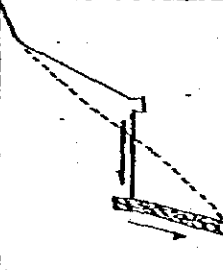
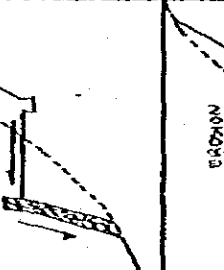
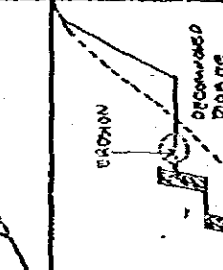
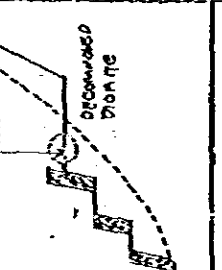
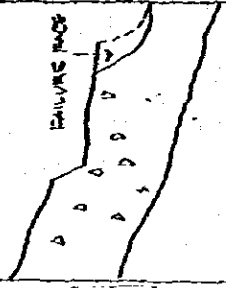
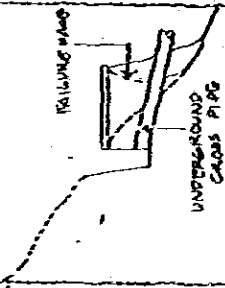
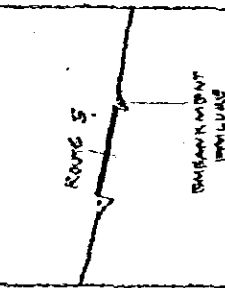


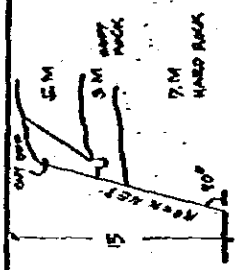
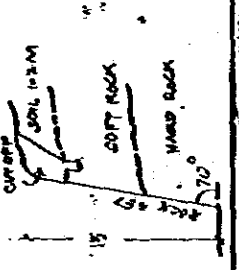
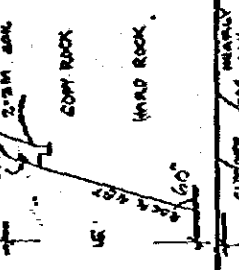
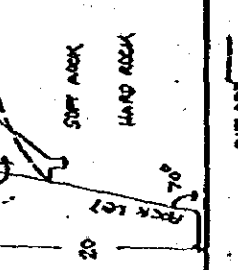
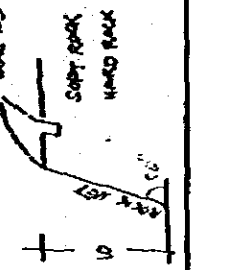
EMBANKMENT SLOPE STABILITY CHECK TABLE

NO.	KM.	LAND FORM FEATURE	CONDITION OF EMBANKMENT FEATURE	CONDITION OF PAYMENT FEATURE	PRESENT LAND FORM	EMBANKMENT TREATMENT	NOTE
65	215.300	cutting and embankment	normal	normal		Gutter repair.	
66	215.600	cutting and embankment	road shoulder is eroded by running water	gully		Gutter and shoulder repair overlay	
67	215.900	cutting and embankment	normal	cracks and settlements		Gutter repair	
68	216.800	cutting and embankment	Foot of Embankment is weasws by Sta. Fe river	cracks and settlements		revetment	

EMBANKMENT SLOPE STABILITY CHECK TABLE

NO.	KM.	LAND FORM FEATURE	CONDITION OF EMBANKMENT FEATURE	CONDITION OF PAVEMENT FEATURE	PRESENT LAND FORM	EMBANKMENT TREATMENT	NOTE
69	223-700	cutting and embankment	slope failure is observed at the toe of hill	cracks on the pavements		failure zone repair by masonry	
70	225-100	half cut and fill	Toot of Embankment is eroded by Sta. Fe river running water from pipe	normal		revetment vertical drain	
71	225-400	plain	erosion by stream	normal		Boxculvert	

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
1	167.000	-Diorite -upper half consists of strongly weathered soil -slope surface leaves open cracks -Covered soil -depth 2-3m	-weathered zone surface failure -hard rock, rock fall		-soil, soft rock zone remove 1:1 -hard rock -rock net protection -need drainage on top of the slope.	-failure surface erosion of stream -rock fall occur by exfoliation -land slide type (B)
2	167.300	-Diorite -occurs hard rock -cracks present -Covered soil -very thin about 2-3m -Gully present along the slope	-soft rock zone -surface exfoliate along open joint -failures occur due to surface stream erosion.		-drainage establish -unstable zone remove -and rock net protection -tion (soft rock and hard rock)	-failures occur due to surface erosion of water stream -rock fall -land slide type (B)
3	167.500	-Diorite -very hard -cracks present -Covered soil -weathered soil 3-5m	-rock fall exfoliate along the crack. there are many crack in the diorite bed.		-drainage establish -unstable zone remove -and rock net protection	-failure in small scale -rock fall often occurred -land slide type (B)
4	167.500	-Andesite -strongly weathered -presence of many brecciated gullies -rocks -Covered soil -very thick	-failures occur due to surface stream erosion and exfoliation from joint		-drainage establish -unstable zone remove -and rock net protection	-failures occur along joint (exfoliate) and large scale -land slide type (B)
5	167.800	-Gneiss -joint bedding abundant which exfoliated soft rock -Andesite -strongly weathered -Covered soil -thin	-failure occur on the slope consist of soil and slope of andesite that exfoliate along joints -all soft rock		-drainage establish -unstable zone remove -and rock net protection	-failure in small scale -land slide type (B)

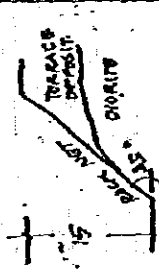

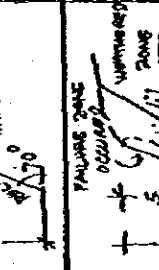
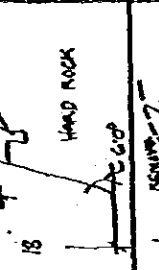
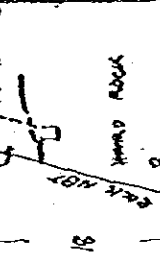
CUTTING SOIL STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slopes)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
6	167.000	<ul style="list-style-type: none"> *Andesite -Dissected -exfoliated, also strongly weathered (nearly soil) *Covered soil -very thick about 5-10m 	<ul style="list-style-type: none"> *bad rock strongly weathered because many gullies exist. failure occurs. 		<ul style="list-style-type: none"> *drainage establish *unstable zone remove *rock net protection 	<ul style="list-style-type: none"> *failure occurred mainly due to water surface stream action *land slide type (A)
7	169.200	<ul style="list-style-type: none"> *Andesite -strongly weathered (nearly soil) -agglomerate occurs *Covered soil -very thick 	<ul style="list-style-type: none"> *weathered zone exfoliation and swamp stream action 		<ul style="list-style-type: none"> *drainage establish *swamp treatment 	<ul style="list-style-type: none"> *mainly needs swamp treatment *land slide type (B)
8	170.700	<ul style="list-style-type: none"> *Sand stone shale many joint bedding exist, easy to exfoliate *Covered soil thickness about 5m 	<ul style="list-style-type: none"> *exfoliation occur from schistosity joint *failure occurs due to surface water action and to intense weathering. 		<ul style="list-style-type: none"> *unstable zone remove *hard rock zone Rock net protection 	<ul style="list-style-type: none"> *failure on the middle scale *land slide type (A)
9	171.000	<ul style="list-style-type: none"> *Schist -corners very abundant (bedding joint) -upper half is andesite *Lower Schist *Covered soil about 5-8m thick 	<ul style="list-style-type: none"> *there are many gullies on the slope surface. -failure occurs on top of the upper portion of the weathered zone. 		<ul style="list-style-type: none"> *unstable zone remove *drainage establish *rock net protection 	<ul style="list-style-type: none"> *failure occurred in small scale due to surface water action *land slide type (A)
10	171.500	<ul style="list-style-type: none"> *Schist -strongly weathered (nearly soil) *Covered soil very thick (5-10m) 	<ul style="list-style-type: none"> *failure occurs due to surface water erosion and underground water action (erosion) *failure exfoliates on the schistosity rocks 		<ul style="list-style-type: none"> *drainage establish *weathered soil remove and hard rock net protection 	<ul style="list-style-type: none"> *very gully exists *failure often occurred *land slide type (A)

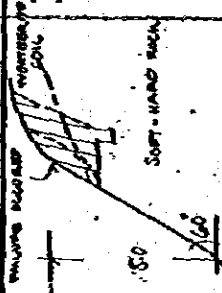
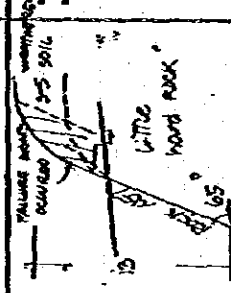
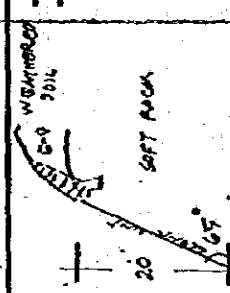
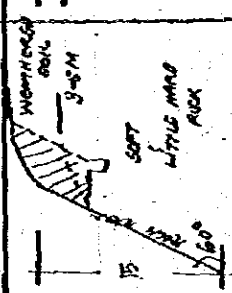
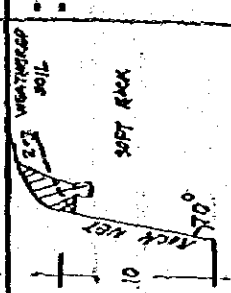
CUTTING ALONG STABILITY CHECK CIRCLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
21	173.500	"Terrace deposits (upper) and andesite (lower). "Covered soil very thin (without terrace deposits)	"failure occurs by surface oxidation "water springs and the boundary terrace deposits (covered with andesite) are the cause of failure."		"drainage establish "weathered soil (unstable zone) removed	"there are underground water on the slope "land slide type (A)
22	173.900	"Terrace deposits -debris present along the slope	"failure occurs with gully on surface slope "failure is due to under-ground water action.		"drainage establish "unstable zone remove	"there are underground water on the slope "land slide type (A)
23	174.000	"Terrace deposits there are many unstable Breccia.	"failure occurs with gullies on surface slope.		"drainage establish "unstable zone remove	"there are underground water on the slope "land slide type (A)
24	182.100	"Talus deposits sand (granitic)	"failure occurs by surface water erosion.		"drainage establish "unstable zone remove	"there are underground water on the slope "land slide type (A)
25	203.500	"Talus deposits and terrace deposits	"failure occurs by surface water erosion.		"drainage establish "unstable zone remove	"land slide type (A)

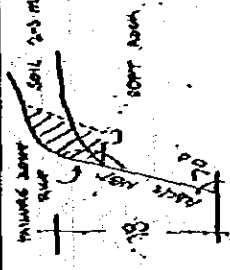
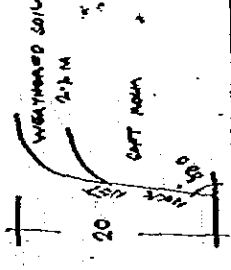
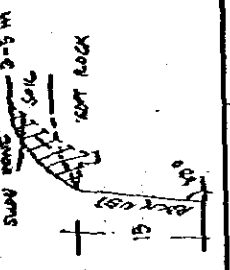
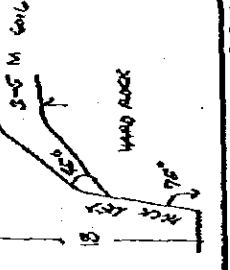
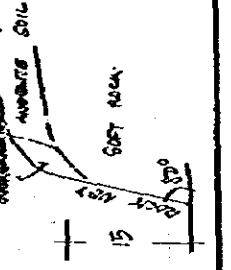
CUTTING SLOPE AT MILITARY CHECK STATION

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in coming slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
16	105.600	Diorite and terrace deposits	Failure occurs by surface water erosion.		drainage establish rock net protection	there are underground water on the slope land slide type
17	105.900	Andesite slightly weathered *Top soil few meters thick	failure occurred by exfoliation joint on the weathered block there are many gullies on the slope (strongly weathered)		drainage establish rock net protection	there are many gullies on the slope land slide type (3)
18	106.100	Andesite, diorite predominant nearly all (slightly) weathered to a depth of about 5m (nearly soil) *Top soil (weathered soil) about 5.0m	failure occurs on the top of slope (weathered soil) failure is due to underground water action, and surface water action.		drainage establish unstable zone remove	there are many gullies on the slope land slide type (1)
19	106.200	Diorite - upper slope very weathered (nearly to soil) - and lower parts of (chill) weathered *Covered soil depth about 5m	failure occurs by exfoliation.		Loose block should be cut off drainage establish	failure is exfoliated from joint because large failure does not occur land slide (C)
20	106.400	Diorite Fault present on the lower slope, because diorite is fractured. *Covered soil few meters thick	failure occurs by sliding along flowing joint and exfoliation along joint.		unstable zone remove rock net protection	failure exfoliated from joints (small scale) land slide type (C)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
21	186.800	-Diorite weathered zone in 3-5m (upper slope)	-failure occurs on weathered zone due to surface and underground water action. there are many gullies on the slope.		-unstable zone remove -drainage establish (there are many gullies on the slope)	-there is water spring on the lower slope -land slide type (B)
22	187.100	-Diorite weathered nearly to soil 3-5m thick	-failure occurs on surface (weathered soil) there are many gullies on the slope due to water surface action.		-unstable zone remove -drainage establish	-there are many gullies on the slope -land slide type (B)
23	187.200	-Andesite and schistose are weathered soil. faults present on the slope because basement rock is sheared.	-failure occurs due to surface water erosion. (gullies present along the slope).		-unstable zone remove -drainage establish (there are many gullies on the slope)	-there is fault because rock is sheared -large scale failure -land slide type (B)
24	187.300	-Diabase, diorite almost weathered to soil there is fault on the slope. -Covered soil 3-5m thick	-failure occurs on the weathered zone. -failure is due to water action on the surface of slope.		-unstable zone remove -drainage establish	-failure occurred on small scale -land slide type (C)
25	187.500	-Diorite, andesite -Generally hard cracks are abundant -Covered soil very thin	-failure occurs on surface -failure is due to water action on the surface of slope.		-unstable zone remove -drainage establish	-small scale failure -there are gullies -land slide type (B)

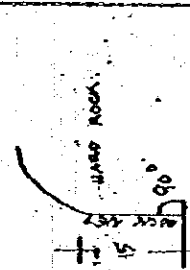
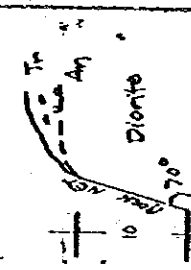
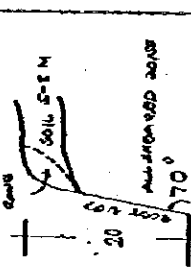
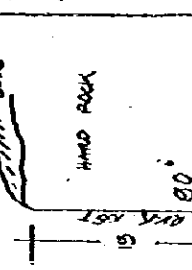

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
26	187.700	Andesite upper half of cutting slope is intensely weathered	weathered zone failure (exfoliate)		unstable zone remove drainage establish	failure occurred on small scale land slide type (B)
27	187.800	Andesite weathered zone is 5-8 meters thick Covered soil very thin	failure occurs on the top of the slope. failure is due to surface water action.		unstable zone remove drainage establish	failure occurred on small scale land slide type (B)
28	187.900	Andesite Generally hard. weathered zone is 3-5 meters Covered soil very thin	failure occurs by exfoliation on weathered andesite (fl-wing joint)		unstable zone remove drainage establish	unstable zone over-hung many gullies land slide type (C)
29	188.100	Micro-diorite is very hard at lower part. But top of slope is intensely weathered.	rock falls exfoliated from flowing joint failure occurs by tension of top half...		unstable weathered zone remove	there are many gullies on the slope land slide type (B)
30	188.200	Andesite strongly weathered Upper half slope strongly weathered (nearly soil)	rock fall occurs by exfoliation failure occurs due to surface water action (there are many gullies)		rock net should be used	there are many gullies land slide type (B)

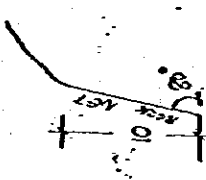
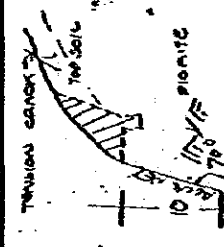
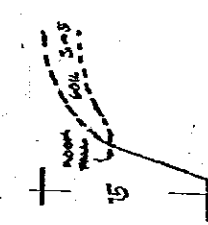
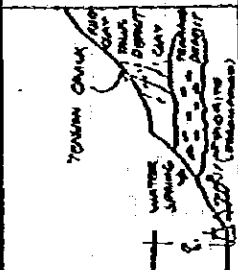
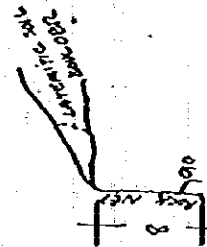
CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
31	189.100	<ul style="list-style-type: none"> Micro Diorite slightly weathered Covered soil 3-5m thick 	<ul style="list-style-type: none"> rock fall rock fall observed along open crack failure observed on weathered zone 		<ul style="list-style-type: none"> unstable zone remove (failure observed on weathered zone) drainage establish 	<ul style="list-style-type: none"> there are many gullies on the slope land slide type (B)
32	190.000	<ul style="list-style-type: none"> Micro diorite numerous cracks (open crack) are present debris few meter depth 	<ul style="list-style-type: none"> failure occur on the middle of the slope and land slide occur on surface slope. 		<ul style="list-style-type: none"> unstable zone remove rock net protection 	<ul style="list-style-type: none"> there are gullies on the slope land slide type (B)
33	190.200	<ul style="list-style-type: none"> Micro diorite little hard rock (moderately weathered) 	<ul style="list-style-type: none"> failure occur on weathered zone (cause by water action) rock falls all exfoliated. 		<ul style="list-style-type: none"> unstable zone remove rock net protection 	<ul style="list-style-type: none"> there are many gullies land slide type (A)
34	190.300	<ul style="list-style-type: none"> Diorite hard rock (moderately weathered) 	<ul style="list-style-type: none"> failure occurs on upper slope due to water action. rock fall occurs exfoliate on lower part of slope. 		<ul style="list-style-type: none"> unstable zone remove rock net protection 	<ul style="list-style-type: none"> large scale failure occurred. many gullies on slope land slide type (B)
35	192.100	<ul style="list-style-type: none"> Micro Diorite little hard rock Covered soil from 3.5m thick 	<ul style="list-style-type: none"> failure exfoliated from open flowing joint due to water action (underground surface) 		<ul style="list-style-type: none"> failure occurred at top of cut slope treatment of water (drainage) rock net protection 	<ul style="list-style-type: none"> there are many gullies on the slope land slide type (A)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
36	192.000	"Schist hard rock (slightly weathered)	"rock fall exfoliate from open joint.		"rock not protection "unstable zone remove	"only rock fall "land slide type (C)
37	192.200	"Diorite, andesite hard rock (slightly weathered) "Covered soil terrace deposit (3-5m)	"failure occurs on terrace deposit "rock fall exfoliate breccia		"unstable zone should be remove "rock not protection	"breccia fall (exfoliate) "land slide type (B)
38	192.900	"Gneiss, schalstein there is fault on the slope because basement rock is sheared.	"failure occurs easily in weathered soil (exfoliate due to schistosity joint)		"unstable zone should be remove "drainage establish "rock not protection	"there are water springs on the upper slope. "land slide type (A)
39	193.000	"Diorite, schalstein hard rock (slightly weathered) "Covered soil about one meter	"weathered zone failure (by water action), there are many gullies on the slope.		"unstable zone should be remove "rock not protection	"present slope is dry "failure occurred in small scale "land slide type (B)
40	193.150	"Diorite strongly weathered (soft rock) "Covered soil on terrace depo- site about 3-5m	"failure occurs on terrace deposits "rock fall exfoliate from joint.		"unstable zone should be remove "drainage establish	"terrace deposits failure (small scale) "land slide type (A)


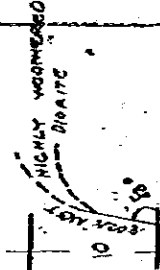
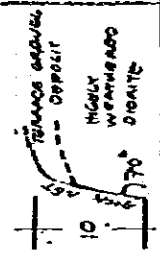
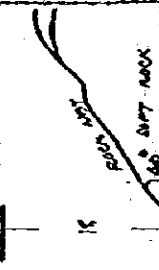

