APPENDIX H

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RURAL ROAD

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APPENDIX H RURAL ROAD

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APPENDIX H RURAL ROAD

THE PRESENT CONDITION OF THE ROADS

The Baguio-Bontoc national road passes through the central area of La Trinidad city where the Study area comprises, provincial roads and barangay road branching of $\ddot{}$ from the national road to communicate with each peripheral villages (see Fig. $H.1.1$).

The Study area, which is located in the Central Cordillera mountains, is composed of three (3) zones, i.e. Zone I, II and III. Zone I is located in the flat land called the Trinidad Valley, whereas the Zone II and III are located in the steep mountainous area. In Zone I, national and provincial loop roads are laid out around the agricultural land of some 200 has. in area distributed in the center of the Trinidad basin, so that the road maintenance and improvement have properly advanced. On the contrary, in Zone II and III, most of the roads are constructed along the ridge on the side of the mountains, and connecting roads with villages are scarce. Verification and inventory data of the provincial and barangay roads are summarized in Table H.1.1 and $H.1.2.$

 (1) Zone I

 $\mathbf{1}$:

Zone I situated in a flat area, an urbanization is well advanced and road rehabilitation and maintenance are also well executed. The Baguio-Bontoc national road passes through the center of La Trinidad city. national (Pico-Puguis) and provincial (Puguis-Poblacion) loop roads with 2-lanes, 6 meters width are laid out around the agricultural land of some 200 ha in area distributed in the center of the Trinidad basin, and thus, the road maintenance and improvement are well in progress. Main public buildings in Zone I are Provincial office, Mayor's office, Benguet State University, General hospital, which are located along the national road. The majority of residences has been built along the loop road, each barangay is communicated by that road.

(2) Zone II

The national road passes at the southern boundary of Zone II, the provincial roads and barangay roads are branching off from the national road to communicate with each barangays. Most of the roads are constructed along the ridge on the side of the mountains, the steep road with gradients more than 15 percent is 2.1 kms (17 percent) out of the total road length. The loop road are constructed in Cruz, Alapang and south of Bahong contiguous to the national road. The total area 310 ha of agricultural land is

expanded in the sloped land, roads for agricultural transportation in agricultural land and communication roads between barangays are very scarce.

Two (2) roads Camp Dangwa-Alno route and Camp Dangwa-Sadag route shown below are the main roads in Zone II, descend steeply toward the north.

a) Camp Dangwa to Alno route

The Camp Dangwa-Alno route leads to Tuel and Pangablan city in the Municipality of Tublay contiguous to the north of the Municipality of La Trinidad. The road is well maintained with the road width of 4.5 meters, there are many automobile traffic for transportation of farm products. As the result of the traffic volume survey in September 1987, some 100 trucks were marked at the intersection on the national road.

Camp Dangwa to Sadag route \mathbf{b}

Camp Dangwa to Sadag route was constructed along the ridge of the mountains, comes to the end at Sadag. The road width is about three (3) meters, 60 percent of the total agricultural lands in the Project area is located along this road.

Tomay-Bahong Proper route $\bf c)$

Tomay-Bahong Proper route is forming a loop road with Camp Dangwa-Sadag route in the north of Bahong. The daily traffic volume attained 30 - 60 vehicles.

d) Samuyao-Alapang route

Samuyao-Alapang route is forming a loop road with Cruz-Samuyao road in Alapang.

 ϵ Cruz-Samuyao-Peril route

Land slide is occurring in the section between Cruz and Samuyao, the slope is very steep. The road width between Samuyao and Peril is some $4 - 5$ a ministration and the meters, and the route comes to the end at Peril.

한 세 음식 나는 자기

 (3) Zone III

Zone III (Bineng) is located at three (3) kms from the north of Zone I. Two (2) roads, Capitol-Bineng Proper-Yapos route and Buyagan-Wangal-Yapos route connect with Zone I and Zone III (Bineng). Capitol-Bineng Proper-Yapos route is shorter than Buyagan-Wangal-Banenbeng route, thus, the former is well used. The roads are constructed along the side of the mountains, the road width excluding shoulder ranges 2.5 - 3.5 meters. Gradients of the road is generally very steep, the section with the gradients of more than 15 percent is 1.85 kms (30 percent) out of the total length of 6.4 km. In addition to the mentioned above, the road surface pavement with calcareous stone causes wheel slip on the road and consequent traffic inexpediency. The daily traffic volume was attained some 30 vehicles caused by a bad condition of the road. The road connecting between Bineng to Boleweng is also in bad condition, the section with the gradients of more than 15 percent is 1.2 kms (46 percent) out of the total length of 2.6 km.

The survey results of the existing road gradient are shown in Table H.1.3.

DEVELOPMENT PLAN OF THE RURAL ROADS $2.$

Basic Concept 2.1

The purposes of the development plan of rural roads are as follows:

- realization of highly productive agriculture 1)
- $2)$ activation of the regional economy
- $3)$ improvement of social capital
- improvement of conditions for settlement $4)$

As for realization of highly productive agriculture mentioned above, the followings are enumerated in detail:

- Saving of vehicle operation cost a)
	- introduction of transportation vehicle with road rehabilitation and new construction
	- speed up with road maintenance
	- shortening of transportation range
- Laborsaving for farming practice $b)$
- c) Improvement of farm production
	- lightening of farm products damage with road widening and pavement
	- prevention farm products from a dust cause by traffic

Increment of land use value, increase of farm products variety and extension of market would be expected, moreover, activation of social activity and increased demand of employment, materials would be accelerated.

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 $2.2.$ **Field Survey**

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2.2.1 The basic data for the road planning

The basic data for the road planning are enumerated below: $\epsilon_{\rm{max}}$

 (1) Temperature

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 $\sim 10^{-1}$

Monthly minimum temperature

The monthly mean temperature ranges between 16.8 and 20.6 °C, and the variation in maximum and minimum temperature ranges within 14 °C.

Rainfall (2)

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Monthly mean rainfall

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Monthly mean rainy days

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2.2.2 Land use

The land use in Project area is classified into five categories i.e., upland crop field, lowland rice field, resident/commercial area, forest/grass land and others. The areas in each Zone are estimated as shown below:

The details are shown in Appendix B 2.5 and 2.6.

Zone I is characterized by large occupation ratios of upland crop fields (73 percent) and resident/commercial area (23 percent). The occupation in Zone II is upland crop fields (46 percent), residential/commercial areas (4 percent) and the rest is mostly

 $\sim \nu_{\rm{g}}$.

 $\mathcal{A}=\mathcal{A}(\mathcal{A})$.

forest and grass land. Agricultural land in Zone III occupies 21 percent of the total area, and the rest is mostly forest and grass land on steep slope.

Gradual slopes have been reclaimed with effective erosion control, terracing, the rest i. e., forest and grass land on steep slope is not suitable for agricultural land exclusive of proceeding to the orcharding.

In Zone I, residential and commercial areas are densely congested along the road, and have a marked tendency to increase in population. Meanwhile, in Zone II and Zone III, majority of the residences are built up along the road, few residences are studding in the agricultural land.

2.2.3 Socio-economic

(1) **Population**

The population in each Zone is shown as below:

The working population by industry (2)

The rate occupied with agriculture in each Zone is as below :

The table shows that the inhabitants in Zone II, III are occupied mainly with agriculture. The rests are occupied with commerce and industry.

Development Plan of the Rural Roads 2.3

2.3.1 Route alignment

Rehabilitation and new construction of roads in the Project area will be designed by taking consideration of harmony with the natural conditions, dimensions and configurations of the beneficial areas, layout and the structure of the existing roads and conditions and density of traffic. Basically, the scheme aims to improve synthetically the regional agricultural productivity and to accelerate the activation of social activity be forming a road network. The routes of the rehabilitation and new construction roads are shown in Fig. H.2.1.

 (1) Zone I

In Zone I situated in a flat area, urbanization is well advanced and road rehabilitation and maintenance are also well executed. The national road from Baguio to Bontok forming a part of loop road is a two-lane road with concrete pavement, its traffic volume marked 7,300 vehicles per day including that of big buses and trucks. Loop road consists of national road and provincial road section. Loop road also has two-lane, and its maintenance and improvement with concrete pavement are well in progress. The majority of residences has been built along the loop road, each barangay is communicated by that loop road. For these reason, a development plan concerning to the road rehabilitation and new construction in Zone I is not schemed.

