

THE REPUBLIC OF PARAGUAY
THE FEASIBILITY STUDY
ON
THE INTEGRATED RURAL INFRASTRUCTURE
IMPROVEMENT PROJECT
IN
LA COLMENA
ANNEX

MAY 1989

JAPAN INTERNATIONAL COOPERATION AGENCY
(JICA)

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**ANNEX A METEOROLOGY
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ANNEX A Meteorology and Hydrology

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ANNEX A. Meteorology and Hydrology

A. 1 Meteorology

The meteorological stations around the study area are shown in Fig. A.1.1 and Table A.1.1. These stations were under the control of the Department of Meteorology, Ministry of National Defense. The meteorological records observed at the said stations are summarized in Table A.1.2 with the monthly basis. As for the rainfall record, there are data which have been observed by the Agricultural Cooperative in La Colmena since 1974. Application of these data will be concluded after verification of its accuracy by using the observed record on the neighboring stations because data at La Colmena were not registered by the Department of Meteorology.

Thiessen method is employed among the neighboring six meteorological stations to clarify the meteorological conditions in the study area. As shown in Fig. A.1.2, the study area situates within the sphere of influence in Villarrica and Carapegua. Correlation factor on the mean monthly and annual rainfall among La Colmena, Villarrica and Carapegua is as follows:

Station	La Colmena	Villarrica	Carapegua
La Colmena	-	0.3460 0.8829	0.2985 0.8954
Villarrica	0.3460 0.8829	-	0.6986 0.8880
Carapegua	0.2985 0.8954	0.6986 0.8880	-

- . Rainfall data employed from 1974 to 1987
- . Upper value shows correlation of annual rainfall
- . Lower value shows correlation of mean monthly rainfall

From the above Table, signification of correlation on the annual rainfall between Carapegua and Villarrica is obviously found, however, there is no signification of correlation between La Colmena and other stations.

On the other hand, high significant levels of correlation on the monthly rainfall can be found among the three stations. These results show that the distribution of annual rainfall for each month develops a same trend among the three stations, however, rainfall record of La Colmena could not be handled with other stations in view of total amount of annual rainfall.

Rainfall record at La Colmena is shown prominently larger values

than the other 2 stations when daily rainfall of more than 150 mm is compared with other stations.

Year	Carapegua	La Colmena	Villarrica
1974/ 5/23	71.5	-	26.8
24	112.3	246	87.4
25	10.0	13	8.6
26	2.0	-	0.7
27	-	-	0.4
Total	195.8 mm	259 mm	123.9 mm

1980/ 5/17	18.9	-	20.4
18	6.6	9	7.8
19	58.9	158	98.8
20	84.5	33	46.9
21	2.4	14	-
22	-	20	-
Total	195.8 mm	234 mm	173.9 mm

1982/11/25	52.4	-	-
26	66.4	98	12.6
27	51.2	50	92.9
28	6.3	163	12.6
29	-	-	71.4
Total	176.3 mm	311 mm	189.5 mm

As shown in Table A.1.1, mean annual rainfall during the observed period is as follows:

Carapegua : 1,619 mm
 La Colmena : 1,755 mm
 Villarrica : 1,592 mm

Mean annual rainfall of La Colmena is also shown larger values than 1,600 mm of annual isohyetal graph in "Mapa Hidrogeologico" which has been published in 1986.

Topographical conditions around the presaid observation stations are

- Elevation of each station varies from 100 to 150 m above mean sea water level
- Distance interval of each station is every 40 km from west to east
- A mountain which elevation is ranging from 400 m to 500 m exists behind the all stations in south and/or east.

It is considered that there are no particular conditions to occur the difference of daily or annual rainfall among the three stations.

Rainfall record to be used for the project will be decided taking the following view points into account:

- Conservative values should be employed for estimation of water requirement
- Long term record is preferable for the analysis.

Therefore, areal rainfall estimated by the area ratio of Villarrica and Carapegua on the basis of Thiessen Polygon is used for the analysis instead of the data on La Colmena.

The missing data of Carapegua station is complemented by the following regression equation constructed on the basis of monthly rainfall of Villarrica:

$$y = 1.846x - 11.7628$$

y: Carapegua daily rainfall
x: Villarrica daily rainfall

Areal rainfall is estimated using the following equation based on the area ratio of 0.33 for Carapegua and of 0.67 for Villarrica:

$$R_a = 0.67 R_v + 0.33 R_c$$

R_a: Daily rainfall in study area, mm
R_v: Daily rainfall in Villarrica, mm
R_c: Daily rainfall in Carapegua, mm

A. 2 Hydrology

The hydrological stations in Paraguay are shown in Fig. A.1.1 together with the meteorological stations. Due to the lack of hydrological data available in and around the study area, new automatic water level gauging stations were established by the project at the three major streams named Rory-Mi, Rory and Tranquera in the study area. River basin diagram in and around the study area and locations of new gauging stations are shown in Tables A.2.1 and A.2.2, respectively.

To estimate the discharge available of pre-said three major streams, runoff analysis is carried out by using the water level record on its commanded streams. The tank model method is employed for analysis because low water discharge is mainly required for the project formulation. The parameters on the model are constructed on the stream basis with the discharge record, daily rainfall and water level record

obtained from September to November, 1988. Data employed for calculation and its computed results are shown in Tables A.2.1 to A.2.3 and illustrated in Fig. A.2.3, respectively.

Calculations on each stream model were performed on the daily basis and presented high co-relation with its observing results as follows :

	co-relation factor
Tranquera	0.9833
Rory	0.9809
Rory-MI	0.9876

Consequently, parameters of the models will be concluded as shown in Fig. A.2.4.

To clarify the stream regimen in the study area, runoff analysis of pre-said three major streams with the tank model is carried out on the daily basis at the duration from 1978 to 1987. Low water flow in each year, thus, estimated and summarized in main report.

A. 3 Water Quality

Water quality analysis is executed by the INTN (INSTITUTO NACIONAL DE TECNOLOGIA Y NORMALIZACION) during the study period at the streams, wells and pond in the study area. Location and number of samples is as follows:

No.	Location	Sample
1.	Tranquera middle stream reach	1
2.	Rory middle stream reach	1
3.	Rory-MI middle stream reach	1
4.	Rory-MI upstream reach	1
5.	Rory upstream reach	1
6.	Tranquera upstream reach	1
7.	Shallow well at Miyamoto,s house	1
8.	Deep well at Miyamoto,s house	1
9.	Pond near the Miyamoto,s house	1

Physical and chemical analysis were performed on the abovementioned samples. Items of analysis and its results are shown in Table A.3.1.

Analyzed results show that the water can be used as the drinking water with the water quality criteria in Paraguay, however, colon bacilli were detected in some samples though values show in allowable range. With these, utilization of these water sources will be made after chlorination.

Table A.1.1 Meteorological Stations Around the Study Area

No.	Station	Coordination			Duration	Remarks
		Lat.	Lon.	EL.		
1	Asuncion					
	Sajonia	25° 16'	57° 38'	63	1881 - 1970	
	Aeropuerto	25° 15'	57° 31'	85	1971 - present	
2	Paraguari	25° 36'	57° 09'	-	1981 - present	
3	Carapegua	25° 48'	57° 14'	116	1970 - present	
4	Villarrica	25° 45'	56° 26'	161	1941 - present	
5	San Juan B. Mios	26° 40'	57° 09'	126	1956 - present	
6	San Lorenzo	25° 22'	57° 34'	120	1957 - 1981	
7	Caacupe	25° 24'	57° 06'	228	1961 - present	
8	Barrerito-Caapucu	26° 17'	57° 03'	-	1975 - 1979	
9	Caazapa	26° 11'	56° 22'	-	1973 - present	

10	Eusebio Ayala	25° 26'	56° 58'	-	1937 - 1949	Rainfall only
11	San Bernardino	25° 18'	57° 17'	-	1940 - 1951	"
12	Altos	25° 16'	57° 14'	-	1940 - 1949	"
13	San Salvador	25° 28'	56° 54'	-	1940 - 1968	"
14	Col. Carlos Pfamul	25° 28'	56° 55'	-	1940 - 1966	"
15	Maciel	26° 08'	56° 30'	-	1940 - 1968	"
16	Isla Saca	26° 28'	56° 24'	-	1940 - 1958	"
17	Sapucaí	25° 39'	56° 56'	-	1940 - 1968	"
18	Yaguaron	25° 34'	57° 17'	-	1940 - 1949	"
19	Guarambare	25° 28'	57° 28'	-	1972 - 1976	"
20	Copiata	25° 16'	57° 30'	-	1940 - 1976	"

Note: 1 - 9 Registered meteorological stations
 10 - 20 Registered rainfall observation stations

Table A.1.2 Mean Monthly Values on Each Meteorological Stations

RAINFALL RECORD				MEAN MONTHLY									UNIT: mm
STATION	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.	TOTAL
VILLARRICA	162	126	158	160	136	113	78	80	108	174	164	133	1592
CARAPEGUA	162	124	137	195	140	106	62	89	77	130	207	145	1619
PARAGUARI	156	144	81	220	86	81	63	72	59	103	218	130	1455
CAACUPE	176	144	143	174	123	95	55	74	99	146	200	162	1590
SAN JUAN B.M.	162	146	146	155	126	104	77	79	100	177	161	134	1607
CAAZAPA	143	132	114	199	138	80	89	85	118	156	192	106	1533
LA COLMENA	172	148	127	198	162	103	60	119	97	159	250	161	1755

RELATIVE HUMIDITY RECORD				MEAN MONTHLY									UNIT: %
STATION	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.	AVE.
VILLARRICA	73	76	77	83	85	84	80	75	71	72	73	73	75
CARAPEGUA	71	74	76	83	83	80	76	75	71	71	70	70	74
PARAGUARI	77	79	78	82	80	81	77	72	79	74	79	77	78
CAACUPE	64	64	65	70	78	77	74	68	64	62	62	59	63
SAN JUAN B.M.	67	70	72	74	75	76	74	71	69	67	67	64	71
CAAZAPA	66	66	69	75	79	80	78	78	77	71	68	66	73

TEMPERATURE RECORD				MEAN MONTHLY									UNIT: °C
STATION	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.	AVE.
VILLARRICA	26.4	26.0	24.6	22.9	20.1	17.8	18.6	17.8	19.5	22.0	23.7	25.4	21.7
CARAPEGUA	27.3	26.8	25.3	22.1	19.3	17.0	17.7	18.0	23.1	22.5	24.4	26.1	22.5
PARAGUARI	27.1	26.6	25.0	22.6	21.6	17.1	18.2	18.5	19.9	23.0	24.9	26.0	22.5
CAACUPE	26.6	26.3	25.0	21.3	19.8	17.4	17.6	18.6	20.2	22.8	24.3	26.1	22.0
SAN JUAN B.M.	27.0	26.5	24.9	21.6	18.2	16.6	16.8	17.4	19.5	22.3	24.2	26.4	21.8
CAAZAPA	25.7	25.5	24.1	20.5	18.1	15.7	16.8	16.8	18.3	21.5	22.8	24.8	20.9

SUNSHINE RECORD				MEAN MONTHLY									UNIT: Hr
STATION	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.	AVE.
VILLARRICA	251	229	226	220	199	163	190	200	181	223	261	257	259
CAACUPE	253	240	228	218	195	171	180	181	185	206	213	243	210
SAN JUAN B.M.	255	226	231	185	179	168	162	175	205	221	246	249	275

EVAPORATION RECORD				MEAN MONTHLY									UNIT: MM
STATION	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.	AVE.
VILLARRICA	82	68	67	50	47	48	65	77	75	75	77	87	817
CARAPEGUA	104	102	85	75	64	52	71	85	101	116	113	112	1057
CAACUPE	156	130	124	110	108	102	126	149	163	177	168	174	1731 (PICHE)
SAN JUAN B.M.	102	79	83	62	55	54	70	85	90	95	101	111	987

Table A.2.1 Results of Runoff Calculation on Rory-Mi

RORY-MI TANK MODEL

DATE	STRAINING RAINFALL		STRAINING		STRAINING		STRAINING		STRAINING		OUTFLOW TOTAL	DISCHARGE m3/sec	OBSERVA- TION m3/sec	DIFFER- ENCE m3/sec
	PREVIOUS DAY	PREVIOUS mm	PREVIOUS DAY	PREVIOUS mm	PREVIOUS DAY	PREVIOUS mm	PREVIOUS DAY	PREVIOUS mm	mm	mm				
9/20	0.000	0.000	20.000	100.000	400.000	427.729	1.252	0.041	0.040	0.001				
21	0.000	0.000	14.630	71.735	427.729	446.602	1.188	0.038	0.038	0.000				
22	0.000	0.000	10.141	51.415	446.602	459.087	1.131	0.037	0.038	-0.001				
23	0.000	0.000	6.388	36.740	459.087	466.945	1.079	0.035	0.038	-0.003				
24	0.000	0.000	3.250	26.089	466.945	471.434	1.034	0.034	0.040	-0.006				
25	0.000	0.000	0.627	18.315	471.434	473.542	1.020	0.033	0.038	-0.005				
26	0.000	0.000	0.000	12.802	473.542	473.994	1.015	0.033	0.038	-0.005				
27	0.000	0.000	0.000	8.949	473.994	473.294	1.010	0.033	0.039	-0.006				
28	0.000	0.000	0.000	6.255	473.294	471.797	1.004	0.033	0.037	-0.004				
29	0.000	0.000	0.000	4.372	471.797	469.749	0.998	0.032	0.037	-0.005				
30	0.000	0.000	0.000	3.056	469.749	467.324	0.991	0.032	0.036	-0.004				
10/1	0.000	0.000	0.000	2.136	467.324	464.643	0.985	0.032	0.035	-0.003				
2	0.000	0.000	0.000	1.493	464.643	461.788	0.978	0.032	0.035	-0.003				
3	0.000	16.000	0.000	1.044	461.788	459.018	1.663	0.054	0.051	0.003				
4	9.661	0.000	3.687	1.192	459.018	456.374	1.049	0.034	0.036	-0.002				
5	5.013	0.000	4.878	1.445	456.374	453.816	1.041	0.034	0.034	0.000				
6	1.759	0.000	4.708	1.601	453.816	451.170	0.987	0.032	0.034	-0.002				
7	0.000	0.000	1.846	1.350	451.170	448.369	0.950	0.031	0.034	-0.003				
8	0.000	0.000	0.000	0.944	448.369	445.466	0.943	0.031	0.033	-0.002				
9	0.000	0.000	0.000	0.660	445.466	442.500	0.937	0.030	0.033	-0.003				
10	0.000	0.000	0.000	0.461	442.500	439.496	0.930	0.030	0.033	-0.003				
11	0.000	57.000	0.000	0.322	439.496	437.218	10.185	0.330	0.320	0.010				
12	29.966	0.000	13.970	1.977	437.218	435.695	4.037	0.131	0.050	0.081				
13	16.427	0.000	18.567	3.711	435.695	434.723	1.743	0.056	0.038	0.018				
14	9.251	0.000	19.015	4.979	434.723	434.060	1.221	0.040	0.035	0.005				
15	4.726	0.000	17.590	5.686	434.060	433.487	1.181	0.038	0.034	0.004				
16	1.558	0.000	15.263	5.889	433.487	432.734	1.102	0.036	0.032	0.004				
17	0.000	0.000	10.670	5.455	432.734	431.651	1.034	0.034	0.032	0.002				
18	0.000	0.000	6.830	4.669	431.651	430.171	0.976	0.032	0.031	0.001				
19	0.000	32.000	3.620	3.718	430.171	428.798	4.441	0.144	0.120	0.024				
20	18.141	0.000	10.726	3.944	428.798	427.617	1.915	0.062	0.055	0.007				
21	10.160	0.000	12.890	4.373	427.617	426.562	1.121	0.036	0.040	-0.004				
22	5.362	0.000	12.697	4.649	426.562	425.524	1.096	0.036	0.036	0.000				
23	2.003	13.000	11.332	4.671	425.524	424.584	1.581	0.051	0.035	0.016				
24	9.133	0.000	12.911	4.884	424.584	423.690	1.112	0.036	0.027	0.009				
25	4.643	11.000	12.457	4.977	423.690	422.913	1.705	0.055	0.050	0.005				
26	9.472	0.000	14.011	5.236	422.913	422.190	1.126	0.036	0.035	0.001				
27	4.880	0.000	13.462	5.348	422.190	421.419	1.097	0.036	0.032	0.004				
28	1.666	0.000	11.851	5.225	421.419	420.401	1.027	0.033	0.032	0.001				
29	0.000	0.000	7.817	4.633	420.401	419.033	0.966	0.031	0.033	-0.002				
30	0.000	0.000	4.445	3.796	419.033	417.276	0.914	0.030	0.031	-0.001				
31	0.000	115.000	1.626	2.857	417.276	416.761	34.840	1.129	0.925	0.204				
11/1	46.206	0.000	29.809	5.744	416.761	417.431	7.769	0.252	0.220	0.032				
2	25.034	0.000	35.169	8.515	417.431	418.877	4.072	0.132	0.055	0.077				
3	13.813	0.000	34.429	10.349	418.877	420.675	1.743	0.056	0.035	0.021				
4	7.866	0.000	31.366	11.199	420.675	422.498	1.371	0.044	0.030	0.014				
5	3.756	0.000	27.568	11.286	422.498	424.108	1.304	0.042	0.028	0.014				
6	0.879	0.000	23.362	10.819	424.108	425.252	1.205	0.039	0.028	0.011				
7	0.000	0.000	17.440	9.750	425.252	425.804	1.122	0.036	0.027	0.009				
8	0.000	0.000	12.490	8.382	425.804	425.724	1.050	0.034	0.026	0.008				
9	0.000	0.000	8.352	6.906	425.724	425.020	0.989	0.032	0.025	0.007				
10	0.000	0.000	4.892	5.441	425.020	423.730	0.935	0.030	0.023	0.007				

Table A.2.2 Results of Runoff Calculation on Rory

RORY TANK MODEL

DATE	STRAGING RAINFALL		STRAGING PREVIOUS		STRAGING PREVIOUS		STRAGING PREVIOUS		OUTFLOW TOTAL		DISCHARGE	OBSERVA-	DIFFER-
	DAY	mm	DAY	mm	DAY	mm	DAY	mm	mm	mm	m3/sec	TION	ENCE
												m3/sec	m3/sec
9/20	0.000	0.000	0.000	100.000	300.000	328.746	0.579	0.078	0.080	-0.002			
21	0.000	0.000	0.000	69.850	328.746	348.372	0.559	0.075	0.078	-0.003			
22	0.000	0.000	0.000	48.790	348.372	361.630	0.545	0.073	0.076	-0.003			
23	0.000	0.000	0.000	34.080	361.630	370.441	0.535	0.072	0.075	-0.003			
24	0.000	0.000	0.000	23.805	370.441	376.147	0.527	0.071	0.077	-0.006			
25	0.000	0.000	0.000	16.628	376.147	379.687	0.520	0.070	0.075	-0.005			
26	0.000	0.000	0.000	11.614	379.687	381.716	0.516	0.069	0.076	-0.007			
27	0.000	0.000	0.000	8.113	381.716	382.690	0.512	0.069	0.078	-0.009			
28	0.000	0.000	0.000	5.667	382.690	382.929	0.508	0.068	0.078	-0.010			
29	0.000	0.000	0.000	3.958	382.929	382.657	0.505	0.068	0.076	-0.008			
30	0.000	0.000	0.000	2.765	382.657	382.029	0.503	0.067	0.075	-0.008			
10/1	0.000	0.000	0.000	1.931	382.029	381.154	0.500	0.067	0.065	0.002			
2	0.000	0.000	0.000	1.349	381.154	380.109	0.498	0.067	0.067	0.000			
3	0.000	16.000	0.000	0.942	380.109	379.408	0.986	0.132	0.140	-0.008			
4	9.957	0.600	2.712	1.736	379.408	379.003	0.673	0.090	0.085	0.005			
5	5.220	0.000	3.044	2.423	379.003	378.690	0.635	0.085	0.078	0.007			
6	1.904	0.000	2.374	2.636	378.690	378.039	0.497	0.067	0.073	-0.006			
7	0.000	0.000	0.000	1.841	378.039	377.153	0.495	0.066	0.073	-0.007			
8	0.000	0.000	0.000	1.286	377.153	376.104	0.493	0.066	0.070	-0.004			
9	0.000	0.000	0.000	0.898	376.104	374.943	0.491	0.066	0.067	-0.001			
10	0.000	0.000	0.000	0.627	374.943	373.706	0.489	0.066	0.067	-0.001			
11	0.000	57.000	0.000	0.438	373.706	374.165	6.531	0.877	0.560	0.317			
12	33.542	0.000	10.277	4.391	374.165	376.104	2.997	0.402	0.200	0.202			
13	19.926	0.000	12.047	7.856	376.104	378.830	1.698	0.228	0.120	0.108			
14	11.620	0.000	10.624	9.711	378.830	381.690	0.987	0.133	0.105	0.028			
15	6.384	0.000	8.216	10.049	381.690	384.224	0.850	0.114	0.095	0.019			
16	2.719	0.000	5.770	9.313	384.224	386.158	0.725	0.097	0.095	0.002			
17	0.153	0.000	3.589	7.932	386.158	387.175	0.556	0.075	0.076	-0.001			
18	0.000	0.000	0.670	5.806	387.175	387.439	0.514	0.069	0.076	-0.007			
19	0.000	32.000	0.000	4.056	387.439	388.142	2.613	0.351	0.600	-0.249			
20	19.717	0.000	5.664	5.085	388.142	389.320	1.460	0.196	0.190	0.006			
21	11.492	0.000	6.660	6.199	389.320	390.672	0.852	0.114	0.120	-0.006			
22	6.295	0.000	5.755	6.618	390.672	391.886	0.766	0.103	0.105	-0.002			
23	2.656	13.000	4.239	6.308	391.886	393.176	1.128	0.151	0.115	0.036			
24	9.747	0.000	5.256	6.495	393.176	394.400	0.788	0.106	0.090	0.016			
25	5.073	11.000	4.570	6.353	394.400	395.742	1.185	0.159	0.155	0.004			
26	10.002	0.000	5.536	6.639	395.742	397.037	0.805	0.108	0.118	-0.010			
27	5.251	0.000	4.789	6.541	397.037	398.070	0.729	0.098	0.105	-0.007			
28	1.926	0.000	3.453	5.941	398.070	398.432	0.563	0.076	0.090	-0.014			
29	0.000	0.000	0.586	4.383	398.432	398.228	0.526	0.071	0.080	-0.009			
30	0.000	0.000	0.000	3.061	398.228	397.630	0.523	0.070	0.078	-0.008			
31	0.000	115.000	0.000	2.138	397.630	400.326	26.905	3.612	3.400	0.212			
11/1	54.422	0.000	20.978	9.833	400.326	405.567	6.146	0.825	0.620	0.205			
2	32.182	0.000	22.341	15.805	405.567	412.017	3.353	0.450	0.350	0.100			
3	19.096	0.000	19.216	18.678	412.017	418.565	1.937	0.260	0.220	0.040			
4	11.114	0.000	14.880	18.962	418.565	424.468	1.203	0.161	0.145	0.016			
5	6.029	0.000	10.740	17.514	424.468	429.323	1.006	0.135	0.130	0.005			
6	2.471	0.000	7.256	15.118	429.323	432.708	0.756	0.102	0.117	-0.015			
7	0.000	0.000	2.925	11.723	432.708	434.611	0.600	0.081	0.113	-0.032			
8	0.000	0.000	0.262	8.292	434.611	435.438	0.581	0.078	0.110	-0.032			
9	0.000	0.000	0.000	5.792	435.438	435.515	0.577	0.077	0.100	-0.023			
10	0.000	0.000	0.000	4.066	435.515	435.069	0.574	0.077	0.096	-0.019			

