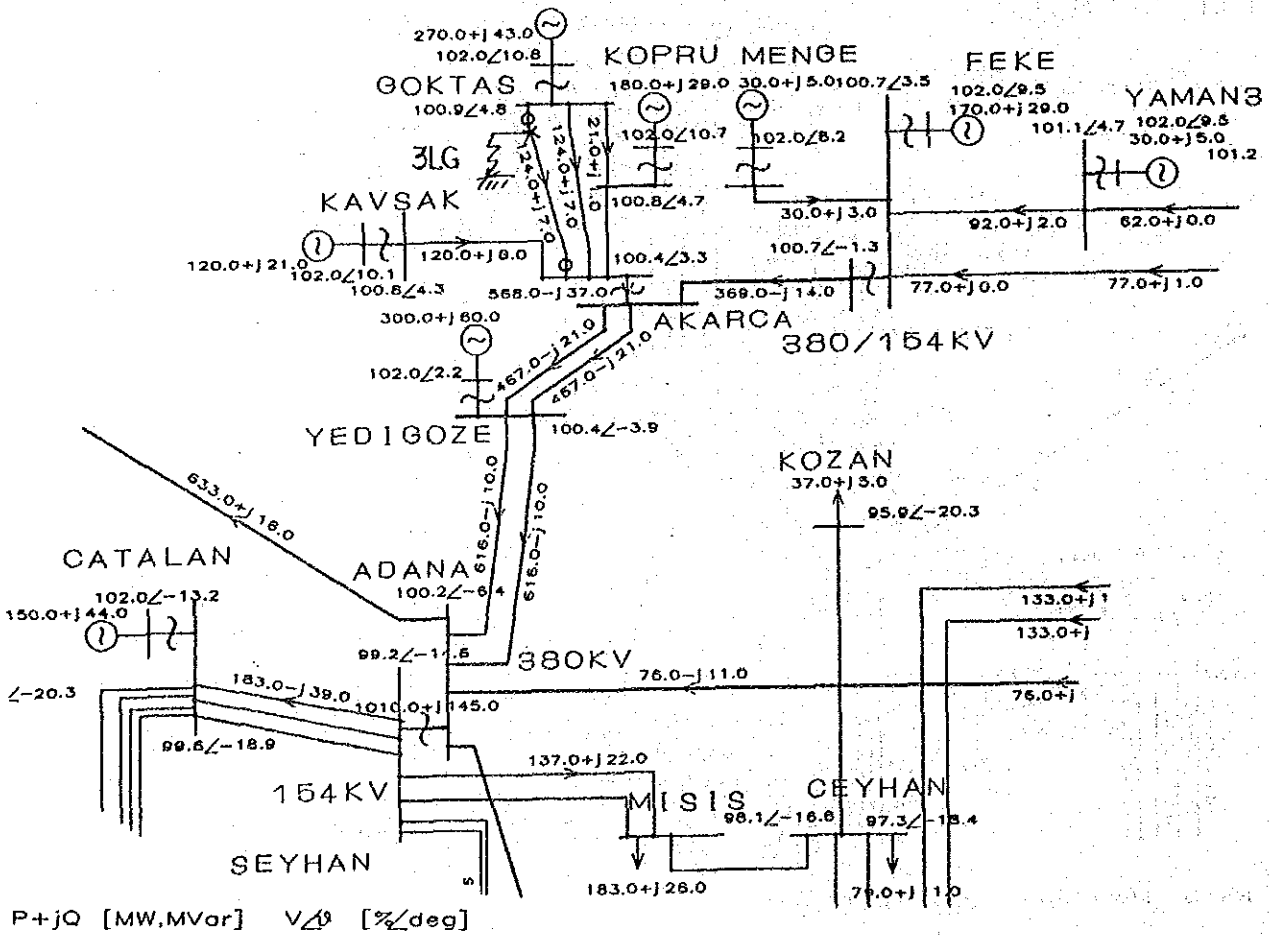
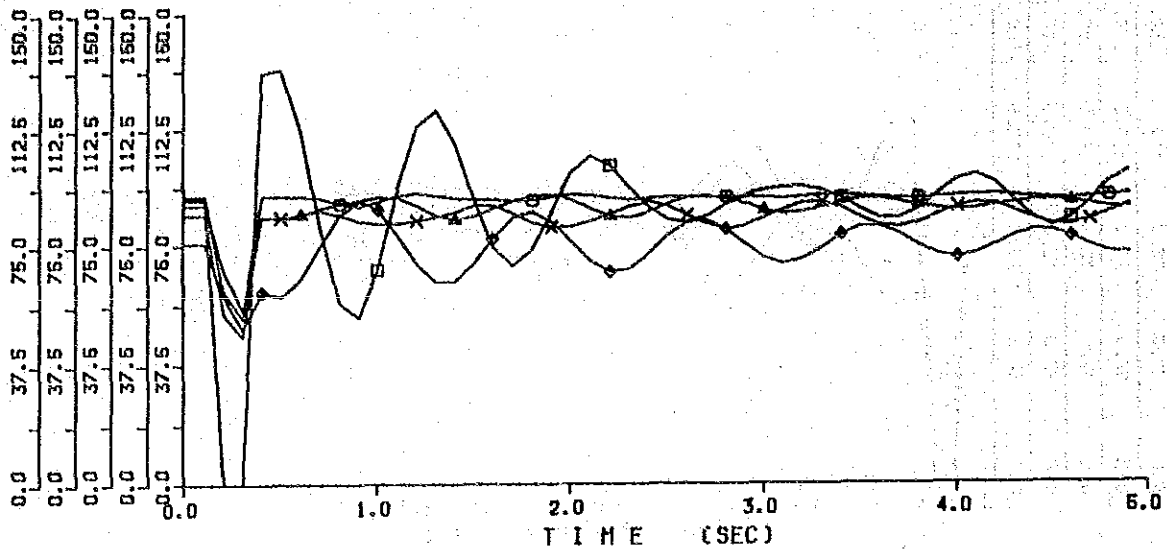