 (2) Zone Π

The loop road was constructed in Cruz, Alapang and south of Bahong contiguous to the national road. While, the roads in Peril (Alapang), Alno, Sadag (Bahong) located away from the national road are constructed on the ridge and the side of mountains, and farm to market roads and communication roads between barangays are very scarce. Thus, rehabilitation works of existing roads and construction of new roads to form a road network are planned in order to improve an agricultural productivity and to accelerate regional social activities. The routes of the construction roads are shown in Fig.H.2.2.

The traffic volume on the main roads, Camp Dangwa-Alno route and Camp Dangwa -Sadag route marked 100 - 150 vehicles and 800 pedestrians per day.

 (3) $\mathop{\mathrm{Zone}}\nolimits\Pi$

 $\label{eq:1} \mathcal{L} \left(\mathcal{L} \right) = \mathcal{L} \left(\mathcal{L} \right) \mathcal{L} \left(\mathcal{L} \right)$

Zone III (Bineng) is located three (3) kms of the distance to the north of Zone I. The Ballli river cuts off a place from communication between Zone III and Zone II. A maintenance condition of the road between the Capitol and Bineng is very poor, This is a serious obstacle to traffic communication and regional development. The traffic volume of Capitol-Bineng route marked 30 vehicles per day.

Thus, the scheme should comprise a rehabilitation works for this road as well as the road between Bineng and Boleweng located to the north of Bineng. In addition to this, the new road which connects each road stretching down the Bineng plateau are planned The route of the construction road is shown in Fig. H.2.3.

The total length to be rehabilitated or new constructed is shown as follows :

The details is below:

The rehabilitation roads

9 routes Total length 22.6 km

Route		Length		
1. Zone II		7 routes	Total length	13.9 km
1)	Samuyao-Peril route			2.0 km
2)	Samuyao-Alapang route			1.1 km
3)	Camp Dangwa-Alno route			3.8 km
4)	Camp Dangwa-Bahong Proper-Sadag route			3.4 km
5)	Tomay-Bahong Proper route			1.3 km
6)	Camp Dangwa-Mae-Bahong route			$0.7 \mathrm{km}$
7)	West Alno-East Alno route			1.6 km
2. Zone III		2 routes	Total length	8.7 km
1)	Capitol-Bineng Proper-Yapos route			6.2 km
2)	Bineng Proper-Boleweng route			2.5 km

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2.3.2 Preliminary design of the rural roads

 (1) Basic design criteria

A standard road section is determined in consideration of the following factors :

 $1)$ Traffic volume estimated at the Project target year (10 years after)

 $2)$ Type of design vehicles

3) Design speed

Design daily traffic volume a.

Daily design traffic volume of the rehabilitated roads and newly constructed roads at the Project target year is estimated to be 150 - 500 vehicles per day on the assumption that an yearly increase rate of vehicles is estimated as 5 percent against the present traffic volume of 100 - 300 vehicles per day.

 $H-10$

Type of design vehicles \mathbf{b} .

Design vehicles are as follows :

Design vehicles were determined taking account of the result of the traffic volume survey.

Design speed

c.

Design speed is set as the maximum safe speed that can be maintained over a specified section of road where conditions are so favorable.

As for the development plan, the design speed is to be designed for a speed of 30 kms per hour. If necessary, due to a topographic condition, in very difficult or steep terrain, it might be useful to reduce the speed to a minimum of 20 kms per hour.

 (2) Standard section of the roads

In consideration of the basic criteria as above-stated, the following dimensions are derived for a road standard section:

Lane width

a.

The lane width is determined on the basis of the design traffic volume taking account of the design vehicle type.

 \mathbf{I} In case by design traffic volume

Design traffic volume and the lane width are given below:

Japan Road Association

 $2)$

In case by type of the design vehicles

 $H - 11$

The lane width is determined by the sum of a vehicle width and 0.3 meter wide of clearance on either side.

 $3)$ Lane width

The lane width of 3 meters is given in consideration of as following:

 $1)$ Existing road ranges from two (2) to three (3) meters width.

 $2)$ Heavy vehicle is not considered as the design vehicle.

 $3)$ Design daily traffic volume is estimated less than 500 vehicles.

In consideration of above-stated, the following dimensions are derived for a road standard section; المستحيل المحاجر

The road width 4.0 meters including shoulders are required to the passing or stopping of vehicles, refuge of pedestrians, protection of facilities laying underground and securing of sight distance.

Widening at sharp curves is desirable due to the fact that, same radius at the front wheel. The following widening should be applied on the inside edge.

In due regard to the fact that the Project area has a seasonal heavy rainfall, the lane and shoulder parts of the roads shall be paved with concrete or gravel in principals.

 (3) Vertical alignment of the roads

Vertical alignment of the existing roads shall not be changed in consideration of the topographical restriction and the increase of a large amount of construction cost involved. The maximum gradient of the roads shall be designed at 8 percent for a newly constructed road.

Pavement and drainage facilities (4)

As the Project area has heavy seasonal rainfall, a severe road surface erosion by rainfall occurs on the steep roads. Therefore, the lanes of the roads rehabilitated shall be paved with concrete and appropriate drainage facilities such as a gutter shall be installed.

In case that the installation of the gutter will cause any disadvantages or difficulties for the Project, a waterway road having a function of waterway for drainage shall be allowed for.

As for the newly constructed roads, the drainage facilities such as gutters shall also be involved, and the roads shall be paved with gravel.

Adequate openings across the road shall be made for the passage of surface water. The capacity of opening i.e. pipe culverts and box culverts is calculated by the Talbot's Formula for expediency. Table H.2.1 and Table H.2.2 show the required sizes of pipe and box culverts necessary for corresponding drainage area.

Table H.1.1 Provincial Road Verification and Inventory (1/2)

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Table.H.1.1 Provincial Road Verification and Inventory (2/2)

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Table H.1.2 Barangay Road Verification and Inventory $(1/5)$

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Table H.1.2 Barangay Road Verification and Inventory (2/5)

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Table H.1.2 Barangay Road Verification and Inventory (3/5)

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Table H.1.2 Barangay Road Verification and Inventory (4/5)

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Table H.2.1 Hectares Drained by Culverts of Various Diameters Talbot's Formula

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Talbot's Formula:

 $A = C 41 M^3 = C (M^3)^{1/4}$ $M = \{ (A/C)^4 \}^{1/3}$

Where: A = Required Area of Waterway in Square Feet

M = Area Drained in Acres (Iha. = 2.471 acres)

 $C = Coefficient$

Table H.2.2 Hectares Drained by Std. Box Culverts of Various Sizes Computed by Talbot's Formula

 $\mathcal{L}_{\mathcal{A}}$

Talbot's Formula:

 \mathcal{A}

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 $A = C 4V M³$

Where: $A = Area of Waterway$ in Square Feet

 $M = Area$ of Drained in Acres (1ha. = 2.471 acres)

 $\sim 10^{-10}$

C = Coefficient of Terrain

 $H - 23$

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Existing Road Networks Fig. H.1.1 $H - 24$

Fig. H.2.1 Plan of Road Rehabilitation and Construction

 $H-28$

APPENDIX I

DRINKING AND DOMESTIC WATER SUPPLY

APPENDIX I DRINKING AND DOMESTIC WATER SUPPLY

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 $\label{eq:1} \mathcal{L}(\mathcal{L}) = \mathcal{L}(\mathcal{L}) \left(\frac{1}{2} \sum_{i=1}^n \mathcal{L}_i \mathcal{L}_i + \mathcal{L}_i \mathcal{L}_i \right) \mathcal{L}(\mathcal{L}_i)$

 $\frac{1}{2} \frac{d^2}{dx^2} = \frac{1}{2} \frac{d^2}{dx^2}$.

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 ~ 100 km s $^{-1}$

 $\mathcal{L}^{\text{max}}_{\text{max}}$

 $\sim 10^7$

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac$

 $\sim 10^6$

 $\mathcal{L}^{\text{max}}_{\text{max}}$ and $\mathcal{L}^{\text{max}}_{\text{max}}$

 $I - i$

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APPENDIX I DRINKING AND DOMESTIC WATER SUPPLY

 ~ 1

PRESENT CONDITIONS 1.

General 1.1

Water sources in the Municipality of La Trinidad can not meet the needs for the water development now and future. The Balili river is not suitable for the water source from the viewpoint of water quality. The domestic water supply sources (see table below) are public springs serving 1,170; water works serving 1,221; private shallow well serving 154; private deep well serving 976; public shallow well serving 181; public deep well serving 491 and stream flow serving 1,967 families. Only 20 percent of the total families are supplied from the waterworks system; the remaining 80 percent are still in want of a continuous and safe water supply. Domestic water sources of La Trinidad Municipality in 1985 are summarized as follows:

* La Trinidad Water District

Water consumption as well as the capacity of sources can not be quantified in as much as the residents have different water sources.

