DESIGN REPORT
ON
THE DETAIL DESIGN SURVEY
FOR
THE AGRICULTURAL COOPERATIVE PROMOTION PROJECT
IN
THAILAND

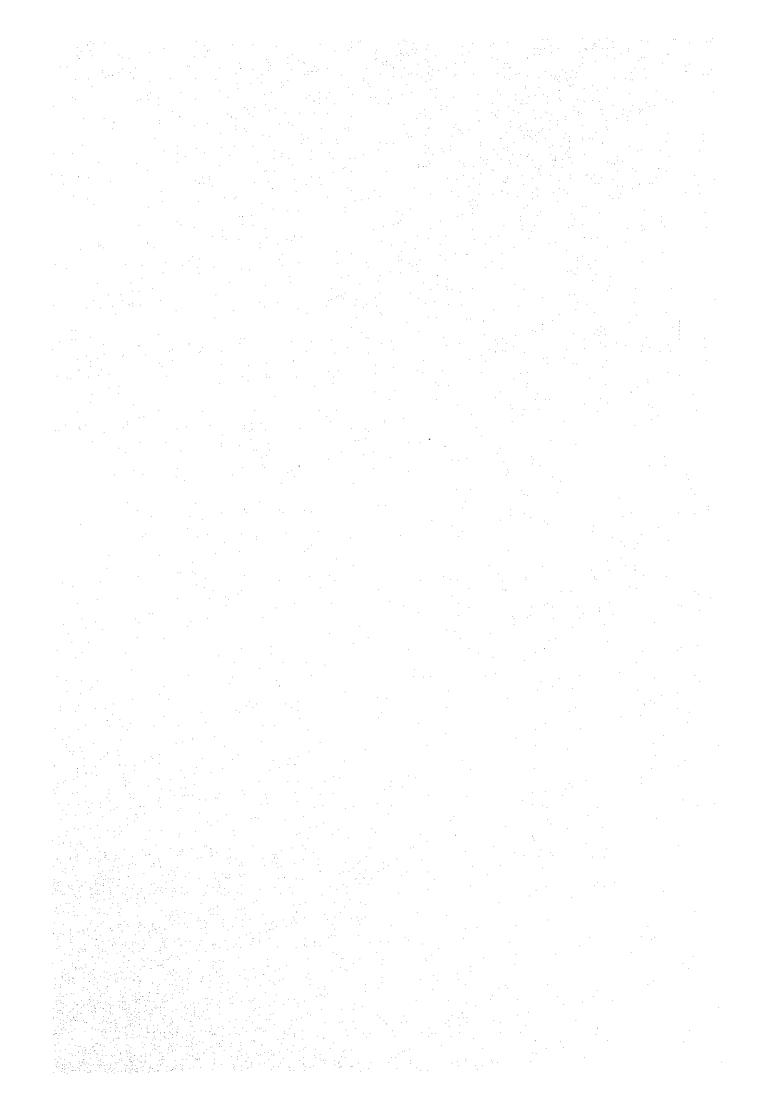
January 1986

Japan International Cooperation Agency

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ON

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FOR

THE AGRICULTURAL COOPERATIVE PROMOTION PROJECT

IN

THAILAND

JANUARY 1986

JAPAN INTERNATIONAL COOPERATION AGENCY

The Government of Kingdom of Thailand has made a request to the Government of Japan a technical cooperation, for the purpose of the promotion of the agricultural cooperative, as a key station for the acceleration of the reorganization planning of agricultural constitution, through introduction of experience and knowledge accumulated by the agricultural cooperative in Japan. The reorganization planning of agricultural construction, for the purpose of improvement of agricultural productivity and raitonalization of distribution, take its place as one of the important policy in the fifth national development plan.

In response to the request, the project in a five year has been started since the record of discussions for the project was signed on July 6, 1984 between both Governments of the Kindom of Thailand and Japan.

At present, the six Japanese experts are engaged in this project. In five model agricultural cooperative at Nakorn Ratchasima Province as the key station of this project, it is required to implement the irrigation water necessary for the agricultural management as the foundation of project activity.

The team was dispatched to the Kingdom of Thailand from October 16 to November 14, 1985 for the purpose of detail design survey of model infrastructures for the improvement of irrigation facilities such as weir and farm pond, in the farm lands of 2 model farmers groups, Chakarat area and Kong Samaki area, selected from the said 5 model farmers groups.

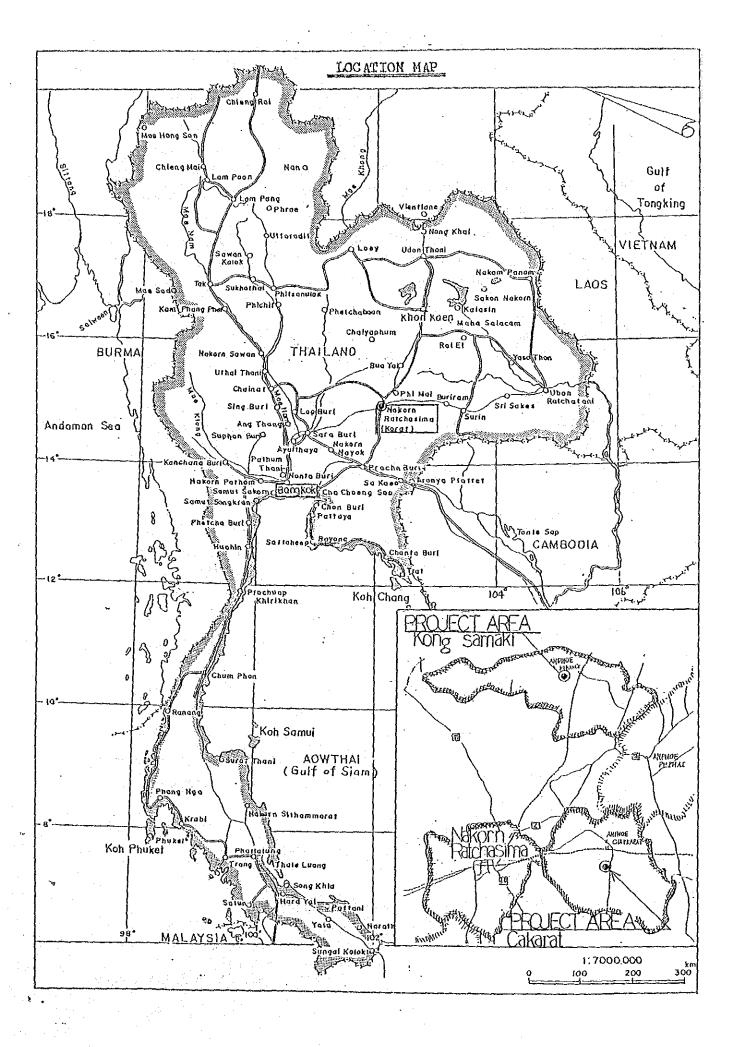
This reports presents the results of the field investigation and the subsequent study in Japan.

We hope that this report will serve as a guideline for the construction of the said irrigation facilities near future.

On behalf of the team members, I wish to take this opportunity to express my heartfelt gratitude to the all the authorities concerned for the valuable cooperation and assistance extended to the team throughout the survey period.

January, 1986

Takashi Tauchi
Director
Agricultural Development
Cooperation Department
Japan International Cooperation Agency



MAIN WORKS OF THE PROJECT

Kong Samaki

1. Construction of Farm Pond

		W	Х	D	X	. Н	Capacity	
Type	Α	. 25m	х	25m	х	5m	1,630 m³	3 place
Туре	В	30m	Х	30m	χ.	5m	2,630 m³	5 place
Type	С	40m	X	40m	Ϋ́Χ	5m	5,380 m ³	4 place
Total							39,600 m ³	12 place

Chakarat

2. Outlet Works 1 place

3. Diversion Works 1 place

4. Gates

Sluice gate B x H : 1,500 x 1,500 4 set

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CHAPTER 1

FIELD INVESTIGATION

1-1. Topographical survey

Topographic map with scale 1/5,000 prepared by C.P.D. was utilized for the general purposes. Additional topographical survey was carried out according to the following specifications to meet with the requirements for the detail designs of infrastructures.

- i) Kong Samaki area
 - 1) Survey area: 270 ha
 - 2) Closed traverse survey covering 270 ha Concrete pegs: 30 pegs
 - 3) Leveling

 Setting of a base line and mesh lines of 50 m x 50 m.

 Leveling survey at intersection points.

 Number of points: 1,300
 - 4) Temporary bench mark was set at No. 25 traverse peg and its elevation was settled at 185 m.
 - 5) Drawings were made based on the above survey results and the C.P.D. map of 1/5,000 scale.

ii) Chakarat area

Survey area was selected according to the design requirement and it was devided into 5 blocks. Plane table survey was carried out at four blocks and cross sectional leveling survey at one block.

- 1) Survey area: 30 ha
- Plane table survey
 Plane table is set at the travers peg (closed or open).
- 3) Cross sectional leveling survey
 Survey line was set along the irrigation canal with
 length about 850 m, and cross sectional interval at
 25 m.

1-2. Topography and Geology

1-2-1. Topography

Kong Samaki and Chakarat areas belong to the Nakorn Ratchasima Prefecture and are located at south west of the Korat Plateau in the Northeast Thailand. Kong Samaki area is located at about 80 km of Nakorn Ratchasima City and extends about 1.5 km from east to west and about 2 km from south to north. The rain-fed paddy field stretches for all of the area. There are no rivers and no irrigation and drainage canals, and rainfall water flows from plot to plot.

Chakarat area is located at about 40 km east of the Nakorn Ratchasima City and also the paddy field area. The distance of this area from east to west is about 1.5 km. At the center of the area, the Chakarat River flows from south to north and eventually meet with the Mun River about 70 km away.

The levees, dams and weirs for irrigation are found at several points but they are all incomplete and small in scale, and have no systematic relations each other.

1-2-2. Geology

The Korat Plateau is composed of fine-graded sandstone and shale strata which are overlain in the valley depressions with alluvium and river terrace deposit.

The investigation of the soil profile was carried out through the excavation of two test pits in Kong Samaki area. The relevant soil is fine textured and poorly drained and suitable for rice cultivation.

1-3. Meteorology and Hydrology

1-3-1. Precipitation

Rainfall data was collected at near the project sites. Period of observation is shown at Fig. 9 and the data are summarized in Table .

Annual average rainfall ranges from 1,000 mm to 1,200 mm, and about 80 % of rainfall is concentrated druing wet season from season from May to October.

Based on the analysis on the rainfall records, the characteristics of rainfall are generally summarized as follows.

- i) During the irrigation period from May to September, the months of May and September have generally greater amount of rainfall about 200 mm/month, and the months of July and August have comparatively small about 150 mm/month. (Fig. 10)
- ii) Monthly fluctuation of rainfall is large in dry season.

1-3-2. Temperature and Humidity

Since June 1985, the cooperative offices at the both Kong Samaki and Chakarat areas have started meteorological observations on rainfall, temperature and humidity. Observation period is still too short for analysis.

The data from meteorological station at Nakorn Ratchasima were utilized for the calculation of evapotranspiration of crops. The Modified Penman method was adopted.

1-3-3. Streamflow

In the Chakarat area, streamflow observations are conducted at 2 gaging stations.

1. Chakarat cooperation office

Location : Fig. 11

Observation : June, 1985 - present

2. RID gaging station

Location: Lat. N 15°10', Long. E 103°00'

about 4 km downstream from the project area

Streamflow observations records are shown in Fig. 13. At the gaging station of the Chakarat cooperation office, it was noticed that the stuff gage would be sometimes submerged during flood runoff. There are data lacking period every year in the observation records of RID gaging station.

As for the flood records, two floods were recorded in the RID observations with peak floods of 221.1 m³/sec at October 16, 1983 and of 164.3 m³/sec at September 10, 1982. Flood mark investigation was also conducted at 5 points along the river course in the project area. Based on the flood mark analysis, the maximum flood flow capacity of the present river was estimated at about 5.3 m³/sec. (Fig. 11)

1-4. Soil Mechanics

1-4-1. Field tests

Location of the test pits excavation is shown at Fig. 15.

Depth and width of the both test pits are 2 m x 2 m x 2 m and additional drilling by hand-auger was also carried out from the bottom of the test pits. Soil profiles are shown at Fig. 16 and Fig. 17. The results of the test pit investigation shows that: top soil is about 50 cm in depth containing grass routs and humus and below the top soil, the particle size of the gravel becomes larger by degrees of depth.

The ground water is observed at 1.6 m to 1.8 m from the ground surface but the obvious aquifer was not formed.

Studying the existing ponds in the both project area, slope of banking is 1: 1 and it keeps steady. The fine sand on the bank surface was washed away by the precipitation and the gravels of 1 to 2 cm are exposed on the surface. Gally erosions of about 20 cm deep was in progress at interval of about 1 m. Consequently it is considered that the soil of these area is very weak against erosion and the slope protection should be required.

The items of the soil tests are as follows.

- Specific gravity test (ASTM D 854)
- 2) Liquid limit test (ASTM D 423)
- 3) Plastic limit test (ASTM D 424)
- 4) Grain size analysis (ASTM D 422)
- 5) Standard compaction test (ASTM D 698 method A)
- 6) Field density test

The soil sample was taken from the test pit at 1.5 m depth below the ground surface, which was considered the typical zone in this area.

The results of the tests are shown at P. 48- P.64. Evaluating from "the coefficient of uniformity" and "the coefficient of curvature", the soil is well-graded and porosity is small. From the results of the liquid and plastic limit test, the soil is classified into cohesive sand (SC). (Table 13)

The compaction test shows as follows:

- i) Wopt = 13.3 %, $\gamma d \max = 1.82 \text{ g/cm}^3$
- ii) Wopt = 15.5 %, $\gamma d \max = 1.63 \text{ g/cm}^3$

Investigation the particle size analysis, consistency limits and compaction curve, the different two results of γ dmax would be originated from the volume of the fine grained materials in the soil.

From the results of the field density test, field water content in this season shows at opeimum moisture ratio. But when the construction works will be carried out in dry season, the moisture content should be carefully checked.

1-5. Water quality

The locations of sampling for the water quality test are shown at Fig. 19.

Area	Point	Site	Soil of Water			
Kong Samaki	A	existing farm pond	storage water			
Kong Samaki	В	No. 1 test pit	grownd water			
Kong Samaki	C	No. 2 test pit	grownd water			
Kong Samaki	D	Kong Provincial Office	grownd water			
Kong Samaki	Ε	Kong Hospital	grownd water			
Chakarat	F.G.H.	Chakarat River	flowing water			

According to "United State Department of Agriculture (USDA)", water samples are classified into four groups as shown in Fig. 20. with respect to sodium hazard depending on the sodium adsorption ration (S A R) value and the specific conductance. The SAR is defined as:

$$SAR = \frac{Na+}{\sqrt{(Ca^{++} + Mg^{++})/2}}$$
 (U S D A)

Where, the concentration of the ions is expressed in equivalents per million (epm).