Table A.2.3 Results of Runoff Calculation on Tranquera

TRANQUERA TANK MODEL

DATE	STRAGING PREVIOUS DAY	RAINFALL PREVIOUS DAY	STRAGING PREVIOUS DAY	STRAGING PREVIOUS DAY	STRAGING PREVIOUS DAY	STRAGING PREVIOUS DAY	OUTFLOW TOTAL	DISCHARGE	OBSERVATION	DIFFERENCE
	mm	mm	mm	mm	mm	mm	mm	m3/sec	m3/sec	m3/sec
9/20	0.000	0.000	0.000	100.000	300.000	328.944	0.496	0.034	0.034	0.000
21	0.000	0.000	0.000	69.900	328.944	348.794	0.490	0.033	0.034	-0.001
22	0.000	0.000	0.000	48.860	348.794	362.289	0.485	0.033	0.034	-0.001
23	0.000	0.000	0.000	34.153	362.289	371.343	0.481	0.033	0.034	-0.001
24	0.000	0.000	0.000	23.873	371.343	377.294	0.478	0.033	0.034	-0.001
25	0.000	0.000	0.000	16.687	377.294	381.077	0.475	0.032	0.035	-0.003
26	0.000	0.000	0.000	11.664	381.077	383.345	0.473	0.032	0.035	-0.003
27	0.000	0.000	0.000	8.153	383.345	384.557	0.471	0.032	0.034	-0.002
28	0.000	0.000	0.000	5.699	384.557	385.031	0.469	0.032	0.034	-0.002
29	0.000	0.000	0.000	3.984	385.031	384.990	0.467	0.032	0.034	-0.002
30	0.000	0.000	0.000	2.785	384.990	384.591	0.466	0.032	0.033	-0.001
10/1	0.000	0.000	0.000	1.946	384.591	383.942	0.464	0.032	0.031	0.001
2	0.000	0.000	0.000	1.361	383.942	383.120	0.463	0.032	0.031	0.001
3	0.000	16.000	0.000	0.951	383.120	382.376	0.894	0.061	0.070	-0.009
4	9.920	0.000	3.687	1.127	382.376	381.755	0.544	0.037	0.039	-0.002
5	5.194	0.000	4.943	1.408	381.755	381.212	0.542	0.037	0.036	0.001
6	1.886	0.000	4.808	1.587	381.212	380.571	0.492	0.034	0.031	0.003
7	0.000	0.000	1.930	1.351	380.571	379.757	0.459	0.031	0.031	0.000
8	0.000	0.000	0.000	0.945	379.757	378.824	0.457	0.031	0.031	0.000
9	0.000	0.000	0.000	0.660	378.824	377.809	0.455	0.031	0.031	0.000
10	0.000	0.000	0.000	0.462	377.809	376.738	0.454	0.031	0.030	0.001
11	0.000	57.000	0.000	0.323	376.738	376.379	6.585	0.450	0.430	0.020
12	33.095	0.000	13.970	1.978	376.379	376.804	2.743	0.187	0.100	0.087
13	19.457	0.000	19.352	3.809	376.804	377.834	1.400	0.096	0.055	0.041
14	11.274	0.000	20.431	5.225	377.834	379.222	0.788	0.054	0.047	0.007
15	6.142	0.000	19.281	6.071	379.222	380.738	0.753	0.051	0.040	0.011
16	2.549	0.000	17.032	6.379	380.738	382.192	0.708	0.048	0.040	0.008
17	0.035	0.000	14.251	6.247	382.192	383.364	0.634	0.043	0.039	0.004
18	0.000	0.000	9.824	5.599	383.364	384.140	0.572	0.039	0.035	0.004
19	0.000	32.000	6.123	4.681	384.140	384.998	2.655	0.181	0.180	0.001
20	19.520	0.000	12.818	4.880	384.998	386.029	1.325	0.090	0.080	0.010
21	11.312	0.000	14.985	5.290	386.029	387.167	0.721	0.049	0.050	-0.001
22	6.168	0.000	14.737	5.546	387.167	388.297	0.697	0.048	0.047	0.001
23	2.568	13.000	13.240	5.537	388.297	389.496	1.049	0.072	0.060	0.012
24	9.661	0.000	14.647	5.708	389.496	390.710	0.714	0.049	0.045	0.004
25	5.013	11.000	14.041	5.751	390.710	392.007	1.110	0.076	0.074	0.002
26	9.928	0.000	15.428	5.955	392.007	393.325	0.729	0.050	0.042	0.008
27	5.199	0.000	14.761	6.014	393.325	394.564	0.701	0.048	0.040	0.008
28	1.889	0.000	13.017	5.836	394.564	395.518	0.631	0.043	0.040	0.003
29	0.000	0.000	8.792	5.182	395.518	396.084	0.571	0.039	0.036	0.003
30	0.000	0.000	5.260	4.282	396.084	396.221	0.520	0.036	0.034	0.002
31	0.000	115.000	2.308	3.283	396.221	397.568	27.320	1.866	1.750	0.116
11/1	53.395	0.000	30.318	6.113	397.568	400.169	6.299	0.430	0.260	0.170
2	31.413	0.000	37.157	9.053	400.169	403.651	3.910	0.267	0.100	0.167
3	18.448	0.000	37.340	11.133	403.651	407.578	2.234	0.153	0.075	0.078
4	10.669	0.000	34.575	12.199	407.578	411.583	1.214	0.083	0.065	0.018
5	5.718	0.000	30.771	12.409	411.583	415.400	0.962	0.066	0.060	0.006
6	2.253	0.000	26.532	12.002	415.400	418.738	0.856	0.058	0.057	0.001
7	0.000	0.000	20.090	10.909	418.738	421.449	0.767	0.052	0.050	0.002
8	0.000	0.000	14.706	9.470	421.449	423.480	0.692	0.047	0.047	0.000
9	0.000	0.000	10.204	7.899	423.480	424.832	0.628	0.043	0.042	0.001
10	0.000	0.000	6.440	6.329	424.832	425.542	0.574	0.039	0.042	-0.003

Table A.3.1 Results of Water Quality Analysis

Location	1	2	3	4	5	6	7	8	9
Temperature (Water) °C	20	22	22	20	21	22	23	22	23
Turbidity (SiO ₂) mg/l	5	2.5	5	2	2	0	0	45	35
pH	6.9	6.9	6.7	6.7	6.5	6.5	6.6	7.2	6.5
Alkalinity (CaCO ₃) mg/l	25	23	19	15	15	15	18	45	15
Hardness (CaCO ₃) mg/l	26	20	20	16	12	10	50	32	14
Solid Total (ST) mg/l	24.5	25.5	25.5	3	41	26	164	147	46.5
Cl ⁻ mg/l	7.1	8.8	7.1	7.1	7.1	7.1	8.8	9	7
SO ₄ ²⁻ mg/l	0.5	1	0.5	1	2	1	2.1	0.7	1
N ⁻ NH ₄ mg/l	0.1	0.1	0.1	0.3	0.1	0.1	0.04	0.02	0.4
EC micromhos/cm	55	40	30	28	29	22	70	80	30
DO mg/l	8.4	6	8	7.5	7.3	6.7	-	-	8.5
BOD mg/l	0.8	0.4	0.6	0.6	0.9	0.6	-	-	5
Na mg/l	0.7	0.6	0.5	0.3	0.3	0.3	0.4	3	0.8
K mg/l	2.0	1.9	1.4	1.5	1.5	0.3	3.3	10	3
Ca mg/l	0.5	1	0.6	0.0	0.0	2.8	2.2	1.8	2.2
Cu mg/l	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Zn mg/l	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Fe mg/l	0.5	0.2	0.1	0.0	0.1	0.0	0.0	3	0.0
Mn mg/l	0.2	0.1	0.0	0.0	0.0	0.0	0.0	0.2	0.1
Colon Bacilli M.P.N	0	4	9	4	23	0	0	0	0

Fig. A.1.1 Hydrological and Meteorological Stations

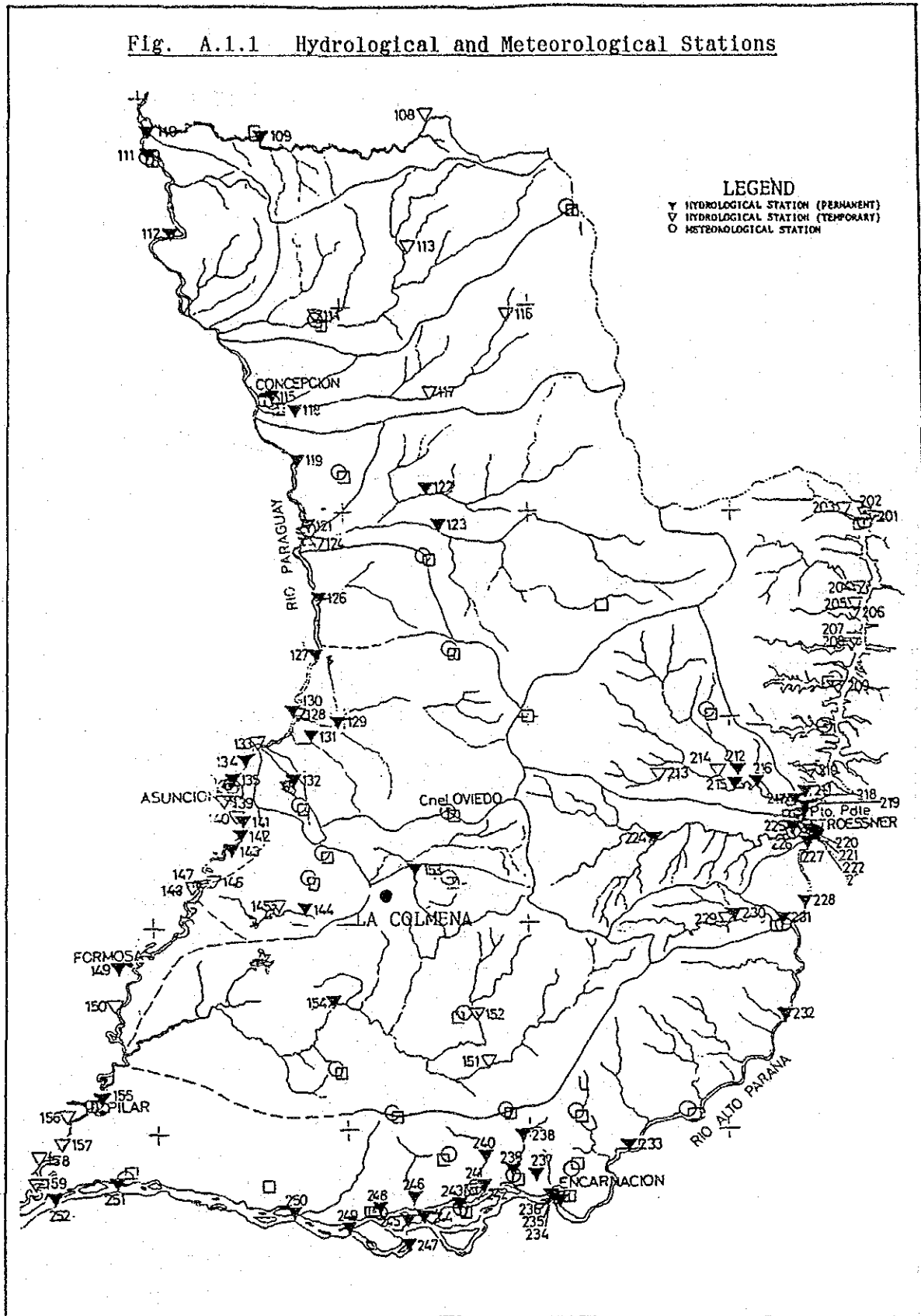
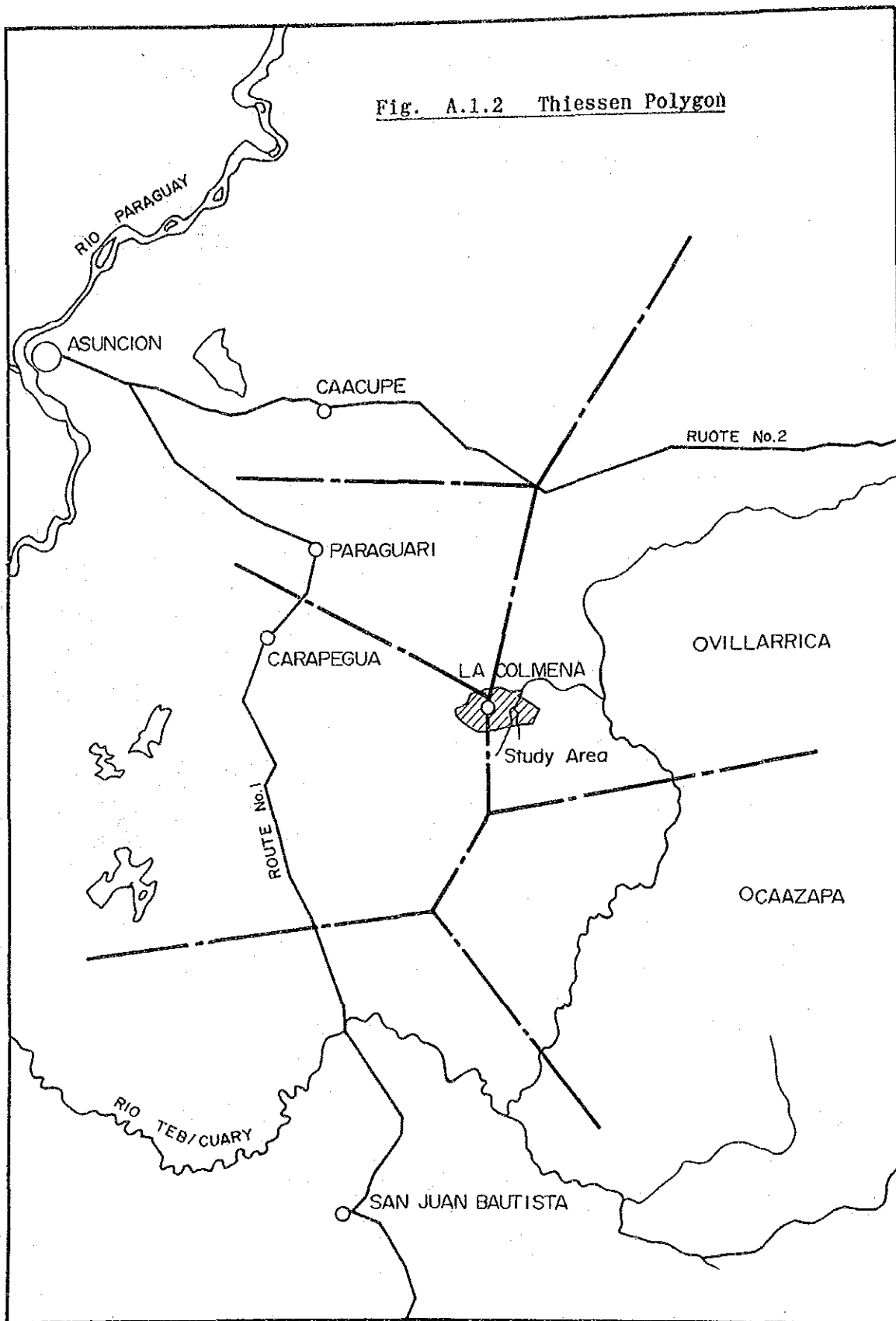


Fig. A.1.2 Thiessen Polygon



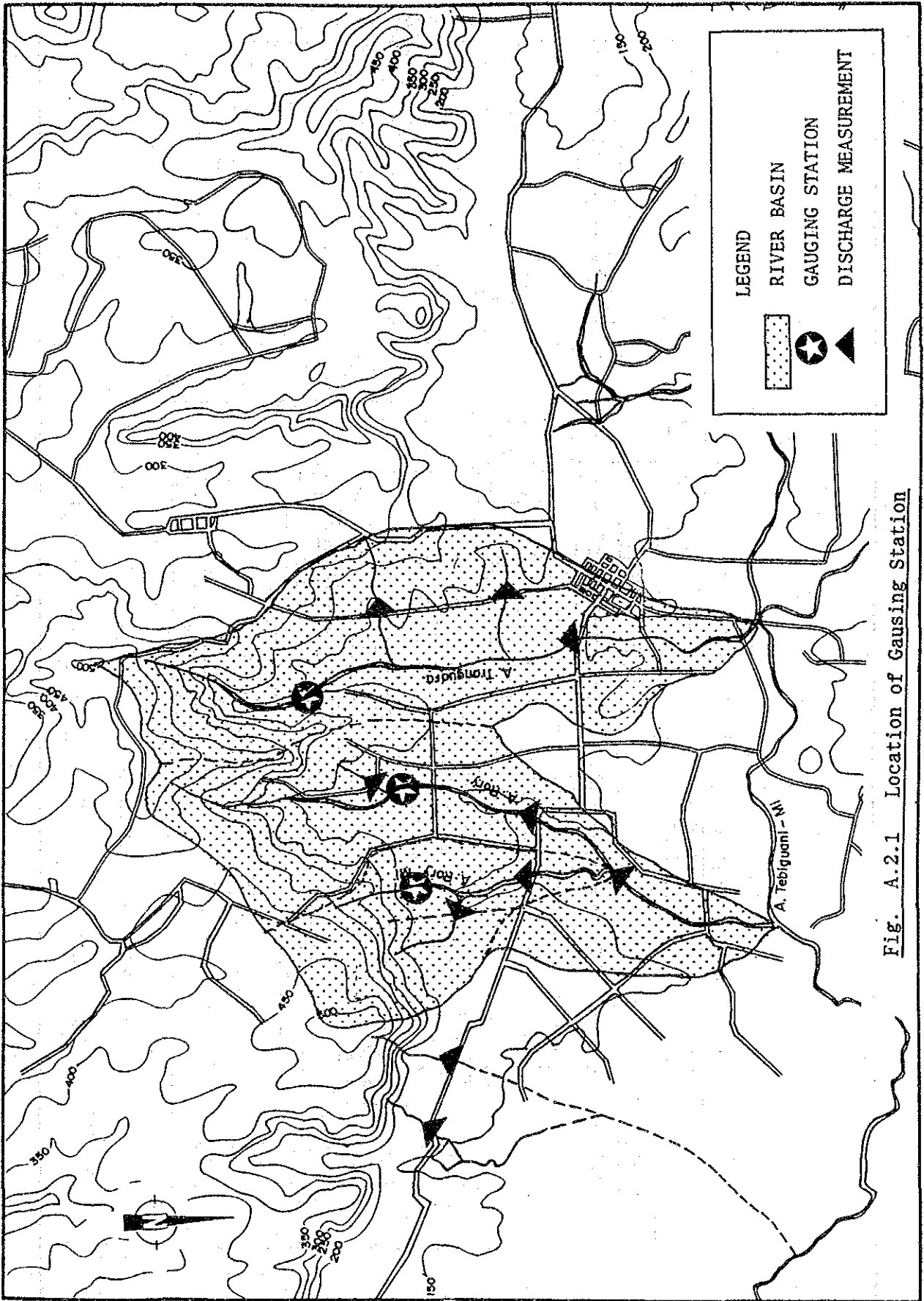


Fig. A.2.1 Location of Gauging Station

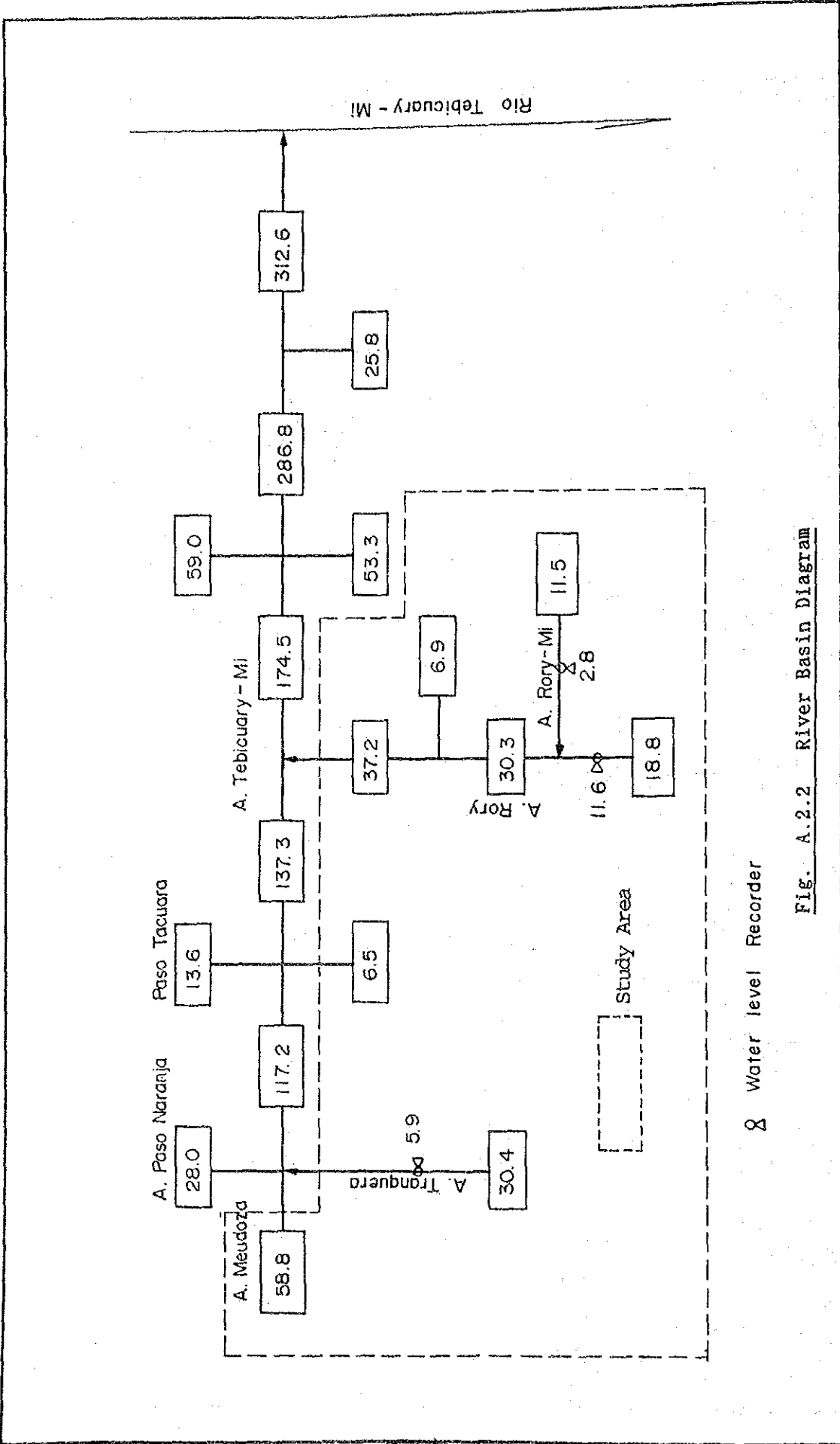
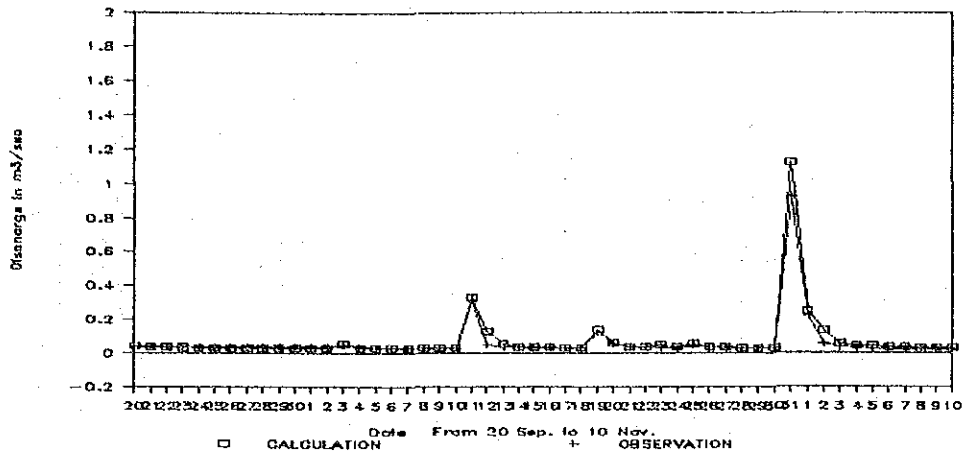