In addition to the scarce water sources, its potability is one factor which needs immediate attention. The majority of the water sources are open springs which are open to waterborne viruses/organisms which affect the respiratory tract and gastro-intestinal parts of the human body, so that adequate water treatment is required. Ingenious water treatment being practiced by a minority of the residents is boiling the water before drinking. The majority take the risk of drinking directly from the source. There is a vital need to develop water sources so as to deliver to the homes potable drinking water.

Water supply in the Study Area 1.2

The water supply system is well equipped in Zone I. The production, however, can not be relied upon to adequately support the needs for increasing water consumption. An 80 lit/capita-day of the water consumption is supplied to the service area including 20 percent of the system losses. Whereas 110 lit/capita-day of the water consumption is required. In addition to the above-mentioned, water supply to the high elevation area, Barangay Cruz, Alapang, is not attained because of the lack of the water capacity and pressure in the dry season.

Meanwhile, Zone II and Zone III areas are out of the service area by LTWD, so that the majority of inhabitants secure the water from springs and ground water. As the land of Zone II and Zone III descends steeply forward the north, only few springs in low elevation area provide a stable water through out the year. Nevertheless, springs in the high elevation area have no water yield in the dry season because of the drawdown of the ground water surface. 21. 不可以解决 is de la

1.3 Water supply by La Trinidad Water District

1.3.1 Water supply system

LTWD (La Trinidad Water District) is operating three (3) water sources, deep well located in BSU compound and intake weirs in Ampasit and Lubas area. Daily production of water sources and the number of service connection are as follows :

a) Production

 $\mathcal{H}^{\mathcal{A}}(\mathcal{H}^{\mathcal{A}}(\mathcal{A}))=\mathcal{H}^{\mathcal{A}}(\mathcal{H}^{\mathcal{A}}(\mathcal{A}))$

 \mathbf{b}

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Service connection: 1,756 nos. (as of 31 May 1987) $\mathcal{L}_{\text{max}} = \mathcal{L}_{\text{max}} + \frac{1}{2} \sum_{i=1}^{2} \mathcal{L}_{\text{max}}$

 $\mathcal{L}_{\rm{max}}$ and $\mathcal{L}_{\rm{max}}$

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 $\label{eq:2.1} \mathcal{L}_{\text{eff}}(\mathcal{M}) = \sin \left(\frac{1}{2} \left(\frac{1}{2} \right) + \frac{1}{2} \left(\frac{1}{2} \right) \left(\frac{1}{2} \right) + \frac{1}{2} \left(\frac{1}{2} \right) \left(\frac{1}{2} \right) \right)$

 $\label{eq:2.1} \mathcal{L}(\mathcal{L}_1,\mathcal{L}_2) = \mathcal{L}(\mathcal{L}_1,\mathcal{L}_2) = \mathcal{L}(\mathcal{L}_2)$

1.3.2 Service area and water consumption

LTWD is supplying potable water to the built-up area along the National and Provincial roads, and Barangay Balili shown in Fig. I.1.1 and Fig. I.1.2. Water sources do not meet the increasing water consumption.

In respect to the above, LTWD has the ten year development plan (1983 to 1993) shown below:

Objectives:

- $1)$ To properly handle and maintain new tools and equipments including relevant technique on the new and existing water supply systems,
- $2)$ To minimize a water loss,
- 3) To maintain good relations between LTWD and consumers,
- 4) To construct administration building,
- 5) To develop another deep wells and water reservoirs to supplement the increasing water demand, and
- $6)$ To supply clean and good drinking water.

Implementation:

- \mathbf{I} To conduct seminars for technical personnel on handling and operating tools, machineries and equipment,
- To maximize a collection efficiency to fund necessary projects $2)$ for the improvement of the facilities,
- To maintain and improve the water shed areas and construct $3)$ filtration tanks and basin to supplement other water sources, and
- $4)$ To constantly check and maintain the deep wells, pumps, machines and equipment.

Based on the above, LTWD has estimated the total production required below:

1.3.3 Operation and maintenance by LTWD

The operation and maintenance of the water supply facilities executed by the LTWD are: $\sim 10^{-1}$ and $\sim 10^{-1}$

flushing of hydrants, and the state of t 1)

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 $\label{eq:2.1} \mathcal{H}(\mathcal{L}) = \mathcal{H}_{\mathcal{L}} = \mathcal{H}_{\mathcal{L} \mathcal{L}} \mathcal{H}_{\mathcal{L} \mathcal{L}}$

- $2)$ improvement of deepwell facilities,
- inspection and maintenance of intake facilities, chlorination facilities and $3)$ pipelines, and

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extension of pipeline. 4)

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Operation and maintenance cost of LTWD in 1987 is given below :

 $\mathcal{L} \in \mathcal{L}^1(\mathbb{R}^d)$, \mathcal{L} Organization chart is shown in Fig. I.1.3. Water rates of Baguio Water District and La Trinidad Water District are shown in Table I.1.2. and Table I.1.3.

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1.3.4 Development of water sources and water supply facilities

To maintain the constant yield of the spring and to maximize a delivery and supply of water, the following should be considered:

> $\langle 1 \rangle$ and preservation of Lubas, Ampasit and the Wangal river watershed,

> > alla katalun

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 $\mathcal{L}(\mathcal{A})$ and $\mathcal{L}(\mathcal{A})$ and $\mathcal{A}(\mathcal{A})$

 $2)$ drilling of additional deepwells, and

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 $3)$ enlarging and improvement of the present intake facilities and the replacement of the existing pipeline. such a superior and

> To meet rapid increasing water demand, deepwells located in the BSU compound around, the Buyagan-Pico loop road or beyond the Camp Dangwa would be recommended.

> > $\alpha\in\mathbb{Z}$

 $\tau=0.5$

I- 5

 \mathcal{L}^{eff}

support to the topic

DEVELOPMENT PLAN $2.$

$2,1$ **Basic Concept**

The majority of the inhabitants in the Project area secure the household water from springs and deepwells. Yield of springs and deep wells becomes to decrease in the dry season to impede the daily life. $\sim 10^6$

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Thus, there should be an urgent needs to develop the domestic water supply system in the area from the viewpoint of basic human needs.

 $\label{eq:3.1} \mathcal{L}_{\text{max}} = \frac{1}{\sqrt{2}} \sum_{i=1}^{N} \frac{1}{2} \left(\mathcal{L}_{\text{max}} \left(\mathcal{L}_{\text{max}} \right) \right)^2$

2.2° Design Terms

2.2.1 General

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Development plan for drinking and domestic water supply should be designed with regards for: 第四章 1000 1000

development level in accordance with that of the adjacent areas, $\left| \right|$

functional stability of the systems, $2)$

 $3)$ operational easiness of the facilities, and

effective use of the water. 4)

2.2.2 Design terms

 (1) Service area

Service areas consist of Zone II and Zone III except LTWD service area. The proposed areas are shown in Fig. I.2.1.

 (2) Target year

Water demand has been set as of a decade after. Therefore, the target year is designed at the 1988.

 $\Delta \sim 10^{-10}$ km

 (3) Service population

Total service population has been estimated at 11,000 persons with annual growth rate of 6 percent on the target year 1988. The present population data are indicated on Table I.2.1. Detailed service population is given in Table I.2.2.

 (4) Water use.

 \mathbf{b} .

The domestic water includes miscellaneous water for agriculture use, such as livestock use, crop washing water and water for spraying fungicide.

- (5) Water consumption
	- Mean daily water consumption (Mean D.W.C) a.

Drinking and domestic water supply $1)$

The water consumption for potable and household water has been estimated, referring from LTWD's criteria, at 120 lit/capita-day including 20 percent loss in consideration of the present water consumption.

 $\sim 10^{11}$

2) Miscellaneous water use for agriculture

Livestock use

Crop washing water: 500 lit/10 a. including 20% loss.

Water for spraying fungicides : 500 lit/10 a. including 20% loss.

Maximum daily water consumption (Max. D. W. C)

Maximum daily water consumption is estimated on the basis of the mean daily water consumption (Mean D. W. C.)

Based on the above, total water consumption is estimated as follows :

Special Card

The detailed results of each water consumption have been estimated in Table I.2.3 through Table I.2.10.