The results of analysis of water samples are summarized below:

Site	Title of Sample	MMHOS/cm	РН	SAR	Sodium Hazard	Salinity Hazard
		25°		in the second of		
Existing farm pond	A	0.11	6	0.85	S 1	C. 1
No. 1 test pit	В	1.15	· 7	14.6	\$ 3	C 3
No. 2 test pit	C	1.20	6	16.2	S 3	C 3
Kong Provincial Office	D	3.20	6	31.5	S 4	C 4
Kong Hospital	Ε	3.30	6	22.4	S 4	C 4
Chakarat R.	F	0.06	6	0.53	S 1	-
Chakarat R.	G	0.06	6	0.58	S 1	· -
Chakarat R.	Н	0.06	6	0.63	S 1	.

From the above table it can be concluded that

- i) The ground water is not adoptable for irrigation water resource
- ii) If the ground water is diluted by the rain water, it may be adoptable for irrigation.

1-6. Irrigation and Drainage

a) Chakarat area

The irrigation water in this area is supplied from the Chakarat River which is originated from Dong Paya yen and flows down to north through the center of the area, then it meets with the Mun River at Pimai. The Chakarat River is meandering and the catchment area is composed of the alluvial soil and the paddy fields are extending at both sides of the river.

Several intake weirs are located along the Cakarat River. The intake facilities are the open diches or culverts combined with the weirs. The irrigation water flow down from the field to the field, so called plot to plot irrigation system.

The construction and protection of the river dike is imperfect. The dike is banked by the earth material near the site. Dike section is not sufficient and some part of it is as small as border dike of paddy field.

During rainy season, the flood water often flows over the dike and breaks it, and the piping has happened in some part of the dike sections.

From June to August, if the rainfall is small and the water level of the river is low, irrigation water is taken by several pumps but amount is insufficient consequently the yield of the crop is damaged considerably.

b) Kong Samaki area

The water resources of this area totally depends upon the rainfall. The irrigation of this area is plot to plot irrigation same as Chakarat.

Implementation of the farm pond with sufficient capacity is most urgently required to sustain the irrigation water.

2-1. Kong Samaki Area

(1) General

In this area, rain-fed farming is practiced during wet season because of no water resource and no irrigation system.

The seasonal fluctuation of rainfall is large and not dependable every year. The largest constraint in this area for developemnt of agriculture is the shortage of dependable water for irrigation. It is, therefore urgently required to implement the water resources and to accelarte motivation of the farmers. In lines with the above purpose of this project, the construction of small scale farm pond is proposed through the following reasons.

- 1. The flat topography of this area does not meet with the construction of the storage dam.
- 2. The river is located too far and the construction of irrigation canal is not economical.
- 3. Ground water is evaluated to be not suitable for irrigation water, because it has high concentration of salt.

(2) Water requirement

- a) Crop evapotranspiration
- i) Potential evapotranspiration (ETo)
 Potential evapotranspiration is calculated by the modified
 Penman method. The clamatic data at Nakorn Ratchasima was
 adopted for calculation. The form of the equation is as
 follows:

ETo = $c\{WxRn + (1-W)xf(u)x(ea-ed)\}$

where, ETo : potential evapotranspiration in mm/day

W : temperature-related weighting factor

Rn : net radiation in equivalent evaporation

in mm/day

f(u) : wind-related function

(ea-ed): difference between the saturation vapour

pressure at mean air temperature and

the mean actual vapour pressure of the

air, both in mbar

: adjustment factor to compensate for the

effect of day and night weather condi-

tions

The process of calculation is shown in Table 18.

As the result of calculation by modified Penman Method, the potential evapotranspiration was evaluated at 5.1 mm/day.

ii) Crop evapotranspiration (ET crop) Crop evapotranspiration was calculated by the following formula;

ET crop = KC x ETo

where, ET crop : crop evapotranpiration in mm/day

ETo : potential evapotranspiration in mm/day

KC : crop coefficient

Crop coefficient was evaluated at 1.05 in consideration of crop characteristics, time of planting or sawing, and stages of crop development. (See IRRIGATION AND DRAINAGE PAPER 24 FAO)

As the result of calculation, the crop evapotranspiration was evaluated at 5.4 mm/day.

b) Water requirement

The net water requirement was calculation by using the following formula;

where, Wn : net water requirement in mm/day

ET crop: crop evapotranspiration in mm/day

Re : effective rainfall in mm/day

Ge : groundwater contribution in mm/day

Wb : stored soil water in mm/day

The value of effective rainfall (Re) was neglected as a safety factor in consideration of the irregular distribution of rainfall. The value of "Ge" and "Wb" are too difficult to evaluate exactly, because these values are affected by method of field cultivation, control of irrigation water, and so on.

In consideration of the above, 2 mm/day was adopted as a percolation loss.

The value of net water requirement is calculated as follows;

Wn =
$$5.4 - 0.0 + 2.0$$

= 7.4
= 8.0 mm/day

The unit design irrigation requirement is calculated by the following formula;

$$Dw = 0.116 \times Wn \times \frac{100}{E}$$

where, Dw : unit design irrigation requirement in lit/sec/ha.

Wn : net water requirement in mm/day

E : irrigation efficieny in %

The value of irrigation efficiency was evaluated at 85%, then the value of the unit design irrigation requirement was concluded as follows;

(3) Planning and design of the facilities

i) Water unitlization plan

In the beginning of wet season, May, rainfall amount is more than sufficient to satisfy the requirement of irrigation water for land preparation including nursery for paddy cultivation. But in June and July, as the rainfall becomes relatively small, the insufficiency of irrigation water occurrs.

Therefore, the small scale farm pond was proposed to store the surplus rainfall during early wet season, and to supply the irrigation water for the rest of the season.

It is also possible to use the reserved water for up-land irrigation after rice cultivation.

- The location of the small scale farm pond

 The location of the pond was sellected to be at the boundary
 of each private benefit areas, considering the cooperative
 management of the farm pond by farmer themselves, in addition to the technical evaluations on the catchment area,
 benefit area and ground surface elevation.
- iii) Storage volume of the farm pond The storage volume of the small scale farm pond was determined to stores the irrigation water for 5 days.
 - v) Dimension of the small scale farm pond The dimension was dicided in consideration of following factors.
 - Prevention of evaporation from the water surface of farm pond
 - 2. Stability of the slope
 - 3. Maintenance of farmpond
 - 4. Active use of the ground water inflow from surrounding

The designed dimensions are as follows.

Type	Storage (cum) volume	size(m)	depth (m)	slope gradient	height of embankment(m)
A B	1,630 2,630	25x25 30x30	1.5 1.5	1:1.5	1.5 (max)
Č	5,380	40x50	1.5	1:1.5	1.5 (max)

2-2. Chakarat Area

(1) General

The Chakarat river flowing through the center of this area has been used as the water resources for irrigation. River water fluctuation is large. In July and August, the middle of wet season, the river flow becomes small and in the end of wet season, September, flood water overflows the paddy field. And of course, during dry season, there is no dependable water in the river for irrigation. There are several weirs and intake structures located along the river. These structures were deteriorated and their structure designs are not suitable to have proper functions.

Weirs have usually no functions for flood passage and for storage. It is therefore required to rehabilitate these existing weirs and intake structures. In this project, site A and B were selected as representative works with high priority for rehabilitation expected. Locations of site A and site B are shown in Fig. 27.

1. Site A

Intake weir at site A has no passage or spillway for flood. Left bank was damaged by overtopping and by pipng. Intake of river water is not functioned properly. Rehabilitation works required are to store the irrigation water at upstream of the weir, to keep intake water level and to have spillway function.

2. Site B

Site B is the diversion point located at right bank irrigation canal. Structure is composed of culvert which is too small to divert the irrigation water. Rehabilitation works are proposed.

(2) Planning and design of irrigation facilities

Site A Outlet works

a) Location

This outlet work is planned as spillway of flood water to downstream and intake of irrigation water to the benefit area.

The location of the outlet work is selected at the right of the bank considering following conditions.

- . Major part of catchment area is located at the right side of the bank
- . The length of the outlet structure at the right side is shorter than it is located at left bank.
- b) Design floodDesign flood (QA) is calculated as follows;

$$QA = QA' \times Fs$$

where, QA': Maximum flood estimated from the existing

flood sluice size

Fs : Safety factor

$$QA' = 5.0 \text{ (m}^3/\text{sec)}, \text{ see Fig. 11}$$

$$Fs = 1.5$$

$$QA = 5.0 \times 1.5 = 7.5 = 8.0 \text{ (m}^3/\text{sec)}$$

c) High water level

High water level is obtained at HWL 199.60 m from the elevation of the existing top bank at EL.199.80 m and free board at 0.2 m.

d) Flood overflow depth and overflow length

The relation between flood overflow depth and overflow
length is obtained by the equation as follows;

$$Q = C B H^{3/2}$$

where, Q : Discharge (m /sec)

C : Overflow coefficient (C=1.7)

B : Overflow length(m)

H : Overflow depth (m)

$$Q = 8.0 \text{ (m}^3/\text{sec)}, B = 3.0 \text{ (m)}$$

$$H = \left(\frac{H}{C \times B}\right)^{2/3} = \left(\frac{8.0}{1.7 \times 3.0}\right)^{2/3} = 1.35 = 1.5 \text{ (m)}$$

e) Design of diversion works

Full water level is determined as high as possible at high-water level, FWL. 199.60 m, in order to have the larger capacity of storage for the irrigation water. Sluice gate is designed for intake works, and two sluice gates are planned considering the overflow length of spillway. The size of a gate is as follows.

Gate width : 1.5 m

Gate height : 1.5 m

Setting the sluice gate, the full water level will be raised by 1.4 m (FWL. 199.60 m) from the existing full water level (FWL. 198.20). The increased capacity of storage will be as follows.

The capacity increase (m^3) = The increase height of the full water level (m) x the length of river (m) x the average width of river (m) = 1.40 x 400 x 15 = 8,400 (m^3)

This capacity is equipvalent volume to supply irrigation water for 7 days to the downstream benefit area.

The number of irrigable days $= \frac{\text{Capacity (m}^3)}{86.4 \text{xUnit design water (1/sec/ha) x irrigation area(ha)} }$ $= \frac{8,400}{86.4 \text{x1.09x13.16}}$ = 6.7 = 7.0 (days)

Site B Design of diversion works

a) Design flood

Design flood (QB) is calculated as follows.

$$QB = QB' \times Fs$$

where, QB': Maximum flood at the site B (m³/sec)

Fs : Safety factor

QB' = 2.4 (
$$m^3/sec$$
), See Fig. 11 .

Fs = 1.5

QB=
$$2.4 \times 1.5 = 3.6 = 4.0 \text{ (m}^3/\text{sec)}$$

b) High water level

High water level is determined at same elevation with the high water level at site A, HWL. 199.60 m.

c) Flood overflow depth and overflow length

The relation between flood overflow depth and overflow length is obtained by the equation as follows.

$$Q = C B H ^{3/2}$$

where, Q : Discharge

 (m^3/sec)

C : Overflow coefficient (C=1.7)

B : Overflow length

(m)

H : Overflow depth

(m)

$$0 = 4.0 \text{ m}^3/\text{sec}$$
, $B = 1.5 \text{ m}$

H =
$$\left(\frac{Q}{C \times B}\right)^{2/3} = \left(\frac{4.0}{1.7 \times 1.5}\right)^{2/3} = 1.35 = 1.5 \text{ (m)}$$

d) Design of outlet works

A sluice gate is designed as outlet works and the size of gate is as follows.

Gate width: 1.5 m, Gate height: 1.5 m

e) Design of diversion works

A sluice gate is adopted as diversion works and the size of gate is selected same as the gate of outlet works.

Gate width: 1.5 m, Gate height: 1.5 m

CHAPTER 3 CONSTRUCTION PLANNING

3-1. Basic assumptions

The following items should be considered prior to make a construction plan,

(1) Workable days

Mean workable day is decided as 21 days per month, considering the suspension days caused by rainfall, Sundays and national holidays.

(2) Conversion rate of earth volume

The conversion rate of earth volume for making the earth moving plan is dicided as 1 vs 1.

(3) Earth moving plan

In principle, the earth materials necessary for embankment are supplied by a excavated earth materials in the site. Earth moving plan is shown in Table .

(4) Application of manpower and construction machinery

Manpower is applied for the work, because the work scale is the comparatively small and the employment opportunity for local labour can be increased. The construction equipment is selected as follows.

Dump Truck (8 ton) transportation

Bull Dozer (11 ton) excavation or spreading

Back-Hoe Shovel (0.35 m³) excavation

Tractor Shovel (1.2 m³) loading

Vibration Roller 92 ton) compaction

Portable Concrete Mixer mixing of concrete

3-2. Construction planning

(1) Earth work

In Kong Samaki area, the construction of small scale farm pond is executed by the excavation method and the excavated materials therefrom is re-used for the embankment materials of up-land reclamation.

Taking account of the big amount of excavation and limited construction period in dry season, the plural construction equipments are required for the construction.

As to the construction of up-land reclamation, the surface soil treatment is planned for keeping the soil fertility and preventing from thrive enevenness. From the result of field investigation, the thickness of fertility soil is decided as 20 cm from ground surface. The excavated fertility soil is stocked near the job site at temporary, and continuously excavation and embankment of the lower soil from the fertility soil is executed. After completion of these works, the stocked fertility soil is spreaded on the surface of the completed embankment. When the construction work is carried out in dry season, it is necessary to add water for getting the field density in accordance with the technical specification and preventing from the reduction of soil strength caused by water seepage.

In Chakarat area, the earth work by manpower is executed because of the following reasons.

- i) The soil moving volume is not so much.
- ii) The distance of moving soil is not so long.
- iii) There are no access road for construction equipment.

(2) Concrete work

Concrete for lining canal, intake weir and so on is produced by portable concrete mixer of about 0.22 m² in capacity, and placed by manpower.

(3) Installation of gate

After completion of concrete works of intake weir, gate and acceraries are carefully installed.

3-3. Bill of Quantities

The bill of quantities of the construction works is shown in Table 21 .

