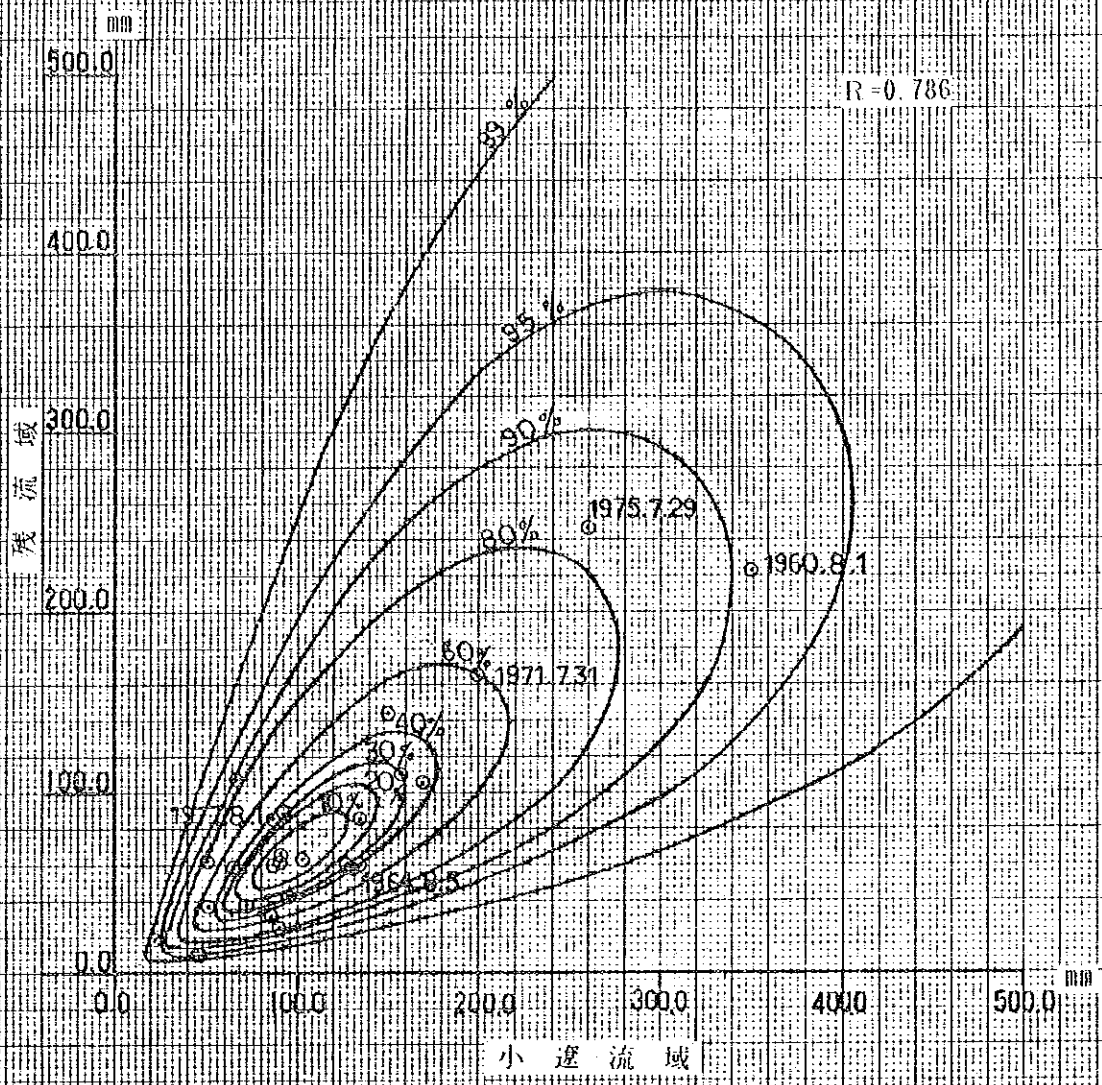


图A.5.6(2) 3日雨量复合概率图 (小遼~残流域)



表A.5.7(1) 分割流域ティーンセン係数表 時間雨量 (1960年)

名目	流域	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22
1	中津山	0.420 406.5	0.031 8.8																				
2	平野子流	0.088 81.7	0.263 74.4	0.376 333.9																			
3	黒子崎	0.472 438.0	0.608 172.1	0.034 30.2	0.118 53.8																		
4	南沢山			0.534 474.2																			
5	津河橋		0.098 27.7	0.011 9.8	0.837 388.4	0.183 42.4	0.094 85.4	0.178 109.1									0.008 9.2						
6	小浜			0.045 39.8	0.047 21.8	0.490 113.7	0.392 347.2	0.354 219.5									0.012 18.4						
7	久才崎					0.327 75.9	0.424 395.4																
8	岡田																						
9	南子崎						0.100 91.0	0.100 92.0		0.293 266.1													
10	本段																						
11	橋							0.370 229.4	0.665 198.6	0.154 171.4							0.110 165.8						
12	下島								0.398 413.0	0.662 32.7													
13	野原									0.034 37.8	0.289 152.6			0.191 44.3									
14	西家台									0.104 115.8	0.557 293.4		0.087 106.8										
15	長家								0.340 71.4	0.017 18.9	0.031 16.4	0.825 212.9		0.206 47.9		0.142 217.8							
16	野河												0.254 311.9	0.533 136.4	0.241 52.1				0.117 53.6	0.022 33.0			
17	二道河子																						
18	花紅橋門										0.051 26.9		0.459 563.7										
19	野家																						
20	真																						
21	橋															0.565 866.7	0.515 193.6						
22	野											0.074 17.1		0.015 3.5	0.759 163.9	0.185 253.1	0.485 162.4	0.776 509.3	0.141 61.6				
23	向山子																						
24	野家																						
25	橋												0.067 82.3					0.224 146.7	0.742 339.8	0.415 624.2			
26	野												0.133 183.3							0.026 42.1	0.376 479.0		
27	八																				0.355 485.0		
28	石門																						
29	野																				0.164 246.7	0.173 226.5	
30	牛																				0.119 179.0	0.072 91.7	
31	野																				0.245 368.5	0.007 8.9	
32	山																				0.007 10.5	0.007 8.9	

上段：ティーンセン係数
下段：支配面積 (km²)

表A.5.7(2) 分割流域ティーンセン係数表 時間雨量 (1964年)

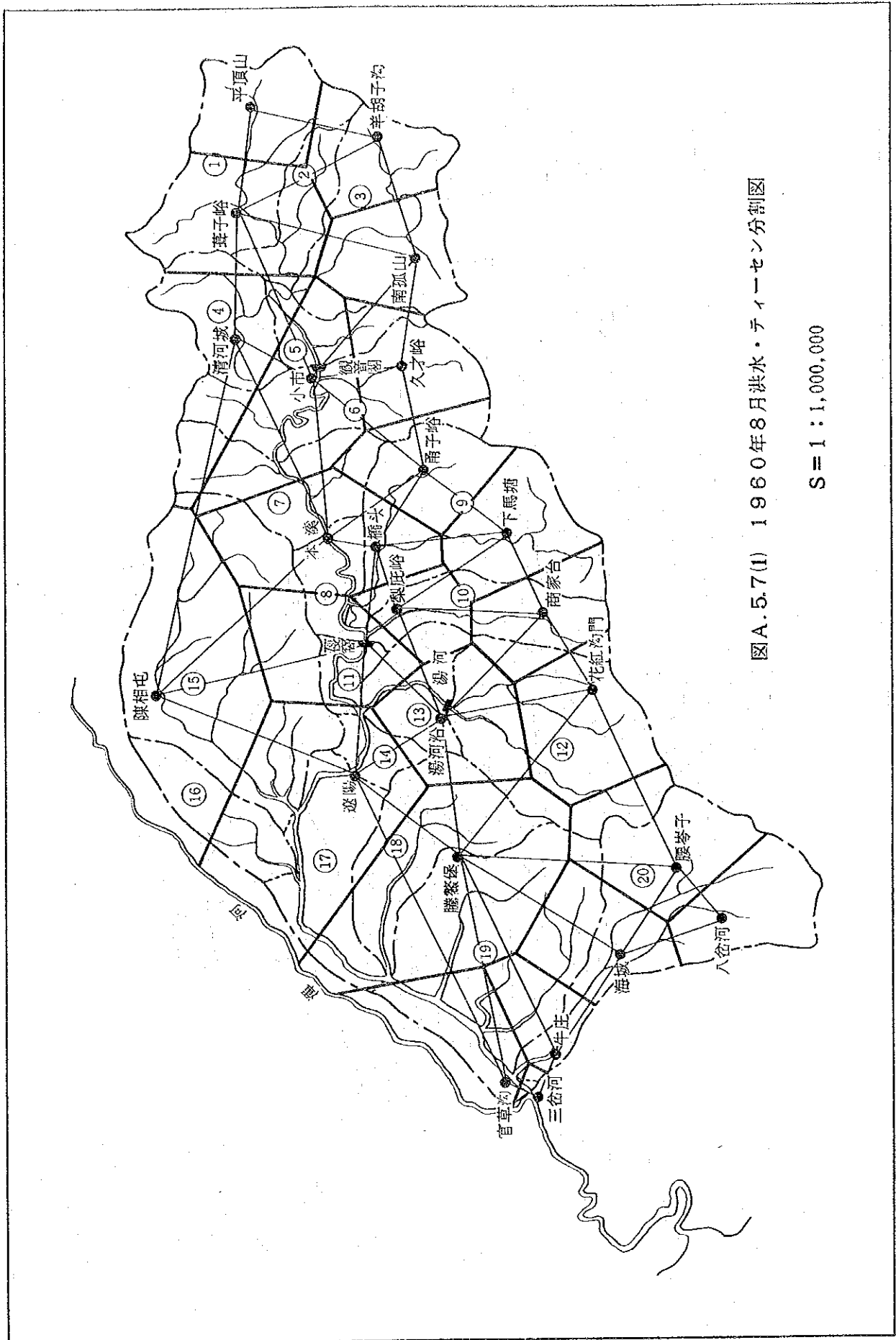
区号	流域名称	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22
1	中込山	0.140 108.3	0.037 10.5																				
2	平瀬子溪	0.088 81.7	0.241 88.2	0.376 339.9																			
3	夏子崎	0.472 438.0	0.885 193.8	0.034 30.2	0.139 81.7																		
4	岡坂山			0.534 474.2																			
5	旗形峠		0.037 10.5	0.011 8.7	0.401 371.7	0.163 37.8	0.078 70.9	0.035 21.7															
6	小市			0.045 40.0	0.022 10.2	0.531 123.2	0.123 111.8																
7	久才崎					0.306 71.0	0.441 400.9																
8	岡坂				0.044 20.4	0.274 249.1	0.558 346.0										0.010 15.3						
9	崎子崎					0.084 76.3	0.071 44.0		0.292 325.0														
10	本深						0.282 181.0	0.605 127.0									0.174 266.9						
11	旗形						0.044 27.3	0.186 39.1	0.192 213.7	0.091 48.0													
12	下系坂									0.384 427.4	0.102 53.9												
13	旗形峠																						
14	岡坂台								0.085 84.6	0.562 286.7		0.087 106.8											
15	旗形							0.209 43.9	0.047 52.3	0.186 97.2	0.863 222.5			0.379 87.5		0.028 43.0							
16	旗形峠											0.201 246.8	0.604 140.6	0.200 43.2						0.008 3.7			
17	二道岡子																						
18	花江橋門										0.061 32.2	0.498 611.6											
19	旗形峠																						
20	旗形																0.400 813.6	0.020 7.6					
21	旗形峠																0.274 420.3	0.480 184.2					
22	旗形										0.037 8.5		0.015 3.5	0.075 162.0	0.114 174.9	0.490 184.2	0.776 508.3	0.080 36.6					
23	岡山子											0.013 16.0	0.050 10.8							0.272 124.6			
24	旗形峠											0.122 149.8								0.224 102.6	0.029 43.6		
25	旗形峠																	0.224 146.7	0.418 180.5	0.410 616.7			
26	旗形峠																						
27	八段河																					0.460 586.0	
28	石門坂											0.078 87.0									0.072 102.3	0.302 284.8	
29	旗形																					0.150 195.5	0.151 192.4
30	牛庄																					0.120 180.5	0.073 83.0
31	旗形峠																					0.232 348.9	0.007 8.9
32	山形河																					0.007 10.5	0.007 8.9

上段：ティーンセン係数
下段：支配面積 (km²)

表A.5.7(3) 分割流域ティーンセン係数表 時間雨量 (1971, 75, 77年)

区号	流域名称	時間雨量																			
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
1	牛 坂 山	0.446	0.037																		
		408.5	10.5																		
2	千 塚 子 城	0.068	0.241	0.370																	
		81.7	88.2	328.5																	
3	甚 子 崎	0.472	0.635	0.034	0.133																
		438.0	193.8	36.2	61.7																
4	南 坂 山			0.534																	
				474.2																	
5	深 河 越		0.037	0.011	0.801	0.163	0.078	0.035													
			10.5	9.8	371.7	37.8	70.9	21.7													
6	小 市			0.051	0.022	0.531	0.123														
				45.3	10.2	123.2	111.8														
7	久 才 崎					0.306	0.441														
						71.0	400.9														
8	崎 越				0.044		0.274	0.558									0.010				
					20.4		248.1	346.0									15.3				
9	崩 子 崎						0.084	0.071		0.294											
							76.3	44.0		327.2											
10	本 城							0.292	0.605								0.174				
								181.0	127.0								266.9				
11	崎 越							0.041	0.185	0.155											
								27.3	39.1	112.5											
12	下 馬 城								0.399	0.052											
									439.7	27.4											
13	坂 越 崎								0.034	0.271				0.212							
									37.8	143.1				49.2							
14	崎 家 台								0.105	0.544		0.010									
									116.6	287.2		12.3									
15	坂 越 越								0.202	0.017	0.063	0.853		0.242		0.028					
									43.9	18.9	33.3	222.5		58.1		43.0					
16	越 河 崎												0.069	0.516	0.200				0.008		
													84.7	119.7	43.2				3.7		
17	二 道 河 子									0.020	0.185	0.015									
										10.6	227.2	3.5									
18	花 紅 越 門										0.050	0.347									
											26.4	426.1									
19	崎 家 店												0.374						0.014	0.008	
													459.3						21.1	10.3	
20	崎 越 越														0.100	0.020					
															613.6	7.6					
21	崎 越 越															0.274	0.490				
																420.3	184.2				
22	越 河 越									0.037		0.015	0.750	0.114	0.490	0.716	0.080				
										8.5		3.5	162.0	174.9	184.2	508.3	36.6				
23	崎 山 子													0.059				0.272			
														10.8				124.6			
24	崎 家 越																	0.224	0.028		
																		102.6	43.6		
25	崎 家 越																	0.224	0.416	0.412	
																		146.7	180.5	619.6	
26	崎 山 子																				
27	八 笠 河											0.015								0.638	
												18.4									812.8
28	石 門 越																				
29	崎 越																			0.190	0.269
																				285.8	342.7
30	牛 庄																			0.118	0.073
																				177.5	93.0
31	崎 越 越																			0.233	0.006
																				350.4	7.6
32	山 立 河																			0.004	0.006
																				6.0	7.6

上段：ティーンセン係数
下段：支配面積 (km²)



圖A.5.7(1) 1960年8月洪水・ティセンサー分割図

S=1:1,000,000

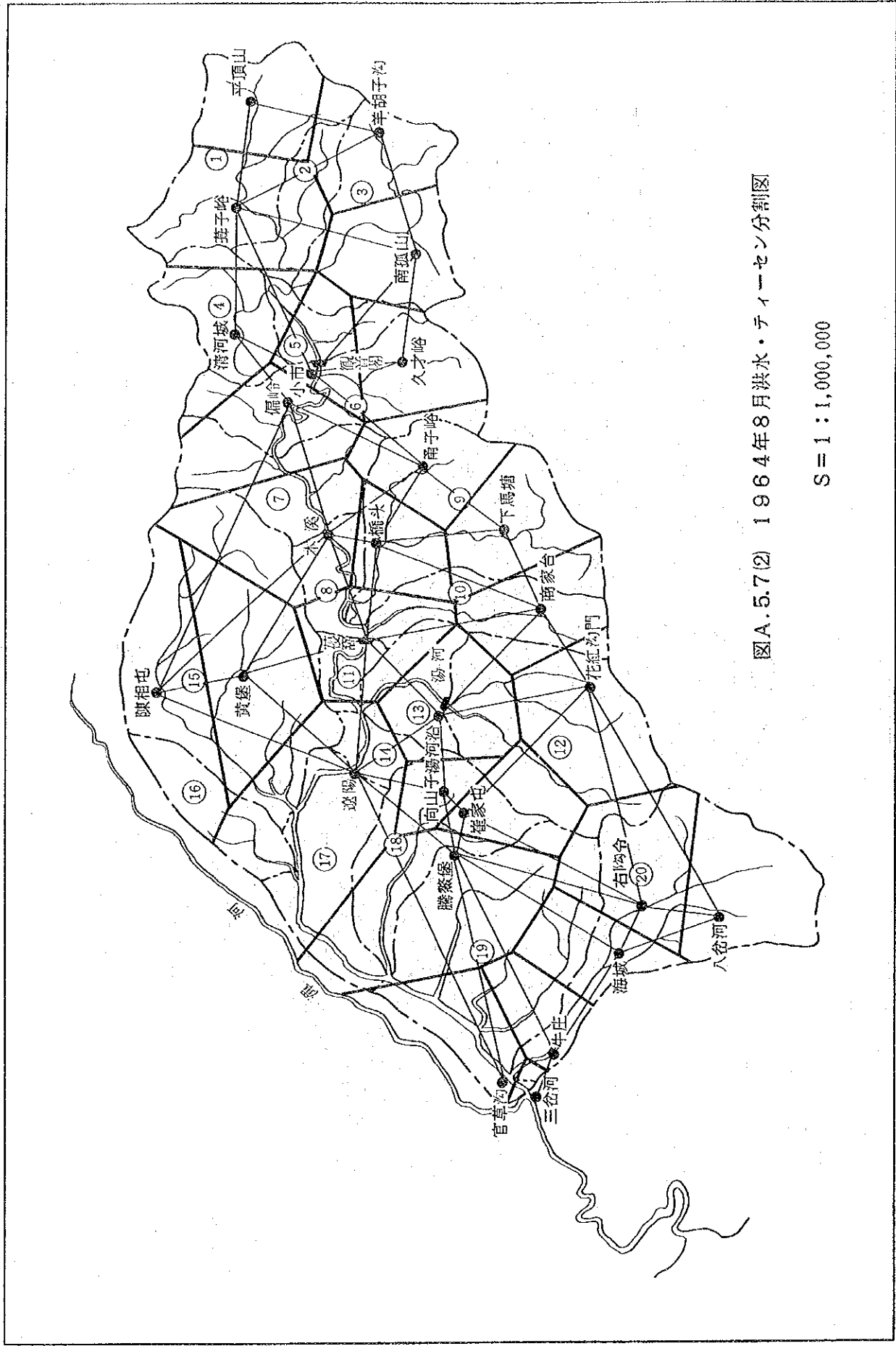
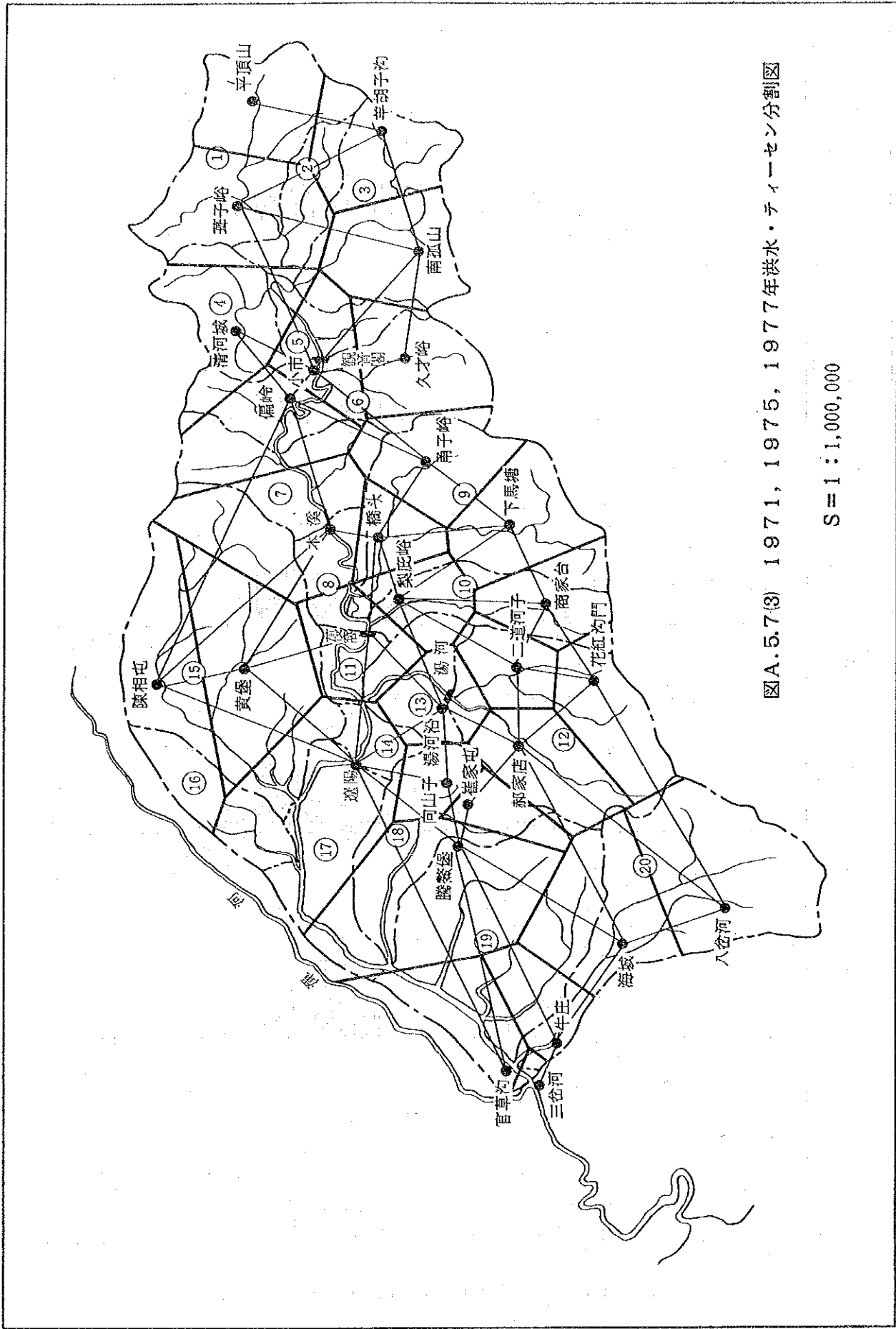


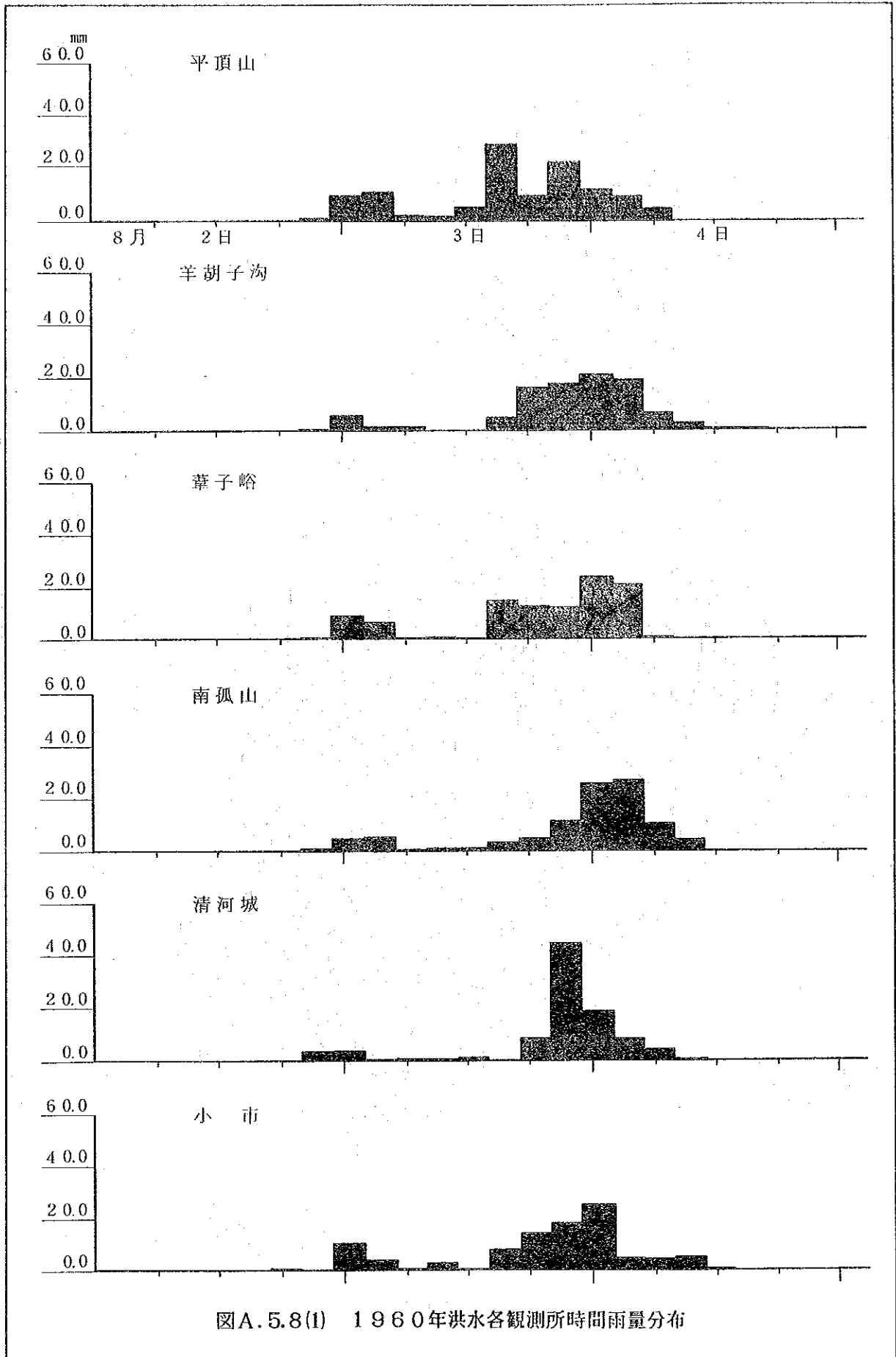
圖 A.5.7(2) 1964年8月洪水・ティーンセン分割圖

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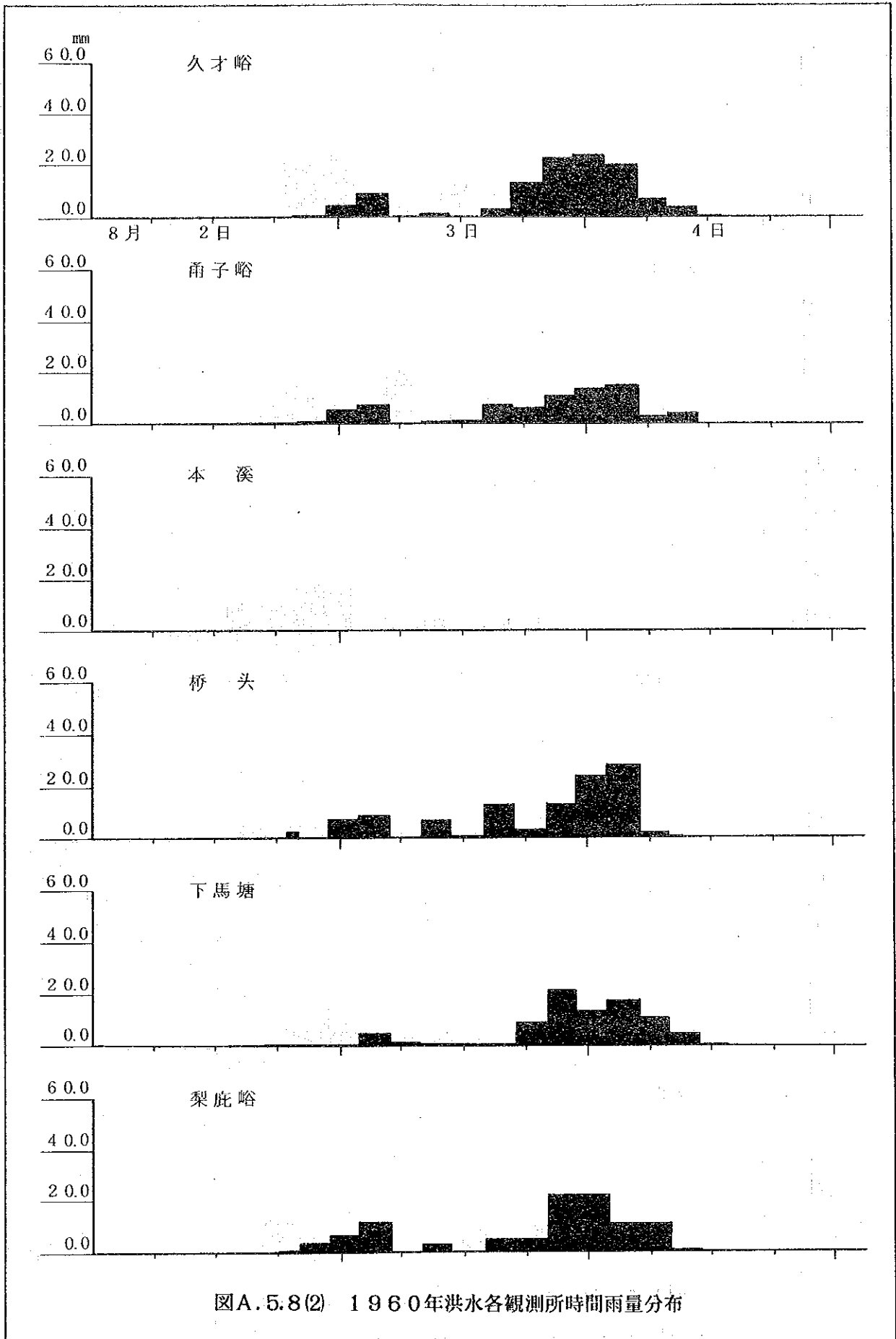