 $\label{eq:2.1} \mathcal{A}_{\mathcal{A}}\left(\mathcal{A}_{\mathcal{A}}\right)=\mathcal{A}_{\mathcal{A}}\left(\mathcal{A}_{\mathcal{A}}\right)=\mathcal{A}_{\mathcal{A}}\left(\mathcal{A}_{\mathcal{A}}\right)$

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 $\frac{1}{2} \left(\frac{1}{2} \left(\frac{1}{2} \frac{1}{2} \right) \right) \left(\frac{1}{2} \right)$

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Proposed Facilities 2.3

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2.3.1 Flow chart of the water supply facilities

Flow chart of the water supply facilities is shown in Fig I.2.6. The flow chart shows the process of producing potable water including water purification facilities

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2.3.2 Proposed facilities

Water source and the state of the (1)

Deep wells are adopted as the water source because of their steady characteristics of ground water in terms of quantity and quality. Almost all creek water in the Project area has been distinguishably contaminated by bacillus. BOD and COD of the creeks water show the value of 2 to 6, and these figures are higher than that of the springs and wells water of 0 to 5. In addition to the above, springs and wells water becomes turbid and muddy even at the slight rainfall.

(2) Pumping facilities

Six (6) deepwells in Zone II and one (1) deepwell in Zone III have been designed based on the topography, geology, the pumping test result and so on. The depth of deep wells are designed at 70 to 120 m. The submersible pump is available for deep well.

(3) Service area

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sta s

Six (6) service areas in Zone II and one (1) service area in Zone III have been proposed in consideration of the production capacity of the deep wells. The service areas are below: 医神经细胞 医血管 医肠管膜炎 医血管囊肿

* Location is indicated in Fig. I.2.2 and Fig. I.2.3.

人名科尔 计中心 医视觉

(4) Water conduit facilities

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Pipeline has been introduced for the water conduit facilities taking account of the topographic conditions, the conveyance effective use and potability.

(5) Water purification facilities

Water purification system has been selected in relation to the quality of water source, volume of water required and easiness of operation and maintenance of the facilities.

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South State State (18 Jan 4 Jan 2) The water purification methods operated in Baguio Water District (BWD) and LTWD are enumerated in Table I.2.9. The chlorination for each deep well and the filtration system in case that the water is turbid and muddy is adopted as the water purification facilities. Slow filtration system is available for water purification. Its capacity is estimated at less than 5 m/day in terms of the filtration velocity. Potable

water standard provided by Baguio Water District is shown in Table I.2.10.

 (6) Water distribution facilities

> Volume of water distribution \mathbf{a} .

Maximum hourly water consumption for drinking and domestic water use and miscellaneous use for agriculture are calculated using the equation as below: and the company of the

dia nombre

and the state

Drinking and domestic water use

Maximum hourly water consumption (max. H.W.C) $= 3$ (coefficient) x Max.D.W.C. **Contract State**

Miscellaneous use for agriculture

Distributing tank b.

Capacity of distributing tank is determined by the following criteria.

Capacity for drinking and domestic water

Capacity for miscellaneous water use for agriculture

 $\frac{1}{2} \frac{1}{2} \frac{d^2}{dx^2}$

The water supply facilities are illustrated by diagrams on Fig. 1.2.4 and Fig. I.2.5.

The salient features of water supply facilities are summarized hereunder:

Table I.1.1 Water Supply Facilities (La Trinidad Water District)

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Classification	Code	Meter Size	Minimum Charge $0 - 10$ cu.m	$1 - 20$	$1 - 30$	1-31 above
Residential A/	$1-2$	$1/2$ "	36.00	3.60/cu.m.	3.65/cu.m.	3.75/cu.m.
Government	$1-3$	3/4"	57.75			
	$1-4$	1 ²	116.00			
	$1 - 5$	$11/2$ "	290.70			
	$1-6$	$2n$.	727.50			
	$1 - 7$	3"	1,309.90			
Residential B	$1-2B$	1/2"	44.50	4.45/cu.m.	4.50/cu.m.	4.60/cu.m.
	$1-3B$	3/4"	71.20			
	$1-4B$	1"	142.90			
	$1-5B$	11/2"	357.90			
	$1-6B$	2"	895.50			
	$1-7B$	2"	1,612.30			
Commercial A	$3-2A$	1/2"	72.50	7.25/cu.m.	7.25/cu.m.	7.55/cu.m.
	$3-3A$	3/4"	116.00			
	$3-4A$	1"	232.50			
	$3-5A$	11/2"	581.90			
	$3-6A$	2"	1,455.50			
	$3-7A$	3"	2,620.30			
Commercial B	$3-2$	1/2"	72.50	4.5 /cu m .	4.5/cu.m.	4.60/cu.m.
	$3 - 3$	3/4 ⁿ	116.00			
	$3-4$	1"	232.50			
	$3 - 5$	11/2 ⁿ	581.90			
	$3-6$	2"	1,455.50			
	$3 - 7$	3 ⁿ	2,620.30			
Wholesale Water delivery		P11.15/cu.m. Residential	P4.00/drum			

Table I.1.2 Water Rates of Baguio Water District ~ 20

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 $\label{eq:2.1} \begin{split} \mathcal{L}_{\text{max}}(\mathbf{r}) = \mathcal{L}_{\text{max}}(\mathbf{r}) \end{split}$

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 \mathbb{R}^3

 $\label{eq:2} \frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1$

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 $\hat{\mathcal{A}}$

Water Rates of La Trinidad Water District Table $1.1.3$

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Table I.2.1 Barangay Population

Popuration growth rate 1975-1985 4.3(%)

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 $\bar{\lambda}$

 $\bar{\beta}$

 $\hat{\boldsymbol{\gamma}}$

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1980-1985 5.8(%) $\mathcal{L}^{\text{max}}_{\text{max}}$

 $\bar{\omega}$.

 $\sim 10^{11}$ km

 $\hat{\boldsymbol{\beta}}$

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Table I.2.2 Designed Population

I- 15

$\mathcal{H}^{\text{c}}(\mathbb{R}^2)$ Table I.2.3 Water Consumption

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 $\epsilon_{\rm{max}}$

 $\frac{1}{2}$

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 $\bar{\gamma}$

 $\frac{1}{2}$

I-16

 $\frac{1}{2}$.

Table I.2.4 Mean of Water Consumption

 $\frac{1}{\sqrt{2}}$

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 $\omega_{\rm{eff}}$

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 \bar{z}

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Barnetti

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 $\sim 10^{11}$

 $\omega_{\rm{eff}}$ and

 $\langle \hat{g} \rangle$ \mathcal{A} $\mathcal{A}^{\mathcal{A}}$

 $\frac{1}{4}$ $\hat{\vec{r}}$

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 $\mathcal{L}^{(1)}$.

 \sim \sim

 $\hat{\boldsymbol{\gamma}}$

 $\hat{\mathcal{L}}$

Unit: Mean daily consumption (cu.m/day)

Max. daily consumption $(cu.m/day)$

I-17

Area	Mean daily water consumption Max. daily water consumption							
				Cattle Swine Fowl Total			Cattle Swine Fowl	Total
Area II-1				$1.5 \t2.6 \t0.4 \t4.5 \t2.3 \t3.9$			0.6	-6.8
Area II-2				1.5 2.6 0.4 4.5	2.3	3.9 ₁	0.6	6.8
Area II-3	2.7	$4.5 -$	0.7	7.9	4.1	6.8	1.1	12
Area II-4	1.6	2.7		0.4 4.7	2.4	4.1	0.6 ¹	7.1
Area $II-5$	2.5	4.2		0.6 7.3	3.8	6.3	0.9 [°]	11
Area Π -6	2.4	-4.1		0.6 7.1	3.6	6.2	0.9 ₂	10.7
Area III-1		$2.1 - 3.5$		$0.5 \t 6.1$		3.2 5.3 0.8		9.3
Total				14.3 24.2 3.6 42.1			21.7 36.5 5.5	63.7

Table I.2.5 Water Consumption for Livestock

 $\label{eq:2.1} \mathcal{A} = \mathcal{A}_{\text{max}} \left(\mathcal{A} \right) \left(\mathcal{A} \right)$

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 $\sim 10^{11}$

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 \mathcal{L}

 $\label{eq:2.1} \frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1$

 $\label{eq:2.1} \mathcal{L}_{\mathcal{A}} = \mathcal{L}_{\mathcal{A}} \left(\mathcal{L}_{\mathcal{A}} \right) \otimes \mathcal{L}_{\mathcal{A}} \left(\mathcal{L}_{\mathcal{A}} \right)$

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 $\hat{\vec{x}} = \vec{r} \cdot \vec{r}$.