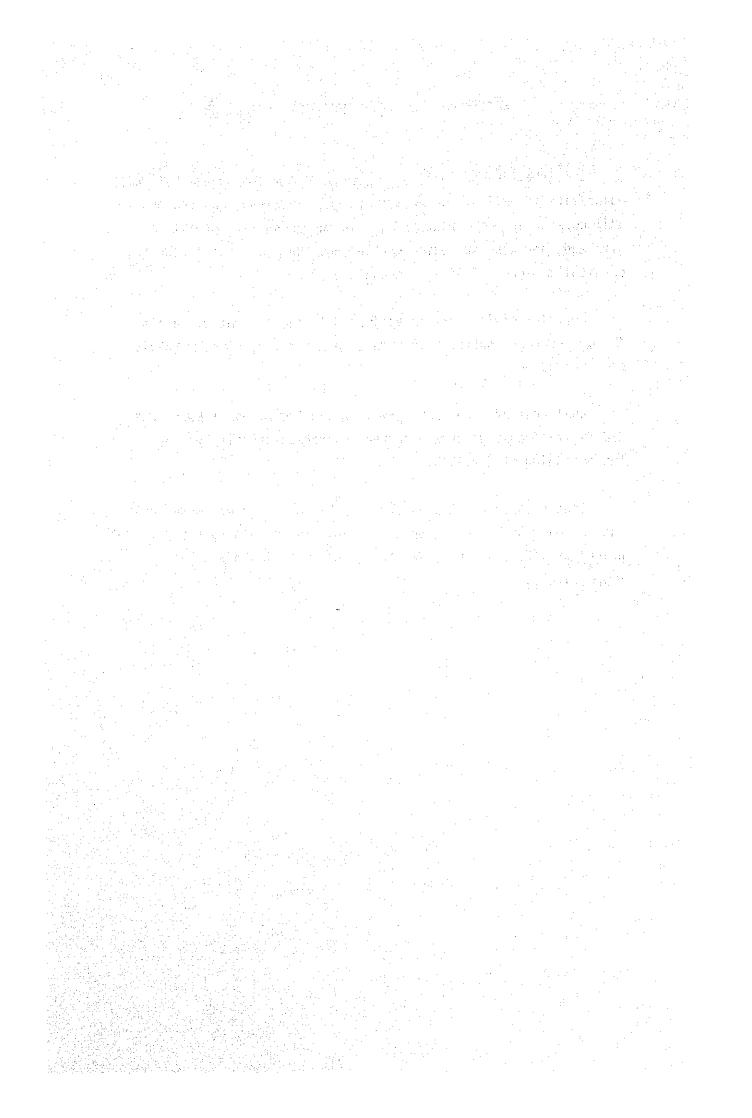
CHAPTER 4 CONSTRUCTION COST ESTIMATION

Construction cost of the project is estimated by use bill of quantities taken from the detail designs, drawings, and reasonable unit costs. The construction cost is including tax, profit and overhead, and also including contingency for price escalation and physical measures of bill of quantities.

Cost for civil works is estimated taking account of various factors such as construction method, earth moving plan, workable days and so on.

Unit cost of each work item is estimated by use labour cost and material cost which are current market prices surveyed on the beginning of Nov.1985.

Construction cost is as shown in Table 22 and estimated unit cost of each work item is shown in Table 25, 26 and surveyed market prices of labour and material are shown in Table 23 and 24, respectively.



APPENDIX

TABLES

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		***** TABLES LIST :	*****
	TABLE NO.	TITLE	보고 있어요. 이번 시험에 전해 시험하고 그리고 있다. 사람들은 이번 교육은 중국 관련하다는 기술이 되었다.
		DAILY RAINFALL	(KONG SAMAKI)
	2	DAILY RAINFALL	(CHAKARAT)
	3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3	DAILY RAINFALL	(CHAKARAT, OFFICE)
	4	MONTHLY RAINFALL	(KONG SAMAKI)
	5	MONTHLY RAINFALL	(CHAKARAT)
	6	MONTHLY RAINFALL	CHAKARAT.OFFICE)
	7	CLIMATOLOGICAL DATA FO	OR THE PERIOD (NAKHON RATCHASIMA)
	8	SPECIFIC GRAVITY OF TE	ST PIT NO.1,2
	ģ	ATTERBERG LIMITS	
	10	COMPACTION TEST	
	1.1.	SIEVE ANALYSIS	and the second s
	12	FIELD DENSITY TEST	
	13	SUITABILITY OF SOIL FO	OR BANKING AND FOUNDATION
	14	DAMAGE CAUSED BY SALT	ON RICE GROWTH
	15	TEMPORARY SALT INJURY	IN EACH GROWING PERIOD
	16	REPORT OF WATER ANALYS	
	1.7	REPORT OF WATER ANALYS	IS BY PHYSICAL AND CHEMICAL EXAMINATIONS
	18	CALCULATION OF EVAPOTE	RANSPIRATION
	19	COMPARISON OF ETO	DAN FUADODATYON METHODS
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Table-4

Monthly Raintall (HM/month)

station: knong samaki

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	Jan	Feb	Har	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Total
1977	0,0	0.0	0.0	41.9	281.6	1209	74.0	119.7	236,6	89.3	135	2.3	979.8
1978	5.6	9.7	14.6	880	2270	46.7	211.7	94.1	z84.1	66.4	IZZ	00	1010.1
1979	0.0	0.0	0.0	127.7	282.5	182.1	99.5	108.9	z36.!	0,0	0.0	0.0	1037.8
1980	0.0	65	65.1	64.5	191.1	224.9	97.7	357.1	142.6	112.5	11.1	0.0	K67.1
1981	0.0	18.1	34.1	128.3	1190	39.4	140.7	61.7	107.9	53.9	41.4	0.0	12.5
1982	0.0	67.9	<i>3</i> 0.7	65.4	289.0	90.3	12.6	1708	3326	57.2	37.5	z.5	1126.5
1983	3,8	16.3	0.0	4.9	183,2	2019	157.0	139,2	369.8	183.7	69.7	0.0	1332.5
1984	00	3Ζ	27.5	61.1	1609	1974	1618	122.2	Z16.4	73.2	1,2	0.0	1024.9
Mean	1.1	15,Z	21.5	72.7	216.8	139.0	1260	1450	240,8	79.5	Z4.3	06	1092.2
Error	108	± 7.9	180	±14.7	t zzs	t 26.3	±17.3	±3/4	± 31.3	t /8.8	±8.9	±04	
A	13	SZ	37	20	10:	19	H	22	1/3	24	37	67	
B	0.1	1.4	2.0	۵.7	20.0	128	11.6	13.5	22.2	23	2.2	0.1	

*Error show "mean square error"

A. Error/Hean , 3: Hear Monthly Rainfail/Moon Stal Fainfail

Table 5		1	Monthly	Raintall	(muy mor	na)
	71	2.3				

(office) station: Chakarat Jan May Feb Har Apr Jun Jul Aug Sep Oct Nov Dec Total 260,9 530 00 1980 00 375 1546 4176 156! Z74.9 232.4 0.0 00 1587.0 690 33! Z60.0 2360 200 1981 00 00 500 900 1700 690 1982 700 579 904 200.Z z89.Z 138.7 1983 73.T 1983 00 42.0 00 05 00 1090,9 1410 1984 00 00 zz.7 335 1728 27158 117.0 240,8 137 0.0 00 1078.3 473 211.1 93.3 1985 345 93.1 333 3//. 00 100 1097 138.9 Z00.4 z/4 74.5 129.7 245/ 185.1 Z3.4 0,0 1252. Hean 00 210 ±669 ±3/9 145.5 ± 9.8 ±*ZZ.9* ±79.0 1/// 137.0 138 / ±17.4 Eron-46 52 50 74 ZI 33 23 35 12 21 9,5 6.5 124 12.1 W3 213 16.1. 2.0 8.\ 1.9 8

Error show "mean square error"

A: Error/yean . 3: Mean Morthly Paintail/Mean total Tainty"

Table 6 Monthly Raintall (MM/Month)

Station: Chakarat

							·		Spring				
	Jan	Feb	Har	Арт	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Total
1978	48.5	1.0	83.6	35.5	176.6	66.Z	1583	182,2	376.	75.0	5,3	0,0	1708. ³
1980	0.0	<i>25.9</i>	0,0	355	1749	201.0	217.7	<i>23</i> 5/8	3/3.0	3/98	23.6	0.0	1547,Z
1981	0.0	21,8	270	409	1336	41.3	223.5	75,4	103.6	109.1	810	0.0	852,2
1982	0.0	00	10Z.b	115.1	25.5	80.5	1785	1916	597.1	1098	0.0	00	1213,8
1983	160	36.6	0.0	9,6	128.5	340.Z	154.3	507.7	1188	z399	60.7	0.0	1412,3
1984	00	00	00	46.7	166.6	129.1	1203	98.5	zs1,4	57.6	16. ^Z	00	885.9
1985	17.0	19.6	21.8	249.6	91.1	634	143.8	41.1	337.9	172.2			_
Hean	11.6	15.0	3Z.9	76!	137.8	131.7	1638	161.8	157.1	1548	3/./	00	1 185.6
*Error	± 6.8	±5.6	±16.1	± 5/4	±13.8	±40,2	±15.5	±35.9	±14.3	±36.1	±/3,3		
14 A	59	37	49	41	10	31	9	ZZ	41	<i>z</i> 3	43	_	
В	1./-1	14	3.1	7.1	12.8	12.3	15.3	15.1	14.6	14.4	2.9		_

Error show "mean square error"

Station NAKHON RATCHASTMA Index Station 48 431 Latitude 14° 58' N. Longitude 102° 05' E. Elevation of station above MSL. 187 meters
Height of barometer above MSL. 188 meters
Height of thermometer above ground 1.25 meters
Height of wind vane above ground 11.30 meters
Height of raingauge 1.00 meters

	Jen	Fab	Kap	yat	Чау	Jun	JuL	yng	Sap	0st	Жом	Dea	Y•ar
Pressure (+ 1000 or 900	uls.)												
V oon	13.93	11,85	10,11	08,61	07.09	06.16	06,22	C6 24	07,71	10,81	13,11	14.39	63,69
Ext. Max,	28.58	24.58	23.88	21.46	15.78	13.86	14.85	13.36	15,26	19,70	22.98	25.66	28,58
Ext. Vin.	05,01	01.78	60,86	98.95	59,34	97.28	97.38	97.26	98.98	01.74	03.68	03,58	97.26
Noun daily range	5.62	6.15	5.94	5.43	4.80	4.32	4,25	4.45	4.65	4.79	4.87	5,34	5.07
Temperature (C.)													
, Kean	22.9	25.7	28,1	53.0	28,4	28.1	27.6	27.3	26.5	25.9	24.2	22.5	26,4
Moan Max.	31.0	55.5	35.9	36.5	35.0	31.1	33.4	32.9	31.9	30.8	29.8	29.6	32.9
llean liin.	16.2	19.3	55.0	23.5	24.0	25.9	53,6	23.4	23.1	22.3	19.5	16,6	21.5
Ext. Hex.	37.8	40.6	42.5	12.7	41.4	40.1	40,0		39.0	35.3	35.3	35.8	42.7
Ext. Vin.	4:9	10.6	11.6	15.7	20.7	21,2	21.1	20.5	19.7	16,2	9.1	6.2	4,9
Relative Humidity (%	1												
Mean	67.0 88.6	65.0 66.4	65.0 86.1	68.0 87.2	76.0 91.3	76.0 91.1	77.0 91.4	78.0 92.2	83.0 95.1	81.0	76.0 92.0	69.0 90.4	73.0 90.5
Hean Mex, Hean Min.	43.0	40.9	40.4	43.9	53.4	55.2	56.5	58.6	64.0	63.0	56.2	48.6	52.0
Ext. Rin.	22.0	14.0	12.0	19.0	23.0	23.0	35,0	35.0	39.0	31.0	27.0	20.0	12.0
	22.0	14.0	12,0	1,7,70	(,,,,	2,10		22.0	///	711,0		2-,-	
Dev Point (°C.)		. :		1.									
Noan	15.8	17.8	19.2	21.8	23.2	23,1	22.0	22,8	23,2	22.2	19,3	16.6	20.6
Evaporation (mm.)													_
Mean - Pan	146.4	152.0	193,0	194.4	182.9	173.4	168.9	159.8	132.2	137.2	134.8	140.5	1915.5
Cloudiness (0 - 8)			1		}	•				}	}		<u> </u>
Youn	2.9	3.4	3.0	4.5	5.6	6,3	6.5	6.8	6.5	5.1	3.9	3.2	4.9
Sunshine Duration (h	r.)				}			ļ	}				·
Roeu	283.0	244.7	248,4	245.3	244.5	207,4	194.7	185.8	166.1	225.0	256.6	277.1	2780,6
Visibility (km.)									<u> </u> 				
0700 L.S.T.	3.7	3.4	3,6	5.1	8.0	9,6	9.6	9.5	7.7	6,4	5.1	4.1	6.5
Wean	7.5	6.3	6.2	7.6	9.8	10.6	10.6	10,3	9.5	9.7	9.2	8,3	8.8
Wind (Knots)			ļ			-				}			
Preveiling wind	36	не	314	S¥	Sit	SW	W	791	l w	HE	NE	НE	_
Mean wind apped	2.5	2,6	2.5	2.9	2,8	3.7	3,8	3.6	2.1	2.7	3.2	2.9	
Wax, wind speed	28 ENG	57 °C	43 SS¥	53 S	46 SE	58 SW	41,¥	35 SE	33 S,	54 SE	14 HE,E		58 5¥
Rainfall (mm.)					}				WS#	1	}	}	
llean	3.5	22.9	55.2	70.0	157.6	116.2	131.0	126,9	263.3	157.7	30.0	3,1	1137.4
Man rainy loya	1.2	2.9	6.1	7.9	15.9	15.0	15.6		19.5	12,1	3.8	0.9	. 117.1
Grootest in 24 he.	17.1	59.7	97.3	. 91,8	134.5	114.8	104.1	72.3	143.7	136.0	108,6	20,6	113,7.
Day/Your ·	26/54	23/65.	10/74	1/73	13/52	27/69	10/75	27/64	12/6B	25/76	9/55	3/70	12/68
Number of days with								:					
Kuzo	27.5	26.9	29.1	22.1	6.5	0.9	0.6	3.1	2.3	9.9	12.3	24.2	168.4
Fod	3.2	3.1	2.6	2.9	1.3	0.3	0.3	0.2	1.0	2.5	2.3	2,4	22,0
Rail	6,4	0.0	0,1	0,0	0.0	0.0	0,0	0,0	0,0	0.0	0,0	0.0	0.1
Thunderst ora	J,/i	2.0	7,5	13.4	16,9	8.5	8.2	7.5	11,3	7.0	0,6	0.0	83.5
Squall	0.0	0,0	0,1	0,0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0,0	0.2
······································	L			1	l	I			<u></u>	<u> </u>	1	L	<u> </u>

