

Table 3.20 Economic Benefit of Irrigation Water Supply

Dam Development Case	Firm Discharge (cms)	Reduction of Annual Deficit (cms)	Increment of Irrigation Area (ha)	Increment of Paddy Production (Mil.M\$/year)	Annual <u>1</u> / Average Benefit (Mil.M\$/year)
Lebir	55	5.6	2,750	0.91	0.51
	60	6.2	3,044	1.01	0.57
	65	6.8	3,339	1.10	0.62
	70	7.3	3,584	1.19	0.65
	75	7.6	3,732	1.23	0.69
80	7.9	3,879	1.28	0.72	
Dabong	160	8.5	4,174	1.38	0.78
	180	8.5	4,174	1.38	0.78
	200	8.5	4,174	1.38	0.78
	220	8.5	4,174	1.38	0.78
	240	8.5	4,174	1.38	0.78
Nenggiri	75	7.7	3,781	1.25	0.70
	80	8.0	3,928	1.30	0.73
	85	8.2	4,026	1.33	0.75
	90	8.3	4,075	1.35	0.76

Note : 1/ Assuming discount rate of 10%, the benefit was calculated in terms of the annual average value for a 57-year period covering the dam construction period of 7 years and the dam project life of 50 years.

Table 3.21 Dam Investment Cost for the Purpose of Water Resources Development

Dam	NHWL, Dam crest		Installed capacity, MW	Plant discharge, m <sup>3</sup> /sec	Investment cost, million MS			
	El.m	El.,m			Dam	Power	Relocation	Total
Lebir	65.0	78.2	67.0	220.0	232.5	131.0	103.5	467.0
	70.0	82.7	87.0	260.0	260.0	155.0	130.5	545.5
	75.0	86.8	112.0	300.0	276.5	179.0	159.0	614.5
	80.0	91.1	126.0	300.0	291.9	204.1	190.1	686.1
Dabong	54.0	69.6	160.0	720.0	59.0	356.8	264.4	680.2
	56.0	71.1	187.0	800.0	64.2	370.8	265.6	700.6
	58.0	72.7	201.0	800.0	70.4	384.2	267.0	721.6
	60.0	74.5	214.0	800.0	77.8	396.0	268.2	742.0
	62.0	76.4	246.0	880.0	83.8	407.6	269.5	760.9
	64.0	78.2	262.0	880.0	88.7	418.7	270.5	777.9
	66.0	78.5	296.0	960.0	89.7	428.9	270.6	789.2
66.7	80.0	302.0	960.0	94.2	431.7	270.9	796.8	
Nenggiri	135.0	151.3	175.0	300.0	251.2	234.0	15.1	500.7
	140.0	155.4	188.0	300.0	263.8	241.4	15.1	520.7
	145.0	159.7	213.0	320.0	280.2	268.4	15.1	564.1
	150.0	164.1	227.0	320.0	299.1	274.5	15.1	589.1
	155.0	168.4	255.0	340.0	318.9	288.1	15.1	622.5
157.0	169.0	275.0	360.0	353.6	292.6	15.1	661.8	

Table 3.22 Economic Evaluation of Dam Development Project

Dam	Normal High Water Level (EL.m)	Firm Discharge (cms)	Dependable Capacity (MW)	Average Annual Energy (GWH)	Cost for Dam Development			Benefit from Dam Development			Net Benefit (B)-(C) 2/ (Mil.M\$/yr)	EIRR (%)	
					Investment Cost (Mil.M\$)	O/M Cost (Mil.M\$/yr)	Annual 2/ Cost (Mil.M\$/yr)	Hydro-power (Mil.M\$/yr)	Irrigation 2/ Supply (Mil.M\$/yr)	Total (Mil.M\$/yr)			(B)/(C) Ratio
Lebir	80	75	110	359	733.6	1.64	41.88	27.85	0.89	28.74	0.59	-20.14	5.30
	75	75	88	322	648.3	1.46	43.20	23.87	0.89	24.76	0.57	-18.44	5.15
	70	65	73	279	570.2	1.13	37.92	20.34	0.75	21.09	0.56	-16.83	4.97
	65	55	59	238	488.5	0.87	32.44	17.01	0.56	17.57	0.54	-14.87	4.82
Dabang	66.7	240	272	935	857.8	3.93	58.19	71.02	1.16	72.18	1.24	13.99	12.78
	66	240	264	918	850.1	3.85	57.64	69.41	1.16	70.57	1.22	12.93	12.59
	64	220	236	870	838.4	3.41	56.65	64.30	1.16	65.46	1.16	8.81	11.77
	62	220	217	822	821.2	3.20	55.42	60.13	1.16	61.29	1.11	5.87	11.20
	60	200	194	773	802.2	2.78	53.96	55.49	1.16	56.65	1.05	2.69	10.56
	58	200	179	728	781.7	2.61	52.53	51.88	1.16	53.04	1.01	0.51	10.11
	56	200	162	680	760.6	2.43	51.06	47.91	1.16	49.07	0.96	-1.98	9.57
	54	180	149	633	739.8	2.08	49.51	44.42	1.16	45.58	0.92	-3.94	9.11
Neng-giri	157	90	266	762	670.2	3.58	45.72	62.50	1.06	63.56	1.39	17.84	14.64
	155	85	249	746	657.8	3.32	44.78	60.00	1.02	61.02	1.36	16.25	14.28
	150	80	221	705	611.7	2.95	41.57	55.21	0.98	56.19	1.35	14.62	14.11
	145	80	206	665	581.1	2.77	39.47	51.82	0.98	52.80	1.34	13.33	13.94
	140	75	182	623	534.0	2.44	36.22	47.40	0.92	48.32	1.33	12.10	13.87
135	75	168	583	499.5	2.28	33.88	44.12	0.92	45.04	1.33	11.16	13.81	

Notes: 1/ O/M Cost = Firm Capacity (MW) x 13(M\$/KW-year)  
 2/ Assuming discount rate of 10%

**Table 3.23 Villages and Population to be Affected  
by Dabong Reservoir Development**

Name of Town / Kampung	Population	Number of Households		
		Farming	Non-Farming	Total
<b>A. Dabong Dam (Jeli District)</b>				
1. Kg. Tunku Abdul Rahman Kuala Balah	2,645	503	26	529
2. Kg. Bukit Tok Ali (Dusun Manal)	608	128	7	135
3. Kg. Bukit Jering	1,527	239	18	257
4. Kg. Jerimbong	580	96	4	100
5. Kg. Tebing Timbah	186	48	6	54
6. Kg. Bukit Selai	334	64	3	67
7. Kg. Kubur Datu	816	157	9	166
8. Kg. Jaber	190	49	3	52
9. Kg. Lubok Bongor	1,057	193	22	215
10. Kg. Renyut	141	30	2	32
11. Kg. Chegar Bedil	229	43	3	46
12. Kg. Pasir Dusun	527	98	5	103
13. Kg. Teluk Bayu	132	24	2	26
14. Kg. Belahat	381	68	8	76
15. Kg. Berdang	684	120	14	134
<b>Total</b>	<b>10,037</b>	<b>1,860</b>	<b>132</b>	<b>1,992</b>
<b>B. Dabong Dam (Kuala Krai District)</b>				
1. Kg. Biak	200	50	-	50
2. Kg. Kl pergau	120	30	-	30
3. Kg. KandeK	533	100	33	133
4. Kg. Jewang	240	50	10	60
5. Kg. Rambai	50	12	-	12
6. Kg. Stong	53	10	7	17
7. Kg. Kl Mahligai	84	21	-	21
8. Kg. Serasa	90	22	-	22
9. Kg. Pulau Layak	18	5	-	5
10. Kg. Sg. Suda	13	4	-	4
11. Kg. Dabong	2,000	350	150	500
12. Kg. Kemubu	1,017	200	55	255
<b>Total</b>	<b>4,418</b>	<b>854</b>	<b>255</b>	<b>1,109</b>

Source: Jeli Distric office, Kuala Krai District office and  
Orang Asli Department (JHEOA), Kelantan

Table 5.1 Flood Mitigation Effect of Storage Dams  
( Unit : cms )

Case	Scheme	Peak Cut Ratio		Peak Discharge at Guillemard Bridge		
		1/		20-year	50-year	
1	Without structures	2/	-	13,437	16,369	
2	R/I	3/	4/	14,350	17,420	
3	Nenggiri + R/I		100	13,367	16,175	
4	- do -		90	13,394	16,206	
5	- do -		80	13,435	16,254	
6	- do -		76	5/	13,456	16,299
7	Kemubu + R/I		40	11,609	13,936	
8	- do -		30	11,689	14,136	
9	- do -		20	12,118	14,719	
10	- do -		15	5/	12,500	15,185
11	Lower Pergau + R/I		30	12,801	15,627	
12	- do -		20	12,971	15,879	
13	- do -		10	13,399	16,314	
14	- do -		9	5/	13,433	16,348
15	Dabong + R/I		80	8,459	10,586	
16	- do -		70	8,545	10,683	
17	- do -		60	8,655	10,802	
18	- do -		59	5/	8,988	11,079
19	Lebir + R/I		70	10,190	12,442	
20	- do -		60	10,606	12,580	
21	- do -		50	10,648	12,817	
22	- do -		37	5/	10,661	13,213
23	Lebir + Nenggiri + R/I	70	100	6/	10,021	11,592
24	- do -	60	90		10,157	11,999
25	- do -	50	80		10,238	12,088
26	- do -	37	76		10,249	12,101
27	Lebir + Kemubu + R/I	70	40		8,429	9,948
28	- do -	60	30		8,456	10,063
29	- do -	50	20		8,789	10,732
30	- do -	37	15		8,896	11,334
31	Lebir + Dabong + R/I	70	80		4,936	6,066
32	- do -	60	70		5,224	6,429
33	- do -	50	60		5,486	6,745
34	- do -	37	59		6,000	7,466

- Notes :
- 1/ Peak-cut ratio = Peak outflow from the spillway for flood mitigation / peak inflow
  - 2/ Flood discharge in natural condition
  - 3/ R/I means river improvement
  - 4/ Inundated flow between Kuala Krai and Guillemard Bridge is confined in the river channel.
  - 5/ An ordinary overflow weir for flood mitigation is not provided to the spillway for the case with the lowest peak-cut ratio of each dam scheme; that is, the flood mitigation to the downstream reaches is only expected with the overflow weir for PMF.
  - 6/ The peak-cut ratio of the Lebir dam scheme is shown in the first column, while the second column for the Nenggiri dam scheme.

Table 5.2 Principal Features of Spillway

Scheme	50-yr peak discharge (cms)	Peak Cut Ratio (%)	Dam type	Dam Crest Elevation (El:m)	Spillway for flood mitigation, m		Spillway for DF, m <sup>1/</sup>			
					NHWL <sup>1/</sup> (El:m)	SWL <sup>1/</sup> (a)	Width (b)	DFWL <sup>1/</sup> (c)		
Nenggiri	100	Rockfill	169.0	150.7	158.6	-	7.9	166.0	75.0	
	4,668 (1984)	90	Rockfill	169.0	152.9	158.8	17.0	5.9	166.0	75.0
	80	Rockfill	169.0	155.0	159.5	50.0	4.5	166.0	75.0	
	76	Rockfill	169.0	157.0	-	-	-	166.0	75.0	
Kemubu <sup>2/</sup>	40	Concrete Gravity	82.0	53.0	70.8	20.0	17.8	80.0	100.0	
	4,943 (1983)	30	Concrete Gravity	82.0	58.4	71.4	37.0	13.0	80.0	100.0
	20	Concrete Gravity	82.0	63.0	72.5	70.0	9.5	80.0	100.0	
	15	Concrete Gravity	82.0	65.7	-	-	-	80.0	100.0	
Dabong	80	Concrete Gravity	80.0	62.4	73.1	25.0	10.7	78.0	100.0	
	8,431 (1983)	70	Concrete Gravity	80.0	64.1	73.7	45.0	9.6	78.0	100.0
	60	Concrete Gravity	80.0	65.6	74.2	70.0	8.6	78.0	100.0	
	59	Concrete Gravity	80.0	66.7	-	-	-	78.0	100.0	
Lebir	70	Rockfill	91.1	76.3	84.2	40.0	7.9	87.6	150.0	
	5,561 (1983)	60	Rockfill	91.1	77.9	84.5	70.0	6.6	87.6	150.0
	50	Rockfill	91.1	79.6	84.7	120.0	5.1	87.6	150.0	
	37	Rockfill	91.1	80.0	-	-	-	87.6	150.0	

Note : <sup>1/</sup> DF : Spillway Design Flood, NHWL : Normal High Water Level  
<sup>2/</sup> SWL : Surchage Water Level, DFWL : Design Flood Water Level  
<sup>3/</sup> The Kemubu project is developed as a single purpose project of flood mitigation.

