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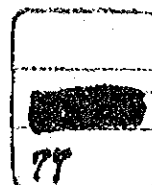
THE KINGDOM OF THAILAND

FEASIBILITY REPORT  
ON  
IRRIGATED AGRICULTURAL DEVELOPMENT PROJECT  
IN  
THE WEST BANK TRACT OF THE GREATER CHAO PHYA

(APPENDIX)

MAY 1977

JAPAN INTERNATIONAL COOPERATION AGENCY



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国際協力事業団	
受入 月日 '84. 9. 24	122
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Table A. 1-1 Personnel Concerns the Mission Contacted

Dr. Thalong Thamlong Navasawat	Deputy Undersecretary of MOAC
Dr. Chaiyong Chuchart	Secretary General of ALRO
Mr. Xujati Pramoolpol	Director General of Department of Technical and Economic Cooperation
Mr. Chamlong Attanatho	Inspector General, MOAC
Mr. Charin Atthayodhin	Deputy Director General, RID
Mr. Chern Banrungwong	Deputy Director General, Department of Cooperative Promotion, MOAC
Mr. John Boonlu	Director of COLC
Mr. Kangwang Dhephasadin Na Ayutthaya	Deputy Secretary General, ALRO
Mr. Paitoon Palayasoot	Deputy Director of COLC
Dr. Suthiporn Chirapanda	Chief of Division of Research & Planning, ALRO
Mr. Leek Jindasangwan	Head of Water Operation Center, RID
Mr. Pitipong Pungbun Na Ayutthaya	Chief of Operation Division, ALRO
Mr. Suraphol Phetlom	Chief of Extension Land Reform Office, Ayutthaya
Mr. Chutchawal Swatdirunk	Assistant Chief of Water Operation & Maintenance Section, RID
Mr. Chalermthep Ratanaprayooh	Project Coordinator, Secretary of Deputy D.G., RID
Mr. Prasart Milintrangul	Head of Research & Applied Hydrology Section, Hydrology Division, RID
Mr. Damrong Jaratwat	Chief of Division of Hydrology, RID
Mr. Danai Triyadhen	Chief of Land Classification Section, RID
Mr. Pradit Ritruongdej	Agronomist of Division of O/M, RID
Mr. Maitri Poolsup	Civil Engineer at Design Section, RID
Mr. Annokul Saravisuta	Project Manager of Sappaya Land Consolidation Project, RID



Mr. Poonsin Lekmanee	Project Engineer of Chao Phya Irrigation Development Project, RID
Mr. Laiad Sainamkeon	Staff of Chanasut Land Consolidation Project, RID
Mr. Sutin Mulphruk	Engineer of Operation Division, ALRO
Mr. Witchitr Warakitpanich	Project Manager of Bangbarn Pumping Irrigation Project, RID
Mr. Phunchai Arworn	Project Manager of Chao-Chet Bangyeehon Project, RID
Mr. Tirasakdi Packdijitt	Agricultural Officer in Chao-Chet Project, RID
Mr. Anurat Ratanakwan	Project Manager of Bhak Hai Irrigation Project, RID
Mr. Pornsak Choloipock	Staff of Bhak Hai Irrigation Office, RID
Mr. Prakit Oon Vimol	Regional Engineer of Region 7, RID
Mr. Suporn Pornprapunth	Chief of O/M of Phraya Banlu Project, RID
Mr. Suphon Chirapuntu	Soil and Civil Engineer, Geotechnical Division, RID
Mr. Kamol Toleb	Staff of Operation Division, ALRO
Mr. Surathep Kowangoon	Staff of Operation Division, ALRO
Mr. Sunthorn Tanthavorn	Chief of Project and Planning Division, Provincial Electricity Authority (PEA)
Mr. Pracherd Sook Kaew	Chief of Project Area No.3 Section, PEA
Mr. Y. Yamada	Electric Expert of PEA, under Colombo Plan, JICA
Mr. Damrongsak Tasanasanta	Staff of Agricultural Land Reform Office
Mr. Peerasak Booranasophon	Planning Division, Dep't of Cooperative Promotion, MOAC
r. T. Hidaka	Entomologist of Tropical Agriculture Research Center, Japan
Mr. Sadao Hatta	Project Leader, Tropical Agriculture Research Center, Japan

Mr. Kohn Chuvanond	Chief of Marketing Section, Agricultural Cooperative Division, MOAC
Mr. Surin Chulpraser	Director General of Dep't of Cooperative Promotion, MOAC
Mr. Hiroshi Takagi	Technical Advisor of BAAC
Mr. Oosot Chanlej	Agronomist of Agricultural Division, RID
Mr. Prayong Prayunhong	Chief of Ayuthaya Provincial Agri- cultural Extension Office
Mr. Vibul Malisen	Provincial Agricultural Extension Officer, Amphoe Lad Bao Laung
Mr. Pirot Vivasanan	Provincial Agricultural Extension Officer, Amphoe Lad Bao Laung
Mr. Srony Taso	Provincial Statistical Officer
Mr. Piroat Nivanont	Staff of Amphoe Lad Bao Laung Office, Cooperative
Mr. Sunthorn Naka	Staff of Experimental Station Ilantra
Mr. Boonlert Klajprayong	Staff of Suphan Buri Rice Experi- mental Station
Mr. Vichien Sasiprapa	Staff of Suphan Buri Rice Experi- mental Station
Dr. Hitoshi Takahashi	Agronomist, Tropical Agriculture Research Centre, Japan
Mr. Thawal Polpuech	Chief of Projects, Colombo Plan Programme, DTEC
Mr. Sutin Sulila	Staff, Colombo Plan Programme, DTEC
Mr. Takamori Igarashi	Soil Scientist, Tropical Agriculture Research Center, Japan
Mr. Kunio Hamamura	Rice Breeder, Tropical Agriculture Research Center, Japan
Mr. Ektrika Kohkongha	Deputy Director General, National Statistical Office
Dr. Thiravira Subhanoi	Staff of Agricultural Land Reform Implementation Division, ALRO

Dr. Gisuke Takahashi	Colombo Plan Expert on Soil Fertility, Dep't of Agriculture, MOAC
Mr. Fumio Kobayashi	Irrigated Agriculture Specialist, Mekong Committee, ESPAP
Mr. Vichak Monsri	Chief Geotechnical Engineer, Krungthep Engineering Consultants, Co., Ltd.
Mr. Tanom Kladkaew	Managing Director, Krungthep Engineering Consultants, Co., Ltd.
Mr. Kitti Phanick	Deputy Manager, Federation of Agricultural Cooperatives
Mr. Koichi Nonaka	Project Manager of Regional Develop- ment Studies Project, JICA
Miss. Supha Sing Inn	Chief of Statistic Section, Planning Division, RID
Mr. Somphol Pithiyakul	Chief of Farm Products Division, Agricultural Co-operatives Federation
Mr. Prom Totieng	Chief of Legal Division, ALRO
Mr. Wera Uholelket	Legal Division, ALRO
Mr. Tasanapong Ettakkapark	Division of Research and Planning LARO
Mr. Hideo Kanamori	Secretary General, Japanese Chamber of Commerce, Bangkok
Mr. Isami Itoh	Unicoop Japan, Bangkok Branch
Mrs. Nongnuch Rochanavedya	Chief of Analysis and Control Data Unit, Statistical Technique Division, National Statistics Office
Mr. Udom Kasetravetin	Chief of Design Section, Village Water Supply Division, Department of Health
Mr. Krairirik Chaliengratchai	Acting Chief of Land Reform Coordination and Farm Development, Land Reform Division, ALRO
Mr. Arli Kuldilokchai	Operation Division, ALRO
Mr. Ammuey Somsin	Hydrologist, O/M Division, RID
Mr. Songpol Suvannadabbe	Land Reform Division, ALRO

Table A. 1-2 Data List used for Feasibility Study

A. Hydrology

Water Level and Discharge

1. Thailand Hydrological Year Book, Volume 5-9, Royal Irrigation Department (RID)
2. Daily Water Level in Chao Phya river at Sing Ha Nat Apr. 1967 - Oct. 1976, Chao-chet Bangyeehon Project Office, RID
3. Daily water level in Suphan river at Phraya Banlu, Apr. 1966 - Oct. 1976, Phraya Banlu Project Office, RID
4. Daily water level in Pasak river, at A. Muang, Ayutthaya, Apr. 1950 - Mar. 1972
5. Daily Water level in Chao Phya river at Wat Chulamani A. Bangbar Ayutthaya, Jun. 1950 - Oct. 1976, Bangbar Project Office, RID
6. Daily Water level and discharge in Chao Phya river, RID
  - at Nakon Sawan Apr. 1967 - Mar. 1976
  - at Chai Nat Apr. 1967 - Mar. 1976
  - at Wat Chulamani Apr. 1967 - Mar. 1976
  - at In Buri Apr. 1967 - Mar. 1969
7. Daily intake discharge of Water and Water level at Pakai Chao-chet, Bang Puraman, Bangyeehon and Lat Chadoo, Oct. 1972 - Oct. 1976, RID
8. Daily water level at inland canal surrounding Project area
  - Phraya Banlu canal at Sing Ha Nat Regulator, Apr. 1967 - Oct. 1976
  - Nai Chat canal at Nai Chat Regulator, May 1975 - Oct. 1976  
Chao-chet Bangyeehon Project Office, RID
  - Phraya Banlu canal at Phraya Banlu Regulator, Apr. 1966 - Oct. 1976
  - Phra Udom canal at Khlong Phra-Udom Regulator, Apr. 1971 - Oct. 1976, Phraya Banlu Project Office, RID

Tide

9. Tidal data of Chao Phya estuary
10. Tide table, Vol. 1, Vol. 2, Thai Water 1976, Hydrographic Department, RID

Salt Water Intrusion

11. Salt water intrusion to Chao Phya river in the dry season (Jan. - May) at Samut Prakarn, Smrong, Pra-Kanhong, Krungtep Bridge and Memorial Bridge, 1964 - 1976, RID

Water Quality

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  - at Sing Ha Nat Regulator, Chao-chet Bangyeehon Project Office, RID Apr. 1952 - Dec. 1976
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39. Chao Phya Irrigated Agriculture Development Project, Northern Chao Phya area, 1/200,000, RID
40. Irrigation and drainage network on the Central Plain of Thailand, 1/200,000, RID
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45. Lad Bao Laung Project area in Ayutthaya, 1/50,000 Land Reform Operation Division, ALRO
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64. Cadastral map of Project area in Lad Bua Laung, 1/5,000, Ayutthaya Extension Land Reform Office
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Table A 3-1 Temperature

(Unit: °C)

	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Year Mean
<u>Monthly Mean Temperature</u>													
Lop Buri	26.4	28.5	30.2	31.1	30.1	29.1	28.4	28.3	27.9	27.7	26.8	25.8	28.3
Suphan Buri	26.1	28.5	30.5	31.8	30.7	29.9	28.2	28.9	28.4	28.0	27.0	25.8	28.7
Bangkok	25.5	27.1	28.6	29.5	28.0	28.5	28.0	27.8	27.5	27.4	26.6	25.3	27.6
Kanchana Buri	25.5	28.1	30.2	31.4	29.9	28.7	28.2	28.1	27.9	27.1	26.1	24.8	28.0
Mean (1)	26.0	28.0	29.8	30.8	29.9	29.2	28.5	28.3	27.9	27.7	26.8	25.6	28.2
Mean (2)	25.9	28.1	29.9	31.0	29.9	29.1	28.5	28.3	27.9	27.6	26.6	25.1	28.2
<u>Monthly Extreme Minimum Temperature</u>													
Lop Buri	8.4	13.5	16.1	17.5	20.0	20.5	21.0	21.0	20.5	19.6	10.5	10.7	8.4
Suphan Buri	9.2	12.0	14.8	19.4	20.9	20.2	21.1	20.8	20.8	19.4	14.5	10.0	9.2
Bangkok	9.9	14.9	16.5	19.9	21.1	21.7	21.9	21.2	21.3	19.8	14.2	10.5	9.9
Kanchana Buri	5.5	12.1	11.2	17.2	21.5	22.0	20.8	21.5	20.8	18.0	11.5	6.8	5.5
Mean (1)	9.2	13.5	15.8	18.9	20.7	20.8	21.3	21.0	20.9	18.6	13.1	10.4	9.2
Mean (2)	8.3	13.1	14.7	16.5	20.9	21.1	21.2	21.1	20.9	18.5	12.7	9.5	8.3
<u>Monthly Mean Minimum Temperature</u>													
Lop Buri	19.0	21.9	23.8	24.8	24.8	24.3	24.0	24.1	24.0	23.5	21.4	19.2	22.8
Suphan Buri	18.7	21.1	23.2	25.0	25.1	24.8	24.5	24.5	24.5	24.2	22.1	19.4	23.1
Bangkok	20.4	22.7	24.5	25.6	25.3	25.0	24.8	24.6	24.4	24.3	22.9	20.6	23.7
Kanchana Buri	17.5	20.4	22.8	24.7	24.9	24.5	24.0	24.0	23.7	23.0	20.8	17.9	22.3
Mean (1)	19.4	21.9	23.8	25.1	25.1	24.7	24.4	24.4	24.3	24.0	22.0	19.7	23.2
Mean (2)	18.9	21.5	23.6	25.0	25.0	24.7	24.3	24.3	24.2	23.6	21.7	19.3	23.0
<u>Monthly Mean Maximum Temperature</u>													
Lop Buri	32.6	34.6	36.2	37.1	35.4	33.6	32.7	32.3	31.7	31.6	31.4	31.4	33.4
Suphan Buri	31.9	34.2	36.2	37.4	35.5	34.3	33.3	32.9	31.8	31.2	30.6	30.4	33.2
Bangkok	31.8	32.7	33.8	34.8	34.1	32.9	32.3	32.1	31.7	31.5	31.1	31.1	32.5
Kanchana Buri	32.2	34.9	37.0	37.8	35.5	33.6	32.8	32.8	32.4	31.5	30.8	30.5	33.5
Mean (1)	32.1	33.8	35.4	36.4	35.0	33.6	32.8	32.4	31.7	31.4	31.0	31.0	33.0
Mean (2)	32.1	34.1	35.8	36.8	35.1	33.6	32.8	32.5	31.9	31.4	31.0	30.9	33.2
<u>Monthly Extreme Maximum Temperature</u>													
Lop Buri	38.4	38.5	40.6	41.8	41.5	38.5	37.0	36.0	35.2	34.4	34.6	34.6	41.8
Suphan Buri	36.7	39.8	41.0	42.2	42.6	39.8	38.0	37.1	35.6	34.5	34.9	35.0	42.6
Bangkok	36.0	36.6	38.9	39.0	39.4	36.8	36.0	35.3	35.7	34.5	35.1	35.2	39.8
Kanchana Buri	37.2	40.3	41.7	43.5	41.6	38.4	37.8	37.5	37.6	37.3	37.5	35.5	43.5
Mean (1)	37.0	38.3	40.5	41.0	41.2	38.4	37.0	36.1	35.5	34.5	34.9	34.9	41.2
Mean (2)	37.1	38.8	40.8	41.6	41.3	38.4	37.2	36.5	36.0	35.2	35.5	35.1	41.6

Note: Mean (1): Mean value of three stations except Kanchana Buri  
Mean (2): Mean value of four stations

Source: Meteorological Department  
Monthly Mean Temperature 25 years (1951 - 1975)  
Monthly Extreme Minimum Temperature 25 years (1951 - 1975)  
Monthly Mean Minimum Temperature 25 years (1951 - 1975)  
Monthly Mean Maximum Temperature 23 years (1951 - 1973)  
Monthly Extreme Maximum Temperature 25 years (1951 - 1975)

Table A 3-2 Relative Humidity, Pan Evaporation and Sunshine Hours Duration

Station	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sept.	Oct.	Nov.	Dec.	Annual Mean
Lop Buri	56.9	60.5	61.5	63.9	71.0	74.5	77.0	78.5	81.0	76.5	69.0	60.8	69.3
Suphan Buri	63.7	64.2	61.4	62.1	68.3	70.0	72.7	74.8	79.5	79.6	74.5	68.3	69.9
Bangkok	72.2	76.0	76.3	76.5	79.9	79.9	80.9	81.7	84.4	83.5	79.5	74.7	78.8
Kanchana Buri	62.5	60.7	57.5	60.0	70.2	72.8	73.9	74.3	77.6	79.9	74.6	67.4	69.1
Mean (1)	64.3	66.9	66.4	67.5	73.1	74.8	76.9	78.3	81.6	79.9	74.3	67.9	72.7
Mean (2)	63.8	65.4	64.2	65.5	72.4	74.3	76.1	77.3	80.6	79.9	74.4	67.8	71.8

Note: Mean (1): mean except Kanchana Buri Mean (2): mean of four stations  
Source: Meteorological Department, 22 years (1951-1972)

Pan Evaporation (mm/month)

Station	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sept.	Oct.	Nov.	Dec.	Annual Mean
Suphan Buri	131.6	131.0	185.1	205.9	191.7	165.8	154.0	146.2	135.9	120.8	121.0	116.1	1,805.1
Bangkok	138.6	137.7	185.8	189.1	165.0	147.9	142.9	137.4	129.8	122.5	121.2	126.5	1,744.4
Mean	135.1	134.4	185.5	197.5	178.4	156.9	148.5	141.8	132.9	121.7	121.1	121.3	1,774.8

Source: Meteorological Department  
Suphan Buri, 8 years (1963-1970)  
Bangkok, 10 years (1961-1970)

Sunshine Hours Duration (hours/day)

Station	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sept.	Oct.	Nov.	Dec.	Annual Mean
Bangkok	8.9	8.8	8.6	8.5	7.3	6.3	5.4	5.4	5.3	5.9	8.2	8.5	7.3

Source: Meteorological Department, 20 years (1951-1972)

Solar Radiation (cal/cm<sup>2</sup>/day)

Station	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sept.	Oct.	Nov.	Dec.	Annual Mean
Bangkok	398	424	457	470	425	409	403	385	370	375	412	401	411

Source: Meteorological Department, 11 years (1965-1975)

Table A 3-3 Wind

	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
<u>Prevailing Wind Direction</u>												
Lop Buri	NE	S	S	S	S	S	S	S	S	NE	NE	NE
Suphan Buri	NE	S	S	S	SW	SW	SW	SW	SW	NE	NE	NE
Bangkok	NE	S	S	S	S	S	SW	SW	SW	NE	N	N
Kanchana Buri	NE	SE	W	W	W	W	W	W	W	W	NE	NE
<u>Mean Wind Speed (knots)</u>												
Lop Buri	6.5	6.4	7.4	7.1	5.9	6.2	5.5	5.2	4.7	5.3	6.8	6.5
Suphan Buri	6.1	6.2	7.2	7.5	7.2	7.8	7.9	7.4	6.6	6.6	7.2	6.6
Bangkok	4.9	5.6	6.6	6.3	5.1	5.4	5.0	5.1	4.5	4.1	4.0	3.9
Kanchana Buri	5.3	3.8	4.2	4.6	4.4	4.6	4.5	5.0	3.9	3.4	3.5	4.1
Mean (1)	5.63	6.07	7.07	6.97	6.07	6.47	6.13	5.90	5.27	5.33	6.00	5.67
Mean (2)	5.05	5.50	6.35	6.38	5.65	6.00	5.73	5.68	4.93	4.85	5.38	5.28
<u>Maximum Wind Speed and Direction (knots)</u>												
Lop Buri	45	52	45	50	40	44	40	40	40	33	45	45
Suphan Buri	30	30	33	45	35	30	34	30	35	35	30	24
Bangkok	31	37	48	56	42	43	43	45	44	40	45	31
Kanchana Buri	25	25	33	50	33	33	55	40	40	30	21	30
Annual Mean	36.13	37.08	44.99	44.11	36.05	35.57	35.28	35.57	35.28	35.57	35.28	35.57

Note: mean (1): mean value except Kanchana Buri  
 mean (2): mean value of four stations

Source: Meteorological Department, 20 years (1951-1970)

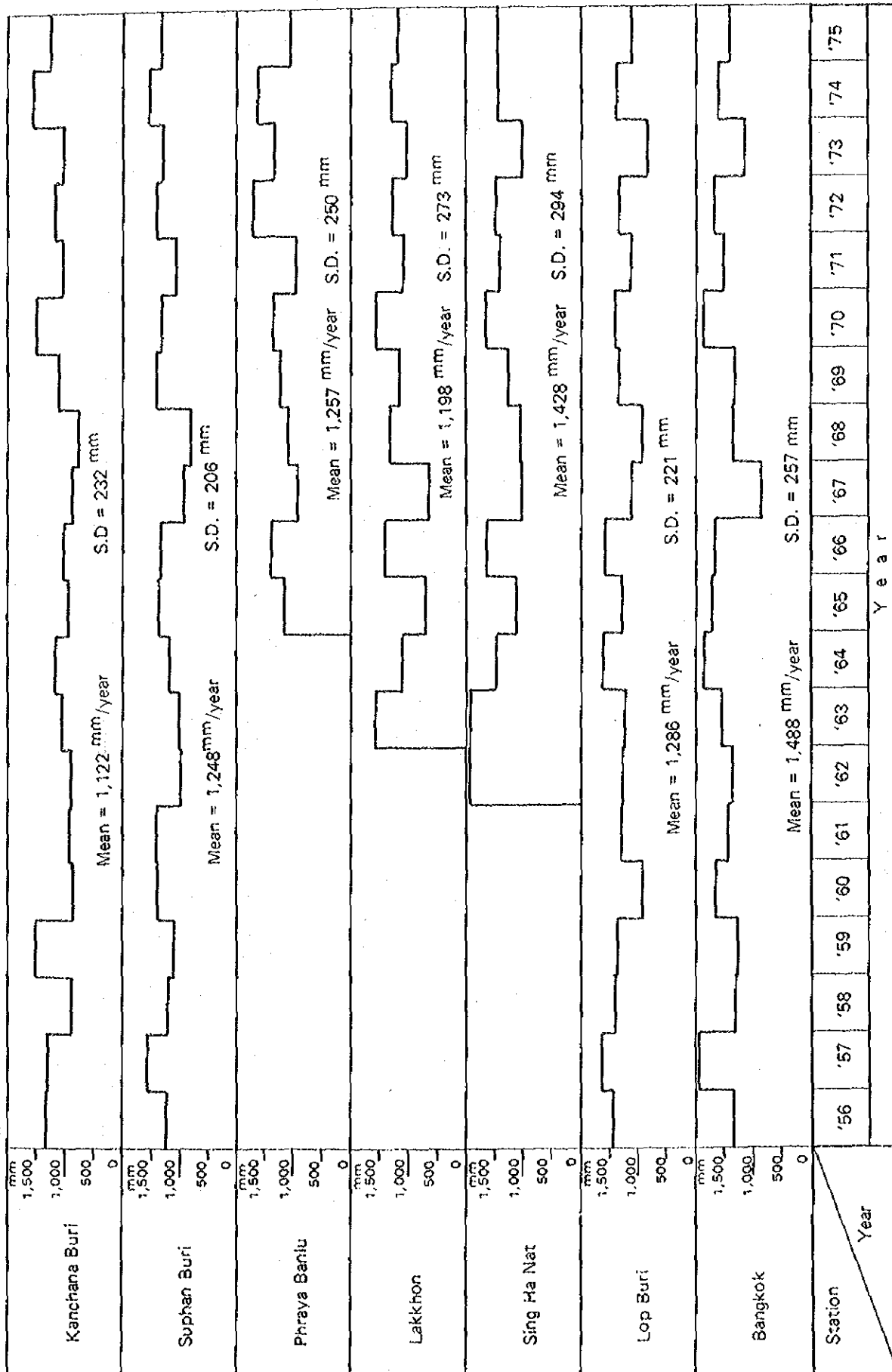
Table A. 3-4 Solar Radiation

(Unit: cal/cm<sup>2</sup>/day)

Year	Daily Solar Radiation												Annual Mean
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1965	426.73	435.52	472.32	536.27	424.67	342.80	420.74	395.01	354.24	372.77	460.45	437.50	423.25
1966	427.12	420.26	516.64	516.79	399.32	466.21	364.44	395.15	418.28	406.79	424.71	378.61	427.86
1967	411.38	449.68	473.71	487.17	433.72	448.25	414.21	355.94	360.04	390.35	398.37	436.68	421.63
1968	366.83	378.88	466.91	437.16	408.66	403.74	380.42	406.15	366.04	324.61	344.72	337.48	386.80
1969	382.46	423.03	-	477.33	406.40	415.29	335.23	355.93	359.26	391.91	426.03	429.06	-
1970	424.27	407.71	368.99	-	-	-	419.45	406.28	371.38	368.84	389.23	377.06	392.58
1971	403.60	364.80	431.86	430.31	425.12	392.70	494.72	402.57	367.57	393.14	447.96	380.94	411.27
1972	403.53	412.19	473.03	429.03	519.43	415.08	370.76	363.78	347.36	351.69	359.02	389.34	402.87
1975	402.27	452.82	421.99	488.01	428.12	417.39	357.03	347.09	312.47	410.15	417.75	448.10	409.43
1974	366.98	415.05	473.47	444.92	411.16	418.88	437.19	407.48	432.46	347.49	404.46	394.09	412.80
1975	340.83	449.23	474.05	451.88	389.71	376.85	431.68	397.88	366.59	370.92	458.85	403.13	409.30
Mean( $\bar{x}$ )	397.82	423.56	457.30	469.93	424.63	409.32	403.26	384.84	369.61	375.35	411.96	401.09	410.63
Standard Deviation(S)	25.52	30.00	38.20	35.38	34.22	32.72	42.58	22.58	30.81	25.20	36.08	32.07	12.06
S/ $\bar{x}$	0.064	0.071	0.084	0.075	0.081	0.080	0.106	0.059	0.083	0.067	0.088	0.080	0.030

Source: Meteorological Department, Station Bangkok, 11 years (1965-1975)

FIGURE A.3-1 ANNUAL RAINFALL DISTRIBUTION



Note : S.D. : Standard Deviation of Annual Rainfall



Table A. 3-5 Annual Rainfall

Year	Stations						
	Sing Ha Nat Regulator	Lakkhon Regulator	Phraya Banlu Regulator	Lop Buri	Suphan Buri	Bang Kapi (Bangkok)	Kanchana Buri
1956				1,465.6	1,267.4	1,362.6	1,357.0
1957				1,639.6	1,584.9	1,946.1	1,296.6
1958				1,391.8	1,217.5	1,307.8	871.0
1959				1,372.8	1,106.8	1,267.2	1,515.8
1960				920.3	1,388.0	1,646.3	867.6
1961				1,289.3	1,432.2	1,454.3	919.9
1962	1,941.5			1,284.3	984.9	1,376.8	901.8
1963	1,931.2	1,568.3		1,222.9	1,004.7	1,540.5	1,055.2
1964	1,486.7	1,118.7		1,552.6	1,182.6	1,866.8	1,179.0
1965	1,130.3	700.7	1,179.5	1,292.8	1,357.0	1,689.0	961.5
1966	1,636.2	1,422.2	1,387.4	1,598.1	1,332.7	1,668.1	1,031.6
1967	1,044.0	659.4	926.4	1,123.7	911.6	865.6	873.3
1968	1,036.1	1,331.7	1,010.8	930.5	800.2	1,361.4	780.2
1969	1,263.8	1,181.8	1,231.6	1,247.8	1,410.8	1,348.1	1,124.1
1970	1,663.6	1,577.7	1,366.1	1,427.8	1,336.2	1,855.4	1,500.4
1971	1,438.4	1,117.2	961.3	1,138.1	1,055.9	1,483.9	1,067.9
1972	1,501.8	1,304.3	1,714.1	1,553.9	1,395.6	1,652.3	1,192.2
1973	1,019.7	1,046.9	1,343.8	855.0	1,300.2	1,103.0	1,063.5
1974	1,450.4	1,332.9	1,639.2	1,390.7	1,552.6	1,579.8	1,581.7
1975	1,449.5	1,212.1	1,073.3	1,114.7	1,335.6	1,377.8	1,288.8
Mean ( $\bar{x}$ )	1,428.1	1,199.0	1,257.1	1,295.6	1,247.9	1,487.6	1,121.5
S	293.9	272.8	250.1	220.5	206.1	256.7	231.9
S/ $\bar{x}$	0.206	0.228	0.199	0.172	0.165	0.173	0.206

Note: S - Standard Deviation

Source: Sing Ha Nat, Phraya Banlu and Lakkhon; RID  
Lop Buri, Suphan Buri, Bangkapi and Kanchana Buri;  
Meteorological Department

Table A. 3-6 Probable Annual Rainfall

(Unit: mm)

Return Period (years)	Probable Maximum Annual Rainfall									
	Sing Ha Nat	Lakkhon	Phraya Banlu	Average of Three Stations	Lop Buri	Suphan Buri	Bangkok	Kanchana Buri	Average of Seven Stations	
2	1,398	1,162	1,233	1,264	1,266	1,229	1,464	1,098	1,264	
5	1,678	1,459	1,468	1,535	1,480	1,433	1,719	1,308	1,506	
10	1,846	1,641	1,607	1,698	1,606	1,552	1,868	1,434	1,651	
15	1,936	1,741	1,682	1,786	1,672	1,615	1,946	1,501	1,728	
20	1,997	1,809	1,733	1,846	1,717	1,658	2,002	1,546	1,780	
25	2,043	1,862	1,771	1,892	1,752	1,690	2,042	1,581	1,820	
30	2,081	1,904	1,802	1,929	1,779	1,716	2,075	1,608	1,852	
40	2,138	1,970	1,850	1,996	1,821	1,756	2,125	1,651	1,902	
50	2,182	2,020	1,885	2,029	1,852	1,786	2,163	1,683	1,939	

Return Period (years)	Probable Minimum Annual Rainfall									
	Sing Ha Nat	Lakkhon	Phraya Banlu	Average of Three Stations	Lop Buri	Suphan Buri	Bangkok	Kanchana Buri	Average of Seven Stations	
2	1,394	1,162	1,222	1,259	1,266	1,229	1,464	1,087	1,261	
5	1,166	927	1,035	1,042	1,083	1,055	1,247	920	1,061	
10	1,050	823	954	946	998	974	1,147	850	972	
15	1,013	776	918	902	958	936	1,100	818	931	
20	984	746	895	875	933	912	1,070	796	914	
25	962	725	879	855	914	894	1,049	784	887	
30	946	709	867	841	901	861	1,032	773	873	
40	922	686	849	819	880	861	1,008	756	852	
50	905	669	836	803	865	846	990	747	837	

Data Source: Table A. 3-8 to Table A. 3-14.

Table A. 3-7 Monthly Mean Rainfall

(Unit: mm)

Station	Monthly Mean Rainfall												Total
	Jan.	Feb.	Mar.	Apr.	May.	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
(1) Sing Ha Nat	7.2	11.9	26.9	63.3	152.0	153.0	165.9	213.4	347.2	245.6	36.1	16.8	1,439.3
(2) Lakkon	4.5	6.9	20.3	42.8	108.0	144.7	131.1	191.5	289.9	205.4	37.0	15.9	1,193.0
(3) Phraya Banlu	2.3	8.0	25.6	58.2	137.7	104.5	108.7	180.2	301.9	245.6	69.6	15.4	1,257.7
(4) Lop Buri	12.3	10.4	49.0	75.4	164.3	148.8	159.2	164.8	281.6	171.9	38.4	10.7	1,286.8
(5) Suphan Buri	5.9	9.4	32.1	83.2	159.6	127.8	112.4	150.5	316.5	204.3	32.3	14.2	1,246.2
(6) Bangkok	8.6	31.8	24.7	67.1	185.4	149.0	148.4	209.4	352.1	236.9	47.5	12.7	1,473.6
(7) Kanchana Buri	5.9	9.6	34.9	77.3	147.2	90.2	97.6	106.3	249.4	229.3	60.7	10.9	1,119.3
Mean (1)	4.7	8.9	24.3	54.8	132.6	134.1	135.2	195.0	313.0	232.2	47.6	16.0	1,298.4
Mean (2)	6.7	12.6	30.5	66.8	150.6	131.1	131.9	173.7	305.5	219.9	45.9	13.8	1,289.0

Note: mean (1): Average of Sing Ha Nat, Lakkon and Phraya Banlu  
mean (2): Average of seven stations

Source: (1)-(3) Royal Irrigation Department  
Sing Ha Nat : 14 years (1962-1975)  
Lakkon : 13 years (1963-1975)  
Phraya Banlu: 11 years (1965-1975)  
(4)-(7) Meteorological Department: 20 years (1956-1975)

Table A. 3-8 Monthly Rainfall Records in the Vicinity of Project Area

Station: Sing Ha Net Regulator

(Unit: mm)

Year	Monthly Rainfall												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1962	0	30.8	2.2	57.5	216.4	161.1	277.6	182.0	645.4	318.7	49.7	0	1,941.5
1963	0	0	9.5	2.0	78.4	213.8	161.1	123.3	531.8	646.8	54.5	0	1,931.2
1964	0	4.0	7.5	138.0	298.9	156.4	292.1	142.1	300.3	91.2	8.8	47.4	1,486.7
1965	0	15.3	107.3	18.4	148.2	68.1	38.2	170.5	313.5	211.4	19.7	19.7	1,130.3
1966	2.5	10.6	0	30.7	339.5	128.6	200.3	209.2	294.3	349.0	32.9	38.6	1,636.2
1967	43.4	0	12.8	40.5	54.8	171.6	166.2	62.9	280.3	171.9	39.6	0	1,044.0
1968	0	34.0	21.5	49.2	132.7	137.2	95.4	210.8	275.0	65.8	13.5	0	1,036.1
1969	7.3	0	29.0	18.0	106.1	121.6	154.3	278.6	258.1	229.2	61.6	0	1,263.8
1970	0	6.1	29.6	182.1	154.9	244.4	127.9	404.6	251.2	163.8	20.0	69.0	1,663.6
1971	0	23.3	49.9	58.5	175.5	167.8	79.0	294.1	309.2	284.1	0	2.0	1,438.4
1972	0	5.2	22.2	163.5	88.5	158.8	112.5	82.6	548.4	171.5	97.9	50.7	1,501.8
1973	0	0	61.3	45.0	66.0	66.5	144.8	182.2	275.1	133.2	41.4	4.2	1,019.7
1974	24.4	0	10.1	114.9	131.7	96.3	236.1	183.1	206.6	401.1	46.1	0	1,450.4
1975	30.7	0	36.4	8.0	129.5	182.5	179.2	252.8	405.8	200.6	20.0	4.0	1,449.5
1976	0	48.5	9.9	23.6	158.9	219.9	223.3	312.6	302.5	194.3	-	-	-
Mean (x)	7.2	11.9	27.3	63.3	152.0	153.0	165.9	213.4	347.2	242.2	36.1	16.8	1,436.3
S	13.4	14.9	27.2	56.0	78.0	50.2	69.1	85.1	122.9	140.2	24.6	23.2	-
S/x	1.86	1.25	0.997	0.885	0.513	0.328	0.416	0.399	0.354	0.579	0.682	1.380	-

Note: S - Standard Deviation

Source: Chao-cher Bangyeehon and Phraya Banlu Project Offices, RID

Table A. 3-9 Monthly Rainfall Record

Station: Lakkhon Regulator													
Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
(Unit: mm)													
1963	0.0	0.0	0.0	0.0	85.6	87.5	178.1	338.7	492.6	374.6	11.2	0.0	1,568.3
1964	0.0	0.0	0.0	101.9	298.0	162.4	103.3	120.7	241.4	39.0	0.0	52.0	1,118.7
1965	0.0	27.4	15.6	1.4	59.5	88.3	3.4	123.7	308.7	67.0	5.2	0.0	700.7
1966	0.0	60.4	3.6	0.0	237.1	191.0	239.3	183.2	174.9	217.3	57.7	57.7	1,422.2
1967	28.7	0.0	26.5	12.6	59.1	62.5	192.9	35.8	165.2	136.1	0.0	0.0	659.4
1968	0.0	1.8	7.4	112.1	139.5	196.9	105.5	203.0	317.5	158.8	88.2	0.0	1,331.7
1969	4.5	0.0	90.8	13.9	61.2	169.4	62.1	213.2	302.1	209.3	55.3	0.0	1,181.8
1970	0.0	0.0	18.4	63.4	108.2	385.8	133.3	369.2	228.5	233.6	18.4	18.9	1,577.7
1971	0.0	0.0	51.5	57.5	134.7	118.1	133.3	282.9	232.2	107.0	0.0	0.0	1,117.2
1972	0.0	0.0	43.2	97.6	33.0	110.2	72.3	90.9	500.3	159.8	118.8	78.2	1,304.3
1973	0.0	0.0	0.0	0.0	46.3	138.8	162.1	186.2	327.8	178.3	7.4	0.0	1,046.9
1974	0.0	0.0	0.0	96.3	46.8	86.6	175.1	117.4	177.1	573.4	59.2	0.0	1,332.9
1975	24.6	0.0	6.4	0.0	94.8	83.3	201.0	225.8	300.6	215.6	60.0	0.0	1,212.1
Mean( $\bar{x}$ )	4.5	6.9	20.3	42.8	108.0	144.7	131.1	191.6	289.9	205.4	37.0	15.9	1,198.0
S	9.58	17.1	26.0	44.4	76.1	81.4	60.6	93.2	103.4	133.4	37.3	26.6	272.8
S/x	2.15	2.48	1.28	1.04	0.70	0.56	0.46	0.49	0.36	0.65	1.01	1.67	0.23

Station: Phraya Banlu Regulator													
Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1965	0.0	74.4	0.0	68.5	131.3	34.0	89.6	112.7	385.0	122.0	120.2	41.8	1,179.5
1966	0.0	0.0	8.5	15.8	313.3	107.0	129.0	357.0	153.6	221.2	24.5	55.5	1,382.4
1967	0.0	0.0	0.0	5.6	112.0	127.6	98.6	74.3	364.0	136.2	8.1	0.0	926.4
1968	0.0	6.0	8.5	29.7	95.6	99.3	61.4	215.1	260.5	139.0	95.7	0.0	1,010.8
1969	5.2	0.0	41.4	5.5	49.4	120.3	135.0	205.2	345.7	189.7	134.2	0.0	1,231.6
1970	0.0	0.0	29.0	16.8	169.1	213.9	102.5	258.3	342.6	156.5	40.8	36.4	1,366.1
1971	0.0	0.0	20.5	126.7	14.1	41.9	67.9	215.9	215.5	258.8	0.0	0.0	961.3
1972	0.0	0.0	11.3	180.1	144.4	74.9	55.0	131.4	701.9	271.1	108.7	35.3	1,714.1
1973	0.0	3.5	159.9	0.0	174.0	129.4	254.0	109.5	210.5	303.0	0.0	0.0	1,343.8
1974	0.0	0.0	2.3	191.4	86.4	109.6	158.1	256.9	118.7	563.6	152.2	0.0	1,639.2
1975	20.1	3.7	0.0	0.0	224.9	91.2	44.3	45.7	221.2	341.0	81.2	0.0	1,073.3
Mean( $\bar{x}$ )	2.3	8.0	25.6	58.2	137.7	104.5	108.7	180.2	301.9	245.6	69.6	15.4	1,257.1
S	5.82	21.1	44.3	70.1	79.2	46.1	57.3	89.2	151.8	122.3	54.1	20.9	250.0
S/x	2.53	2.65	1.73	1.20	0.58	0.44	0.53	0.50	0.50	0.50	0.78	1.36	0.20

Table A. 3-10 Monthly Rainfall Record

Year	(Unit: mm)												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1956	0.0	14.2	63.8	95.0	257.5	192.1	185.0	198.2	343.6	98.9	27.3	0.0	1,465.6
1957	0.0	4.1	0.0	91.8	70.8	140.0	117.1	121.2	496.9	494.7	103.0	0.0	1,639.6
1958	7.8	0.0	271.2	91.8	112.5	214.5	166.7	121.9	317.1	88.3	0.0	0.0	1,391.8
1959	1.3	13.6	123.9	92.4	283.5	70.8	196.9	73.8	321.8	193.7	10.8	0.2	1,372.8
1960	0.0	40.0	25.4	8.5	46.0	113.7	159.3	78.9	120.5	272.6	55.0	0.4	920.3
1961	0.0	10.3	62.3	153.7	165.3	197.7	153.7	164.5	82.5	295.7	12.8	0.8	1,289.3
1962	0.0	0.0	2.1	47.5	289.1	107.0	317.0	167.0	234.8	115.9	1.3	2.0	1,284.3
1963	0.0	0.0	0.8	75.4	64.3	144.8	163.3	118.9	360.2	197.1	98.0	0.1	1,222.9
1964	2.6	31.0	3.9	182.1	352.8	113.7	193.7	176.4	319.4	245.1	9.4	32.5	1,652.6
1965	0.0	12.1	16.2	28.4	189.1	133.8	119.7	285.1	295.0	127.0	75.7	10.7	1,292.8
1966	137.5	1.1	12.0	90.9	340.2	159.7	183.6	281.7	150.5	159.8	13.1	68.0	1,598.1
1967	2.2	0.8	0.0	129.7	125.7	174.6	188.8	67.9	281.2	81.0	71.8	0.0	1,123.7
1968	0.0	20.4	0.3	45.5	182.1	100.8	112.7	130.1	239.4	71.5	27.2	0.5	930.5
1969	59.3	0.0	56.0	4.1	73.5	118.9	213.8	179.2	430.9	97.6	14.3	0.2	1,247.8
1970	0.0	14.8	139.3	70.7	132.8	288.5	160.5	234.6	168.5	158.0	11.0	49.1	1,427.8
1971	0.0	26.6	42.3	68.1	184.7	126.2	51.2	328.0	179.2	120.1	7.5	4.1	1,138.1
1972	0.0	1.1	21.1	91.8	4.9	179.7	98.3	125.4	516.5	172.4	108.8	33.9	1,353.9
1973	0.0	0.0	46.2	0.0	126.7	151.0	97.5	65.8	279.1	67.0	11.6	0.1	855.0
1974	0.0	0.0	67.6	108.7	181.4	155.3	142.7	138.7	297.1	252.4	44.9	2.7	1,390.7
1975	35.2	18.4	24.6	31.0	102.6	93.7	172.3	227.7	197.4	139.2	64.2	8.4	1,114.7
Mean ( $\bar{X}$ )	12.3	10.4	49.0	75.4	164.3	148.8	159.2	164.8	281.6	171.9	38.4	10.7	1,285.6
S	32.2	11.7	64.1	47.2	95.8	46.6	53.5	72.5	113.1	99.2	35.5	18.9	220.5
S/ $\bar{X}$	2.62	1.13	1.31	0.63	0.58	0.33	0.34	0.44	0.40	0.58	0.92	1.77	0.172

Note: S - Standard Deviation  
Source: Meteorological Department

Table A. 3-11 Monthly Rainfall Record

Station: Suphan Buri		(Unit: mm)											
Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1956	0.0	5.1	63.8	61.3	151.2	276.9	144.4	141.3	174.1	223.5	25.8	0.0	1,267.4
1957	0.0	0.0	56.6	89.7	66.0	159.7	102.5	282.8	474.6	287.8	65.2	0.0	1,584.9
1958	0.0	6.0	0.0	92.8	42.8	205.7	92.2	76.2	392.2	309.6	0.0	0.0	1,217.5
1959	0.0	11.8	9.0	41.4	102.4	61.2	133.1	130.4	450.1	125.2	40.3	1.9	1,106.8
1960	0.0	10.1	63.2	24.4	189.9	120.7	128.2	52.3	350.4	346.1	102.7	0.0	1,388.0
1961	24.1	0.0	24.2	194.0	323.8	85.2	184.9	199.6	149.5	239.5	4.4	3.0	1,432.2
1962	0.0	2.5	0.0	22.8	62.5	39.8	84.8	149.4	388.1	234.7	0.3	0.0	984.9
1963	0.0	0.0	66.1	24.4	85.4	137.3	55.5	96.0	279.1	241.8	16.6	2.5	1,004.7
1964	0.0	39.9	1.0	75.3	326.0	56.4	176.1	79.1	266.6	136.8	22.8	2.6	1,182.6
1965	0.0	39.7	0.0	127.1	253.6	83.4	60.8	141.2	442.4	101.6	82.8	24.4	1,357.0
1966	2.8	0.0	0.0	93.2	267.4	130.8	117.6	232.9	250.4	207.7	0.0	30.0	1,332.7
1967	0.9	0.0	4.8	134.8	180.8	55.0	134.9	37.1	227.1	109.8	6.4	0.0	911.6
1968	0.0	29.0	9.6	62.9	116.4	136.5	75.6	130.4	173.0	50.8	15.6	0.0	800.2
1969	0.0	39.4	37.8	126.2	74.2	100.6	197.8	186.5	487.5	95.9	63.9	0.0	1,410.8
1970	0.4	1.8	7.9	38.8	239.8	199.0	123.5	220.3	240.4	189.0	14.8	60.4	1,386.2
1971	3.0	1.4	104.0	138.9	93.8	91.9	54.4	185.4	259.2	221.6	0.6	10.0	1,055.9
1972	0.0	0.2	16.1	71.7	78.8	104.4	71.0	96.8	488.4	235.4	68.9	71.8	1,395.6
1973	0.0	0.0	55.2	5.8	196.3	180.6	127.8	160.2	409.8	154.7	8.4	1.4	1,300.2
1974	0.2	0.0	115.3	227.2	82.7	196.9	102.7	175.4	228.0	380.9	43.3	0.0	1,552.5
1975	86.1	0.0	7.4	10.6	258.3	123.4	80.7	237.4	198.3	193.1	62.4	76.2	1,335.6
Mean( $\bar{x}$ )	5.88	9.35	32.1	83.2	159.6	127.8	112.4	150.5	316.5	204.3	32.3	14.2	1,247.9
S	19.1	14.4	35.2	58.9	89.6	59.1	41.2	64.1	112.7	84.6	30.9	24.7	206.1
S/X	3.25	1.54	1.10	0.71	0.56	0.46	0.37	0.43	0.36	0.41	0.96	1.74	0.165

Note: S - Standard Deviation  
Source: Meteorological Department

Table A. 3-12 Monthly Rainfall Record

Station: Bang Kapi, Phra Na Khon (Bangkok)

Year	(Unit: mm)												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1955	2.2	0.4	0.0	83.7	121.0	162.0	130.0	193.8	365.2	189.0	115.0	0.0	1,362.6
1957	1.0	2.2	47.5	150.7	36.1	244.3	165.0	302.0	448.4	490.1	68.8	0.0	1,946.1
1958	35.5	36.6	3.2	2.7	36.9	175.3	168.0	316.0	278.7	228.7	12.3	0.0	1,307.8
1959	0.0	38.6	20.8	60.6	218.7	138.5	240.2	102.5	210.3	224.6	20.9	0.0	1,267.2
1960	0.0	0.0	34.4	1.8	112.0	79.4	127.4	243.9	499.7	443.2	94.4	10.1	1,645.3
1961	39.3	88.1	60.0	86.2	194.3	208.6	119.1	180.0	219.6	226.0	31.4	1.7	1,454.3
1962	0.0	20.0	43.5	36.4	179.3	117.9	125.5	223.5	464.6	164.6	1.5	0.0	1,376.8
1963	0.0	10.0	40.5	55.5	83.8	122.0	132.4	318.3	386.9	333.4	56.0	2.3	1,540.3
1964	4.6	107.4	15.0	45.9	554.1	100.2	279.6	243.9	403.2	100.2	10.0	2.7	1,866.3
1965	0.4	125.4	2.6	66.2	264.7	96.0	83.1	185.1	543.4	277.5	26.3	18.3	1,689.0
1966	0.0	35.2	1.3	72.0	380.8	214.6	314.7	157.4	257.4	191.3	4.2	39.2	1,668.1
1967	6.3	0.0	4.2	67.6	235.9	28.0	104.7	121.0	165.1	96.6	36.2	0.0	865.6
1968	4.6	51.2	0.4	124.7	124.4	180.1	114.7	269.7	293.7	166.4	31.5	0.0	1,361.4
1969	38.5	0.1	0.4	12.0	68.5	280.0	78.0	95.6	292.8	159.0	93.6	1.6	1,348.1
1970	1.2	68.2	44.6	157.5	283.0	307.7	152.8	144.3	355.2	187.3	50.4	103.2	1,855.4
1971	0.0	26.1	11.0	31.1	236.3	82.2	178.9	352.1	383.2	177.4	2.8	0.8	1,483.9
1972	0.0	19.9	18.8	146.9	55.1	127.8	69.5	157.0	676.3	237.9	88.4	54.7	1,552.3
1973	0.0	0.0	102.6	5.6	157.5	131.4	78.5	97.3	368.3	113.8	36.8	11.2	1,103.0
1974	0.7	0.0	10.2	130.2	179.2	82.3	130.5	160.9	219.7	470.2	134.8	0.0	1,579.3
1975	38.1	1.0	32.4	3.9	186.2	100.7	174.8	323.7	211.5	261.5	35.0	9.0	1,377.8
Mean ( $\bar{x}$ )	8.62	31.8	24.7	67.1	185.4	149.0	148.4	209.4	352.1	236.9	47.5	12.7	1,487.6
S	14.7	37.5	25.7	50.8	121.6	70.6	63.9	80.6	126.9	112.5	36.4	25.1	256.7
S/ $\bar{x}$	1.71	1.18	1.04	0.76	0.66	0.47	0.43	0.38	0.35	0.47	0.81	1.98	0.173

Note: S - Standard Deviation

Source: Meteorological Department



Table A. 3-13 Monthly Rainfall Record

Station:	Kanchana Buri												
Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1956	0.0	0.0	12.0	55.0	149.8	149.4	95.4	186.1	341.7	294.2	73.4	0.0	1,357.0
1957	0.0	0.0	0.0	88.5	5.2	85.6	161.8	152.6	437.4	349.5	15.8	0.0	1,296.6
1958	0.0	46.1	56.4	81.8	80.8	125.1	63.7	99.9	193.1	102.8	21.3	0.0	871.0
1959	3.0	9.0	94.2	43.8	101.3	50.7	166.6	31.8	478.5	505.2	26.8	4.9	1,515.8
1960	10.6	0.2	6.9	23.6	173.3	60.9	86.1	24.2	126.4	255.1	82.0	8.3	857.6
1961	2.4	0.0	10.9	115.2	221.4	144.4	82.7	121.5	55.4	147.4	17.6	0.0	919.9
1962	0.0	0.0	17.4	88.3	101.7	79.0	32.1	121.6	266.2	193.2	2.1	0.2	901.8
1963	0.0	0.0	0.0	6.8	180.5	58.7	89.6	117.3	298.5	204.9	102.1	5.8	1,055.2
1964	0.0	12.1	0.0	119.1	176.8	45.2	192.7	29.0	201.7	182.0	218.7	1.7	1,179.0
1965	17.2	1.5	61.7	25.6	145.6	51.6	38.3	151.7	264.2	144.5	47.5	12.1	961.5
1966	0.0	3.2	2.5	28.5	283.1	100.1	67.1	107.7	145.2	216.5	26.6	51.1	1,031.6
1967	0.0	0.0	0.0	133.8	159.1	32.8	101.9	42.7	90.4	264.4	48.2	0.0	873.3
1968	0.0	2.7	0.0	110.5	115.0	142.3	73.4	9.2	143.7	161.9	20.9	0.5	780.2
1969	3.7	2.0	30.5	125.7	200.1	72.9	92.9	125.3	217.2	121.7	131.5	0.6	1,124.1
1970	0.0	89.6	20.4	103.7	132.4	107.7	90.7	179.4	332.0	265.1	103.7	75.7	1,500.4
1971	0.0	24.3	17.2	107.9	133.5	75.7	88.9	147.1	285.0	178.1	6.2	4.0	1,067.9
1972	0.0	0.0	44.3	115.0	58.2	179.3	64.9	109.9	413.6	89.2	99.9	17.9	1,192.2
1973	1.5	2.0	55.4	6.1	190.0	73.7	84.5	85.7	218.9	238.5	107.2	0.0	1,063.5
1974	1.5	0.0	213.0	146.4	133.0	64.8	151.8	198.6	265.1	366.6	28.9	12.0	1,581.7
1975	77.9	0.0	56.1	59.8	204.1	104.2	126.3	84.6	222.0	295.6	34.3	23.9	1,288.8
Mean ( $\bar{x}$ )	5.89	9.64	34.9	77.3	147.2	90.2	97.6	106.3	249.4	229.3	60.7	10.9	1,121.5
S	17.1	21.4	48.6	43.5	60.4	39.2	41.2	54.7	110.0	98.6	52.7	19.1	231.9
S/ $\bar{x}$	2.90	2.22	1.39	0.55	0.41	0.43	0.42	0.51	0.44	0.43	0.87	1.75	0.206

Note: S - Standard Deviation  
Source: Meteorological Department

Table A. 3-14 Rainfall Records in Wet Season (May - October)

Year	Rainfall in Wet Season		(Unit: mm)
	Sing Ha Nat	Lakkhon	
1962	1,801.2	1,525.8	874.6
1963	1,865.2	1,552.2	1,283.1
1964	1,281.0	964.8	912.5
1965	949.9	650.6	870.9
1966	1,520.9	1,242.8	1,045.3
1967	907.7	591.6	1,243.1
1968	917.9	1,122.2	814.1
1969	1,147.9	1,017.3	1,378.7
1970	1,356.8	1,458.6	1,180.4
1971	1,309.7	1,008.2	1,293.3
1972	1,162.3	966.5	968.3
1973	867.8	1,039.5	1,250.8
1974	1,254.9	1,177.4	1,092.9
1975	1,350.4	1,121.1	191.1
1976	1,411.5	795.5	0.175
Mean ( $\bar{x}$ )	1,273.7	1,082.3	
S.D. (S)	292.2	276.3	
Variation (S/ $\bar{x}$ )	0.229	0.255	

Note: S.D. - Standard Deviation

Table A. 3-15 Probable Rainfall in Wet Season (May - October)

(Unit: mm)

Return Period (years)	Probable Maximum Rainfall			Average
	Sing Ha Nat	Lakkhon	Phraya Banlu	
2	1,227	1,045	1,076	1,116
5	1,512	1,324	1,259	1,365
10	1,697	1,498	1,367	1,521
15	1,800	1,593	1,424	1,606
20	1,871	1,658	1,463	1,664
25	1,927	1,709	1,492	1,709
30	1,971	1,749	1,516	1,745
40	2,042	1,812	1,552	1,802
50	2,096	1,860	1,579	1,845

Return Period (years)	Probable Minimum Rainfall			Average
	Sing Ha Nat	Lakkhon	Phraya Banlu	
2	1,227	1,071	1,083	1,105
5	1,013	836	922	913
10	923	720	843	833
15	883	664	805	797
20	859	628	781	775
25	841	601	761	734
30	828	581	747	718
40	808	550	726	695
50	795	527	711	678

Note: Data Source: Table A. 3-14

Table A. 3-16 Rainfall Records in Dry Season (November-March & November-April)

Year	Stations													
	Sing Ha Nat		Lakkhon		Phraya Banlu		Lop Buri		Suphan Buri		Bang Kapi			
	Regulator	Nov-Mar	Regulator	Nov-Mar	Regulator	Nov-Mar	Nov-Apr	Nov-Mar	Nov-Apr	Nov-Mar	Nov-Apr	Nov-Mar	Nov-Apr	
1952	201.1	201.1					137.0	232.0	126.5	187.8	49.5	133.2	104.0	159.1
1953	0.0	14.3					31.4	123.2	82.4	172.1	165.7	316.4	73.4	161.9
1954	40.8	139.0					382.4	473.8	71.2	164.0	147.1	149.8	118.3	200.1
1955	39.9	136.5					138.8	231.2	20.8	62.2	71.7	132.3	128.0	171.8
1956	0.0	31.7					76.4	84.9	115.5	139.9	55.3	57.1	49.4	73.0
1957	79.7	-					128.0	281.7	151.0	345.0	219.9	378.1	103.6	219.8
1958	-	-					15.7	53.2	9.9	32.7	96.6	133.0	35.0	123.3
1959	92.7	112.9		8.4			4.1	79.3	56.4	90.8	52.0	107.5	2.3	9.4
1960	116.6	279.4		11.2	113.1		135.6	317.7	60.0	135.3	185.3	231.2	120.0	239.1
1961	144.5	202.1		95.0	96.4		70.2	98.6	55.1	192.2	141.1	207.3	300.8	326.4
1962	59.2	61.2		69.2	69.2		237.0	327.9	110.0	203.2	81.1	153.1	55.3	93.8
1963	60.0	198.0		170.6	183.2		84.1	213.8	35.7	170.5	53.9	121.5	77.7	211.5
1964	178.8	197.2		9.2	121.3		92.5	138.0	45.4	108.3	92.4	217.1	50.9	161.4
1965	52.5	83.2		183.5	197.4		143.0	147.1	92.8	219.0	70.5	82.5	57.7	183.4
1966	127.7	168.2		73.7	137.1		168.6	239.3	74.0	112.8	209.2	366.7	242.1	345.8
1967	95.1	144.3		86.8	146.3		129.0	197.1	183.6	322.5	192.7	223.8	220.9	328.9
1968	49.8	67.8		43.2	140.8		33.9	125.7	26.9	98.6	42.3	189.2	54.5	159.5
1969	97.3	279.4		197.0	307.4		188.9	188.9	195.9	201.7	245.7	251.3	176.7	182.8
1970	157.2	215.7		7.4	103.7		79.3	188.0	125.3	352.5	58.9	189.1	321.7	456.1
1971	29.4	192.9		90.2	90.2		125.8	156.8	136.8	147.4	206.3	210.2	174.9	234.7
1972	209.9	217.4		102.9	146.6									
1973	80.1	195.0		82.2	125.1									
1974	113.2	121.2		117.9	172.0		120.1	195.4	89.8	172.9	125.4	192.5	123.9	203.2
1975	82.4	106.0		62.0	49.6		83.3	98.0	51.2	84.8	37.7	84.9	86.9	201.3
Mean( $\bar{x}$ )	91.7	155.2		0.764	0.397		0.694	0.502	0.570	0.491	0.587	0.441	0.701	0.299
S.D.(S)	57.0	71.7		0.703	0.356		0.694	0.502	0.570	0.491	0.587	0.441	0.701	0.299
S/ $\bar{x}$	0.622	0.462		0.703	0.356		0.694	0.502	0.570	0.491	0.587	0.441	0.701	0.299

Table A. 3-17 Probable Maximum Rainfall in Dry Season

(Unit: mm)

Return Period (years)	Probable Maximum Rainfall (November - March)												
	Sing Ha		Phraya		Average of Three Stations		Lop Buri		Suphan		Kanchana		Average of Seven Stations
	Mat	Lakkhon	Banlu	Banlu					Buri	Buri	Buri	Buri	
2	83	*	*	*	*	83	108	82	102	111	97		
5	143	*	*	*	*	143	197	137	176	203	171		
10	182	*	*	*	*	182	255	173	236	265	222		
15	205	*	*	*	*	205	289	193	273	300	252		
20	220	*	*	*	*	220	312	208	301	324	273		
25	232	*	*	*	*	232	330	219	323	343	298		
30	242	*	*	*	*	242	344	228	342	358	303		
40	258	*	*	*	*	258	367	242	373	382	324		
50	269	*	*	*	*	269	385	253	398	400	341		

Return Period (years)	Probable Maximum Rainfall (November - April)												
	Sing Ha		Phraya		Average of Three Stations		Lop Buri		Suphan		Kanchana		Average of Seven Stations
	Mat	Lakkhon	Banlu	Banlu					Buri	Buri	Buri	Buri	
2	145	*	*	159	176	152	176	160	178	188	168		
5	220	*	*	233	267	227	267	251	259	296	254		
10	270	*	*	285	330	278	330	311	312	368	313		
15	298	*	*	315	367	307	367	345	342	408	346		
20	318	*	*	337	392	328	392	368	363	436	369		
25	333	*	*	354	412	344	412	386	379	458	387		
30	346	*	*	367	429	357	429	401	392	476	401		
40	365	*	*	389	455	377	455	425	413	503	425		
50	380	*	*	406	475	393	475	443	429	525	443		

Note: Data Source; Table A. 3-16  
\* ; Data distribution on probability analysis sheet is not normal.

Table A. 3-18 Probable Minimum Rainfall in Dry Season

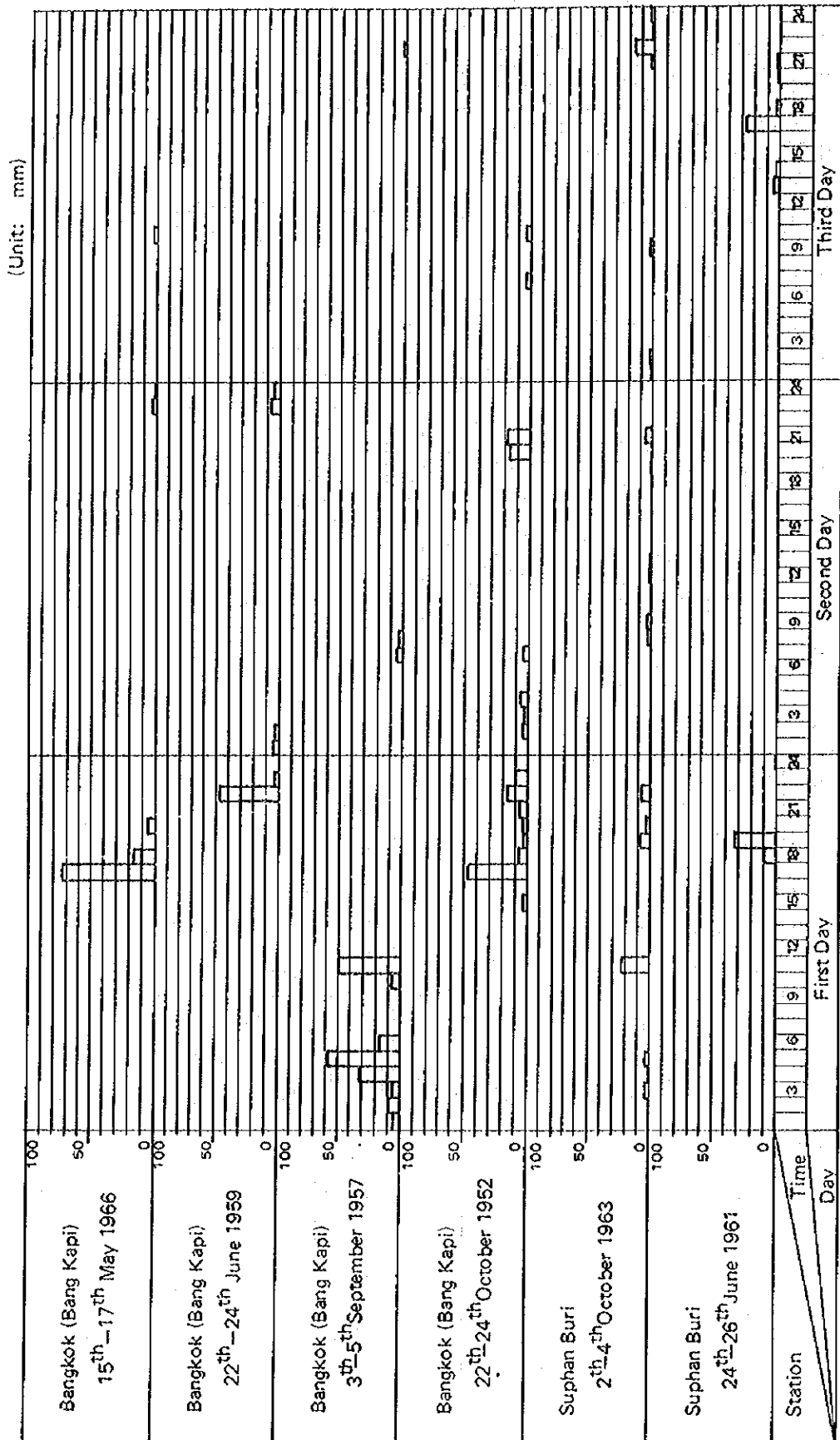
(Unit: mm)

Return Period (years)	Probable Minimum Rainfall (November - March)											
	Sing Ha Nat	Phraya Banlu		Average of Three Stations		Lop Buri		Suphan Buri		Kanchana Buri		Average of Seven Stations
		Lakkhon	Banlu	Phraya	Average of Three Stations	Lop Buri	Suphan Buri	Bangkok	Kanchana Buri			
2	75.1	57.5	89.2	73.9	99.7	79.1	102.4	97.3	85.8			
5	35.4	21.5	34.9	30.6	48.5	43.0	61.9	46.8	41.7			
10	21.6	11.1	17.8	16.8	29.9	28.8	48.5	30.2	26.8			
15	16.0	7.2	11.7	11.6	22.2	22.6	43.3	23.7	21.0			
20	12.8	5.1	7.4	8.4	17.7	18.9	40.3	20.0	17.5			
25	10.6	3.7	4.9	6.4	14.5	16.4	38.2	17.5	15.1			
30	8.9	2.7	3.1	4.9	12.2	14.4	36.7	15.6	13.4			
40	6.6	1.3	0.5	2.8	8.9	11.6	34.6	13.0	10.9			
50	5.0	0.3	0	1.8	6.6	9.6	33.2	11.2	9.4			

Return Period (years)	Probable Minimum Rainfall (November - April)											
	Sing Ha Nat	Phraya Banlu		Average of Three Stations		Lop Buri		Suphan Buri		Kanchana Buri		Average of Seven Stations
		Lakkhon	Banlu	Phraya	Average of Three Stations	Lop Buri	Suphan Buri	Bangkok	Kanchana Buri			
2	148.8	105.5	167.0	140.4	176.0	158.9	178.2	185.9	160.0			
5	89.6	54.3	116.1	86.7	112.7	98.9	118.6	110.8	100.1			
10	62.1	38.4	92.2	64.2	87.6	74.0	93.9	79.8	75.5			
15	49.2	32.3	80.9	54.1	77.1	62.9	83.6	66.1	64.6			
20	41.0	28.8	73.8	47.9	70.6	56.1	76.3	57.7	57.8			
25	34.9	26.5	68.5	43.3	66.0	51.3	71.5	51.6	52.9			
30	30.4	24.8	64.5	39.9	62.6	47.7	68.0	47.3	49.3			
40	23.6	22.5	58.5	34.9	57.7	42.4	62.7	40.6	44.0			
50	18.7	20.9	54.2	31.3	54.2	38.6	59.0	36.1	40.2			

Note: Data Source; Table A. 3-16

FIGURE A.32 HOURLY HYETOGRAPH OF OBSERVED RAINFALLS



Source: Meteorological Department

Table A. 3-19 Probable Maximum Consecutive Rainfall

(unit: mm)

Station	Return Period						
	2 years	5 years	10 years	15 years	20 years	30 years	50 years
<u>Maximum 1 day rainfall</u>							
Lop Buri	( 85) 84	(111) 111	128	138	(144) 145	155	167
Suphan Buri	( 92) 89	(115) 109	121	127	(141) 132	138	146
Kanchana Buri	( 90) 89	(121) 120	141	153	(159) 161	173	188
Bang Kapi (Bangkok)	( 92) 92	(115) 115	129	136	(141) 141	148	156
Sing Ha Nat	92	123	146	160	169	183	201
Phraya Banlu	90	119	143	159	172	191	216
Lakkhon	88	113	128	136	142	150	160
Average	<u>89</u>	<u>116</u>	<u>134</u>	<u>144</u>	<u>152</u>	<u>163</u>	<u>176</u>
<u>Maximum 2 day rainfall</u>							
Lop Buri	( 98) 98	(127) 126	147	159	(172) 169	182	199
Suphan Buri	(114) 111	(135) 135	150	158	(156) 163	171	180
Kanchana Buri	(113) 112	(152) 151	177	192	(202) 202	217	236
Bang Kapi (Bangkok)	(117) 115	(153) 151	177	192	(199) 203	219	239
Sing Ha Nat	124	164	189	203	213	227	224
Phraya Banlu	126	167	196	213	225	242	264
Lakkhon	116	155	181	195	204	218	235
Average	<u>115</u>	<u>150</u>	<u>174</u>	<u>187</u>	<u>197</u>	<u>211</u>	<u>228</u>
<u>Maximum 3 day rainfall</u>							
Lop Buri	(113) 112	(147) 146	171	185	(196) 196	211	230
Suphan Buri	(129) 127	(165) 164	189	203	(209) 213	228	245
Kanchana Buri	(134) 132	(181) 179	209	226	(236) 238	254	274
Bang Kapi (Bangkok)	(140) 140	(186) 185	215	232	(248) 244	261	273
Sing Ha Nat	141	192	225	244	257	275	298
Phraya Banlu	147	196	228	246	259	277	299
Lakkhon	134	181	210	225	236	251	270
Average	<u>133</u>	<u>178</u>	<u>207</u>	<u>223</u>	<u>235</u>	<u>251</u>	<u>270</u>

- Note: 1. Observation Period  
 1952-1975 at four stations  
 Lop Buri, Suphan Buri, Kanchana Buri and  
 Bang Kapi  
 1962-1976 at Sing Ha Nat and Lakkhon  
 1962-1976 (except 1963) at Phraya Banlu

2. Numbers indicated in ( ) are estimated by Hazen  
 Method in Royal Irrigation Department.



Table A. 3-20 Monthly Mean Rainy Days

(Unit: day)

Station	Monthly Mean Rainy Days												Annual Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
(1) Sing Ha Nat	0.7	1.1	1.5	4.6	10.5	11.1	13.1	14.7	16.3	11.5	3.6	1.4	90.1
(2) Lakkhon	0.2	0.4	0.9	2.2	6.9	8.3	8.3	10.0	13.2	9.4	2.3	0.6	62.7
(3) Phraya Banlu	0.5	0.5	1.1	3.0	8.4	8.4	8.1	10.3	14.9	10.9	3.4	1.0	70.5
(4) Iop Buri	0.9	1.7	3.0	6.6	13.0	14.2	16.4	17.0	18.7	13.3	4.3	1.6	110.7
(5) Suphan Buri	0.7	1.2	2.2	5.7	12.4	11.9	14.2	16.0	19.2	13.6	3.9	1.3	102.3
(6) Bangkok	1.5	2.9	3.5	6.8	16.4	16.2	18.5	20.9	21.6	17.4	5.5	1.8	133.0
(7) Kanchana Buri	0.9	1.2	2.7	6.5	13.5	12.9	15.5	17.3	17.9	15.2	5.2	2.0	110.8
Mean (1)	0.5	0.7	1.2	3.3	8.7	9.3	9.8	11.7	14.8	10.6	3.1	1.0	74.7
Mean (2)	0.8	1.3	2.1	5.1	11.6	11.9	13.4	15.2	17.4	13.0	4.0	1.4	97.2

Note: Mean (1): Average of Sing Ha Nat, Lakkhon and Phraya Banlu  
Mean (2): Average of Seven Stations

Source: (1)-(3) Royal Irrigation Department  
Sing Ha Nat : 14 years (1962 - 1975)  
Lakkhon : 13 years (1963 - 1975)  
Phraya Banlu: 11 years (1965 - 1975)  
(4)-(7) Meteorological Department: 20 years (1956 - 1975)

Table A. 3-21 Monthly Rainy Days in the Vicinity of Project Area

Year	Monthly Rainy Days												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1962	0	1	1	5	11	6	14	15	21	11	2	0	87
1963	0	0	1	1	4	11	10	15	20	16	3	0	95
1964	0	1	1	6	18	14	16	13	11	7	2	1	90
1965	0	2	3	4	12	11	8	11	22	12	3	1	89
1966	1	1	0	4	17	13	18	19	14	12	4	4	107
1967	1	0	1	9	8	9	18	12	17	12	3	0	90
1968	0	4	1	12	10	7	11	13	11	6	3	0	78
1969	2	0	1	2	3	14	13	18	14	12	4	0	83
1970	0	2	2	4	13	14	13	14	18	14	1	6	101
1971	0	2	1	4	10	13	8	20	14	11	0	1	84
1972	0	1	2	6	3	12	8	9	17	9	10	3	79
1973	0	0	3	1	20	14	15	16	18	7	5	1	90
1974	1	0	1	7	11	7	14	12	15	19	6	0	93
1975	5	0	2	1	11	13	12	15	18	13	5	2	97
1976	0	2	2	3	16	9	18	19	15	12			
Year ( $\bar{x}$ )	0.667	1.07	1.47	4.60	10.5	11.1	13.1	14.7	16.3	11.5	3.60	1.36	90.3
S	1.30	1.12	0.806	2.98	4.47	2.75	3.45	3.24	3.20	3.30	2.35	1.76	7.85
S/x	1.948	1.05	0.548	0.649	0.426	0.248	0.264	0.220	0.196	0.287	0.652	1.29	0.087

Note: S - Standard Deviation

Source: Sing Ha Nat and Phraya Banlu Project Offices, RID

Table A. 3-22 Monthly Rainy Days in the Vicinity of Project Area

Station: Lakkhon Regulator		(Unit: days)											
Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1963	0	0	0	0	2	8	9	13	18	15	1	0	66
1964	0	0	0	2	13	10	10	10	10	5	0	1	61
1965	0	1	2	1	9	6	1	8	16	6	1	0	51
1966	0	3	1	0	10	8	12	11	9	11	4	3	72
1967	1	0	1	1	8	6	13	5	13	9	0	0	56
1968	0	1	1	5	10	5	7	10	12	9	3	0	63
1969	1	0	2	1	3	9	7	13	11	8	4	0	59
1970	0	0	1	3	9	13	11	11	8	12	1	2	71
1971	0	0	2	4	6	5	7	16	16	6	0	0	62
1972	0	0	1	6	1	9	4	5	14	10	6	2	58
1973	0	0	0	0	6	11	7	9	13	7	1	0	54
1974	0	0	0	6	3	7	11	8	18	17	6	0	76
1975	1	0	1	0	9	9	9	11	13	7	3	0	63
Mean( $\bar{x}$ )	0.231	0.385	0.923	2.23	6.85	8.25	8.31	10.0	13.2	9.38	2.31	0.615	62.5
S	0.421	0.536	0.730	2.22	3.53	2.26	3.20	2.99	3.08	3.45	2.09	1.00	7.01
S/ $\bar{x}$	1.82	2.17	0.79	1.00	0.52	0.28	0.39	0.30	0.23	0.37	0.90	1.63	0.11

Station: Huwaya Banlu Regulator		(Unit: days)											
Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1965	0	2	0	5	12	10	8	10	16	9	2	3	77
1966	0	0	1	1	13	4	8	17	12	10	3	2	71
1967	0	0	0	1	10	4	10	6	16	7	2	0	56
1968	0	2	1	4	8	7	5	8	12	6	3	0	56
1969	1	0	1	1	6	8	8	12	12	11	3	0	64
1970	0	0	1	2	9	15	10	11	15	10	1	2	78
1971	0	0	1	3	2	4	6	14	14	11	0	0	57
1972	0	0	0	3	4	10	8	7	19	14	10	2	84
1973	0	1	0	0	10	13	11	10	16	7	0	0	71
1974	0	0	1	6	7	9	10	13	16	18	10	0	89
1975	3	1	0	0	11	9	5	5	16	17	3	0	72
Mean( $\bar{x}$ )	0.545	0.545	1.09	3.00	6.36	8.36	8.09	10.3	14.9	10.9	3.66	1.00	70.5
S	1.41	0.782	0.900	2.59	3.23	3.44	1.98	3.47	2.11	3.78	3.31	1.41	10.7
S/ $\bar{x}$	2.59	1.43	0.83	0.86	0.51	0.41	0.24	0.34	0.14	0.35	0.91	1.41	0.15

Table A. 3-23 Monthly Rainy Days Record

Station: Lop Buri

(Unit: days)

Year	Monthly Rainy Days												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1956	0	3	4	13	15	13	17	16	19	8	2	0	110
1957	0	1	0	4	2	12	16	15	19	17	4	0	90
1958	1	0	6	7	11	16	13	21	16	9	0	0	100
1959	1	3	5	9	17	12	17	13	24	13	4	1	119
1960	0	1	3	2	12	11	13	16	18	14	9	1	100
1961	0	4	5	9	16	16	19	20	16	23	2	1	131
1962	0	0	2	9	15	16	20	21	21	8	2	2	116
1963	0	0	3	4	8	19	16	18	21	16	11	1	117
1964	2	3	1	7	21	11	12	18	20	18	3	2	118
1965	0	2	1	7	17	16	19	20	19	13	8	2	124
1966	3	2	1	6	22	9	17	21	18	16	4	4	123
1967	1	1	0	9	13	13	22	14	20	13	3	0	109
1968	0	4	2	7	12	18	20	15	19	10	1	1	109
1969	2	0	3	3	8	15	22	14	18	13	3	1	102
1970	0	3	6	6	14	20	18	19	17	10	2	6	121
1971	0	3	2	7	13	17	14	19	16	11	2	1	105
1972	0	2	3	9	3	14	13	16	18	13	12	5	108
1973	0	0	4	0	13	12	14	13	20	8	4	1	89
1974	0	0	5	10	14	8	17	14	20	18	5	1	112
1975	8	1	3	3	14	15	8	17	15	14	4	2	104
Mean( $\bar{x}$ )	0.900	1.650	2.950	6.550	13.000	14.150	16.350	17.000	18.700	13.250	4.250	1.600	110.350
S	1.841	1.388	1.802	3.057	4.868	3.135	3.482	2.702	2.076	3.858	3.176	1.594	10.795
S/ $\bar{x}$	2.046	0.841	0.611	0.467	0.374	0.222	0.213	0.159	0.111	0.291	0.747	0.996	0.098

Note: S - Standard Deviation

A rainy day is one with an amount of rainfall of 0.1 mm or more.

