UNITED NATIONS

COMMITTEE FOR COORDINATION OF INVESTIGATIONS OF THE LOWER MEKONG BASIN

NONG KHAI/VIENTIANE BRIDGE PROJECT

LAOS AND THAILAND

EXAMINATION REPORT

OF THE REVISED

CONSTRUCTION COST ESTIMATE

OVERSEAS TECHNICAL COOPERATION AGENCY

Japan, July 1974



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PREFACE

The Japanease Government, in compliance with the request made by the Mekong Committee (MP/A.2271 TEC 322 (4-17) dated November 30, 1972) in connection with the Nongkhai/Vientiane Bridge Project, agreed to render technical cooperation for the investigation of the Project and entrusted the Overseas technical Cooperation Agency (hereinafter referred to as "OTCA").

The OTCA, in recognition of the importance of the Project in establishing a firm foundation for the economic and social development of both Thailand and Laos, organized a survey team of four Japanese experts head by Mr. Nobuichi Nomoto of the Ministry of Construction. The survey team was despatched to Thailand and Laos for a period of fifteen days from November 18, 1973. The field investigation was carried out successfully with the helpful cooperation given by officials concerned of the Mekong Committee and the Governments of Thailand and Laos.

In March 1974, the survey team submitted to the Mekong Committee the Draft Report containing the results of the examination on the revised construction cost estimate of the Scaled-down Plan for this project received from the Mekong Committee, based on the report concerned with the Nongkhai/Vientiane Bridge Project previously submitted to the Mekong Committee in September 1969 by the OTCA. In addition, a cost estimate regarding the construction of an alternative concrete bridge under the Scaled-down Plan prepared by us was also attached to the report.

This report is to finalize the above-mentioned draft report in accordance with the decision by the Mekong Committee, and there have been no fundamental change to the conclusions reached in the draft report. We sincerely hope that this report will be useful for use as reference in the further study so as to realize this project as soon as possible.

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ACKNOWLEDGEMENTS

The survey team wishes to hereby express deep gratitude to the members of both Laotian and Thai Governments as well as the Mekong Committee who have extended kind cooperation and assistance and provided valuable data to the team, particularly to;

Laotian Government

Mr. Phak Savann Directeur Général, Ministère des

Travaux Publics et des Transports

Mr. Saykham Directeur des Ponts & Chaussées Ministère des Travaux Publics et

des Transports

Mr. Sisonphanh Choummanivong Secrétaire Exéctif du Comité National

Lao du Mékong

Mr. Sounthra Phandanouvong Ingénieur an Service Technique

Thai Government

Mr. Nitipat Jalichan Secretary General

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Mr. Vibul Taweesup Chief of the Mekong River Project

National Energy Administration

Mr. Prasan Vandhanakom Liaison Engineer

National Energy Administration

Mr. Sompongse Chantavorapap Design Engineer

National Energy Administration

Mekong Secretariat

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Division of Engineering Services

Mr. Phlek Chhat Director

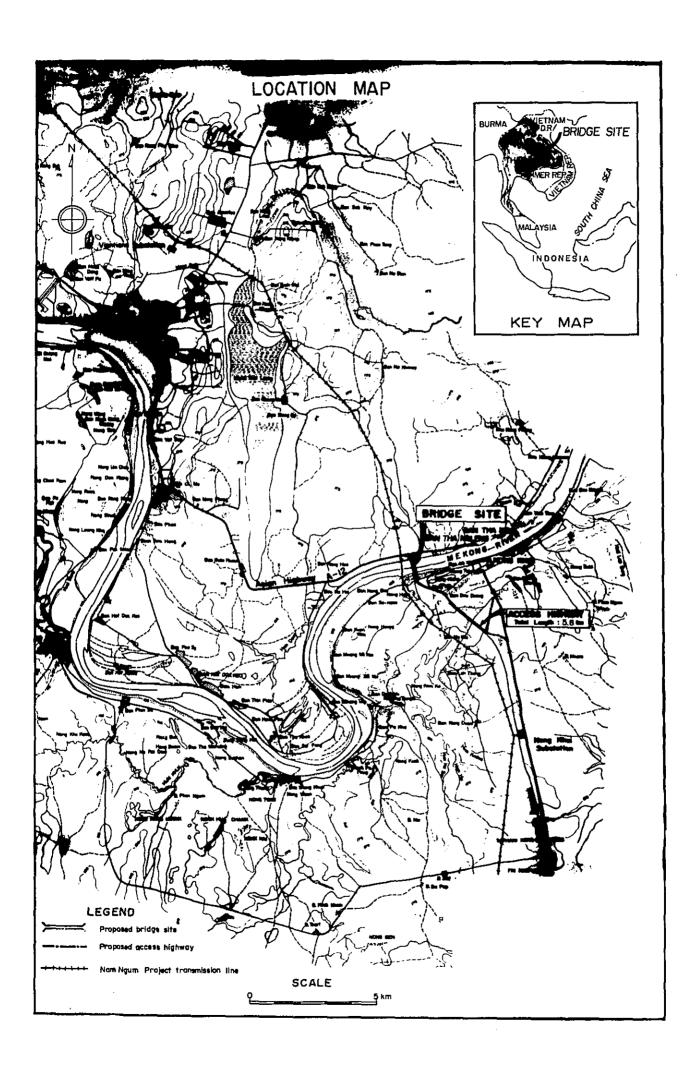
Division of Navigation Improvement

Mr. Somphavan Inthavong Project Planning Engineer

Division of Engineering Services

Mr. Khamsing Luanglath Engineer

Division of Navigation Improvement



OUTLINE OF SCALED-DOWN PLAN

Item Description I. PROJECT 1. Location 600 kilometers northeast of Bangkok, 20 kilometers southeast of Vientiane and 3 kilometers upstream of Nong Khai 2. Purpose To build, across the Mekong, either a rail/ highway bridge of which railway track can be constructed in future or a highway bridge, including all the highway facility. II. BRIDGE 1. Type (i) Main bridge 9-span steel Warren truss continuous over 2 and 3 spans. (ii) Approach viaduct Precast PC box girder 2. Bridge width 17.8 m (i) Highway (ii) Sidewalk 8.0 m 1.5 m 3. Bridge length (i) Main bridge 650 m (ii) Approach viaduct 2x161.3 m 4. Max. pier spacing 90 m 5. Abutment and pier 2 open caissons on both banks, and 8 fundation pneumatic caissons in river channel III. HIGHWAY 1. Access highway Length 5.8 km Width (i) Roadway 7 m (two lanes) (ii) Shoulders 2.5 m each

48.000 m²

IV.

ADMINISTRATIVE FACILITIES

NONG KHAI/VIENTIANE BRIDGE PROJECT

EXAMINATION REPORT OF THE REVISED CONSTRUCTION COST ESTIMATE

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EXAMINATION REPORT OF THE REVISED CONSTRUCTION COST ESTIMATE

\$1 SUMMARY

This report contains the results of an examination of the revised construction cost estimate made by the Mekong Committee on the scaled-down plans for the Nongkhai/Vientiane Bridge Project. The scaled-down plans consist of the Case I (the rail/highway bridge plan under which a highway bridge be initially constructed) and the Case II (the highway bridge plan). Main structure in both cases is a 650 m. long multispan steel-truss bridge.

In November 1973, the Japanese governmental survey team carried out the field investigations in Thailand and Laos to examine the revised cost. After returning to Japan the survey team examined and analyzed the data collected and then newly estimated the construction cost. Both the costs revised before and estimated newly are summarized in Table 1.1. The total construction cost of Case I is nearly US\$22.0 million though the previously revised cost is about US\$9.8 million, and that of Case II is about US\$17.1 million though previously revised to US\$7.8 million.

These great differences are mainly due to 1) the world-wide price escalation since 1972 when the revised estimate was made, for example, the material price of steel became twice in these two years, and 2) the differences of basic standards or criteria applied at this time and at revised estimate.

Table 1.1 Comparative Construction Cost of Scaled-down Plan

L			丄														
		Cause of difference		Diesel generators added				Price of truss member rose	to 2.5 times Price of steel material	rote to 20 times - do -	Price of asphalt rose to	2 times Frice of timber & asphalt rose to 2 times	Direct cost rose as above.	- op -	- op (- ap -	
		Difference	343,000	343,000	٥	6,222,054	5,001,697	3,320,955	1,680,742	353,209	259,225	607,923	394,183	840,273	977,458	528,218	9,305,186
	Case II	Estimated newly	719,000	494,000	225,000	11,443,000	7,873,000	4,773,000	3,100,000	584,000	1,216,000	1,770,000	770,000	1,431,000	1,817,000	970,000	17,110,000
Construction Coat (US\$		Revised before	376,000	151,000	225,000	5,220,946	2,871,303	1,452,045	1,419,258	162,052	956,775	1,162,077	335,817	590,727	859,542	441,782	7,804,814
Construct		Difference	343,000	343,000	0	8,181,936	6,961,579	4,758,085	2,203,494	355,209	259,225	607,923	508,136	1,130,155	1,279,341	683,354	12,125,922
	C889 1	Estimated newly	1,079,000	494,000	585,000	14,477,000	10,907,000	6,829,000	4,078,000	584,000	1,216,000	1,770,000	930,000	1,900,000	2,334,000	1,240,000	21,960,000
		Revised	736,000	151,000	585,000	6,295,064	3,945,421	2,070,915	1,874,506	230,791	956,775	1,162,077	421,864	769,845	1,054,659	556,646	9,634,078
	; 1	M 44 00 8	COVERIMENTS' PREPABATORY WORKS	l. Construction facilities	2. Land and rights	HAIR CONSTRUCTION WORKS	l. Main truss bridge	(1) Superstructure	(11) Substructure	2. Approach wisducts	3. Highway	4. Administrative facilities	. Covernments' administrative expense	engineering service	CONTINCENCY AND RESERVE	Interest during construction	TOTAL
		l	H			Ħ							III.	Ė	Α.	VI.	

\$2. INTRODUCTION

2.1 Outline of the Scaled-down Plan

The project site is located on 20 kilometers southeast of Vientiane and 3 kilometers upstream of Nongkhai.

In 1972, the Mekong Committee prepared two scaled-down plans for this Project and revised the construction cost to reduce the initial investment thereof. The scaled-down plans consisted of Case I and Case II. The Case I is the plan of constructing a dual-purpose rail/highway bridge under which only the highway bridge and related highway facilities will initially be constructed to be provided for the future railway extension. The Case II is the plan of constructing a single-purpose highway bridge.

Each Case comprises four major parts; a main multispan steel-truss bridge with a length of 650 meters sustained by caisson foundations, 320-meter long approach viaducts of precast prestressed concrete girders, 5.8 kilometers long access highway and the administrative facilities in Thailand and Laos with a total area of 4.8 hectares. Width of the main truss bridge in the Case I will be 13.4 meters so as to provide both the highway floor and the space for future railway track on a through-truss bridge not separated though in the Case II 9.3 meters.

The Asian Highway Route A-12 which is now in service but interrupted by the Mekong will become motorable for its entire length upon completion of the bridge.

2.2 Examination Work

2.2.1 Scope of Work

OTCA, entrusted with the study of the scaled-down plan by the Japanese government, despatched a field survey team for 15 days from November 18, 1973.

The scope of work carried out by the survey team included; 1) gathering of necessary data and information for the cost estimate of the scaled-down plan, 2) gathering of opinions on this project from the officials concerned of the Laotian and Thai Governments as well as the Mekong Com-

mittee, 3) discussing in detail with the staff who actually made the revised estimate, 4) inspecting of the bridge construction site and 5) examining of the revised cost estimate after returning to Japan.

2.2.2 Field Survey Team

The field survey team is formed by the following four members.

Mr. Nobuichi NOMOTO Head of the Team.

Head of Second Road Planning Division, Chubu Regional Construction Bureau.

Ministry of Construction

Mr. Hirokazu ITO Highway Engineer,

(Registered Consulting Engineer) Chief of Highway Department,

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Mr. Tohru ITO Civil Engineer,

Project Planning Department,

Nippon Koei Co., Ltd.

Mr. Haruo SUZUKI Liaison and Accounting Officer,

Development Survey Department,

Overseas Technical Cooperation Agency

§3 EXAMINATION OF THE REVISED COST

3.1 Minor Modification of Design

The survey team, after returning to Japan, studied on the bridge design of the scaled-down plan in parallel with examination work of the revised cost estimate, taking into consideration the advanced bridge engineering and the recent circumstances of the countries concerned. As the result of study it was confirmed that no basic revision or change on the design received from the Mekong Committee was required, except on some minor points as mentioned below.