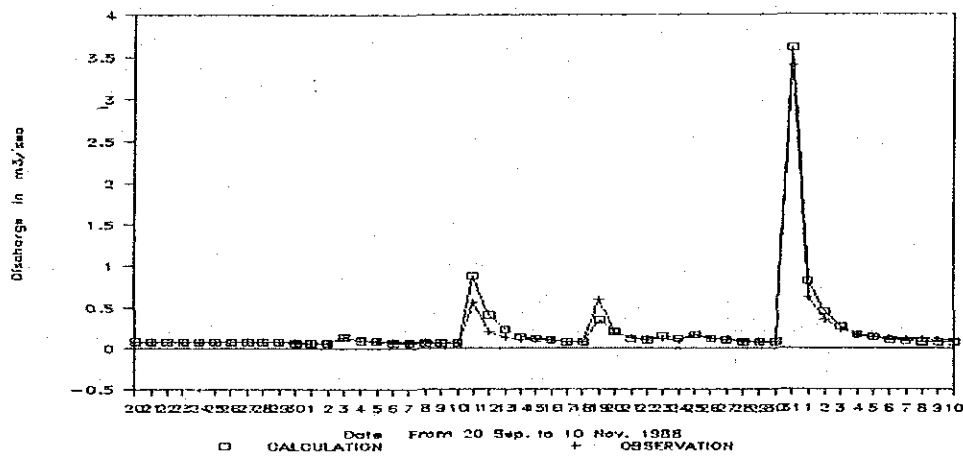
Fig. A.2.2 River Basin Diagram

Water level Recorder

A. RORY-MI



A. RORY



A. TRANQUERA

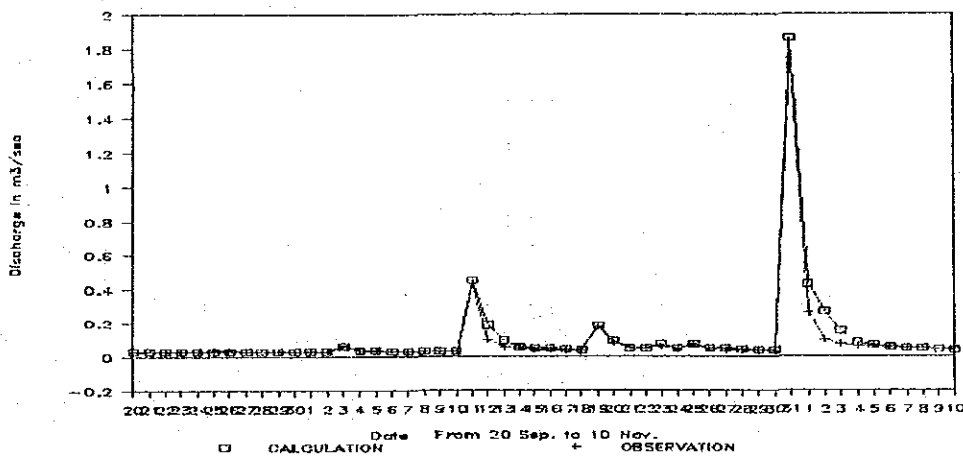


Fig. A.2.3 Verified Results on Tank Model

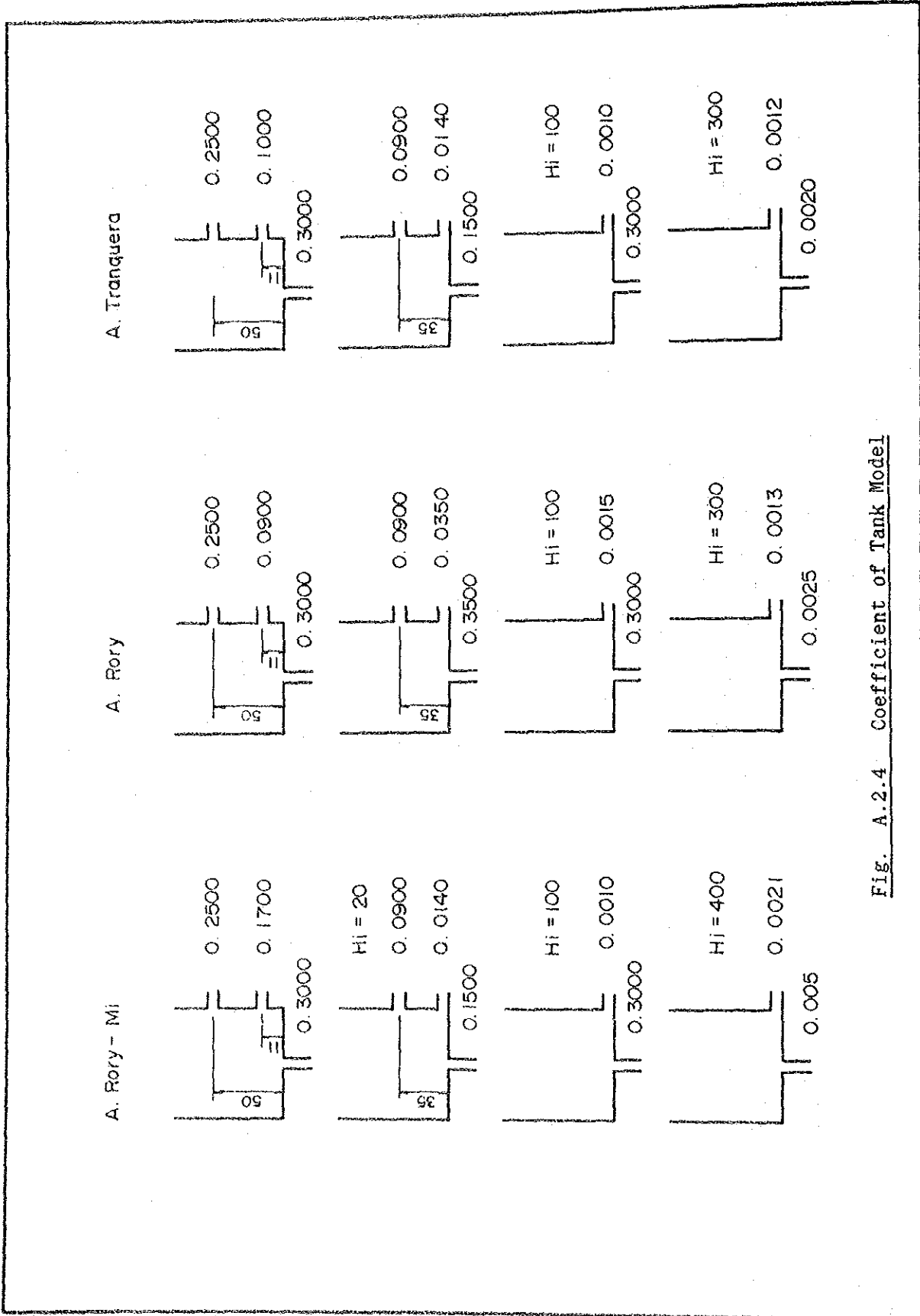


Fig. A.2.4 Coefficient of Tank Model

**ANNEX B GEOLOGY AND
GROUNDWATER**

ANNEX B Geology and Groundwater

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ANNEX B. Geology and Groundwater

B.1. Introduction

The following surveys were carried out in the Study Area to understand its geological and ground water characteristics.

- Geographical and geological surveys of the locations where construction of water-intake dams, regulating ponds and reservoirs is planned in the project.
- Electrical prospecting of the ground and well-digging surveys concerning ground-water development in the Study Area.

B.2 Surveys on Geography and Geology

2.1 Upper Reaches of Arroyo Tranquera, Arroyo Rory and Arroyo Rory-mi

The area's geology is typified by the alternation of conglomerate and sand stone of the Paleozoic period, with the strike running about N30° E and a very gentle gradient of 3-5° to the east or west.

The land appears to have a wavy pattern from west to east.

The streams are rather rapid down to an altitude of 250 meters and their valleys are narrow and shaped like a letter V. The river bed is covered with thin layers of boulders 20-100 cm in size, and rough and medium sand, but no granules. Many small waterfalls are observed, most of them are 1.5 - 5.0 m in height, but very occasionally there are those with a height of up to 7 - 8 m.

The land is an alternation of conglomerate and sand stone strata (30 - 50 cm in thickness, maximum of 1.0 m). The conglomerate consists mostly of well-polished pebbles of chert and quartz. Matrixes comprise about 70 - 80% of the sand stone.

The small water falls are considered a kind of cuesta and are believed to have been formed as a result of the selective erosion of the hard conglomerate. The rock easily crumbles on impact of a hammer and readily becomes sandy without experiencing the granule state.

The valleys of Arroyo Rory and Arroyo Rory-mi are also v-shaped and exhibit rougher terrain than Arroyo Tranquera. This is because of the strong influence of the fault and crushing zones. The streams are rapid and construction of a dam 20 -30 m high will not be economically feasible

as the reservoir capacity of such a dam will be too small for its size. On the other hand, construction of a water-intake (small dam) should not pose any problems, provided that such a dam is located at a certain altitude. Any place with exposed rock will be adequate to serve as a foundation of the dam.

The extension of digging will merely get larger or smaller depending on whether the valley walls have exposed rock or talus cone. If it is the latter, only a 1.0 - 2.0 m trench cut needs to be made at the exposed rock for installation. 0.5 -1.0 m form treatment of the foundation which also serves as a cut off is considered adequate for the lower part of the bank. Others such as curtain grout and contact grout are not necessary.

2.2 Middle Reaches of Arroyo Tranquera

According to the plan, it is possible to construct a small dam 5.0 - 7.0 m (max. 10.0 m) in height with a capacity of 1.5 -2.0 million cubic meters of water at the east of the La Colmena urban area where the valley close to the upper stream becomes narrow. Electrical prospecting were conducted to understand the geological characteristics of the place..

The rock bed is made of hard conglomerate of the Paleozoic period and should pose no problems as to the required strength of the foundation for a dam of the proposed size. Outcrops of the conglomerate strata are observed over about 100 m up and down the stream on the right side of the dam site, but no faults or crushing zones could be found.

The river bed is covered with sedimentary layers 2 - 3 m in thickness, or a little thicker in some parts. No outcrops are observed, an indication that the conglomerate strata is distributed over all the site.

Electrical prospecting conducted at five points to a depth of 50 - 60 m suggested the existence of a foundation which mainly consists of dry, thin surface soil and conglomerate on both sides.

B.3 Ground Water Survey

3.1 Electrical Prospecting

Electrical prospecting, which is one of the many physical surveys, is used to measure natural electric potential or find out the geological structure of the area by carefully studying changes in the electric

current which is made to flow in the ground.

The Wenner method, a kind of specific resistance method, was used to conduct electrical prospecting in La Colmena. Here, electric current electrodes and potential electrodes are symmetrically placed above the point of measurement. The distance between the electrodes is gradually made greater and apparent resistivity is measured each time to determine apparent resistivity directly above the point.

The equation expressed as:

$$\rho = 2 \times 3.14 \times aR$$

where ρ : specific resistivity
a : interval of electrode
R : electrical resistance

3.2 Equipment Used in the Electrical Prospecting

Various devices are used in the electrical prospecting. In this case, highly functional devices most suited to the specific resistance method were employed. Their specifications are shown below.

(1) Transmitter

a. Output voltage : 400 VP-V
b. Output current : 1, 2, 5, 10, 50, 100, 200 mA
c. Operation voltage : 12 VDC

(2) Receiver

a. Input impedance : 1M
b. Measurement potential : $\pm 0.6V$, $\pm 6V$ (AUTO RANGE)
c. Resolution : 10 V
d. Stack frequency : 1, 4, 16, 64 (can be stopped as desired)
e. Cycle time : 3.5 sec.

(3) Data Memory

a. File registration : max. 128 files
b. Data point : max. 1100 data
c. 1 file : max. collected 110 data

(4) Interface : RC-232C

- (5) Power Source : DC12V (built in um 1x8)
External power sources can be used
- (6) Dimensions : 206(W) x 181(H) x 200(D) mm
- (7) Weight : 10 kg

3.3 Survey Outline

The survey was conducted to a level 150 - 200 m below the ground by taking into account the depth (150 m) of the planned wells. As the Wenner's four electrode method was employed, the max. measurement depth of 200 m requires a total length of 600 m (200 m x 3) of C1 and C2 electrodes on a nearly straight line. For ease of measurement, the areas along the main roads were chosen to carry out the approx. 1.0-km square preliminary surveys. In the actual measurement, the key curves were obtained at two places in the urban La Colmena where deep wells are located and near the deep wells owned by Mr. Miyamoto and Mr. Mitsui.

3.4 Survey Results

Only the Paleozoic conglomerate and sand stone layers in the south of the area near the urban La Colmena show rather high resistivity of several 100 Ω m or more. It is about 200 Ω m for the alternation of the very weathered layer of the conglomerate and sand stone of the Mesozoic era, 80 - 100 Ω m for a little fresher rock and mere 10 - 20 Ω m for the fresh foundation rock.

In particular, the specific resistivity is extremely low at 7 -15 Ω m in the vicinity of the urban area, suggesting that the geology of the area contains a mixture of shale and mud stone, or a thick accumulation, forming impermeable layers.

The electrical prospecting started near the deep wells of La Colmena and the measurement points were gradually expanded into the surrounding area. As a result, the Study Area has been divided into the following five blocks according to the specific resistance characteristics: the urban area, west of the urban area, east, south and east of Arroyo Rory-mi.

- (1) The mountains in the south with the highest specific resistivity. This thinly layered block has very few strata which might contain ground water.

- (2) The thick weathered layer to the level of a few dozen meters below the ground of the east block shows relatively high specific resistivity and is considered to contain ground water.
- (3) In this urban block, the sandy layers of the surface weather-eroded zone, which contain ground water, are believed very thin, and the specific resistivity of the foundation rock is extremely low.
- (4) This west block is thought to have relatively thin surface sandy layers but contain a large amount of conglomerate and sand stone with high specific resistivity.
- (5) This block in the east of Arroyo Rory-mi exhibits about the same amount of specific resistivity as case 1. above, but is thought to have thick weather-eroded layers which might contain ground water.

3.5 Analysis of the Survey Results

Brief explanations of each of the above-mentioned five blocks will be given below. First, the relationship between each block and the specific resistivity is as follows.

- a. The mountains in the south : more than 2,000 Ω m
- b. The hilly areas in the east : 100-300, more than 20 Ω m
- c. Near the urban area : 10-20, 50-200 Ω m
- d. The flat land in the west : 100-500, 40 Ω m
- e. Near Arroyo Rory-mi : 15-30, 100-800 Ω m

(1) The mountains in the south

Conglomerate and sand stone of the Paleozoic era predominate in the mountains in the south. The surface layer is soft due to erosion by strong winds and belongs to the CL or D class in terms of rock bed strength.

In the valley, some hard rock in the CM class appears. Although no faults and crushing zones are visible, possibility is high that they are hidden behind the foot of the mountains or talus cone of the sloping areas. Faults and crushing zones may appear in the northern part such as EPNo. 13, 14, but this is not clear. Likewise, there are few joints or cracks and the foundation rock is not likely to contain much ground water

(excluding the part of the mountains south of Rory-mi). The surface soil covers a limited area of land and the accumulation of talus cone is thin and also limited in its distribution. With these geological characteristics, the ground in this block can only temporarily retain water after rainfalls but will quickly lose it after some consecutive sunny days.

However, some part of the mountains have sand layers 5 - 10 m in thickness such as arose in texture, and are capable of holding water in the form of free underground water. Such water can be used for rural water supply on a limited scale.

(2) Hills in the east

The Paleozoic conglomerate and sand stone in the south are distributed below the hills between EPNo. 10 and EPNo. 9 at an angle of about 30° (this angle has been determined from examples of the other areas). ON top of these are the nonconforming strata of conglomerate and sand stone of the Mesozoic period.

The area bordering Arroyo Tranquera up to EL 140 m north of this block shows higher specific resistivity than the urban district. At any measurement point, this weather-eroded Mesozoic conglomerate layer which mostly consists of sand stone is found to contain clayish sand strata 30 - 40 m in thickness. This result has been confirmed by the survey conducted at the present No. 1 well. It is also learned that the ground enters the strata of mostly less-method sand stone about 40 m from the surface.

The results of the electrical exploration show that the high-erosion zones often have a specific resistivity of about 300 m, suggesting the existence of a large pool of water, but 80 - 100 Ω m in some part. The value of resistivity becomes smaller as the measurement point enters the fresh rock bed. It is about 30 - 50 Ω m at the depth of 50 - 60 m, then furthers declines to approx. 20 Ω m, which changes very little to a depth of 150 - 200 meters.

It can be easily understood that the ground 50 - 60 meters from the surface and below has experienced less erosion and had fewer cracks and joints, and therefore does not contain much ground water. For this reason, it does not make sense to dig a well deep into these strata. At the moment, there are two wells, each providing a pumping capacity of approx. 100 m³/day (max. 150 m³/day).

(3) Vicinity of the urban area

This block west of Arroyo Tranquera is located in the hills extending north from EPNo. 14, 30 and has basically the same geological characteristics as the other blocks. Conglomerate and sand stone of the Paleozoic era are believed to be distributed between EPNo. 14 and EPNo. 30 at an angle of 30° in nonconformity. The surface layers mainly consist of conglomerate and sand stone of the Mesozoic period and are thinner than those in the east block (only several meters in thickness, max. 20 m).

(4) Flat land in the west

The block can be geographically further divided into three zones: northern, central and southern part. The p-a curve of the central and the southern zones exhibits the tendency similar to that of the east. On the other hand, the situations in EPNo. 55, 56 (near the graveyard) are similar to those in the urban La Colmena; the heavily weather-eroded sandy surface layer is thin and the specific resistivity declines to 12 -15 Ω m at a depth of about 10 m, which later rises to about 20 m at a depth of 50 m and deeper.

It has been judged that ground water is very difficult to obtain near the graveyard and the area extending to north. The foundation rock shows signs of alteration of conglomerate and sand stone, but the absence of geological data in the area concerning ground water, such as records of deep wells, makes it impossible to make a valid comparison.

The flat land near the national roads and the south exhibits characteristics that are basically similar to the mixture of the data of various places of the urban area; it consists of the very weather-eroded layer of sand and clay 20 - 30 m below the surface, the less weather-eroded and firmer strata up to a depth of 50 - 60 m, and the fresh alterations of conglomerate and sand stone from there.

Such pieces of information suggest that not much can be expected in obtaining water from under ground with such geological characteristics. The stratigraphic column of a survey well shows that the ground is clayish to the depth of 6.0 m, sandy clay to 29.0 m in depth, loose sand to 37.0 m where underground water is stored, medium-hard sand stone and very hard sand stone strata at the depth of 55 m and deeper.

This data almost completely corresponds to the analysis of the above-mentioned ρ -a curve, proving that the overall analysis results are correct.

(5) East of Arroyo Rory-m1

Layers of conglomerate and sand stone of the Paleozoic period appears at the depth of about 12 m in EPNo. 73 in the south, as in the case of No. 10, 13, 14. Naturally, very little ground water can be expected from such strata, but a well can be dug in the area to get water for home use from the eroded sand layers in the upper part.

EPNo. 75, 79 and 80, which are north of the national roads, have a ρ -a curve very close to that in the urban area; therefore, it cannot be expected at all to obtain water form under the ground. However, strongly weather-eroded sandy layers exist to a depth of 40 m at the other measurement points and water can be obtained from these strata in the same manner as in the eastern part of the urban area.

3.6 Summary

(1) In the southern mountains, the only available form of underground water is the free ground water contained in the arose sand 5 - 10 m in thickness. Therefore, it is next to impossible to use it for agricultural irrigation.

(2) Underground water resources are extremely poor near the foot of the mountains a little to the north.

(3) It will be possible to pump 200 m³/day of water form a large-diameter well to be dug to a depth of 40 - 50 m, or a very large well with a bore of 2.0 m or more can be dug to the depth of 10 - 15 m to obtain the same amount of water. In the latter case, more water can be obtained by boring the ground radially from the lower levels.

(4) Large-scale development of the underground water in the urban area cannot be expected. It is possible to pump 50 m³/day of water from a well with a depth of 30 - 50 m, but that amount of water can also be obtained from a shallow large-bore well with a depth of 10 - 15 m.

(5) Construction of a small dam 5 -10 m high at Arroyo Tranquera is expected to greatly enhance recharging of the free underground water. Moreover, large numbers of large-diameter wells and conduits can be constructed to serve as reservoirs.

(6) It is difficult to secure large volumes of underground water in the western part of the area. However, water contained in the weather-eroded sand stone and the clayish sand layers to the depth of 5 - 10 m can be utilized.

(7) The summary above can be further condensed into the following:

- It is not possible to obtain free underground water contained in the loose sand stone of the weather-eroded surface layers.
- Pooling of this underground water should be actively promoted.
- It will be very difficult to obtain ground water from the foundation rock bed 50 m or more below the surface. Water can be obtained if a well happens to encounter faults or crushing zones, but another survey must be made to confirm their existence.