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
42	194.200	-Diorite, andesite hard rock (slightly weathered) -Covered soil (minimal)	-exfoliation failure, rock falls do not occur on the slope.		-unstable zone should be removed and rock net protection should be used.	-no problem -land slide type (C)
42	199.300	-Diorite slightly weathered hard rock (partly soft rock) -Covered soil 3-5 meter	-failure occurs on top of slope (soil failure) -rock fall exfoliate from joint.		-unstable zone should be removed -drainage establish -rock net protection	-there is tension crack on the natural slope. -failure occurs easily when it rains
43	195.850	-Micro joint hard rock -Covered soil 3-5m	-failure occurs on top of slope (weathered soil) many gullies present.		-unstable zone should be removed -rock net protection	-there are many gullies on the slope -land slide type (B)
44	196.100	-Terrace deposit and diorite (decomposed) all nearly weathered soil	-failure occurs due to under-ground water action and under ground water stream, through upper diorite.		-unstable zone should be removed -drainage establish around by (mason) establish (if failure occurs, debris will not reach road surface)	-very rich underground water -land slide type (D)
45	196.200	-Micro diorite little hard rock (slightly weathered) weathering zone depth 3-5m (nearly soil)	-failure occurs due to surface water action (weathered soil) and slide		-unstable zone remove -rock net protection	-failure occurs easily -land slide type (B)

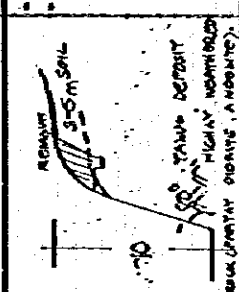
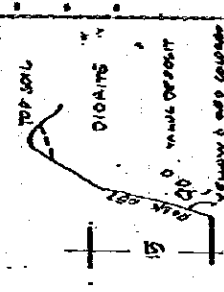
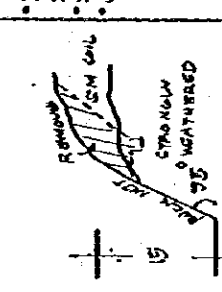
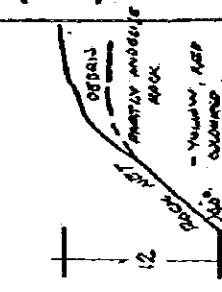
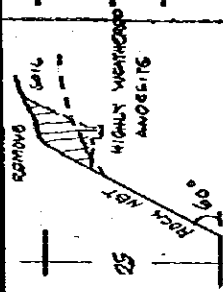
CUTTING SLOPE STABILITY CHECK TABLE

NO	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
46	196.450	-Micro diorite (Hornfels) there are many cracks but rock place very hard -Covered soil depth 3-5m	-rock fall -exfoliation along joint		-unstable zone should be remove -rock net protection	-there is water spring on the lower slope. -land slide type (O)
47	196.500	-Micro diorite hard rock there is a small fault on the slope	-rock fall -exfoliation along open crack		-unstable zone should be remove -rock net protection	-there is water spring on the slope -land slide type (O)
48	196.750	-Diorite slightly weathered -Covered soil depth 3-5m	-failure exfoliate on top of slope due to water action -rock fall -exfoliate along joint		-unstable zone on top of slope should be remove -rock net protection	-land slide type (A)
49	197.000	-Diorite strongly weathered (heavy soil) -Covered soil 2m thick	-failure occurred on top of slope -rock fall -exfoliate		-unstable zone on top of slope should be remove -rock net protection	-sheared zone -land slide type (C)
50	197.300	-Micro diorite fresh (hard) -Covered soil little	-rock fall -exfoliate from flowing joint		-Rock net protection	-no problem -land slide type (C)

CUTTING SLOPE STABILITY CHECK TABLE

NO	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
51	201.300	-Diorite strongly weathered -Covered soil 2-5m thick Terrace deposit	-failure occurred on the weathered zone (exfoliate)		-rock not protection	-there are gullies at terrace deposit -land slide type (C)
52	201.600	-Diorite strongly weathered (soil, little hard rock) -Covered soil 3-5 meter depth (weathered soil)	-failure occurs at weathered soil along the surface slope (by surface water action)		-unstable zone should be remove.	-there is no problem -land slide type (B)
53	202.200	-Diorite strongly weathered (little hard rock) -Covered soil depth 1.5m	-failure occurs due to surface stream action on the slope (there are many gullies)		-unstable zone should be remove -drainage establish /	-land slide type (C)
54	202.950	-Diorite Quartz porphyrite Hydrothermally altered zone (bed rock breccia) soft rock b -Covered soil thin	-failure occurs along joint (rock fall)		-rock not protection -unstable zone should be remove	-all soft rock -large scale failure occurs -land slide type (B)
55	203.200	-Hydrothermally altered zone -Diorite strongly weathered -Covered soil depth 3-5m	-weathered zone failure surface observed to exfoliate		-unstable zone remove -rock not protection	-shattered zone -land slide type (A)

CUTTING SLOPE STABILITY CHECK TABLE

NO	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
56	203.200	Andesite strongly weathered (sheared zone) Covered soil thin	failure occur due to surface water action (gullies) and exfoliation along joints.		unstable zone remove rock net protection	hydrothermally altered zone there are gullies land slide type (B)
57	203.300	Diorite strongly weathered sheared zone (hydrothermally altered zone) hard rock	exfoliation occurs from open cracks erode surface water action.		loose block should be remove rock net used as protection drainage established	there are many gullies on the slope land slide type (B)
58	203.500	Diorite soft rock sheared zone (Breccia rock) strongly weathered	exfoliate cause by surface water slope surface exfoliated all sheared zone cut-off of surface protection		loose and unstable zone remove rock net protection drainage establish	there are many gullies on the slope land slide type (B)
59	203.700	sheared zone diorite, andesite porphyry (all soil, breccia)	failure occurs at surface		drainage establish (there are many water spring points) loose and unstable zone remove	there are many gullies on the slope, and water spring land slide type (B)
60	203.800	andesite strongly weathered (hydrothermally altered) moderately hard	top of slope observed to have failed due to exfoliation along surface.		drainage establish (there are water spring on the slope) loose and unstable zone remove rock net protection	there are many water spring at the slope land slide type (B)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slopes)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
61	204.000	Andesite strongly weathered Frequently fractured loose	exfoliation unstable zone should be remove and rock net protec- tion should be used.		unstable zone remove rock net protection	failure in small scale land slide type (3)
62	204.101	Diorite sheared fracture zone Top soil 5-10m (debris)	weathered soil failure and debris fall occurs due to underground surface water action.		unstable zone remove rock net protection	there are many fault because movement rock breccia land slide type (2) (1)
63	204.300	Diorase nearly weathered soil	failure cause by water action (gullies) exfoliation (intense)		unstable zone remove there are many gullies on the slope.	there are many gullies on the slope land slide type (A) (B)
64	204.500	Diorase strongly weathered	slide (slump) weathered zone		unstable zone should be remove drainage establish rock net protection	there are many gullies on the slope land slide type (B)
65	204.700	Diorase/porphyry frequently fractured weathered breccia	surface exfoliate there are many gullies on slope surface, failure occured on the surface.		unstable zone remove drainage establish rock net protection	there are many gullies on the slope and water spring land slide type (3)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
66	205.000	<ul style="list-style-type: none"> Quartz porphyry (Op) (clay) frequently crack loose (nearly soil alteration) 	<ul style="list-style-type: none"> surface is observed to exfoliate. failure occurs along joint (inside) with clay and block. 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there is water spring hydrothermal alteration rock are all nearly soil land slide type (B)
67	206.000	<ul style="list-style-type: none"> Diabase nearly soil frequently (crack) nearly soil weathered 	<ul style="list-style-type: none"> failure occurs at slope surface due to surface water action. 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there are many gullies on the slope land slide type (B)
68	206.300	<ul style="list-style-type: none"> Diabase/porphyrite nearly soil frequently crack nearly soil 	<ul style="list-style-type: none"> surface failure is due to water action there are many gullies in the slope due to surface water. 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there are many gullies on the slope land slide type (A) land slide type (B)
69	206.900	<ul style="list-style-type: none"> Diabase/porphyrite frequently, very loose 	<ul style="list-style-type: none"> surface exfoliate by many gully (surface water stream) sheared zone, failure occurred at surface of slope. (exfoliate) 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there are many gullies on the slope rocks have many cracks land slide type (B)
70	207.100	<ul style="list-style-type: none"> Diorite/porphyrite nearly soil (sheared zone) frequently, very loose 	<ul style="list-style-type: none"> Failure observed on the slope surface (exfoliate from flowing joint) 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there are many gullies and water spring on the slope. all sheared zone land slide type (B)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
71	207.700	<ul style="list-style-type: none"> Andesite, dioritic porphyritic sheared zone nearly soil frequently crack 	<ul style="list-style-type: none"> failure occurs due to under ground water, and surface water action. rock is exfoliated 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there is water spring at the slope. land slide type (S)
72	207.900	<ul style="list-style-type: none"> Andesite strongly weathered frequently crack zone 	<ul style="list-style-type: none"> surface failure is observed to exfoliate. failure occurs on weathered soil due to water action. 		<ul style="list-style-type: none"> unstable zone remove drainage establish rock net protection 	<ul style="list-style-type: none"> there are many water spring quantity and gullies. land slide type (S)
73	208.600	<ul style="list-style-type: none"> Andesite strongly weathered to soil 	<ul style="list-style-type: none"> exfoliate from joint or surface (in small scale) 		<ul style="list-style-type: none"> unstable zone remove 	<ul style="list-style-type: none"> no problem land slide zone type (A)
74	208.800	<ul style="list-style-type: none"> Diorite sheared zone frequently crack very loose 	<ul style="list-style-type: none"> observed to exfoliate from flowing joint. 		<ul style="list-style-type: none"> unstable zone remove 	<ul style="list-style-type: none"> no problem land slide type (A)
75	209.000	<ul style="list-style-type: none"> Gabbro, diorite strongly weathered 	<ul style="list-style-type: none"> weathered zone failure (top of slope) exfoliates. 		<ul style="list-style-type: none"> unstable zone remove 	<ul style="list-style-type: none"> no problem land slide type (C)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
76	209.200	Andesite slightly weathered Frequently rock piece is very hard	exfoliate from joint		<ul style="list-style-type: none"> rock net protection unstable zone remove 	land slide type (C)
77	209.600	Andesite strongly weathered All soft rock.	weathered soil zone failure by water action.		<ul style="list-style-type: none"> unstable zone remove there are gullies on the slope because drain land slide type (B) drainage needs treatment. 	there are many hollow on the slope
78	209.900	Porphyritic andesite fresh	exfoliation occurs along joint (rock fall)		rock net protection	land slide type (C)
79	211.400	Andesite strongly weathered	exfoliation		<ul style="list-style-type: none"> there is a plain space side of road because 	land slide type (C)
80	213.400	Quartz Diorite strongly weathered (decompose diorite like mesa)	surface exfoliation from joint and surface erosion		<ul style="list-style-type: none"> there are many gullies on the slope (surface water) treatment surface water is needed. mound up (small) 	there is a little problem in drainage land slide type (B)

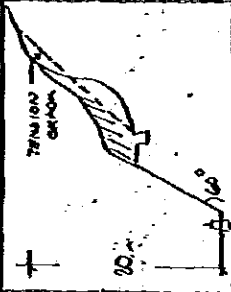
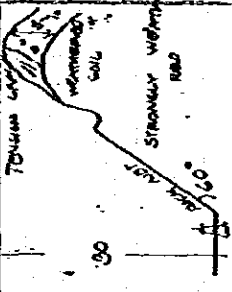
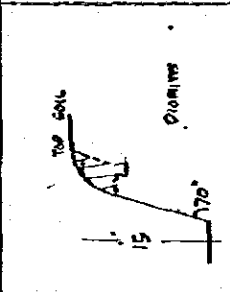
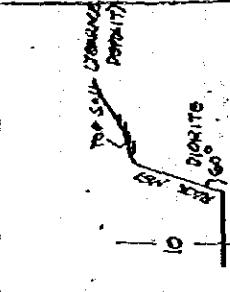
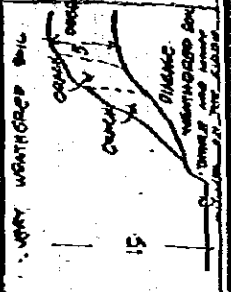
CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
81	213.800	*Debris (Limestone) strongly weathered (all soil) Limestone bed rock	*clay and boulder sands by ground surface water action		*unstable zone remove *needs treatment of drainage	*there is water spring on the slope *land slide type (B)
82	213.900	*Limestone - Quartz diorite strongly weathered very loose	*slump failure is due to water action.		*loose soil slump type. *needs drainage at the slope	*soil is calcicous, and have high water content *land slide type (C)
83	213.600	*Quartz Diorite strongly weathered *Frequently crack loose	*erosion due to stream action *There are many gullies present on the cutting slope		*drainage establish *mound up establish	*surface eroded out due to water action (small scale) *land slide type (B)
84	216.000	*Limestone, above quartz diorite strongly weathered little soft rock	*rock fall observed. limestone boulders		*unstable zone should be remove *rock not protection *mound up establish	*Rock falls only *land slide type (A)
85	216.100	*Quartz Diorite strongly weathered (decomposed)	*surface exfoliate (erosion by surface water) there are many gullies.		*drainage establish (water spring) *mound up establish	*surface exfoliate (small scale) *land slide type (B)

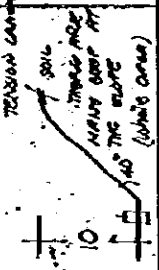
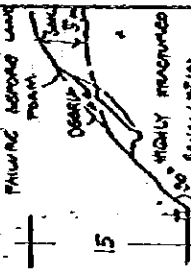
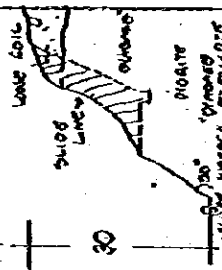
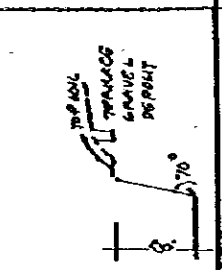
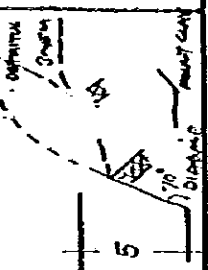
CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
86	216.700	Granodiorite strongly weathered (decomposed)	exfoliation is observed along joints and gullies failure occurs at the top of the slope.		<ul style="list-style-type: none"> • Mound up • drainage establish • rock net protection • unstable zone remove 	<ul style="list-style-type: none"> • surface exfoliate • land slide type (B)
87	216.800	Diorite strongly weathered (decomposed)	exfoliate from joints (exfoliation)		<ul style="list-style-type: none"> • Mound up • drainage establish • unstable zone remove 	<ul style="list-style-type: none"> • this exfoliate from joint surface • land slide type (B)
88	217.200	Schalestein/andesite nearly soil (weathered) Frequently loose	surface failure exfoliate from joint (open) highly weathered schalestein failure is observed due to ground water activities which occurs on top of the natural slope.		<ul style="list-style-type: none"> • Mound up (concrete wall) • drainage establish 	<ul style="list-style-type: none"> • surface failure will occur easily at the slope • land slide type (A)
89	217.600	Schalestein/andesite nearly soil strongly weathered	failure occurs on top of the slope by exfoliation from joint and surface water action (there are many gullies on the slope).		<ul style="list-style-type: none"> • unstable zone remove • drainage establish • mound up (or concrete wall) 	<ul style="list-style-type: none"> • surface failure will occur easily at the slope • land slide type (A)
90	219.200	Schalestein/andesite Diorite Frequently crack very loose	failure on weathered soil is observed. natural slope have gully relief, slope surface		<ul style="list-style-type: none"> • unstable zone remove • drainage establish • mound up (or concrete wall) 	<ul style="list-style-type: none"> • there are tension crack on the natural slope • land slide type (A)

CUTTING SLOPE STABILITY CHECK TABLE

NO	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
92	219.350	Schalestein nearly soil upper part of slope lower parts have little hard rock	top soil failures due to underground water stream action bed rock exfoliate from joint		unstable zone remove drainage establish mound up (concrete wall) rock not establish	surface failure easily occur due to water action land slide type (A)
92	219.900	Schalstein/Andosite strongly weathered nearly to soil	failure is observed on top of weathered soil along the slope by surface and ground water action. rock fall occur by ex- foliation of flowing joint.		unstable zone remove drainage establish mound up (concrete wall) rock not establish	there are many gullies on the slope land slide type (A)
93	220.000	Schalstein moderately hard rock slightly weathered	erosion by surface water		unstable zone remove drainage establish rock not establish	little problem land slide type (B)
94	220.300	Diorite strongly weathered (decomposed)	surface exfoliated terrace deposit		unstable zone remove drainage establish	little problem land slide type (B)
95	220.900	Diorite nearly soil (weathered)	failure occurs on the sur- face of slope by ground water activities		unstable zone remove drainage establish mound up	large scale failure land slide type (A)

CUTTING SLOPE STABILITY CHECK TABLE

NO.	KM	GEOLOGICAL FEATURE	CAUSE OF FAILURE (in cutting slope)	PRESENT LAND FORM	CUTTING SLOPE TREATMENT	REMARKS
96	221.200	"Soil (schalstein) nearly soil (sheared zone)	"surface exfoliate "there are many tension cracks on the upper portion of the slope.		"unstable zone remove "drainage establish "mound up	"bed rock do not "withstand mainly sheared clay "land slide clay (A)
97	223.150	"Schalstein strongly weathered (nearly soil)	"failure occurs along natural water way surface of slope "rock fall along cracks of bed rock		"unstable zone remove "drainage establish "mound up (concrete wall)	"there are many gullies on the slope "land slide type (A)
98	223.250	"Diabase strongly weathered "cracks are observed, very loose.	"slope surface is observed to pop off. "heavy rain led to the exfoliation of slope surface.		"unstable zone remove "drainage establish "mound up (concrete wall)	"there are many gullies on the slope "land slide type (A)
99	224.000	"Terrace deposit gravel (boulder cobbles and sand)	"failure occurs due to underground water flowing out and exfoliation from surface of slope.		"unstable zone remove "drainage establish	"there are many gullies on the slope "land slide type (B)
200	224.160	"Diabase (fault exist) nearly soil "Covered soil detritus (Gravel...)	"failure occurs as a result of underground water flowing out and surface water stream action.		"unstable zone remove "drainage establish	"detritus have many under ground water "land slide type (A)