Source: Meteorological Department

Table A. 3-24 Monthly Rainy Days Record

Year	Monthly Rainy Days												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1956	0	2	4	7	17	12	17	16	16	8	2	0	101
1957	0	0	4	5	4	14	9	22	24	18	6	0	106
1958	0	2	0	5	9	11	12	15	18	11	0	0	83
1959	0	2	1	6	12	7	13	12	20	12	2	1	88
1960	0	1	6	1	9	9	11	9	21	15	11	0	93
1961	1	0	3	6	17	9	15	18	10	18	1	1	99
1962	0	1	0	8	12	11	11	12	22	12	1	0	90
1963	0	0	3	2	6	14	13	17	19	17	5	1	97
1964	0	2	1	5	22	8	16	16	14	12	7	1	104
1965	0	4	0	6	13	15	14	18	21	12	3	3	109
1966	2	0	0	6	21	11	13	18	17	11	0	4	103
1967	1	0	1	7	13	10	15	16	23	13	1	0	100
1968	1	5	1	10	10	9	13	15	19	9	3	0	95
1969	0	1	3	7	7	12	18	12	22	13	4	0	99
1970	1	1	2	7	16	19	16	22	21	16	2	5	132
1971	1	2	1	8	14	12	15	17	17	13	2	3	105
1972	0	1	4	6	3	11	15	14	19	15	10	3	101
1973	0	0	3	2	13	16	15	16	22	7	5	1	100
1974	1	0	4	8	13	15	21	18	22	21	6	0	129
1975	6	0	2	2	15	12	10	16	17	18	7	3	108
Mean( $\bar{x}$ )	0.700	1.200	2.150	5.700	12.400	11.850	14.200	15.950	19.200	13.550	3.900	1.300	102.100
S	1.345	1.364	1.682	2.304	5.044	2.886	2.874	3.105	3.295	3.570	3.081	1.552	11.463
S/ $\bar{x}$	1.921	1.137	0.782	0.404	0.407	0.244	0.202	0.195	0.172	0.263	0.790	1.194	0.112

(Unit: days)

Station: Suphan Buri

Note: S - Standard Deviation

A rainy day is one with an amount of rainfall of 0.1 mm or more.

Source: Meteorological Department

Table A. 3-25 Monthly Rainy Days Record

(Unit: days)

Year	Monthly Rainy Days												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1956	2	1	0	14	20	17	16	23	25	13	4	0	135
1957	2	2	7	7	8	17	16	25	25	18	6	0	133
1958	4	5	3	4	10	16	20	22	20	20	1	0	125
1959	0	2	4	3	14	11	23	18	21	19	3	0	118
1960	0	0	4	1	13	13	16	21	26	17	10	2	123
1961	1	8	8	6	18	17	18	21	17	16	3	1	134
1962	0	2	3	7	12	14	19	20	25	14	5	0	121
1963	0	3	1	2	8	18	19	23	22	23	11	1	131
1964	3	3	2	9	25	15	19	25	22	19	7	2	151
1965	1	7	2	5	20	15	15	22	22	22	3	3	143
1966	0	3	1	4	22	22	22	24	20	15	3	5	141
1967	3	0	2	12	21	9	19	18	20	13	5	0	122
1968	2	8	2	10	11	17	19	15	16	16	7	0	129
1969	2	1	2	4	18	19	19	17	20	15	4	1	122
1970	3	3	4	10	17	19	23	23	21	15	2	10	150
1971	0	5	4	5	22	16	18	24	20	15	1	2	132
1972	0	2	4	15	17	20	19	17	21	20	16	5	156
1973	0	0	9	2	18	16	14	20	25	15	5	1	125
1974	1	0	4	13	17	13	18	22	19	23	9	0	139
1975	6	2	3	2	17	19	18	18	19	20	4	2	130
Year(X)	1.500	2.850	3.450	6.750	16.400	16.150	18.500	20.900	21.600	17.400	5.450	1.750	132.700
S	1.628	2.475	2.247	4.229	4.695	3.038	2.365	2.844	3.056	3.105	3.626	2.426	10.720
S/X	1.085	0.868	0.651	0.627	0.286	0.188	0.126	0.136	0.141	0.178	0.665	1.386	0.081

Note: S - Standard Deviation

A rainy day is one with an amount of rainfall of 0.1 mm or more

Source: Meteorological Department

Table A. 3-26 Monthly Rainy Days Record

Year	Monthly Rainy Days												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1956	0	0	3	7	19	13	10	20	21	14	4	0	111
1957	0	0	0	5	5	12	17	19	23	17	2	0	100
1958	0	2	5	3	13	13	18	16	21	19	1	0	111
1959	1	2	7	6	12	13	20	13	21	17	6	2	120
1960	1	1	3	2	15	13	9	16	16	14	7	2	99
1961	1	0	2	5	16	10	16	15	11	15	2	0	93
1962	0	0	1	6	7	13	13	14	12	10	1	1	78
1963	0	0	0	1	6	11	21	18	17	20	9	2	105
1964	0	2	0	5	22	12	19	17	18	15	9	2	121
1965	2	1	5	7	18	15	12	24	19	10	8	6	127
1966	0	3	1	6	20	8	14	21	20	15	5	3	116
1967	0	0	0	9	14	9	15	16	13	18	3	0	97
1968	0	4	0	11	10	13	13	7	17	15	3	1	94
1969	1	1	4	5	13	13	12	17	17	13	5	1	102
1970	0	3	3	8	13	13	22	21	20	13	2	7	125
1971	0	3	5	8	16	15	15	20	17	17	3	3	122
1972	0	0	4	9	6	18	18	17	19	15	16	7	129
1973	1	1	2	3	11	10	16	15	22	10	6	0	97
1974	1	0	5	16	16	19	16	19	21	16	5	1	135
1975	9	0	4	7	18	14	14	20	13	20	7	2	128
Mean( $\bar{x}$ )	0.850	1.150	2.700	6.450	13.500	12.850	15.500	17.250	17.900	15.150	5.200	2.000	110.500
S	1.956	1.276	2.100	3.263	4.770	2.594	3.428	3.548	3.390	2.937	3.467	2.191	14.898
S/ $\bar{x}$	2.301	1.110	0.778	0.506	0.353	0.202	0.221	0.206	0.189	0.194	0.671	1.096	0.135

(Unit: days)

Station: Kanchana Buri

Note: S - Standard Deviation

A rainy day is one with an amount of rainfall

Source: Meteorological Department

Table A. 3-27 Maximum Drought Days in Wet Season

(Unit: days)

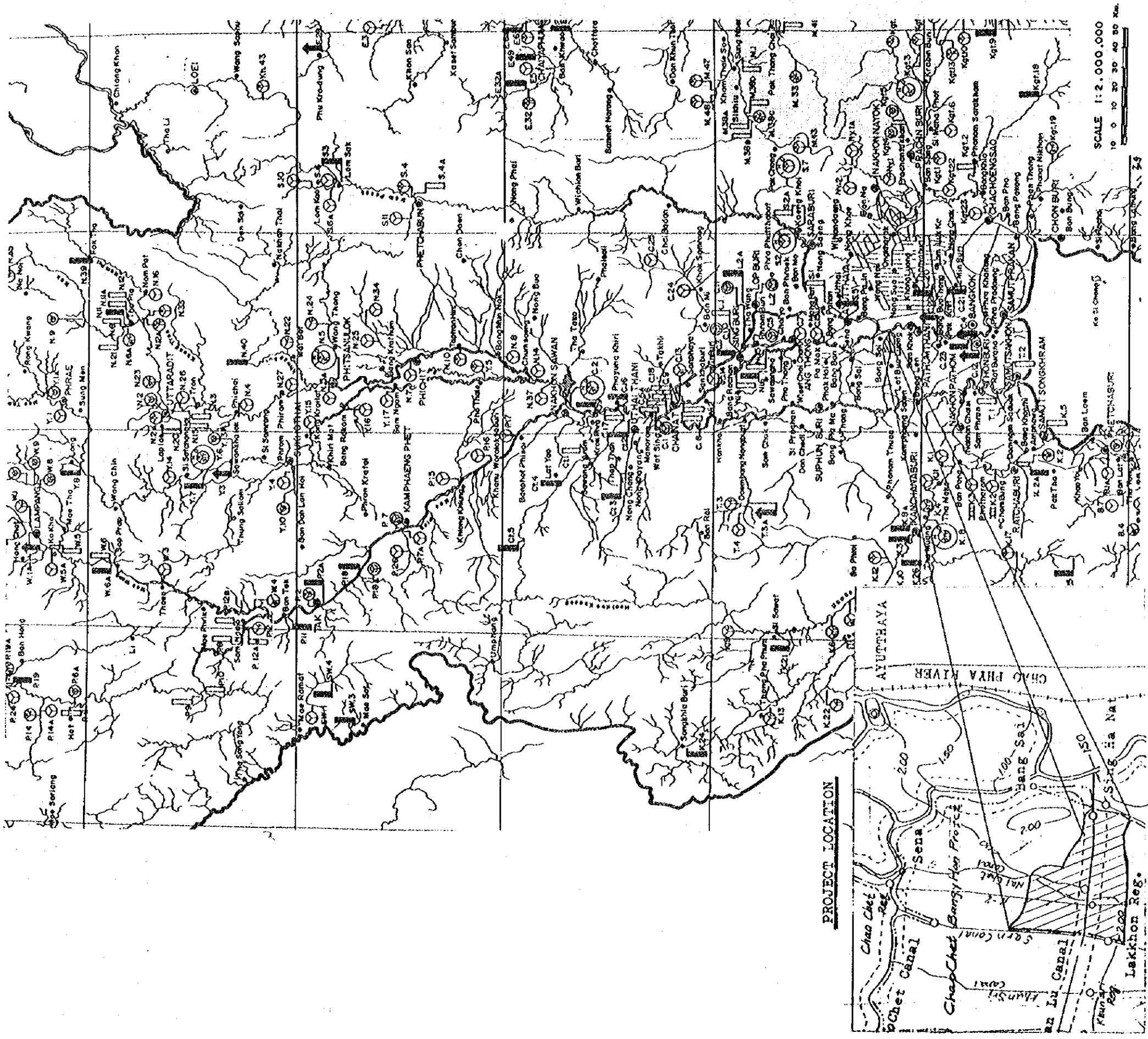
Year	Sing Ha Nat		Station		Phraya Banlu	
	Drought	Period	Drought	Period	Drought	Period
1962	15	6 Oct. - 30 Oct.	13	12 May - 2 Jun.		
1963	15	4 May - 18 May	15	4 May - 18 May		
1964	8	1 Sep. - 8 Sep.	14	4 Oct. - 17 Oct.		
1965	16	29 Jun. - 14 Jul.	34	25 Jun. - 25 Jul.	13	9 Oct. - 22 Oct.
1966	6	26 Jul. - 31 Jul.	15	22 May - 5 Jun.	12	22 Jun. - 2 Jul.
1967	11	5 May - 15 May	17	17 Aug. - 2 Sep.	22	29 May - 19 Jun.
1968	22	11 May - 1 Jun.	21	13 May - 2 Jun.	21	12 May - 1 Jun.
1969	15	7 May - 21 May	15	7 May - 21 May	19	14 Jul. - 2 Aug.
1970	11	1 May - 11 May	15	27 Aug. - 10 Sep.	10	3 May - 12 May
1971	10	4 Jul. - 15 Jul.	15	1 May - 15 May	18	1 May - 18 May
1972	22	21 Jun. - 13 Aug.	30	1 May - 30 May	16	1 May - 16 May
1973	22	10 Oct. - 31 Oct.	16	9 Oct. - 25 Oct.	16	10 Oct. - 25 Oct.
1974	10	1 May - 10 May	13	13 Aug. - 25 Aug.	12	3 May - 14 May
1975	11	13 May - 23 May	10	29 Jul. - 7 Aug.	18	27 Jul. - 13 Aug.
1976	12	13 Jun. - 25 Jun.	21	4 Jul. - 24 Jul.	10	16 Jun. - 25 Jun.
Mean:	13.7		17.6		15.6	
		7 times (46.7 %)		7 times (46.7 %)		6 times (50.0 %)
May	3	" (20.0)	1	" (6.7)	2	" (16.7)
June	2	" (13.3)	2	" (13.3)	2	" (16.7)
July	0	" (-)	3	" (20.0)	0	" (-)
August	1	" (6.7)	0	" (-)	0	" (-)
September	2	" (13.3)	2	" (13.3)	2	" (16.7)
October	15 years		15 years		12 years	

Source: Chao-chet Bangyeehon Office and Phraya Banlu Project Office, RID



FIGURE A.3-3

LOCATIONS OF HYDROLOGIC OBSERVATION STATIONS

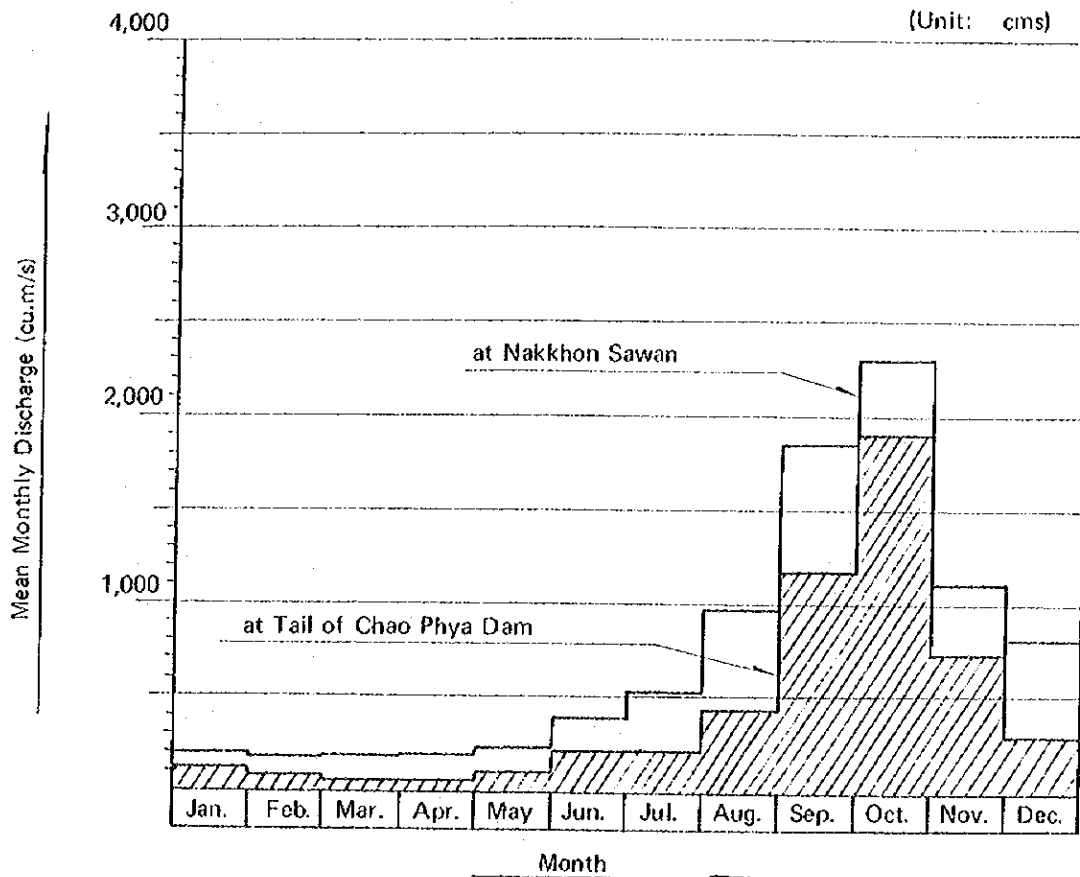


LEGENDS

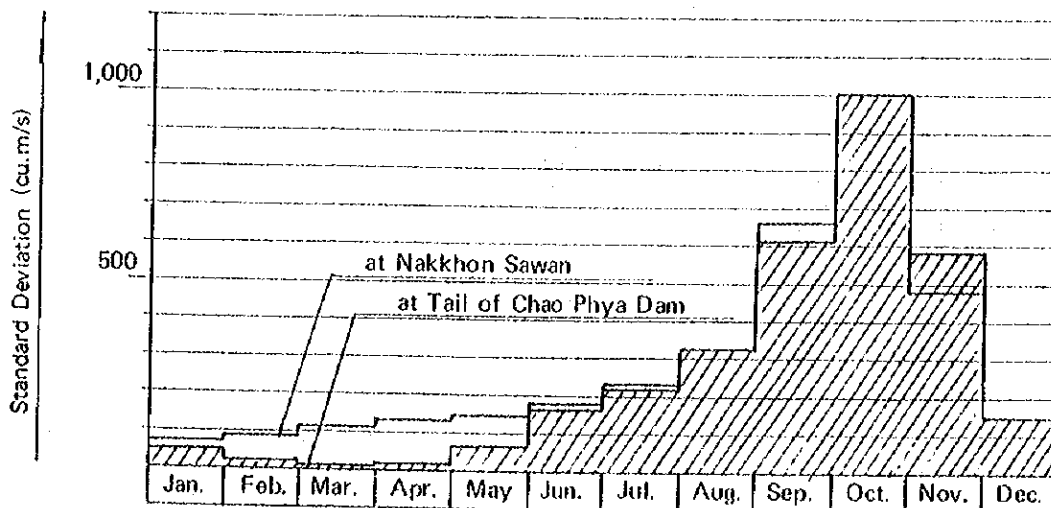
- DISCONTINUED STREAMFLOW RATING STATION
- STREAMFLOW RATING STATION WITH STAFF GAGE
- STREAMFLOW RATING STATION WITH RECORDER (R)
- SEDIMENT SAMPLING (S) & TEMPERATURE (T)
- INTERNATIONAL HYDROLOGIC DECADE STATION
- DISCONTINUED STAFF GAGE
- STAFF GAGE
- RECORDING GAGE

Source: Hydrology Section, Survey Division, RFD

FIGURE A. 3-4 MONTHLY MEAN DISCHARGE OF CHAO PHYA RIVER



FLUCTUATION OF MONTHLY MEAN DISCHARGE



Note: Data period 19 years (1957 - 1975)

Table A. 3-28 Monthly and Annual Mean Discharge of Chao Phya River at Nakorn Sawan

Drainage Area: 110,559 square km  
(Unit: cms)

Year	Monthly Mean Discharge												Annual Mean Discharge
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1957				54.3	61.4	236	365	571	1,789	2,034	726	203	
1958	109	86.5	45.7	34.3	50.1	176	413	730	1,493	1,146	340	162	399
1959	81.5	51.2	47.3	33.8	86.5	267	357	1,274	2,245	3,292	805	226	731
1960	114	87.2	47.8	32.6	82.5	207	315	927	2,034	2,383	939	386	630
1961	144	90.5	67.1	62.4	161	550	612	1,153	2,719	4,336	2,165	439	1,042
1962	241	124	74.7	53.7	109	195	372	872	1,921	3,504	1,424	243	761
1963	87.9	59.1	54.4	45.2	42.2	145	331	1,511	1,882	2,382	1,784	659	749
1964	147	149	137	150	216	380	589	689	1,662	3,974	1,669	431	799
1965	200	190	227	234	268	390	486	757	1,340	1,132	758	302	525
1966	175	189	216	197	268	716	528	1,196	2,497	1,614	819	375	733
1967	231	205	220	217	226	329	387	572	1,169	2,218	817	400	582
1968	242	189	192	216	420	518	624	922	950	728	369	279	469
1969	204	178	144	158	155	324	580	938	1,768	1,974	922	378	644
1970	253	271	260	284	446	763	1,381	1,908	3,463	3,225	1,283	594	1,178
1971	325	290	283	298	371	518	630	1,183	1,911	1,929	1,098	476	776
1972	296	279	284	290	280	380	436	681	785	1,010	756	476	496
1973	245	274	328	344	334	479	533	929	1,557	2,085	947	414	706
1974	272	317	355	396	451	427	417	779	1,173	1,437	1,539	616	682
1975	311	300	401	492	477	595	720	1,057	2,846	3,890	1,747	743	1,132
$\bar{x}$	204.4	165.0	188.6	189.1	237.6	399.7	530.3	982.1	1,851.3	2,299.6	1,201.0	410.6	714.9
S	74.58	86.06	113.68	134.07	142.85	174.4	251.41	327.63	664.76	1,014.21	485.68	155.09	
S/x	0.365	0.465	0.503	0.709	0.601	0.436	0.436	0.334	0.359	0.441	0.441	0.378	

Note:  $\bar{x}$  - Mean    S - Standard Deviation

Source: Hydrology Division, Royal Irrigation Department

Table A. 3-29 Monthly and Annual Mean Discharge of Chao Phya River at Chai Nat

Drainage Area: 120,693 square km  
(Unit: cms)

Year	Monthly Mean Discharge												Annual Mean Discharge
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1957				63.2	66.1	170	242	314	1,429	2,010	416	124	
1958	48.7	60.3	33.7	35.8	71.7	83.9	281	434	1,256	1,001	230	71.6	292
1959	58.6	41.0	47.5	43.2	42.7	192	200	919	1,767	3,381	560	153	617
1960	82.7	66.8	47.1	46.9	46.3	105	166	346	1,252	1,892	627	359	419
1961	119	59.6	53.7	72.4	130.0	490	418	861	1,858	3,406	2,180	465	843
1962	223	105	75.9	-	54.9	84.0	169	412	1,533	3,183	1,212	202	-
1963	63.5	32.5	-	41.2	35.6	33.8	57.6	808	1,153	1,994	1,419	549	-
1964	106	87.2	71.0	78.4	106	231	296	153	941	3,202	1,518	391	598
1965	128	75.9	86.0	84	110	149	66	140	620	564	150	125	192
1966	88	90	80	82.9	159	534	143	591	1,848	1,357	357	264	466
1967	101	97.3	89.4	80.9	85.3	76.0	71.6	82.6	430	1,468	228	196	251
1968	124	85.5	80.1	78.2	274	238	115	180	159	136	83.8	128	140
1969	107	71	59	56.0	52.5	49.9	227	281	997	1,224	301	155	298
1970	119	91.9	80.5	91.5	212	549	1,054	1,351	2,417	3,229	1,007	464	889
1971	208	112	88.2	77.8	121	187	120	529	1,356	1,199	523	410	469
1972	240	107	83.3	97.26	75.45	96.70	66.29	93.70	389.46	597.32	191.33	371.09	201
1973	115.32	69.64	71.06	68.83	79.48	141.90	81.22	193.93	628.03	1,507.87	252.16	325.67	295
1974	144.22	80.30	73.70	79.30	168	139	77.6	209	509	1,397	1,312	524	393
1975	217	103	79.0	87.9	131	281	267	363	2,044	3,575	1,415	613	765
$\bar{x}$	127.7	80.9	70.5	70.4	106.4	201.6	216.8	434.8	1,188.8	1,911.2	731.2	310.0	454.2
S	55.94	19.69	15.93	17.83	50.85	153.79	219.44	329.31	613.19	1,067.01	593.48	162.16	
S/x	0.438	0.243	0.226	0.253	0.572	1.012	0.757	0.757	0.516	0.558	0.812	0.523	

Note:  $\bar{x}$  - Mean S - Standard Deviation

Source: Hydrology Division, Royal Irrigation Department

Table A. 3-30 Monthly and Annual Specific Runoff of Chao Phya River at Nakhon Sawan

Drainage Area: 110,559 square km  
(Unit: mm)

Year	Monthly Specific Runoff												Annual Specific Runoff
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1957	2.7	1.9	1.1	0.8	1.5	5.5	8.8	13.8	41.9	49.3	17.0	4.9	
1958	2.0	1.1	1.1	0.8	1.5	4.1	10.0	17.7	34.8	27.8	8.0	3.9	114.3
1959	2.8	2.0	1.2	0.8	2.0	6.3	8.7	30.9	52.6	79.8	18.9	5.5	209.8
1960	3.5	2.0	1.6	1.5	3.9	4.8	7.6	22.5	47.7	57.7	22.0	9.4	180.5
1961	5.9	2.7	1.8	1.3	2.6	12.9	14.8	28.2	63.7	105.0	50.7	10.6	298.4
1962	2.1	1.3	1.3	1.1	1.0	4.6	9.0	21.1	45.0	84.9	33.4	5.9	218.2
1963	3.6	3.4	3.3	3.5	5.2	3.4	8.0	36.6	42.9	57.7	41.8	16.0	213.2
1964	4.8	4.2	5.5	5.5	6.5	8.9	14.3	16.7	39.0	81.7	39.1	10.4	229.1
1965	4.3	4.1	5.2	4.6	6.5	9.1	11.8	18.3	31.4	27.4	18.0	7.3	149.8
1966	5.6	4.5	5.3	5.1	6.5	16.8	12.8	29.0	58.5	39.1	19.2	9.1	209.2
1967	5.9	4.3	4.7	5.1	5.5	7.7	9.4	13.9	27.4	53.7	19.2	9.7	167.0
1968	4.9	3.9	3.5	3.7	3.8	12.0	15.1	22.3	21.8	17.6	8.7	6.8	134.5
1969	6.1	5.9	6.3	6.7	10.8	7.6	14.1	22.7	41.5	47.8	21.6	9.2	184.3
1970	7.9	6.4	6.9	7.0	9.0	17.9	33.5	46.2	81.2	78.1	30.0	14.4	337.1
1971	7.2	6.3	6.9	6.8	6.8	12.1	15.2	28.7	44.8	46.7	25.8	11.5	222.0
1972	5.9	6.0	6.0	6.1	8.1	8.9	10.6	16.5	18.4	24.5	17.8	11.1	141.8
1973	6.6	6.9	8.8	9.3	10.9	11.2	12.9	22.5	36.5	50.5	22.2	10.0	201.9
1974	7.5	6.5	9.7	11.5	11.6	10.0	10.1	18.9	27.5	34.8	36.0	14.9	194.7
1975	5.0	4.1	4.6	4.4	5.8	14.0	17.4	25.6	66.7	94.2	41.0	18.0	323.8
$\bar{x}$	1.80	1.89	2.76	3.14	3.46	9.4	12.8	23.8	43.3	55.7	25.8	9.9	204.6
S	0.360	0.461	0.600	0.714	0.595	4.09	5.61	7.94	15.58	24.56	11.37	3.75	
S/X						0.435	0.438	0.334	0.360	0.441	0.441	0.379	
Mean before 1964	3.2	1.8	1.4	1.1	2.1	5.9	9.6	24.4	46.9	66.0	27.4	8.0	197.8
Mean after 1964	5.9	5.2	6.2	6.4	7.9	11.4	14.8	23.4	41.2	49.7	24.9	11.0	208.0

Note:  $\bar{x}$  - Mean    S - Standard Deviation  
Source: Hydrology Division, Royal Irrigation Department

Table A. 3-31 Monthly and Annual Specific Runoff of Chao Phya River at Chai Nat

Drainage Area: 120,693 square km  
(Unit: mm)

Year	Monthly Specific Runoff												Annual Specific Runoff
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1957	1.1	1.2	0.7	1.4	1.5	3.7	5.4	7.0	30.7	44.6	8.9	2.8	
1958	1.3	0.8	1.0	0.8	1.6	1.8	6.2	9.6	27.0	22.2	2.8	1.6	76.6
1959	1.8	1.4	1.0	0.9	0.9	4.1	6.3	20.4	38.0	75.0	12.0	3.4	164.2
1960	2.6	1.2	1.2	1.6	2.9	2.3	3.7	7.7	26.9	41.8	13.5	8.0	110.1
1961	5.0	2.1	1.7	-	1.2	10.5	9.3	19.1	39.9	75.6	46.8	10.3	221.0
1962	1.5	1.1	-	0.9	0.8	1.8	3.8	9.1	33.4	70.6	26.0	4.5	-
1963	2.4	1.8	1.6	1.7	2.3	0.7	1.3	17.9	24.8	44.3	30.5	12.2	-
1964	2.9	1.5	1.9	1.8	2.4	5.0	6.6	3.4	20.2	71.1	32.6	8.7	157.4
1965	2.0	1.8	1.8	1.9	3.5	3.2	1.5	3.1	13.3	12.5	3.4	2.8	50.2
1966	2.2	2.0	2.0	1.7	1.9	11.5	3.2	13.1	39.7	30.1	7.7	5.9	122.2
1967	2.8	1.8	1.8	1.7	1.9	1.6	1.6	1.8	9.2	32.6	4.9	4.4	65.9
1968	2.4	1.4	1.3	1.2	1.2	5.1	2.6	4.0	3.4	3.0	1.8	2.8	36.9
1969	2.6	1.8	1.8	2.0	4.7	1.1	5.1	6.3	21.4	27.2	6.5	3.5	78.6
1970	4.6	2.2	2.0	1.7	2.7	11.8	23.4	30.0	51.9	71.7	21.6	10.3	233.6
1971	5.3	2.2	1.8	2.1	1.7	4.0	2.7	11.7	28.7	26.6	11.2	9.1	107.2
1972	2.6	1.4	1.6	1.5	1.8	2.1	1.5	2.1	8.4	13.3	4.1	6.2	52.8
1973	3.2	1.6	1.6	1.7	3.7	3.0	1.8	4.3	13.5	33.5	5.4	7.2	77.6
1974	4.8	2.1	1.7	1.9	2.9	3.0	1.7	4.6	10.9	31.0	28.2	11.6	102.8
1975	2.8	1.6	1.6	1.5	2.4	6.0	5.9	8.1	43.9	79.3	30.4	13.6	200.6
$\bar{x}$	1.24	0.40	0.35	0.39	1.37	4.3	4.9	9.6	25.5	42.4	15.7	6.9	119.2
S	0.443	0.250	0.225	0.260	0.571	0.767	0.996	0.761	0.517	0.558	0.812	0.520	

Note:  $\bar{x}$  - Mean    S - Standard Deviation  
Source: Hydrology Division, Royal Irrigation Department

Table A. 3-32 Extreme Momentary Discharge Records of the Chao Phya River

(Unit: cu.m/sec)

Year	Stations											
	Nakhon Sawan C.2				Chao Phya Dam C.13				Wat Chulamani		Ayutthaya C.15	
	Max.		Min.		Max.		Min.		Max.		Min.	
1957	2,592	10 Oct.	-	-	2,670	12 Oct.	-	-	438	7 Oct.	-	-
1958	2,066	30 Sep.	28	6,7 Apr.	2,295	30 Sep.	25	7 May	422	6 Oct.	8.15	27 May
1959	4,509	8 Oct.	25	22 Apr.	4,325	9 Oct.	22	15 May	572	19 Oct.	13	7 Feb.
1960	2,567	20 Oct.	15	6,7 Apr.	2,095	13 Oct.	30	24 Apr. 11-16 May	444	19 Oct.	11	8,19 May
1961	4,712	13 Oct.	45	20 Apr.	3,985	15 Oct.	36	12 Feb.	531	3 Nov.	9.40	26 Apr.
1962	3,812	17 Oct.	39	17 Apr.	3,609	15 Oct.	22	9-12 Jul.	537	24 Oct.	8.50	10 Jul.
1963	2,946	12 Oct.	32	22,23 May	2,628	11 Oct.	25	1-4 Jul.	508	19 Oct.	8.50	6,19 Jun.
1964	3,825	11 Oct.	90	5,6 Feb.	3,475	18 Oct.	49	14-18 Aug.	546	9 Oct.	26	17 Aug.
1965	1,537	2 Oct.	153	6 Feb.	1,071	3 Oct.	44	14-22 Aug.	-	-	-	-
1966	2,930	24 Sep.	131	26,27 Jan.	2,349	25 Sep.	55	21,22 Feb.	485	28 Sep.	32	30 Apr.
1967	2,768	8 Oct.	133	10 Apr.	1,985	7 Oct.	53	9 May	452	16 Oct.	30	19 Jul.
1968	1,263	21 Sep.	165	3,4 Feb.	588	13 May	65	25 Apr.	214	14 May	28	26 Jul.
1969	2,827	28 Sep.	122	9 May	2,047	6 Oct.	27	30,31 May	499	7 Oct.	14	22 Jun.
1970	4,420	30 Sep.	200	2 Apr.	4,049	7 Oct.	59	5 Apr.	558	16 Oct.	36	22 Mar.
1971	2,370	9 Oct.	242	17 Apr.	1,560	13 Oct.	57	23,24 Apr. 1,2 Jul.	443	11 Oct.	21	5 Jul.
1972	1,310	8 Oct.	197	9 May	1,129	9 Oct.	58	30 Jul.	395	10 Oct.	14	26 May
1973	2,600	5 Oct.	183	5 Jan.	1,910	7 Oct.	64	14 Feb. 12,13 Jul.	488	10 Oct.	16	15 Jul.
1974	1,930	9 Nov.	186	5 Jan.	2,089	19 Oct.	71	18 Jun.	511	21 Oct.	33	19,20 Jul.
1975	4,355	17 Oct.	231	13 Feb.	3,977	22 Oct.	69	7-9, 13-15 Feb.	616	20-23 Oct.	24	10 Feb.
1957-1975	m=2,913	S=1,059	m=123	S=75	m=2,518	S=1,076	m=46	S=17	m=481	S= 86	m=19.6	S=9.4
1957-1963	m=3,315	S= 956	m= 30.7	S= 9.7	m=3,087	S= 81.1	m=27	S= 5.0	m=493	S= 54	m= 9.8	S=1.7
1964-1975	m=2,678	S=1,045	m=169	S=44	m=2,186	S=1,073	m=56	S=11.5	m=473	S=100	m=24.9	S=7.4

Note : m; mean, s; standard deviation

Source: Royal Irrigation Department

Probable Discharge and Water Level of the Chao Phya River

Study on maximum and minimum probable discharges has been conducted based on the mean monthly discharge record observed at Nakorn Sawan and Chao Phya Dam from 1964 - 1975. Its results are as shown below:

Probable Max. Monthly Mean Discharge

(Unit: cu.m/s)

<u>Return Period</u>	<u>Nakorn Sawan</u>	<u>Chao Phya Dam</u>
2 years	2,025	1,596
5 years	2,940	2,583
10 years	3,548	3,291
15 years	4,132	4,007

Probable Min. Monthly Mean Discharge at Nakorn Sawan

(unit: cu.m/s)

<u>Return Period</u>	<u>Discharge</u>
2 years	221
5 years	173
10 years	149
15 years	137
20 yerars	129

Continuous maximum discharges in long term such as monthly maximum discharges have a relation with a flood duration in low-lying area, while momentary maximum discharges, which is larger than an averaged value of the maximum discharges in a long term, have relation with the highest flood level, which give a big influence on facilities such as dikes. Table A.3-33 shows momentary probable maximum discharges and maximum water levels of the Chao Phya river.



Table A. 3-33 Probable Maximum Momentary Discharges and Maximum Water Levels of the Chao Phya River

(Unit: cu.m/s)

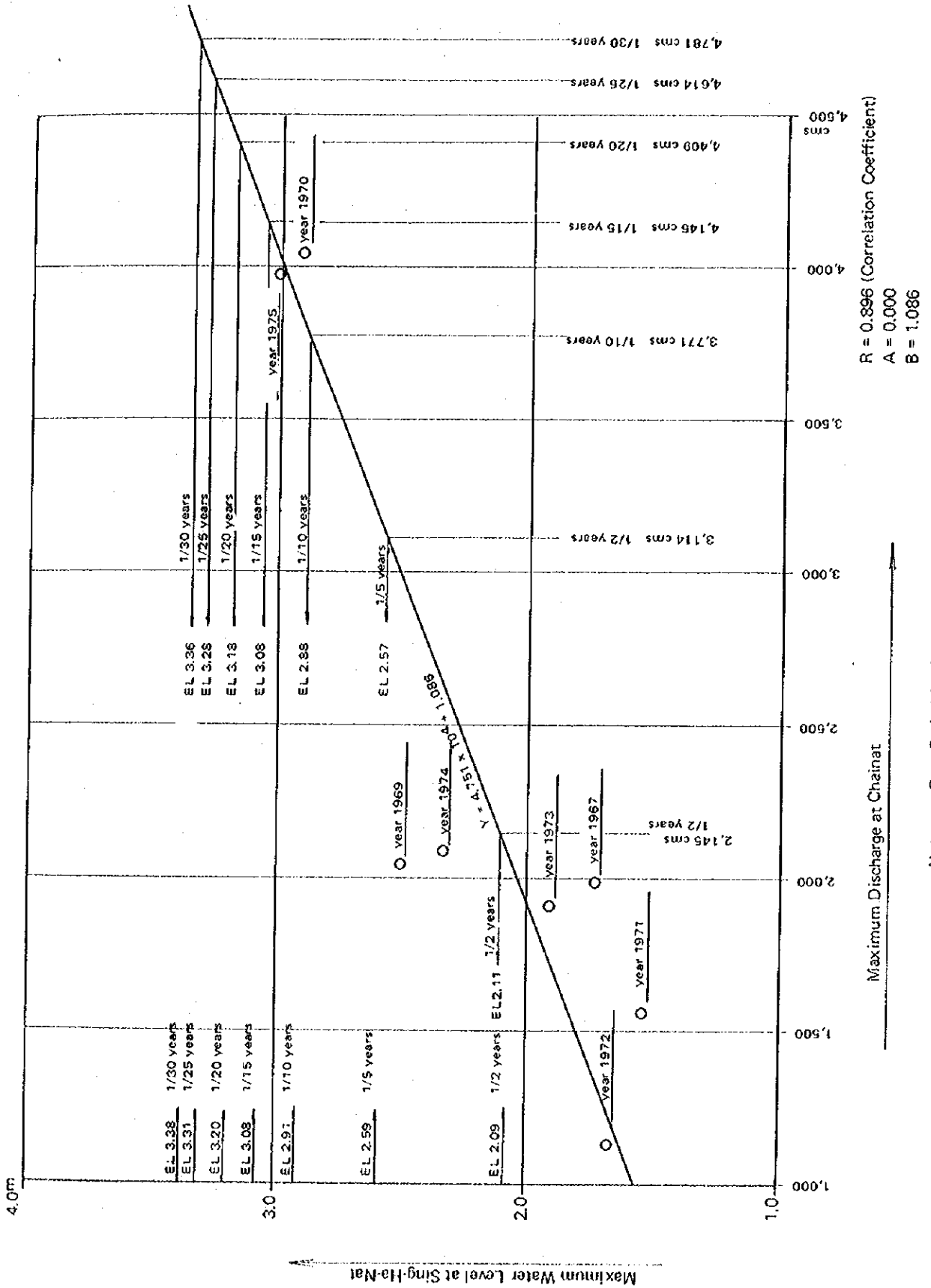
Return Period	Probable Maximum Momentary Discharges									
	Nakorn Sawan		Chai Nat Dam		Wat Chulamani I/		Phraya Banlu		Pak Kret	
	1957-1975	1964-1975	1957-1975	1964-1975	1957-1975	1964-1975	1957-1975	1964-1975	1953-75	1964-75
2 years	2,702	2,515	2,425	2,145	494	490	496			
5 years	3,811	3,550	3,457	3,114	546	542	552			
10 years	4,583	4,218	4,160	3,771	575	571	583			
15 years	4,989	4,590	4,563	4,145	590	587	599			
20 years	5,290	4,849	4,847	4,409	600	598	610			
25 years	5,527	-	5,070	-	607	-	618			
30 years	5,715	5,048	5,249	4,614	613	-	624			
40 years	6,021	-	5,537	-	622	-	-			
50 years	6,252	5,209	5,757	4,781	629	-	-			
100 years	6,993	-	6,460	-	649	-	-			

Return Period	Probable Maximum Water Levels (Meter + Mean Sea Level)											
	Wat Chulamani I/		Ayutthaya		Sing Ha Nat 1967-1976		1966-1976		1953-75		1964-75	
	1957-75	1964-75	1957-63	1964-75	1967-1976	(exc. 1968)	1966-1976	1953-75	1964-75	1953-75	1964-75	
2 years	4.48	4.51	4.08	3.48	2.09	2.58	2.55	1.84	1.80			
5 years	4.75	4.79	4.40	4.18	2.59	2.77	3.12	2.01	2.02			
10 years	4.89	4.95	4.58	4.58	2.91	2.91	3.46	2.11	2.15			
15 years	4.96	5.03	4.67	4.79	3.08	2.99	3.55	2.15	2.22			
20 years	5.01	5.08	4.73	4.93	3.20	3.56	3.77	2.18	2.27			
25 years	5.06	-	-	-	-	-	-	-	-			
30 years	5.08	-	4.77	5.03	3.31	-	3.88	2.23	-			
40 years	5.13	-	-	-	-	-	-	-	-			
50 years	5.15	-	4.81	5.11	3.38	-	3.95	2.28	-			

Note: 1/ Two years data of 1967 and 1968 are excluded.

FIGURE A. 3-5 CORRELATION BETWEEN MAXIMUM WATER LEVEL AT SING-HA WAT AND MAXIMUM DISCHARGE AT CHAINAT (Chao Phya Dam)



Note: Data Period for 9 years (1967 - 1975 exc. 1968)

Influence on Lower Basin Caused by the Construction of Bhumiphol and Sirikit Dams

In order to clarify the influence on the Chao Phya river lower basin caused by the operation of Bhumiphol dam whose construction was completed in 1964, mean monthly discharges of the Chao Phya river observed at Nakorn Sawan before December 1963 and after January 1964 are tabulated as follows:

Monthly Mean Discharge at Chao Phya River at Nokorn Sawan (Dry Season)

(unit: cu.m/s)

<u>Period</u>	<u>Jan.</u>	<u>Feb.</u>	<u>Mar.</u>	<u>Apr.</u>	<u>May</u>	<u>Jun.</u>	<u>Jul.</u>	<u>Aug.</u>
1957 - 1975	204	185	189	189	238	400	530	982
before Dec. 1963	130	83	56	45	86	254	390	1,007
after Jan. 1964	240	236	255	273	326	485	609	968
		<u>Sep.</u>	<u>Oct.</u>	<u>Nov.</u>	<u>Dec.</u>	<u>Annual Mean</u>		
		1,851	2,300	1,101	411	715		
		2,010	2,725	1,169	331	691		
		1,758	2,051	1,061	457	727		

Fluctuation of Mean Discharge (Standard Deviation) (unit: cu.m/s)

<u>Period</u>	<u>Jan.</u>	<u>Feb.</u>	<u>Mar.</u>	<u>Apr.</u>	<u>May</u>	<u>Jun.</u>	<u>Jul.</u>	<u>Aug.</u>
1957-1975	75	86	114	134	143	174	231	328
before Dec. 1963	54	24	11	11	37	126	93	303
after Jan. 1964	52	55	79	96	102	138	251	341
		<u>Sep.</u>	<u>Oct.</u>	<u>Nov.</u>	<u>Dec.</u>			
		665	1,014	486	155			
		360	980	598	163			
		775	949	401	129			

As is clear in the above table, the discharges of the Chao Phya river in the dry season have been stabilized and strengthened to a considerable extent especially in April since 1964 due to the storage effect by Bhumiphol dam.

The minimum monthly mean discharges of the Chao Phya river observed at Nakorn Sawan and at the immediately downstream reach of Chao Phya Dam during the dry season have been picked up and tabulated as follows:

Minimum Monthly Mean Discharge of The Chao Phya River (Dry season)

Year	Nakorn Sawan			Chai Nat (at the tail of Chao Phya Dam)		
	Discharge	Month	Order	Discharge	Month	Order
1958	34.3 <sup>cu.m/s</sup>	Apr.	3	33.7 <sup>cu.m/s</sup>	Mar.	1
1959	33.8	Apr.	2	41.0	Feb.	3
1960	32.6	Apr.	1	46.9	Apr.	4
1961	62.4	Apr.	6	53.7	Mar.	5
1962	53.7	Apr.	5	54.9	May	6
1963	45.2	Apr.	4	33.8	Jun.	2
1958-1963	<u>M=43.7</u>	<u>S=11.3</u>		<u>M=44.0</u>	<u>S=8.6</u>	
1964	137	Mar.	1	71.0	Mar.	4
1965	190	Feb.	5	75.9	Feb.	7
1966	175	Jan.	3	80.0	Mar.	11
1967	205	Feb.	6	71.6	Jul.	5
1968	189	Feb.	4	78.2	Apr.	9
1969	144	Mar.	2	49.9	Jun.	1
1970	253	Jan.	8	80.5	Mar.	12
1971	283	Mar.	11	77.8	Apr.	8
1972	279	Feb.	10	66.3	Jul.	2
1973	245	Jan.	7	68.8	Apr.	3
1974	272	Jan.	9	73.7	Mar.	6
1975	300	Feb.	12	79.0	Mar.	10
1964-1975	<u>M=223</u>	<u>S=54</u>		<u>M=72.7</u>	<u>S=8.2</u>	

Note: M: mean      S: Standard deviation

Minimum monthly mean discharge had occurred in April every year before 1963, but it has been scattered over the three months from Jan. to Mar. since 1964.

An averaged value of the minimum monthly mean discharges observed since 1964 is about 5 times of that of the same before 1964, which clearly prove the storage effect by this dam. According the discharge data observed at the immediate downstream reaches of the Chao Phya dam the minimum monthly mean discharges have been scattered over some months and especially in 1967 and 1972 it occurred in July due to the artificial intake of the river water to cover the water requirement in the whole Central Plain. It seems that occurrence of the minimum monthly mean discharges at the downstream reaches from Chao Phya dam has a close relation with the maximum consecutive draughty days, most of which have been recorded from May to July, and the absolute volume of rainy water in the beginning of the wet season.

An averaged value of the minimum monthly mean discharges observed since 1964 at Chai Nat is 73 cu.m/sec, and the minimum monthly mean discharges are much stabilized as expressed by the standard deviation of 18 cu.m/sec except that of 50 cu.m/sec in 1969 when the salt intrusion became a big problem.

The major objectives to stabilize the minimum discharges in the lower reaches from Chao Phya dam are to secure the discharges of 25 cu.m/sec being released at Ayutthaya to cover the water requirement in Bangkok and to prevent the salt intrusion.

On the contrary, in order to clarify the effect of flood control by Bhumiphol and Sirikit dam operation, maximum monthly mean discharges of the Chao Phya river in the dry season before 1963 and after 1964 have been tabulated below. Reduction from the averaged maximum monthly mean discharge before 1963 to that after 1964 is 586 cu.m/sec at Nakorn Sawan and 761 cu.m/sec at the immediate downstream reaches of Chao Phya dam. By the way Sirikit dam was constructed in 1973, its storage effect of this dam is not yet clear due to the data available are quite limited.

Table A. 3-34 Maximum Monthly Mean Discharge of The Chao Phy River  
(Wet Season)

Year	Nakorn Sawan			Chai Nat (Tail of Chao Phy Dam)		
	Maximum Discharge cu.m/s	Month	Order	Maximum Discharge cu.m/s	Month	Order
1957	2,034	Oct.	6	2,010	Oct.	4
1958	1,483	Sep.	7	1,256	Sep.	7
1959	3,292	Oct.	3	3,381	Oct.	2
1960	2,382	Oct.	4	1,882	Oct.	6
1961	4,336	Oct.	1	3,406	Oct.	1
1962	3,504	Oct.	2	3,183	Oct.	3
1963	2,382	Oct.	4	1,994	Oct.	5
1957-1963	<u>M=2,773</u>		<u>S=907</u>	<u>M=2,445</u>		<u>S=799</u>
1964	3,374	Oct.	3	3,202	Oct.	3
1965	1,340	Sep.	10	620	Sep.	10
1966	2,497	Sep.	4	1,848	Sep.	4
1967	2,218	Oct.	5	1,468	Oct.	6
1968	930	Sep.	12	180	Aug.	12
1969	1,974	Oct.	7	1,224	Oct.	9
1970	3,463	Sep.	2	3,229	Oct.	2
1971	1,929	Oct.	8	1,356	Sep.	8
1972	1,010	Oct.	11	597	Oct.	11
1973	2,085	Oct.	6	1,508	Oct.	5
1974	1,539	Nov.	9	1,397	Oct.	7
1975	3,890	Oct.	1	3,575	Oct.	1
1964-1975	<u>M=2,187</u>		<u>S=925</u>	<u>M=1,684</u>		<u>S=1,055</u>

Note: M: Mean S: Standard Deviation

FIGURE A.3-6. PROFILE OF THE CHAO PHYA RIVER IN LOWER CENTRAL PLAIN

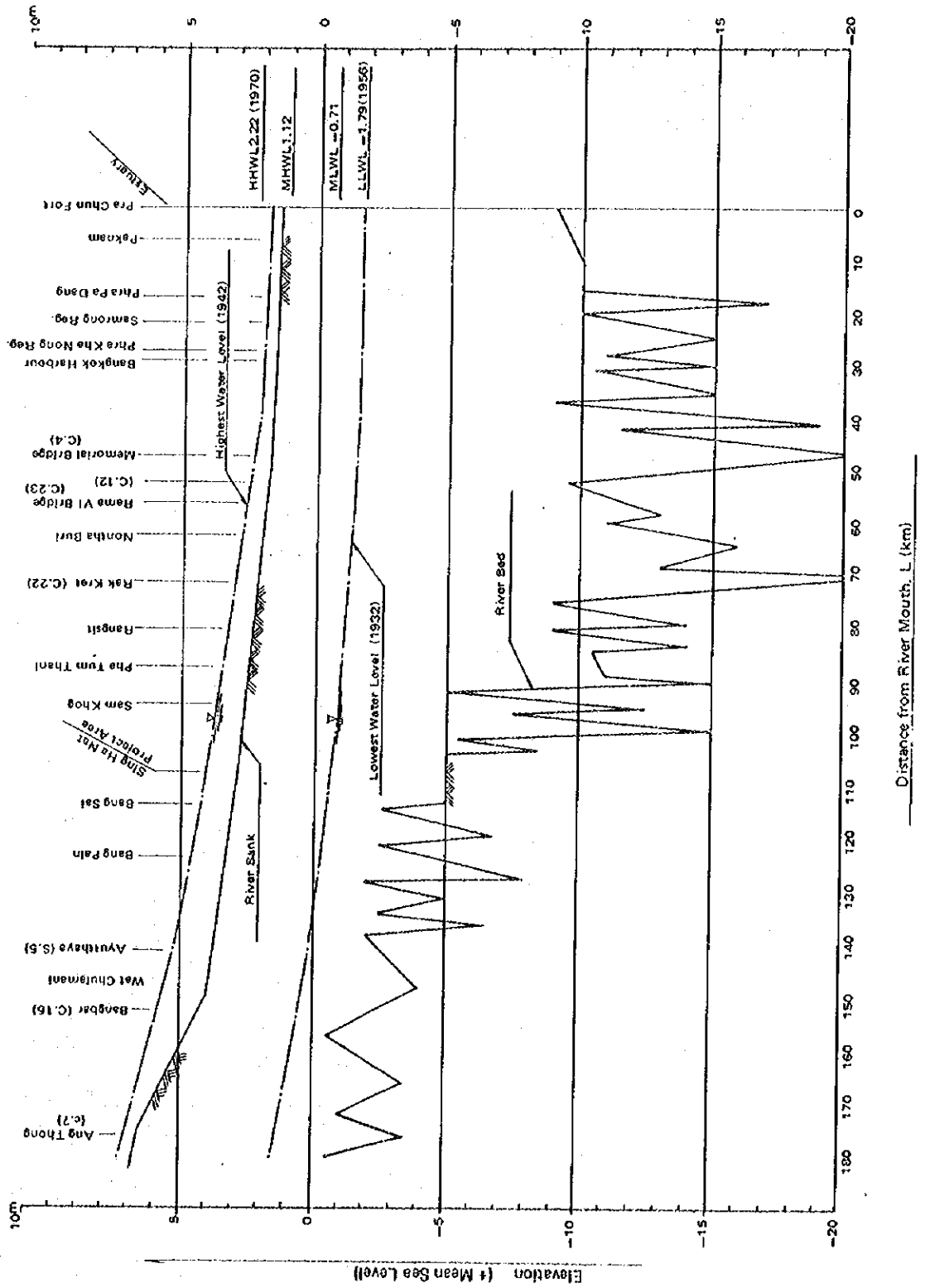
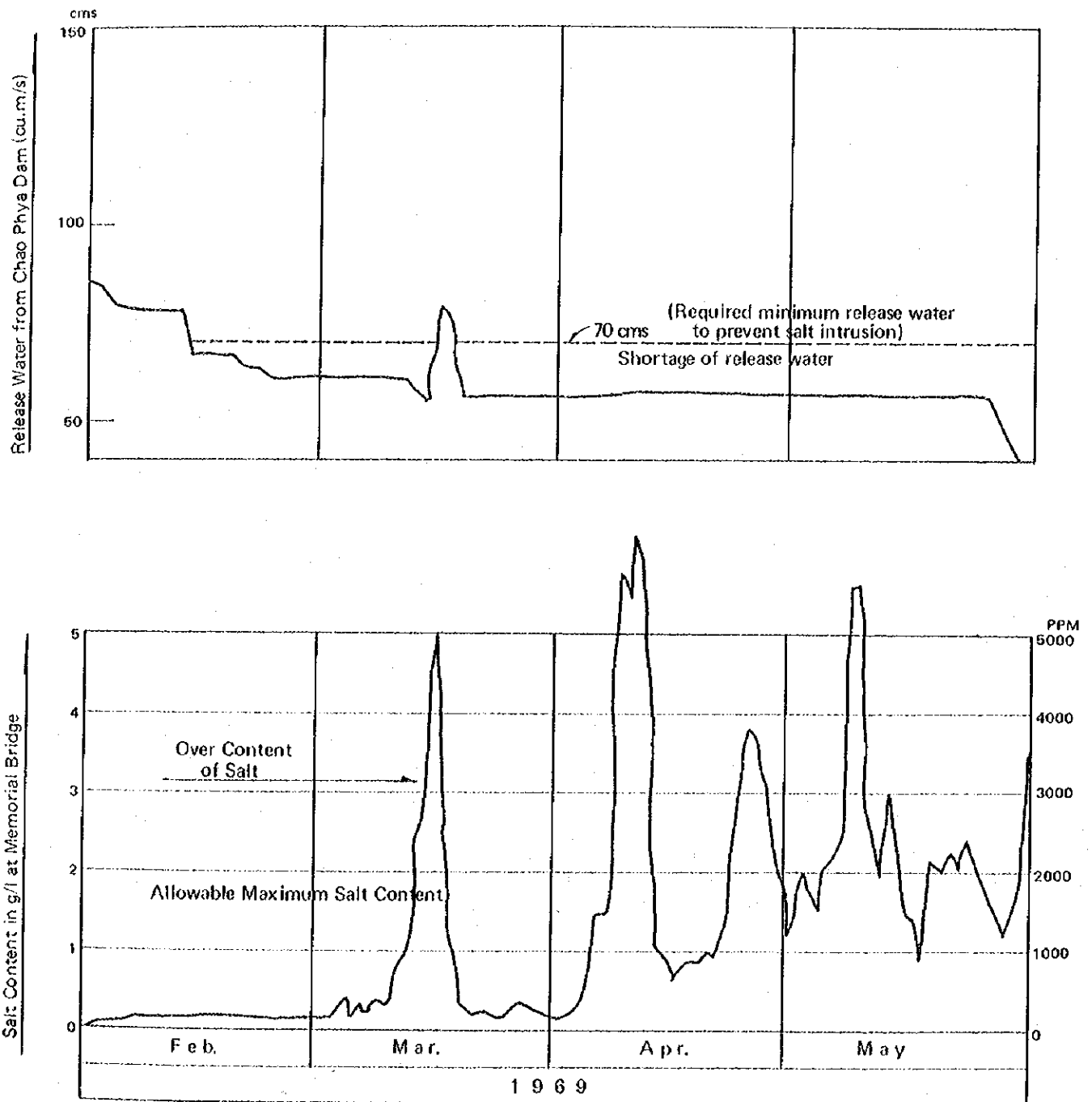


FIGURE A. 3-7

OPERATION OF SALT CONTROL AT MEMORIAL BRIDGE (observed in 1969)



Note: Observation is started from 1964

Source: Operation Center, RID



Table A. 3-35 Tides at Estuary of Chao Phya River

(Unit: m - Mean Sea Level)

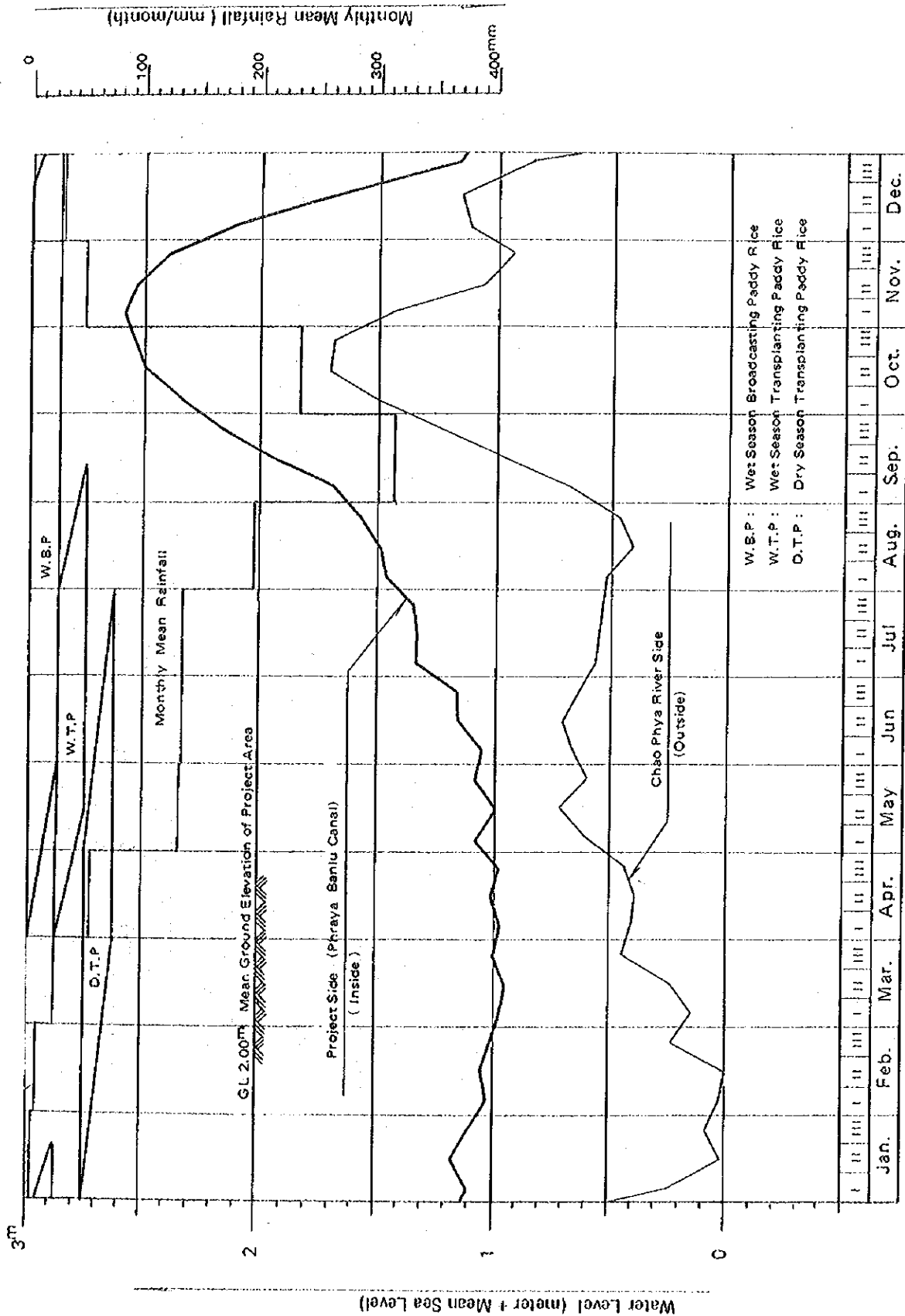
Tides	Bangkok Bar (km.-14)	Fort Phrachul (km.1)	Paknum (km.7)	Phra Pradeng (km.18)	Klong Toey Harbour (km.27)	Hydro. Dept. (km.49) <sup>a/</sup>
H'est. HW	+2.28 (1970)	+2.22 (1970)	+2.04 (1970)	+1.93 (1970)	+1.89 (1970)	+2.42 (1942)
MHW	+1.14	+1.12	+1.10	+1.13	+1.11	+1.05
MHS	+1.07	+1.07	-	-	+1.06	+1.03
MWN	+0.71	+0.71	-	-	+0.72	+0.75
MLWN	-0.65	-0.57	-	-	-0.38	-0.21
MLS	-0.91	-0.71	-	-	-0.54	-0.32
MLLW	-1.30	-1.07	-1.02	-0.90	-0.85	-0.61
L'est LW	-2.42 (1956)	-1.79 (1956)	-1.82 (1964)	-1.78 (1967)	-1.72 (1968)	-1.74 (1964)
M Sp. Range	1.98	1.78	-	-	1.60	1.35
M Np. Range	1.36	1.28	-	-	1.10	0.96

Note:	H'est HW	Highest High Water	31 years (1940 - 1970)
MHW	Mean Higher High Water	Mean Higher High Water	10 years (1940 - 1949)
MHS	Mean High Water Spring	Mean High Water Spring	- do -
MWN	Mean High Water Neap	Mean High Water Neap	- do -
MLWN	Mean Low Water Neap	Mean Low Water Neap	- do -
MLS	Mean Low Water Spring	Mean Low Water Spring	- do -
MLLW	Mean Lower Low Water	Mean Lower Low Water	- do -
L'est LW	Lowest Low Water	Lowest Low Water	31 years (1940 - 1970)

<sup>a/</sup> : Distance from river estuary

Source: RID

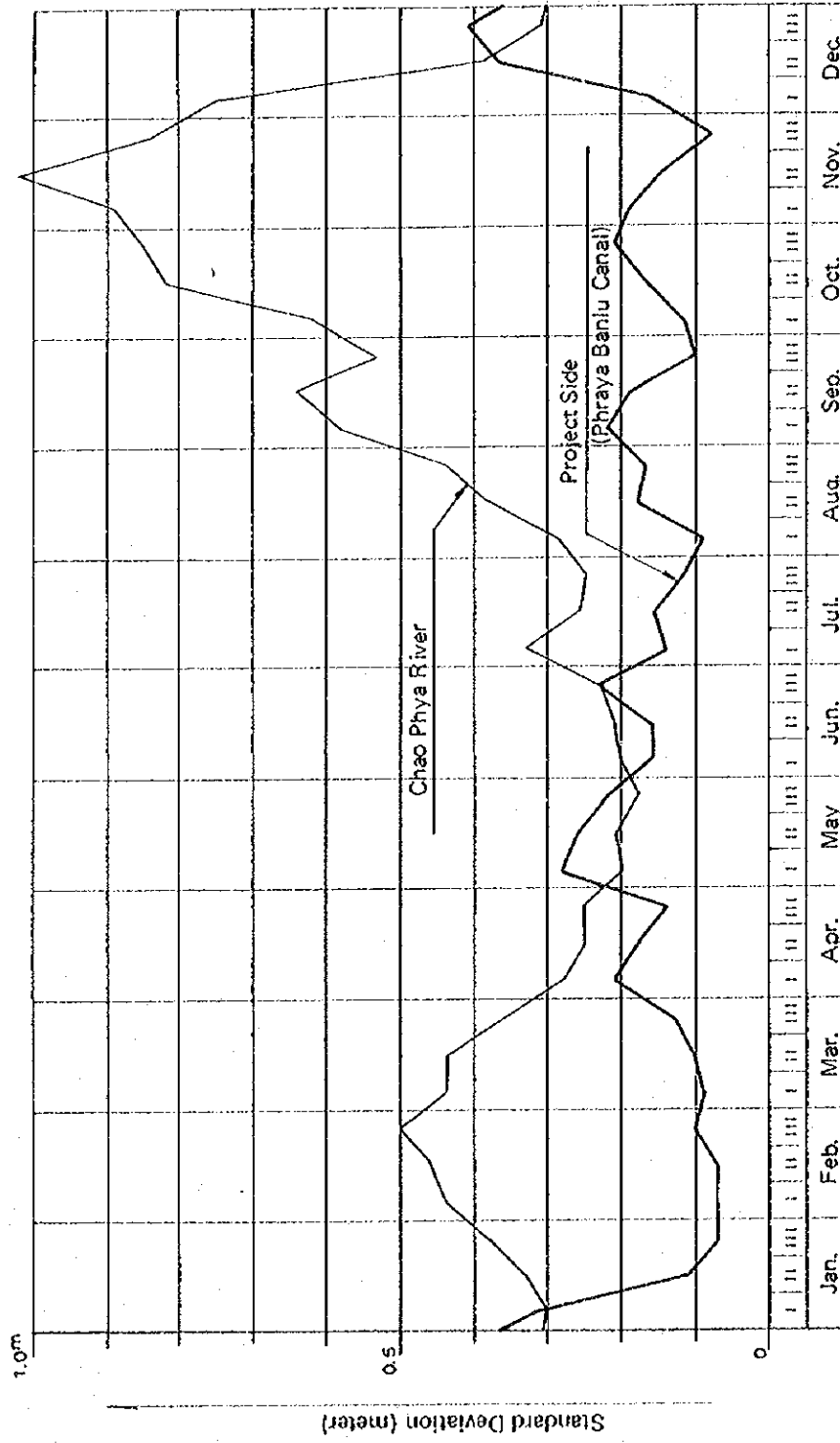
FIGURE A-3-8 MONTHLY MEAN WATER LEVEL AT SING HA WAT



Note: Average for 10 years (1967 - 1976)

Source: Chao-Chet Bangyeehon Project Office, RID

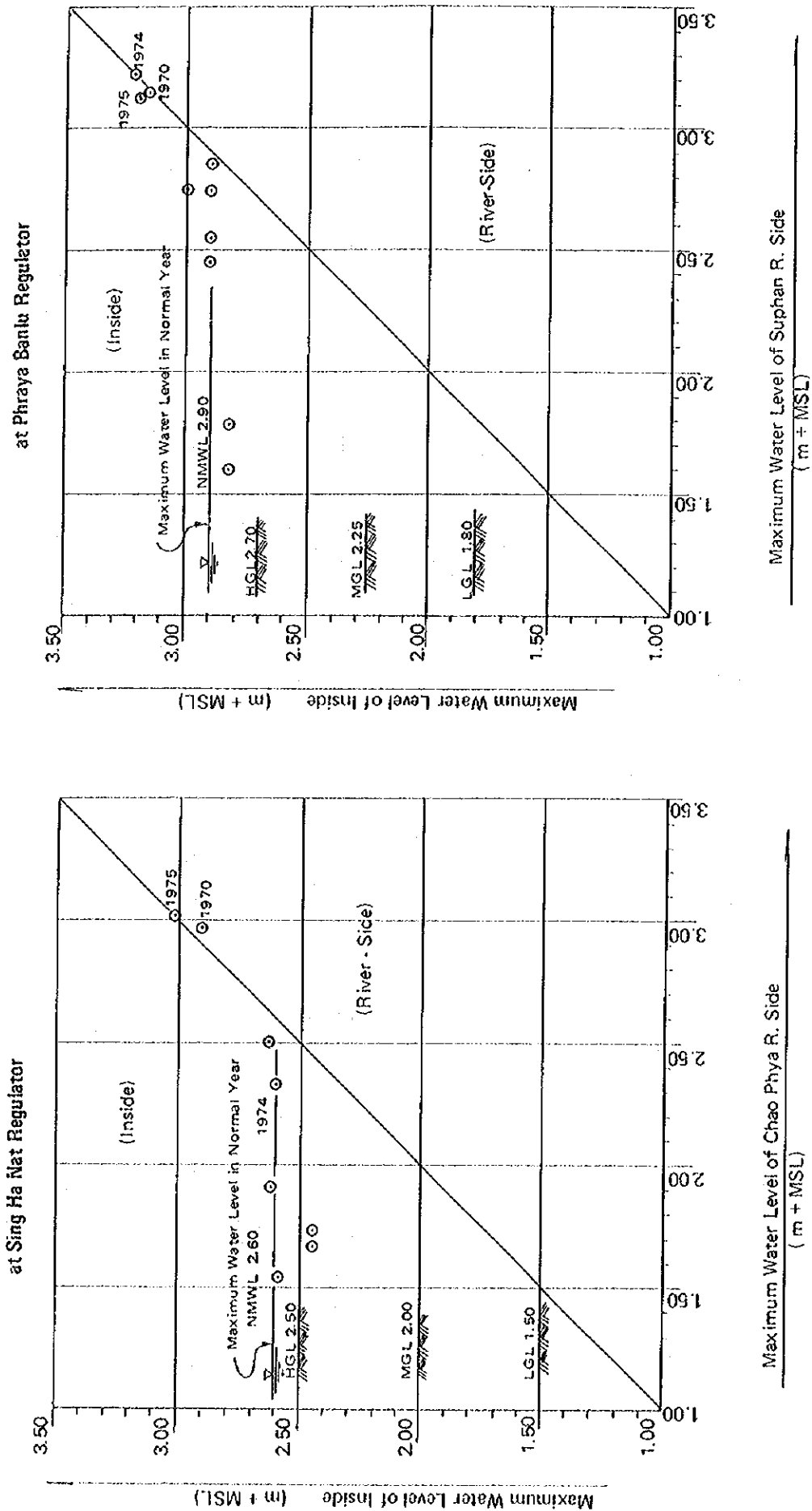
FIGURE A.3-9 FLUCTUATION OF MONTHLY MEAN WATER LEVEL AT SING HA NAT



Note: Average for 10 years (1967 - 1976)

Source: Chao-Chet Bangyeehon Project Office, RID

FIGURE A. 3-10 RELATIONSHIP BETWEEN MAXIMUM WATER LEVELS OF INSIDE OF WEST BANK TRACT AND RIVER SIDE IN THE VICINITY OF PROJECT AREA



Note: NMWL : Normal Maximum Water Level  
 HGL : Highest Ground Level, MGL : Mean Ground Level, LGL : Lowest Ground Level

Table A. 3-36 Maximum and Minimum Probable Water Level

(Unit: m)

Return Period	Maximum Water Level			
	Phraya Banlu		Sing Ha Net	
	Outside (Suphan River)	Inside (Project Side)	Outside (Chao Phya River)	Inside (Project Side)
2 years	2.55	2.95	2.09	2.58
5 years	3.12	3.09	2.59	2.77
10 years	3.46	3.18	2.91	2.91
15 years	3.65	3.23	3.08	2.99
20 years	3.77	3.25	3.20	3.05

Return Period	Minimum Water Level			
	Phraya Banlu		Sing Ha Net	
	Outside (Suphan River)	Inside (Project Side)	Outside/ (Chao Phya River)	Inside (Project Side)
2 years	0.14	0.95	-0.64	0.55
5 years	-0.03	0.80	-0.74	0.55
10 years	-0.12	0.74	-0.79	0.49
15 years	-0.16	0.71	-0.82	0.47
20 years	-0.19	0.70	-0.83	0.45

Note: Phraya Banlu: 1966 - 1975

Sing Ha Net: 1968 - 1975

1/ : the data in 1970 is excluded.