Remark 1 1, Evaporation 1762 - 1980

METOLOGICAL DEPARTMENT MINISTRY OF COMMUNICATIONS

original by "CLIMATOLOGICAL DATA OF THAILAND 30 YEARS PERIOD

การหลุขอบหาความถวงจำเพาะของลิ้น

กินตัวอย่าง คัวอย่างที่ (/) แหล่งที่เก็บ อ.คง จ.นครราชส์มา ทกสอบเมื่อ ๓๐ ๆลาคม ๑๕๖๔

ลำดับที่	Determination การทุกสุขา	สัญญลักษณ์	หนวย	ยุงมู •	alin a
1	บบ.ขวก+ น้า + ก็บ Vegit at pycnometer + Vater + vail	W ₁	กรับ	992.70	919.50
2 :	อณภาม Temperature	T	C	27	28.5
3	William + Win Weight at perometer + Water	W ₂	กรับ	665.80	662.10
4	นน. กินแหง แษเดน. of dry soil	Яз	กรับ	417.70	417.70
5	NY. BOULTHOUMAN T Specific gravity of temperature oc	$G_{\mathbf{T}}$	•••	.9965	. 9961
6	กพ. ของคิน Specific gravity	G _s	<u> </u>	2,588	2.596

พมายเหตุ $G_S = \frac{G_T \cdot W_S}{W_S - W_1 + W_2}$

ν	- Bar
นูพกสอบ •	
วิศวกร .	
หัวหน้างานวิเคราะห์วิจัย	. The true
	4.1. we 1. 65

งานวิเคภาะหวิจัย ผ่ายสาภวจและออกแบบ ได้ข้าระเงินก่าทดสอนย์สัดกรบถือนถูกต้องแล้ว สูนย์ ๆ ภพช.นคภภาชสีมา

Table -8-1 Specific gravity of Test pit 1

การทุกสอบความถวงจำเพาะของกิน

กินทั่วอย่าง

แหลงที่เก็บ

ทกสอบ เมอ

ลำคับที่	การทกสอบ	ส่ญญลักบณ์	หน่วย	ยน กรุงที่	HINN to
1	บน. ขวด + น้ำ+ ก็แ	W ₁	กรับ	921.50	918,50
2	อุณหภูมิ	T	. . .	27	28,5
3	นน.ขวก ⊦ น้ำ	W ₂	กรับ	665.80	662.10
	นน. คินแหง	Ws	กรับ	415 . 00	415.00
5	ถพ.ของน้ำทอุณหภูมิ ซ	G _T	1	•9%5	. 9965
6	ถพ. ของคิน	G	-	2.596	2.606

$$\frac{\text{Nuneum}}{\text{Nuneum}}$$
 $G_s = \frac{G_T, W_s}{W - W + W_s}$

นหกสอบ	->/&~		
1		M	
วิศากร	م	- Ze,	
หัวหน้างานวีเคราะห์	17984		

ได้ชาวะเงินค่าทดสอบบัสดุควบถ้วนถูกต้องแล้ว ลงจืด ทั้งหน้าฝานฯ

งานวิเคราะห์วิจัย ้ ป่ายสำรวจและออกแบบ ศูนย์ ๆ รพช.นครราชสีมา

การทดสอบ ATTERBERG LIMITS [HESTURE HO PO CLOSTICON ON BANTER PA ตัวอย่างที่ ชาวอยุจภาน ที่ ชนิดของวัสดุ ภาฟ วันที่ทดสอบ 30 ๑ ๑ 2 3 PLASTIC LIMIT (P.L) ทุกสถบุครั้งที่ Determination 2 ... 5 : หมายเลขภาชแะ Number of container 93 46 105 น.น. ดินชั้น + ภาชนะ Wet soil + container กรัม 3776 37.00 36.15 น.น. ดินแห้ง + ภาชนะ Drysoil + container กรัม 35.16 34.88 34.18 11.11. 11 weight of Water กรัม 2.30 2.12 1.97 11.11. May weight of container กรัม 20.73 24.30 24.58 น.น. ดินแห้ง Weight of dry soil กรับ 11 12 10.15 9.90 เปอร์เซนต์ความซึ้น (P.L) Water content % 20.68 20.89 19.90 เปอร์เซนต์ความขึ้นเฉลี่ย Average of 20.49 LIQUID LIMIT (L.L) หมายเลขภาชนะ Number of container A60 A 66 A83 A 142 แ.น. ดินชื่น + ภาชนะ Wet soil + container กรับ 53.13 52.25 60.30 53.84 แ.น. ดินแห้ง + ภาชนะDiysoil + containerกรัม 12.22 12.00 10.89 1315 11.11. 117 weight of voter กรัม 9.11 10.69 1091 10.25 11.11. Man Weight of container กรัม 2458 25.11 25.02 20.68 แ.น. ดินแห้ง Weight of dry soil กรัม 16.89 17.61 18.47 15.87 เปอร์เซนต์ความชื้น Water content 60.69 59.29 57.88 จำนวนที่เคาะ 30 40 LL = 61.80 x 11561.80% $p_{\rm L} = 20.49$ % 60 PI = Δ1.31วิศวกร /เายช่วงผู้ควบคุมหัวหน้างานวิเคราะห์ wa. 10 15 25 30 35 40 45 50 20

จำนวนครั้งที่เคาะ ได้ชำระเงินค่าทดสอนวัสดุครบถ้วนถูกต้องแล้ว ลงชื่อ หัวหน้าฝ่ายฯ

แบบฟอร์ม วส. 12 ฝ่ายสำรวจและออกแบบ ศูนย์ปฏิบัติการ รพช. นครราชสีมา

lable 2	>	4	1/0/0	(2) 100	D	
Table -9-2 ตัวอย่างที่ ชาวิธยางเกษา ที่ ? ชเโดของวัสดุ () ดาษ		โครงกั	75 3/840 ((A) ~ (A) (A)	Srr. Day	8\N\U2\!
ชนิดของวัสดุ 💮 😕		วันที่ทั้ง	าสอบ3	O 17 01. 2	<i>Y</i>	
	بسندا المحادث والمحادث والمحاد			PLASTIC L	IMIT (P.L)	
ทดสอบครั้งที่	,	1	2	3	4	5
หมายเลขภาชนะ		A16	1//	A 75		
น.น. ดินชั้น + กาชนะ	กรัม	36.26	36.80	35.68		
น.น. ดินแห้ง + ภาชนะ	กรับ	31.15	34.60	33.60		
น.น. น้ำ	กรับ	2.11	2.20	2.08		
น.น. ภาชนะ	กรับ	24.21	24.78	21.32		
น.น. ดินแห้ง	กรับ	9.91	9.82	9.28		
เปอร์เซนด์ความชื้น (P.L)	%	21.23	22.40	22.11		
เปอร์เซนต์ความชื้นเฉลี่ย	%		22.01			
	· · · · · · · · · · · · · · · · · · ·	··· ·		LIQUID LIM	IT (L.L)	
หมายเลขกาชนะ		91	087	A2,	123	
น.น. ดินชิ้น + ภาชนะ	กรัม	53.20	81.20	52.41		
น.น. ดินแห้ง + ภาชนะ	กรัม	1100	13.00		13.95	
น.น. น้ำ	กรับ	9.20	8.20	8.13		
น.น. ภาชนะ	กรัม	24.33	20.70			
น.น. ดินแห้ง	กรับ	19.67	18.26		19.54	
เปอร์เซนต์ความซื้น	%	1644	111.91			
จำนวนที่เคาะ		20	28	38	(17	
		PL	= 22.0	1		
2.2 = 21.60 %		44 43 42 41 2 40	22.50 N. Gu	ak 3	ุ้ทดสอบ สวกร ∕ม่วชชี วันน้างานวิเ	
10 15 2 จำนวนครั้งที่เคาะ	0 25 30 3	5 40 45 50				
	ψψ		-		บฟอร์ม วส. •	
ได้ชำระเงินด้าทดสอบ โสดุครบถ้วนถู					การวจและออ	
n Göre	วหน้าฝ่ายฯ	51		ศูนย์ปฏิบัติ	การ รพช. เ	เครราชส์

COMPACTION TEST

Date. BOTOBER 1985: Tested by MR. SUTHER TANIONG TINURUK Location KONG., RACHASRIMA. Type Test Dry preparation and repetitive methods and superitive methods. Boring No. 2. Mold: volume 956.141 cm Sample No
Tosted by MR. SUTHER TAMOMOTHURUK Location KONG., RACHASRIMA. Type Tost Dry preparation and repetitive methods and top and to
Location KONG. RACHASRIMA. Type Tost Dry preparation and repetitive methods in the state of the
Boring No. 2. Mold: volume 956.44 cm Sample No. : Weight data and gm. Specific gravity. Gs = d.60 RAMNER: WEIGHT 5.5 158 NUMBER OF LAYER : 3 Determination No. : 2 3 4 5 6
Sample No. Specific gravity. Gs = d.60 RAMNER: WEIGHT 5.5 163 NUNBER OF LAYER 5.3 Determination No. 1 2 3 4 5 6
Specific gravity. Gs = d-60 RAMNER: WEIGHT 5.5 163 NUNBER OF LAYER 5.3 Determination No. 1 2 3 4 5 6
Specific gravity. Gs = d.60 RAMBER: WEIGHT S.5 1158 NUMBER OF LAYER : 3 NUMBER OF TAMPING PER BACH AVER Determination No. 1 2 3 4 5 6
DENSITY NUMBER OF TAMPING PER BACH— Determination No. 1 2 3 4 5 6 1 2 3 4 5 6
Determination No. 1 2 3 4 5 6
Determination No. 1 2 3 4 5 6
Weight mold + compacted soil g 3405.00 3835.00 3969 4007. 3915 3875
Weight mold g 2017-10 dod9-10 dod9-10 dod9-10 dod9-10 dod9-10
Weight compacted soil 9 1697-80 1807-80 1941-80 1939-80 1887-80 1844-80
Wet density 9/cc 1.7514 1.890 4.030 4.090 1.974 1.934
Dry density, 8d g/cc 1.653 1.438 1.812 1.993 1.613 1.560
Void rotio e 0.57 0.50 0.43 0.45 0.60 0.67
Porosity n % 36 33 30 31 38 MO
WATER CONTENT
Determination No.
Container No. NO3-1 NO3-1 NO4-1 NO4-1 NO4-2 NO4-2
Weight container + wet soil 9 85.64 81.45 73.01 62.09 94.10 130.00
Weight container + dry soil 9 81.65 75.99 66.80 55.30 80.00 109.00
Weight water, Ww. 9 4.19 5.26 6.29 14.10 23.00
Weight container g 12.59 15.68. 15.45 11.35 14.20 10.55
Weight dry soil, Ws g 69.86 60.31 51.55 43.95 65.30 96.45
Water content, w . % 6.088 8.712 12.047 15.449 21.593 23.849
REMARKS:- $e = \frac{\gamma_w}{\gamma_1} G_3 - 1$, $\gamma_{u} = \frac{\gamma_w}{\gamma_1} G_3 - 1$
$\perp + \underline{W}$
$\mathcal{N} = \frac{e}{1+e} \times 000 \text{ (%)}$
I+R

Table - 10-1 Compaction Test

	k																							19	88			,
Boring No		1	٤٠			•		· ·					То	st	No :)					1.						-, -	
Type Test											į.)	"To	ste	d	by	+	1R.	78	TH	₽P		IAN OHC	MON	(ଜା (ଜା (ଜା	FNU F ^S 7	RUI TES	< .
			· ·	}	, ,		· · · .		سنسم	, -	· · · · · ·		· -	···-			r	 -	 _		<u>`</u>		γ	;		ı1	 r	•
33/6									<u> </u>							-	\ \{\},	ı m	aχ,	=	1.81	5	l g ∕o	c.				
density, X d										-				_				Э.М 										•
25 [1.8]		_		М,	X	DR.	<u>y</u>	下(181	y	z)-	-8 -8	15	8	<u>E</u>	-												
, (, , , ,			_				/	7	_			- - -		4	7	1	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	- C										
[[7	-					/	72			-				_	_	Zi,	120		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7	-							-
•					<u>/</u>							1			_	 		Ö	7	7	7							
16.								· 	0	771	MC	14	5	01: MT	II.	RE T	-	3.2	7	Ž —	7		-		_			
				-							_				-	-						-					-	
15	<i> </i> -		- 1		 		_				_	-	-		-						-	-			·			
		 							 2			V	 - -					,									,	•
	٠		. 5	; ·			. 1	10)				Ì	5					O:				2					
																		W	ata	r	con	tor	ıt,	เบ	%	· ·		:

SOIL MECHANICS LABORATORY

DEPARTMENT OF COOPERATION PROMOTION, ENGINEERING DIVISION .