APPENDIX C

**QUANTITATIVE ANALYSIS OF
SELECTED ALTERNATIVE ROUTES**

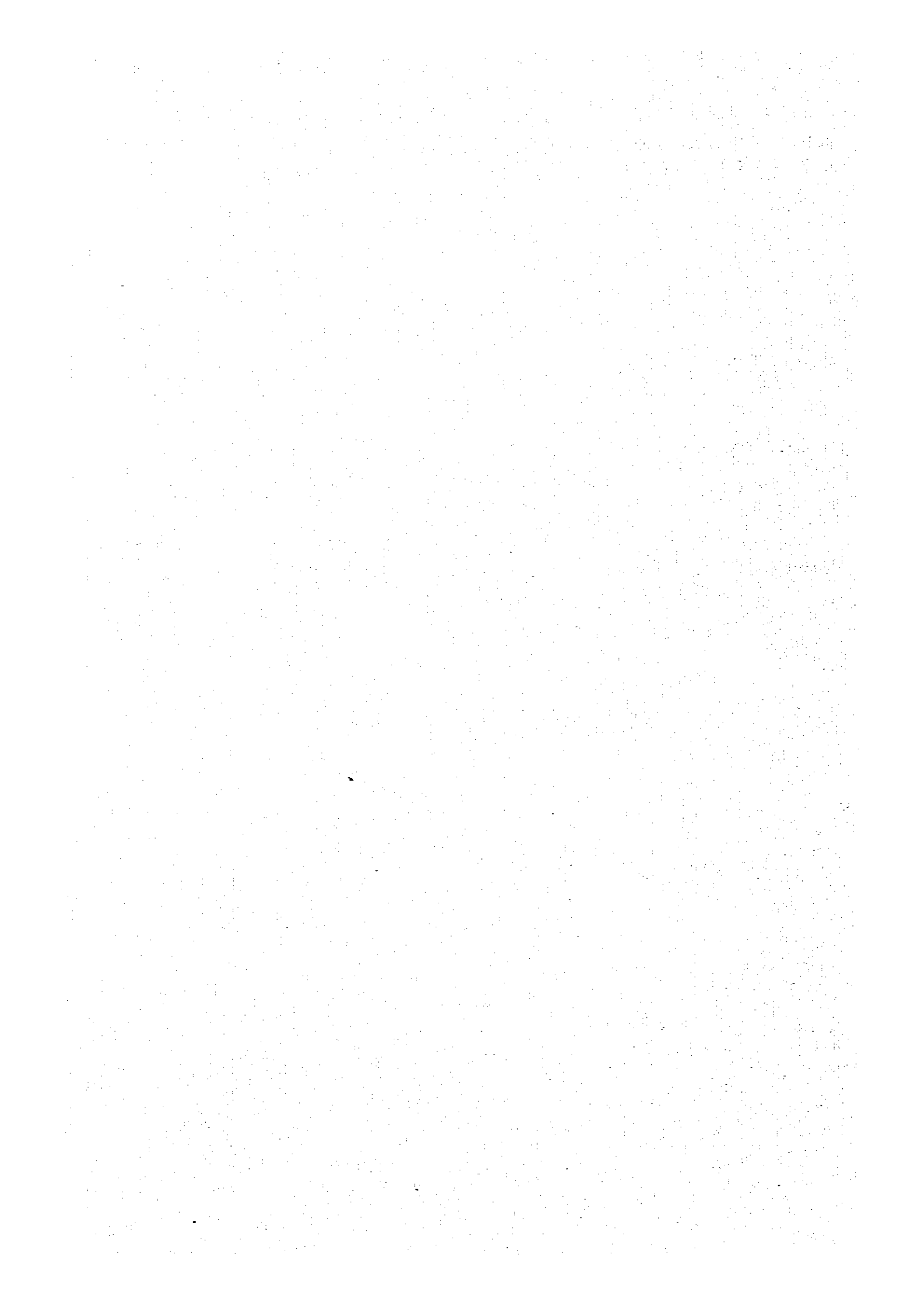


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1. Summary of Lengths (Alternative Route)

ITEM	ROUTE	II	II'	II"	V	II & V
	Total Length (m)	10,160.0	10,130.0	10,060.0	10,882.5	10,850.0
	NEW CONSTRUCTION SECTION	8,240.0	8,210.0	8,140.0	6,462.5	6,430.0
	Earth Work SECTION	5,610.0	5,340.0	4,955.0	3,352.5	3,570.0
	Cut (m)	1,992.0	1,879.0	1,912.0	1,552.0	1,647.0
	Details	1,797.5	2,090.0	1,750.0	1,122.5	1,090.0
	Fill (m)	1,820.5	1,371.0	1,303.0	678.0	833.0
	Half Cut 8' Half Fill	2,630.0	2,870.0	3,175.0	3,110.0	2,860.0
	Structure Section	760.0	760.0	795.0	1,270.0	990.0
	Bridge (m)	1,870.0	2,110.0	2,380.0	1,840.0	1,870.0
	Tunnel (m)	1,920.0	1,920.0	1,920.0	4,420.0	4,420.0
	TO-BE IMPROVEMENT SECTION					

KM 202 218 + 000

1-1 List of Items

Route II

STA.	NEW CONSTRUCTION					IMPROVEMENT
	Cut	Fill	C & F	Bridge	Tunnel	
II ₁	202 + 00					500.00
	+ 500					
	+ 950		450.0			
	203 + 170				220.0 II-1	
	+ 300		130.0			
	+ 600	300				
	+ 647.5		47.5			
	+ 747.5				100.0 II-2	
	+ 800.0		52.5			
	204 + 000			200.0		
+ 135.0	135.0					
+ 225.0				90.0 II-3		
+ 450.0	225.0					
205 + 000			550.0			
Sub total	660.0	680.0	750.0	410.0		500.00
II ₂	205 + 000					
	205 + 37.5			37.5		
	+ 152.5				115.0 II-4	
	+ 200		47.5			
	+ 350	150.0				
	+ 600			250.0		
	+ 830			230.0		
	207 + 700					
Sub total	150.0	47.5	517.5	115.0	1,870.0	
II ₃	207 + 700					
	+ 850	150.0				
	+ 872.5		22.5			
	+ 932.5				60.0 II-5	
	208 + 000		67.5			
	+ 350.0			350.0		
	+ 450.0	100.0				
	+ 600.0		150.0			
209 + 532.0	932.0				35.0 II-6	
+ 567.0						

Route II

STA.	NEW CONSTRUCTION					IMPROVEMENT
	Cut	Fill	C & F	Bridge	Tunnel	
202 + 00						
+ 500						
+ 950		450.0				500.00
203 + 170				220.0 II-1		
+ 300		130.0				
+ 600	300					
+ 647.5		47.5				
+ 747.5				100.0 II-2		
+ 800.0		52.5				
204 + 000			550.0			
+ 135.0	135.0					
+ 225.0				90.0 II-3		
+ 450.0	225.0					
205 + 000			550.0			
Sub total	660.0	680.0	750.0	410.0		500.00

I'

STA.						IMPROVEMENT
	CUT	FILL	C/F	BRIDGE	TUNNEL	
205+000 +035 +110 +450 +518 +558 +560 207+670	35.0 2.0	340.0	68.0	75 X'-1 40 X'-2	2110.0	
SUB TOTAL	37.0	340.0	68.0	115.0	2110.0	
TOTAL		2090.0		760.0	2110.0	
II"						
205+000 +035 +185 +220 207+600	35.0 35.0			150.0 X'-1	2380.0	
SUB TOTAL	70.0			150.0	2380.0	
TOTAL		1750.0		795.0	2380.0	

ROUTE X

STA.	NEW CONSTRUCTION					IMPROVEMENT
	CUT	FILL	C&F	BRIDGE	TUNNEL	
X 205+000 ~205+110 ~205+680 ~205+710 ~205+770 ~205+865 ~205+940 ~206+030 ~206+080 ~206+165 ~206+335 ~206+430 ~206+482.5 ~206+582.5	110.0 40.0 75.0 95.0	 52.5	 30.0 45.0	 570.0 600 75.0 50.0 180.0 100.0		
	370.0	52.5	125.0	1035.0		
			Ø 20.0M L= 1840 M	TUNNEL		
Y 205+000 ~205+110 ~205+680 ~205+710 ~205+770 ~205+870 ~205+995 ~206+100 ~206+120 ~206+250 ~206+300 ~206+400 ~206+500	110.0 100.0 105.0 50.0 100.0	 20.0	 30.0 150.0 100.0	 570.0 600 125.0		
	465.0	20.0	280.0	755.0		

2 Earth Works.

2-1

Route 11

1) Total of Cut,
540,095.^om³

Total F: 11,
245,035.^om³

Common 540,095.^o x 30% x 0.9 = 145,825.^o
Soft Rock 540,095.^o x 30% x 1.1 = 178,331.^o
Hard Rock 540,095.^o x 40% x 1.3 = 280,849.^o
604,905.^om³

2) Tunnel

$1870^n \times 77.5^m / n = 144,925.^o$
Common 144,925.^o x 20% x 1.0 = 28,985.^o
Soft Rock 144,925.^o x 30% x 1.3 = 56,520.^o
Hard Rock 144,925.^o x 40% x 1.5 = 86,925.^o
172,460.^om³

D + 2) = 777,365.^o

3) Excavation of Surplus Material

$777,365.^o - 245,035.^o = 532,330.^om³$

4) Formation of Embankment From Roadway Excavation

245,035.^o

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
202 + 500	450				0			
203 + 950	450				45.0	22.5	10,125.0	
203 + 120	11-1 Br.							
203 + 300	130					176.0	22,880.0	
+ 600	300		1000.0	30,000.0				
+ 647.5	47.5					20.0	95.0	
+ 747.5	11-2 Br.							
+ 800.0	52.5					20.0	1,050.0	
204 + 000	200.0		100.0	20,000.0		20.0	4,000.0	
+ 135.0	135.0		144.0	19,440.0				
+ 225.0	11-3 Br.							
+ 450.0	225.0		184	41,400				
205 + 000.	550.0		20.0	11,000		200.0	110,000	
	Sub-Total			121,840			149,005	

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205 + 000								
205 + 325	37.5		170.0	6,375.0		10.0	375.0	
+ 252.5	11.4 Br.							
+ 200	47.5					18.0	855.0	
+ 350	150.0		136.0	20,400.0				
+ 600	250.0		20.0	5,000		10.0	2,500.0	
+ 830	230.0		80.0	18,400		10.0	2,300.0	
207 + 700	TUNNEL		$1870^2 \times 77.5^2 / \pi$	144,925.0				
				50,175			6,030.	
				144,925.0				
			Sub-Total	195,100.0				

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
207 + 700								
+ 850	150.0		500.0	75,000				
+ 872.5	22.5					20.0	450	
+ 932.5	11-5 Br.							
208 + 000	67.5					20.0	1,350	
+ 350	350.0		120.0	42,000		20.0	7,000	
+ 450	100.0		186.0	18,600				
+ 600.0	150.0					40.0	6,000	
209 + 572.0	932.0			280,000				
+ 567.0	11-6 Br.							
+ 770	203.0		160	32,480				
+ 790	11-7 Br.							
210 + 090	300.0					180.0	54,000	
+ 180	11-8 Br.							

11-3

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
210 + 180								
+ 410	230				40		9,200	
+ 440	11-9							
+ 740	300				40		12,000	
				368,080			90,000	
				540,095.0			245,035.0	
				Tunnel		244,925.0		

2-2 Route 11'

1) Total of Cut

$$499,270.0 \text{ m}^3$$

Fill

$$275,725.0 \text{ m}^3$$

Common $499,270.0 \times 30\% \times 0.9 = 134,803.0$

Soft Rock $499,270.0 \times 30\% \times 1.1 = 164,759.0$

Hard Rock $499,270.0 \times 40\% \times 1.3 = \underline{259,620.0}$

$$559,182.0 \text{ m}^3$$

2) Tunnel

$$2110^m \times 77.5 \text{ m}^2/n = 163,525.0 \text{ m}^3$$

Common $163,525.0 \times 20\% \times 1.0 = 32,705.0$

Soft Rock $163,525.0 \times 30\% \times 1.3 = 63,774.0$

Hard Rock $163,525.0 \times 40\% \times 1.5 = \underline{98,115.0}$

$$194,594.0$$

$$1) + 2) = 753,776$$

3) Excavation of Surplus Material

$$753,776 - 275,725.0 = 478,051 \text{ m}^3$$

4) Formation of Embankment From Broadway Excavation

$$275,725.0 \text{ m}^3$$

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205 + 000								
+ 035	35.0		170.0	5,950				
+ 110	11'-2"							
+ 540	340				100		34,000	
+ 518	680		50.0	3,400			2,720	
+ 558	11'-2"							
+ 560	2.0		0					
207 + 670	TUNNEL		2110 ^m x 77.5 ^m	163,525.0				
				9,350			36,720	
				163,525				
			Sub-Total	172,875				
	Total		Tunnel	163,525.0			275,725.0	
				499,270.0				

2-3 Route 11¹¹

1) Total of Cut	496,570.0 m ³	Fill	239,005.0 m ³
-----------------	--------------------------	------	--------------------------

Common 496,570.0 x 30% x 0.9 = 134,073.0

Soft Rock 496,570.0 x 30% x 1.1 = 163,868.0

Hard Rock 496,570.0 x 40% x 1.3 = 258,216.0

556,157.0 m³

2) Tunnel

2380 m x 77.5 m³/m = 184,450.0 m³

Common 184,450.0 x 20% x 1.0 = 36,890.0

Soft Rock 184,450.0 x 30% x 1.3 = 71,935.0

Hard Rock 184,450.0 x 40% x 1.5 = 110,670.0

219,495.0 m³

1) + 2) = 775,652.0 m³

3) Excavation of Surplus Material

775,652.0 - 239,005.0 = 536,647.0 m³

4) Formation of Embankment From Roadway Excavation

= 239,005.0 m³

" 11"

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205 + 000								
+ 035	35.0		170.0	5,950				
+ 185	11' - 1"							
+ 220	35.0		20.0	700				
207 + 600	TUNNEL	2380	$77.5 \frac{m^2}{ft}$	184,450.0				
				6,650				
				184,450				
			Sub-Total	191,100				
Total				496,570.0			239,005.0	
			Tunnel	184,450.0				

2 - 4 Route V

1) Total of Cut

$$429,255.0 \text{ m}^3$$

$$\text{Common } 429,255.0 \times 30\% \times 0.9 = 115,899.0$$

$$\text{Soft - Rock } \times 30\% \times 1.1 = 141,654.0$$

$$\text{Hard - Rock } \times 40\% \times 1.3 = \frac{223,213.0}{480,766.0 \text{ m}^3}$$

Total Fill

$$92,370.0 \text{ m}^3$$

2) Tunnel

$$1,840 \text{ m} \times 77.5 \text{ m}^3/\text{m} = 142,600.0$$

$$\text{Common } 142,600 \times 20\% \times 1.0 = 28,520.0$$

$$\text{Soft - Rock } \times 30\% \times 1.3 = 55,614.0$$

$$\text{Hard - Rock } \times 40\% \times 1.5 = \frac{85,560.0}{169,694.0 \text{ m}^3}$$

$$1) + 2) = 650,460.0 \text{ m}^3$$

3) Excavation of Surplus Material

$$650,460.0 - 92,370.0 = 558,090.0 \text{ m}^3$$

4) Formation of Embankment from Roadway Excavation

$$92,370.0$$

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205 + 000								
+ 110	110.		50.0	5,500.0				
+ 680	Br							
+ 710	30.0		20.0	600.0		7.5	225.0	
+ 770	Br							
+ 865	95.0		35.0	3,325.0		6.0	570.0	
+ 940	Br							
206 + 030	90.0		200.0	18,000.0				
+ 080	Br							
+ 155	75.0		70.0	5,250				
+ 335	Br							
+ 430	95.0		300.0	28,500				
+ 482.5	52.5					30.0	1,575.0	
+ 582.5	Br		Sub Total	61,175.0			2,370.0	
	Total	Tunnel 184.0 m x 77.5		142,600.0			92,370.0	

2 - 5 Route II & V

(1) Total of Cut

430,080.0 m³

Total Fill

140,000 m³

Common 430,080.0 x 30% x 0.9 = 116,122.0

S - Rock x 30% x 1.1 = 141,926.0

H - Rock x 40% x 1.3 = 223,642.0
481,690.0 m³

(2) Tunnel

144,925

Common = 28,985.0

Soft - Rock = 56,520.0

Hard - Rock = 86,955.0
172,460.0

1) + 2) = 653,150.0

(3) Excavation of Surplus Material

653,150.0 - 140,000 = 513,150 m³

(4) Formation of Embankment from Roadway Excavation

140,000 m³

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
206 + 000 206 + 100	100.0		400.0	40,000				
206 + 300	200		20.0	4,000		250	50,000	
206 + 500	200		90.0	18,000				
							50,000	
				62,000				
			11'3"	368,080			90,000	
				430,080.0			140,000	
			Tunnel	144,925.0				

3 Slope protection

Slope protection No. I For Embankment (Placing Topsoil)						
Route II						
	STA	Length	Left		Right	
			m	m ²	m	m ²
II ₁	202 + 500	450	5.0	2250.0	5.0	2250.0
	+ 950					
	203 + 170	130	5.0	650.0	12.0	1560.0
	+ 300					
	203 + 600	47.5	5.0	237.5	5.0	237.5
	+ 647.5					
	+ 747.5					
	+ 800					
204 + 000	200.0	-	-	-	5.0	1000.0
204 + 450	550.0	-	-	-	24.0	13000.0
205 + 000						
				3,400.0		18,510.0
	Sub Total			Left + Right =	21,910.0 m ²	
II ₂	205 + 000	37.5	-	-	-	-
	+ 37.5					
	+ 152.5	47.5	-	-	8.0	380.0
	+ 200					
	+ 350	250.0	-	-	3.0	750.0
	+ 600					
+ 600	230.0	-	-	3.0	690.0	
+ 830						
						1,820.0
	Sub Total			L + R =	18,200 m ²	
II ₃	207 + 850	22.5	5.0	112.5	5.0	112.5
	+ 872.5					
	+ 932.5					
	208 + 000	67.5	5.0	337.5	5.0	337.5
	+ 350					
	+ 450	350.0	-	-	-	-
	+ 600					
	209 + 790	150.0	12.0	1,800.0	-	-
210 + 090	300.0	24.0	7,200.0	3.0	900.0	-
+ 180						
+ 410						
	230.0				24.0	5,520.0
	Sub Total			9,450.0		6,870.0
				L+R=16,320.0 m ²		

Slope protection

For Embankment (Placing Topsoil)

Route II'

	STA	Length	Left		Right	
			m	m ²	m	m ²
II'	202 + 110					
	+ 450	340.0	-		24.0	8,160.0
	+ 518	68.0	-		18.0	1,224.0
						9,384.0
V				L + R = 9,384.0 m ²		
V	206 + 430					
	206 + 482.5	52.5	5.0	262.5	5.0	262.5
				L + R = 525.0 m ²		
II&V	206 + 100					
	+ 300	200.0			20.0	4,000.0

Slope protection No. I For Cut Section (Planting Work)
Length of Cut x 2m/m

	STA	Length		Unit	Length of Planting Work
		Left	Right		
II ₁	203 + 300 203 + 600	300.0	300.0	2m/m	1,200 m
	204 + 450 205 + 000	500.0	-	2m/m	1,000 m
					2,200 m
II ₂	205 + 000 205 + 37.5	37.5	-	2m/m	75 m
	205 + 350 205 + 600	250.0	-	2m/m	500 m
	205 + 600 205 + 830	230.0	-	2m/m	460 m
II ₃	207 + 700 + 850	150.0	150.0	2m/m	600 m
	208 + 350 + 450	100.0	100.0	2m/m	400 m
	208 + 600 209 + 532.0	932.0		7m/m	6,524 m
					7,524 m
II'	205 + 000 + 035	35.0		4	140 m
	+ 450 + 518	68.0		2	136 m
					276 m
II''	205 + 000 + 035	35.0		4	140 m
V	205 + 000 205 + 110		110	2	220
	205 + 710		30.0	2	60
V	205 + 770 205 + 865		95.0	2	190
	205 + 940 206 + 030		90.0	5	450
	205 + 080 206 + 155		75.0	2	150

1070 m

Slope protection No. 3 For Cut Section (Concrete spraying)

Route II

	STA	Length	Left		Right	
			m	m	m	m
II ₁	203 + 800					
	204 + 000	200.0	10.0	2000.0	-	
	+ 135.0	135.0	7.0	985.0	19.0	2505.0
	+ 225					
	+ 450.0	225.0	20.0	4500.0	-	
	205 + 00	50.0	20.0	1000.0		
				8,445.0		2,505.0
	Sub total		L + R =	11,010.0m ²		
II ₂	205 + 200					
	+ 350	150.0	15.0	2,250.0		
	Sub total			2,250.0m ²		
II ₃	208 + 000					
	+ 350	350.0	-		15.0	5,250.0
	209 + 567 + 770	203.0			15.0	3,015.0
	Sub Total		L			8,295.0m
V	206 + 335 206 + 430	95.0	-		20.0	1,900.0

Slope protection No. 4 (Precast Concrete Frame)

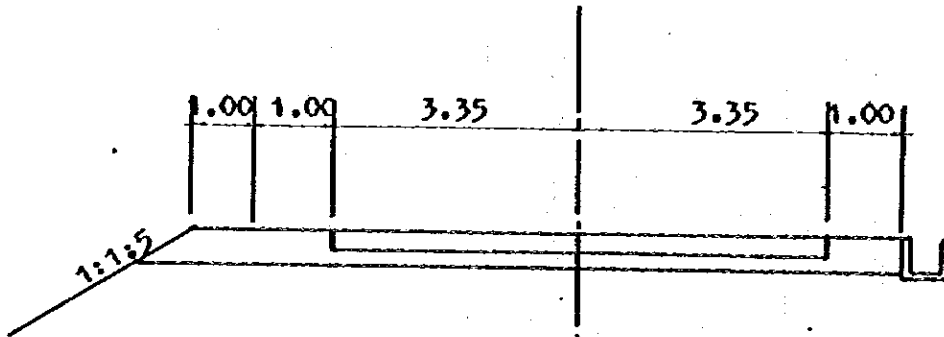
II₃ STA. 207 + 600 H=10m x 200m = 2,000m²

4 MINOR STRUCTURES

Retaining Wall For Embankment

Section	S.P.A.	STONE MASONRY		RETAINING WALL			
		H=3m	H=5m	H=5m	H=7m	H=8m	H=10m
II ₁	203 + 170 + 300	500.0	130.0m				50.0m
	204 + 150 204 + 500 204 + 500 205 + 000						
	Sub Total	500.0m	130.0m				50.0m
II ₂	205 + 152.5 205 + 200			47.5 47.5			
	Sub Total			95.0m			
II ₃	208 + 000 208 + 100				100.0		
	208 + 200 208 + 800				100.0		
	209 + 180 209 + 200						20.0
	209 + 450 209 + 532					82.0	
	209 + 567.0 209 + 650						83.0
	210 + 310 210 + 410						100.0
	210 + 440 210 + 600				160.0		
	Sub Total				360.0m	82.0m	203.0m
V	205 + 630 205 + 710			30.0			

5. PAVEMENT.



1) PORTLAND CEMENT CONCRETE PAVEMENT 0.23 m

$$1\text{m} \times 6.70\text{m} = 6.70 \text{ m}^2/\text{m}$$

2) CRUSHED GRAVEL BASE COURSE 0.20 m

$$1\text{m} \times 6.70\text{m} \times 0.20 \text{ m} = 1.34 \text{ m}^3/\text{m}$$

$$1\text{m} \times 1.00\text{m} \times 0.43 \text{ m} = 0.43 \text{ m}^3/\text{m}$$

$$1\text{m} \times (2.00 + 2.645) \text{ m} \times 0.43 \text{ m} + 2 = 1.00 \text{ m}^3/\text{m}$$

$$\underline{2.77 \text{ m}^3/\text{m}}$$

3) Prime Coat

$$1\text{m} \times 7.16 \text{ m}$$

No.	Unit Cost P	Quantity	P/m Cost	Cost 000 P/Km.
1	161.38	6.70 m ²	1,081 P/m	
2	115.43	2.77 m ³	318 P/m	
3	4.03	7.16 m ²	29 P/m	
			<u>1,428 P/m</u>	1,428 P/m

6. Safety facility

1) Traffic Road Sign

STA. 202 + 500	intersection	2 set	
203 + 150	Steep Grade	2	
	Tunnel	2	
	Steep Grade	2	
210 + 740	intersection	<u>2</u>	Santa Fe
		10	

2) Guard Rail

Lined on length of Embankment

7. Drainage

Gross Drains per Km:

1) Pipe Culvert

Rain faced Concrete Pipe Culvert = 1.22 m

(P.C.) = 8

L = 13m

2) Reinforced Concrete Box Culvert.

