3.3 RELATED STRUCTURES

3.3.1 Crossings

The main and secondary canals would cross at many places over or under the tributaries, canals, roads and railway. Aqueducts, inverted siphons, culverts and bridges have been designed at these crossings, as required. All the crossings along the main canals are listed in Table 3.3.1 and Table 3.3.2. The hydraulic calculations relating to these crossings are presented in Table 3.3.3.

Design consideration on these crossings are as follows:

a) Aqueducts

The canals would cross a number of tributaries by aqueducts as far as possible (sufficient clearance can be maintained above the maximum flood water level). The design velocity at the aqueduct has been decided twice as much as that for the upper and lower canals from the economic point-of-view. Aqueduct has been designed rectangular in its shape and its top width has been made not less than 4 m so that it would serve as operation and maintenance bridge, too. Wasteway has been added as an accessory structure. The standard designs of aqueducts are shown in Dwg. WI-018 to Dwg. WI-020.

b) Siphons

A siphon is used where an aqueduct will not work. The design velocity at the siphon has been decided twice as much as that for the upper and lower canals from the economic point-of-view as well as non-silt nature of the flow. The shape of the siphon was made rectangular, with 1 barrel if the discharge is less than 10 m³/s and 2 barrels if over 10 m³/s. The top of the barrel (barrels) should be at least 2.0 m below river bed where the siphon passes under the tributary. The siphon will have accessory structures such as the gates, trashrack, wasteway, sedimentation basin and scouring sluice. As the siphon will cut the maintenance road into two, a maintenance bridge will be built at such point. (See Dwg. WI-014 to Dwg. WI-017.)

c) Culverts

A culvert will be used for railway crossing (see Dwg. WI-024 and Dwg. WI-025).

d) Bridges

Where the main canal passes under the road or crosses the tributaries by way of siphon, a composite girder bridge (concrete and H-steel beam) has been proposed. The T-20 wheel loading is applicable to highway bridge and T-9 for the maintenance bridge; the width will be 9.0 m for the former and 4.0 m for the latter. (See Dwgs. WI-O21 to WI-O23.)

e) Cross siphon

A circular concrete pipe was adopted for a small size siphon which serves for the existing canal passing under the main canal. The principal cross-siphon at the crossing point of the main canal and the existing canal will be equipped with an upper-stream gate to divert the main canal flow as required. (See Dwg. WI-O31)

3.3.2 Turnouts

The designs of the turnouts for the secondary canals are standardized into 4 types of gate diversion works to facilitate for controlling of discharge and maintenance work. A sluice gate and Parshall flume are used for a large-scale turnout and a Romain gate for an ordinary or small-sized turnout. The standardized designs of these structures are shown in Dwg. WI-026 and Dwg. WI-027.

3.3.3 Checks

Check gates are provided at downstream of the major turnouts to maintain the water surface, to lessen rapid changes in water surface, and to prevent high flow velocity. The check gate is a combination of the fixed overflow and manual operating gate types. They are standardized into 4 types as shown in Dwg. WI-028 and Dwg. WI-029.

3.3.4 Water Measurement Facilities

Installation of the water measuring facilities within the irrigation canal system is imperative for the effective use of the water stored in the Wonogiri Reservoir and for proper water management along the lengthy earthen canals. To meet these requirements, the Colo Weir intake will be equipped with a measuring weir by which to grasp the intake discharge therefrom and all together 4 measuring weirs will be built along the main canals: 3 for the Upper Sala main canal and 1 for the Dengkeng main canal; water measurement is also made possible, though roughly, at many check-gates along the main canals, and the diversion discharge will be known at each turnout. These water measurement facilities are illustrated in Dwg. WI-030.

3.3.5 Others

Apart from the structures mentioned above, it will be desirable to consider planting of shrubs and putting of fences along the canals as well as building of descending stairs into the canals, to prevent any possible loss of human and animal lives, for the amenities of the inhabitants and for the better operation and maintenance of the canals.

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Table 3.3.1 Number of proposed Irrigation Facilities

:										- 1	80 -			: · · ·	٠.				<pre>+ r; .</pre>		
Cross	1 2			2 13	2 15	2 18	2 20	2 6	1	9 1	1 7	13 92		J.	7	6	1 6	1 5	2 32	15 124	
Bridge (B)	2 3 4	(11)	3 15 5	2 10	4 23 5	5 15 5	8 10	9 13 3	18 1	2 13	2 5	35 122 19	(63)	7 20 2	5 23 1	3 4 4	5 8 2	7 1	21 62 9	56 184 28	
Culvert (C)	1 2 3 1	(13)				1	2	3	1	7	61	6 4 3 1	(1)	r r					r~l	6 5 3 2	
 Siphon	1 2 3 4	(19)	RV.		FU.	īv		8	r-i			5 5 5 4	(6)	2	ref	4	7		6	5 5 5 13	
 Aqueduce (A.)	2 3 4 5	(16)						1 6	H	n		1 6 1 3								7 Q J	
(W.M.F.)	1 2 1	(3)					1 5	1				3	(1)		p=4				г.	3 1 5	
Check-Gate (C.G)	1 2 3 4	(6)	1	1	Н	7	1	J			r	3 2 4	(4)		1		r -1	Ţ	4	3 2 4 4	
H	1 2 3 4	(36)	1 2	1 1 2	2 4	1.1.5	2 4	7	1 1	1	r-I	2 4 11 19	(12)	1 1 1	1.2	1 2	5	r-1 · .	80 FL	2 5 14 27	
Canal Type		Upper-solo Main canal		ΙΊ	III	IV	Λ	IV	IIA	VIII	IX	Sub Total	Dengkeng Main canal	Ħ	ΙΊ	I I	ΛI	Δ	Sub Total	Total	

Table 3.3.2 Type and Number of Crossings

										111		Sx XX	5 17.5 28.5		þ		3 1 3		ķ	•	
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			- ' '							ii	· ·		11 51 10		6		5 5 		5 1		3
M	9 1 51	ең о	HTUM	uor	siph S 1	seo. IXI	žΟ		θĮ	i tor'	đ	ave)	TO VVN	\$	чтрт.	ų :	rilli I	ng	Abe q&e	i III L	C-Section D 2.5x2.5x3 D 2.5x1.3x2 D 2.5x1.95x2 1 2.5x1.55 1 0.5x1.55
ً اے	1 1		. : 	1	I. I	in. Haral		· . · 	•	· -	i . I		. , . 		· ·		: '				C-Section - 2.5x1 - 2.
Siphon				13	15	1.8.	50	9	2	9	77	65	: -	2	7	6	. 9	2	32	124	Suffix C
Cross				5	2	2	2	73	7	H	1	13					н,	4	01	15	1
	2 3	6)		-				3	1			4	(6)	2	T :	4			2 7	6.7	Calvert 1ype
	3 1	(1	5		ŧ۲	5				13	10	18 15			23	4	PC .	7	42	60 15	
e (B)	1 2	(122)	15	10	23	15	10	13	18			63 41 3	(62)	ဂ္ဂ					20 4	63 61 6	4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
Bridge	2 6	(35)	1	1	(1	-	60	6		63	.01	17 4 ((21)	7	5	3	ľ.	ı	7 14	24 18 (2 Length 23 35 35 35 35 35 35 35 35 35 35 35 35 35
	2 1	. 4	6	63	4	ıń						4	9 (1)	FI				·	гH	1 14	C-Section 2.5x2.5x2 1 2.2x2.2x2 1 2.0x2.0x2 1 1.5x1.5
	1	-				П		-				-								۲	S
rt (C)	2	(3)						r	in Ev	-	71	6								3	S
Culvert(C)	1 7	(6) (4)				4	77		1	1.2		6 1	(3)				- ,		1	6	
	7	(4)		-				n				4	(6)		7	23	H		5	6	Siphon Type 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
ŀ	2		:			ਜ			_		-	7				2	H'		4	2 1 4	
(S) u	2 3 1 2	(5)			1	2 2					<u> </u>	1 2 2	_						1	1 2	
Sipho	7 7	(5)			3 1							3 7								٦ 1	Remark Verteway(W.W.) Non-W.W. Non-W.W.
	7 7 1	T.	4 1	:			:		7.		: '	4								1	Renark Vesteva, Non-W.W Non-W.W
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(A)	4	(7)						73	1	1		, C3								2 1	9 3 3 5 <u>8 8 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 </u>
Aqueduct	51 51 13	(1) (9) (1)						1 4				4		 						4	C.Section 1.2.0x2.2x2 1.2.8x1.95 1.3.5x1.8 1.8x1.55
Aq	m	(1 1					1 1								1 1	Suffix C-Section Profile 2
	1 2						2 1					7 7								2 L	1 W// 11V/11W/
Γ'	Canal type	Upper-solo			III							Sub total	Dengkeng Main Canal				2 -	Δ	Sub fotal	Totel	nd Agueduct Type 2 2 2 4 4 4 5 6 6 6 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7

녆		,,,	,																			
No. (m3/sec)		Water Depth(m	Type Water V Type Depth(m) (m/sec)	(m)	I 3,8.4.C		Trunsition Se Length(m)	Section (m)	А (m2)	я (m)	V)	hv 1 (m) (1x	I S. (1x10 ⁻³) Le	S.A.C E Length(m)	Велд	114	hec	b£	qų	Σhx1.1	şų	Total
NO.0+600 24.30	- 1	1.92	0.751	0.029	0.1667	w	18	25×25×2	12,340	0.647	1,969 0.	0.198 1.		35	2	0.031	0.085	0,055	0.040	0.232	0,150	0.38
NO.0+385 "	н.	=	I.	=	=	=	=	=	=	=		=	=	=	=	_	=	-	-	=	E	=
NO.2+965 23.76	76 I-2	1.90	0.747	0.028	11	£	=	ŧ	=	=	1.925 0.	0.189 1.	1.490	. =		0.030	0,081	0.052	0.038	0.221	=	0.37
No.4+300 23.	23.49 I-3	1.89	0.745	#	Ξ	=	<u>-</u>	£	=	-	1,904 0	0.185 1.	1.458	-	-	0.029	0.079	0.061	0.037	0.216	=	=
NO.54715 23.16	16 I.4	1.87	0.740	н	41		ĸ	ŧ	ч	±	1.877 0.	0.180 1.	1.417	55	<u>.</u>		0.076	0.078	0.036	0.241	=	0.39
NO.10+825 16.88	88 III-1	1 1,70	0.689	0.024	u	=	15	22x22x2	9.520	0.571	1.773 0.	0.160 1.	1.493	. 35	=	0.025	390.0	0,052	0.032	0.195	=	0.35
NO.11+875 16.88	: 88	E	2	=	=	±	#		н	: · =	=	=	=	=	=	-	F	 	=	=	= ,	=
NO.13+620 16.69	69 III-2	-2 1.69	0.687	Į.	t	=		=	н	=	1.753 0,	0.157 1.	1.460	-	=	0.024	0.067	0.051	0.031	0,190	=	0.3
NO.15+110 16.60	60 III-3	£	Ε	=:	. =	±	£	4	1	÷.	1.744 0.	0.155 1.	1.445	55	E	=	990.0	0.079	=	0.220	=	0.37
NO.19+450 15.09	9-III 60	6 1.60	0,666	0,023	a .	1	E.	=	11	±	1.585 0,	0.128 1.	1.193	45	=	0,020 (0.053	0.054	0.026	0.168	=	0.32
NO.22+15 14.44	44 IV-2	1.58	0,660	0.022	ŧ	=	=	20x20x2	7.910	0.517	1.826 0.	0,170 1,	1,808	35	-	0.030	0.074	0.063	0.034	0.221	<u>-</u>	0.37
NO.23+20 14.34	34 IV-3	1.57	0,658	=	E	=	=	E	н		1.813 0.	0.168 1.	1,782	45	,	0,029	0.073	0.080	=	0.238	=	0.39
NO.23+680 14.22	22 IV	1.56	0,656	E	=	ပ	t ~	25×23×3	11.580 (0.717	1,228 0.	0.077 0.	0.5287	4.70	1	0,005 (0,028	0.002	,	0.039		0.0
NO.24+15 14.22	22 IV-4	1.55	0.656	÷	±	S.	15	20x20x2	7.910 (0.517	1.798 0.	0.165 1.	1,753	35	2	0.029	0.072	0,061	0.033	0.215	0.150	0.37
NO.26+785 13.77	77 IV-6	1.54	0.651	0.021	# 1 E	=	= :	· · =	=	=	1.741 0.	0.155 1.	1.644	55	=	0.027	0.067	060*0	0.031	0.237	=	0.39
NO.27+740 "	A	.=	=	. =	#	٥	7	25x23x3	11,430 (0,713	1,205 0.	0.074 0.	0,5129	4.70		0.00.0	0.027	0,002	,	0.037	,	9.0
NO.28+60 13.12	12 "	1.49	0.639		=	=	=	ı.	11,055 (0.702	1.187 0.	0.072 0.	0,5081				0.026	=	,	0.036		-
NO.28+630 "	7-VI	Ħ.	F	=	z.	υ	15	20x20x2	7.910 0	0.517	1,659 0.	0.140 1.	1,492	45	2	0.025 (0,060	290.0	0,028	0,198	0.150	0.35
No.29+300 "	A	=	=	=	<u>n</u>	Đ	7	25×23×3	11,055 0	0.702	1.187 0.	0.072 0.	0,5081	4.70		0.005	0,026	0.002	,	0.036		0.0
NO.30+80 11.55	55 V	-	299.0	0.023	0.1818	=	۲۵	25x22x3	11	" 1	1.045 0.	0.056 0.	3938	=	0	0.003	0.017	=	1	0.024		0.02
NO.30+550 "	V-1	Ξ	E	2	#	Ą	12	20x22x2	5,880 (0,620	1,964 0.	0.197 1.	1.642	40		0.022 (780.0	990.0	1	0.193		0.19
No.31+425 11.40	40 V-2	3,48	0,664	‡	=	,:=	÷	н	5.840 (0,618. 1	1.952 0.	0.194 1.	1,629	50	1.		0.086	0.081		0.208	,	0.21
NO.32+435 "	=	=	=	*	ı.	=.	=	н	п	=	r.	=	=	40		=	=	0,065	۱,	0,190	,	0.19
NO.35+30 10.63	53 V-4	1.42	0.649	0,021	=	2	=	ŧ	5.600 0	0,608	1.898 0.	0.184 1.	1.574	-	١	0.021	0.082	0.063		0,183		0.18
Notes by m Privation lane of the said																						

Table 3.3.3 (continued)

and Many	ог неад	hf hb Zhxl.1 hs Total	0.079 - 0.200 - 0.20	0.002 0.021 - 0.02	0.074 - 0.163 - 0.16	0.053 - 0.140 - 0.14	0.059 0.030 0.188 0.150 0.34	0.053 - 0.140 - 0.14	0,042 - 0,128 - 0,13		F F	0.002 - 0.021 - 0.02	0.055 0.027 0.173 0.150 0.32	0.042 - 0.124 - 0.12 5	0,002 - 0,019 - 0,02 (0.046 0.023 0.142 0.150 0.29	0.038 - 0.105 - 0.11	0.002 - 0.011 - 0.01	0.056 0.021 0.151 0.150 0.30	0.055 - 0.11 0.13:	0.044 - 0.099 - 0.10		0,039 - 0,086 - 0,09	- 0.086 -	- 0.086 - 0.023 - 0.004 -
1	Loss	hec l	0.082 0.	0.014	590.0	۳ 0	0	11	n 0	u		0.014	0.059	0,062	0.012	H.	0.047	050.0	900.0	0.045	0.040	۵ "		0.033	0.033	0.033
		Bend ht	- 0.021	0.003	600.0 -	=	2 0.017	600*0 -		#	=	0.0003	2 0.016	600*0 -	0000	£	2 0.014	0.007	0,002	2 0.015	900*0 -	=		900°0 -		
	ĺ	S.A.C Length(m)	20	4.70	70	50	35	20	40	£	В	4.70	35	40	4.70	ij	35	94	8.0	35	50	40		40	40	4
) (m) (1×10 ⁻³)	0.184	0.047 0.3566	0.148 1.061	=	0.146 1.684	1,061		=	=	0.046 0.3585	0,137 1,560	0.143 1.046	0.042 0.3383	11	0.113 1.281	0.117 0.9495	0.029 0.2371	0.103 1.592	0.095 1.091			3 0.079 0.9812		1 1 1
	i t	R V (m) (m/sec)	0,608 1,898	0.669 0.963	0.694 1.702	# . E	0.492 1.705	0.694 1.702	= .	#	11	0.657 0.954	0,492 1,641	0.686 1.677	0,632 0,903	=	0.492 1.487	0.632 1.513	0.756	0.390 1.420	0,486 1,361	# #		0.462 1.248	1 1	1 1 1
	Siphon, Aqueduct & Culvert	Section A (m) (m2)	5.600	25x22x3 9.930	28x1.95 3.572	=	1.9x1.9 3.565	2.8x1.95 3.572	=	# #	п,	25x1.95x2 6.370	1.9x1.9 3.565	2.8x1.95 3.488	25x1.95x2 5.870	#	1.9x1.9 3.565	25×1.8 2.935	2.5x1.8x2 5.870	1.5x1.5 2.205	l.8xl.55 l.742	#		н 1,562	1.55	1 1 1
	Siphon,	Transition Section Length (m) (m)	12	10	7	r	6	7	11	-	=	5 2	1 6	7 2	5 2	£	9 1	6	5 2	8 1	4.5	#				
		Ix10-3) S.A.C	¥	. 0 #	0,2500 A	.a	σ 3	٧		+	1	0	e W	Ψ μ	D =	=	S	V .	D #	50	0.2857 A	# #		-		333
) (u)	0,021	0.020	0 610°0	=	z	· .	=	Ξ	=	±	= :::::::::::::::::::::::::::::::::::::	=	£	=	=	0.017	Ξ	0.014	0.015	ŝ		0.013		0.013
	Canal	er v h(m) (m/sec)	1	1.34 0.629	1.29 0.613	п		=	F	-		ii ii	1.26 0.605		1,19 0,587	#	= = = = = = = = = = = = = = = = = = = =	" 0.573	μ	0.980 0.516	" 0.535	11	-	0.880 0.506	1 1	1 1 1
	ģ	Type Depth(m)	4	Δ	vi-1	±	=	11	=	=	=	ΙΛ	VI-2	=	ΔI	n	VI-3	VII-1	VII	VII-2	VIII-2	=		VIII-2	VIII-2 VIII	VIII-2 VIII
		(m3/sec)	10, 10,63	80 9.56	90-9 01	310 "	ı. 068	046	525	375 "	385 "		700 5.85	1 088	150 5.30	# 009	760 5.30	735 4.44	330 "	100 3.13	30 2.37	125 "		450 1.95		1 1
	40+3	No.	NO.36+40	NO.39-280	NO.40-210	NO.40+810	NO.42+390	NO.42+940	NO.43+625	NO.43+875	NO.44+385	NO.45	NO.45+700	NO.45+880	NO.47+450	NO.48+600	NO.48+760	NO.50+715	NO.51+330	NO.52+100	NO.54+30	NO.55+125		NO.57+450	NO.57+450 NO.58+450	NO.57+450 NO.58+450 NO.59+150

Table 3.3.3 (continued)

Discharge		ł	ကျ	Canal				Siph	Siphon, Aqueduct & Culvert	t & Cul	vert							ŭ	Loss of Head	ađ			
No. (m ³ /sec) Type Nater V hv I -3 S.A.C Transition (m) (1xlO-3) S.A.C Tength(m)	hv (п)	hv (п)	hv (п)	hv (п)	(1×10 ⁻³) S.A.C	S.A.C	ਰਮ∣	ransition ength(m)	on Section (m)	A (m2)	R (m)	(m/sec)	hv (m)	I S.A.C (1x10-3) Length(m)	S.A.C Length(m)	Вепд	1 4	hec	३प	рp	Zhxl.1	sų	Totel
0.018	0.018	0.018	0.018	0.018	0,2500 0	D			2.5x1,8x2 5.920 0.634 0.878 0.039	5.920	0,634	0,878	650.0	0.3185	8.0	ŧ	0.003	0,011	0.003	ı	0.019	1	0.02
NO.2+815 " I(L)-1 " " " s	11 H H 11	11 H H 11	И	н		ß		. 6	1.9x1.9	3.565	0,492	1.459	3.565 0.492 1.459 0.109 1.233	1.233	35	73	0,013	0.046	0.043	0.022	0,136 0,150	0,150	0.29
			T II II II	п п	II.	F		ı	1	n.	÷	#	=	±	30.	ŧ	±	=	0.037	ε	0.130	£	0.28
NO.11+210 2.96 $\overset{11}{1}^{(L)}$ - 1.050 0.550 0.015 0.2702 "	1,050 0,550 0,015	1,050 0,550 0,015				Ę.		6]	l.5x1.5	2,205 0,390	0.390	1.342	1.342 0.092 1.422	1.422	35	=	0.010 0.039	1	0,050 0,018	0.018	0.129	F	
NO.20+130 1.78 III(L)- 0.850 0.481 0.012 " " "	H H	H H	H H	H H	Į.	#	-		1.1x1.1	1,165	0.288	1.528	1,165 0,288 1,528 0,119 2,762	2,762	30	=	0.018 0.054 0.083	0.054	0,083	0.024 0.197	261.0	=	0.35
NO.21+30 1.55 III(L)-0.790 0.463 0.011 " " "	0.011 " "	0.011 " "	0.011 " "	п	u u	n t	-		· #	έ	=	1,330	1.330 0.090 2.093	2,093	30	£	0.014	0.040	0.063	0,018	0.149	=	0.30
No.21+660 " " " " " " " " " " " " " " " " " "							-		Ξ	12			ŧ	±	35	Ξ	£	Ξ	0.073	¥	0,160	F	0,31
NO.224125 " " " " " " " "		н н н н		- F	£	F	-	_		±	ı.	Ţ.	ii	E	÷	; =	£	=	- -	F	-		
NO.25+190 0.94 V(L)-1 " 0.437 0.010 0.2857 "	± '	± '	0.437 0.010 0.2857 "	0.010 0.2857 "	0.2857 "	= '		3	0.85x0.85 0.703	0.703	0,222	0,222 1,337 0,091		2,992	# .	-	0.010 0.041		0,105	Ŧ	0.191	=	0.34
0.81 V(L)-2 0.720 0.417 0.009 "	V(L)-2 0,720 0,417 0,009				- - -	=	-	_	=	= .	- -	1,152	0.068	2,222	20	=	0.008	0.030	0.067	0.014	0.131	=	0.28
							ŀ																

4. CONSTRUCTION PLANS

4.1 BASIC CONSIDERATIONS

Basic conditions taken into account for planning the construction method and schedule are presumed as follows.