COMPACTION TEST

Soil sample		Tost	No	$t_{\rm col}$		
		····· Date.	17	NOVEM	SER 198	5
	. ,		Me	U. J. M. M. P. SUTHEI	TEXADUST S	JI EITAURUK
Location					on and tepe	ľ
Boring No. /			: volumo		42 cm	·
Sample No.			. woight .	Loda	idg gn	
Specific gravity, $G_5 = 2.59$		Ramm	er : wei	ght	2.2	
		MONB	BR OF LAY	/er		3
DENSITY		NUMBE	R CF TAMP	ing per ba	CH LAYERS	<u>ar,</u>
Determination No.	1	2	3	4	5	6
Weight mold + compacted soil g	3712.00	3849.80	3865	3968	360/	
'Weight mold g	4027.19	2027-29	9047·19	201729	7097.73	
Weight compacted soil W g	1694.71	1798-71	1837.71	1740.71	1603.71	
Wet density Yt g/cc	1.772	1.881	1.922	1.830	11697	
Dry density, 8 d g/cc	1.593	1.634	1.603	/. IAYO	1.306	
Void: rotio e	0.63	0.59	0.62	0,76	0.98	
Porosity n	39	37	38	43	49	
WATER CONTENT				 	, 	
Determination No.						
Container No.	NO3-L.	NG4-1	102-1	10/-d	NO4-2-	
Weight container + wet soil g	65:31	79.61	以别	64.00	60.40	
Weight container + dry soil g	60.30	70.61	59.00	\$6.09	49.00	<u></u> .
Woight water, Ww g	5.0/	9.00	8.92	9.91	11.00	
Weight container 0	.15.69	//.0/	14 20	14.45	10.30	
Weight dry soil, Ws 9	14.61	59.60	144.80	121.64	38.70	
Water content, w . %	11.13	15.10	19.91	13.80	19, 121	
REMARKS: $e = \frac{\chi_{m}}{\delta d} Gs$ $ \eta = \frac{e}{1+2} $	_1 <100 (%))	78 - 1 Gs	1/w - + W - Sr		

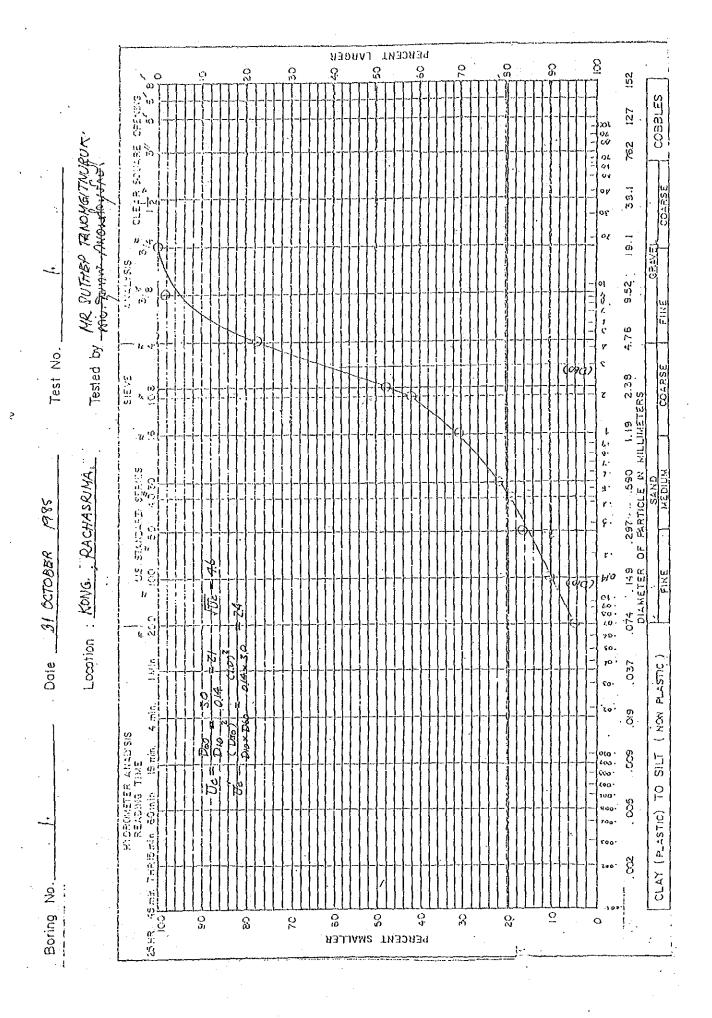
Location		angline (nicona) pamangaga di Palana, palangan pamban da di Palana da	Date.	1 ST NOVENBE	5R 1985.
, ,	<u> </u>		.Test No		, h ;'
Týpa Tast	STANDARD CO	ONPACTION.	Tested by	HR-SUTHER TA	yonentanens
22/6				Y _d max. = /1.634g	/cc
sity, & d				O.M.C. = 15-1 %	
Ory censity,	NAX DE	DENSITY = /	1634 axc		
16					
15					
<u> </u>					
· ·		OPTIMUM MOI	STURE 15./	0%	
(3				6	
	10	16	20	25	30
			· .	Water conten	, w % ··
		•	•		
,					

Toble -	I-I SIEVE ANALISIS	
Soil sample :	Soil sample weight.	Test No.
	Wt.Container + Dry soil in a 1474.88 gm	Date 31. 00108ER /985
Location : Konto, RACHASPINA.	Wt. Container in g 394.88 gm	
Boring No /	Wt. Dry soil Ws, in g. 1000.00 gm	Tested by HR. SUTHSP TANOWGITHURUK
Sample No	Specific Gravity, Gs,	10. 2011 M. 13 MOS/ W45/120
•		
-		

•	ga.com		***********	,						, ,			
	- Percent Finer	00.00/	98.58	42.54	143-43	00.77	30.32	86.18	80.61	\$5.91	(0.45	4.89	6.00
,	Cumulative Percent Retained	 	841	25.77	ts.ps	00 35 ·	89:69	70°85	60.03	87.58	56.68	11:56	op-as/
	Percent Retained		P41')	9∕-/⁄6	\$9.99	5.45	. 89.//	8.80	24.45	3.45	.: . 6:33	2.36	4-89
	Wt.soil returned in g		া //১ বত	d1/-30	199.30	-51° 78	116.60	88.do.	571.62	~\\rho.50	63.14	28.50	48.82
	₩t. sieve ÷ soil in g.	<i>৯৯</i> ৮০১	493.90	· 643.40 c	or ext	कर-४ <i>६५</i>	278-80	455.90	397 UD	ος· ππε	374:49	976 DO	343.40
	Wt. sieve in g.	297,42	06.68H	4640	M23.90	50.8/4	Op.6/4	352.00	11 349,98	390:00	309.95	304.50	6 88·76P
	Sieve opening in mm.	19.00 min	9.50 moun	~~~ × 6.9 × ···	ø.36	٠ ٩٠٥٥	/18.	09.0	Style	0:30		5.045	
	Sieve No.	3/4′	3/8	र्य	Go.	9/	<i>(</i> e)	50	\$ O.	0%	160	007	PAN.

5 978:05 BRRER = 1.916 gm

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The second secon				A		وبريد بندهور ونشست	<u></u>
		.586	ONGITHURUK.			Percent Finer	\ Co.801
	6.	1 ST NOVEHBER 1985:	Tested by ANG STOW. DHOUNDY Thos.		C. 12. 12. 12. 12. 12. 12. 12. 12. 12. 12	Percent Reformed	
	Test No.	of am Daie				Percent Retained	
ALYSIS	4	Wit Container + Dry soil in a 1494.95 gm Date.	Ö	Gs,		Wt. soil returned in g	
SIEVE AHALYSIS	Soil sample weight. Container No.	Wf.Container + Dry so	Wt. Dry soil Ws., in g.	Specific Gravity, Gs.		Wf. sieve + soil	
Table - 11-2		HASKIMA.			. 677	wt. sieve in g.	57. to-5
	e(d:	Comples KONKS., RACHASRIMA	10.	20		Sieve opening in mm.	19.00 on.m
	elombe lico	[contion	Boring No.	Sample No		Sieve No.	3/4
				····	_	_	

•							
Sieve No.	Sieve opening	Wt. sieve	Wf. sieve + soil	Wt. soil returned	Percent	Cumulative	Fercent
	in mm.	in g,	g rī	o ri	Retained	Reformed	Finer
3/4"	19.00 mm	Sn.tas	-				/ 00.00/
3/0/1	9.50 orm	09.664	486.09	67.9	59.0	29.0	99.35
4	St-4	\$66.30	ot. 969	04.0/p	to.14	41.72	30:35
©s	2.36	H43.80	69.669	63.568	89.60	49.35	20.05
0/	J. 80	46.80	88·164	53.88	5.40	<u> २६-५८</u>	45.25
/6.	81.7	14/4.50	85.185	80-60/	10.92	49:59	34.33
30	9.60	351.85	08-/44 ×	54.68	8.96	7463	45.32
, C3	6.40dS.	्र ०४·६म४ : हा	384.35	34.55	3.46	78.09	16.18
- 50	0.300	340,00	०६.६व.ह	00°67	4.73	98.03	19.14
/00/	05/-9	309.50	389.40	06.tt	2:80	20.88	11.34
රුණ	St0.0	302.90.	358.45	کرد. در در	5.86	94.43	84.5
PAN.		56.467	354-50	\$6.6S	86.5	00.00/	0.00

ERROR 136 gan

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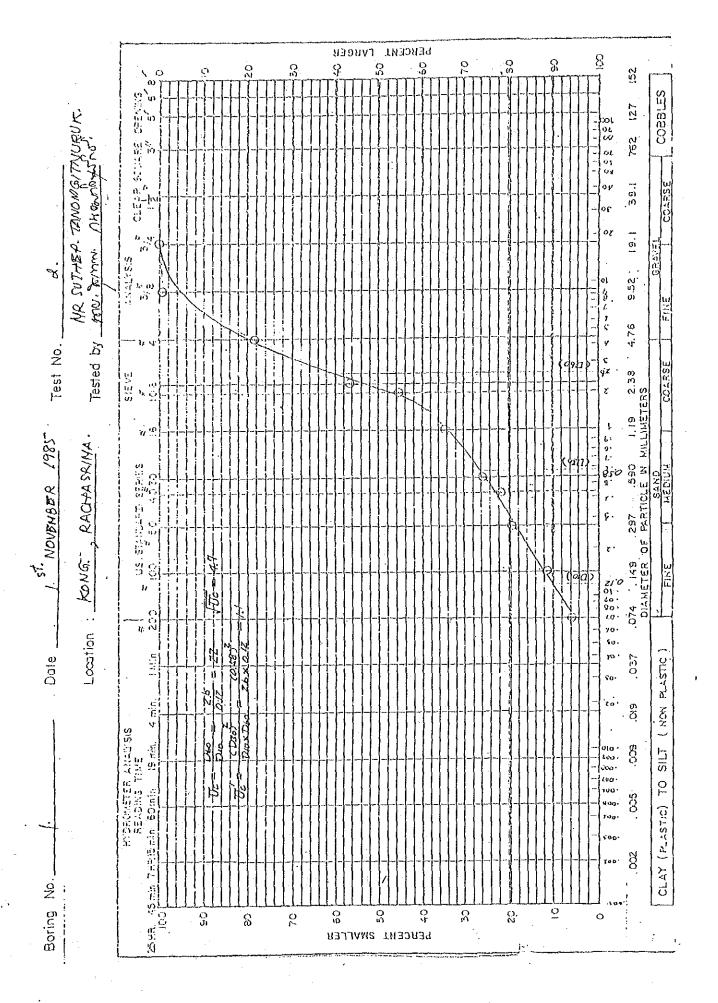
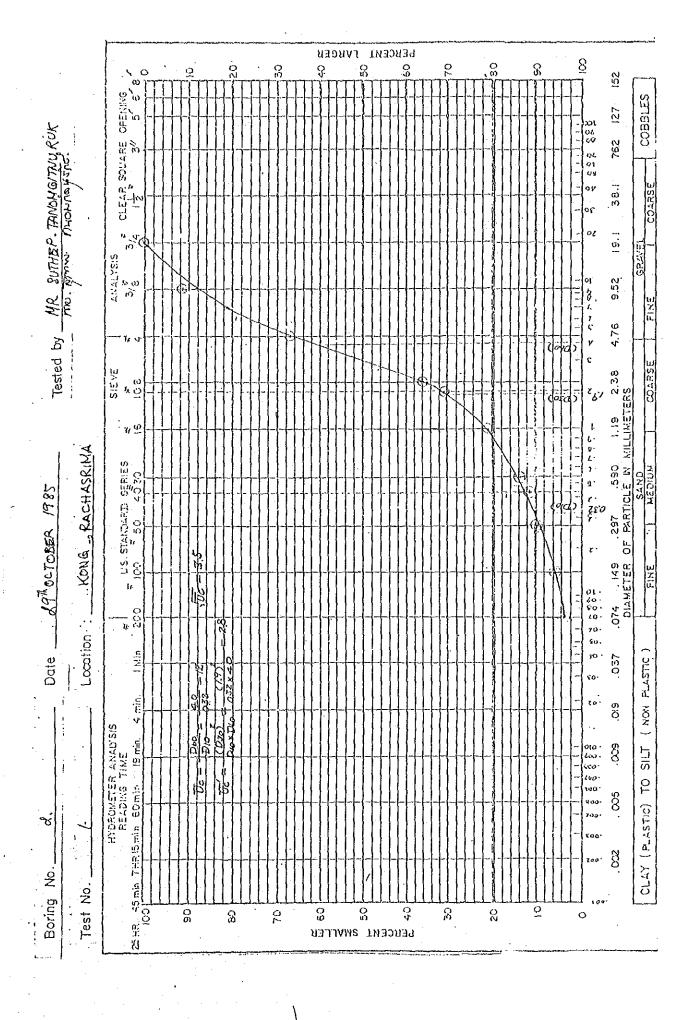


TABLE - 11-3 SIEVE ANALYSIS

	R. 1985	TANOME TRYBUK		Pacent Finer	00-08/	91.39.	. co.tg	35.89	30.69	/t.op	13.83	11.51	£8.6	5.95	3./2	0.00
	19th October	NO. FORM. DAGARATING 1800 FORMS TANGENTAL	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	Cumulative Percent Retained		19.8	38.33	इस-२१९	18:31	79.29	86.19	671.88	51.0%	50.76	28.96	100-00
Test No.	1.70 gm Date	gm. Tested by		Percent Retained .		8:61	29,42	db.10	88·83	86.6	88.0	2.32	1.64	. 3.92	4.80	8.75
	soil in g 1894.) g. (600		Wt.soil returned in g		\$6.0K	30.48%	260.82	48.80	64.80	St.39 .	29.22	01.91	39.15	00.37	31.48
Sall sample weight. Container No.	Wt.Container + Dry soil in g 1894-70 gm Wt.Container, in g 894-70 gm	Wt. Dry soil Ws., in g.		Wt. sieve + soil in g.	c8.40's	-S8:395	. 763.20 C	75.7189	0±.99h	511.89	7,024	390.82	326-350	3.48.35	330.40	325.18
	SRIMA			Wf. sieve in g.	\$06.80	449.30	51,994	A43.40	DF.4/4	M12.30	357.40	Str. 60	319.90	20d.30	302.70	०८ भारत
6)	KONG, RACHASRIMA	6 . 6		Sieve opening in mm.	19.00 m.m	9.50	メヤ:ヤ	4.36	d.60	8) 11	0.60		6.30	:845	5,60.0	-
elomos lico	Lection	Boring No.		Sieve No.	3/5"	3/8"	+	0,4	- 0/	. 9/	90	40 .	50.	/00-	900	, Van