圖A.5.7(3) 1971, 1975, 1977年洪水・ティーンセン分割図

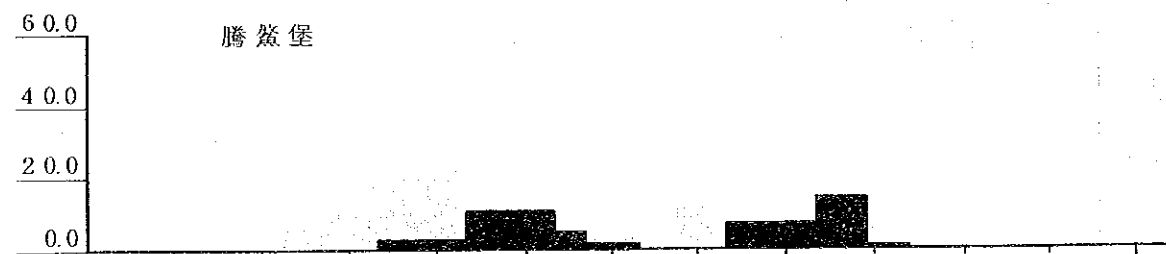
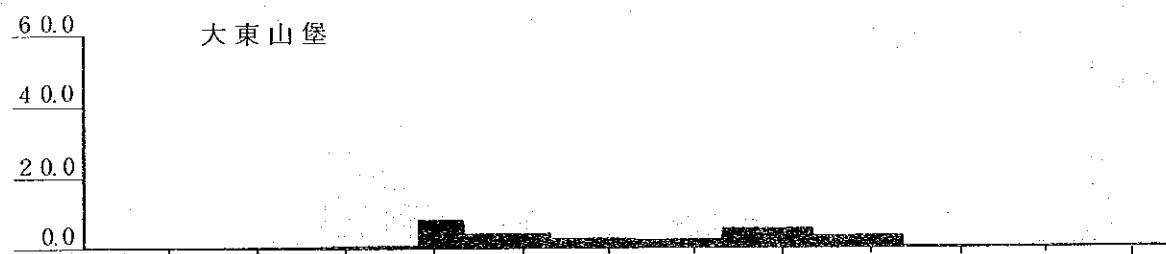
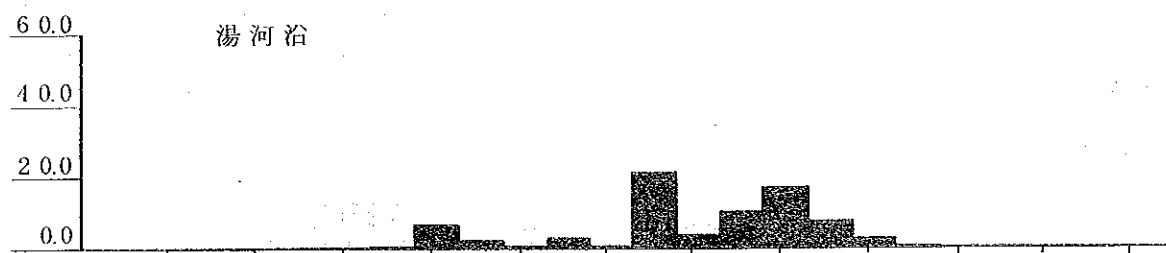
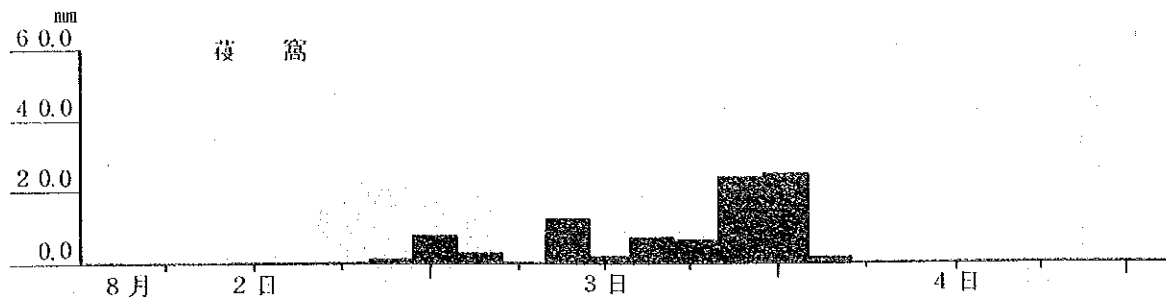
S = 1 : 1,000,000



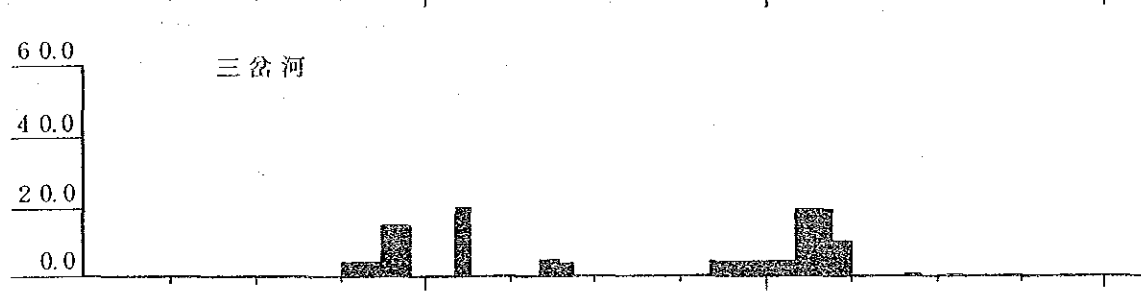
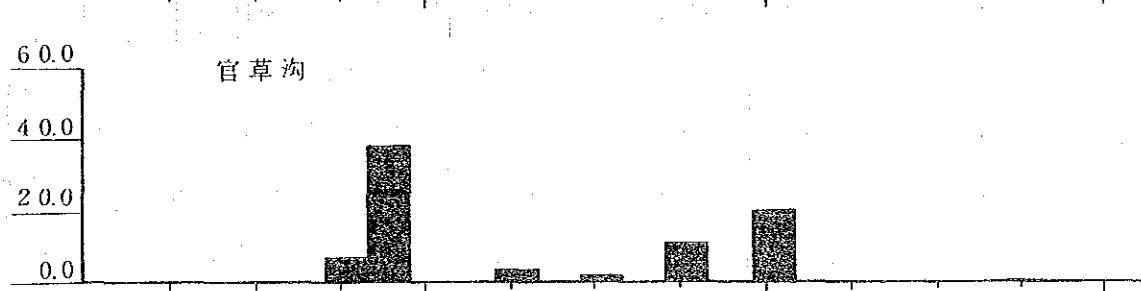
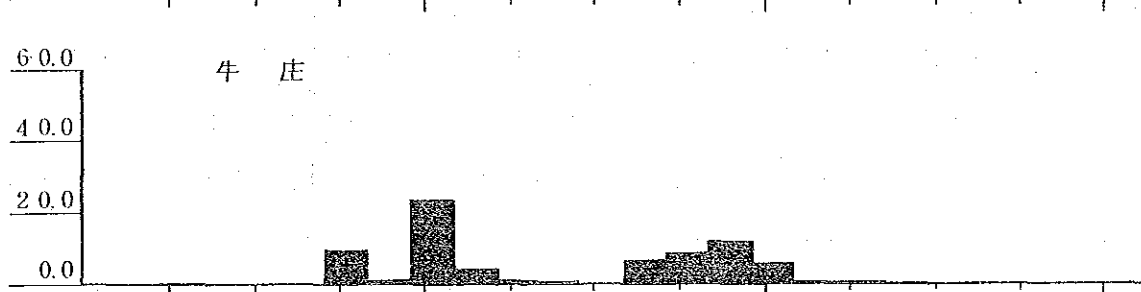
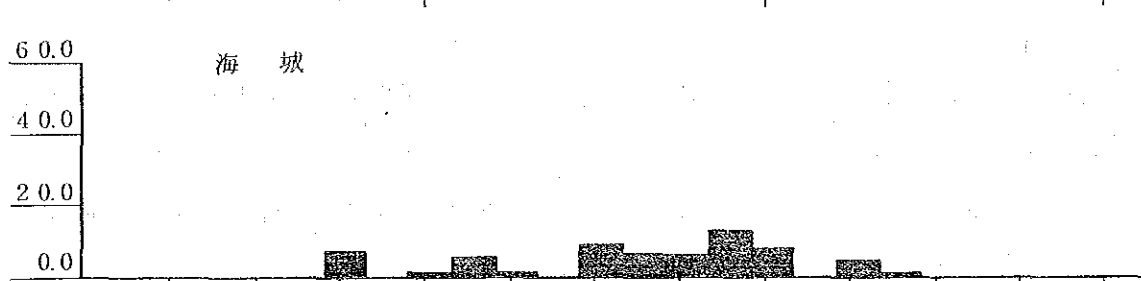
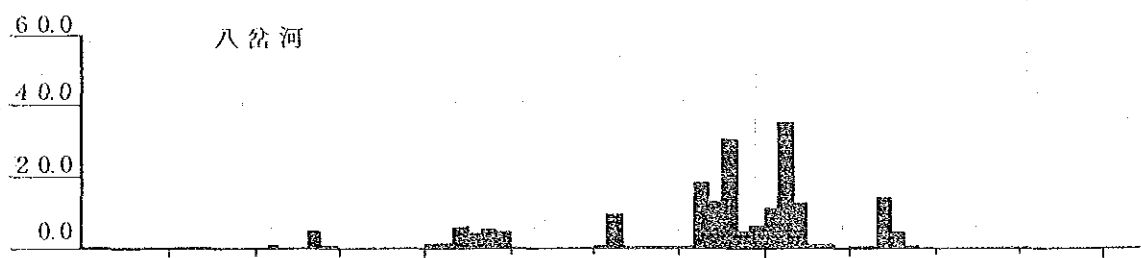
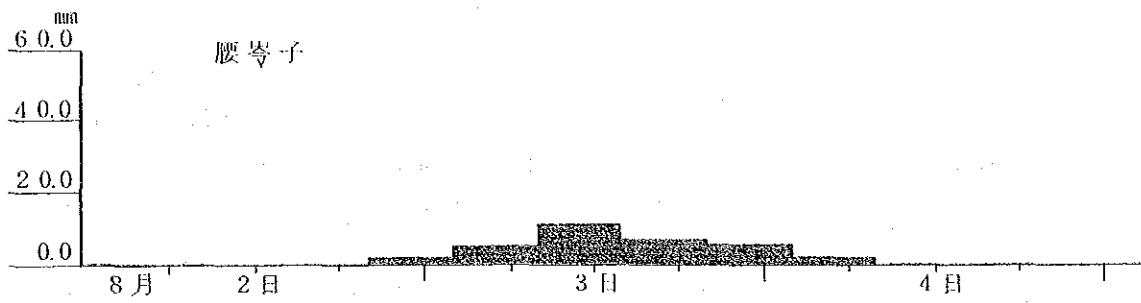
图A.5.8(1) 1960年洪水各观测所时间雨量分布



图A.5.8(2) 1960年洪水各观测所时间雨量分布



圖A.5.8(3) 1960年洪水各觀測所時間雨量分布



图A.5.8(4) 1960年洪水各观测所时间雨量分布

表A.5.8(1) 確率計算結果

(單位：mm)
小市流域3日雨量

確 率	岩井法	HAZEN 法	順 序 確率法	PEARSON Ⅲ型法	HAZEN& CHOW法	GUMBEL 法	CHOW 法
W=1/ 2	97.9	95.0	97.9	97.9	104.2	103.3	102.4
1/ 5	157.6	152.4	165.3	156.2	161.2	167.9	157.7
1/ 10	202.4	197.1	217.8	199.4	196.8	210.7	194.2
1/ 20	249.0	244.9	273.7	243.9	229.6	251.7	229.3
1/ 30	277.5	274.8	308.3	270.9	248.0	275.3	249.5
1/ 50	314.7	314.6	354.1	306.0	270.7	304.8	274.7
1/100	367.9	372.9	420.6	356.0	300.8	344.6	308.7
1/200	424.5	436.8	492.5	408.8	330.4	384.3	342.6
1/500	505.1	530.5	596.3	483.4	369.2	436.6	387.3
1/10000	819.3	927.1	1015.9	770.7	495.8	607.5	533.4

表 A. 5. 8 (2) 確率計算結果

(單位：mm)
遼陽流域 3 日雨量

確 率	岩井法	HAZEN 法	順 序 確率法	PEARSON Ⅲ型法	HAZEN& CHOW法	GUMBEL 法	CHOW 法
W=1/ 2	89.2	85.8	89.2	89.2	94.7	94.3	93.5
1/ 5	143.2	139.8	150.2	142.8	148.7	155.4	145.7
1/ 10	183.5	132.6	197.3	182.6	182.6	195.8	180.3
1/ 20	225.2	228.9	247.3	223.6	214.2	234.7	213.5
1/ 30	250.6	258.1	278.1	248.6	232.0	257.0	232.6
1/ 50	283.8	297.2	318.9	281.0	253.9	284.9	256.4
1/100	331.1	354.9	377.8	327.3	283.2	332.5	288.6
1/200	381.3	418.7	441.3	376.2	312.2	360.1	320.6
1/500	452.4	513.2	532.8	445.5	350.2	409.5	362.9
1/10000	728.3	921.4	899.8	712.6	475.6	571.1	501.0

表A.5.9 太子河流域平均確率3日雨量

Pearson III型法

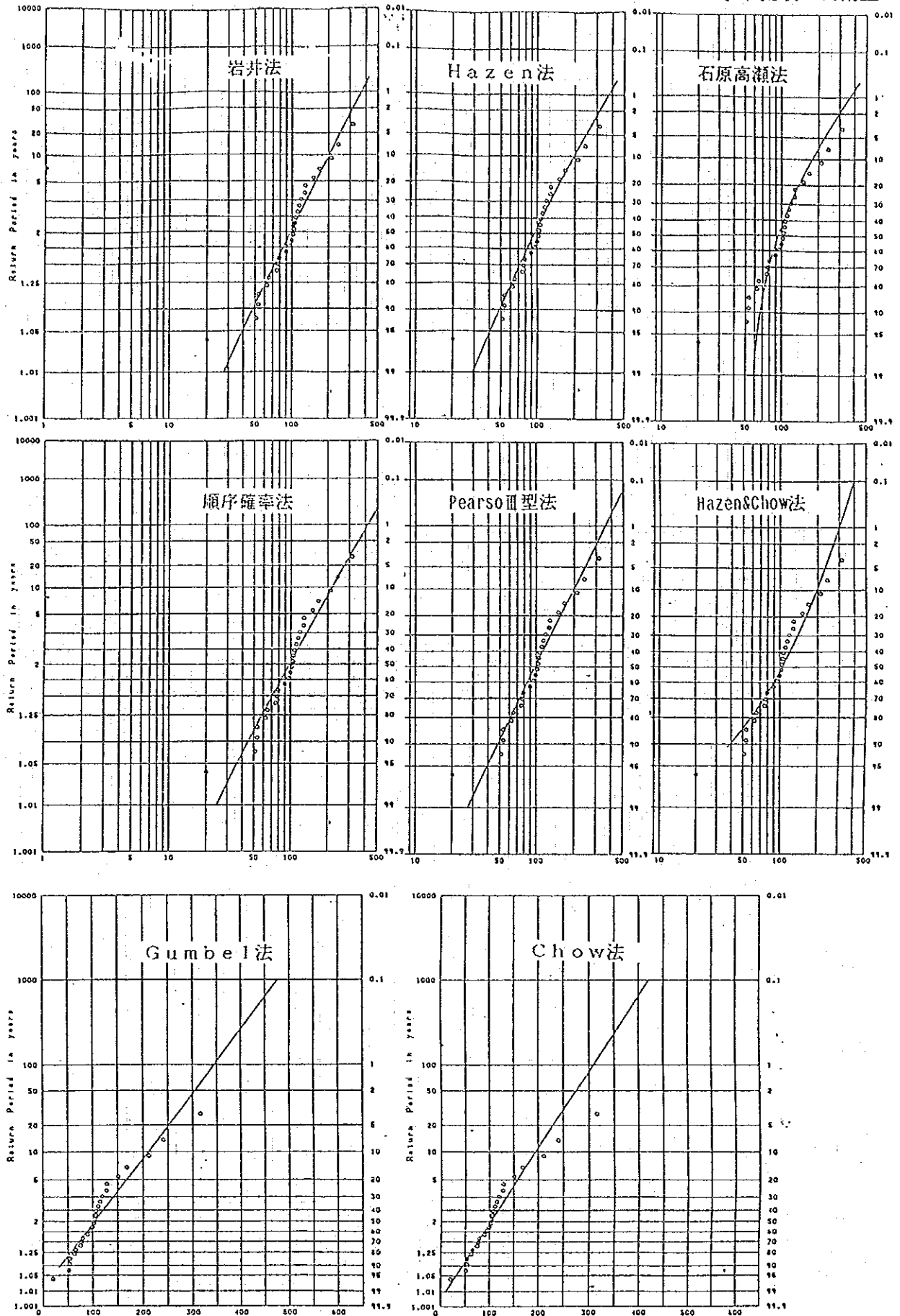
(單位：mm)

確率年	小市流域	小遼流域	殘流域	遼陽流域	備考
W= 1/ 2	97.9	99.5	80.1	89.2	
5	156.2	161.0	127.7	142.8	
10	199.4	207.1	162.9	182.6	
20	243.9	255.0	199.3	223.6	
30	270.9	284.1	221.3	248.6	
50	306.0	322.2	245.0	281.0	
100	356.0	376.6	290.8	327.3	
200	408.8	434.3	333.9	376.2	
500	483.4	516.4	394.8	445.5	
1000	544.1	585.4	444.3	502.2	
10000	770.7	835.6	629.3	712.6	

図A.5.9(1) 各確率手法による流域平均3日雨量確率

(単位: mm)

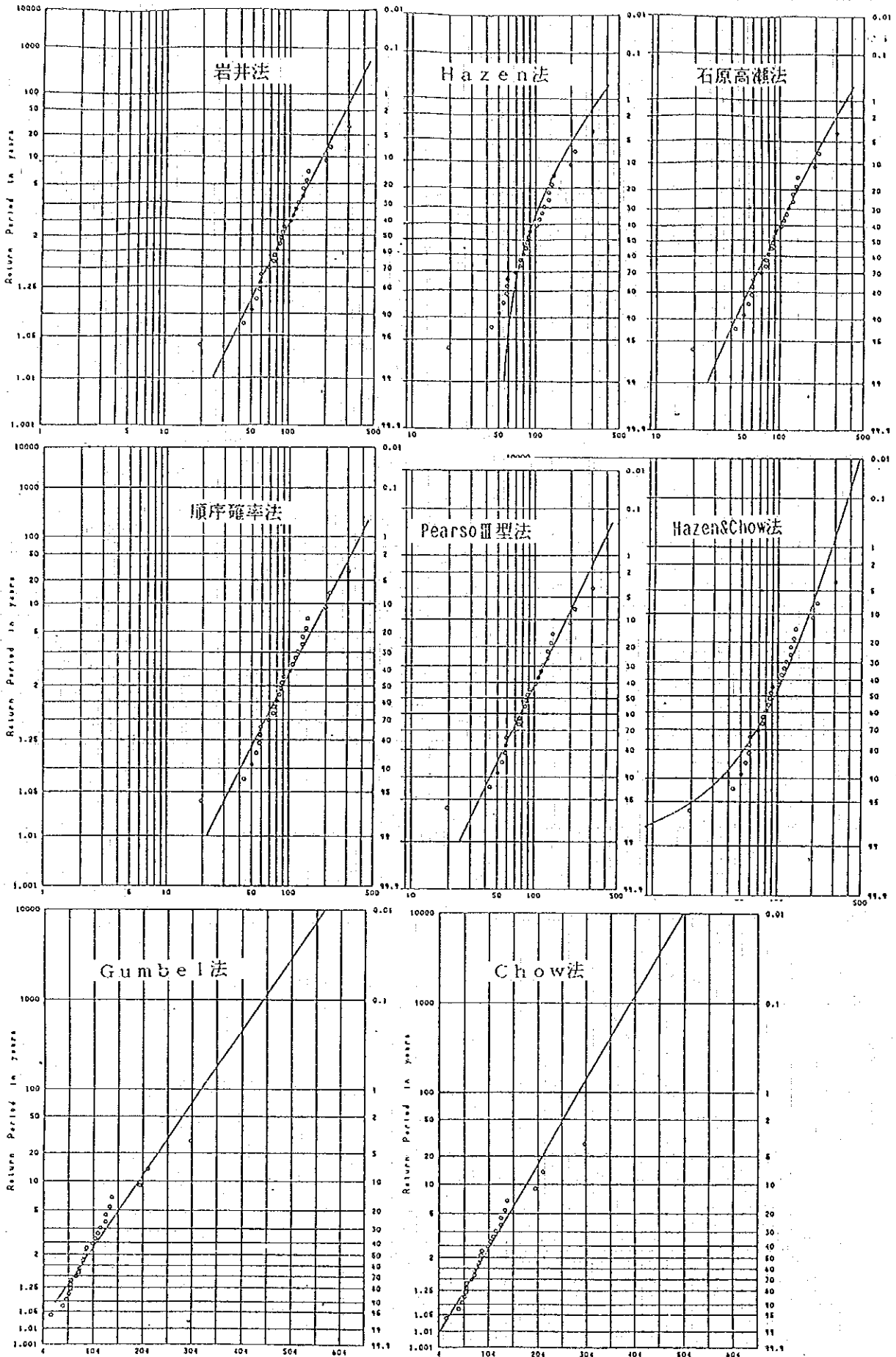
小市流域3日雨量



図A. 5.9(2) 各確率手法による流域平均3日雨量確率

(単位: mm)

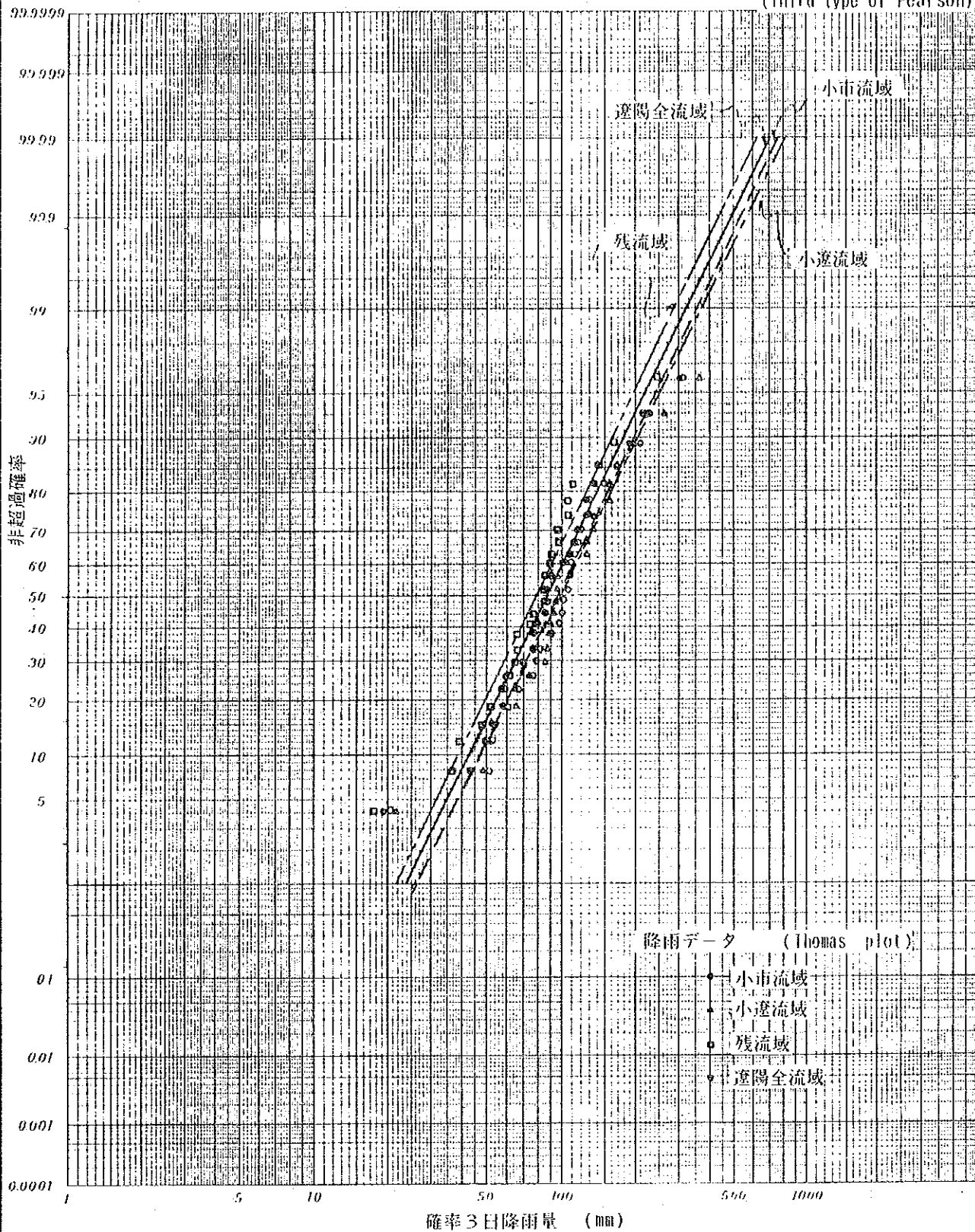
遼陽流域3日雨量



对 数 確 率 紙

図A.5.10 年最大3日降雨量非超過確率

(third type of Pearson)



降雨データ (Thomas plot)

- 小市流域
- ▲ 小遼流域
- 残流域
- ◇ 遼陽全流域

$$f(x) = \frac{M}{x \cdot \ln 10} e^{-\frac{(\log x - \log \xi)^2}{2\sigma^2}}$$

(M: log10 σ , x > 0)