- (1) Workable days: 180 days a year (May Nov)
- (2) Construction period: 5 years
- (3) Division of main canal construction work:

4 sections on the Upper Sala main canal and 1 section on the Dengkeng main canal;

The sequence of construction is shown below

Table 4-1 Division of Work and Sequence of Construction of Main Canal

		Main		Construc-			Related 8	Struct	ure	
Secti	ion	Canal No.	Length	tion year)	Siphon	Aque- duct	Culvert	Turn- out	Check gate	Bridge
1	Righ	t No.0-60 - No.15	O 16,600) lst	9	. 	 	10	2	61
2	11	No.15 - No.30+50	0 15,500) 2nd	6		. 5	11	4	34
3	n	No.30+5 - No.45+5	00 00 ¹⁵ ,000	3rd	1	11	2	8		29
4	B	No.45+5 - No.60+8	00 00 ¹⁵ ,300	5th	3	5	6	7	3	53
5	Left		31,350	4th	9	٠ 🗕	1	12	4	. 93

Table 4-2 Division of Work on Secondary Canal

<u>.</u>	ection	Lengt	h (m)		Construction
96	\$ C 0 T O I I	Secondary canal	Rehabilitation canal	Total	year
	1	5,200	10,000	15,200	lst
	2	5,150	7,850	13,000	2nd
	3	19,300	7,700	27,000	3rd
	4 .	1,950	7,050	9,000	$5 \mathrm{th}$
	5	9,600	7,400	17,000	4th

(4) Employment of Manpower throughout the Construction Period

Although good efforts have been made to maximize labor employment for the construction work, a constant labor force which will be daily mobilized would remain at about 8,500, as some part of the work requires an even and exact finish which can be expected only by use of machinery (such as embankment work) and, moreover, the entire construction work must be completed within five years. Hence, most of the earth work called for construction of Colo Weir and the Main Canals would primarily be done by machinery. The total labor force required for the construction work would amount to 15,000/days, if the entire work should be done mainly by man-power.

Allocation of Construction Force:

Colo Weir and Main Canals

Excavation and embankment: 10 % man-power & 90 % mechanical

Secondary canals

Excavation and embankment: 100 % man-power

Farm ditches, Drains & Roads: 100 % man-power

4.2 COLO DIVERSION WEIR

The construction of Colo Weir shall be started in the second year and completed within 2 years, as per Fig.4-1: Construction Schedule. Its start in the second year has been determined from two reasons: the one is the availability of the construction road (from Surakarta-Wonogiri highway to Colo Weir site) through the main canal work (the first construction section) which is scheduled to be completed in the first year, and the other is unavailability of construction machinery through wear and tear during the Wonogiri Dam construction inspite of the assumptions made in the preliminary feasibility study on the latter. The construction period lasting for 2 years will be divided into the first stage (1st year) and the second stage (2nd year) as follows:

First Stage

This stage will be commenced by constructing the temporary structures, provision of power supply facilities, jungle clearing, etc., and followed by excavation of the weir-site. Excavation shall be made by use of bulldozers for the first 1-2 meter depth and, then, by crushing using shovels and back-hoes. The final finishing to the designed section will be carried out by man-power. The spoil (excavated earth) shall be moved out toward both the upper - and the lower - streams as the material for constructing the closing levees, and the remainder shall be deposited at the places shown on the map. After the excavation work will have progressed to the designed ground level, a series of concrete work such as pouring of concrete for levelling - bar arrangement - shuttering - concrete pouring - removal of shuttering, will follow until the weir and the intake will be built in their final shape. Then, gate will be installed. In the meanwhile, riprapping and slope protection work shall be carried on so that such work will be completed simultaneously with that of the weir. Drainage of the groundwater by pumps will become necessary during the excavation work.

Second Stage

Embankment work along the right bank will be started by pushing out the excavated earth toward the river-bed for construction of the closing-levee; wet masonry work will follow along the right bank. After completing the right bank, the closing levees at both the upper - and the lower-streams of the weir and along the right bank shall be removed and a new closing-levee needs to be built across the Sala river to divert the river water into the new channel. Wet masonry work along the left bank built by use of the deposited spoil as well as the material used for the closing-levees will follow. Because of the high-banking required for construction of the embankment work, geological conditions must be confirmed through boring at several points along the bank-alignments to avoid sliding or seepage of the banks.

4.3 CANALS

Canal construction work will be divided into 5 sections: 4 along the Upper Sala Main Canal and 1 for the entire Dengkeng Main Canal. For the perfection of their earth work, the construction period will be limited during dry season, starting in May and finishing in November. Each construction section takes 1 year for completion and their work schedule is as shown in Fig.4-1: Construction Schedule. The initial work for canal construction will be the excavation which is to be started from the point easily accessible to the construction machinery, and all the excavated earth shall be used for embankment. The embankment material shall be layered as

soon as excavated and well compacted; spraying of water by Water Tanker may be required to give the material the optimal moisture content. Embankment work needs uniform compaction; otherwise cracking will cause leakage after the work. When the spoil does not meet full requirement of embankment, the material earth will have to be brought in from elsewhere and if surplus, vice-versa. Crossing structures across the tributaries will be worked out by Copure Method as far as possible to do without closing-levees during their construction period. Their construction will be easier by Copure Method, although it will require more excavation work. Each construction-section shall be linked at its end to the existing tributary and/or drainage canal so that thorough running of irrigation water will be made possible.

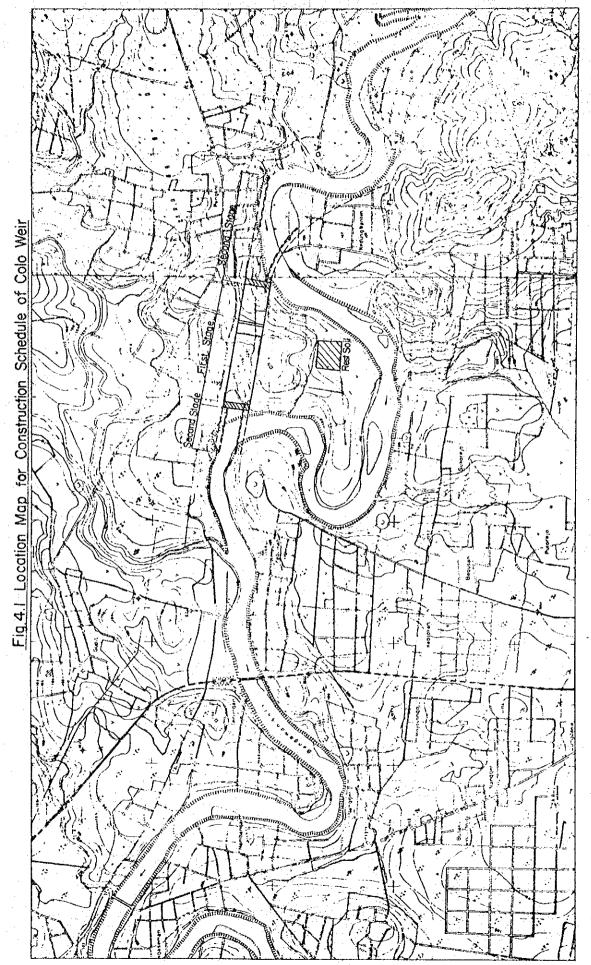
Secondary canals are to be built entirely by man-power, but their embankment needs to be compacted as carefully as with the case of main canal; necessary material will have to be brought in by dumptrucks.

4.4 FARM DITCHES, DRAINAGE AND ROADS

3 years' time is allocated for construction of farm ditches and drainage, and 4 years' time for road construction. Construction of farm-ditches and drainage must be very much facilitated if road network will have been completed in advance of 1 year's time. Their work depends entirely on man-power.

Table 4-3 Construction Plants and Equipment for Irrigation Project

No.	Equipment	<u>Capacity</u>	<u>Quantity</u>
1.	Bulldozer	20 ton	15
2.	Crawler loader	2 m^3	5
3.	Back hoe	1.2 m^3	6
4.	Dump truck	8 ton	90
5.	Motor grader	3.7 m	3
6.	Vibration roller	5 ton	3
7.	Fuel tanker	8 ton	2
8.	Water tanker	8 ton	2
9.	Trailer truck	30 ton	1.
10.	Cargo truck	6 ton	10
11.	Grease car	6 ton	5
12.	Truck crane	30 ton	1
13.	Portable concrete mixer	0.1 m ³	4
14.	Concrete plant	14 cft x 2	1
15.	Agitator truck	3.2 m ³	3
16.	Pump w/engine	4 inch	8
17.	Road roller	8 ton	2
18.	Screening plant	30 ton/hr	1
19.	Repair shop		1
20.	Saw mill		1
21.	Ripper attachment Bull	20 ton_	2
22.	Dragline attachment show	. 17	3
23.	Power shovel attachment	1.2 m ³	3
24.	Miscellaneous		1



SCALE = 1:20,000

Proposed Construction Schedule Fig. 4-2

								- 191	<u>.</u>		
	1982	ASONDUFMAMUJASOND									
	1861	JEMAMJJASOND.									
	0861	FMAMJJASONDJI									
٠.	79	FIMAMUJAISIONOJE									
on Schedule	-	UDNOSVO									
osed Construction		JFIMAMJJASIONIDJF									
-2 Proposed		Q' 7 Y	11			429000m3 313000m3 310000m3 4 000m2 6 00s 8 cm	844 O O m3 10 0 0 35 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	οα - 44C ΣΕ ΣΕ	867.000m ² 2.29 62.000m ²	2000 10	2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
Fig. 4		TEM	SPECIFICATIONS	8 COMPENSATION	S	Excovation Embankment Concrete Structue Revetment Riprop Gates Endes	7.8 [= 6600m Gate augs [= 5200 E= 5500m		Main Cendis L=15 000m Siphon Aqueduct Aqueduct Luncut Tumout Burdon Secondary Canals 1=27000	Moin Carals L=15300m Siphon Aqueduct Culvert Turnout Check Gate Bridge Secondary Carals L=9000	8
		WORK	DESIGNS & SPECIFIC	LAND ACQUISITION 6	PREPARATORY WORKS	COLO DIVERSION WEIR	SECTION 1 (RIGHT)	SECTION 2 (RIGHT)	SECTION 3 (RIGHT)	SECTION 4 (RIGHT)	SECTION 5 (LEFT) FARM DITCH DRAINAGE FARM ROAD

5. CONSTRUCTION COST ESTIMATES

IRRIGATION PROJECT

Estimation of the construction cost has been made on the grounds as enumerated below:

i) Unit cost

References have been made to the following projects, especially a) and b):

- a) Sempor Dam (comparison of unit prices and lump sum);
- b) Brantas (Dec. 1975), and
- c) Karangkates Dam.

ii) Machinery cost

Unit price = Domestic price in Japan + Freight + Inland transport Cost (Surabaja - Surakarta)

Depreciation cost = $\left(\text{unit price} - \left(0.1 \times \text{unit price}\right)\right) \times \frac{\text{used year}}{\text{total life year}}$

Spare parts and consumables = 20% of the depreciation cost.

- iii) In such kind of work where excavation is done by use of the machinery, 10% of the total amount of the earth will be handled by man-power in its finish.
- iv) Embankment is to be done by using the excavated earth and, only when it is insufficient, the necessary amount of the material earth will be brought in by dump trucks from the borrow pits.

The cost of land acquisition and compensation has been estimated from the sizes of the land deemed appropriately necessary for the Colo Weir, the main and secondary canals, farm-ditches, drains and the roads as is shown in Table 5.1 .

Table 5.1 Cost Estimate for the Irrigation Project (Economic cost)

US\$1000) (Unit: Item Foreign Local Total 18,974 19,326 38,300 ī, Civil Works (270) (270) Preparatory Works (2,490)(1,680) (4,170)Colo Diversion Weir (10,150)Main Canal (8,570) (18,720)(620) (1,510)(2,130)Secondary Canal (1,330) (1,330)Farm Ditch Drainage Facilities (2,060) (2,060) (1,100)(3,750)(4,850)Farm Road (4,614)(156) (4,770) Construction Machinery 200 200 II. Land Acquisition & Compensation 5,700 2,816 2,884 III. Contingency IV. Engineering & 2,000 500 2,500 Administrative Expenses 23,790 22,910 46,700 Total

Table 5.2 Fund Requirement (Contract Base)

(Unit: US\$ 1,000) Item Foreign Local Total Civil Works 19,230 33,620 52,850 Construction Machinery 6,210 260 6,470 5,270 5,270 Land Aquisition Contingencies 2,880 4,490 7,370 Engineering & Administration 3,200 790 3,990 Expenses 44,430 Total 31,520 75,950

Table 5.3 Fund Requirement (Force Account Base)

		(Unit:)	US\$ 1,000)
Item	Foreign	Local	Total
Civil Works	19,230	33,620	52,850
Construction Machinery	6,640	2,040	8,680
Land Aquisition	-	5,270	5,270
Contingencies	4,000	4,480	8,480
Engineering & Administration Expenses	4,400	2,470	6,870
Total	34,270	47,880	82,150

		ran nyawan), mangkarangi a dalah sarihid ad Sakain dalah dal	(Unit: US\$
	Item Quantity	& Unit Cost	Cost
I	Irrigation office personnel Expenses 20 x 120 Others L.	00 \$/year S.	24,000 22,000
	Sub total		46,000
II	Colo Weiv office personnel Expenses 4 x 1200 Others L.S	0 \$/year •	4,800 1,200
	Sub total		6,000
III	0. & M for turnout personnel Expenses 50 x 96 (canal rider)	O \$/year	48,000
IV	O. & M for Main canals & secondary canals Desilting work & treatment Fuel Expenses of Vehicles Sub total	L.S. L.S.	30,000 20,000 50,000
V	O. & M for tertiary canals, farm ditches & drainage ditches. Operation cost for irri, canal Desilting work & treatment for irri, canal and drainage. Sub total	L,S. L,S.	50,000 40,000 90,000
VI	Depreciation Cost of 0 & M facilities (office building, wireless equips & gates)	L.S.	100,000

Table 5.5 Statement of Items

. Preparatory Works

					190 -
 Foreign Currency	Amount		, 1 - , - , - , - , - , - , - , - , - ,	I	I.
Foreign	Unit Price	 	ı	i	
Local Currency	Amount	130,000 55,000 37,000	222,000	48,000	270,000
Local	Unit Price	130.0			
Total	Amount	130,000 55,000 37,000	222,000	48,000	270,000
Ψ	Unit Price	130.0	·		
	Quantity	1,000			
	Unit	m2 " sum			
	Work	Office camp house Field camp house Attendant construction (20%)	Sub total	General Expenses (20%)	Total
1.4.7	No.				