(R.C.) 2.40 x 2.40 = 3

L=15m

3) Inlet and Outlet Headwall and Wingwall.

8 + 3 = 11 set

No.	Unit Cost	Quantity	P/m Cost	Cost 000 P/km.
1	1,312.44	8		10.5 000 P/km.
2	5,397.64	3		16.2
3	27,074.0	11		297.8

4) Side Ditch

Used on length of Cut Section

5) Sub Drainage

Used on length of Cut Section

8. TUNNEL							('000 ₱)
ITEM	ROUTE	II (L=7870 m)	II' (L=2110 m)	II'' (2380 m)	V (1840 m)	II + V (L=1870 m)	REMARKS
Length of Tunnel		1,870 m	2,110 m	2,380	1,840	1,870	
Total Cost		233,000	266,000	291,000	230,000	233,000	
1. Main Work		134,000	152,000	171,000	133,000	134,000	
1) Excavation, Lining and Drain		133,000	151,000	170,000	132,000	133,000	
2) Portal		1,000	1,000	1,000	1,000	1,000	
2. Traffic Safety and Control Facilities		88,000	103,000	109,000	86,000	88,000	
1) Lighting Facility		3,500	3,700	4,000	3,300	3,500	
2) Power Receiving Distribution		5,700	5,900	6,000	5,500	5,700	
3) Stand-by Generator Facility		1,100	1,100	1,100	1,000	1,100	
4) Communication Facilities		5,100	5,100	5,100	5,100	5,100	
5) Fire-Preventing Facility		3,900	4,300	4,900	3,800	3,900	
6) Measuring Control Facility		5,200	5,400	5,700	5,100	5,200	
7) Ventilation Facilities		63,500	77,500	82,200	62,200	63,500	
Vertical Shaft		12,000	18,000	20,100	12,000	12,000	
Horizontal Shaft		11,300	19,800	21,500	10,000	11,300	
Ventilating Station		14,500	15,000	14,900	14,500	14,500	
Fan		25,700	25,700	25,700	25,700	25,700	
3. High Tension Line		11,000	11,000	11,000	11,000	11,000	

7 Bridge Route II				
Section	STA.	NAME	CONSTRUCTION COST	LENGTH
II ₁	202 + 950 203 + 170	II - 1	'000 P 7,283.0	L = 220m
	203 + 647.5 203 + 747.5	II - 2	2,474.0	100m
	204 + 135 204 + 225	II - 3	2,434.0	90m
			12,191.0	
II ₂	205 + 37.5 205 + 152.5	II - 4	2,737.0	115m
II ₃	207 + 872.5 207 + 932.5	II - 5	1,698.0	60m
	209 + 532.0 209 + 557.0	II - 6	1,182.0	(S) 35m
	209 + 770 209 + 790	II - 7	843.0	(S) 20m
	210 + 90 210 + 180	II - 8	2,597.0	90m
	210 + 410 210 + 440	II - 9	706.0	(S) 30m
			7,026.0	
		Total	21,954.0	

Bridges				
Section	STA	NAME	CONSTRUCTION COST	LENGTH
II'	205 + 035	II' - 1	1,629.0	75m
	205 + 110			
	205 + 518	II' - 2	1,251.0	40m
	205 + 558			
			2,880.0	
		Total	22,097.0	
II''	205 + 035	II'' - 1	3,808.0	150m
	205 + 185			
		Total	23,025.0	
II&V	205 + 870	II + V - 1	3,814.0	125m
	205 + 995			
V	205 + 110	V - 1	29,984.0	570m
	205 + 680			
	205 + 710	V - 2	2,205.0	60m
	205 + 770			
	205 + 865	V - 3	2,704.0	75m
	205 + 940			
	206 + 030	V - 4	1,769.0	50m
	206 + 080			
206 + 155	V - 5	6,498.0	180m	
206 + 335				
206 + 482.5	V - 6	2,824.0	100m	
206 + 382.5				
			45,984.0	
		Total	53,010.0	

10 Sabo

Approximate Cost Estimation of Erosion/Sediment Control Works

1) On the southern side of Dalton Pass

Four Sabo dams shall be delineated at the proper sites in the extreme upper reaches of Digdig River for the following reasons:

- a) To support the foundation of the newly planned road, especially for the parcel of land where the land slide is feared to occur (KM 204 205)
- b) To support the foundation of existing road, especially at the site where a fierce land slide and gully erosion is prevailing (KM 206 207)
- c) To treat the muck of Tunnel Construction properly at the site adjacent to the Tunnel Entrance.
- d) To maintain the natural environment of the rivers and mountain slopes or its vegetation that might be deteriorated totally by the construction works of the Project.

The main purpose for each dam is as follows:

- UAPINTALAN No. 1: To control the sediment,
No. 2: To control the sediment and the erosion
No. 3: To check the further erosion
4: To check the further erosion.

No. 1 - dam $H = 20^m$ (From foundation to Spillway crest)

$L = 101^m$ (Total crest length)

$V_c = 12,500^{m^3}$ (concrete volume)

$V_e = 7,000^{m^3}$ (Excavation volume)

$$\begin{aligned} 12,500 \times 20,000 + 7,000 \times 7,000 &= 250 \times 10^6 + 49 \times 10^6 \\ &= 299 \times 10^6 \\ &= (10 \times 10^6) \end{aligned}$$

Assuming alternative site ----- 7×10^6

No. 2 - dam $H = 13^m$ (From foundation to Spillway crest)

$L = 40^m$ (Total crest length)

$V_c = 2,400^{m^3}$ (Concrete volume)

$V_e = 900^{m^3}$ (Excavation volume)

$$\begin{aligned} 2,400 \times 20,000 + 900 \times 7,000 &= 48 \times 10^6 + 6.3 \times 10^6 \\ &= 54.3 \times 10^6 \\ &= (1.8 \times 10^6) \end{aligned}$$

No. 3 - dam $H = 9^m$ (From foundation to Spillway crest)

$L = 47^m$ (Total crest length)

$V_c = 1,800^{m^3}$ (Concrete volume)

$V_e = 1,000^{m^3}$ (Excavation volume)

$$\begin{aligned} 1,800 \times 20,000 + 1,000 \times 7,000 &= 36 \times 10^6 + 7 \times 10^6 \\ &= 43 \times 10^6 \\ &= (1.4 \times 10^6) \end{aligned}$$

No. 4

Assume the same with No. 3

$$\begin{aligned} &= 43 \times 10^6 \\ &= (1.4 \times 10^6) \end{aligned}$$

2) On the northern side of Dalton Pass

One Sabo dam, three consolidation works, and three ground sills shall be delineated in the rivercourse adjacent to the Santa Fe Bridge. The length of channel works which is related to the above mentioned works is to extend one thousand meters. The purpose of these works consists in the following matters.

- a) To support and stabilize the parcel of land where the newly planned road is passing by.
- b) To protect the existing Santa Fe Bridge from the sediment flow.
- c) To lend support to the treatment of muck of Tunnel Construction.
- d) To maintain the natural environment of the center of Municipality Santa Fe.

Sabo dam $H = 8^m$ (From foundation to Spillway crest)

$L = 75^m$ (Total crest length)

$V_c = 2,200^m^3$ (Concrete volume)

$V_e = 1,500^m^3$ (Excavation volume)

$$\begin{aligned} 2,200 \times 20,000 + 1,500 \times 7,000 &= 44 \times 10^6 + 10.5 \times 10^6 \\ &= 54.5 \times 10^6 \\ &= (1.8 \times 10^6) \end{aligned}$$

Consolidations $V_c = 330^m^3 = 1,000^m^3)$

Ground Sills $V_c = 100 \times 3 = 300^m^3)$

$$(1,300 \times 20,000) \times 1.5 = 39 \times 10^6$$

Rivetment Works $2 \times 1000 \times 66,000 = 132 \times 10^6$

$$= 171 \times 10^6$$

$$= (5.7 \times 10^6)$$

Total Construction Cost for Sabo

$$\begin{array}{r} 7 \times 10^6 \\ 1.8 \times 10^6 \\ 1.4 \times 10^6 \\ 1.4 \times 10^6 \\ 1.8 \times 10^6 \\ 5.7 \times 10^6 \\ \hline 19.1 \times 10^6 = 20 \times 10^6 \end{array}$$

// IMPROVEMENT

- 1) For Route II, II' & II''

Section (a) + Section (o)

$$507.0 + 869.0 = 1,376.0 \text{ '000 P}$$

- 2) For Route V

Section (a) + (b) + (o)

$$507.0 + 12,608.0 + 869.0 = 13,984.0 \text{ '000 P}$$

IMPROVEMENT

(a) KH. 202 + 000 To KH. 202 + 500

L = 500m

(1) OVER LAY n P 750.0

'000 P

500m X 750.0

(375.0)

(2) Drainage

(132.0)

Concrete side Ditch

L = 300m

300m x 433.18 P

130.0

Clean and repair Existing Culvert

3 Boxes x 13.0m x 62.38 P

2.0

507

(3) Total

'000 P

507.0

(b) KH 202 + 500 To KH. 205 + 000

L = 2,500m

(1) OVER LAY

'000P

2,500m x 750.0 P

(1,875.0)

(2) Drainage

(924.0)

Side ditch: 2,100m x 433.18 P

910.0

Clean Out Existing Side Ditch

1,100m x 4.21 P

5.0

Clean and repair Existing Culvert

11 pipes x 13.0m x 62.38P

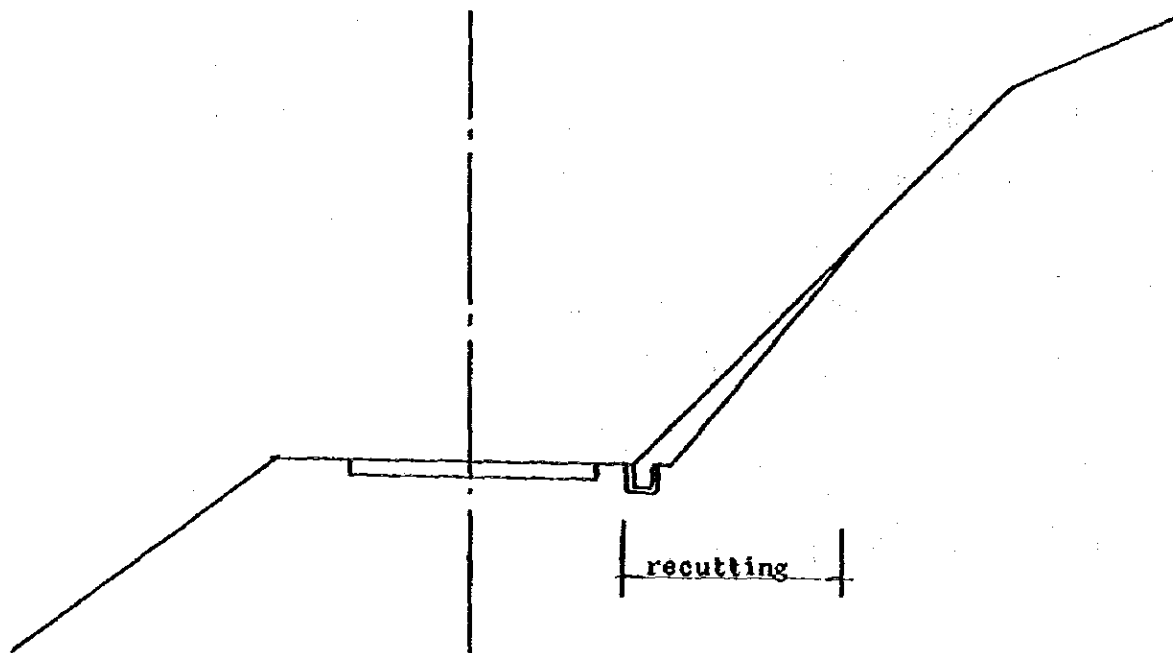
9.0

3) Slope protection (Concrete Spraying)

Cut Section

L = 2,100m

(9,809.0) '000 P



Recutting (Soft - Rock)

$$1.0m \times 6.0m \times \frac{1}{2} = 3m^2$$

$$3m^2 \times 2,100m \times 41.40 P$$

$$= 216.0 \text{ '000 P}$$

Concrete Spraying

H L

$$20m \times 280m = 5,600.0m^2$$

$$30 \times 300 = 9,000.0$$

$$20 \times 150 = 3,000.0$$

$$10 \times 200 = 2,000.0$$

$$20 \times 200 = 4,000.0$$

$$50 \times 150 = 7,500.0$$

$$30 \times 100 = 3,000.0$$

$$34,100.0m^2$$

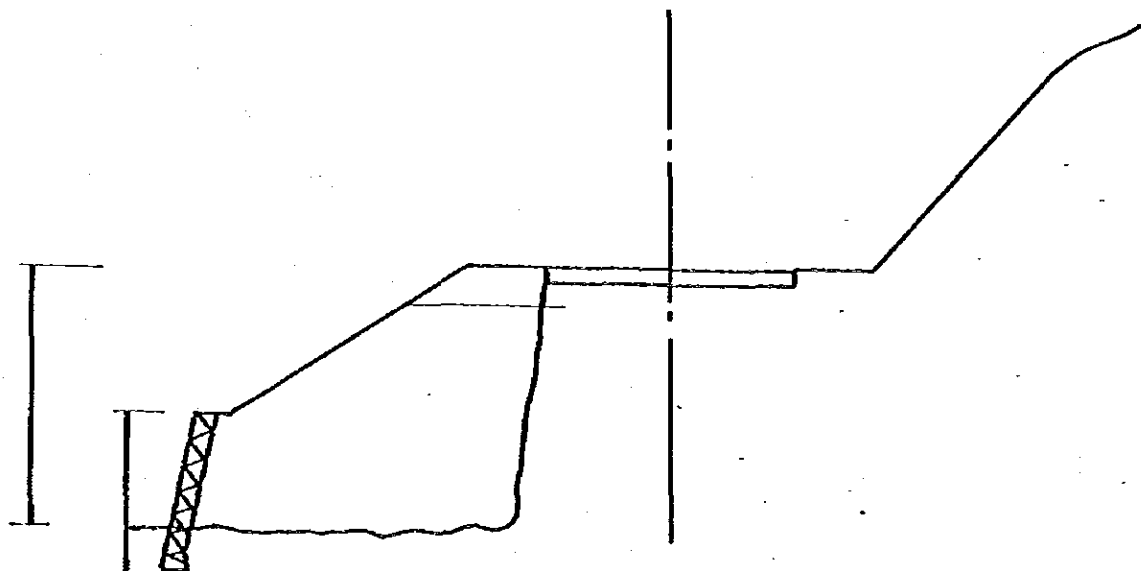
$$34,100.0m^2 \times 280.0 P$$

$$= 9,548.0 \text{ '000 P}$$

4) Total

$$= 12,608.0 \text{ '000 P}$$

(C) KM 216 + 580 To KM. 218 + 000	L = 1,420m	
1) Drainage		(285.0 '000P)
Side Ditch 650m x 433.18 ^P		282.0 '000P
Clean and repair Existing Culvert		
4 pipes x 13.0m x 62.38 ^P		3.0
2) KM. 216 + 800	Shoulder Scorded	(475.0)
L = 160 m		



Earth Work	$36 \text{ m}^2/\text{m} \times 160 \text{ m} \times 34.1^{\text{P}}$	196.0
Crushed Gravel Base Course	$3 \text{ m}^2/\text{m} \times 160 \text{ m} \times 115.43^{\text{P}}$	55.0
Stone Masonry H = 4 m	$160 \text{ m} \times 14000^{\text{P}}$	224.0

3) KH 217 + 250

Land slided

L = 70m (1090)

'000?

Recutting 70m x 70m² = 4,900 (common)

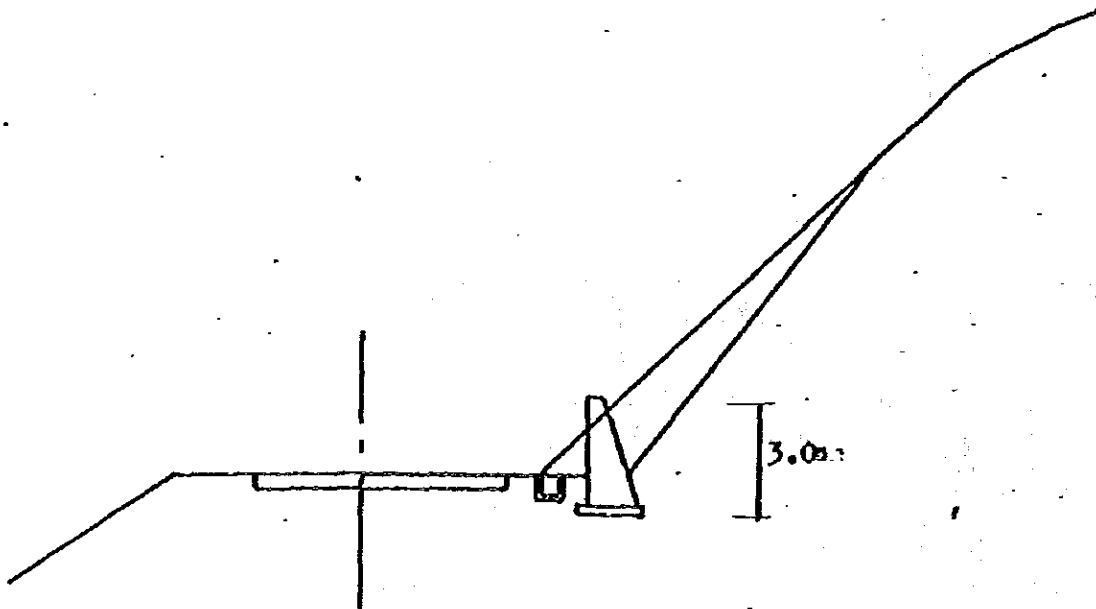
49,000m² x 0.5 x 13.05²

= 320 '000?

Retaining Wall H = 3m

70m x 1100²

= 77.0



4) Total

869.0 '000?