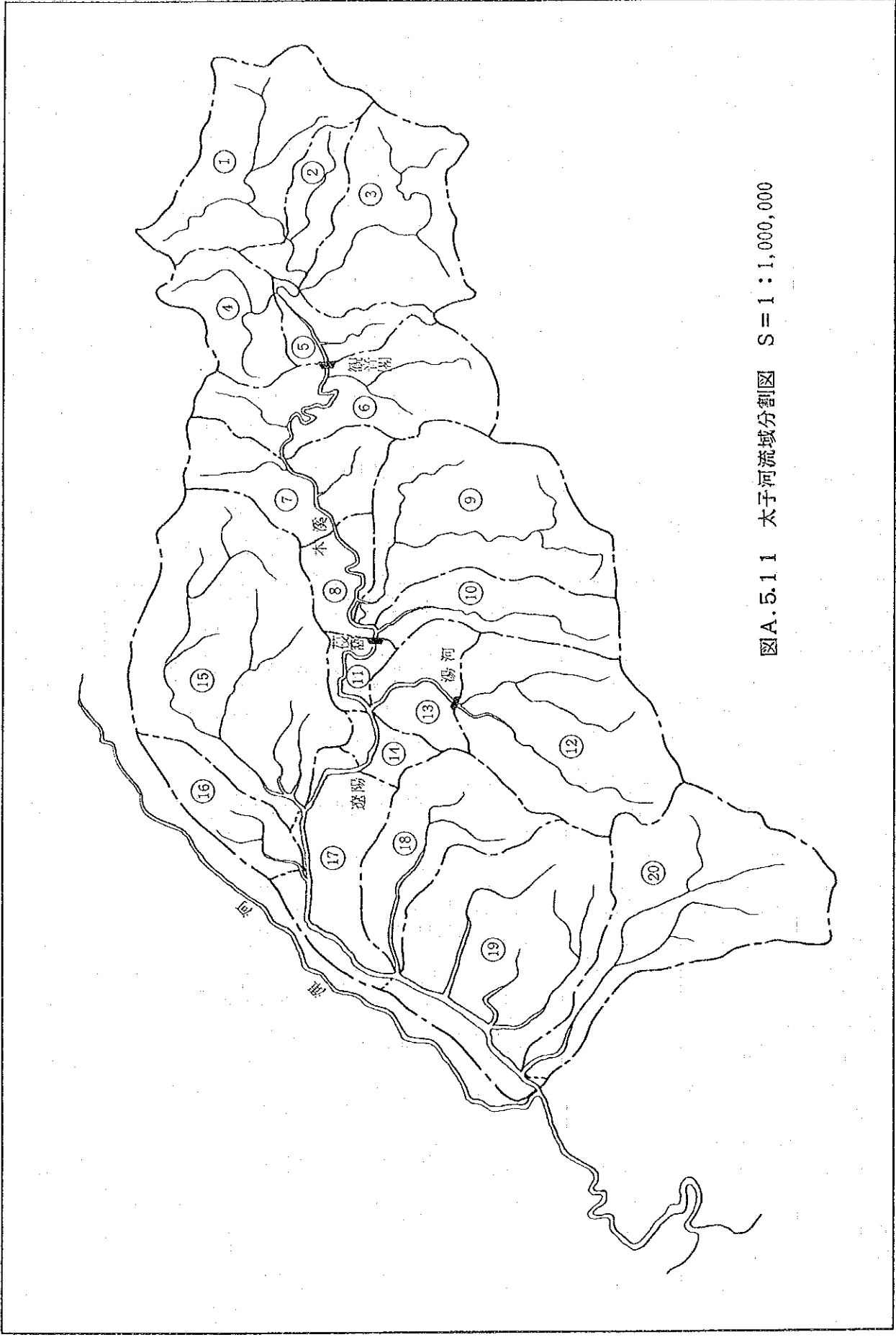
表A.5.10 太子河流域確率別降雨引伸ばし率 (1960年洪水)

1960年洪水 3日雨量
Pearson III型法

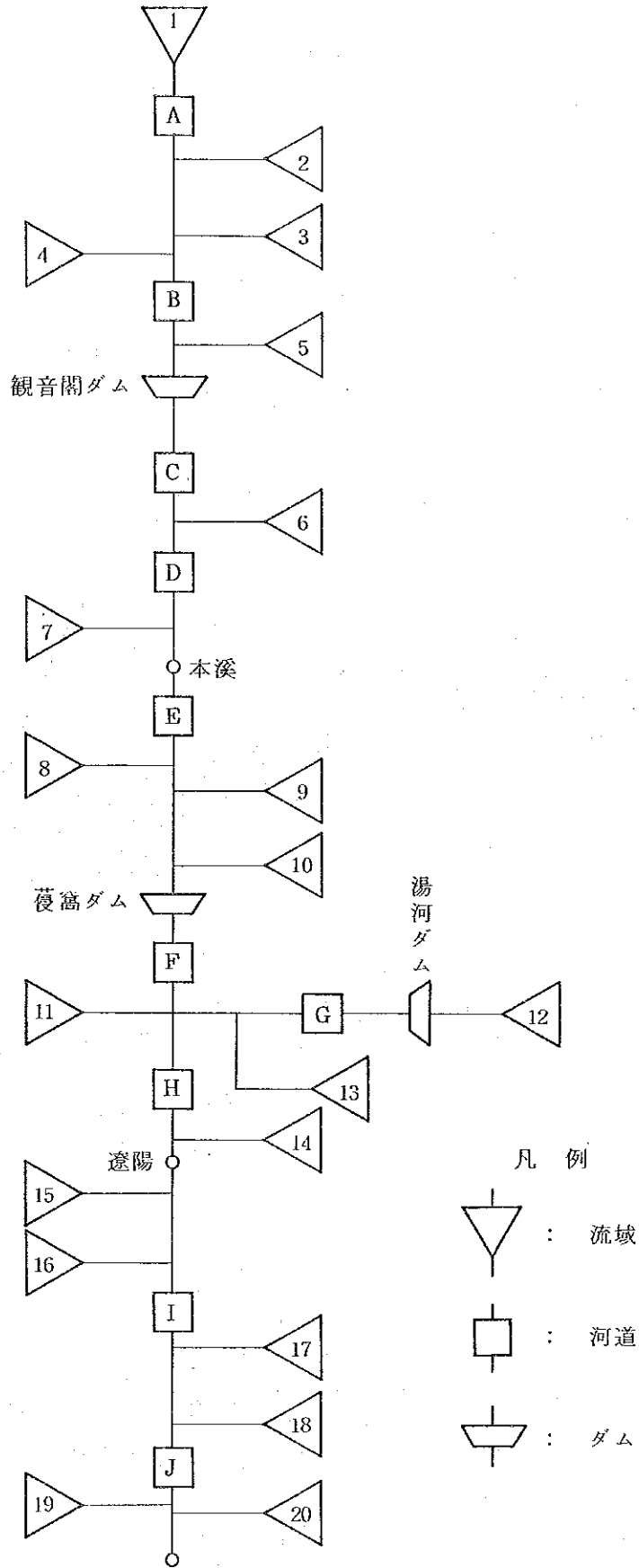
確率年	小市流域	小遼流域	残流域	遼陽流域	備考
実績3日雨量	315.78	352.29	223.94	301.75	
W= 1/ 2	0.3100	0.2824	0.3577	0.2956	
5	0.4946	0.4570	0.5702	0.4732	
10	0.6315	0.5879	0.7274	0.6051	
20	0.7724	0.7238	0.8900	0.7410	
30	0.8579	0.8064	0.9882	0.8239	
50	0.9690	0.9146	1.0940	0.9312	
100	1.1274	1.0690	1.2986	1.0847	
200	1.2946	1.2328	1.4910	1.2467	
500	1.5308	1.4658	1.7630	1.4764	
1000	1.7230	1.6617	1.9840	1.6643	
10000	2.4406	2.3719	2.8101	2.3616	

表A.5.1 1 分割流域勾配算定表

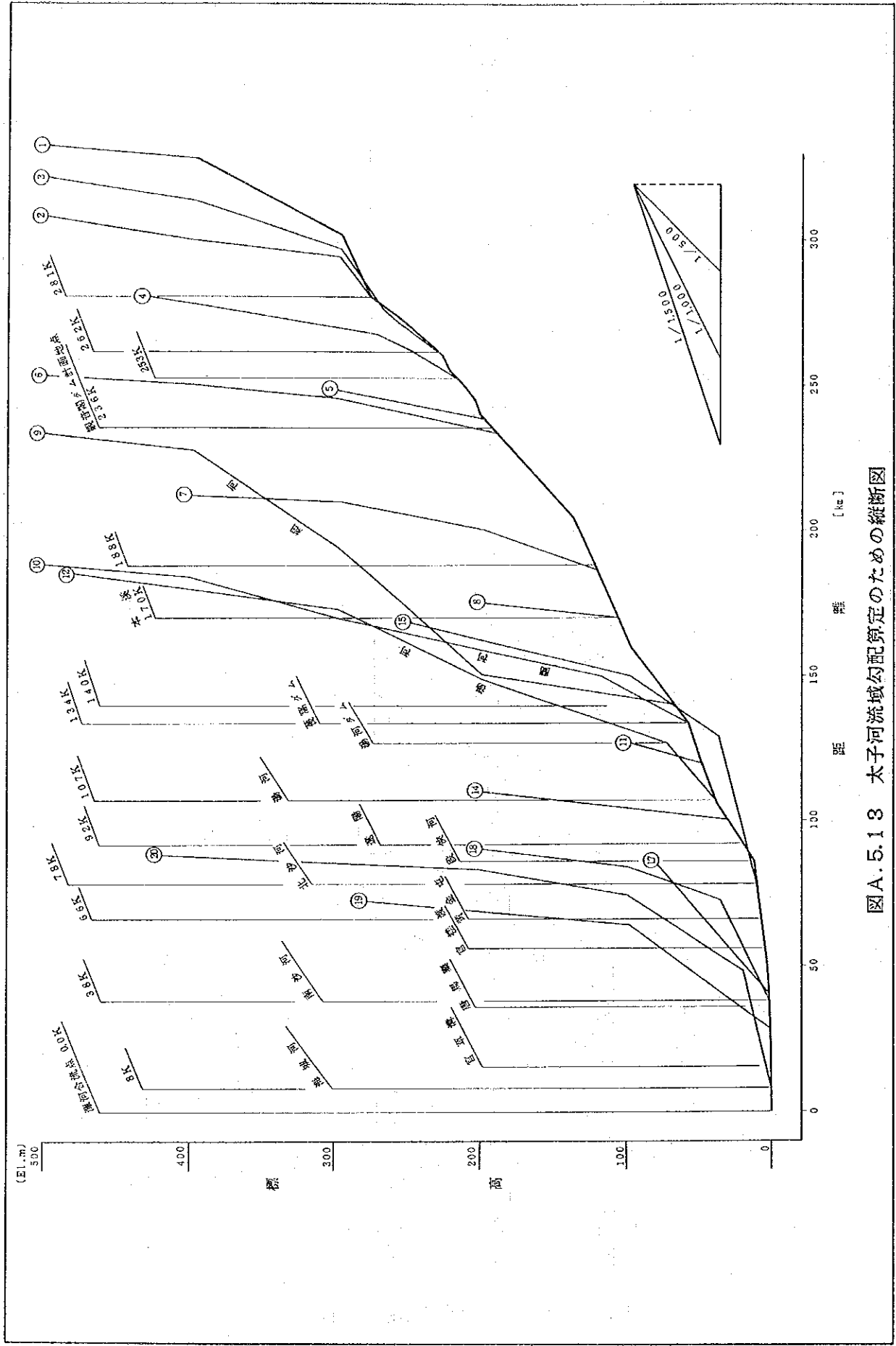
流域	下流端位標	上流端位標	下流端位標	計 算 式	流域勾配
1 流域	275 km	333 km	284 m	$\frac{6 \times 10 / 2 + (10 + 36) \times 22 / 2 + (36 + 136) \times 26 / 2 + (136 + 236) \times 4 / 2}{58 \times H / 2 = 3600.0}$ $H = 124.1$	1/ 467.4
2 流域	262 km	309 km	225 m	$\frac{19 \times 55 / 2 + (55 + 75) \times 14 / 2 + (75 + 275) \times 14 / 2 = 3882.5}{47 \times H / 2 = 3882.5}$ $H = 165.2$	1/ 284.5
3 流域	262 km	322 km	225 m	$\frac{10 \times 35 / 2 + (35 + 45) \times 5 / 2 + (45 + 75) \times 21 / 2 + (75 + 175) \times 16 / 2 + (175 + 275) \times 8 / 2 = 5435.0}{60 \times H / 2 = 5435.0}$ $H = 181.2$	1/ 331.1
4 流域	253 km	281 km	219 m	$\frac{10 \times 34 / 2 + (34 + 57) \times 5 / 2 + (57 + 211) \times 13 / 2 = 2139.5}{28 \times H / 2 = 2139.5}$ $H = 152.8$	1/ 183.2
5 流域	236 km	249 km	195 m	$\frac{3 \times 5 / 2 + (5 + 105) \times 10 / 2 = 557.5}{13 \times H / 2 = 557.5}$ $H = 85.8$	1/ 151.5
6 流域	188 km	254 km	124 m	$\frac{17 \times 16 / 2 + (16 + 66) \times 29 / 2 + (66 + 176) \times 12 / 2 + (176 + 276) \times 5 / 2 + (276 + 376) \times 3 / 2 = 4885.0}{66 \times H / 2 = 4885.0}$ $H = 148.0$	1/ 445.9
7 流域	170 km	212 km	108 m	$\frac{16 \times 14 / 2 + (14 + 92) \times 14 / 2 + (92 + 192) \times 10 / 2 + (192 + 292) \times 2 / 2 = 2758.0}{42 \times H / 2 = 2758.0}$ $H = 131.3$	1/ 319.9
8 流域	140 km	175 km	70 m	$\frac{20 \times 30 / 2 + (30 + 38) \times 10 / 2 + (38 + 130) \times 5 / 2 = 1060.0}{35 \times H / 2 = 1060.0}$ $H = 60.6$	1/ 577.6
9 流域	140 km	234 km	70 m	$\frac{10 \times 130 / 2 + (130 + 230) \times 44 / 2 + (230 + 330) \times 34 / 2 + (330 + 430) \times 6 / 2 = 20370.0}{94 \times H / 2 = 20370.0}$ $H = 433.4$	1/ 216.9
10 流域	134 km	188 km	60 m	$\frac{16 \times 60 / 2 + (60 + 240) \times 20 / 2 + (240 + 340) \times 14 / 2 + (340 + 440) \times 4 / 2 = 9100.0}{54 \times H / 2 = 9100.0}$ $H = 337.0$	1/ 160.2
11 流域	107 km	127 km	40 m	$\frac{13 \times 10 / 2 + (10 + 60) \times 7 / 2 = 310.0}{20 \times H / 2 = 310.0}$ $H = 31.0$	1/ 645.1
12 流域	127 km	185 km	75 m	$\frac{22 \times 125 / 2 + (125 + 225) \times 21 / 2 + (225 + 405) \times 12 / 2 = 9355.0}{58 \times H / 2 = 9355.0}$ $H = 322.6$	1/ 179.8
13 流域	107 km	127 km	40 m	$\frac{20 \times 35 / 2 = 350.0}{20 \times H / 2 = 350.0}$ $H = 350.0$	1/ 571.4
14 流域	92 km	110 km	20 m	$\frac{8 \times 12 / 2 + (12 + 180) \times 10 / 2 = 1008.0}{18 \times H / 2 = 1008.0}$ $H = 112.0$	1/ 822.7
15 流域	78 km	168 km	10 m	$\frac{51 \times 28 / 2 + (28 + 90) \times 21 / 2 + (90 + 240) \times 18 / 2 = 4923.0}{90 \times H / 2 = 4923.0}$ $H = 109.4$	1/ 691.1
16 流域	60 km	111 km	7 m	$\frac{51 \times 27 / 2 = 688.5}{51 \times H / 2 = 688.5}$ $H = 27.0$	1/ 1888.9
17 流域	38 km	85 km	10 m	$\frac{47 \times 68 / 2 = 1598.0}{47 \times H / 2 = 1598.0}$ $H = 68.0$	1/ 691.1
18 流域	38 km	90 km	2 m	$\frac{35 \times 34 / 2 + (34 + 98) \times 11 / 2 + (98 + 198) \times 6 / 2 = 2209.0}{52 \times H / 2 = 2209.0}$ $H = 85.0$	1/ 611.8
19 流域	8 km	72 km	0 m	$\frac{19 \times 1 / 2 + (1 + 99) \times 36 / 2 + (99 + 279) \times 8 / 2 = 3321.5}{64 \times H / 2 = 3321.5}$ $H = 103.8$	1/ 616.6
20 流域	8 km	88 km	0 m	$\frac{40 \times 20 / 2 + (20 + 100) \times 26 / 2 + (100 + 200) \times 9 / 2 + (200 + 420) \times 5 / 2 = 4860.0}{80 \times H / 2 = 4860.0}$ $H = 121.5$	1/ 658.4



图A.5.1.1 太子河流域分割图 S=1:1,000,000



図A. 5.1 2 太子河洪水追跡系統図



図A.5.13 太子河流域勾配算定のための縦断面図

表A.5.12 流域貯留関数定数一覧表

	流域面積 (km ²)	流路長 (km)	勾配	(K)	(P)	(T1)	(F1)	(Rsa)	(Q)
1	928	58	1/ 467.4	10.8	0.90	4.4	0.5	80.0	37
2	283	47	1/ 284.5	13.4	0.78	3.2	"	"	11
3	888	60	1/ 331.1	12.5	0.80	4.6	"	"	36
4	464	28	1/ 183.2	16.0	0.68	1.6	"	"	19
5	232	13	1/ 151.5	17.4	0.66	1.2	"	"	9
6	909	66	1/ 445.9	11.0	0.88	5.0	"	"	36
7	620	42	1/ 319.9	12.6	0.80	2.8	"	"	25
8	210	35	1/ 577.6	9.8	0.94	2.2	"	"	8
9	1,113	94	1/ 216.9	15.0	0.72	7.8	"	"	44
10	528	54	1/ 160.2	17.0	0.66	4.0	"	"	21
11	231	20	1/ 645.1	9.3	0.98	0.8	"	"	9
12	1,228	58	1/ 179.8	16.2	0.68	4.4	"	"	49
13	232	20	1/ 571.4	9.8	0.94	0.8	"	"	9
14	216	18	1/ 160.7	17.0	0.66	0.6	"	"	9
15	1,534	90	1/ 822.7	8.4	1.05	7.4	"	"	61
16	376	51	1/1888.9	5.6	1.32	3.6	"	"	15
17	655	47	1/ 691.1	9.1	1.00	3.2	"	"	26
18	458	52	1/ 611.8	9.6	0.96	3.8	"	"	18
19	1,504	64	1/ 616.6	9.5	0.96	4.8	"	"	60
20	1,274	80	1/ 658.4	9.3	0.98	6.4	"	"	51

(km) (m) 河道貯留関数定数一覧表

	流路長	河幅	勾配	K	P	T1	不定流扱い
A	19.0		1/ 400.	53.96	0.5999	0.6	しない
B	26.0		1/ 500.	304.63	0.4880	1.0	"
C	48.0	625.0	1/ 647.	114.92	0.6059	2.0	"
D	16.0	1240.0	1/ 800.	54.88	0.6023	0.7	"
E	40.0	718.0	1/ 909.	112.61	0.6052	2.0	"
F	24.0	828.0	1/1091.	75.91	0.6044	1.3	"
G	16.0	1550.0	1/ 452.	50.70	0.6017	0.6	"
H	20.0	1460.0	1/1250.	83.69	0.6022	1.2	"
I	40.0	1800.0	1/2857.	233.62	0.6020	3.5	"
J	48.0	2177.0	1/5000.	445.92	0.6023	5.6	"

表A.5.1 3(1) 河道貯留定数算定内訳表 (1)

等流計算 n=0.030

C-河道 流量

D-河道 流量

水深 (H)	河 積 (H2)	流 量 (H3/S)	容 量 (10**6 M3)
0.5	313.25	258.14	15.04
1.0	628.00	820.08	30.14
1.5	944.25	1612.86	45.32
2.0	1262.00	2608.99	60.58
2.5	1581.25	3783.92	75.90
3.0	1902.00	5130.98	91.30
3.5	2224.25	6638.51	105.76
4.0	2548.00	8298.88	122.30
4.5	2873.25	10105.80	137.92
5.0	3200.00	12054.10	153.60
5.5	3528.25	14139.30	169.36
6.0	3858.00	16357.40	185.20
6.5	4189.25	18705.10	201.08
7.0	4522.00	21179.00	217.06
7.5	4856.25	23777.40	233.10
8.0	5192.00	26496.00	249.22
8.5	5529.25	29335.70	265.40
9.0	5868.00	32291.70	281.66
9.5	6208.25	35363.00	298.00
10.0	6550.00	38548.10	314.40

水 深 (H)	河 積 (H2)	流 量 (H3/S)	容 量 (10**6 M3)
0.5	620.75	480.44	9.93
1.0	1243.00	1482.28	19.89
1.5	1866.75	2875.13	29.87
2.0	2492.00	4645.46	39.87
2.5	3118.75	6740.41	48.90
3.0	3747.00	9136.86	59.95
3.5	4376.75	11817.30	70.03
4.0	5008.00	14767.80	80.13
4.5	5640.75	17976.80	90.25
5.0	6275.00	21434.80	100.40
5.5	6910.75	25133.50	110.57
6.0	7548.00	29065.70	120.77
6.5	8186.75	33225.00	130.99
7.0	8827.00	37605.70	141.23
7.5	9468.75	42202.80	151.50
8.0	10112.00	47011.60	161.79
8.5	10756.80	52027.90	172.11
9.0	11403.00	57247.90	182.45
9.5	12050.80	62668.10	192.81
10.0	12700.00	68285.10	203.20

表A.5.1 3(2) 河道貯留定数算定内訳表 (2)

F-河道 流量

水深 (M)	河積 (M ²)	流量 (M ³ /S)	容量 (10**6 M ³)
0.5	414.75	263.32	9.95
1.0	831.00	836.40	19.94
1.5	1248.75	1644.78	29.97
2.0	1668.00	2657.98	40.03
2.5	2088.75	3857.28	50.13
3.0	2511.00	5229.57	60.24
3.5	2884.75	6764.90	70.43
4.0	3350.00	8455.38	80.64
4.5	3786.75	10294.60	90.88
5.0	4215.00	12277.00	101.16
5.5	4644.75	14398.10	111.47
6.0	5076.00	16653.70	121.82
6.5	5508.75	19040.30	132.21
7.0	5943.00	21554.80	142.63
7.5	6378.75	24194.30	153.09
8.0	6816.00	26856.20	163.58
8.5	7254.75	29638.30	174.11
9.0	7695.00	32638.30	184.68
9.5	8136.75	35854.30	195.28
10.0	8580.00	39184.70	205.92

E-河道 流量

水深 (M)	河積 (M ²)	流量 (M ³ /S)	容量 (10**6 M ³)
0.5	359.75	250.17	14.39
1.0	721.00	794.89	28.84
1.5	1083.75	1562.89	43.35
2.0	1448.00	2525.84	57.02
2.5	1813.75	3665.82	72.55
3.0	2181.00	4970.38	87.24
3.5	2549.75	6430.14	101.99
4.0	2920.00	8037.63	116.80
4.5	3291.75	9786.77	131.67
5.0	3665.00	11672.40	146.60
5.5	4029.75	13690.20	161.59
6.0	4416.00	15836.30	176.64
6.5	4793.75	18107.40	191.75
7.0	5173.00	20500.50	206.92
7.5	5553.75	23012.90	222.15
8.0	5936.00	25642.30	237.44
8.5	6319.75	28386.50	252.79
9.0	6705.00	31243.50	268.20
9.5	7091.75	34211.50	283.67
10.0	7480.00	37286.70	299.20

表 A. 5. 1 3 (3) 河道貯留定数算定内訳表 (3)

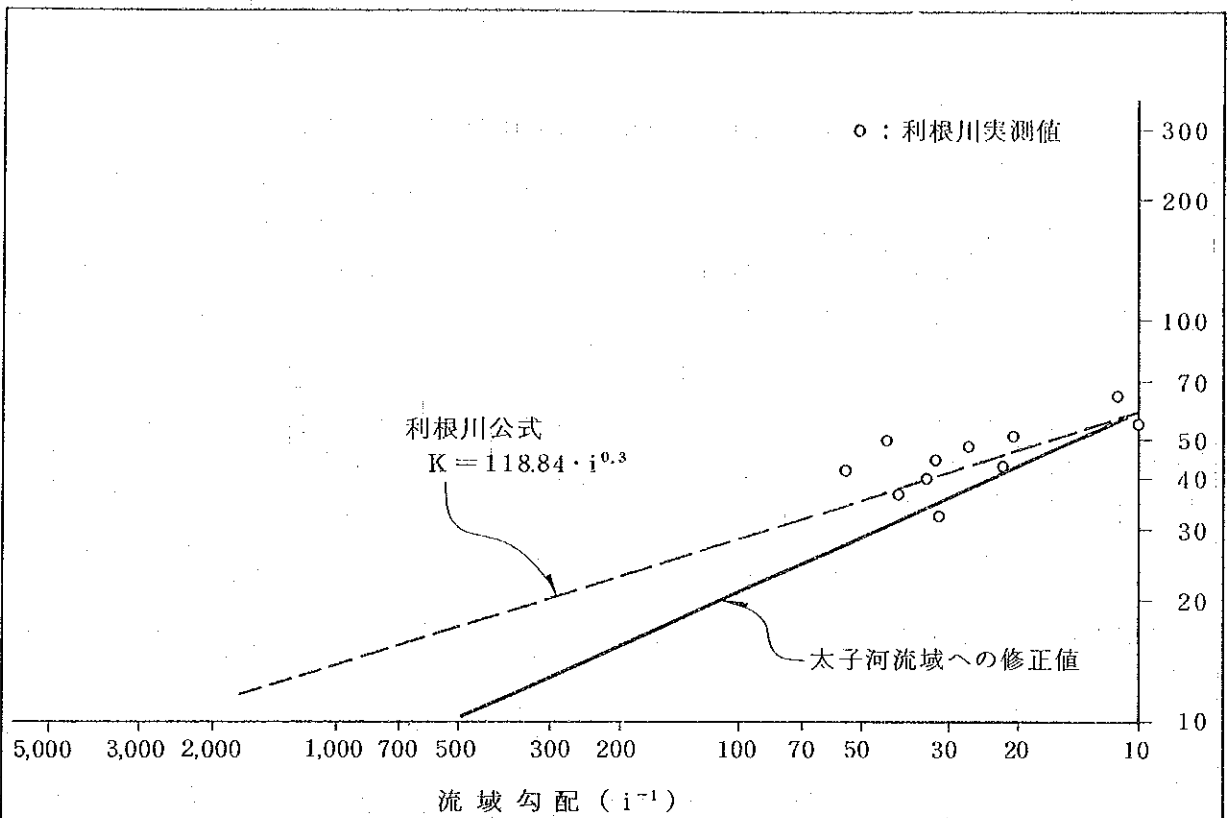
G-河道 流量

水深 (H)	河 積 (H2)	流 量 (H3/S)	容 量 (10 ⁺ 6 H3)
0.5	775.75	755.66	12.41
1.0	1553.00	2431.43	24.85
1.5	2331.75	4780.34	37.31
2.0	3112.00	7723.29	49.79
2.5	3893.75	11295.50	62.03
3.0	4677.00	15188.40	74.83
3.5	5461.75	19042.80	87.39
4.0	6248.00	24545.50	99.97
4.5	7035.75	29877.20	112.57
5.0	7825.00	35621.90	125.20
5.5	8615.75	41765.70	137.85
6.0	9408.00	48298.60	150.53
6.5	10201.80	55203.90	163.23
7.0	10997.00	62478.10	175.95
7.5	11793.80	70110.60	188.70
8.0	12592.00	78093.70	201.47
8.5	13391.80	86420.20	214.27
9.0	14193.00	95083.80	227.09
9.5	14995.80	104078.00	239.93
10.0	15800.00	113399.00	252.80

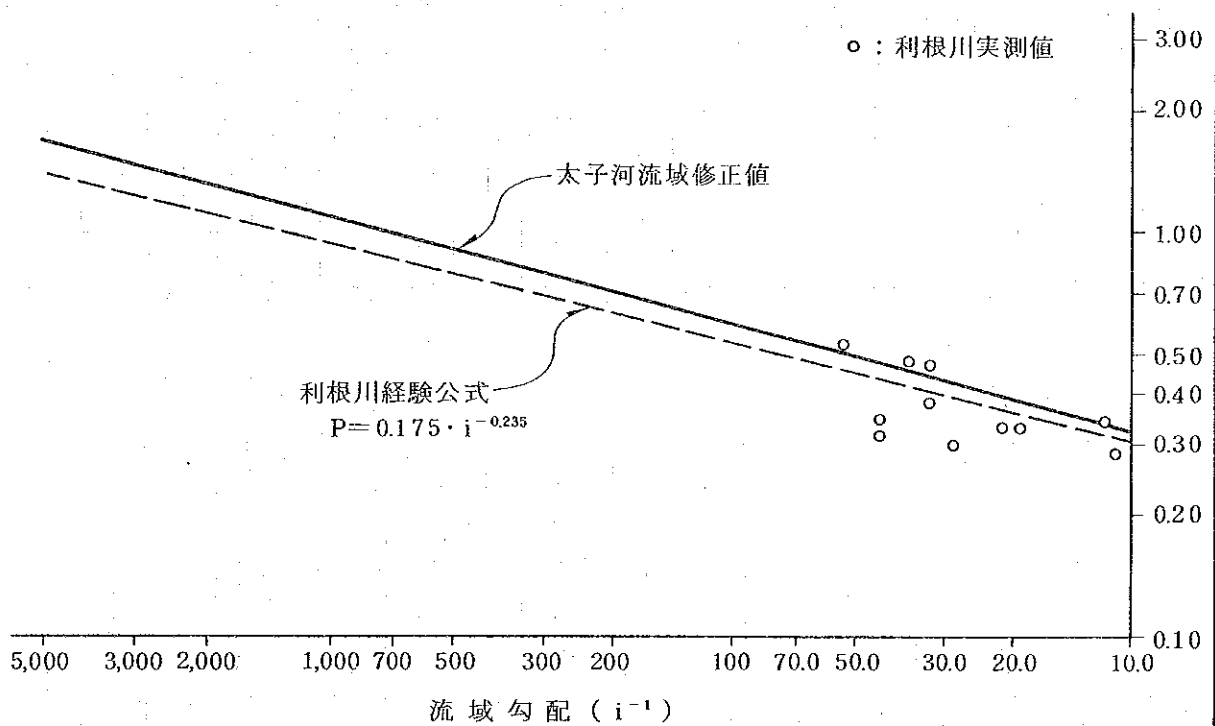
等流計算 n=0.030

H-河堤 流量

水 深 (H)	河 積 (H2)	流 量 (H3/S)	容 量 (10 ⁺ 6 H3)
0.5	730.75	433.68	14.62
1.0	1463.00	1377.24	29.26
1.5	2196.75	2707.79	43.94
2.0	2932.00	4374.87	58.64
2.5	3668.75	6347.48	73.38
3.0	4407.00	8603.80	88.14
3.5	5146.75	11127.30	102.94
4.0	5888.00	13904.70	117.76
4.5	6630.75	16925.40	132.52
5.0	7375.00	20180.10	147.50
5.5	8120.75	23661.10	162.42
6.0	8868.00	27361.40	177.36
6.5	9616.75	31275.10	192.34
7.0	10367.00	35396.80	207.34
7.5	11118.80	39721.70	222.38
8.0	11872.00	44245.40	237.44
8.5	12626.80	48963.80	252.54
9.0	13383.00	53873.40	267.66
9.5	14140.80	58970.70	282.82
10.0	14900.00	64252.60	298.00