II. Colo Diversion Weir

				∏ 0±0]:	1000	1501010101		
Work	Unit	Quantity	Unit Price	Amount	Unit Price	Amount	Unit Price	Amount
Excavation (Man)	Е	12,000	9.0	7,200	9.0	7,200	l	1
" (Mechanical)	=	108,000	0	54,000	4.0	43,200	0.1	10,800
" Breccia-Taff (Man)	±	15,000	1.5	22,500	1.5	22,500	1	1.
" (Mechanical)	ŧ	294,000	0,	294,000	8.0	235,200	0.2	58,800
Compacted fill (Man)	E	3,000	0.5	1,500	0.5	1,500	1	1
" (Mechanical)	£	25,000	4.0	10,000	0.3	7,500	0.1	2,500
Embankment (Man)	=	31,000	8.0	24,800	8.0	24,800	1	
" (Mechanical)	E	282,000	6.0	253,800	9.0	169,200	0.3	84,600
Plain concrete	=	23,364	42.0	981,288	10.0	233,640	32.0	747,648
Reinforced concrete	E	7,695	46.0	353,970	10.0	76,950	36.0	277,020
Reinforcing bar	ton	316	460.0	145,360	70.07	22,120	390.0	123,240
Form (A)	m2	15,116	8.0	120,928	0*9	90,696	2.0	30,232
Revetment (stone pithing)	=	23,602	13.0	306,826	11.0	259,622	2.0	47,204
" (sod facing)	E	56,675	0.3	17,002	0.3	17,002	1	1.
Riprap	=	4,181	17.0	71,077	0.2	29,267	10.0	41,810
Screen	=	7.7	10.0	770	0.9	462	4.0	308
Concrete pipe (ø 1800)	Ħ	170	100.0	17,000	80,0	13,600	20.0	3,400
(6 1100)	=	96	70.0	6,720	55.0	5,280	15.0	1,440
Roller type Gate (7.5 x 655m)	sum	7	150,000	300,000	20,000	40,000	130,000	260,000
(5.0×2.7)	E	<u>ش</u>	000,06	270,000	15,000	45,000	75,000	225,000
(4.5 x 2.7)	=	н	80,000	80,000	13,000	13,000	67,000	67,000
Gate valve $(\phi 1100)$	=	6 1	13,000	26,000	2,000	4,000	11,000	22,000
Operation house	m2	100	100	10,000	100	10,000		1
" Bridge (H-Steel)	دب	66	880.0	87,120	220	21,780	099	65,340

					(Unit: US\$)
Item	Work	Unit Quantity	Total Unit	Local Currency Unit	Foreign Currency Unit
.			Price	Price	Price
	Temporay Works	sum 1	10,000	000,6	1,000
	Other		2,889	1,231	1,658
	Sub total		3,480,000	1,405,000	2,075,000
	General Expenses (20%)		000,069	275,000	415,000
	Total		4,170,000	1,680,000	2,490,000

III. Main Canals

			III. Main	Main Canals				(A)	(Unit: US\$)	
				Ţ	Tota1	Local	. Currency	Foreign	1 Currency	
Ltem No.	Work	Unit	Quantity	Unit Price	Amount	Unit Price	Amount	Unit Price	1 1	
2	Excavation (Man)	E E	163,000	9.0	97,800	9.0	97,800	ŧ		
, H	" (Mechanical)	=	1,465,000	0.5	732,500	4.0	586,000	0°1	146,500	
∞	Embankment (")	=	1,628,000	6.0	1,465,200	9.0	976,800	0.3	488,400	
ī.	" (Truck-Mecha)	=	570,000	1,5	855,000	1,0	570,000	0.5	285,000	
4	Sod facing	m2	817,000	0.3	245,100	0.3	245,100		1	
	Related Structure									
3-1	Turnout	s um	48		625,800		476,900		148,900	
3-2	Check gate	t	13		489,500		147,900		341,600	-
3-3	Aqueduct	=	16		1,343,400	r *	510,400		833,000	
3-4	Siphon	£.	28		3,093,500		1,321,100		1,772,400	19
3-5	Culvert	· =	Н 4		115,000		58,200		56,800	9 -
36	Bridge	£	270		3,837,000		1,099,400		2,737,600	
3-7	Water Measurement Facility	E	4		32,900		21,600		11,300	
8-8	Inverted Siphon	F ,	139		2,677,700		1,034,800		1,642,900	
	Sub total			٠.	15,610,400	<i>:</i>	7,146,000		8,464,400	
							· :			
	General Expenses (20%)				3,109,600		1,424,000		1,685,600	
					. *					
÷	Total				18,720,000		8,570,000		10,150,000	
					:					

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	(Unit: US\$)	Foreign Currency	Amount Price Amount	31,860 -	42,480	300 0.2 13,700	27,600	300 2,000 206,000	300 2,400 247,200	50,000	160 47,100	514,000	20	106,000	000 620,000
		Local Curren	Onit Price Amo	9.0	0.8 42,	1.8 123,300	0.3 57,	7,100 731,300	1,100 113,300	50,	114,160	1,264,000		246,000	1,510,000
	F	Total	ce Amount	6 31,860	8 42,480	0 137,000	3 57,600	937,300	360,500	50,000	161,260	1,778,000		352,000	2,130,000
IV. Secondary Canal		+ : -11	guantisy unit Price	53,100 0.6	53,100 0.8	68,500 2.0	192,000 0.3	103 9,100	103 3,500	1					
뒤		**************************************	3 1170	m E	t	(Truck-Man)	C1 E	Sum	3-3)	canal "	action (10%)			(20%)	
		Item	No.	2 Excavation (Man)	O Embankment (")	onal) " o (Truo)	4 Sod facing	3-1-4 Turnout (Type 4)	3-6-8 Bridge (" 3-	Rehabilitation of canal	Attendant construction (10%)	Sub total		General Expenses (20%)	Total
			İ		10	_	14	۲ ۲	۲ ۲	4-1					

			V. Farm Ditch		L = 928,000 m			-	
								(Unit: US\$)	
-					Total	Local Currency	ncy	Foreign Currency	
Ltem No.	Work	Unit	Quantity	Unit Price	Amount	Unit Price A	Amount	Unit Price Amount	:
V-1	Rehavilitation Ditch	E	475,200	1.04	494,208	1.04 49	494,208	1	
V-2	New "	E	452,800	1.14	516,192	1.14 51	516,192	l .	
	Attendant Construction (10%)	Sum	д		101,000	10	101,000		
	Sub total	·			1,111,400	1,11	1,111,400	1	
	General Expenses (20%)				218,600	21	218,600	1 · ·	
	Total	.e.		·	1,330,000	1,33	1,330,000		

е (
000
= 928
))
Drainage
VI.

				*		¥		- 202 -	
(Unit: US\$)	Foreign Currency Unit Amount Price	1	1	1	i T	· · · · · · · · · · · · · · · · · · ·			
(Unit	Foreign Unit Price	1	ŧ	1	1	. 1	i		
	Currency Amount	437,184	1,124,608	156,208	1,718,000	342,000	2,060,000	٠.	
·	Local Cu Unit Price	1.38	1.84		7				
(Total Amount	437,184	1,124,608	156,208	1,718,000	342,000	2,060,000		
(L = 928,000 m	To Unit Price	1.38	1.84		러		21		
VI. Drainage (Quantity	316,800	611,200						
VI	Unit	Ħ	=	Sum					
	Work	Rehabilitation Drain	New "	Attendant Construction (10%) Sum	Sub total	General Expenses (20%)	Total		
	Item No.	VI-1	VI-2		٠				

							D)	(Unit: US\$)	;
ī					Total	Local Currency		Foreign Currency	
ltem No.	Work	Unit	Quanti ty	Unit Price	Amount	Unit Amount Price Amount	nt	t ce Amount	
VII-1	VII-1 Rehabilitation Road	H	548,800	4.86	4.86 2,667,168	3.66 2,008,608		1.20 658,560	
VII-2	New "	E	147,200	6.88	6.88 1,012,736	5.68 836,096		1.20 176,640	
	Attendant Construction (10%)				367,096	284,296	96	82,800	
	Sub total				4,047,000	3,129,000	0.0	918,000	
	General Expenses (20%)				803,000	621,000	00	182,000	
	Total				4,850,000	3,750,000	00	1,100,000	

(L = 708,000 m)

VII. Farm Road

VIII. Depreciation cost of construction plant & Equipment

v .							. '					- 2	04						: :				
	· ·	Depreciation	cost	675,000	206,200	501,400	1,417,500	78,700	18,000	45,000	45,000	52,500	101,200	62,500	82,500	30,000	56,300	59,400	12,000	25,700	96,400	000,6	4,500
	(Unit: US\$	Depr	year	75	25	30	450 1	ZI.	10	10	10	ľV	50	10	. rv	50	ι Λ	15	40	10	ľΩ		1
ment		Total	Life year	06	30	42	360	18	12	∞	Ø	9	. 04	∞	9	12.	∞	15	24	14	Ľ	· I	l I
plant & Equipment		Total Price	Amount	000,006	275,000	780,000	1,260,000	105,000	47 000 000	300	+ 400,000,000,000,000,000,000,000,000,000	70,000	+ +	+ +	110,000	+ 5,000	100,000	66,000	8,000 1,000 1,000 1,000	40,000	150,000	10,000	5,000
construction		L	Unit Price	60,000	55,000	130,000	14,000	35,000	12,000	20,000	20,000		000,6	25,000		2,000		22,000	1,000	20,000			
st of			Quantity	15	ľV	9	90 +22	٣	7	2 +1	7 + 7	H	10 +1	5 + +	٦.	4+	rđ	m	& + 17		Н	Н	٦
Depreciation co			Unit	No	Ξ	F	=	E	=	ŧ	F	Ε	=	ŧ	Þ	5 .	=	E	÷	Ξ	Ε	r.s	ւ. Տ
VIII.	:			20 _¢	20m3	1.2 ^{m3}	8ton	3.7m	حا حا	۰ حد	±2 €	30 _¢	+ >	ۍ ب	30 t	0.1	$14^{cft}x_2$	3.2m3	4 inch	φ Q	30/hr		
	. *		Work	Bulldozer	Crawler loader	Back hoe	Dump truck	Motor grader	Vibration roller	Fuel tanker	Water tanker	Trailer truck	Cargo truck	Grease car	Truck crane	Portable concrete mixer	Concrete plant 14	Agitator truck	Pump w/engine	Road roller	Screening plant	Repair shop	Saw mill
			vem No.			•							-										

(to be continued)

			-				(Unit: US\$)	
				Total Price	Total		Depreciation	
Work	Unit	Quantity	Unit Price	Amount	Life year	λĈ	year cost	
Ripper attachment 20 ^t Bull	No	. 2	8,000	16,000	2.5		10 12,000	
Dragline attachment 1.2 ^{m3} Shovel	=		10,000	30,000	21		15 19,300	
Power Shovel attachment	ي تــ = ـ :	м	10,000	30,000	21		15 19,300	
Sub total				+ 410,000 4,459,000 (4,869,000)			3,845,400	
Torce account) Spare parts & Consumables				821,000			769,000	- 205
Labour				1,200,000	(1,200x200x5)		156,000	
Sub total				2,021,000			925,000	
Total			٠.	6,890,000			4,770,000	

IX. Land Acquisition & Compensation

US\$		Amount					• 1										
(Unit: US\$		Unit Price												•			
	1 Price	Amount		2,237,066	822,051	19,280	90,100	3,168,497			144,600	72,300	62,640		279,540	3,448,037 /7	3,450,000
	Financial Price	Unit Price		3,614	2,169	964	2,650				723	1,446	3,132				
	ic Price	Unit Price Amount		. 1	ı	I	27,846	27,846			86,800	43,350	37,600		167,750	195,596	200,000
	Есопош	Unit Price		1	ı	1	819				434	867	1,880				
		Quantity		619	379	50	34	1,052			200	500	20		270		
		Unit		ਲ .ਧ	=	E	Ξ		٠.		No	=	E				
						,											
		Work	Land Acquisition	Sawa – Irrigated	" - Rainfed	Тедај	Yard	Sub total		Compensation	Ватьоо	Wooden	Brick		Sub total	Total	
		No.					•										

Note: /1 This amount does not include price escalation.

Cos	
Unit	:
	r
Table 5.6	
Enil	

Excavation Nork Unit Quantity Total Unit Price Portion Portion Post					**************************************
Excavation Soil (Mechanical) " (Man) Breccia—Taff (Man) " 1 0.6 0.6 Breccia—Taff (Man) " 1 1.5 1.0 Bibbankment Dring Soil (Truck)(Mechanical) " 1 1.5 Excavation Divert D=300m " 1 1.3 Excavation Divert D=300m " 1 0.9 (Mechanical) " (Mechanical) " 1 0.7 0.5 (Man) Compacted fill Soil (Mechanical) " 1 0.4 Compacted fill Soil (Mechanical) " 1 0.5 Breccia—Taff (Mechanical) " 1 0.5 Breccia—Taff (Mechanical) " 1 0.4 Sod facing Bainforced Concrete " 1 0.5 Bright Concrete " 1 0.0 Blain concrete " 1 0.0 Blain concrete " 1 0.0	Work			Local Currency Portion	Foreign Currency Portion
Soil (Mechanical) " (Man) Breccia-Taff (Mechanical) " 1 1.0 Bheankment Dring Soil (Truck) (Mechanical) " (Cart) (Man) " (Mechanical) " 1 1.3 Excavation Divert D=300m " 1 1.3 Excavation Divert D=300m " 1 0.9 (Mechanical) " 1 0.7 (Mechanical) " 1 0.7 (Man) Compacted fill Soil (Mechanical) " 1 0.4 O.5 Breccia-Taff (Mechanical) " 1 0.5 Britin concrete " 1 42.0 10.0					
Preccia_Taff (Mechanical)	Soil (Mechanical)		0.5	4.0	0.1
Breccia-Taff (Mechanical) " 1 1.0 0.8 " (Man) " 1 1.5 1.5 Embankment " 1 1.5 1.0 " (Man) " 1 2.0 1.8 " (Cart)(Man) " 1 1.3 1.8 Excavation Divert D=300m " 1 0.9 0.6 (Mechanical) D=100m " 1 0.7 0.5 (Man) " 1 0.7 0.5 Compacted fill Soil (Mechanical) m³ 1 0.4 0.3 Breccia-Taff (Mechanical) " 1 0.5 0.4 Soid facing " 1 0.5 0.4 Brainforced Concrete " 1 0.5 0.3 Plain concrete " 1 45.0 10.0 10.0 10.0 10.0 10.0	" (Man)		9.0	9.0	ı
Embankment	Breccia-Taff (Mechanical)	T	1.0	8.0	0.2
Embankment Dring Soil (Truck)(Mechanical) m³ 1 1.5 1.0 " (Cart)(Man) " 1 2.0 1.8 " (Cart)(Man) " 1 0.9 0.6 (Mechanical) D=100m " 1 0.7 0.5 (Mechanical) D=30m " 1 0.7 0.5 (Man) D=30m " 1 0.4 0.3 compacted fill " 1 0.4 0.3 Soil (Mechanical) " 1 0.5 0.4 Breccia-Taff (Mechanical) " 1 0.5 0.4 Sod facing m² 1 0.5 0.4 Sad facing m² 1 0.5 0.3 Plain concrete " 1 42.0 10.0 2 Plain concrete " 1 42.0 10.0 3		-	1.5	1.5	·
uck)(Mechanical) m ³ 1 1.5 1.0 ")(Man) " 1 2.0 1.8 rt)(Man) " 1 0.9 0.6 ert D=300m " 1 0.7 0.5 D=100m " 1 0.7 0.5 al) m ³ 1 0.4 0.3 crete m ³ 1 46.0 10.0 3 10.0 3	Embankment				
" (Cart)(Man) " 1 2.0 1.3 Excavation Divert D=300m " 1 0.9 0.6 (Mechanical) D=100m " 1 0.7 0.5 (Mechanical) D=30m " 1 0.8 0.5 (Man) " 1 0.4 0.3 (Man) " 1 0.5 0.5 Breccia-Taff (Mechanical) " 1 0.5 Bainforced Concrete " 1 46.0 10.0 Plain concrete " 1 42.0 10.0	Dring Soil (Truck) (Mechanical)	. ∟ . ~ a	1.5	1.0.	0.5
Excavation Divert (Man) " 1 1.3 1.3 1.3 Excavation Divert D=300m " 1 0.9 0.6 (Mechanical)) =====================================	ri •	2.0	8.1	0.2
Excavation Divert D=300m " 1 0.9 0.6 (Mechanical)	٠		1.3	H G	1
(Mechanical) D=100m " 1 0.7 0.5 (Man) D=30m " 1 0.8 0.8 Compacted fill Soil (Mechanical) m³ 1 0.4 0.3 Breccia-Taff (Mechanical) " 1 0.5 0.5 Breccia-Taff (Mechanical) " 1 0.5 0.4 Sod facing m² 1 0.5 0.3 Rainforced Concrete m³ 1 46.0 10.0 Plain concrete " 1 42.0 10.0	Excavation Divert (Mechanical)	.=- !	6.0	9.0	0.3
" (Man) D=30m " 1 0.8 0.8 Compacted fill 3 1 0.4 0.3 Soil (Mechanical) " 1 0.5 0.5 Breccia-Taff (Mechanical) " 1 0.5 0.4 Sod facing m² 1 0.5 0.4 Rainforced Concrete m³ 1 46.0 10.0 Plain concrete " 1 42.0 10.0	" " (Mechanical)	` ∄ `	0.7	0.5	0.2
Compacted fill Soil (Mechanical) " 0.4 0.3 Breccia-Taff (Mechanical) " 1 0.5 0.4 Sod facing " 1 0.3 0.3 Rainforced Concrete " 1 46.0 10.0 Plain concrete " 1 42.0 10.0	" (Man)	<u>-</u>	8.0	8.0	1
Soil (Mechanical) m³ 1 0.4 0.3 " 1 0.5 0.5 Breccia_Taff (Mechanical) " 1 0.5 0.4 Sod facing m² 1 0.3 0.3 Rainforced Concrete m³ 1 46.0 10.0 Plain concrete " 1 42.0 10.0					
" (Man) " 1 0.5 Breccia-Taff (Mechanical) " 1 0.5 Sod facing m Rainforced Concrete m Plain concrete " 1 46.0	Soil (Mechanical)	. T	4.0	0.3	۳.0
Breccia-Taff (Mechanical) " 1 0.5 0.4 Sod facing m 1 0.3 0.3 Bainforced Concrete n 1 46.0 10.0 Plain concrete " 1 42.0 10.0	" (Man)		0.5	0.5	E
Sod facing m ² 1 0.3 0.3 Rainforced Concrete m ³ 1 46.0 10.0 Plain concrete " 1 42.0 10.0			0.5	4.0	0.1
Rainforced Concrete m ³ 1 46.0 10.0 Plain concrete " 1 42.0 10.0	Sod facing	n 2 1	0.3	0.3	F
Plain concrete " 1 42.0 10.0	Rainforced Concrete	e u	46.0	10.0	36.0
	Plain concrete	<u>-</u>	42.0	10.0	32.0

(to be continued)

Item No.	Work	Unit.	Quantity	Quantity Total Unit Price	Local Currency	Foreign Currency
	Form				1010101	LOLLOLD
. 21	Structure (A)	e B	<i>-</i> -1	0.8	6.0	2.0
18	Simple (B)	=	1	7.0	5.0	2.0
19	Rainfocing bar	ton	7	460.0	70.0	390.0
20	Stone Pitching	Cl E	ч	13.0	11.0	2.0
21	Gravel bedding	e E	r-f	0.9	4.0	2.0

Table 5.7 Land Acquisition & Compensation

Land Acquisition

(unit: ha)

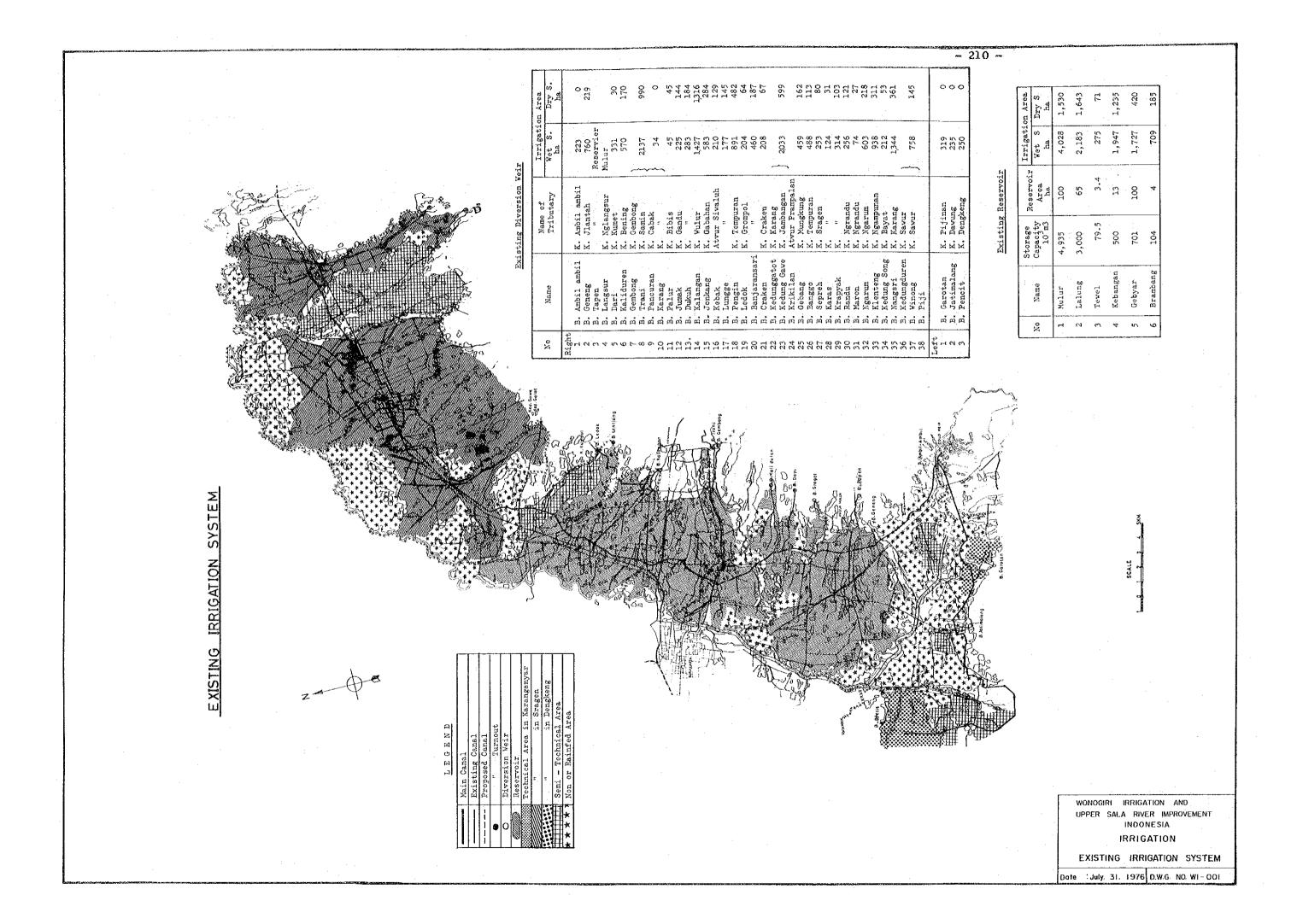
	~~···~~~ ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			
Sawa	a	Тодої	Vand	Total
irrigated	Rainfed	regar	raru	10041
140	82	-	23	245
28	19		_	47
451	274	<u> </u>		725
-	4	20	11	35
619	379	20	34	1,052
	140 28 451	140 82 28 19 451 274 - 4	Tegal irrigated Rainfed 140 82 - 28 19 - 451 274 - 4 20	Tegal Yard irrigated Rainfed 140 82 - 23 28 19 451 274 - 4 20 11