- The 30-meter long composite girder bridge forming a part of the approach viaduct is considered to be not so severe in designing the joint of different types of bridge structures such as steel-truss and concrete girder. So, it is changed to more economical bridge structure, that is, precast, prestressed-concrete, multi-box-girder bridge with pier spacing of 16 meters, as shown in the attached PLATE 5.
- Sodding on side slopes of the highway embankment is provided for slope protection instead of soil-cement mixed embankment. The embankment soil is sufficiently stable even when it is saturated with water, according to experiences at other highway embankments by the authorities concerned of the Thai government.
- Total building area of temporary facilities for construction use is reduced to 1,000 sq. meters from 1,500 sq. meters which would be required for the simultaneous construction of both railway and highway bridges.

3.2 Examination of the Unit Prices

Main effort of the survey team was concentrated on this examination of the unit prices which were used in the revised cost estimate received from the Mekong Committee. Based on the data collected in the field survey and the current material price records of Japan, the unit prices are newly estimated at this time. Wages of labors and prices of material

domestically available are assessed in terms of Thai Baht and Laotian Kip but exchanged to US dollar at the rates of 20 Bahts and 500 Kips a dollar respectinely. Structural steel of truss members and sheet piles, etc., and heavy construction equipment such as a cable crane, traveller cranes, emergency power plants, aircompressors and hospital lock are assessed in the term of US dollar. Price escalation after now (Mar. 1974) is not taken into account in this estimate, but the import, commodity and other taxes are included.

Followings are the major unit prices assessed at this time and comments on the prices used in the revised cost estimate.

1) Structural steel for truss member

The unit price of 525 US dollars per ton of steel material for bridge, which was used in the revised cost estimate, is considered to be too cheap even though it was assessed two years ago. Such price is applicable only to a steel material to be used for some light-loading structures. Truss members of bridge are made of special high grade steel from which the weight of truss members had been calculated and require complicated fabrication and welding by skilled technicians.

The unit price estimated at this time is as follows:

Items	Unit price per ton
	(US\$ equiv.)
Material including fabrication and painting:	1,000
Transportation:	130
Erection(including traveller and cable cranes and other equipment):	420
General expense	250
Total:	1,800

2) Concrete

Unit price of concrete in this estimate is based on the assumption that ready-mixed concrete with required qualities will be

purchased from a domestic company. The unit price of concrete, including forms, scaffoldings and placing equipment, vary according to the kind of particular structure. The estimated unit prices range from US\$70 to 160 per m³, for instance;

	Unit price	per m ³
Items	Caisson concrete (Cement 300 kg/m ³)	
		(US\$ equiv.)
Concrete mixed and carried to site:	45	35
Forms and scaffolding:	50	20
Labors:	7	2
Chutes, conveyors, bucket and other equipment:	32	3
General expense:	26	10
Total:	160	70

3) Other steel

The import price of steel material such as reinforcement bar and sheet pile became twice or more at Bangkok for these two years, and accordingly the present unit prices including erection and other related costs are as follows;

- Reinforcement bar

Items	Unit price per ton
	(US\$ equiv.)
Material	410
Inland transportation	10
Fabrication and erection	120
General expense	90
Total:	630

- Sheet pile

Items	Unit price per ton
	(US\$ equiv.)
Materials	450
Inland transportation	10
Erection and driving	170
General expense	120
Total;	750

4) Excavation for pier foundations

The previously revised estimate classified the excavation cost of pier foundations into three classes according to types of caisson and compositions of soil, sand, gravel and rock to be excavated. Unit price of upper layer excavation of river bed and bank was US\$7.5/m³ in the rivesed estimate. The price is, however, considered to be too cheap to excavate under-water layers by using the open caisson that could be changed to the pneumatic caisson after reaching to gravel layer or bed rock. Special measures to dewater or special machines workable in water will additionally be required and their costs will exceed the labor cost for pneumatic caisson. Furthermore, if the big amount of pneumatic equipment cost is allotted to a little excavation volume only of shale and gravel, the unit price of gravel and shale excavation will increase greatly compared with US\$30/m³ in the revised estimate.

In this estimate cost of the pneumatic equipment is allotted to whole volume to be excavated by the pneumatic caissons regardless of soil or rock. Hence, the unit prices of excavation by the pneumatic caisson and the open caisson are estimated at US\$35/m³ and US\$15/m³ respectively.

5) Power supply for construction use

The unit price per kilometer of the power distribution line from the Nongkhai substation to the bridge site was newly estimated and resulted in US\$6,000/km including the costs for material, equipment and wiring, which increased to three times from the revised

estimate. This is especially due to price escalation of material.

Substation equipment at the site, such as a 1,000 KVA transformer, cubicle and so on, will cost US\$40,000 including installation cost, which is rather low than the revised estimate.

In addition, two Diesel power plants (500 KW + 250 KW) will be required for emergency use though these were eliminated in the revised estimate. Their costs will be approximately US\$313,000.

§4 RE-ESTIMATE OF CONSTRUCTION COST

Based on the bridge design modified and new unit prices as described in the previous section 3, the construction costs of both Case I and Case II are estimated herein and tabulated in Tables 4.1 and 4.2 respectively. As seen in the tables, the cost of Case I includes compensation cost of land and right for the access railways to Vientiane to be constructed in the future. For easy comparisons, the revised estimates received from the Mekong Committee are concurrently presented in the tables.

Consequently, the total construction costs of Case I and Case II amount to US\$21.96 million and US\$17.11 million respectively whereas those were estimated at US\$9.83 million and US\$7.80 million respectively in the previously revised estimate.

The construction cost of main truss bridge including substructure increases to US\$10.91 million from US\$3.95 million of the revised estimate for Case I and to US\$7.87 million from US\$2.87 million for Case II. These newly estimated costs correspond US\$16,800 and US\$12,100 per meter of bridge length for Case I and Case II respectively.

Table 4.1 Itemized Comparative Construction Cost of Scaled-down Plan (Case 1)

Note: Quantities in () show the quantities revised before.

Item No.	Nork	Unit	Quantity	Revised be Unit Price	Revised before (US\$)	Estimated newly(US\$) Unit Price Amor	ly(US\$) Amount	Difference(USE)	Remarks
I. COVE	COVERNERUTS PREPARATORT WORKS				736,000		1,079,000	347,000	
1. Cons	Construction facilities				151,000		494,000	345,000	
(a) Tez	Temporary buildings	ß	1,000 (1,500)	9	000*09	O ₃	40,000	-20,000	Offices and laboratory
(b) Wat	Water supply system	s.			15,000		35,000	20,000	Well and pump
(c) E14	Electric power supply system								
1-6	22 kV distribution line	Ξ.	60	2,000	16,000	6,000	48,000	32,000	22 kV distr. line 8 km long
?	Substation equipments	1.5.			50,000		40,000	-10,000	One 1,000 kVA trans, and others
ĵ	c-3 Diesel power plants	I.3,					313,000	313,000	500 kW and 250 kW generators
(a) Com	Comminication system	L.S.			10,000		18,000	9,000	One automatic telephone exchanger
2. Land	Land and rights	7 E	1.3	450,000	585,000	450,000	585,000	•	
II. MAIN	MAIN CONSTRUCTION WORKS				6,235,064		14,477,000	8, 181, 936	
1. Bridge	9.9				4,176,212		11,491,000	7,314,788	650 m long
(a) Yes	Main truss bridge				3,945,421		10,907,000	6,961,579	
ing.	Superstructure				2,070,915		6,829,000	4,758,085	
1	Steel for truss members	ton	3,320	525	1,743,000	1,800	5,976,000	4,233,000	
7	Concrete	۳è	2,250	40	000*06	8	180,000	000*06	Cement 300 kg/m ³
8 -8	Reinforcement steels	ton	200	250	125,000	630	315,000	190,000	
7	Asphalt pavement	섐	7,150	2	14,300	4	28,600	14,300	
4	Miscellaneous	L.S.			98,615		329,400	230,785	#K 15
	Substructure				1,874,506		4,078,000	2,203,494	
9	Excavation for pler, Class I Class II Class II Class II	25 2	6,575 6,245 2,680	7.5 30	49,311 46,845 80,388	%%%	98,625 218,575 93,800	49,314 171,730 13,412	2 piera 8 piera 8 piera
7-0	Cutting edge steels for calssons	30.	70		•	4,050	40,500	40,500	4.5 ton edge
8	Congrete for pneumatic calssons	ገ	6,200 (B,200)	09	492,000	160	992,000	200,000	Cement 300 kg/m ³
9	Contrete for open calasons	"E	2,000		•	140	290,000	280,000	Cement 300 kg/m ³
9-10	Concrete for piers	ří	6,700	ß	335,000	6	469,000	134,000	Cement 250 kg/m ³
11-8	Reinforcement steels	ton	1,100	250	275,000	630	693,000	418,000	
B-12	Steel sheetpiles	ton	750	350	262,500	750	562,500	300,000	74 kg/a

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Item Mo.	Hork	Unit	Quantity	Rewised before (USS) Unit Price Amount	nre (US\$) Amount	Estimated new Unit Price	Estimated newly(US\$) Unit Price Amount	Difference (US\$)	Remarks	rk sa
a-13	a-13 Structural steel for temporary bridge	ton	200	320	64,000	207	140,000	76,000	H-shape	ŀ
A-14	a-14 Wooden mattresses for pler protection	Ą	6,200	21	130,200	40	248,000	117,880		
a-15	a-15 Gabions for bank protection	7 _h	1,000	20	20,000	95	65,000	35,000		
a-16	a-16 Miscellaneous	1.5.			89,262		157,000	67,738	N.	
2	Approach viaduct (Composite girder bridges for highway)				270,791 (72,188)		584,000	353,209 (-72,188)		
ī	b-1 Steel for composite girders	ton	8	525	47,250		1	-47,250		
ĩ	b-2 Concrete	۴	500	40	8,000		•	-B,000		
፲	b-7 Reinforcement steels	ton	50	250	12,500		1	-12,500		
1	b-4 Asphalt pavement	2	200	2	1,000		•	-1,000		
ĭ	b-5 Mecellaneous	L.S.			3,438		•	-3,438	₽¢.	
~	(Precest PC box girder bridge for highway)	<u>د</u>			(158,603)		(584,000)	(425,397)	2 x 161.3 m long	
Ţ	b-6 Excavation, common, for piers	ra	1,850 (100)	1.5	150	2	3,700	3,550		
Ä	b-7 Concrete for superstructure	r _{is}	1,800 (1,300)	40	52,000	160	288,000	236,000	Cement 350 kg/m3	
Ţ	b-8 Concrete for substructure	ń	850 (200)	35	7,000	22	59,500	52,500	Cement 250 kg/m3	
Ţ	b-9 Reinforcement steels	ton	(061) 08	250	47,500	630	50,400	2,900		
ĭ	b-10 Concrete piles for piers	Xo.	300 (160)	250	40,000	30	000 * 06	50,000		
7	b-11 Asphalt pavement	2	2,580 (2,200)	۵.	4,400	4	10,320	5,920		
₽ - 12	b-12 Miscellaneous	r. s.			7,553		82,080	74,527	1 K	
	H. ghway				956,775		1,216,000	259,225	5.6 km long	
2-1	2-1 Clearing and stripping	r _š i	72,500	0.3	21,750	0.05	3,625	-18,125		
2	Excavation, common	r _{ja}	26,500	Ħ	26,500	H	26,500	1		
7	2-3 Embankment, earth	ĽĦ	104,100 (59,800)	2	119,600	8	208,200	009*88	Including sodding	to.
ĭ	2-4 Embankment, soil-cement mix	r _B	44,300	9	265,800	•	1	-265,800		
2 - 5	Subbase course	댸	47,450	ю	142,350	₩.	142,350	•		
2	2-6 Base course	7₫	38,850	1.5	58,275	3.5	135,975	001,11		
2-7	2-7 Asphalt pavement	¹ 2	44,520	21	89,040	4	178,080	89,040		
۹ %	2-8 Concrete for box culverts	r _{ia}	2,300	40	92,000	8	184,000	95,000	Cement 280 kg/m³	

Table 4.1 (Continued)