£8808 059 gm

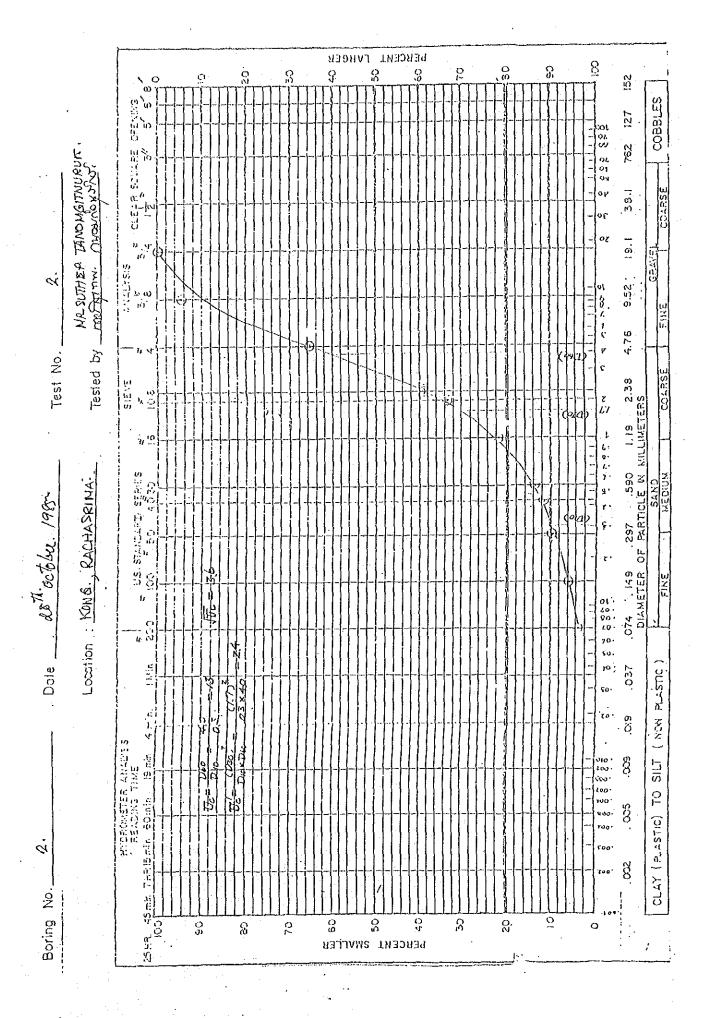
60



			HR. SUTHER TANOMOSTNURUT. MC. 701 MM. MEANOY STON		Parcent Finer	00.00/	34.44.	74.29	38-87	33.14	91.60	13.85	11.08	6.53	2.94	8.1/	0.00
	7		Dy HR. SUTHER TANON MEN MEN		Cumulative Percent Retained	ļ į	45.2	9t:118	81.19	98.99	Od:8t	51.98	55.88	90.28	90.70	68.96	100.00
	Test	o gram Date	्र च च च च च च च च च च च च च च च च च च च		Percent Retained		2.56	29.30	46.42	89.5	71.51	7.95	4.40	1.68	2.83	4.83	3.11
ALYSIS	1.	soil in a lague go gm	in g.		Wt. soil returned in g		\$5.57	441.43	468.64	26.90	115.20	75.31	43.99	16.80	. 38.43	₹. œ.	3/22
SIEVE ANALYSIS	Soil sample weight. Container No.	Wf.Container + Dry soil in g. Wf.Container in g. $d9u$	Wt. Dry soil Ms, in g. Specific Gravity, Gs,		Wt. sieve + soil		\$35:00	5 ts-tst .	983.39	०५.५६३	543.40	449.51	13-1-5	. 337.00	3 48.05	330.80	385.75
Table - 11-4		RACHASAIMA.			Wt. sieve	01.605	かなっちゃれ	466.10	25.88H	14/8·00	44.20	354.00	38.80 ···	BO. 20	309.82	202.50	294.20
† }]	φ <u>ι</u>	Location : KONG. RAC	%		Sieve opening in mm.	19:00 m-m	8,	400	8.36 8.36	9.7	8/./	0.40	・のチュバー・・・	0.50		569.0	
	Soil some	Location	Boring No. Sample No	-	Sieve No.	3/4		4	8	2	16	%	Š	20	@a/	ବ୍ୟବପ	PAN.

ERROR 1.9 gm

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		1		1	0
Ľ		See Sic		lest pic	7
1	reset in pration.	1.04	NO.1	NO.3	X0, A
Secimen	Weight m. 188	1.580	ω\$ ^γ	0/0,5	05.4.5
_	Volume T; cm3	8/6.1	703.7	1,436.5	D.747.
Wet	Wet density pt, m (%m)	. 936	5.732	2,095	2,024
Water	Water contents (%)	14.67	02.71	15.99	15.09
Dry	Dry density 20d 26/(1+4/00) (\$/cm²)	889~/	687	1.806	1, 259
(y)	Specific gravity Gs	2.23	2.892	109.2	2.601
P!0/	Void ratio e (fasiou/le / /	555.0	0.425	0,440	0.479
2 g	Regree of saturation Sr Grs. W/e (%)	70.94	8,8	94.52	46.18

Table 17:1 Field density test

SOIL MECHANICS LABORATORY

DEPARTMENT OF COOPERATILIVE PROMOTION, ENGINEERING DIVISION

WATER CONTENT OF SOIL

Soil sample		Test	No			
		Date	. ds	October.	1985.	
			NA	SUTHEP.	TANDAGIT	NURUK
10			d by "A X) h • MAXIAA • •	- X X X O Y V I A	14 24 15.
Location KONG. KAC	HASKIN	4,			· · · · · · · · · · · · · · · · · · ·	
Boring No.				•		1 .
Lia da Ma	- MA).			•	. •	
Sample No. [100] 1002, 1009	IVU).		-	•		
Specific grayity. Ge		<u></u>	· · · · · · · · · · · · · · · · · · ·			
WATER CONTENT	No	<u> </u>	NO	2.	Ν	03
Determination No.	1	d	7	7	1	۵
Container No.	A-1	A-d	8-1	B-d	C-/	C-4
'Weight container + wet soil g	18.40	100.27	85.90	100.00	99.00	102.39
Weight containor + dry soil g	61.05	89.40	75.40	89.90	87.00	90.59
Weight water, Ww 9	7.35	10.89	10,00	14.10	14:00	11.80
Weight container g	11.30	14.75	15:30	16:40	12.80	15.99
Weight dry soil, Ws g	49.95	74.65	6010	71.50	71/190	74.60
Water content, W %	14.77	14.56	19.49	16.92	16.17	15.81
		L	<u> </u>		L	
WATER CONTENT	· _ ^	104				·
Octormination No.	D=1	D-1				
Container No.		<u> </u>			 	<u> </u>
Weight container + wet soil g	116.90	85.60	ļ 			
Weight container + dry soil g	103,40	11.60	ļ <u></u>		- <u></u>	
Woight water, Ww 9	13:50	ļ			 	ļ
Weight container 0	.11.40 92.00	74.80	<u> </u>			<u> </u>
Weight dry soil, Ws 9 Weter content W %	14.67	15.50				
	l	1 .7 3 . 3	<u>L</u>	<u> </u>	l	<u> </u>
REMARKS: NO.1, NO.Z : Test	pil 1 T= 1 (47)	7+1456+111	47+1592 \:	= 159 1d=	3	
No.3, No.4 : Test		· · · · · · · · · · · · · · · · · · ·				
	7" = { (16!"	1+15.81+14.	67+1550)	= 155 010)	
	. 4		•			

Table 12-2 Water soutent of soil

Take 13 suitability or soil for banking and foundation

Sym-	Suitability for banking	Compaction	Dry density (t/m³)	Permo- ability cm/sec.	Suitabili~ ty for foundation	Adjustment for perme- ability
ત્ય	. Very good used for pervious zone of bank or dam	Good by tractor, rubber tired roller, rteel wheel roller	2.00 ~ 2.16	> 10 ⁻²	Good	Cut off wall re- quired
GP	Good used for pervious zone of bank or dam	Good by tractor, rubber tired roller, steel wheel roller	1.84 [%] 2.00	> 10_5	Good ,	Cut off wall re- quised
СМ	Fair not so suitable as impervious zone, but used for impervious core or blanket	Good by close management, by rubber tired roller, sheeps food roller ecc.	1.92 ∿ 2.16	10 ⁻¹	Good	Toe trench required ∿ needless
GC	Barely fair used for impervious core	fair by rubber tired roller, sheeps foot roller	1.84 ∿ 2.08	↑ 10 ₋₈	Good	Needless
SW	Very good used for pervious zone with slope pro- tection	, Good by tractor	1.76 ° 2.08	> 10 ⁻³	Good	Upstream blanket, toe drain or drain well re- quired
SP	Fair used for gentle slo- pe banking	Good by tractor	1.60 ∿ L.92	> 10 ⁻¹	Good-poor according their den- sity	Upstream blanket, toe drain or drain well re- quired
SM	Barely fair not so suitable for impervious zone used for impervious core or bank	Good careful operation re- quired, by rubber tired roller, sheeps foot roller	1.72 ∿ 2.00	10 ⁻³ √ 10 ⁻⁸	Good-poor according their den- sity	Opstream blanket, toe drain or drain well re- quired
\$C-	Barely fair used for impervious core of flood pro- tection bank	Fair by sheeps foot roller, rubber tired roller	1.60 ∿ 2.00	. 10 ⁻⁶	Good∽poor	Needless
ML	Poor used on proper adjustment	Good - poor careful operation is important. by rubber tired coller, sheeps foot roller	1.52 ∿ 1.92	10 ⁻¹ √ 10 ⁻⁶	Very poor in danger of Lique faction	Toe drain ∿ need- less
CF .	Barely fair used for impervious core or blanket	Fair - good by sheeps foor roller, rubber tired roller	1.52 ∿ 1.92	√ 10 ⁻⁸	Cood-poor	Needless
OL	Unsuitable for banking materials	Fair - poor by sheeps foot roller	1.26 % 1.60	10 ⁻⁴	Fair-poor in danger large sec- tlement	Needless
МН	Poor used for core in hy- draulic fill but un- suitable for coll fil	Poor - unsuitable by sheeps foot roller	1.12 ∿ 1.52	10-4 ~ 10-6	Poor	Needless
CH	Fair for gentle slope, used for thin core, blanket	Fair - poor by sheeps foot roller	1.20 ∿ 1.60	10 ⁻⁶ ∿ 10 ⁻⁸	Fair-poor	Nuedless
ОН	Unsuitable for bank- ing materials	Poor - unsuitable by sheeps foot roller	1.04 ° 1.60	10 ⁻⁶	Very poor	Needless
Pt.	Can't use for cons- truction materials	Pracrically impossible	~	-	Can't use for foun- dation	_

Table 14

Damage caused by salt on rice growth

Salt concentration and rice growth

Height of rice (cm)	Tillering (number)	Weight ratio of unhulled rice (%)	Weight ratio of unhulled rice (%)	Weight of straw (g)
80.9	21.0	53.5	100	59.3 59.0
81.4	19.5	47.0	87	59.0 55.0
79.9	20.5	46.5	86	56.0 49.0
77.5 wither	.22.0	36.0	-	45.0
	80.9 82.6 81.4 78.1 79.9	80.9 21.0 82.6 19.5 81.4 19.5 78.1 21.5 79.9 20.5 77.5 22.0	### Reight of rillering of unhulled rice (%) ### 80.9	Height of rice (cm) (number) of unhulled rice (%) rice (%) 80.9 21.0 53.5 100. 82.6 19.5 52.0 97. 81.4 19.5 47.0 87. 78.1 21.5 47.5 88. 79.9 20.5 46.5 86. 77.5 22.0 36.0 67

Table 15

Temporary salt injury in each growing period

Concentration	Nursery	period	Yield ratio (%)				
(ppm)	Germination ratio (%)	Height after 30 days (cm)	in trans- planting tíme	After setting	in boot- ing period		
0	100	13.0	100	100	100		
1,000	100	13.0	54	109	112		
2,500	100	13.0	34	79	100		
5,000	100	12.0	0	9	106		
7,500	80	9.5	0	. 0	67		
10,000	50	8.0	0	0	57		
15,000	0		0	.0.	27		

TTEMS	A		В		C		D
					7.		
1.PH Value	. 6		7	-,	6 6		6
2.Electrical Conductivity	110		1,150		1,200		3,200
3.Total Hardness (as GaCO3)	45	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	23	2.5 5.1	235		278
4.Mg ⁺⁺ ion	10.8		5.4		56.4		24
5.Ca ⁺⁺ ion	0	•	0		0		71.8
6.Na [†] ion	2		. 24		86		218
7.SAR	0.85	e de la companya de La companya de la co	14.6		16.2		31.5
		* 1					
					. *	* 8	
				•			
ITEMS	E		F		G		н
					· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·
L.PH Value	6	•	6	•	6		. 6
2.Electrical Conductivity 3	, 300		60		60		60
3.Total Hardness (as CaCO ₃)	580		30		25	4	20
i.Mg ⁺⁺ ion	96		7.2		6		4.8
	74.5		0		0		0
6.Na ⁺ ion	206		- 1	1 1	1		2
7.SAR	22.4		0.53	•	0.58	4	0.63

Table 16 Report of water analysis

REPORT OF WATER ANALYSIS Table -17-1 BY PHYSICAL AND CHEMICAL EXAMINATIONS

<u></u>		
Sampling date	October 24,1985	
Sampling place	Existing Reservoir	
Sample name	<u>A</u>	
	Analybis	RESULT (8)
ltem :	Sample name	A · · · · · · · · · · · · · · · · · · ·
1. Appearance		Little White Nud
2. Colour		0:
3. Odour		
4. Turbidity		401
S. PH value		6
6. Electrical conductivity		110
at 20°C, micromhos/cm		
7. Total solids	(ppm)	213
8. Suspended solids	(ppm)	167
9. Dissolved solids	(ppm)	46
10. Total hardness	(ppm as CaCo3)	45
11. Temporary hardness	(ppm as CaCo ₃)	45
12. Permanent hardness	(ppm as CaCo3)	0
13. M—Alkalinity	(ppm as CuCo3)	. 62
14. P-Alkalinity	(ppm as CaCo3)	0
15. Residual - Alkalinity	(ppm as CaCo3)	0
16. Chlorides Ion	(ppm as Cl ⁻)	2
17. Sulfates lon	(ppm us SO ₄ - 2)	
18. Phosphates Ion	(ppm as PO ₄ ⁻³)	
19. Nitrates Ion	(ppm as NO 1)	Detection
20. Nitriter Ion	(lppm as NO ₂)	Detection
21. Ammonium Ion	(ppm as N11 ₄ +)	None
22. Silica	(ppm as Slo ₂)	
23, Total Iron	(ppm)	3.5
24. Total Manganese	(bbæ)	
25. Residual Chlorine	(ppm)	0
26. COD-Mn		
REMARKS.		LABORATORY MANAGER
		essa de la companya d
		When Road 1951

<u>Table : 1 (-2 B)</u>	Y PHYSICAL AND CHEMICAL	
Sampling date Oc	:tober,1985	
	0.1 TEST PI'T	
Sample name B		,
	analysis resul	r (8)
Nem	Sample name	В
I. Appearance .		Little White Mud
2. Colour		0'
3. Odour		
4. Turbidity	**************************************	9 .
5. Pil value		7
6. Electrical conductivity		1,150
at 20°C, micromhos/cm		
7. Total solids	(ppm) '	1,685
8. Suspended solids	(ppm)	71.
9. Dissolved solids	(ppm)	1,614
10. Total bardness (p	om aj CaCo3)	23
11. Temporary bardness (pr	om as CaCo ₃)	23
12. Permanent hardness (p.	om as CaCo ₃)	0
13. M-Alkalinity (p	pm as CuCo ₃)	1,300
14. P-Alkalinity (p	pm as CuCo ₃)	0
15. Residual-Alkalinity (p	om as CaCo ₃)	
16. Chlorides Ion (p	om as Cl)	<u>-</u> 52
	pm as SO ₄ ⁻²)	
18. Phosphates Ion (p	om as PO ₄ -3)	
19. Nitrates Ion (pp	m as NO 3)	None
	m as NO ₂)	Detection
	om as NII ₄ ⁺)	None
· · · · · · · · · · · · · · · · · · ·	om as Sio ₂)	
23. Total Iron	(ppm)	25
	(ppm)	
25. Residual Chlorine	(ppm)	0
26. COD-Mn		(3)
REMARKS.		LABORATORY MANAGER