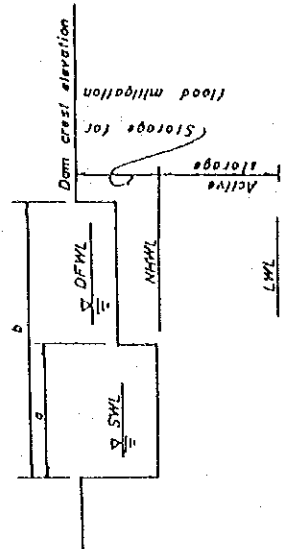


Table 5.3 Cost of Combination Plans including Water Resources Development (1/2)

Combination	Peak cut ratio	Cost, million M\$										Grand Total	
		Dam	Power	Relocation	Total	R/I	Compensation	Total	Dam Project	River improvement	Total		
1. R/I only	-	-	-	-	-	751	132	883	883.0				
2. Nenggiri + R/I	100	353.6	278.0	49.5	647.1	707	125	832	1,479.1				
	90	353.6	284.5	49.5	653.6	708	125	833	1,486.6				
	80	353.6	290.8	49.5	659.9	710	125	835	1,494.9				
	76	353.6	297.0	49.5	656.1	713	126	839	1,505.1				
3. Kemubu + R/I	40	84.9	-	54.7	139.6	626	111	737	876.6				
	30	84.9	-	54.7	139.6	634	112	746	885.6				
	20	84.9	-	54.7	139.6	655	116	771	910.6				
	15	84.9	-	54.7	139.6	671	119	790	929.6				
4. Dabong + R/I	80	94.2	410.1	270.9	775.2	481	85	566	1,341.2				
	70	94.2	419.3	270.9	784.4	485	86	571	1,355.4				
	60	94.2	427.0	270.9	792.1	499	88	587	1,379.1				
	59	94.2	431.7	270.9	796.8	510	90	600	1,396.8				
5. Lebir + R/I	70	291.9	184.7	190.1	666.7	570	101	671	1,337.7				
	60	291.9	193.1	190.1	675.1	576	102	678	1,353.1				
	50	291.9	202.0	190.1	684.0	586	103	689	1,373.0				
	37	291.9	204.1	190.1	686.1	599	106	705	1,391.1				
6. Lebir + Nenggiri + R/I	70 100	645.5	462.7	205.6	1,313.8	530	93	623	1,936.8				
	60 90	645.5	477.6	205.6	1,328.7	555	98	653	1,981.7				
	50 80	645.5	492.8	205.6	1,343.9	557	98	655	1,998.9				
	37 76	645.5	501.1	205.6	1,352.2	558	98	656	2,008.2				

Table 5.3 Cost of Combination Plans including Water Resources Development (2/2)

Combination	Peak cut ratio	Cost, million M\$										
		Dam		Power		Relocation		Total		River improvement		Grand Total
		Dam	Power	Relocation	Total	R/I	Compensation	Total				
7. Lebir + Kemubu + R/I	70 60 50 37	15 20 30 40	376.8 376.8 376.8 376.8	184.7 193.1 202.0 204.1	244.8 244.8 244.8 244.8	806.3 814.7 823.6 825.7	445 450 490 521	78 80 86 92	523 530 576 613	1,329.3 1,344.7 1,399.6 1,438.7		
8. Lebir + Dabong + R/I	70 60 50 37	80 70 60 59	386.1 386.1 386.1 386.1	594.8 612.4 629.0 635.8	461.0 461.0 461.0 461.0	1,441.9 1,459.5 1,476.1 1,482.9	156 184 210 269	28 33 37 48	184 217 247 317	1,625.9 1,676.5 1,723.1 1,799.9		

Note : 1/ The specific cost of flood mitigation is as follows:

M\$ 132.0 million for Nenggiri

M\$ 193.0 million for Dabong

M\$ 191.0 million for Lebir



Table 5.4 Annual Financial Cost of Combination Plan

( Unit : million M\$ )

Malaysia Plan

Combination Plan	Malaysia Plan														
	5 th	6 th	7 th	8 th	9 th	00	'01	'02	'03	'04	'05	'06	'07	'08	
1. R/I only			45.8	45.8	45.8	45.8	45.8	45.8	45.8	45.8	45.8	45.8	45.8	45.8	45.8
2. Nenggiri + R/I			33.5	124.5	225.1	191.5	124.5	90.9	57.4	57.4	57.4	57.4	57.4	57.4	57.4
3. Kemubu + R/I			83.9	129.1	99.0	53.8	53.8	53.8	53.8	53.8	53.8	53.8	53.8	53.8	53.8
4. Dabong + R/I			94.1	136.9	265.5	265.5	222.6	136.9	94.1	51.2	51.2	51.2	51.2	51.2	51.2
5. Lebir + R/I			36.7	117.2	227.3	190.6	117.2	80.5	43.8	43.8	43.8	43.8	43.8	43.8	43.8
6. Lebir + Nenggiri + R/I			36.7	125.9	236.0	199.3	125.9	122.7	119.6	220.2	220.2	186.6	67.1	33.5	33.5
7. Lebir + Kemubu + R/I			36.7	115.2	225.3	188.6	115.2	78.5	71.9	117.1	87.0	41.8	41.8	41.8	41.8
8. Lebir + Dabong + R/I			36.7	103.1	213.2	176.5	103.1	109.3	115.4	214.3	214.3	171.4	85.7	42.9	42.9

Table 5.5 Economic Comparison of Combination Plans

Case	Scheme	Peak-cut ratio, %		EIRR %
1.	R/I	-		5.34
2.	Nenggiri + R/I	100		9.91
3.	- do -	90		10.33
4.	- do -	80		10.53
5.	- do -	76		10.87
6.	Kemubu + R/I	40		4.44
7.	- do -	30		4.38
8.	- do -	20		4.22
9.	- do -	15		4.06
10.	Dabong + R/I	80		11.01
11.	- do -	70		11.31
12.	- do -	60		11.78
13.	- do -	59		11.93
14.	Lebir + R/I	70		6.11
15.	- do -	60		6.20
16.	- do -	50		6.29
17.	- do -	37		6.27
18.	Lebir + Nenggiri + R/I	70	100	9.24
19.	- do -	60	90	9.49
20.	- do -	50	80	9.66
21.	- do -	37	76	9.89
22.	Lebir + Kemubu + R/I	70	15	5.55
23.	- do -	60	20	6.06
24.	- do -	50	30	6.32
25.	- do -	37	40	6.34
26.	Lebir + Dabong + R/I	70	80	11.08
27.	- do -	60	70	11.19
28.	- do -	50	60	11.37
29.	- do -	37	59	11.19

Table 6.1 Variation of Scale of Storage Dam

Storage Dam	Catchment Area (sq. km)	Riverbed Spillway		Dimension (El:m)		Dam Height (m)	Storage (MCM)		Peak Discharge (cms)		Peakcut Ratio (%)			
		Scale	El. (m)	Width (m)	NHWL		DFWL	at NHWL	at DFHL	Inflow		Outflow		
Nenggiri	3,690	Maximum		75	157.0	166.0	169.0	108.0	3,101	4,213	4,668	1,120	76	
		Medium	61	75	126.0	141.0	144.0	144.0	83.0	899	1,686	4,668	2,087	55
		Minimum		75	95.0	115.0	119.0	119.0	58.0	152	532	4,668	3,552	24
Kemubu	5,630	Maximum		100	65.7	80.0	82.0	46.0	726	2,163	4,943	4,184	15.4	
		Medium	36	100	59.6	75.7	77.7	41.7	352	1,461	4,943	4,215	14.7	
		Minimum		100	55.0	71.4	73.4	37.4	250	1,139	4,943	4,389	11	
Dabong	7,480	Maximum		100	66.7	78.0	80.0	58.0	3,707	6,631	8,431	3,457	59	
		Medium	22	85	54.8	69.0	71.0	49.0	1,532	4,294	8,431	4,768	43	
		Minimum		70	40.0	50.0	62.0	40.0	307	2,121	8,431	6,319	25	
Lebir	2,480	Maximum		150	80.0	87.6	91.1	61.1	2,393	3,917	5,561	3,503	37	
		Medium	30	150	63.3	73.2	76.7	46.7	726	1,563	5,561	4,942	11	
		Minimum		150	47.0	58.8	62.3	52.3	102	463	5,561	5,322	4	

Note : 1/ Peak discharge of 50-year probable flood.

Table 6.2 Flood Mitigation Effect of Storage Dam

Storage Dam	Catchment Area (km <sup>2</sup> )	Spillway Width (m)	Scale	Peak Discharge (cms)	Peakcut Ratio (%)	Peak Discharge at Guillemard Bridge (cms)
				Inflow <u>1</u> / Outflow		
Nenggiri	3,690	75	Maximum	4,668	76	16,299 (1,121)
			Medium	4,668	55	16,550 ( 870)
			Minimum	4,668	24	16,890 ( 530)
Kemubu	5,630	100	Maximum	4,943	15.4	15,185 (2,235)
			Medium	4,943	14.7	15,279 (2,141)
			Minimum	4,943	11	15,802 (1,618)
Dabong	7,480	100	Maximum	8,431	59	11,079 (6,341)
			Medium	8,431	43	12,334 (5,086)
			Minimum	8,431	25	13,602 (3,818)
Lebir	2,480	150	Maximum	5,561	37	13,213 (4,207)
			Medium	5,561	11	15,265 (2,155)
			Minimum	5,561	4	16,257 (1,163)

Note : 1/ Peak discharge of 50-year probable flood.

2/ Parenthesized figures are obtained by subtracting peak discharge at Guillemard Bridge from that of river improvement only. ( 17,420 cms )

Table 6.3 Peak Discharge at Guillemard Bridge  
by the Combination of Dam Plan

No.	Combination	Peak Discharge at Guillemard Bridge (cms)
1	R/I only	17,420
2	Ds	13,602
3	Dm	12,334
4	Dl	11,079
5	Ls	16,257
6	Lm	15,265
7	Ll	13,213
8	Ns	16,890
9	Nm	16,550
10	Nl	16,229
11	Ks	15,802
12	Km	15,279
13	Kl	15,185
14	Ds + Ls	13,033
15	Dm + Ls	11,765
16	Dl + Ls	10,510
17	Ds + Lm	12,014
18	Dm + Lm	10,746
19	Dl + Lm	9,491
20	Ds + Ll	9,989
21	Dm + Ll	8,721
22	Dl + Ll	7,466
23	Ds + Ls + Ns	11,928
24	Ds + Lm + Ns	11,648
25	Ds + Ll + Ns	11,327
26	Ds + Ls + Nm	10,926
27	Ds + Lm + Nm	10,656
28	Ds + Ll + Nm	10,335
29	Ds + Ls + Nl	8,874
30	Ds + Lm + Nl	8,604
31	Ds + Ll + Nl	8,283
32	Ks + Ls	13,768
33	Km + Ls	13,245
34	Kl + Ls	13,151
35	Ks + Lm	12,776
36	Km + Lm	12,253
37	Kl + Lm	12,159
38	Ks + Ll	10,724
39	Km + Ll	10,201
40	Kl + Ll	10,107
41	Ns + Ls	15,736
42	Nm + Ls	15,466
43	Nl + Ls	15,145
44	Ns + Lm	14,744
45	Nm + Lm	14,474
46	Nl + Lm	14,153
47	Ns + Ll	12,692
48	Nm + Ll	12,422
49	Nl + Ll	12,101

Remarks ; Dam scheme D : Dabong N : Nenggiri  
L : Lebir K : Kemubu  
Dam scale l : maximum m : medium  
s : minimum