Table A. 3-37 Extreme Water Level in the Vicinity of Project Area

Year	Phraya Banlu Regulator				Lakkhon Regulator				Sing Ha Nat Regulator											
	Outside (Suphan River)		Inside (Project Side)		Upstream (Project Side)		Outside (Chao Phra River)		Inside (Project side)											
	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.								
1966	2.73	20 Oct.	-0.05	8 Apr.	2.90	11 Oct.	0.90	20 Apr.	-	-	-	-	2.44	12 Nov.	0.75	11 Apr.				
1967	1.78	1 Jan.	-0.19	24 Mar.	2.82	31 Oct.	1.00	12 Apr.	-	-	1.73	25 Oct.	-	2.44	1 Nov.	0.65	23 May			
1968	1.60	15 Jan.	0.00	27 Mar.	2.82	31 Oct.	0.87	22 May	-	-	1.19	18 May	-0.72	1 Feb.	2.63	27 Oct.	0.47	30 Apr.		
1969	2.85	5 Nov.	0.10	1 Apr.	2.90	14 Nov.	0.80	3 Apr.	-	-	2.50	15 Oct.	-0.80	17 Feb.	2.97	20 Oct.	0.10	9 Jan.		
1970	3.14	5 Nov.	0.12	4 Apr.	3.16	31 Oct.	1.06	19 Jan.	-	-	2.91	25 Oct.	-0.80	21 Feb.	2.58	30 Oct.	0.60	30 Dec.		
1971	2.74	4 Nov.	0.10	6 Apr.	3.00	31 Oct.	0.70	9 Feb.	2.74	31 Oct.	0.92	15 May	1.54	7 Dec.	-0.60	26 Feb.	2.44	20 Oct.	0.64	13 May
1972	2.44	14 Dec.	0.38	11 May	2.91	12 Oct.	0.88	12 May	2.64	25 Oct.	0.92	31 May	1.67	13 Dec.	-0.60	15 Feb.	2.62	15 Nov.	0.52	3 Jan.
1973	2.54	19 Oct.	0.24	1 Apr.	2.90	22 Oct.	0.99	16 May	2.64	15 Nov.	0.74	16 May	1.91	20 Oct.	-0.65	5 Mar.	2.60	1 Nov.	0.87	6 Jun.
1974	3.22	28 Oct.	0.45	12 Mar.	3.22	28 Oct.	1.10	5 Feb.	2.76	1 Nov.	0.90	26 May	2.33	21 Oct.	-0.45	27 Feb.	3.01	30 Oct.	0.78	16 Mar.
1975	3.12	16 Nov.	0.35	25 Apr.	3.19	15 Nov.	1.06	9 Mar.	3.00	30 Oct.	0.86	16 Mar.	3.02	30 Oct.	-0.56	12 Feb.	-	-	-	-
1976	-	-	-	-	-	-	1.02	2 Mar.	-	-	-	-	1.93	17 Oct.	-0.30	16 Feb.	-	-	-	-
Mean	2.61		0.15		2.98		0.94		2.76		0.87		2.01		-0.61		2.63		0.59	
S	0.52		0.19		0.14		0.12		0.13		0.07		0.57		0.15		0.20		0.21	

Note: Based on the record of daily mean water level

S; Standard Deviation

Elevations of canal and river beds are EL.(-)2.00 m at the both regulator of Phraya Banlu and Sing Ha Nat

Source: Chao-chet Bangyechon and Phraya Banlu Project Offices, RID

FIGURE A. 3-11 ANNUAL FLUCTUATION OF FREE SURFACE GROUNDWATER TABLE  
IN UPPER WEST BANK TRACT

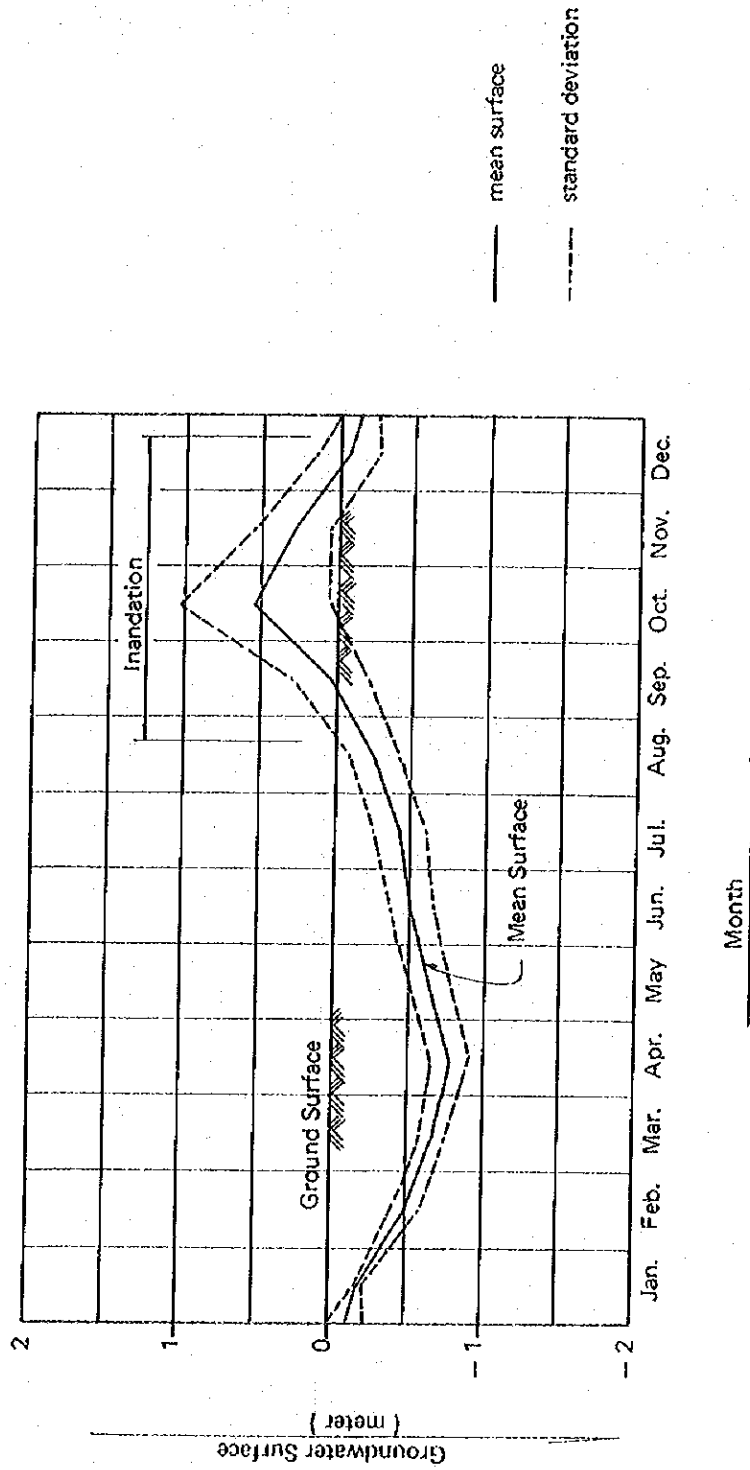


Table A. 3-38 Free Surface Ground Water Tables in Upper West Bank Tract (at Sena)

(Unit: m)

Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
1966	0.164	0.558	0.793	0.862	0.473	0.321	0.272	0.091	+0.405	+1.375	+0.645	0.357
1967	0.237	0.570	0.730	0.885	0.812	0.546	0.653	0.336	0.259	+0.617	+0.241	0.135
1968	0.148	0.329	0.597	0.654	0.567	0.588	0.609	0.494	0.097	+0.215	+0.250	+0.102
1969	0.196	0.562	0.664	0.880	0.773	0.726	0.369	0.226	0.081	0.052	0.081	0.094
1970	0.187	0.437	0.557	0.579	0.552	0.371	0.203	0.046	+0.270	+0.562	+0.452	+0.265
Mean ( $\bar{x}$ )	0.186	0.491	0.663	0.772	0.635	0.510	0.421	0.239	+0.048	+0.543	+0.303	0.044
Standard Deviation (S)	0.030	0.095	0.086	0.129	0.132	0.148	0.180	0.164	0.248	0.482	0.242	0.213

- Note:
1. All measurements show depth in meters from ground surface to ground water table.
  2. The measurements with mark of (+) are height from ground surface to inundation water surface.

Source: Technical Division, RID



Table A. 3-39 Water Quality of Canals in the West Bank Tract

Station	PH												
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Mean
Sena	7.2	7.2	7.2	7.3	7.4	7.5	7.4	7.1	7.3	7.4			
Bang Bua Thong	6.5	7.1	7.3	7.3	7.2	7.4	7.2	7.1	7.1	7.1	7.2	7.5	7.2
Mean	6.9	7.2	7.3	7.3	7.3	7.5	7.3	7.1	7.1	7.2	7.2	7.5	7.2

Station	Electrical Conductivity (ECx10 <sup>6</sup> , micromhos/cm at 20°C)												
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Mean
Sena	290	300	295	300	260	280	250	240	240	220			
Bang Bua Thong	270	550	380	340	700	600	600	900	530	280	280	220	220
Mean	250	425	338	320	480	540	430	570	407	467	280	220	394

Station	Nitrate, expressed as nitrogen (PPM)												
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Mean
Sena	nil	0.004	0.04	0.025	nil	0.004	0.012	0.005	0.010	0.016			0.012

Note: Data Period  
 Sena: Jan. - Oct. 1976  
 Bang Bua Thong: Sep. 1975 - Oct. 1976  
 Source: Sena Domestic Water Supply Station and Bang Bua Thong Domestic Water Supply Station

Table A. 3-40 Water Quality of the Chao Phya River

Station	PH							Mean
	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
Chai Nat				7.6	7.8			
Chao Phya Dam				7.3	7.5	7.6	8.2	
Sing Buri	8.5	7.7	7.8	7.4	7.9	7.9	8.7	
Ang Thong	8.2			7.5	7.7	7.9	8.3	
Ayutthaya	8.8	7.5	8.3	7.4	7.5	7.7	8.5	
Bang Sai		7.5	8.2					
Pathum Thani				7.5	7.9	7.7	8.7	
Non Tha Buri				7.3	8.0	7.6	8.5	
Mean	8.50	7.57	8.10	7.43	7.76	7.73	8.48	7.94
Standard Deviation (S)	0.24	0.09	0.22	0.10	0.18	0.12	0.19	

Station	Electrical Conductivity (ECx10 <sup>6</sup> , micromhos/cm at 25°C)							Mean
	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
Chai Nat				115	128			
Chao Phya Dam				116	115	138	178	
Sing Buri	132	129	111	118	122	136	136	
Ang Thong	136			118	116	135	154	
Ayutthaya	150	123	135	128	105	130	155	
Bang Sai		124	136					
Pathum Thani				123	117	132	154	
Non Tha Buri				123	129	139	161	
Mean	139	125	127	120	119	135	156	132
(S)	8	3	12	4	8	3	12	

Station	Total Solids (PPM)							Mean
	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
Chai Nat				368	165			
Chao Phya Dam				375	172	262	167	
Sing Buri	-	663	468	433	209	240	222	
Ang Thong	-			467	241	219	258	
Ayutthaya	-	824	469	306	139	142	259	
Bang Sai		341	522					
Pathum Thani				342	167	161	255	
Non Tha Buri				208	181	164	261	
Mean		609	486	357	182	198	237	345
(S)		201	25	79	31	45	34	

Note: Observed in 1961  
S; Standard deviation

Source: Enclosed Data on Water Analysis, Greater Chao Phya Project  
Refer to Lab. No.194/1962, RID

## Water Quality of the Chao Phya River (cont'd)

Station	Total dissolved Solids (PPM)							Mean
	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
Chai Nat				126	112			
Chao Phya Dam				115	113	131	143	
Sing Buri	129	115	81	98	117	139	114	
Ang Thong	106			86	103	138	98	
Ayutthaya	132	110	125	98	95	116	124	
Bang Sai		126	146					
Pathum Thani				100	99	124	144	
Non Tha Buri				98	126	125	138	
Mean	122	117	117	103	110	129	127	118
(S)	12	7	27	12	11	8	17	

Station	Suspended Solids (PPM)							Mean
	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
Chai Nat				242	53			
Chao Phya Dam				260	59	131	24	
Sing Buri	-	547	407	335	92	101	98	
Ang Thong	-	381		381	138	81	160	
Ayutthaya	-	714	336	217	44	28	123	
Bang Sai		268	377					
Pathum Thani				241	68	37	121	
Non Tha Buri				110	55	38	134	
Mean		478	373	255	73	69	110	226
(S)		169	29	80	30	38	43	

Station	Calcium (PPM)							Mean
	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
Chai Nat				14.56	14.59			
Chao Phya Dam				15.10	13.93	17.32	18.14	
Sing Buri	16.17	17.35	15.27	14.89	14.73	17.29	16.27	
Ang Thong	14.76			16.04	13.02	14.38	18.50	
Ayutthaya	17.12	13.81	16.49	17.53	12.91	13.36	19.04	
Bang Sai		15.29	16.93					
Pathum Thani				16.69	15.17	13.36	18.55	
Non Tha Buri				16.53	14.48	14.61	19.32	
Mean	16.02	15.48	16.23	15.91	14.12	15.05	18.31	15.87
(S)	0.97	1.45	0.70	1.01	0.80	1.66	0.99	

FIGURE A.3-12 GEOLOGICAL MAP OF THE SOUTHERN CENTRAL PLAIN

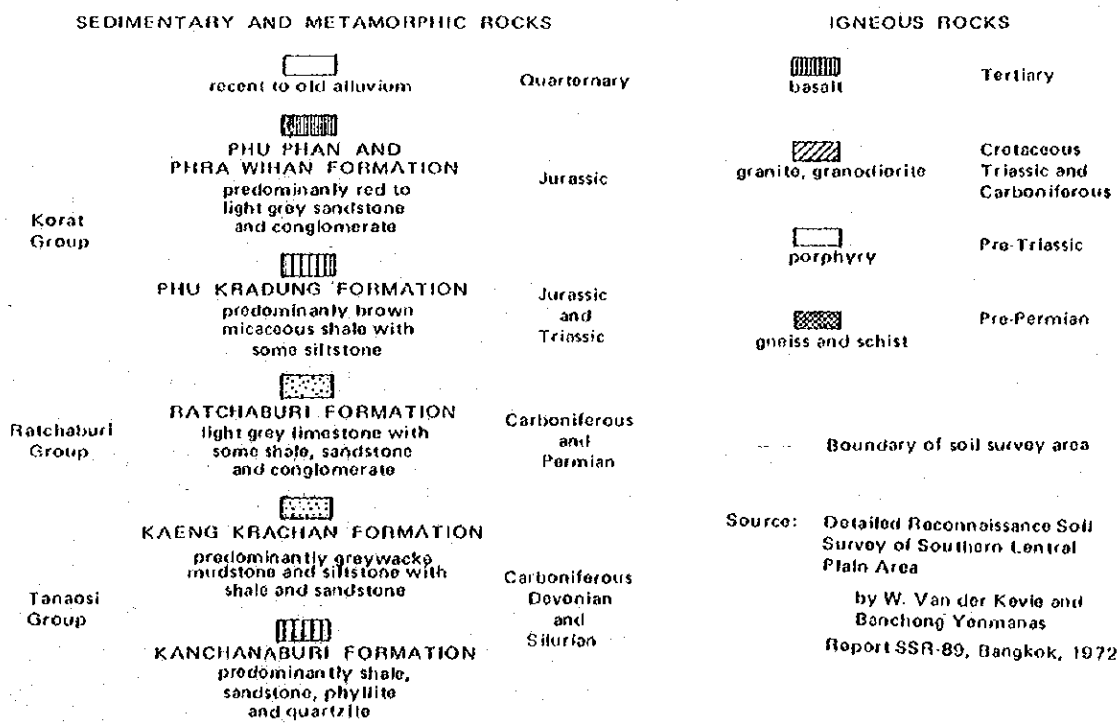
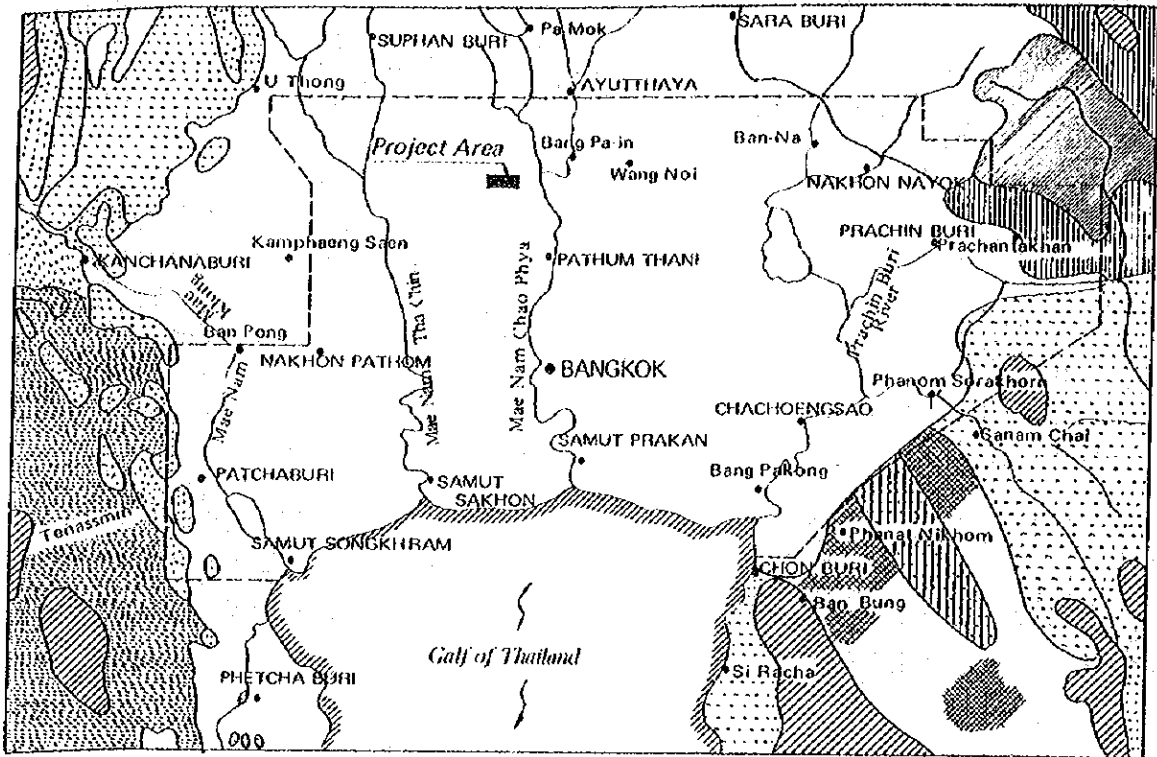
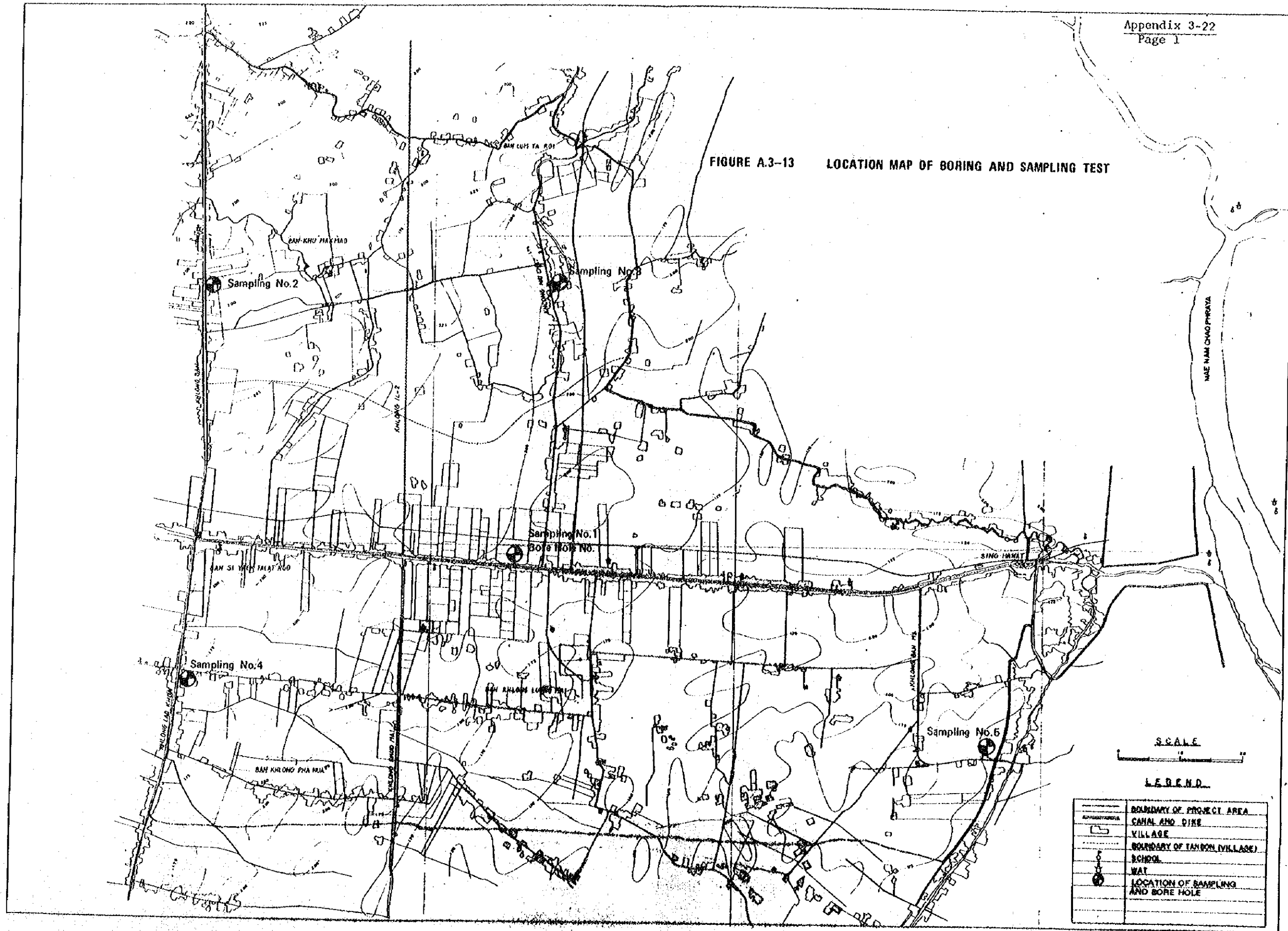


FIGURE A.3-13 LOCATION MAP OF BORING AND SAMPLING TEST



SCALE

LEGEND

	BOUNDARY OF PROJECT AREA
	CANAL AND DIKE
	VILLAGE
	BOUNDARY OF TAMBON (VILLAGE)
	SCHOOL
	WAT
	LOCATION OF SAMPLING AND BORE HOLE

FIGURE A.3-14 SOIL AND MATERIAL LABORATORY BORING LOG

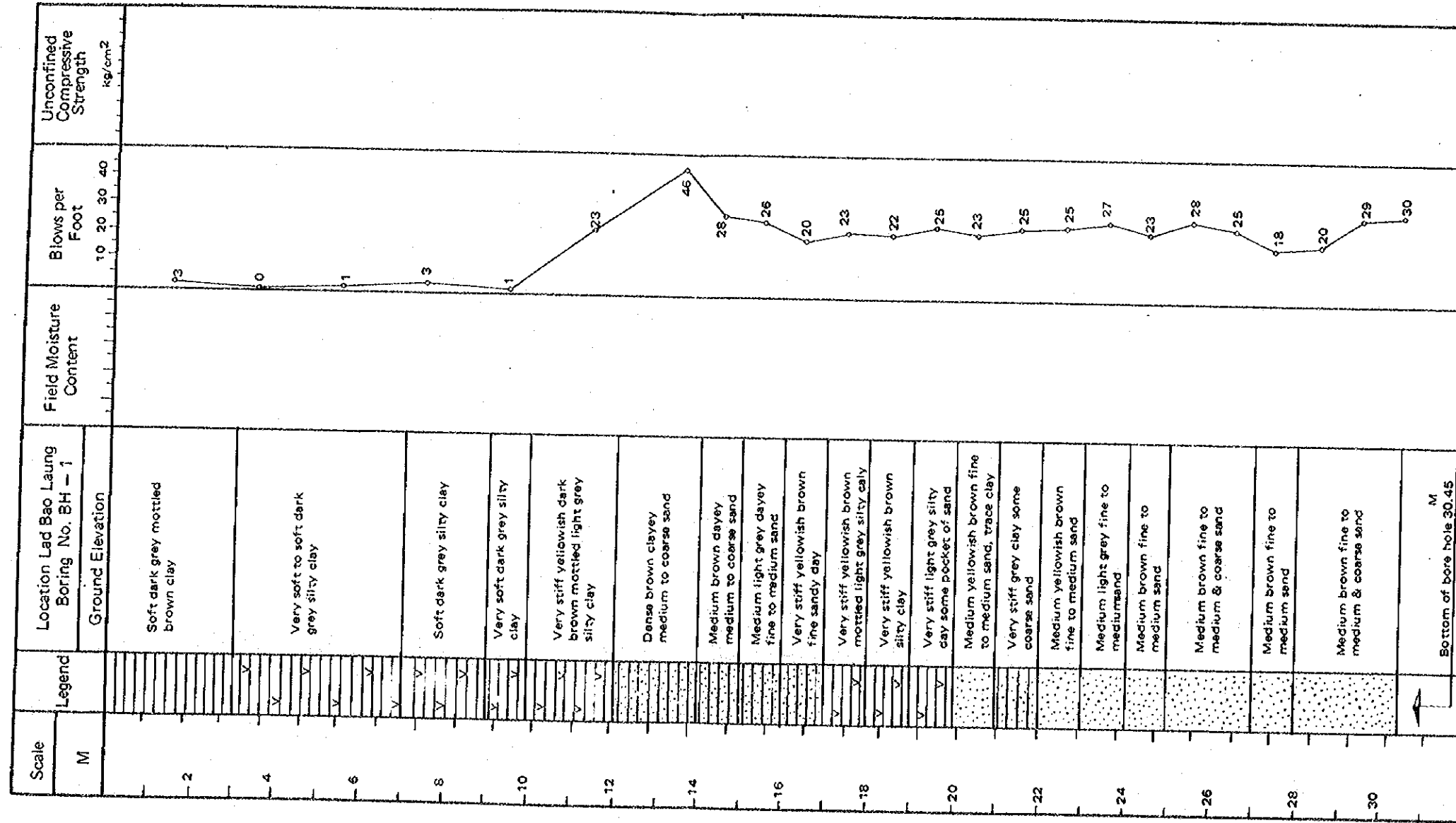
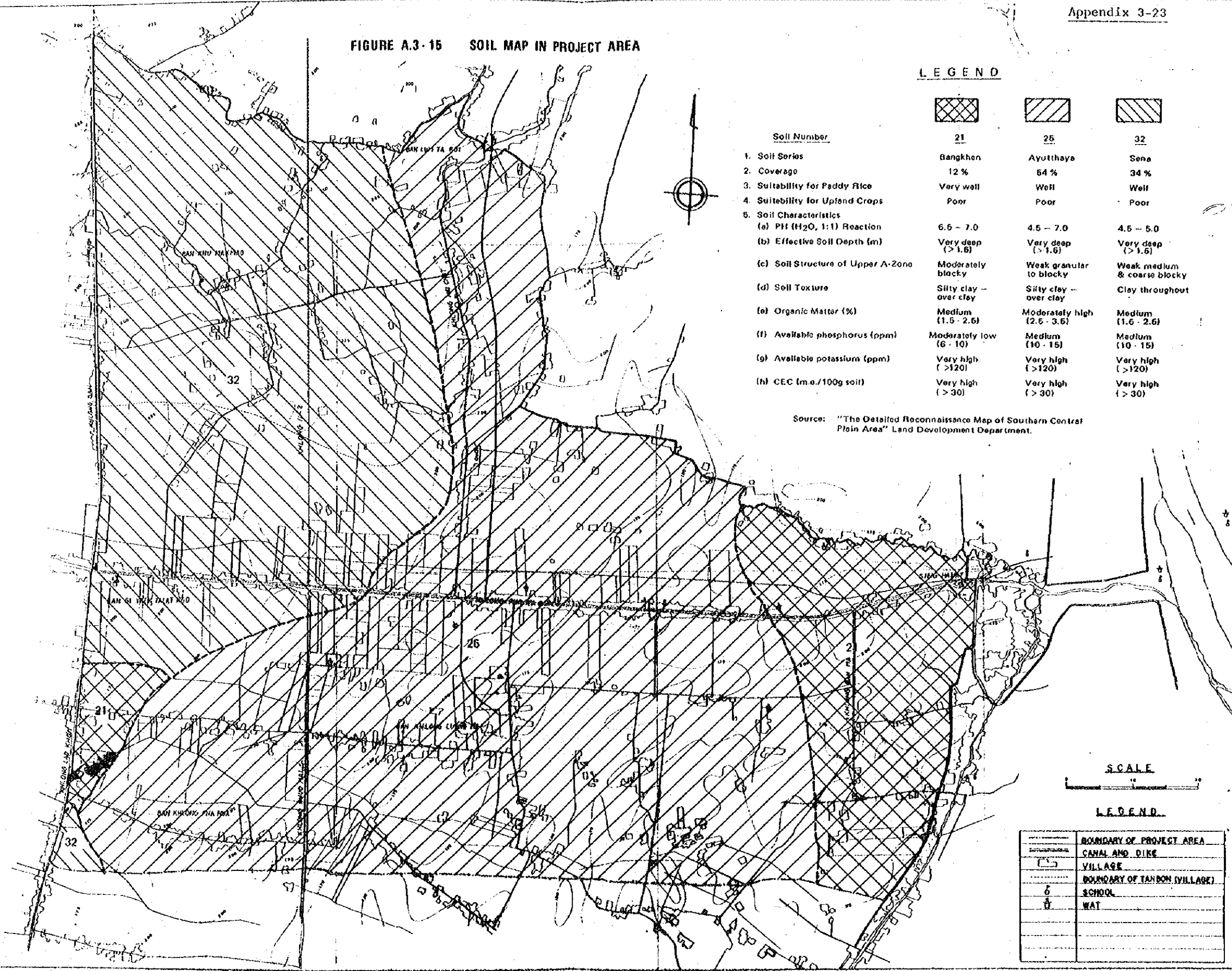


Table A. 3-41 Results of Soil Mechanical Test

<u>Item</u>	<u>No.1</u>	<u>No.2</u>	<u>No.3</u>	<u>No.4</u>	<u>No.5</u>	<u>Remarks</u>
Natural moisture content (%)	76.4	60.2	54.2	42.0	22.2	
Consistency						
Liquid limit WL (%)	84.2	74.0	92.0	85.2	71.5	
Plastic limit Wp (%)	34.6	32.8	36.9	32.0	31.1	
Shrinkage limit Ws (%)	28.3	24.0	23.8	23.8	28.6	
Plasticity index Ip (%)	49.6	41.2	55.1	54.2	40.4	Ip = WI - Wp
Grain size analysis						
Gravel (%)	1	-	-	-	1	4.76 mm over
Sand (%)	6	10	4	4	1	4.76 - 0.074 mm
Silt and clay (%)	93	90	96	96	98	0.74 mm under
Specific gravity	2.57	2.58	2.56	2.54	2.58	
Natural unit density ( $\tau/m^3$ )	1.64	1.84	1.89	1.64	1.82	
Classification of soil (USCS)	CH	CH	CH	CH	CH	Inorganic clays of
(USDA)	Silty clay	Silty clay	Silty clay	Silty clay	Silty clay	high plasticity

Note: USCS: base on Unified Soil Classification  
USDA: base on U.S. Bureau of Soil Triangular Classification Chart

FIGURE A.3-15 SOIL MAP IN PROJECT AREA



LEGEND

Soil Number	21	25	32
1. Soil Series	Bangkhen	Ayutthaya	Sena
2. Coverage	12 %	54 %	34 %
3. Suitability for Paddy Rice	Very well	Well	Well
4. Suitability for Upland Crops	Poor	Poor	Poor
5. Soil Characteristics			
(a) PH (H <sub>2</sub> O, 1:1) Reaction	6.6 - 7.0	4.5 - 7.0	4.5 - 5.0
(b) Effective Soil Depth (m)	Very deep (>1.6)	Very deep (>1.6)	Very deep (>1.6)
(c) Soil Structure of Upper A-Zone	Moderately blocky	Weak granular to blocky	Weak medium & coarse blocky
(d) Soil Texture	Silty clay - over clay	Silty clay - over clay	Clay throughout
(e) Organic Matter (%)	Medium (1.5 - 2.6)	Moderately high (2.6 - 3.6)	Medium (1.6 - 2.6)
(f) Available phosphorus (ppm)	Moderately low (6 - 10)	Medium (10 - 15)	Medium (10 - 15)
(g) Available potassium (ppm)	Very high (>120)	Very high (>120)	Very high (>120)
(h) CEC (m.e./100g soil)	Very high (>30)	Very high (>30)	Very high (>30)

Source: "The Detailed Reconnaissance Map of Southern Central Plain Area" Land Development Department.

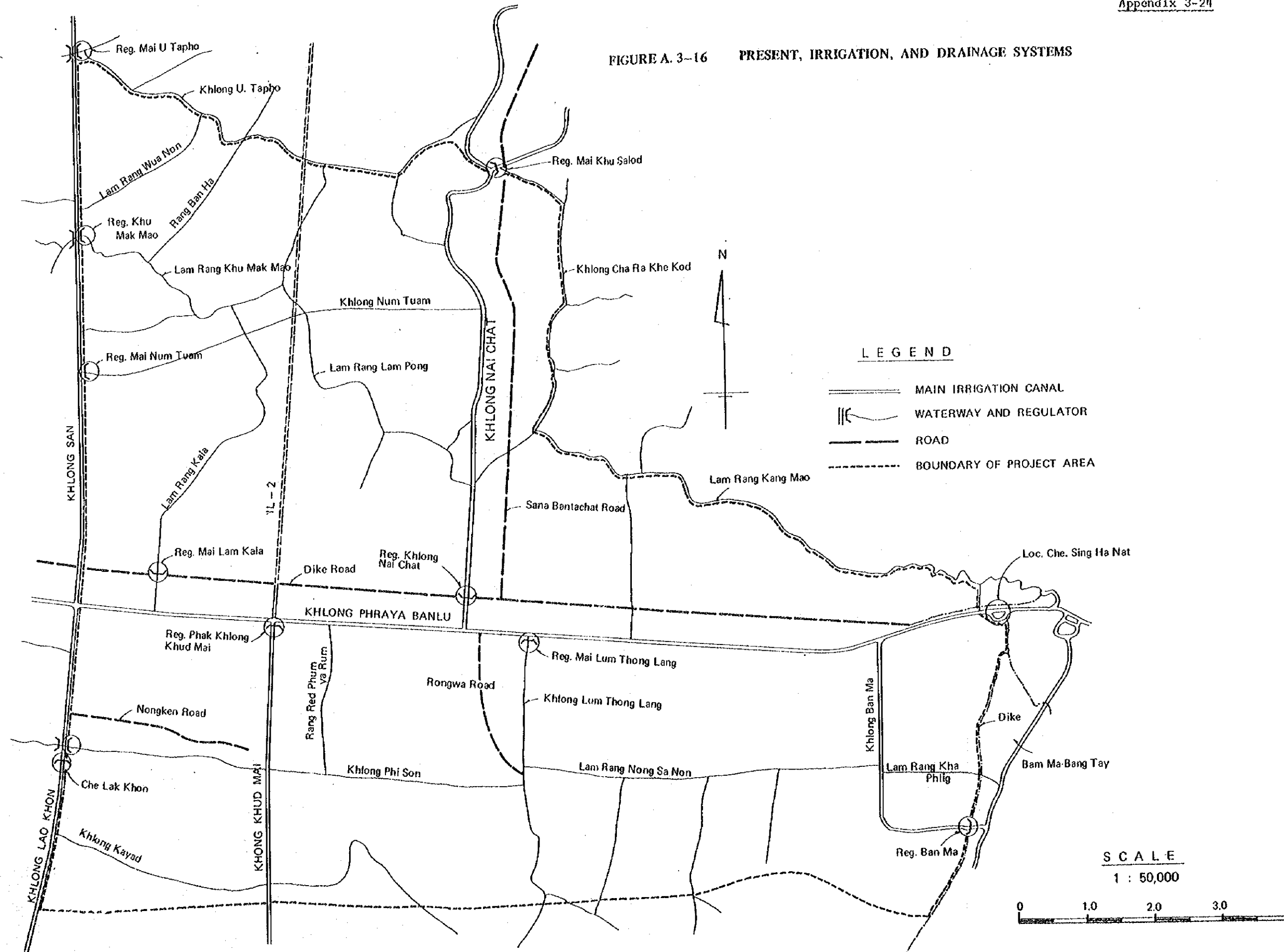
SCALE  
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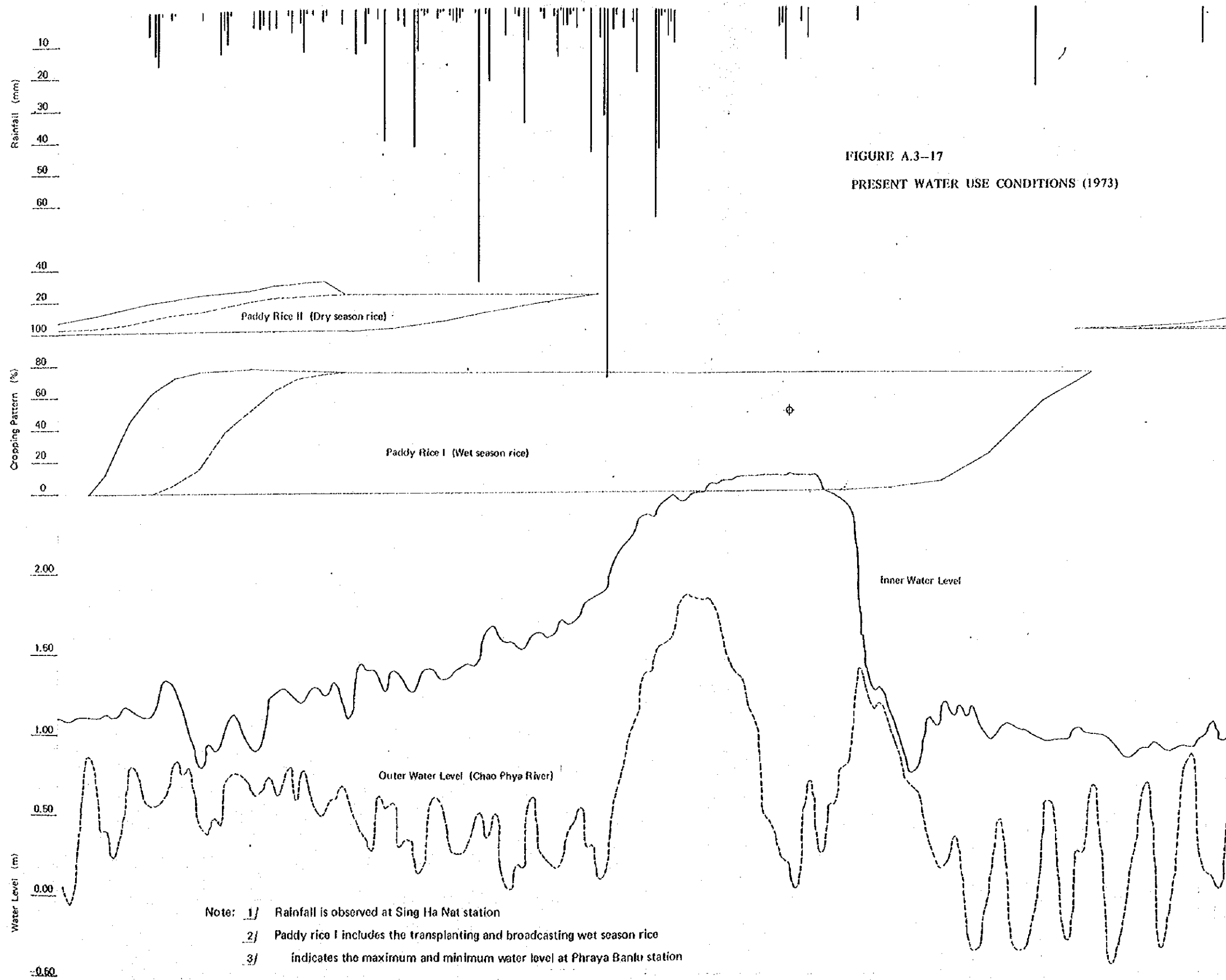
LEGEND

-----	BOUNDARY OF PROJECT AREA
-----	CANAL AND DIKE
-----	VILLAGE
-----	BOUNDARY OF TAMBON (VILLAGE)
○	SCHOOL
⊕	WAT



FIGURE A. 3-16 PRESENT, IRRIGATION, AND DRAINAGE SYSTEMS





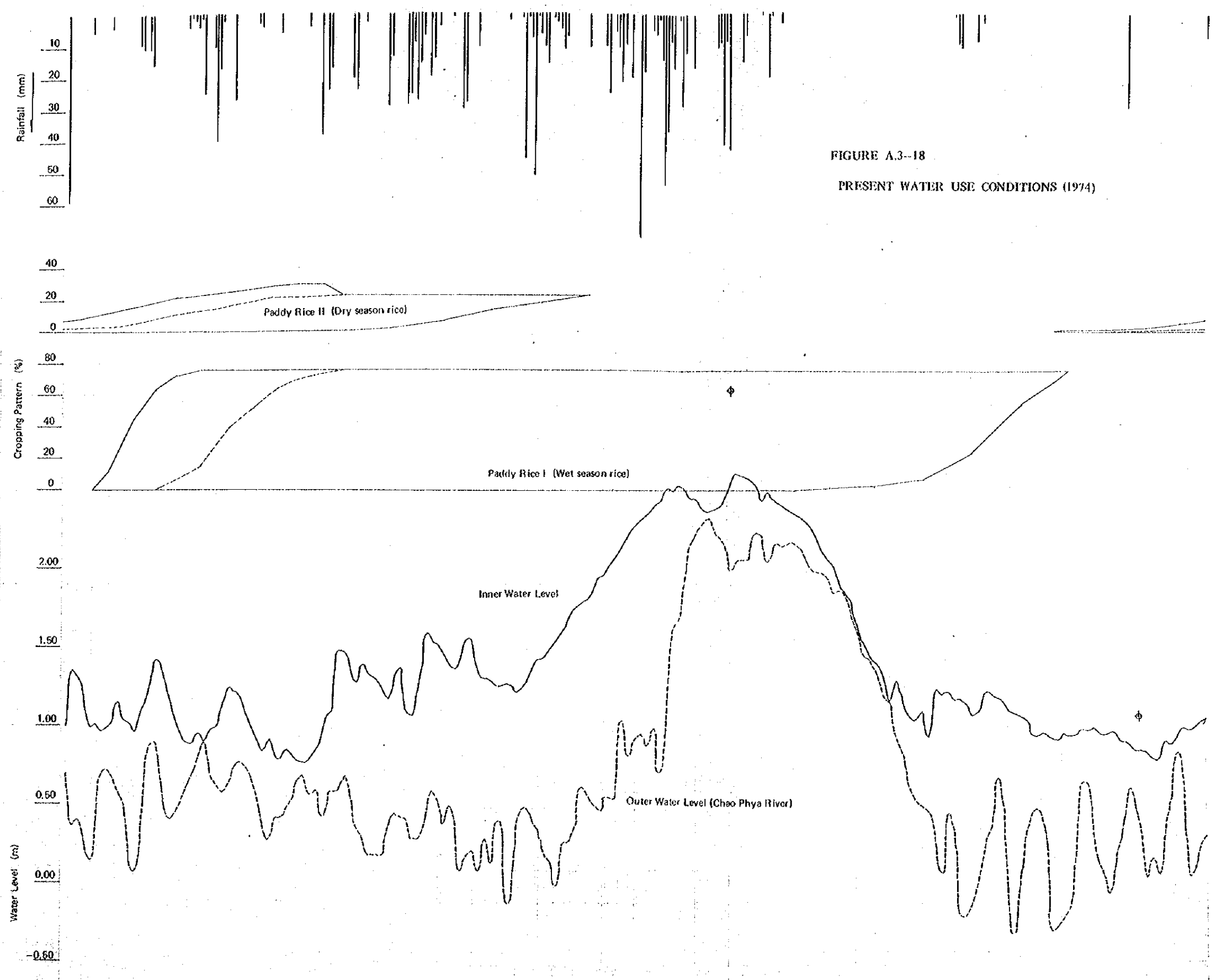


FIGURE A.3-18  
PRESENT WATER USE CONDITIONS (1974)

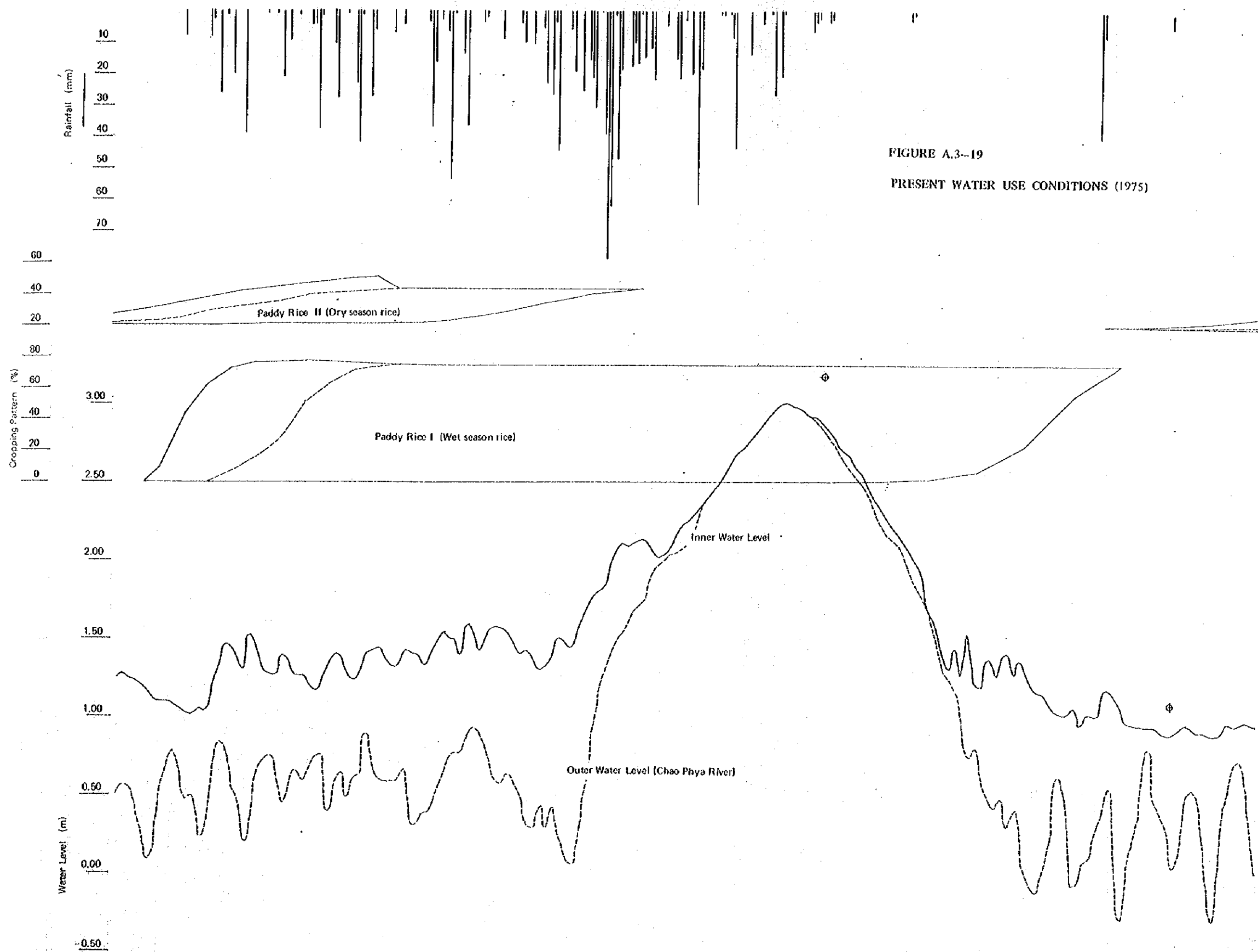


FIGURE A.3-19  
PRESENT WATER USE CONDITIONS (1975)

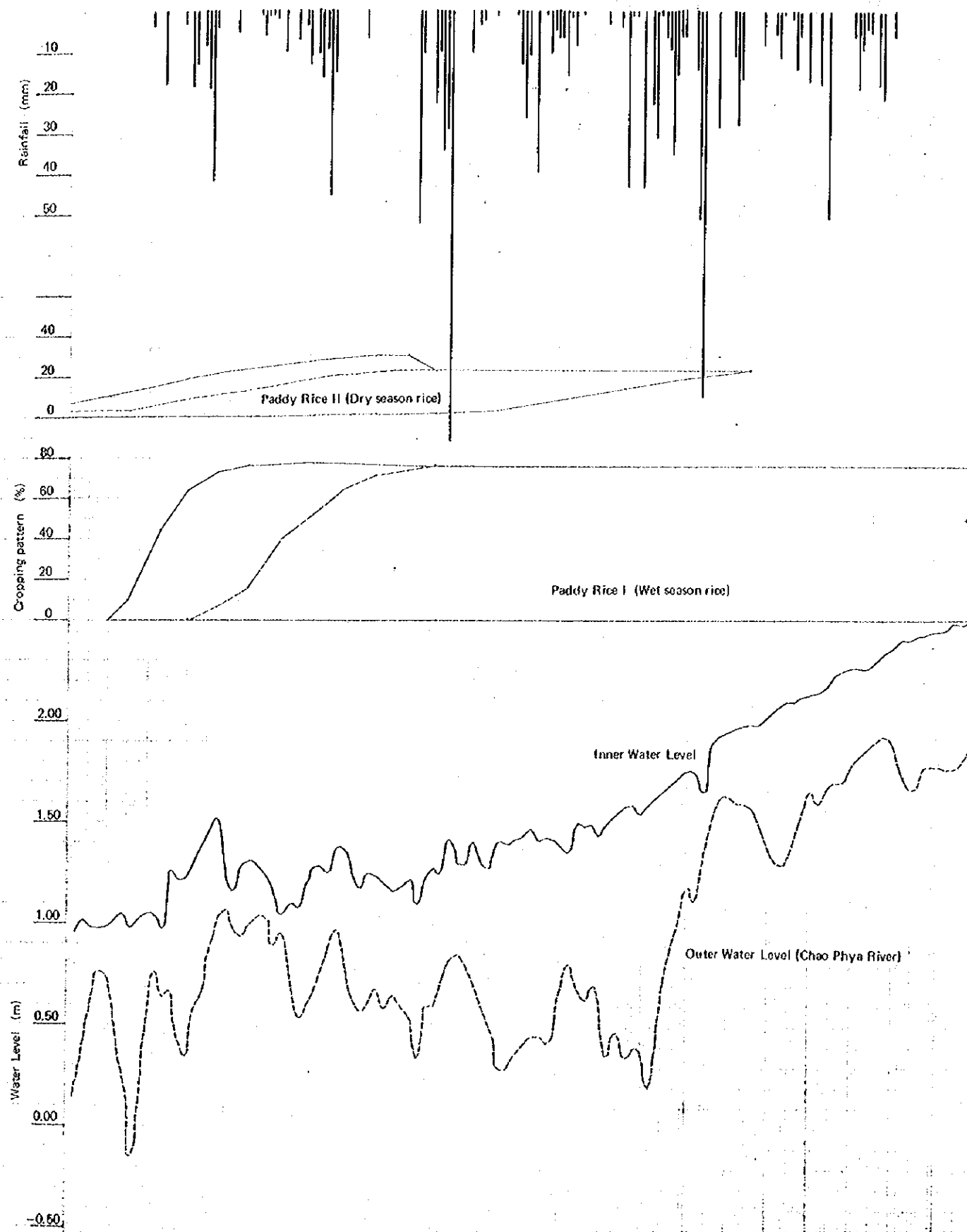
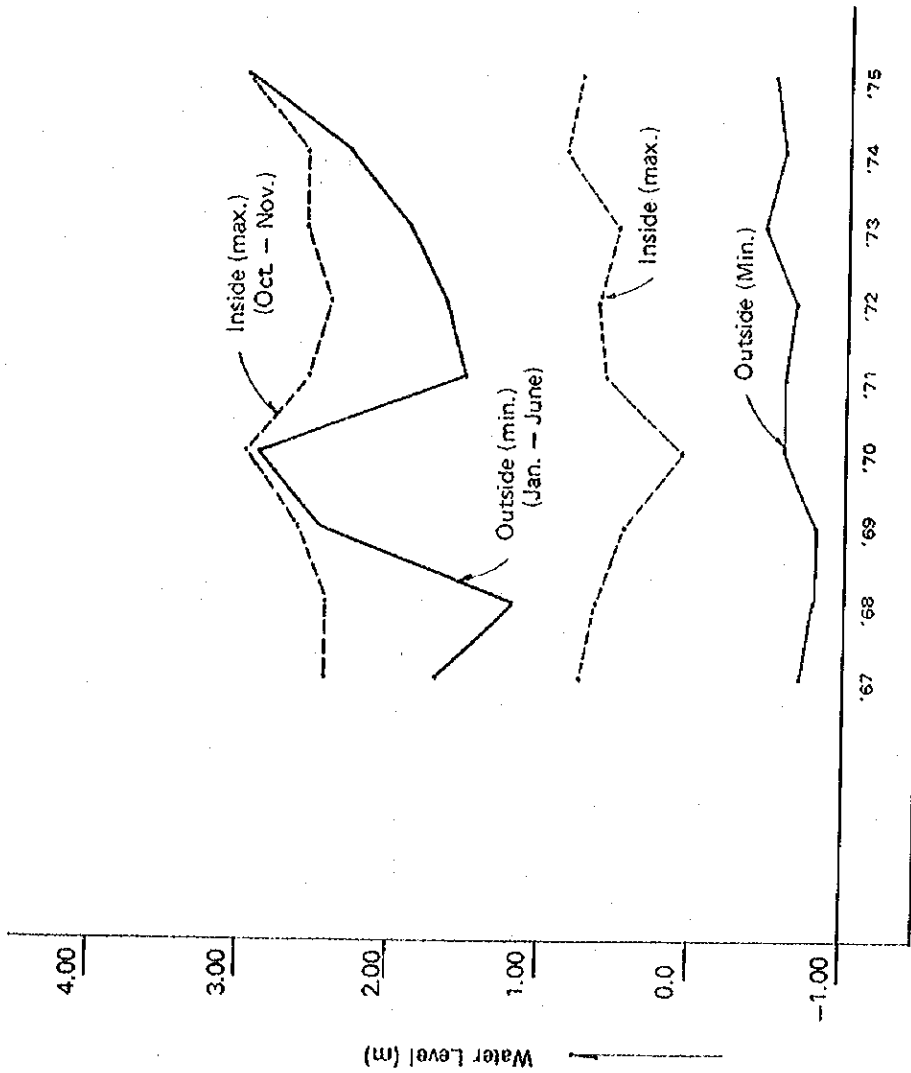


FIGURE A.3-20  
PRESENT WATER USE CONDITIONS (1976)

Month	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Rainfall (mm)	15	20	25	30	35	40	45	50	45	35	25	15
Paddy Rice I (%)	0	10	20	35	50	65	75	80	80	80	80	80
Paddy Rice II (%)	0	0	0	0	0	0	0	0	0	0	0	0
Inner Water Level (m)	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1
Outer Water Level (Chao Phya River) (m)	0.5	0.6	0.7	0.8	0.9	1.0	1.1	1.2	1.3	1.4	1.5	1.6

FIGURE A-321 MAXIMUM & MINIMUM WATER LEVEL AT SING HA NAT REG.



Year	INSIDE		OUTSIDE	
	Max. W.L.	Min. W.L.	Max. W.L.	Min. W.L.
1967	2.44	0.75	1.73	-0.72
1968	2.44	0.65	1.19	-0.80
1969	2.63	0.47	2.50	-0.80
1970	2.97	0.10	2.91	-0.60
1971	2.58	0.60	1.54	-0.60
1972	2.44	0.64	1.67	-0.65
1973	2.62	0.52	1.91	-0.45
1974	2.60	0.87	2.33	-0.56
1975	3.01	0.78	3.02	-0.50
Mean	2.64	0.60	2.09	-0.63

Source: Chao-ched Bangyeehon Irrigation Office

Inside: Inside of regulator

Outside: Outside of regulator (Chao Phya river)

Table A. 3-42 Proposed Land Consolidation Area and Completed Area in Thailand in 1976

Project	Area (ha)	Implemented area (ha)					Total
		1971	1972	1973	1974	1975	
Suppeya	7,200	124	162	346	213	459	368
Channasutra (I)	9,600						
Channasutra (II)	56,700	176	531	1,149	1,216	2,368	1,168
Nong Wai	12,000	-	-	-	-	-	160
<b>Total</b>	<b>85,500</b>	<b>300</b>	<b>693</b>	<b>1,495</b>	<b>1,429</b>	<b>2,807</b>	<b>1,696</b>
							<b>8,420</b>

Source: Central Land Consolidation Office

Table A. 3-43 Comparison of Major Features of Sappaya Project and Channasutra Project

<u>Items</u>	<u>Sappaya Project</u>	<u>Channasutra Project</u>	<u>Remarks</u>
Project area (ha)	7,200	Stage (I) : 9,600 Stage (II) : 56,700 <u>66,300</u>	
Construction periods	1971 - 1980	Stage (I) : 1971-1977 Stage (II) : 1978-1982	
Implemented area (ha)	1,652 (23 %)	5,608 (69 %)	
Construction cost (\$/ha)	2,200	1,500	
Major Features of On-Farm			Exclusive of depreciation cost of machineries
Cost per ha (\$/ha)	720 (2,380 B/rai)	470 (1,500 B/rai)	
size of plot (ha)	1.0 - 0.15	6.0 - 0.15	
length of Run (m)	130 - 150	200 - 250	
width of parcel (m)	15 - 70	10 - 240	
ratio of roads (m/ha)	72.5 (11.6 m/rai)	36.3 (5.8 m/rai)	
ratio of canal (m)	98.1 (15.7 m/rai)	68.1 (10.9 m/rai)	
Earth moving volume (m <sup>3</sup> /ha)	481 (77 m <sup>3</sup> /rai)	313 (50 m <sup>3</sup> /rai)	
Width of road (m)	3.0, 4.0, 6.0	4.0, 5.5	

Source: Royal Irrigation Department, Central Land Consolidation Office



FIGURE A. 3-22 TYPICAL LAYOUT OF LAND CONSOLIDATION  
PLAN IN SAPPAYA PROJECT  
SCALE 1:10,000

LEGEND

- TORN OUT GATE
- +— DIVISION BOX
- - - MINOR DRAIN
- ==== NEW PLOT BOUNDARY
- ==== MINOR ROAD

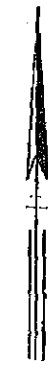
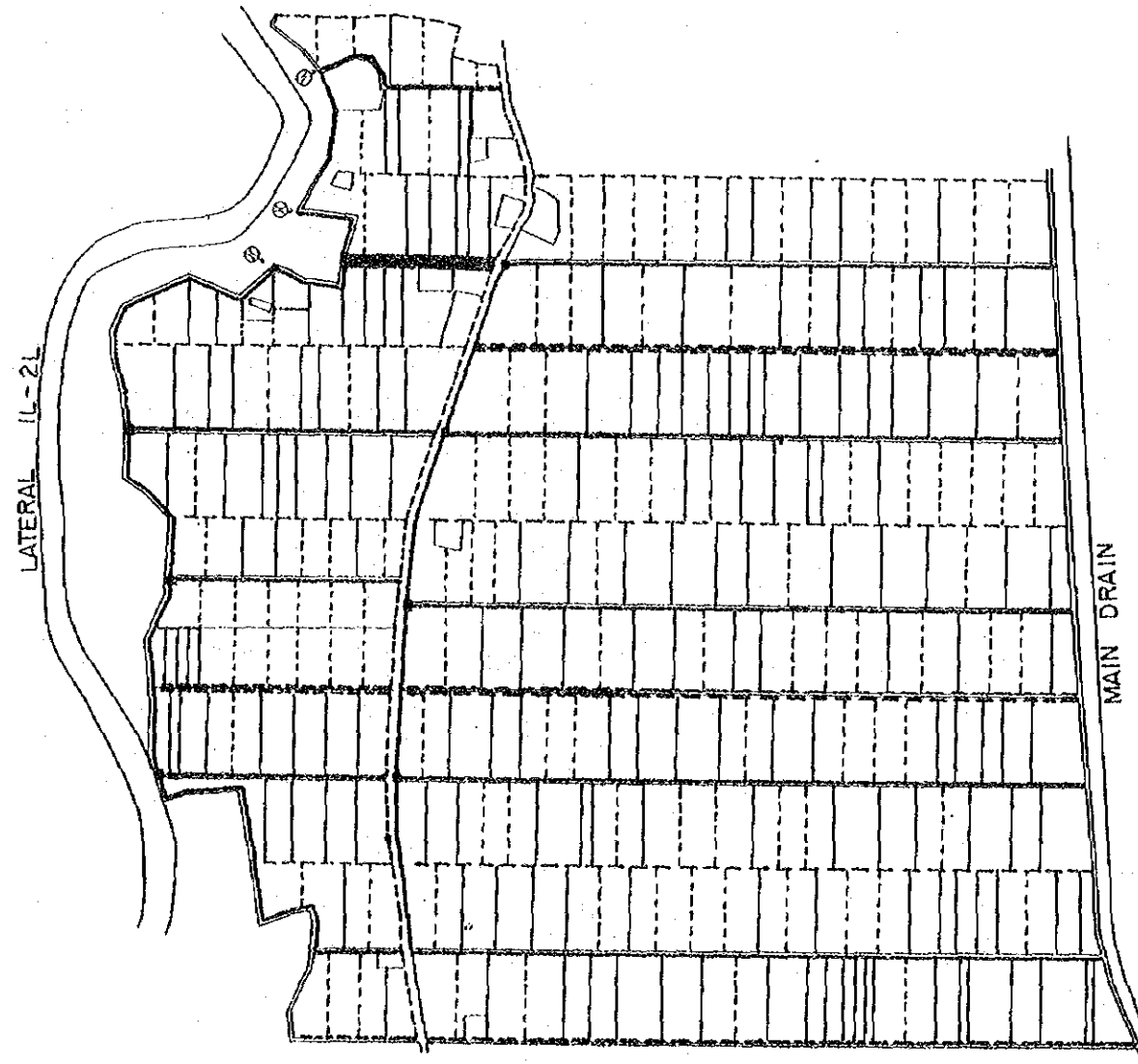



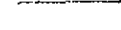

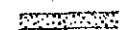



FIGURE A.3-23 TYPICAL LAYOUT OF LAND CONSOLIDATION  
PLAN IN CHANNASUTRA PROJECT  
(ALTERNATIVE 2)  
SCALE 1:10,000

LEGEND

-  IRRIGATION DITCH WITH EXISTING OFFTAKE
-  DIVISION BOX
-  MINOR DRAIN
-  FARM ROAD
-  CADASTRAL BOUNDARY
-  CROSS REGULATOR
-  EXISTING MAIN DRAIN

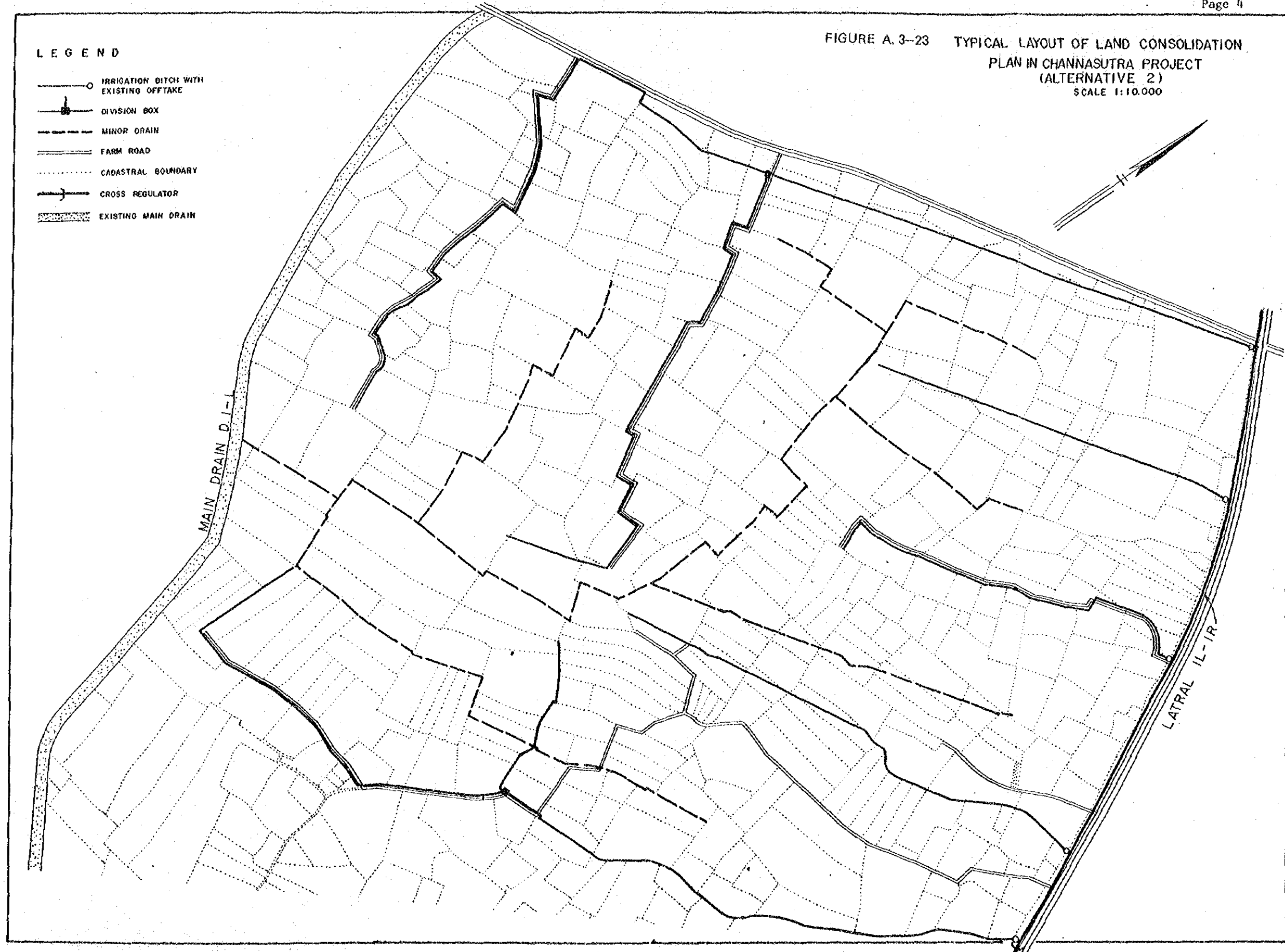


FIGURE A. 3-24 TYPICAL LAYOUT OF LAND CONSOLIDATION  
PLAN IN CHANNASUTRA PROJECT (ALTERNATIVE 4 AND 5)  
SCALE 1:10,000

- LEGEND
- IRRIGATION DITCH WITH CONSTANT HEAD ORIFICE
  - DIVISION BOX
  - — MINOR DRAIN
  - — NEW PLOT BOUNDARY
  - — MINOR ROAD
  - CADASTRAL BOUNDARY
  - >— CROSS REGULATOR
  - ▨ EXISTING MAIN DRAIN

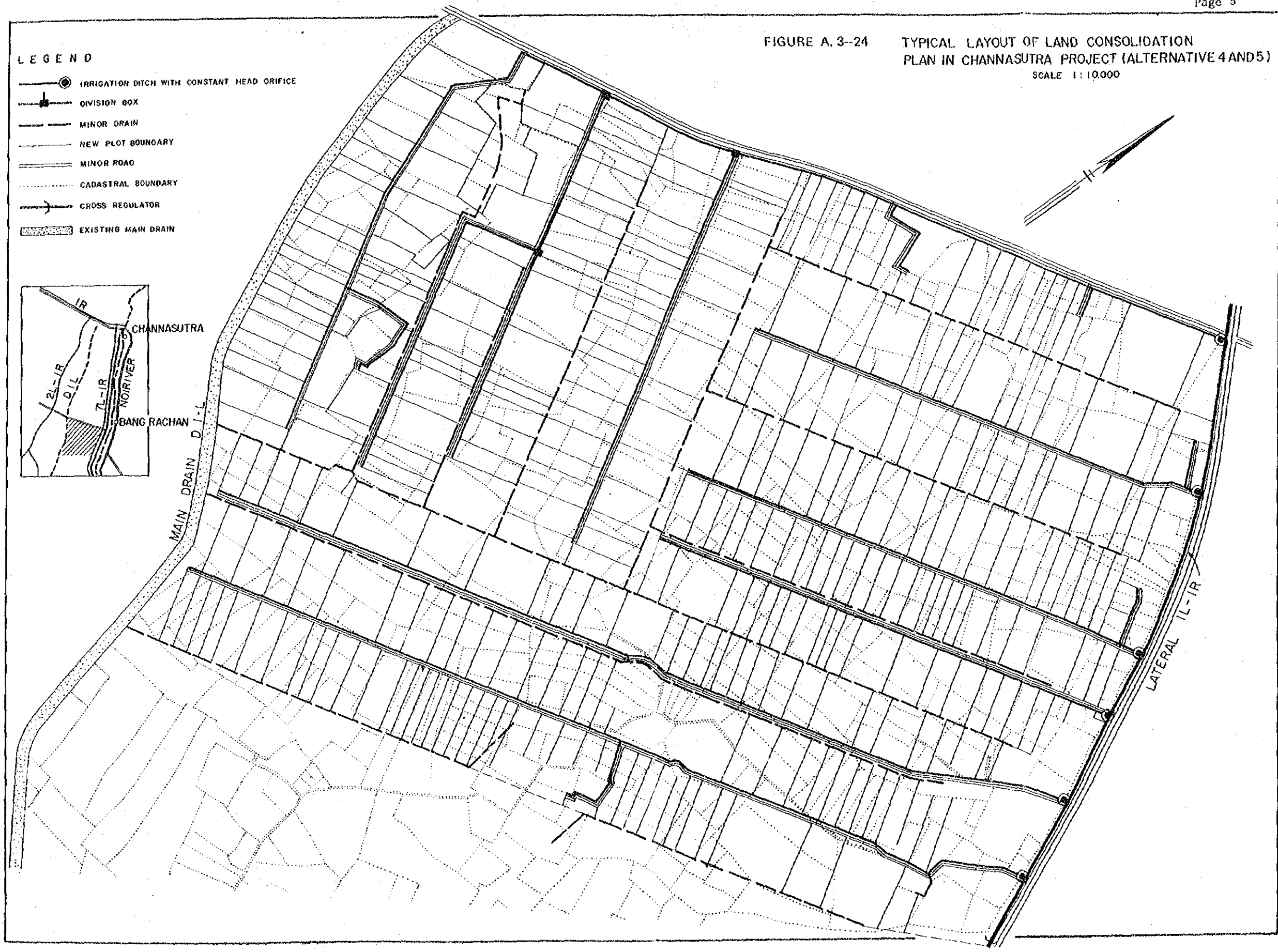
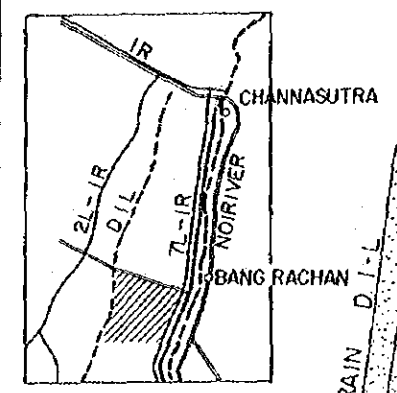
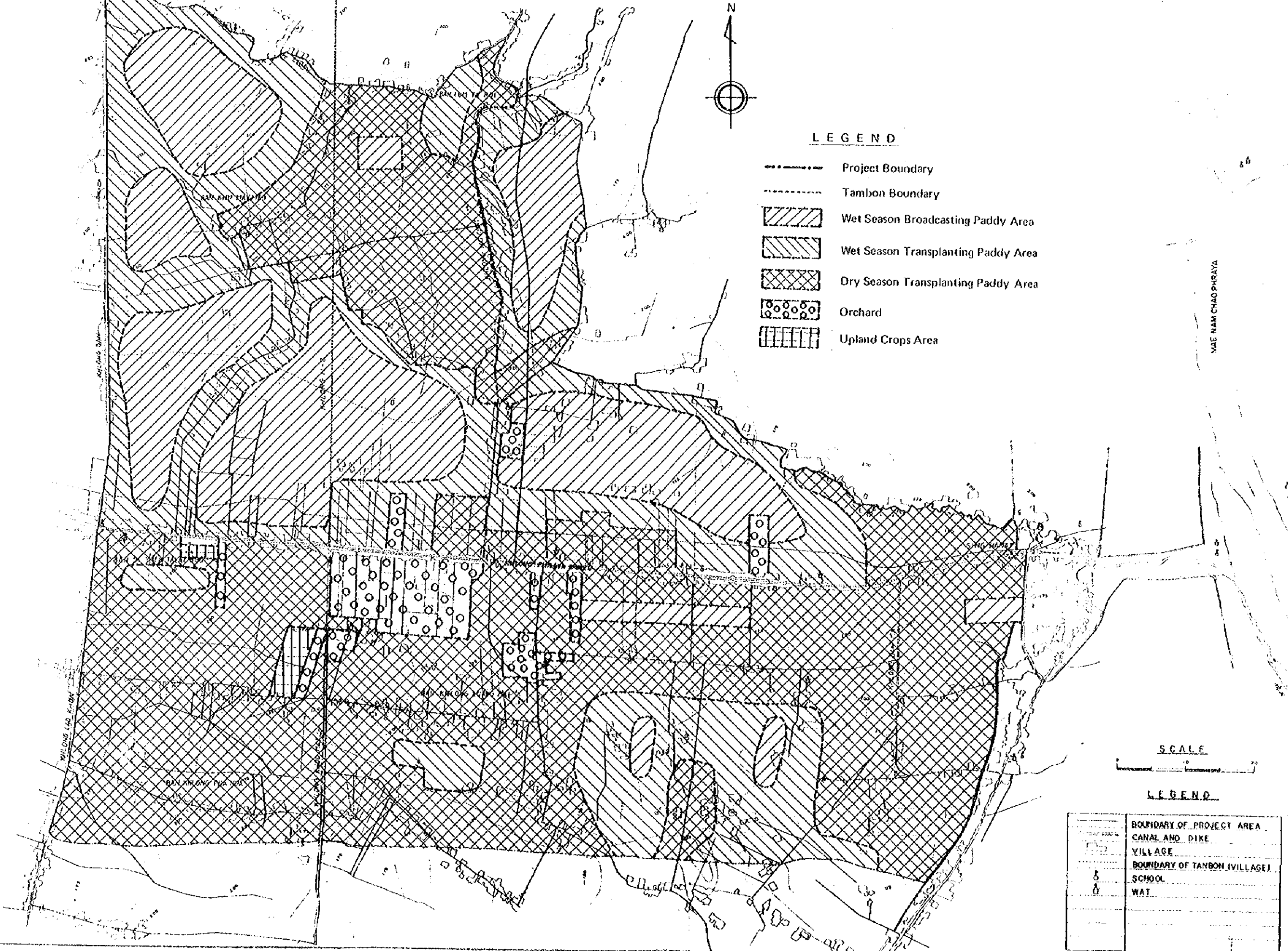


FIGURE A.3-25 PRESENT SIZE AND SHAPE OF FARM FIELD



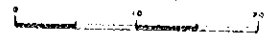
FIGURE A.3-25-1 PRESENT LAND USE MAP



LEGEND

- Project Boundary
- - - - - Tambon Boundary
- ▨ Wet Season Broadcasting Paddy Area
- ▧ Wet Season Transplanting Paddy Area
- ▩ Dry Season Transplanting Paddy Area
- ⊙ Orchard
- ▤ Upland Crops Area

SCALE



LEGEND

---	BOUNDARY OF PROJECT AREA
- - - - -	CANAL AND DIKE
▭	VILLAGE
- - - - -	BOUNDARY OF TANBON (VILLAGE)
⊙	SCHOOL
⊙	WAT

### Crop Yield

#### Yield of Paddy Rice

##### Yield in Ayutthaya Province

As shown in Table A. 3-40, the Paddy Yields in the Ayutthaya Province are lower than those in the Central Plain as well as whole Thailand. There are about one million hectares of paddy cultivated for the floating type paddy in Thailand, and these are concentrated to the lower Chao Phya area. In the Ayutthaya Province, especially, this type of paddy occupies about 88 per cent of the paddy cropping area, because of low-lying area with flooding on in the wet season.