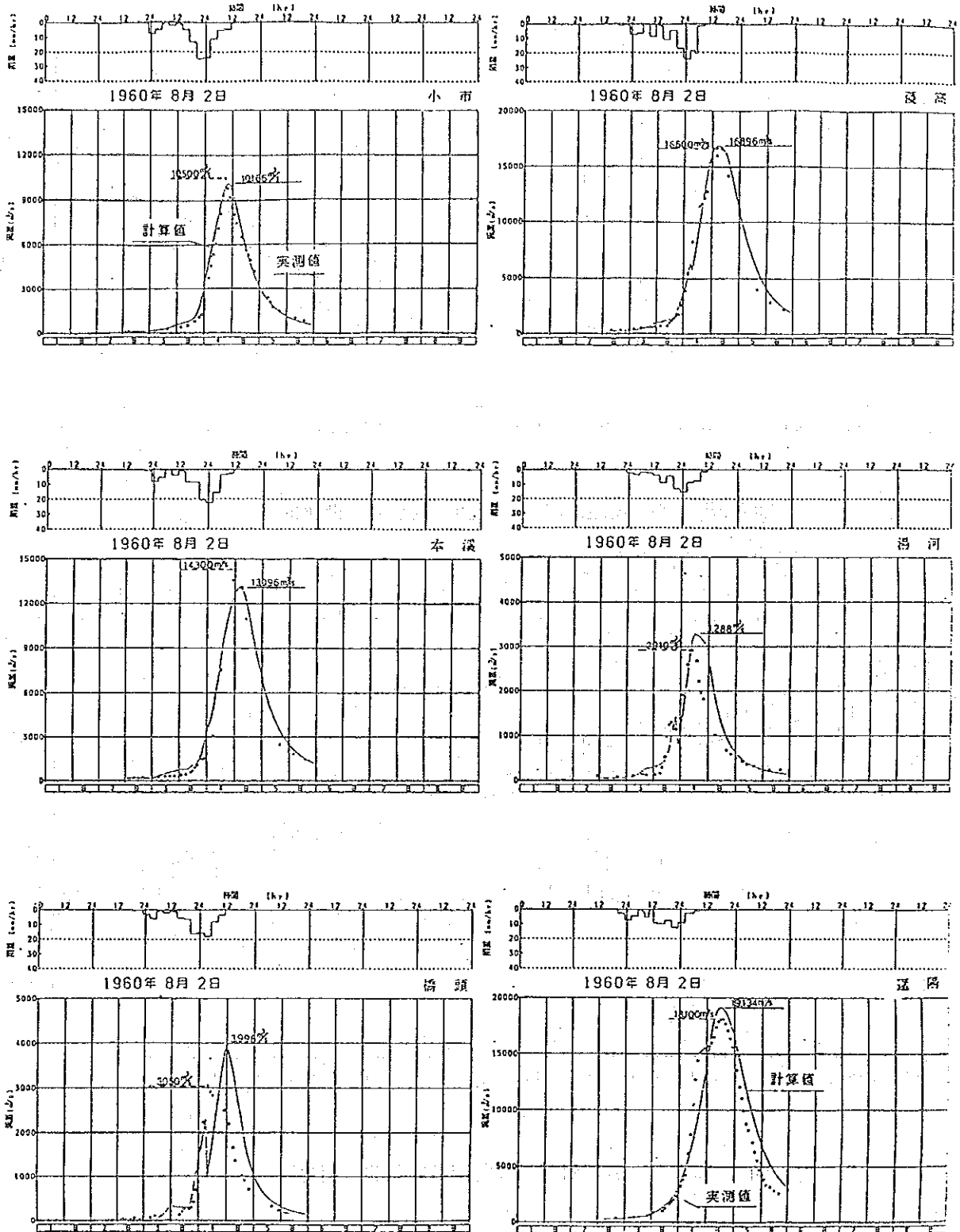
図A.5.14 流域勾配～貯留関数定数K 関係図



図A.5.15 流域勾配～貯留関数定数P 関係図

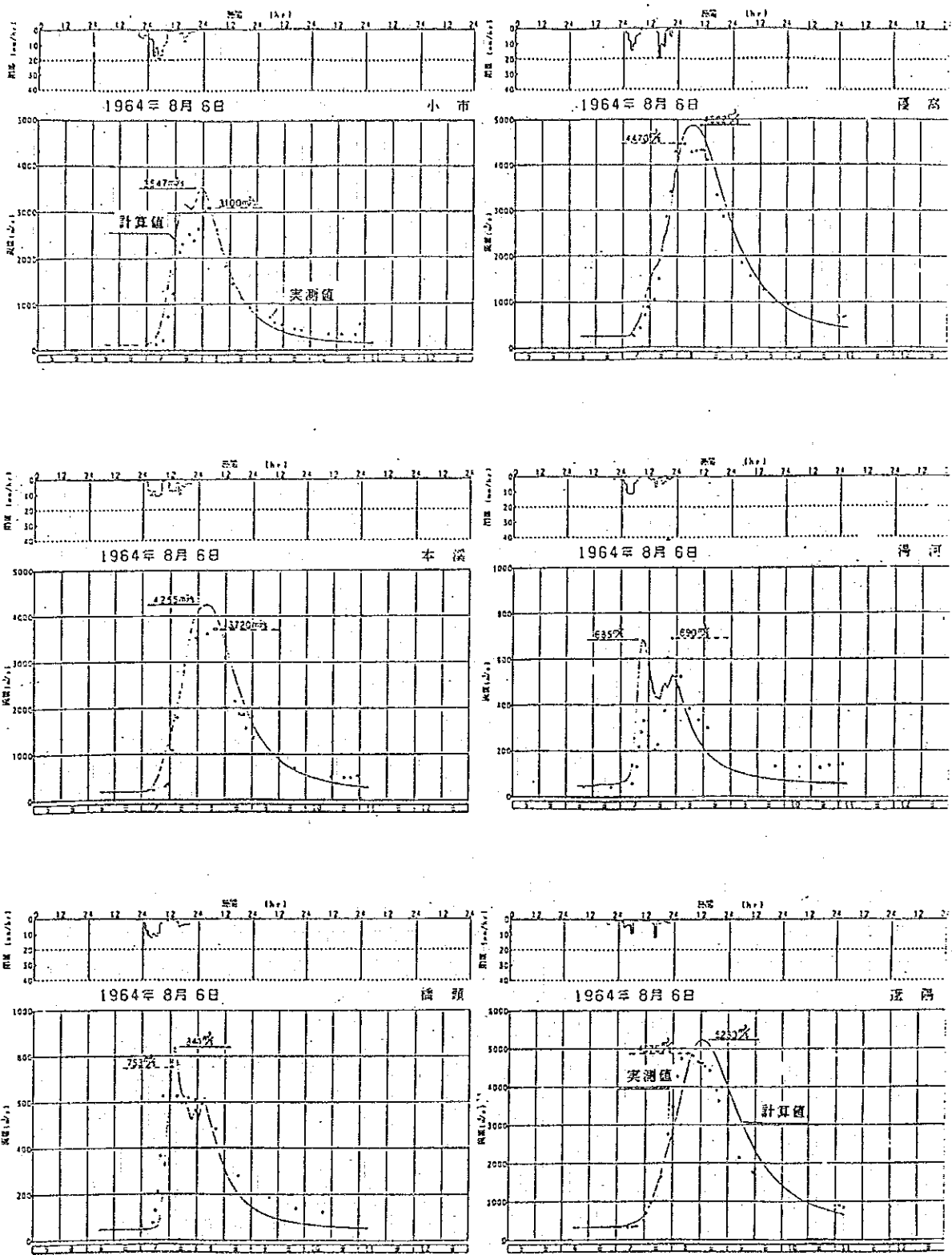
図A.5.16(1) 定数解析結果 1960年8月2日洪水

一次流出率 $r_1 = 0.5$ 越和雨量 $R_{11} = 80\text{mm}$



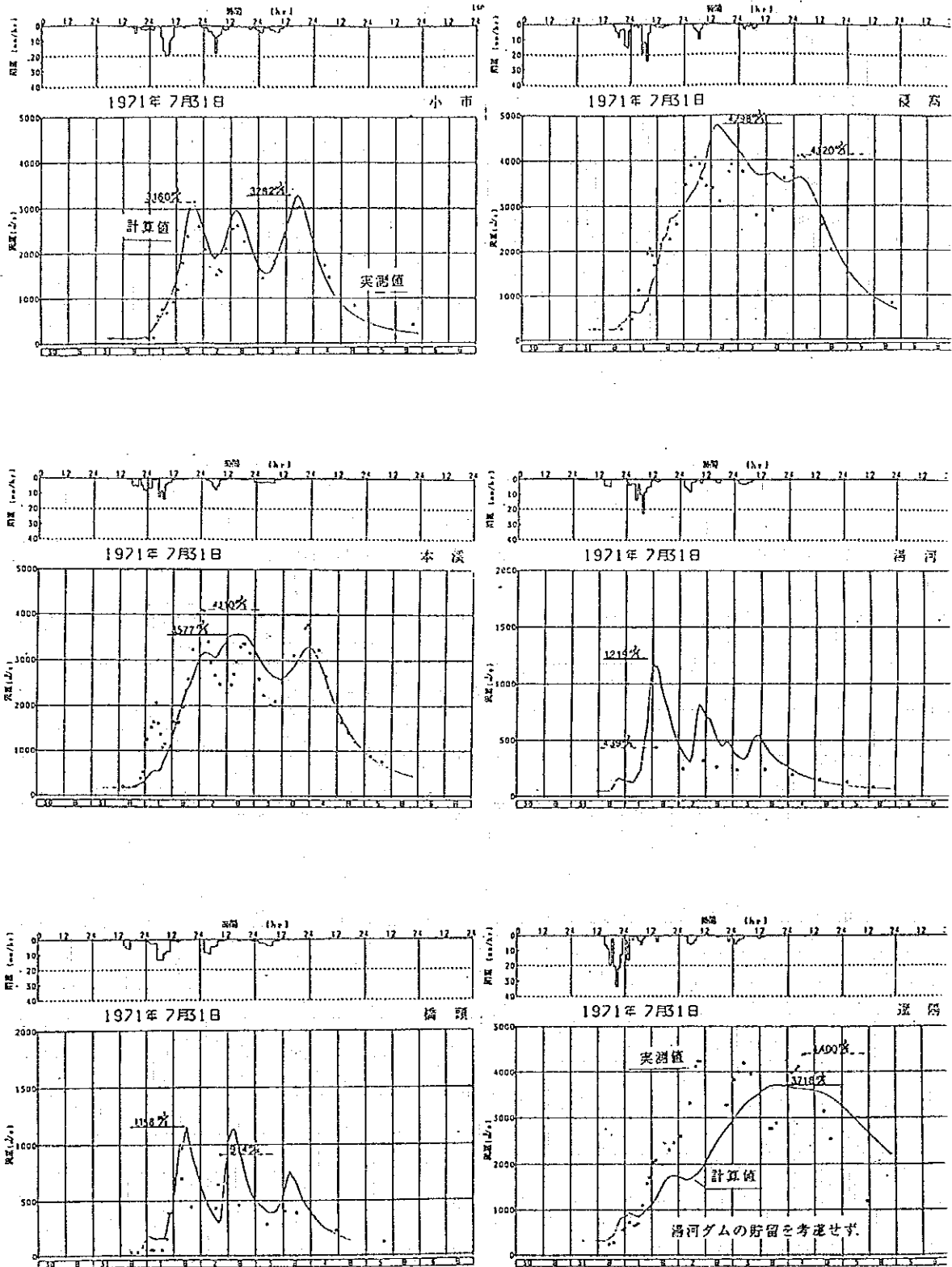
圖A.5.16(2) 定数解析結果 1964年8月6日洪水

一次流出率 $\lambda = 0.5$ 飽和雨量 $R_s = 80\text{mm}$



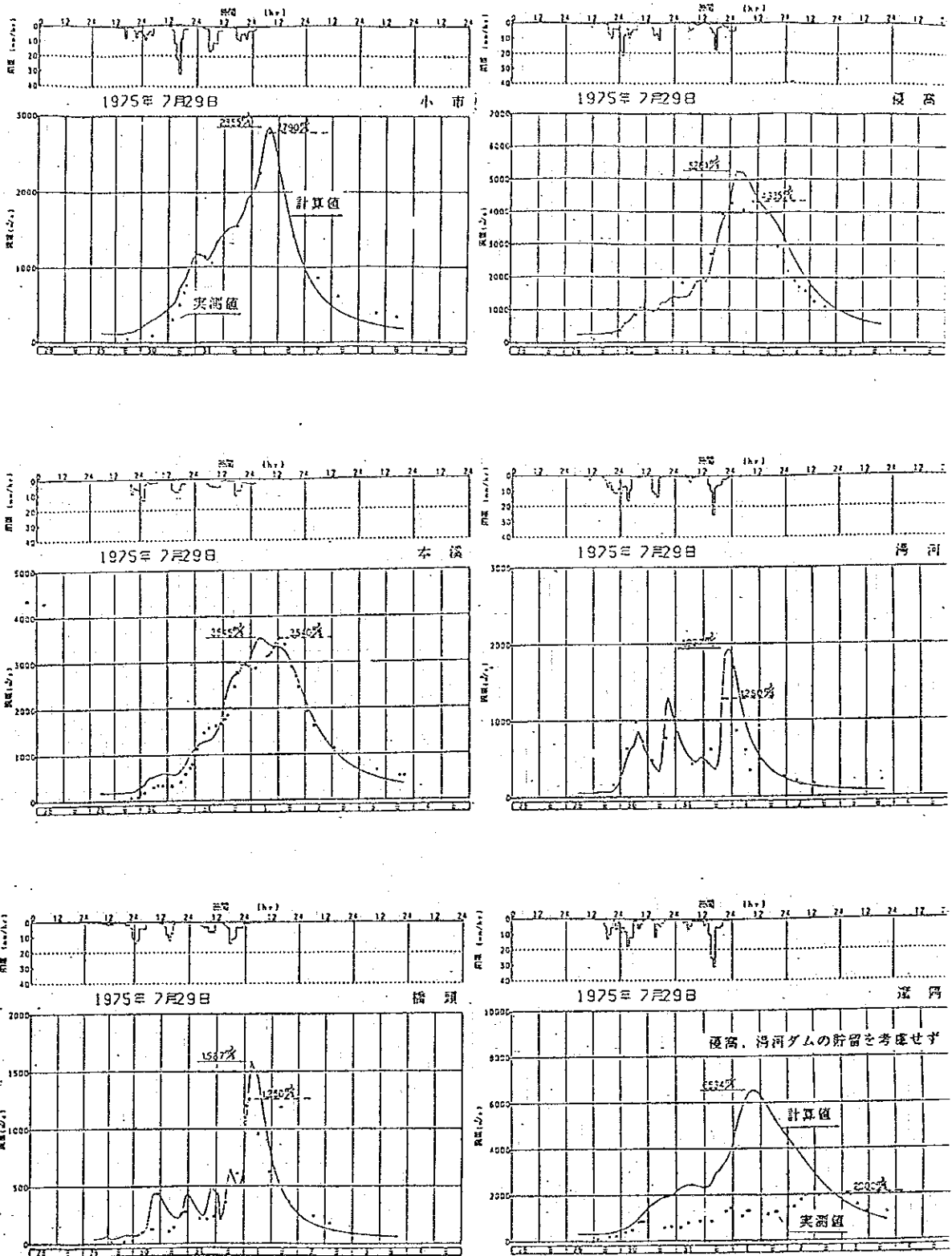
図A.5.16(3) 定数解析結果 1971年7月31日洪水

一次流出率 $f_1 = 0.5$ 飽和雨量 $R_{sA} = 80\text{mm}$



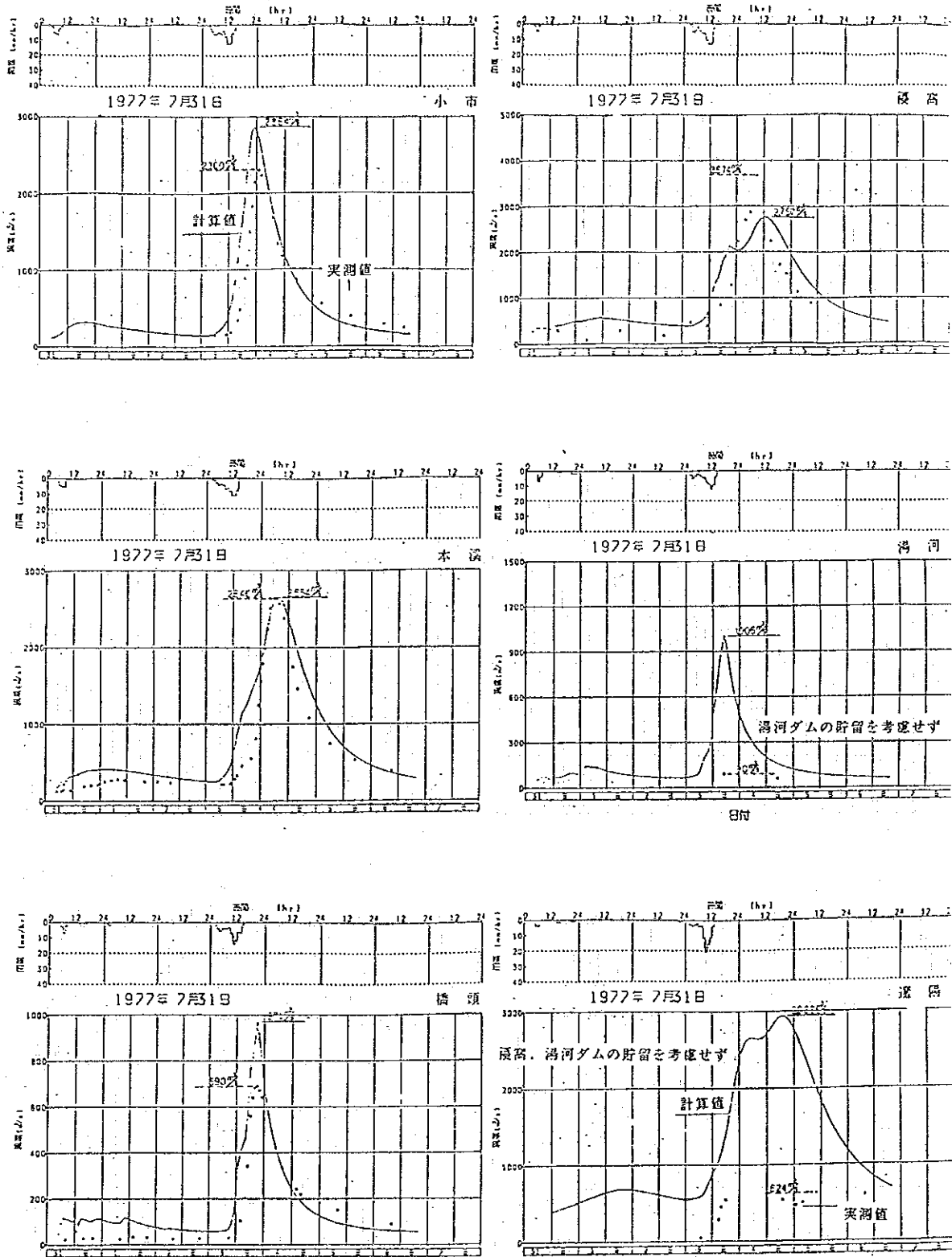
図A.5.16(4) 定数解析結果 1975年7月29日洪水

一次流出率 $\lambda_1 = 0.3$ 飽和雨量 $R_{sa} = 110 \text{ mm}$



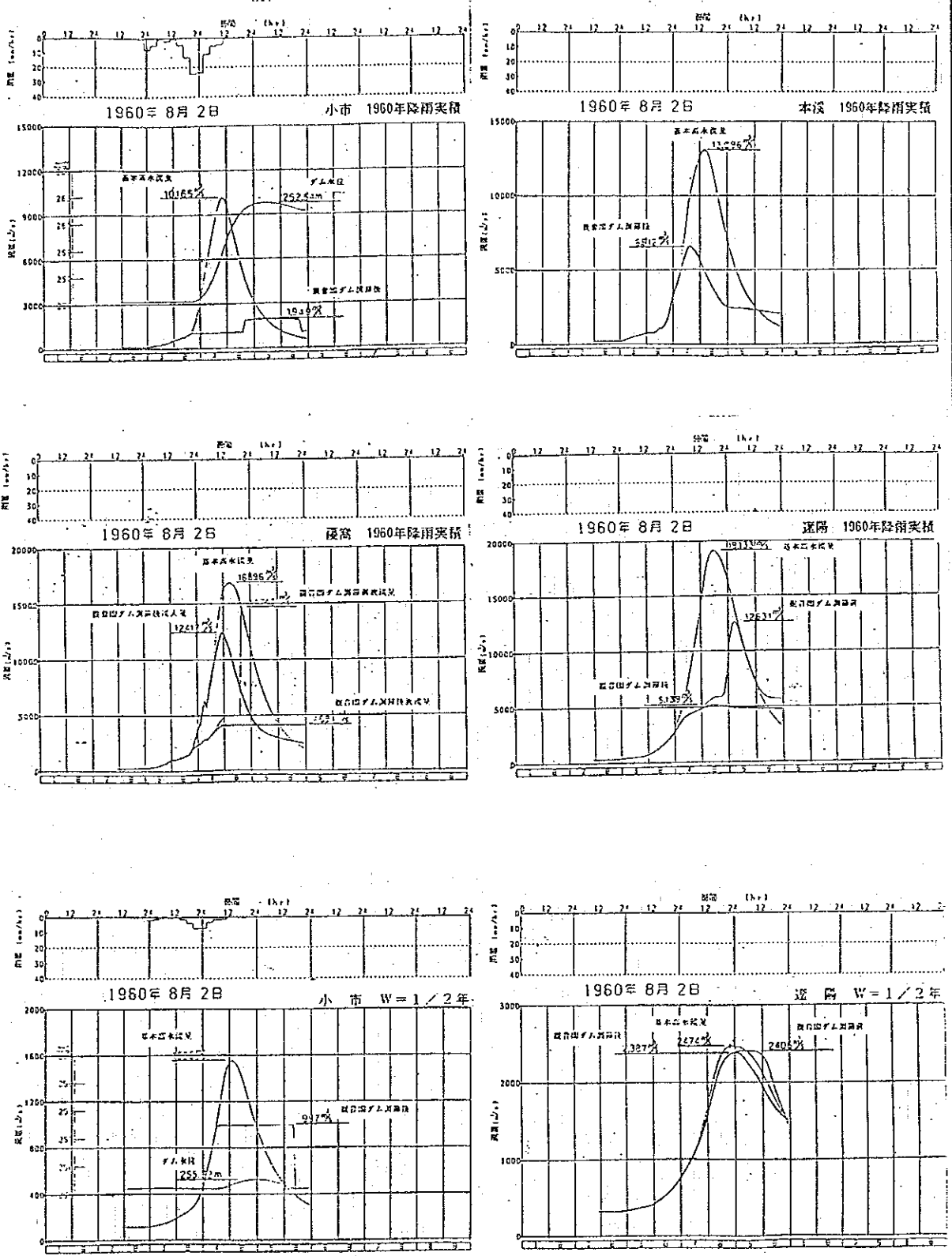