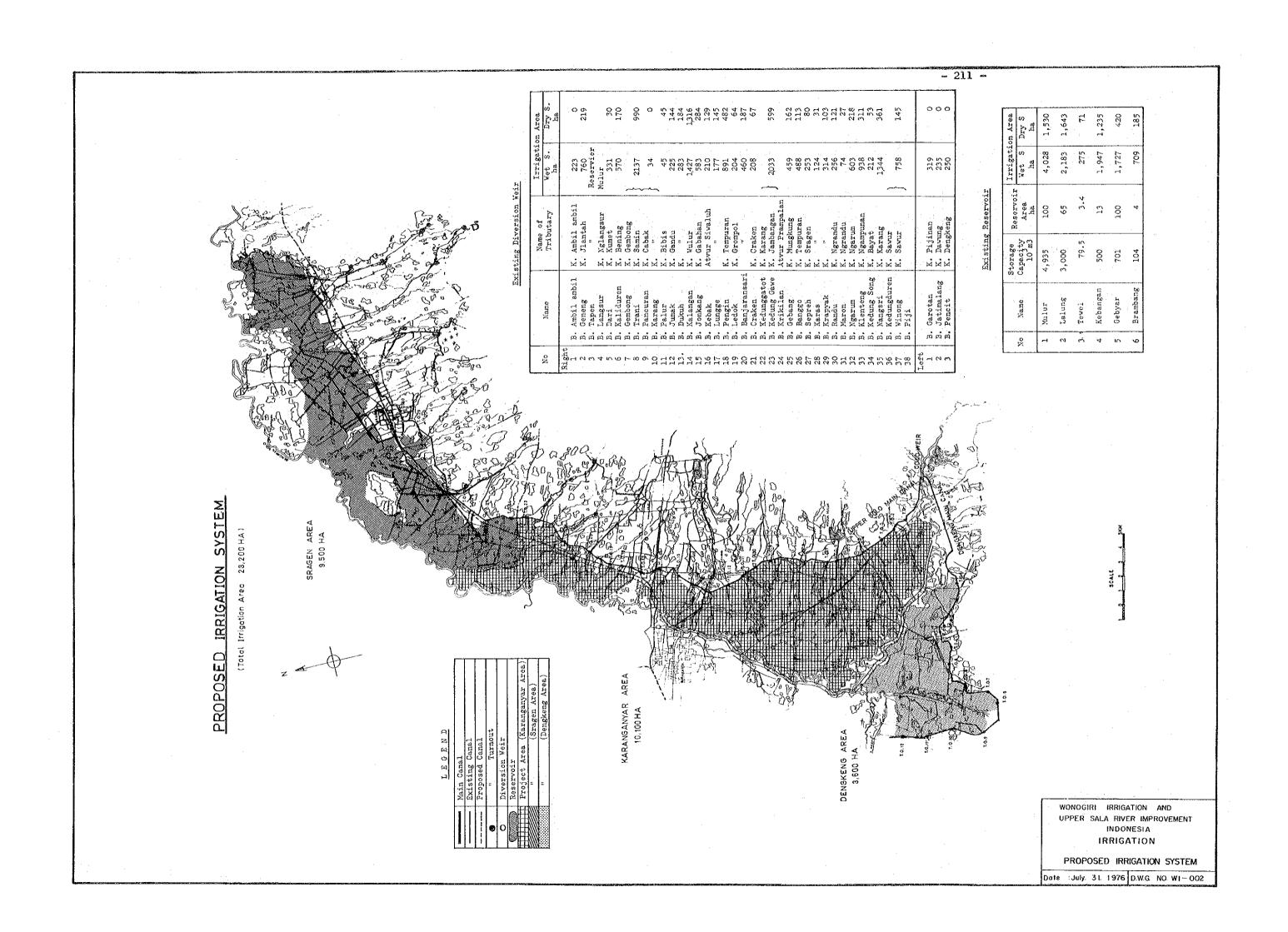
Compensation

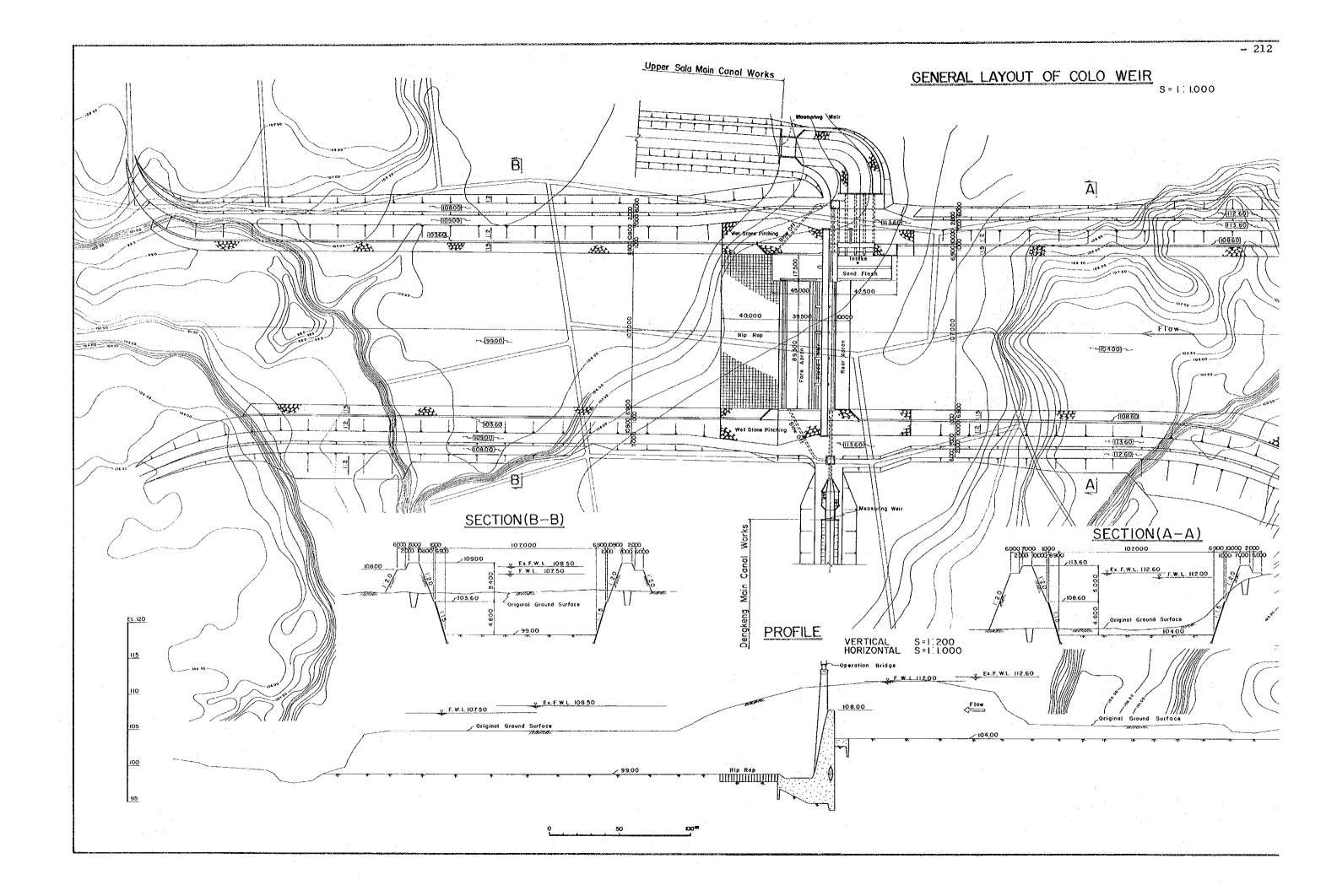
(unit: a house)

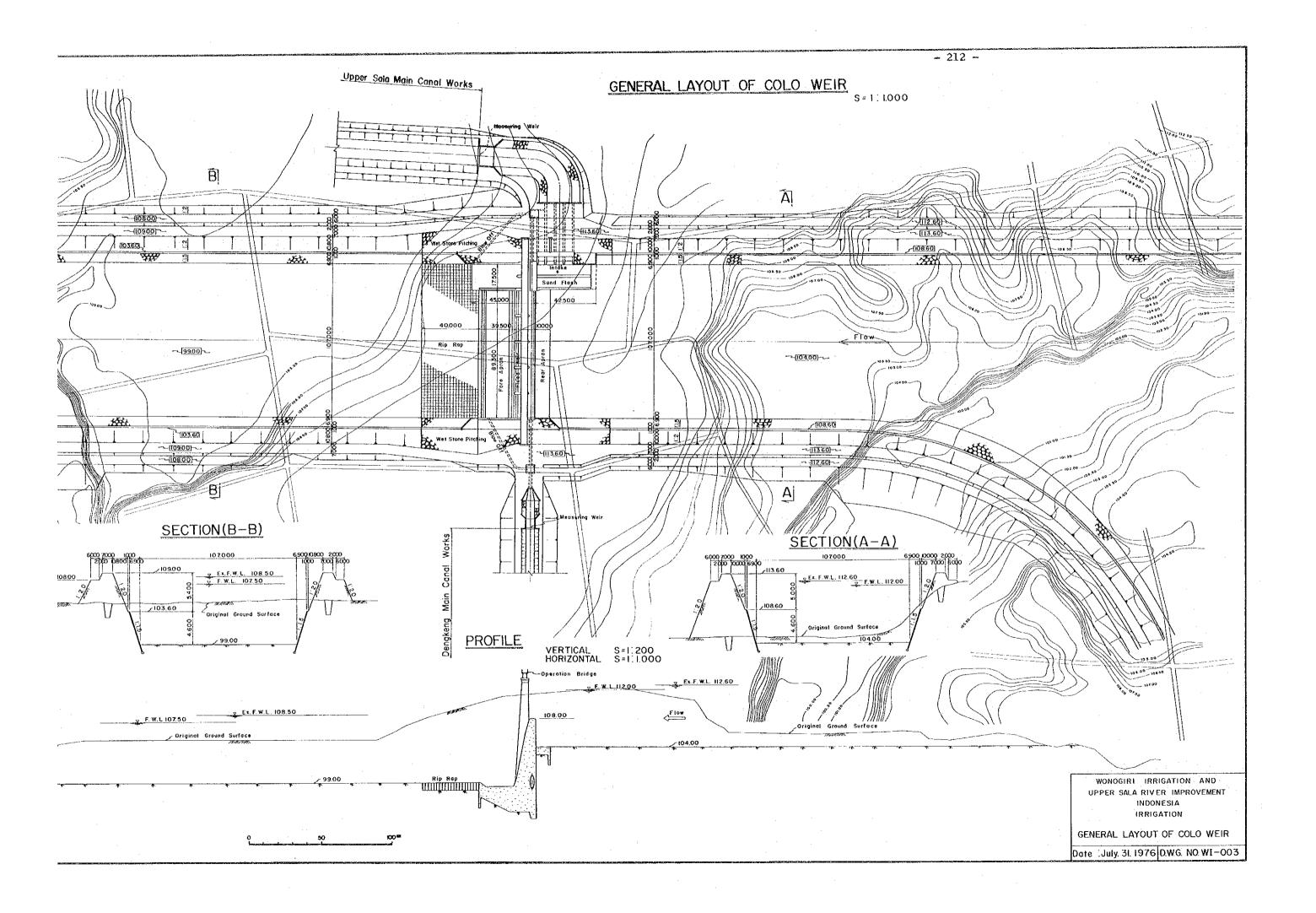
Item Work	Banboo house	Wooden house	Brick house	Total
Main Canal	150	40	20	210
Colo Weir	50	10	-	60
Total	200	50	20	270