3.7 Groundwater Level of the Project Area

Surveys were conducted on the distribution of ground water in shallow strata, mainly at farmers' shallows scattered over most of La Colmena area. The actual places of measurement, a total of 176 points were shown in Fig. B.1.8. However, wells at 26 points were excluded from the measurement because of the following reasons.

- (1) Discarded wells
- (2) Drained up wells about which measurement cannot be performed
- (3) Use of streams to obtain water and therefore cannot be classified as well

The strata which contain water, located 5 - 10 m below the ground, consist mainly of sandy clay, sandy clay with gravels or weather-eroded sand stone and conglomerate. Distribution of such water-containing strata is uneven over the area, with layers running as thick as 20 m in some places where there are thick weather-eroded strata.

The wells are on average 5 - 10 m in depth, excluding the extremely shallow ones in some part of the flat land in the north, each containing

0.5 - 1.5 m of water. Quite a few wells were found short of water as the survey was conducted in August 1988 after more than 40 consecutive days of dry days. However, they can adequately serve as a source of water during the wet season. It has been learned during the field survey that the water level in the well drops 1.5 - 2.0 m throughout a year. Fig. B.1.9 shows the contour line of the water level in the shallow wells.

Groundwater levels have been found to follow the land contours, indicating distribution of the water-containing shallow strata with relatively uniform thickness. On the other hand, the permeability coefficient is judged to be 10^{-3} - 10^{-4} cm/s and much water cannot be expected from the wells.

It has been learned that as many as one third of the wells in the area run short of water during the dry season. Therefore, it is necessary to construct facilities to obtain water from rivers and deep wells in order to secure supply of water.

B.4 Reference Data of Well-Digging Surveys

- . Bodegas "La Colmenita"
- . Propiedad de Hayashi
- . Propiedad de Seki
- . Propiedad de Chavez

1. INTRODUCCION

De acuerdo a las indicaciones recibidas de los interesados en la construcción de dos pozos tubulares profundos, se ha trasladado la máquina perforadora INGERSOLL RAND, hasta el sitio indicado por los mismos. Atendiendo, al parecer, los resultados de SEV (sondeos eléctricos verticales) practicados en la zona por técnicos japoneses.

Los trabajos tenían por objetivo perforar hasta 100 m. de profundidad (cada pozo), encamisarlo y realizar pruebas de suficiencia de los acuíferos alumbrados.

Hubiera resultado de gran valor contar con los resultados de los SEV para orientar con mayor efectividad los trabajos de perforación.

2. PERFORACION

Las experiencias anteriores de perforaciones realizadas en la zona indicaban la conformación de un subsuelo bastante difícil, con gravas y cantos rodados en matriz arcilloso y arenoso, lo cual significa un medio sumamente inestable. Razón por la cual, se decidió iniciar los trabajos en el sistema de rotación con broca tricórnica de 12 1/4" de diámetro, utilizando "bentonita" para contener las paredes.

2.1. Pozo de la Bodega

Los trabajos con el sistema de rotación avanzaron con muchas dificultades hasta los 16 m. de profundidad, cuando la herramienta dejó de avanzar, por lo cual se decidió cambiar al sistema de percusión, utilizando martillo de fondo (DHD 16 A, INGERSOLL RAND) con broca de botones de 8 1/2" de diámetro, llegando hasta los 87,00 m. de profundidad.

La perforación no pudo continuar debido a las dificultades ocasionadas por la alta inestabilidad de las paredes del pozo

que produjeron el continuo atascamiento de la herramienta, con el consiguiente peligro para hombres y máquina operando en el lugar.

Después de varios días de intentos fallidos se decidió, con acuerdo de los Interesados, encamisar la longitud perforada, previa realización del perfilaje eléctrico del pozo.

El encamisado no alcanzó a cubrir toda la columna perforada, lográndose entubar hasta los 60 m. de profundidad, debido al derrumbamiento de las paredes del pozo en su longitud inferior.

Características del pozo

Durante los trabajos de perforación del pozo ya se tuvo conocimiento de su rendimiento debido al trabajo de la máquina, que utiliza un martillo de fondo para romper la roca y aire a presión para el desalojo de los detritos. Si se detecta un acuífero, el aire empieza a desalojar agua en forma continua, en caudales casi igual al rendimiento del acuífero alumbrado.

En el pozo de referencia no se logró captar ningún aporte de importancia, apenas el acuífero libre de los 20 m. y otros de menor importancia más abajo. Este hecho fue profusamente informado a los Interesados de manera a evitar gastos mayores con el encamisado y terminado del pozo. No obstante los mismos decidieron continuar los trabajos hasta su completa terminación.

2.2. Pozo 2 de Hayashi

La perforación realizada en este sitio no ha variado substancialmente en relación al anterior pozo. Sin embargo, existió mejor cementación de la formación que permitió perforar 57.4 m. en el sistema de rotación y por supuesto utilizando tiempos mucho mayores que los normales.

Se decidió terminar el pozo en la profundidad de 57,4 m., para lo cual se realizó el perfilaje eléctrico correspondiente.

Posteriormente se procedió al encamisado y engravado del pozo para iniciar la limpieza y desarrollo del mismo.

3. PRUEBAS DE BOMBEO

. Equipo utilizado: Compresor portátil SPIRO FLO de INGERSOLL RAND.

. Medidor de nivel de agua

. Cronómetro.

3.1. Pozo 1 de la Bodega

Como ya se adelantara, este pozo no tiene un rendimiento importante.

Durante el bombeo el agua se mantuvo turbia durante unas 5 horas y posteriormente ya se mantuvo limpia hasta la completa terminación de la prueba, es decir, hasta 24 hs.

El caudal de bombeo fue de 2.000 lts/h.

Posteriormente se realizó la prueba de recuperación del nivel del agua del pozo.

3.2. Pozo 2 Hayashi

El rendimiento de este pozo fue superior al anterior. En efecto, durante la limpieza y desarrollo del mismo ya se midió un caudal de 6.000 lts./h., utilizando el compresor de la máquina.

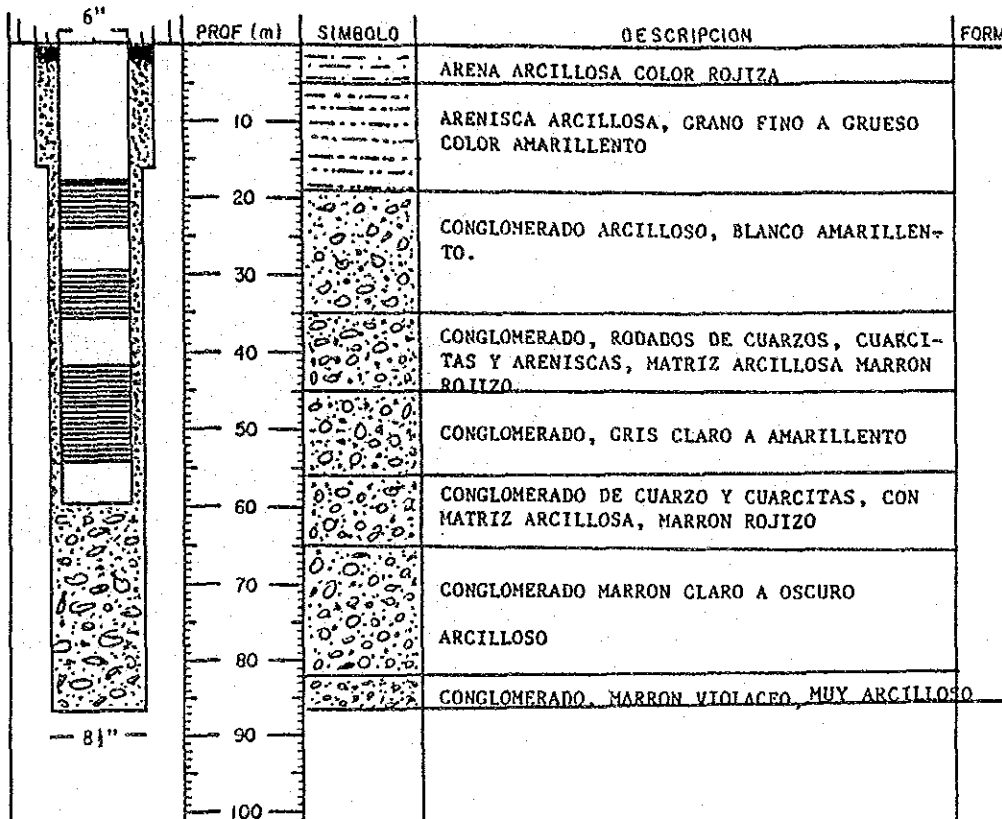
Posteriormente, utilizando el equipo de prueba de bombeo, se realizó un ensayo durante 24 hs.

El caudal de bombeo fue de 4.000 lts./h.

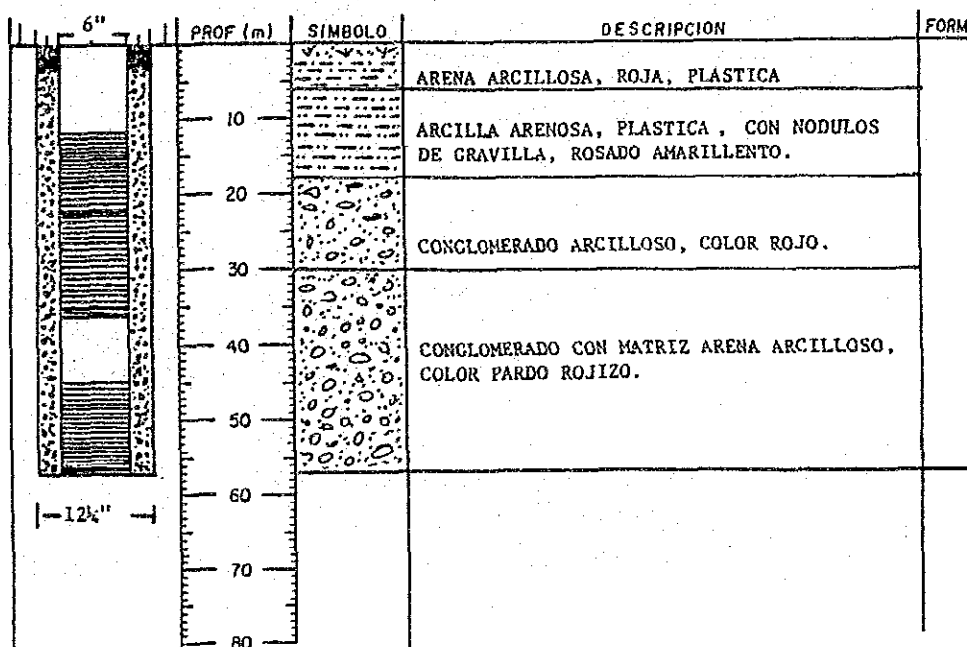
Al parar el equipo de bombeo se realizaron mediciones de la recuperación del nivel de agua del pozo.

PERFIL DE POZO PERFORADO

Nombre: LA COLMENA - POZO DE LA BODEGA



Nombre: LA COLMENA - POZO N° 2 DE HAYASHI



PLANILLA DE BOMBEO

POZO N° 2 : HAYASHI
 PROFUNDIDAD : 57 m. DIAMETRO: 6"
 NIVEL ESTÁTICO : 19,55 m. FECHA DE PRUEBA: 22 - 23 - XII - 88
 HORAS DE BOMBEO : 24 hs.
 EQUIPO UTILIZADO : COMPRESOR INGERSOLL RAND

TIEMPO (min)	NIV. EST. (m)	NIV. DINAM. (m)	ABATIM. (m)	CAUDAL l/h.	CARACT. DEL AGUA
	19,55				
1		23,67	4,12		
2		24,00	4,45		
3		24,28	4,73		Turbio
4		24,48	4,93		"
6		24,58	5,03		"
8		24,71	5,16		"
10		24,98	5,43	4,000	"
15		25,40	5,85	"	"
20		25,61	6,06	"	"
30		25,77	6,22	"	"
60		25,90	6,35	"	"
90		26,08	6,53	"	"
120		26,10	6,55	"	Aclarando
150		26,12	6,57	"	"
180		26,14	6,59	"	Claro
240		26,20	6,65	"	"
300		26,25	6,70	"	"
380		26,32	6,77	"	"
420		26,38	6,83	"	"
480		26,42	6,87	"	"
540		26,47	6,92	"	"
600		26,50	6,95	"	"
660		26,52	6,97	"	"
720		26,50	6,95	"	"
780		26,50	6,95	"	"
840		26,51	6,96	"	"
900		26,50	6,95	"	"
960		26,50	6,95	"	"
1020		26,51	6,96	"	"
1080		26,51	6,96	"	"
1140		26,51	6,96	"	"
1200		26,50	6,95	"	"
1260		26,51	6,96	"	"
1320		26,51	6,96	"	"
1380		26,51	6,96	"	"
1440		26,51	6,96	"	"

PLANILLA DE RECUPERACION

POZO N° 2 : HAYASHI
 NIVEL DINAMICO: 26,51 m

TIEMPO (min.)	RECUPERACION (m)	NIVEL DINAMICO (m)
0,5	1,95	26,51
1,0	2,51	24,56
2,0	2,72	24,00
3,0	3,53	23,79
5,0	4,45	22,98
7,0	4,92	22,06
9,0	5,30	21,59
11	5,48	21,21
14	5,58	21,03
17	5,67	20,93
20	5,76	20,84
23	5,81	20,75
26	5,84	20,70
30	5,89	20,67
35	5,93	20,62
40	5,97	20,58
45	6,03	20,54
55	6,09	20,48
65	6,16	20,42
		20,35

GRAFICO ABATIMIENTO - BODEGA

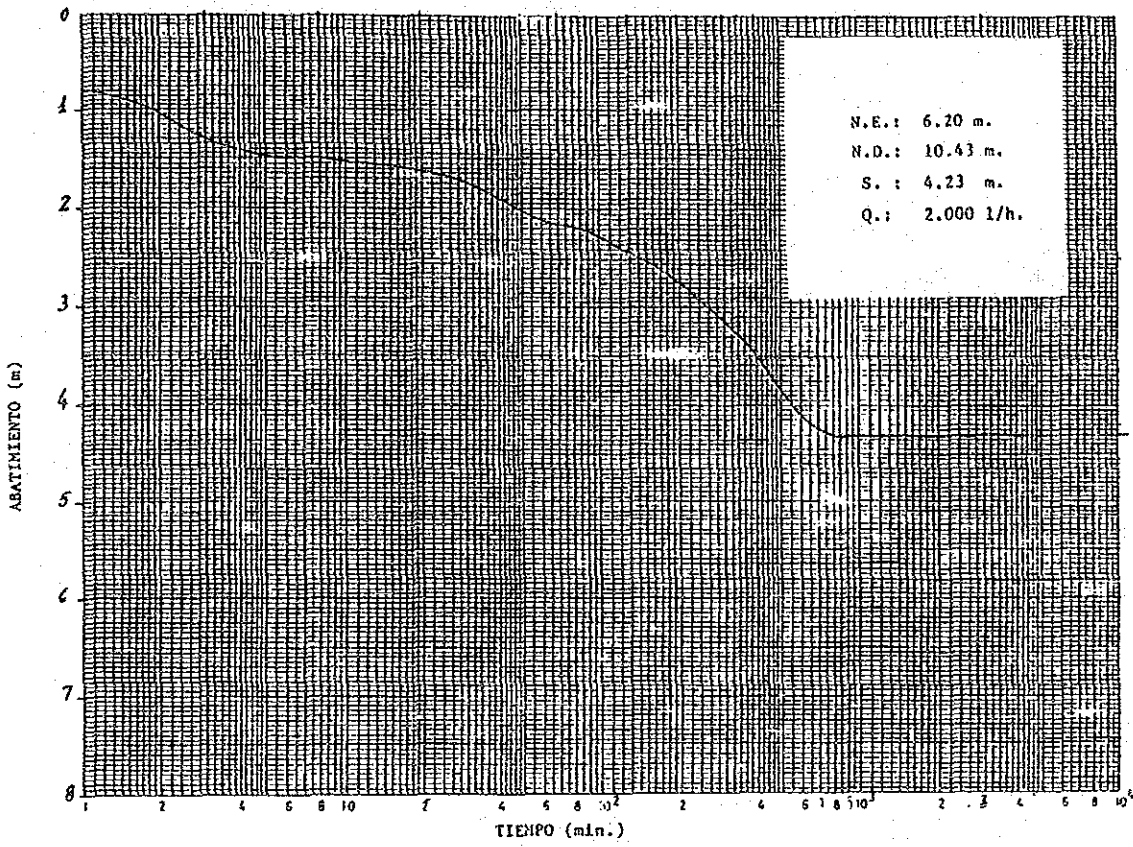
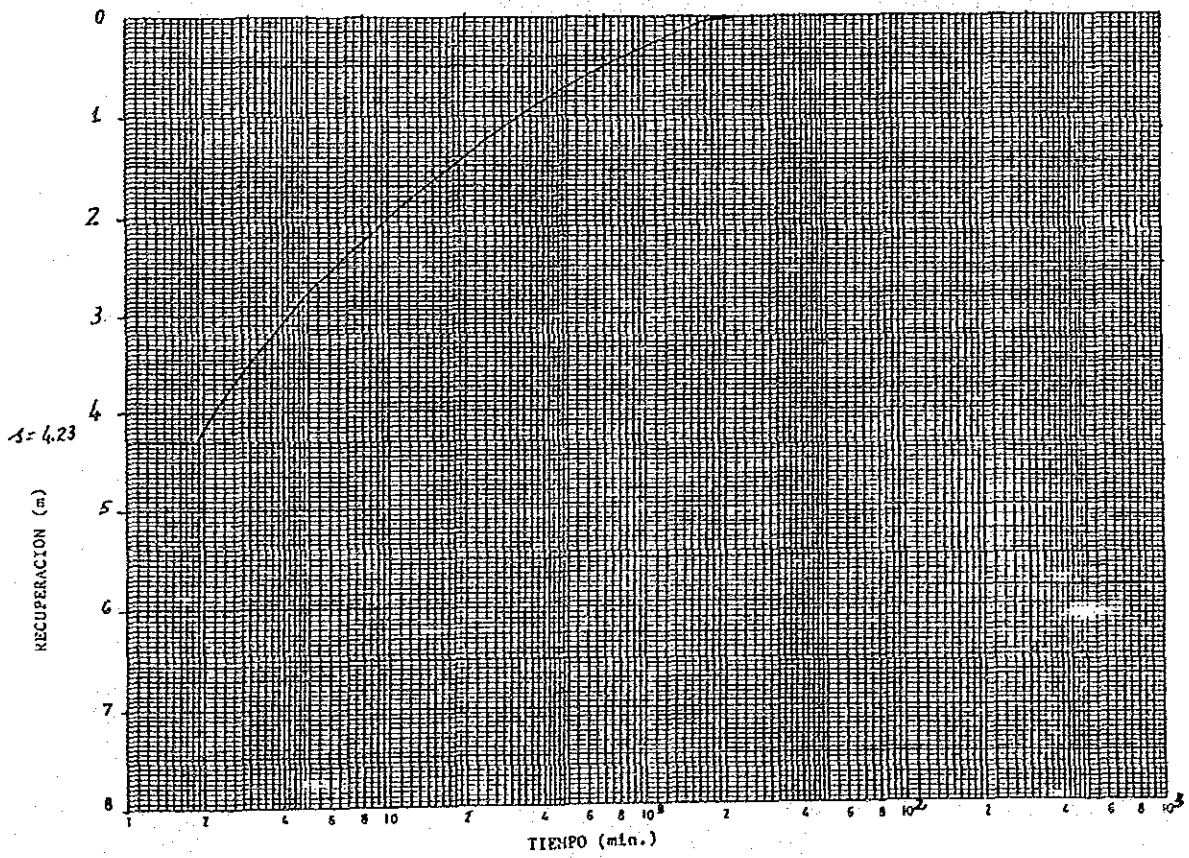


GRAFICO RECUPERACION - BODEGA



PLANILLA DE BOMBEO

POZO N° 1 : BODEGA
 PROFUNDIDAD : 60 m. DIAMETRO: 6"
 NIVEL ESTÁTICO : 6.20 m. FECHA DE PRUEBA: 20 - 21 - XII - 88
 HORAS DE BOMBEO : 24 hs.
 EQUIPO UTILIZADO : COMPRESOR INGENSOLL RAND

TIEMPO MIN.	NIV. EST. (m)	NIV. DINAM. (m)	ABATIM. (m)	CAUDAL l/h.	CARACT. DEL AGUA
	6.20				
1		7.00	0.80		
2		7.20	1.00		
3		7.50	1.30		Turbio
4		7.60	1.40		"
6		7.66	1.46		"
8		7.68	1.48		"
10		7.70	1.50		"
15		7.75	1.55	1.800	"
20		7.81	1.61		"
30		7.89	1.69		"
60		8.32	2.12		"
80		8.53	2.33		"
120		8.66	2.46		"
150		8.76	2.56	1.800	"
180		8.85	2.65		"
240		9.10	2.90		"
300		9.30	3.10	1.800	Aclarando
360		9.50	3.30		Claro
420		9.75	3.55	2.000	"
480		10.00	3.85	"	"
540		10.20	4.05	"	"
600		10.32	4.12	"	"
660		10.42	4.22	"	"
720		10.42	4.22	"	"
780		10.43	4.23	"	"
840		10.43	4.23	"	"
900		10.43	4.23	"	"
960		10.42	4.22	"	"
1020		10.43	4.23	"	"
1080		10.44	4.24	"	"
1140		10.43	4.23	"	"
1200		10.43	4.23	"	"
1260		10.43	4.23	"	"
1320		10.43	4.23	"	"
1380		10.42	4.22	"	"
1440		10.43	4.23	"	"

PLANILLA DE RECUPERACION

POZO N° 1 : BODEGA
 NIVEL DINAMICO : 10.43

TIEMPO (min.)	RECUPERACION (m)	NIVEL DINAMICO (m)
		10.43
0.5	1.43	9.00
1.0	2.18	8.25
1.5	2.48	7.95
2.0	2.76	7.67
2.5	2.96	7.47
3.0	3.18	7.25
4.0	3.42	7.01
5.0	3.79	6.64
6.0	3.89	6.54
10.0	3.97	6.46
12.0	4.02	6.41
14.0	4.07	6.36
16.0	4.19	6.24
18.0	4.21	6.22