Table 6.4 Construction Cost and Social Impact for Combination Plan

No.	Combination	CONSTRUCTION COST (million M\$)						SOCIAL IMPACT																						
		Dam				R/I	Total	Dabong					Lebir				Nenggiri		Kemubu				R/I		(A)	(B)	(C)			
		Dabong	Lebir	Nenggiri	Kemubu			H(nos) (1)	P(ha)	OP(ha)	RP(ha)	F(ha)	R(km)	PR(km)	H(nos) (2)	OP(ha)	RP(ha)	F(ha)	PR(km)	H(nos) (3)	F(ha)	H(nos) (4)	OP(ha)	RP(ha)	F(ha)	R(km)	H(nos) (5)	B(nos)	Total for H (nos) (1)-(4)	Total for H (nos) (1)-(5)
1	R/I only					883	883																			800	3	0	800	
2	Ds + R/I	445				726	1,171	4,800	40	390	5,580	4,110	30	26												800	2	4,800	5600	5,970
3	Dm + R/I	555				668	1,223	6,100	40	540	6,090	6,030	35	44												800	2	6,100	6900	6,630
4	Dl + R/I	745				603	1,348	7,300	40	1,400	9,850	11,230	55	57												770	1	7,300	8070	11,250
5	Ls + R/I		220			838	1,058								90	2,100	1,200	2,300								800	3	90	890	3,300
6	Lm + R/I		351			798	1,149								140	5,400	2,900	4,600								800	3	140	940	8,300
7	Ll + R/I		611			708	1,319								165	11,800	5,300	8,600	5							800	2	165	965	17,100
8	Ns + R/I			106		862	968													320	1,600					800	3	320	1120	
9	Nm + R/I			246		848	1,094													510	6,100					800	3	510	1310	
10	Nl + R/I			403		835	1,238													640	13,900					800	3	640	1440	
11	Ks + R/I			139		819	958														1,000	180	790	1,910	16	800	3	1,000	1800	970
12	Km + R/I			189		798	987														1,200	560	1,660	3,780	23	800	3	1,200	2000	2,220
13	Kl + R/I			246		793	1,039														1,295	1,160	2,990	6,600	28	800	3	1,295	2095	4,150
14	Ds + Ls + R/I	445	220			698	1,363	4,800	40	390	5,580	4,110	30	26	90	2,100	1,200	2,300								800	2	4,890	5690	9,270
15	Dm + Ls + R/I	555	220			638	1,413	6,100	40	540	6,090	6,030	35	44	90	2,100	1,200	2,300								780	1	6,190	6970	9,930
16	Dl + Ls + R/I	745	220			563	1,528	7,300	40	1,400	9,850	11,230	55	57	90	2,100	1,200	2,300								770	1	7,390	8160	14,550
17	Ds + Lm + R/I	445	351			652	1,448	4,800	40	390	5,580	4,110	30	26	140	5,400	2,900	4,600								800	2	4,940	5740	14,270
18	Dm + Lm + R/I	555	351			580	1,486	6,100	40	540	6,090	6,030	35	44	140	5,400	2,900	4,600								770	1	6,240	7010	14,930
19	Dl + Lm + R/I	745	351			492	1,588	7,300	40	1,400	9,850	11,230	55	57	140	5,400	2,900	4,600								750	1	7,440	8190	19,550
20	Ds + Ll + R/I	445	611			529	1,585	4,800	40	390	5,580	4,110	30	26	165	11,800	5,300	8,600	5							760	1	4,965	5725	23,070
21	Dm + Ll + R/I	555	611			428	1,594	6,100	40	540	6,090	6,030	35	44	165	11,800	5,300	8,600	5							740	1	6,265	7005	23,730
22	Dl + Ll + R/I	745	611			318	1,674	7,300	40	1,400	9,850	11,230	55	57	165	11,800	5,300	8,600	5							670	1	7,465	8135	28,350
23	Ds + Ls + Ns + R/I	445	220	106		647	1,418	4,800	40	390	5,580	4,110	30	26	90	2,100	1,200	2,300			320	1,600				800	1	5,210	6010	9,270
24	Ds + Lm + Ns + R/I	445	351	106		634	1,536	4,800	40	390	5,580	4,110	30	26	140	5,400	2,900	4,600			320	1,600				790	1	5,260	6050	14,270
25	Ds + Ll + Ns + R/I	445	611	106		613	1,775	4,800	40	390	5,580	4,110	30	26	165	11,800	5,300	8,600	5		320	1,600				780	1	5,285	6065	23,070
26	Ds + Ls + Nm + R/I	445	220	246		590	1,501	4,800	40	390	5,580	4,110	30	26	90	2,100	1,200	2,300			510	6,100				770	1	5,400	6170	9,270
27	Ds + Lm + Nm + R/I	445	351	246		573	1,615	4,800	40	390	5,580	4,110	30	26	140	5,400	2,900	4,600			510	6,100				770	1	5,450	6220	14,270
28	Ds + Ll + Nm + R/I	445	611	246		555	1,857	4,800	40	390	5,580	4,110	30	26	165	11,800	5,300	8,600	5		510	6,100				760	1	5,475	6235	23,070
29	Ds + Ls + Nl + R/I	445	220	403		443	1,511	4,800	40	390	5,580	4,110	30	26	90	2,100	1,200	2,300			640	13,900				740	1	5,530	6270	9,270
30	Ds + Lm + Nl + R/I	445	351	403		420	1,619	4,800	40	390	5,580	4,110	30	26	140	5,400	2,900	4,600			640	13,900				730	1	5,580	6310	14,270
31	Ds + Ll + Nl + R/I	445	611	403		393	1,852	4,800	40	390	5,580	4,110	30	26	165	11,800	5,300	8,600	5		640	13,900				720	1	5,605	6325	23,070
32	Ks + Ls + R/I		220		139	732	1,091								90	2,100	1,200	2,300			1,000	180	790	1,910	16	800	2	1,090	1890	4,270
33	Km + Ls + R/I		220		189	710	1,119								90	2,100	1,200	2,300			1,200	560	1,660	3,780	23	800	2	1,290	2090	5,520
34	Kl + Ls + R/I		220		246	705	1,171								90	2,100	1,200	2,300			1,295	1,160	2,990	6,600	28	800	2	1,385	2185	7,450
35	Ks + Lm + R/I		351		139	688	1,178								140	5,400	2,900	4,600			1,000	180	790	1,910	16	800	2	1,140	1940	9,270
36	Km + Lm + R/I		351		189	665	1,205								140	5,400	2,900	4,600			1,200	560	1,660	3,780	23	800	2	1,340	2140	10,520
37	Kl + Lm + R/I		351		246	658	1,255								140	5,400	2,900	4,600			1,295	1,160	2,990	6,600	28	800	2	1,435	2235	12,450
38	Ks + Ll + R/I		611		139	577	1,327								165	11,800	5,300	8,600	5		1,000	180	790	1,910	16	770	1	1,165	1935	18,070
39	Km + Ll + R/I		611		189	540	1,340								165	11,800	5,300	8,600	5		1,200	560	1,660	3,780	23	760	1	1,365	2125	19,320
40	Kl + Ll + R/I		611		246	537	1,394								165	11,800	5,300	8,600	5		1,295	1,160	2,990	6,600	28	760	1	1,460	2220	21,250
41	Ns + Ls + R/I		220	106		816	1,142								90	2,100	1,200	2,300			320	1,600				800	3	410	1210	3,300
42	Nm + Ls + R/I		220	246		805	1,271								90	2,100	1,200	2,300			510	6,100				800	3	600	1400	3,300
43	Nl + Ls + R/I		220	403		792	1,415								90	2,100	1,200	2,300			640	13,900				800	3	730	1530	3,300
44	Ns + Lm + R/I		351	106		775	1,232								140	5,400	2,900	4,600			320	1,600				800	3	460	1260	8,300
45	Nm + Lm + R/I		351	246		764	1,361								140	5,400	2,900	4,600			510	6,100				800	3	650	1450	8,300
46	Nl + Lm + R/I		351	403		748	1,502								140	5,400	2,900	4,600			640	13,900				800	3	780	1580	8,300
47	Ns + Ll + R/I		611	106		684	1,401								165	11,800	5,300	8,600	5		320	1,600				800	2	485	1285	17,100
48	Nm + Ll + R/I		611	246		670	1,527								165	11,800	5,300	8,600	5		510	6,100				800	2	675	1475	17,100
49	Nl + Ll + R/I		611	403		655	1,669								165	11,800	5,300	8,600	5		640	13,900				800	2	805	1605	17,100

Remarks ; Dam Scheme D : Dabong L : Lebir N : Nenggiri K : Kemubu  
Dam Scale s : small m : medium l : large  
Compensation H : houses P : Paddy OP : Oil palm RP : Rubber R : Railway PR : Public road F : Forest B : Bridge  
EIRR F.M. : Flood mitigation P.G. : Power generation







**Table 6.5 Combinations to Meet the Basic Concept on Peak Discharge at Guillemard Bridge**

No.	Combination	Peak discharge at Guillemard Bridge, m <sup>3</sup> /sec	Households to be submerged, nos
1	Dl + Ls	10,510	6,190
2	Dm + Lm	10,746	6,240
3	Dl + Lm	9,491	7,440
4	Ds + Ll	9,989	4,965
5	Dm + Ll	8,721	6,265
6	Dl + Ll	7,466	7,465
7	Ds + Ls + Nm	10,926	5,400
8	Ds + Lm + Nm	10,656	5,450
9	Ds + Ll + Nm	10,335	5,475
10	Ds + Ls + Nl	8,874	5,530
11	Ds + Lm + Nl	8,604	5,580
12	Ds + Ll + Nl	8,283	5,605
13	Ks + Ll	10,724	1,165
14	Km + Ll	10,201	1,365
15	Kl + Ll	10,107	1,460

Remarks : Dam scheme      D : Dabong      L : Lebir  
    K : Kemubu      N : Nenggiri  
    Dam scale      l : maximum      m : medium      s : minimum

Table 6.6 Flood Mitigation Effect at Guillemard Bridge

Case Combination	Probability				
	1/5	1/10	1/20	1/30	1/50
1. Natural condition	8,680	11,430	13,470	14,770	16,370
2. R/I only <u>1/</u>	9,190	12,100	14,350	15,760	17,420
3. Lebir + R/I	6,860	8,840	10,520	11,530	12,910
4. Kemubu + R/I	8,630	11,440	13,180	14,290	15,800
5. Lebir + Kemubu + R/I	6,260	8,060	9,270	9,940	10,650

Note : 1/ Flood discharge inundated at the reaches between Kuala Krai and Guillemard Bridge is confined in the river channel by river improvement.

**Table 6.7 River Division for Implementation**

River stretches	Urban/Rural	Distance, km (A)	Area, km <sup>2</sup>	Population, persons	Population density, persons/km <sup>2</sup>	Annual potential damage (50-year flood), 10 <sup>6</sup> M\$ (B)	Annual potential damage (B)/(A) (10 <sup>6</sup> M\$/km)
<u>Left bank</u>							
DL1	Rural	25.0	239.1	130,084	544	8.70	0.348
DL2	Urban	5.0	62.8 (19.9)	38,217 (23,145)	609 (1,166)	3.81 (0.80)	0.762
DL3	Rural	18.0	69.8	18,590	266	1.38	0.077
DL4	Rural	11.0	19.6	5,665	290	0.95	0.086
DL5	Urban	9.0	34.2	31,206	912	1.53	0.170
DL6	Rural	33.0	31.5	6,508	207	1.34	0.041
<u>Right bank</u>							
DR1	Rural	6.5	25.3	20,965	829	0.33	0.051
DR2	Urban	9.5	163.5 (10.9)	237,317 (41,869)	1,451 (3,852)	12.38 (9.54)	1.303
DR3	Rural	19.0	174.2	94,681	544	5.06	0.266
DR4	Rural	11.0	124.8	67,806	543	2.47	0.225
DR5	Rural	52.0	141.2	43,943	311	3.51	0.068
DR6	Urban	3.0	17.2	38,750	2,252	1.11	0.370

Note: Figures in parentheses show the information in the township area excluding the out-skirts.

**Table 6.8 Disbursement Schedule for the Flood Mitigation Plan of the Kelantan River Basin**

Unit : Million M\$

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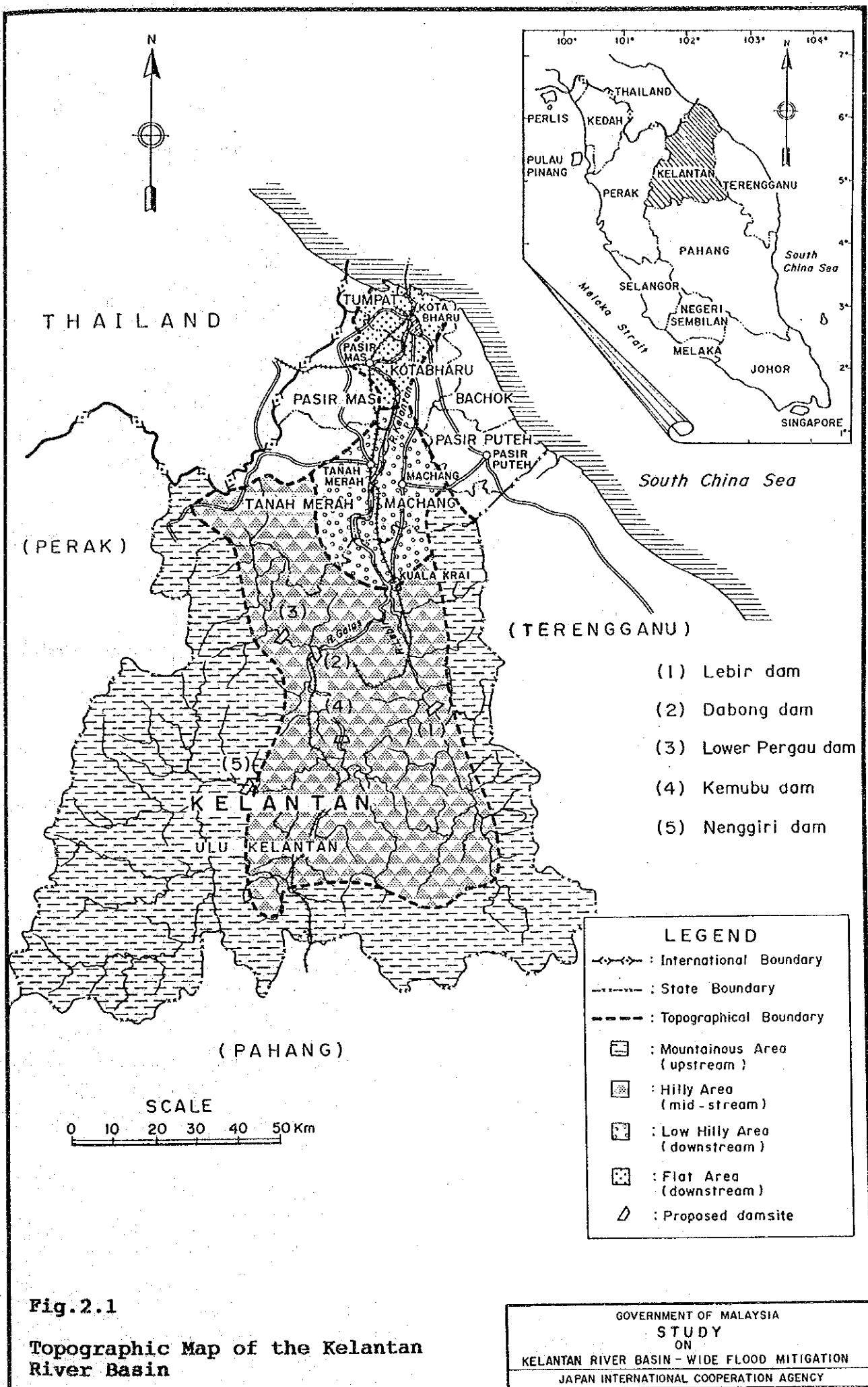
Malaysia Plan