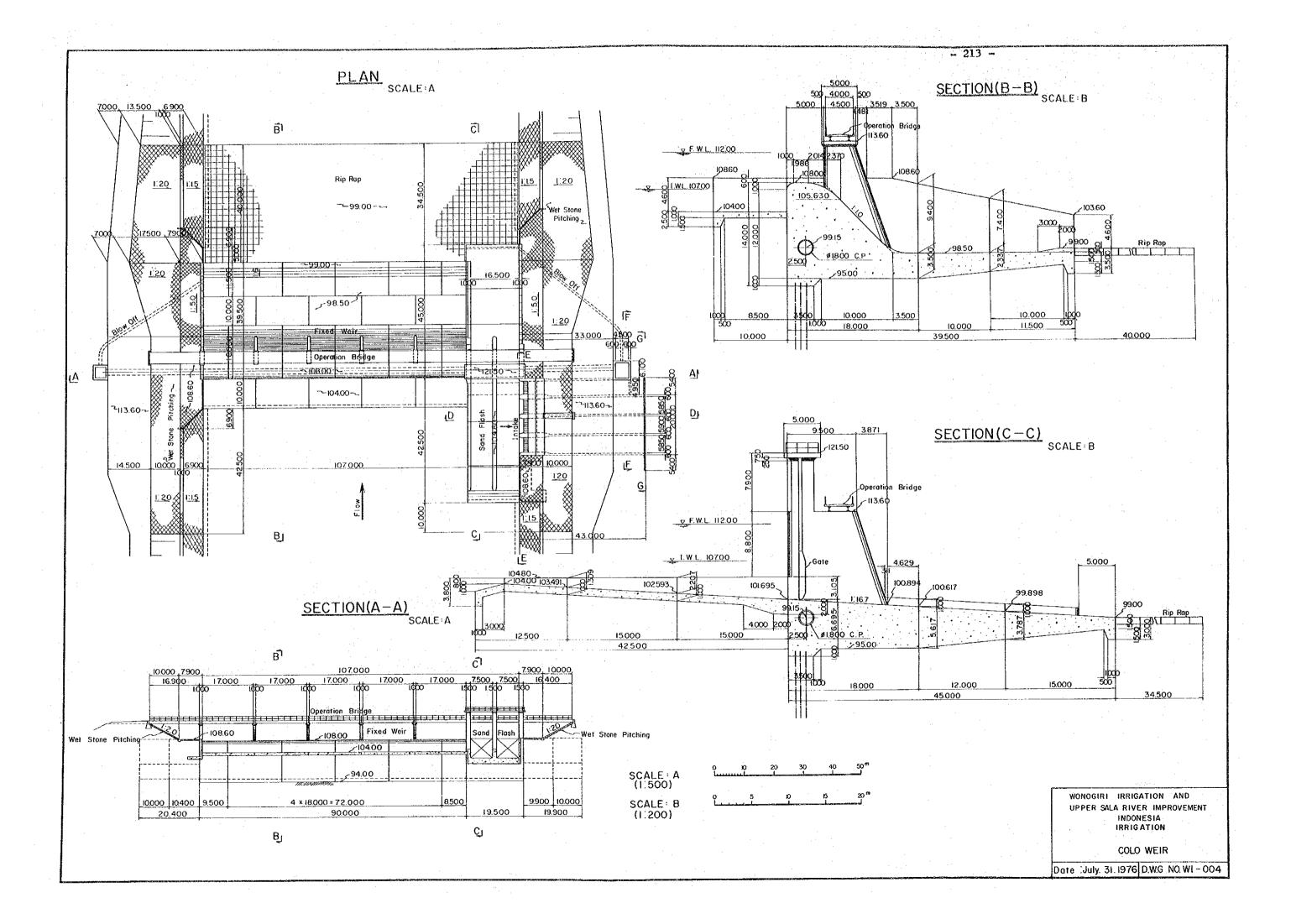
6. DRAWINGS

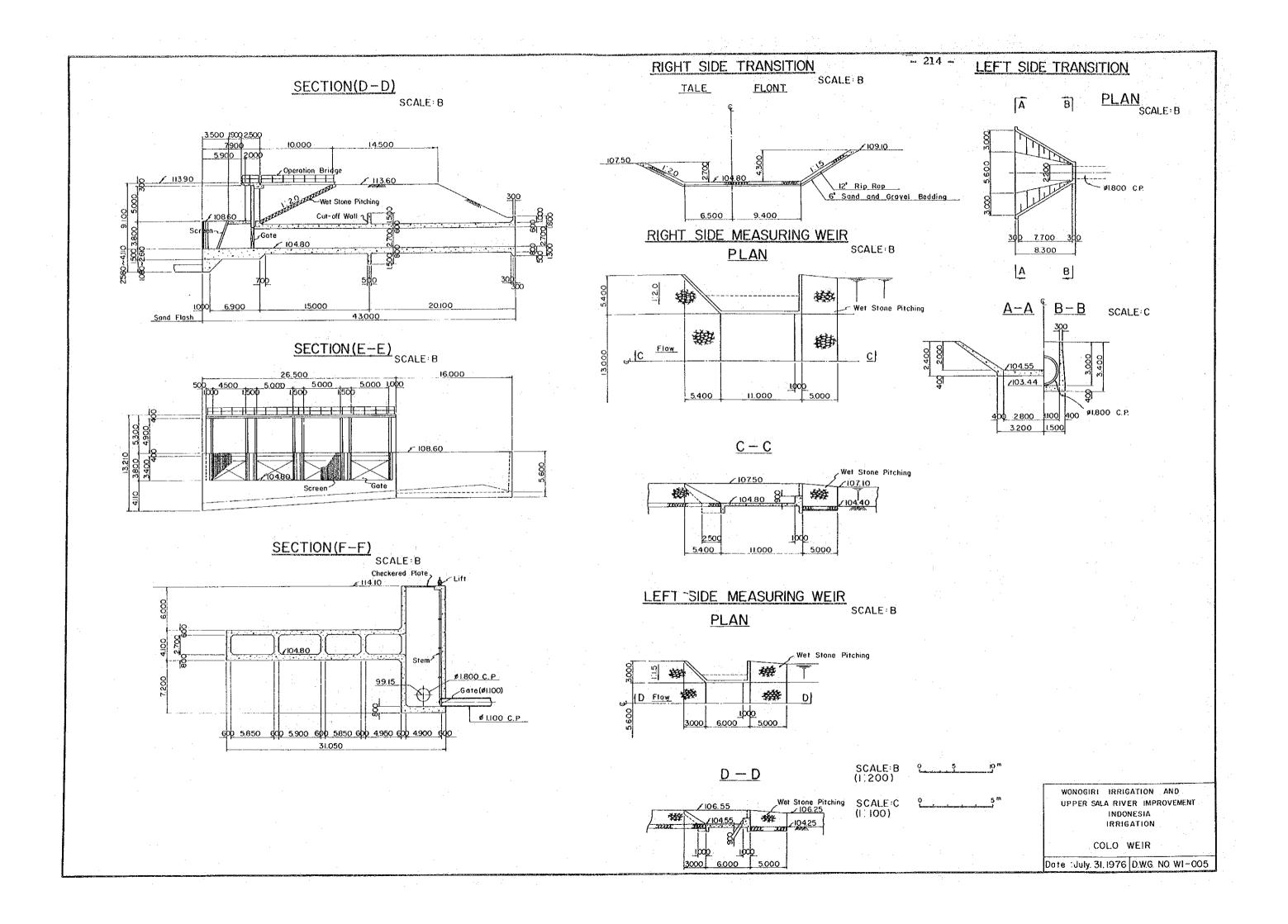




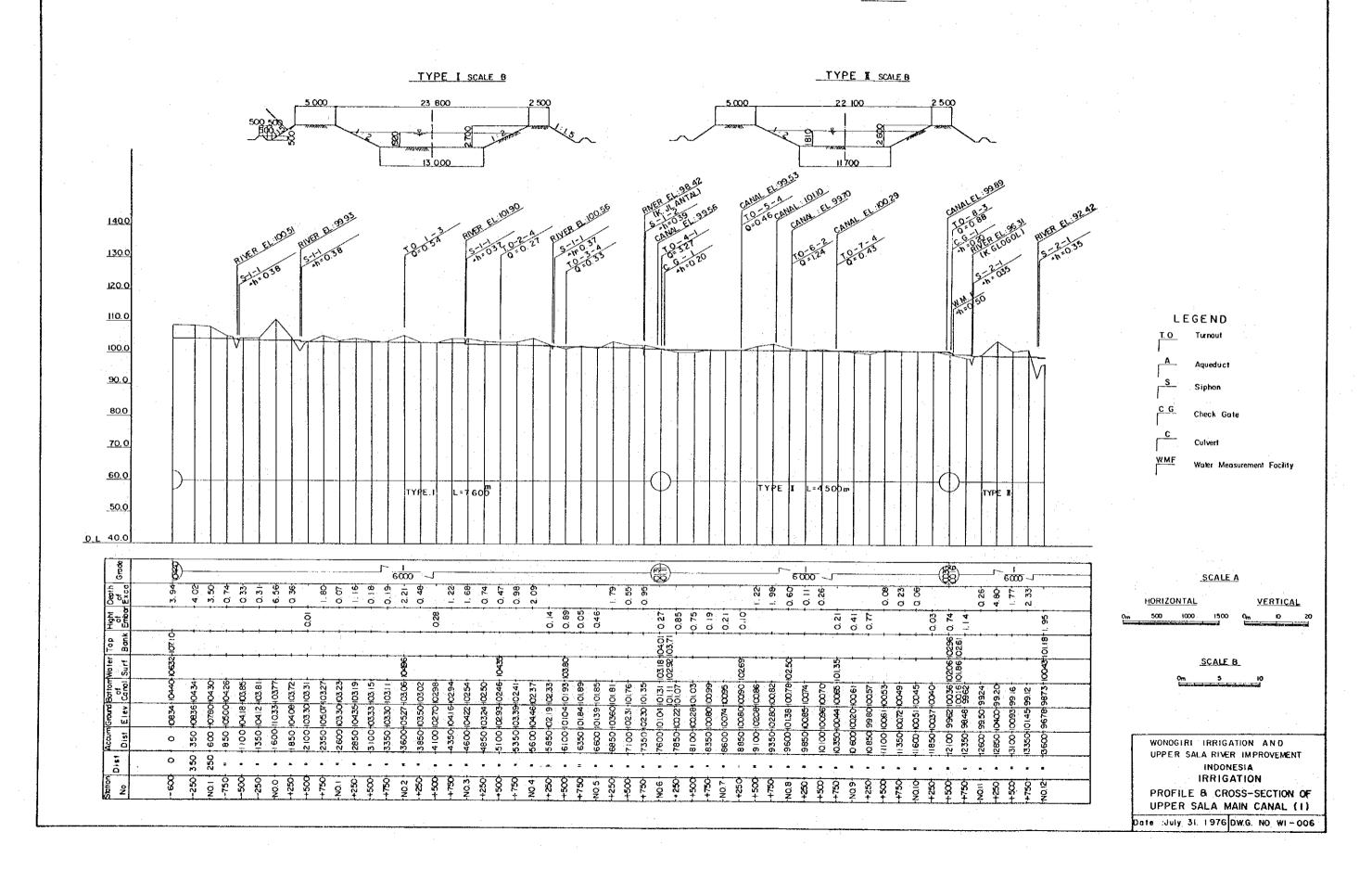








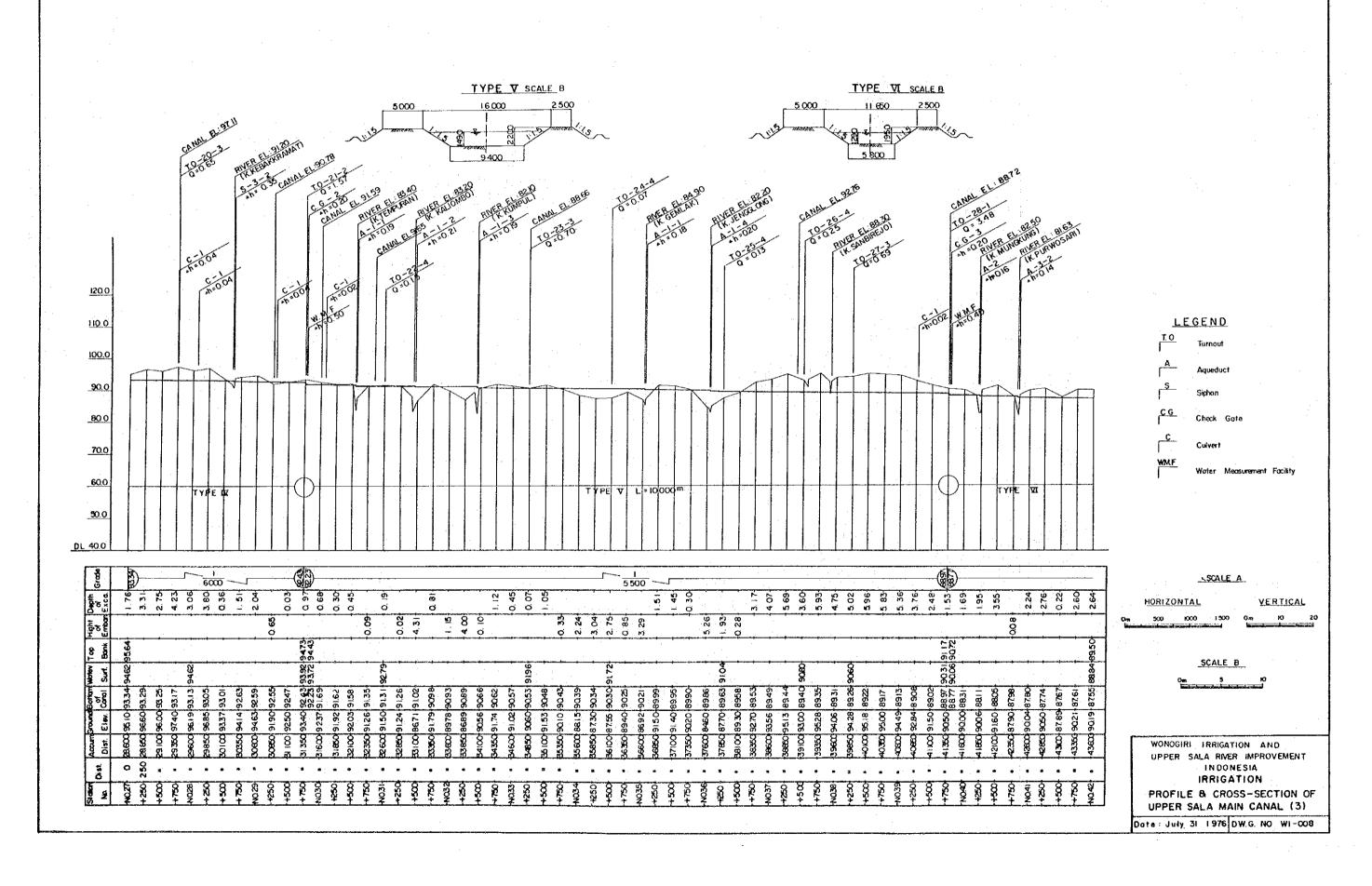
PROFILE OF UPPER SALA MAIN CANAL (I) SCALE A



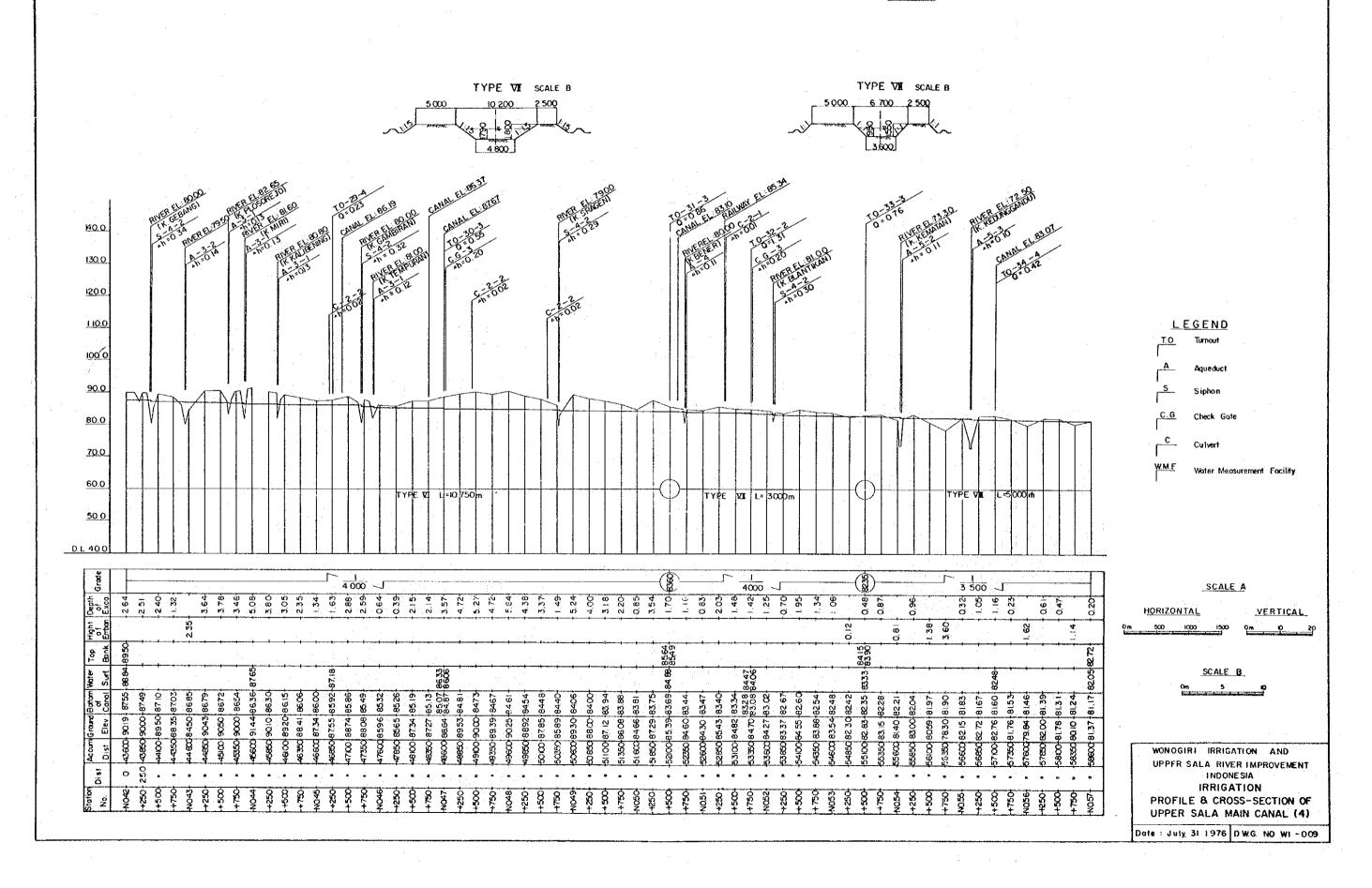
Dote: July 31 1976 D.W.G NO WI. 007

PROFILE OF UPPER SALA MAIN CANAL (2) SCALE A TYPE M SCALEB TYPE IV SCALEB 5 000 5 000 11000 CAMALEL 3746 19-030 CANAL EL 95 BA CWW E 3850 CANAL EL STAT 18:02 RIVER REALISE 120.0 110.0 LEGEND Turnout 100.0 Aqueduct 90.0 80.0 Check Gote 70.0 Water Measurement Facility 60.0 50.0 <u>~</u> <u>e000</u>0 ---1 8 4 8 032 - 79 S CALE A HORIZONTAL VERTICAL 98.69 98.69 SCALE B WONOGIRI IRRIGATION AND UPPER SALA RIVER IMPROVEMENT ဝရ္က INDONESIA IRRIGATION PROFILE & CROSS-SECTION OF UPPER SALA MAIN CANAL (2)

PROFILE OF UPPER SALA MAIN CANAL (3) SCALE A



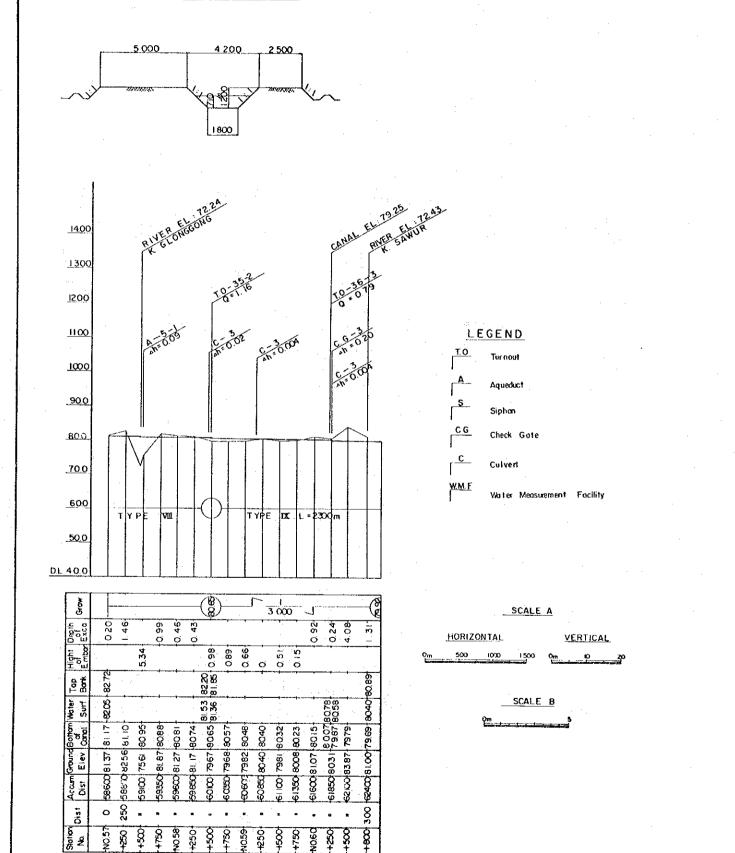
PROFILE OF UPPER SALA MAIN CANAL (4) SCALE A



PROFILE OF UPPER SALA MAIN CANAL (5)

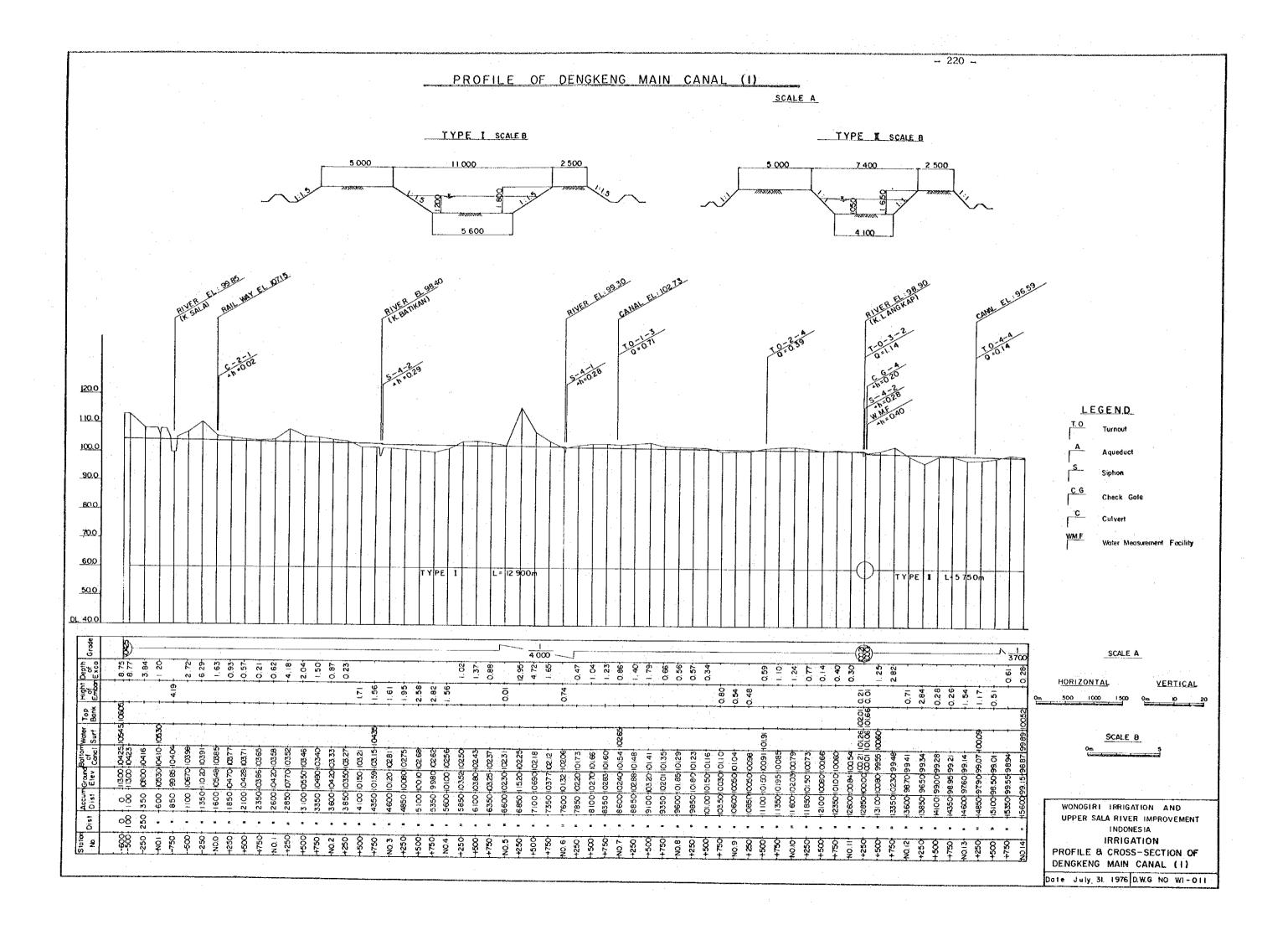
SCALE A

TYPE IX SCALE B



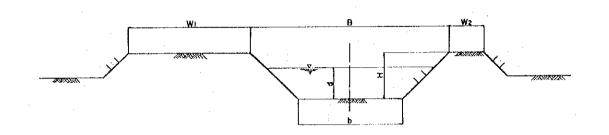
WONOGIRI IRRIGATION AND
UPPER SALA RIVER IMPROVEMENT
INDONESIA
IRRIGATION
PROFILE & CROSS-SECTION OF
UPPER SALA MAIN CANAL (5)

Date: July, 31, 1976 D.W. G. NO. WI = 010



STANDARD CROSS SECTION

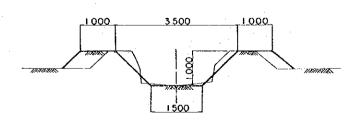
SECONDARY CANAL



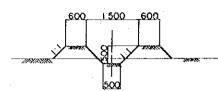
___Data__Table

							- (0	imension m)
TYPE	o	b	В	đ	н	Wı	Ms	CANAL LENGTH
Ι.	1.62 1.40	3.0	5.70	0.90	1.35	3.50	1.00	4 600
2	0.92 0.70	2.1	4.40	0.75	1.15	*	*	9 900
3	0.68 0.50	1.8	4.00	0.70	1.10		ti	11 100
4	0.42 0.30	1.3	3.20	0.60	0.95	21	11	2 650
5	0.29 0.20	. 0.9	2.70	0.55	0.90	19	h ·	5 350
6	0.19 0.10	0.7	2.40	0.50	0.85	н	ж	e e00
7	0.07	0.5	1.90	0.40	0.70	*	*	1 000