Fig. A-4-3 (7) Stability Study

380KV PLAN(2006) 3LG-0 100MS AT YEDIGOZE 380KV BUS

CASE	NAME	機 名	MAX	MIN
LINE BTW	G-GOKTAS	PG	98.1	6.1
YEDIGOZE	G-YEDIGO	PG	104.2	0.0
ADANA	G-CATALA	PG	91.3	28.6
I-CCT	G-SIR	PG	91.1	25.2
TRIPPED	G-YUMURT	PG	86.0	18.6

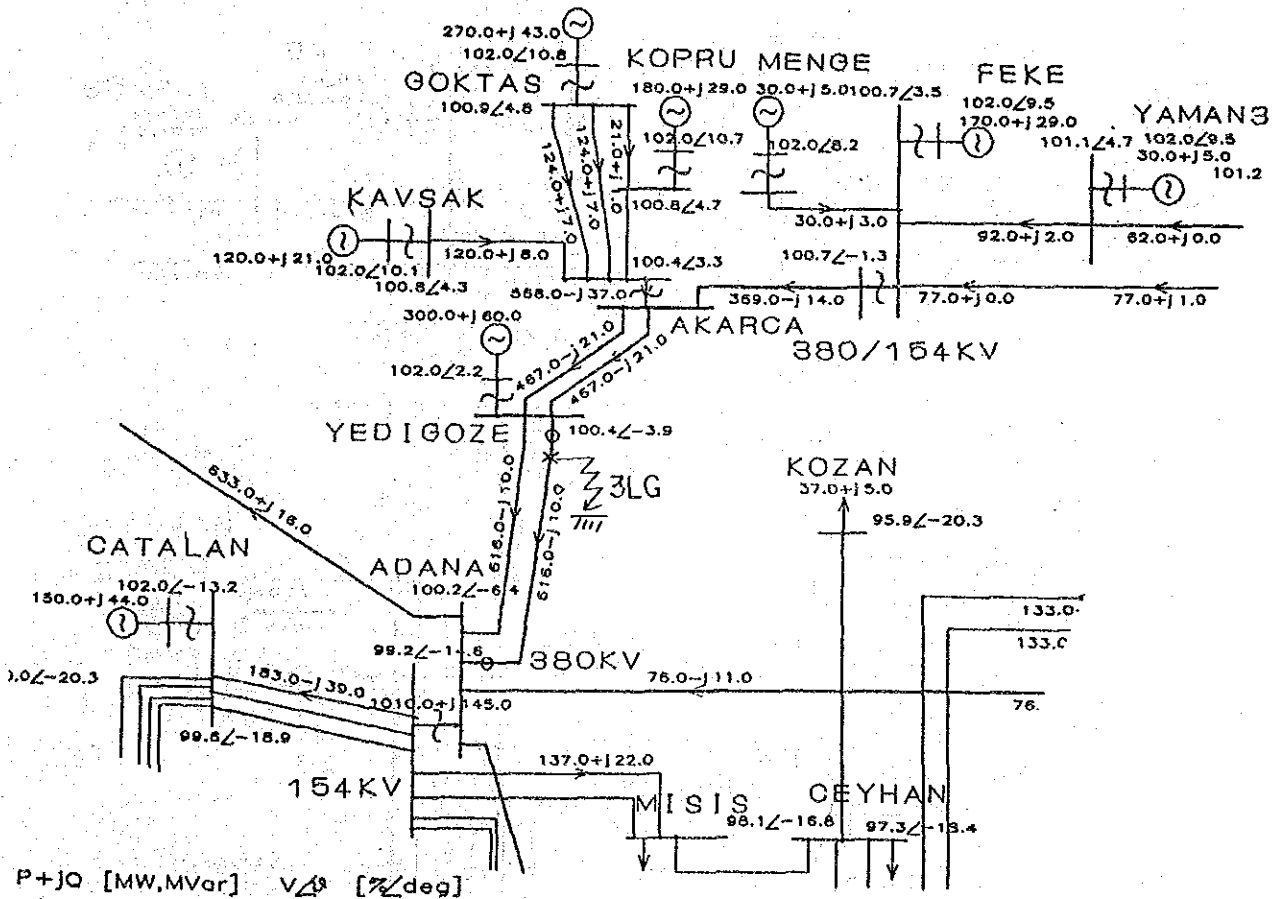
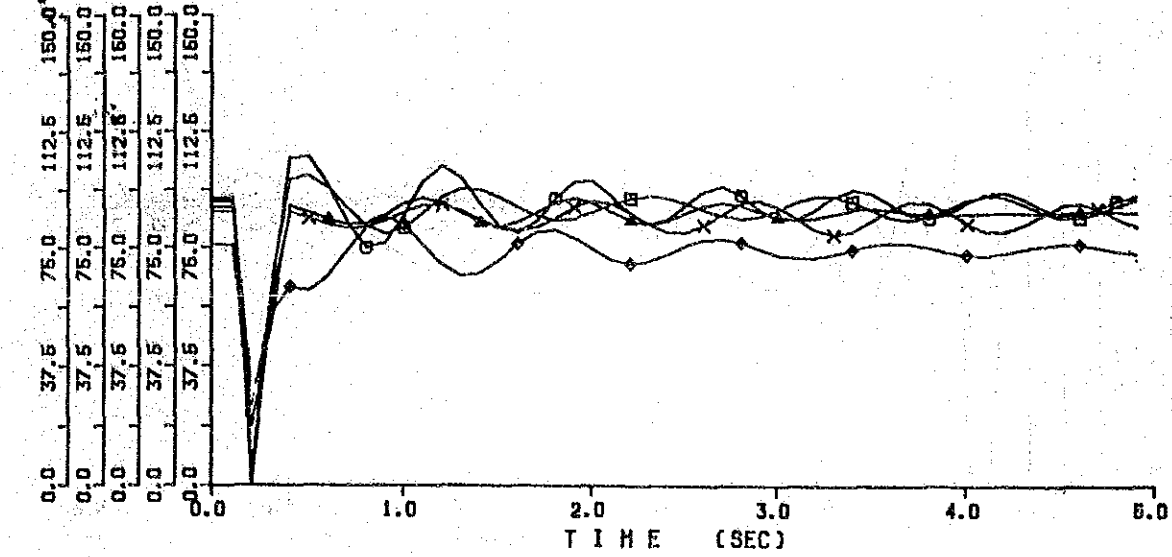