図A.5.16(5) 定数解析結果 1977年7月31日洪水

一次流出率 $f_1 = 0.5$ 総和雨量 $R_{\Sigma} = 80\text{mm}$



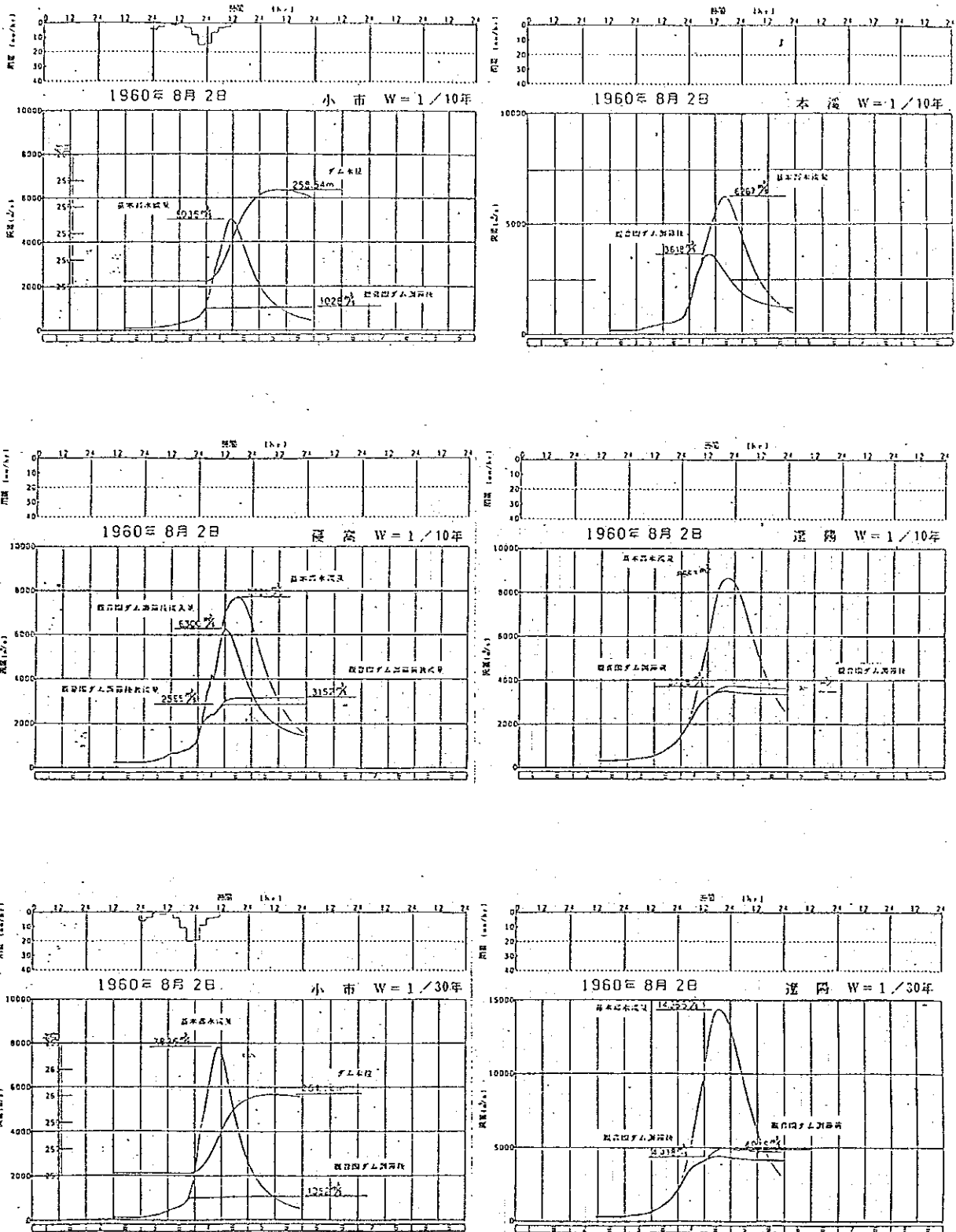
図A.5.17(1) 各確率規模別流出計算結果

1960年8月洪水



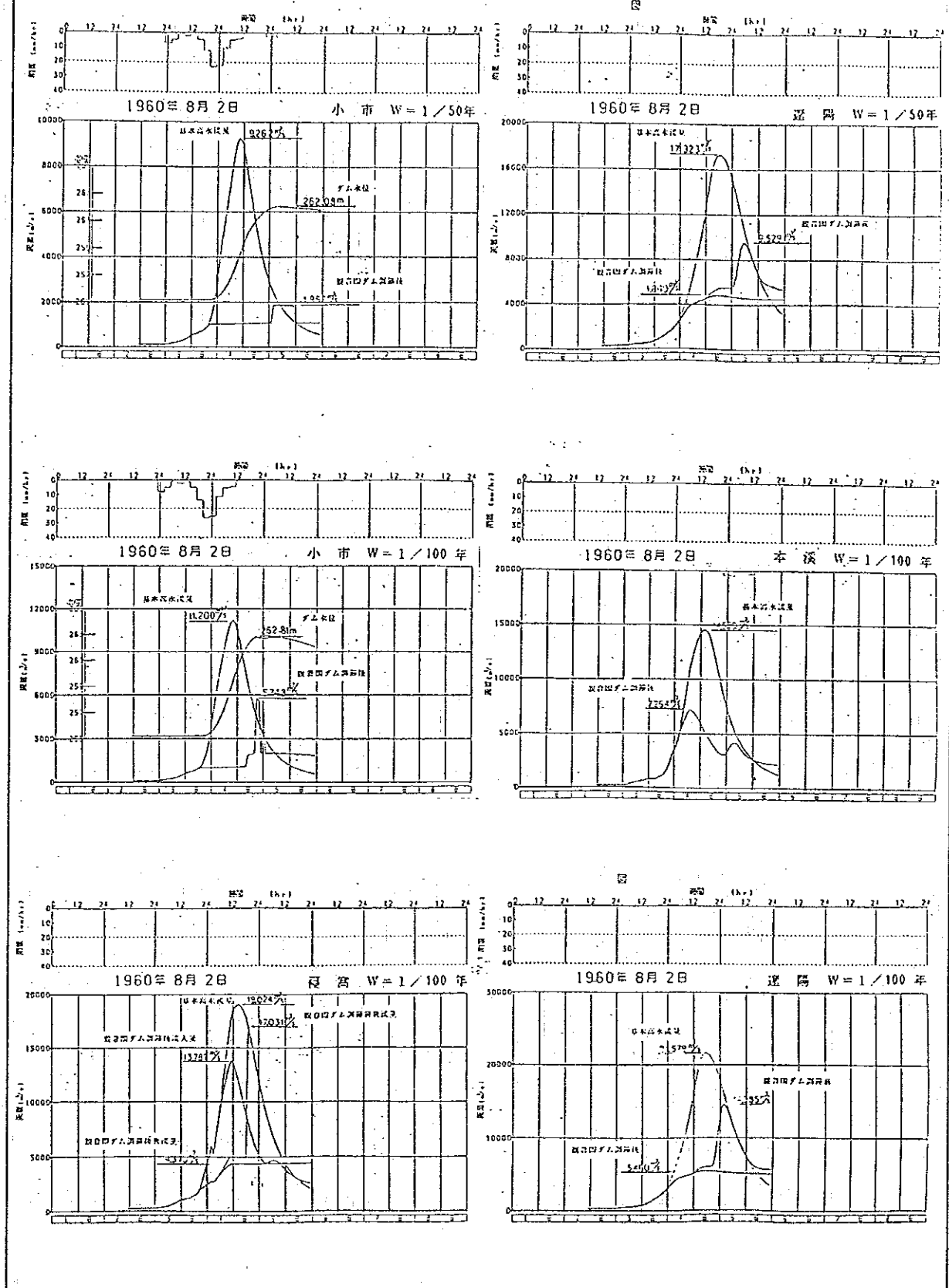
図A.5.17(2) 各確率規模別流出計算結果

1960年8月洪水



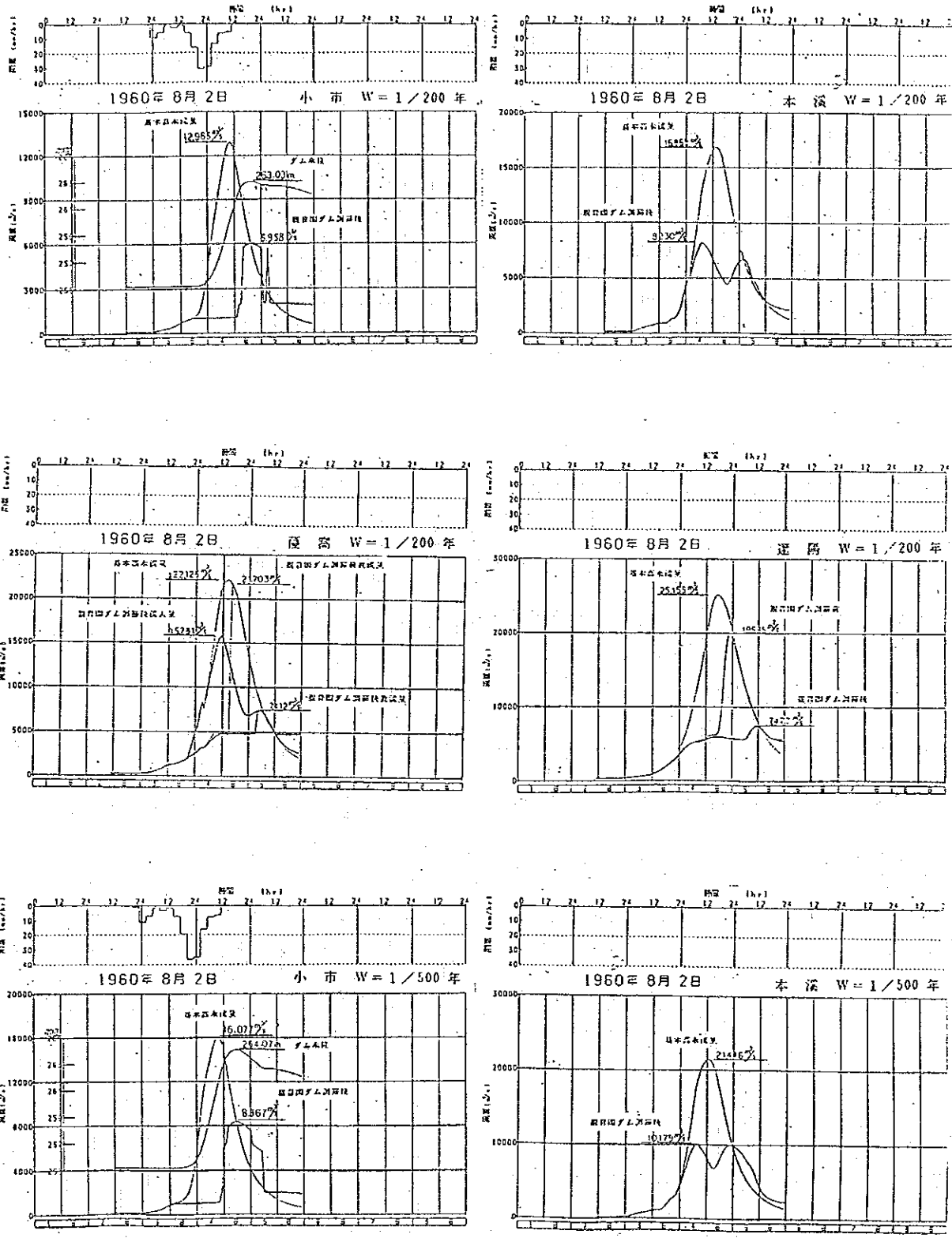
図A.5.17(3) 各確率規模別流出計算結果

1960年8月洪水



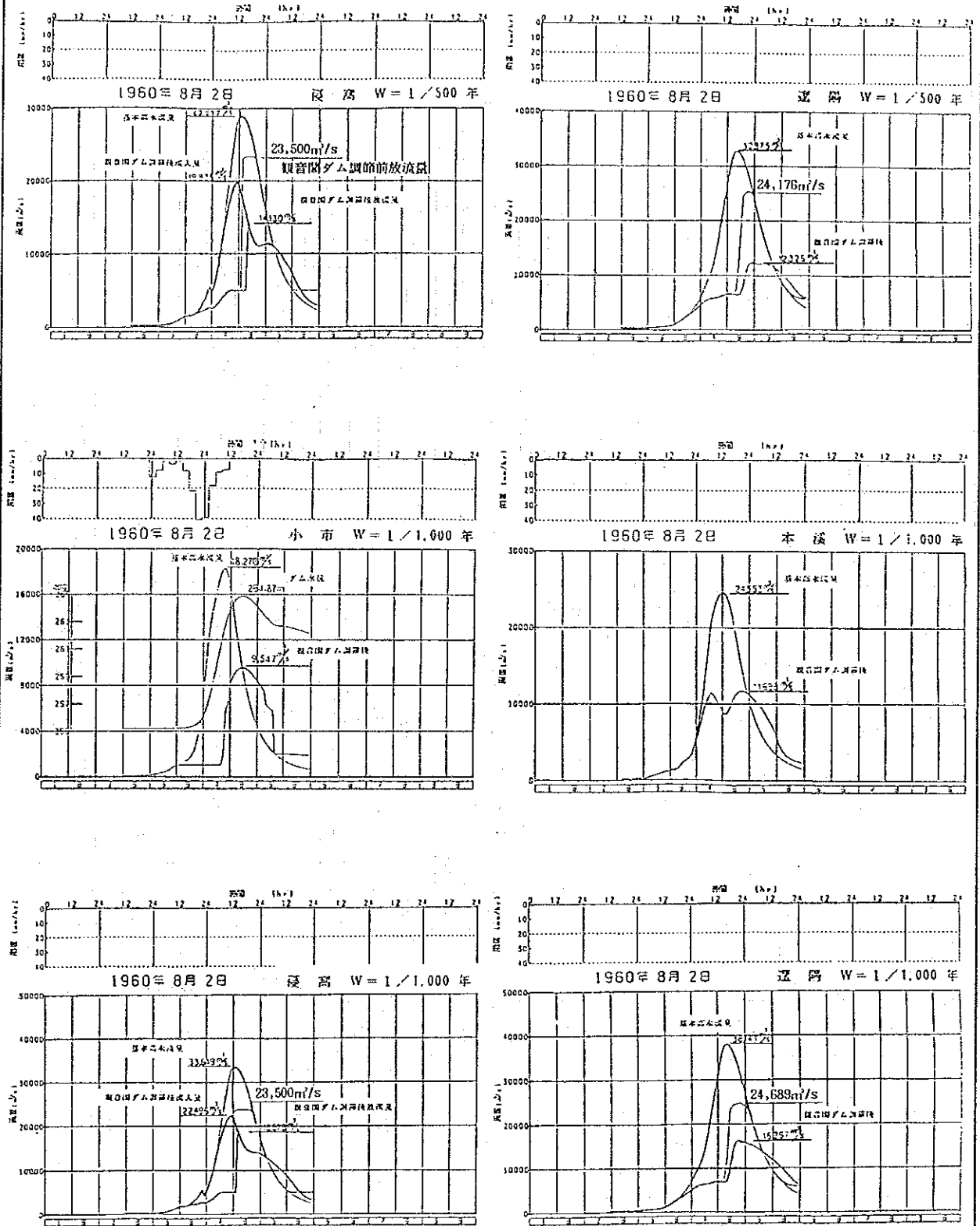
図A.5.17(4) 各確率規模別流出計算結果

1960年8月洪水



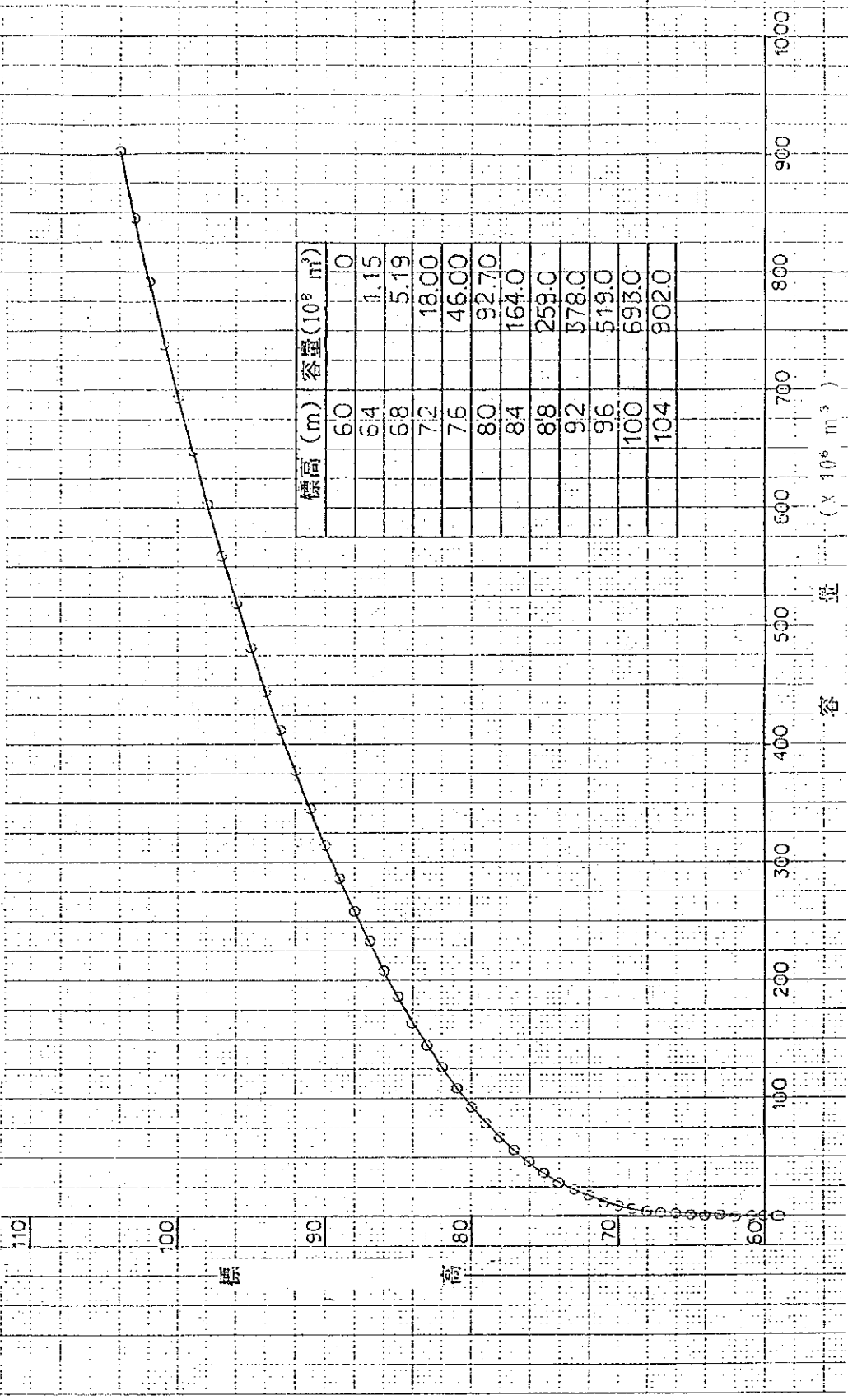
図A.5.17(5) 各確率規模別流出計算結果

1960年8月洪水

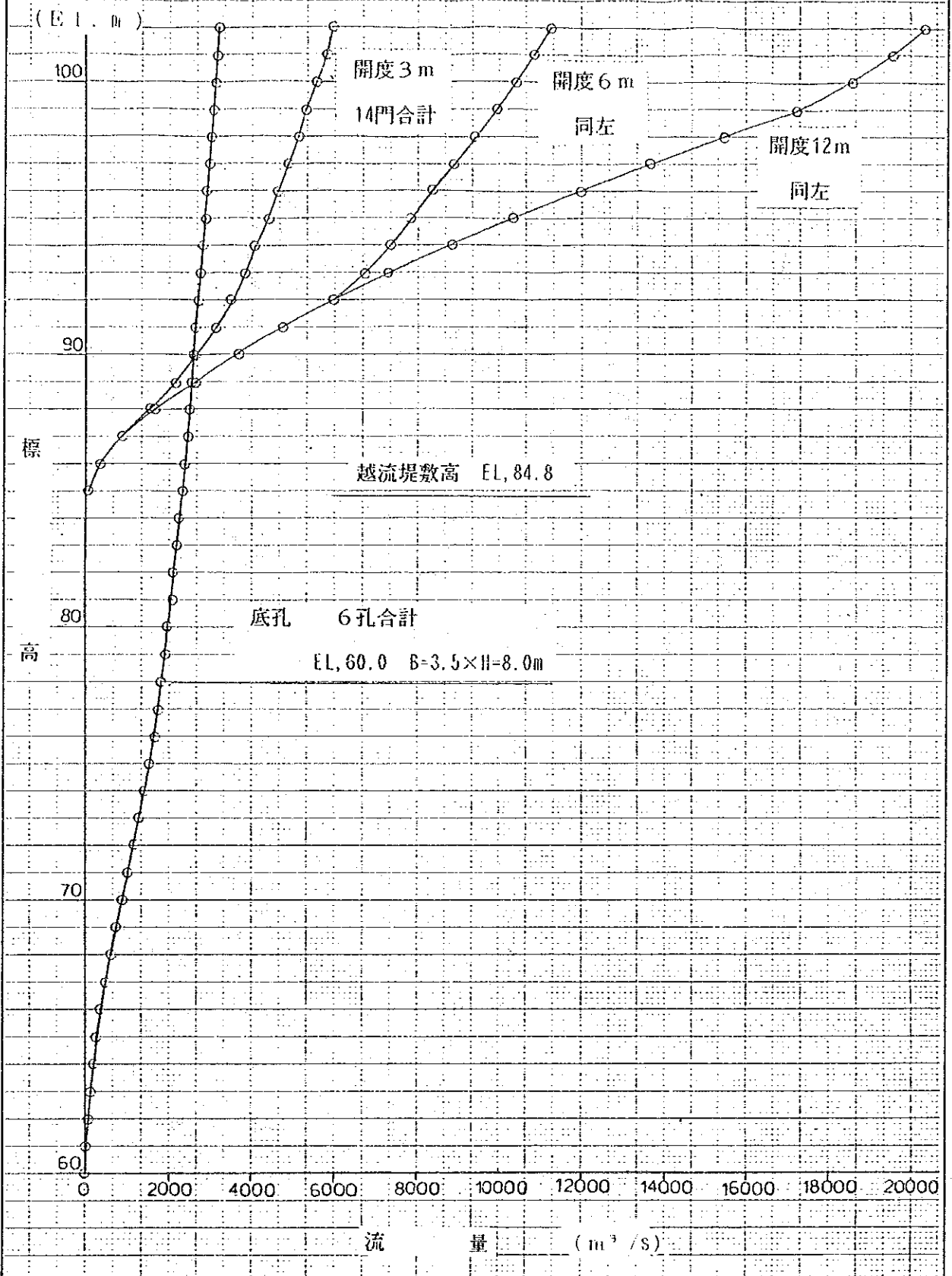


図A.5.18(1) 複窩ダム水位容量曲線

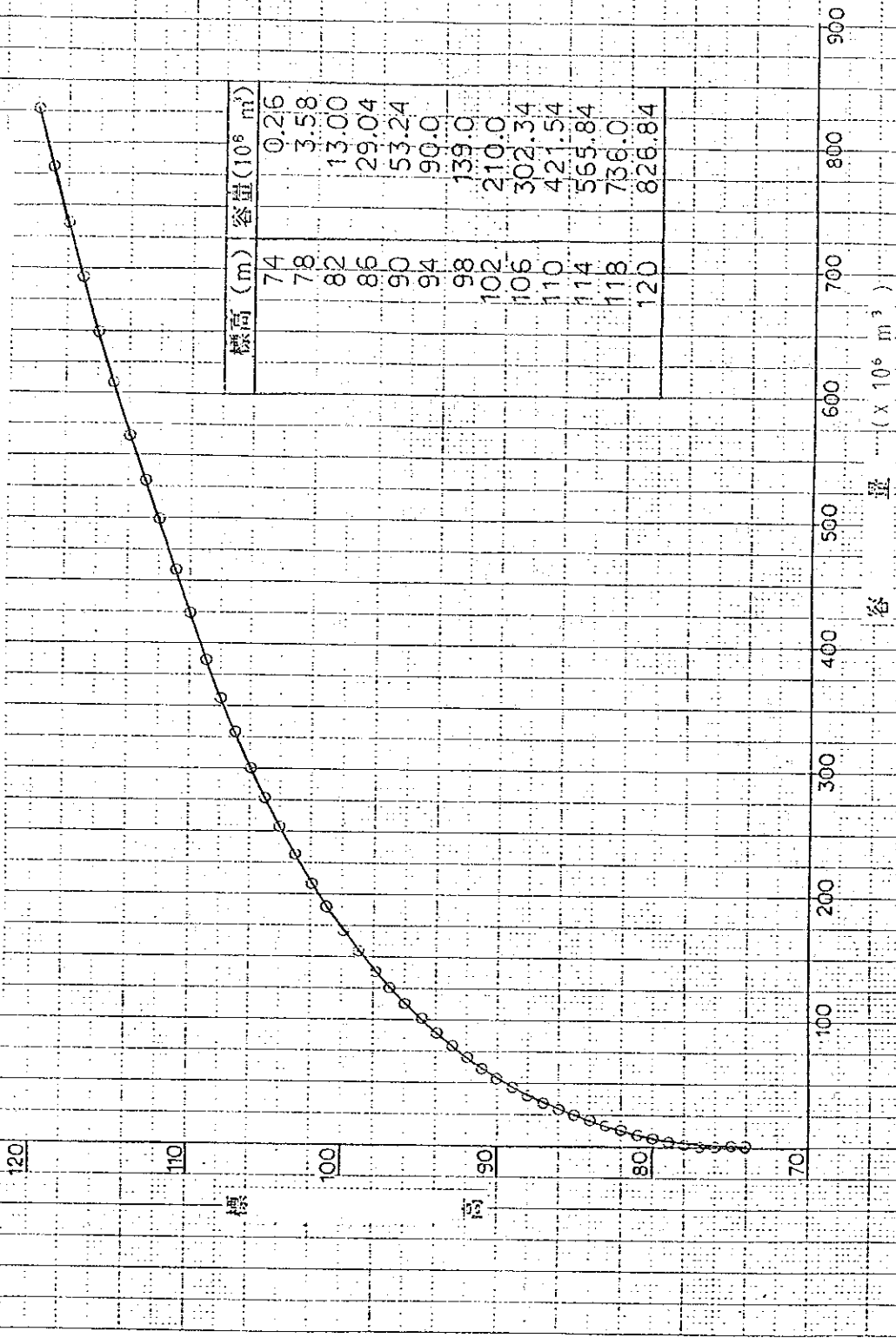
(E. 冊)