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2004年4月

 $\mathcal{L}^{\text{max}}_{\text{max}}$ and $\mathcal{L}^{\text{max}}_{\text{max}}$

 $\label{eq:2.1} \mathcal{L}=\mathcal{L}^2\left(\mathcal{L}^2\right) \left(\mathcal{L}^2\right) \left(\mathcal{L}^2\right) \left(\mathcal{L}^2\right)$

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 $\hat{\mathcal{A}}$

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 $\omega \rightarrow \omega$

 $\mathcal{F}=\frac{1}{2}\mathcal{F}_{\mathrm{d}}(\mathcal{F})$

Area			No. of cattle No. of farmhouse Water consumption (lit./nos/day)	Mean daily water consumption $(cu \n m/day)$	Max. daily water consumption (cu.m/day)	
Area II-1	0.3	170	30.0	1.5	2.3	
Area Π -2	0.3	170	30.0	1.5	2.3	
Area $II-3$	0.3	300	30.0	2.7	4.1	
Area Π -4	0,3	180	30.0	1.6	2.4	
Area II-5	0.3	280	30.0	2.5	3.8	
Area II-6	0.3	270	30.0	2.4	3.6	
Area III-1	0.3	230	30.0	2.1	3.2 ₂	
Total		1600		14.3	21.7	

Table I.2.6 Water Consumption for Cattle

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 $\label{eq:2} \frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\left(\frac{1}{\sqrt{2}}\right)^2\left(\frac{1}{\sqrt{2}}\right)^2\left(\frac{1}{\sqrt{2}}\right)^2.$

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^{2} \left(\frac{1}{\sqrt{2}}\right)^{2} \left(\$

Table I.2.9 Water Consumption for Washing and Spraying Fungicide

 $\hat{\boldsymbol{\beta}}$

Mean daily water consumption = Farm area/15days x 5.0 cu.m/ha.day

 $\epsilon_{\rm{max}}$ $\tau \rightarrow \infty$

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 $\hat{\boldsymbol{\alpha}}$

 \bar{z}

k,

 $\hat{\boldsymbol{\beta}}$

 $\frac{1}{2}^{2}$, $\frac{1}{2}$ \bar{z}

 $\frac{1}{\epsilon}$

 \bar{z}

 \mathbb{R}^2

 $\ddot{}$

 \mathbb{R}^3 a

 \mathcal{A}

Table I.2.10 Max. Hourly Water Consumption

 $\mathcal{L}(\mathcal{I},\mathcal{L},\mathcal{L})$, and $\mathcal{L}(\mathcal{I},\mathcal{L},\mathcal{L})$, and $\mathcal{L}(\mathcal{I},\mathcal{L})$

 $\bar{\mathcal{A}}$

 $\ddot{}$

Value of *5 is the biggest water consumption among No.2 - No. 4.

Value of *6 means the 1.5 hr. water consumption of total amount of max. hourly water consumption on No.2 - No. 4.

Water Sources and Purification Facilities Operated by Water District Table I.2.11

 $\alpha_{\rm c} = \alpha_{\rm c} = 2\pi$

 $\label{eq:2} \mathcal{L}_{\text{max}} = \frac{1}{2} \sum_{i=1}^{N} \mathcal{L}_{\text{max}}$

Open Sources (Rainy Season)

1. Amliang 2. Lamut Spring

3. Idisan Spring

4. Amsing Spring

Boosters

- 1. Military Cut Off
- 2. Bonifacio
- 3. DPWH
- 4. Upper Quezon Hill
5. City Camp
6. Stage I

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-
-
- 7. Phil-Am

Gas Chlorination Gas Chlorination **Gas Chlorination Gas Chlorination**

Boosters

Open Sources

1. Lubas Spring

2. Ampasit

 α

1. Tawang

Table I.2.12 Potable Water Standard (Baguio Water District)

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2.2 count per 100 ml

Allowable chlorine residential : $0.2 - 1.0$ mg/lit. \sim

Permissible limit for chemical substances

 $\frac{1}{2}$

Bacteriocidal examination of water

Multiple fermentation
tube technic

La Trinidad Water District Organization Chart Fig. 1.1.3

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 $I-28$

 $I - 29$

 $I - 30$

 $\overline{1-31}$

 $1 - 32$

Water Source - Deep Well

 $I - 33$

 $\mathcal{L}^{\text{max}}_{\text{max}}$, $\mathcal{L}^{\text{max}}_{\text{max}}$

APPENDIX J

CONSTRUCTION PLAN AND COST ESTIMATE

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2.$

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2.$

 $\mathcal{L}^{\text{max}}_{\text{max}}$ and $\mathcal{L}^{\text{max}}_{\text{max}}$

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APPENDIX J CONSTRUCTION PLAN AND COST ESTIMATE

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Fig. J.1.1

Implementation Schedule of the Project ... $J-29$

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APPENDIX J CONSTRUCTION PLAN AND COST ESTIMATE

CONSTRUCTION PLAN $\mathbf{1}$.

 1.1 General

The constructions of the Project are given below:

 \mathbf{I} **Irrigation** facilities

> Irrigation facilities consist of ponds as the water source, intake weirs, water conduit pipes, irrigation canals, and on-farm irrigation structures. Eleven (11) ponds and eight (8) intake weirs are proposed in the Project area. Intake weirs, ponds and farm are connected by pipeline.

 $2)$ Drainage improvement

> Drainage improvement is applied to the Balili river, the Bolo creek, the Bayabas creek and a part of the small creeks in Zone I.

> As a major cause of flood inundation is a lack of discharge capacity of the Balili river and the Bolo creek, widening of river section is proposed and bank lining is also planned so as to flow the flood smoothly. Moreover, installation of a screen at the entrance of the Dinog cave is proposed to prevent the floating matter from flowing into the cave.

 $3)$ Rural road

4)

The rehabilitation and construction of rural roads are mainly intended to improve the agricultural and social infrastructures. Concrete pavement for the existing road and gravel pavement for the newly proposed road should be constructed to preclude the road surface from an erosion.

Drinking and domestic water supply facilities

Drinking and domestic water supply facilities are proposed in Zones II, III. They are composed of deep wells as a water source, water purification facilities and water distributing facilities.

 $J - 1$

$5)$ Rural electrification

Electric power supply is proposed in Zone III. Electricity is necessary for domestic electrification. Extension of the electric wires from Zone I to Zone III is to be executed.

$6)$ Sewerage

The sewage canals are proposed as a countermeasure against the sewage problem in Zone I. Drainage canal is lined with concrete.

7) Rural community center

Seven (7) rural community centers are selected to be constructed to promote the rural development programs. Rural community centers will be utilized for the barangay associations and council, training of agricultural management and health service.

As described above, the construction works of the Project consist of various works such as deep wells, earth, concrete and piping works. Construction site is located in the mountainous area, where annual rainfall has been recorded 3,500 - 4,000 mm. As existing road conditions are not adapted to transport of construction machineries and materials due to the rough surface and narrow road width, it is therefore extremely important to adjust the road construction schedule in consideration of the other construction schedules.

Basic Assumption of Construction Planning 12

1.2.1 Workable days

As for the general works such as concrete works, drilling works and piping works, etc., 25 days per month are applied for the standard workable day.

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On the other hand, earth works are mostly affected by heavy rainfall. Since embankment of impervious or semi-impervious materials are controlled by moisture content, special attentions must be paid on executing earth works. Suspension days of earth works caused by rainfall are studied based on the following criteria concerning daily rainfall intensity.

Annual mean workable days were estimated based on the daily rainfall records at BSU PAGASA for recent 11 years (1977 - 1987). The results are shown in Table J.1.1.

The results show that less than 25 days of the standard workable days concentrate in the wet season from May to October, especially, the workable days from May to September are estimated at less than 15 days.

Workable days for impervious materials were obtained from computed days in wet season and 25 days in dry season, i.e., in total, 235 days in a year.

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1.2.2 Earth moving plan

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The construction works of the irrigation, drainage improvement and rural road include a large quantity of the earth works. The total amount of 58,000 cu.m. of the embankment materials are required. The amount of 40,000 cu.m. will mainly be obtained from the excavated materials and the rest amount of 18,000 cu.m. will be obtained from the borrow area selected around the working site.