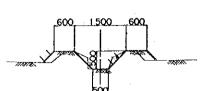
REHAVILITATION
Existing Canal



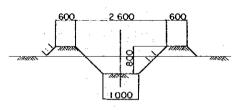
FARM DITCH



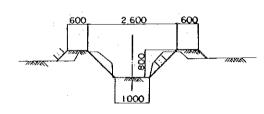
REHAVILITATION Existing Ditch



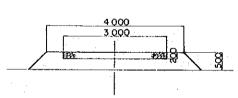
FARM DRAIN



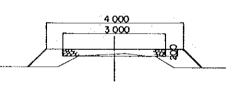
REHAVIL ITATION Existing Drain



FARM ROAD



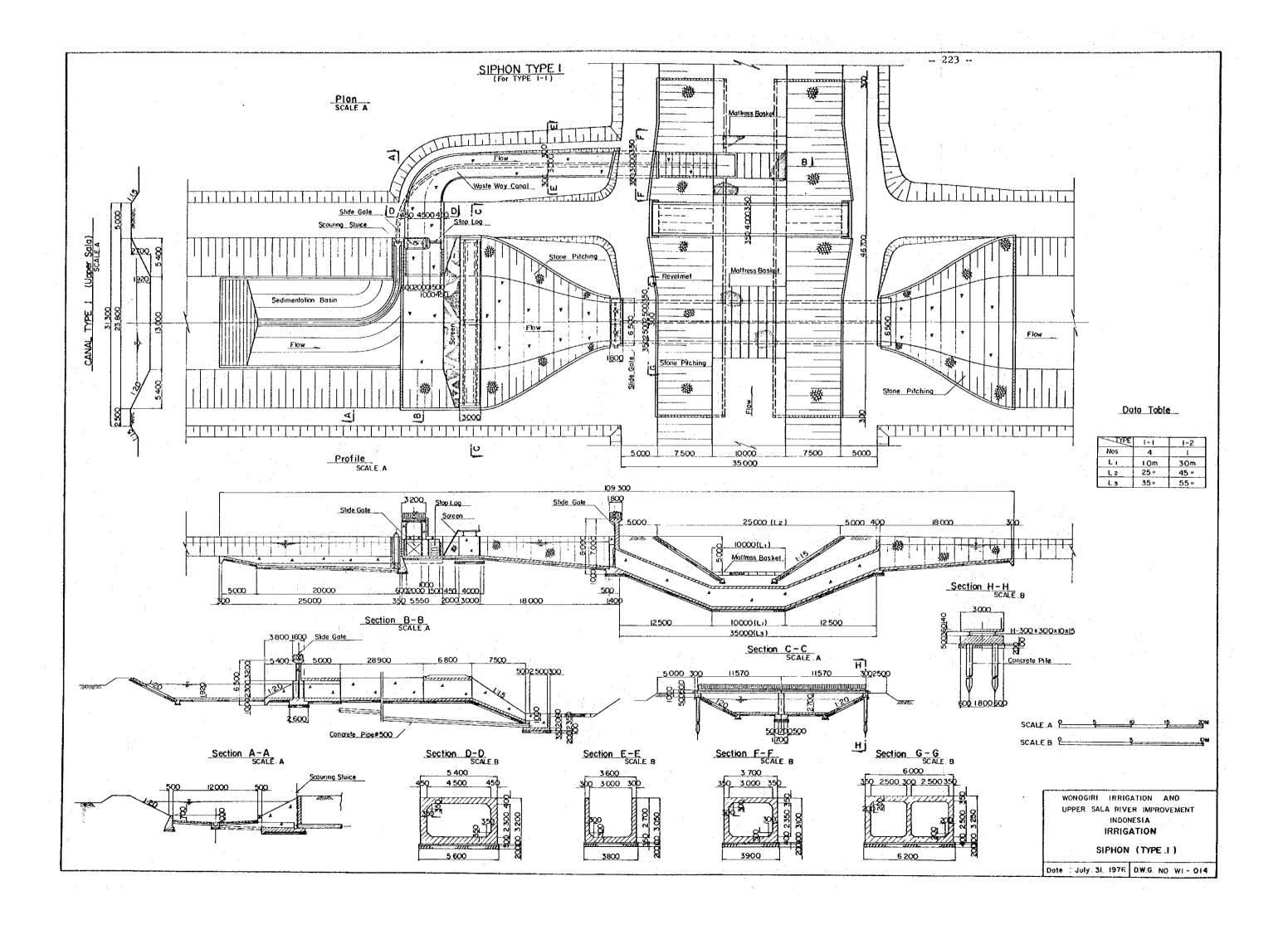
REHAVILITATION Existing Road

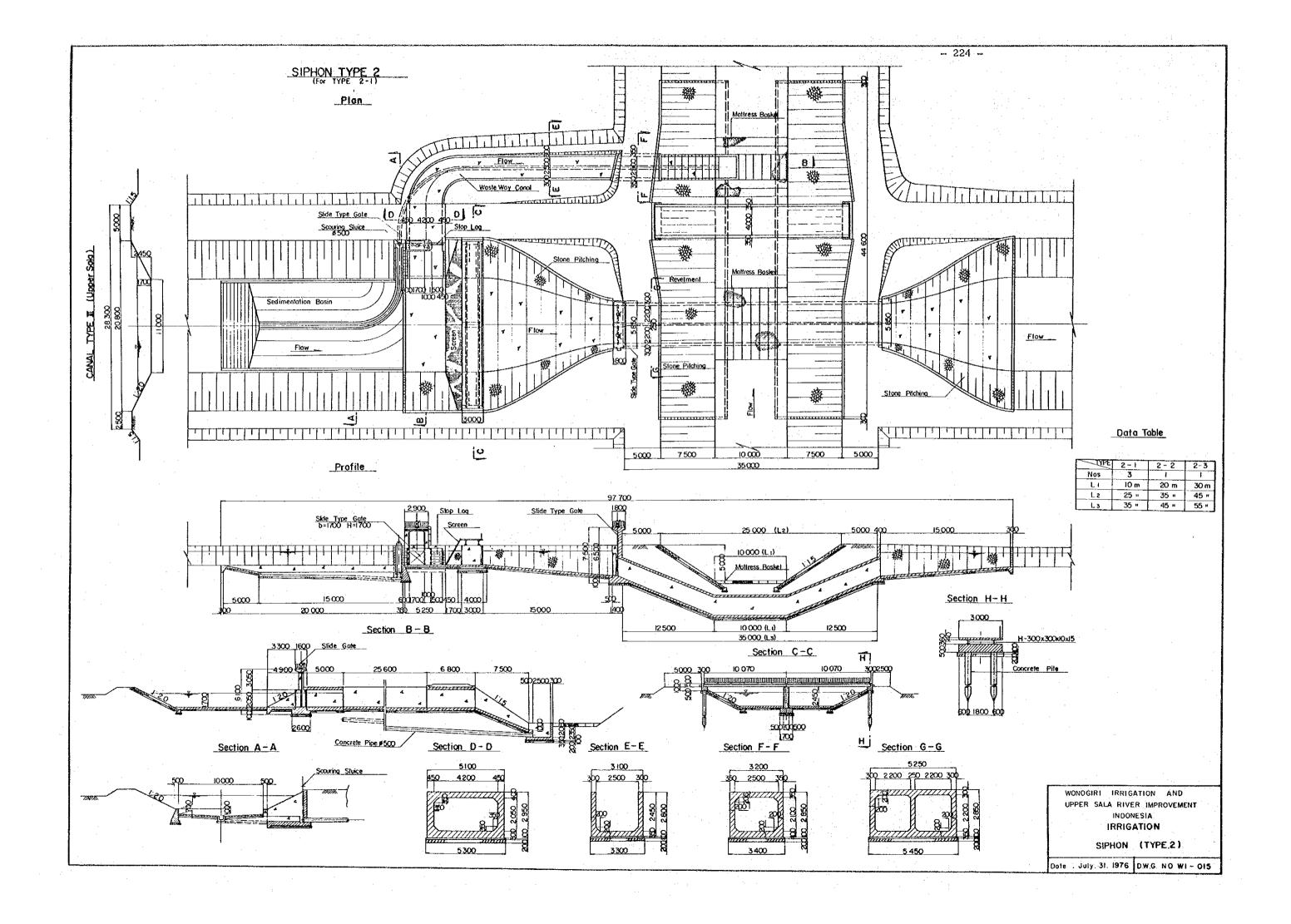


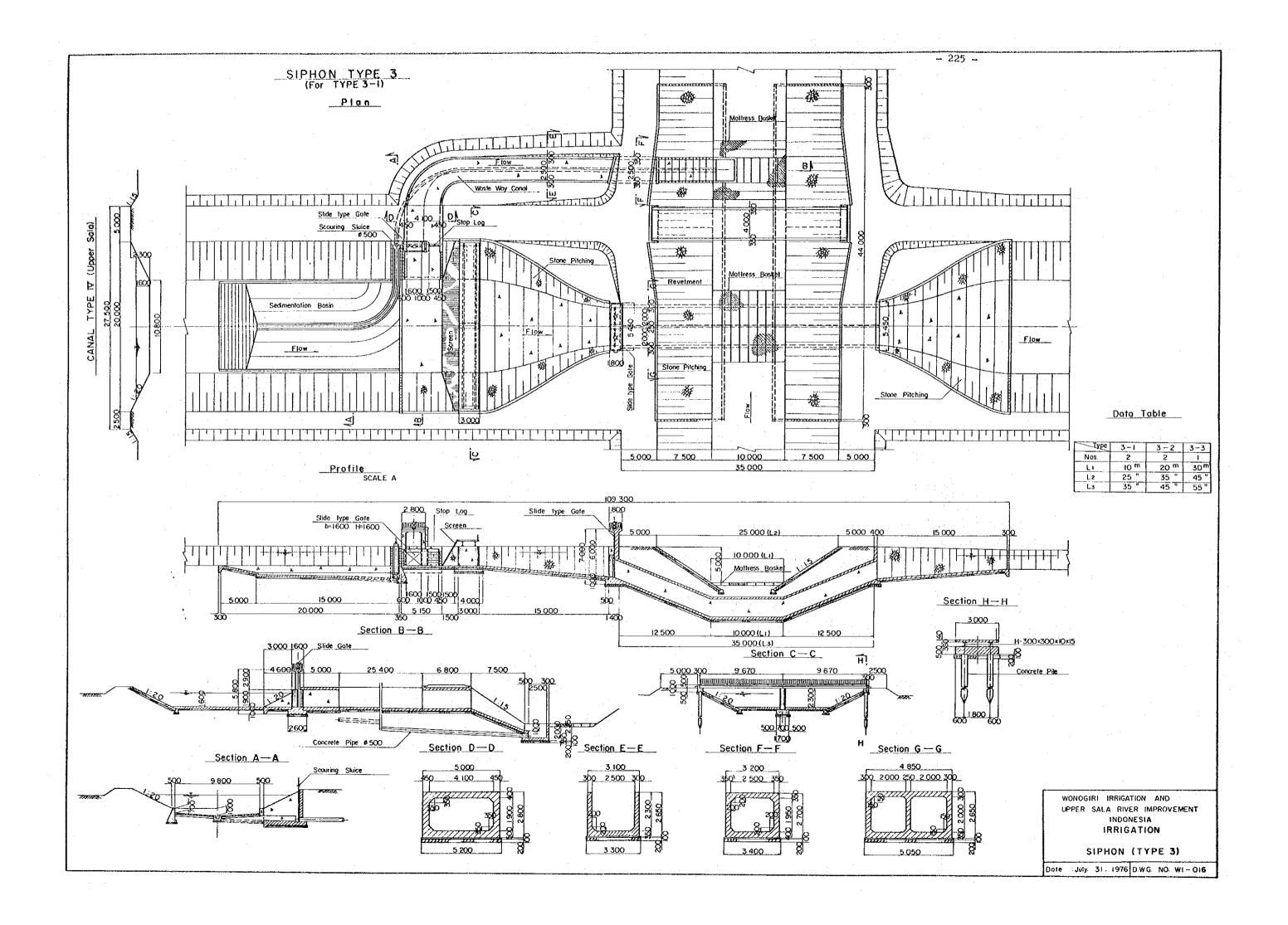
SCALE 2 3 4 5

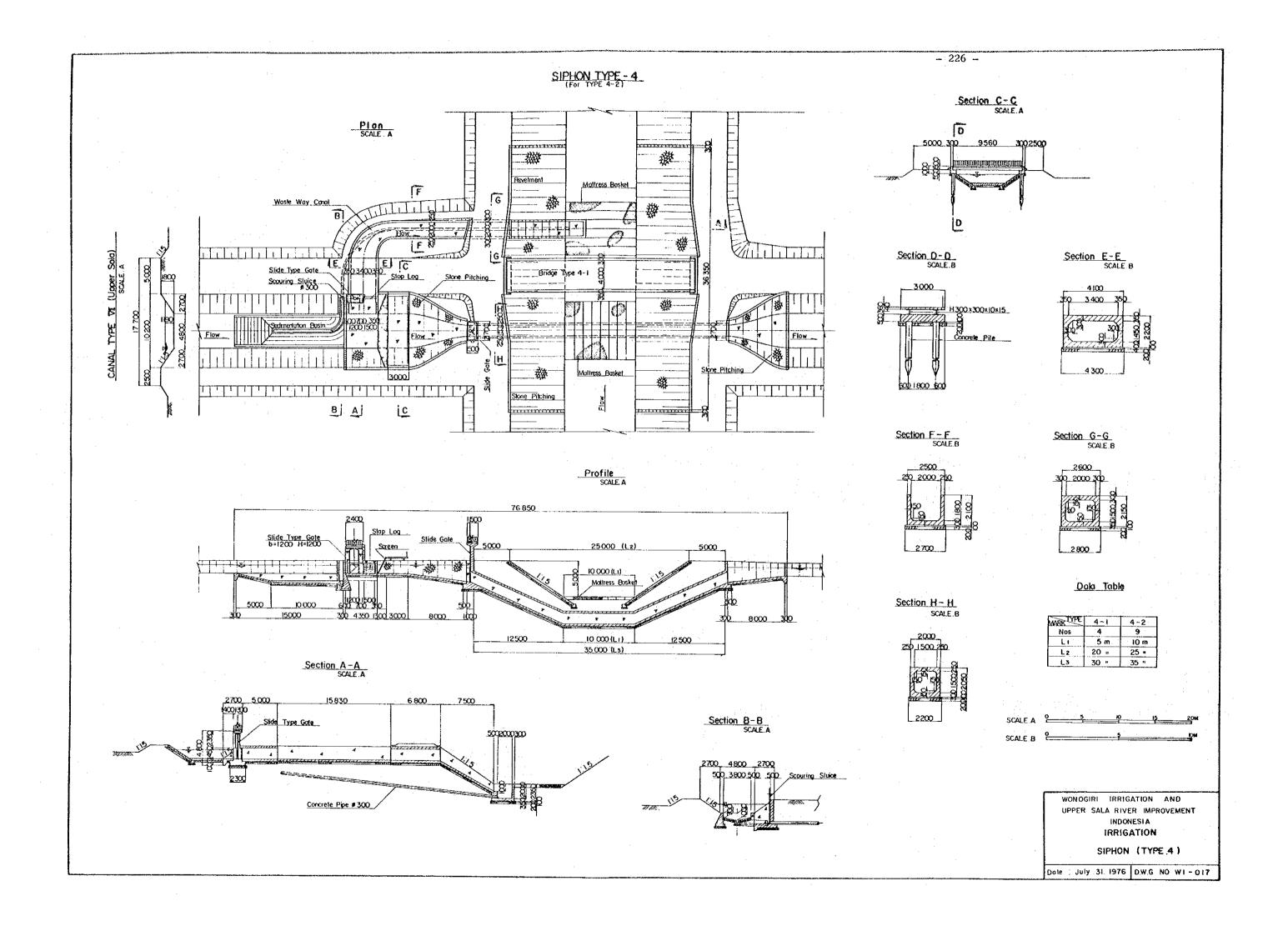
WONOGIRI IRRIGATION AND UPPER SALA RIVER IMPROVEMENT
INDONESIA
IRRIGATION
STANDARD CROSS-SECTION OF
SECONDARY CANAL & OTHERS

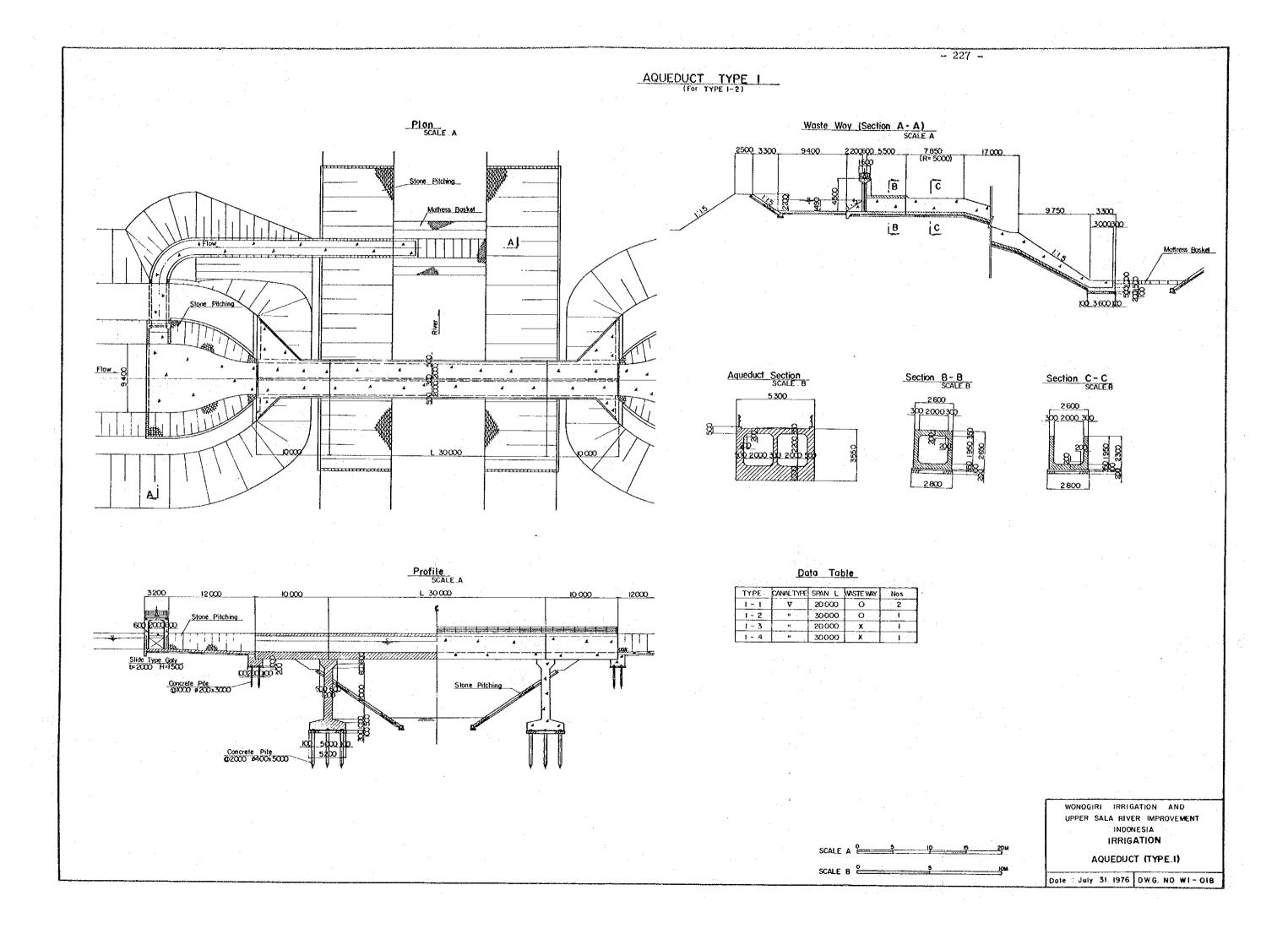
Date : July. 31. 1976 D.W.G. NO. WI - 013