1/2 Right-of-Way

W=40.0 m

as January 1981 P

Section	Kilo-Post	Length	Land				Buildings	Total
			2.5 P/m ²	6 P/m ²	Forest	Crops	Nipa	
					7.5 P/m ²	13P/hill	44 P/m ²	
II	202 + 600	400.0		96,000.0			1(44.0 ^P)	
	203 + 000							
	203 + 100	600.0		144,000			1(44.0 ^P)	
	203 + 700							
	203 + 750 + 800	50.0		12,000				
	204 + 000	200.0		48,000			1(44.0)	
	204 + 500	500.0	4,000.0		10,000			
205 + 000	500.0		10,000					
			4,000.0	300,000.0	10,000		132.0	314.0 ^{'000P}
II ₂	205 + 000	800.0		240,000				
	205 + 800							
								240 ^{'000P}
II ₃	207 + 600	2,500.0	200,000					200 ^{'000P}
	210 + 100							
II'	205 + 000	560.0			168,000			168 ^{'000P}
	205 + 560							
II''	205 + 000	220.0			66,000			66
	205 + 220							
V	205 + 100	700.0			210,000			
	V ⁿ 205 + 800							
	206 + 400	600.0			180,000			
	206 + 400							
	206 + 500	100.0		24,000				
206 + 500								
206 + 600	100.0	8,000						
			8,000	24,000	390,000			427.0
II+								

ROAD RIGHT -OF- WAY
1978 Price Level

<u>RURAL</u>	<u>TYPE</u>	<u>UNIT</u>	<u>COST</u>
Land	a) Uncultivated	P/hectare	P20,000.00
	b) Cultivated	P/hectare	P50,000.00
	c) Forest	P/hectare	P60,000.00
	d) Crops and improvement	P/hectare	P30/tree - coconut P10/hill - banana P60/grove - bamboo P30/tree - mango P10/tree - star apple
Buildings	a) Residential	P/m ²	P35/sq. m/ - Nipa/mix mat P1,000-P1,500 - conc. P500-700 - semi-conc.

APPENDIX D

QUANTITIES AND COST ESTIMATES
OF THE MOST LIKELY ROUTE

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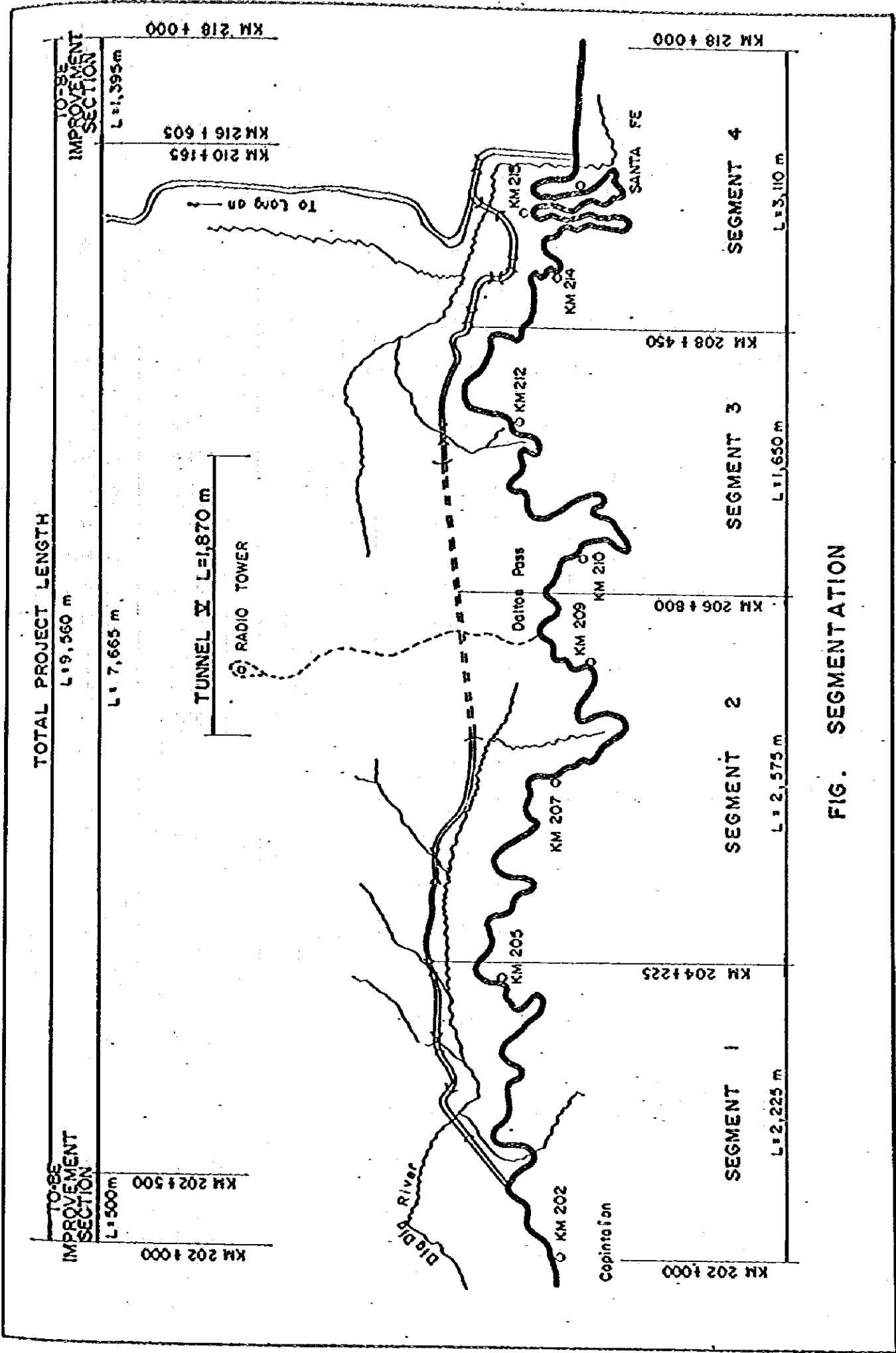


FIG. SEGMENTATION

I. SUMMARY OF LENGTH

SECTION	SECTION				(n)
	1	2	3	4	
ITEM	KM 202 + 000				
	KM 204 + 225	KM 206 + 800	KM 208 + 450	KM 218 + 000	TOTAL
TOTAL LENGTH	2,225.0	2,575.0	1,650.0	3,110.0	9,560.0
NEW CONSTRUCTION SECTION	1,725.0	2,575.0	1,650.0	1,715.0	7,665.0
EARTH WORK	1,325.0	1,470.0	705.0	1,367.0	4,867.0
Cut					
Left Side	650.0	1,412.5	250.0	425.0	2,747.5
Right Side	605.0	332.5	567.5	667.0	2,172.0
Average	632.5	872.5	408.75	546.0	2,459.75
Fill					
Left Side	665.0	77.5	455.0	42.0	2,139.50
Right Side	720.0	1,157.5	137.5	700.0	2,715.0
Average	612.5	617.5	235.25	821.0	2,427.25
STRUCTURE	400.0	1,085.0	915.0	378.0	2,808.0
Tunnel	0	770.0	100.0	0	1,870.0
Bridge					
L <= 50	0	0	45.0 (1)	108.0 (3)	153.0 (4)
L >= 50	400.0 (3)	115.0 (1)	0	270.0 (3)	785.0 (7)
To - Be - Improvement Section	500.0	0	0	1,375.0	1,875.0

* including Santa Fe Bridge (L = 300)

R = Cut & Fill

Station Kilo-Post	Left		Right		Bridge or Tunnel
	Cut	Fill	Cut	Fill	
km. 202+000					
202+500	To Be Improvement Section (L=500')				
202+950		450.0		450.0	
203+170					Br No 1 220°
203+300		130.0		130.0	
x 203+330	30.0			30.0	
203+600	270.0		270.0		
x 203+625	25.0			25.0	
203+657.5		32.5		32.5	
203+747.5					Br No 2 90°
203+800		52.5		52.5	
REQ 1 204+135	335.0		335.0		
204+225					Br No 3 90°
Sub Total	660.0	665.0	605.0	720.0	400.0
REQ 2 204+370	145.0		145.0		
x 204+450	80.0			80.0	
204+460		10.0		10.0	
x 204+880	420.0			420.0	
204+900		20.0		20.0	
x 205+000	100.0			100.0	
205+037.5	37.5		37.5		
205+152.5					Br No 4 115°
205+200.0		47.5		47.5	

	Station Kilo.-Post	Left		Right		Bridge or Tunnel
		Cut	Fill	Cut	Fill	
	205 + 200.0					
	+ 350.0	150.0		150.0		
SEG 2	* 205 + 830	480.0			480.0	
	206 + 800					TUNNEL 970.0
	sub Total	1412.5	77.5	332.5	1152.5	Br 1150 T 970
	207 + 700					TUNNEL 900
SEG 3	+ 830	130.0		130.0		
	+ 877.5		47.5		47.5	
	+ 922.5					Br 1105 450
	* 208 + 050		127.5	127.5		
	+ 170	120.0		120.0		
	* + 200		30.0	30.0		
	* + 260		60.0	60.0		
	+ 300		40.0		40.0	
	* + 400		100.0	100.0		
SEG 3	+ 450		50.0		50.0	
	sub Total	250.0	455.0	567.5	137.5	Br 450 T 970
	+ 498					Br 1106 480
SEG 4	* + 630		132.0	132.0		
	+ 680	50.0		50.0		
	* + 845		165.0	165.0		
	+ 900					Br 1107 55.0
	* 209 + 070		170.0	170.0		
	+ 130	60.0		60.0		

Station Kilo.-Post	Left		Right		Bridges or Turns
	Cut	Fill	Cut	Fill	
209+130					
+190					Br No 8 60°
+240		50°		50°	
+300	60°		60°		
x +330		30°	30°		
209+481		151°		151°	
+636					Br No 9 155°
+815		179°		179°	
+845					Br No 10 30°
x +950	105°			105°	
x 210+100	150°			150°	
+165		65°		65°	
216+605					
218+100	To - Be - Improvement (L: 1395)				
Sub Total	425°	942°	667°	700	Br 348°

II CONSTRUCTION COST ESTIMATE

CONSTRUCTION COST					
	NO.1	SEG 1		AS JANUARY: 1981	200 P
DESCRIPTION	UNT	UNT. COST	QUANTITY	COST	REMARKS
I. EARTHWORKS				1,237.0	
Clearing & Grubbing	SqM	0.85	10,770.0	9.2	
Stripping	CuM	20.83	1,077.0	22.4	
Excavation of Common Surplus Material	CuM	23.6	4,444.8	104.9	
Excavation of Common Soft-Rock Material	CuM				
Excavation of Common Hard-Rock Material	CuM				
Formation of Embankment From Roadway Excavation In Common Material	CuM	34.1	32,272.5	1,100.5	
Formation of Embankment From Roadway Excavation In Soft Material	CuM				
Formation of Embankment From Roadway Excavation In Hard Material	CuM				
Formation of Embankment From Borrow Excavation In Common Material	CuM	23.39	—		
Compaction of Existing Ground	SqM	2.83	—		
Compaction of Cut Section	SqM	2.25	0		
I-1 Slope Protection				2,058.6	
Gabions	each	455.85	—		
Matted Gabions	each	533.51	—		
Clearing of Boulder Stone	SqM	2.60	—		
Piling Top Soil	SqM	6.18	4,316.5	26.7	
Seeding	SqM	0.97	4,316.5	4.2	
Planting Work	L.M.	180.30	1,083.0	195.3	
Fencing For Dropped Stone	L.M.		—		

CONSTRUCTION COST					
		NO.2	SEG 1		
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Netting for Slope Protection	SqM	20. ⁰	6,108. ⁰	122. ²	
Concrete Splying	SqM	280. ⁰	6,108. ⁰	1,710. ²	
Precast Concrete Frame	SqM	220. ⁰	0	0	
II MINOR STRUCTURE				0	
Retaining Wall H=4m	M	4230. ⁰	0		
Retaining Wall H=7m	M	9000. ⁰	0		
Retaining Wall H=8m	M	10000. ⁰	0		
Stone Masonry H=2m	M	800. ⁰	0		
" H=3m	M	1100. ⁰	0		
" H=4m	M	1250. ⁰	0		
Stone Masonry H=5m	M	1750. ⁰	0		
" H=7m	M	2500. ⁰	0		
" H=7x2m	M	5000. ⁰	0		
III PAVEMENT					
	M	1,428. ⁰	1325. ⁰	1,892. ¹	
Subbase Course	CuM				
Crushed Gravel Base Course	CuM				
Prime Coat	SqM				
Tack Coat	SqM				
Portland Cement Concrete Pavement 0.23m	SqM				
Side Walk	L.M		0		
II-1 Safety Facilities					
Pavement Marking					
White W=0.15m	M				
Yellow W=0.15m	M				
Traffic Road Sign	each				
Kilometer Post	each				
Pavement Stud	each				
Delineoler	each				

CONSTRUCTION COST					
NO.3					
SEG 1					
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Metal Bear, Type Guard Rail	M				
Guard Rail End Piece	each				
Concrete Post for Guard Rail	each				
IV OVERLAY	M	750.0			
V DRAINAGE				917.1	
Grouted Side Ditch	L.M.	171.76			
Concrete Side Ditch w=0.5m.	L.M.	433.18	1,265.0	548.0	
Sub - Drainage	L.M.	170.85	1,265.0	216.1	
Reinforced Concrete Pipe Culvert Ø 1.2Cm.	L.M.	1330.0	54.0	71.8	
Reinforced Concrete Box Culvert					
150 X 1.50	L.M.				
2.00 X 2.00	L.M.	55000.0			
2-3.00 X 3.00	L.M.				
Clean and Repair Existing Culvert	L.M.				
Clean Out Existing Ditches	L.M.				
Inlet and Outlet Headwall and Wingwall	set	27,074.0	3	81.2	
VI TUNNEL	M				
VII BRIDGE					
VII-1 Sobou					
VIII DIRECT COST					Total I to VII
IX MISCELLANEOUS MAJOR WORKS					VIII X 0.15
X SUB-TOTAL ECONOMIC COST					VIII + IX

CONSTRUCTION COST					
NO.1		SEG 2		AS JANUARY: 1981	
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
I. EARTHWORKS				4,661.3	
Clearing & Grubbing	SqM	0.85	25,893.8	22.0	
Stripping	CuM	20.83	2,589.4	53.9	
Excavation of Common Surplus Material	CuM	23.6	110,943.1	2,618.3	
Excavation of Common Soft-Rock Material	CuM				
Excavation of Common Hard-Rock Material	CuM				
Formation of Embankment From Roadway Excavation In Common Material	CuM	34.1	56,625.6	1,930.9	
Formation of Embankment From Roadway Excavation In Soft Material	CuM				
Formation of Embankment From Roadway Excavation In Hard Material	CuM				
Formation of Embankment From Borrow Excavation In Common Material	CuM	23.39			
Compaction of Existing Ground	SqM	2.83			
Compaction of Cut Section	SqM	2.25	16,103.5	36.2	
I-1 Slope Protection				2,975.4	
Gobions	each	455.85	—		
Matted Gobions	each	533.51	—		
Clearing of Boulder Stone	SqM	2.60	—		
Paving Top Soil	SqM	6.18	9,603.3	59.3	
Seeding	SqM	0.97	9,603.3	9.3	
Planting Work	L.M.	180.30	5,042.0	909.1	
Fencing For Dropped Stone	L.M.		—		

CONSTRUCTION COST					
		NO.2	SE 9.2		
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Netting for Slope Protection	SqM	20.0	4,965.0	99.3	
Concrete Spraying	SqM	280.0	4,965.0	1,390.2	
Precast Concrete Frame	SqM	220.0	2,310.0	508.2	
II MINOR STRUCTURE				1,100.4	
Retaining Wall H=4m	M	4230.0	4.5.0	190.4	
Retaining Wall H=7m	M	9000.0	0	0	
Retaining Wall H=8m	M	10000.0	60.0	600.0	
Stone Masonry H=2m	M	800.0	0	0	
• H=3m	M	1100.0	100.0	110.0	
• H=4m	M	1250.0	160.0	200.0	
Stone Masonry H=5m	M	1750.0	0	0	
• H=7m	M	2500.0	0	0	
• H=7x2m	M	5000.0	0	0	
III PAVEMENT		M	1,428.0	2,460.0	3,512.9
Subbase Course	CuM				
Crushed Gravel Base Course	CuM				
Prime Coat	SqM				
Tack Coat	SqM				
Portland Cement Concrete Pavement 0.23 m	SqM				
III - I Safety Facilities					
Pavement Marking					
White W=0.15m	M				
Yellow W=0.15m	M				
Traffic Road Sign	each				
Kilometer Post	each				
Pavement Stud	each				
Delineator	each				

CONSTRUCTION COST					
		NO.3	SEG 2		
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Metal Bear, Type Guard Rail	M				
Guard Rail End Piece	each				
Concrete Post for Guard Rail	each				
V OVERLAY	M	750.0			
V DRAINAGE				1345.0	
Grouted Side Ditch	L.M.	171.76			
Concrete Side Ditch w=0.5m.	L.M.	433.18	1.745.0	755.9	
Sub - Drainage	L.M.	170.85	1.745.0	298.1	
Reinforced Concrete Pipe Culvert Ø 1.20m.	L.M.	1330.0	117.0	155.6	
Reinforced Concrete Box Culvert 1.50 X 1.50	L.M.				
2.00 X 2.00	L.M.	55000.0			
2-3.00 X 3.00	L.M.				
Clean and Repair Existing Culvert	L.M.				
Clean Out Existing Ditches	L.M.				
Inlet and Outlet Headwall and Wingwall	set	27074.0	5	135.4	
VI TUNNEL	M				
VII BRIDGE					
VIII Sobou					
VIII DIRECT COST					Total I to VII
IX MISCELLANEOUS MINOR WORKS					VIII X 0.15
X SUB-TOTAL ECONOMIC COST					VIII + IX

CONSTRUCTION COST					
NO.1		SEG 3		AS JANUARY: 1981	
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
I. EARTHWORKS				6,055.4	
Clearing & Grubbing	SqM	0.85	24,662.5	21.0	
Stripping	CuM	20.83	2,466.3	51.4	
Excavation of Common Surplus Material	CuM	23.6	196,065.4	4,627.7	
Excavation of Common Soft-Rock Material	CuM				
Excavation of Common Hard-Rock Material	CuM				
Formation of Embankment From Roadway Excavation In Common Material	CuM	34.1	3,9087.5	1,332.9	
Formation of Embankment From Roadway Excavation In Soft Material	CuM				
Formation of Embankment From Roadway Excavation In Hard Material	CuM				
Formation of Embankment From Borrow Excavation In Common Material	CuM	23.39	—		
Compaction of Existing Ground	SqM	2.83	—		
Compaction of Cul Section	SqM	2.25	10,221.2	23.0	
I-1 Slope Protection				2,454.5	
Gabions	each	455.85	—		
Matted Gabions	each	533.51	—		
Clearing of Boulder Stone	SqM	2.60	—		
Picking Top Soil	SqM	6.18	5,900.8	36.5	
Seeding	SqM	0.97	5,900.8	5.7	
Planting Work	L.M.	180.30	1,243.0	224.1	
Fencing For Dropped Stone	L.M.				