The total amount of excavated materials, i.e., 180,000 cu.m., comprising 105,000 cu.m. from the drainage improvement construction, 75,000 cu.m. from the ponds construction, will be hauled to the spoil area. The swamp area would be available for the spoil area with the surface soil handling.

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1.3 **Irrigation Facilities**

1.3.1 Pond construction

(1) **Earth works**

Stripping and surface excavation would be mainly executed by bulldozer and sub-surface and deep excavation would be executed by back-hoe shovel, drag line depending on the soil condition at the pond site. Manpower would contribute to face smoothing and compacting of the foundation of appurtenant structures.

The excavated materials excessing embankment requirement would be transported to a spoil area. In case of lacking the embankment materials, the materials would be supplemented from borrow area selected around the working site. embankment materials would be spread by bulldozer and compacted by tire roller. As for the compacting impervious materials, tamping roller would be adopted.

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 (2) Concrete works

Intake and spillway would be constructed by concrete or stone masonry. Concrete works would be started after completion of earth works. Concrete would be produced by portable concrete mixer and placed using bucket hanged by truck-crane.

1.3.2 Intake weirs

The construction works of intake weirs would be executed during dry season. Intake weirs are designed to be a fixed type weir made by concrete. Concrete would be produced by concrete plant or portable concrete mixer and placed using bucket hanged by truck-crane.

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1.4 Drainage Improvement

The construction works for drainage improvement would be mainly executed during the dry season in consideration of magnitude of flooding in the river. The constructions consist of widening of river section and bank protection. The foundation of the river bed consists of rock, gravel and river load. The excavation of gravel and river load would be executed by bulldozer and back-hoe, while, rock materials would be broken by blasting and gathered by bulldozer. Excavated rock would be useful for

Supported

riprap and so forth. Bank would be protected by retaining wall. Concrete would be produced by concrete plant and placed by concrete pump or bucket hanged by truck crane.

1.5 Rural Road

The constructions of rural road mainly consist of the excavation for road widening, concrete/gravel pavement and appurtenant structure, such as bridges, drainage culverts and so on.

Excavation works would be mainly made by bulldozer, back-hoe shovel and supplementary manpower. Excavated materials would be hauled by dump truck to a spoil area.

Pavement consists of subbase course and concrete pavement slab. Subbase course of crushed stone would be spread by bulldozer and compacted by vibratory roller and pneumatic type roller. Concrete would be produced by concrete plant, and hauled by truck mixer.

Drinking and Domestic Water Supply Facilities 1.6

The construction of drinking and domestic water supply facilities mainly consists of deep wells, slow filtration basins, distributing tanks and water conduit and supply pipe. Deep well drilling, 70-120 meters in depth, would be executed by percussion method and submersible pumps would be installed in deep well. Water conduit and supply pipes would be laid 0.6-1.0 meters below from the ground surface. Excavation for lying pipe would be made by back-hoe shovel and manpower.

1.7 **Rural Electrification**

Electrification is proposed in Zone III. Extension of the electric wires from Zone I to Zone III is to be executed.

1.8 Sewerage

The sewage canals would be lined with concrete. Concrete would be produced by portable mixer. Excavation would be made by back-hoe shovel and manpower.

1.9 **Implementation Schedule**

The implementation schedule of the Project is shown in Fig. J.1.1. The first one (1) year would be necessary for the preparatory works including tendering, survey and mapping works, detailed design works, mobilization and construction of offices and quarters. The actual construction works would be commenced from the second year. The construction works for irrigation, drainage improvement, drinking and domestic water supply facilities and rural road will need two (2) years in total.

Construction of the irrigation facilities, the drainage improvement and rural road would be executed during the dry season in consideration of the damage caused by the heavy rainfall and flooding in the river.

 $J - 6$

2. **COST ESTIMATE**

2.1 **Basic Assumptions**

The construction costs have been estimated on following assumptions:

Unit prices are analyzed on the basis of average price level for first half $1)$ of 1988, forming the price basis prior to preparing this cost estimate.

Exchange rate used in the estimate is shown as follows: 2)

$US $ 1.0 = P 21.0$

Construction works is assumed to be executed within the full contract 3) basis. The machinery and equipment required for construction works shall be provided by the contractors. Therefore, depreciation costs of machinery and equipment are considered in the estimate of the construction unit cost.

Import tax for construction materials, machinery and equipment shall be $4)$ excluded from the cost estimate.

The construction cost based on unit costs shall be divided into foreign 5) and local currency portions. Local currency portion is estimated on the basis of the current price in La Trinidad in the first of 1988 and of the data collected from the on-going projects around the project area. Foreign currency portion is estimated based on the CIF prices at Manila.

The physical contingency, 10 percent of the direct construction cost shall 6) be included in the construction cost in view of preliminary nature of the estimate.

Price contingency shall also be taken into account at an annual escalation 7) rate of three(3) percent for the foreign currency portion and five(5) percent for the local currency portion.

The associated costs to be financed by the Government, such as the 8) costs for strengthening the extension services and facilities of the water user's association shall be not included in the estimate.

$2,2$ **Financial Construction Cost**

Financial construction cost comprises direct construction cost, land acquisition, O&M equipment, administration and engineering services, physical contingency and price contingency.

The direct construction cost consists of construction costs of irrigation facilities, sewage canals, drainage facilities, drinking and domestic water supply facilities, rehabilitation and construction of rural roads, barangay halls and electric facilities, including contractor's profit, overhead and taxes.

The total construction costs of the project are estimated at 301.5 million peso, comprising 175.2 million peso (equivalent to 58.1 percent of the total construction $\cos(s)$ for the foreign currency portion and 126.3 million peso (equivalent to 41.9) percent the total construction costs) for the local currency portion. The summary of the construction cost are shown in Table J.2.1. Breakdown of the cost estimate is shown in Table J.2.3 through Table J.2.11.

2.3 **Annual Disbursement Schedule**

The annual disbursement schedule has been established on the basis of the construction implementation schedule. Details are stated in Table J.2.2.

2.4 **Annual Operation and Maintenance Costs**

Annual operation and maintenance costs include salaries for project administration and staff, materials and labor costs for repair and maintenance of O&M equipment, and running costs of project facilities. The annual operation and maintenance costs are estimated at 2.2 million peso (Table J.2.12).

2.5 **Replacement Cost**

Some of the facilities, especially mechanical equipment have a shorter useful life than the civil constructions and are likely to require replacement at some time within the project's useful life. Table J.2.13 shows the useful life and replacement cost for the mechanical equipment.

Suspention and Workable Days by Rainfall Intensity
for Impervious Materials for Pond Table J.I.I

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Total 124 108 112 95 129 136 115 250 123 $\overline{5}$ 113 102 1,264 121 Dec. $\frac{8}{10}$ 29 $\mathbf{\Omega}$ \circ \sim \sim \bullet \circ $\mathbf{\Omega}$ \circ \circ \circ \sim \mathfrak{g} Nov. 25 S \mathbf{v} \mathbf{a} \circ \bullet ∞ **SU** 113 \mathfrak{Q} \overline{z} $\mathbf{\hat{z}}$ Ω \mathbf{S} \mathbf{r} \bullet Ω σ \mathbf{I} \mathcal{C} t \mathbf{I} Sep. 191 \mathbf{C} 16 \mathbf{L} 26 Γ \supseteq Ω $\frac{6}{16}$ \mathbf{I} $\overline{4}$ Γ \overline{z} \overline{z} Aug. 243 σ 22 23 25 28 22 \boldsymbol{z} \tilde{z} 24 $\frac{9}{1}$ 23 \overline{z} Ξ \vec{a} 202 $\frac{3}{2}$ 22 $\frac{6}{10}$ $\frac{8}{10}$ $\overline{1}$ $\overline{15}$ \tilde{a} $\vec{4}$ Ë, $\mathbf{14}$ 24 $\overline{21}$ $\overline{21}$ Jun. 178 16 $\vec{4}$ \overline{c} $\overline{1}$ \tilde{L} \tilde{L} $\frac{3}{2}$ $\overline{\mathcal{Z}}$ \sum Γ $\overline{21}$ $\overline{12}$ May 171 \tilde{a} $\frac{5}{1}$ \mathbb{C} $\frac{9}{1}$ $\frac{6}{1}$ \sum 4 \mathcal{Z} \mathbf{r} $\tilde{1}$ $\frac{3}{2}$ \mathbf{r} $\overline{21}$ Apr. 60 \mathbf{v} 25 ω \sim ∞ \sim ∞ O ¢ $\overline{\mathbf{C}}$ $\mathbf{1}$ ∞ Mar. 22 29 \circ \circ \circ $\mathbf{\Omega}$ $\overline{ }$ \sim $\mathbf{\sim}$ ∞ $\left\langle \cdot \right\rangle$ \sim Feb. 27 \circ ŀ. \circ \circ \circ $\mathbf{\Omega}$ \circ $\mathbf{\Omega}$ $\overline{}$ \circ Jan. \mathfrak{D} \bullet σ $\mathbf{\sim}$ O Mean Suspension Workable days 1984 1985 1986 Total 1978 1979 1980 1982 1983 1987 Year 1977 1981