The yield of floating paddy rice, in general, is quite low, consequently, the average yield in the Ayutthaya Province becomes low when compared with those in Central Plain or Thailand.

##### Yield in Project Area

The paddy field in the Project Area is cultivated with two kinds of paddy cultivation, and the yield of each kind of that is shown below, based on the collected data and field survey conducted by the mission.

#### Wet Season broadcasting paddy

The best paddy field in the Ayutthaya Province is cultivated with broadcasting paddy, so that, it is reasonable to make assumption that the paddy yield of about 1.6 ton/ha in Ayutthaya Province for the years from 1963 to 1972 can be regarded as that in the Project Area. This figure is approximately same to the result of the field survey conducted.

#### Transplanting paddy

In the transplanting paddy field, high yielding varieties are used under the application of fertilizer and agricultural

Table A. 3-44 Statistical Data on Paddy Yield

Statistic	Year	Planted Area (ha)	Damaged Area (ha)	Harvested Area (ha)	Production (ton)	Average
						Yield/ (ton/ha)
Ayutthaya Province/ (1963 - 1972)	1963	217,475	5,028	212,446	362,566	1.67
	1964	220,754	99,984	120,770	149,493	0.68
	1965	220,455	13,966	206,489	350,190	1.59
	1966	220,200	11,656	208,544	374,638	1.70
	1967	219,797	17,245	202,552	383,583	1.75
	1968	220,855	16,558	204,299	331,136	1.50
	1969	220,435	37,742	182,693	361,788	1.64
	1970	215,839	33,837	182,002	385,274	1.79
	1971	219,317	5,452	213,865	401,398	1.83
	1972	212,819	13,670	199,149	351,322	1.65
Total	2,187,947	255,138	1,932,809	3,451,388	15.80	
Mean	218,795	25,514	193,281	345,139	1.58	
Annual Rice Production in Central Plain <sup>2/</sup> (1962 - 1975)	2,219,000	N.A.	N.A.	4,190,000	1.89	
Annual Rice Production in Thailand <sup>3/</sup> (1962 - 1975)	7,342,000	551,000	6,791,000	12,397,000	1.69	

Source: <sup>1/</sup> Dep. of Agricultural Extension, MOAC  
("Statistical Reports of Ayutthaya Province", National Statistical Office)

<sup>2/</sup>, <sup>3/</sup> Div. of Agricultural Economics, MOAC  
("Agricultural Statistics of Thailand, Crop Year 1975/76")

<sup>4/</sup> Basing on planted area.

Table A. 3-45 Data on Paddy Yield in the Project Area

Data Source	Paddy Yield			Remarks	
	Season	No. of surveyed farm families	Planted area		Yield
(1) The survey on socio-economic conditions of farmers in Lad Rao Laung district. (Div. of Research & Planning, ALRO, Oct. 1976)	Wet	} 50	157 ha	1.20 ton/ha	Broadcasting
	Dry		164 ha	*2.48 ton/ha	Transplanting
(2) Statistical survey in each village (Muban) concerned with Project Area (Statistical Office of Ayutthaya province, 1976)	Tambon	Yield	Coverage of transplanting paddy		Number in parenthesis: % of dry season trans-planting paddy
	Khusalod	2.9 ton/ha	67%	(31%)	
	Phraya Banlu	2.5 ton/ha	54%	(20%)	
	Sing Ha Nat	*2.9 ton/ha	80%	(60%)	
(3) Agro-economy survey conducted by this Mission in Project area	Crop	No. of surveyed farmers	Planted area	Yield	Crop year: 1975/76
	Wet season broadcasting	6	33.0 ha	1.0 ton/ha	
	Wet and dry season transplanting	21	89.6 ha	*3.2 ton/ha	
(4) Agricultural extension office of Ayutthaya province	Tambon	Planted area	Yield	Mean yield of 1974 and 1975	
	Khusalod	2,342 ha	2.3 ton/ha		
	Phraya Banlu	1,150 ha	2.4 ton/ha		
	Sing Ha Nat	3,132 ha	*2.7 ton/ha		



Data on Paddy Yield in the Project Area (continued)

	<u>Crop</u>	<u>Planted area</u>	<u>Yield</u>	<u>Remarks</u>
(5) Agricultural extension office of Led Bao Laung	Wet season broadcasting	4,338 ha	2.1 ton/ha	Mean yield from harvested area in 1971 - 1975
	Wet & dry season transplanting	4,248 ha	*3.6 ton/ha	
(6) Yield survey by quadrat sampling in Project area, which was conducted by this Mission	<u>Crop</u>		<u>Yield</u>	
	Wet season broadcasting		1.6 ton/ha	
	Dry season transplanting		3.0 ton/ha	

chemicals in the dry season and the beginning of the wet season. The paddy cultivation in the dry season is called as the dry season transplanting paddy which is blessed with favorable conditions of high yield in comparison with the yield of the wet season transplanting paddy.

As is seen in Table A. 3-45, yield of transplanting paddy, but if the yield marked by asterisks was regarded as the yield in the dry season transplanting paddy, 3.0 ton/ha of yield can be supposed to be an average yield of dry season transplanting paddy in the Project Area. On the other hand, the yield of wet season transplanting paddy rice is assumed at 2.2 ton/ha from the data.

## 2. Yield of Other Crops

### Yield of Upland Crops

The cropping area of upland crops is very small in the Project Area, but many kind of crops are cultivated mainly for the farm households' self consumption. The yield of these upland crops in the Project Area and its neighbourhood are tabulated as follows:

<u>Crops</u>	<u>Yield per crop</u>	<u>Data Source</u>
Corn	2.3 ton/ha	Averaged yield of Upland Crops from planted area, Ayutthaya Province (1974-1976), prepared by Agricultural Extension Office of Ayutthaya province
Soybean	2.6 ton/ha	
Sweet potato	5.0 ton/ha	
Big pepper	1.7 ton/ha	
Cili pepper	1.2 ton/ha	
Tomato	5.6 ton/ha	
Coconut	100 fruit/tree	Socio-economic conditions of farmers in Lad Boa Laung district, prepared by ALRO
Banana (smelling)	60 fruit/tree	
Banana (cooking)	100 fruit/tree	
Water melon	5.2 ton/ha	

Out of these upland crops, water melon is one of the main upland crop in the Project Area, so the yield of 5.0 ton/ha is regarded as the representative of upland crops based on the above table.

#### Yield of Fruits

Citrus is the representative fruits in the Project Area, and the following shows the yield of citrus;

<u>Yield</u>	<u>Data Source</u>
12.6 ton/ha	Socio-economic conditions of farmers in Lad Boi Laung district prepared by ALRO.
7.0 ton/ha	Agricultural extension office of Ayutthaya Province
20.0 ton/ha (basing on harvested area)	The result of interview to some farmers growing citrus in the project area

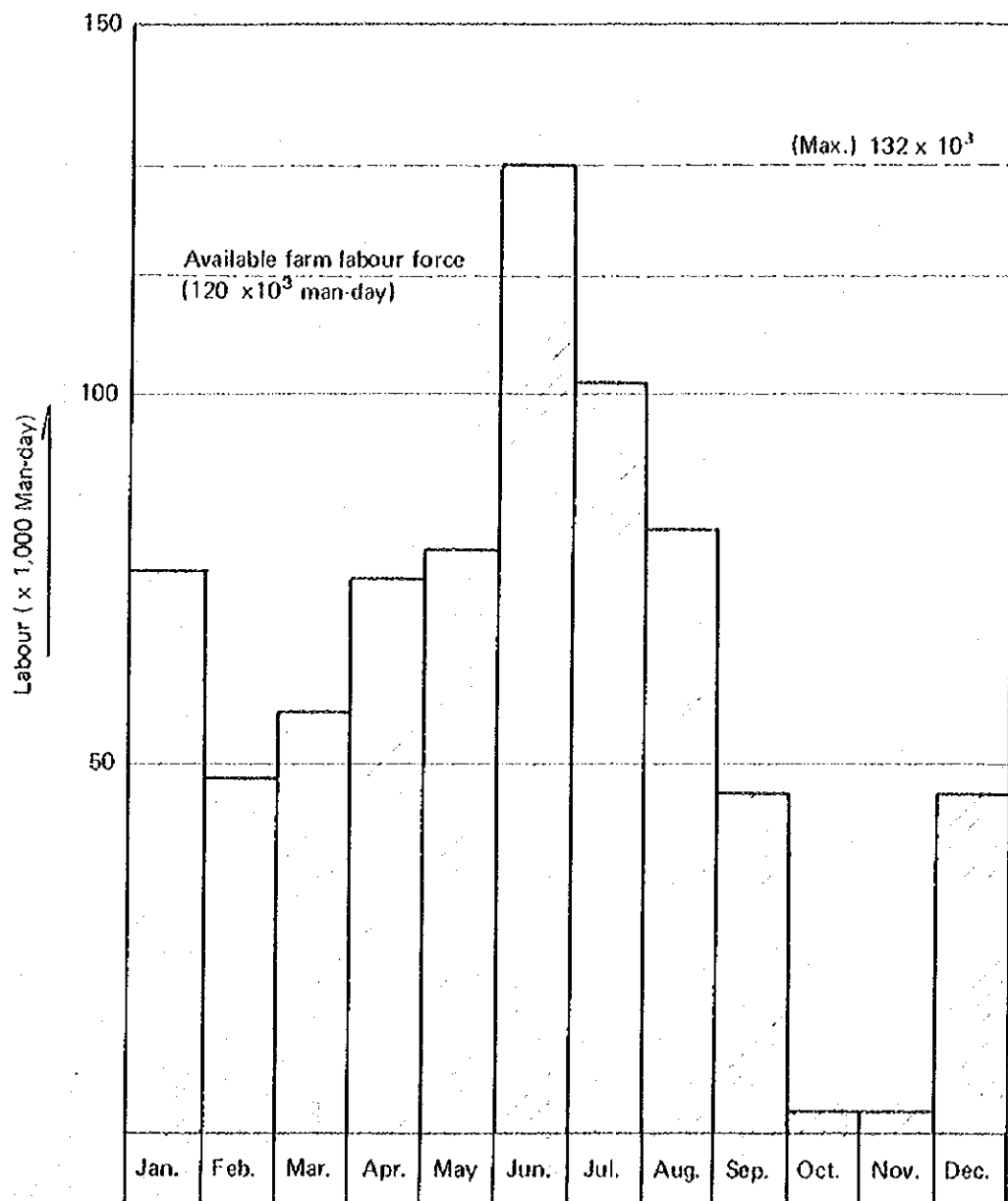
As is seen in the above table, the yield of citrus varies largely from 7.0 ton/ha to 20 ton/ha, and it is considered that these fluctuations of yields are caused by the tree age, cultivation technique and amount of input materials. In the Project, the yield of citrus is estimated at 15.0 ton/ha based on the above table.

Table A. 3-46 Number of Domestic Animals and Breeding Households  
in Each Tambon in the Project Area

Animal	Tambon	No. of Domestic Animal (head)					No. of Household	
		1973	1974	1975	1976	1977	1976	Proportion <sup>1/</sup> (%)
Buffalo	Khusalod	254	123	119	126	64	13	
	Phraya Banlu	192	138	87	86	57	14	
	Sing Ha Nat	316	259	567	95	42	6	
	Total	762	520	773	307	163	11	
Cattle	Khusalod	17	13	73	20	9	2	
	Phraya Banlu	174	174	210	245	60	15	
	Sing Ha Nat	195	502	998	251	69	10	
	Total	386	689	1,281	516	138	9	
Swine	Khusalod	528	280	520	166	31	6	
	Phraya Banlu	1,059	341	239	220	40	10	
	Sing Ha Nat	288	155	136	175	34	5	
	Total	1,875	776	895	561	105	7	

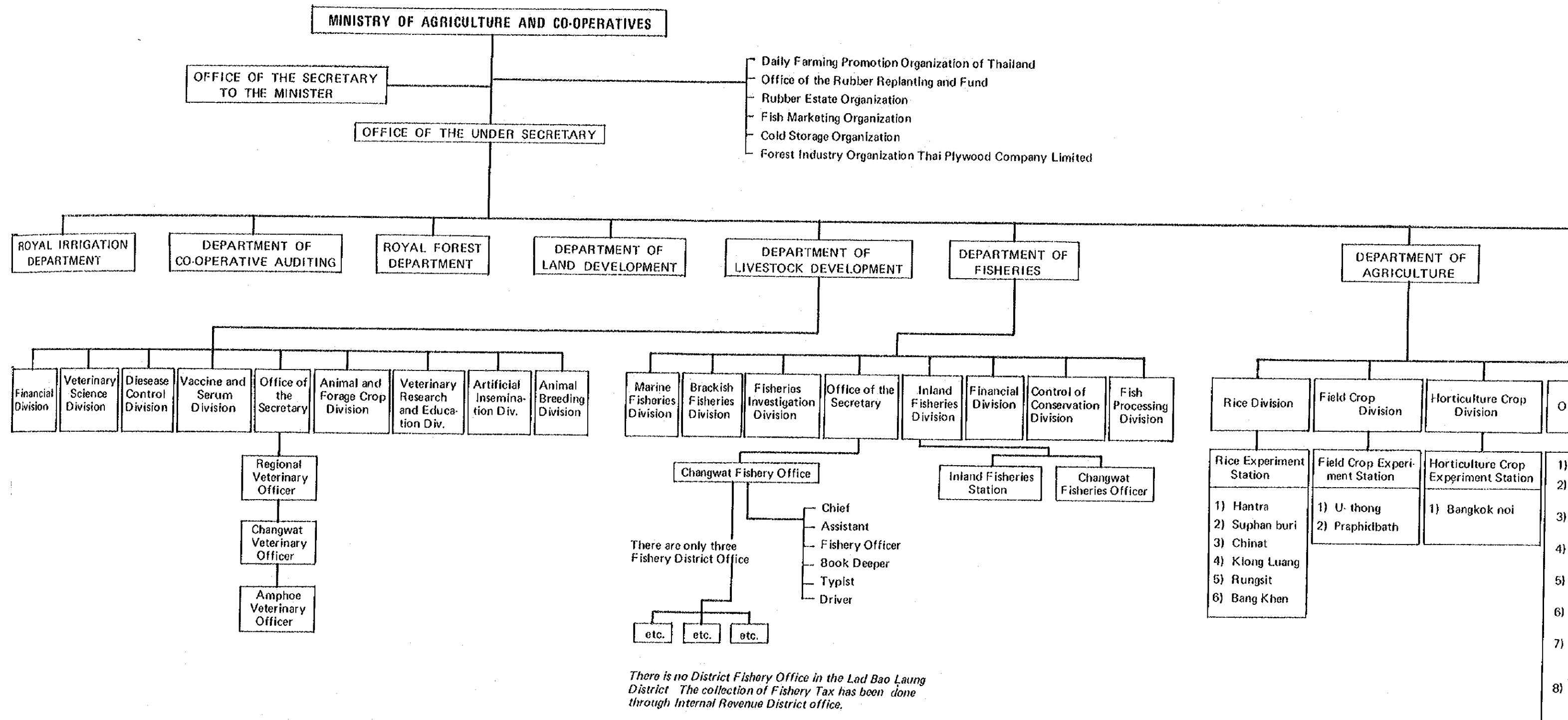
Note: 1/ Proportion of the households breeding domestic animals to total farm households  
(Khusalod 486 households, Phraya Banlu 396 households, Sing Ha Nat 666 households)

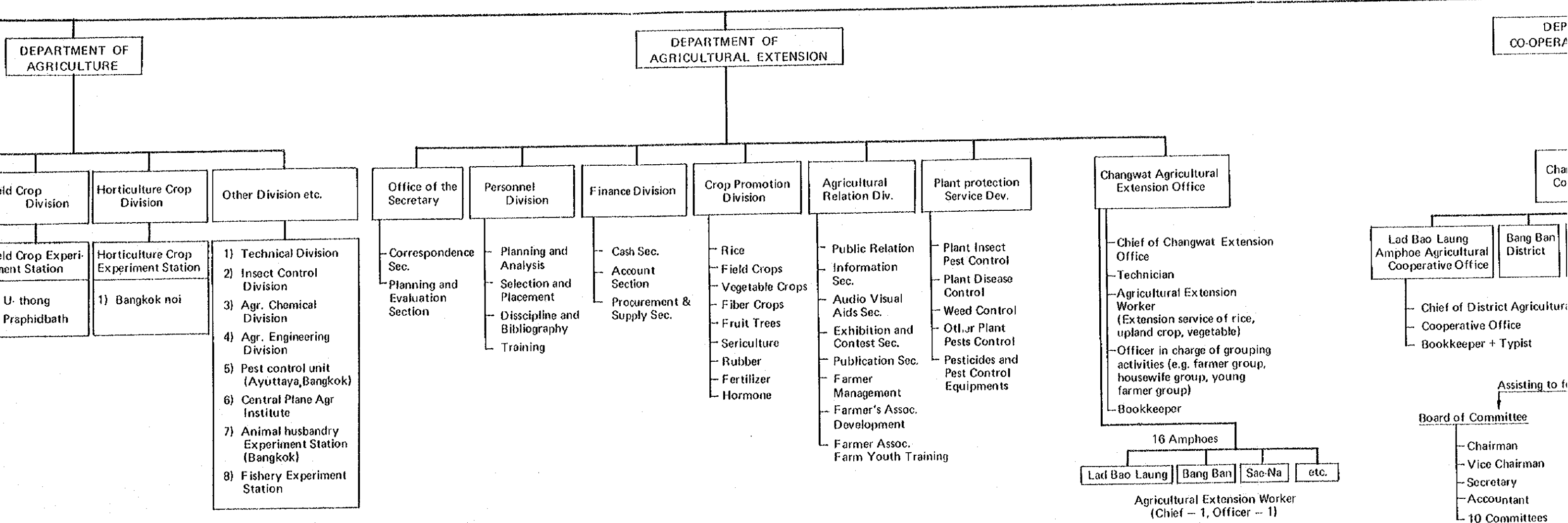
FIGURE A.3-26 FARM LABOUR BALANCE AT PRESENT



Note: Based on the calculation in Appendix 6-2

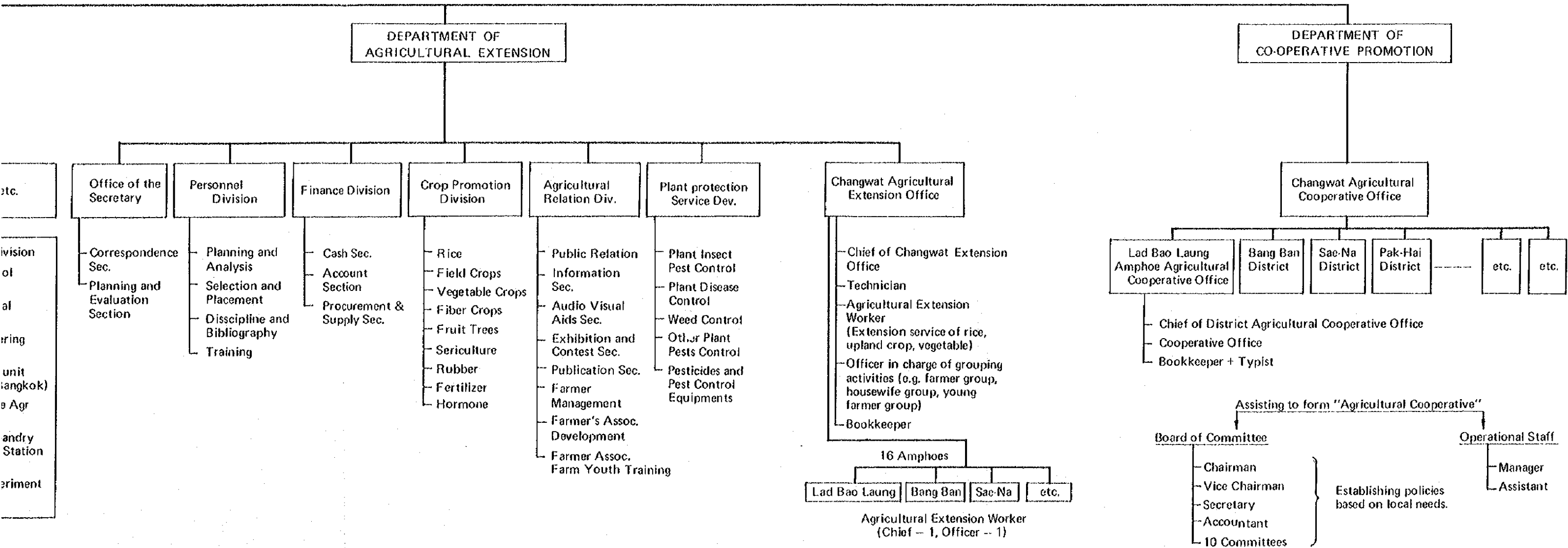
FIGURE A.3 - 27 ORGANIZATION CHART OF MINISTRY OF AGRICULTURE AND CO-OPERATIVES





Work Load of Extension Worker in Amphoe Level

- 50% routine work (o.g. statistical survey book keeping, etc)
- 25% Agricultural extension servicing,
- 25% Working for other departments in Amphur Level



Work Load of Extension Worker in Amphoe Level

- 50% routine work (e.g. statistical survey book keeping, etc)
- 25% Agricultural extension servicing,
- 25% Working for other departments in Amphur Level



Table A.3-47 Number of Unit and Cost of Farm Machinery and Tools per House-hold

Tool and Machinery	Unit and Price			Unit and Price per hectare		Cost of Machinery and Tool		
	(Unit) (1)	(£/unit) (2)	(£) (3)	(unit/ha) (4)	(£/ha) (5)	Durable year (6)	Annual cost/ha (7)	annual cost /house-hold (8)
Plow	2	150	300	0.50	75	5	15	60
Hoe (large)	2	50	100	0.50	25	5	5	20
Hoe (small)	2	20	40	0.50	10	5	2	8
Sickle	5	25	125	1.25	31	3	10	40
Boat, (with engine)	0.39	3,000	1,170	0.10	300	8	38	152
Boat, (small)	1	500	500	0.25	125	10	13	52
Hand Tractor, 8HP	0.35	14,500	5,075	0.09	1,305	7	186	744
Water pump	0.35	2,000	700	0.09	180	8	23	92
Fuel and Other materials							519	2,072
Total			8,010		2,051		811	3,240

Note: (3) = (1) x (2)

(4) = (1)/4.0 ha

average farm land holding per farm house-hold is assumed at 4.0 ha

(5) = (2) x (4)

(7) = (5)/(6)

(8) = (7) x 4.0 ha

Proposed Cropping Pattern for Alternative Studies

Following cropping patterns are proposed for each alternative study,  
(see Figure A. 4-1)

Without Dike Plan

- Cropping Pattern - 1: Cropping intensity 67%
- Paddy Rice I: Broadcasted floating rice in the wet season
  - Paddy Rice II: Transplanted new variety rice in the dry season
- Cropping Pattern - 2: Cropping intensity 130%
- Paddy Rice I: Transplanted floating rice in the wet season (30% of area)
  - Paddy Rice II: Transplanted new variety rice in the dry season (100% of area)

With Dike Plan

- Cropping Pattern - 3: Cropping intensity 200%
- Paddy Rice I: Transplanted new variety rice in the wet season (100% of area)
  - Paddy Rice II: Transplanted new variety rice in the dry season (100% of area)

With and without Dike Plan

- Cropping Pattern - 4: Cropping intensity 180%
- Paddy Rice I-1: Transplanted floating rice in the wet season (30% of Block A)
  - Paddy Rice I-2: Transplanted new variety rice in the wet season (100% of Block B and C)
  - Paddy Rice II-1: Transplanted new variety rice in the dry season (100% of Block A)
  - Paddy Rice II-2: Transplanted new variety rice in the dry season (100% of Block B and C)

FIGURE A.4 — 1. PROPOSED CROPPING PATTERN

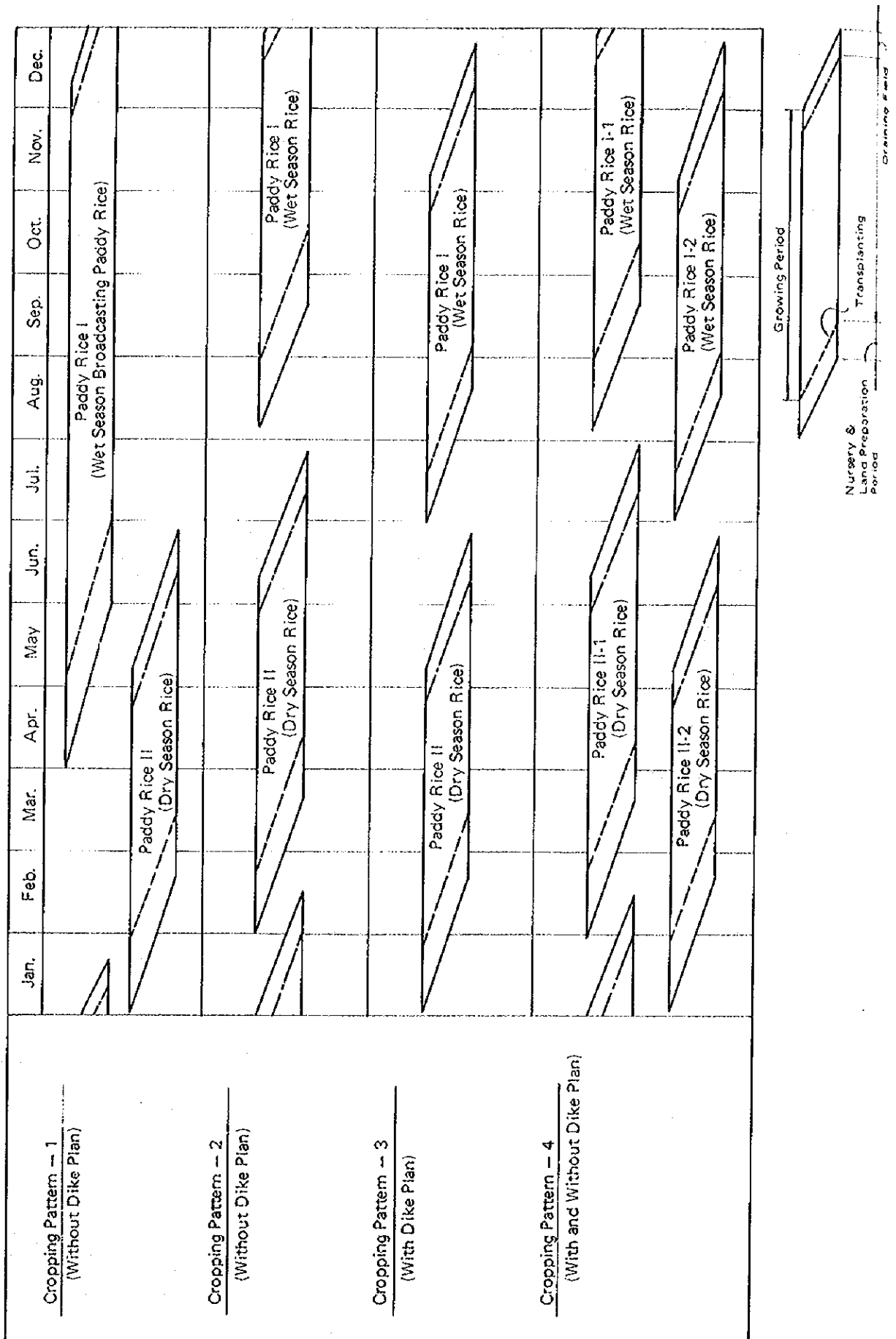


FIGURE A.4-2 CROPPING PATTERN OF PADDY RICE WITHOUT DIKE

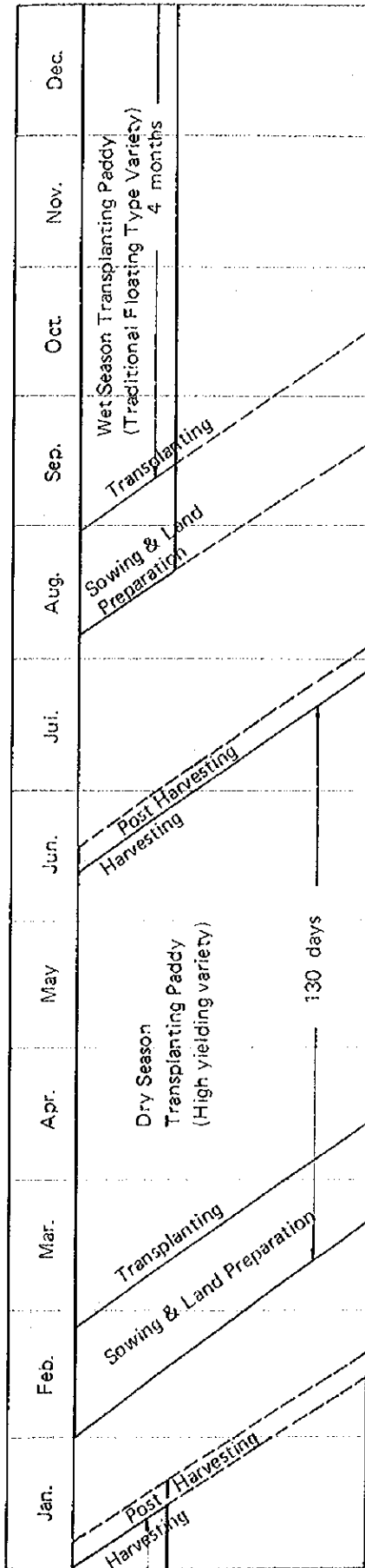


Table A. 4-1 Major Features of the Alternative Plan

Description	Without Dike Plan			With Dike Plan			With and Without Dike Plan
	Plan I-1	Plan I-2	Plan I-3	Plan II-1	Plan II-2	Plan II-3	
<b>1. Cropping Area (ha)</b>							
Without Project							
Block A	3,571	1,336	1,903	3,571	1,336	1,903	1,903
B	3,128	995	1,666	3,128	995	1,666	1,666
C	-	1,238	3,130	-	1,238	3,130	3,130
D	-	1,602	-	-	1,602	-	-
E	-	1,528	-	-	1,528	-	-
Total	6,699	6,699	6,699	6,699	6,699	6,699	6,699
<b>With Project</b>							
Block A	7,621	2,851	4,102	11,220	4,204	6,054	3,935
B	6,664	2,137	3,519	9,864	3,130	5,166	5,166
C	-	2,664	6,664	-	3,896	9,864	9,864
D	-	3,425	-	-	5,044	-	-
E	-	3,209	-	-	4,810	-	-
Total	14,285	14,285	14,285	21,084	21,084	21,084	18,965
<b>2. Proposed Cropping Pattern 1/</b>							
	2	2	2	3	3	3	4
<b>3. Proposed Facilities</b>							
Irrigation and drainage Canals (km)2/	206	244	235	206	244	235	213
Road and Dike	153	153	153	241	260	268	237
Pumping Station (Place)	2	5	3	2	5	3	3

Note: 1/ : See Figure A. 4-1

2/ : No inclusive of farm ditch and drainage ditch

Table A. 4-2. Construction Cost of Alternative Plans

(Unit: US\$'000)

Works	Without Dike Plan			With Dike Plan		
	Plan I-1	Plan I-2	Plan I-3	Plan II-1	Plan II-2	Plan II-3
1. Civil Works						
Land consolidation	9,130	9,110	9,120	8,850	8,810	8,830
Dikes, roads and canals	2,520	2,350	2,420	4,010	3,920	4,080
Pumping station	3,020	4,840	3,720	8,640	10,150	9,020
Collateral works	1,510	1,380	1,450	2,400	2,350	2,650
Sub-total	16,180	17,680	16,710	26,910	25,230	24,580
2. Agricultural Development	740	740	740	1,200 <sup>a/</sup>	1,200	1,200
3. Project Administration	1,140	1,140	1,140	1,140	1,140	1,140
Total	18,060	19,560	18,590	28,250	27,570	26,920
						24,260

Note: a/ inclusive of required cost for animal husbandry and inland fishery as shown belows;

Animal husbandry : B 25,833/unit x 250 units = US\$635,000  
 Inland fishery : B 10,000/unit x 250 units = US\$25,000



Table A. 4-4 Internal Rate of Return (Plan I-1)

Year	No.	Construction		Agricultural Net incre- mental Benefit	Net Incre- mental Benefit	Rate of Return (12%)		Rate of Return (13%)	
		Cost	O & M Cost			Benefit	Cost	Benefit	Cost
1978	1	1,500a/							
1979	2	3,612							1,328
1980	3	3,612	54c/	-125d/	-71	-51			2,828
1981	4	3,612	108	-25	83	53			2,503
1982	5	3,612	162	200	362	205			2,214
1983	6	3,612	216	625	841	426			1,961
1984	7	(18,060)	903	1,225	2,128	962			1,734
1985	8		903	1,700	2,603	1,052			904
1986	9		903	2,150	3,053	1,102			979
1987	10		903	2,375	3,278	1,056			1,017
1988	11		903	2,500	3,403	980			967
1989	12-50		903	2,500	3,403	8,054			888
2002	25	2,240b/					132		6,766
									105
									(12,673)
									-549
									(12,124)
									+740
									(13,839)
									(13,099)
									(12,124)
									-549
									(12 + $\frac{740}{1,289}$ ) = 12.6%

Note: a/ Engineering fee is assumed at 1,500 x 10<sup>3</sup>US\$

b/ Replacement cost of pumping facilities

c/ Operation and maintenance cost is estimated based on the following assumption:  
during construction period: 1.5% of construction cost  
after construction period: 5.0% of total construction cost (without dike plan)

d/ Estimated by applying the annual proportion of agricultural net incremental benefit to reach target, which was obtained from the economic evaluation analyzed in Chapter VI.  
-do- : 3.0% of total construction cost (with dike plan)



Table A. 4-5 Internal Rate of Return (Plan I-2)

Year	No.	Construction		Agricultural Net Incre- mental Benefit	Net Incre- mental Benefit	Rate of Return (12%)		Rate of Return (13%)	
		Cost	O & M Cost			Benefit	Cost	Benefit	Cost
1978	1	1,500							
1979	2	3,912							1,328
1980	3	3,912	59	-125	-66	-47	1,340		3,068
1981	4	3,912	118	-25	93	59	2,785	-46	2,711
1982	5	3,912	177	200	377	213	2,488	57	2,398
1983	6	3,912	236	625	861	437	2,218	205	2,124
1984	7	(19,560)	978	1,225	2,203	996	1,983	413	1,878
1985	8		978	1,700	2,678	1,082		936	
1986	9		978	2,150	3,128	1,129		1,007	
1987	10		978	2,375	3,353	1,080		1,042	
1988	11		978	2,500	3,478	1,002		989	
1989	12-50		978	2,500	3,478	8,231		308	
2002	25	3,580					211	6,915	168
						(14,182)	(14,143)	(12,426)	(13,670)
						+39			-1,244
						$(12 + \frac{39}{1,283}) = 12.0$			

Table A. 4-6 Internal Rate of Return (Plan I-3)

Year	No.	Construction Cost	O & M Cost	Agricultural Net Incremental Benefit	Net Incremental Benefit	Rate of Return (12%)	Rate of Return (13%)
					Benefit	Benefit	Benefit
					Cost	Cost	Cost
1978	1	1,500					1,328
1979	2	3,718					2,911
1980	3	3,718	56	-69	-49		2,577
1981	4	3,718	112	87	55		2,279
1982	5	3,718	168	368	209		2,019
1983	6	3,718	224	849	430		1,785
1984	7	(18,590)	930	2,155	974		
1985	8		930	2,630	1,063		
1986	9		930	3,080	1,112		
1987	10		930	2,375	1,064		1,026
1988	11		930	3,430	988		975
1989	12-50		930	3,430	8,118		895
2002	25	2,750				162	6,820
					(13,964)	(13,470)	(13,028)
					+494		-794

$$(12 + \frac{494}{1,288}) = 12.4\%$$

Table A. 4-7 Internal Rate of Return (Plan II-1)

Year	No.	Construction Cost	O & M Cost	Agricultural Net Incremental Benefit	Incremental Net Benefit	Rate of Return (16%)	Rate of Return (17%)
		Cost			Benefit	Benefit	Cost
1978	1	1,500					1,293
1979	2	5,250					3,838
1980	3	5,250	79	-300	-221	-142	3,276
1981	4	5,250	158	-60	98	54	2,804
1982	5	5,250	237	479	716	341	2,394
1983	6	5,250	316	1,498	1,814	744	2,048
1984	7	(26,250)	788	2,935	3,723	1,318	
1985	8		788	4,073	4,861	1,483	
1986	9		788	5,151	5,939	1,562	
1987	10		788	5,691	6,479	1,471	
1988	11		788	5,990	6,778	1,922	
1989	12-50		788	5,990	6,778	8,254	
2002	25	6,400					128
					(16,407)	(16,269)	(14,643)
					+138		(15,771)
							-1,128

$$(16 + \frac{138}{1,266}) = 16.1\%$$

Table A. 4-8 Internal Rate of Return (Plan II-2)

Year	No.	Construction		O & M Cost	Agricultural Net Increase- mental Benefit	Incremental Net Benefit	Rate of Return (15%)		Rate of Return (16%)	
		Cost	Benefit				Benefit	Cost	Benefit	Cost
1978	1	1,500								
1979	2	5,514								1,293
1980	3	5,514	82		-300	-216				4,097
1981	4	5,514	164		60	104	-143			3,535
1982	5	5,514	246		479	725	59			3,044
1983	6	5,514	320		1,498	1,818	360			2,625
1984	7	(27,570)	827		2,935	3,762	785			2,261
1985	8		827		4,073	4,900	1,415			1,332
1986	9		827		5,151	5,978	1,602			1,495
1987	10		827		5,750	6,577	1,698			1,572
1988	11		827		5,990	6,817	1,625			1,493
1989	12-50		827		5,990	6,817	1,466			1,329
2002	25		7,520		5,990	6,817	9,725			8,301
							(18,592)			188
							+988			(16,529)
							(17,604)			-514
							(15 + $\frac{988}{1,502}$ ) = 15.6%			

Table A. 4-9 Internal Rate of Return (Plan II-3)

Year	No.	Construction Cost	O & M Cost	Agricultural Net Incremental	Net Benefit	Rate of Return (15%)		Rate of Return (16%)	
						Benefit	Cost	Benefit	Cost
1978	1	1,500							
1979	2	5,384							
1980	3	5,384	81	-300	-219	-144	1,305	-140	1,293
1981	4	5,384	162	-60	102	58	4,070	52	4,000
1982	5	5,384	243	479	722	359	3,543	344	3,451
1983	6	5,384	324	1,498	1,822	787	3,080	747	2,971
1984	7	(26,920)	807	2,939	3,746	1,408	2,676	1,326	2,563
1985	8		807	4,073	4,880	1,596	2,326	1,488	2,207
1986	9		807	5,151	5,958	1,692		1,567	
1987	10		807	5,691	6,498	1,505		1,475	
1988	11		807	5,990	6,797	1,461		1,325	
1989	12-50		807	5,990	6,797	9,697		8,277	
2002	25	6,680					200		167
						(18,519)	(17,200)	(16,461)	(16,652)
						+1,319			-191
						(15 + $\frac{1,319}{1,510}$ ) = 15.9%			

Table A. 4-10 Internal Rate of Return (Plan III-1)

Year	No.	Construction		Agricultural		Net Incre-		Rate of		Rate of	
		Cost	O & M Cost	Net Incre- mental Benefit	mental Benefit	Return (14%) Benefit	Cost	Return (15%) Benefit	Cost		
1978	1	1,500									1,305
1979	2	4,852									3,668
1980	3	4,852	73	-248	-175	-118	3,275	-115			3,193
1981	4	4,852	146	-50	96	57	2,873	55			2,775
1982	5	4,852	219	396	615	319	2,518	306			2,411
1983	6	4,852	292	1,238	1,530	698	2,213	661			2,096
1984	7	(24,260)	728	2,425	3,135	1,254		1,179			
1985	8		728	3,366	4,094	1,437		1,339			
1986	9		728	4,257	4,985	1,535		1,416			
1987	10		728	4,703	5,431	1,466		1,341			
1988	11		728	4,950	5,678	1,346		1,221			
1989	12-50		728	4,950	5,678	9,538		8,100			
2002	25		5,630								169
										(15,503) (15,617)	
										-114	
										214	
										(17,532) (16,145)	
										+1,387	
										(14 + $\frac{1,387}{1,501}$ ) = 14.9	

Table A. 4-11 Consumptive Use of Crops

Month	Potential ETP (mm/day)	Paddy Rice								Upland-type Crops							
		Plant A		Plant B		Plant C		Plant D		Plant A		Plant B		Vegetable, Orchard			
		K	C.U	K	C.U	K	C.U	K	C.U	K	C.U	K	C.U	K	C.U		
Jan. I	3.9	0.85	3.3													0.80	3.1
Jan. II	3.9	0.85	3.3	0.85	3.3											0.80	3.1
Feb. I	4.7	0.90	4.2	0.85	4.0	0.35	4.0	0.85	4.0							0.80	3.7
Feb. II	4.7	1.06	5.0	0.90	4.2	0.85	4.0	0.85	4.4					0.30	1.4	0.80	3.7
Mar. I	5.2	1.17	6.1	1.06	5.5	0.90	4.7	0.85	4.4					0.50	2.6	0.80	4.2
Mar. II	5.2	1.19	6.2	1.17	6.1	1.06	5.5	0.90	4.7					0.85	4.4	0.80	4.2
Apr. I	5.7	1.13	6.4	1.19	6.7	1.17	6.8	1.06	6.0					1.02	5.9	0.80	4.6
Apr. II	5.7	1.03	5.9	1.13	6.4	1.19	6.8	1.17	6.7					0.95	5.4	0.80	4.6
May I	5.0	0.90	4.5	1.03	5.2	1.13	5.7	1.19	6.0					0.80	4.0	0.80	4.0
May II	5.0	0.90	4.5	0.90	4.5	1.03	5.2	1.13	5.7					0.65	3.3	0.80	4.0
Jun. I	4.6													1.0	0.9	0.80	3.7
Jun. II	4.6													0.20	0.9	0.80	3.7
Jul. I	4.2	0.85	3.6													0.80	3.4
Jul. II	4.2	0.85	3.6	0.85	3.6											0.80	3.4
Aug. I	4.0	0.90	3.6	0.85	3.4	0.85	3.4	0.85	3.4							0.80	3.2
Aug. II	4.0	1.06	4.2	0.90	3.6	0.85	3.4	0.85	3.4	0.85	3.4					0.80	3.2
Sep. I	3.8	1.17	4.4	1.06	4.0	0.90	3.4	0.90	3.2							0.80	3.0
Sep. II	3.8	1.19	4.5	1.17	4.4	1.06	4.0	0.90	3.4							0.80	3.0
Oct. I	3.9	1.13	4.4	1.19	4.6	1.17	4.6	1.06	4.1							0.80	3.1
Oct. II	3.9	1.03	4.0	1.13	4.4	1.19	4.6	1.17	4.6							0.80	3.1
Nov. I	4.0	0.90	3.6	1.03	4.1	1.13	4.5	1.19	4.8							0.80	3.2
Nov. II	4.0	0.90	3.6	0.90	3.6	1.03	4.1	1.13	4.5							0.80	3.2
Dec. I	3.9															0.80	3.1
Dec. II	3.9															0.80	3.1

Note: K: Coefficient  
C.U: Consumptive use, in millimeter per day

Comparison of Consumptive Use of Rice

The comparison of the consumptive use which has been authorized in the reports submitted already is made, in order to confirmed the estimated values of the consumptive use of the project. Following table indicates the monthly consumptive use of rice at the four areas inclusive of the Project Area.

Comparison of Consumptive Use of Rice  
(millimeter per day)

<u>Month</u>	<u>Project Area</u>	<u>Northern Chao Phya Project</u>	<u>Sam Chook Experimental Farm</u>	<u>Mae-Kuang Water Use Experimental Station</u>
Jan.	4.7 <sup>a/</sup>	4.6 <sup>b/</sup>	-	-
Feb.	5.6	5.4	3.7 <sup>c/</sup>	5.9 <sup>d/</sup>
Mar.	6.3	6.4	6.8	6.3
Apr.	6.9	6.7	9.5	8.5
May	6.0	5.3	8.0	8.1
Jun.	5.5	5.0	3.8	-
Jul.	5.0	4.6	5.9	-
Aug.	4.8	4.4	7.0	4.2
Sep.	4.5	4.1	5.3	5.3
Oct.	4.7	4.4	5.6	6.5
Nov.	5.4	4.7	6.0	5.6
Dec.	5.0	4.5	-	-

Note: a/ Calculated by multiplying the potential evapotranspiration, E<sub>tp</sub> (see Table A.4-11) by crop coefficient of K = 1.2

b/ Prepared by NEDECO in 1970 (K = 1.2)

c/ See Table A. 4-12

d/ See Table A. 4-12

From the Table, the estimated values are almost the same to that of the Northern Chap Phya project, which is located on an adjacent upstream of the Project Area. However, the values observed at two experimental farms, Sam Chook Experimental Farm and Mae-Kuang Water Use Experimental Station, is relatively large, especially at the peak periods during the growing seasons. But those values will be varied depending upon the varieties of paddy, growing season, water management and so forth. After consideration of the facts mentioned above, the estimated values appear to be adequate for the Project.



Table A. 4-12 Observed Evapotranspiration (ETa) of Rice

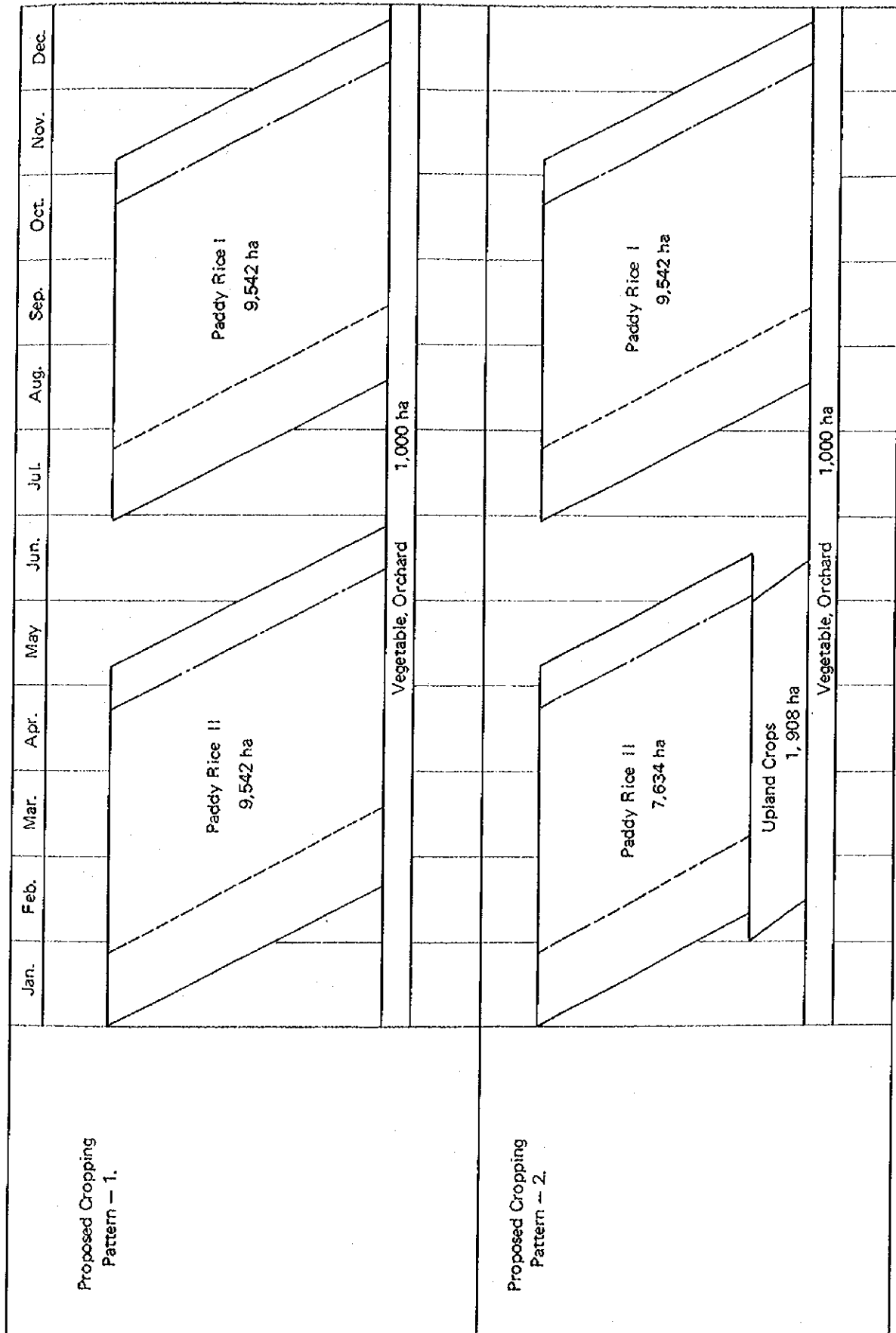
Item	Irrigation Period	Observed ETa (mm)		Remarks
		Total	Daily Mean	
<b>Station: Sam Chook Experimental Farm</b>				
<b>Wet season rice</b>				
(one year of data, 1964)	Jun. 25 - 30	22.6	3.76	Variety: Bang Phae (5 month rice)
	Jul. 1 - 31	182.9	5.90	Transplanting Date: June 25, 1964
	Aug. 1 - 31	216.8	6.99	Irrigation Period: Continuously from June 26 to November 22, 1964
	Sep. 1 - 31	159.5	5.31	Harvesting: December 11, 1964
	Oct. 1 - 31	172.5	5.56	
	Nov. 1 - 22	130.8	5.95	
	Average 151 days	865.1	5.86	
<b>Dry season rice</b>				
(one year of data, 1965)	Feb. 20 - 26	32.8	3.65	Variety: Leong Kamin (4 month rice)
	Mar. 1 - 31	209.8	6.77	Transplanting Date: Feb. 20, 1965
	Apr. 1 - 30	283.9	9.46	Irrigation Period: Continuously from Feb. 21 to May 21, 1965
	May 1 - 21	168.0	8.00	Harvesting: June 8, 1965
	Average 91 days	694.5	7.63	
<b>Station: Mae-Kuang Water Use Experimental Station, Chiang Mai</b>				
<b>Wet season rice</b>				
(average two years of data, 1966 - 1967)	Aug. 10 - 31	92.5	4.20	Variety: Kam Pai
	Sep. 1 - 30	159.8	5.32	Transplanting Date: Aug. 10
	Oct. 1 - 31	200.5	6.47	Irrigation Period: Continuously from Aug. 10 to Nov. 21
	Nov. 1 - 21	117.2	5.58	
	Average 104 days	570.0	5.48	
<b>Dry season rice</b>				
(average two years of data, 1967 - 1968)	Feb. 24 - 28	29.3	5.86	Variety: Kam Pai
	Mar. 1 - 31	194.2	6.26	Transplanting Date: Feb. 24
	Apr. 1 - 30	256.2	8.54	Irrigation Period: Continuously from Feb. 24 to May 28
	May 1 - 28	227.9	8.14	
	Average 94 days	707.6	7.53	

Table A. 4-13 Field Water Requirements of Crops

Month	Perco- lation (1)	R i c e																Upland-type Crops					
		Plant A			Plant B			Plant C				Plant D			Average Field Water Requirements (6)	Upland Crops		Vegetable, Orchard					
		C.U (2)	A.S (3)	Total (4) (5)	C.U (2)	A.S (3)	Total (4) (5)	C.U (2)	A.S (3)	Total (4) (5)	C.U (2)	A.S (3)	Total (4) (5)	Plant A		Plant B	Average F.W.R.						
Jan. I	15	50	30	95	6.3												2.0			3.1			
II	16	53	160	229	14.3	53	30	99	6.2								6.5			3.1			
Feb. I	15	63		78	5.2	60	160	235	15.7	60	30	105	7.0				9.0	1.4		0.7	3.7		
II	13	65		78	5.0	55		68	5.2	52	160	225	17.3	52	30	95	7.3	9.0	2.4	1.4	1.9	3.7	
Mar. I	15	98		113	7.5	83		98	6.5	71		86	5.7	66	160	241	16.1	7.0	1/	4.4	2.6	3.5	4.2
II	16	99		115	7.2	92		108	6.8	88		104	6.5	75		91	5.7	6.8	5.3	4.4	4.9	4.2	
Apr. I	15	96		111	7.4	101		116	7.7	102		117	7.8	93		105	7.0	7.6	5.4	5.8	5.6	4.6	
II	15	89		104	6.9	96		111	7.4	102		117	7.8	101		116	7.7	7.4	4.6	5.4	5.0	4.6	
May I	15	68		83	(5.5)	78		93	6.2	86		101	6.7	90		105	7.0	4.6	3.3	4.0	3.7	4.0	
II	16					72		88	(5.5)	83		99	6.2	91		107	6.7	2.3	1.0	3.3	2.2	4.0	
Jun. I	15									62		77	(5.1)	71		86	5.7	0.2		0.9	0.5	3.7	
II	15													62		77	(5.1)	-				3.7	
Jul. I	15	54	15	84	5.6												1.8					3.4	
II	16	58	160	234	14.6	58	15	89	5.6								6.4					3.4	
Aug. I	15	54		69	4.6	51	160	226	15.1	51	15	81	5.4				8.2					3.2	
II	16	67		83	5.2	58		74	4.6	51	160	227	14.2	54	15	85	5.3	7.8				3.2	
Sep. I	15	66		81	5.4	60		75	5.0	54		69	4.6	48	160	223	14.9	5.4				3.0	
II	15	68		83	5.5	66		81	5.4	60		75	5.0	51		66	4.4	5.3				3.0	
Oct. I	15	66		81	5.4	69		84	5.6	69		84	5.6	62		77	5.1	5.5				3.1	
II	16	64		80	5.0	70		86	5.4	74		90	5.6	74		90	5.6	5.4				3.1	
Nov. I	15	54		69	(4.6)	62		77	5.1	68		83	5.5	72		87	5.8	3.7				3.2	
II	15					54		69	(4.6)	62		77	5.1	68		83	5.5	1.8				3.2	
Dec. I	15									54		69	(4.6)	60		75	5.0	0.2				3.1	
II	16													56		(71)	4.4	-				3.1	

- Note: (1) Percolation in paddy fields is assumed at 1.0 mm/day  
 (2) See Table A. 4-11  
 (3) Additional Water Supply (A.S) for nursery bed and land preparation is assumed as follows:
- |                         |   |
|-------------------------|---|
| <u>Nursery bed</u>      | Area : one-twentieth of paddy field area  |
|                         | Water : requirement: 300 mm   |
|                         | water : requirements to whole paddy area: 15 mm   |
| <u>Land Preparation</u> | Pre-irrigation for plowing 15 mm (plowing depth 150 mm, increasing soil moisture from 10 per cent to 20 per cent) land preparation: |
|                         | Saturation of surface soil layer: 60 mm (plowing depth 150 mm, with 40 per cent of soil prosoity)                                   |
|                         | Standing water above soil surface: 100 mm   |
- (4) Total field water requirements, in millimeter per half-month, (1)+(2)+(3)  
 (5) Total field water requirements, in millimeter per day  
 ( ) shows the water requirements of rice during the period of drying field.  
 (6) Estimated by applying the following proportion of planting area in half month;  
 plant A: 31.25%    plant B: 33.33%    plant C: 31.25%    plant D: 4.17%
- Maximum average field water requirements:  
 $7.1 \times 15/48 + 6.5 \times 16/48 + 5.7 \times 15/48 + 5.4 \times 1/48 + 160 \times 1/48 = 9.6 \text{ mm/day}$   
 1/ Maximum 9.6 mm/day

FIGURE A.4-3 PROPOSED CROPPING PATTERN



Effective Rainfall

Three rainfall data observed at Sing Ha Nat, Phraya Banlu and Lakkhon regulators are available around the Project Area for 14 years from 1962 to 1975. By using these rainfall data, Probability analysis was made to determine the design rainfall for the irrigation plan, that is, 2-years probable rainfall equivalent to once every two years and 10-years probable rainfall equivalent to once every ten years are selected as the rainfalls of normal year and the design year for the Project respectively, and they are summarized as given below;

Station	<u>Probable Rainfall</u>			
	Return Period: 2-years		Return Period: 10-years	
	Rainfall	Corresponding year	Rainfall	Corresponding year
Sing Ha Nat Reg.	1,394	1971	1,060	1967
Phraya Banlu Reg.	1,222	1969	954	1971
Lakkhon Reg.	1,162	1964	823	1965
Average	1,259		946	

As is seen in the above Table, the rainfall in the normal year and design year are estimated at 1,259 mm/annum and 946 mm/annum on average, and the corresponding year at each station to meet the average rainfall estimated above is found out respectively. The effective rainfalls for paddy fields and upland fields are computed by applying the calculation method of effective rainfalls<sup>1/</sup> used in the irrigation Project under the Royal Irrigation Department.

<sup>1/</sup> Paddy field:

Monthly rainfall x coefficient (K)

Coefficient K;

Jan.:	0.90	Jul.:	0.75
Feb.:	0.90	Aug.:	0.75
Mar.:	0.90	Sep.:	0.75
Apr.:	0.75	Oct.:	0.65
May :	0.75	Nov.:	0.80
Jun.:	0.75	Dec.:	0.90

Upland field:

Based on Figure A, 4-4

Table A. 4-14 and A. 4-15 show the effective rainfalls of paddy fields and upland fields, in case of the normal year and design year.

Table A.4-14 Effective Rainfall (Return Period: 2-years)

(Unit: mm)

Month	Rainfall				Effective Rainfall		
	Sing Ha Nat Reg.	Phraya Ban Lu Reg.	Lakkhon Reg.	Average (1)	Paddy Field		Upland Field (4)
					Ratio of Effective Rainfall (2)	Effective Rainfall (3)=(1)x(2)	
Nov. I	-	134.2	-	44.7	0.80	35.8	32.2
II	-	-	-	-	0.80	-	-
Dec. I	-	-	52.0	17.3	0.90	15.6	12.2
II	2.0	-	-	0.7	0.90	0.6	0.4
Jan. I	-	-	-	-	0.90	-	-
II	-	-	-	-	0.90	-	-
Feb. I	5.2	-	-	1.7	0.90	1.5	1.1
II	-	-	27.4	9.1	0.90	8.2	6.2
Mar. I	-	-	8.3	2.8	0.90	2.5	1.9
II	22.2	29.0	7.3	19.5	0.90	17.6	13.8
Apr. I	12.0	5.5	-	5.8	0.75	4.4	4.0
II	46.5	-	101.9	49.5	0.75	37.1	36.0
May I	31.4	27.5	254.7	104.5	0.75	78.4	70.0
II	144.1	21.9	43.3	69.8	0.75	52.4	49.0
Jun. I	120.5	71.0	70.2	87.2	0.75	65.4	59.7
II	47.3	49.3	92.2	62.9	0.75	47.2	44.7
Jul. I	21.1	135.0	56.0	70.7	0.75	53.0	49.6
II	57.9	-	47.3	35.1	0.75	26.3	25.9
Aug. I	68.6	104.9	35.1	69.5	0.75	52.1	48.8
II	225.5	100.3	85.6	137.1	0.75	102.8	87.4
Sep. I	157.3	145.9	15.0	106.1	0.75	79.6	70.9
II	151.9	199.8	226.4	192.7	0.75	144.5	109.8
Oct. I	72.5	120.5	19.5	70.8	0.65	46.0	49.5
II	211.6	69.2	19.5	100.1	0.65	65.1	67.2
Total	<u>1,397.6</u>	<u>1,214.0</u>	<u>1,161.7</u>	<u>1,257.6</u>		<u>936.1</u>	<u>840.3</u>

Note: (4); estimated based on Figure A. 4-4

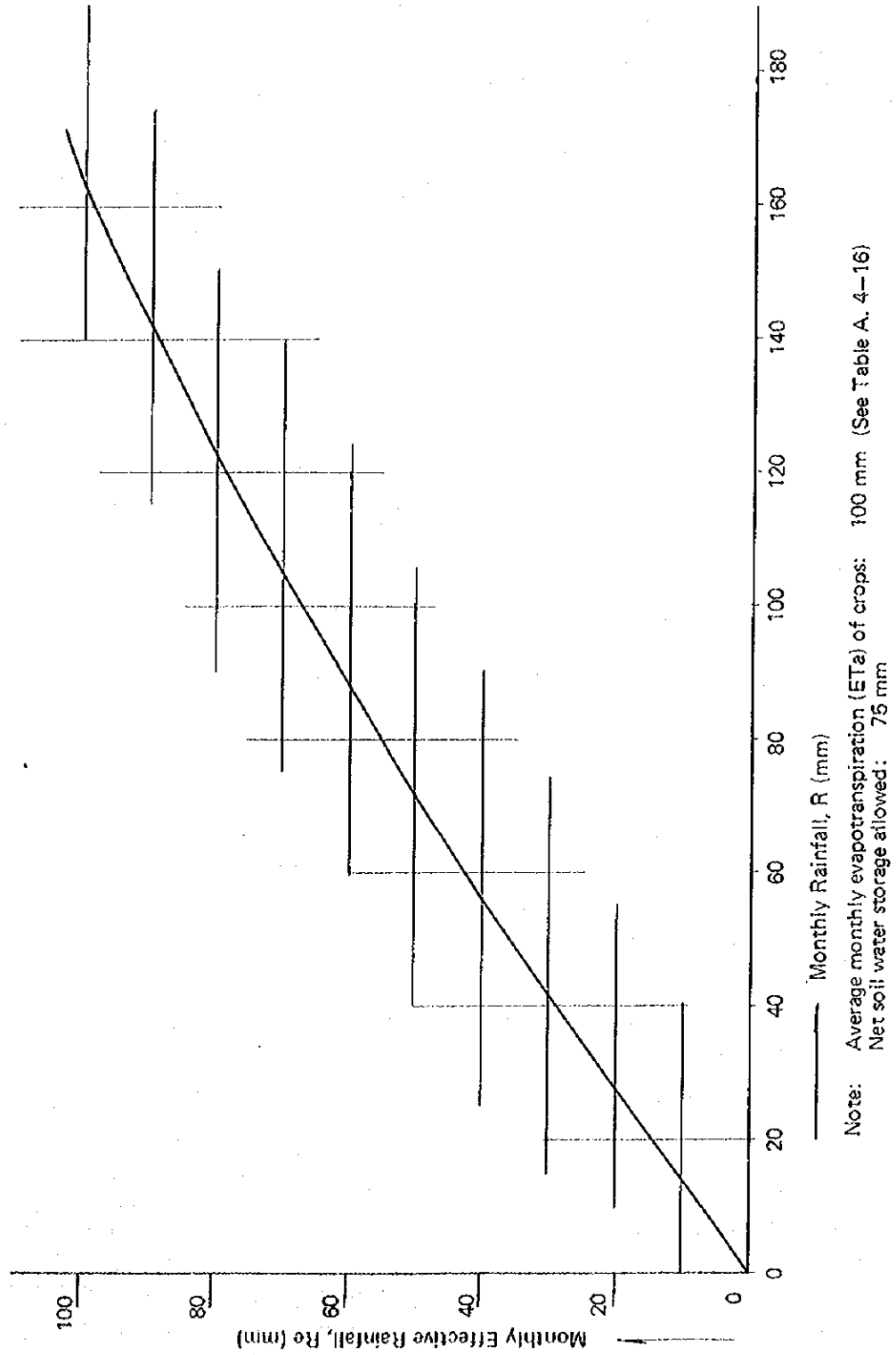
Table A.4-15 Effective Rainfall (Return Period: 10 years)

(Unit: mm)

Month	Rainfall				Effective Rainfall			
	Sing Ha Mat Reg.	Phraya		Average	Paddy Field			
		Banlu Reg.	Lakkhon Reg.		Ratio of Effective Rainfall	Effective Rainfall	Upland Field	
				(1)	(2)	(3)=(1)x(2)	(4)	
Nov.	I	38.8	-	-	12.9	0.80	10.3	9.0
	II	0.8	-	5.2	2.0	0.80	1.6	1.3
Dec.	I	-	-	-	-	0.90	-	-
	II	-	-	-	-	0.90	-	-
Jan.	I	-	-	-	-	0.90	-	-
	II	-	-	-	-	0.90	-	-
Feb.	I	-	-	43.6	14.5	0.90	13.1	10.0
	II	34.0	-	16.8	16.9	0.90	15.2	12.0
Mar.	I	21.5	-	-	7.1	0.90	6.4	5.0
	II	-	11.3	3.6	5.0	0.90	4.5	3.5
Apr.	I	5.0	34.0	1.4	13.5	0.75	10.1	9.4
	II	35.5	92.7	-	42.7	0.75	30.0	31.0
May	I	3.3	-	7.4	3.6	0.75	2.7	2.5
	II	51.5	14.1	52.1	39.2	0.75	29.4	28.5
Jun.	I	131.2	34.4	48.6	71.4	0.75	53.6	50.0
	II	40.2	7.5	39.7	29.1	0.75	21.8	21.0
Jul.	I	89.9	19.5	-	36.1	0.75	27.1	26.1
	II	76.3	48.4	3.4	42.7	0.75	30.0	30.9
Aug.	I	28.8	23.0	41.0	30.9	0.75	23.2	22.1
	II	34.1	192.9	82.7	103.2	0.75	77.4	69.0
Sep.	I	129.3	119.0	89.9	112.7	0.75	84.5	74.4
	II	151.0	96.5	218.8	155.4	0.65	116.6	96.1
Oct.	I	143.0	46.0	58.2	82.4	0.65	53.6	56.8
	II	28.9	212.8	9.3	83.7	0.65	54.4	57.4
Total	1,043.1	952.1	721.7	905.0		665.5	616.0	

Note: (4); estimated based on Figure A. 4-4

FIGURE A.4-4 RELATION CURVE BETWEEN RAINFALL AND EFFECTIVE RAINFALL FOR UPLAND FIELDS



Source: Lam Pra Pierrig Irrigation Project report prepared by Engineering Consultants, 1971



Table A. 4-16 Average monthly effective rainfall as related to mean monthly rainfall and average monthly crop evapotranspiration,  $E_{Ta}$ , for upland-type crop with net soil water storage allowed of 75 mm <sup>1/</sup>

Mean Monthly Rainfall R (mm)	Average monthly $E_{Ta}$ (mm) (upland-type crops)									
	25	50	75	100	125	150	175	200	225	250
Average monthly effective rainfall, $R_e$ (mm)										
15	9	10	10	11	11	12	12	13	14	15
20	12	13	14	14	15	16	17	18	19	20
30	18	19	21	22	22	23	24	26	28	30
40	23	25	27	29	30	31	32	35	38	40
50	25	32	34	35	36	38	40	43	46	49
60		38	40	42	43	45	47	51	55	59
70		43	46	49	51	53	55	59	63	68
80		48	52	55	58	60	63	67	71	77
90		50	57	61	64	67	70	75	79	85
100			63	67	71	74	78	82	87	94
110			68	73	78	80	84	89	95	102
120			73	78	84	86	91	97	102	110
130			75	83	89	92	98	104	110	118
140				89	95	99	105	112	118	126
150				94	101	105	110	120	125	134
160				99	106	110	117	125	132	142
170				100	111	116	123	131	138	149
180					116	121	129	136	144	155
190					121	126	134	142	150	161
200					125	132	140	148	157	168

<sup>1/</sup> For other net soil water storage allowed, multiply by the factor shown below:

Net soil water storage allowed, mm										
	25	30	40	50	60	75	100	125	150	175
Factor	0.74	0.82	0.88	0.93	0.96	1.00	1.02	1.04	1.06	1.07

Note: Average monthly effective rainfall cannot exceed average monthly rainfall or average monthly crop evapotranspiration. Where mean monthly rainfall is less than the minimum effective rainfall in the Table above it is assumed to be 100 percent effective.

Water Losses

Water losses for paddy fields consist of the following two losses;

- i) Application losses
- ii) Conveyance losses

The former is considered to be on-farm losses due to mostly the farmer's capacity of farm water management, and in the project, the 20 per cent of the average field irrigation water requirements is taken as the on-farm losses, based on the following assumption.

	<u>Application losses</u>
Surface run-off	10 <sup>%</sup>
Feeder loss	5
Operation loss	5
Total (1)	<u>20</u>

On the other hand, the latter which is water losses during conveyance stage, furthermore, can be classified into two factors namely: physical and non-physical factors, and physical factors are composed of seepage, leakage and evaporation losses, while non-physical factors are rather related to operational factors such as over-application of irrigation water in the fields, unscheduled drainage and illegal diversion.

However, in the project area, no data on the water conveyance losses exist entirely, so that the conveyance losses for main canals and laterals are decided as shown below, although the Morits empirical formula<sup>1/</sup> could be considered as the procedure for estimation of seepage losses of canal.

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1/ Morits Formula: Water losses (cu.m/km) = 0.038.C.(Q/V)<sup>1/2</sup>  
 where, Q = canal discharge (cu.m/sec), V = velocity of flow (m/sec)  
 and C = 0.20 for sandy canal and 0.13 for clay and clayey canals.

	<u>Conveyance loss (%)</u>
Water loss in main canals	
physical	2.5
Non-physical	5.0
Sub-total (2)	<u>7.5</u>
Water loss in laterals	10.0
Sub-total (3)	<u>10.0</u>
Total (4) = (2) + (3)	<u><u>17.5</u></u>
Total loss (5) <sup>1/</sup>	<u><u>33.4</u></u>

As a result, total water loss for paddy field irrigation are decided to be 33.4 per cent of farm turn-out requirements in the project.

On the other hand, the irrigation efficiency for upland fields is assumed to be 65 per cent, in view of irregular parcellation patterns resulting in diverse lengths of runs, non-uniform distribution of water, expected operation and percolation losses.