REPORT OF WATER ANALYSIS Table -17-3 BY PHYSICAL AND CHEMICAL EXAMINATIONS

Sampling date	October, 1985	
Sampling place	NO.2 TEST PIT	
Sample name	C	

ANALYSIS RESULT (8)

hem Sample name	c
1. Appearance	Little White Mud
2. Colour	01
3. Odour	-
4. Turbidity	17*
S. PIL value	6
6. Electrical conductivity	1,200
at 20°C, micromhos/cm	
7. Total solids (ppm)	781
8. Suspended solids (ppm)	135
9. Dissolved solids (ppm)	646
10. Total liardness (ppm as CaCo 3)	235
11. Temporary hardness (ppm as CaCo 3)	235
12. Permanent hardness (ppm as CaCo 3)	0
13. M-Alkalinity (ppm as CaCo 3)	. 296
14. P-Alkalinity (ppm as CaCo ₃)	0
15. Residual-Alkalinity (ppm us CaCo 3)	0
16. Chlorides Ion (ppm as Cl)	162
17. Súlfates lon (ppm as SO ₄ ⁻²)	
18. Phosphates Ion (ppm as PO ₄ ⁻³)	
19. Nitrates Ion (ppm as NO 3)	None
20. Nitrites (on (ppm as NO ₂)	None
21. Ammonium Ion (ppm as NII ₄ ⁺)	None
22. Silica (ppm as Sio ₂)	
23. Total Iron (ppm)	1
24. Total Manganese (ppm)	
25. Residual Chlorine (ppm)	0
26. COD-Mn	2
REMARKS.	LABORATORY MANAGER
	की कि

REPORT OF WATER ANALYSIS Table -17-4 BY PHYSICAL AND CHEMICAL EXAMINATIONS October 24,1985 Sampling date Sampling place District public office Sample name ANALYSIS RESULT (8) Sample name D 1. Appearance Little Brown Mud 2. Colour 0 1 1. Odour 4. Turbidity 30 t 5 PH value 6. Electrical conductivity 3, 200. at 20°C, micromhos/cm 1,708 7. Total solids (բբա) 8. Suspended solids (ppm) 211 9. Dissolved solids (ppm) 1,497 10. Total hardness (opin us CaCo3) 278 11. Temporary hardness (ppm as CaCo3) 278 12. Permanent hardness (ppm as CaCo3) 13. M-Alkalinity (ppm as CaCo3) 532 14. P-Alkalinity (ppm as CaCo3) Ω 15. Residual-Alkalinity (ppm as CaCo3) 0 16. Chlorides Ion (ppm as Cl) 661 (ppm as SO₄ -2) 17. Sulfates Ion $(ppm as PO_4^{-3})$ 18. Phosphates ion (ppm as NO 37) 19. Nitrates Ion None 20. Nitrites Ion (ppm as NO₂-) None 21. Ammonium fon (ppm as NII₄+) 22. Silica (opm as Slo₂) 23. Total Iron (թթտ) 10.5 24. Total Manganese (ppm) 25. Residual Chlorine (ppm) 26. COD-Mn REMARKS.

REPORT OF WATER ANALYSIS Table -17-5 BY PHYSICAL AND CHEMICAL EXAMINATIONS Sampling date October 24,1985 Sampling place Deep Well of Un Samaki Hospital Sample name ANALYSIS RESULT (S) Sample name 1. Appearance Little White Mud 2 Colour 01 3. Odour 4. Turbidity 31 5 PH value 6. Electrical conductivity at 20°C, micrombos/cm 2,072 7. Total solids (ppm) 14 8. Suspended solids (ppm) 9. Dissolved solids (mqq) 2,058 10. Total hardness (ppm as CaCo₁) 580 11. Temporary hardness (ppm as CaCo 3) 580 12. Permanent hardness (ppm as CaCo 3) _0 13. M-Alkalinity (ppm as CaCo 3) (ppm as CaCo3) 14. P-Alkalinity (opm as CaCo 3) 15. Residual-Alkalinity (ppm as Cl) 16. Chlorides ion (ppm as SO₄⁻²) 17. Sulfates Ion (ppm as PO₄ -3) 18. Phosphates fon (ppm as NO 3) 19, Nitrates ion None 20, Nitrites Ion (ppm as NO₂-) None (ppm as NH₄ +) 21. Ammonium Ion None 22. Silica (ppm as Slo₂) 23. Total Iron (ppm) (ppm) 24. Total Manganese 25. Residual Chlorine (ppm) 26. COD-Mn REMARKS.

REPORT OF WA	TER ANALYSIS
Table -17-6 BY PHYSICAL AND CHE	EMICAL EXAMINATIONS
Stapling date October 30, 1985	
Scupling place Chakaya River Point A	
Sample name F	
ANALYSIS J	RESULT (8)
Sample name	F
1. Appearance	Little Brown Mud
2. Colour	01
3. Odour	<u> </u>
4. Turbidity	131
5. PH value	6
6. Electrical conductivity	60
at 20°C, micromhos/cm	nu .
7. Total solids (ppm)	2/0
8. Suspended solids (ppm)	
9. Dissolved solids (ppm)	71
10. Total liardness (ppin as CaCo3)	30
11. Temporary hardness (ppm as CaCo3)	30
12. Permanent hardness (ppm as CaCo 3)	0
13. M-Alkalinity , (ppm as CaCo 3)	34
14. P-Alkalinity (ppm as CuCo3)	0
15. Residual-Alkalinity (ppm as CaCo 3)	0
16. Chlorides Ion (ppm as Cl-)	
17. Sulfates fon (ppm at SO ₄ ⁻²)	
18. Phosphates fon (ppm as PO ₄ ⁻³)	
19. Nitrates Ion (ppm as NO 3)	None
20. Nitrites Ion (ppm as NO ₂)	None
21. Ammonium ton (ppm as NH ₄ +)	None
22. Silica (ppm as Slo ₂)	
23. Total Iron (ppm)	1.25
24. Total Manganese (ppm)	
25. Residual Chlorino (ppm)	0 - mile (1714)
26. GOD-Nn	K. W. Colon Service Se
REMARKS.	LABORATORY MANAGER
	at Tymn Belat

REPORT OF WATER ANALYSIS Table -17-7 BY PHYSICAL AND CHEMICAL EXAMINATIONS

Sampling date	October 29,1985
Sampling place	Chakara River Point R
Sample name	G
	ANALYSIS RESULT (8)

liem Sample name	G
1. Appearance	Little Brown Mud
2. Colour	01
3. Odour	_
4. Turbidity	91
5. PH value	6
6. Electrical conductivity	60
at 20°C, micrombos/cm	
7. Total solids (ppm)	115
8. Suspended solids (ppm)	57
9. Dissolved solids (ppm) ,	58
10. Total bardness (ppm as CaCo 3)	25
11. Temporary hardness (ppm as CaCo 3)	25
12. Permanent hardness (ppm as CaCo 3)	0
13, M-Alkalinity (ppm as CaCo 3)	30
14. P-Alkalinity (ppm as CaCo3)	0
15. Residual-Alkalinity (ppm as CaCo3)	0
16. Chlorides Ion (ppm as Cl)	4
17. Sulfates Ion (ppm as SO ₄ ⁻²)	
18. Phosphates fou (ppm as PO ₄ -3)	
19. Nitrates ion (ppm as NO ₃)	None
20. Nitrites Ion (ppm as NO ₂ -)	None
21. Ammonium Ion (ppm as NH ₄ ⁺)	None
22. Silica (ppm as Slo ₂)	
23. Total Iron (ppm)	5
24. Total Manganese (ppm)	
25. Residual Chlorine (ppm)	allum
26. COD-Mn	3
REMARKS.	LABORATORY MANAGER

REPORT OF WATER ANALYSIS BY PHYSICAL AND CHEMICAL EXAMINATIONS Simpling date October 30,1985 Sampling place Chakara River under the Bridge Sumple name ANALYSIS RESULT (8) Sample name н 1. Appearance Little Brown Mud 2. Colour 3. Odour 4. Turbidity 111 5. PH value 6 6. Electrical conductivity at 20°C, micromhos/cm 7. Total solids (ppm) 136 8. Suspended solids (ppm) 9. Dissolved solids (ppm) 10. Total hardness (ppm as CaCo 3) 20 (ppm as CaCo3) 11. Temporary hardness 20 12. Permanent hardness (ppm as CaCo 1) 13. M-Alkalinity (ppm as CaCo3) 14. P-Alkalinity (opm as CaCo 3) 15. Residual - Alkalinity (ppm as CaCo3) 16. Chlorides Ion (ppm as Cl) (ppm as SO₄ -2) 17. Sulfates Ion $(ppm as PO_4^{-3})$ 18. Phosphates Ion 19. Nitrates Ion (ppm as NO 1) <u>None</u> 20. Nitriter Ion (ppm as NO 2) None 21. Ammonium Ion (ppm as NH₄ ⁺-) None 22. Silica (ppm as Sio₂) 23. Total Iron (ppm) 24. Total Manganese (ppm) 25, Residual Chlorine (ppm) 26. COD-Hn REMARKS.

Table 18

CALCULATION OF EVAPOTRANSPIRATION

(1) Modified Penman Method

(ETo = C { $W \cdot Rn + (1-W) \cdot f(u) \cdot (ea-ed)}$)

	Jan.	Feb.	Mar.	Apr.	May	Jun.	Júl.	Aug.	Sep.	Oct.	Nov.	Dec.	Remarks
I. METEOROLOGICAL DATA													
Air Temperature (°c)	22.9	25.7	1.82	29.0	28.4	28.1	27.6	223	26.5	25.9	242	22.5	
Relative Humidity (%)	62.0	65.0	45.0	68.0	76.0	76.0	27.0	78.0	83.0	81.0	76.0	69.0	
Wind Velocity (m/s)	1.0	07	1.0	7.2	1.7	1.5	1.6	57	0-/	/ /	හ ·<	7.2	
Sun Shine Duration (n/N)	0.81	0.75	0.67	0.65	29.0	0.53	0.49	84.0	0.45	29.0	0.78	0,000	
					-								
II. CALCULATION													
ea (mbar)	27.9	33.0	38.0	0.0	38.7	38.0	37.0	36.8	34.7	4.55	30.5	523	
ed=eaxRHmean/100	/8.7	9/2	24.7	27.3	29.4	28.9	28.5	5.8.3	23.8	1.22	23.0	Ø, &,	
(ea-ed)	3.2	17.5	ارې چ	12.8	9.3	9.1	8.5	0,8	6.5	Ŋ.	22	19.00	
ī	86.4	86.4	36.4	4.501	95.0	9.621	138.5	129.6	86.4	95.0	11.5.13	103.7	
f(u)=0.27(1+u/100)	0.50	0.50	0.50	0.55	0.83	0.62	0.64	29.0	0.50	0.53	057	0.55	
(1-W)	0.28	52.0	52.0	0.23	0.23	0.23	0.23	2.24	0.25	0.25	0.27	0.29	
×.	0.72	0.75	0.77	0.77	0.22	0.77	0.27	0.76	0.75	0.75	0.73	0.71	
ω α	2.2/	In rij	4.8	15.7	15.9	15.8	15.8	15.7	18.1	Ď, Ď	5.21	80 /	
Rs=(0.25+0.50n/N)Ra	٥	∞,4	7. 7	0.0	8.9	8./	90	7.7	27	200	2.9	7.7	
Rns= (1-α)Rs	4,0	6.3	15	80	6.7	6.1	5,9	5,8	Ą	0,	5.9	8.5	
Bol=f(T).f(ed).f(n/N)	2:51	15.8	/6.3	16.5	43,4	16.3	79.5	76.2	0.9/	15.5	15.5	15.1	
	0-13	2/-0	11.0	0.71	0-11	0.11	11:0	0-11	21.0	0.12	2/-0	0.13	
(N/N)	5.83	0.78	0.70	69.0	99.0	0.58	0.00	0.53	15.0	0.66	0.78	58.0	
Rnl	7.6	りべ	1,3	8	2 7	0,	0 \	6-0	0 </td <td>7.3</td> <td>19</td> <td></td> <td></td>	7.3	19		
Rn=Rns-Rn]	4,4	4,00	5.2	5:5	5.5	5-1	4,9	4-9	A	4:6	Ą	N 4	
U	1.05	1.06	1.07	1.06	1.07	4	1.02	1.03	, k	1.04	\$:	40.	av s
ETo=C{W·Rn+(1-W)·f(u)·(ea-ed)}	トン・	5.3	6.5	6.2	5.7	رم. 4	5.1	1.5	4.2	A, C)	400	A. Sh	

: temperature-related weighting factor

: net radiation in equivalent evaporation in mm/day

R

: difference between in saturation vapour pressure at mean air temperature and : wind-related function (ea-ed) F(u)

: adjustment factor to compensate for the effect of day and night weather the mean actual vapour pressure of the air, both in mbar

conditions

Ç

continue

		<u> </u>	
19	22.0	39	3 69 9
	اما	38	66.3
17 18	19.4 20.6	37	62.8
16	8.2	36 37 38	.2 59.4 62
15	16.1 17.0	35	Nai
1.4	16.1	32 33 34	53.2
13	2	33	50.3
12 13	14.0 1	32	47.6
1.1	13.1	3	4 44.9 42.
1.1 01	12.3	30	75.7
ov.	11.5	29 30	6.77 7.77 107
c 3	10.7	28	37.8
7	10.0	27.	6 35.7
9	9.3	26 2	33.6
'n	8.7	25	31.7 33.6
4	8.1	24	29.8
3	7.6	23	28.1
7	7.1	22	26.4
	6.6	2)	24.9
0	6.1	20	23.4
per-	nbar	per-	nbar
Temp	P. 1	Ten	e a

1/ Also actual vapour pressure (ed) can be obtained from this table using available Tdewpoint data. (Example: Tdewpoint is 180C; ed is 20.6 mbar)

Saturation Vapour Pressure (ea) in mbar as Function of Mean Air Temperature (T) in oC 1/