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Table J.2.1 Summary of Construction Cost

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Table J.2.3 Breakdown of Direct Construction Cost (1/5)

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Table J.2.3 Breakdown of Direct Construction Cost (2/5)

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 $\label{eq:2} \begin{split} \mathcal{L}_{\text{max}}(\mathbf{r}) & = \frac{1}{2} \sum_{\mathbf{r} \in \mathcal{R}^{(n)}} \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \\ & = \frac{1}{2} \sum_{\mathbf{r} \in \mathcal{R}^{(n)}} \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}$

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Table J.2.3 Breakdown of Direct Construction Cost (4/5)

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Table J.2.3 Breakdown of Direct Construction Cost (5/5)

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$\frac{1}{2} \sum_{i=1}^{n} \frac{1}{2} \sum_{j=1}^{n} \frac{1}{2} \sum_{j=1}^{n$ Table J.2.4 Cost for Land Acquisition

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Procurement Cost of O & M Equipment Table J.2.5

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Procurement Cost of Project Office Equipment
for Implementation and O & M Table J.2.6

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Procurement Cost of O & M Equipment
for Agricultural Extension Service Table J.2.7

Remarks

*1 : Agricultural equipment include shovel, knopsack sprayers, hoe, etc..

Table J.2.8 Procurement Cost of O & M Equipment for Garbage Disposal

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Table J.2.9 Administration and Engineering Costs

 $\label{eq:2} \frac{1}{\sqrt{2}}\int_{0}^{\infty}\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^{2}d\theta.$

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Table J. 2. 10 Required Man-Months of the Project Staff

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Table J. 2. 11 Required Man-Months of Consultant Engineers

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 $\sim 10^4$

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 $\sim 10^6$

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Table J. 2. 13 Replacement Cost

		Items	Useful life (year)	Replacement Cost $(1,000$ pesos)
		O&M Equipment		
		Garbage Disposal Trucks	10	Carlos Contract 1,600
2.	Project Facilities			
	$2-1$	Irrigation Facilities		and the company
		Pump	20	-990
		Vulves and Others	20	6,837
	$2-2$	Drainage Facilities		
		Gate	30	2,400
	$2 - 3$	Drinking and Domestic Water Supply Facilities		
		Pump	20	1,270
		Pipe, Valve and Others	20	17,200

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Table J.2.14 Labors cost

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Materials Cost

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 $\label{eq:2} \frac{1}{2} \sum_{i=1}^n \frac{1}{2} \sum_{j=1}^n \frac{1}{$

Materials Cost

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 $\label{eq:2.1} \frac{d\mu}{d\mu} = \frac{1}{2\pi\mu}$

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Materials Cost

Table J.2.15

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Fig. J.1.1 Implementation Schedule of the Project

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 $\label{eq:2} \frac{1}{2} \int_{\mathbb{R}^3} \frac{d^2\mathbf{r}}{|\mathbf{r}|^2} \, \mathrm{d}\mathbf{r} \, \mathrm$

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APPENDIX K

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PROJECT EVALUATION

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APPENDIX K PROJECT EVALUATION

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 $\label{eq:2} \frac{d\mathbf{r}}{dt} = \frac{d\mathbf{r}}{dt} \left[\frac{d\mathbf{r}}{dt} - \frac{d\mathbf{r}}{dt} \right] \, ,$

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 $\label{eq:2} \begin{split} \mathcal{L}_{\text{max}}(\mathcal{L}_{\text{max}}) & = \mathcal{L}_{\text{max}}(\mathcal{L}_{\text{max}}) \,, \end{split}$
LIST OF TABLES

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LIST OF FIGURES

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 $\label{eq:2.1} \frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2\pi}}\int_{\mathbb{R}^3}\frac{1$

APPENDIX K PROJECT EVALUATION.

GENERAL 1.

The Government of the Philippines considers agricultural and rural development as the highest priority among all other national medium-term development plans (1987-1992) to stabilize and improve living of farmers. Conformably the Government of the Philippines paid attention to assist highland farmers who had long been neglected from the development.

Under this situation, the feasibility study of the Highland Integrated Rural Development Project in La Trinidad, Province of Benguet, was carried out with the intention of:

- increasing land and labor productivities through the improvement of (i) agricultural infrastructures;
- increasing farm income through the improvement of agricultural (ii) infrastructures and strengthening of agricultural extension services; and
- raising quality of life of farmers through income increase, upgrading and (iii) construction of rural infrastructures and improvement of access to social and cultural amenities.

The evaluation of the Project was carried out in terms of economic, financial and socio-economic aspects. The economic feasibility was evaluated by calculating the economic internal rate of return (EIRR), net present value (NPV) and benefit cost ratio (B/C). Sensitivity analyses were also carried out in order to elucidate the economic viability of the Project against the changes in the economic benefits, costs and construction period. The financial feasibility was evaluated by analyzing the effects of the Project on a typical farm budgets. The socio-economic impacts from the implementation of the Project were studied including the qualitative evaluation of the social components.

$2.$ **ECONOMIC EVALUATION**

2.1 **Basic Assumptions**

The economic evaluation was carried out on the following basic assumptions :

- A conventional assessment ways will be adopted. That is the economic (1) evaluation will be carried out on the directly productive components of the Project as: (i) irrigation; (ii) drainage; and (iii) rural roads.
- The costs in the economic evaluation will be studied on directly productive (2) components in accordance with the assumption above mentioned.
- From the viewpoint of national economy as a whole, the transfer payments such (3) as contract taxes, duties, subsidies, interests and depreciations should be considered as a domestic monetary movement without direct productivity. These transfer payments should be, therefore, excluded from the economic costs.
- (4) Price contingencies should be excluded from the economic costs.
- (5) The construction period will be three (3) years including one (1) year for $\frac{1}{2} \log \frac{1}{2} \delta$ detailed design.
- The economic useful life of the Project will be 35 years. (6)
- All prices are expressed in constant 1987 prices. (7)
- An exchange rate of US\$ $1.00 = P$ 21.0 = \frac{1}{30.0 is used throughout the (8) report.

2.2 **Economic Factors**

For evaluation of economic prices and costs, the following criteria were used. Each economic factor was studied based on the Appraisal Report on the Highland Agriculture Development Project (HADP) performed by the Asian Development Bank $(ADB).$

2.2.1 Economic prices for agricultural outputs and inputs

Prices of internationally traded inputs (fertilizers) were estimated on the basis of projected international market prices by the World Bank with the necessary adjustments, and are all expressed in 1987 constant value. Economic prices of all imported farm inputs except fertilizers were converted from the current market price, assuming that 90 percent of the market price consists of foreign exchange and 10 percent of local currency which is adjusted using a standard conversion factor of 0.86. Economic prices of locally produced farm inputs was also converted from the farmgate prices using a standard conversion factor. A shadow wage rate of 0.80 was used for both family and hired farm labor. All economic prices of outputs were converted into economic prices using a standard consumption conversion factor of 0.85. Financial farmgate prices of farm inputs and outputs were estimated on the basis of current farmgate prices prevailing in the Project area as of 1987.

Conversion factors of construction $2.2.2$

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The individual financial costs were split into four (4) categories as transfer payment, foreign exchange, unskilled labor and others in local currency.

The economic opportunity cost of unskilled construction labor might be assumed to equal that of hired farm labor of \overline{P} 35/man-day. Related to the financial wage rate of $\frac{1}{2}$ 55/man-day for construction labor, this would give a conversion factor of (\angle 35/ \angle 55) x 0.86 (SCF) = 0.55. The conversion factors for each cost component were as follows:

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2.3 **Economic Benefits**

2.3.1 Irrigation

The agricultural benefits are primarily derived from the increased and stabilized crop production attributable to: 医异常性 经工具经营 医耳朵反应 想见

- improved technologies and farm management practices coupled with (i) strengthening agricultural extension system;
- (ii) reduction of flooded area and provision of irrigation water;
- guidance of extension team to minimize the variation of the crop prices; (iii)

promotion of farmer's cooperative activities; and (iv)

reduction of transportation loss by rehabilitating of the rural roads. (v)

The benefits are estimated as the difference of the annual net crop production values under with and without Project conditions. The net production value is defined as the difference between the gross production value and the crop production cost (for details, see APPENDIX B).