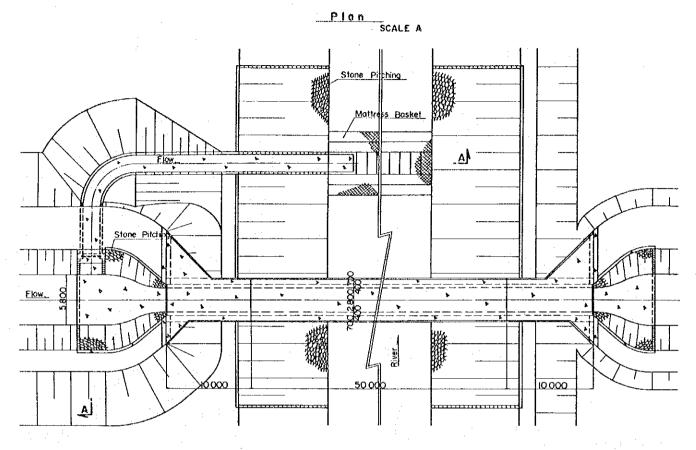


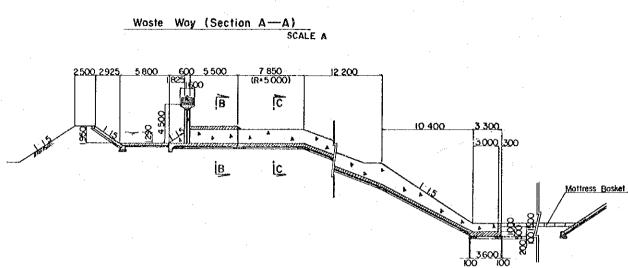


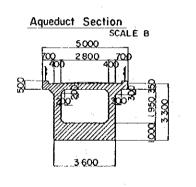


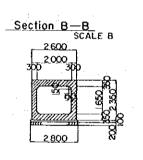


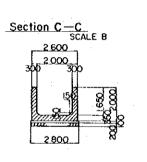
AQUEDUCT TYPE 2

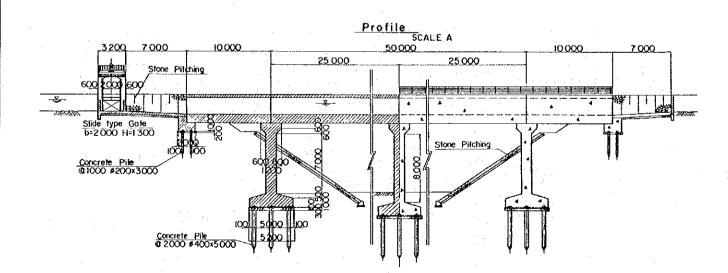












SCALE A 2 5 19 30 SCALE B 2 5 19 M

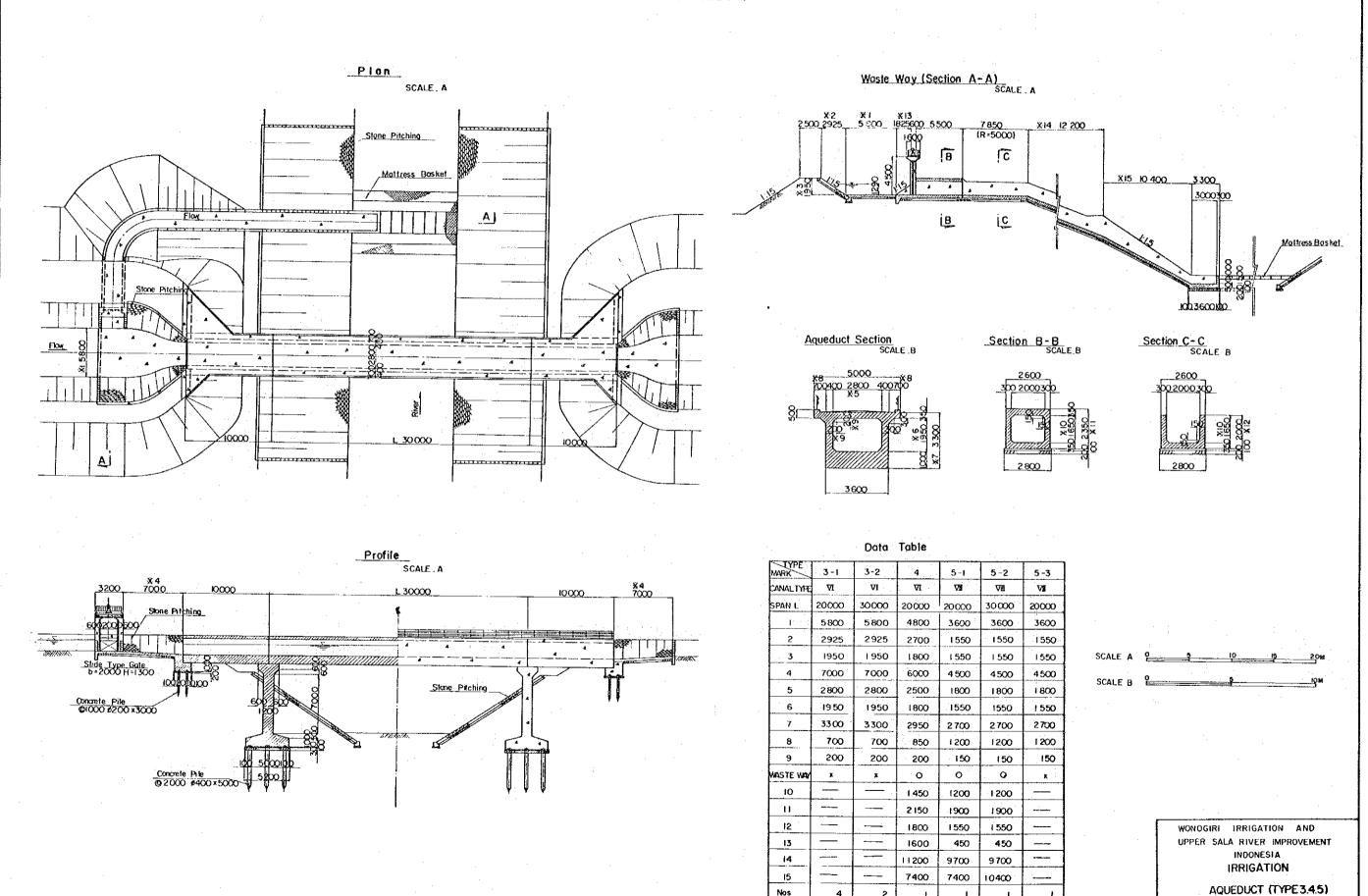
WONOGIRI IRRIGATION AND
UPPER SALA RIVER IMPROVEMENT
INDONESIA
IRRIGATION

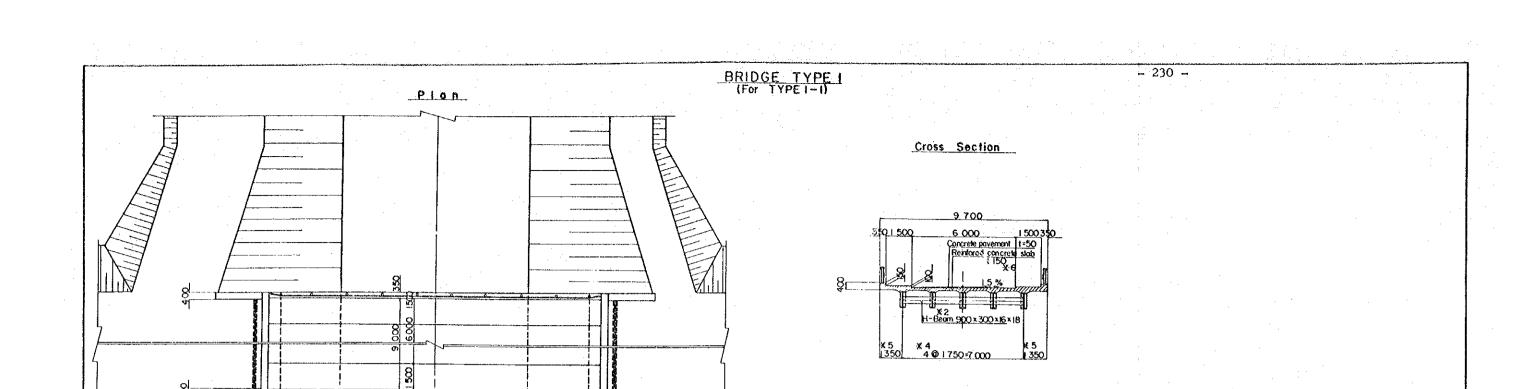
AQUEDUCT (TYPE 2)

Date July, 31, 1976 D.W.G. NO. WI- 019

Date : July 31, 1976 D.W.G. NO WI - 020

AQUEDUCT TYPE 3 (For TYPE 3-2)





X 8 00 00

5,000

¥8 2 700

Profil<u>e</u>

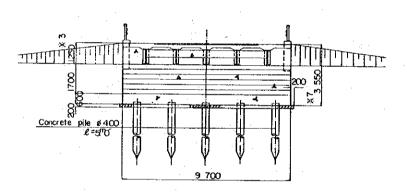
SCALE

¥ 2 H-Bean 900 x300 x 6 x 18 L≈19[™]16

400 2 500

Wing

Abutment



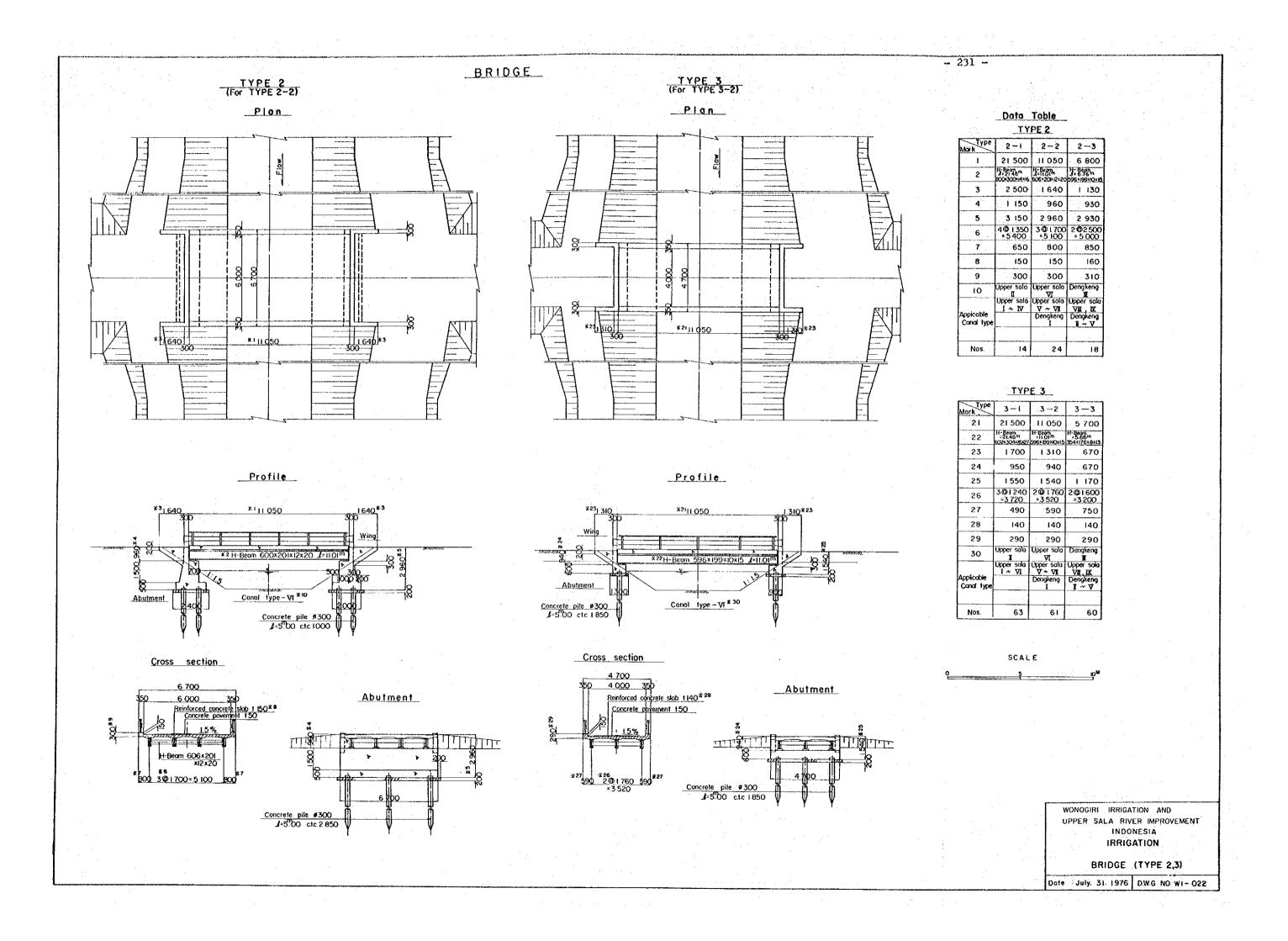
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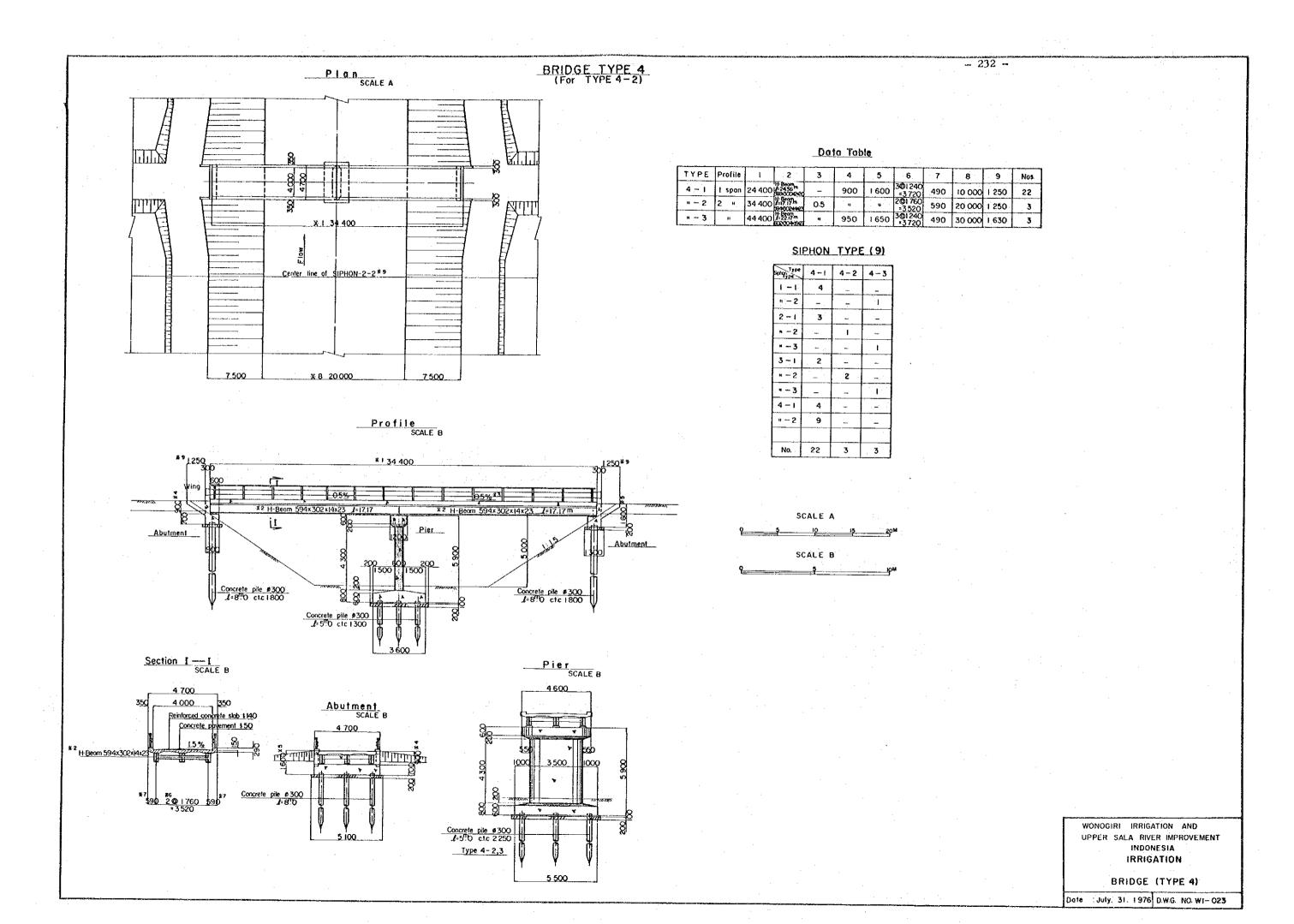
										
TYPE	- I	2	3	4	. 5	6	7	8	Nos	1
1-1_	19 2 00	H-Beom#19)6 900×300×6+8	1 250	4 © 1750 ¯ =7 000	1.350	150	3 5 5 0	2 7 00		l
1 - 2	10 2 00	#016 792×300×4/22	1170	3 0 2300	1 400	170	3 4 7 0	1950	ŀ	

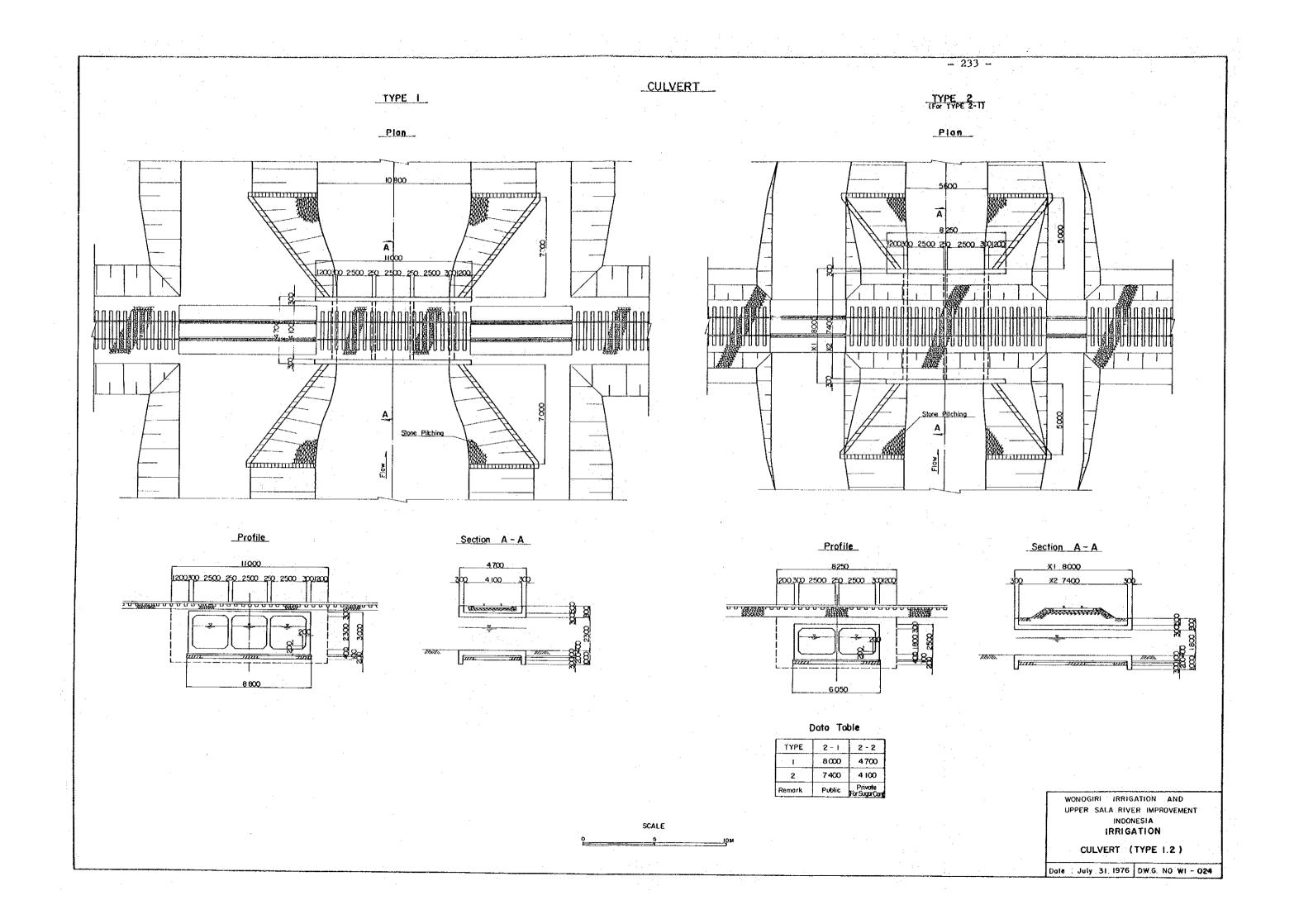
WONOGIRI IRRIGATION AND UPPER SALA RIVER IMPROVEMENT INDONESIA IRRIGATION

BRIDGE (TYPE I)