CONSTRUCTION COST					
		NO.2	SEG3		
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Netting for Slope Protection	SqM	20.°	6216.9	124.3	
Concrete Spraying	SqM	280.°	6216.°	1,740.5	
Precast Concrete Frame	SqM	220.°	1,470.°	323.4	
II MINOR STRUCTURE				1080.°	
Retaining Wall H=4m	M	4230.°	0.	0	
Retaining Wall H=7m	M	9000.°	0	0	
Retaining Wall H=8m	M	10000.0	0	0	
Stone Masonry H=2m	M	200.0	3500	700.0	
" H=3m	M	1100.0	0	0	
" H=4m	M	1250.0	80.°	100.0	
Stone Masonry H=5m	M	1750.0	0	0	
" H=7m	M	2500.0	280.°	700.0	
" H=7x2m	M	5000.0	0	0	
II PAVEMENT	M	1,428.0	1,605.°	2,291.9	
Subbase Course	CuM				
Crushed Gravel Base Course	CuM				
Prime Coat	SqM				
Tack Coat	SqM				
Portland Cement Concrete Pavement 0.23m	SqM				
II-1 Safety Facilities					
Pavement Marking					
White \bar{w} = 0.15m	M				
Yellow \bar{w} = 0.15m	M				
Traffic Road Sign	each				
Kilometer Post	each				
Pavement Stud	each				
Delineator	each				

CONSTRUCTION COST					
NO.3 5193					
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Metal Bear, Type Guard Roll	M				
Guard Roll End Piece	each				
Concrete Post for Guard Roll	each				
IV OVERLAY	M	750.0			
V DRAINAGE				704.0	
Grouted Side Ditch	L.M.	171.76			
Concrete Side Ditch w=0.5m.	L.M.	433.18	817.5	354.1	
Sub - Drainage	L.M.	170.85	817.5	139.7	
Reinforced Concrete Pipe Culvert Ø 1.2Cm.	L.M.	1330.0	97.0	129.0	
Reinforced Concrete Box Culvert					
1.50 X 1.50	L.M.				
2.00 X 2.00	L.M.	55000.0			
2-3.00 X 3.00	L.M.				
Clean and Repair Existing Culvert	L.M.				
Clean Out Existing Ditches	L.M.				
Inlet and Outlet Headwall and Wingwall	set	27,074.0	3	81.2	
VI TUNNEL	M				
VI BRIDGE					
VII Sobou					
VII DIRECT COST					Total I to VII
IX MISCELLANEOUS MINOR WORKS					VIII X 0.15
X SUB-TOTAL ECONOMIC COST					VIII + IX

CONSTRUCTION COST					
NO.1		5194		AS JANUARY: 1981	
DESCRIPTION	UNT	UNIT COST	QUANTITY	COST	REMARKS
I. EARTHWORKS				4,385.6	
Clearing & Grubbing	SqM	0.85	32,405.5	27.5	
Stripping	CuM	20.83	3,240.6	67.5	
Excavation of Common Surplus Material	CuM	23.6	110,930.4	2,618.0	
Excavation of Common Soft-Rock Material	CuM				
Excavation of Common Hard-Rock Material	CuM				
Formation of Embankment From Roadway Excavation in Common Material	CuM	34.1	49,050.2	1,672.6	
Formation of Embankment From Roadway Excavation in Soft Material	CuM				
Formation of Embankment From Roadway Excavation in Hard Material	CuM				
Formation of Embankment From Borrow Excavation in Common Material	CuM	23.39	—		
Compaction of Existing Ground	SqM	2.83	—		
Compaction of Cut Section	SqM	2.25	0	0	
I-1 Slope Protection				1,418.8	
Gabions	each	455.85	—		
Matted Gabions	each	533.51	—		
Clearing of Boulder Stone	SqM	2.60	—		
Paving Top Soil	SqM	6.18	9,077.5	56.1	
Seeding	SqM	0.97	9,077.5	8.8	
Planting Work	L.M.	180.30	7,130.0	1,285.5	
Fencing For Dropped Stone	L.M.				

CONSTRUCTION COST					
		NO.2	SEG 4		
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Netting for Slope Protection	SqM	20.0	228.0	4.6	
Concrete Sproying	SqM	280.0	228.0	63.8	
Precast Concrete Frame	SqM	220.0	0	0	
II MINOR STRUCTURE				4,345.1	
Retaining Wall H=4m	M	4230.0	0	0	
Retaining Wall H=7m	M	9000.0	245.0	2,205.0	
Retaining Wall H=8m	M	10000.0	70.0	700.0	
Stone Masonry H=2m	M	800.0	0	0	
" H=3m	M	1100.0	300.0	330.0	
" H=4m	M	1250.0	265.0	331.3	
Stone Masonry H=5m	M	1750.0	45.0	78.8	
" H=7m	M	2500.0	0	0	
" H=7x2m	M	5000.0	140.0	700.0	
III PAVEMENT				2,016.7	
	M	1,428.0	1,367.0	1,952.1	
Subbase Course	CuM				
Crushed Gravel Base Course	CuM				
Prime Coat	SqM				
Tack Coat	SqM				
Portland Cement Concrete Pavement 0.23m Side Walk	SqM L.M	169.9	380.0	64.6	
III-1 Safety Facilities					
Pavement Marking					
White W=0.15m	M				
Yellow W=0.15m	M				
Traffic Road Sign	each				
Kilometer Post	each				
Pavement Stud	each				
Delineator	each				

CONSTRUCTION COST					
		NO.3	SEG 4		
DESCRIPTION	UNIT	UNIT COST	QUANTITY	COST	REMARKS
Metal Beor, Type Guard Rail	M				
Guard Rail End Piece	each				
Concrete Post for Guard Rail	each				
V OVERLAY	M	750.0		—	
V DRAINAGE				3,031.8	
Grouted Side Ditch	L.M.	171.76	—	—	
Concrete Side Ditch w=0.5m.	L.M.	433.18	1,092.0	473.0	
Sub-Drainage	L.M.	170.85	1,092.0	186.6	
Reinforced Concrete Pipe Culvert Ø 1.20m.	L.M.	1330.0	69.0	91.8	
Reinforced Concrete Box Culvert 1.50 X 1.50	L.M.		—		
2.00 X 2.00	L.M.	55,000.0	39.0	2,145.0	
2-3.00 X 3.00	L.M.		—		
Clean and Repair Existing Culvert	L.M.		—		
Clean Out Existing Ditches	L.M.		—		
Inlet and Outlet Headwall and Wingwall	set	27,074.0	5	135.4	
VI TUNNEL	M				
VII BRIDGE					
VIII Sabou					
VIII DIRECT COST					Total I to VII
IX MISCELLANEOUS MINOR WORKS					VIII X 0.15
X SUB-TOTAL ECONOMIC COST					VIII + IX

III QUANTITIES

1. EARTHWORKS

1) Clearing & Grubbing, Stripping (0.1 m)

STA	WIDTH	HEIGHT	AVG. WIDTH	AREA
203 + 310		0		
+ 400	90.0	26.0		1,170.0
+ 510	140.0	0		1,820.0
+ 580	40.0	48.0		960.0
+ 610	30.0	0		720.0
203 + 790		0		
+ 800	10.0	13.0		65.0
+ 900	100.0	0		650.0
+ 950	50.0	27.0		675.0
204 + 000	50.0	16.0		1,075.0
+ 100	100.0	42.0		2,900.0
+ 135	35.0	0		735.0
			590.1	10,770.0 m ²
				1,977.0 m ³
+ 225		0		
+ 300	75.0	43.0		1,512.5
+ 400	100.0	18.0		3,050.0
+ 450	50.0	0		450.0
+ 450		0		
+ 500	40.0	13.0		260.0
+ 610	100.0	0		650.0
+ 700	100.0	13.0		650.0
+ 750	50.0	4.0		425.0
+ 850	100.0	13.0		850.0
+ 900	50.0	0		325.0
205 + 000	100.0	51.0		2,550.0
+ 37.5	37.5	0		956.3

STA.	LNTH	WIDE	AVE. WIDE	AREA
205 + 200				
+ 230	30.0	0		
+ 300	70.0	41.0		1,435.0
+ 350	50.0	0		1,025.0
+ 400	50.0	20.0		500.0
+ 500	100.0	11.0		1,550.0
+ 600	100.0	25.0		2,000.0
+ 700	100.0	30.0		2,550.0
+ 800	100.0	34.0		3,200.0
+ 840	30.0	63.0		1,455.0
			Seg. 2	
			Total	25,893.8e ²
			x 0.1	2,589.4e ³
207 + 650		55.0		
+ 750	60.0	110.0		4,280.0
+ 800	50.0	110.0		5,500.0
+ 830	30.0	50.0		2,400.0
+ 877.5	47.5	0		1,187.5
+ 922.5		0		
+ 950	27.5	35.0		495.0
208 + 000	50.0	35.0		1,800.0
+ 100	100.0	55.0		4,550.0
+ 200	100.0	0		2,750.0
+ 300	100.0	0		
+ 350	50.0	20.0		500.0
+ 400	50.0	0		500.0
			Seg. 3	24,662.5e ²
				2,466.3e ³

STA.	LENGTH	WIDTH	Avg. WIDTH	AREA
208 + 478		0		
+ 550	52.0	40.0		1,040.0
+ 600	50.0	80.0		3,000.0
+ 650	50.0	75.0		8,750.0
+ 750	100.0	30.0		6,250.0
+ 845	5.0	0		1,425.0
+ 897		0		
+ 950	51.0	75.0		1,912.5
209 + 000	50.0	23.0		2,450.0
+ 100	100.0	35.0		2,900.0
+ 130	30.0	0		525.0
+ 240		0		
+ 270	30.0	65.0		975.0
+ 310	40.0	0		1,300.0
209 + 636		0		
+ 700	64.0	4.0		128.0
+ 750	50.0	0		100.0
+ 700		0		
210 + 000	100.0	20.0		1,000.0
+ 165	65.0	0		650.0
			Seg. 4	32,405.50 ²
			Total	3,240.60 ³
			Total	63,731.80 ²
				9,373.20 ³

2) Cut and Embankment Volume

SEGMENT	CUT	FILL	TUNNEL	SURPLUS MATERIAL OF EXCAVATION	EMBANKMENT
1	+ 36,717.3	- 32,272.5	0	4,444.8	32,272.5
2	+ 78,107.8	- 56,625.0	185,458.3	110,443.1	56,625.0
3	152,150.4	- 32,087.5	83,002.5	106,065.4	32,087.5
4	152,080.4	- 49,050.0	0	110,030.4	49,050.0
Total	426,957.1	-177,035.0	172,460.8	422,333.7	177,035.0

SEGMENT	CUT					TUNNEL					Fill	Others For Deposit Area	Remarks	
	VOLUME	Common x40% x1.0	Soft-rock x30% x1.1	Hard-rock x30% x1.3	Total	VOLUME	Common x30% x1.0	Soft-rock x30% x1.3	Hard-rock x40% x1.5	Total				
1	33,997.5	12,239.1	11,219.2	13,259.0	36,717.3	-	-	-	-	-	32,272.5			
2	72,323.2	26,036.6	23,866.9	28,206.3	78,109.8	79,175.0	15,035.0	29,318.3	45,103.0	89,438.3	36,625.0		No. 1 to No. 5 S480 DAM	
3	140,880.0	50,716.8	46,490.4	54,943.2	152,150.4	69,750.0	23,930.0	27,502.5	41,930.0	83,002.5	39,087.5			
4	148,130.0	53,326.8	48,882.9	57,770.7	139,980.4	-	-	-	-	-	49,050.0		237,225.0	
Total	395,331.3	142,319.3	139,459.4	154,179.2	426,937.9	144,925.0	28,965.0	56,820.8	86,933.0	172,460.0	177,033.0			

SEGMENT /

STA.	LENGTH	CUT			FILL			TOTALS
		AREA	AVG. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
201 + 500					0			
+ 800	300.0				2.0		300.0	
+ 900	100.0				18.0		1000.0	
+ 950	50.0				20.0		9500.0	
				Br No 1		4,220.0		
203 + 170					176.0			
+ 250	80.0				176.0		14000.0	
+ 310	60.0	0			0		5,550.0	
+ 400	90.0	100.0		4,500.0				
+ 540	140.0	0		1000.0				
+ 580	40.0	140.0		2,000.0				
+ 610	30.0	0		2,100.0		0		
+ 657.5	47.5				20.0		475	
				Br No 2		1,900.0		
								30,655.0

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
Rm 203 + 797.5					30.0			
+ 770	42.5	0			8.0		807.5	
+ 800	10.0	17.0		25.0	8.0		80.0	
+ 900	100.0	0		850.0	0		400.0	
+ 950	50.0	55.0	0.5	1375.0	7.0		175.0	
204 + 1000	50.0	55.0		2750.0	0		175.0	
+ 100	100.0	145.0		10,000.0				
+ 135	35.0	0		2,537.5				
				Bm No 3	4.0	90.0"		
+ 225		0						
+ 300	75.0	184.0	0.5	6,900.0				
+ 400	100.0	15.0		9,950.0				
+ 450	50.0	0		375.0	0			
+ 460	10.0			300.0			1,500.0	
				34,822.5			3,137.5	

↑
SEG 1

↓
SEG 2

SEGMENT 1

STA.	LENGTH KM	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
204 + 460		0			300.0			
+ 500	40.0	14.0		280.0	300.0		6600.0	
+ 600	100.0	0		700.0	40.0		3500.0	
+ 700	100.0	18.0		900.0	50.0		4500.0	
+ 750	50.0	2.0		500.0	138.0		4700.0	
+ 850	100.0	14.0		800.0	200.0		16700.0	
+ 900	50.0	0		350.0	144.0		8600.0	
205 + 000	100.0	165.0		8250.0	0		2200.0	
+ 037.5	37.5	0		3093.3				
				Bx 100 x 4.115.0 =				
+ 152.5					18.0			
+ 200	47.5				18.0		555.0	
+ 230	30.0	0			0		270.0	
+ 300	70.0	140.0	CS	4700.0				
				19773.8				53/25.0

SECTION 1

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205 + 300		140.0	0.5		20.0			
+ 350	50.0	0		3,500.0	20.0		1,000.0	
+ 400	50.0	45.0		1,125.0	0		500.0	
+ 500	100.0	15.0		3,000.0	5.0		250.0	
+ 600	100.0	53.0		3,400.0	0		250.0	
+ 700	100.0	76.0		6,450.0				
+ 800	100.0	110.0		9,300.0				
+ 830	30.0	460.0		8,550.0				
							2,000.0	
				35,325.0			88,897.5	
			TOTAL	106,321.3				
206 + 500	77.5	1970		75,175.0				
	(TURNPIKE)			775				
				1870				
	121	1900		69,750.0				

NEG 2

NEG 3

SE 4 2

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
207 + 690		576.0						
+ 750	60.0	870.0		43,380				
+ 800	50.0	690.0		39,000				
+ 830	30.0	150.0		12,600	0			
+ 877.5	47.5	0		35,625	20.0		475.0	
					Br. No. 5	4.45.0		
+ 922.5		0	7		0			
+ 950.0	27.5	70.0		962.5	150.0		2062.5	
208 + 000	50.0	115.0	6.5	4625.0	0		3,750.0	
+ 100	100.0	260.0		18,750.0				
+ 200	100.0	0		13,000.0	0			
+ 300	100.0	0			304.0		15,700.0	
+ 350	50.0	100.0		2500.0	100.0		10,100.0	
+ 400	50.0	0		2500.0	0		2,500.0	
				140,580.0			34,057.5	

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
208 + 400		0			0			
+ 420	20.0				200.0		2000.0	
+ 450	30.0				0		2000.0	
			Br 16.6	48.0				
+ 498		0			10.0			
+ 550	52.0	148.0		3848.0	1.0		286.0	
+ 600	50.0	620.0		19,200.0	0.8		45.0	
+ 650	50.0	780.0		35,000.0	0		20.0	
+ 750	100.0	48.0		41,400.0	0.8		80.0	
+ 845	95.0	0		2,280.0	10.0		85.5	
			Br	110.7	4.540			
+ 899		0						
+ 950	51.0	270.0		6,885.0	0			
209 + 000	50.0	38.0		7,700.0	10.0		250.0	
				116,513.0			5,726.5	

NEG 3

NEG 4

SF 2

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
209 + 000		38.0			10.0			
209 + 100	100.0	188.9		11345.0	0		944.5	
+ 130	30.0	0		2833.5				
+ 190			B+ MO 8	L= 60.0 ~	0			
+ 200	10.0				248.0		1240.0	
+ 240	40.0	0			0		4960.0	
+ 270	30.0	456.0		6840.0				
+ 310	40.0	0		9120.0	0			
+ 350	40.0				226.0		4520.0	
+ 400	50.0				80.0		7650.0	
+ 481	81.0				288.0		14904.0	
+ 636			B+ MO 9	L= 155.0 ~				
				30138.5			34215.5	

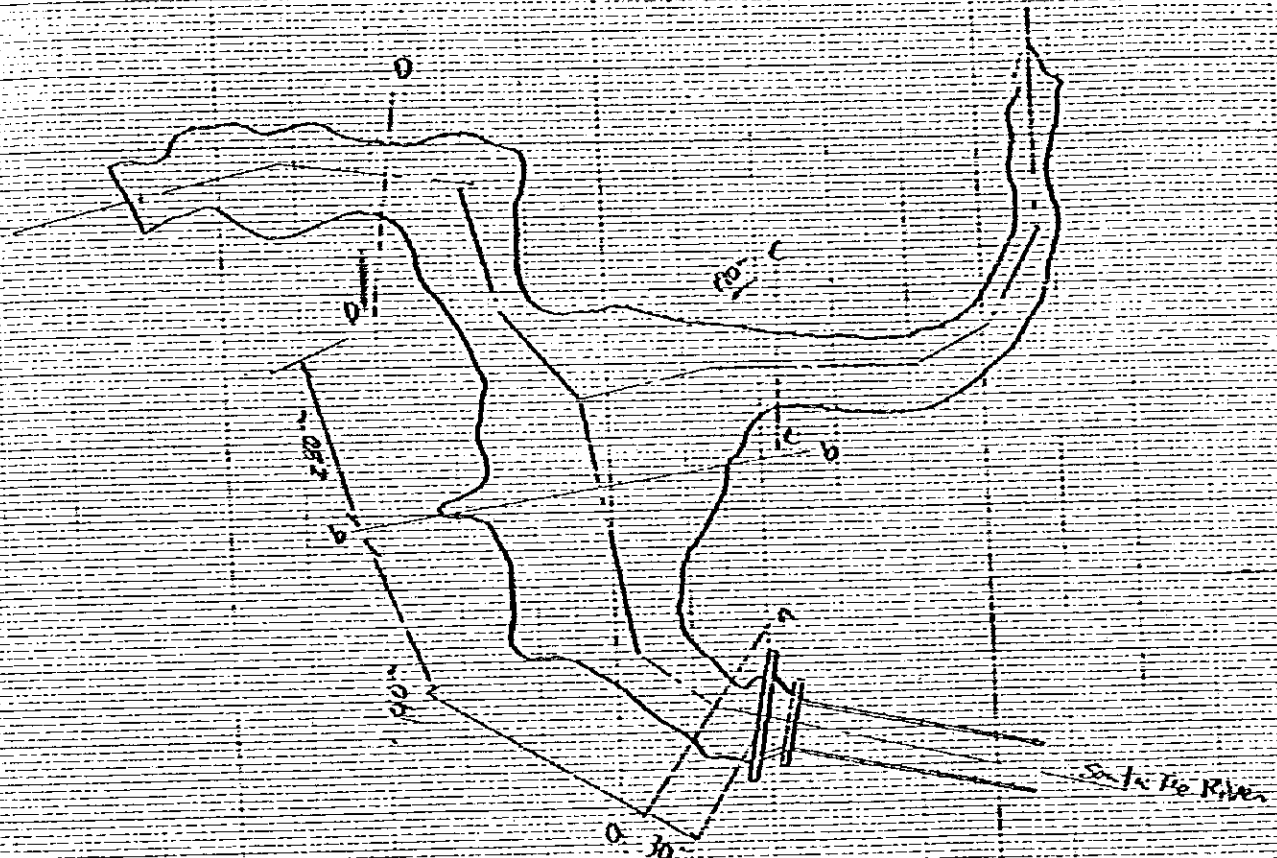
SFC 2

STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
209+636		0			0			
+700	64.0	0.5	CS	16.0	15.0		480.0	
+750	50.0	0	-	12.5	72.0		2,175.0	
+800	50.0				80.0		3,800.0	
+815	15.0				80.0		1,200.0	
				Br No. 10 L	30.0			
+845					80.0			
+900	55.0	0			40.0		3,300.0	
210+000	100.0	20.0		1,000.0	10.0		2,500.0	
+165	65.0	0		650.0	10.0		650.0	
				1678.5			14105.0	
				SEGMENT :	TOTAL		56,137.5	