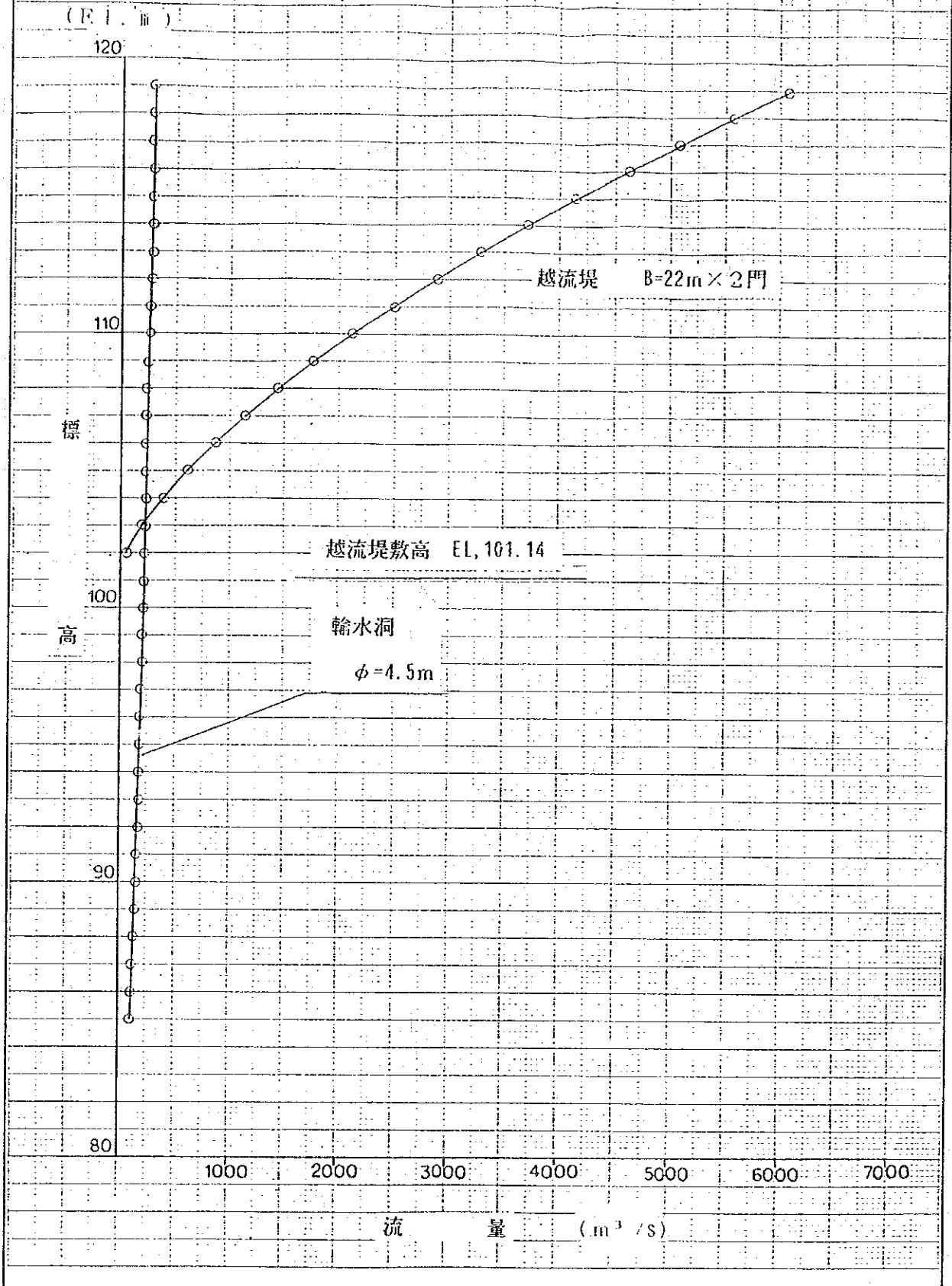
図A.5.18(2) 後高ダム水位流量曲線



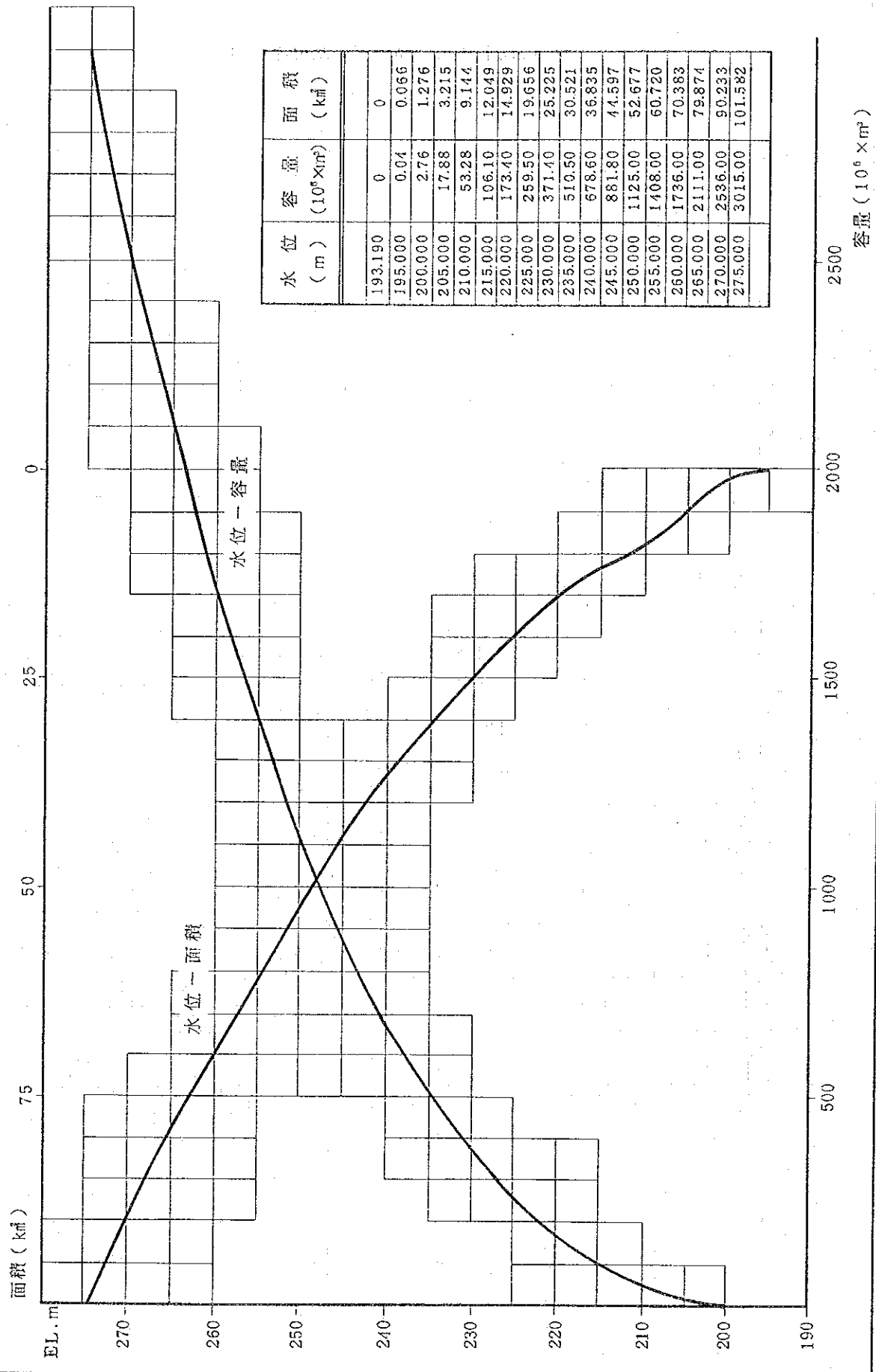
図A.5.18(3) 湯河ダム水位容量曲線



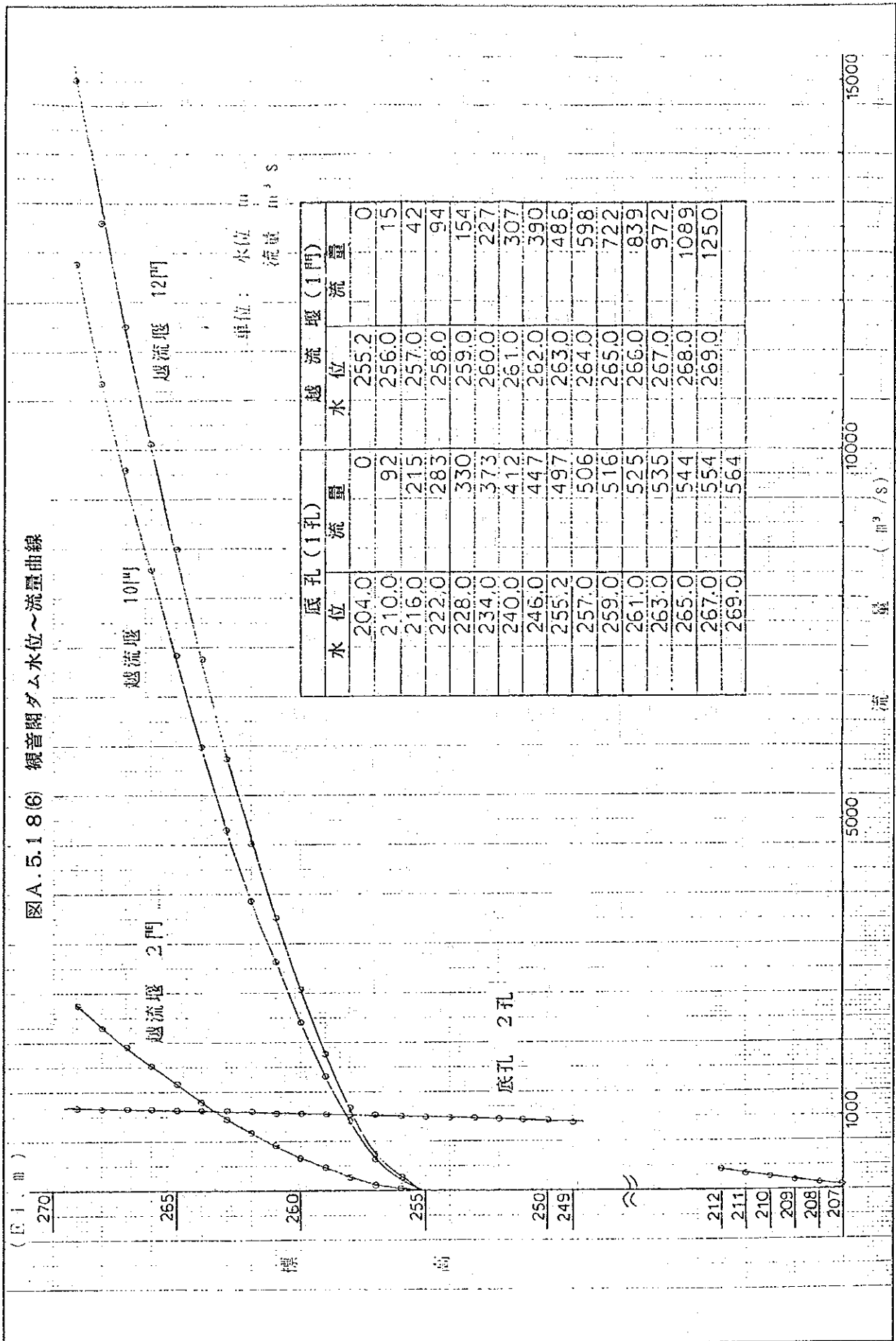
図A.5.18(4) 湯河ダム水位流量曲線



図A.5.18(5) 観音閣ダムH-A, H-V曲線



図A.5.18(6) 観音閣ダム水位～流量曲線



表A.5.14 既往主要5洪水雨量流出量一覽表

洪水名	観測所名	流域面積 (km^2)	3日雨量 (mm)	流出容量 (10^6m^3)	流出率	流出高 (mm)
1960年 8月4日	小市	2.795	309.20	6.615	0.765	236.5
	本溪	4.324	310.36	9.405	0.701	217.6
	橋頭	1.023	262.78	2.340	0.870	228.6
	侵窩	6.175	298.41	15.570	0.845	252.2
	湯河	1.228	231.34	2.025	0.713	164.9
	遼陽	8.082	283.94	16.335	0.712	202.2
1964年 8月8日	小市	2.795	167.63	3.720	0.794	133.1
	本溪	4.324	155.55	4.272	0.635	98.8
	橋頭	1.023	126.63	0.883	0.682	86.4
	侵窩	6.175	140.78	5.648	0.650	91.5
	湯河	1.228	86.01	1.584	0.667	57.4
	遼陽	8.082	128.36	6.368	0.614	78.8
1971年 8月1日	小市	2.795	240.57	6.096	0.907	218.2
	本溪	4.324	225.37	8.208	0.842	189.8
	橋頭	1.023	174.30	1.152	0.646	112.6
	侵窩	6.175	207.79	10.104	0.787	163.5
	遼陽	8.082	197.58	10.377	0.650	128.4
1975年 8月1日	小市	2.795	211.37	3.984	0.674	142.5
	本溪	4.324	214.08	6.064	0.655	140.2
	橋頭	1.023	191.83	1.072	0.546	104.7
	侵窩	6.175	206.97	3.120	0.244	50.5
	遼陽	8.082	214.16	4.848	0.280	59.9
1977年 8月4日	小市	2.795	102.20	1.848	0.647	66.1
	本溪	4.324	94.16	2.076	0.510	48.0
	橋頭	1.023	78.42	0.588	0.733	46.1
	侵窩	6.175	89.21	2.100	0.381	34.0
	遼陽	8.082	86.04	1.452	0.209	18.0

表 A.5.15 想定洪水被害額算定結果

(1) 遼陽市下流農業被害額

確率規模	調節前		非越流部天端高					
			E l. 2 6 3.8 m		E l. 2 6 7.0 m		E l. 2 7 0.3 m	
	氾濫容量 (億m ³)	被害額 (百萬元)	氾濫容量 (億m ³)	被害額 (百萬元)	氾濫容量 (億m ³)	被害額 (百萬元)	氾濫容量 (億m ³)	被害額 (百萬元)
1/30	0.00	0	0.00	0	0.00	0	0.00	0
1/50	1.80	250	0.40	80	0.00	0	0.00	0
1/100	5.37	630	2.99	410	0.25	70	0.00	0
1/200	7.26	760	4.89	590	1.58	250	1.99	300
1/500	10.59	870	6.50	700	6.03	670	4.87	590
1/1000	12.43	910	11.54	890	8.95	850	5.30	620
年平均 洪水被害額	—	13.00	—	8.21	—	3.29	—	2.69

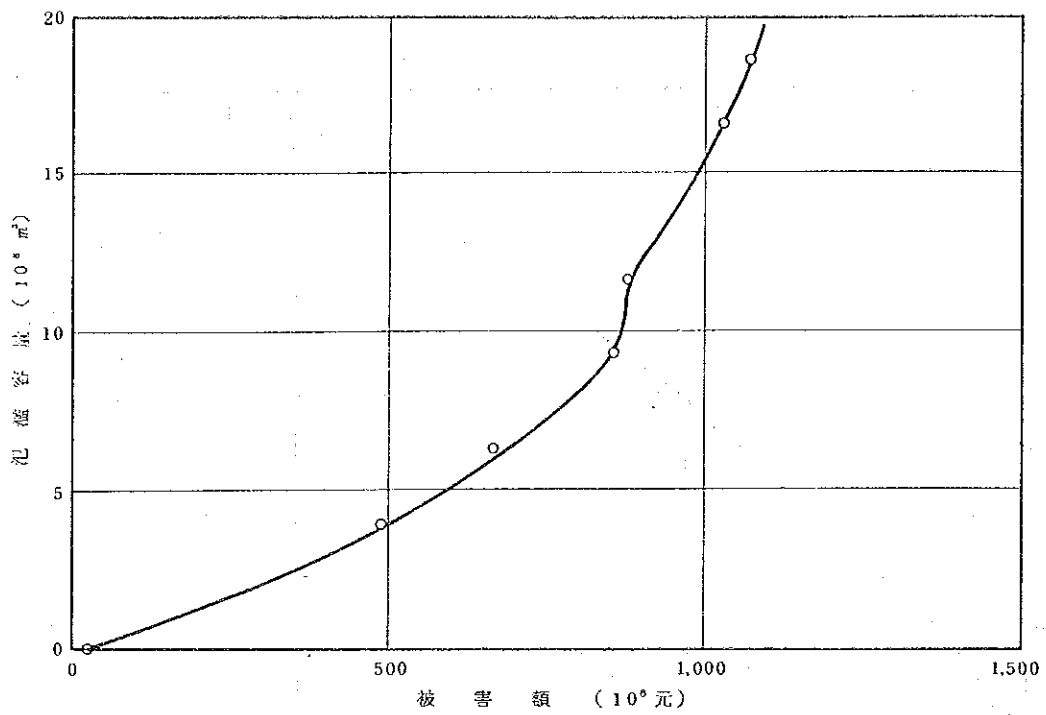
△1 外水氾濫容量

(2) 本溪市都市被害額

確率規模	調節前		非越流部天端高					
			E l. 2 6 3.8 m		E l. 2 6 7.0 m		E l. 2 7 0.3 m	
	流量 (m ³ /s)	被害額 (百萬元)	流量 (m ³ /s)	被害額 (百萬元)	流量 (m ³ /s)	被害額 (百萬元)	流量 (m ³ /s)	被害額 (百萬元)
1/30	9,998	0	5,310	0	5,310	0	5,310	0
1/50	11,889	290	6,133	0	6,133	0	6,133	0
1/100	14,559	440	7,264	0	7,264	0	7,264	0
1/200	16,869	555	8,230	0	8,230	0	8,230	0
1/500	21,446	670	10,179	0	10,179	0	10,179	0
1/1000	24,553	770	11,698	250	11,698	250	11,465	230
年平均 洪水被害額	—	11.00	—	0.13	—	0.13	—	0.12

(3) 交通, 電力, 輸送被害額

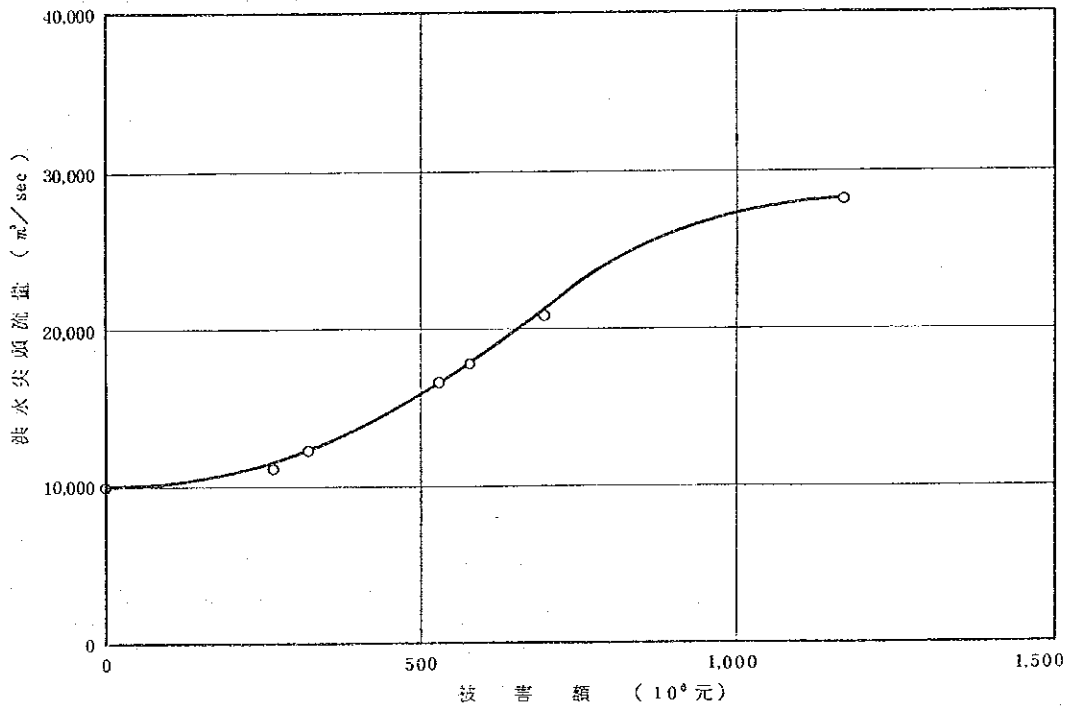
確率規模	調節前		非越流部天端高					
	流量 (m ³ /s)	被害額 (百萬元)	E 1. 2 6 3.8 m		E 1. 2 6 7.0 m		E 1. 2 7 0.3 m	
			流量 (m ³ /s)	被害額 (百萬元)	流量 (m ³ /s)	被害額 (百萬元)	流量 (m ³ /s)	被害額 (百萬元)
1/30	4,915	0	4,418	0	4,418	0	4,418	0
1/50	9,529	120	5,852	50	4,860	0	4,860	0
1/100	14,585	255	11,141	150	5,460	40	5,460	0
1/200	19,885	390	13,611	220	7,477	90	5,941	55
1/500	24,176	520	18,944	360	12,326	180	11,763	165
1/1000	24,689	550	22,935	460	16,267	285	15,651	275
年平均 洪水被害額	—	6.00	—	3.54	—	1.16	—	0.69



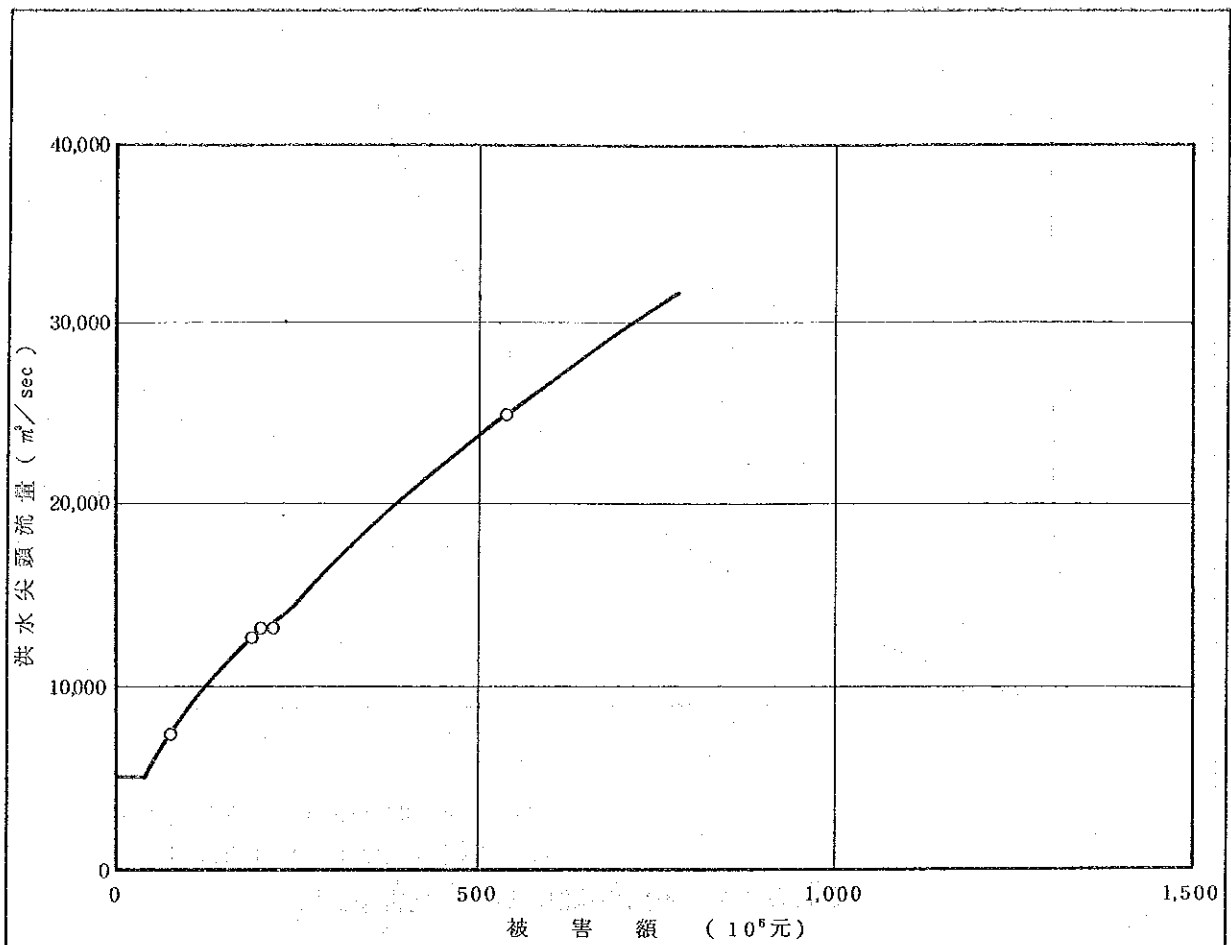
出典：觀音閣水庫可行性研究報告書

注：被害額は1988年価格

図A.5.19 遼陽下流氾濫容量～被害額曲線



図A.5.20 本溪市・洪水尖頭流量～被害額曲線



圖A.5.21 遼陽市・洪水尖頭流量~交通・電力輸送被害額曲線

表A.5.16 ダム規模別事業費

項目	El.262.0		El.267.0		El.272.0	
	内貨 財務	外貨 経済	内貨 財務	外貨 経済	内貨 財務	外貨 経済
1. 直接工事費						
1) 国内入札工事 (準備工事)	216.75	195.08	216.75	195.08	216.75	195.08
2) 国際入札工事 土木建築工事 鋼構造工事 発電電機器 管理設備	63.40 5.84 3.50 14.70	57.06 5.26 3.15 13.23	68.94 5.84 3.50 14.70	62.05 5.26 3.15 13.23	400.62 17.50 22.20 22.00	71.10 5.26 3.15 13.23
小計	304.19	273.77	309.73	278.76	772.05	491.92
2. 間接工事費	85.48	76.93	85.48	76.93	108.60	100.05
3. 補償費	152.01	136.81	168.43	151.59	168.43	203.54
4. 予備費 物理的予備費 價格予備費	17.78 57.91	16.00 0.00	18.10 60.22	16.29 0.00	46.23 67.94	16.82 0.00
5. 建設中利子	49.45	0.00	52.28	0.00	52.28	0.00
合計	666.82	503.51	694.24	523.57	1293.85	1149.33
逸失便益			-6.70	-7.60		-8.60

表A.5.18 各雨量観測所単相関解析結果

XX YY	1 羊胡子溝	2 葦子峪	3 小市	4 木溪	5 侵窩	6 湯河	7 遼陽	8 騰熬堡	9 海城
		1.047	1.070	1.050	1.004	1.085	1.020	1.031	1.033
		8.801	6.850	10.138	13.333	11.126	15.343	20.229	15.820
1 羊胡子溝		0.9032	0.9064	0.8678	0.8523	0.8482	0.8475	0.8089	0.8346
		293	298	294	297	299	289	272	291
	0.779		0.957	0.944	0.909	0.963	0.911	0.929	0.912
	5.533		2.821	5.234	7.605	6.805	10.321	13.822	11.457
2 葦子峪	0.9032		0.9398	0.9044	0.8955	0.8736	0.8777	0.8459	0.8558
	293		296	292	296	297	290	273	290
	0.768	0.923		0.949	0.914	0.985	0.932	0.937	0.930
	6.682	5.244		5.005	7.411	5.581	9.081	13.281	10.319
3 小市	0.9064	0.9398		0.9257	0.9160	0.9105	0.9146	0.8681	0.8873
	298	296		299	303	305	295	279	296
	0.717	0.867	0.903		0.918	0.958	0.908	0.926	0.901
	9.019	7.552	4.845		5.584	5.639	9.060	12.302	10.422
4 木溪	0.8678	0.9044	0.9257		0.9423	0.9072	0.9133	0.8802	0.8821
	294	292	299		298	299	291	275	291
	0.723	0.882	0.918	0.968		1.020	0.958	0.984	0.942
	8.178	6.267	3.592	1.935		1.612	5.546	8.378	7.867
5 侵窩	0.8523	0.8955	0.9160	0.9423		0.9405	0.9386	0.9091	0.8988
	297	296	303	298		303	296	279	295
	0.663	0.793	0.841	0.859	0.867		0.858	0.915	0.876
	9.971	9.396	5.794	6.203	5.724		8.957	9.665	9.099
6 湯河	0.8482	0.8736	0.9105	0.9072	0.9405		0.9118	0.9187	0.9067
	299	297	305	299	303		296	278	296
	0.704	0.846	0.897	0.919	0.919	0.969		0.969	0.927
	7.243	5.912	2.192	2.276	2.402	1.952		6.481	6.056
7 遼陽	0.8475	0.8777	0.9146	0.9133	0.9386	0.9118		0.9149	0.9036
	289	290	295	291	296	296		276	290
	0.635	0.770	0.805	0.837	0.840	0.922	0.863		0.912
	8.644	7.062	4.394	3.665	3.583	0.668	4.482		2.529
8 騰熬堡	0.8089	0.8459	0.8681	0.8802	0.9091	0.9187	0.9149		0.9408
	272	273	279	275	279	278	276		277
	0.675	0.803	0.847	0.863	0.857	0.939	0.881	0.970	
	8.184	7.478	4.362	4.873	5.129	2.416	6.191	4.982	
9 海城	0.8346	0.8558	0.8873	0.8821	0.8988	0.9067	0.9036	0.9408	
	291	290	296	291	295	296	290	277	

1段 A: Y = A · X + B

一次回帰式

$$y = A \cdot x + B$$

2段 B: "

3段 R: 相関係数

4段 N: 資料個数 (半旬)

相関係数 R

$$A = \frac{n \cdot \sum x \cdot y - \sum x \cdot \sum y}{n \cdot \sum x^2 - \sum x \cdot \sum y}$$

$$B = \frac{\sum y - \sum x^2 - \sum x \cdot \sum x \cdot y}{n \cdot \sum x^2 - \sum x \cdot \sum x}$$

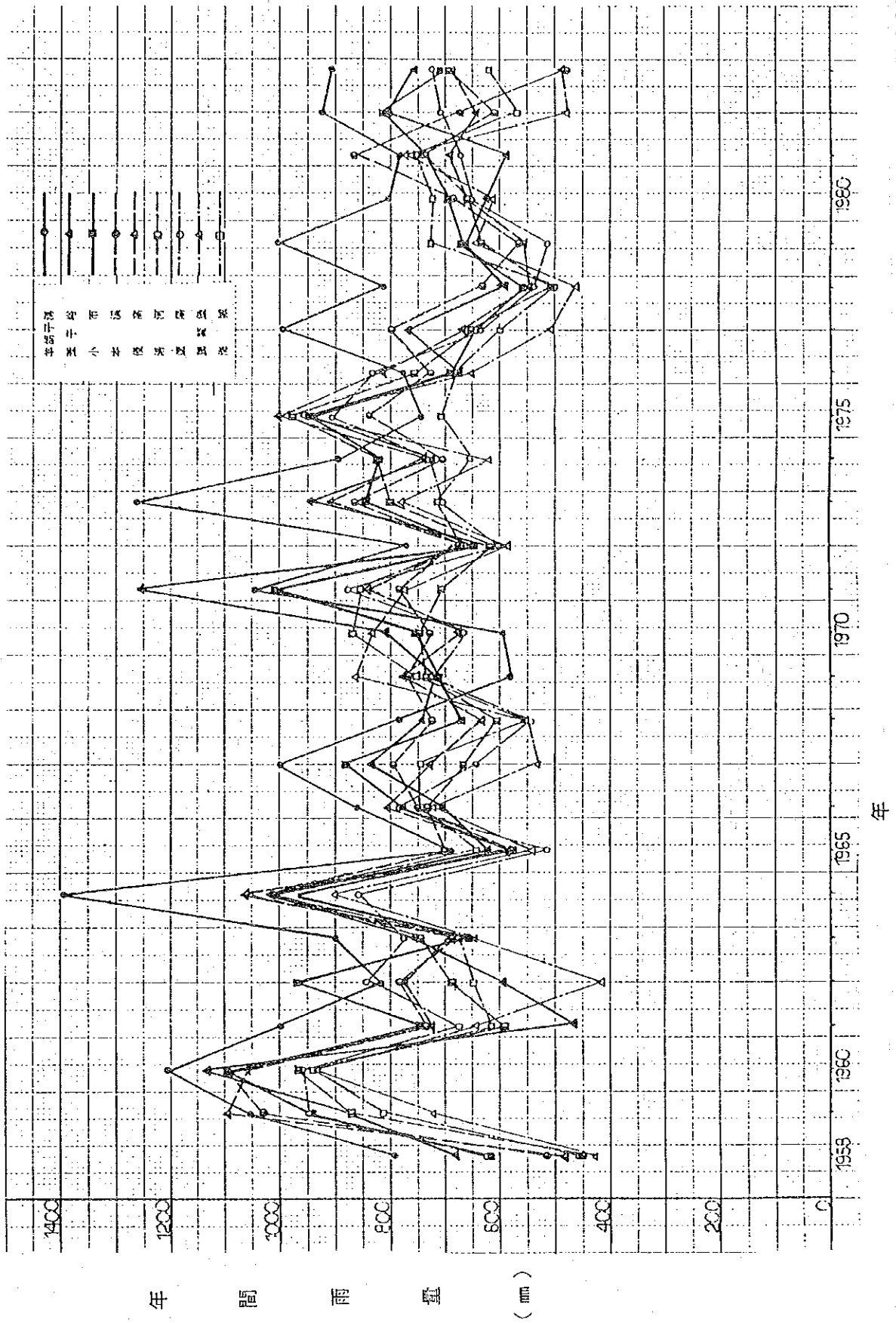
相関係数 R

$$R = \frac{\sum x \cdot y - \frac{\sum x \cdot \sum y}{n}}{\sqrt{(\sum x^2 - \frac{(\sum x)^2}{n})(\sum y^2 - \frac{(\sum y)^2}{n})}}$$