GRAFICO RECUPERACION - HAYASHI

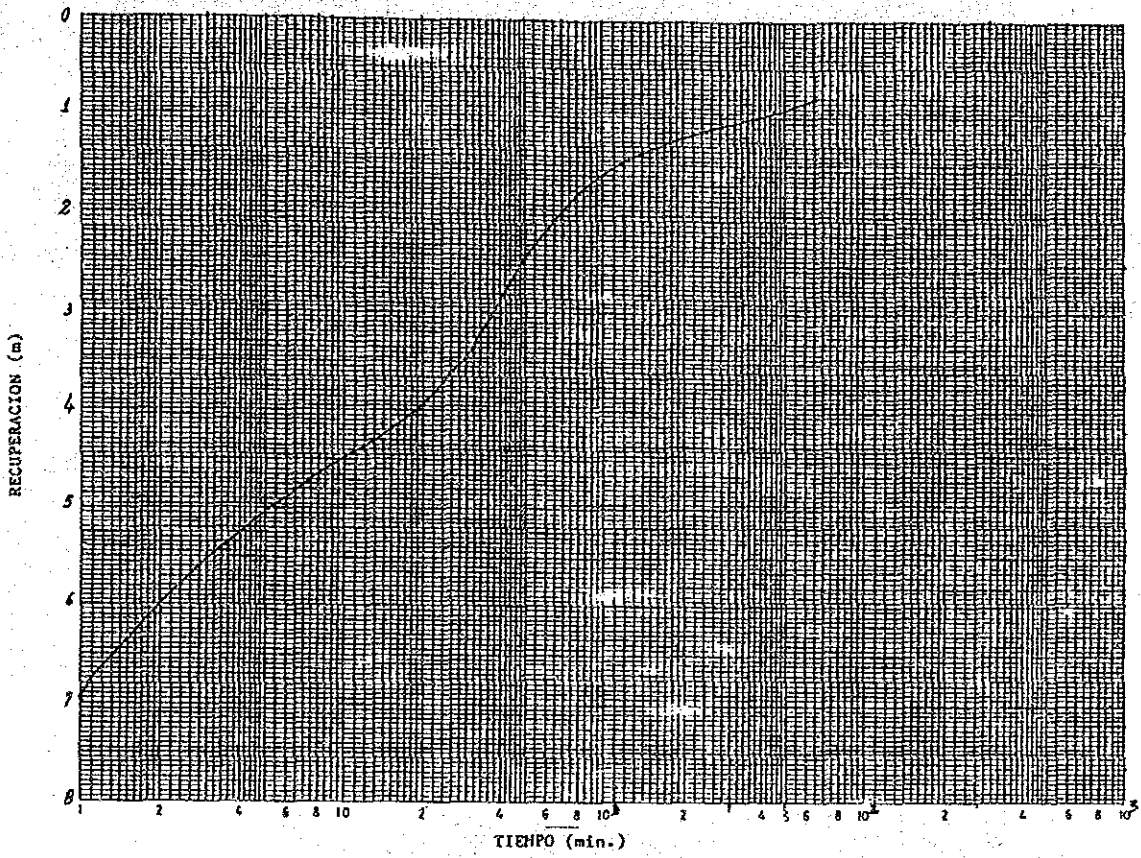
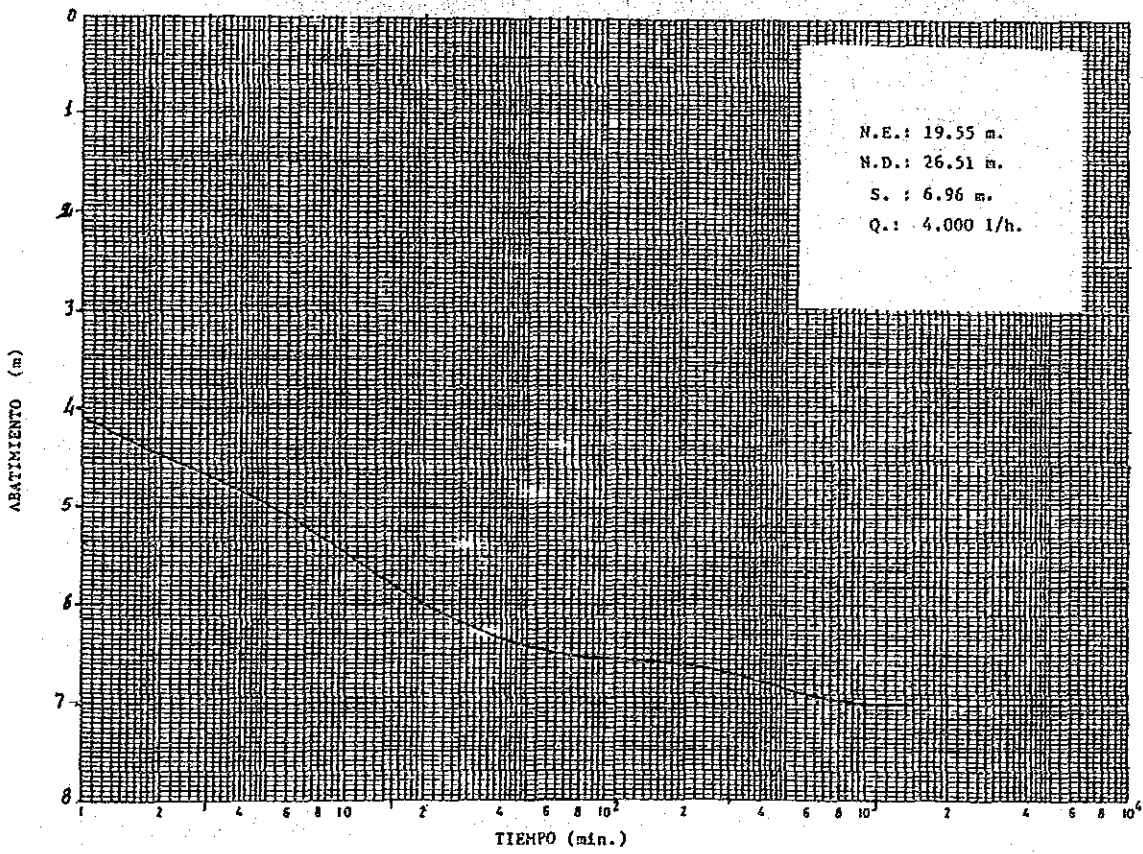
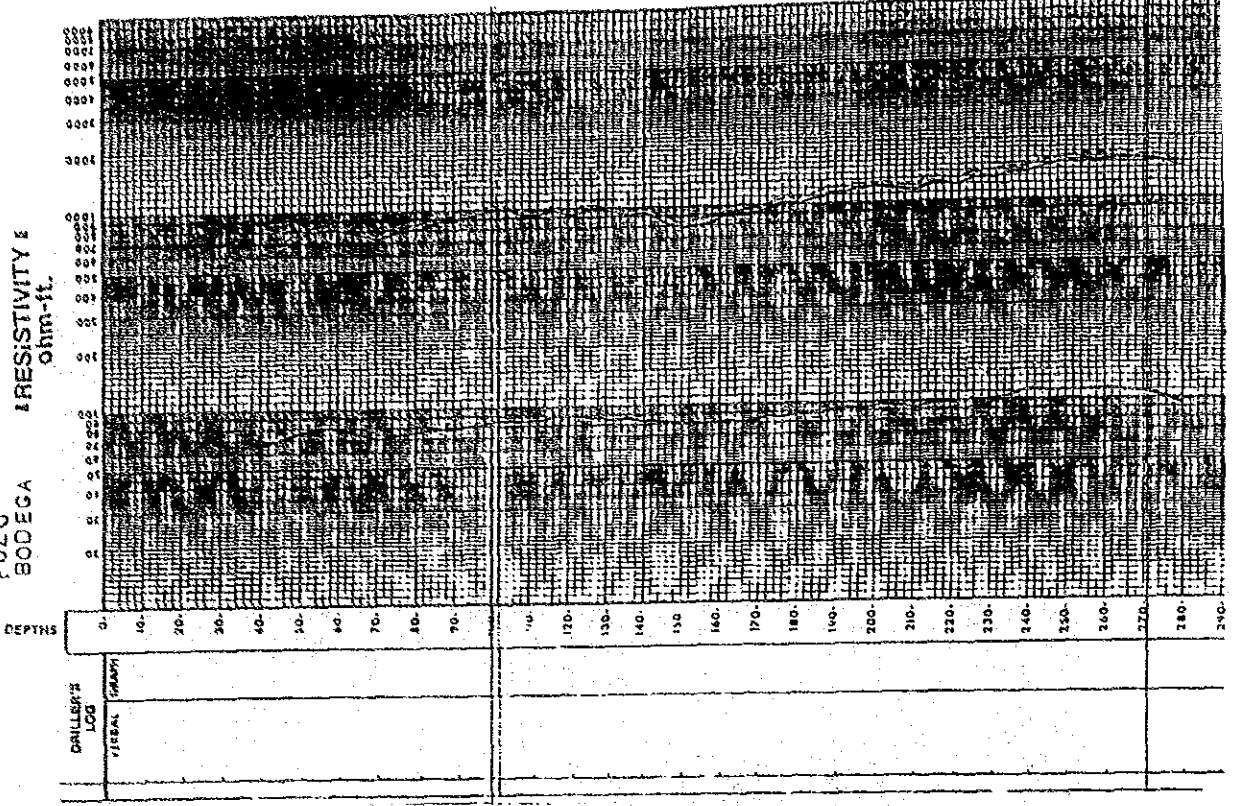


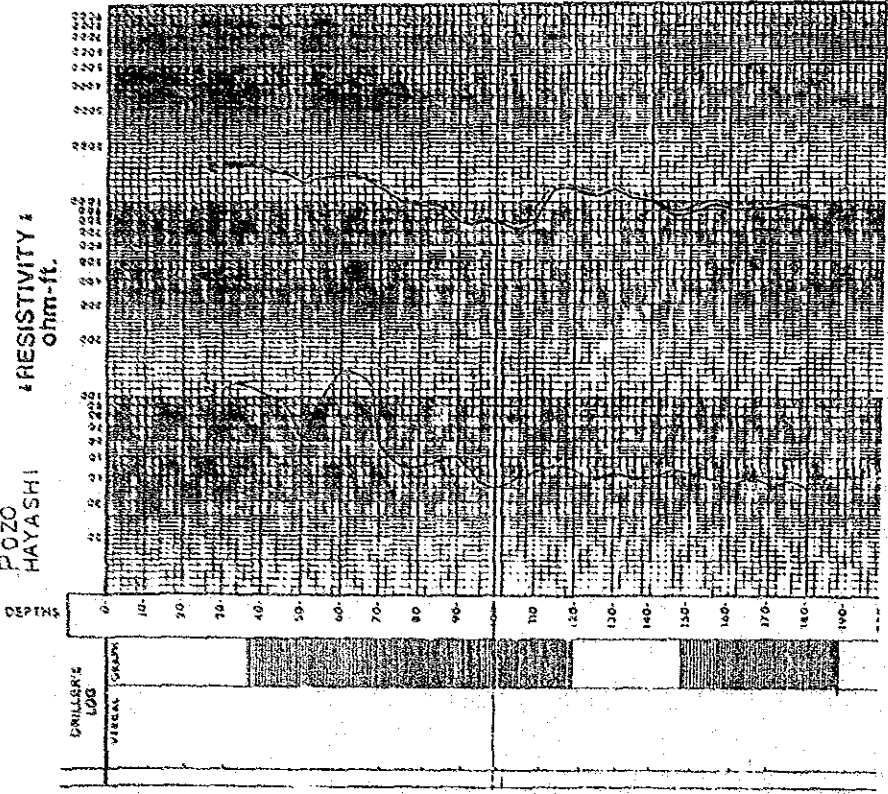
GRAFICO ABATIMIENTO - HAYASHI



POZO BODEGA



POZO HAYASHI



P O Z O S E K I

El primer pozo a ser perforado en la propiedad del señor Seki fue iniciado el día 1° de septiembre de 1988 con una máquina a percusión. El día antes fue realizada la limpieza del terreno para la introducción a la misma de la máquina perforadora.

El día 3 de septiembre del mismo año la perforación se encontraba en 8 metros de profundidad y se presentaban problemas de estabilidad del suelo perforado, por lo tanto hubo necesidad de utilizar bentonita para contrarrestar el desmoronamiento del terreno que estaba siendo perforado.

El día 6 de septiembre del mismo año la perforación se encontraba en 20 metros de profundidad pero continuaban los problemas de desmoronamiento.

Hasta el día 14 de septiembre del mismo año no se habían podido superar los problemas de desmoronamiento en el pozo y el mismo continuaba en 20 metros.

Debido a todos los problemas anteriormente citados se decidió avanzar la máquina perforadora unos 10 metros al frente e iniciar una nueva perforación a percusión, tampoco se tuvo éxito.

Finalmente el día 20 de octubre de 1988 con otra máquina esta vez rotativa se consigue perforar hasta los 42 metros encontrándose grandes cantos rodados que aprisionaban la herramienta de perforación por lo cual no fue posible continuar la perforación.

Cuando se decidió el encamisado del pozo de 42 metros de profundidad la sorpresa fue que peones mal intencionados echaron madera de grandes dimensiones dentro del pozo y por lo tanto fue necesario perforar otro pozo, el tercero en la misma propiedad, el cual se inició con una máquina rotativa a las 06;00 horas del día 3 de diciembre concluyéndose a las 18;00 horas del día 4 de diciembre de 1988.

El bombeo del mismo fue realizado los días 5, 6 y 15 de diciembre de 1988 y los resultados son los siguientes:

Pozo Seki (continuación)

Caudal de Ensayo:	10.000 litros/hora = 2,78 l/s
Profundidad:	40 metros
Tiempo de Ensayo:	26 horas + 7 horas
Nivel Estático:	9,40 metros
Nivel Dinámico 1 hora:	15,40 metros
Nivel Dinámico 2 horas:	15,80 metros
Nivel Dinámico 3 horas:	15,80 metros
Nivel Dinámico 4 horas:	15,80 metros
.....	
Nivel Dinámico 26 horas:	15,80 metros
Abatimiento:	6,40 metros
Caudal Específico:	1.563 l/hxm = 0,43 l/sxm
Recuperación:	5 minutos 15 segundos

Como en las cercanías de este pozo no existían otros pozos de agua no fue posible hacer la medición de los descensos de nivel de los mismos.

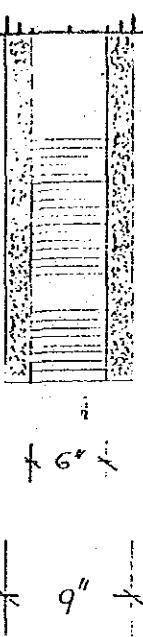
Cuando fue realizada la segunda perforación, la cual no pudo ser encamisada, la arenisca recién fue encontrada a los 30,50 metros y sin embargo en la tercera perforación ella fue hallada desde los 24,30 metros en adelante.

En la segunda perforación a los 42 metros se encontraron grandes cantos rodados que atrapaban la herramienta y en la tercera perforación se halló CONGLOMERADO hacia el final del pozo.

Los pozos no encamisados fueron sellados con hormigón armado para evitar posibles accidentes y además se instaló una alambrada con un cartel indicador de la expresa prohibición de ingresar a la propiedad donde se encuentra el pozo ordenado por LA MISION.

PERFIL DE POZO PERFORADO

Nombre: S E K I

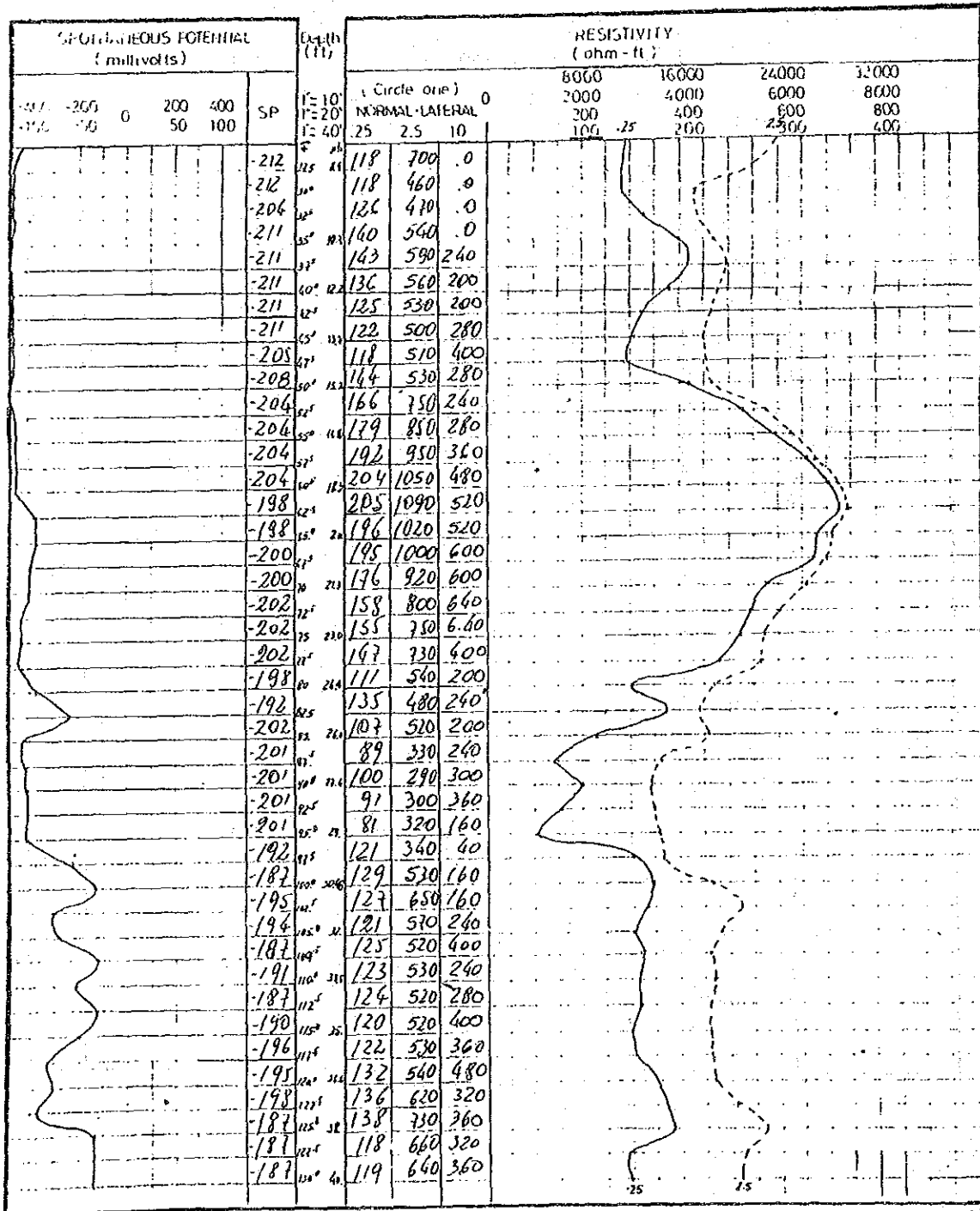
	PROF (m)	SIMBOLO	DESCRIPCION	FORM
	0		Arcilla limosa, color rojizo. Cuat.	
	10		Arenisca limosa, parda amarillenta, granos brechosos de cuarzo.	
	20		Arenisca, grano fino a medio.	
	30		Arenisca con predominio de cristales brechosos de sílice. Color claro a oscuro. CONGLOMERADO.	
	40			
	50			
	60			
	70			
	80			
	90			
	100			
	110			
	120			
	130			
	140			
	150			
	160			
	170			

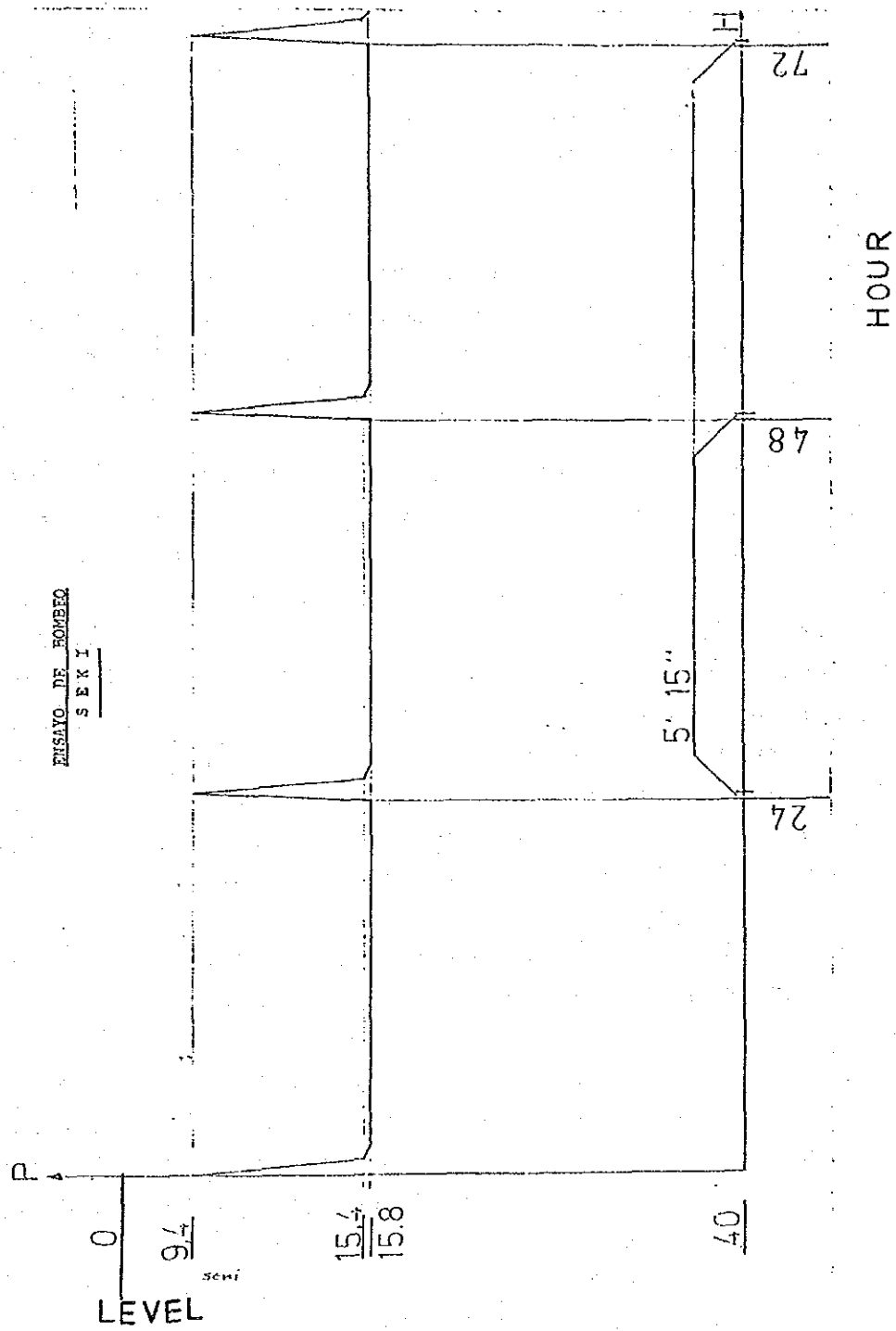
Seccion tecnica
 ADERACION - TABLA 11

SISTEMA DE PERFILAJE ELECTRICO MANUAL
 (Johnson-Reel, DR 70)

POZO 119
 UBICACION " LA COLMENA "
 FECHA DE LA PERFORACION 3-XII-88
 INICIO 3-XII-88 FIN 4-XII-88
 OPERADOR DEL PERFILAJE
 FECHA DE PERFILAJE 4-XII-88
 NIVEL ESTATICO 9,40 m
 PROFUNDIDAD DE LA PERFORACION 40 m
 PROFUNDIDAD PERFILADA 40 m

PROPIEDAD Seki
 MAQUINA QUE PERFORO FAILING 250
 OPERADOR Edgar Ello Franco
 DIAMETRO DE LA PERFORACION 9"
 INYECCION USADA DURANTE LA PERFORACION
 Arcilla Natural (Nai-6)
 RESISTIVIDAD DEL LODO:
 METODO DE PERFORACION Rotativo





P O Z O C H A V E Z

El día 23 de septiembre de 1988 fue marcado en el terreno el lugar de la perforación, el 26 de septiembre fue instalada en el lugar la máquina perforadora a percusión que haría los trabajos de perforación.