Date July 31, 1976 D.W.G NO WI - 021

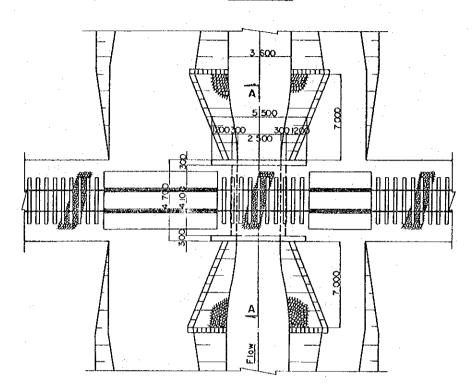


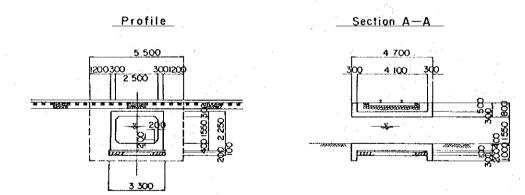




TYPE 3

Plon



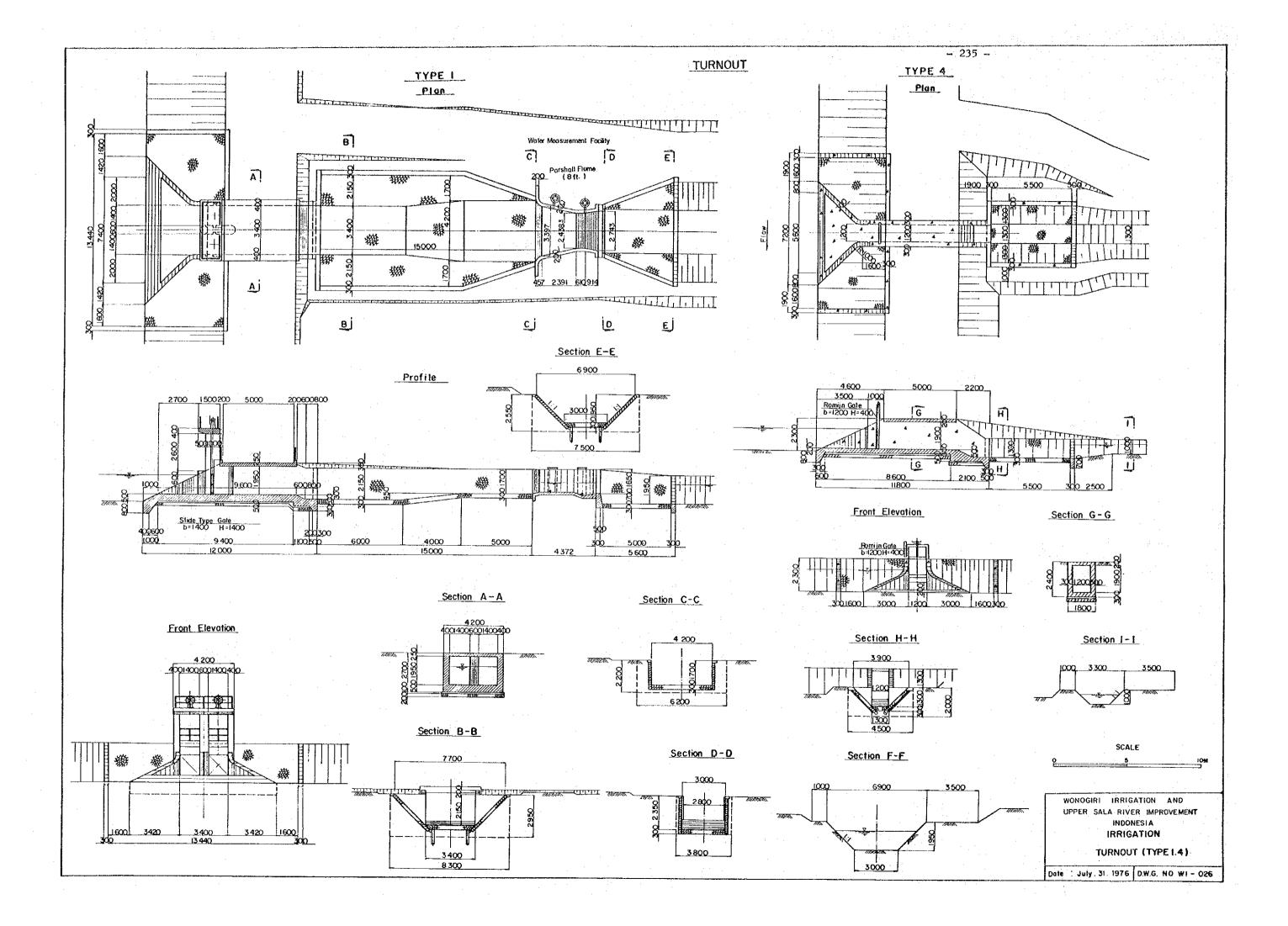


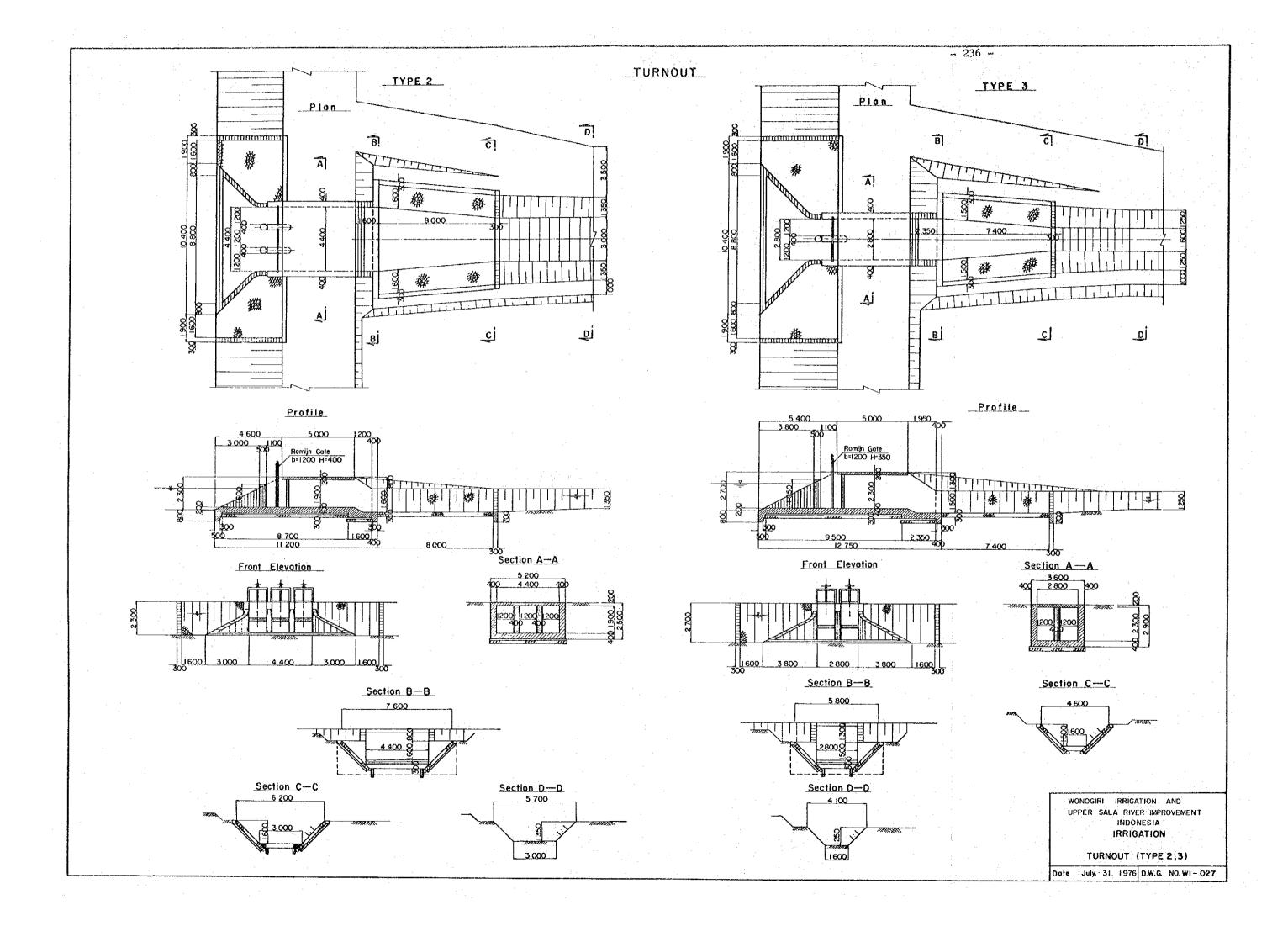
SCALE

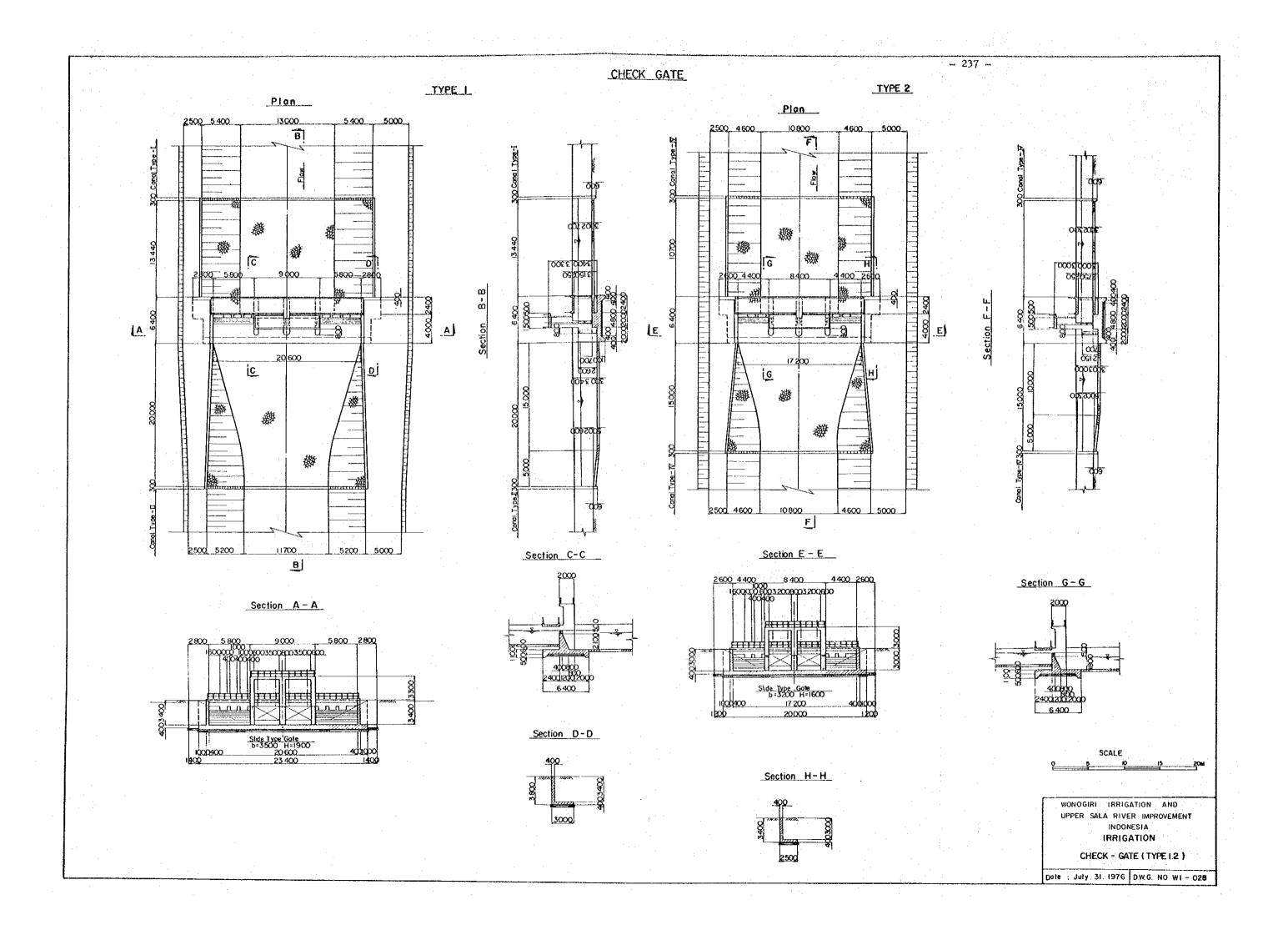
WONDGIRI IRRIGATION AND UPPER SALA RIVER IMPROVEMENT INDONESIA IRRIGATION

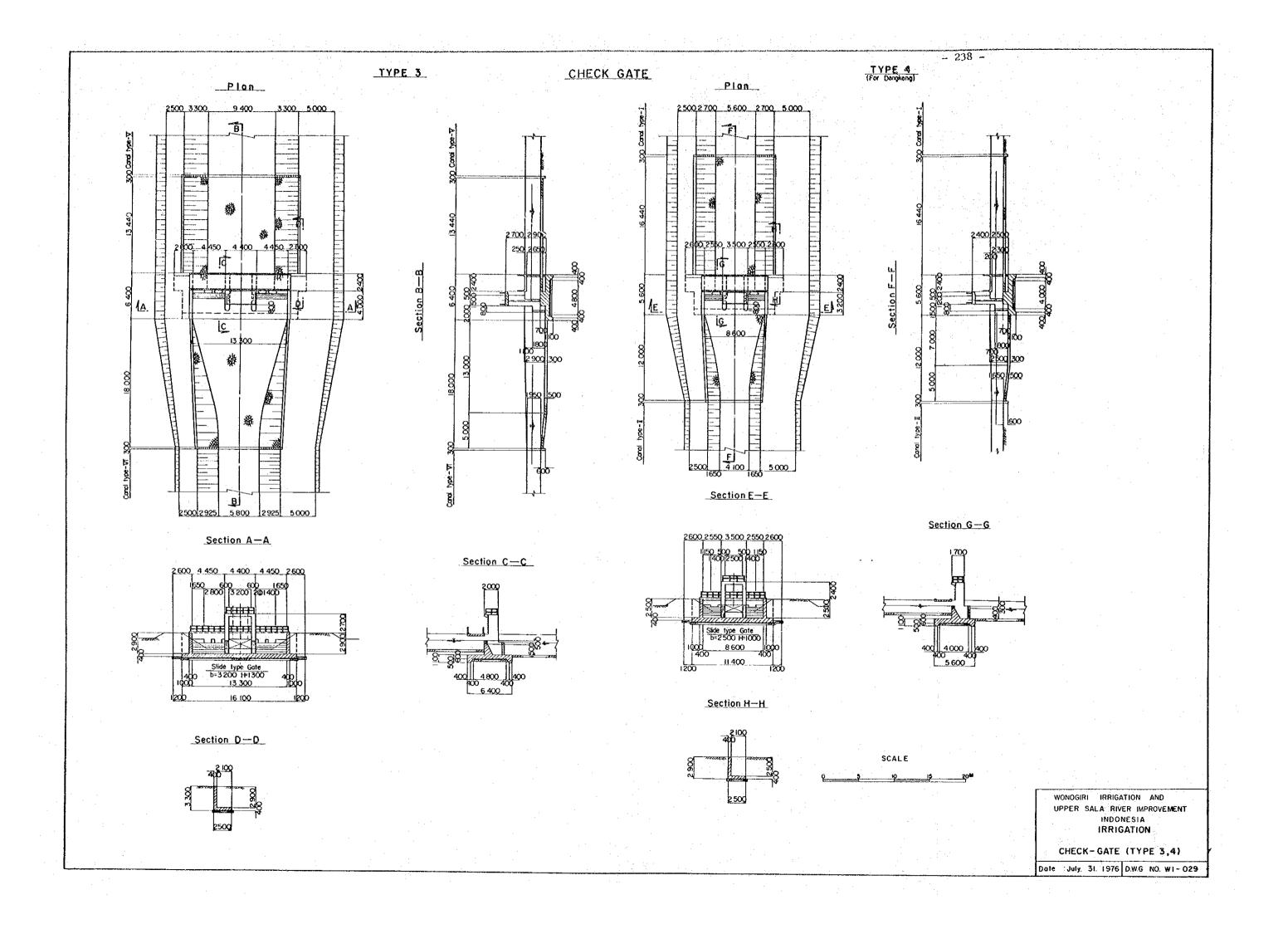
CULVER (TYPE 3)

Date July 31: 1976 D.W.G. NO. WI - 025



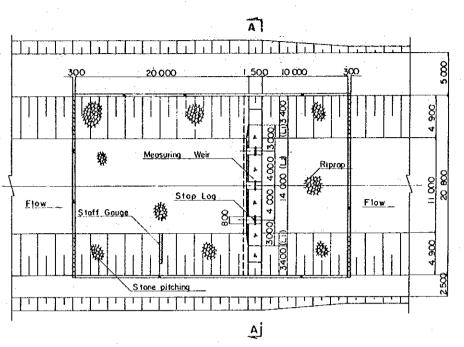






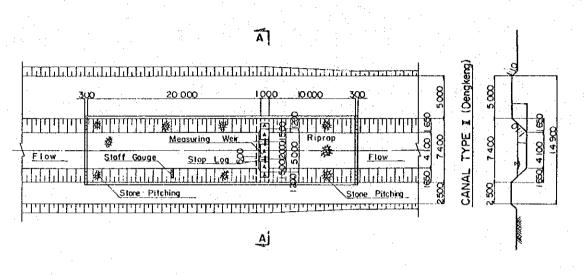
TYPE 4

Plon

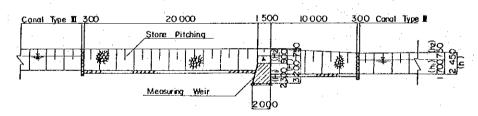


2500 2 20 800 5 000 2

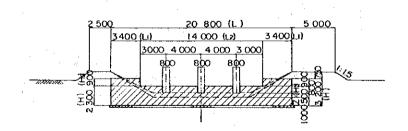
Plan



Profile



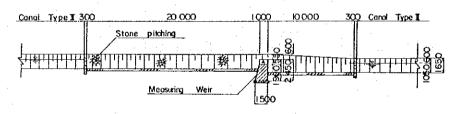
Section A-A



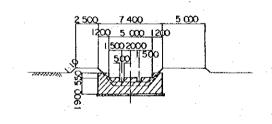
__Data_Table

			, .		<u> </u>									
Type	Canal Type	No	Z	L	L	Lz	н	Hi	H2 .		h	hı	hz	
	T	1	0.50 ^m	20.80	3.40 ^m	14.00	3, 20	2.30	0.90 ^m	0.80 m	2.45 ^m	1.70	0.75	
2	V.	ı	0.50	16 00	2.50	11.00	2.99	2.20	0.79	0.70	2.20	1,49	0.71	
3	VΙ	ì	0.40	11.66	2.08	7.50	2.69	2.00	0.69	0.60	1.95	1.29	0.66	

Profile



Section A-A



SCALE 0 5 10 15 20°

WONOGIRI IRRIGATION AND
UPPER SALA RIVER IMPROVEMENT
INDONESIA
IRRIGATION
WATER MEASUREMENT FACILITY
(TYPE 1,2,3,4)

Date July 31, 1976 D.W.G. NO WI-030