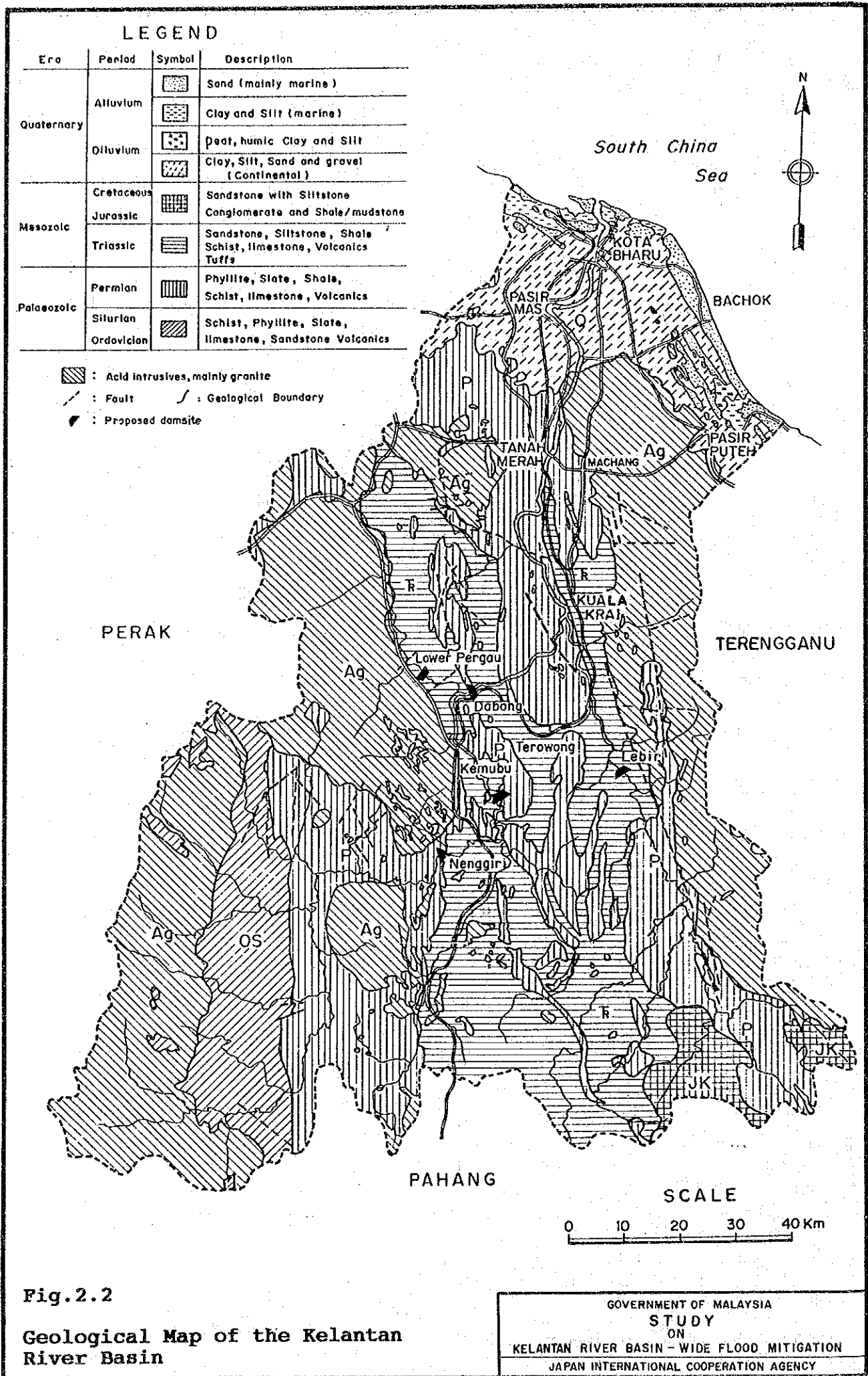
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	6th	7th	8th	9th																
Schemes	'91 '92 '93 '94 '95 '96 '97 '98 '99 2000 '01 '02 '03 '04 '05 '06 '07 '08 '09 '10																			
Urban area	31.7	23.5	23.5	23.5	16.2	16.2	15.2	15.2												
Rural area	39.4	11.5	11.5	11.5	18.9	18.9	19.9	19.9	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0			
Schemes																				
Urban area	117.3	57.5	71.1	84.7	112.0	57.5														
Rural area																				
Total	188.4	92.5	106.1	119.7	147.1	92.6	35.1	35.1	26.0	26.0	26.0	26.0	26.0	26.0	26.0	95.6	77.2	78.7	78.0	
Each Malaysian Plan	387.0				429.6											130.0				355.6

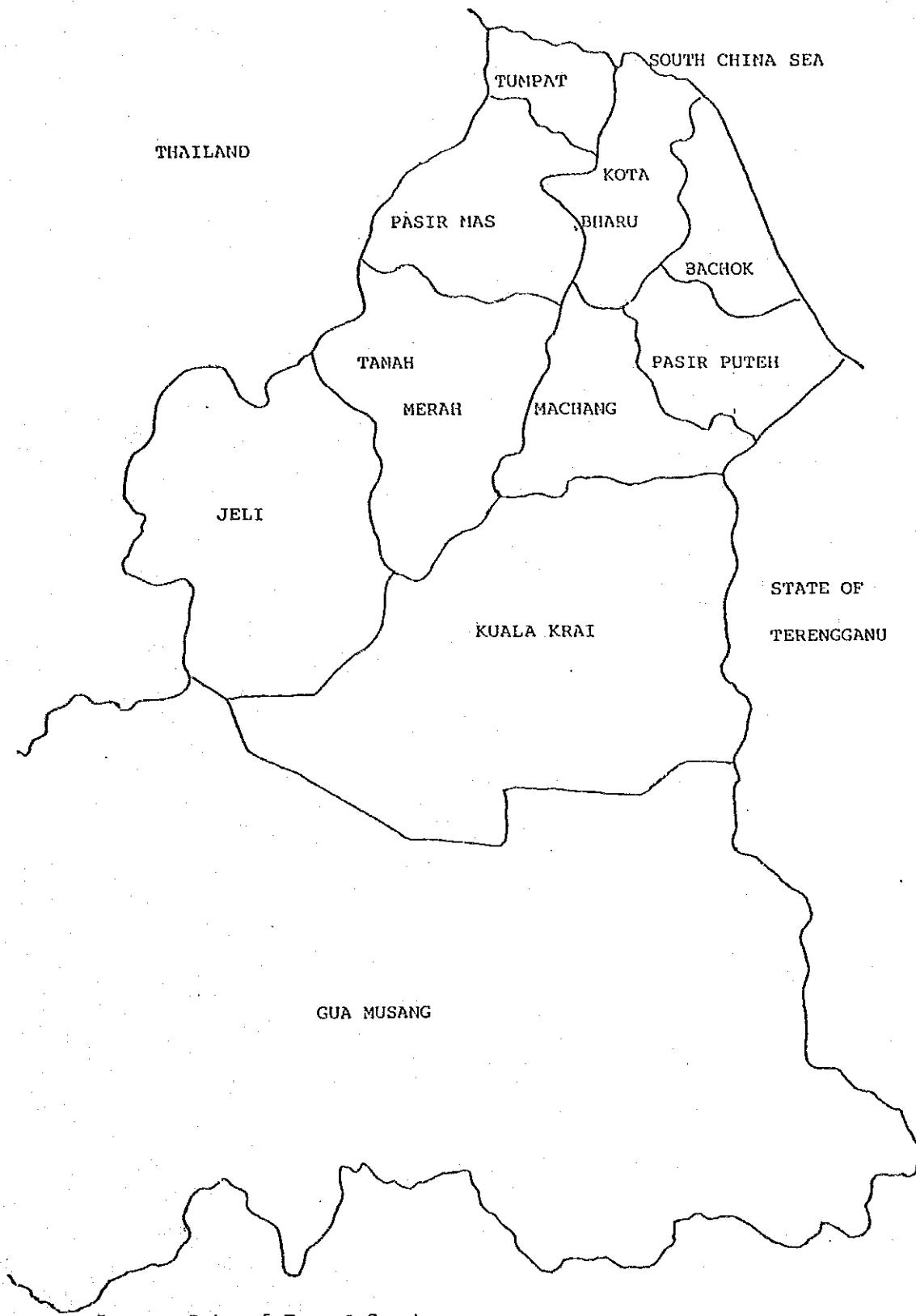
## ***FIGURES***











Source: Dpt. of Town & Country Planning, Kelantan

**Fig.2.3**  
**Districts of Kelantan**

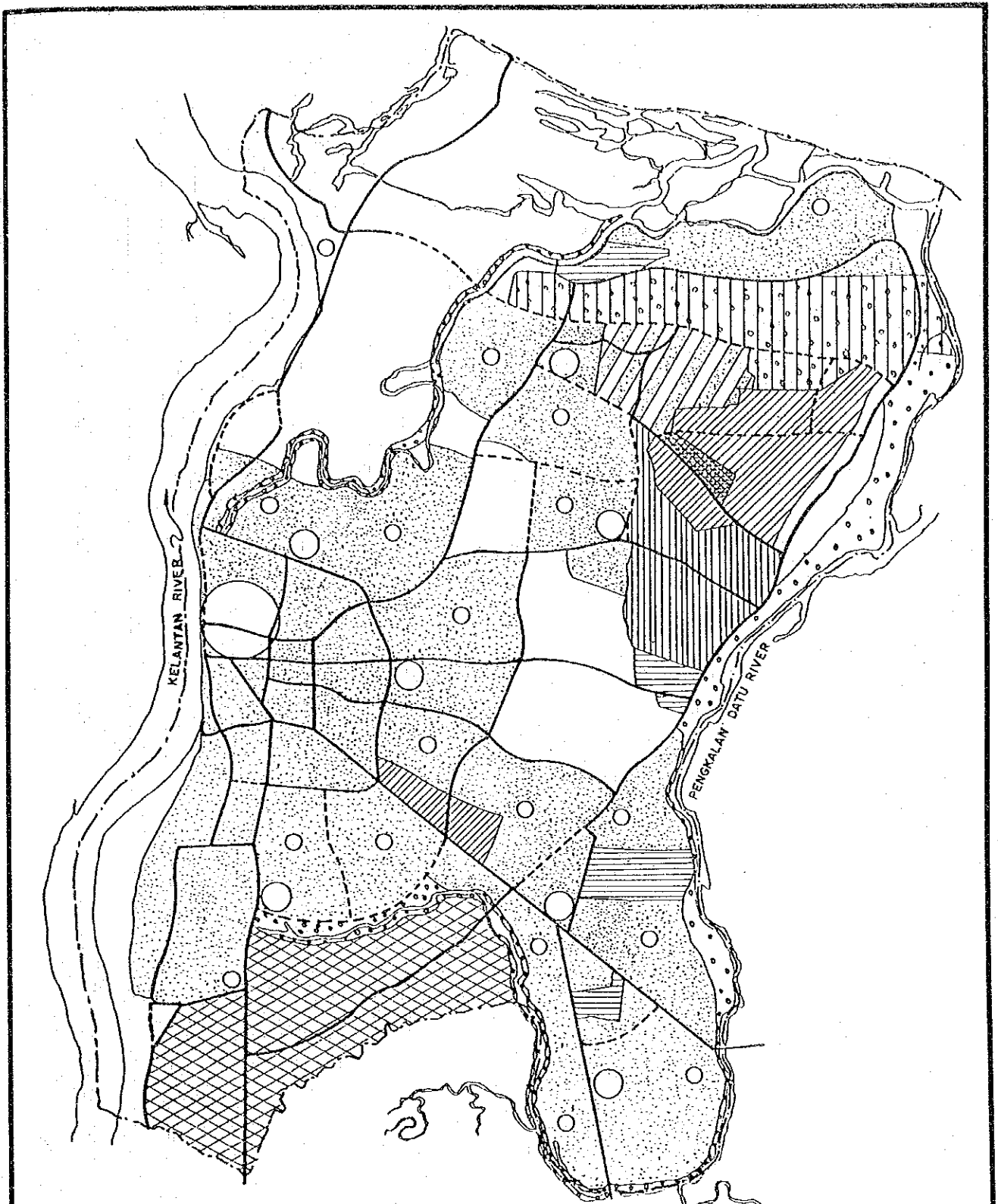
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**LEGEND:**