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<sup>1/</sup> (5) = 100% - [100 - (1)%] x [100 - (2)%] x [100 - (3)%]

Table A. 4-17 Irrigation Water Requirements for Paddy Fields  
(Return Period: 2-years)

Month	Field Water Requirements		Effective Rainfall (mm/half mon)	Net Water Requirements (mm/half mon)	Gross Water Requirements			Irrigation Water Requirements			
	(mm/day) (1)	(mm/half mon) (2)			Farm Gate Water Requirements (mm/half mon) (5)	Diversion Water Requirements (mm/half mon) (6)	( $\ell$ /sec/ha) (7)	Block A (cu.m/s) (8)	Block B (cu.m/s) (9)	Block C (cu.m/s) (10)	Total (cu.m/s) (11)
Jan. I	2.0	30.0	-	30.0	37.5	45.0	0.347	1.00	0.81	1.50	3.31
II	6.5	104.0	-	104.0	130.0	156.2	1.130	3.26	2.64	4.88	10.78
Feb. I	9.0	135.0	1.5	133.5	166.9	200.5	1.547	4.46	3.61	6.69	14.76
II	9.0	117.0	8.2	108.8	136.0	163.4	1.455	4.20	3.40	6.29	13.89
Mar. I	7.0	105.0	2.5	102.5	128.1	153.9	1.187	3.42	2.77	5.13	11.32
II	6.8	108.8	17.6	91.2	114.0	136.9	0.990	2.86	2.31	4.28	9.45
Apr. I	7.6	114.0	4.4	109.6	137.0	164.6	1.270	3.66	2.96	5.49	12.11
II	7.4	111.0	37.1	73.9	92.4	111.0	0.803	2.32	1.87	3.47	7.66
May I	4.6	69.0	78.4	-	-	-	-	-	-	-	-
II	2.3	36.8	52.4	-	-	-	-	-	-	-	-
Jun. I	0.2	3.0	65.4	-	-	-	-	-	-	-	-
II	-	-	47.2	-	-	-	-	-	-	-	-
Jul. I	1.8	27.0	53.0	-	-	-	-	-	-	-	-
II	6.4	102.4	26.3	76.1	95.1	114.2	0.826	2.38	1.93	3.57	7.88
Aug. I	8.2	123.0	52.1	70.9	88.6	106.4	0.821	2.37	1.92	3.55	7.84
II	7.8	104.8	102.8	2.0	2.5	3.0	0.022	0.06	0.05	0.09	0.20
Sep. I	5.4	81.0	79.6	1.4	1.8	2.2	0.017	0.05	0.04	0.07	0.16
II	5.3	84.8	144.5	-	-	-	-	-	-	-	-
Oct. I	5.5	82.5	46.0	36.5	45.6	54.8	0.423	1.22	0.99	1.83	4.04
II	5.4	86.4	65.1	21.3	26.6	32.0	0.231	0.67	0.54	1.00	2.21
Nov. I	3.7	55.5	35.8	19.7	24.6	29.5	0.228	0.66	0.53	0.99	2.18
II	1.8	27.0	-	27.0	33.8	40.6	0.313	0.90	0.73	1.35	2.98
Dec. I	0.2	3.0	15.6	-	-	-	-	-	-	-	-
II	-	-	0.6	-	-	-	-	-	-	-	-

Note: (1): Average field water requirement, in millimeter per day (see Table A. 4-13)  
(2): Average field water requirement, in millimeter per half month  
(3): Effective rainfall in millimeter per half month (see Appendix A. 4-14)  
(4): (2) - (3)  
(5): (4)/(1 - 0.2)  
(6): (5)/(1 - 0.075)(1 - 0.10)  
(7): (6)/8.64 x n  
(8): Block A; irrigable area 2,885 ha x (7)  
(9): Block B; irrigable area 2,334 ha x (7)  
(10): Block C; irrigable area 4,323 ha x (7)  
(11): (8) + (9) + (10)

Table A. 4-18 Irrigation Water Requirements for Upland Crops  
(Return Period: 2-years)

Month	Field Water Requirements		Effective Rainfall (mm/half mon)	Net Water Requirements (mm/half mon)	Gross Water Requirements			Irrigation Water Requirements			
	(mm/day) (1)	(mm/half mon) (2)			Farm Gate Water Requirements (mm/half mon) (5)	Diversion Water Requirements (mm/half mon) (6)	(l/sec/ha) (7)	Block A (cu.m/s) (8)	Block B (cu.m/s) (9)	Block C (cu.m/s) (10)	Total (cu.m/s) (11)
Jan. I	3.1	46.5	-	46.5	71.5	85.9	0.663	0.09	0.17	0.40	0.66
II	3.1	49.6	-	49.6	76.3	91.7	0.663	0.09	0.17	0.40	0.66
Feb. I	3.7	55.6	1.1	54.5	83.9	100.8	0.778	0.11	0.19	0.47	0.77
II	3.7	48.1	6.2	41.9	64.5	77.5	0.690	0.10	0.17	0.42	0.69
Mar. I	4.2	63.0	1.9	61.1	94.0	112.9	0.871	0.12	0.22	0.53	0.87
II	4.2	67.2	13.8	53.4	82.2	98.7	0.714	0.10	0.18	0.43	0.71
Apr. I	4.6	69.0	4.0	65.0	100.0	120.1	0.927	0.13	0.23	0.56	0.92
II	4.6	69.0	36.0	33.0	50.8	61.0	0.471	0.07	0.12	0.29	0.48
May I	4.0	60.0	70.0	-	-	-	-	-	-	-	-
II	4.0	64.4	49.0	15.4	23.7	28.5	0.206	0.03	0.05	0.13	0.21
Jun. I	3.7	55.5	59.7	-	-	-	-	-	-	-	-
II	3.7	55.5	44.7	10.8	16.6	19.9	0.154	0.02	0.04	0.09	0.15
Jul. I	3.4	51.0	49.6	1.4	2.2	2.6	-	-	-	-	-
II	3.4	54.4	25.9	28.5	43.9	52.7	0.381	0.05	0.09	0.23	0.37
Aug. I	3.2	48.0	48.8	-	-	-	-	-	-	-	-
II	3.2	51.2	87.4	-	-	-	-	-	-	-	-
Sep. I	3.0	45.0	70.9	-	-	-	-	-	-	-	-
II	3.0	45.0	109.8	-	-	-	-	-	-	-	-
Oct. I	3.1	46.5	49.5	-	-	-	-	-	-	-	-
II	3.1	49.6	67.2	-	-	-	-	-	-	-	-
Nov. I	3.2	48.0	32.2	15.8	24.3	29.2	0.225	0.03	0.06	0.14	0.23
II	3.2	48.0	-	48.0	73.9	88.8	0.685	0.10	0.17	0.42	0.69
Dec. I	3.1	46.5	12.2	34.3	52.8	63.4	0.489	0.07	0.12	0.30	0.49
II	3.1	49.6	0.4	49.2	75.7	90.9	0.658	0.09	0.16	0.40	0.65

Note: (1): Average field water requirements, in millimeter per day (see Table A. 4-13)  
(2): Average field water requirements, in millimeter per half month  
(3): Effective rainfall, in millimeter per half month, (see Appendix A. 4-14)  
(4): (2) - (3)  
(5): (4)/(1 - 0.35)  
(6): (5)/(1 - 0.075)(1 - 0.10)  
(7): (6)/8.64 x n  
(8): Block A; irrigable area 142 ha x (7)  
(9): Block B; irrigable area 249 ha x (7)  
(10): Block C; irrigable area 609 ha x (7)  
(11): (8) + (9) + (10)

Table A. 4-19 Irrigation Water Requirements for Paddy Fields  
(Return Period: 10-years)

Month	Field Water Requirements (mm/day) (mm/half mon)		Effective Rainfall (mm/half mon)	Net Water Requirements (mm/half mon)	Gross Water Requirements			Irrigation Water Requirements			
					Farm Gate Water Requirements (mm/half mon)	Diversion Water Requirements		Block A (cu.m/s)	Block B (cu.m/s)	Block C (cu.m/s)	Total (cu.m/s)
						(mm/half mon)	(l/sec/ha)				
Jan. I	2.0	30.0	-	30.0	37.5	45.0	0.347	1.00	0.81	1.50	3.31
II	6.5	104.0	-	104.0	130.0	156.2	1.130	3.26	2.64	4.88	10.78
Feb. I	9.0	135.0	13.1	121.9	152.4	183.1	1.413	4.08	3.30	6.11	13.49
II	9.0	117.0	15.2	101.8	127.3	152.9	1.362	3.93	3.18	5.89	13.00
Mar. I	7.0	105.0	6.4	98.6	123.3	148.1	1.143	3.30	2.67	4.94	10.91
II	6.8	108.8	4.5	104.3	130.4	156.6	1.133	3.27	2.64	4.90	10.81
Apr. I	7.6	114.0	10.1	103.9	130.0	156.2	1.205	3.48	2.81	5.21	11.50
II	7.4	111.0	30.0	81.0	101.3	121.7	0.939	2.71	2.19	4.06	8.96
May I	4.6	69.0	2.7	66.3	82.9	99.6	0.769	2.22	1.79	3.32	7.33
II	2.3	36.8	29.4	7.4	9.3	11.2	0.081	0.23	0.19	0.35	0.77
Jun. I	0.2	3.0	53.6	-	-	-	-	-	-	-	-
II	-	-	21.8	-	-	-	-	-	-	-	-
Jul. I	1.8	27.0	27.1	-	-	-	-	-	-	-	-
II	6.4	102.4	30.0	72.4	90.5	108.7	0.786	2.27	1.83	3.40	7.50
Aug. I	8.2	123.0	23.2	99.8	124.8	149.9	1.157	3.34	2.70	5.00	11.04
II	7.8	104.8	77.4	27.4	34.3	41.2	0.298	0.86	0.70	1.29	2.85
Sep. I	5.4	81.0	84.5	-	-	-	-	-	-	-	-
II	5.3	84.8	116.6	-	-	-	-	-	-	-	-
Oct. I	5.5	82.5	53.6	28.9	36.1	43.4	0.335	0.97	0.78	1.45	3.20
II	5.4	86.4	54.4	32.0	40.0	48.0	0.347	1.00	0.81	1.50	3.31
Nov. I	3.7	55.5	10.3	45.2	56.5	67.9	0.524	1.51	1.22	2.27	5.00
II	1.8	27.0	1.6	25.4	31.8	38.2	0.295	0.85	0.69	1.28	
Dec. I	0.2	3.0	-	3.0	3.8	4.6	0.035	0.10	0.08	0.15	0.33
II	-	-	-	-	-	-	-	-	-	-	-

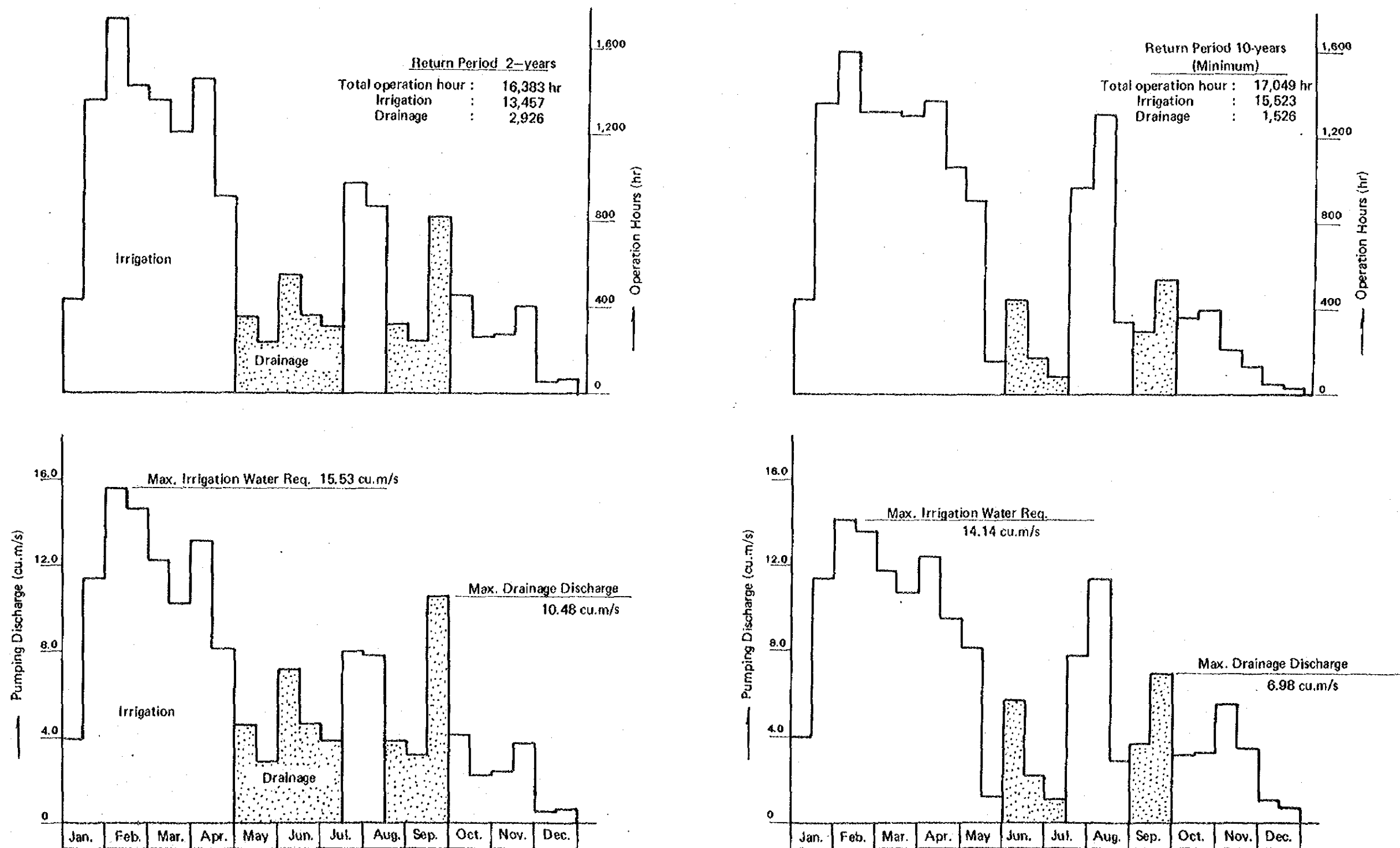
Note: (1): Average field water requirement, in millimeter per day (see Table A. 4-13)  
(2): Average field water requirement, in millimeter per half month  
(3): Effective rainfall in millimeter per half month (see Appendix A. 4-15)  
(4): (2) - (3)  
(5): (4)/(1 - 0.2)  
(6): (5)/(1 - 0.075)(1 - 0.10)  
(7): (6)/8.64 x n  
(8): Block A; irrigable area 2,885 ha x (7)  
(9): Block B; irrigable area 2,334 ha x (7)  
(10): Block C; irrigable area 4,323 ha x (7)  
(11): (8) + (9) + (10)

Table A. 4-20 Irrigation Water Requirements for Upland Crops  
(Return Period: 10-years)

Month	Field Water Requirements		Effective Rainfall (mm/half mon)	Net Water Requirements (mm/half mon)	Gross Water Requirements			Irrigation Water Requirements			Total (cu.m/s)	
	(mm/day)	(mm/half mon)			Farm Gate Water Requirements (mm/half mon)	Diversion Water Requirements		Block A (cu.m/s)	Block B (cu.m/s)	Block C (cu.m/s)		
	(1)	(2)			(5)	(6)	(7)	(8)	(9)	(10)		(11)
Jan.	I	3.1	46.5	-	46.5	71.5	85.9	0.663	0.09	0.17	0.40	0.66
	II	3.1	49.6	-	49.6	76.3	91.7	0.663	0.09	0.17	0.40	0.66
Feb.	I	3.7	55.6	10.0	45.6	70.2	84.2	0.650	0.09	0.16	0.40	0.65
	II	3.7	48.1	12.0	36.1	55.5	66.7	0.594	0.08	0.15	0.36	0.59
Mar.	I	4.2	63.0	5.0	58.0	89.2	107.1	0.826	0.11	0.21	0.50	0.82
	II	4.2	67.2	3.5	63.7	98.0	117.7	0.851	0.12	0.21	0.52	0.85
Apr.	I	4.6	69.0	9.4	59.6	91.7	110.2	0.850	0.12	0.21	0.52	0.85
	II	4.6	69.0	31.0	38.0	58.5	70.3	0.542	0.08	0.13	0.33	0.54
May	I	4.0	60.0	2.5	57.5	88.5	106.3	0.820	0.12	0.20	0.50	0.82
	II	4.0	64.4	28.5	35.9	55.2	66.3	0.480	0.07	0.12	0.29	0.48
Jun.	I	3.7	55.5	50.0	5.5	8.5	10.2	0.078	0.01	0.02	0.08	0.11
	II	3.7	55.5	21.0	34.5	53.1	63.8	0.492	0.07	0.12	0.30	0.49
Jul.	I	3.4	51.0	26.1	24.9	38.3	46.0	0.355	0.05	0.09	0.22	0.39
	II	3.4	54.4	30.9	23.5	36.2	43.5	0.315	0.04	0.08	0.19	0.31
Aug.	I	3.2	48.0	22.1	25.9	39.8	47.8	0.369	0.05	0.09	0.22	0.36
	II	3.2	51.2	69.0	-	-	-	-	-	-	-	-
Sep.	I	3.0	45.0	74.4	-	-	-	-	-	-	-	-
	II	3.0	45.0	96.1	-	-	-	-	-	-	-	-
Oct.	I	3.1	46.5	56.8	-	-	-	-	-	-	-	-
	II	3.1	49.6	57.4	-	-	-	-	-	-	-	-
Nov.	I	3.2	48.0	9.0	39.0	60.0	72.1	0.556	0.08	0.14	0.34	0.56
	II	3.2	48.0	1.3	46.7	71.8	86.2	0.665	0.09	0.17	0.40	0.66
Dec.	I	3.1	46.5	-	46.5	71.5	85.9	0.663	0.09	0.17	0.40	0.66
	II	3.1	49.6	-	49.6	76.3	91.7	0.663	0.09	0.17	0.40	0.66

Note: (1): Average field water requirements, in millimeter per day (see Table A.4-13)  
 (2): Average field water requirements, in millimeter per half month  
 (3): Effective rainfall, in millimeter per half month, (see Appendix A. 4-15)  
 (4): (2) - (3)  
 (5): (4)/(1 - 0.35)  
 (6): (5)/(1 - 0.075)(1 - 0.10)  
 (7): (6)/8.64 x n  
 (8): Block A; irrigable area 142 ha x (7)  
 (9): Block B; irrigable area 249 ha x (7)  
 (10): Block C; irrigable area 609 ha x (7)  
 (11): (8) + (9) + (10)

FIGURE A.4-6 AVERAGE PUMPING DISCHARGE AND PUMP OPERATION HOURS



Note: Detail estimation is given in Table A.4 - 21



Return Period: 2-years

Month	Pumping Discharge (cu.m/s)			Operation Hours of Pumps (hr)			
	Block A	Block B	Block C	Block A	Block B	Block C	Total
Jan. I	1.09 <sup>a/</sup>	0.98	1.90	141	147	151	439
Jan. II	3.35	2.81	5.28	461	448	450	1,359
Feb. I	4.57	3.80	7.16	590	567	572	1,729
Feb. II	4.30	3.57	6.71	481	463	464	1,408
Mar. I	3.54	2.99	5.56	458	447	452	1,357
Mar. II	2.96	2.49	4.71	408	397	402	1,207
Apr. I	3.79	3.19	6.05	489	477	483	1,449
Apr. II	2.39	1.99	3.76	308	297	300	905
May I	(1.29) <sup>b/</sup>	(1.12)	(2.07)	(117)	(117)	(116)	(350)
May II	(0.81)	(0.70)	(1.50)	(78)	(78)	(77)	(233)
Jun. I	(2.03)	(1.75)	(3.29)	(183)	(183)	(180)	(546)
Jun. II	(1.33)	(1.14)	(2.14)	(120)	(120)	(120)	(360)
Jul. I	(1.12)	(0.97)	(1.81)	(101)	(102)	(101)	(304)
Jul. II	2.43	2.02	3.80	334	321	324	979
Aug. I	2.37	1.92	3.55	305	287	283	875
Aug. II	(1.09)	(0.95)	(1.75)	(106)	(106)	(104)	(316)
Sep. I	(0.89)	(0.78)	(1.43)	(81)	(81)	(80)	(242)
Sep. II	(3.02)	(2.59)	(4.87)	(273)	(272)	(272)	(817)
Oct. I	1.22	0.99	1.83	158	149	146	453
Oct. II	0.67	0.54	1.00	93	86	85	264
Nov. I	0.59	0.59	1.13	89	89	90	268
Nov. II	1.00	0.90	1.77	129	135	141	405
Dec. I	0.07	0.12	0.30	9	18	24	51
Dec. II	0.09	0.12	0.40	13	19	34	66
Total				4,548 (978)	4,428 (978)	4,461 (970)	13,437 (2,926)
				5,526	5,406	5,451	16,383

Note: a/ : derived from Table A. 4-17 and A. 4-18

b/ : derived from Table A. 4-23

c/ : 1.09 cu.m/s x 86,400 x 15/2.79 cu.m/s (pump capacity) x 3,600 pump capacity:

Block A: 2.79 cu.m/s (3.99) x 3 units (41,350)

Block B: 2.41 cu.m/s (3.44) x 3 units (41,350)

Block C: 4.51 cu.m/s (6.44) x 3 units (41,650)

( ) shows the drainage discharge and operation hours of pumps for drainage

Table A. 4-22 Average Operation Hours of the Proposed Pump

Return Period: 10-years (Minimum)	Month	Pumping Discharge (cu.m/s)			Operation Hours of Pumps (hr)			
		Block A	Block B	Block C	Block A	Block B	Block C	Total
Jan.	I	1.09	0.98	1.90	141	147	152	440
	II	3.35	2.81	5.28	461	448	450	1,359
Feb.	I	4.17	3.46	6.51	540	534	527	1,601
	II	4.01	3.33	6.25	449	432	433	1,314
Mar.	I	3.41	2.88	5.44	440	431	434	1,305
	II	3.39	2.85	4.42	467	454	376	1,297
Apr.	I	3.60	3.02	5.73	465	452	458	1,375
	II	2.79	2.32	4.39	360	347	351	1,058
May	I	2.34	1.99	3.82	302	297	305	904
	II	0.30	0.31	0.64	42	50	54	146
Jun.	I	(1.63)	(1.40)	(2.64)	(147)	(147)	(147)	(441)
	II	(0.62)	(0.54)	(1.00)	(56)	(57)	(56)	(159)
Jul.	I	(0.31)	(0.27)	(0.49)	(29)	(29)	(27)	(85)
	II	2.31	1.91	3.59	318	342	306	966
Aug.	I	3.39	2.79	5.22	438	459	417	1,314
	II	0.86	0.70	1.29	118	112	110	340
Sep.	I	(1.07)	(0.93)	(1.71)	(96)	(98)	(95)	(289)
	II	(2.01)	(1.74)	(3.23)	(180)	(182)	(180)	(542)
Oct.	I	0.97	0.78	1.45	125	117	116	358
	II	1.00	0.81	1.50	138	130	128	396
Nov.	I	1.59	1.36	2.61	206	266	209	681
	II	0.94	0.86	1.68	130	216	134	480
Dec.	I	0.21	0.27	0.59	27	41	47	115
	II	0.09	0.17	0.40	13	27	34	74
		Total			5,180	5,302	5,041	15,523
					(508)	(513)	(505)	(1,526)
					5,688	5,815	5,546	17,049

Table A. 4-23 Drainage Discharge for Project Area

Return Period: 2-years

Month	Rainfall (mm) (1)	Cultivated Area					Others					Total Drainage Discharge (cu.m/s) (17)					
		Field Water Requirements			Drain Water		Drainage Discharge			Drain Water			Drainage Discharge				
		Paddy Fields (mm) (2)	Upland Fields (mm) (3)	Average (mm) (4)	mm (5)	l/sec/ha (6)	Block A (cu.m/s) (7)	Block B (cu.m/s) (8)	Block C (cu.m/s) (9)	Sub- Total (cu.m/s) (10)	mm (11)		l/sec/ha (12)	Block A (cu.m/s) (13)	Block B (cu.m/s) (14)	Block C (cu.m/s) (15)	Sub- Total (cu.m/s) (16)
		(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)	(14)	(15)	(16)
May I	113.1	69.0	60.0	68.1	45.0	0.347	1.05	0.90	1.71	3.66	73.5	0.567	0.24	0.22	0.36	0.82	4.48
II	69.9	36.8	64.4	39.4	30.5	0.221	0.67	0.57	1.09	2.33	45.4	0.328	0.14	0.13	0.21	0.48	2.81
Jun. I	87.2	3.0	55.5	8.0	79.2	0.611	1.85	1.58	3.01	6.44	56.7	0.437	0.18	9.17	0.28	0.63	7.07
II	56.8	-	55.5	5.3	51.5	0.397	1.21	1.03	1.96	4.20	36.9	0.285	0.12	0.11	0.18	0.41	4.61
Jul. I	70.9	27.0	51.0	29.3	41.6	0.321	0.97	0.83	1.58	3.38	46.1	0.356	0.15	0.14	0.23	0.52	3.90
II	35.1	102.4	54.4	97.8	-	-	-	-	-	-	22.8	0.165	0.07	0.06	0.11	0.24	0.24
Aug. I	69.5	123.0	48.0	115.9	-	-	-	-	-	-	45.2	0.349	0.15	0.14	0.22	0.51	0.51
II	137.2	104.8	51.2	99.7	37.5	0.271	0.82	0.70	1.34	2.86	39.2	0.645	0.27	0.25	0.41	0.93	3.79
Sep. I	106.1	81.0	45.0	77.6	28.5	0.220	0.67	0.57	1.09	2.33	69.0	0.532	0.22	0.21	0.34	0.77	3.10
II	192.6	84.8	45.0	81.0	111.6	0.861	2.61	2.22	4.25	9.08	125.2	0.966	0.41	0.37	0.62	1.40	10.48
Oct. I	70.9	82.5	46.5	79.1	-	-	-	-	-	-	46.1	0.356	0.15	0.14	0.23	0.52	0.52
II	85.6	86.4	49.6	82.9	2.7	0.020	0.06	0.05	0.10	0.21	55.6	0.402	0.17	0.16	0.26	0.59	0.80

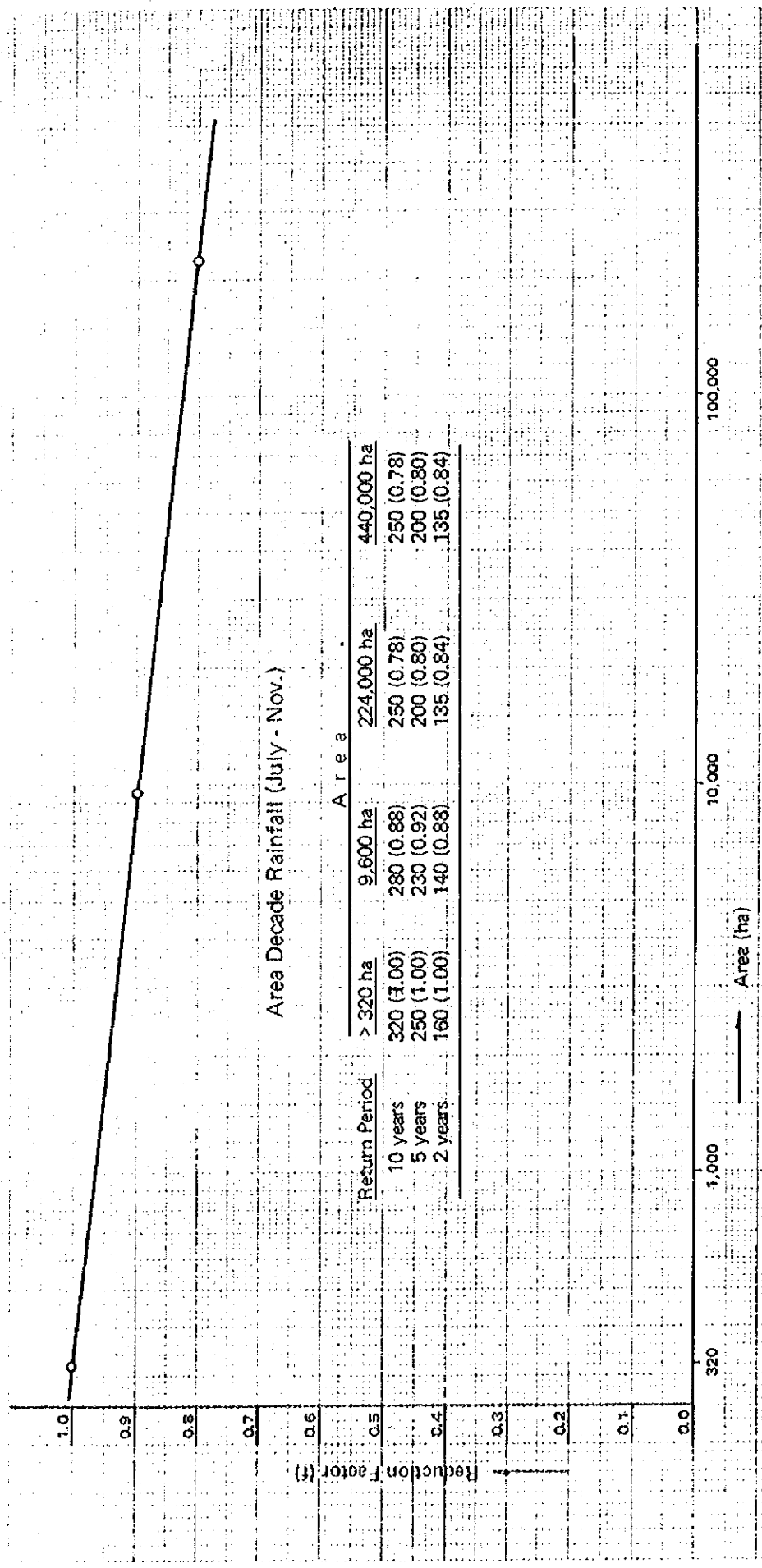
Return Period: 10-years (Minimum)

Month	Rainfall (mm) (1)	Cultivated Area					Others					Total Drainage Discharge (cu.m/s) (17)					
		Field Water Requirements			Drain Water		Drainage Discharge			Drain Water			Drainage Discharge				
		Paddy Fields (mm) (2)	Upland Fields (mm) (3)	Average (mm) (4)	mm (5)	l/sec/ha (6)	Block A (cu.m/s) (7)	Block B (cu.m/s) (8)	Block C (cu.m/s) (9)	Sub- Total (cu.m/s) (10)	mm (11)		l/sec/ha (12)	Block A (cu.m/s) (13)	Block B (cu.m/s) (14)	Block C (cu.m/s) (15)	Sub- Total (cu.m/s) (16)
		(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)	(14)	(15)	(16)
May I	3.6	69.0	60.0	68.1	-	-	-	-	-	-	2.3	0.018	0.01	0.01	0.01	0.03	0.03
II	39.1	36.8	64.4	39.4	-	-	-	-	-	-	25.4	0.184	0.08	0.07	0.12	0.27	0.27
Jun. I	71.4	3.0	55.5	8.0	63.4	0.489	1.48	1.26	2.41	5.15	46.4	0.358	0.15	0.14	0.23	0.52	5.67
II	29.1	-	55.5	5.3	23.8	0.184	0.56	0.48	0.91	1.95	18.9	0.146	0.06	0.06	0.09	0.21	2.16
Jul. I	39.0	27.0	51.0	29.3	9.7	0.075	0.23	0.19	0.37	0.79	25.4	0.196	0.08	0.08	0.12	0.28	1.07
II	40.1	102.4	54.4	97.8	-	-	-	-	-	-	26.1	0.189	0.08	0.07	0.12	0.27	0.27
Aug. I	31.1	123.0	48.0	115.9	-	-	-	-	-	-	20.2	0.156	0.07	0.06	0.10	0.23	0.23
II	103.2	104.8	51.2	99.7	3.5	0.025	0.08	0.06	0.12	0.26	67.1	0.485	0.21	0.19	0.31	0.71	0.97
Sep. I	113.1	81.0	45.0	77.6	35.5	0.274	0.83	0.71	1.35	2.89	73.5	0.567	0.24	0.22	0.36	0.82	3.71
II	152.9	84.8	45.0	81.0	71.9	0.555	1.68	1.44	2.74	5.86	99.4	0.767	0.33	0.30	0.49	1.12	6.98
Oct. I	76.9	82.5	46.5	79.1	-	-	-	-	-	-	50.0	0.386	0.16	0.15	0.25	0.56	0.56
II	83.0	86.4	49.6	83.0	-	-	-	-	-	-	54.0	0.391	0.16	0.15	0.25	0.56	0.56

Note: (1) Average half month rainfall at three stations, Sing Ha Nat, Phraya Banlu and Lakkhon, in each probable year  
 (2) See Table A. 4-17  
 (3) See Table A. 4-18  
 (4) (2)x0.905+(3)x0.095  
 (5) (1)-(4)  
 (6) (5)/8.64 x n  
 (7) 3,027 ha x (6)  
 (8) 2,583 ha x (6)  
 (9) 4,932 ha x (6)

(10) (7) + (8) + (9)  
 (11) (1) x run-off coefficient 0.65 (assumed)  
 (12) (11)/8.64 x n  
 (13) 422 ha x (12)  
 (14) 387 ha x (12)  
 (15) 637 ha x (12)  
 (16) (13) + (14) + (15)  
 (17) (10) + (16)

FIGURE A.4-6 AREA - REDUCTION FACTOR FOR DRAINAGE MODULUS



Source: Northern Chao Phya study report prepared by NEDECO, 1970

Runoff Analysis by Mononobe's Graphic Method

Basic equation is shown as follows;

$$\frac{(I_1 + I_2)}{2} \Delta t - \frac{(Q_1 + Q_2)}{2} \Delta t = V_2 - V_1 \dots\dots (1)$$

- Where;  $I_1$ : Inflow at time  $t_1$   
 $I_2$ : Inflow at time  $t_2$   
 $Q_1$ : Outflow at time  $t_1$   
 $Q_2$ : Outflow at time  $t_2$   
 $t$ : too short time considered to be linear in changing of inflow and outflow  
 $V_1$ : Field surface storage at time  $t_1$   
 $V_2$ : Field surface storage at time  $t_2$

The above equation shall be transformed into:

$$\phi = V + Q \frac{\Delta t}{2}, \quad \psi = V - Q \frac{\Delta t}{2}$$

and presented in a form of a general equation as below:

$$\phi_{n+1} = \phi_n + (I_n + I_{n+1}) \frac{\Delta t}{2} \dots\dots\dots (2)$$

The equation (2) is expressed in the following diagram which indicates the procedure of estimation of run-off discharges through notches of the paddy fields.

**ILLUSTRATION OF PROCEDURE FOR RUN-OFF DISCHARGE**

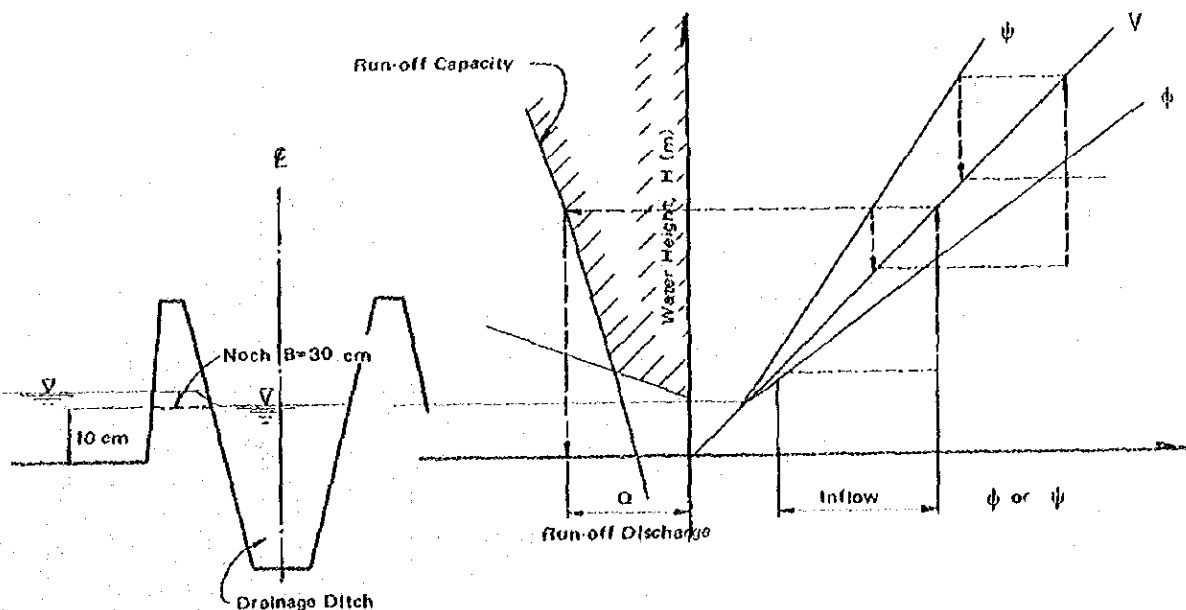


Table A. 4-24 Drainage Conditions at Each Pumping Rate (Block A)

Pumping Rate mm/hr	Maximum 1/ Water Level m	Maximum 2/ Water Depth m	Inundation 3/ Period hr	Inundation 4/ Area ha	Max. Water Depth 5/ of Each Field m
<b>Case 1 : B = 0.20 m (width of notch)</b>					
q = 1.20	2.24	0.34	105 (4.4) days	2,132	0.190
1.25	2.24	0.34	100 (4.2)	2,052	0.190
1.30	2.23	0.33	95 (4.0)	1,972	0.180
1.35	2.23	0.33	90 (3.8)	1,893	0.180
1.40	2.22	0.32	85 (3.5)	1,815	0.170
1.45	2.22	0.32	81 (3.4)	1,738	0.170
1.50	2.21	0.21	76 (3.2)	1,662	0.160
<b>Case 2 : B = 0.30 m (width of notch)</b>					
q = 1.20	2.26	0.36	114 (4.8) days	2,465	0.210
1.25	2.25	0.35	109 (4.5)	2,387	0.200
1.30	2.25	0.35	104 (4.3)	2,310	0.200
1.35	2.25	0.35	100 (4.2)	2,233	0.200
1.40	2.24	0.34	96 (4.0)	2,157	0.190
1.45	2.24	0.34	92 (3.8)	2,081	0.190
1.50	2.23	0.33	89 (3.7)	2,007	0.180

Note:  
 1/ : Lowest elevation of paddy field: EL. 1.70 m  
 2/ : Water depth above basic elevation of paddy field EL. 1.90 m  
 3/ : Inundation periods above allowable water level EL. 2.15 m  
 4/ : Inundation area at maximum water level.  
 5/ : Maximum water depth of each paddy fields controlled by notch and drainage ditch.

Table A. 4-25 Drainage Conditions at Each Pumping Rate (Block B)

Pumping Rate mm/hr	Maximum 1/		Maximum 2/ Water Depth m	Inundation 3/ Period hr	Inundation 4/ Area ha	Max. Water Depth 5/ of Each Field m
	Water Level m	Width of notch				
<b>Case 1 : B = 0.20 m (Width of notch)</b>						
q = 1.20	2.00		0.30	64 (2.6)	1,594	0.215
1.25	1.99		0.29	59 (2.5)	1,557	0.215
1.30	1.99		0.29	53 (2.2)	1,520	0.215
1.35	1.98		0.28	47 (2.0)	1,484	0.215
1.40	1.97		0.27	41 (1.7)	1,448	0.215
1.45	1.97		0.27	34 (1.4)	1,413	0.215
1.50	1.96		0.26	14 (0.6)	1,378	0.215
<b>Case 2 : B = 0.30 m (Width of notch)</b>						
q = 1.20	2.02		0.32	81 (3.4)	1,710	0.211
1.25	2.01		0.31	76 (3.2)	1,683	0.211
1.30	2.01		0.31	68 (2.8)	1,657	0.211
1.35	2.01		0.31	63 (2.6)	1,630	0.211
1.40	2.00		0.30	59 (2.5)	1,604	0.211
1.45	1.99		0.29	55 (2.3)	1,571	0.211
1.50	1.99		0.29	51 (2.1)	1,537	0.211

Note: 1/ : Lowest elevation of paddy field : EL. 1.65 m  
 2/ : Water depth above basic elevation of paddy field EL. 1.70 m  
 3/ : Inundation periods above allowable water level EL. 1.95 m  
 4/ : Inundation area at maximum water level  
 5/ : Maximum water depth of each paddy fields controlled by notch and drainage ditch

Table A. 4-26 Drainage Conditions at Each Pumping Rate (Block C)

Pumping Rate mm/hr	Maximum 1/ Water Level m	Maximum 2/ Water Depth m	Inundation 3/ Period days	Inundation 4/ Area ha	Max. Water Depth 5/ of Each Field m
<u>Case 1 : B = 0.20 m (width of notch)</u>					
q = 1.20	1.83	0.33	91 (3.8)	2,915	0.213
1.25	1.83	0.33	85 (3.5)	2,844	0.213
1.30	1.82	0.32	80 (3.3)	2,774	0.213
1.35	1.82	0.32	73 (3.0)	2,704	0.213
1.40	1.81	0.31	66 (2.8)	2,636	0.213
1.45	1.81	0.31	61 (2.5)	2,568	0.213
1.50	1.80	0.30	57 (2.4)	2,501	0.213
<u>Case 2 : B = 0.30 m (width of notch)</u>					
q = 1.20	1.86	0.36	104 (4.3)	3,208	0.209
1.25	1.85	0.35	99 (4.1)	3,139	0.209
1.30	1.85	0.35	95 (4.0)	3,071	0.209
1.35	1.84	0.34	91 (3.8)	3,003	0.209
1.40	1.84	0.34	86 (3.6)	2,936	0.209
1.45	1.83	0.33	82 (3.4)	2,870	0.209
1.50	1.83	0.33	79 (3.3)	2,799	0.209

Note: 1/ : Lowest elevation of paddy field : EL. 1.40 m  
 2/ : Water depth above basic elevation of paddy field EL. 1.50 m  
 3/ : Inundation periods above allowable water level EL. 1.75 m  
 4/ : Inundation area at maximum water level.  
 5/ : Maximum water depth of each paddy fields controlled by notch and drainage ditch



FIGURE A.4-7 STAGE - AREA AND STORAGE CURVE (BLOCK A)

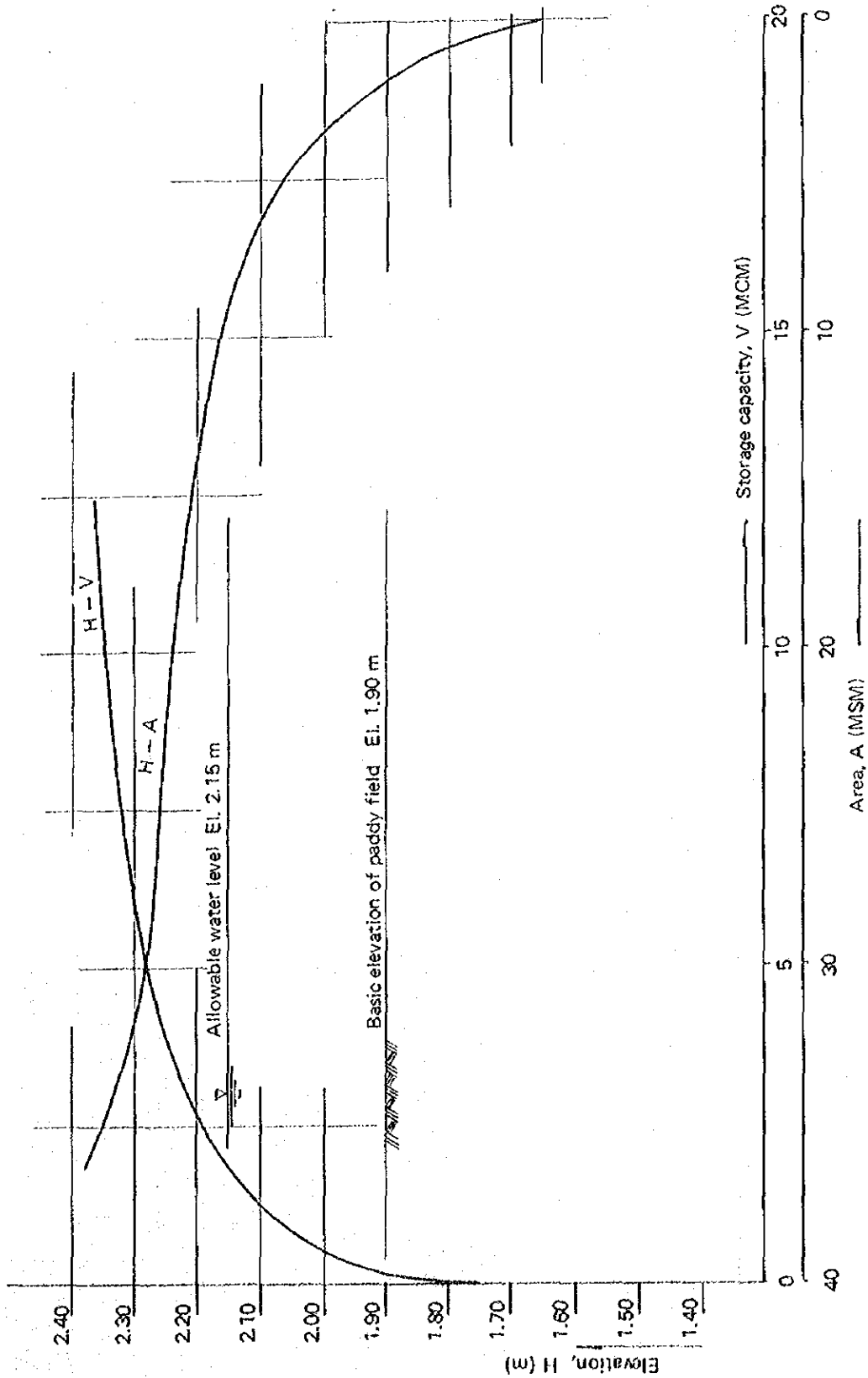


FIGURE A.4-8 STAGE - AREA AND STORAGE CURVE (BLOCK B)

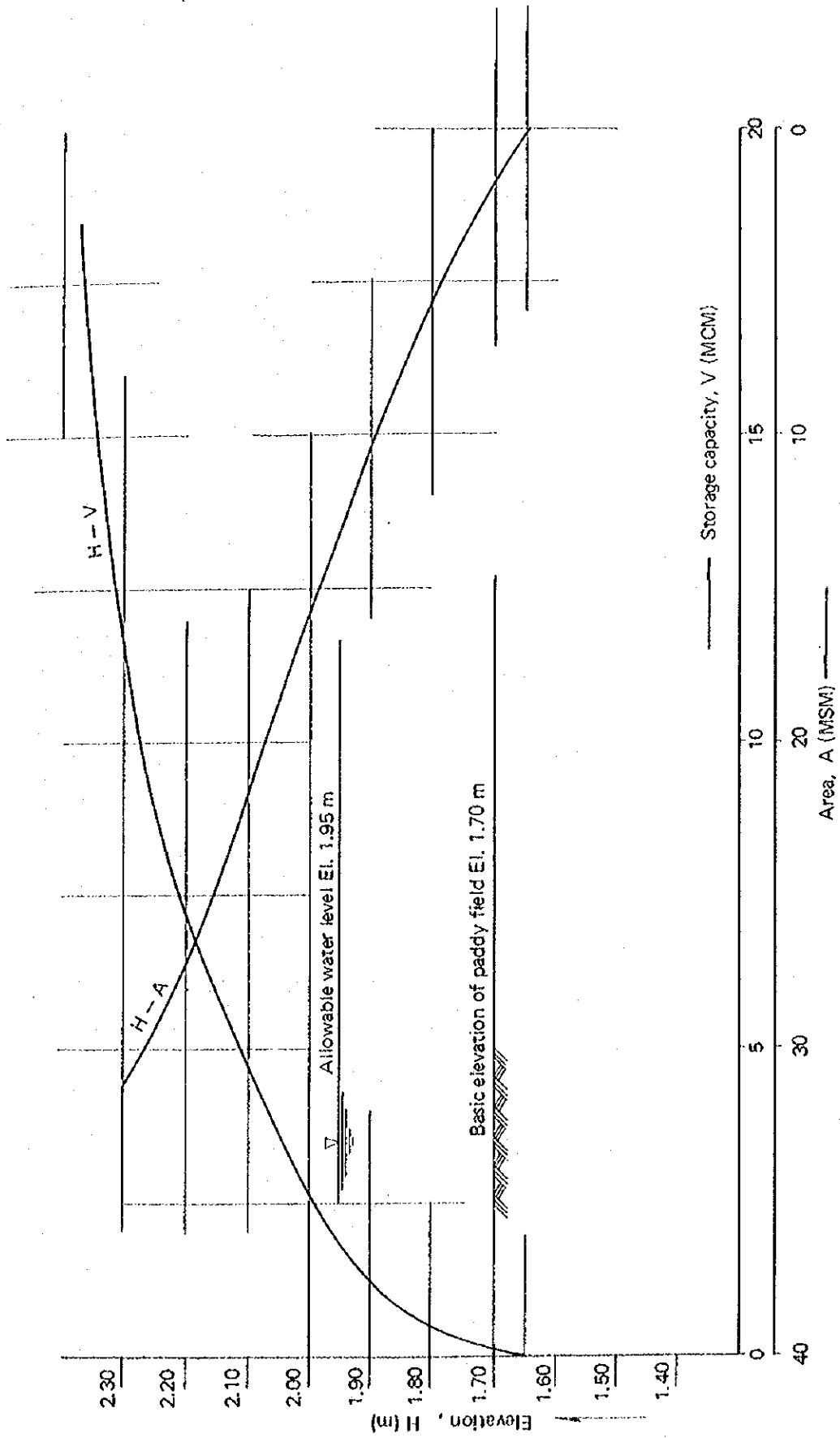
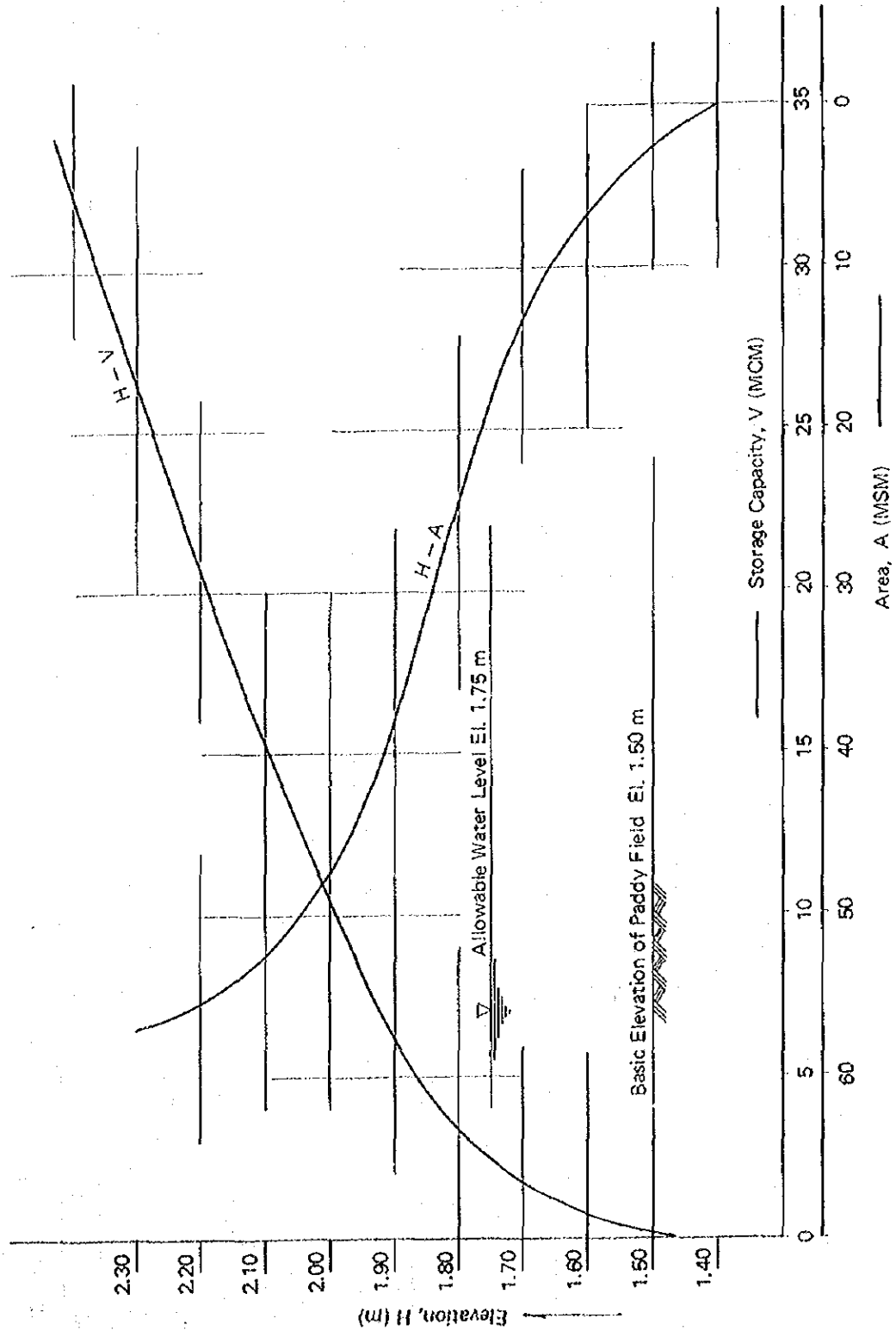
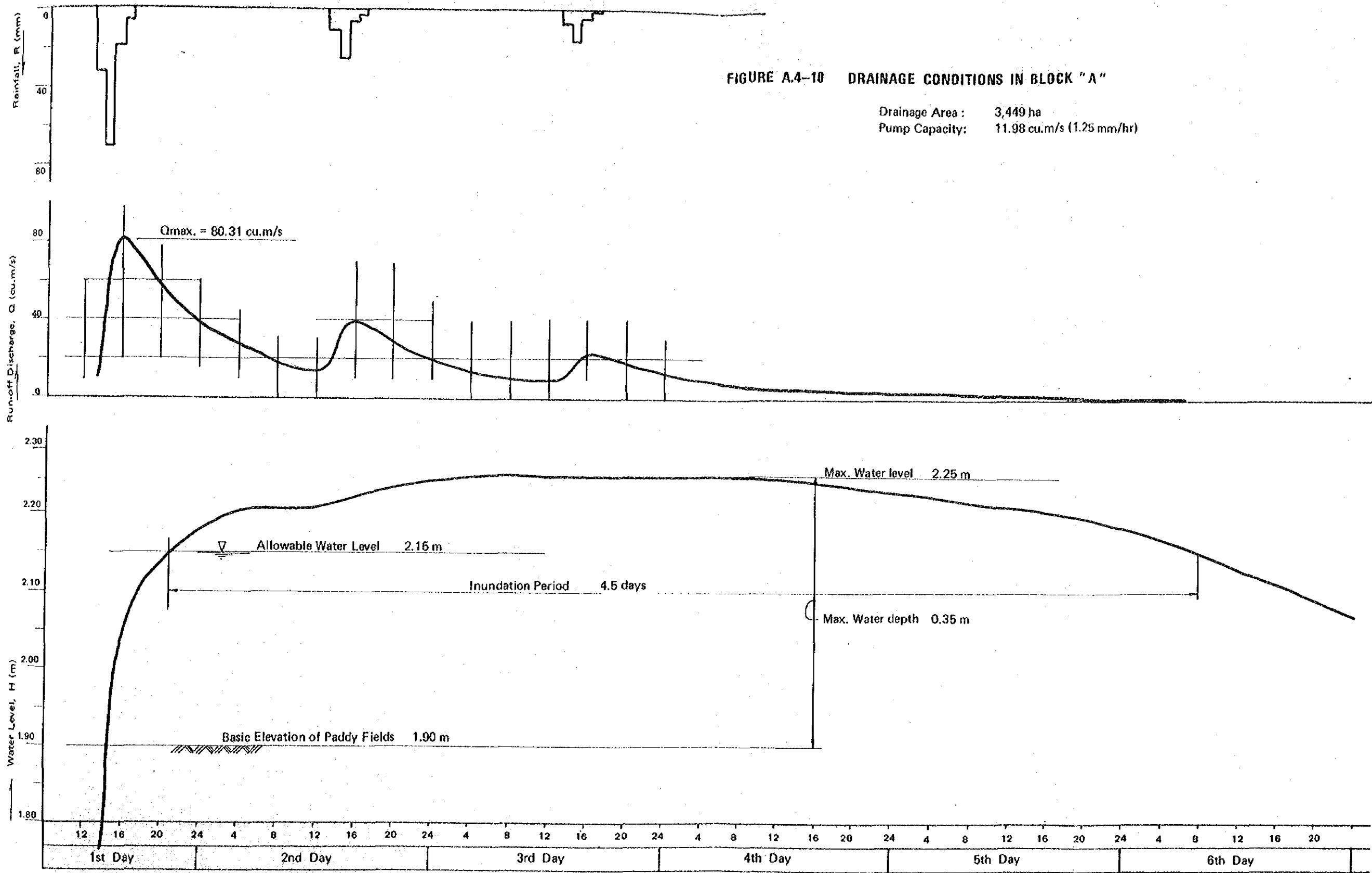
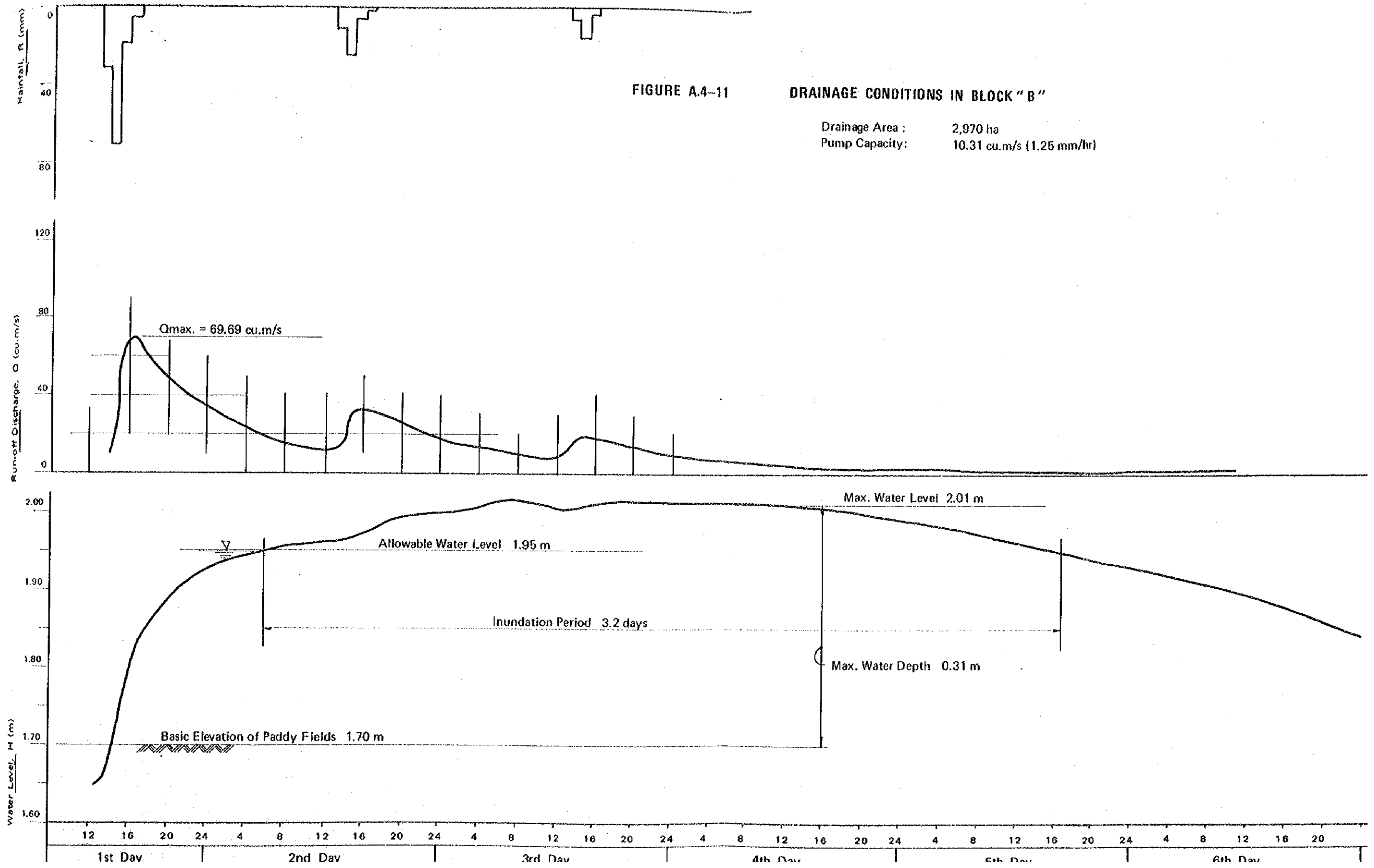


FIGURE A. 4-9 STAGE - AREA AND STORAGE CURVE (BLOCK C)







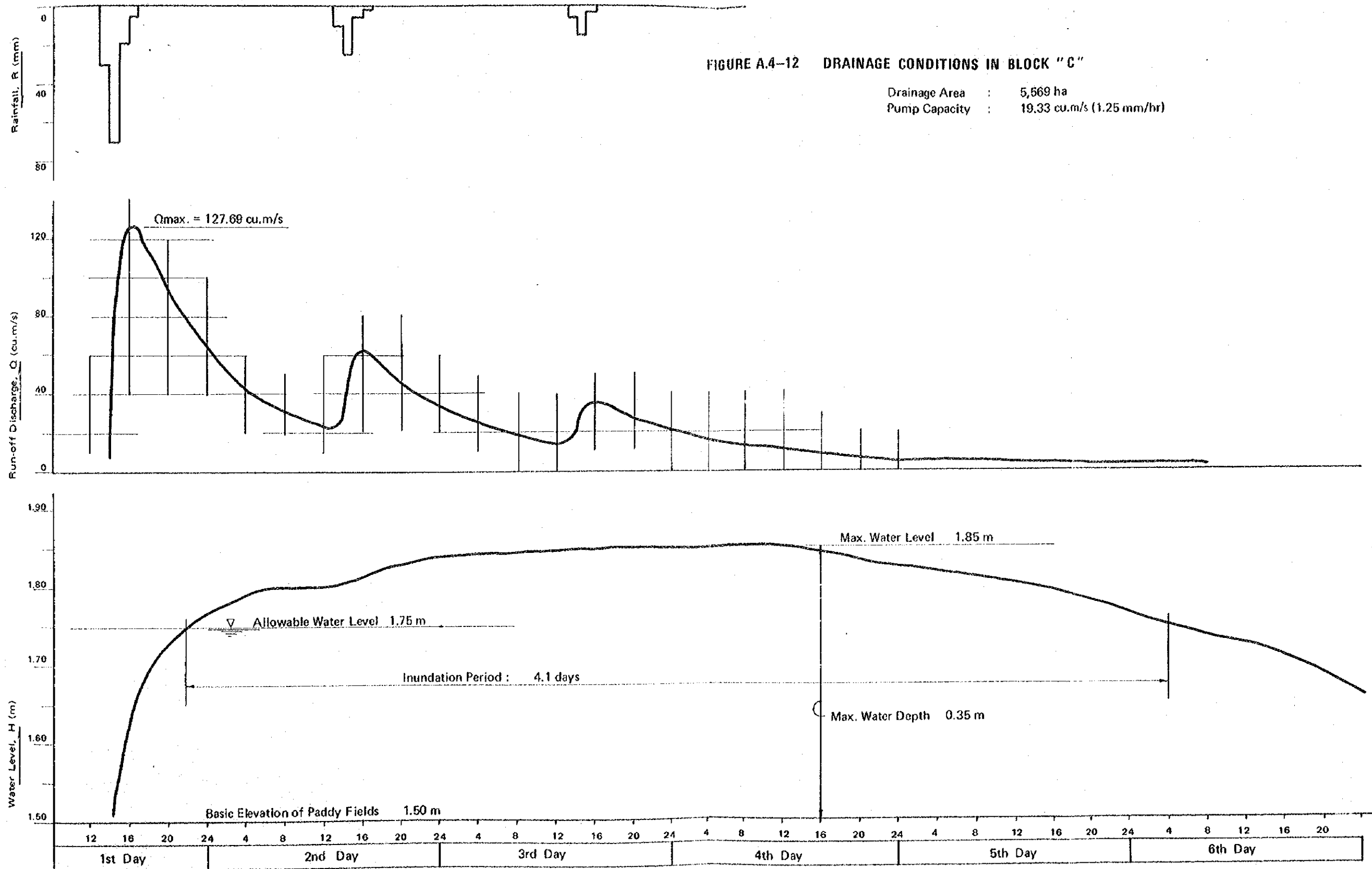


FIGURE A.4-13 AVERAGE PUMPING DISCHARGE AND OPERATION HOURS

Return Period:  
10 - years (maximum)  
Total operation hour: 17,620 hr  
Irrigation : 12,391  
Drainage : 5,229

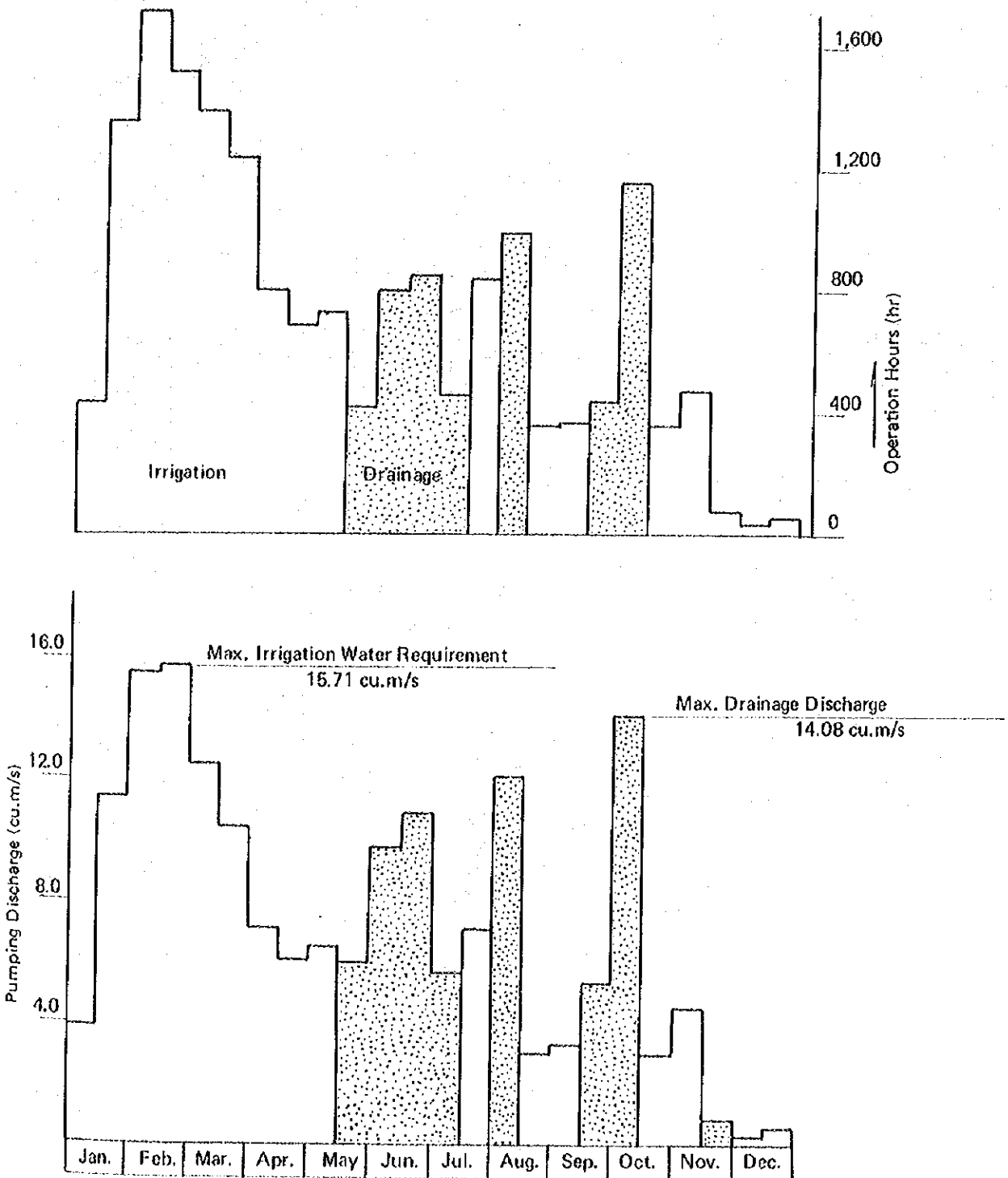


Table A. 4-27 Average Operation Hours of the Proposed Pump

Month	Pumping Discharge (cu.m/s)			Operation Hours of Pumps			
	Block A	Block B	Block C	Block A	Block B	Block C	Total
Jan. I	1.08	0.98	1.90	141	147	151	439
Jan. II	3.35	2.81	5.28	461	448	450	1,359
Feb. I	4.56	3.79	7.14	588	566	570	1,724
Feb. II	4.82	3.85	7.24	517	499	501	1,516
Mar. I	3.84	3.06	5.81	470	457	464	1,391
Mar. II	3.05	2.56	4.85	420	408	413	1,242
Apr. I	2.12	1.75	3.30	274	261	263	798
Apr. II	1.82	1.51	2.82	235	226	225	686
May I	1.88	1.61	3.09	242	240	247	729
May II	(1.44) $\frac{2}{3}$	(1.24)	(2.32)	(139)	(138)	(138)	(415)
Jun. I	(2.82)	(2.42)	(4.56)	(294)	(295)	(285)	(804)
Jun. II	(3.15)	(2.71)	(5.10)	(284)	(284)	(285)	(853)
Jul. I	(1.63)	(1.41)	(2.64)	(147)	(170)	(148)	(465)
Jul. II	2.11	1.73	3.26	290	275	278	843
Aug. I	(3.47)	(3.00)	(5.61)	(313)	(363)	(314)	(990)
Aug. II	0.92	0.74	1.37	127	119	116	362
Sep. I	1.02	0.82	1.53	122	122	122	375
Sep. II	(1.56)	(1.34)	(2.49)	(141)	(153)	(139)	(443)
Oct. I	(4.05)	(3.49)	(6.54)	(390)	(424)	(366)	(1,180)
Oct. II	0.91	0.73	1.36	125	116	116	357
Nov. I	1.23	1.04	2.01	158	155	160	473
Nov. II	(0.28)	(0.24)	(0.45)	(25)	(29)	(25)	(79)
Dec. I	0.05	0.09	0.22	6	13	18	37
Dec. II	0.09	0.17	0.40	13	14	34	61
Total				4,198	4,065	4,128	12,391
				(1,693)	(1,866)	(1,670)	(5,229)
				5,891	5,961	5,798	17,620

Note: a/ : derived from Table A. 4-28 and A. 4-29

b/ : derived from Table A. 4-31

c/ : 1.09 cu.m/s x 96,400 x 15/2.79 cu.m/s (pump capacity) x 3,600

pump capacity:

Block A : 2.79 cu.m/s (3.99) x 3 units (61,350)

Block B : 2.41 cu.m/s (3.24) x 3 units (61,350)

Block C : 4.51 cu.m/s (6.44) x 3 units (61,350)

( ) shows the drainage discharge and summation hours of pumps for drainage



Table A. 4-28 Irrigation Water Requirements for Paddy Fields  
(Return Period: 10-years)

Month	Field Water Requirement		Effective Rainfall (mm/half mon)	Net Water Requirements (mm/half mon)	Gross Water Requirements			Irrigation Water Requirements			Total (cu.m/s) (11)
	(mm/day)	(mm/half mon)			Farm Gate Water Requirements (mm/half mon)	Diversion Water Requirements (mm/half mon)	Requirements (l/sec/ha)	Block A (cu.m/s)	Block B (cu.m/s)	Block C (cu.m/s)	
	(1)	(2)			(5)	(6)	(7)	(8)	(9)	(10)	
Jan. I	2.0	30.0	-	30.0	37.5	45.0	0.347	1.00	0.81	1.50	3.31
II	6.5	104.0	-	104.0	130.0	156.2	1.130	3.26	2.64	4.88	10.78
Feb. I	9.0	135.0	1.8	133.2	166.5	200.0	1.543	4.45	3.60	6.67	14.72
II	9.0	117.0	-	117.0	146.3	175.7	1.564	4.51	3.65	6.76	14.92
Mar. I	7.0	105.0	-	105.0	131.3	157.7	1.217	3.51	2.84	5.26	11.61
II	6.8	108.8	15.1	93.7	117.1	140.7	1.018	2.94	2.38	4.40	9.72
Apr. I	7.6	114.0	51.8	62.2	77.8	93.5	0.721	2.08	1.68	3.12	6.88
II	7.4	111.0	57.5	53.5	66.9	80.4	0.620	1.79	1.45	2.68	5.92
May I	4.6	69.0	15.3	53.7	67.1	80.6	0.622	1.79	1.45	2.69	5.93
II	2.3	36.8	72.2	-	-	-	-	-	-	-	-
Jun. I	0.2	3.0	98.4	-	-	-	-	-	-	-	-
II	-	-	96.5	-	-	-	-	-	-	-	-
Jul. I	1.8	27.0	68.3	-	-	-	-	-	-	-	-
II	6.4	102.4	36.5	65.9	82.4	99.0	0.716	2.07	1.67	3.10	6.84
Aug. I	8.2	123.0	182.1	-	-	-	-	-	-	-	-
II	7.8	104.8	75.5	29.3	36.6	44.0	0.318	0.92	0.74	1.37	3.03
Sep. I	5.4	81.0	50.5	30.5	38.1	45.8	0.353	1.02	0.82	1.53	3.37
II	5.3	84.8	87.9	-	-	-	-	-	-	-	-
Oct. I	5.5	82.5	150.5	-	-	-	-	-	-	-	-
II	5.4	86.4	57.5	28.9	36.1	43.4	0.314	0.91	0.73	1.36	3.00
Nov. I	3.7	55.5	20.7	34.8	43.5	52.3	0.404	1.17	0.94	1.75	3.86
II	1.8	27.0	30.0	-	-	-	-	-	-	-	-
Dec. I	0.2	3.0	25.9	-	-	-	-	-	-	-	-
II	-	-	0.5	-	-	-	-	-	-	-	-

Note: (1): Average field water requirement, in millimeter per day (see Table A. 4-13)  
 (2): Average field water requirement, in millimeter per half month  
 (3): Effective rainfall in millimeter per half month (Table A. 4-30)  
 (4): (2) - (3)  
 (5): (4)/(1 - 0.2)  
 (6): (5)/(1 - 0.075)(1 - 0.10)  
 (7): (6)/8.64 x n  
 (8): Block A; irrigable area 2,885 ha x (7)  
 (9): Block B; irrigable area 2,334 ha x (7)  
 (10): Block C; irrigable area 4,323 ha x (7)  
 (11): (8) + (9) + (10)

Table A. 4-29 Irrigation Water Requirements for Upland Crops  
(Return Period: 10-years)

Month	Field Water Requirement		Effective Rainfall (mm/half mon)	Net Water Requirements (mm/half mon)	Gross Water Requirements			Irrigation Water Requirements			
	(mm/day)	(mm/half mon)			Farm Gate Water Requirements (mm/half mon)	Diversion Water Requirements (mm/half mon)	( $\ell$ /sec/ha)	Block A (cu.m/s)	Block B (cu.m/s)	Block C (cu.m/s)	Total (cu.m/s)
	(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)
Jan. I	3.1	46.5	-	46.5	71.5	85.9	0.663	0.09	0.17	0.40	0.66
II	3.1	49.6	-	49.6	76.3	91.7	0.663	0.09	0.17	0.40	0.66
Feb. I	3.7	55.6	1.5	54.1	83.2	99.9	0.771	0.11	0.19	0.47	0.77
II	3.7	48.1	-	48.1	74.0	88.9	0.791	0.11	0.20	0.48	0.79
Mar. I	4.2	63.0	-	63.0	96.9	116.4	0.898	0.13	0.22	0.55	0.90
II	4.2	67.2	11.7	55.5	85.4	102.6	0.742	0.11	0.18	0.45	0.74
Apr. I	4.6	69.0	48.5	20.5	31.5	37.8	0.292	0.04	0.07	0.18	0.29
II	4.6	69.0	53.0	16.0	24.6	29.5	0.227	0.03	0.06	0.14	0.23
May I	4.0	60.0	14.5	45.5	70.0	84.1	0.649	0.09	0.16	0.40	0.65
II	4.0	64.4	65.0	-	-	-	-	-	-	-	-
Jun. I	3.7	55.5	77.5	-	-	-	-	-	-	-	-
II	3.7	55.5	83.0	-	-	-	-	-	-	-	-
Jul. I	3.4	51.0	61.0	-	-	-	-	-	-	-	-
II	3.4	54.4	35.0	19.4	29.8	35.8	0.259	0.04	0.06	0.16	0.26
Aug. I	3.2	48.0	129.0	-	-	-	-	-	-	-	-
II	3.2	51.2	67.5	-	-	-	-	-	-	-	-
Sep. I	3.0	45.0	47.5	-	-	-	-	-	-	-	-
II	3.0	45.0	86.3	-	-	-	-	-	-	-	-
Oct. I	3.1	46.5	125.5	-	-	-	-	-	-	-	-
II	3.1	49.6	60.5	-	-	-	-	-	-	-	-
Nov. I	3.2	48.0	18.5	29.5	45.4	54.5	0.421	0.06	0.10	0.26	0.42
II	3.2	48.0	27.0	21.0	32.3	38.8	0.299	0.04	0.07	0.18	0.29
Dec. I	3.1	46.5	21.0	25.5	39.2	47.1	0.363	0.05	0.09	0.22	0.36
II	3.1	49.6	-	49.6	76.3	91.7	0.663	0.09	0.17	0.40	0.66

Note: (1): Average field water requirements, in millimeter per day (see Table A. 4-13)  
 (2): Average field water requirements, in millimeter per half month  
 (3): Effective rainfall, in millimeter per half month, (Table A. 4-30)  
 (4): (2) - (3)  
 (5): (4)/(1 - 0.35)  
 (6): (5)/(1 - 0.075)(1 - 0.10)  
 (7): (6)/8.64 x n  
 (8): Block A; irrigable area 142 ha x (7)  
 (9): Block B; irrigable area 249 ha x (7)  
 (10): Block C; irrigable area 609 ha x (7)  
 (11): (8) + (9) + (10)

Table A.4-30

Effective Rainfall (Return Period: 10-years)

(Unit: mm)

Month	Rainfall				Effective Rainfall			
	Sing Ha Nat. Reg.	Phraya Banlu Reg.	Lakkhon Reg.	Average (1)	Ratio of Effective Rainfall (2)	Effective Rainfall (3)=(1)x(2)	Upland Field (4)	
Nov.	I	0	77.7	0	25.9	0.80	20.7	18.5
	II	20.0	74.5	18.4	37.6	0.80	30.0	27.0
Dec.	I	67.5	0	18.9	28.8	0.90	25.9	21.0
	II	1.5	0	0	0.5	0.90	0.5	0
Jan.	I	0	0	0	0	0.90	0	0
	II	0	0	0	0	0.90	0	0
Feb.	I	6.1	0	0	2.0	0.90	1.8	1.5
	II	0	0	0	0	0.90	0	0
Mar.	I	0	0	0	0	0.90	0	0
	II	29.6	2.3	18.4	16.8	0.90	15.1	11.7
Apr.	I	89.5	69.9	47.6	69.0	0.75	51.8	48.5
	II	92.6	121.5	15.8	76.6	0.75	57.5	53.0
May	I	24.7	6.5	29.7	20.4	0.75	15.3	14.5
	II	130.2	79.9	78.5	96.3	0.75	72.2	65.0
Jun.	I	149.5	23.1	181.0	117.9	0.75	98.4	77.5
	II	94.9	86.5	204.8	128.6	0.75	96.5	83.0
Jul.	I	105.5	46.0	121.7	91.1	0.75	68.3	61.0
	II	22.4	112.1	11.6	48.6	0.75	36.5	35.0
Aug.	I	330.3	101.5	296.8	242.8	0.75	182.1	129.0
	II	74.3	155.4	72.4	100.7	0.75	75.5	67.5
Sep.	I	64.6	57.0	80.7	67.3	0.75	50.5	47.5
	II	196.6	61.7	147.8	135.2	0.65	87.9	86.3
Oct.	I	117.6	415.3	161.8	231.6	0.65	150.5	125.5
	II	46.2	148.3	71.8	88.7	0.65	57.5	60.5

Note: (4); estimated based on Figure A.4-4

Table A. 4-31 Drainage Discharge for Project Area

Return Period: 10-years (Maximum)

Month	Rainfall mm (1)	Cultivated Area									Others					Total Drainage Discharge cu.m/s (17)	
		Field Water Requirements			Drain Water		Drainage Discharge				Drain Water		Sub-				
		Paddy Fields mm (2)	Upland Fields mm (3)	Average mm (4)	mm (5)	ℓ/sec/ha (6)	Block A cu.m/s (7)	Block B cu.m/s (8)	Block C cu.m/s (9)	Sub- Total cu.m/s (10)	mm (11)	ℓ/sec/ha (12)	Block A cu.m/s (13)	Block B cu.m/s (14)	Block C cu.m/s (15)		Sub- Total cu.m/s (16)
Apr. I	69.0	114.0	69.0	109.7	-	-	-	-	-	-	44.9	0.346	0.15	0.13	0.22	0.50	0.50
II	76.6	111.0	69.0	107.0	-	-	-	-	-	-	49.8	0.384	0.16	0.15	0.24	0.55	0.55
May I	20.4	69.0	60.0	68.1	-	-	-	-	-	-	13.3	0.103	0.04	0.04	0.07	0.15	0.15
II	96.3	36.8	64.4	39.4	56.9	0.412	1.25	1.06	2.03	4.34	62.6	0.453	0.19	0.18	0.29	0.66	5.00
Jun. I	117.9	3.0	55.5	8.0	109.9	0.848	2.57	2.19	4.18	8.94	76.6	0.591	0.25	0.23	0.38	0.86	9.80
II	128.6	-	55.5	5.3	123.3	0.951	2.88	2.45	4.69	10.03	83.6	0.645	0.27	0.25	0.41	0.93	10.96
Jul. I	91.1	27.0	51.0	29.3	61.8	0.477	1.44	1.23	2.36	5.03	59.2	0.457	0.19	0.18	0.29	0.66	5.69
II	48.6	102.4	54.4	97.8	-	-	-	-	-	-	31.6	0.229	0.10	0.09	0.15	0.34	0.34
Aug. I	242.8	123.0	48.0	115.9	126.9	0.979	2.96	2.53	4.83	10.32	157.8	1.218	0.51	0.47	0.78	1.76	12.08
II	100.7	104.8	51.2	99.7	1.0	0.07	0.21	0.18	0.35	0.74	65.5	0.474	0.20	0.18	0.30	0.68	1.42
Sep. I	67.3	81.0	45.0	77.6	-	-	-	-	-	-	43.7	0.338	0.14	0.13	0.22	0.49	0.49
II	135.2	84.8	45.0	81.0	54.2	0.418	1.27	1.08	2.06	4.41	87.9	0.678	0.29	0.26	0.43	0.98	5.39
Oct. I	231.6	82.5	46.5	79.1	152.5	1.177	3.57	3.04	5.80	12.41	150.5	1.161	0.49	0.45	0.74	1.68	14.09
II	88.7	88.4	49.6	84.7	4.0	0.029	0.09	0.08	0.14	0.31	57.7	0.417	0.18	0.16	0.27	0.61	0.92
Nov. I	25.9	55.5	48.0	54.8	-	-	-	-	-	-	16.8	0.130	0.05	0.05	0.08	0.18	0.18
II	37.6	27.0	48.0	29.0	8.6	0.066	0.20	0.17	0.32	0.69	24.4	0.188	0.08	0.07	0.12	0.27	0.96

Note: (1) Average half month rainfall at three stations, sing Ha Nat, Phraya Banlu and Lakkhon, in probable 10-years

- |                               |  |
|-------------------------------|--|
| (2) See Table A.4-17          | (11) (1) x runoff coefficient 0.65 (assumed) |
| (3) See Table A.4-18          | (12) (11)/8.64 x n                           |
| (4) (2) x 0.905 + (3) x 0.095 | (13) 422 ha x (12)                           |
| (5) (1) - (4)                 | (14) 387 ha x (12)                           |
| (6) (5)/8.64 x n              | (15) 637 ha x (12)                           |
| (7) 3,027 ha x (6)            | (16) (13) + (14) + (15)                      |
| (8) 2,583 ha x (6)            | (17) (10) + (16)                             |
| (9) 4,932 ha x (6)            |  |
| (10) (7) + (8) + (9)          |  |

Necessity and Possibility for Separation of Irrigation and Drainage System

Necessity

The modernized paddy cultivation requires systematic irrigation and drainage systems controlled intentionally in each rotation block. The puddling period has been planned at 48 days so that not only transplanting but also harvesting of paddy should be completed within 48 days in one rotation block. For this purpose, each plot has to control irrigation water freely under the separated irrigation and drainage systems. In case of planning a dual purpose canal system, it is virtually difficult to control irrigation water freely, because the irrigation water supply in plot by plot are different due to the difference of growing stage of paddy.