Item No.		¥07k	Unit	Quantity	Revised before (USS) Unit Price Amount	Amount	Estimated newly(US\$) Unit Price Amo	1y(US\$) Amount	Difference (USS)	Renarko
	6-5	2-9 Reinforcement steels	tons	230	250	57,300	630	144,900	87,400	
	2-10	2-10 Guardrail	н	3,840		38,400	ĸ	134,400	96,000	
	2-11	2-11 Miscellaneous	L.S.			45,560		57,970	12,410	**
~		Administrative facilities			편	1,162,077		1,770,000	607,923	
	7	5-1 Clearing and stripping	r¦s	50,300	0.3	15,090	0.1	5,030	-10,060	
	Z,	Erbankment	ግ	73,900	1.5	110,850	~	147,800	36,950	
	ŗ	Subbase course	۲ ₃	39,700		001,611	٣	119,100		
	ĭ	Base course	~	41,600	1.5	62,400	3.5	145,600	63,200	
	ኚ	Asphalt pavement	Z _i	39,650	۲.	79,300	4	158,600	79,300	
	9	Immigration offices and customhouses m2	ses m ²	6,500	8	585,000	120	780,000	195,000	
	Ţ	Warehouses, booths and others	ъ	3,000	45	135,000	110	330,000	195,000	
	ę,	Miscellaneous	r.s.			55,337		83,870	28,533	3 4.
III.		COVERNMENTS' ADMINISTRATIVE EXPENSE	r.S.			421,864		930,000	508,136	6 % of (1) and (11)
*	ENGINE	Engineering Service	r. s.			769,845		1,900,000	1,130,155	5 % of (1), 15% of (11) -1(a), 7 % of (11) - 1(b), 7 % of (11) - 2
Α.	CONTEN	CONTENGENCY AND RESERVE	L.S.		ר	1,054,659		2,334,000	1,279,341	EΞ
vi.	INTERE	INTEREST DURING CONSTRUCTION	1.3			556,646		1,240,000	683,354	6 ≸ of (I) to (V)
		Total			δí,	9,834,078		21,960,000	12,125,922	

Table 4.2 Itemized Communitive Construction Cost of Scaled-down Plan (Case II)

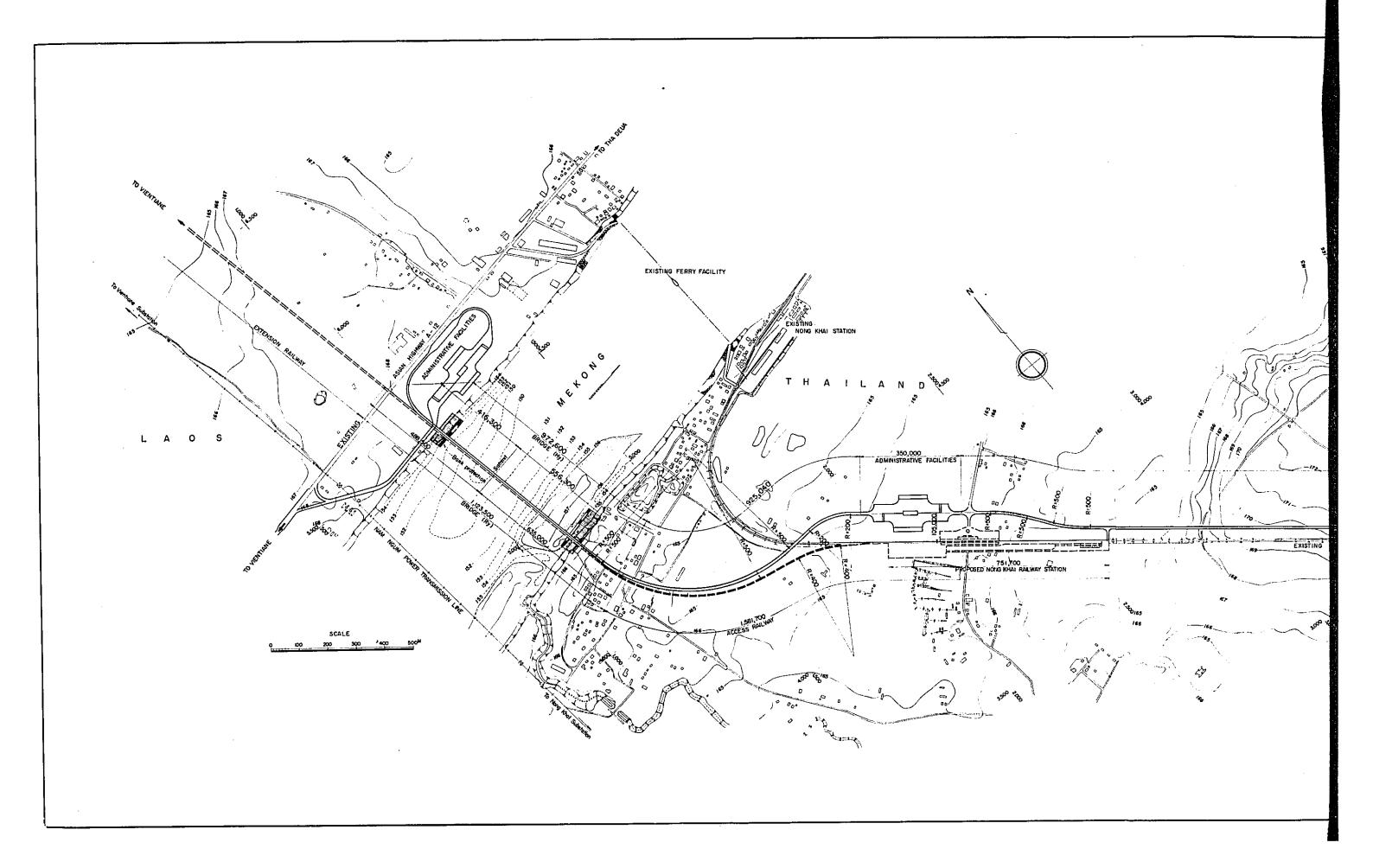
							Note: Cuan	tities in ()	Note: Quantities in () show the quantities review hefore.
Iten					1				
No.	Work	Unit	Quantity	Unit Price	Meyised before (USS) Unit Price Amount	Estimate Unit Price	Estimated Newly(US\$) Unit Price Amount	Difference (US\$)	Remarks
ī.	GOVERNMENTS! PREPARATORY WORKS				376,000		719,000	343,000	
ä	Construction facilities				151,000		494,000	343,000	
3	Temporary buildings	~h	1,000 (1,500)	40	000*09	40	40,000	-20,000	Offices and laboratory
2	Water supply system	si N			15,000		35,000	20,000	Well and pump
(0)	Electric power supply system								
-	c-1 22 kV distribution line	Ē	ω	2,000	16,000	6,000	48,000	32,000	22 kV distri. Mine 8 km long
-	c-2 Substation equipments	1.5.			20,000		40,000	-10,000	One 1,000 kVA trans and others
•	c-3 Diesel power plants	L, 5,			•		313,000	313,000	500 kW and 250 kW generators
(F)	Communication system	.3			10,000		18,000	8,000	One automatic telephone exchanger
%	Land and rights	ўщ ₂	0.5	450,000	225,000	450,000	225,000	i	
i.	MAIN CONSTRUCTION WORKS				5,220,946		11,443,000	6,222,054	
i	Bridge				3,102,094		8,457,000	5,354,906	650 m long
3	Main truss bridge				2,871,303		7,873,000	5,001,697	
	Supera truc ture				1,452,045		4,773,000	3,320,955	
4	a-1 Steel for truss members	ton	2,220	525	1,165,500	1,900	4,050,000	2,884,500	
-	s-2 Concrete	r _B	2,140	40	85,600	8	171,200	85,600	Cement 300 kg/m ³
4	a-3 Reinforcement steels	tg	470	250	117,500	630	296,100	178,600	
4	a-4 Asphalt pavement	~	7,150	۲	14,300	4	28,600	14,300	
•	a-5 Miscellaneous	L.S.			69,145		227,100	157,955	₩. w
	Substructure				1,419,258		3,100,000	1,660,742	
•	a-6 Excavation for plers, Class II Class II Class III	ลูลล	3,700 3,500 1,510	7.5	27,750 26,325 45,300	322	55,500 122,850 52,850	27,750 96,525 7,550	2 piere B piera B piera
e)	a-7 Cutting edge steels for caissons	Жо.	10		1	3,800	38,000	38,000	3.8 ton edge
٠	a-8 Concrete for preumatic calasons	£	3,310 (4,910)	9	294, 600	160	529,600	235,000	Cement 300 kg/m3
4	a-9 Concrete for open calesons	r _Z a	1,600		•	140	224,000	224,000	Cement 300 kg/m3
÷	a-10 Concrete for plera	₆	4,520	ይ	226,000	٤	316,400	90,400	Cement 250 kg/m³
å	s-11 Reinforcement steels	ton	006	250	225,000	630	567,000	342,000	
٠	s-12 Steel sheetpiles	to 2	750	350	262,500	750	562,500	300,000	74 ¥E/m
è	a-13 Structural steel for temporary bridge	tot t	800	320	64,000	<u>σ</u>	140,000	76,000	H-shape
à	a-14 Mooden mattresses for pier protection	Th.	6,200	22	130,200	40	248,000	117,800	
4	a-15 Cabions for bank protection	ሤ	1,000	50	50,000	8	85,000	35,000	

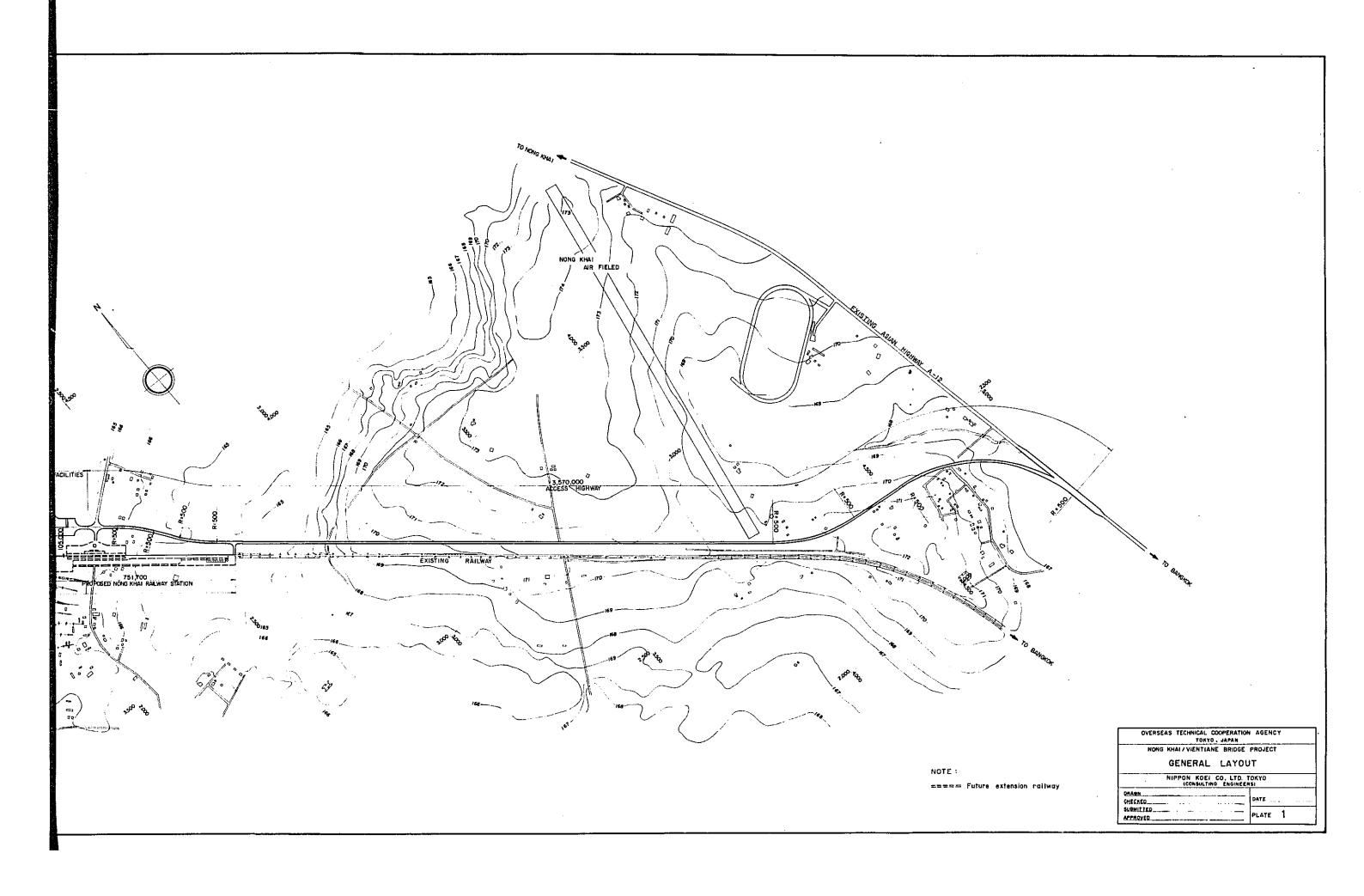
Table 4.2 (Continued)