- |                                |                                  |
|--------------------------------|----------------------------------|
| ○ CENTRAL AREA                 | ⊙ FLOODWAY                       |
| ○ DISTRICT CENTRE              | ⊗ IRRIGATION SCHEME AREA         |
| ○ COMMUNITY CENTRE             | ✈ AIRPORT                        |
| ⊙ RESIDENTIAL AREA             | ▬ SPECIAL USE AREA               |
| ▨ INDUSTRIAL AREA              | ▨ RESIDENTIAL/INSTITUTIONAL AREA |
| ▬ FUTURE URBAN AREA            | — ROAD (--- PROPOSED)            |
| ▬ INDUSTRIAL ESTATE            | --- BOUNDARY OF MPKB             |
| ⊗ WORKER ACCOMODATION          | (KOTA BHARU MUNICIPAL COUNCIL)   |
| □ OPEN SPACE/AGRICULTURAL AREA |                                  |

**Fig.2.5**

**Development Plan of Majlis  
Perbandaran Kota Bharu**

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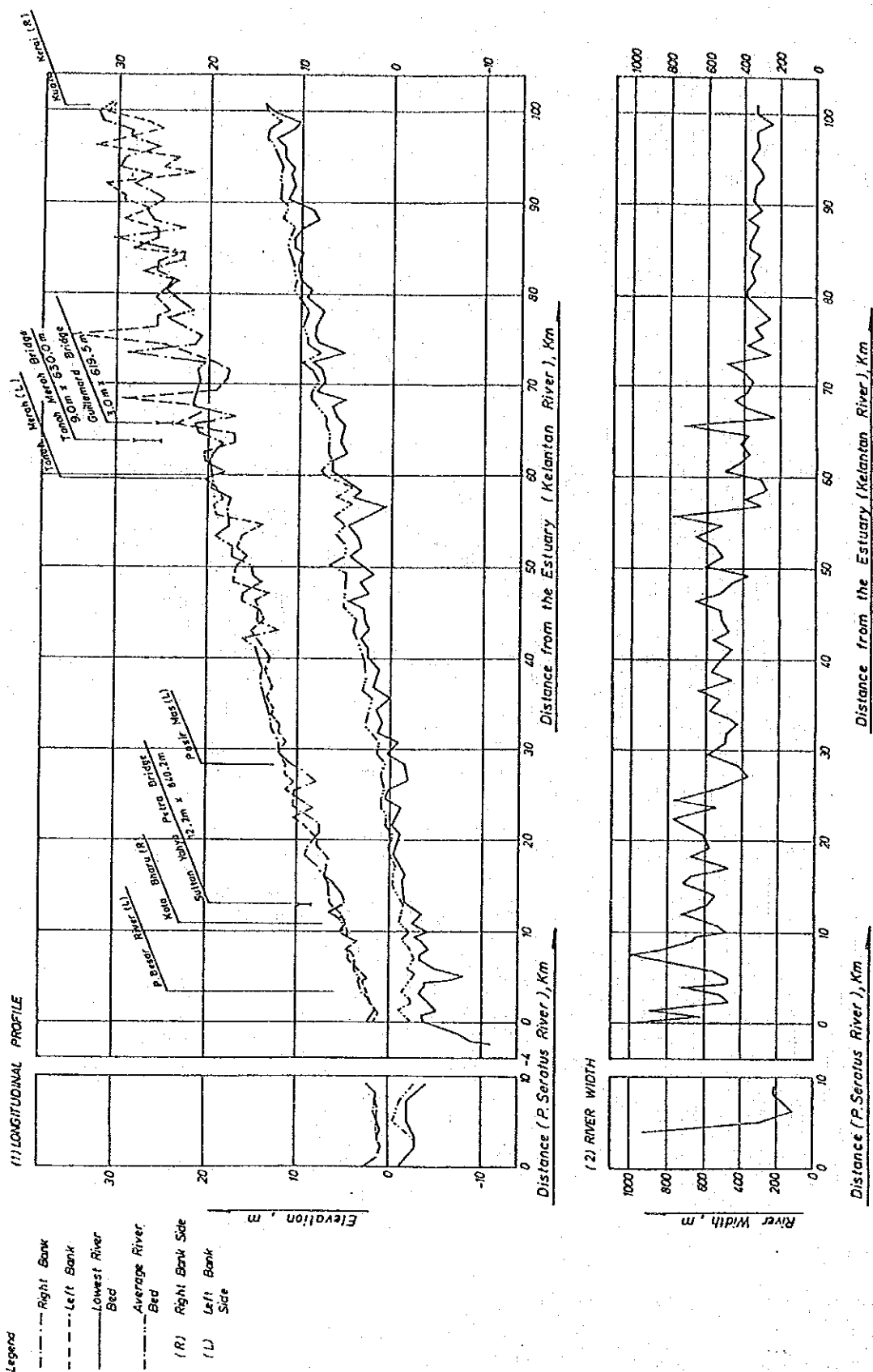


Fig.2.6

Characteristics of the Kelantan River

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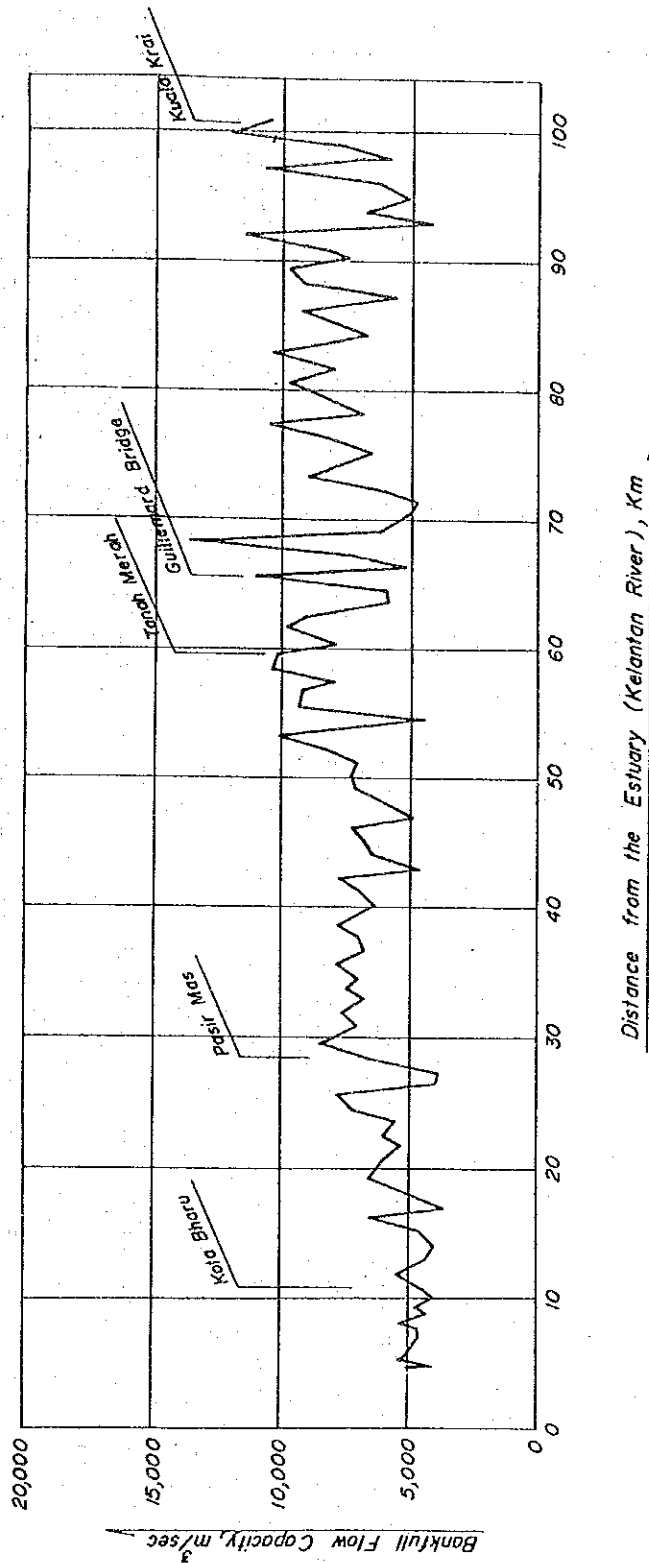


Fig.2.7

Bankfull Flow Capacity of the Kelantan River

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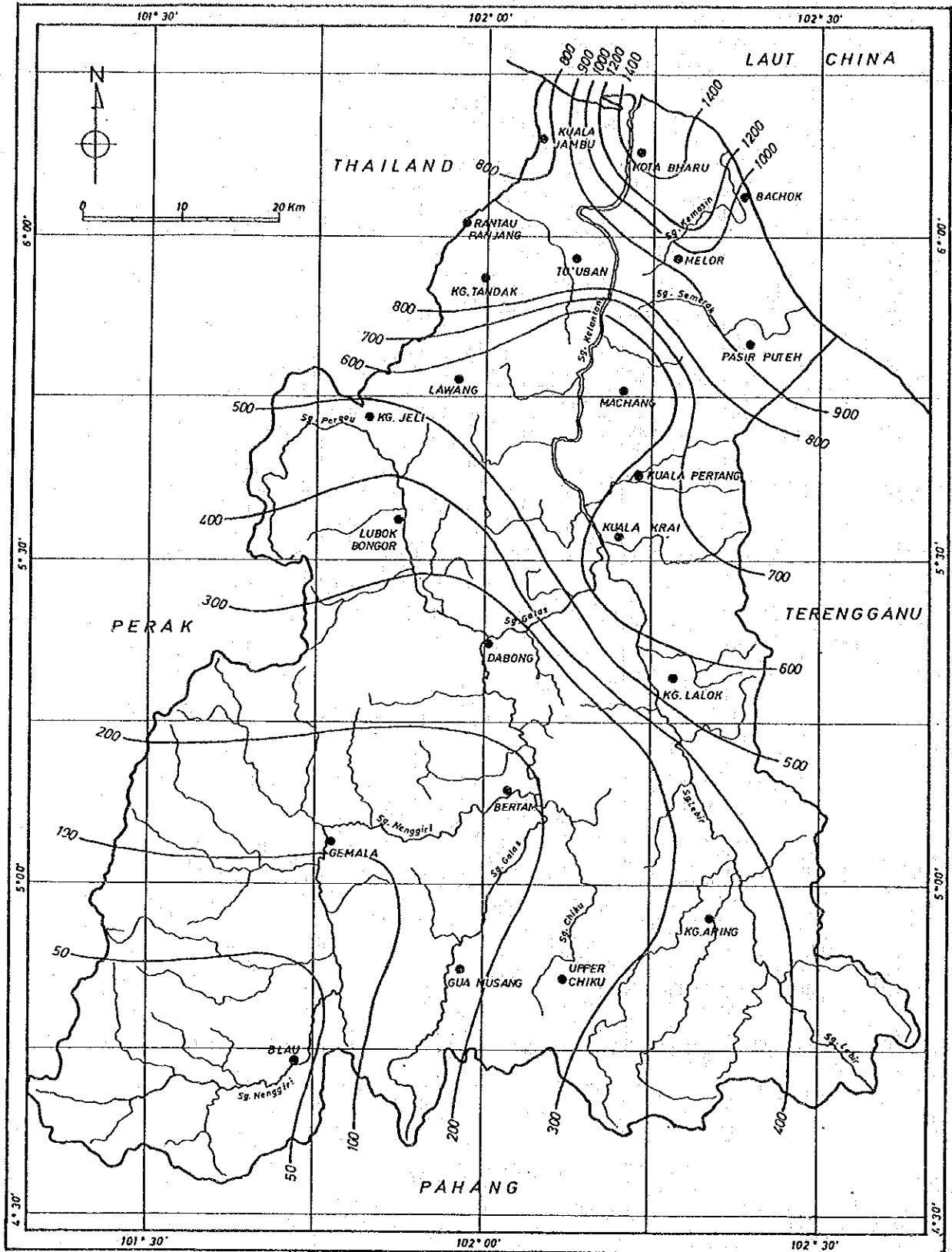