Temperature OC	U	2	7	4 6 8	ထ	10	12	14.	16	18	20	22	24	26	28	30	32	34	36	88	70
M at altitude	E								1.			. '		• .	.:						
3 3 3 3 3 3	:0	0.43 .46	7.0	7.	.52	55	58	.61	. 64	99.	69	.71	.73	.75	.77.			. 82	8	27	ເດ ເດ
	. S	77	87	ſſ	7	75	9	. 62	.65	.67	.70	.72	77	.76	.78		:	82	. 84	85	98
	000	9 %	0 0	် က	ע ע	, e	· 50	79	99	69	.7)	.73	75	77	.79	1		.83	.85	.85	.87
• •	000.6	2 0) (c		, r,	9	979	99	69	7	.73	.75	.77.	.79	8.	July		85	.86	.87	-88
ı m	3 000		, K	 	9	6	. 66	69	.71	73	75	77	79	8.	. 82	. 84	85	.86	.87	.88	-89
› া	4 000	, %	28	.61		. 66	69.	.71	.73	.75	.77	.79	.8	.82	.84	, 11.		.87	89	8	8
					: : :																7

Values of Weighting Factor (W) for the Effect of Radiation on ETo at Different Temperatures

_ continue

	36	8.1
	34	7.7
	32	7.2 1
	30	16.7 17.
	28	6.3 1
	22 24 26 28 30 32 34 36	5.9 1
	24	5.4
	22	5.0
•	14 16 18 20	12.4 12.7 13.1 13.5 13.8 14.2 14.6 15.0 15.4 15.9 16.3 16.7 17.2 17.7 18.1
-	18	14.2
	16	13.8
	14	13.5
	12	13.1
	10 12	12.7
	8	12.4
-	9 .	12.0
	7	11.7
	7	11.4
	0	11.0
		Tk4
	TOC	(T) = σ

flect of Temperature I(T) on Longwave Radiation (Rul

40.	90.
38	07
36	.08
34	. 80. 80.
32 34	60
90	10
28	.11 .10
26 28	.12
24	. 12
22	. I3
20	. 14
	15
16 18	16
17	. 18
12	. 19
10	.20 .19
8	. 22
Q	0.23
	f(ed) - 0.34 - 0.044 ed 0.23 .22 .
	0.0
Jar	.0.34
ed mbar	f(ed) = 0
	·

Essect of Vapour Pressure seed) on Longwave Radiation (Rul)

(N) - 0.1 + 0.9 n/N 0.10 .15 .19 .24 .28 .33 .37 .42 ,46 .51 .55 .60 .64 .69 .73 .78 .82 .87 .91 .	7	0	.05		.15	.15 .2 .25 .3 .35	.25	<u>.</u>	.35	.445	. 45	. 5	. 55	. 9	.55 .6 .65 .7]	75	æ	6. 28.	ō.	0.1 26.	0.0
	.0.1 + 0.9 n/N	0.10	.15	. 19	: 24		.33	.37	.42	.46	.51	. 55	.60	79.	69.	.73	.78	.82	.87	.9.	96.	1.0

Iffect of the Ratio Actual and Maximum Bright Sunshine Hours ((n/N) on Longway

– continue –

Adiustment Factor (c) in Presented Penman Equation

								,	
30%	12		1.32		1.10		1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.		0000
	6		1.10		1.10		01.11		01.1 100.1 100.8 758.
RHmax	9		1.06		1.06		1.06 .98 .92 .81		1.06
	က		 200. 488.		1.02 .94 .86 .78		1.02 .89 .79		1,000
20%	12		1.05	. 0	1.05	0	0000 0000 0000	0	000000 0000000000000000000000000000000
Hmax = 6	o.	ght = 4.0	1.05	1 3.	1.05 1.05 1.05 1.05 1.05	12.	000000 00044	, i	09/21/2
RHm	Q	Un)	800 888 888	ay/Unight	8,88,17	ay/Unign	828.0	ay/Unigh	8,8,7,9
	п	Uday	7,800	Uday	96 77 77 67	Uday	8888	Uday	88568
30%	12		00.1 00.0 00.0 00.0 00.0		1.00 28.88 28.82 28.82		0.00.00.00.00.00.00.00.00.00.00.00.00.0		0.00
a x	o,		1.00 .92 .78		1.00 .88 .81 .72		00.1 08.7 7.7 10		 08.3.0. 08.80.0.
RHm	ω		0.817.0		8.28.8		8.6.28		8533
	n		0 0 0 10 10 0 10		81-0-1		8685		8.045 045C
	m/day	m/sec					-		
1	Rs mm,	Uday	OMOD		OMIDA		ଠ ମଧ୍ୟ ଦ		റ നശ ത

		1/1	-,,,				٠,		<u>۲</u>	١	0	
	Dec	A0.6	4.0	6, ي	85.0	141.7	4.6	3.9	£.0	7	~	A
	ZoZ	134.8	4.5	% %	0.78	135.0	4,5	8.ي	/3/.0	4.4	ر. ي	Ą
	Oct	137.2	A.	8-5	84.0	δ. 0.0	A.5	3.8	135.0	Δ Δ	3.7	A ()
:	0.00	/32.2	A	200	0/8	/35.0	4.5	3.8	135.0	A.S.	დ ზ	Λ.
	Ace	159.8	5:2	A'	8 0	166.7	4.5	4.6	159.0	5.1	4,4	V
	13.77	7.88.9	ね	4.6	104.0	173.3	5.8	8,4	167.0	4	4.6	L/
	75	173.4	8	4.0	108.0	180.0	6.0	1-5	185.0 179.0	6.0	5.	Y
	May	182.9	6.5	5.0	0.40	173.3	6,6	4,0	185.0	0,8	1.5	r .
	Apr	4.4	6.5	5.5	116.0	193.3	4.	5.5	0.261	4.4	η, Δ '	/ /
	Mar	193.0	2.8	5.3	0.8//	193.3	2.8	5.3	0761	5.5	2.5	o L
	Te b	152.0	4.5	4.6	92.0	153.3	5.5	4.7	153.0	5.5	4.6	4
	Jan	484	4.7	A 0.	91.0	151.7	A.	7, 4	14.0	4.7	A O	
	month	ţ.	Pan Euporation may	×1 = 10 = mm/day	Bon Exponetion xous miny fronth	POTENTIAL PROPERTION For Exponetion month 151.7	Pan Esporation my	* Z * * T = T = T = T = T = T = T = T = T =	Pan Evaporation month	Ran Elaporation my	*3 E70	Total Control of the second of
•		CLIMATOLOGICAL For Evaporation my	DATA OF THILAND Pan Euporation Wasy	[1951-1980] ETO	ECTIMATEO	POTENTIAL EVAROTRANSPIRATION	IN THE	707		Δ		

(Pan evaporation data was observed in Nichon Rachasima) Epon = pan exappration in mm/day and represents the mean daily value *1, *2, *3 FTO (Reference crop evapotranspiration) = KP. Epar. where

kp = pan coefficient

of the period considered

= 0.85 (see FAO IRRIGATION AND DRAINAGE PAPER 24 Table 13

; case A, RHmon high, wind Light, Windward side distance

of green crop 1000 m

				10 to 10	see fig	27	
)					the number
Furm Pond	Area of	Total Area at	Volume at	Volume of	Volume at	Total Volume	ot days
1.1	poddy field		Irrigation	existing	planning	of	Inigation
No.				Farm pond	Farmyond	Farm par	possible
	() ~ ha	ha	<um <="" td=""><td>CUM</td><td>cum</td><td>CUM</td><td>days</td></um>	CUM	cum	CUM	days
	Ø5.04		Zany				
	34.91			(A) 1.000	Type C		
Pl	(3) 7.06	17.01	1,601	B 2.280	<u>380</u>	8 660	5.4
					R		
	(2) / 20		-0.0		туре В	n	0.77
PZ	4 6.20	6.20	584	C) 122	2 630	2 752	4.7
					туре А		
P3	34.16	4.16	392		1 630	1630	4,2
					PACT CONTRACT		:
	6 2.08				Type C	5 150	. 2
PS	(7) 7.31	9.39	884	D 90	5 380	5 470	8.2
1					Type B		
P5	8 7.20	7.20	678		2 630	2 630	3.9
<u> </u>							
1.							
	9 3.26						
	10 S.OZ				Type C		
PS	1 4.00	12.28	1.156		5 380	5 320	4.7
-		/2.20	7.790			0 00.	
	(Z) 4.30				Type B		
P7	(3·3.86	8.76	768	E)1.036	5 8730	3 666	4.8
					Type B		
P8	(A) 5.57	5.57	524		2 630	2 630	50
1.0			<u> </u>	 	2 350	1000	
	(S) 4.00						
1	6 5.57		,	1	Type C		
P9	(1) J. 64	13.21	1,243		5380	5 380	4.3
	® 3.77	1			1		
	1 _	0.90			Туре В		
PIO	♠ 4.∞	<u>7.77</u>	731	 	2 630	2 630	3.6
					ТуреА		\
PII	20) 4.07	4.07	383	•	1630	1 630	4,3
	60 400				Туре А		
PIZ	(E) 4.49	4,49	423		1630	1 630	3.9
							(mean)
Total		99.51	9.367	4.528	19560	44 088	4.7
l	. 		-4321		121200	124000	1

Table 20 Calculation of Farm Fond potential

Table-21-1 BILL OF QUANTITIES (kong samaki)

· commence and the second seco	<u> Item</u>		unit	Quantities
1. Construction of Fa	r'in pand			
Type A (3 place)				
	Excavation (top soil) 480×3	cum	1.440
	Excavation	300×3	cum	900
	Excavation	1.200 x 3	cum	3.600
				1. 194
	Spreading	1,200 x3	cum	3.6c0
	Spreading (top soil)	480 x 3	cum	1,440
4	Compaction.	1.980 x3	cum	5,940
			ļ	
	Smoothing tace	1.165 x 3	SAM	3.495
			<u> </u>	
			<u> </u>	
Type B (Splace)	Excavation (top soil)	720 x 5	cum	3.600
	Excavation	545 x 5	cum	2.725
	Excavation	1,900 x5	cum	9,500
			\ <u> </u>	
	Sprending	1.900 xS	cum	9,500
	Spreading (top scil)	720 ×5	cum	3.6 <i>0</i> 0
	Compaction.	3.165×5	cum	15.825
				
	Smoothing tace	1,460×5	szm	<u>7,300</u>
		· · · · · · · · · · · · · · · · · · ·		
		·		
Type C (4 place)	Excavation (top oil)	1,175 xa	CHM	4,500
	Excavation.	1.055 x4	sum	4,220
	Excavation.	4,000 x4	cum	16.CCO
	Spreading	4:000 :4	sum	15, aco
	Spreading (top soil)	1,125 x 4	CUM	4,500

the second of th				
	ltem		unit	Quantities
:	Compaction	6.180×4	cum	24.720
			·	
	Smoothing tace	2.416 x4	Sam	9,664
		ļ		
2. Inlet works				
Type A (6 place)	Excavation	24×6	CUM	144
	Smoothing face	30×6	S8M	/රිଠ
	Concrete	(4.7+0.4) × 6	cum	<u> </u>
· · · · · · · · · · · · · · · · · · ·	whoden jorn	34×6	Sam	20.4
	Slope protection	36×6	58 M	216
				<u> </u>
Type B (10 place)	Excavation	30×10	<u>cum</u>	300
	Smoothing face	38×10	SRW	<u> </u>
	Concrete	15.2÷0.€) × 10	CNW	56
	}		}	34
•	Slope projection	36×10	23m	360
			<u> </u>	
Type C (11 place)	}	24 × 11	CUM	704
	Smoothing Jace		sam	759
	Concrete		1	83.6
	1		-	37.4
	Slope protection	39×11	<u> </u>	396
				
				<u></u>
		·		
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				<u> </u>
			<u></u>	<u> </u>
			 	
	<u>85</u>		L	<u> </u>
	Type B (10 place)	Compaction Smoothing face Inlet works Type A (6 place) Exavation Smoothing face Concrete Worken form Slope protection Excavation Smoothing face Concrete Worken form Slope protection Type C (11 place) Excavation Excavation Smoothing face Concrete Worken form Slope protection Excavation Smoothing face	Compaction 6.180×4 Smoothing face 2.416×4 2. Inlet works Type A (6 place) Examination 24×6 Smoothing face 30×6 Concrete (4.7+0.4)×6 Ideal form 34×6 Slope protection 36×6 Type B (10 place) Examption 20×10 Concrete 15.2+0.4)×10 Worden form 34×10 Slope protection 36×10 Type C (11 place) Examption 36×11 Smoothing face 69×11 Concrete (7.2+0.4)×11 Worden form 3.4×11	Compaction 6.180×4 cum Smoothing face 2.416×4 S&m 2. Inlet works Type A (6 place) Exavation 30×6 S&m Concrete (4.7+0.4)×6 cum Worken form 3.4×6 S&m Shope protection 36×6 S&m Concrete 15.2+0.4)×10 cum Worken form 3.4×10 S&m Concrete 15.2+0.4)×10 cum Worken form 3.4×10 S&m Slope protection 36×10 S&m Slope protection 36×10 S&m Slope protection 36×10 S&m Concrete 15.2+0.4)×11 cum Smoothing face 69×11 S&m Concrete (7.2+0.4)×11 cum Jacken form 3.4×11 S&m

Table-21-2 BILL OF QUANTITIES (Chakarat)

The same of the sa	<u> </u>	unit	Quantities
1. Outlet works		:	
	Exervation 110+50+190+610	= um	960
	Embankment 540:100	Sum	440
	Slope protection 240+350	Sam	590
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		ļ	
	Concrete	cum	59.
	Wooden torm	sem	30.
		ļ	
9 🗅		 	
2. Diversion works		 	
	Excavation 180+200+90+190	CUM	610
	Enbankment 200+160	CHW	360
	Slope protection 480-120	szm	600
			,,,,,,
	Carata	 	89.5
	Concrete	Sam.	90.5
•	ALGERTA GOLIN	szm	70.
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6 Page 1			

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Table 22	Project Cost			
	Quantity	Equipment Cost (B)	Construction	Remarks
1. Construction Cost				
A. Direct cost Kong Samaki				
1. Construction of farm pond 2. Appertenant structure	39,600 m³		1,706,000	
Sub-Total Chakarat			2,158,000	(<u>)</u>
3. Outlet works 4. Division works 5. Annortenant structure	set set	000	139,000	4
Sub-Total	ט ט מ	000.5	362,000	(2)
B. Indirect cost			504,000	$(3)=\{(1)+(2)\}\times$
C. Physical contingency			211,000	{(1)+(2)+(3)}x7%
Sub-Total			715,000	
Total			3,235,000	(4)
II. Others		-	305,000	$(4)\times9.4\%(\pm)$
Grand Total		1,000,000 8	3,540,000 &	