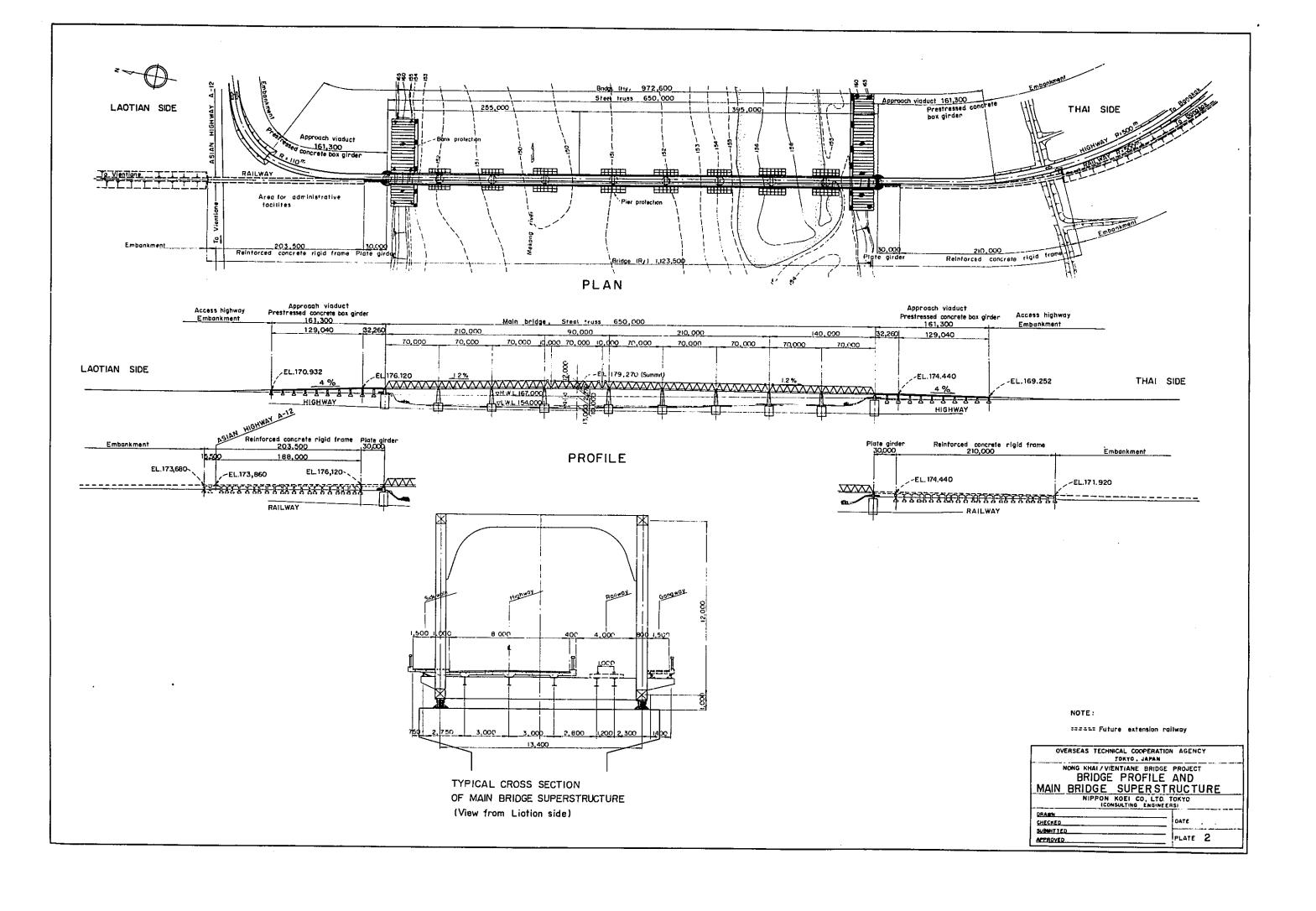
Item No.	Vork	Unit	Quantity	Revised before (USS) Unit Price Amount	ore (USS) Amount	Estimated Newly(UGS) Unit Price Amount	Kewly(UGS) Amount	Difference (UCS)	Remarks
	a-16 Miscellaneous	L.S.			67,583		158,300	717.06	z.
(P)	Approach visducts (composite girder bridges for highway)				230,791 (72,188)		584,000	353,209 (-72,188)	
1	l Steel for composite girder	ton	8	525	47,250		•	47,250	
ĩ	2 Concrete	r _e	200	40	9,000		•	-8,000	
7	3 Reinforcement steel	ton	20	250	12,500		1	-12,500	
1	4 Asphalt pavement	샙	200	2	1,000		1	-1,000	
ì	5 Miscellaneous	ដូន			3,438		1	-3,438	
	(Precast PC box girder bridge for highway)	ray)			(158,603)		(584,000)	(425,397)	2 x 161.3 m long
Ï		r _a	1,850 (100)	1.5	150	٧	3,700	3,550	
Ţ		'n	1,800 (1,700)	40	52,000	160	288,000	236,000	Cement 350 kg/m ³
Ţ	b-8 Concrete for substructure	ř _B	850 (200)	35	7,000	70	29,500	52,500	Cement 250 kg/m3
¥	b-9 Reinforcement steels	ton	80 (190)	250	47,500	630	50,400	2,900	
L	b-10 Concrete piles for plors	No.	300 (1060)	250	40,000	300	000*06	50,000	
4	b-11 Asphalt pavement	n _k	2,580 (2,200)	64	4,400	4	10,320	5,920	
ል	b-12 Miscellaneous	L. 5.			7,553		82,090	74,527	₩. ₩.
2. 18	Highway				956,775		1,216,000	259,225	5.6 km long
7	2-1 Clearing and stripping	γį	72,500	0.3	21,750	90.0	3,625	-19,125	
2-5	-2 Excavation, common	r _E	26,500	н	26,500	٦	26,500		
-5	2-3 Enbankment, earth	r _{în}	104,100 (59,800)	r,	119,600	64	208,200	88,500	Including sodding
5-	2-4 Entankment, soil-cement mix	r, ^{El}	44,300	9	265,800		•	-265,800	
-5	2-5 Subbase course	겉	47,450	۳	142,350	r	142,350	•	
ċ.	2-6 Base course	엖	38,850	1.5	58,275	3.5	135,975	77,700	
4	2-7 Asphalt pavement	₁ 2	44,520	8	89,040	4	178,080	69,040	
.5	2-8 Concrete for box culverta	ď	2,300	40	92,000	8	184,000	92,000	Cement 280 kg/m³
	2-9 Reinforcement steels	ton	0£2	250	57,500	630	144,900	87,400	
	2-10 Gurdre11	В	3,840	10	38,400	35	134,400	000*96	,
	2-11 Miscellaneous	r. 5.			45,560		57,970	12,410	R.
3. Ad	Administrative facilities				1,162,077		1,770,000	607,923	-
Y	3-1 Clearing and stripping	2₫	50,300	0.3	15,090	0.1	5,030	-10,060	
Υ.	3-2 Enbankment	E3	73,900	1.5	110,650	8	147,800	36,950	
ķ	5-5 Subbase course	ሳ	39,700	۴	119,100	iv.	119,100	•	

Table 4.2 (Continued)

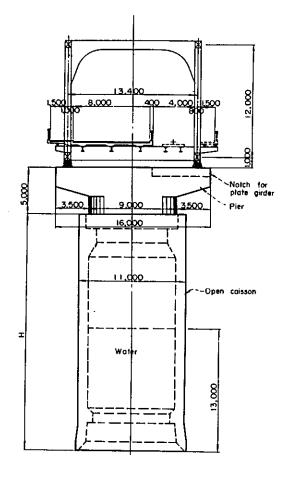
İ		ĺ							
Item No.	Work	Uni t	Quanti ty	Revised before Unit Price	ore (US#)	Revised before (US\$) Estimated Kewly(US\$) Unit Price Amount Unit Frice Amount	س.	Difference (USS)	Renarks
	3-4 Base course	డ	41,600	1.5	62,400	3.5	145,600	83,200	
	3-5 Asphalt pavement	~	39,650	2	79,300	4	158,600	79,300	
	3-6 Immigration offices and customhouses	^{‡5}	6,500	&	585,000	120	790,000	195,000	
	3-7 Warehouses, bouths and others	ሤ	3,000	45	135,000	011	330,000	195,000	
	3-8 Muscellaneous	L.S.			55,337		83,870	28,533	缺い
.111.	Covernments' administrative expense	.5.			335,817		730,000	394,183	6 % of (I) and (II)
IV.	Encineering Service	. s.			590,727		1,431,000	840,273	5 % of (1) 15 % of (11) - 1(a), 7 % of (11) - 1(b), 7 % of (11) - 2
Α.	CONTENCENCY AND RESERVE	r.s.			839,542		1,817,000	977,458	5 % of (II) + 3 15 % of (I) and (II)
VI.	INTEREST DURING CONSTRUCTION	1.5			441,782		970,000	528,218	ō % of (I) to (V)
	lote1			7.	7,804,814	•	17,110,000	9,305,186	

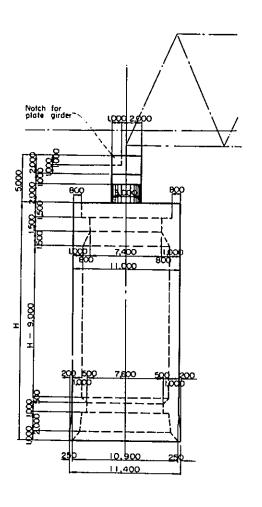


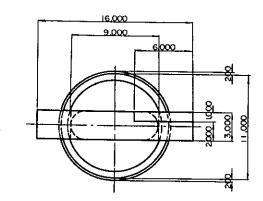




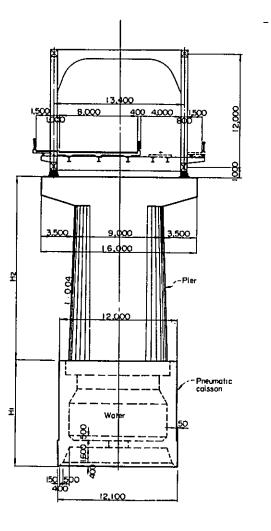
PIER AND OPEN CAISSON

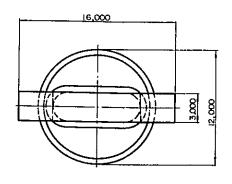






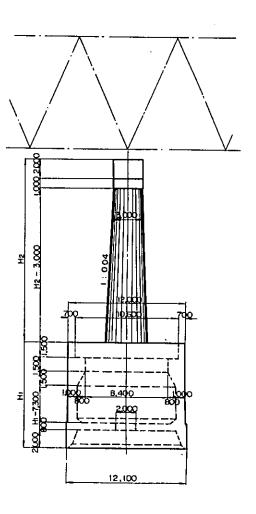
PIER AND PNEUMATIC CAISSON

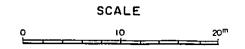




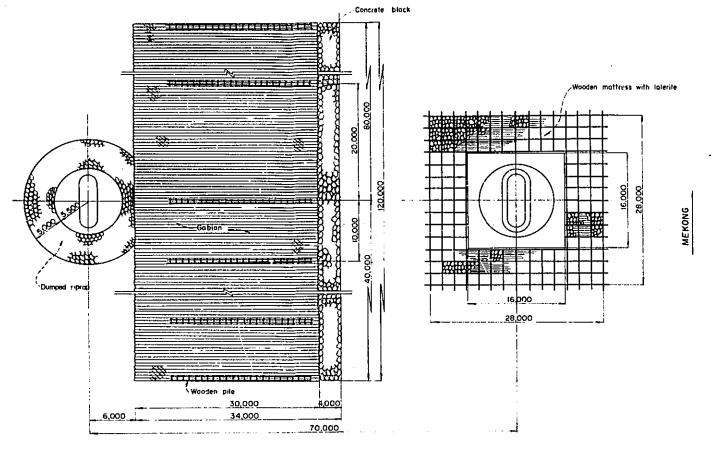


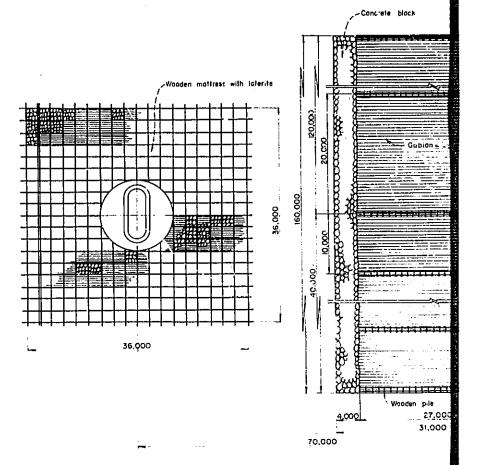
Both open and pneumatic caissons are planned to be founded on the fresh siltstone excavated Its 2-mater surface.