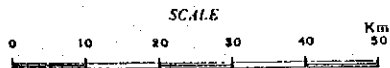
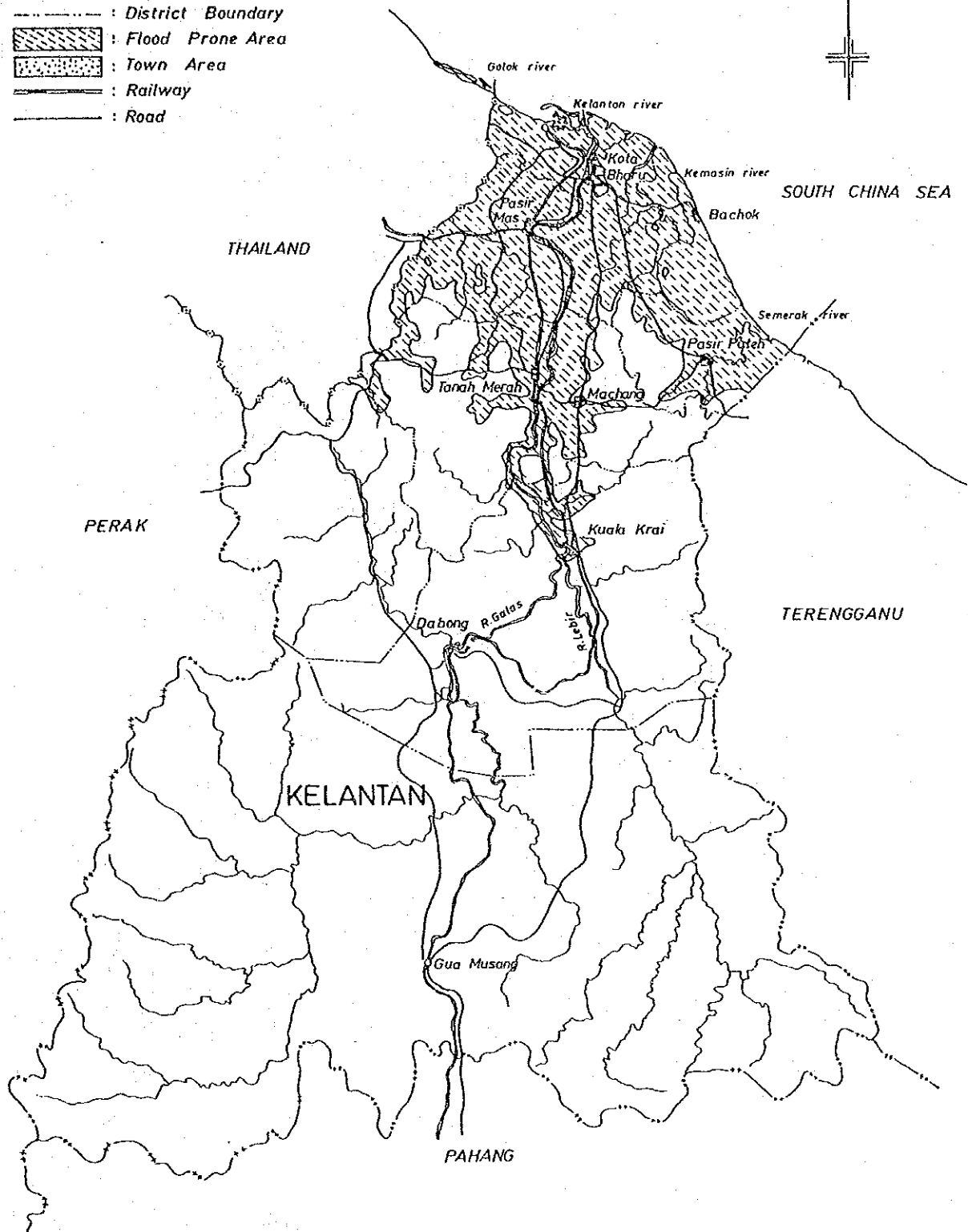
Fig.2.8

**Isohyetal Map of Monsoon Rainfall  
( Nov.26 to Dec.1, 1986 )**

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**LEGEND**

-  : International Boundary
-  : State Boundary
-  : District Boundary
-  : Flood Prone Area
-  : Town Area
-  : Railway
-  : Road



**Fig.2.9**

**Flooded Area by January 1967 Flood**

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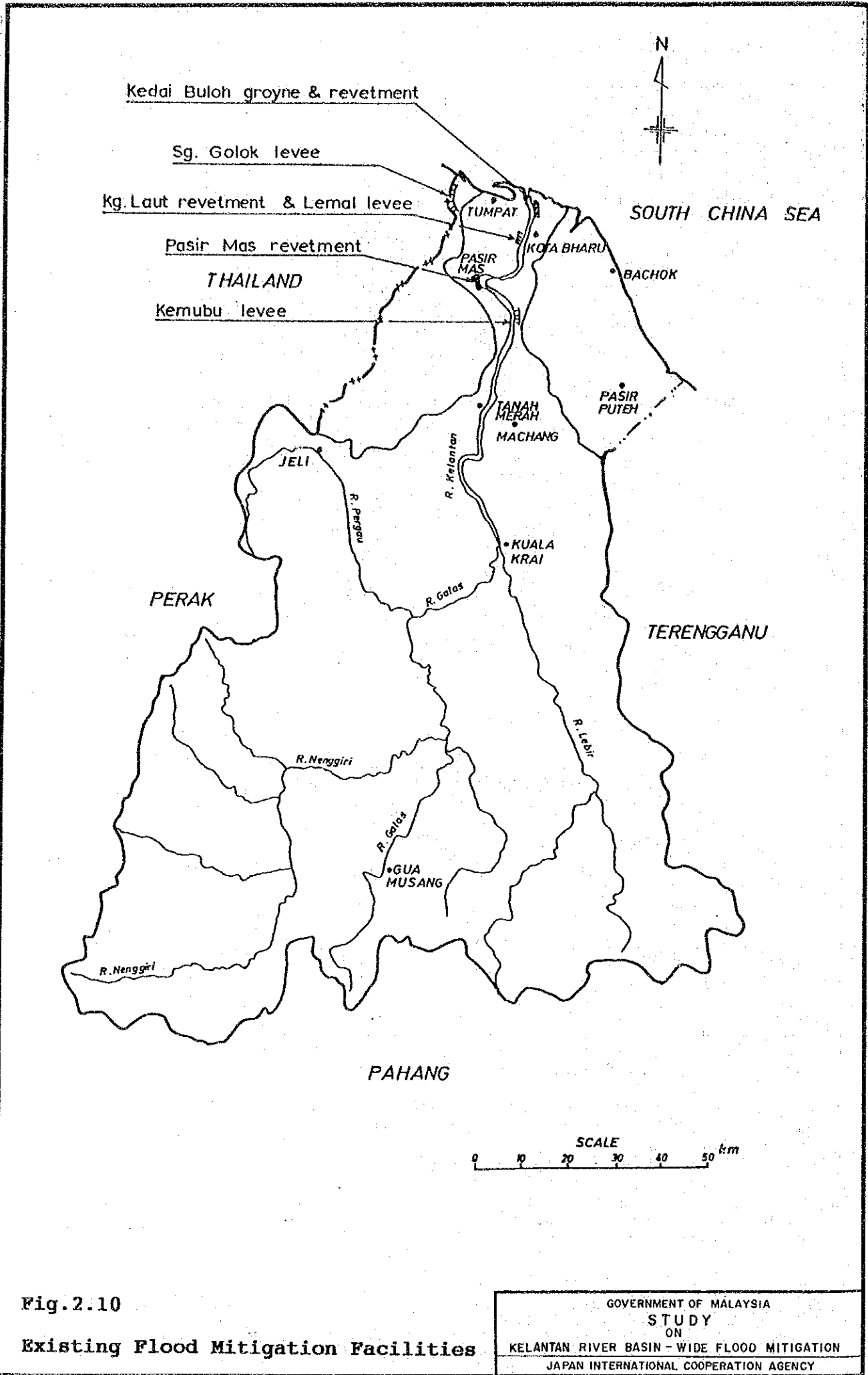


Fig. 2.10

Existing Flood Mitigation Facilities

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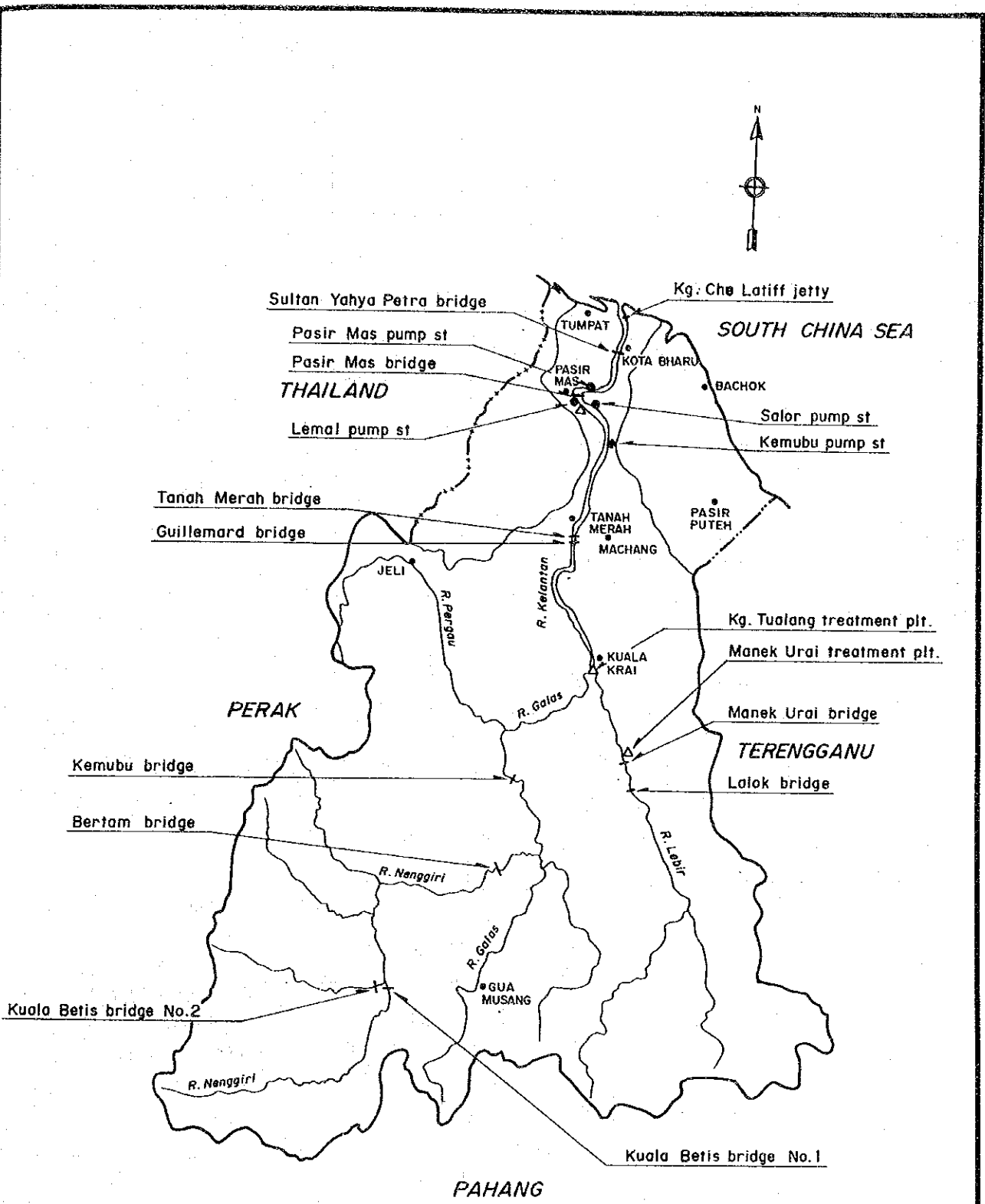


Fig.2.11

Existing Structures along the Kelantan River

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Rainfall / water level data from the telemetric stations ( 6 for rainfall and 7 for water level )

Telemetric system  
via a relay station  
at Bukit Bakar

D.I.D Flood operation room in Kota Bharu ( key station of telemetric system )

Water level forecast (Tank Model and correlation diagram of water levels)

Issue of flood warning by State Flood warning committes

LEVEL OF WARNING

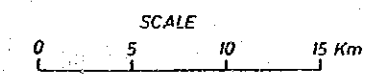
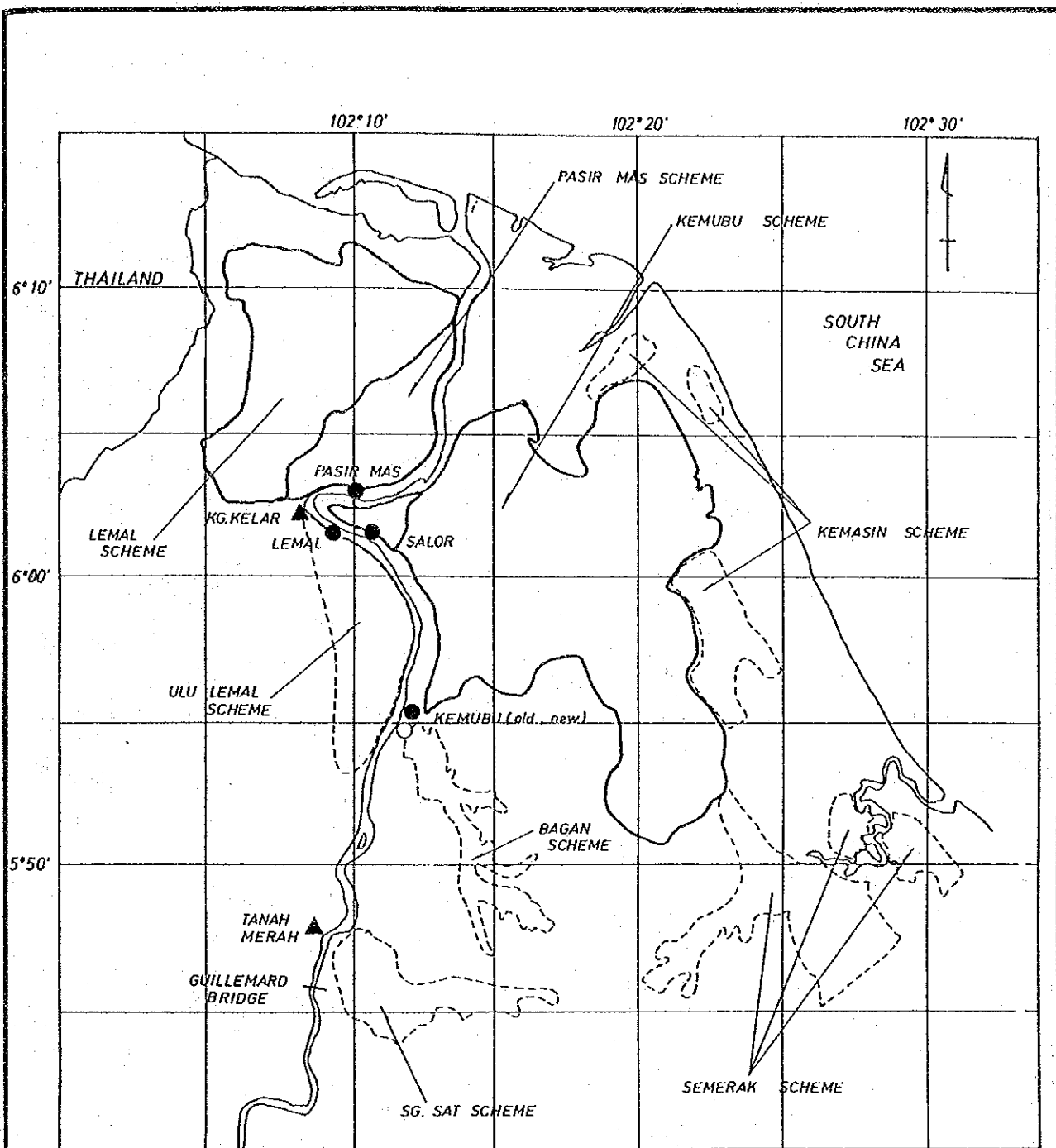
1. Alert (WL 65 feet at Kuala Krai )
2. Warning (WL 75 feet at Kuala Krai )
3. Emergency ( WL 85 feet at Kuala Krai )