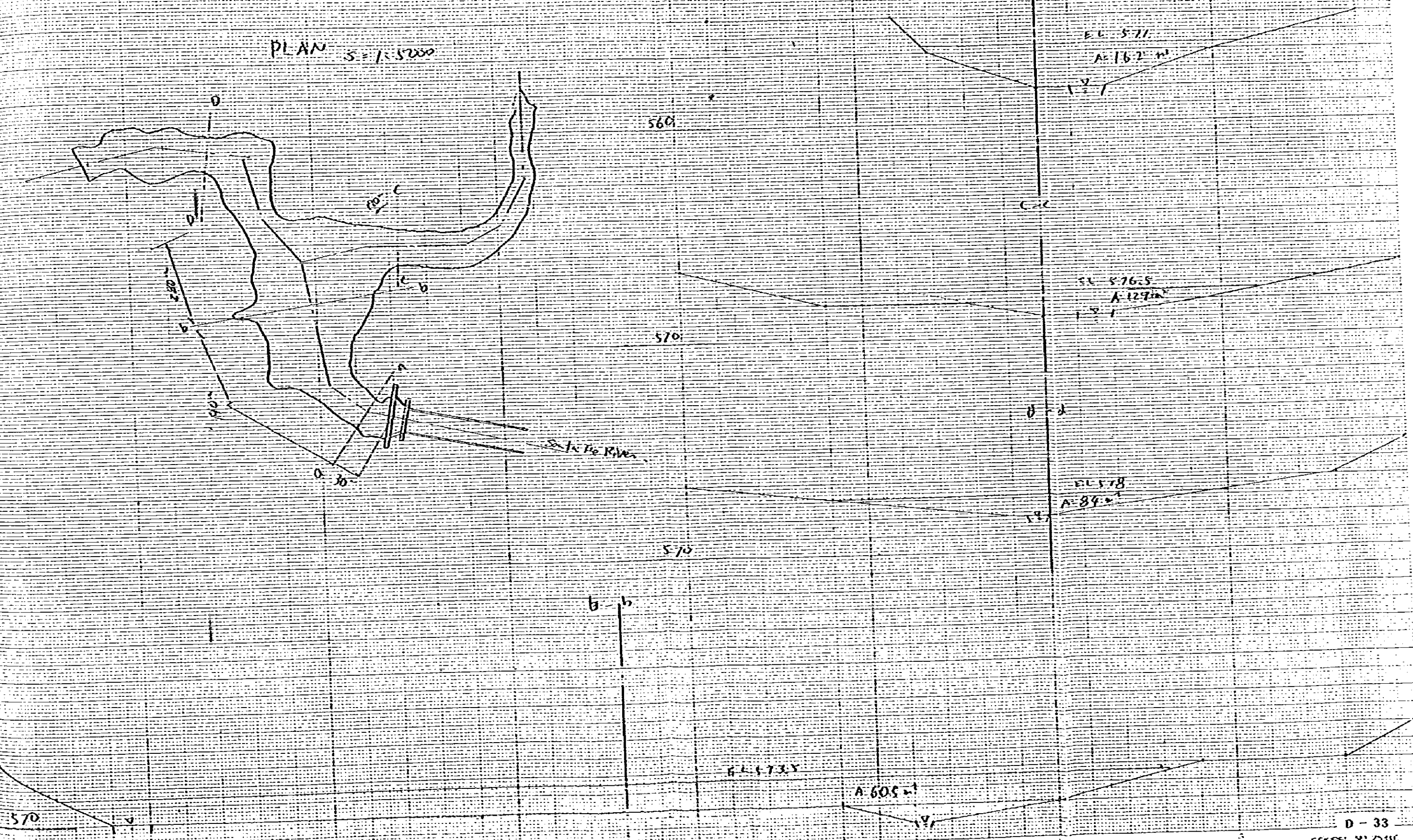
Deposition Area for San Joaquin River

$A = 70000 \text{ m}^2$

PLAN $S = 1:5000$



CROSS SECTION $S = 1:500$



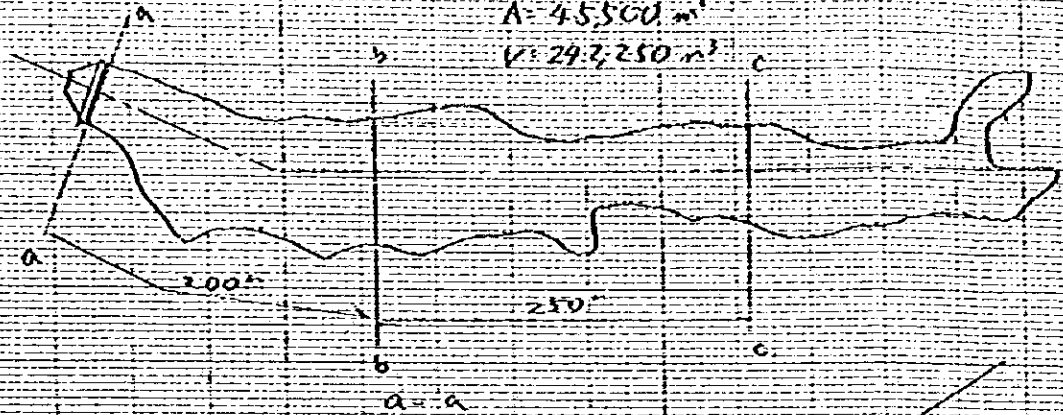
STA.	LENGTH	CUT			FILL			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
No. 1 SARO DAM 0					420.0			
+ 200	200.0				540.0		96000.0	
+ 450	250.0				350.0		111250.0	
+ 650	200.0				0		35000.0	
						Sub Total	242350.0	
No. 2 0					270.0			
+ 050	50.0				110.0		9500.0	
+ 190	140.0				5.0		8050.0	
+ 260	70.0				0		175.0	
						Sub Total	12725.0	
No. 3								
							2360.0	
No. 4							8200.0	
No. 5							6000.0	
								Total 277835.0

SOUTH SIDE

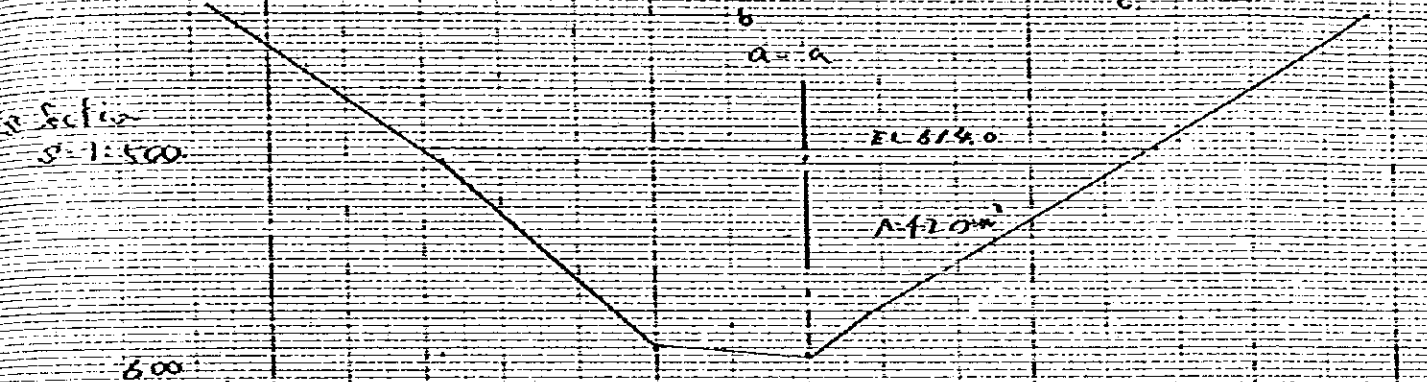
NO. 1 SABO DAM

$A = 45,500 \text{ m}^2$
 $V = 29,7250 \text{ m}^3$

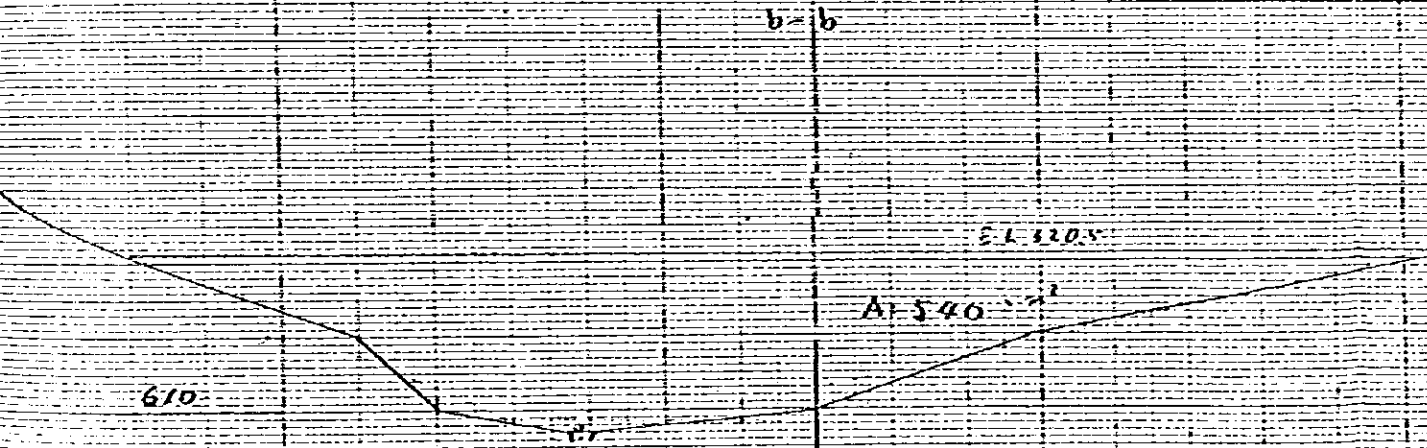
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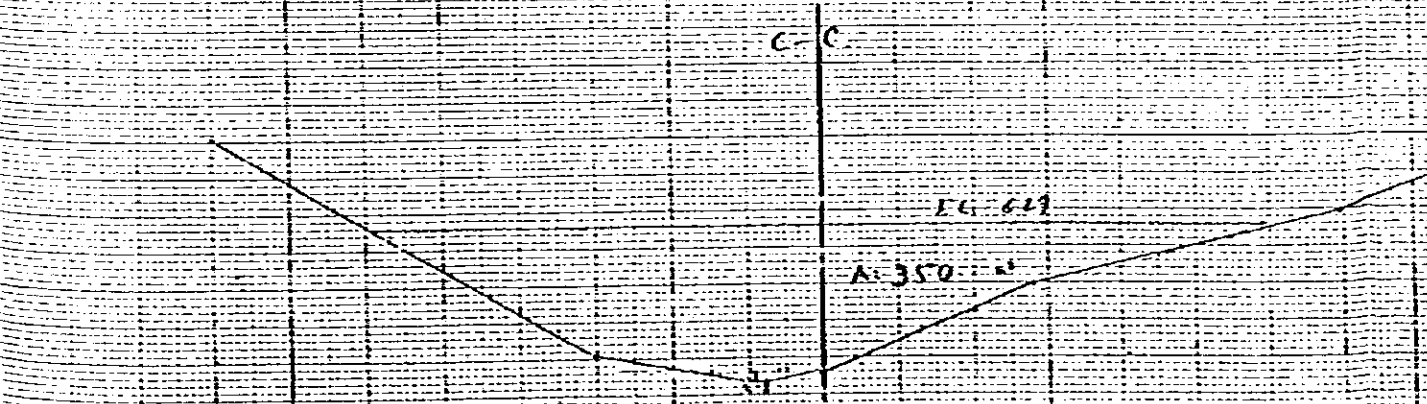
Section S: 1:500



600



610

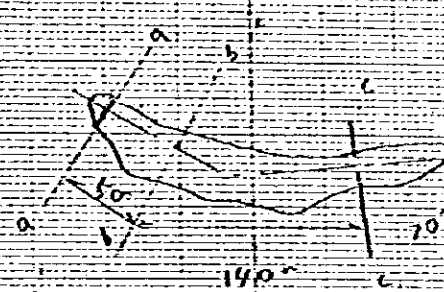


610

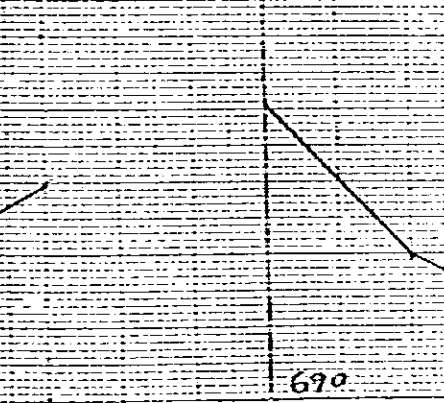
NO. 2 SABO DAM

$A = 9000 \text{ m}^2$

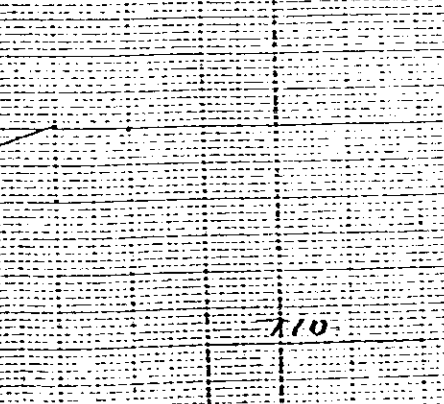
a-a See Cross section KM 205+300
 b-b See " KM 205+700
 c-c See " KM 205+500
 $V = 17725.0 \text{ m}^3$



Section S: 1:500



690

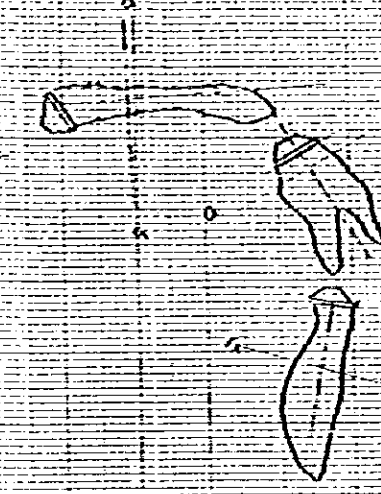


710

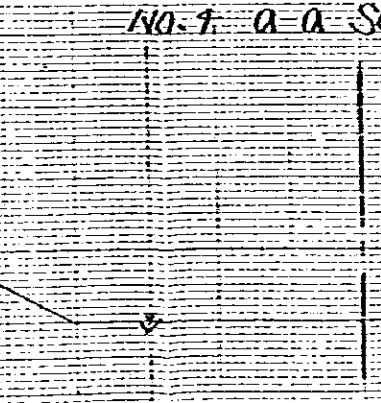
NO. 3 SABO DAM

$A = 3700 \text{ m}^2$

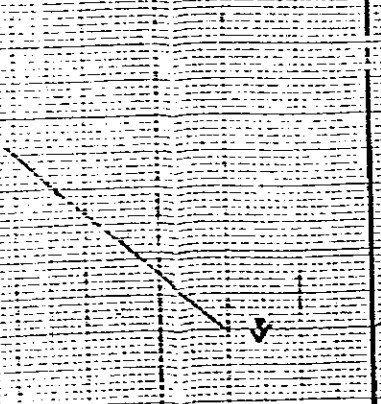
a-a See Cross Section KM 205+700
 $V = 3360.0 \text{ m}^3$



Section S: 1:500



690



710

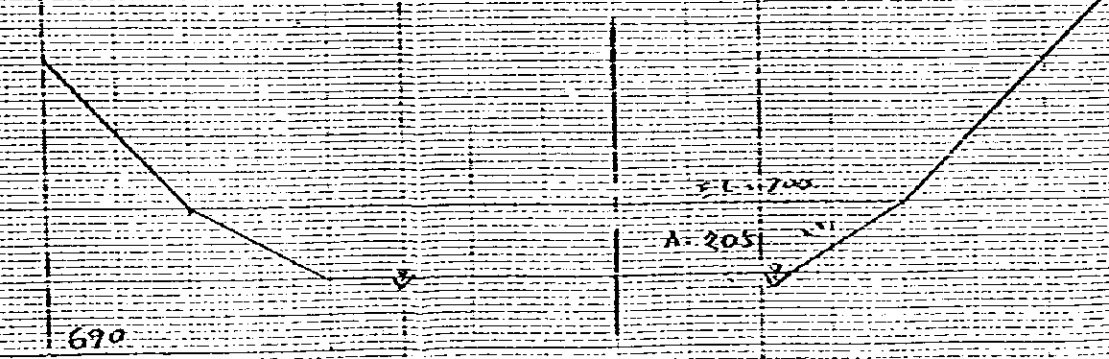
NO. 4 SABO DAM

$A = 7800 \text{ m}^2$
 $V = 8200 \text{ m}^3$

NO. 5 SABO DAM

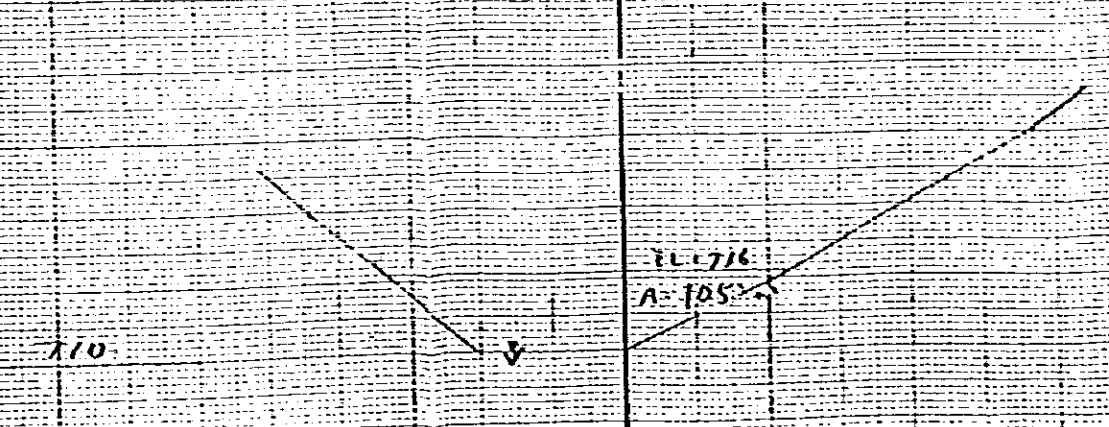
$A = 6000 \text{ m}^2$
 $V = 6300 \text{ m}^3$

NO. 4 a-a Section



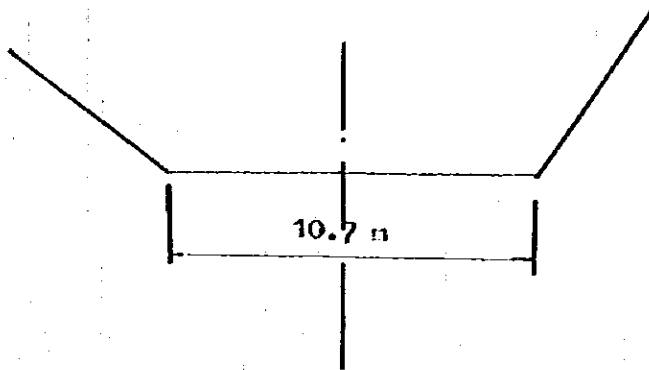
690

NO. 5 a-a Section



710

3) Compaction of Cut Section



SEG. 2	$1505.0\text{m} \times 10.7\text{m} = 16,103.5\text{m}^2$
3	$55.25 \times 10.7 = \frac{10,221.2\text{m}^2}{26,324.7\text{m}^2}$
Total	

2. Slope Protection

ITBI \ SEGMENT	1	2	3	4	Total
For Embankment					
1) Placing Topsoil (Sq.m)	4,316.5	9,603.3	5,100.8	3,077.5	28,898.1
Seeding (Sq.m)	4,316.5	9,603.3	5,100.8	3,077.5	28,898.1
For Cut					
2) Planting Work and Vegetation (Sq.m)	3,250.0	15,125.0	3,730.0	21,380.5	43,485.5
W = 3.0m (L.H)	1,083.0	5,042.0	1,243.0	7,130.0	14,498.0
3) Concrete Spraying (Sq.m)	6,108.0	4,665.0	6,216.0	228.0	17,517.0
Netting (Sq.m)	6,108.0	4,665.0	6,216.0	228.0	17,517.0
4) Precast Concrete Prean (Sq.m)	0	2,310.0	1,470.0	0	3,780.0

1) Slope protection for embankment (Placing Topsoil)

SEGMENT 1

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
KM 202 + 500		0			0			
+ 800	300.0	1.0		150.0	0			
+ 900	100.0	2.5		175.0	2.5		125.0	
+ 950	50.0	2.5		125.0	2.5		125.0	
				Br No. 1	6:220.0			
203 + 170		5.0			11.5			
+ 250	80.0	5.0		400.0	11.5		920.0	
+ 310	60.0	0		150.0	0		345.0	
+ 610		0			0			
+ 657.5	47.5	5.0		119.0	11.5		273.0	
				Br No. 2	6:90.0			
+ 747.5		5.0			11.5			
+ 790	42.5	5.0		213.0	11.5		489.0	

Slope protection For Embankment (Placing Topsoil)