Fig. A-4-3 (8) Stability Study

380KV PATTERN-2 3LG-0 120MS AT GOKTAS 164KV BUSC2006

CASE	NAME	機名	MAX	MIN
□	LINE BTW	G-GOKTAS	PG 129.8	0.0
○	GOKTAS	G-YEDIGO	PG 91.9	49.2
△	AKARCA	G-CATALA	PG 90.0	49.6
X	I-CCT	G-SIR	PG 92.7	52.0
◇	TRIPPED	G-YUMURT	PG 88.8	45.6

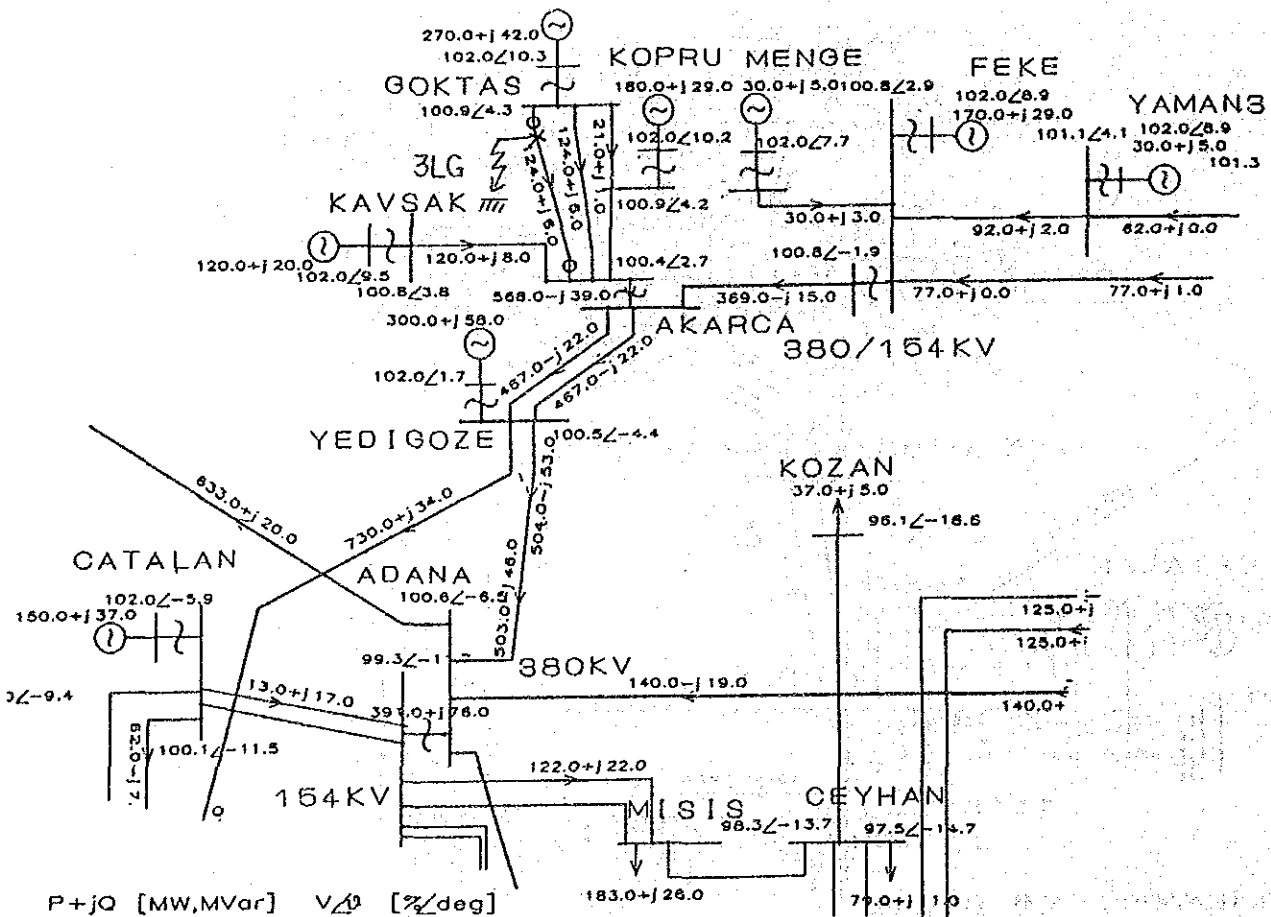