Public warning to people affected with such media as TV, radio, loud-speaker and so on

Fig.2.12

Flood Forecasting and Warning System  
in the Entire Kelantan River Basin

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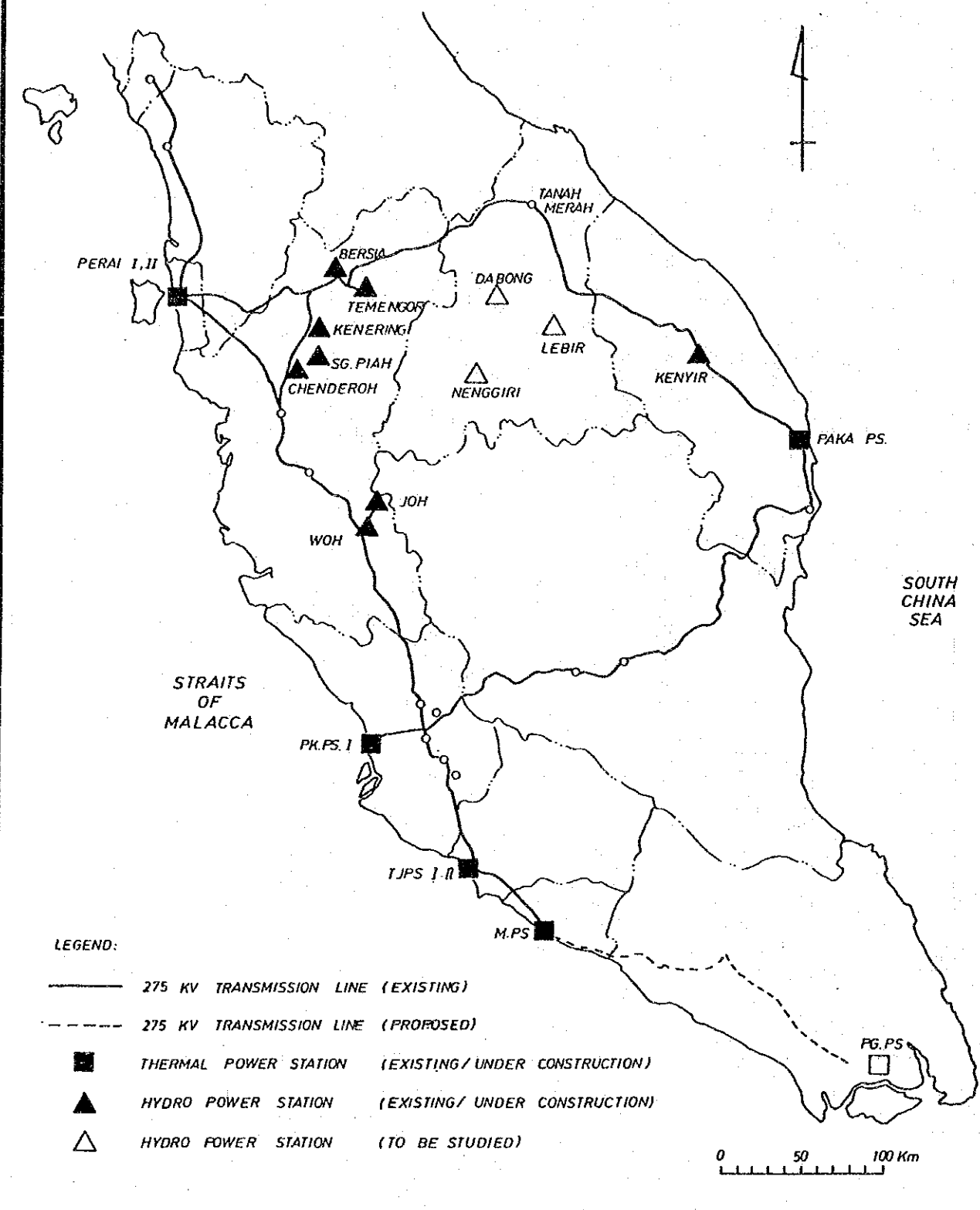


- LEGEND**
- Existing Pumping Station For Irrigation
  - Project Pumping Station For Irrigation
  - ▲ Existing Pumping Station For Domestic And Industrial Water
  - Boundary Of Existing Irrigation Area
  - - - Boundary Of Projected Irrigation Area

**Fig.3.1**

**Location of Pumping Station and Irrigation Area**

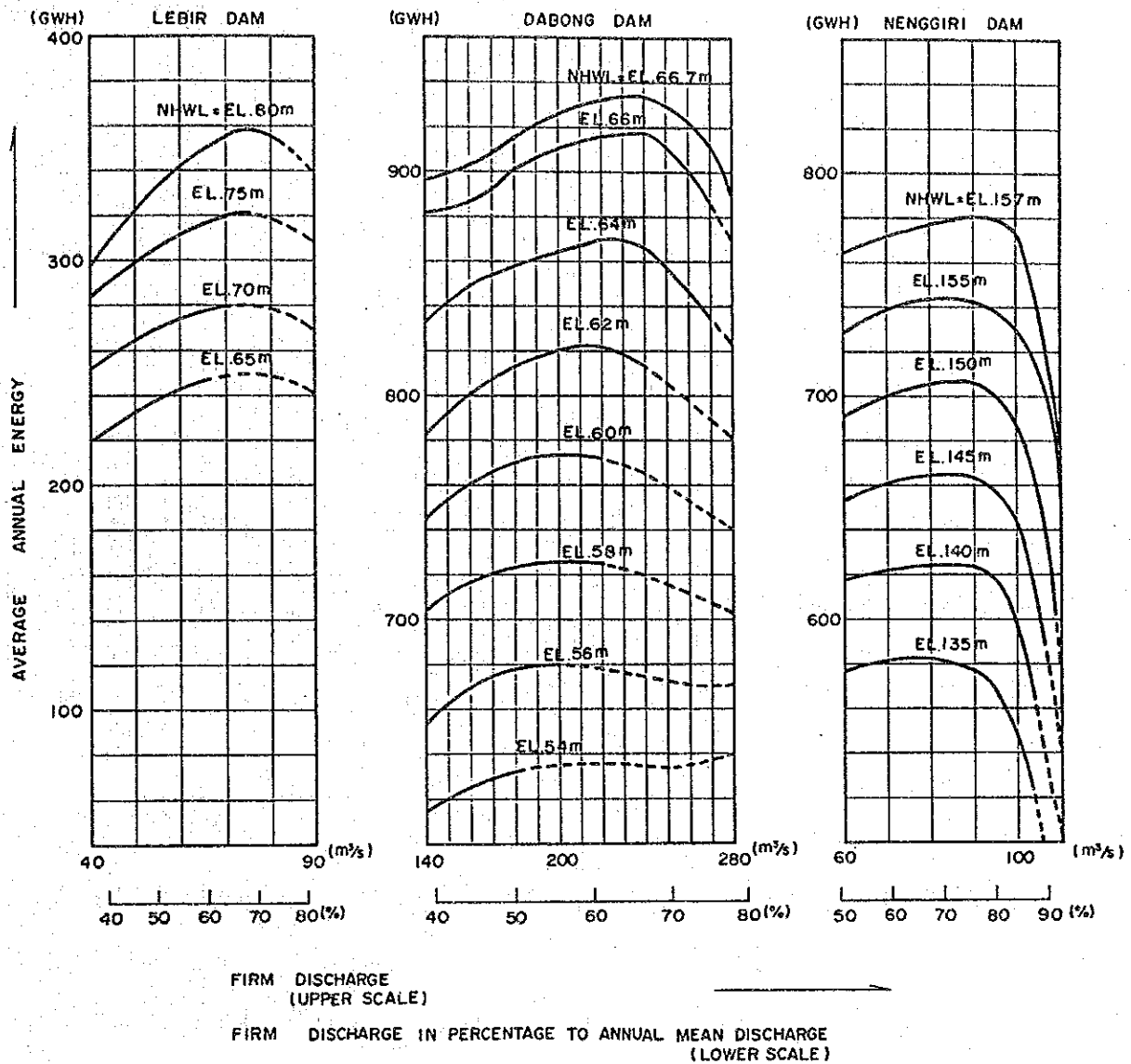
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- LEGEND:**
- 275 KV TRANSMISSION LINE (EXISTING)
  - - - - - 275 KV TRANSMISSION LINE (PROPOSED)
  - THERMAL POWER STATION (EXISTING/ UNDER CONSTRUCTION)
  - ▲ HYDRO POWER STATION (EXISTING/ UNDER CONSTRUCTION)
  - △ HYDRO POWER STATION (TO BE STUDIED)