Iniciada la perforación en el día 28 de septiembre esta se encontraba en 3 metros y para el día 30 alcanzaba los 7 metros.

El día 11 de octubre del mismo año la perforación se encontraba en 52 metros y a esa profundidad se cortó la barra de perforación. Inmediatamente se inició la tarea de pesca de la barra la cual fue extraída del pozo el día 23 de octubre de 1988.

El día 29 de octubre del mismo año la perforación alcanzaba 57 metros y la herramienta quedó atrapada en el pozo a causa de los cantos rodados que había en el mismo. Esta herramienta no pudo ser recuperada pese al esfuerzo que fue realizado y recién el día 22 de noviembre se inició un nuevo pozo el cual fue concluido el día 9 de diciembre de 1988 con una profundidad de 52 metros y con el siguiente resultado: el pozo no tiene agua y por lo tanto se ordenó el encamisado del pozo viejo para realizar los estudios de acuífero correspondiente. El encamisado del pozo viejo se hizo el día 12 de diciembre de 1988 y en una longitud de 28 metros.

Fueron realizados los ensayos correspondientes con resultados totalmente negativos ya que ninguno de los dos pozos contenían agua.

El agua que fluía a los pozos respondía al de un estero que se encuentra a 17,50 metros de distancia de los pozos y solamente a los 3 metros de profundidad se detectó una pequeña entrada de agua y analizada esta agua resultó ser filtración del agua del estero hacia los pozos.

En el pozo de 52 metros fluían 400 litros/hora.

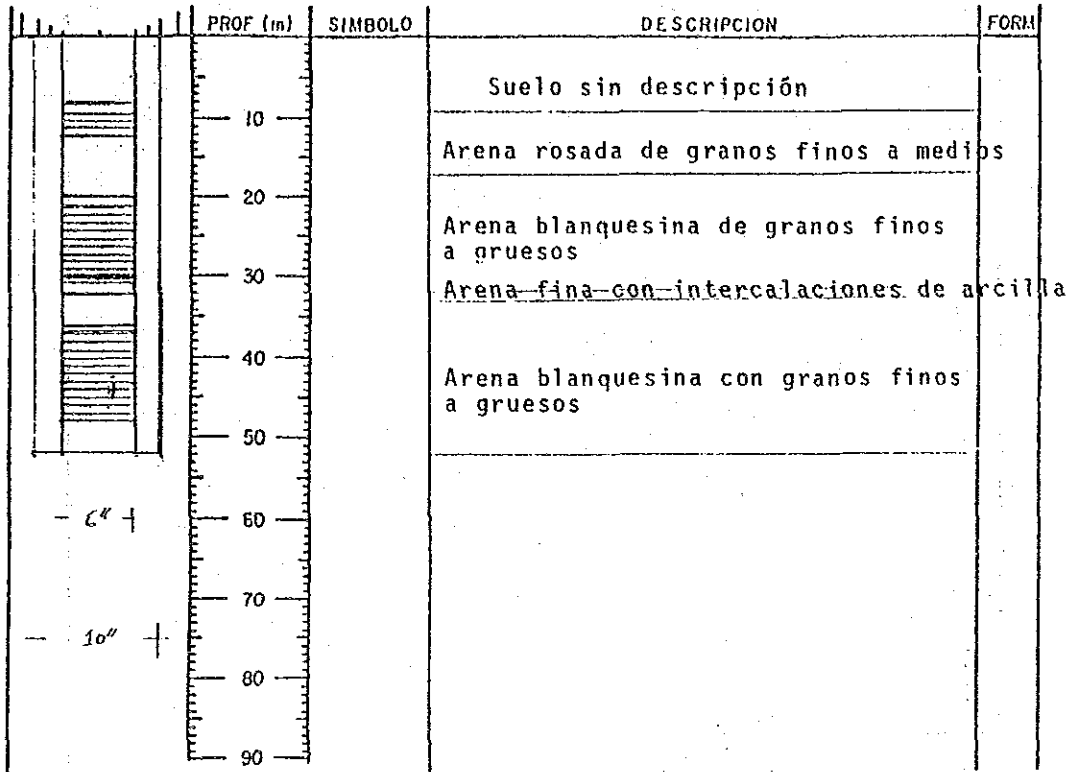
En el pozo de 28 metros fluían 700 litros/hora.

En ningún momento el agua de ninguno de los dos pozos adquirió transparencia, siempre tuvo el color GRIS del agua del estero adyacente a los pozos.

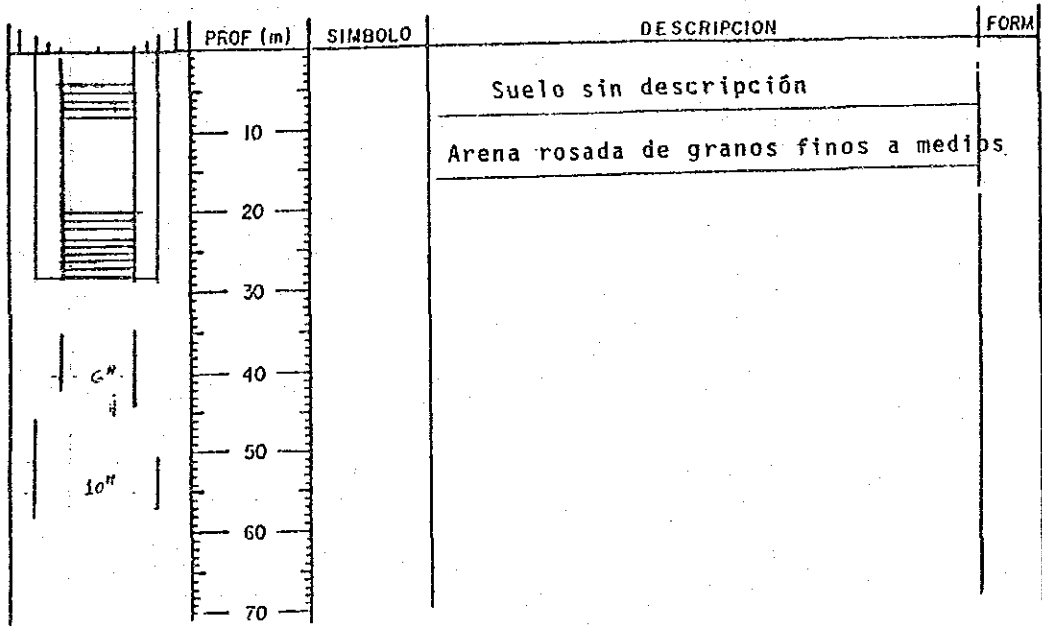
La recuperación del nivel de los pozos era de 25 horas 10 minutos y el nivel estático es de 1,00 metros.

PERFIL DE POZO PERFORADO

Nombre: C H A V E Z

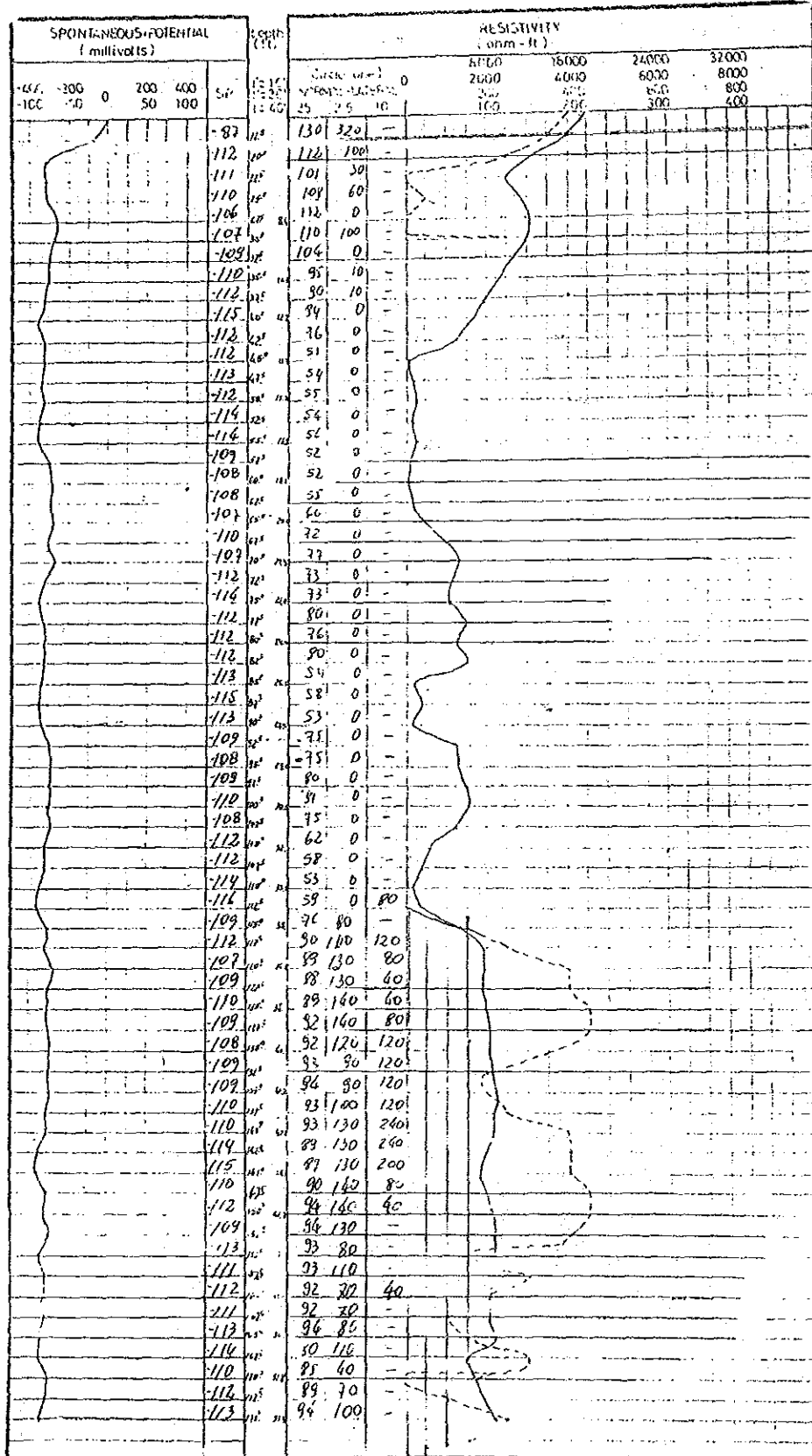


Nombre: C H A V E Z



UBICACIÓN: La Colmena
 FECHA DE LA PERFORACIÓN:
 INICIO: Fin
 OPERACIÓN DEL PERFORAJE:
 FECHA DE PERFORACIÓN: 8-diciembre-1988
 NIVEL ESTÁTICO: 1,00 metro
 PROFUNDIDAD DE LA PERFORACIÓN: 52 m.
 PROFUNDIDAD PERFORADA:

PROPIEDAD: Chávez
 MÁQUINA QUE PERFORÓ: Felling 250
 OPERADOR: Edgar Elío Franco
 DIÁMETRO DE LA PERFORACIÓN: 10"
 SOLUCIÓN USADA DURANTE LA PERFORACIÓN:
 VELOCIDAD DEL LOG:
 MÉTODO DE PERFORACIÓN: Combinado



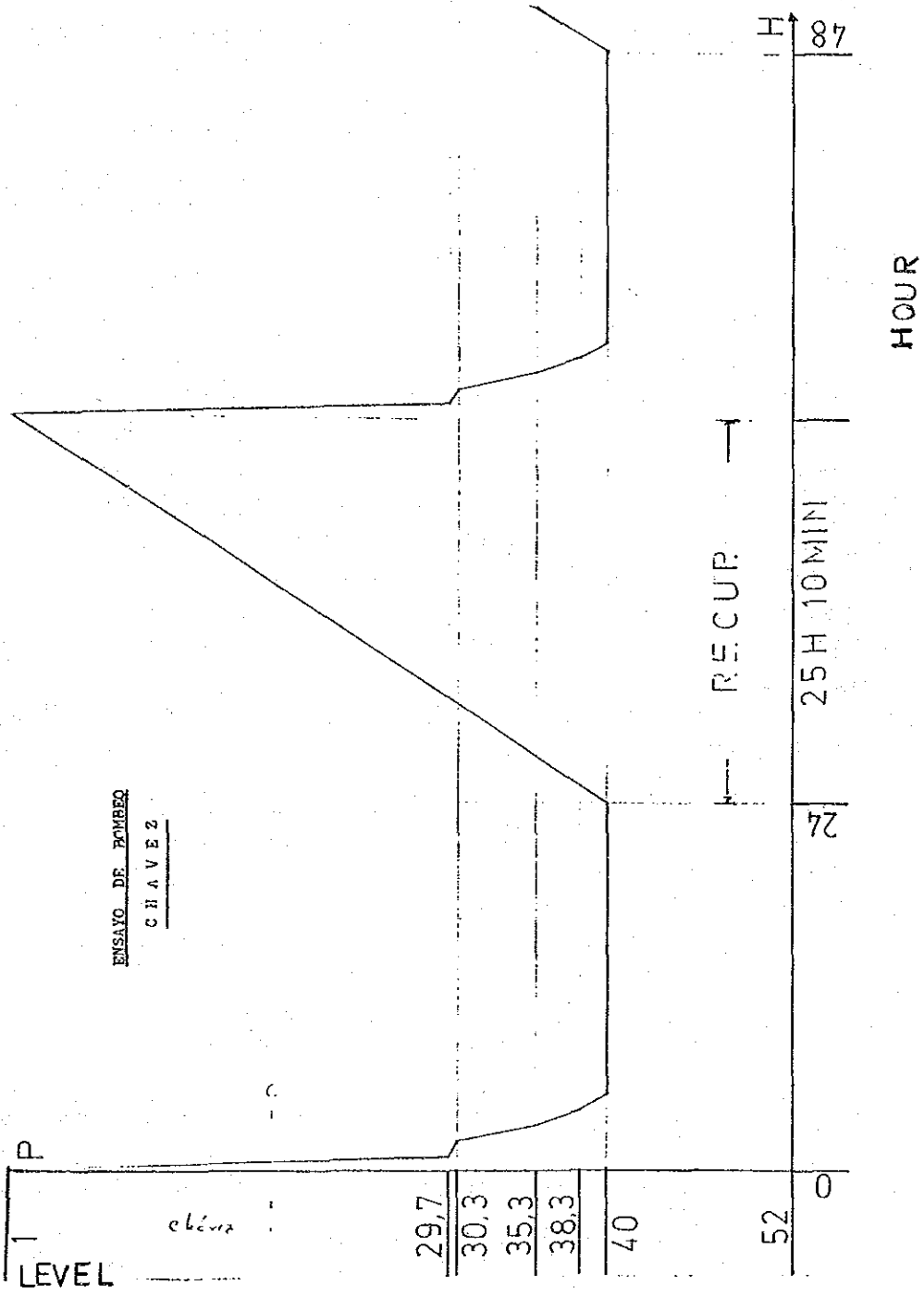


Table B.1.1 Data of Electrical Prospecting Survey (4)

a(m)	2πa	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82
1	6.28	389.621	181.710	78.668	41.900	434.420	75.084	312.956	4010.580	302.724	338.096	892.841	390.689	1938.160	220.791	398.543	164.305	115.422	360.021	1719.910	765.921	170.714	131.947
2	12.57	135.717	172.030	81.230	21.319	121.406	105.164	332.250	1487.990	82.542	227.703	298.766	189.007	757.357	163.237	368.521	142.619	96.558	112.344	3371.900	474.241	147.547	101.411
3	18.85	87.013	117.089	107.958	59.249	71.096	110.784	251.827	958.106	80.961	183.238	248.773	109.207	497.529	131.353	388.921	132.069	72.528	333.444	3717.831	154.567	106.240	
4	25.14	64.976	86.698	140.319	47.278	52.230	107.588	187.018	678.258	80.367	141.287	200.078	78.263	365.842	104.616	367.080	126.529	51.038	273.880	3976.859	176.138	101.812	
5	31.43	49.573	66.785	170.207	38.122	39.039	102.319	133.728	518.412	80.700	112.977	160.372	58.198	251.018	84.101	300.543	118.886	36.608	240.258	4200.258	195.946	98.978	
6	37.72	39.876	52.569	202.366	31.342	31.107	102.020	103.043	389.758	48.160	90.394	177.093	48.306	173.903	72.265	235.428	130.429	30.799	201.067	4540.258	238.500	82.068	
7	44.01	31.633	39.789	230.385	26.037	26.037	99.939	85.043	302.306	43.322	77.364	199.810	38.336	130.900	60.019	180.478	124.768	24.242	142.450	4840.258	250.070	67.242	
8	50.30	24.986	30.729	257.994	22.268	22.268	98.414	73.820	241.122	41.160	68.114	188.968	34.242	105.619	50.222	168.558	109.701	20.007	124.752	5140.258	271.774	53.800	
9	56.59	20.012	24.141	284.584	19.198	19.198	97.846	63.822	184.412	40.000	61.672	195.098	32.784	95.379	44.842	153.855	95.379	17.000	108.908	5340.258	293.578	43.401	
10	62.88	16.458	19.717	310.884	16.458	16.458	97.278	58.422	157.584	38.876	57.078	204.098	31.558	87.842	41.842	145.222	90.689	15.300	96.774	5540.258	315.482	36.000	
11	69.17	13.844	16.241	337.184	14.322	14.322	96.710	54.000	136.412	38.000	54.608	213.198	30.798	79.842	39.842	137.842	84.558	13.900	91.608	5740.258	337.394	30.000	
12	75.46	11.844	13.422	363.484	12.570	12.570	96.142	50.000	115.242	37.142	51.142	222.318	29.642	73.842	38.798	130.000	80.000	12.700	87.558	5940.258	359.306	27.000	
13	81.75	10.344	11.344	389.784	11.344	11.344	95.574	46.142	104.098	36.286	48.286	231.438	28.642	67.842	37.754	122.142	80.000	11.600	83.500	6140.258	381.218	24.000	
14	88.04	9.144	9.844	416.084	10.422	10.422	95.006	42.286	93.000	35.422	45.422	240.558	27.642	63.842	36.710	114.286	80.000	10.500	79.458	6340.258	403.130	21.000	
15	94.33	8.144	8.644	442.384	9.644	9.644	94.438	38.574	81.842	34.574	42.574	249.678	26.642	60.000	35.574	106.422	80.000	9.400	75.400	6540.258	425.042	18.000	
16	100.62	7.344	7.844	468.684	8.944	8.944	93.884	35.000	70.642	33.718	39.642	258.798	25.642	56.000	34.438	98.574	80.000	8.300	71.358	6740.258	446.954	15.000	
17	106.91	6.644	7.144	494.984	8.344	8.344	93.338	31.574	59.486	32.862	36.710	267.918	24.642	52.000	33.386	90.642	80.000	7.200	67.300	6940.258	468.866	12.000	
18	113.20	6.044	6.544	521.284	7.844	7.844	92.792	28.142	48.342	32.006	33.754	277.038	23.642	48.000	32.342	82.700	80.000	6.100	63.158	7140.258	490.778	9.000	
19	119.49	5.544	6.044	547.584	7.444	7.444	92.246	24.718	37.200	31.150	30.642	286.158	22.642	44.000	31.386	74.842	80.000	5.000	59.000	7340.258	512.690	6.000	
20	125.78	5.144	5.644	573.884	7.044	7.044	91.700	21.286	26.042	30.294	29.574	295.278	21.642	40.000	30.438	66.942	80.000	4.000	54.858	7540.258	534.602	3.000	
21	132.07	4.744	5.244	600.184	6.644	6.644	91.154	17.862	14.942	29.446	28.506	304.398	20.642	36.000	29.586	59.000	80.000	3.000	50.700	7740.258	556.514	0.000	
22	138.36	4.444	4.944	626.484	6.244	6.244	90.608	14.438	10.842	28.700	27.438	313.518	19.642	32.000	28.638	53.000	80.000	2.000	46.558	7940.258	578.426	0.000	
23	144.65	4.144	4.644	652.784	5.844	5.844	90.062	11.022	7.742	28.000	26.386	322.638	18.642	28.000	27.686	47.000	80.000	1.000	42.400	8140.258	600.338	0.000	
24	150.94	3.844	4.344	679.084	5.444	5.444	89.516	7.606	5.642	27.254	25.338	331.758	17.642	24.000	26.838	41.000	80.000	0.500	38.258	8340.258	624.250	0.000	
25	157.23	3.644	4.144	705.384	5.144	5.144	89.000	5.000	4.142	26.506	24.186	340.878	16.642	20.000	25.986	35.000	80.000	0.000	34.100	8540.258	648.162	0.000	
26	163.52	3.444	3.944	731.684	4.844	4.844	88.454	3.574	2.942	25.758	23.038	350.000	15.642	16.000	25.138	29.000	80.000	0.000	29.958	8740.258	672.074	0.000	
27	169.81	3.244	3.744	757.984	4.544	4.544	87.908	2.142	1.842	25.010	21.886	359.120	14.642	12.000	24.286	23.000	80.000	0.000	25.800	8940.258	695.986	0.000	
28	176.10	3.044	3.544	784.284	4.244	4.244	87.362	0.942	1.242	24.262	20.738	368.240	13.642	8.000	23.438	17.000	80.000	0.000	21.658	9140.258	719.898	0.000	
29	182.39	2.844	3.344	810.584	3.944	3.944	86.816	0.000	0.542	23.514	19.586	377.360	12.642	4.000	22.586	11.000	80.000	0.000	17.500	9340.258	743.810	0.000	
30	188.68	2.644	3.144	836.884	3.644	3.644	86.270	0.000	0.142	22.766	18.438	386.480	11.642	0.000	21.738	5.000	80.000	0.000	13.358	9540.258	767.722	0.000	
31	194.97	2.444	2.944	863.184	3.344	3.344	85.724	0.000	0.000	22.018	17.286	395.600	10.642	0.000	20.886	1.000	80.000	0.000	9.200	9740.258	791.634	0.000	
32	201.26	2.244	2.744	889.484	3.044	3.044	85.178	0.000	0.000	21.270	16.138	404.720	9.642	0.000	20.038	0.000	80.000	0.000	5.058	9940.258	815.546	0.000	
33	207.55	2.044	2.544	915.784	2.744	2.744	84.632	0.000	0.000	20.522	15.000	413.840	8.642	0.000	19.186	0.000	80.000	0.000	0.900	10140.258	839.458	0.000	
34	213.84	1.844	2.344	942.084	2.444	2.444	84.086	0.000	0.000	19.774	13.858	422.960	7.642	0.000	18.336	0.000	80.000	0.000	0.000	10340.258	863.370	0.000	
35	220.13	1.644	2.144	968.384	2.144	2.144	83.540	0.000	0.000	19.026	12.710	432.080	6.642	0.000	17.486	0.000	80.000	0.000	0.000	10540.258	887.282	0.000	
36	226.42	1.444	1.944	994.684	1.844	1.844	83.000	0.000	0.000	18.278	11.562	441.200	5.642	0.000	16.636	0.000	80.000	0.000	0.000	10740.258	911.194	0.000	
37	232.71	1.244	1.744	1020.984	1.544	1.544	82.454	0.000	0.000	17.530	10.414	450.320	4.642	0.000	15.786	0.000	80.000	0.000	0.000	10940.258	935.106	0.000	
38	239.00	1.044	1.544	1047.284	1.244	1.244	81.908	0.000	0.000	16.782	9.266	459.440	3.642	0.000	14.936	0.000	80.000	0.000	0.000	11140.258	959.018	0.000	
39	245.29	0.844	1.344	1073.584	0.944	0.944	81.362	0.000	0.000	16.034	8.118	468.560	2.642	0.000	14.086	0.000	80.000	0.000	0.000	11340.258	982.930	0.000	
40	251.58	0.644	1.144	1109.884	0.644	0.644	80.816	0.000	0.000	15.286	7.000	477.680	1.642	0.000	13.236	0.000	80.000	0.000	0.000	11540.258	1006.842	0.000	
41	257.87	0.444	0.944	1136.184	0.344	0.344	80.270	0.000	0.000	14.538	5.852	486.800	0.642	0.000	12.386	0.000	80.000	0.000	0.000	11740.258	1030.754	0.000	
42	264.16	0.244	0.744	1162.484	0.044	0.044	79.724	0.000	0.000	13.790	4.704	495.920	0.000	0.000	11.536	0.000	80.000	0.000	0.000	11940.258	1054.666	0.000	
43	270.45	0.044	0.544	1188.784	0.000	0.000	79.178	0.000	0.000	13.042	3.556	505.040	0.000	0.000	10.686	0.000	80.000	0.000	0.000	12140.258	1078.578	0.000	
44	276.74	0.000	0.																				