The annual incremental net production values creditable to irrigation and agricultural extension services were estimated at P_1 17.325 million as shown in Table K.2.1. The benefits may be expected to increase linearly year by year, and to reach its full benefits about five (5) years after the completion of physical implementation. The implementation of the Project would require lands for construction. These lands presently used for agricultural production were considered in the planted area under with project conditions.

2.3.2 Drainage

The annual flood control and drainage benefits creditable to the Project were estimated at \mathbb{R} 7.335 million. This presents estimated average annual reduction in flood damages and incremental net production values of agricultural crops in Zone I as follows:

Flood control (1)

Monetary estimates were made including damages to residential and nonresidential houses, personal property and real property, etc.. No estimates of intangible damages such as loss of life, impairment to health, etc. were included.

Average annual reduction in flood damages was estimated as a expected value using the formula as follows:

 $I = \sum (Dn - dn) \cdot Fn$

Where: $I = Average$ annual reduction in flood damages,

Dn = Estimated flood damages under without project conditions in return period (n) ,

 $dn =$ Estimated flood damages under with project conditions in return period (n),

 $Fn = Probability of flood occurrence in return period (n).$

The procedures of calculation of average annual reduction in flood damages are shown in Table K.2.2, Table K.2.3 and Table K.2.4. The flood control benefits may be expected to reach its full extent just after the construction of the flood control facilities.

 (2) Drainage

Through the agricultural survey and interviews under Work II, it was revealed that hardly anything is planted in flood-prone area in Zone I during the wet season,

because the farmers don't want to risk losing the crops in case an unusually heavy rain comes along and floods the area.

The drainage problem is a yearly phenomenon and the solution of which would enable the farmers to plant all throughout the wet season. The additional production during this period would be a direct benefit of the drainage. The quantification of this benefit is difficult because the ultimate total incremental value of production that would accrue from the Project is a function of management decision and management capability. However, a reasonable enough estimate can be made with the use of the agricultural survey data. Based on the survey, it was estimated at $P_{0.315}$ million by multiplying net return per ha. of 29,100 pesos with increase of vegetables cropping area of 217 ha. The drainage benefit may be expected to increase linearly year by year, and to reach its full benefit about five (5) years after completion of the physical implementation.

2.3.3 Rural roads

On the basis of the location of current vegetable areas, their production and the existing road network, the rural roads works would consist of the following:

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Benefits due to rehabilitation of roads arise from vehicle operating cost savings (VOC savings) on agricultural and non-agricultural transport. On the other hand, benefit from opening of new roads in vegetable producing areas are obtained by substitution of head-carrying by vehicle transport. The results of the benefits accrued from rural roads are summarized as follows:

 $\label{eq:2.1} \frac{1}{\sqrt{2\pi}}\frac{1}{2\pi}\left(\frac{1}{2\pi}\right)^{2}\frac{1}{2\pi}\left(\frac{1}{2\pi}\right)^{2}\frac{1}{2\pi}\left(\frac{1}{2\pi}\right)^{2}.$

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The rural roads benefits of agricultural transport concerned and non-agricultural transport may be expected to increase linearly year by year, and to reach its full extent about five (5) and ten (10) years after completion of physical implementation respectively.

A number of case studies undertaken in the Philippines have shown that there is a close correlation between improved roads and higher farmgate prices for agricultural commodities, as well as increased transfer of modern farming techniques through agricultural extension services. From this past experience, it is expected that the farmers, local population, and transport operators providing services in the areas will be influenced.

Many of the farmers in the less developed areas are currently farming at subsistence level and improved access is expected to increase farm incomes through the encouragement of better farming techniques and greater opportunity for marketing surpluses. The road improvements will also enable other small-scale producers to participate in the market economy stimulating trade and development in other sectors of the economy.

(1) Internal influence area

The internal influence area of a project road consists of the area directly being served by the road. In other words, the internal area is the area surrounding the road. The exact delineation of that area depends on the natural terrain boundaries, such as mountains and rivers, as well as crop areas, competing road network and the location of local market centers to/from which the farmers bring their produce and carry other goods.

In the Project area, Zone II and Zone III will be served by more than one road. Therefore, Zone II and Zone III would be split into internal influence areas according to the respective homogeneous road sections. The boundaries of the influence areas of rehabilitation and new opening rural roads are shown in Fig. K.2.2 and Fig. K.2.3 respectively. Land use patterns are also plotted on these figures, and crop type areas in each internal influence area are measured. The results are indicated in Fig. K.2.4 and Fig. K.2.5 in the form of schematic diagram.

(2) Individual traffic costs

Basic traffic costs in the Philippines are defined as the costs which would be incurred by vehicles using roads under ideal conditions. The basic traffic costs can be divided into groups as $:$ (i) running costs; (ii) fixed costs; (iii) time costs; and (iv) accident costs. Accident costs were not taken into consideration in this study because of the low vehicle operating speed and small traffic volume in the Project area.

Individual traffic costs for specific project roads with road and traffic elements which deviate from the ideal road conditions are calculated according to the dl-system concerning running costs based on the basic traffic costs calculated by DPWH in 1982. It was no use of dt-system in this study because of low vehicle operating speed. The updated individual traffic costs of the project roads are shown in Table K.2.5.

Representative vehicles by make and model in the Project area were selected based on the results of the traffic survey as : (i) small trucks; (ii) light cars; (iii) jeeps; and (iv) jeepneys.

 (3) VOC savings on agricultural transport

After farmers have harvested and packed the products, they deliver them to the market in La Trinidad and Baguio City by means of jeepney transport at present time. The road conditions in Zone II and Zone III are so bad that there are plenty of spoilage of vegetables during jeepney transport.

With implementation of the Project, provincial and barangay roads in the area will be rehabilitated with concrete pavement. In addition to the improvement of rural roads, provision of irrigation water and agricultural extension services will increase and stabilize crop production in the area. These conditions will also encourage the introduction of vehicles with higher loading capacity (2 tons truck) as partial substitution of jeepneys.

Benefits from the roads rehabilitation materialize as savings in vehicle operating costs (VOC savings), which are derived from individual traffic costs. The VOC savings were calculated under the hypothesis that a half of the produce transported on rehabilitated roads will shift from jeepneys to small trucks. The production used in the calculation was anticipated target around the five years after completion of the Project. The results and procedures of the calculation are shown in Table K.2.6 and Table $K.2.7.$

The VOC savings due to the rehabilitation of rural roads were \triangleright 0.163 million. $\mathcal{L}^{\mathcal{L}}$ This savings can be increased by 10 percent to take into account the transport of agricultural inputs. The final VOC savings was therefore $P(0.179)$ million.

(4) VOC savings on non-agricultural transport

Non-agricultural transport saving on rehabilitated roads is due to the difference in VOC between poor and good conditions road. In the following calculation it was assumed that the traffic volume will increase with the growth rate of 5 percent p.a., and the traffic volume of ten years later was used in the calculation based on the results of the traffic survey.

The results and procedures of the calculation are shown in Table K.2.8 and Table K.2.9. The VOC savings due to rehabilitation of rural roads on non-agricultural transport were estimated at P 1.053 million.

Transport savings from opening of new roads (5)

The transport savings in the vegetable producing areas obtained from the opening of new roads derived from the conversion of a footpath into a passable vehicle were calculated by the reduction of head-carrying costs. The unit cost of head-carrying has been estimated at $\angle P$ 10 - 15 /trip where the average trip consists of 1 km and 50 kg of weight to be carried. The majority of households prefer to use family labor to work in the field and hired labor for head-carrying of their produce.

In the following calculation it was assumed that moving each head load (50 kg) of produce by 1 km costs to the farmer P 10. The crop production used in the calculation was anticipated target around the five years after completion of the Project.

In order to calculate the savings obtained from the opening of new roads, it has been assumed that head-carrying will be substituted by vehicle transport, jeepneys and small trucks sharing the production equally.

The results and procedures of the calculation are shown in Table K.2.10 and Table K.2.11. The transport savings were estimated at P 0.324 million. This amount can be increased by 10 percent to take into account the transport of agricultural input. The total transport cost savings were P 0.356 million.