Possibility

It is necessary to examine thoroughly the possibility for the separation of irrigation and drainage systems in the project, because the project area lies on generally low-lying and flat area, and the outer water level is high during the wet season even after the Project. During the wet seasons when the outer water level is higher than the inner one, it is planned that drainage will be made forcedly by pumps. On the other hand, in the dry season, the irrigation water is lifted by pumps and is conveyed to the field with effective hydraulic potential in canals provided along the roads. These plans will make it possible to separate irrigation and drainage systems in the field.

Table A. 4-32 Comparison of Land Consolidation Cost for Each Length of Run

Alternatives	Area		On-farm Facilities				Construction Cost		
	Gross ha	Net ha	Land Levelling	On-farm Road	Drainage Ditch	Leading Ditch	Community Road	Total Cost	Cost/ha
Case (1) (100x50m)	102.3	96.03/	\$44,160	4,800m \$4,320	3,600m \$5,400	833.4m \$5,567	833.4m \$2,083	\$61,630	\$642 (106)
Case (2) (130x50m)	131.9	124.8	\$57,454	4,800m \$4,320	3,600m \$5,400	1,033.4m \$7,027	1,033.4m \$2,583	\$76,784	\$615 (102)
Case (3) (150x50m)	151.5	144.0	\$65,240	4,800m \$4,320	3,600m \$5,400	1,233.4m \$8,387	1,233.4 \$3,083	\$87,430	\$607 (101)
Case (4) (160x50m)	161.3	153.6	\$70,656	4,800m \$4,320	3,600m \$5,400	1,313.4m \$8,931	1,313.4m \$3,283	\$92,590	\$603 (100)

Note: a/ Gross area minus area of on-farm facilities

- 1) Unit cost of on-farm facilities:
- |                |           |
|----------------|-----------|
| Land levelling | 460 \$/ha |
| On-farm road   | 0.9 \$/m  |
| Drainage ditch | 1.5 \$/m  |
| Leading ditch  | 6.8 \$/m  |
| Community road | 2.5 \$/m  |

- 2) Costs of survey, preparation, land clearing, structures, and overhead are not included in the above costs.

Table A. 4-33 Comparison of Deduction Ratio of Land for Each Length of Run

Alternatives	Gross Area (ha)	Area of On-farm Facilities (ha)				Sub-total	Deduction Ratio of Land (%)	
		On-farm Road	Drairage Ditch	Leading Ditch	Community Road		Main Road	Total
Case (1)	102.3 <del>e</del>	2.5 <del>b</del>	1.48 <del>c</del>	1.17 <del>d</del>	1.17 <del>e</del>	6.36 <del>f</del> (6.2%)	1.8	8.0
Case (2)	131.9	2.54	1.48	1.50	1.50	7.02 (5.5%)	1.8	7.3
Case (3)	151.5	2.54	1.48	1.73	1.73	7.49 (4.9%)	1.8	6.7
Case (4)	161.3	2.54	1.48	1.84	1.84	7.70 (4.8%)	1.8	6.6

Note: a: A x B

b: 600m x 8 x 5.3m (width of on-farm road) including farm ditch

c: 600m x 6 x 4.1m (width of drainage ditch)

d: A x 14 (width of leading ditch including road)

e: A x 14 (width of community road including lateral irrigation canal)

f:  $\frac{b}{a} + \frac{c}{a} + \frac{d}{a} + \frac{e}{a}$  ( ) shows the percentage of area of on-farm facilities

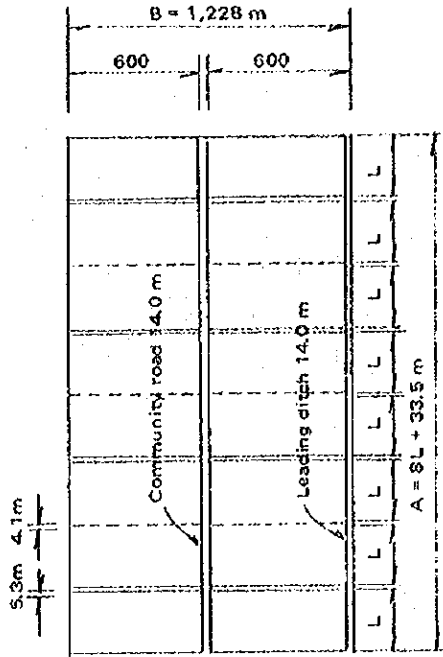


FIGURE A. 4-14 ALTERNATIVES ON SIZE OF PLOT AND REPARCELING OF LAND

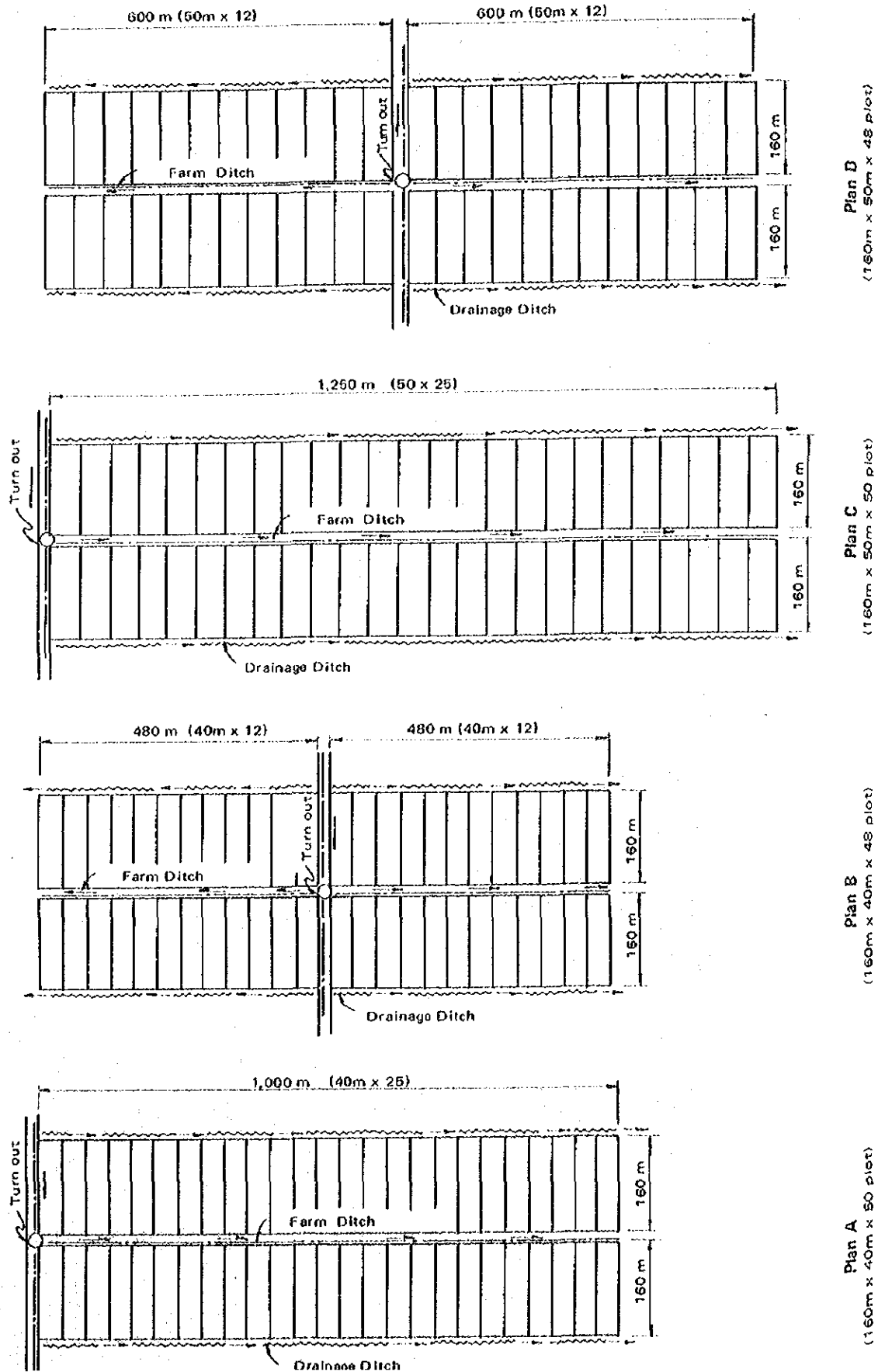




Table A. 4-34 Comparison of Land Consolidation Cost for Each Width of Plot

Case	Area		On-farm Facilities				Construction Cost		
	Gross (ha)	Net (ha)	Land Levelling	On-farm Road	Drainage Ditch	Leading Ditch	Community Road	Total Cost	Cost/ha
Case B	129.8	122.9	\$56,534	3,840m \$3,456	2,880m \$4,320	1,313.4m \$8,931	1,313.4m \$3,283	\$76,524	\$623 (103)
Case D	161.3	153.6	70,656	4,800m \$4,320	3,600m \$5,400	1,313.4m \$8,931	1,313.4m \$3,283	\$92,590	\$603 (100)

Note: 1) Unit cost of on-farm facilities;

Land levelling	460 \$/ha
On-farm road	0.9 \$/m
Drainage ditch	1.5 \$/m
Leading ditch	6.8 \$/m
Community road	2.5 \$/m

2) Costs of survey, preparation, land clearing, structures and overhead are not included in the above costs.

Table A. 4-35 Comparison of Deduction Ratio of Land for Each Width of Plot

Case	Gross Area(ha)	Area of On-farm Facilities (ha)				Sub-total	Deduction Ratio of Land (%)	
		On-farm Road	Drainage Ditch	Leading Ditch	Community Road		Main Road	Total
Case B	129.8	2.04 <sub>a/</sub>	1.18 <sub>b/</sub>	1.84 <sub>c/</sub>	1.84	6.90 (5.3%)	1.8	7.1'
Case D	161.3	2.54	1.48	1.84	1.84	7.70 (4.8%)	1.8	6.6

Note: a/: b m x 8 x 5.3 (Width of on-farm road including farm ditch)

b/: b m x 6 x 4.1 (Width of drainage ditch)

c/: A x 14m (Width of community road including lateral irrigation canal)

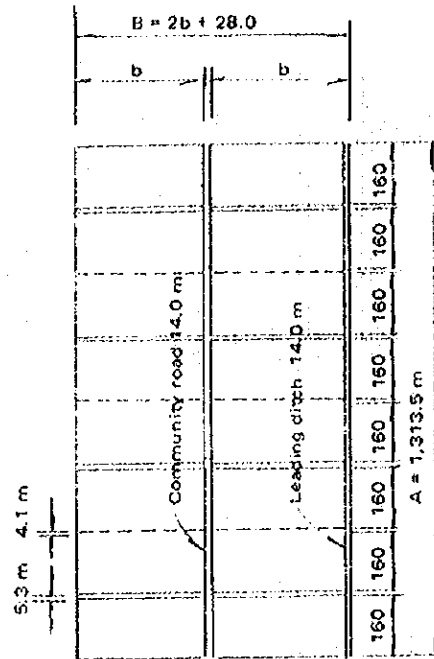
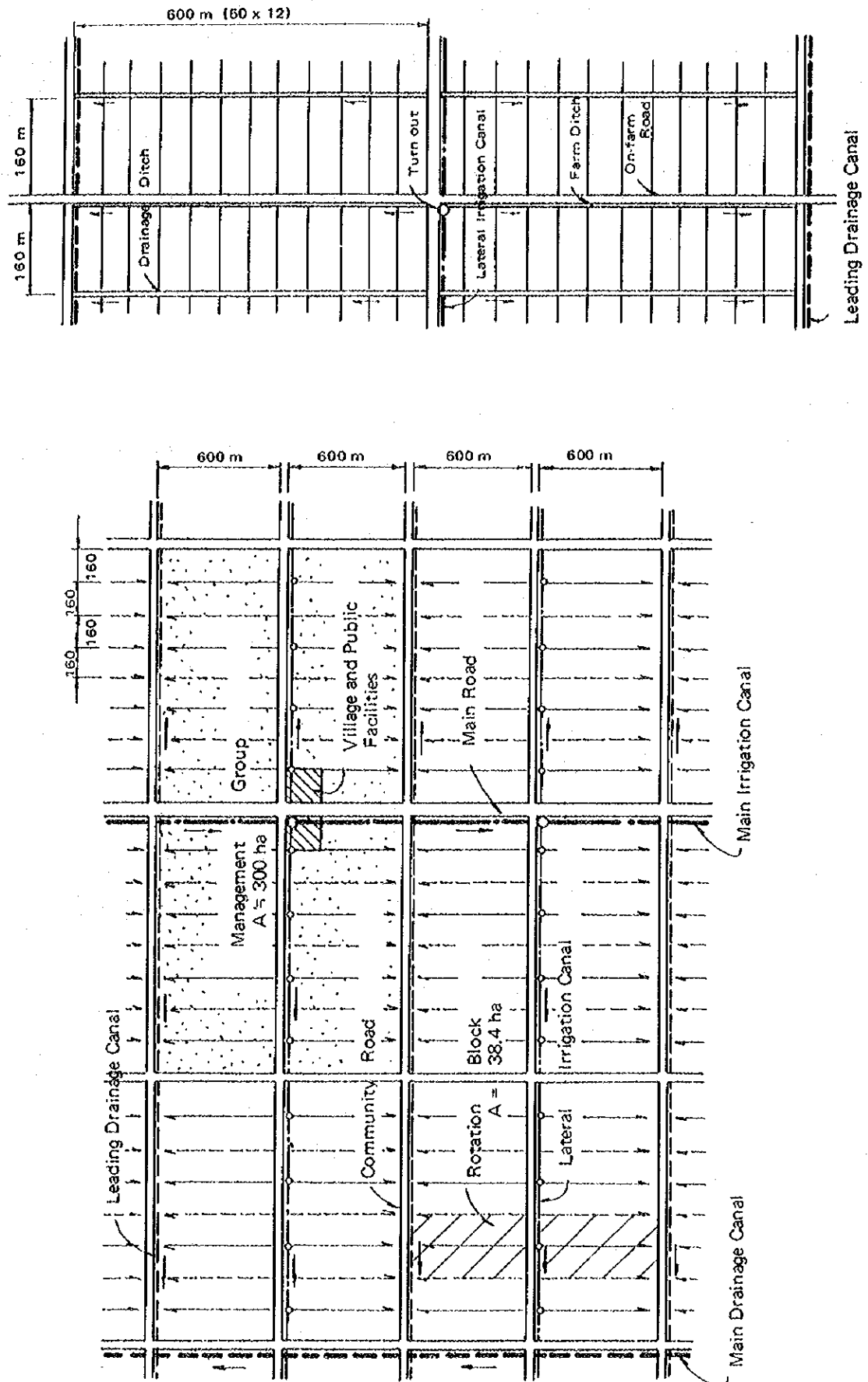


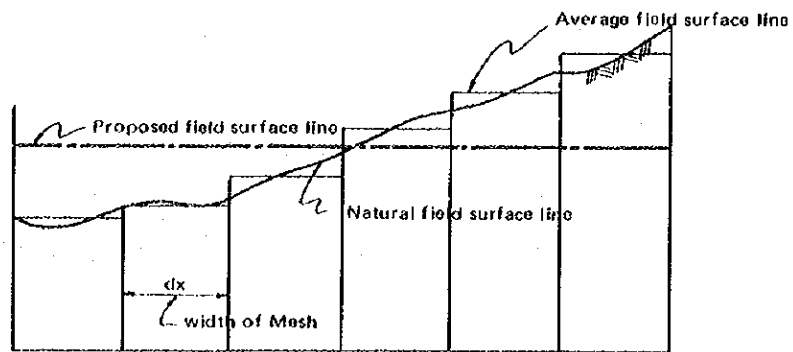
FIGURE A.4-15 TYPICAL LAYOUT OF FARM LAND DEVELOPMENT



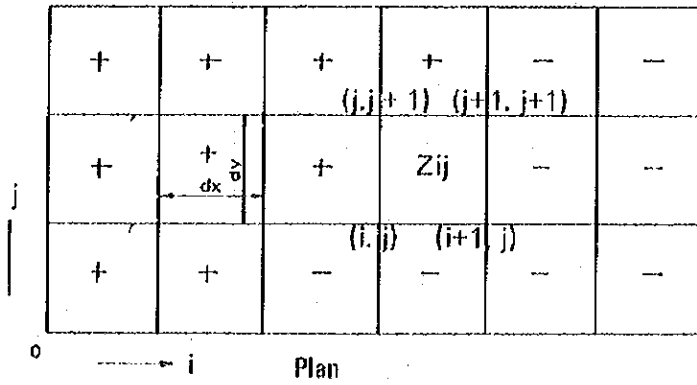
Procedure for Computation of Land Leveling

The computation of land leveling is made by applying the mech method. According to this method, it is assumed that the topography is levelled within the same mech after partitioning off the present field surface into several meshes with regular intervals, as shown in the following figures, and each moving volume and hauling distance can be computed by the following formula;

Area	$A_{iJ}$ (mesh area surrounded by coordinates)	
Centroid coordinate:	$X_{iJ}, Y_{iJ}$	
Ground elevation, $Z_{iJ}$ :	$1/4 \times (Z_{iJ} + Z_{i+1,J} + Z_{i,J+1} + Z_{i+1,J+1})$	
Designed elevation F:	$\frac{\sum(A_{iJ} + Z_{iJ})}{\sum A_{iJ}}$	
Height of cut and bank, $H_c, H_B$ :	Designed elevation minus $Z_{iJ}$	
Earth moving volume, $V_c, V_B$ :	Height of cut and bank $\times A_{iJ}$	
Hauling distance D:	$\{(X_c - X_B)^2 + (Y_c - Y_B)^2\}^{1/2}$	
	$X_c = \frac{\sum V_c \times X_{iJ}}{\sum V_c}$	$Y_c = \frac{\sum V_c \times Y_{iJ}}{\sum V_c}$
	$X_B = \frac{\sum V_B \times X_{iJ}}{\sum V_B}$	$Y_B = \frac{\sum V_B \times Y_{iJ}}{\sum V_B}$



Section



Flow chart for the above computation is shown in Figure A. 4-16.

FIGURE A. 4-16 FLOW CHART OF ESTIMATION FOR EARTH MOVING VOLUME

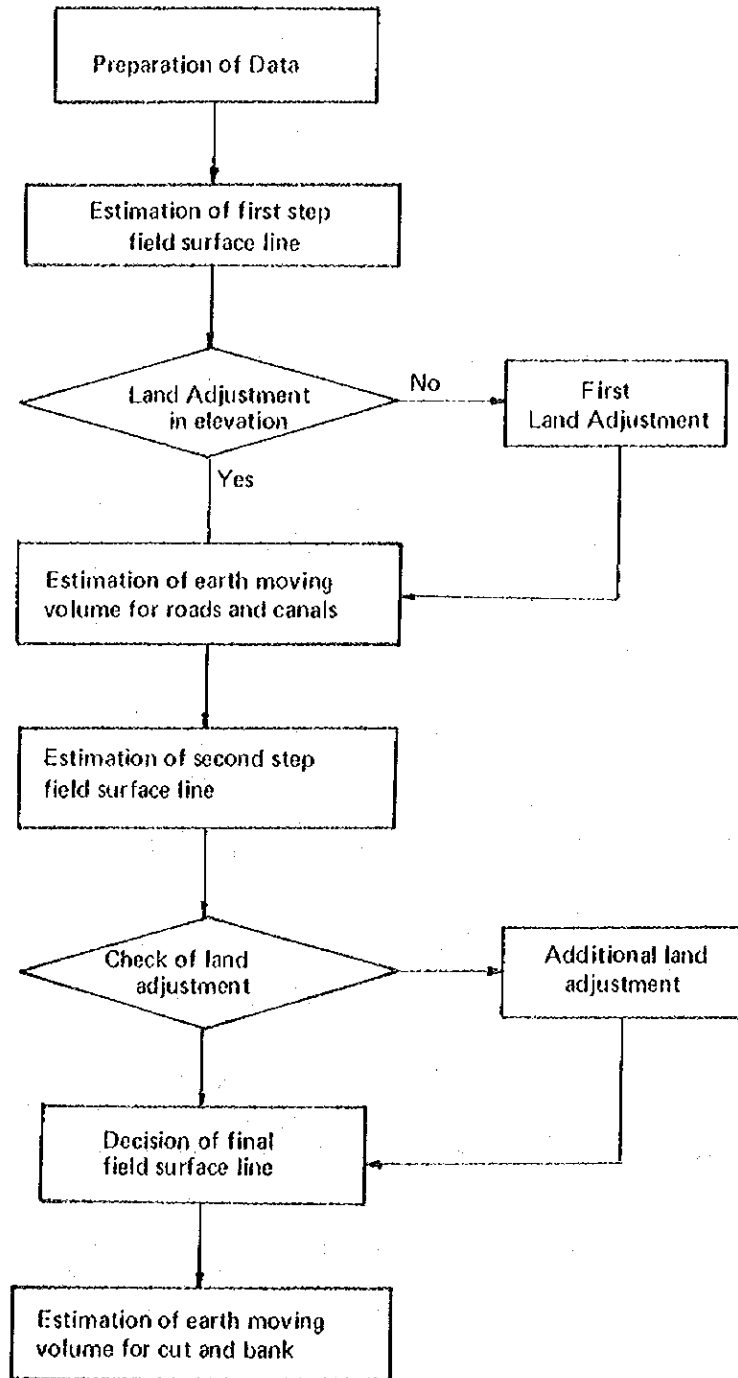


Table A. 4-36 Summary of Estimation for Earth Moving Volume and Hauling Distance

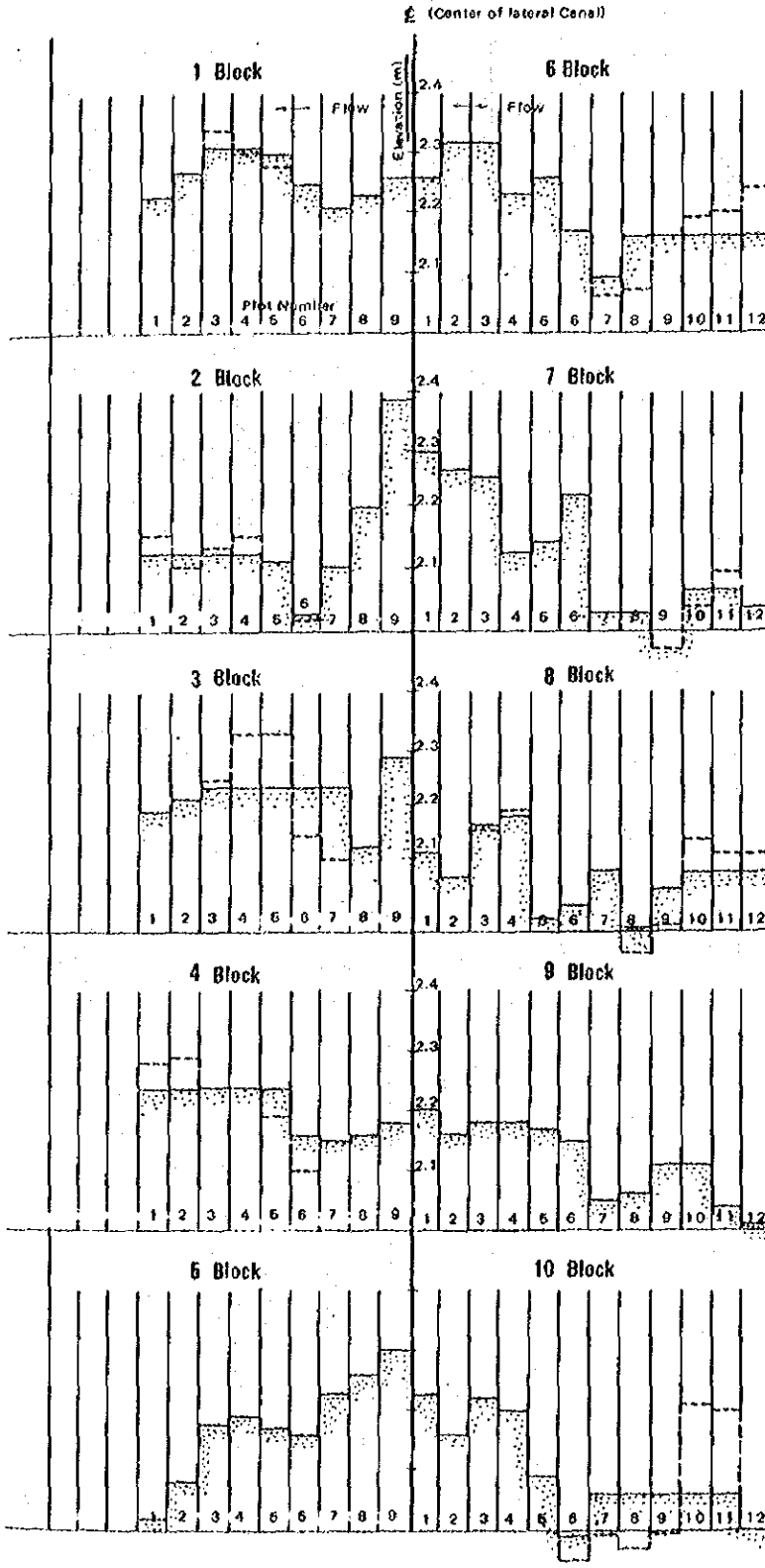
Description	Case (E)	Case (F)	Case (G)	Case (H)
<b>1. Area (ha)</b>				
Site I	84.0	84.0	84.0	83.2
Site II	96.0	96.0	96.0	96.0
Total	<u>180.0</u>	<u>180.0</u>	<u>180.0</u>	<u>179.2</u>
<b>2. Size of plot (length of run x width of spot) 160mx50m</b>				
	160mx50m	160mx50m	160mx50m	160mx40m
<b>3. Allowable difference in field elevation (cm)</b>				
	no land adjustment	10	5	10
<b>4. Land leveling works (cu.m)</b>				
Cut volume, Site I	26,833	29,731	31,519	29,443
Site II	50,645	63,930	65,649	61,234
Total	<u>77,478(430m<sup>3</sup>/ha)</u>	<u>93,661(520)</u>	<u>97,168(540)</u>	<u>90,677(506)</u>
Bank volume, Site I	28,571	31,470	33,259	30,491
Site II	52,487	65,773	67,493	63,077
Total	<u>81,058(450)</u>	<u>97,243(540)</u>	<u>100,752(560)</u>	<u>93,568(522)</u>
<b>Land adjustment in elevation</b>				
Site I	-	9,315	16,490	10,133
Site II	-	34,303	49,940	41,013
Total	-	<u>43,618(242)</u>	<u>66,430(369)</u>	<u>51,146(285)</u>
<b>5. Average hauling distance (m)</b>				
Cutting works Site I	61	63	64	63
Site II	70	66	68	70
Average	<u>67</u>	<u>65</u>	<u>66</u>	<u>67</u>
<b>Land adjustment in elevation</b>				
Site I	-	140	173	149
Site II	-	213	219	200
Average	-	<u>197</u>	<u>208</u>	<u>190</u>

Table A. 4-37 Earth Moving Volume and Hauling Distance

Block Location Number	Case (E)			Case (F)			Case (G)			Case (H)		
	Cut (m <sup>3</sup> )	Bank (m <sup>3</sup> )	Dist. (m)	Cut (m <sup>3</sup> )	Bank (m <sup>3</sup> )	Dist. (m)	Cut (m <sup>3</sup> )	Bank (m <sup>3</sup> )	Dist. (m)	Cut (m <sup>3</sup> )	Bank (m <sup>3</sup> )	Dist. (m)
Site I Land Levelling												
1	1,222	1,068	58	1,284	1,131	60	1,572	1,418	68	1,487	1,195	73
2	2,491	2,540	84	2,594	2,643	87	2,757	2,806	99	2,587	2,097	82
3	2,218	2,542	52	2,894	3,218	75	2,294	3,218	79	2,765	3,085	70
4	2,666	2,989	40	2,873	3,197	42	3,403	3,726	42	2,814	3,134	44
5	1,955	2,230	65	1,955	2,230	65	1,955	2,230	65	1,699	1,968	65
6	2,685	2,456	67	3,280	3,051	70	3,186	2,958	69	3,291	3,063	64
7	2,751	2,792	57	2,829	2,870	58	3,042	3,084	56	2,938	2,979	54
8	3,703	4,087	62	3,816	4,200	65	4,058	4,442	67	3,908	4,291	75
9	3,614	3,997	72	3,614	3,997	72	3,640	4,023	72	3,571	3,955	72
10	3,528	3,870	54	4,592	4,933	45	5,012	5,354	46	4,383	4,724	42
Total	26,833	28,571	61	29,731	31,470	63	31,519	33,259	64	29,443	30,491	63
Land adjustment in elevation	-	-	-	9,315	-	140	16,490	-	173	10,133	-	149
Site II Land levelling												
1	6,948	6,719	58	6,943	6,715	58	6,984	6,755	58	7,033	6,805	64
2	5,037	5,078	45	5,754	5,795	50	6,118	6,160	52	5,733	5,774	42
3	4,111	4,495	68	4,662	5,045	67	4,932	5,316	73	4,335	4,718	78
4	4,518	4,701	75	6,307	6,691	78	5,723	7,106	78	5,971	6,354	80
5	3,865	4,206	66	6,904	7,245	47	7,115	7,458	46	6,335	6,677	63
6	3,804	3,576	40	7,025	6,796	49	6,274	6,045	47	5,552	5,324	56
7	3,734	3,776	60	4,519	4,560	66	4,770	4,812	73	4,446	4,488	72
8	8,270	8,653	95	8,505	8,889	95	8,596	8,980	95	8,405	8,788	95
9	2,728	3,112	71	4,278	4,662	43	4,636	5,020	51	4,541	4,924	55
10	7,830	8,171	88	9,033	9,378	88	9,500	9,841	89	8,883	9,225	80
Total	50,645	52,487	70	63,930	65,773	66	65,849	67,493	68	61,234	63,077	70
Land Adjustment in elevation	-	-	-	34,303	-	213	49,940	-	219	41,013	-	200



FIGURE A.4-17  
PROPOSED ELEVATION OF PLOT IN CASE OF WITH AND WITHOUT  
LAND ADJUSTMENT IN ELEVATION (SITE I)



Case (E) (Without land adjustment)

	Cut m <sup>3</sup>	Bank m <sup>3</sup>	Dist. m
1 Block	1,222	1,068	58
2 Block	2,491	2,540	84
3 Block	2,218	2,542	62
4 Block	2,686	2,989	40
5 Block	1,955	2,230	65
6 Block	2,684	2,456	67
7 Block	2,751	2,792	57
8 Block	3,703	4,087	62
9 Block	3,614	3,997	72
10 Block	3,529	3,870	54
<b>Total</b>	<b>26,833</b>	<b>28,671</b>	<b>61</b>

Case (F) (with land adjustment, 10 cm)

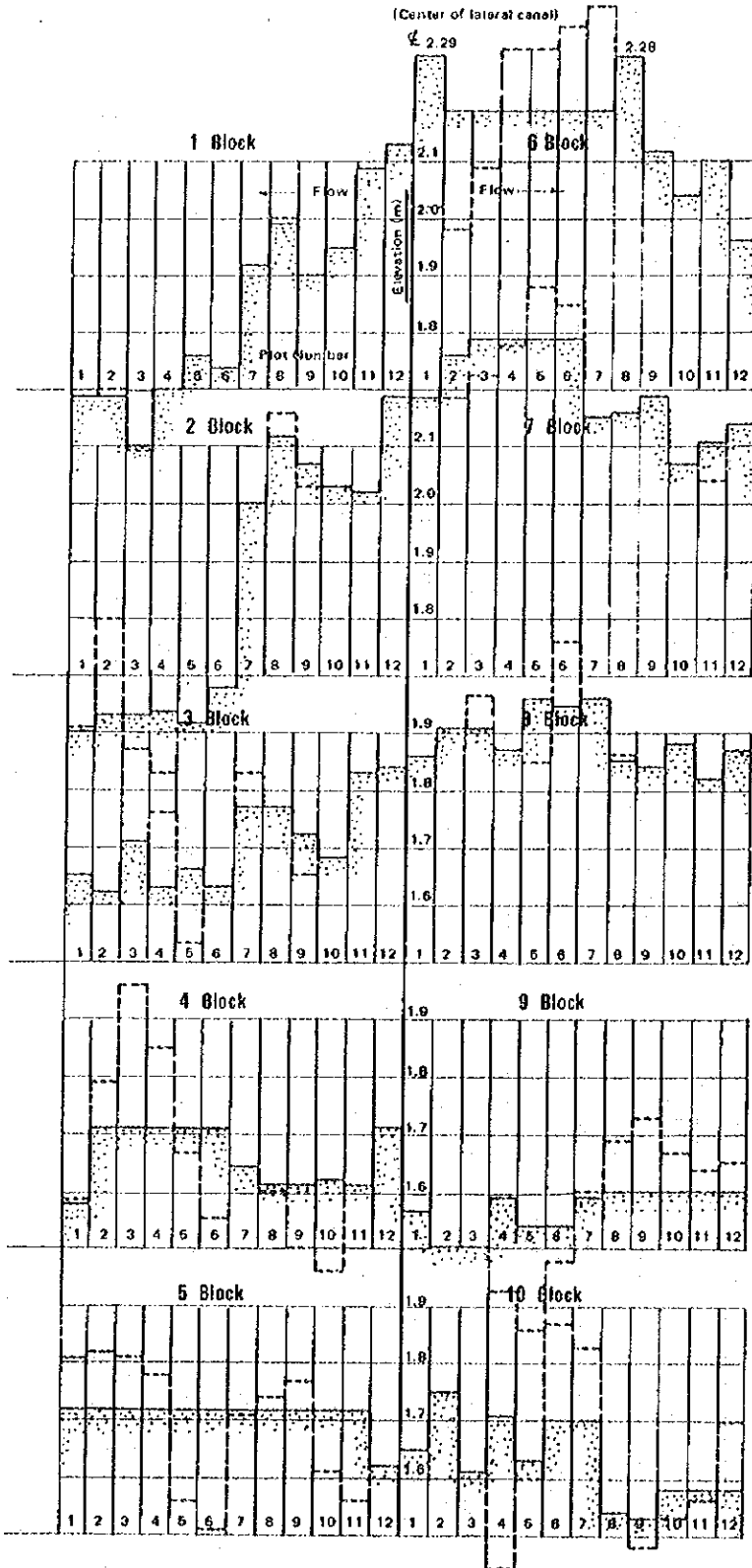
1 Block	1,284	1,131	60
2 Block	2,594	2,643	87
3 Block	2,894	3,218	75
4 Block	2,873	3,197	42
5 Block	1,955	2,230	65
6 Block	3,280	3,051	70
7 Block	2,829	2,870	58
8 Block	3,816	4,200	66
9 Block	3,614	3,997	72
10 Block	4,592	4,933	46
<b>Total</b>	<b>29,731</b>	<b>31,470</b>	<b>63</b>

LEGEND

- Case E
- ..... Case F

Numbers of block and plot are given in the drawing No. 1 in main report.

FIGURE A.4 - 18  
PROPOSED ELEVATION OF PLOT IN CASE OF WITH AND WITHOUT  
LAND ADJUSTMENT IN ELEVATION (SITE II)



Case (E) (without land adjustment)

	Out m <sup>3</sup>	Bank m <sup>3</sup>	Dist. m
1 Block	6,948	6,719	68
2 Block	5,037	5,078	45
3 Block	4,111	4,495	68
4 Block	4,318	4,701	76
5 Block	3,865	4,206	66
6 Block	3,804	3,576	40
7 Block	3,734	3,776	60
8 Block	8,270	8,653	95
9 Block	2,728	3,112	71
10 Block	7,830	8,171	88
<b>Total</b>	<b>50,646</b>	<b>52,487</b>	<b>70</b>

Case (F) (with land adjustment, 10cm)

1 Block	6,943	6,716	68
2 Block	5,764	5,795	50
3 Block	4,662	5,045	67
4 Block	6,307	6,691	78
5 Block	6,904	7,245	47
6 Block	7,025	6,796	49
7 Block	4,519	4,560	66
8 Block	8,505	8,889	95
9 Block	4,278	4,662	43
10 Block	9,033	9,375	88
<b>Total</b>	<b>63,930</b>	<b>65,773</b>	<b>66</b>

LEGEND

- Case E
- ..... Case F

Numbers of block and plot are given in the drawing No. 2 in the main report

FIGURE A.4-19 LONGITUDINAL SECTION OF LATERAL CANAL (SITE 1)

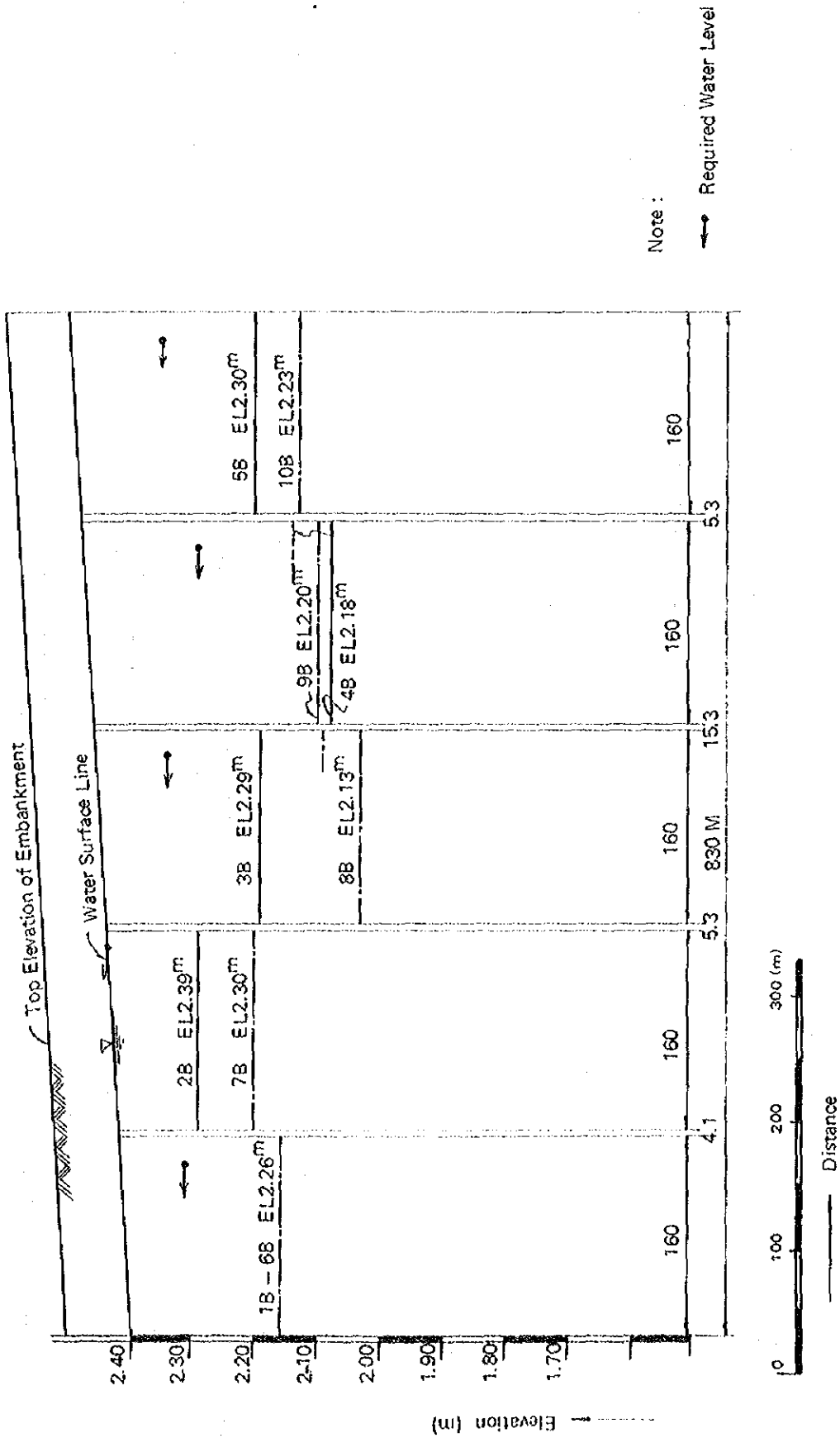


FIGURE A. 4-20 LONGITUDINAL SECTION OF LATERAL CANAL (SITE II)

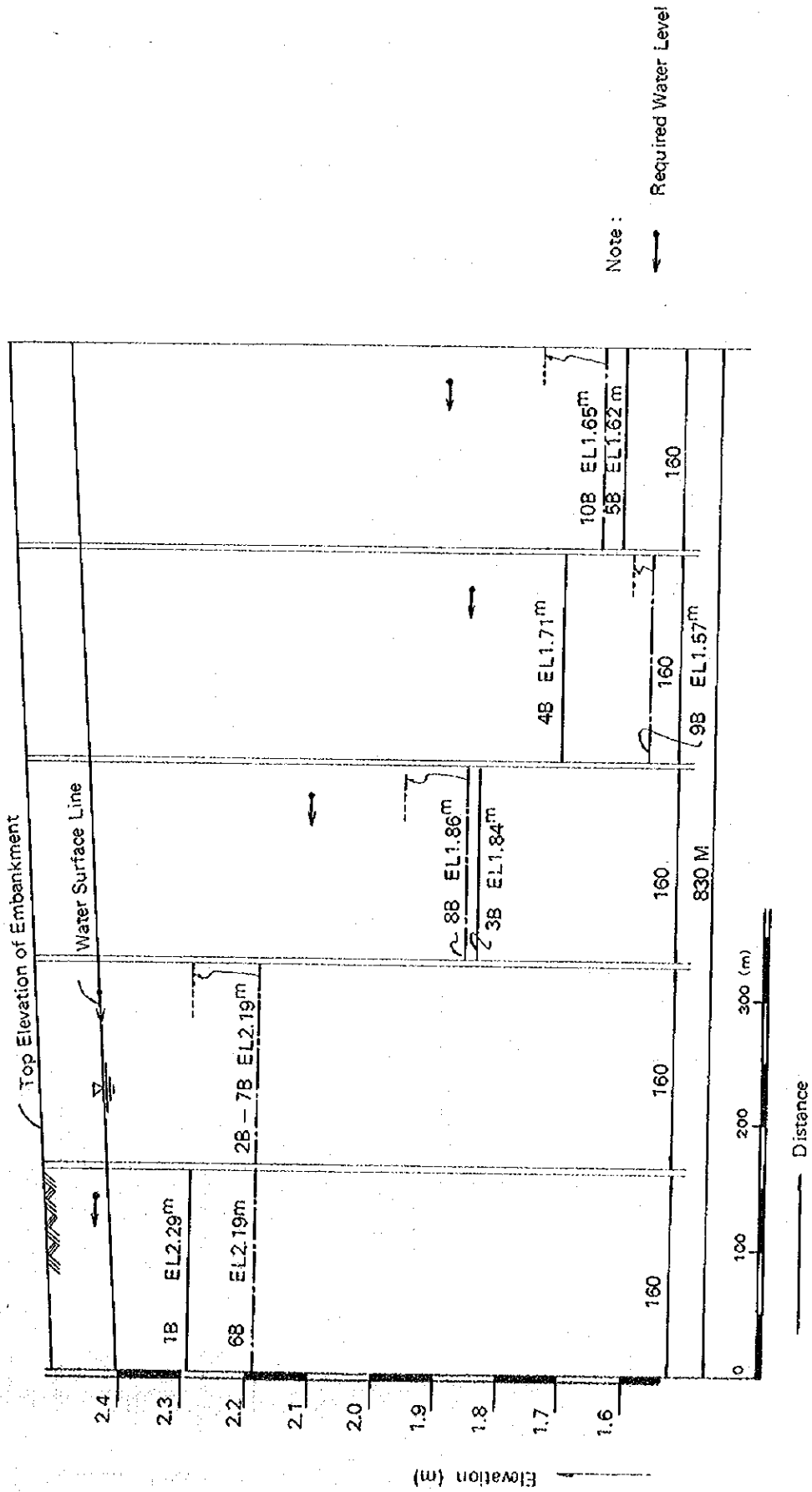


FIGURE A. 4-21  
LAYOUT OF VILLAGE WATER SUPPLY PLANT

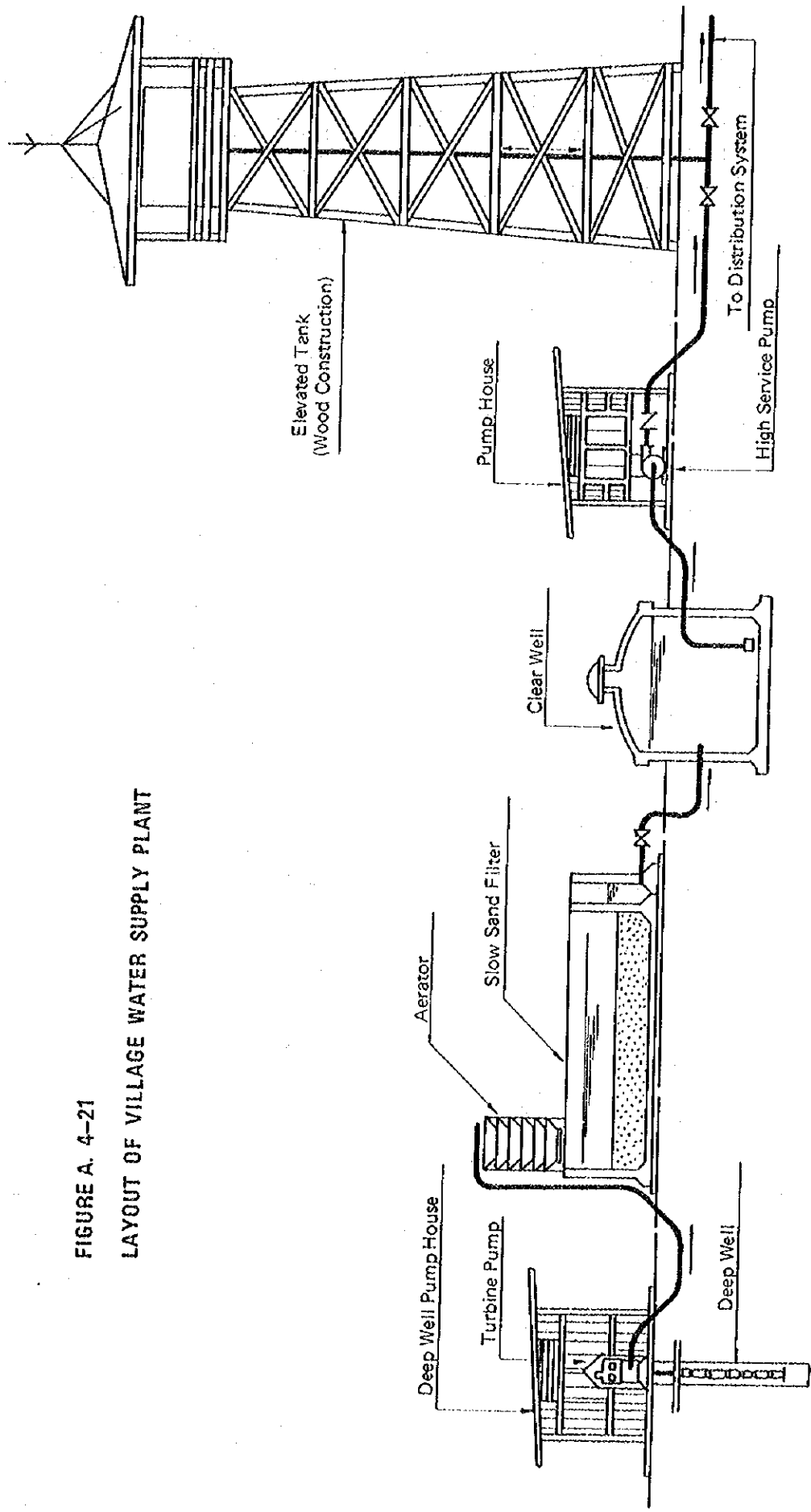


Table A. 4-38 Recommended Farm Practices for Paddy Rice

<u>Field Operation</u>	<u>Recommended Practice</u>	<u>Inputs per Ha</u>
1. Sowing (1st day after sowing)	<p>(a) Form and size of nursery: wet-seedbeds with the width of 120-150 cm (area for nursery is 500 m<sup>2</sup> per 1 ha of transplanting area)</p> <p>(b) Seed rate and the method of seed preparation</p> <ul style="list-style-type: none"> <li>- Seed rate : 60 g per one m<sup>2</sup> of seed-bed</li> <li>- Selection : select seed with water</li> <li>- Soaking : in clean or fungicided water for 24 hr.</li> <li>- Incubation: keep seeds moist and warm in half filled, loosely tied sacks for 36 to 48 hr.</li> </ul>	<p>Fertilizer for nursery: 9 Kg of Ammophos (16-20-0)</p> <p>Seed: 40 kg</p> <p>Fungicide for seed: Ditane M-45 1.5 kg or any other kinds of chemicals</p>
2. Care of Seedlings (8-20 days)	<p>(a) Apply recommended insecticides at 8 to 10 days after sowing</p> <p>(b) After sowing, the beds should be covered by filmy shallow water</p> <p>(c) 3 to 4 days after sowing, drain the water in the nursery and make the surface of beds dry for 1 to 2 days to make the sprouting seeds fix to soil.</p> <p>(d) Then, water should be deepened gradually in a range between 2 to 3 cm, so that seedling are not submerged.</p>	<p>Insecticides for seedlings: 0.1 kg of Carbaryl (Sevin) 85%, WSP or any other kinds of chemicals.</p>

Field Operation

Recommended Practice

Inputs per Ha

3. Land Preparation  
(1st-25 days)

Prepare land by soaking (if land is too dry to plow), plowing and harrowing. The harrowing is to be performed as:

1st step: after flooding with enough water, harrow the field longitudinal and cross-wise

2nd step: more than 6 days after 1st harrowing, harrow with applying basal fertilizer finally.

Basal Fertilizer:  
Wet - 156 kg of Ammophos  
(16-20-0)  
(N: 25kg, P<sub>2</sub>O<sub>5</sub>: 31kg)  
Dry - 188 kg of Ammophos  
(16-20-0)  
(N: 30kg, P<sub>2</sub>O<sub>5</sub>: 38kg)

4. Transplanting  
(25-30 days)

After final harrowing, transplant immediately in the way as:

(a) Planting distance: 25cm x 25cm (wet season), 20cm x 20cm (dry season) in the way of regular planting with using 3 to 5 seedlings per hill.

(b) Seedlings should be planted as shallow as possible

5. Spraying herbicides  
(3-5 days & 20-25days)

(a) 3 to 5 days after transplanting (before the weeds can be seen), apply herbicide and maintain water 3 to 5 cm deep for 10 days at least.

(b) 20 to 25 days after transplanting (when weeds are showing), apply herbicide to the completely drained field and keep the field dry for 2 days.

Herbicides of pre-emergency application: 25kg of Benthocarb 8%, G or any other kind of herbicides.

Herbicides of post-emergency application; 0.8-1.0 kg of 2.4D sodium salt 95%, WSP or any other kinds of chemical chemicals.

6. Weeding  
(45-60 days)

Weed between hills by hand or rotary weeder

Field Operation

Recommended Practices

Inputs per Ha

7. Additional Fertilizing (65-70 days) 20 to 10 days before heading, apply additional fertilizer.
- Additional Fertilizer:  
Wet - 55kg of Urea (46-0-0) (N: 25kg)  
Dry - 65kg of Urea (46-0-0) (N: 30kg)

8. Spraying Insecticides (30 & 65 days)
- (a) 1st: 30 days after transplanting  
(b) 2nd: 65 days after transplanting (for the field where much harmful insects would be found).
- Insecticides  
1st: 2 kg of Carbaryl (Sevin) 85%, WSP or any other kind of insecticides

These insecticide should be applied after checking the number of each harmful insects in the field.

2nd: 30 kg of carbofuran (Furadan) 3%, G or any other kind of insecticides

9. Drainage
- After milk-ripe stage (14 days before harvesting), stop irrigation and drain the water from the field.

10. Harvesting (125-130 days)
- When 80 % of panicle turns yellowish and become firm to touch, harvest the rice

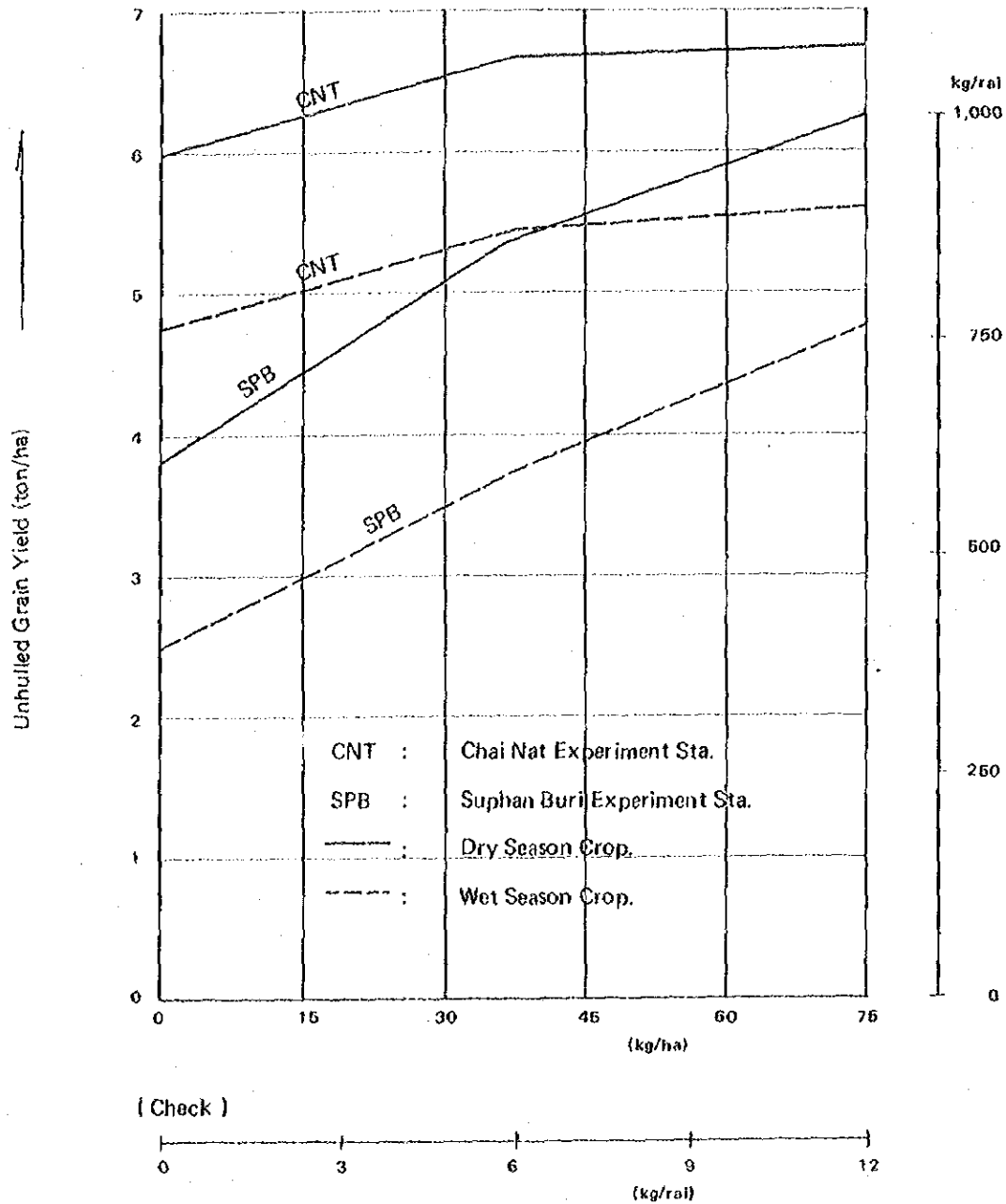
11. Drying
- Dry up harvested paddy hills in the field for 2 to 4 days by sunshine. Bring them to dry place and pile up.

12. Threshing
- After drying paddy hills, thresh them. Threshed paddy must be reduced in their moisture contents to 14 % by sunshine or dryer.

Note: (1) Besides above mentioned chemicals, 0.25kg of Warfaring or any other kind of rodenticides per one times of applying will be needed to control rats  
(2) WSP: Water soluble powder. G: Granule



FIGURE A.4-22 PADDY YIELD RESPONSE TO NITROGEN

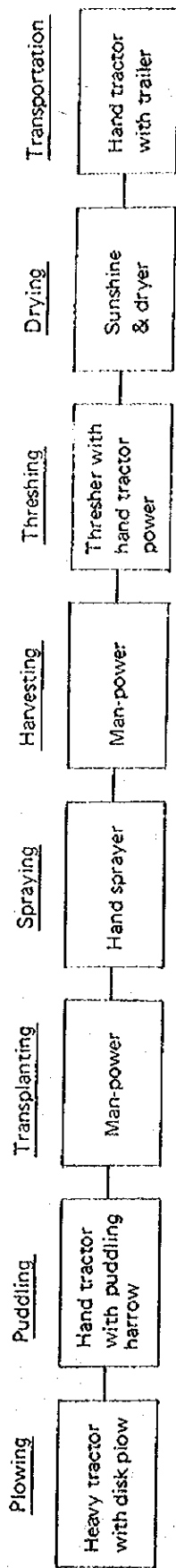


Remarks: (1) Source: "Annual Research Report of 1969" Rice Dep.  
 (2) Yield : Mean yield of RD1 and C4-63  
 (3) Another fertilizer application: P<sub>2</sub>O<sub>5</sub> 75 kg/ha, K<sub>2</sub>O 6 kg/ha

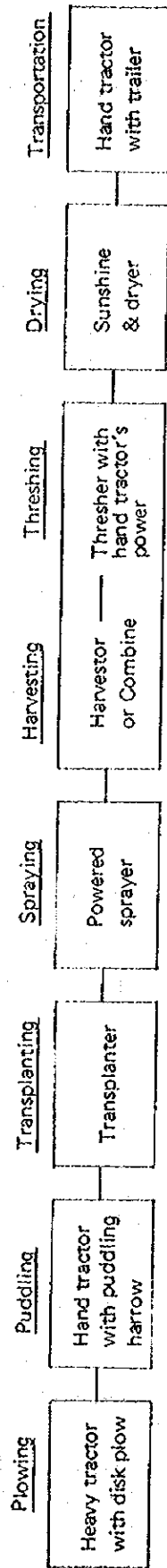
FIGURE A.4 - 23 FARM MECHANIZATION SYSTEM WITH PROJECT

1. Paddy

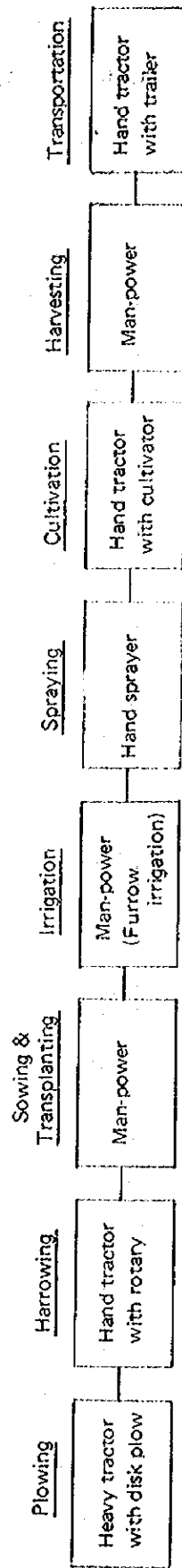
(a) Type I



(b) Type II



2. Upland Crops (Vegetable) & Fruit



Remarks: (1) Heavy tractor; 4 wheel tractor (more than 50 PH)  
(2) Hand tractor; 2 wheel tractor (7 - 8 PH)

Table A.4-39 Capacity of Farm Machinery for Paddy Rice Cultivation  
(Mechanization System Type 1)

<u>Items</u>	<u>Unit</u>	<u>Plowing</u>	<u>Puddling</u>	<u>Threshing</u>	<u>Drying</u>
<u>1. Machinery</u>					
(a) Power		~50 PH 4 wheel tractor	7-8 PH 2 wheel hand tractor	7-8 PH 2 wheel hand tractor	5PH Gasoline engine, kerosine burner
(b) Implement		Disc plow  26"x6 <sup>disc</sup>	Puddling harrow  1.2m width	Throw-in type thresher (IRRI type) 1.8m drum	Flat type rice dryer (UP type) 2 ton bin
<u>2. Capacity of Machinery</u>					
(a) Operational width	m	1.58 (=26"x2/5x6)	0.97	-	-
(b) Opc. speed	km/hr	5.0	2.9	-	-
(c) Theoretical ope. capacity	ha/hr	0.79	0.28	-	-
(d) Field ope. efficiency	%	85	80	-	-
(e) Field ope. capacity	ha/hr	0.67	0.22	0.16 (=0.8ton)	0.20 (=1.0 ton)
(f) Field ope. hours	2/ hr/ha	1.49	4.54	6.25	5.00
(g) Real ope. efficiency	%	80	80	75	80
(h) Real ope. hours	hr/ha	1.86	5.68	8.33	6.25
(i) Ope. times	time	2	3	1	1
(j) Real ope. hours	hr/ha	3.72	17.0	8.33	6.25
(k) Real ope. capacity	ha/hr	0.269	0.059	0.120	0.160
(l) Real ope. days per ha					
3/	day/ha	0.5	2.2	1.1	0.4
(m) Real ope. capacity per day					
	ha/day	2.2	0.5	1.0	2.5
(n) Possible ope. days	day	70	48	40	40
<u>3. Coverage per one unit of machinery</u>					
		one unit	one unit	Two units	Three units
		Per two	per one	per one	one manage-
		farming	farm	farming	ment group
		groups	household	group	

- Note: 1/ Field operational efficiency is based on the data for 50m x 160m field plot
- 2/ Real operational efficiency means the efficiency inclusive of loss time outside the field
- 3/ One day = 8 hours, but in the case of drying, one day is 16 hours

Table A.4-40 Farm Machinery Cost per Hectare for Paddy Rice Cultivation

Machinery	(1) Purchasing Price (£)	(2) Durable Period (year)	(3) Depreciation Cost 2/ (£)	(4) Repair Cost (£)	(5) Other Fixed Cost 3/ (£)	(Unit: £/ha/crop)	
						(6) Total (£/unit/year)	(7) Coverage per Unit (ha)
Heavy tractor	208,000	10	19,720	(1) x 0.08 = 16,640	2,080	37,440	924/
Disc plow	25,000	5	4,500	(1) x 0.04 = 1,000	250	5,750	19
Sub-total	233,000		23,220	15,560	2,330	41,110	111
7-8PH Hand tractor	12,000 1/	5	2,160	(1) x 0.08 = 960	120		
Puddling harrow	1,500	5	270	(1) x 0.02 = 30	15		
Trailer	1,000	5	180	(1) x 0.02 = 20	10	3,765	471
Sub-total	14,500		2,610	1,010	145	3,765	471
Thresher	12,000	8	1,350	(1) x 0.03 = 350	120	1,830	24
Dryer	15,000	8	1,688	(1) x 0.02 = 300	150	2,138	10
Total							<u>616</u>

Note:

- 1/ Price of Thai made hand tractor = £4,000(Body) + £8,000(7-8PH diesel engine)
- 2/ (1) x 0.9 ÷ (2)
- 3/ (1) x 1% = 0.025%(Insurance fee) + 0.5%(Garage cost) + 0.1%(Lubricating oil cost)
- 4/ £92 = £122 x 1,026hr / 1,360hr; Annual average hours of tractor operation in Thailand  
 1,360hr - 334hr of estimated operational hours besides paddy rice cultivation.