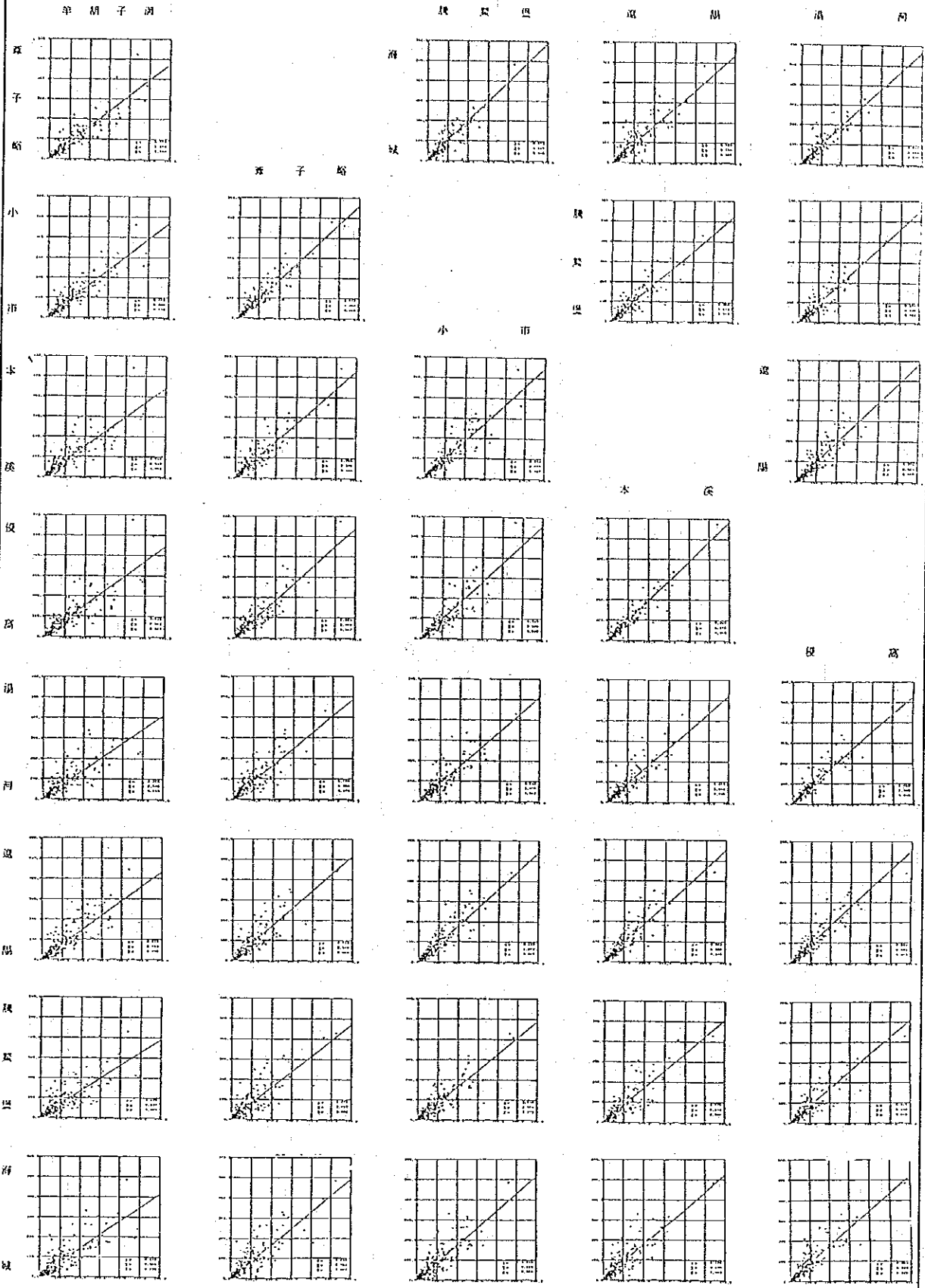
Rが 0.85以上が丸

0.9以上が二重丸

图A.5.2.2 各雨量观测所年間雨量經年变化图



圖A.5.23 各雨量觀測所單相關解析結果



表A.5.19 流量観測所流域・ティーン係数表

流域 雨量観測所	小市 2795 Km ²	本溪 4324 Km ²	橋頭 1023 Km ²	侵窩 6175 Km ²	湯河 1228 Km ²	遼陽 3082 Km ²
羊胡子溝	0.328	0.212	—	0.149	—	0.113
	917	917	—	917	—	917
葦子峪	0.418	0.270	—	0.189	—	0.145
	1168	1168	—	1168	—	1168
小市	0.254	0.426	0.061	0.309	—	0.236
	710	1843	68	1911	—	1911
本溪	—	0.092	0.531	0.181	—	0.139
	—	396	591	1119	—	1119
侵窩	—	—	0.204	0.080	—	0.093
	—	—	227	493	—	755
湯河	—	—	0.204	0.092	0.727	0.215
	—	—	227	567	893	1735
遼陽	—	—	—	—	—	0.018
	—	—	—	—	—	142
騰熬堡	—	—	—	—	0.145	0.022
	—	—	—	—	178	178
海城	—	—	—	—	0.128	0.019
	—	—	—	—	157	157

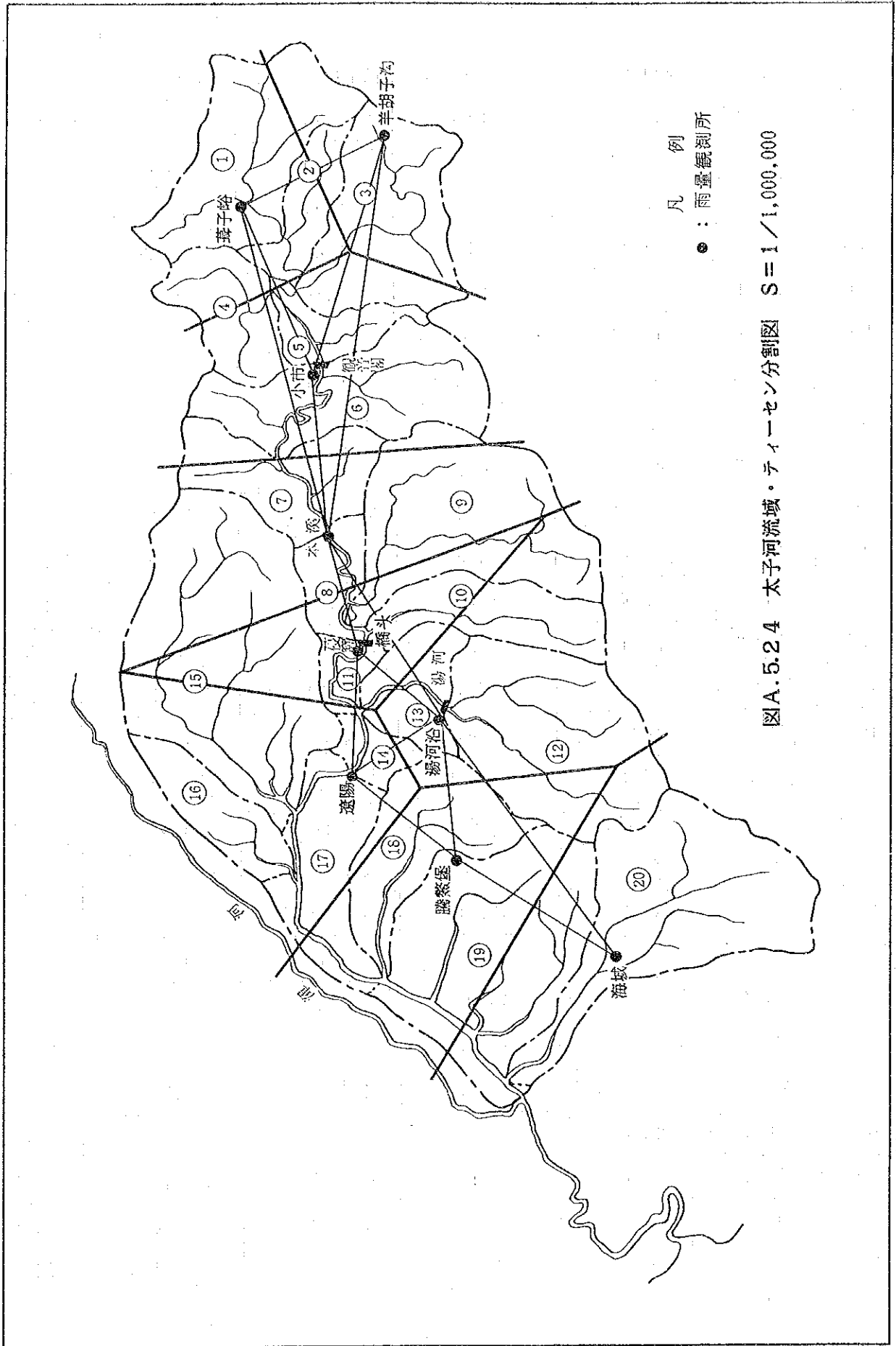
上段：ティーン係数

下段：支配面積 (km²)

表A.5.20 各流量観測所流域の年間降雨量, 流出量, 流出率

(降雨量: mm 流出量: 億m³)

年	小市流域		本溪流域		橋頭流域		俊高流域		湯河流域		遼陽流域							
	降雨量	流出率	降雨量	流出率	降雨量	流出率	降雨量	流出率	降雨量	流出率	降雨量	流出率						
1958	701.2	5.13	0.261	662.2	7.61	0.265	503.1	1.22	0.236	611.2	9.96	0.263	448.7	1.06	0.191	577.1	8.66	0.185
59	951.7	11.42	0.429	957.1	16.27	0.393	1006.0	2.62	0.254	968.6	19.50	0.326	839.7	1.64	0.159	949.3	21.55	0.280
60	1148.1	19.40	0.504	1129.4	19.65	0.402	1062.7	6.82	0.027	1105.0	41.23	0.504	961.1	3.63	0.493	1074.5	47.18	0.543
61	711.9	9.23	0.463	722.8	11.32	0.362	704.5	1.92	0.266	712.8	14.65	0.332	601.9	1.04	0.140	691.2	15.56	0.276
62	762.9	9.52	0.446	819.1	15.11	0.426	775.1	4.96	0.023	801.5	21.89	0.442	645.1	4.13	0.520	773.8	25.02	0.400
63	777.8	9.20	0.423	737.3	13.00	0.407	696.5	2.92	0.409	727.5	16.99	0.378	722.6	1.53	0.172	727.0	16.72	0.284
64	1129.4	21.65	0.585	1081.5	31.27	0.668	1043.6	7.42	0.694	1069.6	42.38	0.641	993.2	7.31	0.599	1051.6	49.04	0.576
65	633.1	5.43	0.306	613.3	7.90	0.297	605.2	1.68	0.272	615.0	11.29	0.297	670.4	1.99	0.241	623.6	11.13	0.220
66	775.8	10.82	0.498	774.6	15.32	0.457	761.3	4.48	0.575	771.0	22.87	0.480	733.7	3.87	0.429	765.3	24.60	0.397
67	901.4	13.56	0.538	886.6	19.24	0.501	762.3	4.51	0.577	843.6	26.57	0.512	659.4	2.61	0.322	802.3	25.60	0.394
68	738.2	7.31	0.354	719.1	10.58	0.340	677.3	1.89	0.273	701.6	13.89	0.320	590.8	1.49	0.205	676.5	11.62	0.212
69	672.6	8.95	0.476	692.8	14.19	0.473	762.0	3.39	0.435	713.0	19.43	0.441	758.2	0.36	0.038	723.1	16.71	0.285
70	725.0	8.44	0.416	733.7	12.30	0.387	723.4	3.65	0.493	729.8	17.47	0.387	775.6	2.19	0.230	734.6	16.54	0.278
71	1124.1	20.43	0.650	1062.7	29.25	0.636	794.3	4.76	0.585	979.4	36.05	0.596	735.2	2.65	0.293	926.3	35.92	0.479
72	661.3	6.78	0.366	659.0	10.35	0.363	666.3	2.10	0.307	659.3	11.52	0.283	620.2	3.62	0.475	651.3	11.47	0.217
73	1024.8	17.17	0.599	964.4	24.61	0.590	860.9	5.48	0.622	932.3	31.13	0.540	785.5	1.44	0.148	900.4	29.31	0.482
74	804.7	10.04	0.416	799.6	15.01	0.434	744.1	4.23	0.562	786.3	19.44	0.400	771.0	2.69	0.304	782.5	19.32	0.305
75	877.9	10.40	0.423	850.3	15.27	0.396	807.2	3.60	0.387	902.3	21.65	0.369	940.8	3.57	0.309	914.3	22.75	0.307
76	711.0	6.21	0.312	706.2	9.55	0.312	749.8	1.77	0.230	722.2	13.81	0.309	732.0	1.55	0.172	729.9	10.63	0.180
77	811.5	10.09	0.444	766.2	13.45	0.405	751.8	2.96	0.395	750.2	19.30	0.416	625.2	4.37	0.568	723.2	19.22	0.328
78	653.9	4.71	0.257	626.0	6.62	0.244	586.0	1.44	0.239	609.2	10.79	0.286	499.5	1.32	0.214	565.8	7.18	0.151
79	776.1	9.22	0.425	729.3	11.98	0.379	583.9	2.45	0.410	687.2	16.48	0.369	647.3	0.81	0.102	671.9	12.11	0.223
80	701.3	5.91	0.301	696.9	7.87	0.261	673.7	1.39	0.202	688.8	6.41	0.197	660.9	0.78	0.096	681.9	3.96	0.071
81	689.7	7.25	0.376	718.2	12.20	0.392	798.4	3.41	0.417	735.8	19.19	0.422	744.6	0.51	0.055	734.6	13.67	0.230
82	647.6	7.63	0.322	622.6	10.58	0.297	603.6	3.96	0.583	771.4	17.59	0.369	583.4	1.53	0.220	732.1	12.67	0.214
83	791.8	9.34	0.421	737.9	12.72	0.398	584.7	3.64	0.608	701.5	19.20	0.443	665.5	1.11	0.135	696.0	14.79	0.262
最小値	633.1	4.71	0.257	613.3	6.62	0.244	503.1	1.22	0.202	609.2	6.41	0.197	448.7	0.51	0.038	577.1	3.96	0.071
平均値	811.7	10.20	0.432	796.5	14.35	0.403	747.2	3.30	0.434	780.6	20.11	0.402	708.2	2.36	0.263	765.4	19.34	0.289
最大値	1146.1	21.65	0.685	1129.4	31.27	0.668	1062.7	7.42	0.628	1105.0	42.38	0.641	993.2	7.31	0.599	1074.5	49.04	0.576



凡 例
● : 雨量観測所

図A.5.24 太子河流域・ティーンセン分割図 S=1/1,000,000

表A.5.21 小市流量観測所各年流況 (1958~1983年)

単位 m^3/s

年	最大流量	豊水量(95日)	平水量(185日)	低水量(275日)	渇水量(355日)	平均流量
1958	299.0	17.90	9.93	※ 4.75	2.45	16.28
1959	383.0	40.20	18.10	5.60	2.23	36.20
1960	6,290.0	43.00	16.90	9.44	3.65	61.52
1961	663.0	27.80	16.90	8.60	3.18	29.27
1962	513.0	32.30	17.00	7.58	3.06	30.20
1963	1,250.0	17.40	11.50	5.87	3.24	29.16
1964	1,800.0	56.60	16.70	6.34	3.20	68.65
1965	785.0	12.70	7.50	※ 3.76	2.02	17.21
1966	538.0	46.10	10.60	5.81	1.46	34.30
1967	811.0	39.90	8.64	※ 3.45	1.65	43.00
1968	192.0	23.80	11.70	7.00	1.65	23.19
1969	696.0	25.60	10.90	※ 4.86	2.17	28.39
1970	678.0	16.10	9.52	※ 3.25	2.13	26.76
1971	2,300.0	50.40	27.70	7.08	2.70	64.78
1972	118.0	24.60	15.90	5.97	3.71	21.50
1973	1,280.0	50.90	27.90	6.77	2.93	54.45
1974	537.0	34.00	17.00	※ 4.66	1.98	31.83
1975	1,980.0	19.10	9.90	※ 3.63	1.93	32.98
1976	246.0	26.30	13.30	※ 4.18	1.57	19.71
1977	1,310.0	21.80	9.19	5.85	1.82	31.98
1978	156.0	19.60	10.60	※ 4.62	2.12	14.94
1979	481.0	30.70	10.90	※ 3.47	1.60	29.25
1980	254.0	22.20	8.73	※ 3.70	1.30	18.76
1981	135.0	32.50	14.80	5.29	2.83	22.98
1982	732.0	15.00	7.73	※ 2.46	1.29	24.21
1983	300.0	32.40	11.00	※ 4.03	2.20	29.61
平均値	951.04	29.96	13.48	5.31	2.31	32.35

※

……平均値よりも少ない年

表A.5.2.2 小市流量観測所年間流出総量 確率計算結果 (X10⁶ m³)

EXCESS PROBABLE DISCHARGE

STATION 2SHOSHI CHINA RYONEI-SHO
 RIVER SYSTEM 2 TAISHIGA DRAINAGE AREA 2795KM2
 KIND OF RECORD 2VOL IN YEAR PERIOD OF RECORD FROM 195& TO 1983

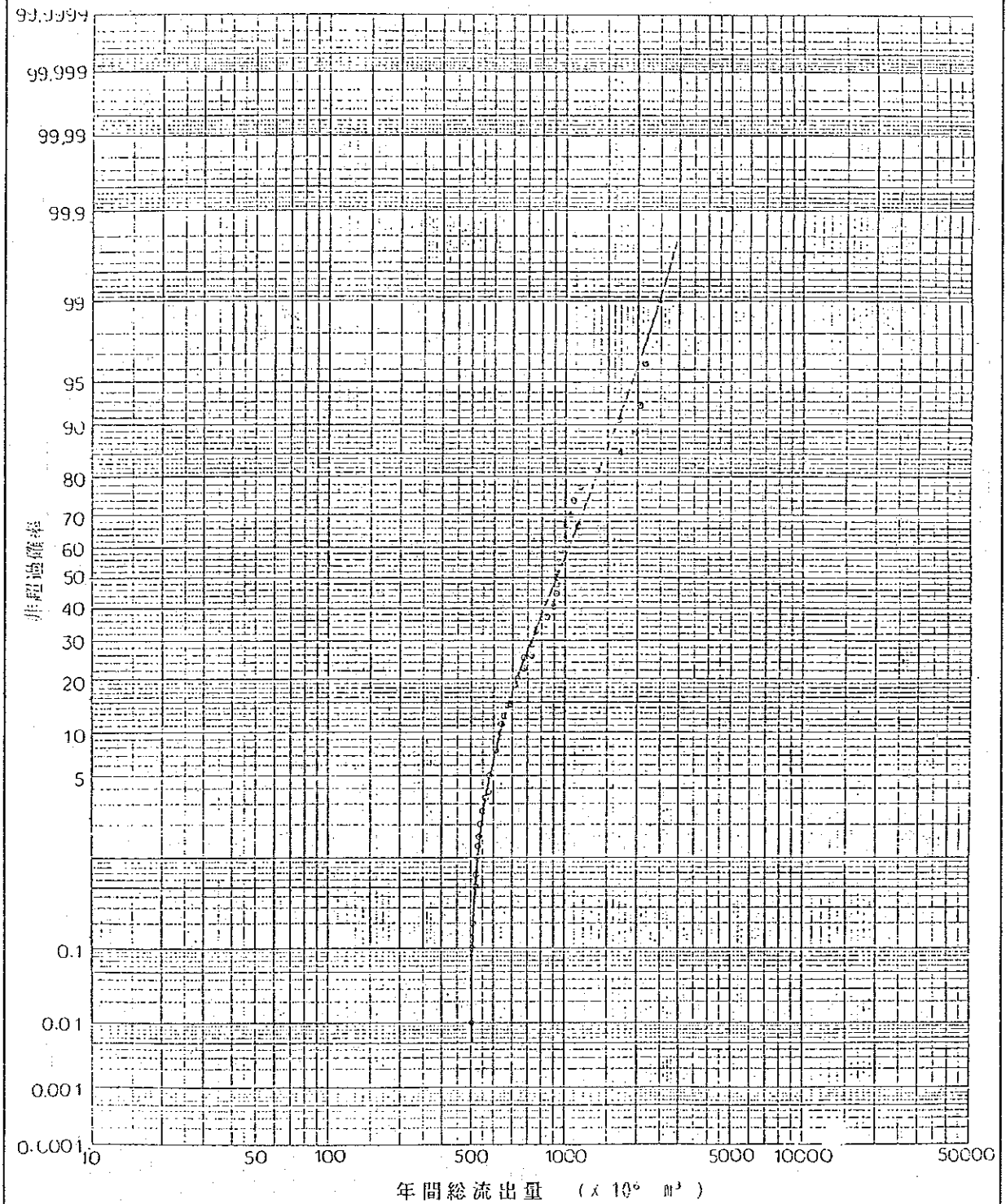
COMPUTATION METHODS

RETURN PERIOD	PROBABILITY	IWAI	HAZEN	MOMENT	ORDER-PROBABILITY	THIRD TYPE OF PEASON	HAZEN & CHOW	GUMBEL	CHOW'S METHOD	MINIMUM DISTRIBUTION
1.01	0.9901	2755.615	3049.677	2774.291	3093.162	2097.715	2500.024	2714.484	2452.213	2464.268
1.10	0.9091	1686.063	1742.953	1593.843	1809.476	1570.999	1663.669	1775.691	1649.853	1679.866
1.50	0.6667	1124.864	1109.818	1053.168	1146.336	1143.641	1180.050	1174.117	1135.704	1124.985
2.	0.5000	936.249	912.214	892.343	936.249	968.392	977.142	951.333	945.297	918.951
3.	0.3333	726.915	739.240	774.453	773.767	809.276	790.046	759.979	781.752	752.156
4.	0.2500	716.633	638.080	721.749	698.712	727.411	690.699	685.343	699.160	675.089
5.	0.2000	673.541	644.554	690.500	653.209	674.691	625.172	601.332	646.161	629.301
7.	0.1429	621.158	592.016	653.694	593.479	607.775	539.801	522.455	578.747	576.166
8.	0.1250	603.517	574.315	641.610	580.204	524.498	509.423	494.880	555.179	559.119
10.	0.1000	577.129	547.848	623.842	553.026	548.941	462.234	452.529	518.983	534.628
20.	0.0500	513.137	483.634	582.404	487.972	453.825	337.493	343.192	425.536	481.677
30.	0.0333	484.501	454.630	564.685	459.311	416.603	275.873	290.452	360.460	461.721
40.	0.0250	466.976	437.163	554.117	441.918	390.157	236.000	256.717	351.627	450.879
50.	0.0200	454.693	424.756	546.840	429.799	371.340	206.939	232.315	330.772	443.950
70.	0.0143	438.001	407.888	537.132	413.434	345.395	165.817	198.033	301.472	435.466
80.	0.0125	431.913	401.682	533.645	407.495	335.824	150.308	185.176	290.483	432.647
100.	0.0100	422.330	391.956	528.222	398.191	320.655	125.324	164.539	272.846	425.517
150.	0.0067	406.636	375.059	519.493	383.027	295.466	82.627	129.480	242.882	422.557
200.	0.0050	396.639	365.742	514.047	373.439	279.233	54.215	106.285	223.058	419.289
500.	0.0020	369.814	338.187	499.694	347.695	227.930	---	40.010	166.414	412.492
1000.	0.0010	353.455	321.269	491.618	332.681	207.420	---	---	128.865	409.667
10000.	0.0001	314.673	280.818	473.234	297.130	141.063	---	---	26.707	406.191

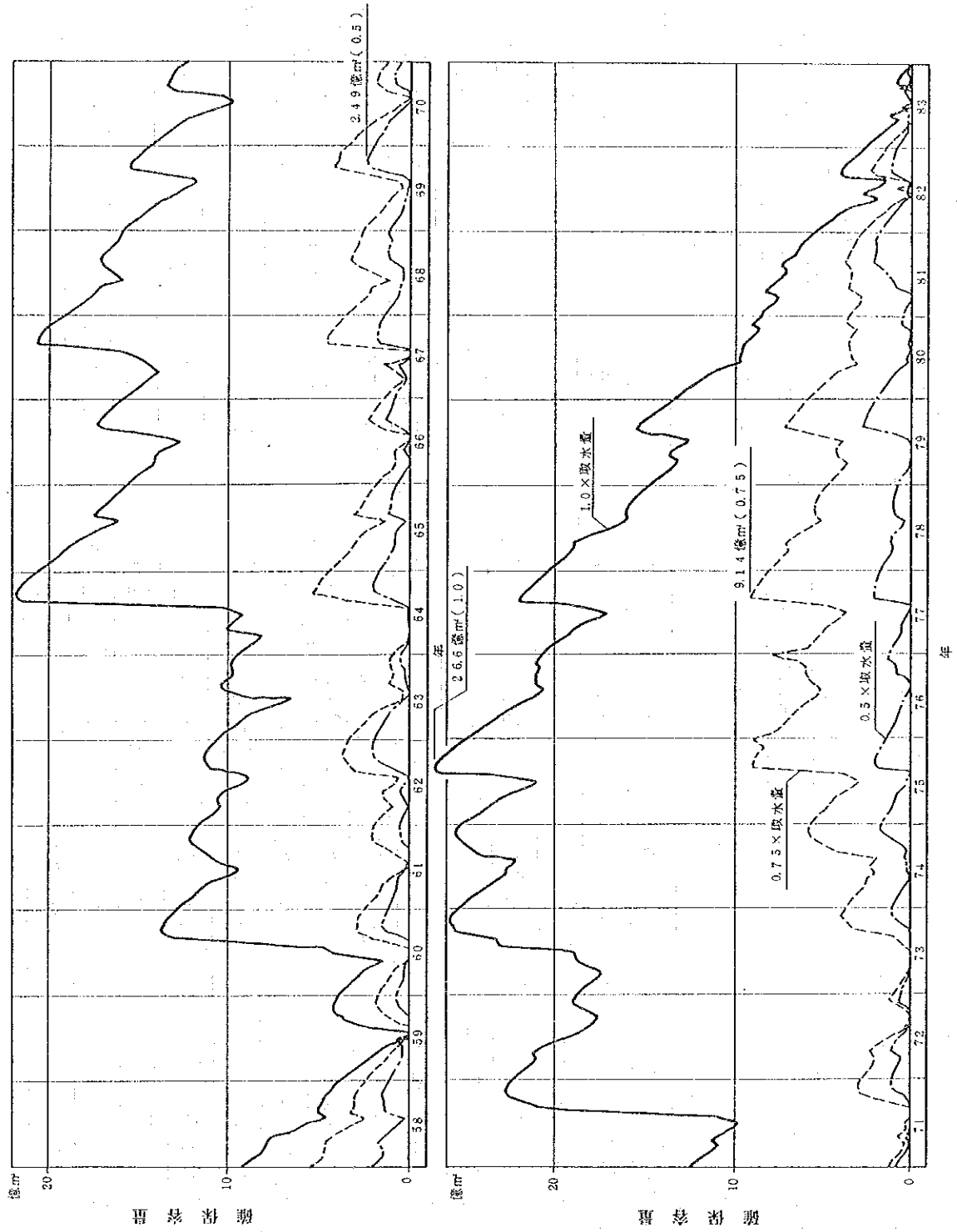
NOTE: NUMBER OF SAMPLES: 26

図A.5.25 小市流量観測所年間総流出量非超過確率

1958~1983年 最小値分布



図A.5.2.6 利水計算確保容量曲線の算定 節水率 0%



表A.5.23 中国主要電力系統，網，他の設備容量及び発生電力量，他（1984年）

	設備容量		発生電力量		最大負荷 (MW)
	全体 (MW)	水力の割合 (%)	全体 (TWh)	水力の割合 (%)	
東北電力系統 (NEPN)	11288	23.5	57.001	10.9	7941
北部中国電力系統 (NCPN)	10413	6.9	59.084	1.7	8218
東北中国電力系統 (ECPN)	11274	16.5	62.935	8.0	8427
中部中国電力系統 (CCPN)	11828	39.2	56.315	39.6	4020
北西部中国電力系統 (NWPN)	4768	46.4	23.147	47.6	3380
南西部中国電力系統 (SWPN)	4877	49.9	22.716	50.2	3210
山東地方網 (SDPG)	3952	1.2	23.450	0.1	3110
福建地方網 (FJPG)	1312	62.3	5.227	61.1	860
雲南地方網 (YNPG)	1345	59.7	6.013	49.7	888
広東地方網 (GDPG)	2123	43.3	11.653	30.4	1689
広西地方網 (GXPG)	1449	65.7	5.550	66.1	832
東黒龍江網 (EHG)	1281	10.2	7.274	5.6	-
新疆维吾尔自治区 (XJAR)	1063	31.5	3.417	23.9	-
西藏自治区 (XZAR)	110	81.1	0.227	73.6	-