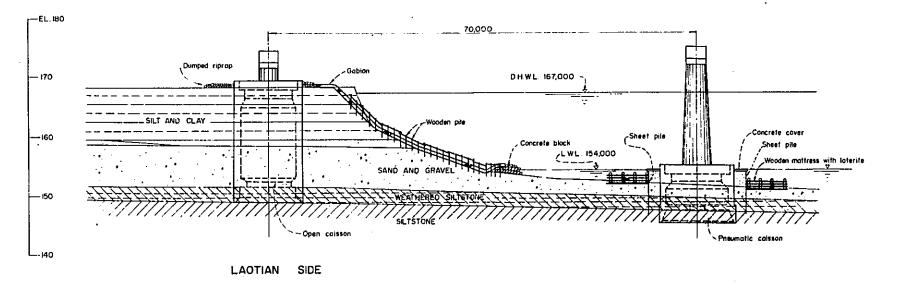


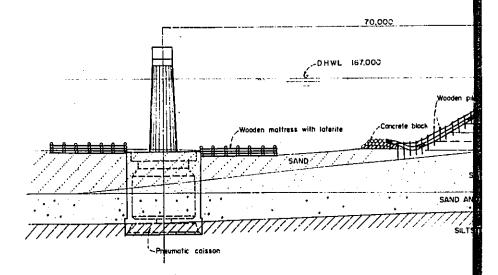
OVERSEAS TECHNICAL COOPERATIO TOKYO, JAPAN	N AGENCY
NONG KHAI/VIENTIANE BRIDGE	PROJECT
MAIN BRIDGE SUBSTRUCT	TURE
NIPPON KOEL CO., LTD.	
DRIAWN	DATE
APPROVED	PLATE 3