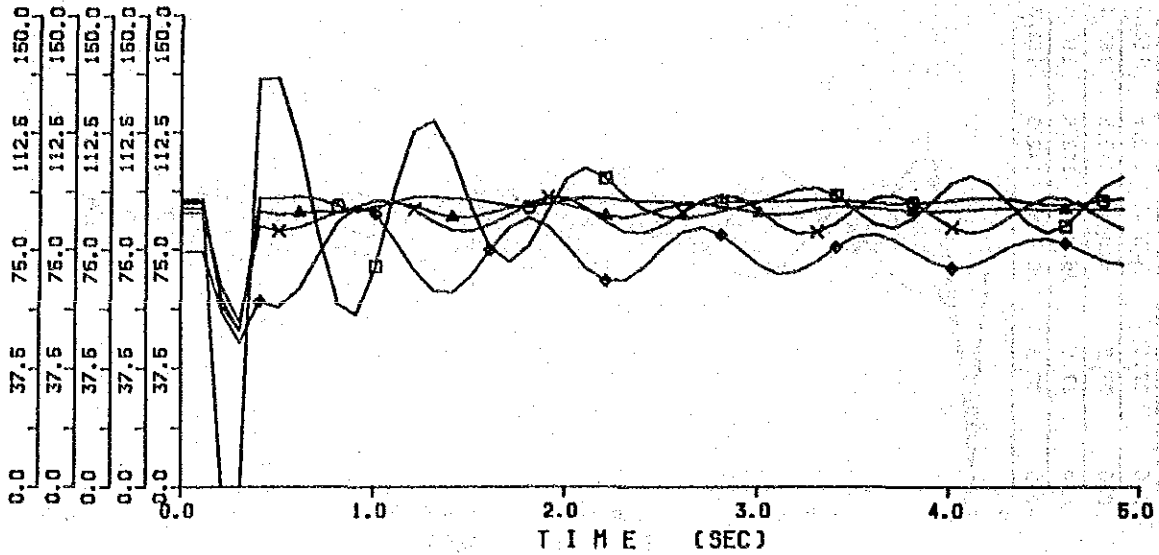
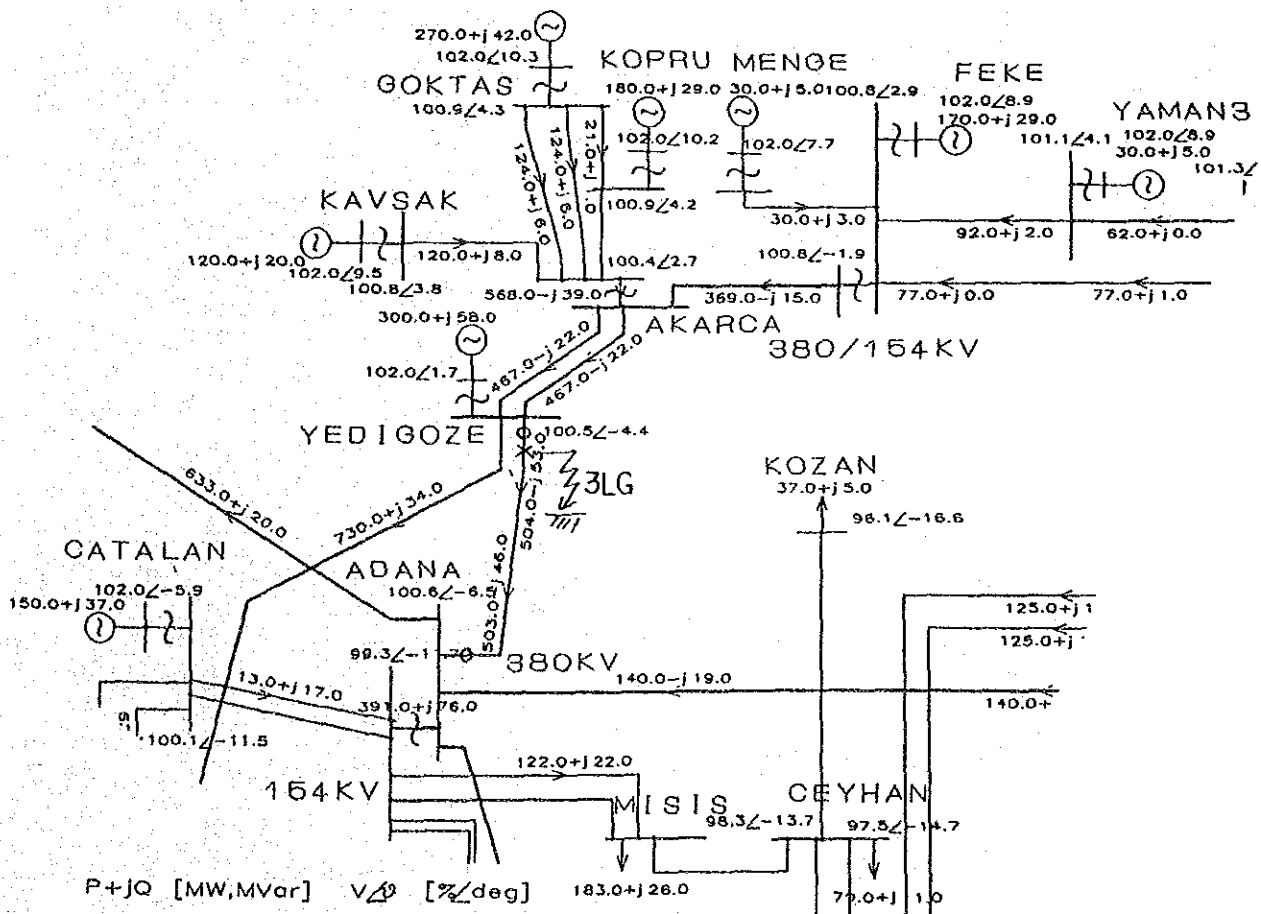
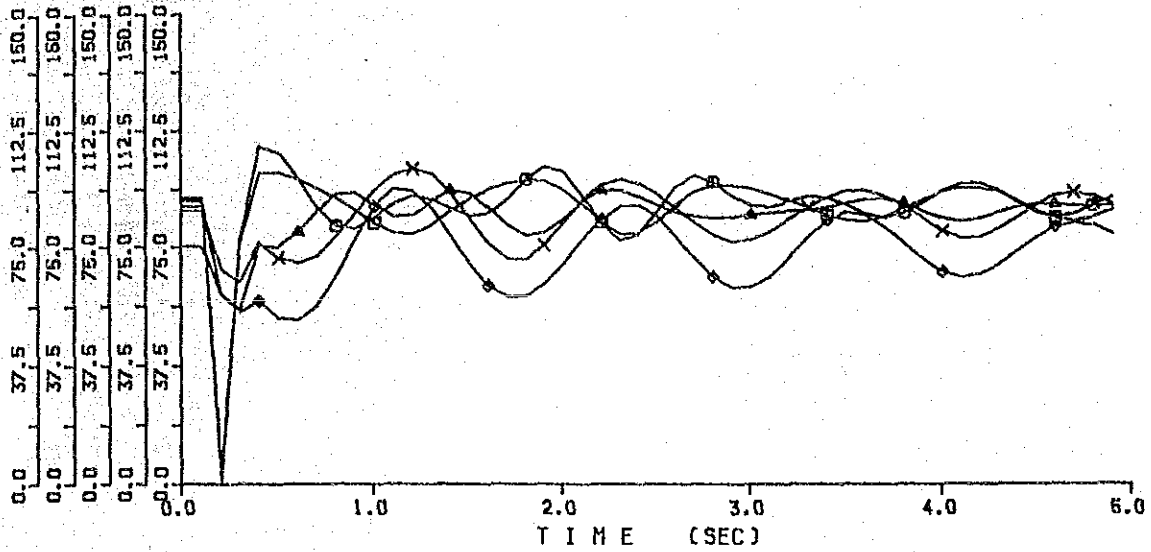