**Fig.3.2**  
**National Grid Network of Power Supply**

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Note

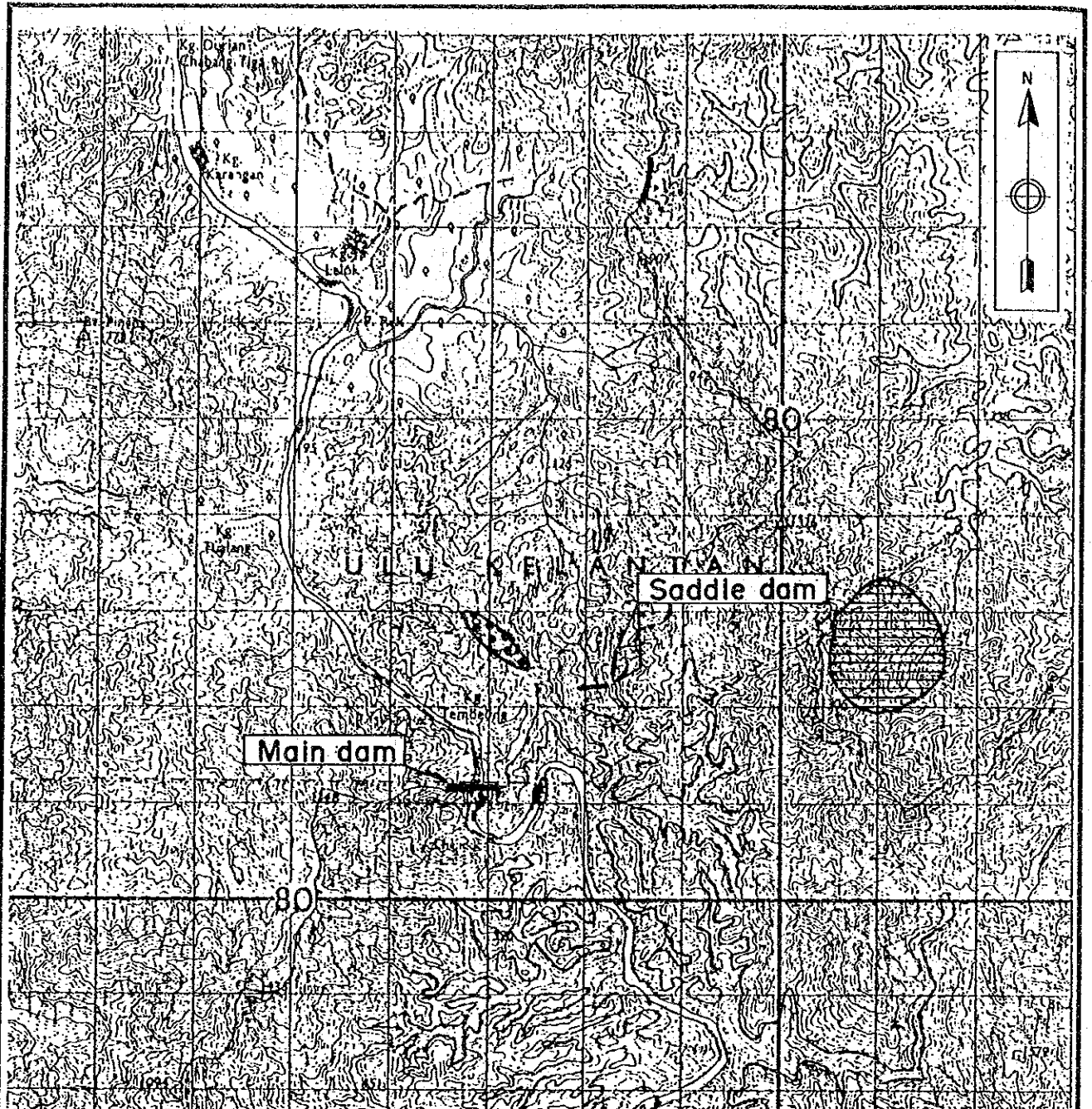
----- : Firm Discharge is not insured

Fig. 3.3





Average Annual Energy Generated by  
Alternative Normal High Water Level

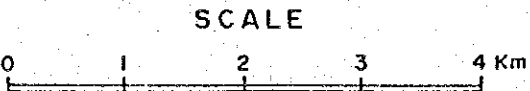
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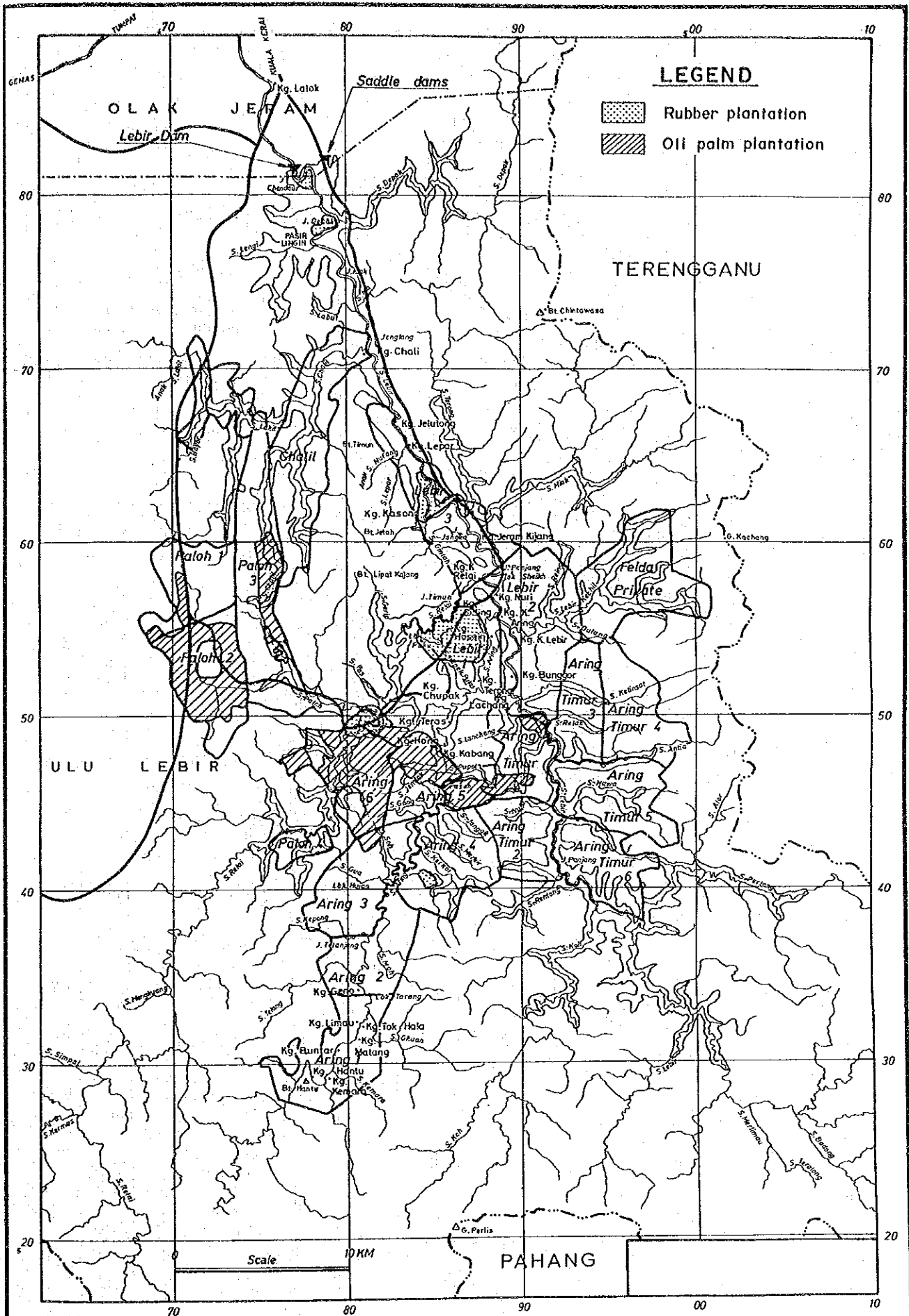
**LEGEND**

-  Damsite
-  Borrow Site (Core material)
-  Borrow Site (filter, Sand material)
-  Quarry Site (rock material concrete aggregate)



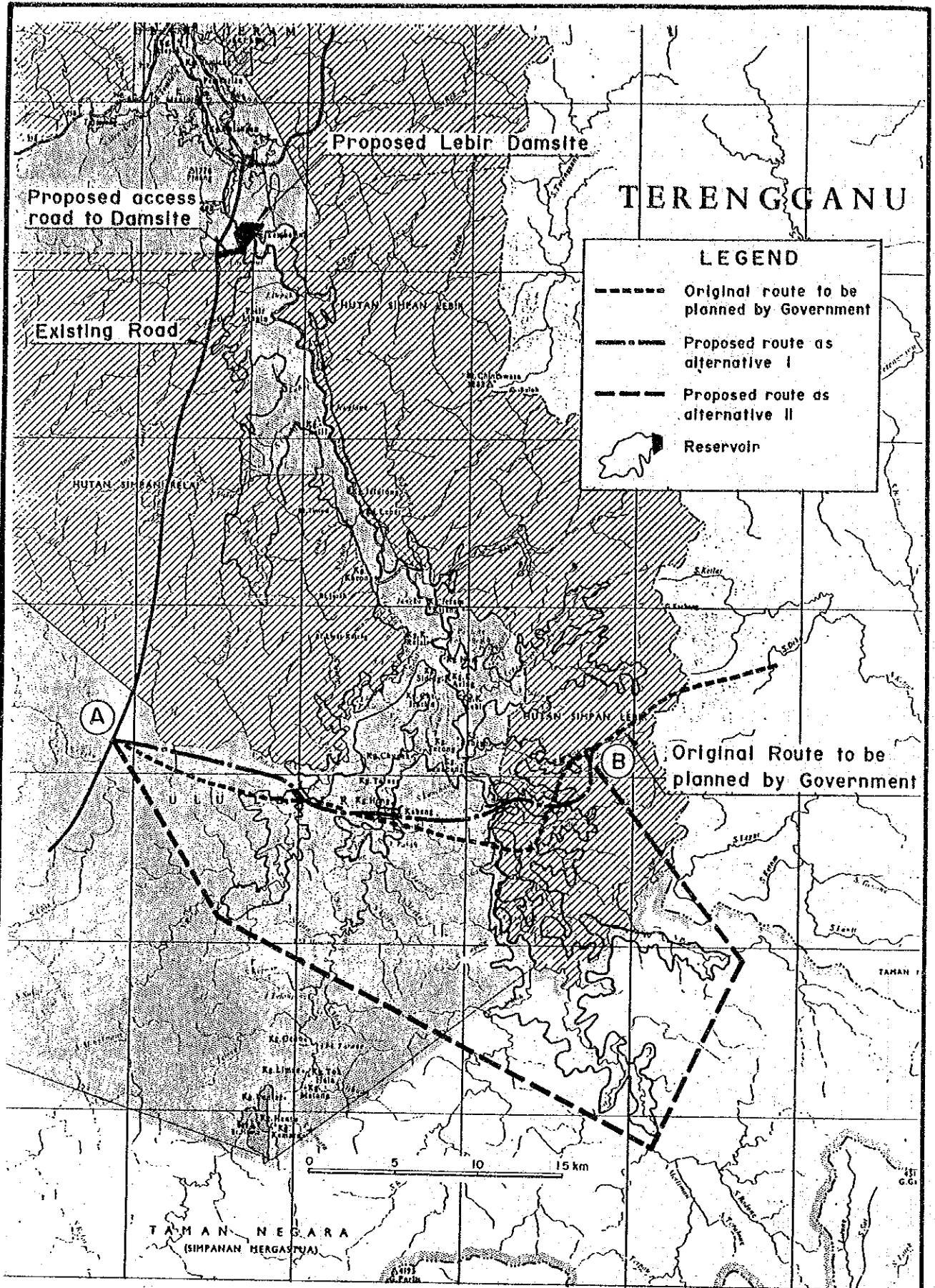
**Fig.3.4**  
**Location Map of Lebir Dam**

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**Fig.3.5**  
**Reservoir Area, Lebir Dam**

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**Fig.3.6**

**Re-planned Highway Route  
between Chiku and Kuala Brang**

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Area Km<sup>2</sup>

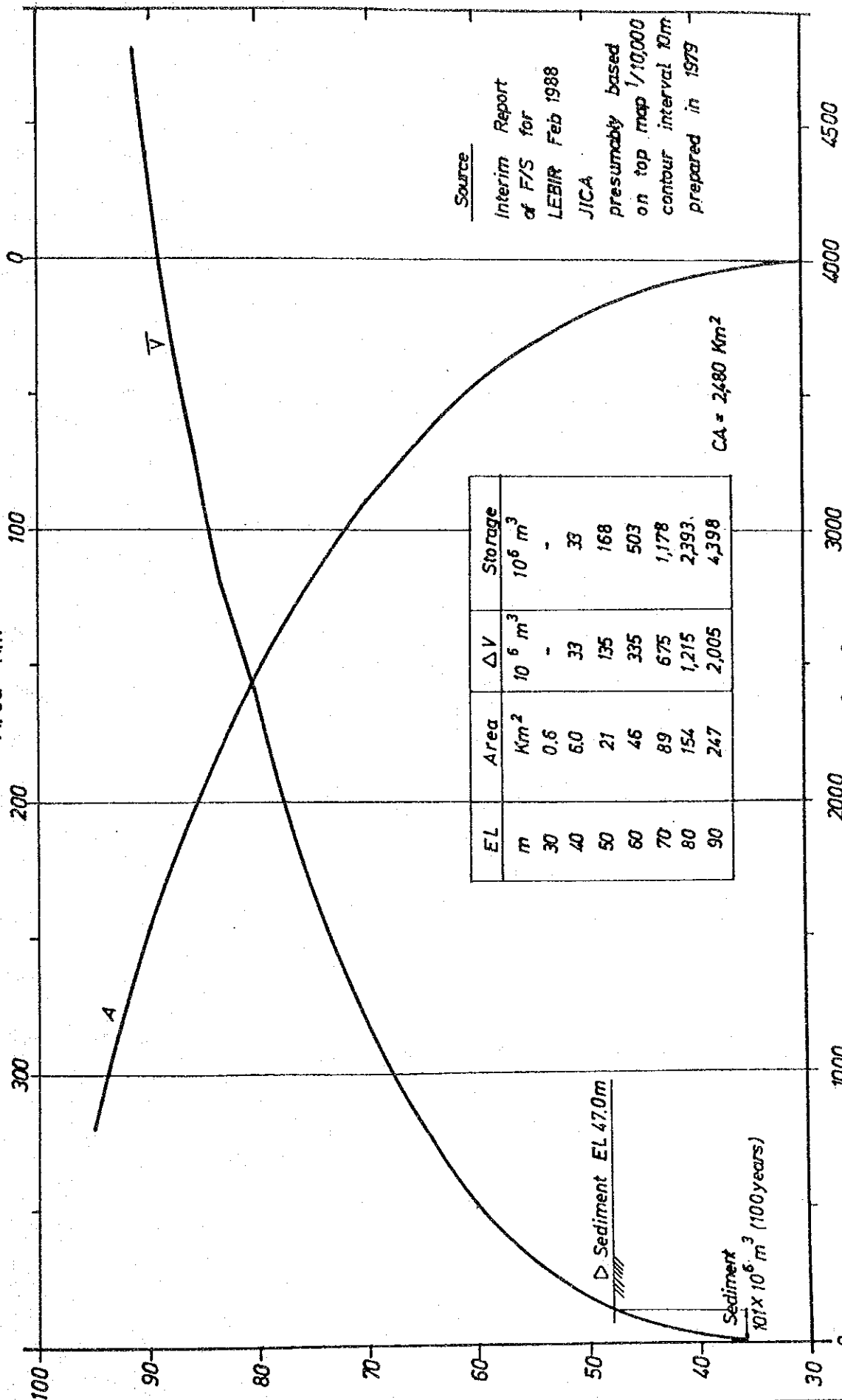


Fig. 3.7

Storage Capacity, Lebir Dam

EL m

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