Table B.1.1 Data of Electrical Prospecting Survey

No.	No. TRANQUERA										unit: $\rho \tau \Omega$	
	101	102	103	104	105	201	202	203	204	205		
1	5.28	1528.070	1034.840	60.394	177.626	826.233	1086.990	2554.120	260.081	2585.530	9424.780	-
2	12.57	1893.900	491.345	41.491	153.938	321.071	1864.850	4380.370	279.225	1641.170	9872.150	-
3	18.85	1310.420	266.156	37.077	157.300	170.853	2177.130	3604.040	285.341	642.016	9036.800	-
4	25.13	781.880	176.181	26.906	137.476	118.979	2073.700	2651.510	274.701	550.712	8241.320	-
5	31.42	562.031	133.288	40.841	105.620	99.714	1885.470	1728.510	332.695	454.589	5881.070	-
6	37.70	242.028	121.994	40.263	91.458	82.486	1541.140	1152.840	433.163	346.116	4339.170	-
7	43.98	111.803	331.451	38.177	74.110	67.821	1073.170	972.010	454.337	346.449	3637.340	-
8	50.27	72.282	100.531	41.678	66.149	55.946	1136.000	890.202	476.819	367.743	2463.510	-
9	56.55	48.179	87.424	41.337	63.674	47.840	1124.190	838.052	478.628	394.314	1659.890	-
10	62.83	33.886	75.901	40.087	54.224	42.726	1105.210	764.035	507.745	415.507	1502.940	-
11	69.12	26.264	70.359	39.603	41.581	33.244	1045.710	697.371	534.882	436.531	1271.790	-
12	75.40	19.830	62.807	38.694	34.382	34.457	919.105	670.592	506.752	460.307	1074.430	-
13	81.68	15.785	56.769	37.492	28.282	30.467	830.701	662.894	474.161	469.342	939.337	-
14	87.96	14.386	52.603	36.065	25.598	24.102	807.340	677.240	432.434	502.542	879.647	-
15	94.25	15.645	50.423	36.097	23.656	22.242	790.457	658.604	429.016	507.619	779.995	-
16	100.53	14.778	48.054	35.467	22.418	23.323	759.311	632.340	439.421	541.953	676.473	-
17	106.81	13.352	37.171	35.035	20.722	20.508	702.187	573.058	423.198	509.037	719.394	-
18	113.39	15.155	34.138	34.668	20.131	18.887	628.030	535.969	431.240	523.641	512.218	-
19	119.36	15.758	30.800	31.546	20.414	18.623	583.652	510.472	443.738	519.783	499.847	-
20	125.66	14.451	28.526	31.919	19.352	19.352	530.804	464.811	432.535	473.124	454.777	-
22	136.23	15.820	25.987	31.240	18.938	20.162	478.553	-	435.702	-	445.931	-
24	150.83	18.095	22.770	29.858	18.397	17.945	-	-	-	-	477.723	-
26	161.36	16.683	21.401	28.262	18.623	17.643	-	-	-	-	456.926	-
28	175.33	15.482	19.860	26.917	18.297	15.834	-	-	-	-	459.703	-
30	188.50	14.703	19.981	25.070	17.719	17.530	-	-	-	-	465.207	-
32	201.06	13.873	18.900	23.926	17.695	15.080	-	-	-	-	411.491	-
34	213.63	14.740	17.845	23.926	18.759	14.099	-	-	-	-	-	-
36	226.79	13.119	17.417	25.334	18.096	14.703	-	-	-	-	-	-
38	238.76	-	16.713	-	18.385	15.042	-	-	-	-	-	-
40	251.33	-	16.085	-	20.609	15.331	-	-	-	-	-	-
42	263.89	-	-	-	20.584	15.306	-	-	-	-	-	-
44	276.46	-	-	-	19.076	14.652	-	-	-	-	-	-
46	289.03	-	-	-	16.764	14.740	-	-	-	-	-	-
48	301.59	-	-	-	17.482	15.080	-	-	-	-	-	-
50	314.16	-	-	-	16.022	14.765	-	-	-	-	-	-
52	326.73	-	-	-	16.651	-	-	-	-	-	-	-
54	339.29	-	-	-	14.250	-	-	-	-	-	-	-
56	351.86	-	-	-	16.165	-	-	-	-	-	-	-
58	364.43	-	-	-	14.577	-	-	-	-	-	-	-
60	376.99	-	-	-	15.080	-	-	-	-	-	-	-
64	402.12	-	-	-	14.476	-	-	-	-	-	-	-

Table B.1.2 Relationship between Specific Resistance and Permeable/Impermeable Layers

Rock	Specific Resistance		
	Dry(- m)	Wet(- m)	
Permeable Layer	Gravel I	1,000 - 15,000	200 - 10,000
	Gravel & Sand	1,000 - 7,000	200 - 5,000
	Sand	300 - 7,000	100 - 700
	Conglomerate	300 - 1,800	100 - 500
	Sand Stone	200 - 2,500	100 - 500
Difficult permeate layer(A)	Loam	500 - 5,000	100 - 1,000
	Tuff	100 - 1,000	-
Difficult permeate layer(B)	Marlstone	-	100
	Oil shale	-	100
Impermeable Layer	Granite	1,000 - 10,000	-
	Andesite	200 - 10,000	-
	Basalt	20,000	-
	Crystalline schist	200 - 20,000	-
	Gneiss	200 - 20,000	-
	Lava	1,000 - 20,000	-
	Limestone	60 - 500,000	-

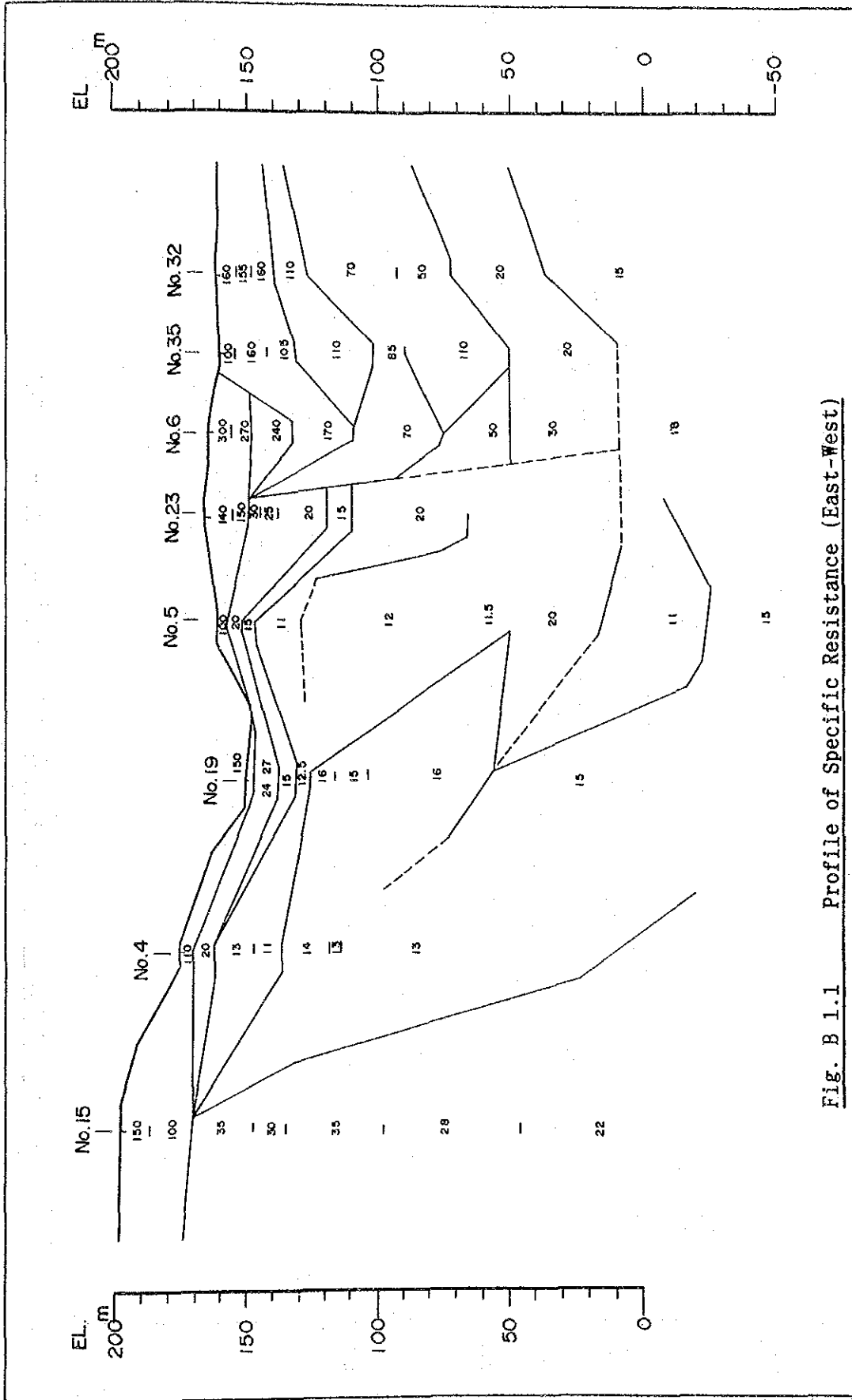


Fig. B 1.1 Profile of Specific Resistance (East-West)

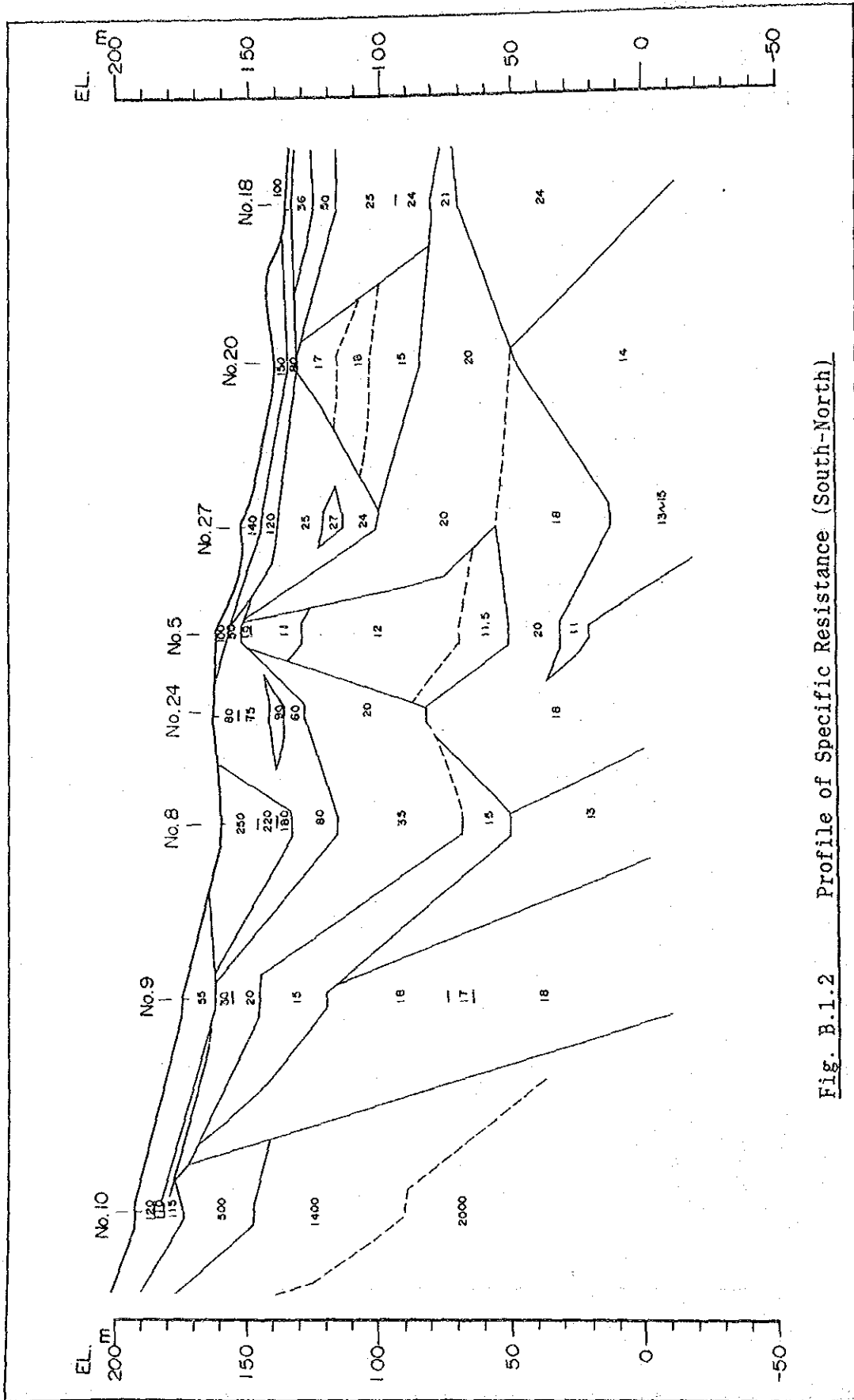


Fig. B.1.2 Profile of Specific Resistance (South-North)

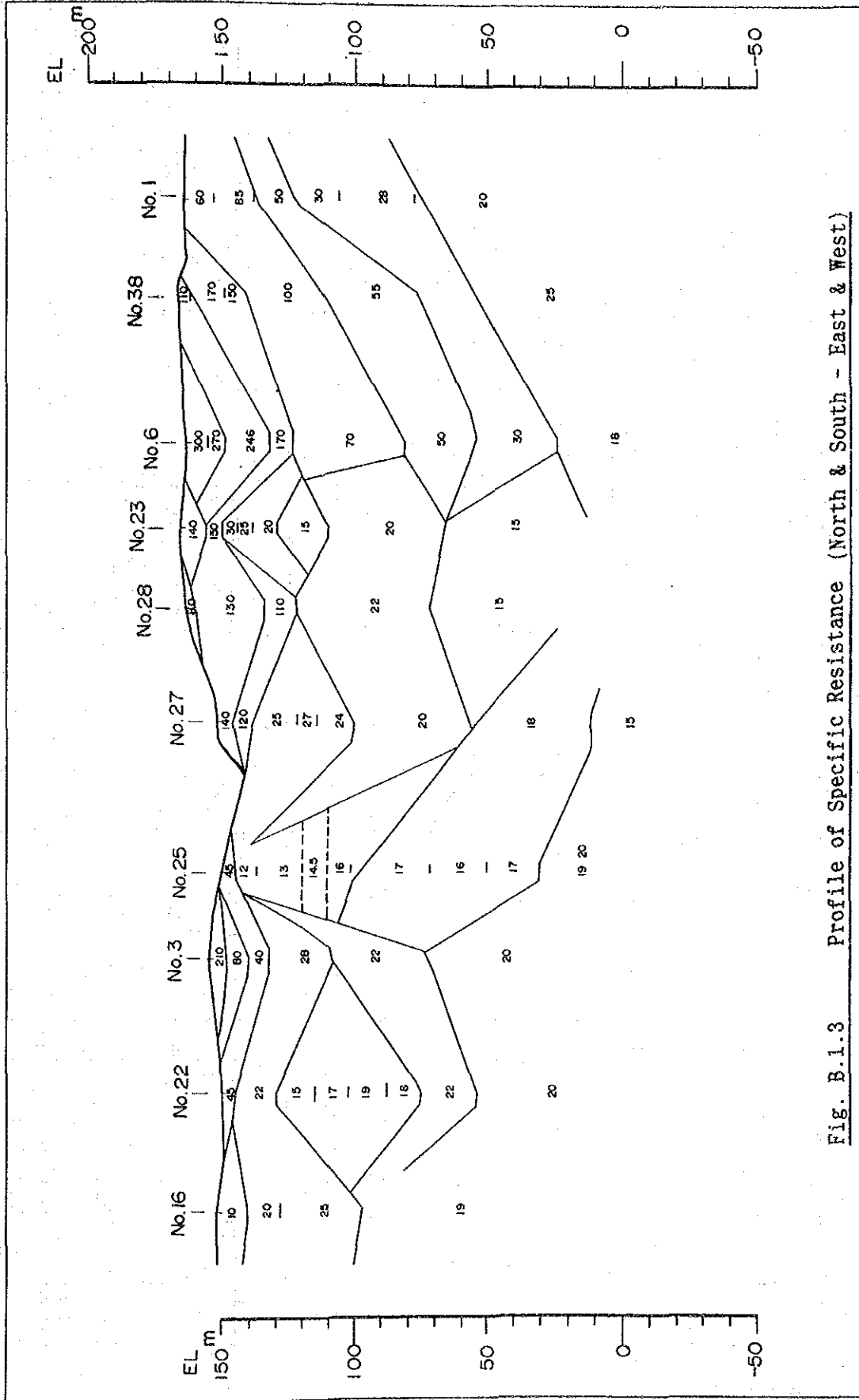


Fig. B.1.1.3 Profile of Specific Resistance (North & South - East & West)

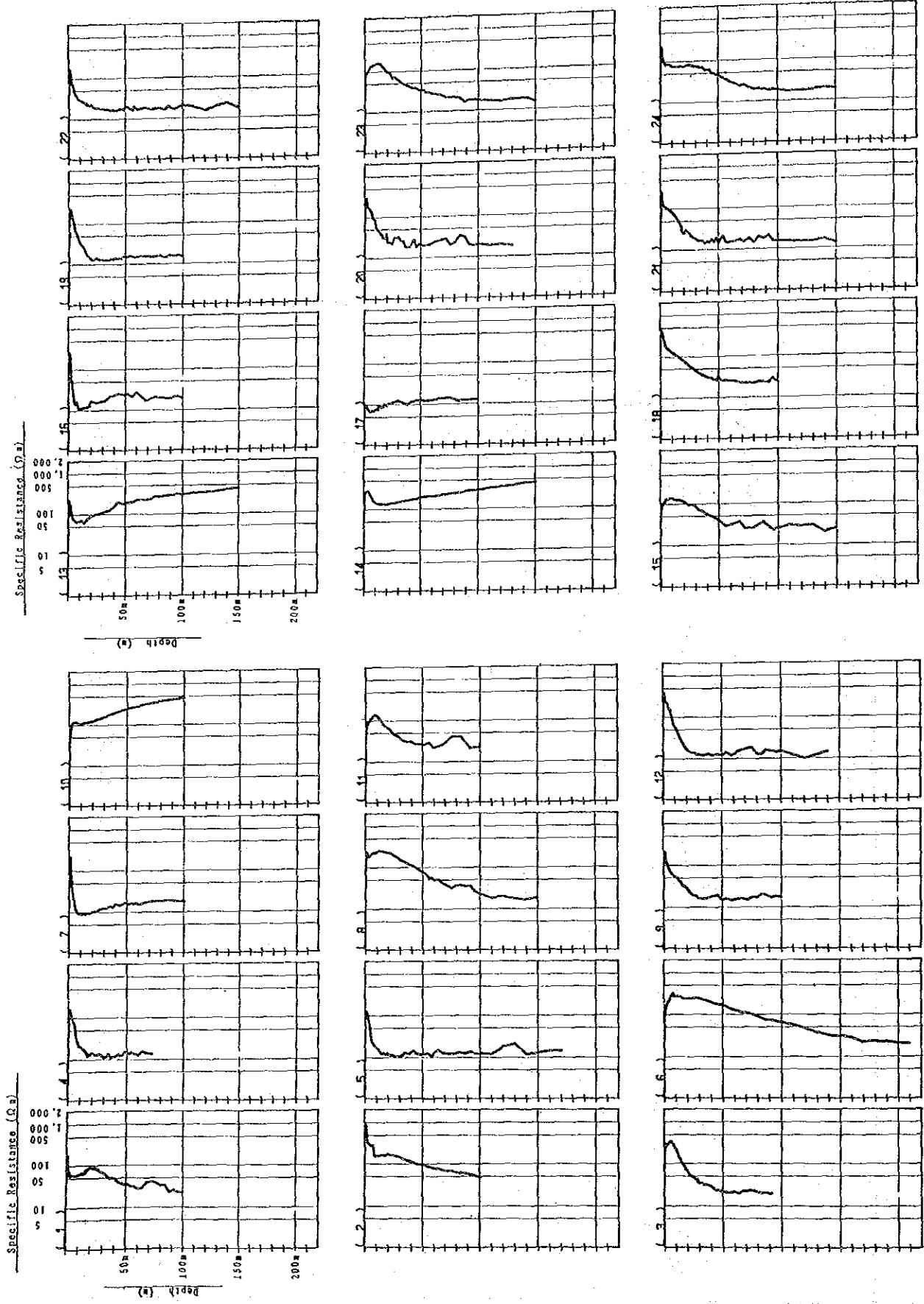


Fig. B.1.4 ρ -a Curve of Electric Potential (1)