2. Variable Cost

Operation	Machinery	(1) Operation Hour per ha (hr/ha)	(2) $\frac{L}{\text{Consumption}}$ (l/hr)	(3) Fuel Price (₹/l)	(4) $= (1) \times (2) \times (3)$ Fuel Cost (₹/ha)	(5) $= (4) \times 13$ Fuel Cost Inclusive of Lubricat- ing Oil Cost (₹/ha)
1. Land preparation for seed-bed	Hand tractor with puddling harrow	2.6	L 2.0	2.6	13.5	17.6
2. Plowing	Heavy tractor with disc plow	3.7	L 5.5	2.6	52.9	68.8
3. Puddling	Hand tractor with puddling harrow	17.0	L 2.0	2.6	88.4	114.9
4. Threshing	Thresher with engine of hand tractor	8.3	L 2.0	2.5	43.2	56.2
5. Drying	Dryer	2.5	2/ G+O 0.75 K 1.5	4.0 1.2	7.5 4.5	9.8 5.9
6. Transportation	Hand tractor with trailer	7.3	L 2.0	2.6	38.0	49.4
Total					<u>322.6</u>	

Remarks:  $\frac{1}{L}$  Light oil, G+O: Gasoline mixed with lubricating oil, K: Kerosene  
 $\frac{2}{}$  Average hours of 1st and 2nd crops

3. Farm Machinery Cost per Hectare

$3616 + 3323 = \underline{3939}$

Table A.4-41 Labour Requirement of Paddy Rice per Hectare  
(With Project)

<u>Operation</u>	<u>Man-day</u>	<u>Machinery-day</u>	<u>Remarks</u>
1. Nursery			
a) Preparation & sowing	3.0	0.3	Hand tractor with puddling harrow (three times)
b) Care of seedling	2.0		
Sub-total	<u>5.0</u>	<u>0.3</u>	
2. Land Operation			
a) Plowing	0.5	0.5	Heavy tractor with discplow (cross-wise)
b) First puddling	1.5	1.5	Hand tractor with puddling harrow (cross-wise)
c) Second puddling	0.8	0.6	-do- (one time)
d) Repair of dikes	1.8		
Sub-total	<u>4.6</u>	<u>2.6</u>	
3. Transplanting			
a) Pulling seedlings	8.0		Regular planting
b) Transplanting	22.0		
Sub-total	<u>30.0</u>		
4. Fertilization			
a) Basal fertilization	1.0		
b) Additional fert.	1.0		
Sub-total	<u>2.0</u>		
5. Spraying			
a) Inseticides	2.0		
b) Herbicides	1.0		
Sub-total	<u>3.0</u>		
6. Weeding	11.0		
7. Irrigation/Drainage	6.0		
8. Harvesting			
a) Cutting/Bundling	20.8		Hand tractor with trailer
b) Hauling/Piling	3.7	0.9	
c) Threshing	5.5	1.0	
Sub-total	<u>30.0</u>	<u>1.9</u>	Thresher with hand tractor's engine
9. Post Harvesting			
a) Drying	4.0	2.5	Dryer
b) Sacking	2.0		
c) Piling/Delivery	1.5	0.3	Hand tractor with trailer
Sub-total	<u>7.5</u>	<u>2.8</u>	
Total	<u>99.1</u>	<u>7.6</u>	

Table A.4-42 Farm Labour Balance with Project

	Area	(Unit: man-day)												
		Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
Double cropping of paddy rice	1 ha	9.0	23.5	20.4	7.0	16.3	19.6	16.8	25.0	15.1	7.5	19.8	18.2	198.2
Vegetables & Fruits	1 ha	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	360.0
Animal husbandry	1 unit	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	180.0
Inland fishery	1 unit	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	180.0
Double cropping of paddy rice	9,452 ha	85,878	224,237	194,657	66,794	155,535	187,023	160,000	238,550	144,084	71,565	188,932	173,664	1,891,225
Vegetables	500 ha	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	180,000
Fruits	500 ha	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	15,000	180,000
Animal husbandry	250 unit	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	45,000
Inland fishery	250 unit	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	3,750	45,000
Total labour requirement	(1)	123,378	261,737	232,157	104,294	193,035	224,523	197,806	276,050	181,580	109,065	226,432	211,164	2,341,221
Available farm labour force	(2)	191,250	191,250	191,250	191,250	191,250	191,250	191,250	191,250	191,250	191,250	191,250	191,250	2,295,000
Balance	(2)-(1)	67,872	-70,487	-40,907	86,956	-1,785	-33,273	-6,556	-84,800	9,670	82,185	-35,182	-19,914	-46,221

Remarks: (1) Available farm labour force = 2,550 household x 3.0 workablemen per household x 25 days per month = 191,250 man-days per month

FIGURE A.4-24 FARM LABOUR BALANCE WITH PROJECT

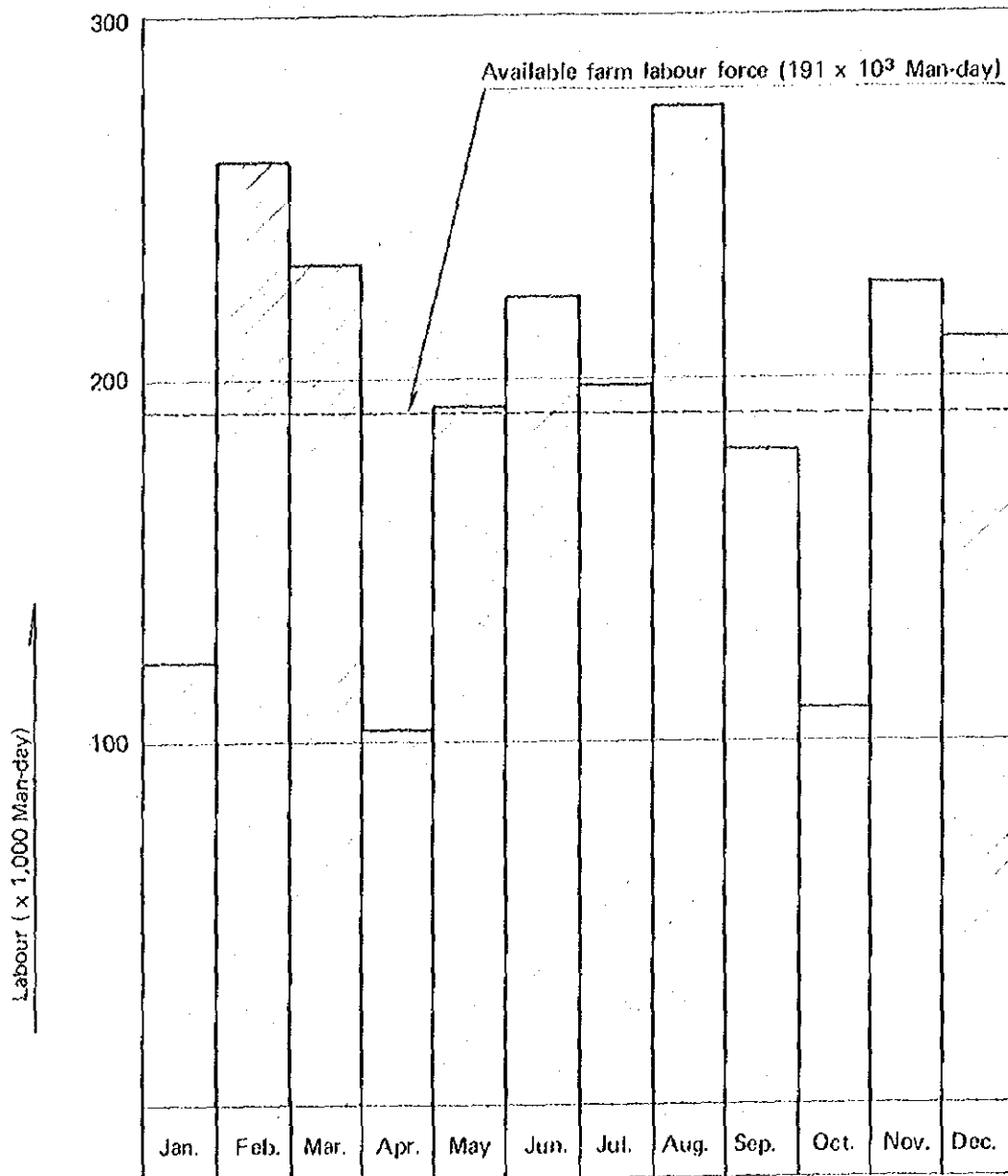




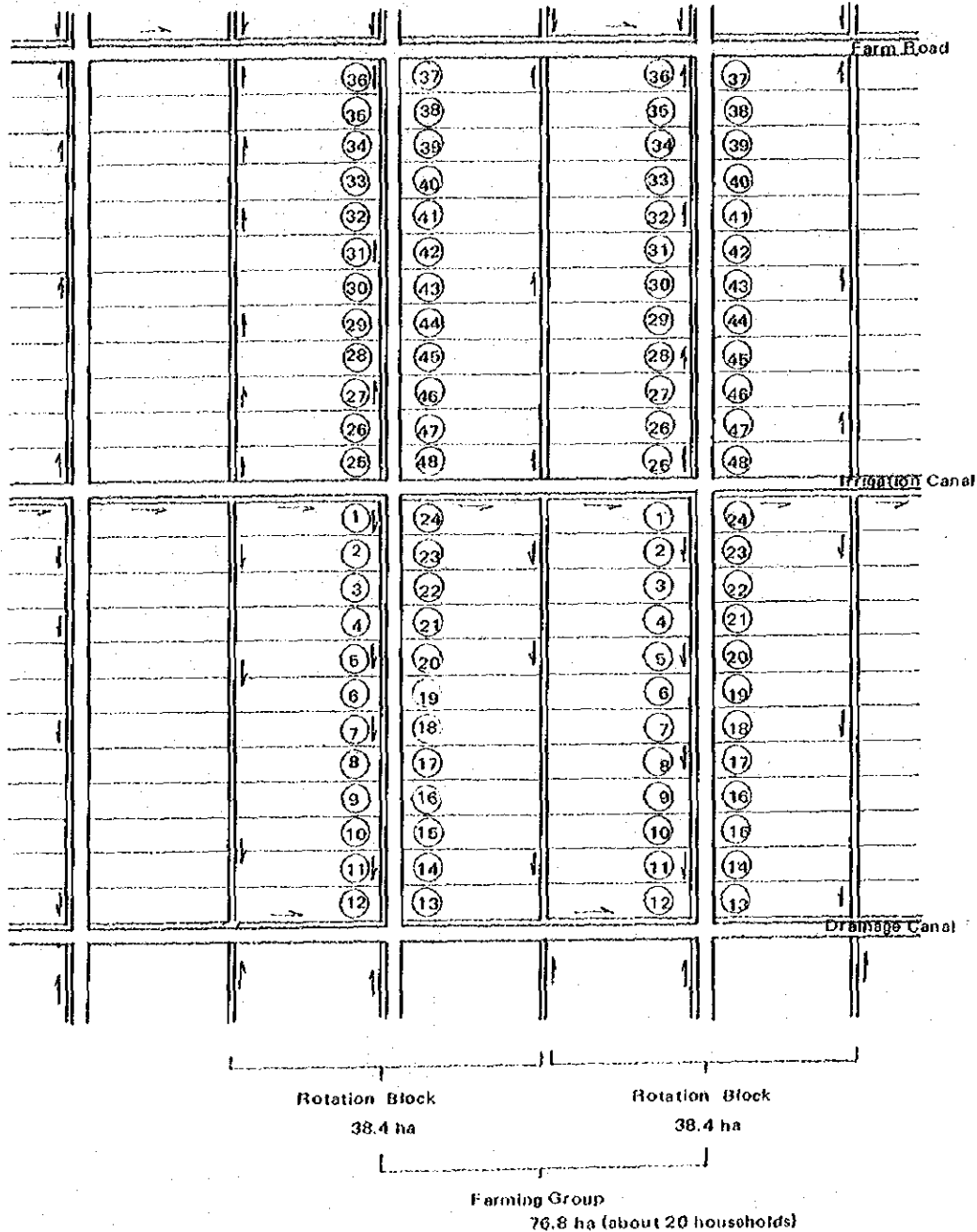
Table A.4-43 Introduction Plan of Animal Husbandry with Project

Kind	Managing Method	Production Unit			No. of unit in Project Area
		Raising Scale	Facility	Annual Production	
Swine	Integrated hog raising from breeding to fattening	Gilts(♀): 5 head Sire (♂): 1 head Breeded pigs: 80 head/year (7 head, usually) Fattening hogs: 78 head/year (10 head, usually)	Pen of gilts, sire & pigs: 48 m <sup>2</sup> Grazing ground of gilts & sire: 180 m <sup>2</sup> Pen of fattening hogs etc.: 35 m <sup>2</sup> Site area: 580 m <sup>2</sup>	Fattening hogs: 78 head (weight, 100 kg/head) Scraped gilt & sire: 2 head (weight, 140 kg/head)	83
Chicken	Raising "egg and meat type" chicken with introduction of 140 days old chicks	Chicks: 530 bird/year (177 bird, usually) Matured hens: 500 bird, usually	Pen of matured hens: 110 m <sup>2</sup> Pen of chicks: 25 m <sup>2</sup> Other facility: 40 m <sup>2</sup> Site area: 350 m <sup>2</sup>	Eggs: 108,000 eggs (216 egg/year x 500 head) Meat: 1,000 kg (2 kg/head x 500 head)	83
Duck	Raising "egg and meat type" ducks	Matured ducks: 500 bird, usually	Pen of matured ducks: 70 m <sup>2</sup> Site area: 140 m <sup>2</sup>	Eggs: 67,500 egg (150 egg x 500 head x 0.9) Meat: 675 kg (1.5 kg/head x 500 head x 0.9)	84
<u>Total</u>					<u>250</u>

Table A.4-44 Introduction Plan of Fresh-Water Fish Culture with Project

Kind	Managing Method	Production Unit			No. of unit in Project Area
		Raising Scale	Facility	Annual Production	
Carp	Raising 2 crops per year	No. of fry fishes per crop: 8,000 fish	Fish pond: 1,600 m <sup>2</sup> (one rai)	Fresh fishes: 1,500 kg x 2 (weight: 0.7 kg/fish)	83
Ciclid (Tilapia)	Raising 2 crops per year	No. of fry fishes per crop: 8,000 fish	-do-	Fresh fishes: 1,500 kg x 2 (weight: 0.5 kg/fish)	83
Cat-fish	Raising one crop per year	No. of fry fishes per crop: 6,000 fish	-do-	Fresh fishes: 7,500 kg (weight: 2.0 kg/fish)	84
<u>Total</u>					<u>250</u>

FIGURE A.4 - 25 PLAN OF FARMING GROUP



- Ⓢ : Plot number
- 1 plot : 160 x 50 m = 0.8 ha
- Rotation Block : 0.8ha x 48 plots = 38.4 ha (Period of land preparation 48 days)
- Farming Group : 2 Rotation Blocks = 38.4 ha x 2 = 76.8 ha (about 20 farm households)
- Management Group: 4 Farming Groups = 76.8 x 4 = 307.2 ha  
(about 80 farm households)

Table A.4-45. Required Capacity and Cost for Rice Processing Facilities

1. Increment of Paddy Yield		(unit: tons)		
	<u>Planned Yield</u>	<u>Present Yield</u>		<u>Increment</u>
Dry Season	44,847 (9,542ha x 4.7ton/ha)	7,170	=	37,677
Wet Season	40,076 (9,542ha x 4.2ton/ha)	7,404	=	32,674
Total	<u>84,923</u>	<u>14,574</u>		<u>70,349</u>

## 2. Capacity and Equipment Cost of Warehouse (Unit: ¥'000)

37,677 tons (Dry Season) x 0.77 % = 29,000 tons is the required capacity of warehouse

20,000 tons of paddy will be stored by agricultural cooperatives

Capacity per warehouse : 500 tons (300 m<sup>2</sup> x 3 m)  
 Number of warehouse : 40 (= 20,000 tons/500 tons)  
 Cost of warehouse : ¥20,000 (= 40 warehouses x 2¥500)

Remaining paddy of 9,000 tons will be stored by each farm household

Capacity per warehouse : 15 tons (10 m<sup>2</sup> x 3 m)  
 Number of warehouse : 600 (= 9,000 tons/15 tons)  
 Cost of warehouse : ¥3,630 (= 600 warehouses x 2¥3,630)

## 3. Capacity and Equipment Cost of Rice Mill

A half of stored paddy rice (29,000 tons) in the dry season is sold without milling and the remaining half is milled. The yield of paddy in the wet season is less than that in the dry season. So, the required capacity of mill is as follows:

$$29,000 \text{ tons} \times 1/2 = 15,000 \text{ tons}$$

Paddy of 3,000 tons is polished in the dry season by a mill with the annual capacity of 6,000 tons (per 200 days) and 2,000 tons polished by a mill with the annual capacity of 4,000 tons. Thus, three sets of each milling machine are required to polish paddy of 15,000 tons in the dry season. The equipment costs are as follows:

6,000 tons per year : 3 sets x @¥1,900 = ¥5,700 (Unit: ¥'000)

4,000 tons per year : 3 sets x @¥1,600 = ¥4,800

Total ¥10,500

Table A.4-46. Seeds Demand in Planning Stage

	<u>Cultivation Acreage</u> (ha)	<u>Seeds Demand per Ha.</u> (ton)	<u>Seeds Demand</u> (ton)	<u>Unit Price</u> (¥'000)	<u>Amount</u> (¥'000)
<b>Paddy</b>					
Wet Season	9,542	0.04	382	2.6	992
Dry Season	9,542	0.04	382	2.6	992
Sub-total	<u>19,084</u>	-	<u>764</u>	-	<u>1,984</u>
<b>Vegetables</b>					
Chinese Cabbage	500	0.004	2.0	158	316
Cabbage	500	0.0005	0.3	400	120
Cauliflower	500	0.0004	0.2	388	78
Sub-total	<u>1,500</u>	-	<u>2.5</u>	-	<u>517</u>
		(trees)	(trees)		
Fruits	260	560	14,560	0.010	1,456
<b>Fish</b>					
Carp	14	50 x 2*	1,400	0.2 *	280
Tilapia	13	50 x 2*	1,400	0.14*	196
Catfish	13	38 *	494*	2.0 *	988
Sub-total	<u>40</u>	-	<u>3,294*</u>	-	<u>1,464</u>
<b>Livestock</b>					
Swine	11	455	5	100	500
Chicken	6	10	62.5	1	63
Duck	3	10	62.5	1	63
Sub-total	<u>20</u>	-	<u>130.0</u>	-	<u>626</u>
Total					<u>7,330</u>

Note: \* expressed in thousand fingerlings or ¥'000 per thousand fingerlings

FIGURE A.4 - 26 PLAN OF SEED CENTER

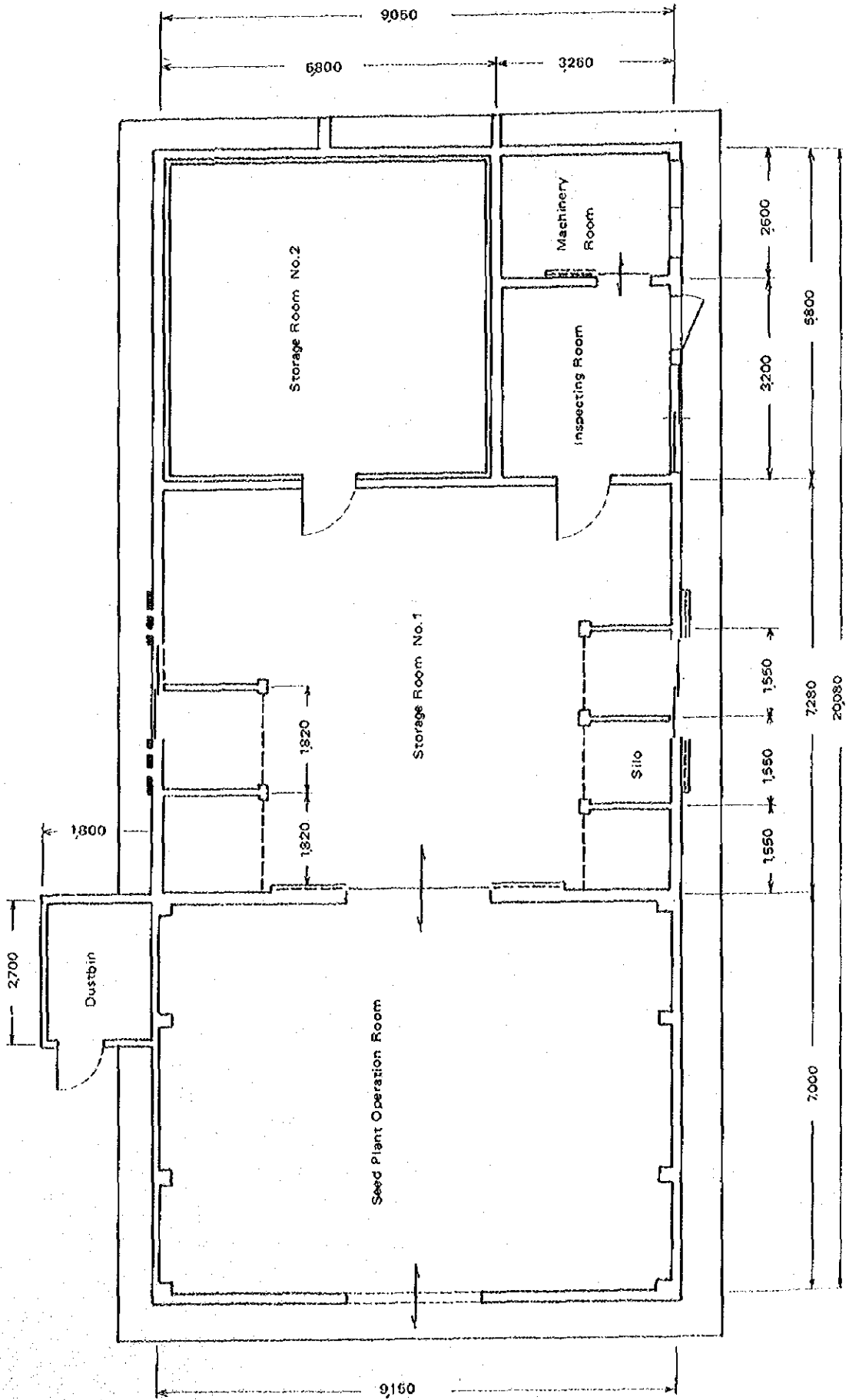


Table A.4-47. Demand of Fertilizers and Agricultural Chemicals

<u>Crops</u>	<u>Input Qty. per ha.</u> (kg)	<u>Cultivation Acreage</u> (ha)	<u>Input Qty.</u> (ton)	<u>Cost per ha.</u> (₱'000)	<u>Amount</u> (₱'000)
<u>Fertilizers</u>					
Dry season paddy	262	9,542	2,500	1.049	10,010
Wet season paddy	220	9,542	2,099	0.881	8,407
Fruits	1,960	500	980	8.120	4,060
Vegetables	1,908	500	954	9.540	4,770
Total			<u>6,533</u>		<u>27,247</u>
<u>Other Materials</u>					
Lime for paddy	3,000	9,542	28,626	0.270	2,576
Lime for fruits & vegetables	6,000	1,000	6,000	0.936	936
Total			<u>34,626</u>		<u>30,759</u>
<u>Agricultural Chemicals</u>					
Dry season paddy	21.2	9,542	202	0.763	7,280
Wet season paddy	21.2	9,542	202	0.763	7,280
Fruits	1,285	500	642	14.805	7,403
Vegetables	1,117 (372 x 3)	500	559	19.962	9,981
Total			<u>1,605</u>		<u>31,944</u>

Table A.4-48. Number of Farm Machinery to be introduced in the Project Area

## 1. Number of Machinery for One Farming Group (76.8 ha)

<u>Kinds of Machinery</u>	<u>Case I (set)</u>	<u>Case II (set)</u>	<u>Operational Capacity</u>
Heavy Tractor	1/2	-	Plowing: 2.0 ha/day (2 times)
Medium-size Tractor	-	1	Plowing: 1.2 ha/day (1 time)
Hand Tractor (7-8 Hp)	20	20	Puddling: 1.0 ha/day
7-8 Hp Class Thresher	2	2	Threshing: 1.0 ha/day
2 tons Bin Drier	1	1	Drying: 2.5 ha/day

## 2. Number of Farm Machinery in the Project (For Farming Groups of Paddy - 9,542 ha, 125 groups)

<u>Kinds of Machinery</u>	<u>Case I (set)</u>	<u>Case II (set)</u>	<u>Remarks</u>
Heavy Tractor	63	-	with disc plow & trailer
Medium-size Tractor	-	124	with rotary plow & trailer
Hand tractor (7-8 Hp)	2,500	2,500	with puddling harrow and trailer
7-8 Hp Class Thresher	248	248	without engine
2 tons Bin Dryer	125	125	

## 3. Number of Farm Machinery in the Project (For Farming Groups of Orchard and Vegetables - 500 ha and 6 groups respectively)

<u>Kinds of Machinery</u>	<u>Case I (set)</u>	<u>Case II (set)</u>	<u>Remarks</u>
Medium-size Tractor	6	6	one set per group
7-8 Hp Class Power Tiller	12	12	two sets per group
Powered Sprayer	12	12	two sets per group

Note: Heavy tractor - over 50 Hp, four wheel type

Medium-size tractor - 20 Hp class, two wheel type

Table A.4-19. Cost of Farm Machinery

<u>Farm Machinery</u>	<u>Unit</u>	<u>Unit Price</u> <u>(¥'000)</u>	<u>Amount</u> <u>(¥'000)</u>	<u>Unit</u>	<u>Unit Price</u> <u>(¥'000)</u>	<u>Amount</u> <u>(¥'000)</u>
Heavy Tractor	63	253	15,939	-	-	-
Medium Size Tractor	6	98	588	130	98	12,740
7-8 HP Hand Tractor	2,500	15	37,500	2,500	15	37,500
7-8 HP Power Tiller	12	25	300	12	25	300
7-8 HP Class Thresher	248	12	2,976	248	12	2,976
Powered Sprayer	12	20	240	12	20	240
2 Ton Bin Dryer	125	15	1,875	125	15	1,875
Total			<u>59,418</u>			<u>55,631</u>

- Note: 1) The prices of tractors are including the prices of their attachments.
- 2) The price of heavy tractor exclusive of trailer is ¥233,000.
- 3) Heavy tractor - over 50 hp, four wheel type  
Medium-size tractor - 20 hp class, four wheel type.



FIGURE A.4 - 27 STANDARD PLAN OF REPAIR SHOP FOR FARM MACHINERY

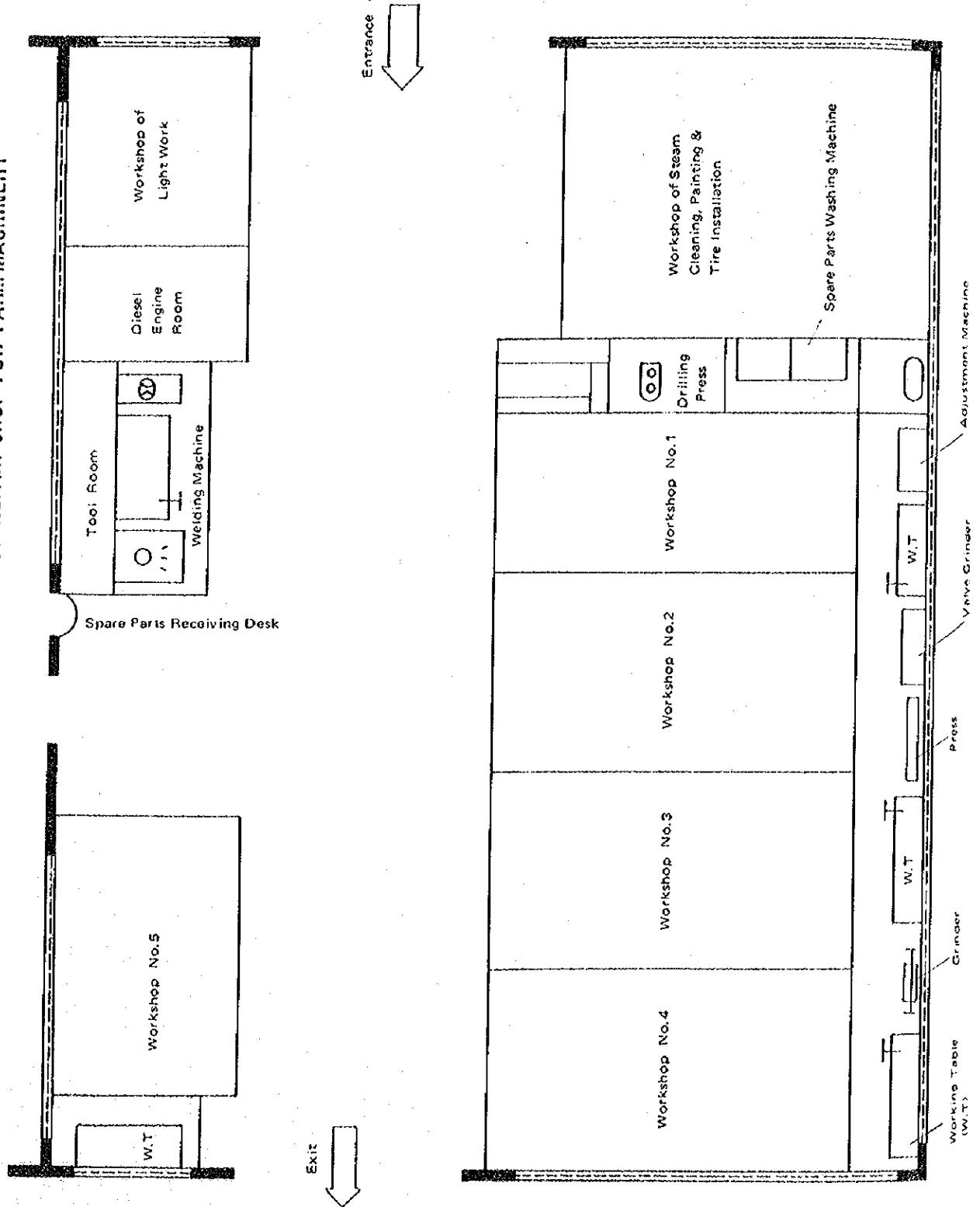


FIGURE A.4-28. ANNUAL TRAINING SCHEDULE

Dry season puddling & transplanting period: Jan. 15 - Mar. 15 (60 days)  
 Wet season puddling & transplanting period: Jul. 15 - Sept. 10 (58 days)

Training Items	1	2	3	4	5	6	7	8	9	10	11	12	Total
Water Management (To Common Irrigators)	■						■						twice 80
Preparation & Guidance for Establishment of ALRC (To ALRC staff)			■										once 40
Management of Farming Group (To Farming Group Leaders)					■	■				■	■		four 160
Guidance on Seed Breeding & Paddy Planting (To Persons-in-Charge of Seed Breeding and Farmers)													once 40
Guidance on Agricultural Mechanization (To Persons-in-Charge of Agricultural Mechanization)						■							twice 80
General Guidance (To Farmers)	Occasional inspections and meetings are applied to farmers' training												100
Total													500

Table A.4-50. Organization and Operation Expenses of ALRC

1. Organization of ALRC

<u>Personnel</u>	<u>No. of Personnel</u>	<u>Man</u>	<u>Woman</u>	<u>Truck &amp; Boat</u>	<u>Motor Bike</u>	<u>Wireless Telephone</u>	<u>Office Space</u>
<u>1. Director</u>							
Director	10 <sup>1/</sup>	-	-	-	-	-	160 <sup>2/</sup>
Deputy Director	2	-	-	-	-	-	
<u>2. Staff</u>							
General Manager	1	1	-	-	-	-	10
Div. of General Affairs	5	3	2	-	1	-	30
Div. of Extension	8	6	2	-	4	-	50
Div. of Credits	9	5	4	-	1	-	50
Div. of Sales & Purchasing	12	7	5	2 <sup>3/</sup>	2	-	50
Div. of Utilization	15	12	3	1 <sup>4/</sup>	4	-	50
Div. of Farm Machinery	10	7	3	1 <sup>5/</sup>	3	34	50
Div. of Irrigation Water Control	7	5	2	1 <sup>6/</sup>	5		50
Branch (Production Unit)	8	4	4	-	4	-	-
Total	<u>75</u> (87)	<u>50</u>	<u>25</u>	<u>5</u>	<u>24</u>	<u>34</u>	<u>500</u>

Note: <sup>1/</sup> including each general and vice-general director of ALRC

<sup>2/</sup> 160 m<sup>2</sup> = 30 m<sup>2</sup> (room for general director of ALRC + 50 m<sup>2</sup> (meeting room) + 50 m<sup>2</sup> (directors' room) + 30 m<sup>2</sup> (preparatory room)

<sup>3/</sup> 6-ton truck

<sup>4/</sup> boat

<sup>5/</sup> 2-ton truck

<sup>6/</sup> 2-ton truck

2. Personnel Expenses of ALRC

<u>Personnel</u>	<u>Yearly Unit Expenses</u> (¥'000)	<u>Number</u>	<u>Amount</u> (¥'000)
1) Director			
General Director	12	1	12
Vice General Director	6	1	6
Director	3	8	24
Deputy Director	3	2	6
Sub-total		<u>12</u>	<u>48</u>
2) Man Staff			
General Manager	72	1	* 72
Division Chief	50	7	*350
Clerk - 1st Class	30	10	300
- 2nd Class	25	12	300
- 3rd Class	15	9	135
Expert	45	11	*495
Sub-total		<u>50</u>	<u>1,652</u>
3) Woman Staff			
Woman Clerk - 1st Class	10	10	100
- 2nd Class	96	15	144
Sub-total		<u>25</u>	<u>244</u>
4) Others			
Common Irrigator	3	29	97
Leader of Management Group	3	29	97
Leader of Farming Group	1	136	136
Hired labor	(1,917 man x ¥30 x 12 mos.)		690
Sub-total		<u>194</u>	<u>1,020</u>
5) Total		<u>281</u>	<u>2,964</u>

\* Paid by the Government (¥917,000). The deduction of ¥917,000 from the total amount of ¥2,964,000 produces ¥2,047,000.

3. Expenses for Preparation and Establishment of ALRC

<u>Year</u>	<u>Amount (¥'000)</u>
1977	500
1978	500
1979	500
Total	<u>1,500</u>

Remarks: Each amount is including the expenses for preparation for land register books and execution of disposition for substitute plots

4. Expenses for the Training by ALRC

<u>Year</u>	<u>Amount (¥'000)</u>
1977	500
1978	500
1979	500
1980	500
1981	500
1982	500
1983	500
Total	<u>3,500</u>

5. Annual Operational Expenses for ALRC

<u>Expense</u>	<u>Amount (¥'000)</u>
Travel	990
Office Maintenance	593
Meeting	100
Fuel & Others	200
Contingency	300
Total	<u>2,183</u>

6. Personnel and Operation Expenses Paid by the Government for Execution of Supporting Service

Government Office	Manager Class @¥72,000	Expert @¥45,000	1st Class Clerk @¥50,000	2nd Class Clerk @¥30,000	3rd Class Clerk @¥15,000	Amount
Changwat	-	1	-	-	-	2*
	-	45	-	30	-	75**
Amphoe	-	1	-	1	-	2*
	-	45	-	30	-	75**
Tambon	-	-	-	-	3	3*
	-	-	-	-	45	45**
RID	-	-	1	3	-	4*
	-	-	(1)	-	-	(1)
	-	-	50	90	-	140
	-	-	(50)	-	-	(50)
Agric. Cooperative	-	6	3	1	1	11*
	-	(5)	(2)	-	-	(7)
	-	270	150	30	15	465**
	-	(225)	(100)	-	-	(325)
Extension	-	4	1	3	-	8*
	-	(1)	-	-	-	(1)
	-	180	50	90	-	320**
	-	(45)	-	-	-	(45)
ALRO	1	6	5	4	4	20*
	(1)	(5)	(4)	-	-	(10)
	72	270	250	120	60	772**
	(72)	(225)	(200)	-	-	(497)
Operational Expenses for above expenses						608**
Total	1	18	10	13	8	50*
	(1)	(11)	(7)	-	-	(19)
	72	810	500	390	120	2,500**
	(72)	(495)	(35)	-	-	(917)

Note: The expenses in parentheses are for ALRC.  
\* No. of person  
\*\* Amount in ¥1,000

Table A.4-51. Proposed Cropping Area in Accordance with Land Consolidation

Crops	Block	Unit	1979		1980		1981		1982		1983		1984		1985		1986		1987		1988		1989		
			Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	
Cereals	Broadcasting	ton/ha	-	1.6	-	1.6	-	1.6	-	1.6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
		Area(ha)	-	2,190	-	1,800	-	850	-	400	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	Transplanting	A	ton/ha	3.0	2.2	3.0	2.6*	3.4*	3.0	3.7	3.4	4.0	3.8	4.3	4.2**	4.7**	4.2	4.7	4.2	4.7	4.2	4.7	4.2	4.7	4.2
		Area(ha)	540	580	-	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500	2,500
	Transplanting	A+B	ton/ha	3.0	2.2	3.0	2.2	3.0	2.6*	3.4*	3.0	3.7	3.4	4.0	3.8	4.3	4.2**	4.7**	4.2	4.7	4.2	4.7	4.2	4.7	4.2
		Area(ha)	290	660	290	660	-	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860	2,860
	Transplanting	C	ton/ha	3.0	2.2	3.0	2.2	3.0	2.2	3.0	2.6*	3.4*	3.0	3.7	3.4	4.0	3.8	4.3	4.2**	4.7**	4.2	4.7	4.2	4.7	4.2
		Area(ha)	780	268	780	268	780	268	-	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000	2,000
	Transplanting	C	ton/ha	3.0	2.2	3.0	2.2	3.0	2.2	3.0	2.2	3.0	2.6*	3.4*	3.0	3.7	3.4	4.0	3.8	4.3	4.2**	4.7**	4.2	4.7	4.2
		Area(ha)	780	265	780	265	780	265	-	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182	2,182
	Fruits	B	ton/ha			(-)		(-)		(15)		(21)		(24)		(27)		(30)		(30)		(30)		(30)	
			Area(ha)	15		18		21		24		27		30		30		30		30		30		30	
C		ton/ha							(-)		(-)		(15)		(21)		(24)		(27)		(30)		(30)		(30)
		Area(ha)	15		15		15		18		21		24		27		30		30		30		30		30
Vegetables		A	ton/ha	8		8		14		20		26		32		38		38		38		38		38	
		Area(ha)	39		39		150		150		150		150		150		150		150		150		150		150
B	ton/ha	8		8		8		14		20		26		32		38		38		38		38		38	
	Area(ha)	20		20		20		150		150		150		150		150		150		150		150		150	
C	ton/ha	8		8		8		8		14		20		26		32		38		38		38		38	
	Area(ha)	47		47		47		47		200		200		200		200		200		200		200		200	
Total				<u>6,699</u>		<u>7,729</u>		<u>8,800</u>		<u>9,552</u>		<u>10,542</u>		<u>10,542</u>		<u>10,542</u>		<u>10,542</u>		<u>10,542</u>		<u>10,542</u>		<u>10,542</u>	

Note: \* Initial year after the completion of land consolidation works  
 \*\* Target year to display full benefits after the completion of land consolidation works  
 ( ) New planting

Table A.4-52. Expected Yield in Accordance with Land Consolidation

		(Unit: ton)																						
Crop	Block	1979		1980		1981		1982		1983		1984		1985		1986		1987		1988		1989		
		Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	
<b>Waddy</b>																								
Broadcasting	ABC	-	3,504	-	2,880	-	1,360	-	640	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Transplanting	A	1,620	1,276	-	6,500	8,500	7,500	9,250	8,500	10,000	9,500	10,750	10,500	11,750	10,750	11,750	10,500	11,750	10,500	11,750	10,500	11,750	10,500	
Transplanting	A+B	870	1,452	870	1,452	-	7,436	9,724	8,580	10,582	9,724	11,440	10,868	12,298	12,012	13,442	12,012	13,442	12,012	13,442	12,012	13,442	12,012	
Transplanting	C	2,340	590	2,340	590	2,340	590	-	5,200	6,800	6,000	7,400	6,800	8,000	7,600	8,600	8,400	9,400	8,400	9,400	8,400	9,400	8,400	
Transplanting	C	2,340	583	2,340	583	2,340	583	2,340	583	-	5,673	7,419	6,546	8,073	7,419	8,728	8,292	9,383	9,164	10,255	9,164	10,255	9,164	
Total		<u>7,170</u>	<u>7,405</u>	<u>5,550</u>	<u>12,005</u>	<u>13,180</u>	<u>17,469</u>	<u>21,314</u>	<u>23,503</u>	<u>27,382</u>	<u>30,897</u>	<u>37,009</u>	<u>34,714</u>	<u>40,121</u>	<u>37,531</u>	<u>42,520</u>	<u>39,312</u>	<u>43,975</u>	<u>40,076</u>	<u>44,847</u>	<u>40,076</u>	<u>44,847</u>	<u>40,076</u>	
<b>Fruits</b>																								
	B	900		1,080		1,260		2,040		2,460		2,760		2,880		3,000		3,000		3,000		3,000		3,000
	C	2,700		2,700		2,700		3,240		3,780		8,160		9,480		10,680		11,340		12,000		12,000		12,000
Total		<u>3,600</u>		<u>3,870</u>		<u>3,960</u>		<u>5,280</u>		<u>6,240</u>		<u>10,920</u>		<u>12,360</u>		<u>13,680</u>		<u>14,340</u>		<u>15,000</u>		<u>15,000</u>		<u>15,000</u>
<b>Vegetable</b>																								
	A	312		312		2,100		3,000		3,900		4,800		5,700		5,700		5,700		5,700		5,700		5,700
	B	160		160		160		2,100		3,000		3,900		4,800		5,700		5,700		5,700		5,700		5,700
	C	376		376		376		376		2,800		4,000		5,200		6,400		7,600		7,600		7,600		7,600
Total		<u>848</u>		<u>848</u>		<u>2,636</u>		<u>5,476</u>		<u>9,700</u>		<u>12,700</u>		<u>15,700</u>		<u>17,800</u>		<u>19,000</u>		<u>19,000</u>		<u>19,000</u>		<u>19,000</u>



Table A.4-53. Annual Input Materials with Project - Seeds

Crop	1979		1980		1981		1982		1983		1984		Remarks
	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	
Paddy													
Qty (ton)	(108)	(365)	( 83)	(290)	( 70)	(135)	( 35)	( 64)	-	-	-	-	
Qty (ton)	-	-	100	100	100	215	215	295	295	382	382	382	
Amount (£'000)	260	949	216	1,014	442	910	650	933	767	992	992	992	Unit Price: £2,600
Fruits													
Qty (tree)	-	-	22,400	-	67,200	-	56,000	-	-	-	-	-	Total of tree: 145,600
Amount (£'000)	-	-	224	-	672	-	560	-	-	-	-	-	
Vegetables													
Qty (ton)	0.212	-	0.212	-	1.034	-	1.894	-	2.5	-	2.5	-	Unit Price: £205.6
Amount (£'000)	44	-	44	-	213	-	389	-	514	-	514	-	
Fish													
Amount (£'000)	-	-	-	-	320	-	430	-	714	-	1,464	-	
Livestock													
Amount (£'000)	-	-	-	-	230	-	250	-	567	-	626	-	
Total													
Amount (£'000)	1,253	-	1,498	-	2,868	-	3,212	-	3,554	-	4,588	-	

Note: Figure in parenthesis refers to the quantity of input materials before land consolidation

Table A.4-54. Annual Input Materials with Project — Agricultural Chemicals

Crops	1979		1980		1981		1982		1983		1984	
	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet
<b>Paddy</b>												
Qty (ton)	(152)	(172)	(118)	(232)	(99)	(61)	(50)	(31)				
Qty (ton)		53		53	53	114	114	156	156	202	202	202
Amount (£'000)	1,389	1,823	1,076	3,325	3,393	5,019	5,019	6,081	5,616	7,280	7,280	7,280
<b>Fruits</b>												
Qty (ton)	(216)		(162)		(162)							
Qty (ton)			128		128		385		514		642	
Amount (£'000)	2,487		3,335		3,335		4,428		5,911		7,403	
<b>Vegetables</b>												
Qty (ton)	-	-	-	-	168		335		559		559	
Amount (£'000)	-	-	-	-	2,994		5,989		9,981		9,981	
<b>Total</b>												
Qty (ton)	540		693		785		1,071		1,431		1,605	
Amount (£'000)	5,699		7,646		14,741		21,517		28,788		31,944	

Note: Figures in parenthesis refers to the quantity of input materials before land consolidation

Table A.4-55. Annual Input Material with Project - Fertilizers

	1979		1980		1981		1982		1983		1984	
	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet	Dry	Wet
<b>Crops</b>												
Paddy												
Qty (ton)	(449)	(334)	(348)	(223)	(293)	(100)	(147)	( 50)				
Qty (ton)		550		655		1,179		1,404		1,928		2,090
Amount (B'000)	1,843	1,367	1,392	3,092	3,792	5,116	6,204	6,676	8,395	7,712	10,010	8,407
<b>Fruits</b>												
Qty (ton)	(329)		(247)		(247)							
Qty (ton)		196		196		196		980		980		980
Amount (B'000)	1,363	1,816	1,816	1,816	1,816	1,816	4,050	4,050	4,050	4,050	4,050	4,050
<b>Vegetables</b>												
Qty (ton)	-	-	-	-	286		572		954		954	
Amount (B'000)	-	-	-	-	1,430		2,860		4,770		4,770	
<b>Others (Lime, Oil)</b>												
Qty (ton)	-	-	-	-	-		-		-		28,626	
Amount (B'000)	-	-	878		1,756		2,634		3,512		3,512	
<b>Total</b>												
Qty (ton)	1,112		1,564		2,956		4,772		5,961		6,533	
Amount (B'000)	4,573		7,178		14,010		22,434		28,450		30,759	

Note: Figure in parenthesis refers to the quantity of input materials before land consolidation



Table A.4-57. Cost of Supporting Services Facilities

Annual Interest: 5%

Unit: ₪'000

Name of Facilities	Cost	Life (year)	Payment per Year	Annual Repair Cost (1%)	Total	₪/Ha
ALRC Office	2,000	30	130	20	150	14
Production Unit Office	1,200	30	78	12	90	9
Training Center	2,000	30	130	20	150	14
Seed Center	2,000	26	139	20	159	15
Motor Pool	1,225	26	85	12	97	9
Spare Machine	1,461	8	226	15	241	23
Assemble House & Warehouse <sup>1/</sup>	2,000	26	140	20	160	15
Truck and Motor Bicycle <sup>2/</sup>	1,576	8	244	16	260	25
Water Supply	1,400	26	97	14	111	11
<b>Total</b>	<b>14,862</b>		<b>1,269</b>	<b>141</b>	<b>1,418</b>	<b>135</b>

Proposed Area

10,542 ha

Farm Machinery	55,631	} 91,861 is not included in Supporting Services' Cost
Hanger	2,100	
Warehouse	23,630	
Rice Mill	10,500	

Note: <sup>1/</sup> Inclusive of truck terminal, navigation center and shopping center

<sup>2/</sup> Truck - 6 ton :	₪260,000	x	2	=	₪520,000
Pick-Up, 2 ton:	₪100,000	x	2	=	₪200,000
Boat :	₪500,000	x	1	=	₪500,000
Motor Bicycle :	₪ 12,000	x	24	=	₪288,000
Wireless Telephone:	₪ 2,000	x	34	=	₪ 68,000
<b>Total</b>	<b>₪1,576,000</b>				

FIGURE A.4 - 28.  
TYPICAL PLAN OF VILLAGE RE-ARRANGEMENT

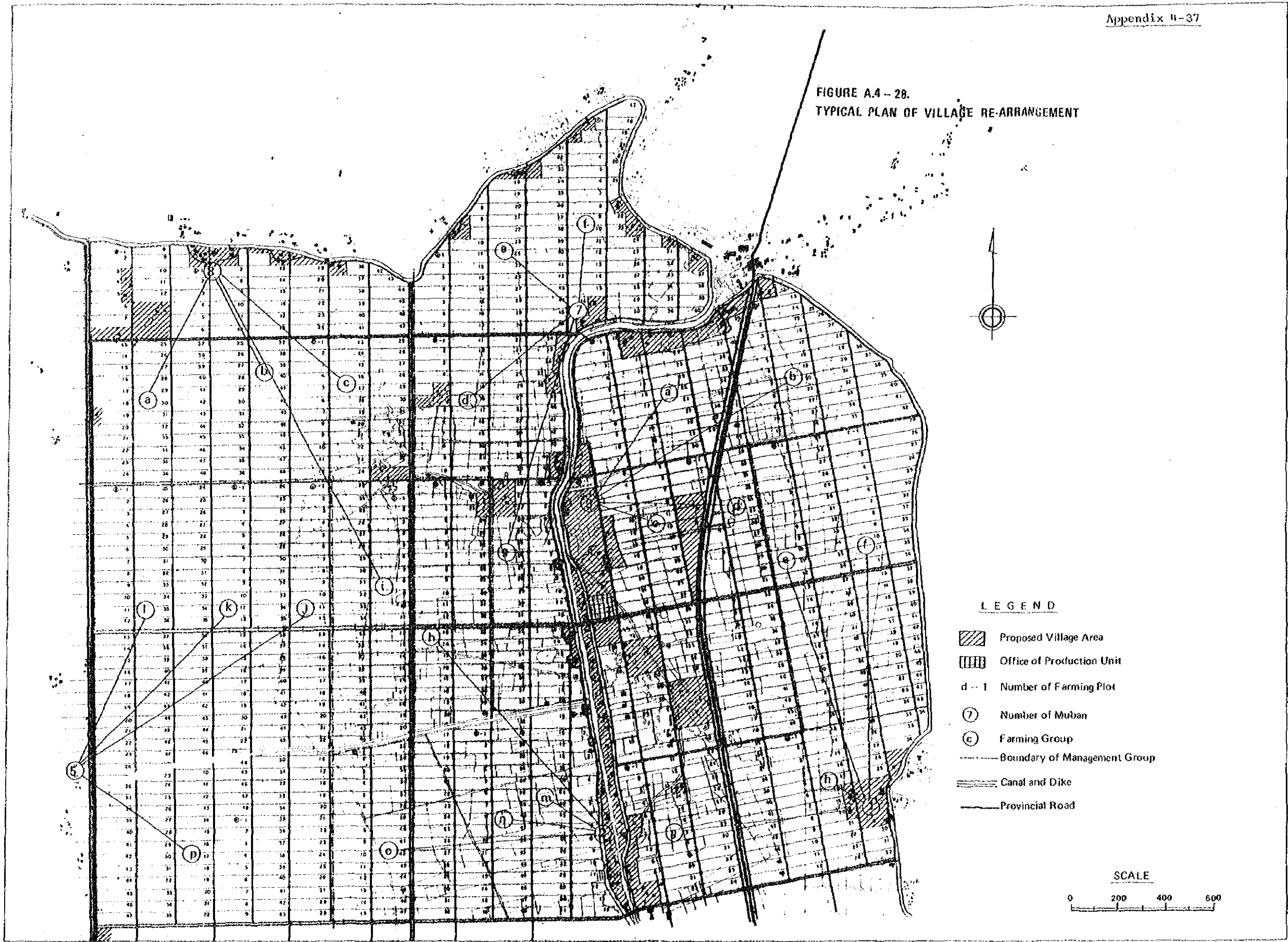


FIGURE A.4 - 30  
LOCATION OF PUBLIC FACILITY AREA AND ROAD NETWORK WITH PROJECT

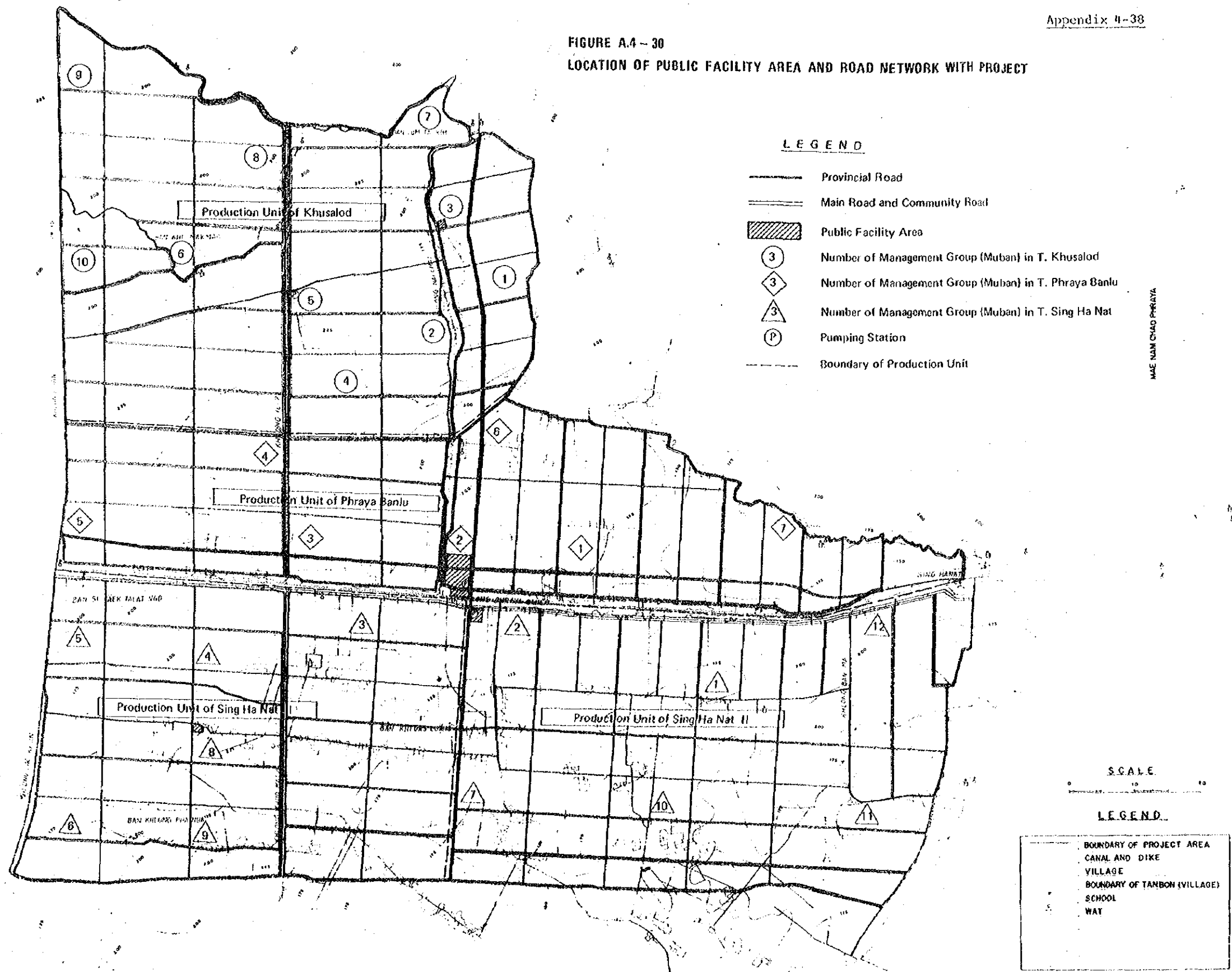
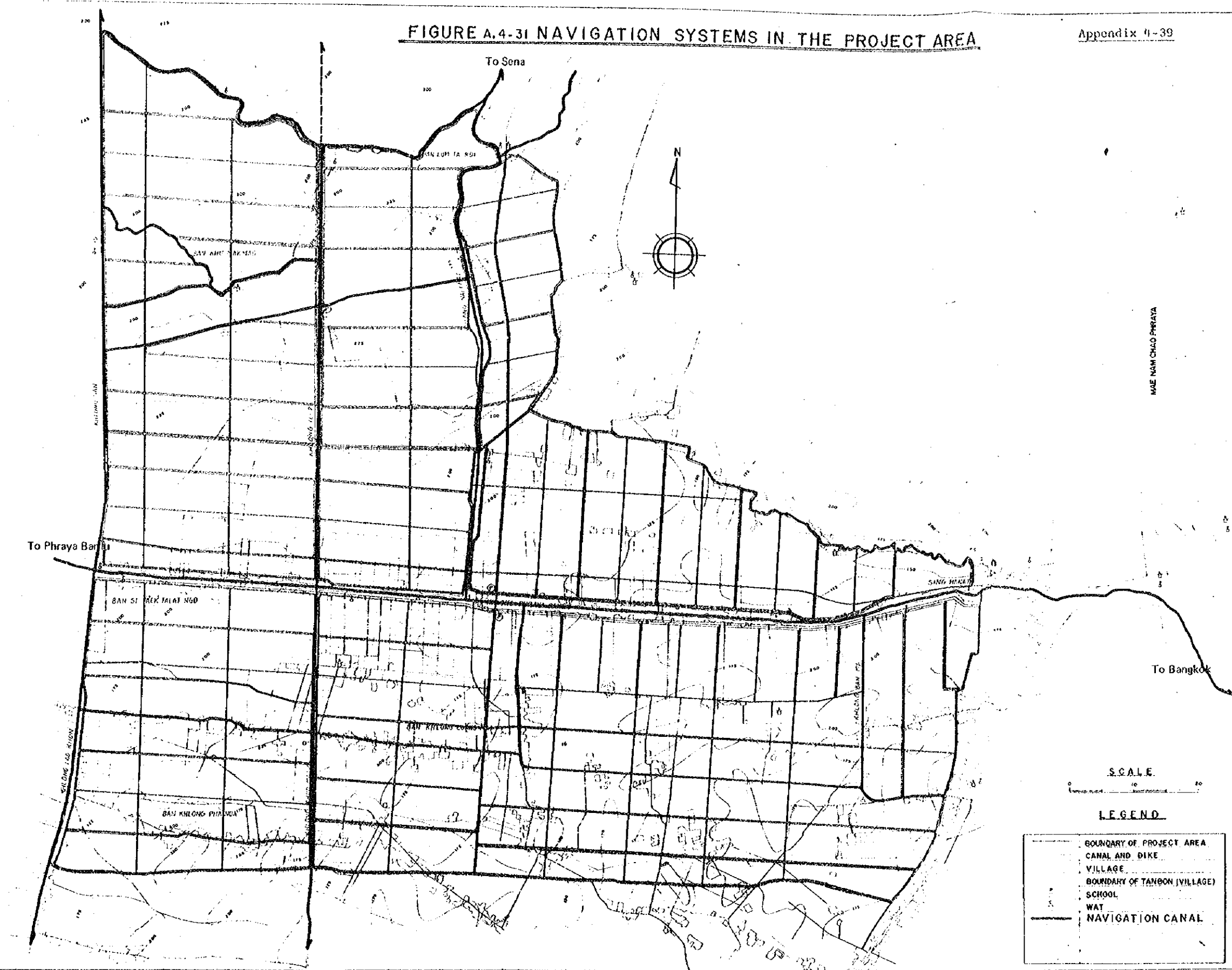


FIGURE A.4-31 NAVIGATION SYSTEMS IN THE PROJECT AREA



SCALE

0 10 20

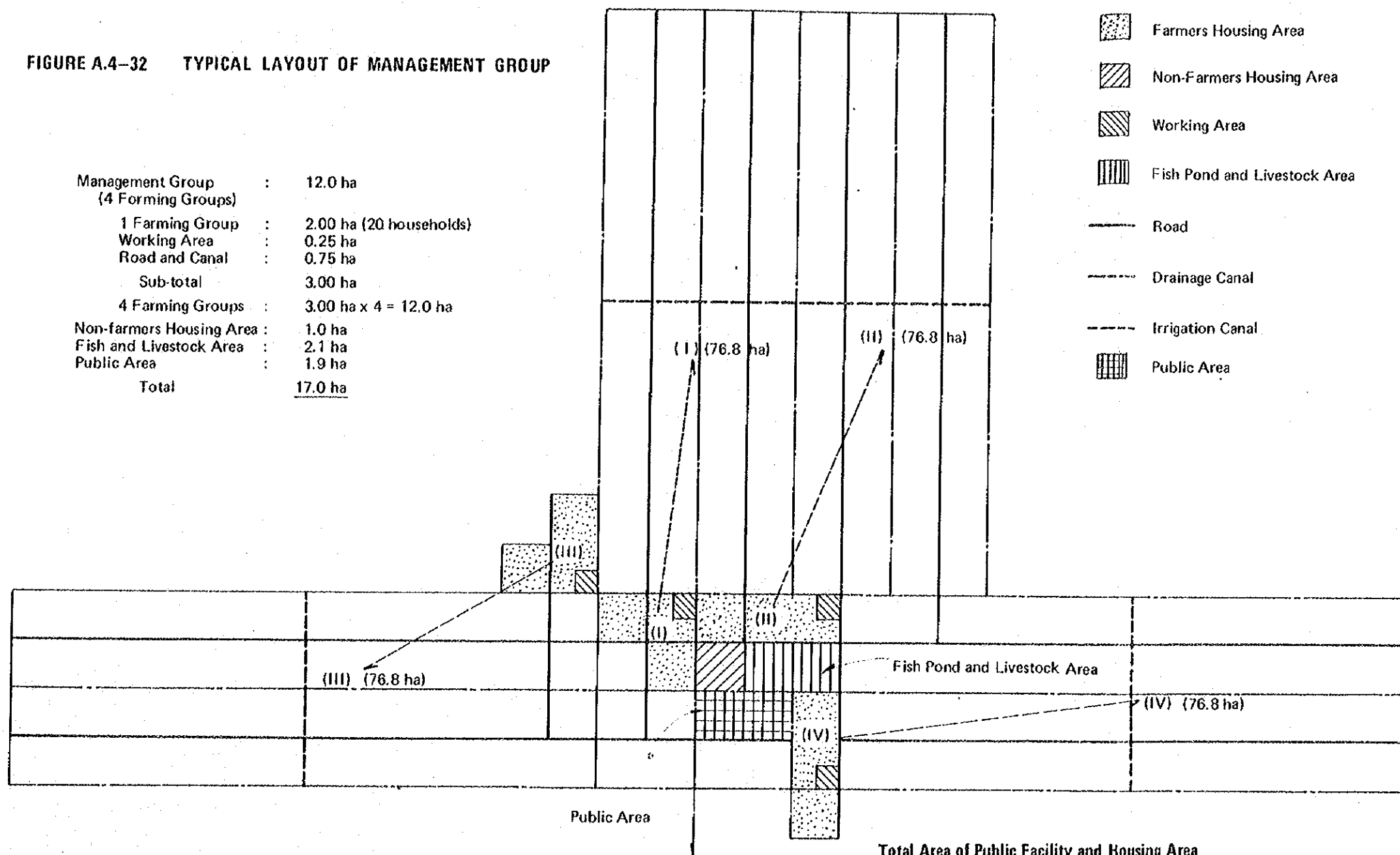
LEGEND

- BOUNDARY OF PROJECT AREA
- CANAL AND DIKE
- VILLAGE
- BOUNDARY OF TANBON (VILLAGE)
- SCHOOL
- WAT
- NAVIGATION CANAL



FIGURE A.4-32 TYPICAL LAYOUT OF MANAGEMENT GROUP

Management Group	:	12.0 ha
(4 Farming Groups)		
1 Farming Group	:	2.00 ha (20 households)
Working Area	:	0.25 ha
Road and Canal	:	0.75 ha
Sub-total	:	3.00 ha
4 Farming Groups	:	3.00 ha x 4 = 12.0 ha
Non-farmers Housing Area	:	1.0 ha
Fish and Livestock Area	:	2.1 ha
Public Area	:	1.9 ha
<b>Total</b>		<b><u>17.0 ha</u></b>



**Total Area of Public Facility and Housing Area**

Village Area	:	17 ha x 29 = 493 ha	
ALRC Center Area	:	*24	(* : Refer to Figure A.4-32)
Production Unit Area	:	*2.5 ha x 4 = 10	
Temple and School	:	85	
Others	:	7	
<b>Total</b>		<b><u>619 ha</u></b>	

FIGURE A.4 - 33 TYPICAL LAYOUT OF PUBLIC FACILITY AREA

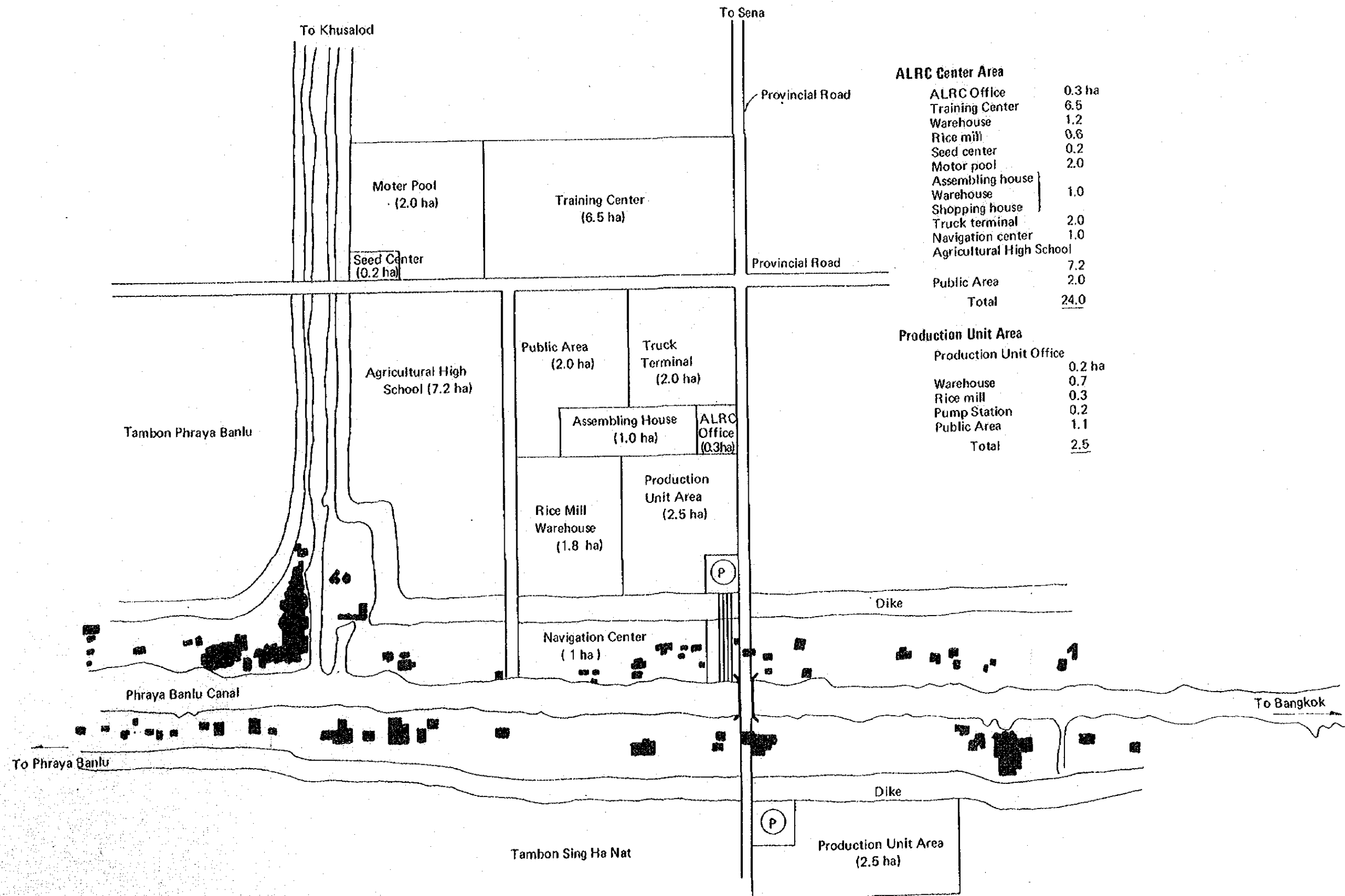
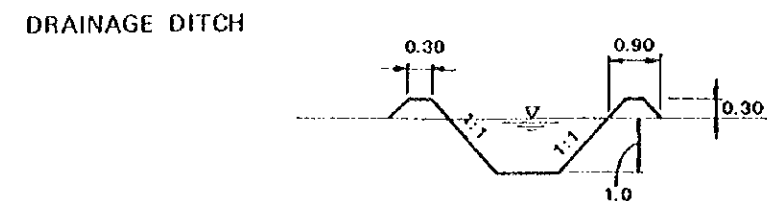
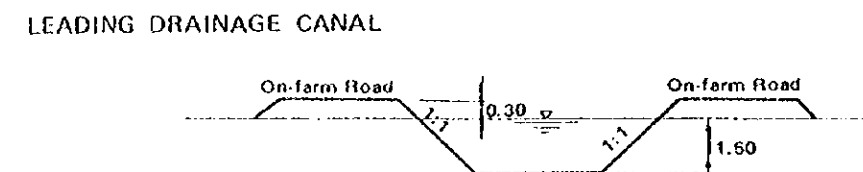
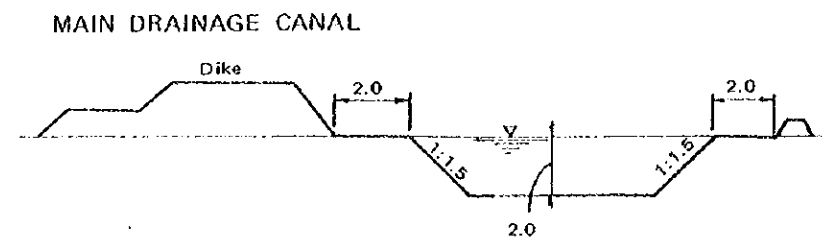
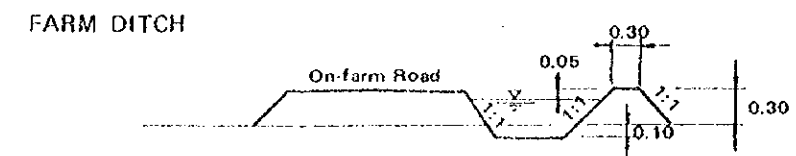
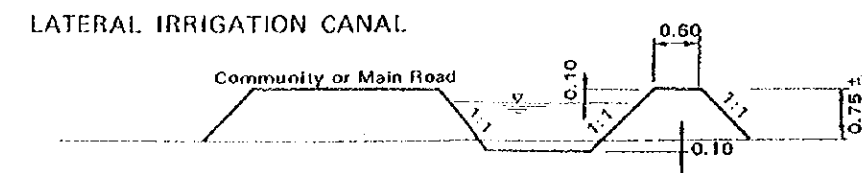
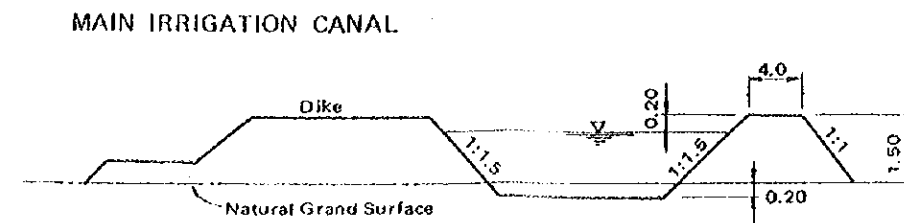
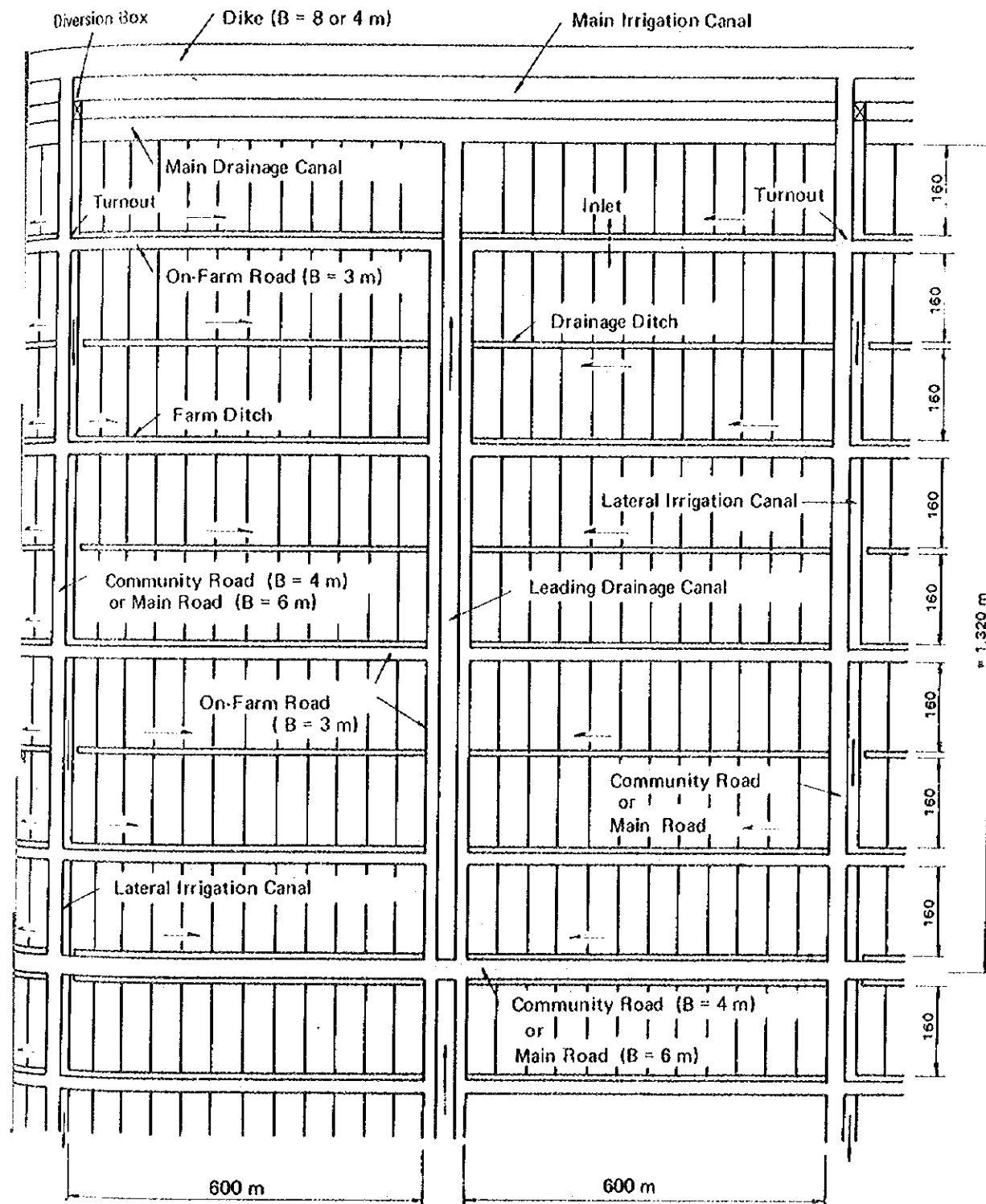


Table A.4-58. Cost for Community Development Plan

Items	Acreage (ha)	Price per Ha.		Remarks
		or House (¥'000)	Cost (¥'000)	
<u>Land Construction</u>				
ALRC Center area	24	20	480	A part of this cost is included in Table A.4-57.
Production Unit Area	10	20	200	- do -
Rural Community Area	375	5	1,875	Reclamation costs are not included.
Fish pond & Livestock Area	60	(42)	(2,500)	Included in the agricultural production cost.
Sub-total	<u>469</u>	-	<u>2,555</u> (5,000)	
<u>House Construction</u>				
Compensation for Transferring	120*	10	1,200	Only transfer cost
New House	940*	40	37,600	For new separate farm house-holds and new comers.
Sub-total	<u>893*</u>		<u>38,800</u>	
Total			<u>41,355</u> (42,655)	

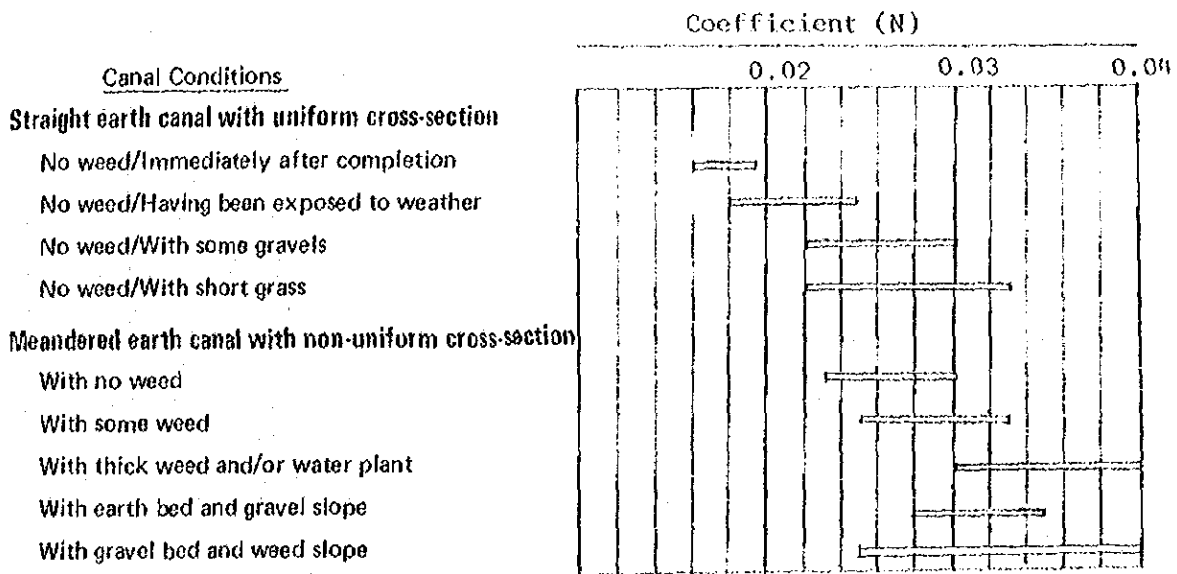
Note: \* houses

FIGURE A.4 - 34 TYPICAL LAYOUT OF CANALS AND ROADS



Roughness Coefficient (N)

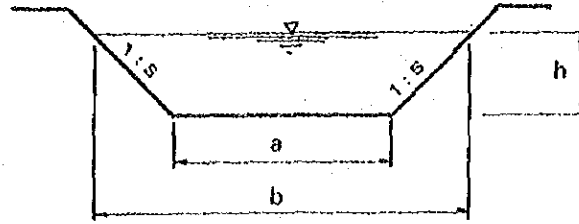
Roughness Coefficient (N) is to be determined taking into full consideration various conditions of the canals, such as roughness of canal surface, vegetation, shape and water velocity, etc. Standard roughness coefficient under various conditions in excavated and dredged canals are shown below.