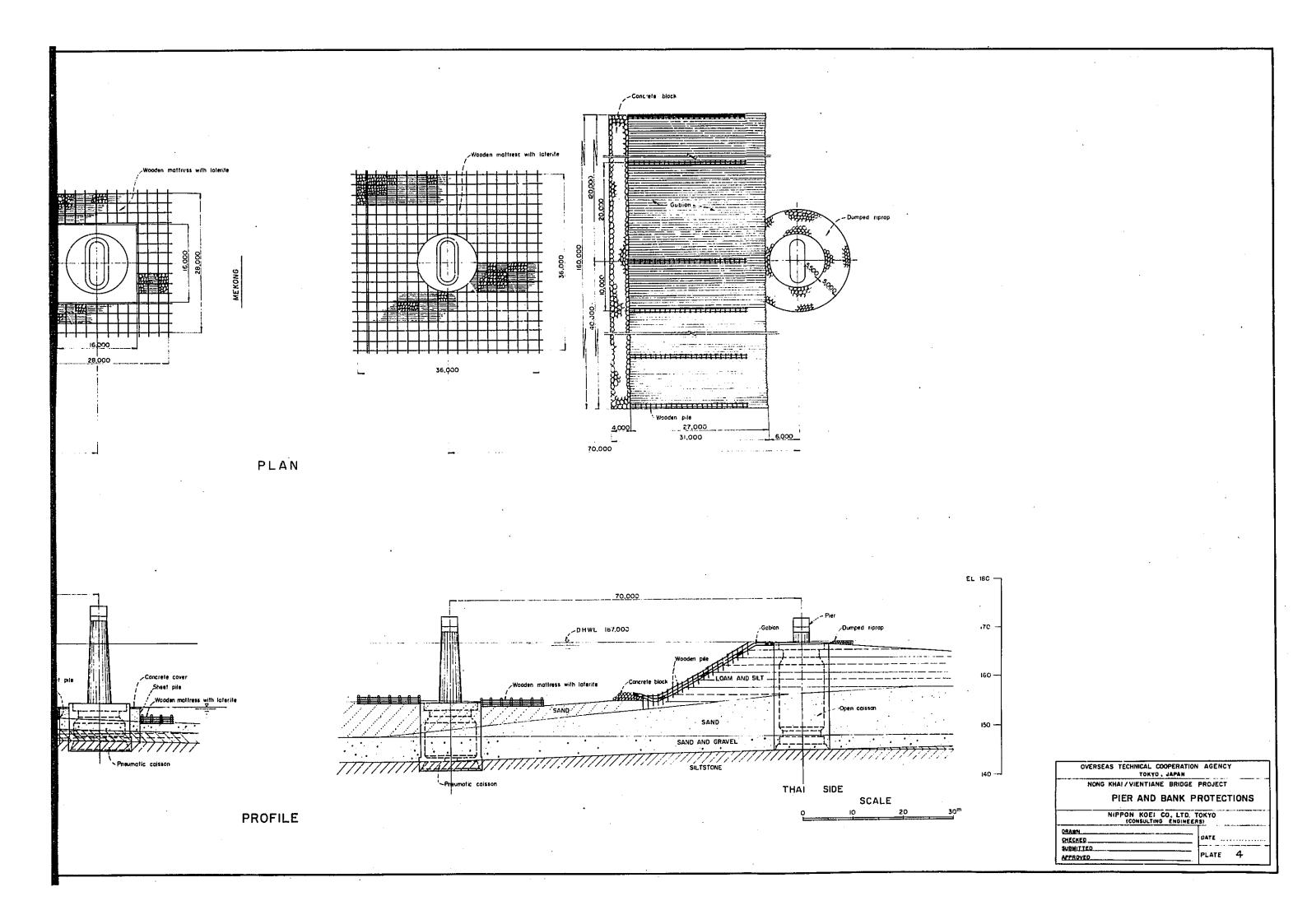


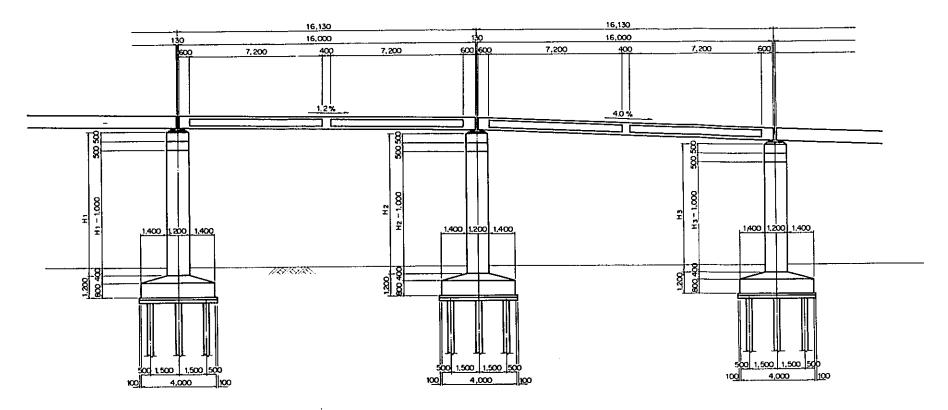
PLAN



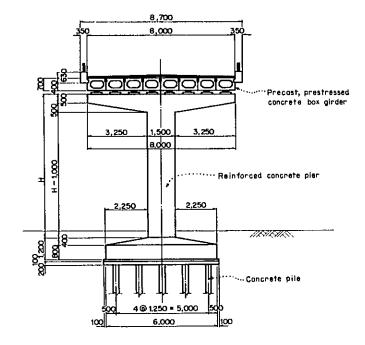


PROFILE



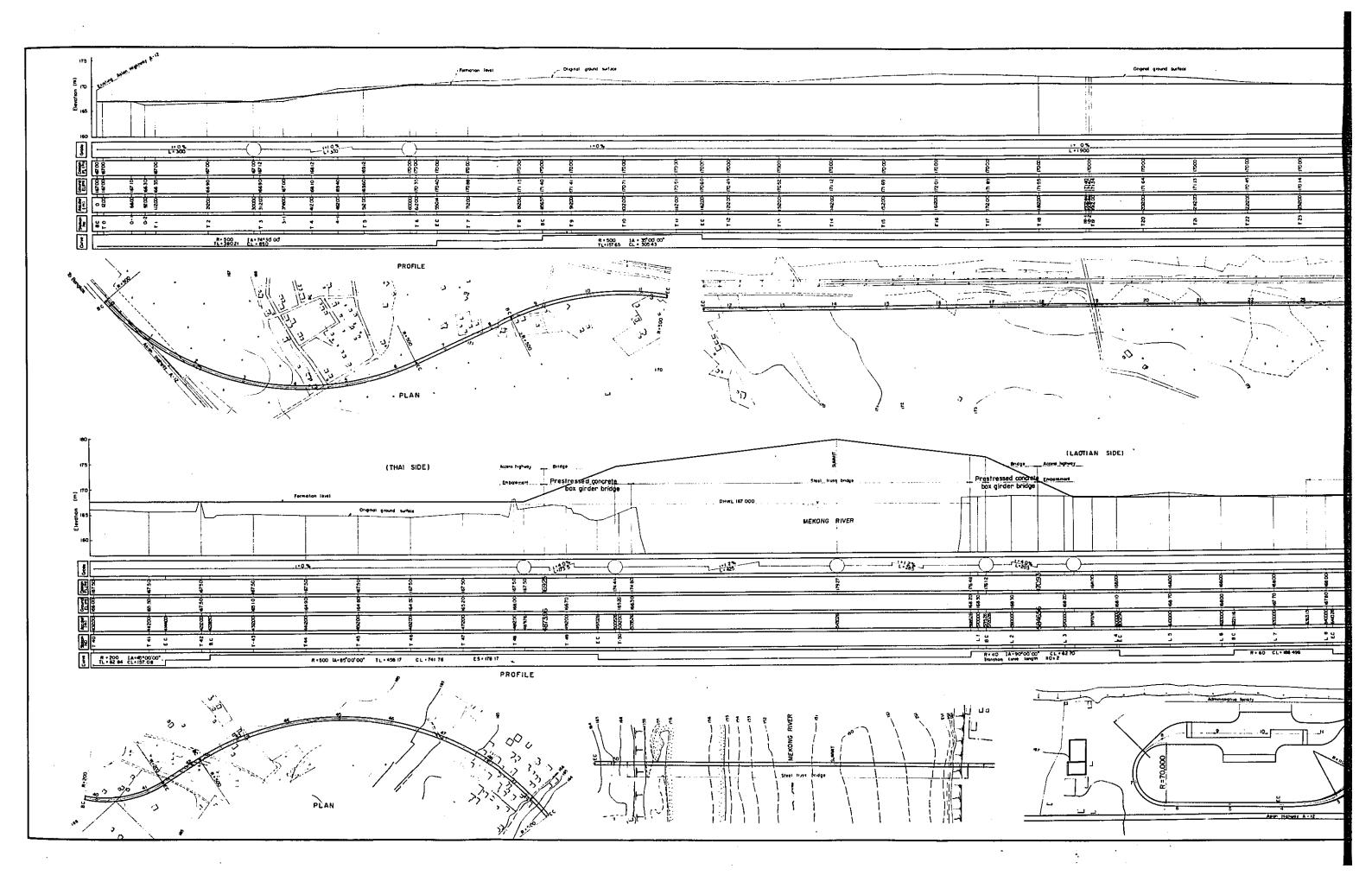


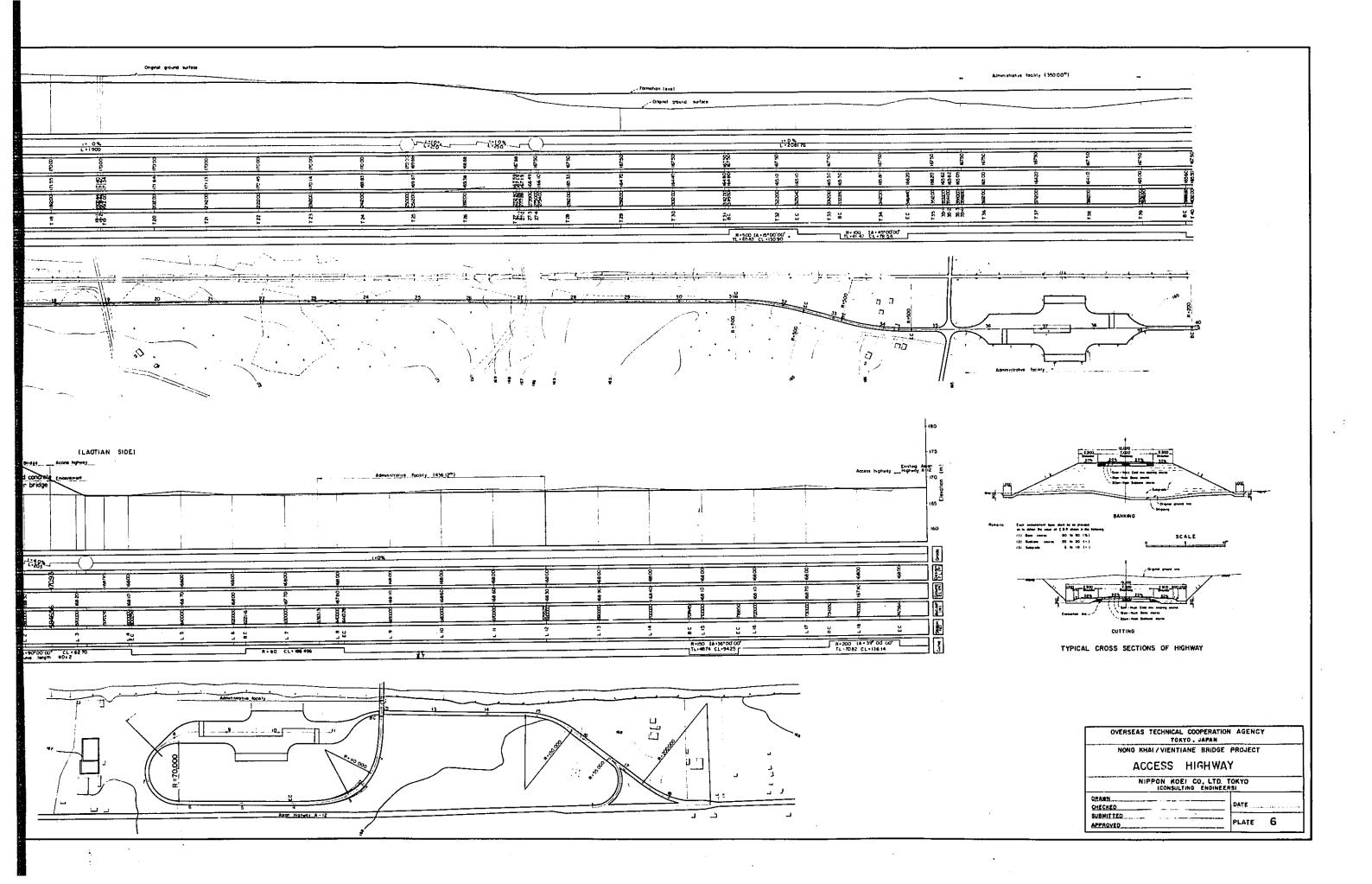
PROFILE

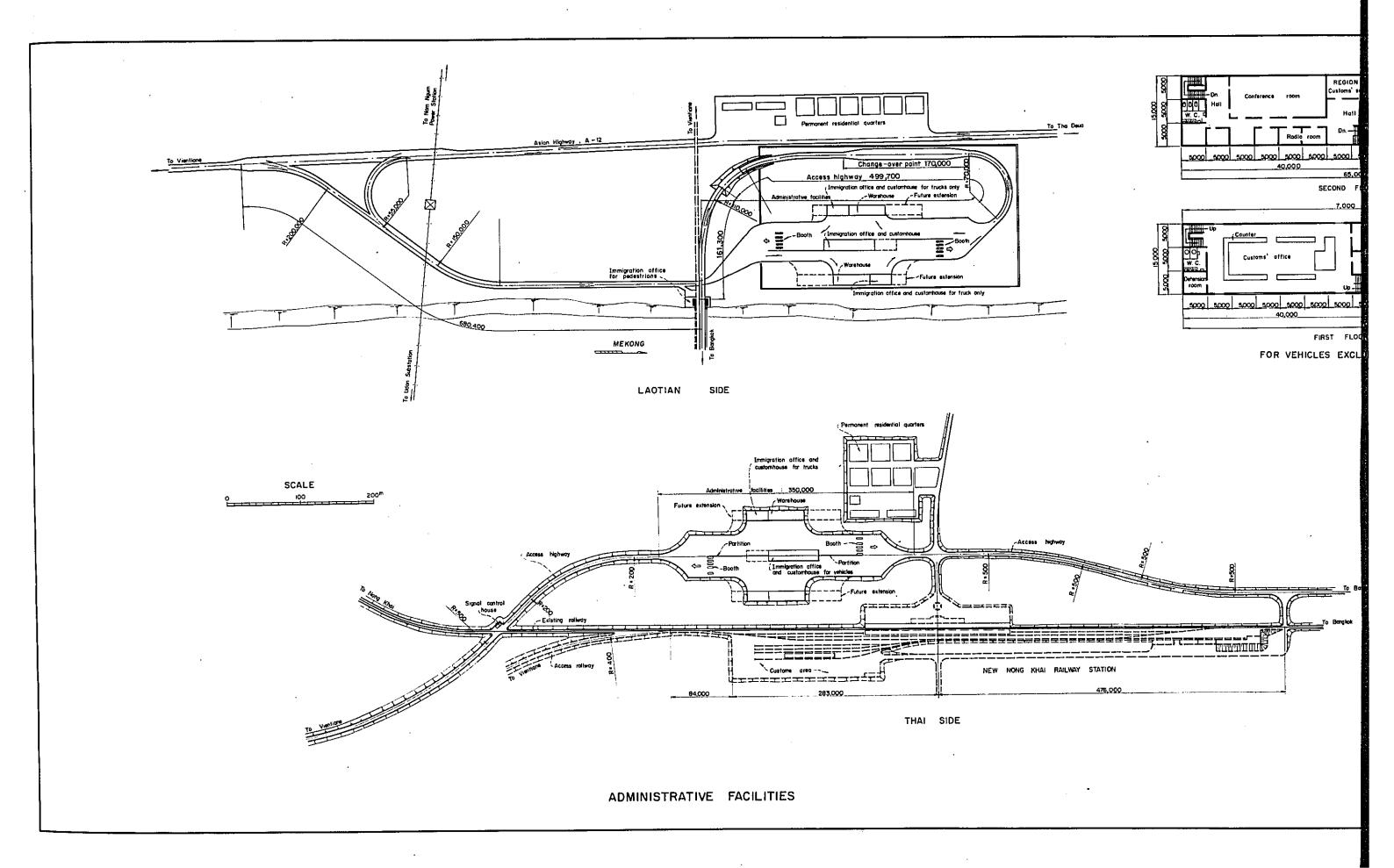


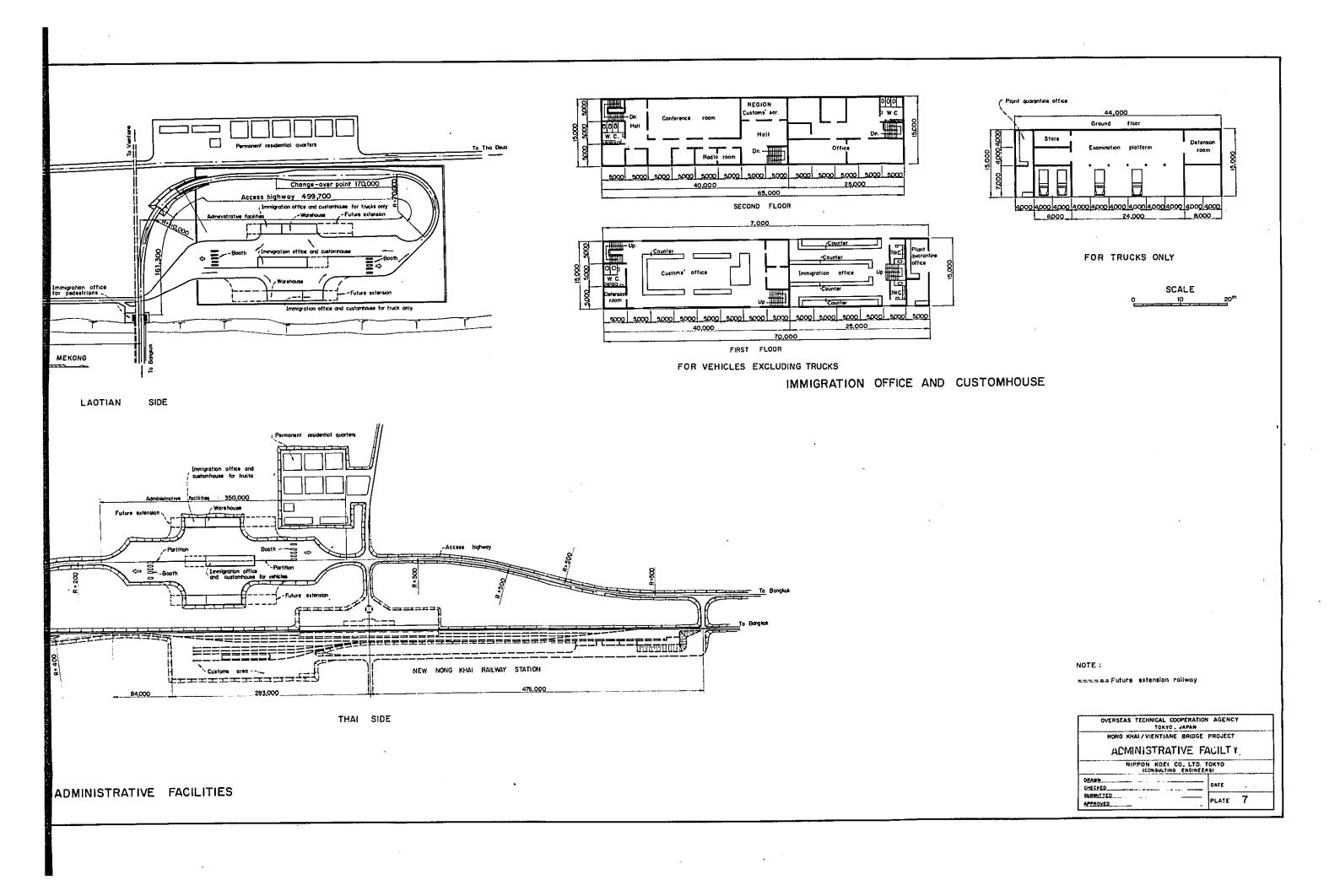
SECTION

OVERSEAS TECHNICAL CO	
NONG KHAI / VIENTIANE	BRIDGE PROJECT
APPROACH \	/IADUCT
NIPPON KOEI CO	LTD, TOKYO
DRAWN	DATE
CHECKED	
SUBMITTED	PLATE 5
APPROVED	PLAIL C









APPENDIX

COST ESTIMATE OF CONCRETE BRIDGE PLAN

APPENDIX

COST ESTIMATE OF CONCRETE BRIDGE PLAN

A.1 SUMMARY

This Appendix is prepared for use as reference by the Mekong Committee in further detailed investigation of this project, and contains the results of a tentative study made on the concrete bridge for the scaled—down plan. A suitable type of concrete bridge has been tentatively select—ed for the combined rail/highway bridge and the highway bridge, and the construction costs thereof have been estimated in this study. The bridge structures studied herein are confined only to the superstructure and piers of the main bridge. Type of pier foundation and other related highway facilities is not altered from the scaled—down plan stated in the main report, and construction period is also unchanged.

A prestressed-concrete box-girder was selected as the suitable type for concrete bridge at this site, taking into account the site topography and geology, the technical feasibility and the economy of construction. The construction cost of the bridge varies with length of pier spacing. Several cases of pier spacing were compared on the basis of the total construction cost of super-and substructures estimated by using the unit prices given in the main report. Among them the most sconomical pier spacing was 60 meters in the case of a rail/highway bridge, and 60 to 90 meters in the case of a highway bridge.

The total construction costs of the rail/highway bridge plan and the highway bridge plan were estimated respectively for the 60-meter pier spacing. It was US\$16.3 million in the case of rail/highway bridge and US\$14.8 million in the case of a highway bridge. In both cases, related highway facilities were included but the railway bridge girder and the railway facilities for former case were not included.

A.2 DESIGN

A.2.1 Design Criteria

Design criteria are the same as in the scaled-down plan dealt with

in the main report with the exception as stated below.

- Horizontal clearance of navigational requirement: no limitation (assumed)
- Design compressive strength of prestressed concrete (28-day age) : 350 kg/cm²

A.2.2 Design Conception and Structural Features

In designing bridge structures, the following conceptions are taken into account.

- Where the river channel is very wide but relatively shallow and foundation rock lies below thin river deposit, a multispan girder bridge is most economical.
- Where the construction of scaffolding for superstructures is impracticable or risky, the construction method is inevitably limited to the cantilever method.
- Where the bridge span is expected to be as long as 50 meters or more and cantilever construction method is applied, the conventional reinforced-concrete girder is uneconomical. In such case, prestressed concrete girder is useful and economical.
- Where the bridge with long spans is to be constructed by cantilever method or it is continuous over piers, box girder is generally suitable because it has high torsional rigidity and girder body becomes slender.
- Where superstructure is longitudinally symmetrical on the supports, thin reinforced-concrete wall is economical as the bridge pier.