注：中国電力年鑑（1984-1985）より抜粋

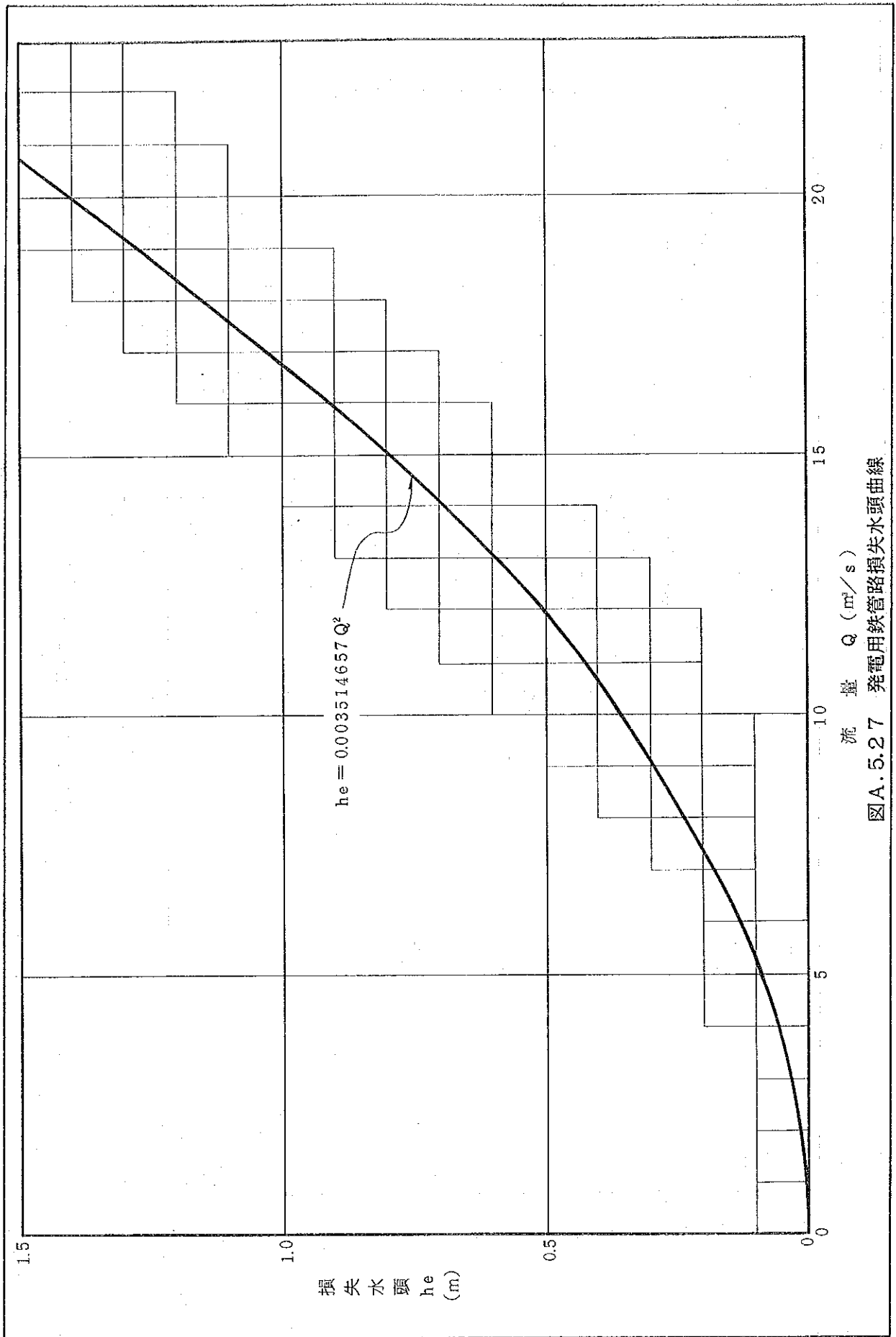


圖 A.5.27 發電用鐵管路損失水頭曲線

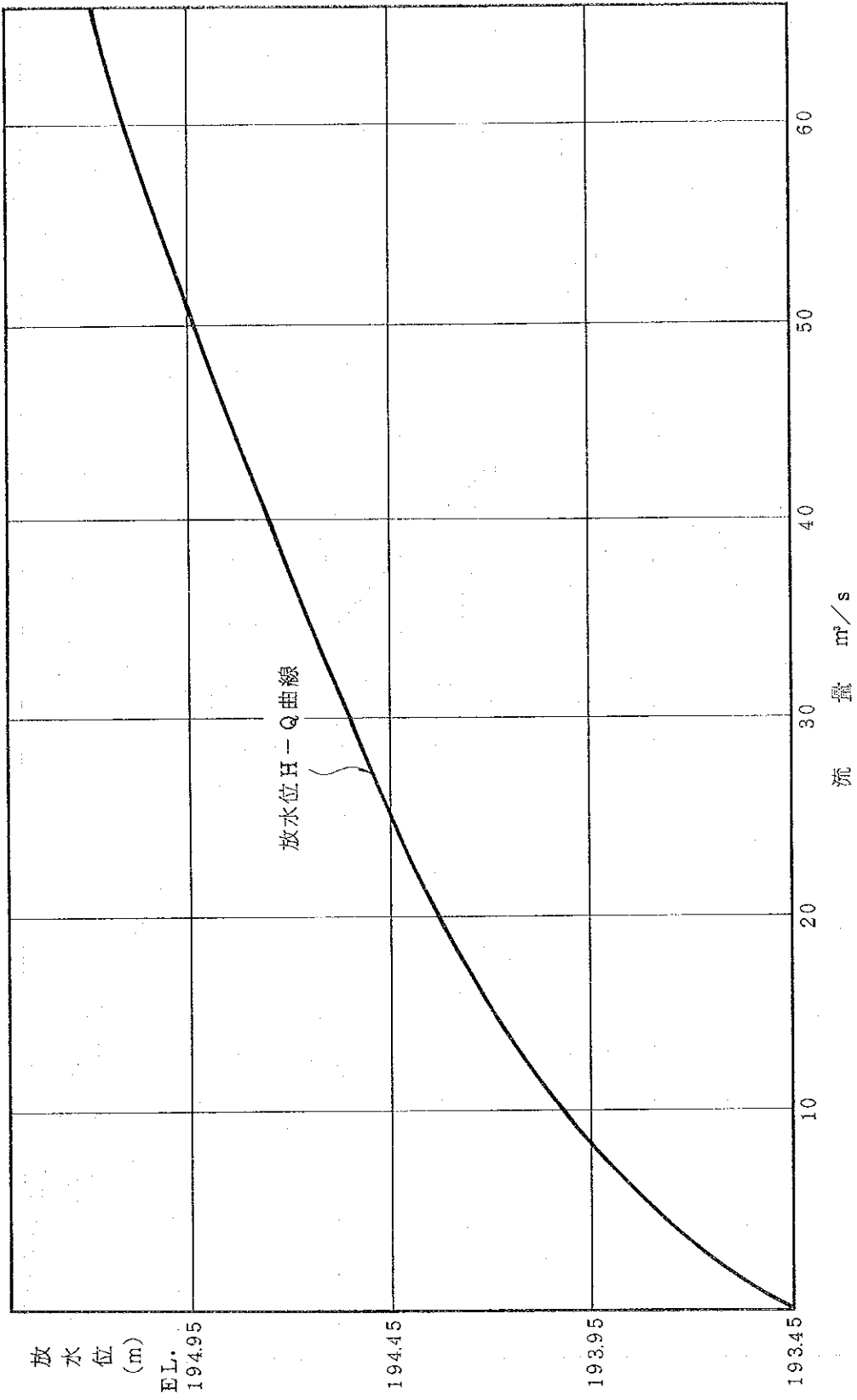
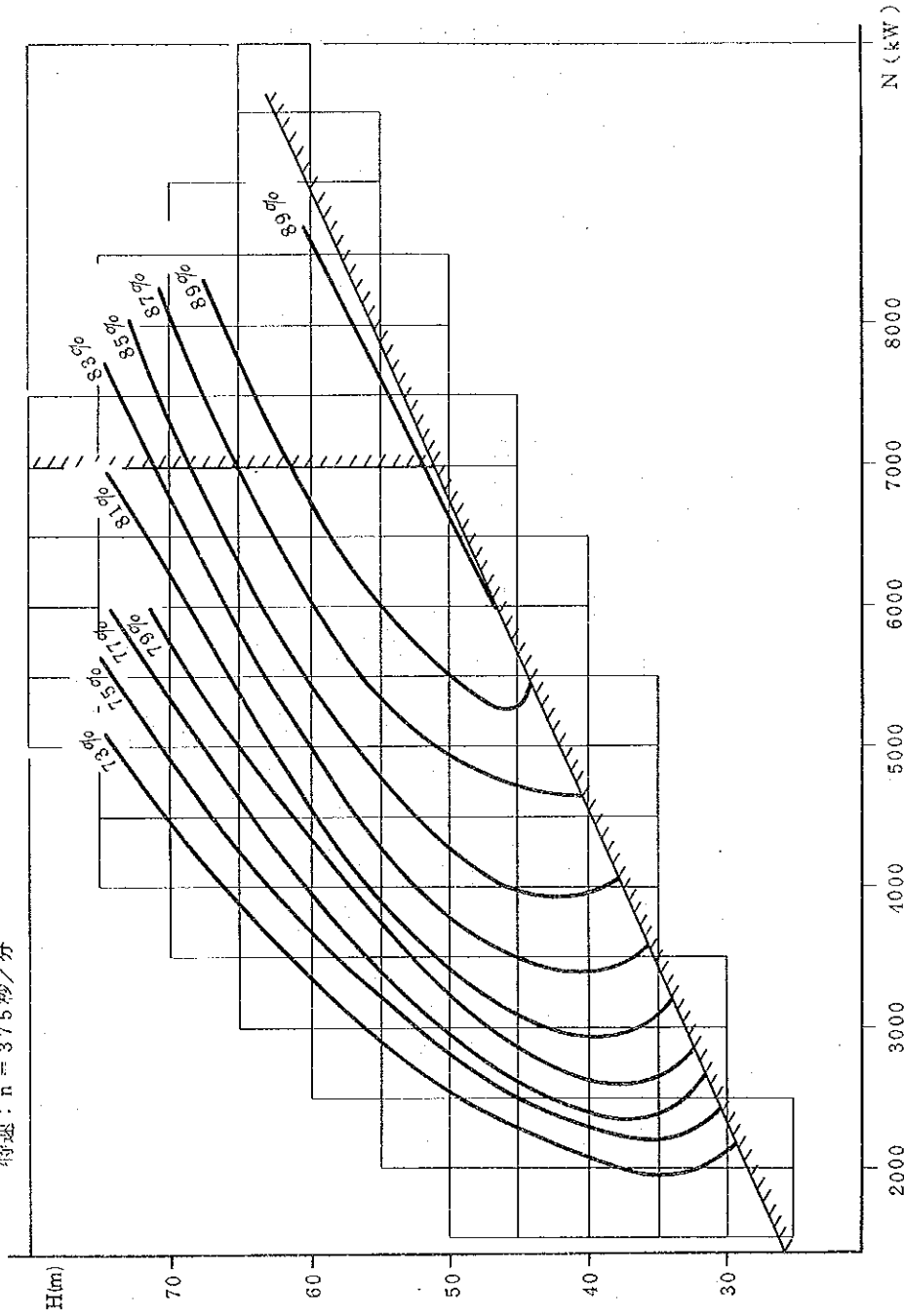


图 A.5.28 放水水位 H-Q 曲线

型号: HL220-LJ-140
 转速: $n = 375$ 秒/分



图A. 5.29 水車效率曲线

表A.5.24 発生電力量計算（フランス水車，3台）

KANAWHAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE -FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1962 - 1963

Table with columns: MONTH, N, DAYS, INFLOW, WATER LOSS, SEQU., G.POWER, R.Outlet, F.L., P, E. Contains monthly data for 1962-1963.

ANNUAL OPERATION PWR = 8769 H
AVERAGE RESERVOIR W.L. =EL. 246.25 M
AVERAGE TAIL W.L. =EL. 194.52 M
AVERAGE EFFECTIVE HEAD = 51.73 M
AVERAGE POWER EFFICIENCY = 0.83

KANAWHAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE -FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1963 - 1964

Table with columns: MONTH, N, DAYS, INFLOW, WATER LOSS, SEQU., G.POWER, R.Outlet, F.L., P, E. Contains monthly data for 1963-1964.

ANNUAL OPERATION PWR = 8784 H
AVERAGE RESERVOIR W.L. =EL. 244.45 M
AVERAGE TAIL W.L. =EL. 194.51 M
AVERAGE EFFECTIVE HEAD = 49.94 M
AVERAGE POWER EFFICIENCY = 0.84

KANAWHAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE -FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1964 - 1965

Table with columns: MONTH, N, DAYS, INFLOW, WATER LOSS, SEQU., G.POWER, R.Outlet, F.L., P, E. Contains monthly data for 1964-1965.

ANNUAL OPERATION PWR = 8769 H
AVERAGE RESERVOIR W.L. =EL. 250.12 M
AVERAGE TAIL W.L. =EL. 194.51 M
AVERAGE EFFECTIVE HEAD = 55.61 M
AVERAGE POWER EFFICIENCY = 0.92

KANAWHAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE -FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1965 - 1966

Table with columns: MONTH, N, DAYS, INFLOW, WATER LOSS, SEQU., G.POWER, R.Outlet, F.L., P, E. Contains monthly data for 1965-1966.

ANNUAL OPERATION PWR = 8769 H
AVERAGE RESERVOIR W.L. =EL. 245.15 M
AVERAGE TAIL W.L. =EL. 194.52 M
AVERAGE EFFECTIVE HEAD = 50.63 M
AVERAGE POWER EFFICIENCY = 0.94

KANONKAWA PROJECT, RESERVOIR OPERATION

KANONKAWA PROJECT, RESERVOIR OPERATION

TURBINE TYPE =FRANCIS
I.F.L.=EL.255.2 L.F.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.B.W.L.=EL.255.2 P.L.F.L.=EL.223.6
NUMBER OF TURBINE = 9
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1965 - 1967

TURBINE TYPE =FRANCIS
I.F.L.=EL.255.2 L.F.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.B.W.L.=EL.255.2 P.L.F.L.=EL.223.6
NUMBER OF TURBINE = 9
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1967 - 1969

Table with 12 columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUL. (m3/s), POWER (Mw/s), R.Outlet (m3/s), T.L. (m), P (kw), E (kwh). Rows represent monthly data from 3/10 to 2/28, plus summary rows for 365 days.

Table with 12 columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUL. (m3/s), POWER (Mw/s), R.Outlet (m3/s), T.L. (m), P (kw), E (kwh). Rows represent monthly data from 3/10 to 2/28, plus summary rows for 365 days.

ANNUAL OPERATION HOUR = 3760 H
AVERAGE RESERVOIR T.L. =EL. 240.63 M
AVERAGE TAIL F.L. =EL. 194.51 M
AVERAGE EFFECTIVE HEAD = 45.69 M
AVERAGE POWER EFFICIENCY = 0.55

ANNUAL OPERATION HOUR = 3764 H
AVERAGE RESERVOIR T.L. =EL. 244.33 M
AVERAGE TAIL F.L. =EL. 194.51 M
AVERAGE EFFECTIVE HEAD = 45.69 M
AVERAGE POWER EFFICIENCY = 0.53

KANONKAWA PROJECT, RESERVOIR OPERATION

KANONKAWA PROJECT, RESERVOIR OPERATION

TURBINE TYPE =FRANCIS
I.F.L.=EL.255.2 L.F.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.B.W.L.=EL.255.2 P.L.F.L.=EL.223.6
NUMBER OF TURBINE = 9
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1968 - 1969

TURBINE TYPE =FRANCIS
I.F.L.=EL.255.2 L.F.L.=EL.207.7
DESIGN F.L.=EL.246.7 P.B.W.L.=EL.255.2 P.L.F.L.=EL.223.6
NUMBER OF TURBINE = 9
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1969 - 1970

Table with 12 columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUL. (m3/s), POWER (Mw/s), R.Outlet (m3/s), T.L. (m), P (kw), E (kwh). Rows represent monthly data from 3/10 to 2/28, plus summary rows for 365 days.

Table with 12 columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUL. (m3/s), POWER (Mw/s), R.Outlet (m3/s), T.L. (m), P (kw), E (kwh). Rows represent monthly data from 3/10 to 2/28, plus summary rows for 365 days.

ANNUAL OPERATION HOUR = 3760 H
AVERAGE RESERVOIR T.L. =EL. 244.81 M
AVERAGE TAIL F.L. =EL. 194.52 M
AVERAGE EFFECTIVE HEAD = 45.76 M
AVERAGE POWER EFFICIENCY = 0.55

ANNUAL OPERATION HOUR = 3765 H
AVERAGE RESERVOIR T.L. =EL. 239.85 M
AVERAGE TAIL F.L. =EL. 194.19 M
AVERAGE EFFECTIVE HEAD = 38.67 M
AVERAGE POWER EFFICIENCY = 0.53

KANNOKAWA PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN W.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S

CALCULATION YEAR: 1974 - 1975

Table with columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQU. POWER (m3/s), R.Outlet (m3/s), W.L. (m), P (kw), E (kwh). Rows for each day of the year 1974-1975.

ANNUAL OPERATION HOUR = 8760 H
AVERAGE RESERVOIR W.L. = EL. 251.49 M
AVERAGE TAIL W.L. = EL. 194.52 M
AVERAGE EFFECTIVE HEAD = 36.45 M
AVERAGE POWER EFFICIENCY = 0.81

KANNOKAWA PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN W.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S

CALCULATION YEAR: 1976 - 1977

Table with columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQU. POWER (m3/s), R.Outlet (m3/s), W.L. (m), P (kw), E (kwh). Rows for each day of the year 1976-1977.

ANNUAL OPERATION HOUR = 8760 H
AVERAGE RESERVOIR W.L. = EL. 244.34 M
AVERAGE TAIL W.L. = EL. 194.52 M
AVERAGE EFFECTIVE HEAD = 49.28 M
AVERAGE POWER EFFICIENCY = 0.55

KANNOKAWA PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN W.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S

CALCULATION YEAR: 1975 - 1976

Table with columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQU. POWER (m3/s), R.Outlet (m3/s), W.L. (m), P (kw), E (kwh). Rows for each day of the year 1975-1976.

ANNUAL OPERATION HOUR = 8784 H
AVERAGE RESERVOIR W.L. = EL. 251.00 M
AVERAGE TAIL W.L. = EL. 194.51 M
AVERAGE EFFECTIVE HEAD = 35.99 M
AVERAGE POWER EFFICIENCY = 0.81

KANNOKAWA PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.W.L.=EL.255.2 L.W.L.=EL.207.7
DESIGN W.L.=EL.246.7 P.H.W.L.=EL.255.2 P.L.W.L.=EL.223.6
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S

CALCULATION YEAR: 1977 - 1978

Table with columns: MONTH, K, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQU. POWER (m3/s), R.Outlet (m3/s), W.L. (m), P (kw), E (kwh). Rows for each day of the year 1977-1978.

ANNUAL OPERATION HOUR = 8760 H
AVERAGE RESERVOIR W.L. = EL. 239.74 M
AVERAGE TAIL W.L. = EL. 194.51 M
AVERAGE EFFECTIVE HEAD = 44.79 M
AVERAGE POWER EFFICIENCY = 0.55

KANONAWAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.P.L.-EL.255.2 L.P.L.-EL.207.7
DESIGN F.L.-EL.246.7 P.M.F.L.-EL.252.5
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1978 - 1979

Table with columns: MONTH, N, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUI. (m3/s), SPWTR (m3/s), R.Outlet (m3/s), F.L. (m), P (kw), E (kwh). Contains 365 rows of daily data.

365 423.59 57.42 587.04 597.39 341.83 45.75
x m11. m3 x m11. m3 x m11. m3 x m11. m3 x m11. m3 0.78

ANNUAL OPERATION HOUR = 5325 H
AVERAGE RESERVOIR F.L. = EL. 224.77 M
AVERAGE TAIL F.L. = EL. 194.56 M
AVERAGE EFFECTIVE HEAD = 35.27 M
AVERAGE POWER EFFICIENCY = 0.83

KANONAWAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.P.L.-EL.255.2 L.P.L.-EL.207.7
DESIGN F.L.-EL.246.7 P.M.F.L.-EL.252.5
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1979 - 1980

Table with columns: MONTH, N, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUI. (m3/s), SPWTR (m3/s), R.Outlet (m3/s), F.L. (m), P (kw), E (kwh). Contains 365 rows of daily data.

365 374.12 57.42 567.05 577.21 211.21 580.76 14.02
x m11. m3 x m11. m3 x m11. m3 x m11. m3 x m11. m3 0.78

ANNUAL OPERATION HOUR = 1983 H
AVERAGE RESERVOIR F.L. = EL. 215.77 M
AVERAGE TAIL F.L. = EL. 194.54 M
AVERAGE EFFECTIVE HEAD = 39.93 M
AVERAGE POWER EFFICIENCY = 0.75

KANONAWAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.P.L.-EL.255.2 L.P.L.-EL.207.7
DESIGN F.L.-EL.246.7 P.M.F.L.-EL.252.5
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1989 - 1991

Table with columns: MONTH, N, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUI. (m3/s), SPWTR (m3/s), R.Outlet (m3/s), F.L. (m), P (kw), E (kwh). Contains 365 rows of daily data.

365 558.31 57.42 587.04 0.00 433.65 0.00
x m11. m3 x m11. m3 x m11. m3 x m11. m3 x m11. m3 0.78

ANNUAL OPERATION HOUR = 0 H
AVERAGE RESERVOIR F.L. = EL. 205.38 M
AVERAGE TAIL F.L. = NO CALCULATION
AVERAGE EFFECTIVE HEAD = NO CALCULATION
AVERAGE POWER EFFICIENCY = NO CALCULATION

KANONAWAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE FRANCIS
H.P.L.-EL.255.2 L.P.L.-EL.207.7
DESIGN F.L.-EL.246.7 P.M.F.L.-EL.252.5
NUMBER OF TURBINE = 3
DESIGN HEAD = 51.00 M
INSTALLED CAPACITY= 19500.0 KW
QMAX = 46.20 M3/S
CALCULATION YEAR= 1991 - 1992

Table with columns: MONTH, N, DAYS, INFLOW (m3/s), WATER LOSS (m3/s), REQUI. (m3/s), SPWTR (m3/s), R.Outlet (m3/s), F.L. (m), P (kw), E (kwh). Contains 365 rows of daily data.

365 865.48 57.42 587.04 0.00 599.29 0.00
x m11. m3 x m11. m3 x m11. m3 x m11. m3 x m11. m3 0.78

ANNUAL OPERATION HOUR = 0 H
AVERAGE RESERVOIR F.L. = EL. 209.04 M
AVERAGE TAIL F.L. = NO CALCULATION
AVERAGE EFFECTIVE HEAD = NO CALCULATION
AVERAGE POWER EFFICIENCY = NO CALCULATION

KANAWANAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE = FRANCIS
 H.F.L. = EL. 255.2 L.F.L. = EL. 207.7
 DESIGN F.L. = EL. 246.7 P.R.F.L. = EL. 255.2 P.L.F.L. = EL. 223.6
 NUMBER OF TURBINE = 3
 DESIGN HEAD = 51.00 M
 INSTALLED CAPACITY = 19500.0 KW
 QMAX = 46.20 M³/S

CALCULATION YEAR: 1982 - 1983

MONTH	K	DAYS	INFLOW (m ³ /s)	WATER LOSS (m ³ /s)	RESOL. (m ³ /s)	GROWER (m ³ /s)	R.Outlet (m ³ /s)	H.L. (m)	P (t/s)	E (kwh)	
1	1	10	1.39	1.79	21.43	0.00	0.00	207.70	0	0	
3	2	10	2.13	1.79	21.43	0.00	0.24	207.70	0	0	
3	3	11	2.30	1.79	21.43	0.00	0.51	207.70	0	0	
4	1	10	2.06	1.85	21.99	0.09	0.21	207.70	0	0	
4	2	10	1.59	1.85	28.49	0.00	0.00	207.70	0	0	
4	3	10	1.97	1.85	30.89	0.00	0.00	207.70	0	0	
5	1	10	1.08	1.79	44.69	0.00	0.00	207.70	0	0	
5	2	10	1.58	1.79	87.39	0.00	0.00	207.70	0	0	
5	3	11	77.21	1.79	78.41	0.00	75.42	207.70	0	0	
5	1	10	39.26	1.85	39.73	0.00	39.79	219.31	0	0	
5	2	10	21.24	1.85	43.79	0.00	43.79	211.91	0	0	
6	3	10	10.30	1.85	41.99	0.00	41.99	209.76	0	0	
7	1	10	7.97	1.85	43.09	0.00	3.85	207.70	0	0	
7	2	10	9.71	1.85	36.99	0.00	7.66	207.70	0	0	
7	3	11	17.64	1.85	48.41	0.00	15.96	207.70	0	0	
8	1	10	109.25	1.85	43.79	0.00	43.79	212.92	0	0	
8	2	10	218.39	1.85	33.89	33.89	0.00	233.99	6670	1600913	
8	3	11	82.58	1.85	29.91	29.91	0.00	226.18	7154	1888753	
9	1	10	38.25	1.85	41.39	35.91	5.48	225.59	8686	2384736	
9	2	10	25.09	1.85	21.99	21.99	0.00	228.04	5331	1293916	
9	3	10	12.77	1.68	21.99	21.99	0.00	225.57	5311	1274501	
10	1	10	9.03	1.85	21.43	21.43	0.00	225.92	5089	1224491	
10	2	10	9.27	1.85	21.43	21.43	0.00	224.28	4425	1062043	
10	3	11	3.25	1.68	21.43	0.00	21.43	223.46	0	0	
11	1	10	7.66	1.85	22.07	0.00	22.07	222.64	0	0	
11	2	10	8.82	1.85	22.07	0.00	22.07	221.58	0	0	
11	3	10	9.52	1.85	22.07	0.00	22.07	221.15	0	0	
12	1	10	6.20	1.79	21.43	0.00	21.43	220.31	0	0	
12	2	10	5.53	1.79	21.43	0.00	21.43	219.32	0	0	
12	3	11	4.61	1.79	21.43	0.00	21.43	218.01	0	0	
1	1	10	2.76	1.85	21.43	0.09	21.43	216.69	0	0	
1	2	10	2.11	1.85	21.43	0.00	21.43	215.33	0	0	
1	3	11	2.10	1.85	21.43	0.09	21.43	213.59	0	0	
2	1	10	2.28	1.79	23.56	0.00	23.56	211.50	0	0	
2	2	10	1.92	1.79	23.56	0.00	23.56	209.84	0	0	
2	3	8	1.94	1.75	23.56	0.00	15.93	207.70	0	0	
265			721.40	57.15	367.04	163.77	163.49			10.43	
			x m11. a3 x m11. a3 x m11. a3 x m11. a3 x m11. a3								078

ANNUAL OPERATION HNR = 1704 H
 AVERAGE RESERVOIR F.L. = EL. 214.97 M
 AVERAGE TAIL F.L. = EL. 194.47 M
 AVERAGE EFFECTIVE HEAD = 30.33 M
 AVERAGE POWER EFFICIENCY = 0.77

KANAWANAU PROJECT, RESERVOIR OPERATION

TURBINE TYPE = FRANCIS
 H.F.L. = EL. 255.2 L.F.L. = EL. 207.7
 DESIGN F.L. = EL. 246.7 P.R.F.L. = EL. 255.2 P.L.F.L. = EL. 223.6
 NUMBER OF TURBINE = 3
 DESIGN HEAD = 51.00 M
 INSTALLED CAPACITY = 19500.0 KW
 QMAX = 46.20 M³/S

ANNUAL AVERAGE ENERGY (3) = 4.61 CFH
 ANNUAL AVERAGE ENERGY (4) = 5.53 CFH
 ANNUAL AVERAGE ENERGY (5) = 8.80 CFH
 ANNUAL AVERAGE ENERGY (6) = 7.23 CFH
 ANNUAL AVERAGE ENERGY (7) = 8.12 CFH
 ANNUAL AVERAGE ENERGY (8) = 8.97 CFH
 ANNUAL AVERAGE ENERGY (9) = 7.06 CFH
 ANNUAL AVERAGE ENERGY (10) = 5.19 CFH
 ANNUAL AVERAGE ENERGY (11) = 5.02 CFH
 ANNUAL AVERAGE ENERGY (12) = 4.32 CFH
 ANNUAL AVERAGE ENERGY (13) = 4.83 CFH
 ANNUAL AVERAGE ENERGY (2) = 4.62 CFH

ANNUAL AVERAGE ENERGY = 74.91 CFH
 PLANT FACTOR = 43.85 %
 ANNUAL AVERAGE OPERATION HOUR = 6770 H
 ANNUAL AVERAGE RESERVOIR F.L. = EL. 238.28 M
 ANNUAL AVERAGE TAIL F.L. = EL. 194.50 M
 ANNUAL AVERAGE EFFECTIVE HEAD = 45.30 M
 ANNUAL AVERAGE EFFICIENCY = 0.82