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
203 + 790		5.0			11.5			
+ 800	10.0	0		25.0	5.0		82.5	
+ 900	100.0	0			0		250.0	
+ 950	50.0	0			7.0		175.0	
204 + 000	50.0	0			0		175.0	
	Sub Total			(13570)			(2959.5)	43/6.5 m ²
+ 450		0			0			
+ 460	10.0	0			4.0		20.0	
+ 500	40.0	0			4.0		160.0	
+ 600	100.0	0			10.0		700.0	
+ 700	100.0	0			23.0		1,650.0	
+ 750	50.0	0			24.5		1,188.0	
+ 850	100.0				28.0		2,625.0	
+ 900	50.0				28.0		1,400.0	
205 + 000	100.0	0			0		1,400.0	

SEG 1

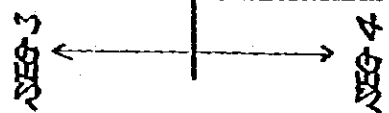
SEG 2

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205 +152.5		0			0			
+200	47.5	1.0		24.0	7.0	166.0		
+230	30.0	0		15.0	0	105.0		
205 +300								
+350	50.0							
+400	50.0	0			0			
+500	100.0	0			1.5	75.0		
+600	100.0	0			0	75.0		
				SEG 2 (39.0)		(9564.3)	96033 m ²	
207 +830		0			0			
+877.5	47.5	5.0		118.0	5.0	119.0		
				8.0	0			
+925.5		0			0			

NEG-2

NEG-3

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
207 + 922.5		0			0			
+ 950	27.5	5.0		69.0	5.0	69.0		
208 + 000	50.0	0		125.0	0	125.0		
+ 200		0			0			
+ 300	100.0	31.0		1550.0	6.0	300.0		
+ 350	50.0	31.0		1550.0	6.0	300.0		
+ 400	50.0	0		775.0	0	150.0		
+ 420	20.0	19.0		190.0	7.0	70.0		
+ 450	30.0	0		285.0	0	105.0	59008 m ²	
				Sub Total (4662.8)		1238.0		
				By No. 6				
+ 498								
+ 550	52.0	0			0			
+ 600	50.0	0			0			



STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
208 + 600								
+ 650	50.0	0			0			
+ 750	100.0	0			0			
+ 845	95.0	0			0			
				Br 140.7				
+ 950		0			0			
209 + 000	50.0	0			0			
+ 100	100.0	0			0			
+ 190		0			0			
+ 200	10.0	36.0		180.0	2.0	10.0		
+ 240	20.0	0		360.0	0	20.0		

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
209 + 310		0			0			
+350	40.0	24.0		480.0	11.0		220.0	
+400	50.0	26.0		1,250.0	2.0		325.0	
+481	81.0	36.0		2,511.0	6.0		324.0	
209 + 636		6.0			13.0			
+700	64.0	0		192.0	16.0		928.0	
+750	50.0	0			5.0		525.0	
+800	50.0	5.0		125.0	1.0		150.0	
+815	15.0	5.0		75.0	1.0		15.0	
+845		5.0	Br	110.10				
+900	55.0	2.0		192.5	1.0		55.0	
210 + 000	100.0	0		100.0	12.0		650.0	
+165	65.0	0			0		390.0	

TOTAL 11,524.5
 5164 (5965.5)

12323 +
 (2612.0)

9077.5 A.S.
 = 2,298.1 m.

2) Slope protection For Cut (Planting Work and Vegetation) 50%

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
203+310		0			0			
+400	90.0	10.0		450.0	7.0		315.0	
+540	140.0	0		700.0	0		490.0	
+580	40.0	36.0		720.0	1.0		20.0	
+610	30.0	0		540.0	0		15.0	
			Total	(2,410.0)			(890.0)	3250.0m
204+460		0			0			
+500	40.0	10.0		200.0	0			
+600	100.0	10.0		1,000.0	0			
+700	100.0	9.0		950.0	0			
+750	50.0	2.0		275.0	0			
+850	100.0	11.0		650.0	0			
+900	50.0	0		275.0	0			
205+000	100.0	40.0		2,000.0	0			
+315	37.5	0		750.0	0			

NEG 1

NEG 2

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205+350		0			0			
+400	50.0	14.0		275.0	2.0		50.0	
+500	100.0	8.0		950.0	0		100.0	
+600	100.0	12.0		2,000.0	8.0		400.0	
+700	100.0	19.0		1,550.0	0		400.0	
+800	100.0	20.0		1,950.0	3.0		150.0	
+850	30.0	39.0		885.0	18.0		315.0	
			5467	(13,710.0)			(1415.0)	
							15,125.0	

SEG 3

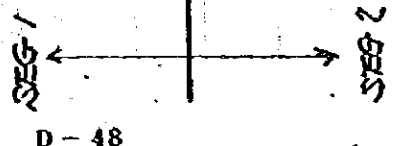
SEG 4

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
207+690		24.0			25.0			
+750	60.0	0		1320.0	12.0		1110.0	
+800	50.0	0			0		300.0	
+830	30.0	0			0			
+877.5	47.5	0			0			
208+300		0			0			
+350	50.0	10.0		250.0	10.0		250.0	
+400	50.0	0		250.0	0		250.0	
				704.1 (1830.0)			(1910.0)	3,730.0
208+498		0			0			
+550	52.0	0			27.0		702.0	
+600	50.0	0			60.0		2,175.0	
+650	50.0	7.0		175.0	60.0		3,000.0	
+750	100.0	0		350.0	22.0		4,100.0	
+845	95.0	0			20.0		1,995.0	

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
208 + 899		0			0			
+ 950	51.0	0			65.0		1657.5	
209 + 1000	50.0	0			20.0		2125.0	
+ 100	100.0	9.0		450.0	8.0		1,400.0	
+ 130	30.0	0		135.0	0		1,200.0	
209 + 240		0			0			
+ 270	30.0	5.0		75	50.0		750.0	
+ 310	40.0	0		100	0		1,000.0	
			559.4	(1,285.0)			(20,104.5)	21389.5 m ³
			Total	19,225.0			24,269.5	43,494.5 m ³

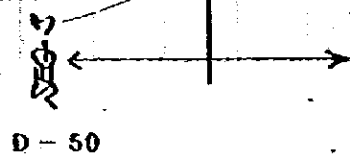
S) Slope protection For Cut (Concrete Spraying and Netting)

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
203+790		0			0			
+800	10.0	4.0		20.0	0			
+900	100.0	0		200.0	0			
+950	50.0	18.0		450.0	0			
204+000	50.0	10.0		700.0	25.0	625.0		
+100	100.0	10.0		1000.0	25.0	2500.0		
+135	35.0	0		175.0	0	438.0		
			70.1-1	(2,545.0)		(3,563.0)		6/08.0
204+225		0			0			
+300	75.0	28.0		1,050.0	2.0	75.0		
+400	100.0	8.0		1,800.0	0	100.0		
+450	50.0	0		200.0	0			



STA.	LENGTH	Left				Right				REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME			
204 + 460		0	↑	↑	0	↑	↑			
+ 500	40.0									
+ 600	100.0	0			0					
+ 700	100.0									
+ 750	50.0									
+ 850	100.0									
+ 900	50.0	0			0					
205 + 000	100.0									
+ 007.5	37.5	0	↓	↓	0	↓	↓			
205 + 230		0			0					
+ 300	70.0	29.0		1015.0	0					
+ 350	50.0	0		725.0	0					
		SEG Z		4790.0				(= 175.0)	4765.0	

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
207+922.5		0			0			
+950	27.5	0			23.0		316.0	
208+000	50.0	0			23.0		1150.0	
+100	100.0	5.0		250.0	31.0		2700.0	
+200	100.0	0		250.0	0		1550.0	
+300	100.0	0			0			
+350	50.0	0						
+400	50.0	0			0			
			Total	(500.0)			(5,716.0)	
209+636		0			0			
+700	64.0	4.0		128.0	0			
+750	50.0	0		100.0	0			
209+900		0			0			
210+000	100.0	0			0			
-145	65.0	0			0			
			Total	(228.0)			(0)	
			Total	8,063.0			9,454.0	
							17,517.0	



4) Precast Concrete Form

STA.	LENGTH	Left			Right			REMARKS
		AREA	AVE. AREA	VOLUME	AREA	AVE. AREA	VOLUME	
205+815		20.0			3.0			
+830	15.0	39.0		442.5	18.0		157.5	
+850	20.0	39.0			18.0			
+860	10.0	39.0		1170.0	18.0		540.0	
				SEGMENT 2		Total	2310.0 m ³	
207+730		24.0			25.0			
+720	10.0	24.0			25.0			
+700	20.0	24.0		720.0	25.0		750	
				SEGMENT 3		Total	1470.0 m ³	
						Total	3780.0 m ³	

3. MINOR STRUCTURES

1) Retaining Wall

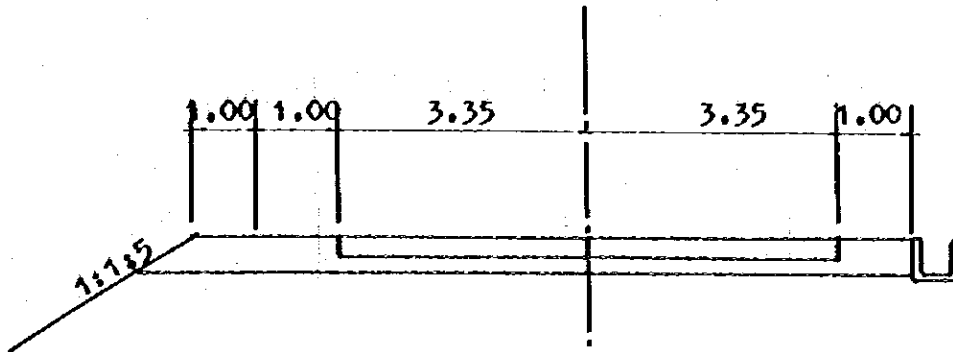
(m)

STA.	Height	H = 4.0m	H = 7.0m	H = 8.0m	
Ki 201 + 450 to Ki 201 + 510				60.0	
205 + 155 to 205 + 200		45.0			
Sub Total SEGMENT 2		45.0		60.0	
Ki 203 + 510 to Ki 203 + 630			120.0		
203 + 685 to 203 + 710			25.0		
203 + 735 to 203 + 780			55.0		
203 + 800 to 203 + 845			45.0		
203 + 880 to 203 + 950				70.0	
Sub Total SEGMENT 4			245.0	70.0	
Total		45.0m	245.0m	130.0m	

2) STONE MASONRY

STA.	Height	Height					
		H=2.0m	H=3.0m	H=4.0m	H=5.0m	H=7.0m	H=7.0m x 2
	KH 202 + 190 to KH 203 + 100						
SEG. 2	KH 203 + 170 to KH 203 + 330 (160 m ³)		100.0 (300m ³)	160.0 (640.0m ³)			
	204 + 510 to 204 + 590			20.0 (320 m ³)			
SEG. 3	204 + 720 to 205 + 000 205 + 450 to 205 + 300	350.0				280.0	
	Sub Total SEGMENT 3 (2,580m ³)	350.0 (700m ³)		240.0 (960m ³)		280.0 (1,960m ³)	
	KH 208 + 500 to KH 208 + 800		300.0				
	208 + 900 to 209 + 040						140.0
	209 + 461 to 209 + 481				45.0		
SEG. 4	209 + 750 to 209 + 815			65.0			
	209 + 845 to 209 + 945			100.0			
	209 + 950 to 210 + 050			100.0			
	Sub Total SEGMENT 4 (4,145m ³)		300.0 (1,000m ³)	265.0 (1,060m ³)	45.0 (225m ³)		140.0 (1,560m ³)
	Total (8,065m ³)	350.0m (700m ³)	400.0m (1,200m)	505.0m (2,020m ³)	45.0m (225m ³)	280.0m (1,960m ³)	140.0m (1,560m ³)

4. PAVEMENT



1) PORTLAND CEMENT CONCRETE PAVEMENT 0.23 m

$$1\text{ m} \times 6.70\text{ m} = 6.70\text{ m}^2/\text{m}$$

2) CRUSHED GRAVEL BASE COURSE 0.20 m

$$1\text{ m} \times 6.70\text{ m} \times 0.20\text{ m} = 1.34\text{ m}^3/\text{m}$$

$$1\text{ m} \times 1.00\text{ m} \times 0.43\text{ m} = 0.43\text{ m}^3/\text{m}$$

$$1\text{ m} \times (2.00 + 2.645)\text{ m} \times 0.43\text{ m} + 2 = \frac{1.00\text{ m}^3/\text{m}}{2.77\text{ m}^3/\text{m}}$$

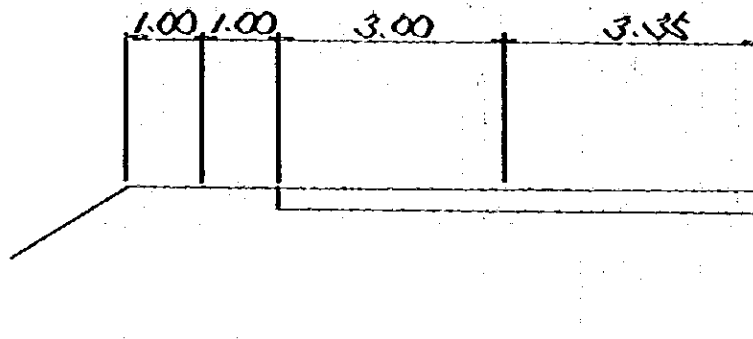
3) Prime Coat

$$1\text{ m} \times 7.16\text{ m}$$

No.	Unit Cost P	Quantity	P/m Cost	Cost 000 P/m.
1)	161.38	6.70 m ²	1,081 P/m	
2)	115.43	2.77 m ³	318 P/m	
3)	4.03	7.16 m ²	29 P/m	
			<u>1,428 P/m</u>	1,428 P/m

CLIMBING LANE

CLIMBING LANE



1. Earth Work Fill : $L = 100^m \times 5^m^2$ Cut $L = 100^m \times 8^m^2$

$$L = 160 \times 48^m^2$$

$$8180^m^3$$

$$800^m^3$$

$$193.0'000 \text{ P}$$

$$34.1 \text{ P}$$

$$27.0'000 \text{ P}$$

$$\text{Total } 220.0'000 \text{ P}$$

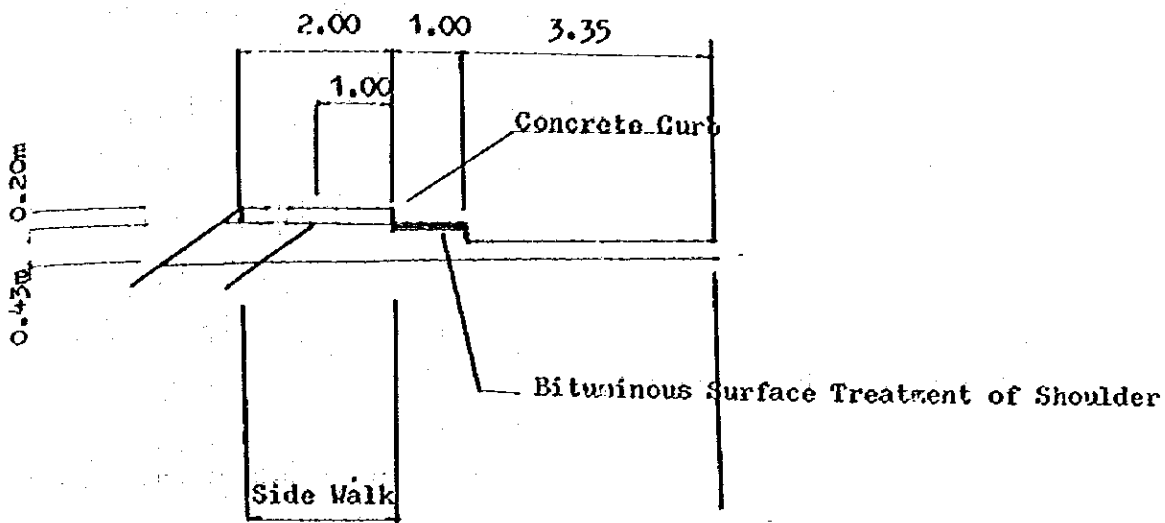
2. Pavement $L = 360^m$

$$161.38 \text{ P}$$

$$58.0 \text{ P}$$

$$\text{Total } 278'000 \text{ P}$$

Side Walk



- 1) Bituminous Surface Treatment of Shoulder $1.0 \times 0.1 = 0.1 \text{ m}^3/\text{m}$
- 2) Concrete Curb $1 \text{ m}/\text{m}$
- 3) CRUSHED GRAVEL BASE COURSE
 - 0.20×2.0 0.4
 - 0.43×1.0 0.43 $0.83 \text{ m}^3/\text{m}$

No.	Unit Cost	Quantity	Cost	Remarks
1	218.81 P/m ³	0.1 m ³ /m	21.9 P/m	
2	52.18 P/m	1 m/m	52.2 P/m	
3	115.43 P/m ³	0.83 m ³ /m	69.8 P/m	
			167.9 P/m	

Length of Side Walks in Santa Fe: 170.0 m
 210.0 m
 Total 380.0 m

5. DRAINAGE

1) Concrete Side Ditch $W = 0.5m$

SEGMENT 1.	$650 + 605 = 1,265.0$	
SEGMENT 2.	$1,412.5 + 332.5 = 1,745.0$	(Length of Cut Area)
SEGMENT 3.	$250.0 + 567.5 = 817.5$	
SEGMENT 4.	$425.0 + 667.0 = 1,092.0$	
Total	$= 4,919.5$	(Length of Cut Area)

2) Sub - Drainage

SEGMENT 1	1,265.0
SEGMENT 2	1,745.0
SEGMENT 3	817.5
SEGMENT 4	1,092.0
Total	4,919.5

3) Reinforced Concrete Pipe Culvert

(a)

STA.	Diameter	Ø 1.20 m	Remarks
KM 207 + 850		12.0	
203 + 200		30.0	
203 + 100		12.0	
SUB TOTAL SEGMENT 1		54.0	
204 + 460		25.0	
204 + 660		30.0	
204 + 800		33.0	
205 + 360		12.0	
205 + 550		17.0	
SUB TOTAL SEGMENT 2		117.0	
KM 207 + 150		35.0	
203 + 280		30.0	
203 + 420		32.0	
SUB TOTAL SEGMENT 3		97.0	
203 + 600		9.0	
203 + 150		10.0	
207 + 210		40.0	
210 + 120		10.0	
SUB TOTAL SEGMENT 4		69.0	
TOTAL		332.0 m	

4) Reinforced Concrete Box Culvert

STA.	Size	2.0 x 2.0	Remarks
KM. 207 + 365		39.0	
SEGMENT 4		39.0 m	
TOTAL		39.0 m	

6. TUNNEL

ITEM	STAGE	FIRST	SECOND	LAST
Construction Cost for Tunnel		P 173,959,000		
Traffic Safety and Control Facilities		50,860,000		
1) Lighting facilities		3,310,000		
2) Fire Preventing facilities		6,010,000		
3) Received Power supply		(13,000,000)		
4) Distribution Line		13,140,000	1,567,000	1,960,000
5) Ventilation Facilities		15,400,000	10,213,000	69,867,000
Access Road				1,600,000
Air Shaft				40,531,000
Ventilating Station				22,936,000
Fan				
Jet Fan		11,240,000	9,636,000	
Electrical Facilities		4,160,000	577,000	4,800,000
Total Cost		224,819,000	11,780,000	71,827,000

7. BRIDGE

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SBB.	STA.	NAME	TYPE	L = 50		L = 50	
				LENGTH (c)	DIRECT COST	LENGTH (c)	DIRECT COST
1	KH						
	202 + 560	S.D.P. No. 1	POG			220.0	5,355.5
	203 + 702.5	No. 2	POG			50.0	2,130.8
	204 + 180.0	No. 3	POG			50.0	2,140.8
		Sub Total				(400.0)	(9,627.1)
2	205 + 55.0	No. 4	BCDG POG			(115.0)	(2,484.5)
3	207 + 500.0	No. 5	BCDG	(45.0)	(563.2)		
4	208 + 474.0	No. 6	POG BCDG	48.0	1,035.3		
	208 + 872.0	No. 7	POG			55.0	1,655.7
	209 + 160.0	No. 8	POG			60.0	1,811.4
	209 + 553.5	No. 9	POG			155.0	3,570.9
	209 + 830.0	No. 10	BCDG	30.0	596.9		
	216 + 400.0	Santa Fe	POG	30.0	814.8		
		Sub Total		(108.0)	(2,447.0)	(270.0)	(7,078.0)
			TOTAL	153.0	3,410.2	785.0	10,189.0