Based on these conceptions, a prestressed-concrete box-girder bridge was selected as the suitable type of bridge structure. However, the pier spacing is flexible, which will be determined by the cost analysis of super-and substructures. For this analysis, six cases with pier spacing of 50, 60, 70, 80, 90 and 100 meters were considered and their structures were provisionally designed as follows.

1) Superstructures

- Highway bridge

The bridge girder is fixed rigidly to each intermediate pier. But, hinged expansion joint is provided at each crown between piers in order to absorb complex stresses due to temperature change and to distribute horizontal force to each pier. Cross-section of the girder is selected from a single box shape having side cantilever decks in order to give high torsional rigidity. Widths of upper and lower flanges are 11.8 meters and 6.3 meters, respectively. Girder depth is gradually reduced from pier end towards each crown, for instance, from 3.4 meters to 1.5 meters in the case of 60-meter pier spacing or from 4.8 meters to 1.9 meters in the case of 90-meter pier spacing.

- Railway bridge

This design of a railway bridge is made only for use as reference in designing the substructures. Taking into account the railway extension in the future, the railway bridge girder is separated from the highway bridge girder though both are supported by the common piers. Single-box type is also used. However, the girder is designed as a continuous unit with no expansion joint over the entire length of the bridge, which is supported by movable shoes on each pier. Widths of upper and lower flanges are designed to be 5.7 meters and 2.7 meters repectively and the girder depth is gradually reduced from pier end towards crown: from 4.0 meters to 3.0 meters for 60-meter span or 6.0 meters to 4.0 meters for 90-meter span.

2) Substructure

The bridge pier is designed as a wall-type reinforced concrete structure with constant thickness of 2 meters since the superstructures are nearly symmetrical in the direction of bridge axis. Height of the pier is determined by vertical clearance of navigational requirement. Top of a pier has two levels stepwise so as to even both floor deck levels of highway and railway bridges. Width of pier in the case of rail/highway bridge is tapered towards foundation from 14.85 meters at the top in order to smoothly transmit the asymmetrical load on the pier to foundation.

But, in the case of highway bridge, such tapering is avoided and the width is 6.6 meters constantly.

Foundation structure to support superstructure and pier as well as live loads is temporarily designed as a circular caisson standing on shale of foundation rock. From the viewpoints of safety and economy of construction, pneumatic sinking caisson is applied for pier foundations in river channel while two abutment caissons are open caissons. The diameter of all the caissons is 12 meters in the case of rail/highway bridge and is 10 meters in the case of highway bridge.

A.3 COST ESTIMATE

Estimate of construction cost is made based on the unit prices described in the main report except the cost of the prestressed-concrete girder. As the unit construction cost of the prestressed concrete girder varies depending on the construction method and the construction period, they are assumed as follows:

Construction method: Dywidag cast-in-place cantilever method
Construction period: 2.5 years

Then, the unit cost of the prestressed concrete is estimated as follows:

	Cost per m3
Concrete purchased from a domestic	(US\$ equivalent)
company (cement 400 kg/m3) and forms: . Special movable scaffolds, cable crane and	90
other related construction equipment and	
labors:	110
General expense:	40
Total	240

In order to select an economical pier spacing from the six cases mentioned in the section A.2, direct construction cost of the main bridge is estimated for each case. The results are illustrated graphically in Fig. A.1. From this figure, it is evident that the economical pier spacing is 60 meters in the case of rail/highway bridge and 60 to 90 meters, with no critical point, in the case of highway bridge.

Therefore, the suitable bridge plan is considered to be an eleven 60-meter span, prestressed-concrete box-girder bridge for both the rail/highway bridge and the highway bridge. These bridges are tentatively designed as shown in PLATES A.1 and A.2.

The total construction cost of the rial/highway bridge plan not including railway bridge-girder and railway facilities to be constructed in the future was estimated and tabulated in Tables A.1 and A.2. On the other hand, the total cost of the highway bridge plan can be easily obtained by estimating the cost reducible, with regard to the substructure, from that of the above rail/highway bridge plan. The total construction cost only of the highway bridge and related highway facilities under each bridge plan thus obtained is summarized as follows:

Construction Cost of Concrete Bridge Plan

	Bridge plan	Foreign currency (US\$)	Domestic currency (US\$)	Total (US\$)
1)	Highway bridge in rail/highway bridge plan:	5,300,000	11,000,000	16,300,000
2)	Highway bridge plan:	4,600,000	10,200,000	14,800,000

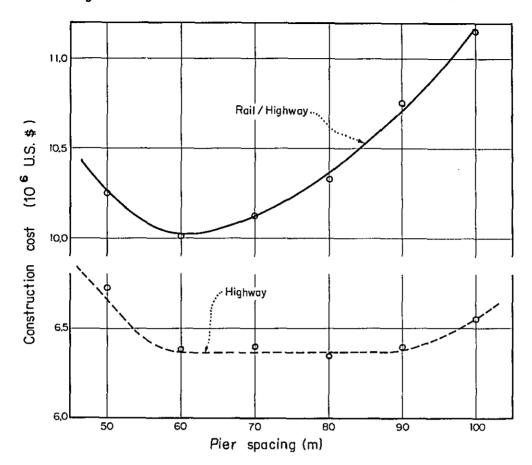


Fig. A.1 SPAN VS. COST OF CONCRETE BRIDGE

Pier	Number	Cons	struction cost (U.S	.\$)
spacing (m)	of pier	Substructure	Superstructure	Total
50	14	4,700,000	5,550,000	10,250,000
	' '	(3,550,000)	(3,170,000)	(6,721,000)
60	12	4,300,000 (3,230,000)	5,710,000 (3,190,000)	10,010,000 (6,420,000)
70	11	4,200,000 (3,160,000)	5,920,000 (3,240,000)	10,120,000 (6,400,000)
80	10	4,060,000 (3,030,000)	6,270,000 (3,320,000)	10,330,000 (6,350,000)
90	9	3,890,000 (2,880,000)	6,870,000 (3,510,000)	10,760,000 (6,390,000)
100	8	3,640,000 (2,750,000)	7,510,000 (3,800,000)	11, 150,000 (6,550,000)

Remarks:

- (1) Cost of approach structures is not included
- (2) The above construction costs do not include such indirect costs as expenses for engineering services, Government's administrative expenses and interests during construction
- (3) The above construction cost in () is of highway

Table A.1 Construction Cost of Concrete Bridge Plan
(Summary for the rail/highway bridge)

		Const	ruction cost	(US\$)
	Work	Foreign currency	Domestic currency	Total
I.	GOVERNMENTS' PREPARATORY WORKS	269,000	810,000	1,079,000
	1. Construction facilities	269,000	225,000	494,000
	2. Land and rights		585,000	585,000
II.	MAIN CONSTRUCTION WORKS	3,439,000	7,698,000	11,137,000
	1. Bridges	3,439,000	4,709,000	8,148,000
	(a) Main prestressed concr bridge (i) Superstructure 1 (ii) Substructure (b) Approach viaduct 2. Highway			3,191,000 4,381,000 576,000 1,219,000
	3. Administrative facilities		1,770,000	1,770,000
III.	GOVERNMENTS' ADMINISTRATIVE EXF	PENSE	730,000	730,000
IV.	ENGINEERING SERVICE	800,000	600,000	1,400,000
٧.	CONTINGENCY AND RESERVE	492,000	1,292,000	1,784,000
VI.	INTEREST DURING CONSTRUCTION	300,000	600,000	900,000
	Total	5,300,000	11,000,000	16,300,000

^{1/} not including railway's girder

Table A.2 Itemized Construction Cost of Concrete Bridge Plan (Rail/highway bridge)

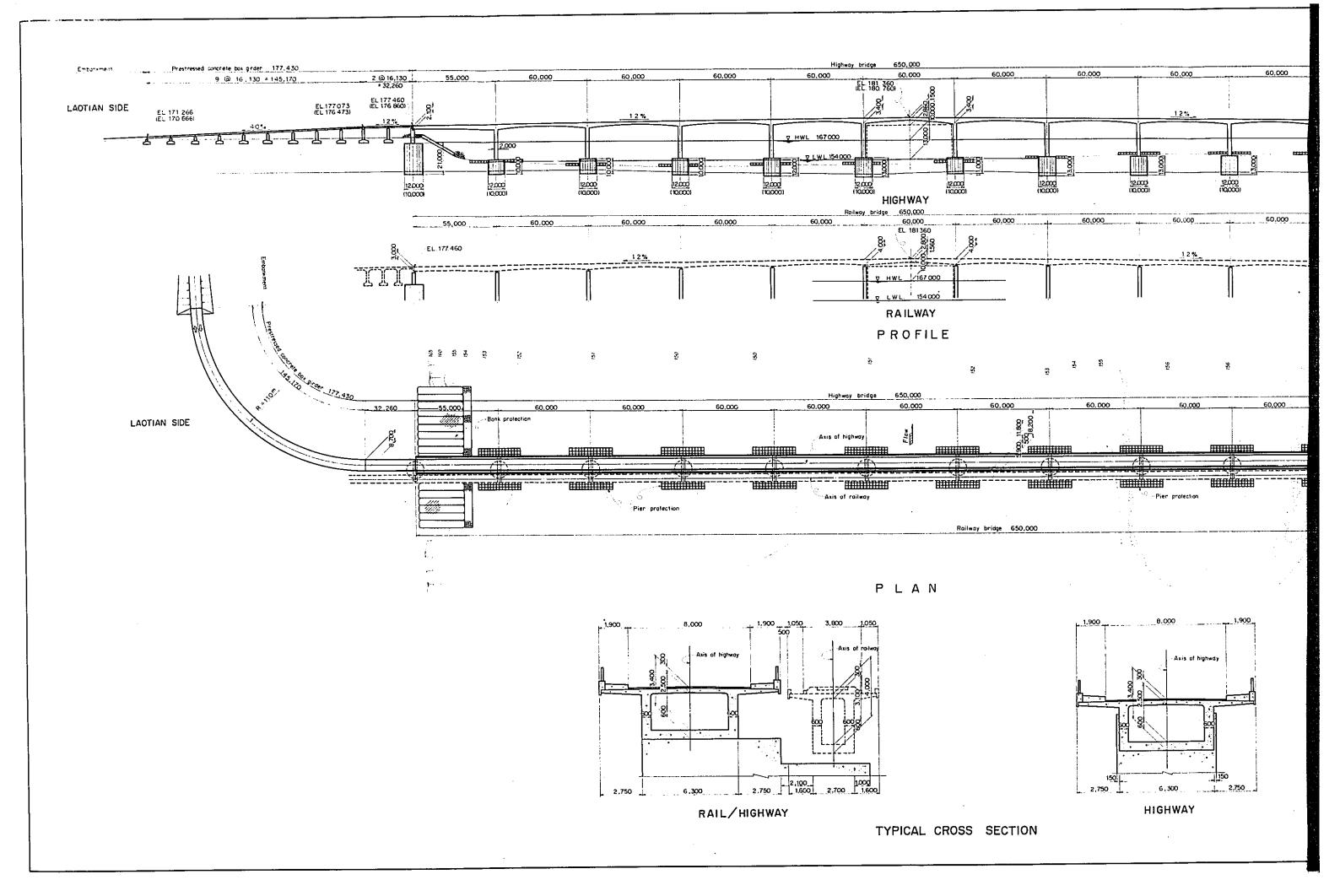
Item No.	Vork	Unit	Quantity	Foreign C	Foreign Currency (US\$) Domestic Currency (US\$) Unit Frice Amount Unit Price Amount	Domestic C Unit Price	urrency (US\$)	Total (USE)	Rezarka
I. COVERA	COVERNMENTS' PREPARATORY WORKS				269,000		810,000	1,079,000	
1. Const	Construction facilities				269,000		225,000	494,000	
(a) Tempo	(a) Temporary buildings	٦	1,000			04	40,000	40,000	Offices and laboratory
(b) Water	(b) Water supply system	L.S.					35,000	35,000	Pump and well
(c) Elect	(c) Electric power supply system								
7	c-1 22 kV distribution line	E X	60			9,000	48,000	48,000	22 kV distr. line 8 km. long
; ;	c-2 Substation equipment	5.3					40,000	40,000	One 1,000 kVA trens and others
7	c-3 Discal power plants	L.S.					44,000	313,000	500 kW and 250 kW generators
(4) Comm	(d) Communication system	2.5					18,000	19,000	One automatic telephone exchanger
2. Land	Land and rights	결	1.3			450,000	585,000	565,000	
II. MAIN	MAIN CONSTRUCTION WORKS				3,439,000		7,698,000	11,157,000	
1. Bridges	10 to				3,439,000		4,709,000	8,148,000	650 m long
(a) Matn	(a) Main prestressed concrete bridge				3,439,000		4,133,000	7,572,000	
Mng)	(Superstructure of highway bridge)				(1,527,000)		(1,664,000)	(3,191,000)	
) [-8	a-1 Concrete	r _e	5,850	88	468,000	160	936,000	1,404,000	Gement 400 kg/m3
A-2 1	A-2 Reinforcement steels	ton	560	001	56,000	530	296,800	352,800	
-	Prestressed steels	# 6	390	2,020	787,800	740	288,600	1,076,400	
1	Shoe and hinged bearings	No.	22	2,790	61,380	990	19,580	096*08	1.3 ton shoe and 1.0 ton hinge
	Expansion joints	a	132	580	76,560	210	27,720	104,280	
, d	Asphalt pavenent	25	2,200	н	5,200	۳	15,600	20,800	
£-6	Miscellaneous	L.S.			72,060		79,700	151,760	**
	(Substructure for rail/highway bridge)	(adpi			(1,912,000)		(2,469,000)	(4,381,000)	
# #	Ercavation for piers, Class I	P.P	4,960	2,52	49,600	202	24,800	74,400	2 piers 10 piers
6 − 6	a-9 Cutting edge steels	70.	12	510	6,120	3,540	42,480	49,600	4.5 ton edge
g-10	a-10 Concrete for pneumatic calssons	n,	6,300	81	630,000	9	378,000	1,008,000	Cement 300 kg/m3
a-11	a-11 Concrete for open catasons	Ę	2,000	8	160,000	09	120,000	280,000	Cement 300 kg/m3
a-12	a-12 Concrete for piers	r _i	5,500	30	165,000	70	220,000	355,000	Cement 250 kg/m³
A-13	a-13 Reinforcement steels	ton	1,360	100	136,000	530	720,900	856,800	
8-14	a-14 Steel sheetpiles	ta:	750	390	292,500	360	270,000	562,500	75 kg/m sheetpiles
a-15	a-15 Structural steel for temporary bridge	ton	80	330	66,000	370	74,000	140,000	94 kg/m H-stape