Fig. A-4-3 (9) Stability Study

PATERN-2, STABILITY, 3LG-0 100MS AT YEDIGOZE 380KV BUS

X AXIS	CASE	NAME	項目名	MAX	HIN
—	2006YEAR	G-GOKTAS	PG	98.9	6.1
—	TRS.LINE	G-YEDIGO	PG	107.3	0.0
—	ADANA TO	G-CATALA	PG	94.0	64.7
—	YEDIGOZE	G-SIR	PG	100.4	63.6
◇	TRIPPED	G-YUMURT	PG	93.9	61.8





## **A-5 FEASIBILITY DESIGN**



## A-5 FEASIBILITY DESIGN

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**List of Drawings**

**DWG. A-5-1      Dam (Gravity Type)      Plan, Elevation and Section**

## 5-1 Stability Analysis of Dam

### 1) General

Stability analysis of the Goktas dam was executed according to load partition method under normal and earthquake conditions. Design loads were considered to act onto the dambody under two stages of construction as shown below:

1st stage      first impounding/before joint grouting  
2nd stage      final impounding/after joint grouting

### 2) Design Conditions

#### . Properties of dam

Elevation of crest	EL. 635.00 m
Height	148 m
Crest length	242 m
Arch radius	220 m
Arch angle	63 deg.
Slope	1 : 0.6
Unit weight of concrete	$\gamma_c = 2.35 \text{ tf/m}^3$
Elastic modulus of concrete	$E_c = 3 \times 10^6 \text{ tf/m}^2$
Poisson's ratio of concrete	$\nu_c = 0.2$

#### . Properties of foundation rock

Elastic modulus of rock	$E_r = 1 \times 10^6 \text{ tf/m}^2$
Poisson's ratio of rock	$\nu_r = 0.2$
Angle of internal friction	$\phi = 55^\circ$
Shear strength	$\tau = 400 \text{ tf/m}^2$

(Refer to Final Report 7.4.4)

#### . Reservoir

Reservoir water level	
First stage	EL. 617.00 m (Spillway crest)
Second stage	EL. 630.00 m (H.W.L.)

Wave height (considered for second stage only)

Normal condition

hw = 0.80 m

Earthquake condition

hw + he = 1.50 m

(Refer to A-5-2 II 3))

Downstream water level

EL. 505.00 m

• Sediment

Sediment level

EL. 607.00 m

Unit weight

$\gamma_s = 1.1 \text{ tf/m}^3$

Coefficient of sediment

Cs = 0.5

pressure

• Earthquake

Seismic coefficient

k = 0.12

Direction

Horizontal

Dimensions of dam are shown in Figs. A-5-1 and A-5-2.