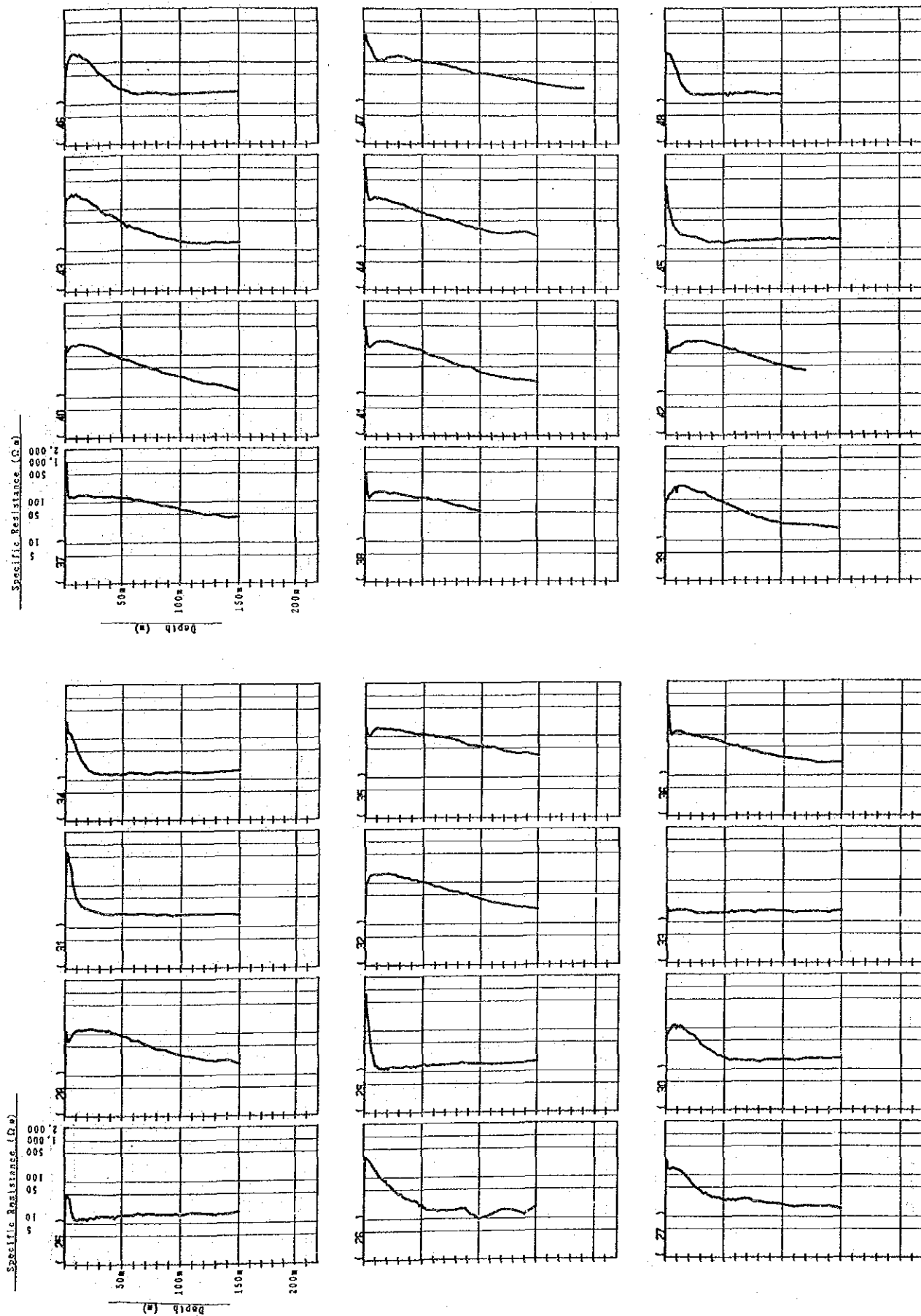


Fig. B.1.4 ρ -a Curve of Electric Potential (2)

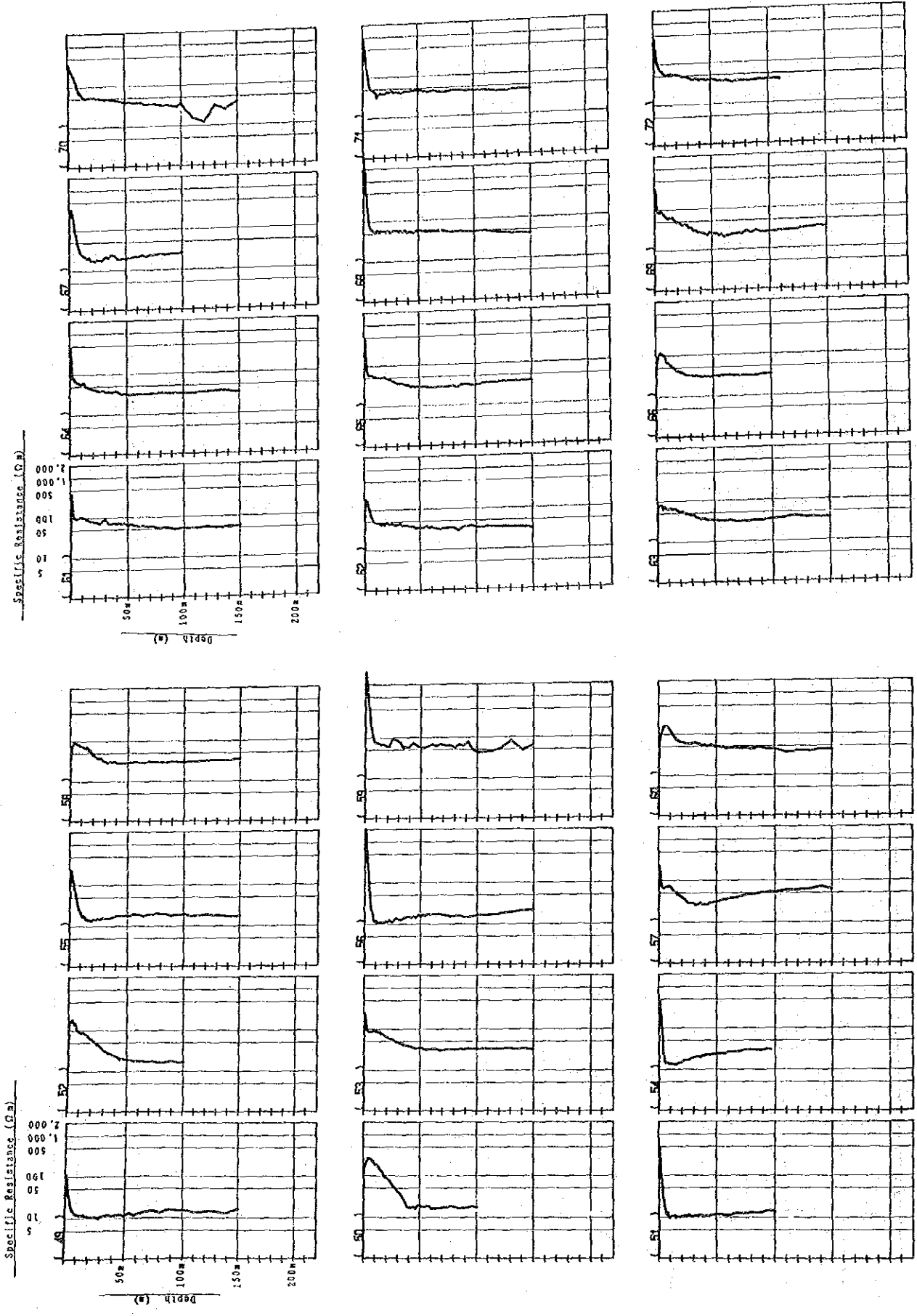


Fig. B.1.4 ρ -a Curve of Electric Potential (3)

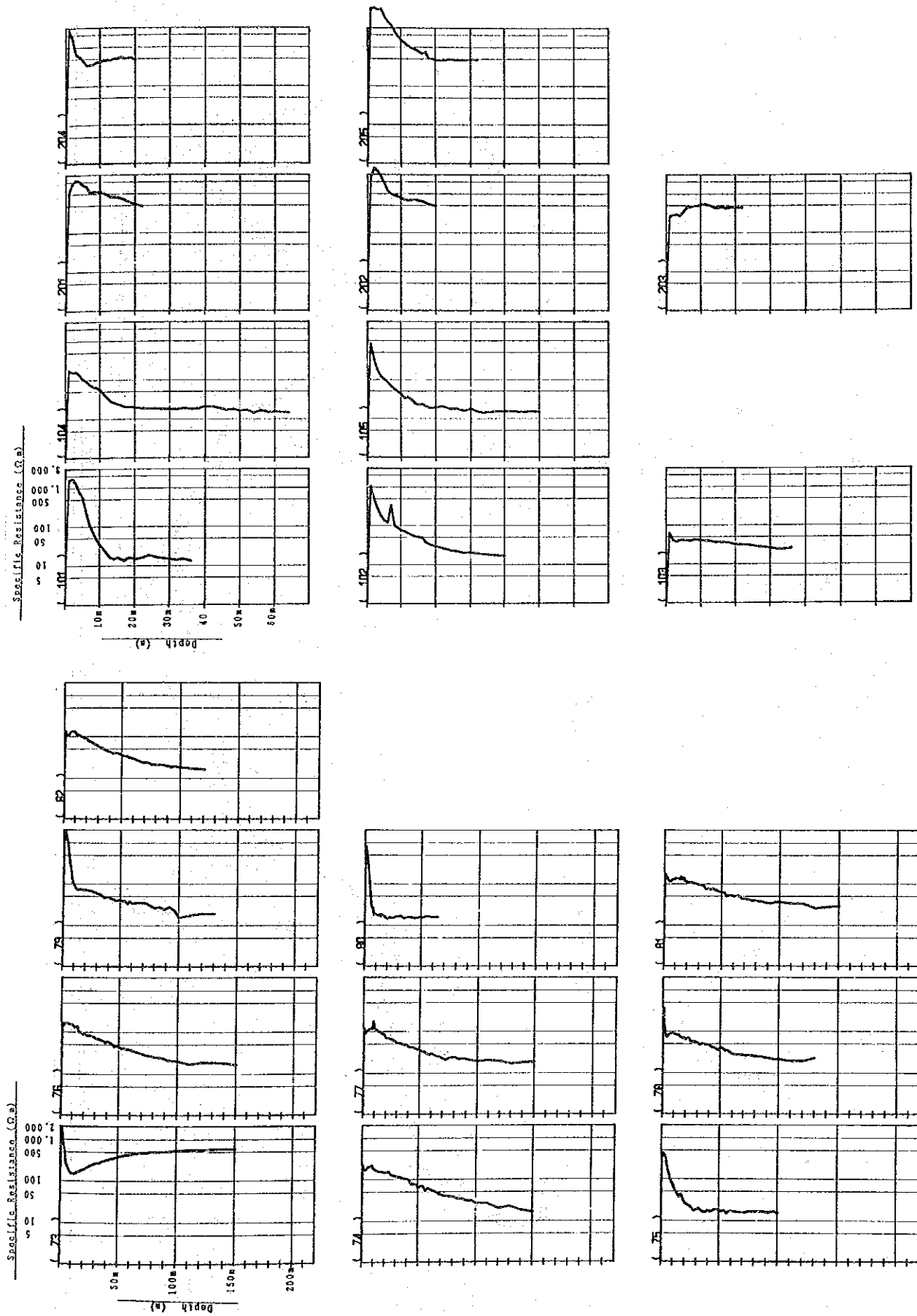


Fig. B.1.4 ρ -a Curve of Electric Potential (4)

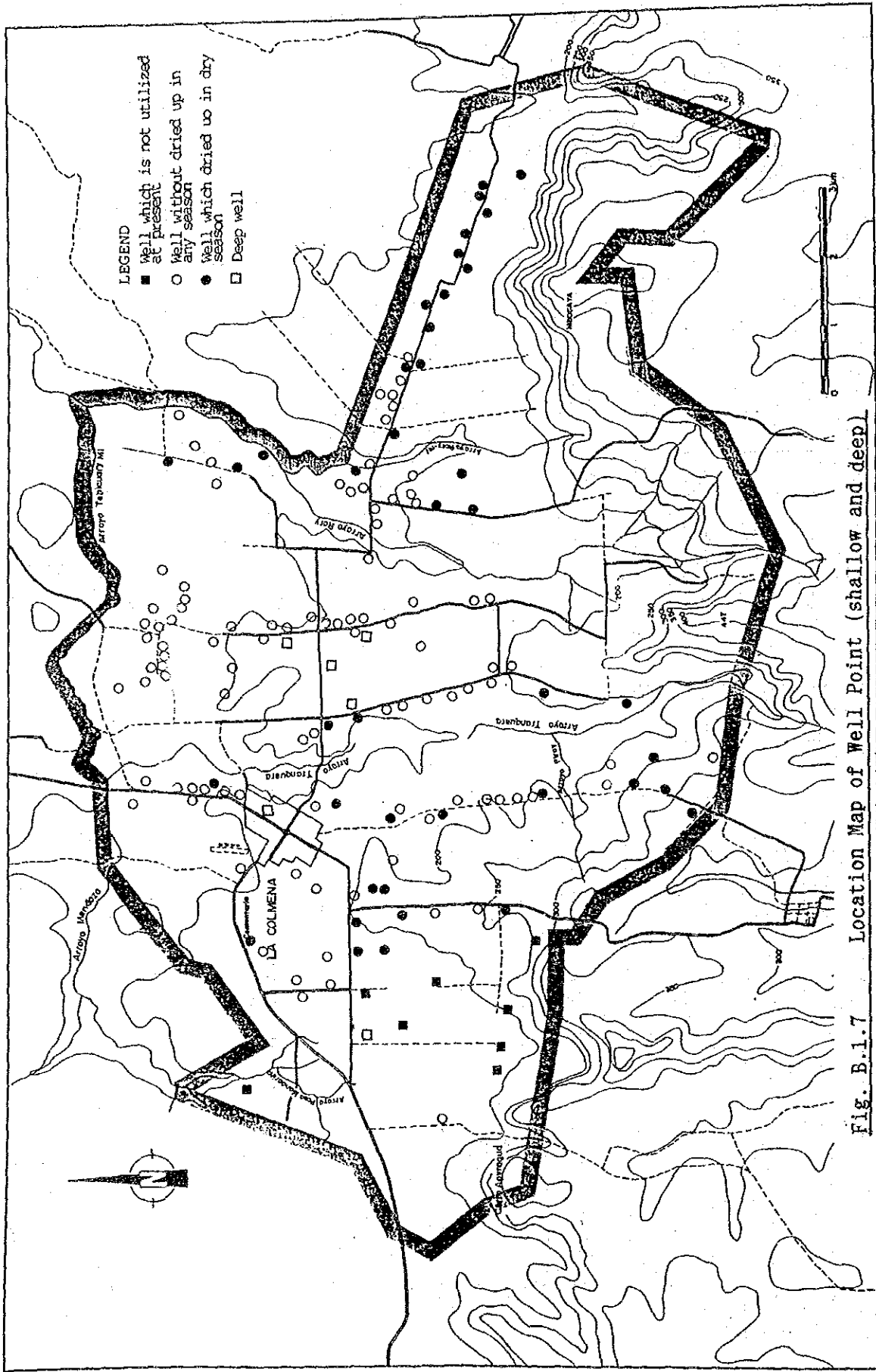


Fig. B.1.7 Location Map of Well Point (shallow and deep)

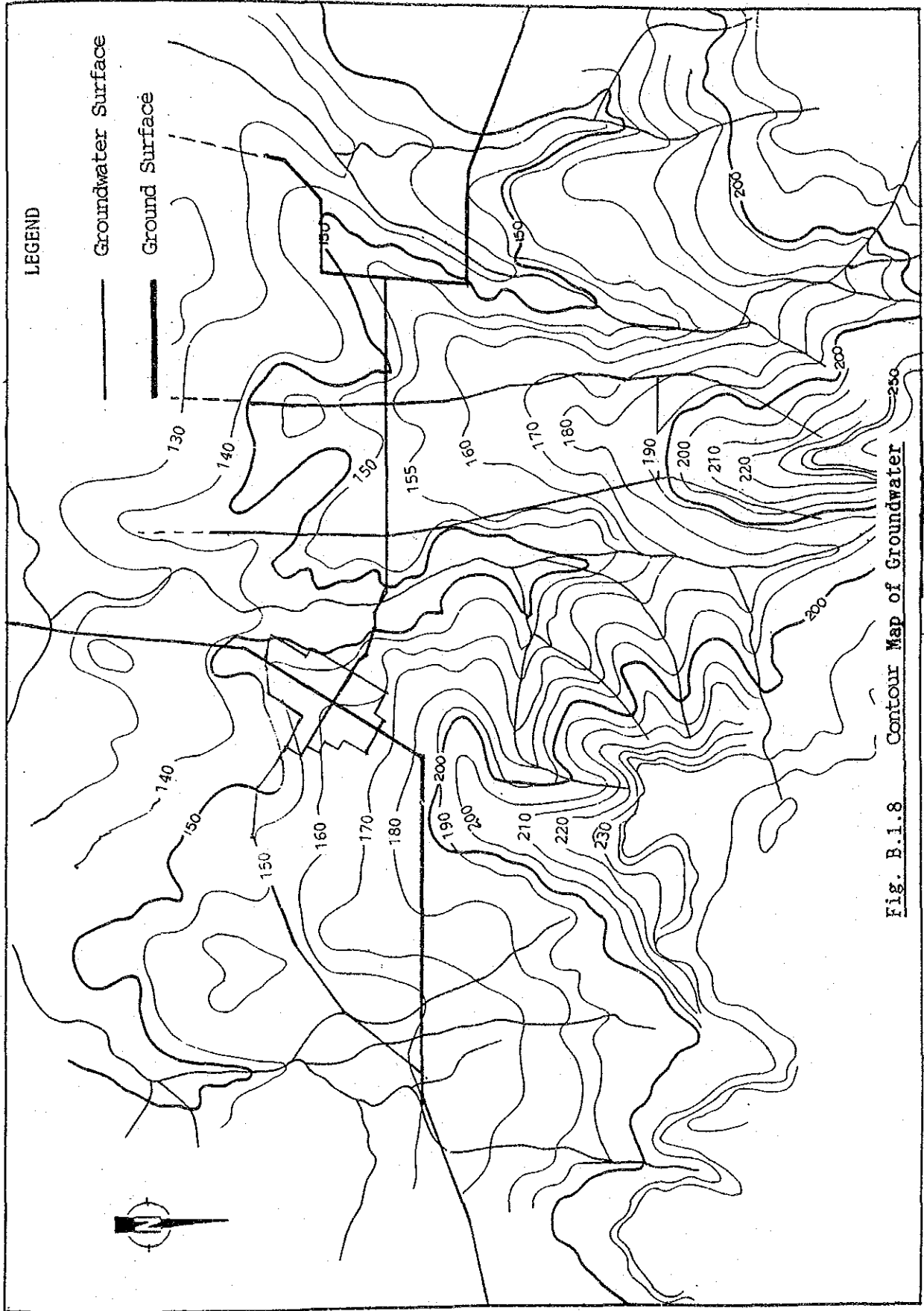


Fig. B.1.8 Contour Map of Groundwater

ANNEX C SOIL AND LAND USE

ANNEX C Soil and Land Use

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ANNEX C. SOIL AND LAND USE

C.1 General

As the preliminary survey had found that the contents of the water soluble salts and the exchangeable sodium in the soils are remarkably low, the measurements about them were omitted after that.

Distribution of the pits are given in Fig. C.1.1.

C.2 Soil Classification

The typical profiles of each series are shown as monoliths in Fig. C.2.1 and the brief description on them are given as below.

The distribution area of the soil types at each basin and administrative section is given in Table C.2.1, C.2.2 (1) and (2) respectively.

Description of the Soil Profiles

The U.CM series

Pit No. 4 : Located in Fatima, hillside with gentle slope with 240 m in elevation, upland field (cotton).

Ap: 0-24 cm : Sand, very fine gravel common, dull reddish brown (5YR 4/3.5). Organic matter (OM) few, no structure (NS) friable, hardness (16) gradual boundary, 0 - 8 cm: extremely dry.

Bt: 24-140 + cm : Sandy loam, very fine gravel common, dark red (10R 3/6), O.M. few, weakly subangular blocky (W.s AB) friable, hardness (19). Increasing very fine gravel and fine particles gradually with depth below 140 cm, gravely layer and at 190 cm, rock bed.

Pit No. 21 : Located in Rory, flate land with 140 m in elevation, upland field (no cropping).

Ap: 0-31 cm : Sand, no gravel, dull reddish brown (5YR 5/2.5 - 5YR 5/4). O.M. common, N.S., friable, hardness (16 - 22).

0 - 5 cm : extremely dry
16 - 31 cm: harder than upper and below layers gradual boundary.

Bt₁: 31-61 cm : Sandy loam, no gravel, dull reddish brown (5YR 5/4), O.M. common, W.s AB, friable, hardness (16), gradual boundary.

Bt₂ 61-115 + cm : Sandy loam, no gravel, reddish brown (2.5YR 4/5), O.M. common, moderate (M) s AB, friable, hardness (10), fine particles increase gradually with depth, below about 135 cm clay layer appears.

The U.Pa series

Pit No. 10 : Located in Ybaroty, flate grassland with 180 m in elevation.

A₁: 0-15 cm : Sandy loam, no gravel, dull reddish brown (5YR 4/3), O.M. few, W. s AB, friable, hardness (10), very fine - fine root many, gradual boundary.

ABt₁: 15-33 cm : Loamy sand, no gravel, dark reddish brown (5YR 4/4), O.M. few, M. s AB very firm, hardness (28) obviously harder than upper and below layers, very fine root many, gradual boundary.

Bt₁: 33-90 : Loamy sand, no gravel, dark reddish brown (10R 3/3.5), O.M. few, M. s AB, firm, hardness (20) very fine root common, gradual boundary.

Bt₂: 90-128 + cm: Sandy clay loam, no gravel, dark red (10R 3/5), O.M. few, strong (S) s AB, very firm, sticky, hardness (28), fine particles and sticky increase with depth.

Pit No. 20 : Located in Rory, flate land with 130 m in elevation, orchard (orange).

A₁: 0-19 cm : Sandy loam, no gravel, grayish brown (7.5YR 4/2), O.M. few, N.S., very friable, hardness (25), fine root common, gradual boundary.

A₂: 19-60 cm : Sandy loam, no gravel, almost similar to A₁ except color, brown (7.5YR 4/5).

Bt: 60-115 cm : Clay loam, no gravel, dark red (10R 3/4), O.M. few,