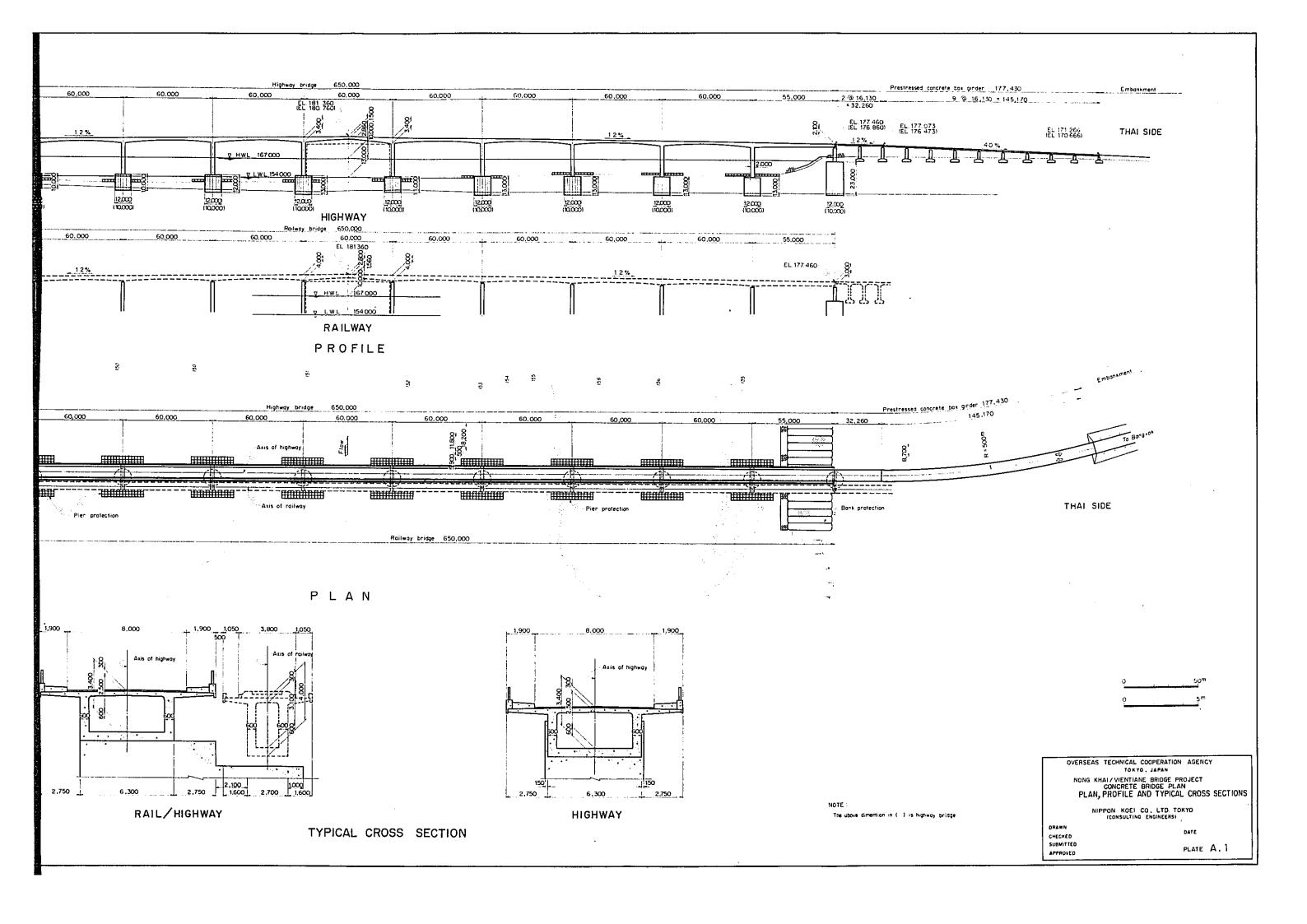
Table A.2 (Continued)

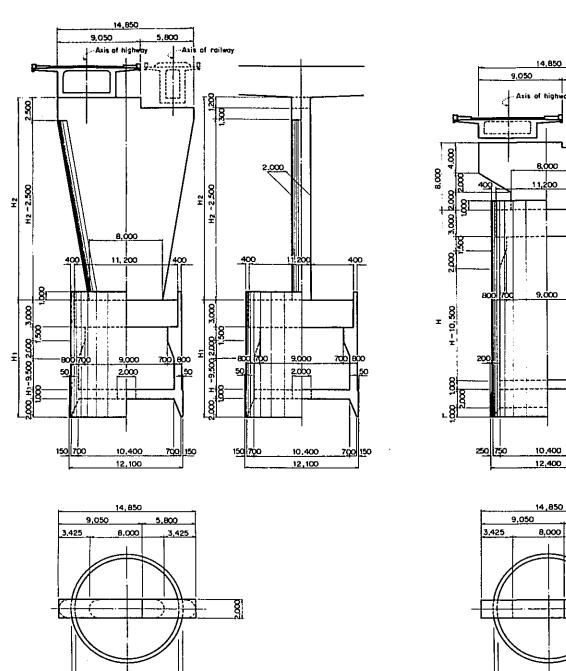
Item No.	Uni t	Quantity	Foreign Unit Pr	Foreign Currency (US\$) Unit Price Amount	Donestic Currency (USS) Unit Price Amount	rency (UCS)	Total (USE)	Remarks
a-15 Wooden mattresses for pler protection	2	7,700		7,700	39	300,300	308,000	
a-17 Gabione for bank protection	Ύa	1,000	ξV	2,000	8	900,000	85,000	
a-18 Miscellaneous	1.3			90,580		117,220	207,800	¥ 5.
(b) Approach visduct (Precast PC box girder bridge for highway)	_					576,000 (576,000)	576,000 (576,000)	2 x 177.43 m long
b-1 Eresystion, common, for plers	۴	2,050			~	4,100	4,100	2 x 11 piers
b-2 Congrete for superstructure	* 6	1,940			160	310,400	310,400	Cement 350 kg/m ³
b-3 Concrete for substructure	r _B	950			70	66,500	66,500	Cement 250 kg/m3
b-4 Reinforcement steels	ton	8			630	56,700	56,700	
b-5 Concrete piles for plers	No.	330			300	000*66	000*66	
b-6 Asphalt pavement	걸	2,840			4	11,360	11,360	
b-7 Miscellanedus	r,					27,940	27,940	₽¢.
2. Highway						1,219,000	1,219,000	5.6 km long
2-1 Clearing and stripping	E 5	72,500			0.05	3,625	3,625	
2-2 Excavation, common	°a	26,500			н	26,500	26,500	
2-3 Embankment	ř	104,600			2	209,200	209,200	Including sodding
2-4 Subbase course	" }	47,900			ĸ	143,700	145,700	
2-5 Base course	r _a	38,900			3.5	136,150	136,150	
2-6 Asphalt pavement	FE .	44,600			4	178,400	178,400	
2-7 Concrete for tox culverts	É	2,300			8	184,000	184,000	Cement 280 kg/m³
2-8 Reinforcement steels	ten	230			630	144,900	144,900	
2-9 Cuerdrail	Ø	3,850			35	134,750	134,750	
2-10 Miscellaneous	1.5					577,775	577,775	₩ 15
3. Administrative facilities						1,770,000	1,770,000	
3-1 Clearing and stripping	r _E	50,300			0.1	5,030	5,030	÷
3-2 Smbankment	Ę	73,900			7	147,600	147,800	
3-3 Subbase course	r _a	39,700			ľ	119,100	119,100	
3-4 Base course	r _e	41,600			3.5	145,600	145,600	
3-5 Asphalt pavenent	⁶¹	39,650			**	158,600	158,600	
3-6 Immigration offices and customhouse	설	6,500			120	780,000	780,000	
3-7 Warebouses, booths and others	Ę	3,000			011	330,000	330,000	
3-8 Miscellaneous	1.5					83,870	67,870	5 A

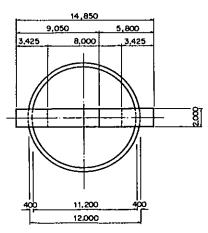
Table A.2 (Continued)

Item No.	Vorks	Uni t	Unit Quantity	Foreign Currency (US\$) Unit Price Amount	Domestic Currency (US\$) Unit Price Amount	Total (US\$)	७ मृ.च्या भु
III.	III. GOVERNYENTS' ADMINISTRATIVE EXPENSE	1.5.			730,000	730,000	6 % of (1) and (11)
IV.	ENGINEERING SERVICE	L.S.		900,000	000,000	1,400,000	5 % of (11) 15 % of (11)-1(b) 7 % of (11)-1(b) 7 % of (11)-2 5 % of (11)-3
A	CONTINGENCY AND RESERVE	1.5.		492,000	1,292,000	1,784,000	15 % of (1) and (II)
vI.	INTEREST DURING CONSTRUCTION	1.5.		300,000	000,000	000,000	6 ≸ of (I) to (V)
-	Total			5,300,000	11,000,000	16,300,000	









10,400

12,400

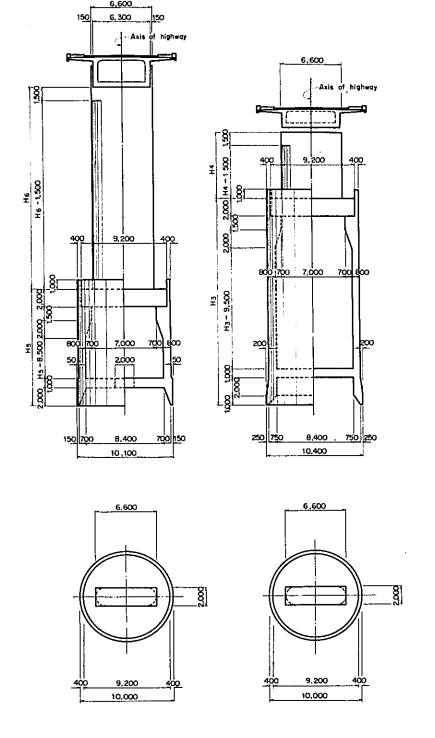
5,800

PIER AND OPEN CAISSON

PIER AND PNEUMATIC CAISSON

12,000

RAIL/HIGHWAY



PIER AND PNEUMATIC CAISSON PIER AND OPEN CAISSON

HIGHWAY

Remarks:

to be founded on the firm siltstone

OVERSEAS TECHNICAL COOPERATIO TOKYO, JAPAN	N AGENCY
NONG KHAI/VIENTIANE BRIDGE F CONCRETE BRIDG PLAN BRIDGE SUBSTRUCTUI	
NIPPON KOEI CO., LTD. T	
ORAWN CHECKED	DATE
SUBMITTED APPROVED	PLATE A. 2