Fig. A-5-1 Dimensions of Göktaş Dam (2-1)

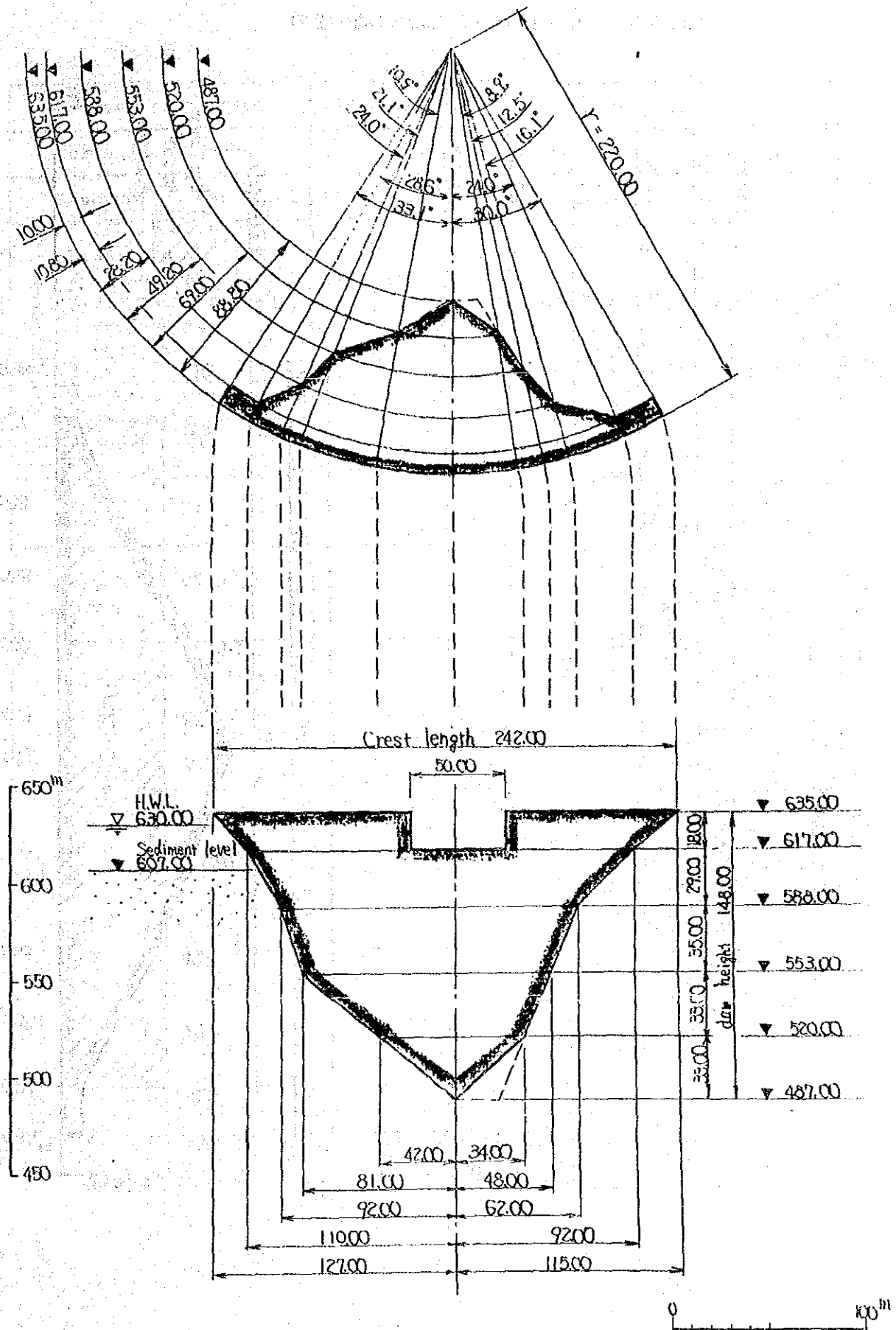


Fig. A-5-2 Dimensions of Göktas Dam (2-2)