In the case of natural flow canal where there is much gravel and weeds, the standard value of N is calculated to be approximately 0.035. In this project the canal will be designed on straight line and uniform cross-section in the implementation of land consolidation, and the water velocity would not be so high. Therefore, the N value has been designed as 0.035.

Table A.4-59

Design Discharge for Each Typical Section of Irrigation Canal



Roughness coefficient  
n = 0.035

Main irrigation canal

S = 1.5      b = (a + 4.5)      A =  $\frac{2a + 4.5}{2} \times 1.5 = 1.5a + 3.375$   
 h = 1.50 m      p = (a + 5.41)

a m	A m <sup>2</sup>	P m	R m	R <sup>2/3</sup>	I=1/10,000 I <sup>1/2</sup> =0.01		I=1/7,500 I <sup>1/2</sup> =0.01155		I=1/5,000 I <sup>1/2</sup> =0.01414	
					V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s
2.0	6.375	7.41	0.860	0.904	0.258	1.645	0.298	1.900	0.365	2.327
3.0	7.875	8.41	0.936	0.957	0.273	2.125	0.316	2.499	0.387	3.048
4.0	9.375	9.41	0.996	0.997	0.285	2.672	0.329	3.084	0.403	3.778
5.0	10.875	10.41	1.045	1.030	0.294	3.197	0.340	3.698	0.416	4.524
6.0	12.375	11.41	1.085	1.056	0.302	3.737	0.348	4.307	0.427	5.284
7.0	13.875	12.41	1.118	1.077	0.308	4.274	0.355	4.926	0.435	6.036

S = 1.5      b = (a + 3.0)      A =  $\frac{2a + 3}{2} \times 1.0 = a + 1.5$   
 h = 1.00 m      p = (a + 3.61)

a m	A m <sup>2</sup>	P m	R m	R <sup>2/3</sup>	I=1/10,000 I <sup>1/2</sup> =0.01		I=1/7,500 I <sup>1/2</sup> =0.01155		I=1/5,000 I <sup>1/2</sup> =0.01414	
					V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s
2.0	3.50	5.61	0.624	0.730	0.209	0.730	0.241	0.844	0.295	1.032
3.0	4.50	6.61	0.681	0.774	0.221	0.995	0.255	1.148	0.313	1.407
4.0	5.50	7.61	0.723	0.805	0.230	1.266	0.266	1.463	0.325	1.789
5.0	6.50	8.61	0.755	0.829	0.237	1.540	0.274	1.781	0.335	2.177
6.0	7.50	9.61	0.780	0.848	0.242	1.816	0.280	2.100	0.343	2.569
7.0	8.50	10.61	0.801	0.863	0.247	2.100	0.285	2.423	0.349	2.967

Lateral irrigation canal

S = 1.0 m      b = a + 1.5      A =  $\frac{2a + 1.5}{2} \times 0.75 = 0.75a + 0.563$   
 h = 0.75      p = a + 2.21

a m	A m <sup>2</sup>	P m	R m	R <sup>2/3</sup>	I=1/10,000 I <sup>1/2</sup> =0.01		I=1/7,500 I <sup>1/2</sup> =0.01155		I=1/5,000 I <sup>1/2</sup> =0.01414	
					V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s
1.0	1.313	3.12	0.421	0.562	0.161	0.211	0.185	0.243	0.227	0.298
2.0	2.063	4.12	0.501	0.631	0.180	0.371	0.209	0.431	0.255	0.526
3.0	2.813	5.12	0.549	0.670	0.191	0.537	0.221	0.622	0.271	0.762
4.0	3.563	6.12	0.582	0.697	0.199	0.709	0.230	0.819	0.282	1.005
5.0	4.313	7.12	0.606	0.716	0.205	0.884	0.236	1.018	0.289	1.246
6.0	5.063	8.12	0.624	0.730	0.209	1.058	0.241	1.220	0.295	1.494

FIGURE A.4 - 35. DIAGRAM OF PROPOSED IRRIGATION SYSTEM (BLOCK A)

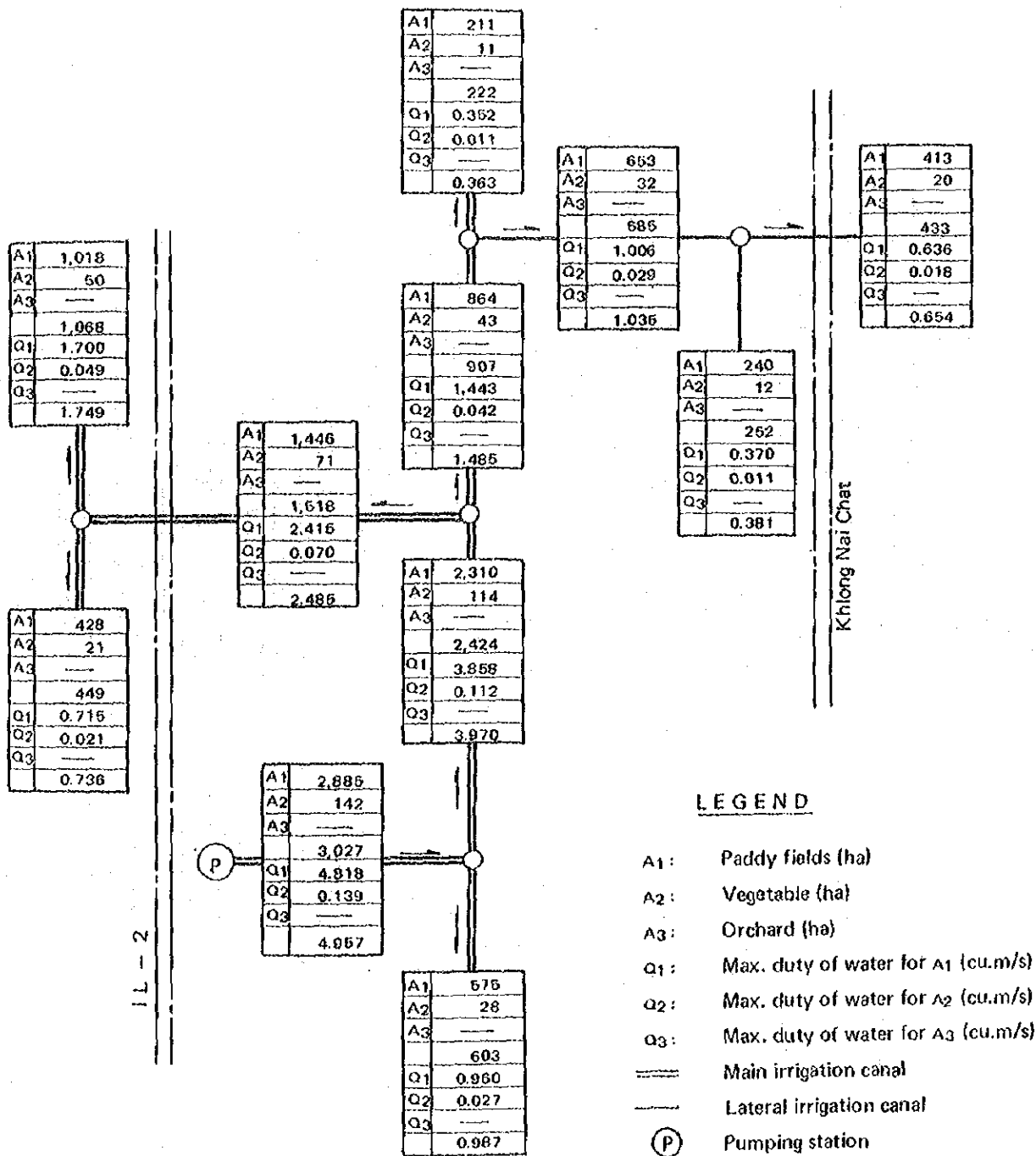


FIGURE A.4 - 36 DIAGRAM OF PROPOSED IRRIGATION SYSTEM (BLOCK B)

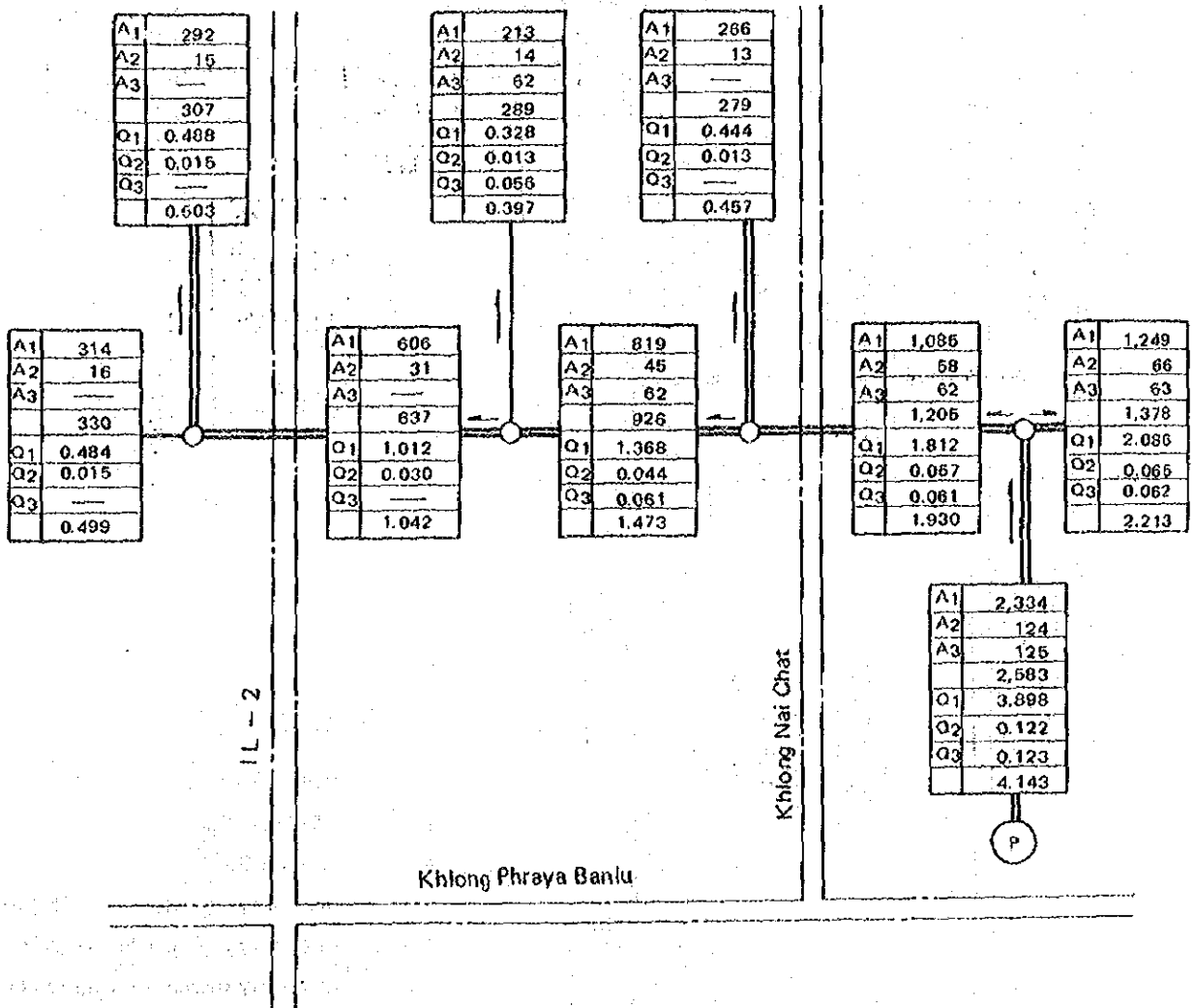
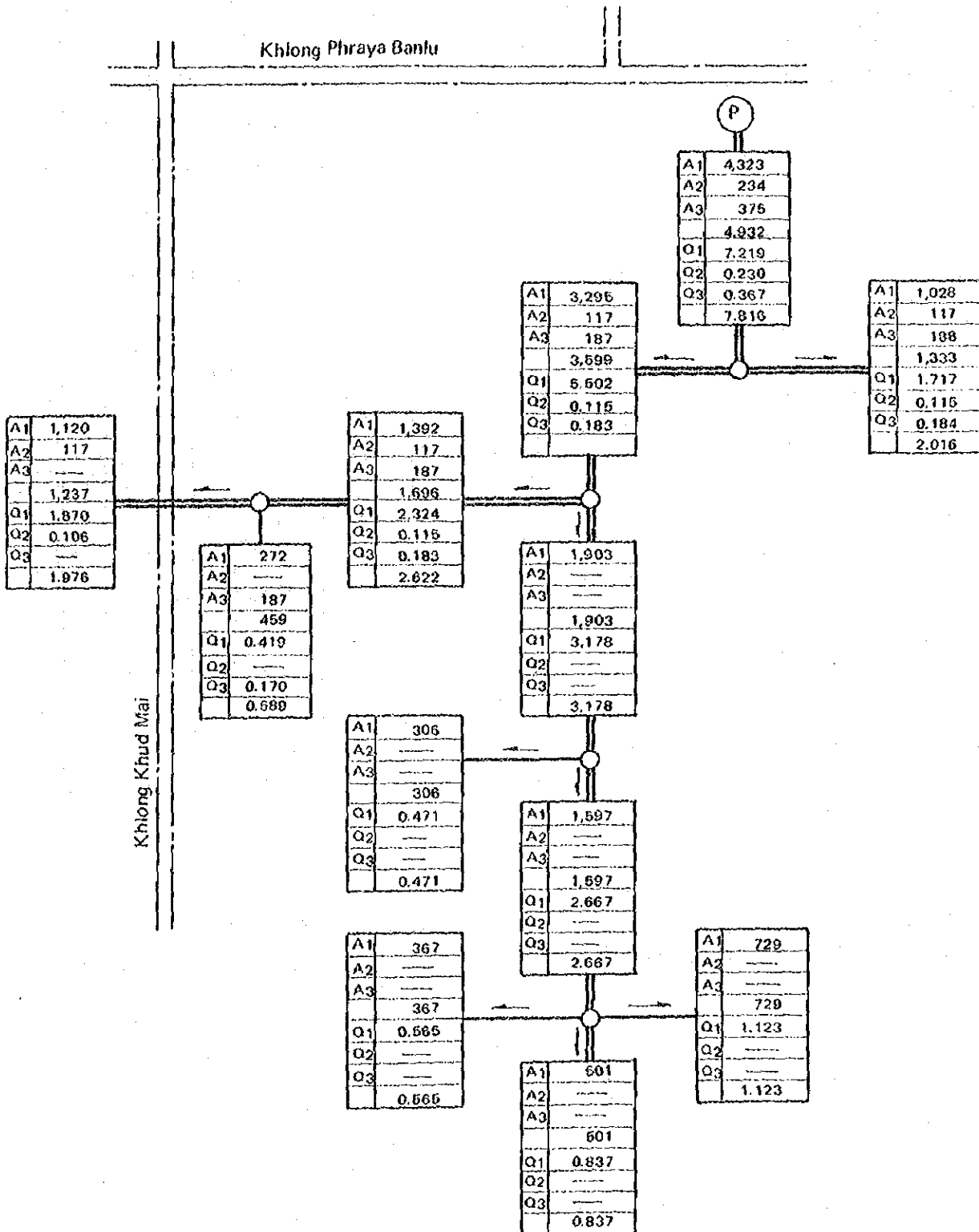




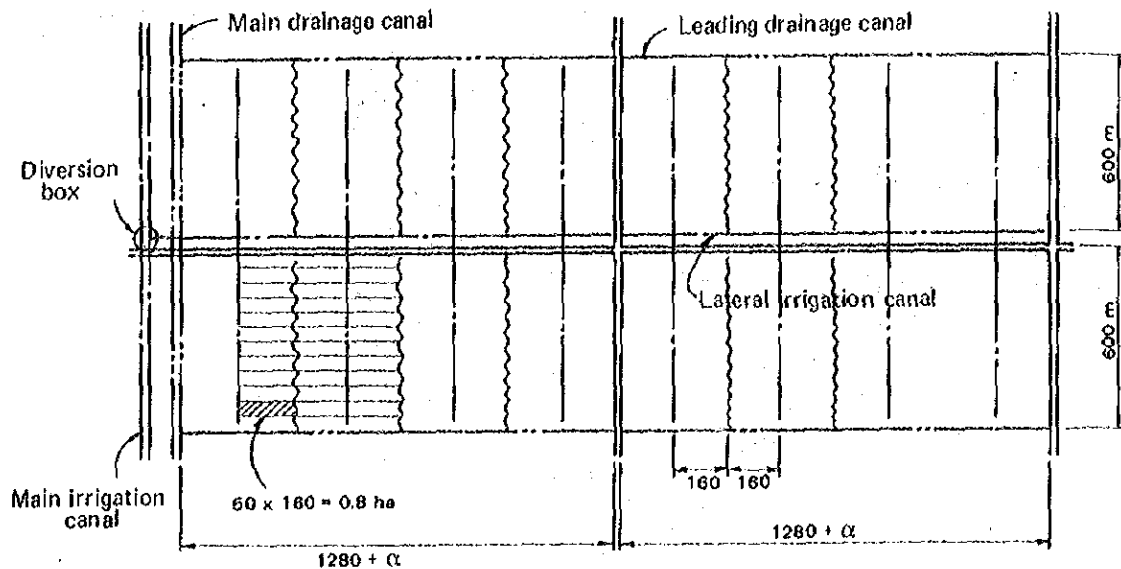
FIGURE A.4 - 37. DIAGRAM OF PROPOSED IRRIGATION SYSTEM (BLOCK C)



Standard Design of Diversion Box

Water distribution from main irrigation canals to lateral canals will be controlled by the diversion box equipped with double gates, in order to execute systematic water distribution. By operating the two gates, orifice gate and diversion gate, constant head (H) between two gates will be kept and these phenomena make it possible to divert the constant irrigation water to meet the design water demand in each irrigation system. Standard design of diversion box is as shown below:

LOCATION OF DIVERSION BOX



1 plot = 0.80 ha

Numbers of plot =  $12 \times 8 \times 4 = 384$

Accordingly,

Standard covering area of a diversion box:

=  $384 \times 0.8 \text{ ha} = 307.2 \text{ ha}$

In lateral irrigation canal, a design discharge Q is:

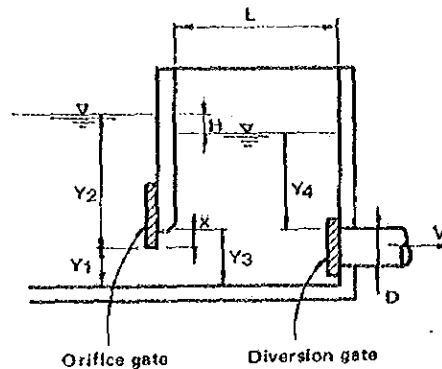
$Q = q \times A = 0.00154 \text{ cu.m/s/ha} \times 307.2 = 0.473 \text{ cu.m/s}$

Diversion box, being a constant head type (double orifice), is designed by using the theory of submerged orifice, namely, an orifice gate shall be installed so that a discharge will be fixed, and then a diversion gate shall be operated in order that the difference of water level  $H$  will be 0.06 m, which is appropriate in standard design to get a required diversion water.

Following is the formula for orifice flow:

$$Q = C.A.\sqrt{2gH}$$

- C: Coefficient, 0.65  
A: Cross-sectional area of orifice (sq.m)  
g: Acceleration of gravity, 9.8 (cu.m/sec<sup>2</sup>)



Accordingly, the required cross-sectional area of orifice is:

$$A = \frac{Q}{C.\sqrt{2gH}} = \frac{0.473}{0.65 \times \sqrt{19.6 \times 0.06}} = \frac{0.473}{0.705} = 0.671 \text{ sq.m}$$

The cross-section of an orifice gate is a regular square, and the length of a side is equal to  $Y_3$ . Notwithstanding, the actual height of cross-sectional area of orifice will be determined so that 80 percent of  $Y_3$  may be maximum.

$$0.8Y_3 \times Y_3 = A \quad 0.8Y_3^2 = 0.671$$

$$Y_3 = \sqrt{\frac{0.671}{0.8}} \approx 0.95 \text{ m}$$

$Y_2$  will be designed to be equal to or more than  $Y_1$  or  $Y_3$ . Inside length of water tank  $L$  will be designed to be more than  $2.75 Y_1$ .

$$L > 2.75Y_1 = 2.75 \times 0.8 \times Y_3 = 2.75 \times 0.8 \times 0.95 = 2.09$$

Accordingly,  $L = 2.1 \text{ m}$

$Y_1$  will be designed to be more than 1.75 times of the depth which is equal to the velocity head at the full flow plus 0.08 m.

$$Y_t > 1.75 \times \left( \frac{V^2}{2g} + 0.08 \right)$$

Where,  $V = 1.2 \text{ m/s}$

$$Y_t > 1.75 (1.2/19.6 + 0.08) = 0.25 \text{ m}$$

Diameter of diversion pipe is:

$$Q = V.A' = V \times \frac{\pi D^2}{4}$$

$A'$ : cross-sectional area of diversion pipe

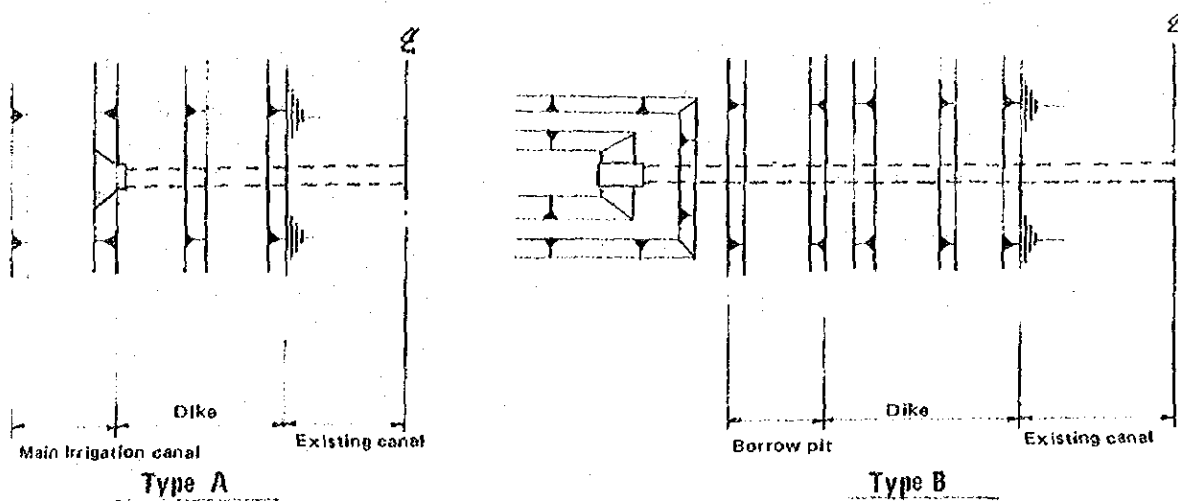
$$D = \sqrt{\frac{Q}{0.7854V}} = \sqrt{\frac{0.473}{0.7854 \times 1.2}} = 0.70 \text{ m}$$

Typical design of diversion box is shown in Drawing No.6 attached to the main report.

Design of Culverts in Irrigation Canal

(1) Design of Culvert in Main Irrigation Canal Crossing Existing Canal

Construction sites and design discharge of each block are shown in the diagram of the proposed irrigation system. As the design discharge is not much, pipe culvert with the combination of following two types has been proposed in this project from the economical point of view.



As the project area is almost flat, the water head should be kept as much as possible. But protection from the sediment load should be considered in the first priority in the design of culvert for maintenance purpose. In this respect, the design will be made so that the velocity may be  $1.0 \text{ m/s}^{\pm}$  for the design discharge and that its diameter may be between  $\phi 600$  and  $\phi 1,500$ , the maximum diameter of ready-made pipes.

Hydraulic dimension and cross-section are summarized below:

Hydraulic Dimension of Culvert

Description	Block A		Block B		Block C
	IL-2	Nai Chat	IL-2	Nai Chat	Khud Mai
Name of existing canal	IL-2	Nai Chat	IL-2	Nai Chat	Khud Mai
Classification of canal	Main	Lateral	Main	Main	Main
Design discharge, Q (cu.m/s)	2.485	0.654	1.042	1.930	1.976
Diameter of culvert, D (mm)	φ1500	φ900	φ1100	φ1500	φ1500
Velocity V (m/s)	1.406	1.028	1.096	1.092	1.118
Velocity head, $v^2/2g$ (m)	0.101	0.054	0.061	0.061	0.064
Combination of Type					
Upstream	A	B	B	B	B
Downstream	A	B	A	A	A
Approximate length of culvert, $l$ (m)	51	89	72	67	72
Loss head, h (m)	0.20	0.20	0.17	0.14	0.15

$$h = (f_e + f_{se} + fl) V^2/2g$$

$f_e$  : Coefficient of inlet loss 0.5

$f_{se}$ : Coefficient of outlet loss 1.0

$f$  : Coefficient of friction loss

<u>D(mm)</u>	<u>f</u>
φ 900	0.0242
φ1100	0.0185
φ1500	0.0123

The culvert will be protected by concrete for safety purpose and at the inlet and outlet of the culvert, reinforced concrete transition will be paved. At the inlet, a gate will be installed to keep the effective water level and also to divert the water. A screen will also be necessary to protect from inflow of materials such as weed, etc.

At the foundation of culvert, concrete pile will be driven for safety purpose. Typical layout of the culvert is shown in the Drawing No.7 attached to the main report.

(2) Road Crossing for Irrigation Canal

Road crossing for irrigation canal will be made by pipe culvert. Considering the safety against the load of vehicles running on the road, the depth from ground surface of road to the top of culvert should be kept at a minimum of 0.6 (standard depth: 1.0 m). To keep such depth, irrigation canal bed should be lowered because of its shallow water depth. At the inlet and outlet of the culvert, a transition having a gentle bed slope of 1:5 should be provided to eliminate a sudden variation of velocity and a sediment load, for which a diameter of culvert will be designed so that the discharge velocity will be 1.0 m/s at the design discharge. To give protection to a slope of road and irrigation canal, gravels should be paved at the inlet and outlet of the culvert.

Road Crossing for Main Irrigation Canal

In case of main irrigation canal, road crossing will be made by using two pipe culverts, because if only one culvert is used, the diameter should be bigger due to much water discharge in the canal and hence lowered. At a point of the main irrigation canal crossing the main road, a gate will be installed so that it can play a role to keep the effective water level. Assuming the irrigation area is 1,000 ha as standard, loss of head is as follows:

Design discharge:  $Q = q \text{ cu.m/s/ha} \times A \text{ ha}$   
 $= 0.00167 \times 1,000 = 1.67 \text{ cu.m/s}$

Diameter of culvert:  $D = 1,000 \text{ mm} \times 2$

Discharge velocity for each culvert:

$$V = 1.067 \text{ m/s}$$

Loss head:

$$h = (f_c + f_{se} + f_l) \frac{V^2}{2g}$$

$$= (0.5 + 1.0 + 0.021(4 \sim 8m)) \frac{1.067^2}{19.6} = 0.08 \text{ m}$$

Road Crossing for lateral Irrigation Canal

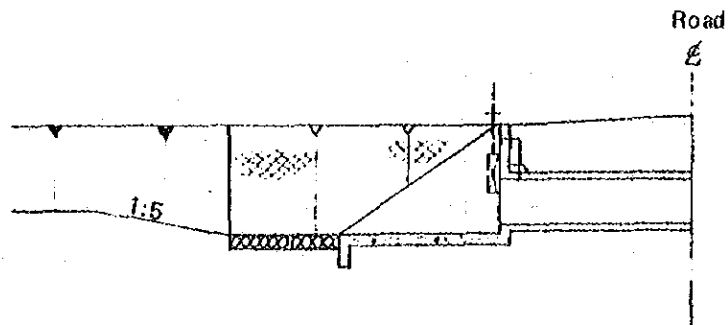
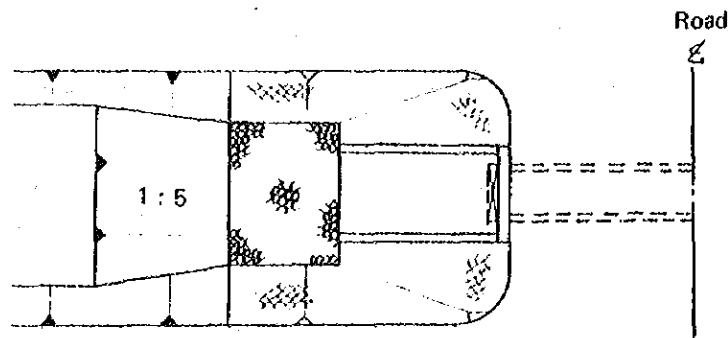
In case of lateral canal, a gate will be installed at every crossing point having each interval of 1.20 km and such gate will serve as a check gate at the same time. Assuming that one standard irrigation area is 150 ha and the diameter of culvert is  $\phi 500$ , the loss of head will be 0.12 m which satisfies every hydraulic dimension, as shown below.

Design discharge:  $Q = q \text{ cu.m/s/ha} \times A \text{ ha}$   
 $= 0.00154 \times 150 = 0.231 \text{ cu.m/s}$

Diameter of culvert:  $D = 500 \text{ m}$

Discharge velocity:  $V = 1.176 \text{ m/s}$

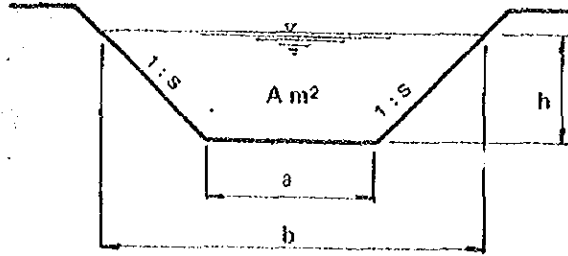
Loss head:  $h = (f_e + f_{se} + f_l) \frac{V^2}{2g}$   
 $= (0.5 + 1.0 + 0.053 (4 \sim 8\text{m})) \frac{1.176^2}{19.6} = 0.12 \text{ m}$



Details are shown in the Drawing No.8 attached to the main report.



Table A.4-60 Design Discharge for Each Typical Section of Drainage Canal



Main drainage canal  $S = 1.5$   $b = (a + 6) \text{ m}$   
 $h = 2.0 \text{ m}$   $A = \frac{2.0}{2}(2a + 6) = 2a + 6$   
 $P = a + 7.21$

a m	A m <sup>2</sup>	P m	R m	R <sup>2/3</sup>	I=1/10,000		I=1/5,000		I=1/7,500	
					V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s
2.0	10.0	9.21	1.086	1.057	0.302	3.020	0.427	4.270	0.349	3.490
4.0	14.0	11.21	1.249	1.160	0.331	4.640	0.469	6.566	0.383	5.362
6.0	18.0	13.21	1.363	1.229	0.351	6.321	0.497	8.946	0.406	7.308
8.0	22.0	15.21	1.446	1.279	0.365	8.039	0.517	11.374	0.422	9.284
10.0	26.0	17.21	1.511	1.317	0.376	9.783	0.532	13.832	0.435	11.310

Lateral drainage canal  $S = 1.0$   $b = (a + 3) \text{ m}$   
 $h = 1.50$   $A = \frac{1.50}{2}(2a + 3) = 1.5a + 2.25$   
 $P = (a + 4.24) \text{ m}$

a m	A m <sup>2</sup>	P m	R m	R <sup>2/3</sup>	I=1/10,000 I <sup>1/2</sup> =0.01		I=1/5,000 I <sup>1/2</sup> =0.01414		I=1/7,500 I <sup>1/2</sup> =0.01155	
					V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s	V m/s	Q m <sup>3</sup> /s
2.0	5.25	6.24	0.841	0.891	0.255	1.337	0.360	1.890	0.294	1.544
4.0	8.25	8.24	1.001	1.001	0.286	2.360	0.404	3.333	0.330	2.723
6.0	11.25	10.24	1.099	1.065	0.304	3.423	0.430	4.838	0.351	3.948
8.0	14.25	12.24	1.164	1.107	0.316	4.507	0.447	6.370	0.365	5.201

FIGURE A.4 - 38 DIAGRAM OF PROPOSED DRAINAGE SYSTEMS (BLOCK A)

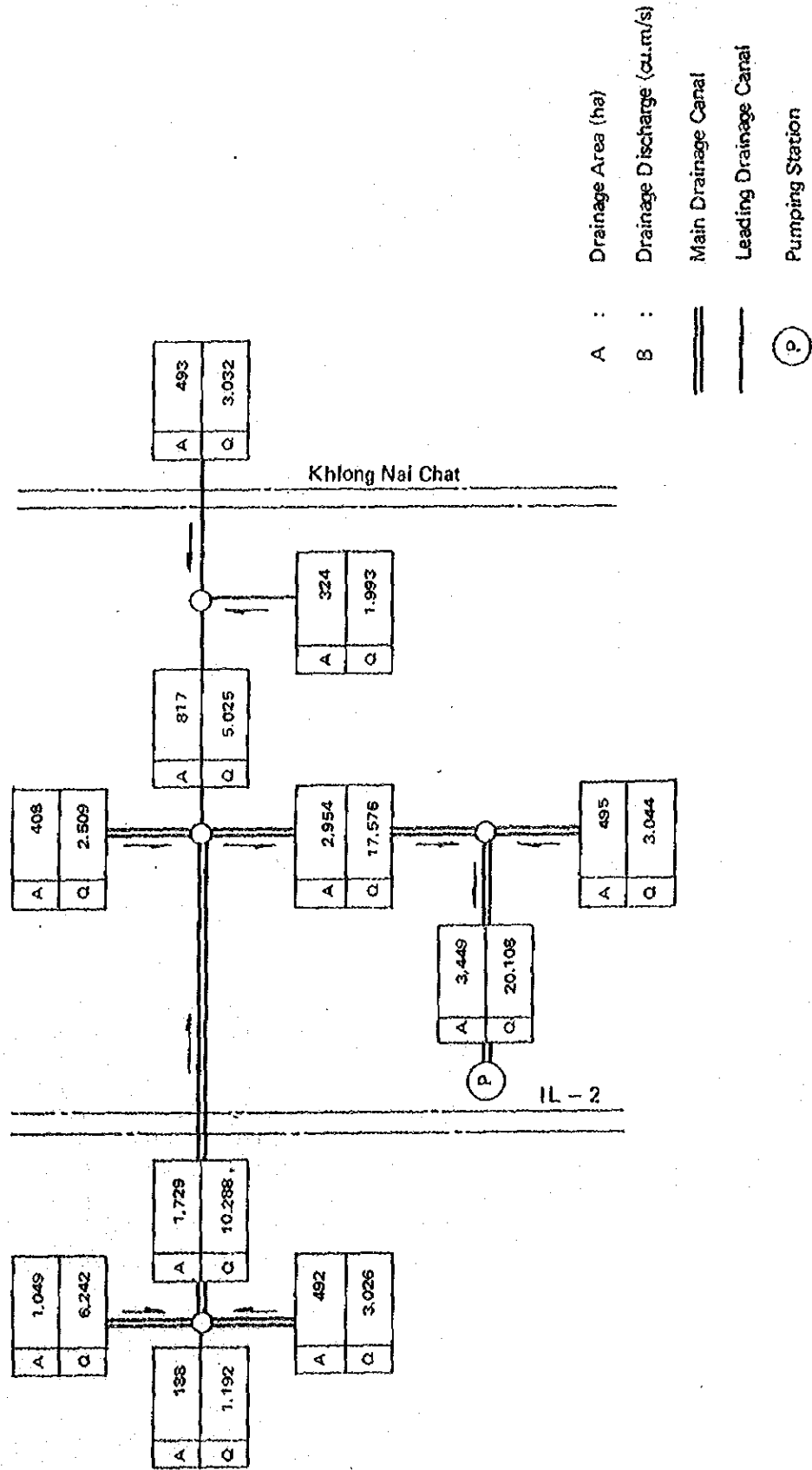


FIGURE A.4 - 39 DIAGRAM OF PROPOSED DRAINAGE SYSTEMS (BLOCK B)

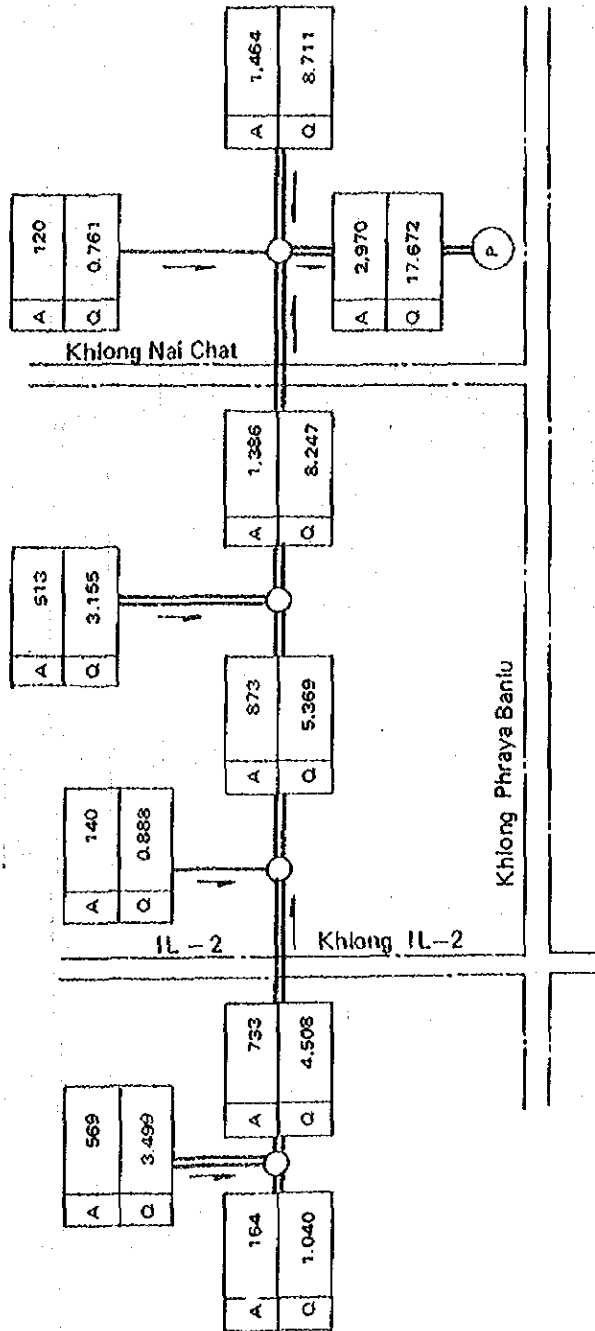
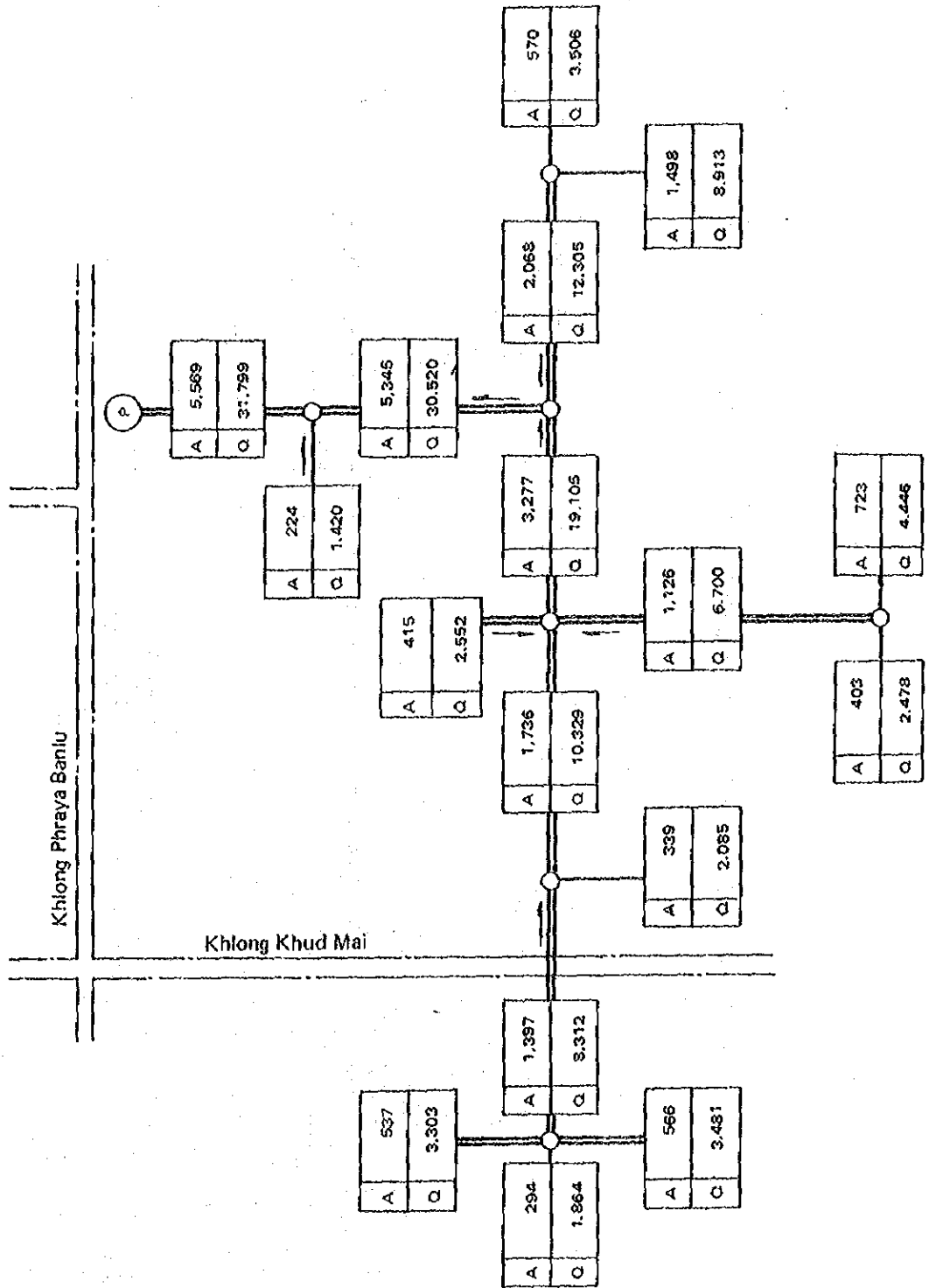


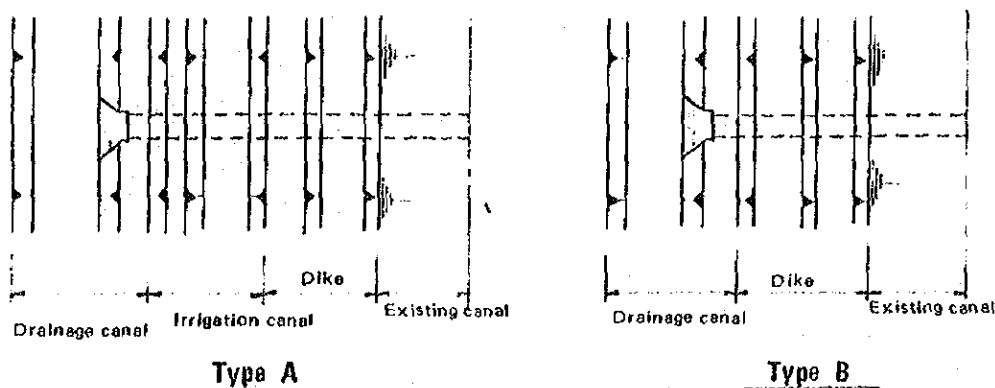
FIGURE A.4 - 40 DIAGRAM OF PROPOSED DRAINAGE SYSTEM (BLOCK C)



### Design of Culverts in Drainage Canal

#### (1) Design of Culvert in Main Drainage Canal Crossing Existing Canal

Construction sites and design discharge of each block are shown in the diagram of proposed drainage system. Because of much discharge volume of the main drainage canal, the culvert will be designed by reinforced concrete culvert with the combination of two types as follows.



For the protection from the sediment load in the culvert, the design will be made so that the discharge velocity may be  $1.0 \text{ m/s} \pm$  against the design discharge, and that the width of cross section of culvert may be more than 1.20 m.

The hydraulic dimension and cross section are summarized below.

Hydraulic Dimension of Culvert

Description	Block A		Block B		Block C
	Il-2	Nai Chat	Il-2	Nai Chat	Khud Mai
Name of existing canal	Il-2	Nai Chat	Il-2	Nai Chat	Khud Mai
Classification of canal	Main	Leading	Main	Main	Main
Design discharge, Q(cu.m/s)	10.288	3.032	4.508	8.247	8.312
Cross-section of culvert					
Width x Height (m)	3.0x3.0	1.7x1.7	2.0x2.0	2.7x2.7	2.7x2.7
Velocity, V (m/sec)	1.143	1.049	1.127	1.131	1.140
Velocity head, $V^2/2g$ (m)	0.067	0.056	0.065	0.065	0.066
Combination of type					
Upstream	A	B	B	A	A
Downstream	A	B	B	B	B
Approximate length of culvert, $L$ (m)	88	53	58	68	73
Loss head, $h$ (m)	0.13	0.12	0.13	0.13	0.14

$$h = (f_e + f_{se}) \frac{V^2}{2g} + I \cdot L$$

$f_e$  : Coefficient of inlet loss 0.5

$f_{se}$  : Coefficient of outlet loss 1.0

$I$  : Hydraulic gradient

$$I = \left( \frac{N^2 V^2}{R^{4/3}} \right)$$

$N$  : Coefficient of roughness 0.015

$R$  : Hydraulic mean depth

The culvert will be made by reinforced ready-mixed concrete, and the cross-section will be regular square. At the inlet and outlet the transition of reinforced concrete will be provided. A screen will be set for the protection from materials such as weed etc.

At the foundation at culvert piles will be driven for safety purpose.

Typical layout is shown in the Drawing No.9 attached to the main report.

(2) Road Crossing for Drainage Canal

Road crossing for drainage canal will also be made by pipe culvert, as the bed of drainage canal is deeper than that of irrigation canal, a sufficient depth from the road surface can be obtained without lowering the canal bed at the inlet and outlet of the culvert. But from the nature of drainage canal the diameter of culvert will be determined so that the discharge velocity may be 1.0 m/s against the design discharge, to prevent from the sediment load.

At the inlet and outlet of culvert a gravel will be paved for the protection of slope of canal.

Road Crossing for Main Drainage Canal

Road crossing of main drainage canal will be made by using two or more pipes having the maximum diameter of  $\phi 1,500$ , as the discharge in drainage canal is more than that in irrigation canal, and the drainage canal has a sufficient cross-sectional area because of its borrow pit nature.

Assuming that one standar drainage area is 1,000 ha, the loss head will be 0.10 m as shown below;

Desing discharge:  $Q = q \text{ cu.m/s/ha} \times A \text{ ha}$   
 $= 0.00615 \times 1,000 = 6.15 \text{ cu.m/s}$

Diameter of culvert:  $D = 1,500 \text{ mm} \times 3 \text{ culverts}$

Discharge velocity per one culvert:

$$V = 1.160 \text{ m/s}$$

Loss head:  $h = (f_e + f_{se} + f_l) \frac{V^2}{2g}$   
 $= \{0.5 + 1.0 + 0.0123 (4 \sim 8\text{m})\} \frac{1.160^2}{19.6} = 0.10 \text{ m}$

Road Crossing for Leading Drainage Canal

Assuming that one standard drainage area is 150 ha, the loss head will be 0.12 m as shown below:

Design discharge:  $Q = q \text{ cu.m/s/ha} \times A \text{ ha}$   
 $= 0.00634 \times 150 = 0.951 \text{ cu.m/s}$

Diameter of culvert:  $D = 1,000 \text{ mm}$

Discharge velocity:  $V = 1.210 \text{ m/s}$

loss head: 
$$h = (f_e + f_{se} + f_l) \frac{v^2}{2g}$$
$$= \{0.5 + 1.0 + 0.021 (4 \sim 8\text{m})\} \times \frac{1.210^2}{19.6} = 0.12 \text{ m}$$

Typical layout is shown in Drawing No.10 attached to the main report.



Required Units of Pumps

Maximum discharge to be required for irrigation and drainage in each block are as follows:

Block	Drainage		Area		Irrigation		
	Area (ha)	Discharge (cu.m/s)	Paddy (ha)	Upland (ha)	for Paddy (cu.m/s)	for Upland (cu.m/s)	Total (cu.m/s)
A	3,449	11.97	2,885	142	4.818	0.139	4.957
B	2,970	10.31	2,334	294	3.898	0.245	4.143
C	5,569	19.32	4,323	609	7.219	0.597	7.816

Note:

Unit drainage discharge  $q = 0.00347$  cu.m/sec/ha (1.25 mm/hour)

Unit irrigation water requirement

Paddy  $q = 0.00167$  cu.m/sec/ha

Upland  $q = 0.00098$  cu.m/sec/ha

Discharge for drainage is bigger than that for irrigation, therefore the units of pumps and bore diameter should be decided based on the quantity of drainage discharge. The units of pump should also be more than two units taking trouble of pumps into consideration.

The relations between discharge and bore diameter are as follows:

Bore, D (mm)	Discharge (cu.m/min)
1,000	115 - 150
1,200	150 - 200
1,350	200 - 255
1,500	255 - 325
1,650	325 - 400
1,800	400 - 480
2,000	480 - 600

The required head of pumps is computed by the following formula:

Total Head = (Delivery water level - Suction water level) + Pipe Loss

Total head for both irrigation and drainage are estimated as follows:

	Delivery Water Level (m)	Suction Water Level (m)	Pipe Loss	Total Head (m)
Drainage	WL3.20 <sup>±</sup>	WL1.50 <sup>±</sup>	1.00	2.70
Irrigation	WL3.00 <sup>±</sup>	WL0.10 <sup>±</sup>	0.70	3.60

If pumps is planned to meet the discharge in case of drainage, total head in case of irrigation will be of  $3.60/2.70 = 130\%$  and the discharge will be about 70-80 % owing to the characteristic of pump.

The following table indicates the lifting capacity for irrigation and drainage in case of various units of pumps ranging from two units to five units.

Lifting Capacity of Pumps

Block	Unit	Drainage Lifting capacity per unit		Diameter x unit	Irrigation Lifting capacity per unit	
		Q (m <sup>3</sup> /s)	Q (m <sup>3</sup> /min)		Q (m <sup>3</sup> /s)	Q (m <sup>3</sup> /min)
A	2	5.98	359	φ1,650 x 2	4.19	251
	3	3.99	239	φ1,350 x 3	2.79	167
	4	2.99	179	φ1,200 x 4	2.09	125
	5	2.39	143	φ1,000 x 5	1.67	100
B	2	5.15	309	φ1,500 x 2	3.61	212
	3	3.44	206	φ1,350 x 3	2.41	145
	4	2.58	155	φ1,200 x 4	1.81	109
	5	2.06	124	φ1,000 x 5	1.44	86
C	2	9.66	580	φ2,000 x 2	6.76	406
	3	6.44	386	φ1,650 x 3	4.51	271
	4	4.83	290	φ1,500 x 4	3.38	203
	5	3.86	232	φ1,350 x 5	2.70	162
	5	3.22	193	φ1,200 x 6	2.25	135

Annual mean pumping operation hours in the normal year (return period 2-years) are shown in Figure A.4-41 to Figure A.4-43, when above pumps are operated.

FIGURE A.4 - 41 OPERATION HOURS OF PUMPS IN NORMAL YEAR (Block A)

Month	Peak	Jan.		Feb.		Mar.		Apr.		May		Jun.		Jul.		Aug.		Sep.		Oct.		Nov.		Dec.	
		I	II	I	II	I	II	I	II	I	II	I	II	I	II	I	II	I	II	I	II	I	II	I	II
Units 1/ 2/ 3/		1.09	3.35	4.57	4.30	3.54	2.96	3.79	2.39	(1.29)	(0.81)	(2.03)	(1.33)	(1.12)	2.43	2.37	(1.09)	(0.89)	(3.02)	1.22	0.67	0.69	1.00	0.07	0.09
	1	6.2	19.2			20.3	17.0	21.7	13.7	5.2	3.3	8.1	5.3	4.5	12.9	13.6	4.4	3.6	12.1	7.0	3.8	3.95	5.7	0.4	0.5
2			2.2	0.6																					
φ 1,350 x 3 units	1	9.4								20.6	7.8	12.2	8.0	6.7	20.9	20.4	6.6	5.4	18.2	10.5	5.8	5.9	8.6	0.6	0.8
	2		4.8	15.3	12.0	6.5	1.5	8.6																	
	3																								
φ 1,200 x 4 units	1	12.5								10.4	6.5	16.3	10.7	9.0		8.7	7.1			14.0	7.7	7.9	11.5	0.8	1.0
	2		14.5			16.7	10.0	19.5	3.4					3.9	3.2				0.2						
	3			4.5	1.4																				
	4																								
φ 1,000 x 5 units	1	15.7								13.0	8.1	20.4	13.4	11.2		10.9	8.9			17.5	9.6	9.9	14.4	1.0	7.3
	2														10.9	10.0			6.3						
	3		0.1	17.7	13.8	2.9	6.5																		
	4																								
	5																								

Note: 1/ : Pumpage of one unit m<sup>3</sup>/s ( ) : Indicates the drainage discharge  
 2/ : Units  
 3/ : The necessary water requirements m<sup>3</sup>/s (See Table A.4 - 61)  
 4/ : Pumping operation hours

FIGURE A.4 - 42 OPERATION HOURS OF PUMPS IN NORMAL YEAR (Block B)

Month	Peak	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
		1	1	1	1	1	1	1	1	1	1	1	1
Units 1/2/3		0.98	3.30	2.99	3.19	1.99	3.19	2.02	1.92	(0.78)(2.59)	0.99	0.54	0.12
		18.7	23.7	19.9	21.2	13.2	3.2	4.5	12.8	4.4	3.6	3.6	3.9
Φ 1,500 x 2 units	1	6.5	23.7	19.9	21.2	13.2	3.2	4.5	12.8	4.4	3.6	3.6	3.9
	2	1.3											0.8
Φ 1,350 x 3 units	1	9.8			19.9	7.8	12.2	6.8	19.1	6.6	5.4	5.9	9.0
	2	4.0	13.8	11.6	5.8	0.8	7.8						
	3												
Φ 1,200 x 4 units	1	13.0			19.9	10.4	16.3	9.0	8.8	7.3	13.1	7.2	7.8
	2	13.3	23.3	15.6	9.0	18.3	2.4	2.8	1.5	0.1			1.6
	3	2.4											
Φ 1,000 x 5 units	1	16.3			19.9	13.0	20.4	11.3	11.1	9.1	16.5	9.0	9.8
	2	22.8			10.2	9.1	9.7	8.0	6.2				2.0
	3		15.3	11.5	1.8	5.2							
	4												
	5												

Note: 1/ : Pumpage of one unit m<sup>3</sup>/s  
 2/ : Units  
 3/ : The necessary water requirements m<sup>3</sup>/s (See Table A.4 - 61)  
 4/ : Pumping operation hours

FIGURE A.4 - 43 OPERATION HOURS OF PUMPS IN NORMAL YEAR (Block C)

Month	Peak	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.											
		Units	Units	Units	Units	Units	Units	Units	Units	Units	Units	Units	Units											
φ 2,000 x 2 units 9.76 m <sup>3</sup> /s (9.66)	1	1.90	7.16	5.66	6.05	3.76	(2.07)	(1.30)	(3.29)	(2.14)	(1.81)	3.80	3.55	(1.75)	(1.43)	(4.87)	1.83	1.00	1.13	1.77	0.30	0.40		
	2	6.7	18.7	23.8	20.1	16.7	21.5	13.3	5.1	3.2	9.2	5.3	4.5	13.5	12.6	4.3	3.6	12.1	6.5	3.6	4.0	6.3	1.1	1.4
φ 1,650 x 3 units (6.44) 4.61 m <sup>3</sup> /s	1	10.1							20.0	7.7	4.8	12.3	3.0	18.9	6.5	5.3	18.1	9.7	5.3	16.0	9.4	1.6	2.1	
	2	4.1	14.1	11.7	6.1	7.1	8.2																	
	3																							
φ 1,500 x 4 units 3.38 (3.83) m <sup>3</sup> /s	1	13.5								10.3	6.5	15.3	10.6	9.0			7.1	13.0	7.1	9.0	12.6	2.1	2.8	
	2	12.5	23.6	16.2	9.4	19.0	2.7					3.0	1.2				0.1							
	3																							
	4																							
φ 1,350 x 5 units 2.70 (3.86) m <sup>3</sup> /s	1	16.9								12.9	8.1	20.5	13.3	11.3			8.9	16.3	8.9	10.0	15.7	2.7	3.6	
	2	22.9															9.8	7.5	16.3					
	3		15.6	17.6	2.3	5.8																		
	4																							
	5																							

Note: 1/ : Pumpage of one unit m<sup>3</sup>/s ( ) : Indicates the drainage discharge  
 2/ : Units  
 3/ : The necessary water requirements m<sup>3</sup>/s (See Table A.4 - 61)  
 4/ : Pumping operation hours

Table A.4-61 Design Capacity for Irrigation and Drainage (Normal year)

(Unit: cu.m/s)

	<u>Block A</u>	<u>Block B</u>	<u>Block C</u>
Jan. I	1.09	0.98	1.90
II	3.35	2.81	5.28
Feb. I	4.57	3.80	7.16
II	4.30	3.57	6.71
Mar. I	3.54	2.99	5.66
II	2.96	2.49	4.71
Apr. I	3.79	3.19	6.05
II	2.39	1.99	3.76
May I	(1.29)	(1.12)	(2.07)
II	(0.81)	(0.70)	(1.30)
Jun. I	(2.03)	(1.75)	(3.29)
II	(1.33)	(1.14)	(2.14)
Jul. I	(1.12)	(0.97)	(1.81)
II	2.43	2.02	3.80
Aug. I	2.37	1.92	3.55
II	(1.09)	(0.95)	(1.75)
Sep. I	(0.89)	(0.78)	(1.43)
II	(3.02)	(2.59)	(4.87)
Oct. I	1.22	0.99	1.83
II	0.67	0.54	1.00
Nov. I	0.69	0.59	1.13
II	1.00	0.90	1.77
Dec. I	0.07	0.12	0.30
II	0.09	0.12	0.40

Note: ( ): indicate the design capacity for drainage

Table A.4-62 Irrigation Water Requirement and Drainage Discharge  
(Normal year)

Irrigation Water Requirement										(Unit: cu.m/s)
	Block A			Block B			Block C			Total
	Paddy	Upland	Total	Paddy	Upland	Total	Paddy	Upland	Total	
	2,885ha	142 ha		2,334ha	249 ha		4,323ha	609 ha		
Q <sub>1</sub>	Q <sub>2</sub>	Q	Q <sub>1</sub>	Q <sub>2</sub>	Q	Q <sub>1</sub>	Q <sub>2</sub>	Q		
Jan. I	1.00	0.09	1.09	0.81	0.17	0.98	1.50	0.40	1.90	
II	3.26	0.09	3.35	2.64	0.17	2.81	4.88	0.40	5.28	
Feb. I	4.46	0.11	4.57	3.61	0.19	3.80	6.69	0.47	7.16	
II	4.20	0.10	4.30	3.40	0.17	3.57	6.29	0.42	6.71	
Mar. I	3.42	0.12	3.54	2.77	0.22	2.99	5.13	0.53	5.66	
II	2.86	0.10	2.96	2.31	0.18	2.49	4.28	0.43	4.71	
Apr. I	3.66	0.13	3.79	2.96	0.23	3.19	5.49	0.56	6.05	
II	2.32	0.07	2.39	1.87	0.12	1.99	3.47	0.29	3.76	
May I	-	-	-	-	-	-	-	-	-	
II	-	0.03	0.03	-	0.05	0.05	-	0.13	0.13	
Jun. I	-	-	-	-	-	-	-	-	-	
II	-	0.02	0.02	-	0.04	0.04	-	0.04	0.04	
Jul. I	-	-	-	-	-	-	-	-	-	
II	2.38	0.05	2.43	1.93	0.09	2.02	3.57	0.23	3.80	
Aug. I	2.37	-	2.37	1.92	-	1.92	3.55	-	3.55	
II	0.06	-	0.06	0.05	-	0.05	0.09	-	0.09	
Sep. I	0.05	-	0.05	0.04	-	0.04	0.07	-	0.07	
II	-	-	-	-	-	-	-	-	-	
Oct. I	1.22	-	1.22	0.99	-	0.99	1.83	-	1.83	
II	0.67	-	0.67	0.54	-	0.54	1.00	-	1.00	
Nov. I	0.66	0.03	0.69	0.53	0.06	0.59	0.99	0.14	1.13	
II	0.90	0.10	1.00	0.73	0.17	0.90	1.35	0.42	1.77	
Dec. I	-	0.07	0.07	-	0.12	0.12	-	0.30	0.30	
II	-	0.09	0.09	-	0.16	0.12	-	0.40	0.40	

Drainage Discharge (Cont'd)

	Block A			Block B			Block C		
	Cultivat-	Others	Total	Cultivat-	Others	Total	Cultivat-	Others	Total
	ed Area 3,027ha	422ha		ed Area 2,583ha	387ha		ed Area 4,932ha	637ha	
May I	1.05	0.24	1.29	0.90	0.22	1.12	1.71	0.36	2.07
II	0.67	0.14	0.81	0.57	0.13	0.70	1.09	0.21	1.30
Jun. I	1.85	0.18	2.03	1.58	0.17	1.75	3.01	0.28	3.29
II	1.21	0.12	1.33	1.03	0.11	1.14	1.96	0.18	2.14
Jul. I	0.97	0.15	1.12	0.83	0.14	0.97	1.58	0.23	1.81
II	-	0.07	0.07	-	0.06	0.06	-	0.11	0.11
Aug. I	-	0.15	0.15	-	0.14	0.14	-	0.22	0.22
II	0.82	0.27	1.09	0.70	0.25	0.95	1.39	0.41	1.75
Sep. I	0.67	0.22	0.89	0.57	0.21	0.78	1.09	0.34	1.43
II	2.61	0.41	3.02	2.22	0.37	2.59	4.25	0.62	4.87
Oct. I	-	0.15	0.15	-	0.14	0.14	-	0.23	0.23
II	0.06	0.17	0.23	0.05	0.16	0.21	0.10	0.26	0.36

Subsequent paragraph deals with the selection of most desirable units of pumps from view point of cost and operation and maintenance,

Cost:

## Running cost;

Electric power charge in Thailand is only the consumed electric power charge, and it is not necessary to pay the contract charge.

## Initial cost;

The total bore diameter of pumps gradually increases in accordance with the increase of number of pumps. Therefore, construction cost including civil works and building gradually increases as units of pumps increases.

From the result, it is found out that less units of pumps are more effective from economic view point.



Operation and Maintenance:

The pumps, which will be used for both purposes of irrigation and drainage, are operated throughout a year in the Project. All units of pump will be operated when the drainage discharge is at peak, but in the case of operation for irrigation, it is desirable that each station has spare units of one or two from operation point of view, because the durability of pumps gets more longer by alternate using of the pumps. If the plan having two units is adopted, there would be no spare units, when the water requirements is at peak, and the bore diameter becomes large and the farmers in the Project Area have no experienced in operating such pumps with the large scale bore. Therefore, the plan having two units is not recommendable. Also, in case that the number of units for each block is more than five units in ordre to adopt the smaller bore diameter than  $\phi 1,000$  mm, it will not be efficient from economic and operation and maintenance points of view.

As mentioned above, three units of pump is the most effective plan considering the economy, safty for operation, the durability and the operation and maintenance. Therefore, plan of the three units installed will be adopted in each block, and following table shows the features of pumps in each block.

<u>Block</u>	<u>Delivery discharge per unit</u>	<u>Diameter</u>	<u>Units</u>
A	3.99 cu.m/sec (239 cu.m/min)	$\phi 1,350$ mm	3
B	3.44 " (206 " )	$\phi 1,350$ "	3
C	6.44 " (386 " )	$\phi 1,650$ "	3

Selection of Prime Mover

Disel engine

Heavy oil and lubricating oil will be used for fuel, and the fuel consumptions are as follows:

	<u>Heavy oil</u>	<u>Lubricating oil</u>
Fuel consumption (g/ps/hr)	190	5
Price (₪/ℓ)	2.6	18.0
Specific gravity	0.9	0.9

<u>Output (ps)</u>	<u>Rate of fuel consumption (g/ps/hr.)</u>
100 - 200	200
200 - 500	190
more 500	180

<u>Block</u>	<u>Output per unit (ps)</u>	<u>Units</u>	<u>Fuel consumption per hour (ℓ/hr/unit)</u>		<u>Operation costs for hour per unit (₪/hr/unit)</u>		
			<u>Heavy oil</u>	<u>Lubricating oil</u>	<u>Heavy oil</u>	<u>Lubricating oil</u>	<u>Total</u>
A	240	3	50.7	1.3	131.8	23.4	155.2
B	200	3	42.2	1.1	109.7	19.8	129.5
C	440	3	92.9	2.4	241.5	43.2	284.7
<u>Total</u>					<u>494.0</u>	<u>86.4</u>	<u>569.4</u>

Annual operating hours in the normal year are as follows.

<u>Block</u>	<u>Total annual operating hours (hr)</u>	<u>Annual operating hours per unit (hr) l/</u>
A	5,226	1,742
B	5,406	1,802
C	5,451	1,817

l/ : Total annual operation hours/unit.

Block	Annual operating hours per unit(hr)	Annual operation costs per unit (¥'000)			Annual operation costs per three units (¥'000)		
		Heavy oil	Lubricat-ing oil	Total	Heavy oil	Lubricat-ing oil	Total
A	1,842	242.7	43.1	285.8	728.1	129.3	857.4
B	1,802	197.6	35.7	233.3	592.8	107.1	699.9
C	1,817	438.8	78.5	517.3	1,316.4	235.5	1,551.9
Total				1,036.4	2,637.3	471.9	3,109.2

Motor

Block	Output per unit (kw)	Annual operating hours per unit (hr)	Annual operating costs per unit (¥'000)	Annual operating costs per three units (¥'000)
A	170	1,842	156.6 <sup>1/</sup>	469.8
B	140	1,802	126.1	378.3
C	300	1,817	272.6	817.8
Total			555.3	1,665.9

<sup>1/</sup>: 170kw x 1,842hr x 0.5 ¥/kwh

Following table indicate the approximate comparison of both type of prime mover from view point of annual cost required.

(Unit: ¥'000)

Items	Diesel Engine	Motor
Amortization <sup>1/</sup>	6,420	7,350
Replacement cost <sup>2/</sup>	2,290	2,420
Operation cost	3,110	1,670
Total	11,820(103)	11,440(100)

Note: <sup>1/</sup> : cost (see Table A.4-63) x 0.08335 (i=10%)  
<sup>2/</sup> : pump cost x 0.02978 (i=10%)

As is seen in the above table, although an annual cost is almost same, motor is recommended in the project considering the easy operation.

Table A. 4-63 Cost of Pumping Facilities

Items	Diesel engine		Motor	
	$\phi 1,350 \times$	$\phi 1,650 \times$	$\phi 1,350 \times$	$\phi 1,650 \times$
	3 units ( '000 $\phi$ )	3 units ( '000 $\phi$ )	3 units ( '000 $\phi$ )	3 units ( '000 $\phi$ )
Pump & Accessories	6,260	9,660	6,260	9,660
Prime mover	1,118	1,680	1,240	2,320
Reduction gear	1,287	1,456	1,220	1,780
Value & Pipe	4,680	7,733	4,680	7,733
Switch board	1,096	1,331	2,193	2,663
Auxiliary equipments	52	48	114	127
Installation & Arrangement	4,531	5,753	4,067	5,353
Packing & Freightage	1,167	1,680	1,167	1,680
Other costs	2,088	3,063	2,273	3,393
Total	<u>22,279</u>	<u>32,404</u>	<u>23,214</u>	<u>34,709</u>

The construction costs for electric power transmission and supplying facilities is  $\phi 7,026,000$  (From PEA)

Total costs for pumping facilities of Project Area is as follows;

Block	Motor ( '000 $\phi$ )	Diesel engine ( '000 $\phi$ )
A $\phi 1,350 \times 3$ units	23,214	22,279
B $\phi 1,350 \times 3$ units	23,214	22,279
C $\phi 1,650 \times 3$ units	34,709	32,404
Total	<u>81,137</u>	<u>76,962</u>

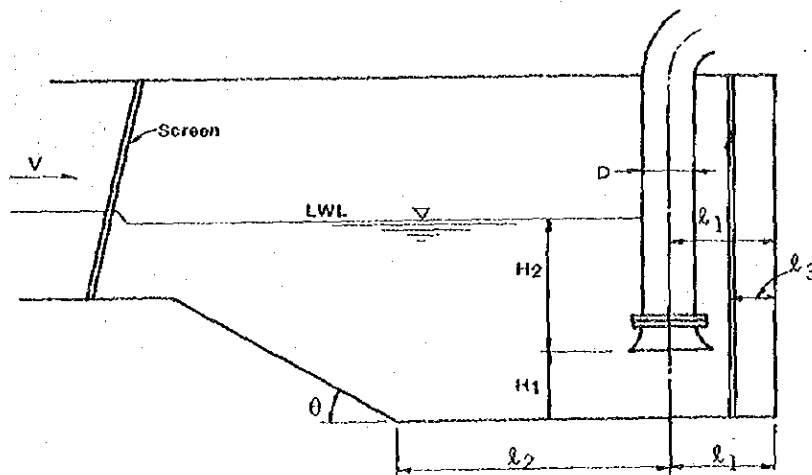
The construction costs in case of motor  
 = Construction cost of pumping facilities  
 + Costs of the electric power transmission facilities  
 =  $\phi 81,137,000 + \phi 7,026,000$   
 =  $\phi 88,163,000$

Plan of Pumping Stations

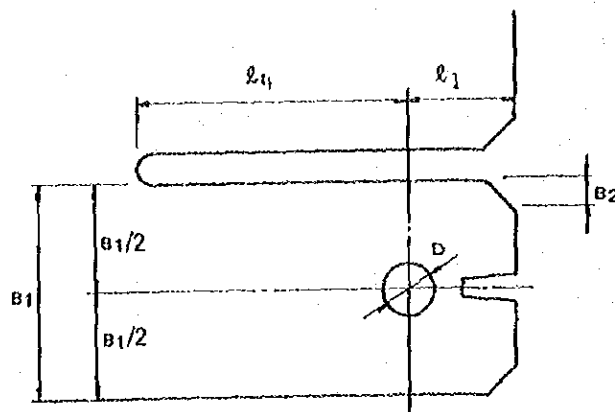
Dimensions of the proposed pumps are as follows;

Block	Design Capacity		units	Boredia- meter	Design Capacity per unit		Type of pump
	Drainage	Irriga- tion			m <sup>3</sup> /s	m <sup>3</sup> /min	
A	11.97	4.96	3	φ1,350	3.99	239	Horizontal mixed flow pump
B	10.31	4.14	3	φ1,350	3.44	206	"
C	19.32	7.82	3	φ1,650	6.44	386	"

(1) Design of suction tank



- V : < 0.5 m/s
- l<sub>1</sub> : 1.5 D
- l<sub>2</sub> : > 3 D
- l<sub>3</sub> : 0.8 D
- l<sub>4</sub> : 3 D ±
- H<sub>1</sub> : 1 D
- H<sub>2</sub> : 1.5 - 2 D
- B<sub>1</sub> : > 3 D
- B<sub>2</sub> : 0.75 D ±

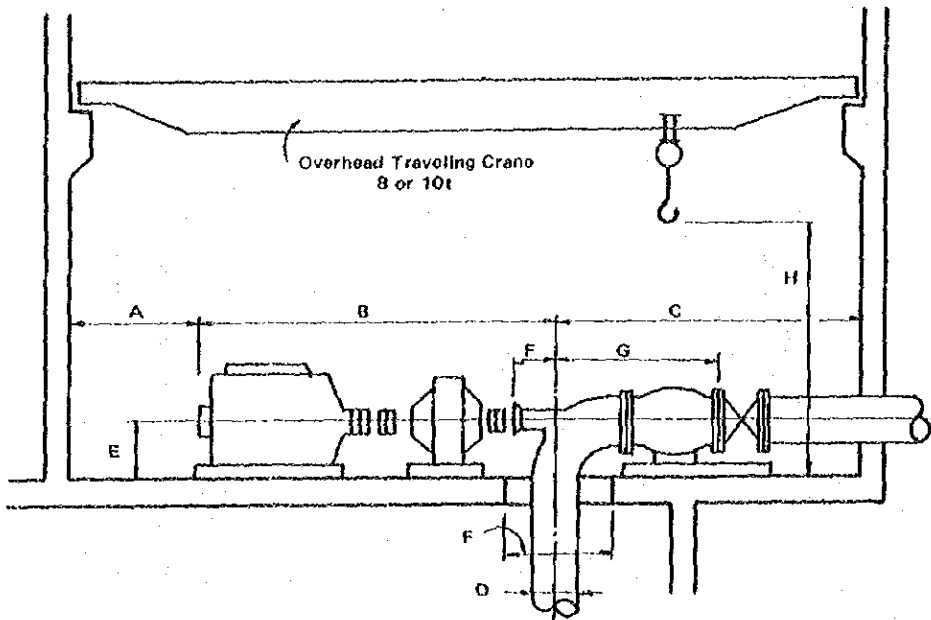


$$l_2 = (\theta = 30^\circ) > 3D$$

$$(\theta = 45^\circ) > 4.5D$$

(2) Design of Pumping House

The building of pumping station is composed of three rooms, namely pumping machine room, frame work room, and operation room. Main facilities set in the machine room are composed of pumps, reduction gear, motor and regulating valves, and the width of the machine room is planned as follows. Also, the overhead travelling crane will be installed in the pumping house.



(Unit: mm)

Block	D	A	B	C	E	F	G	H
A, B	φ1,350	over 1,500	4,000±	6,500	1,300	1,350	3,360	over 4,100
C	φ1,650	over 1,500	4,500±	6,500	1,400	1,600	4,000	over 4,800

Capacity of the overhead travelling crane is as follows;

φ1,350 ----- 8 t crane

φ1,650 ----- 10 t crane

FIGURE A.4 - 44  
DIMENSION OF PUMPING HOUSE

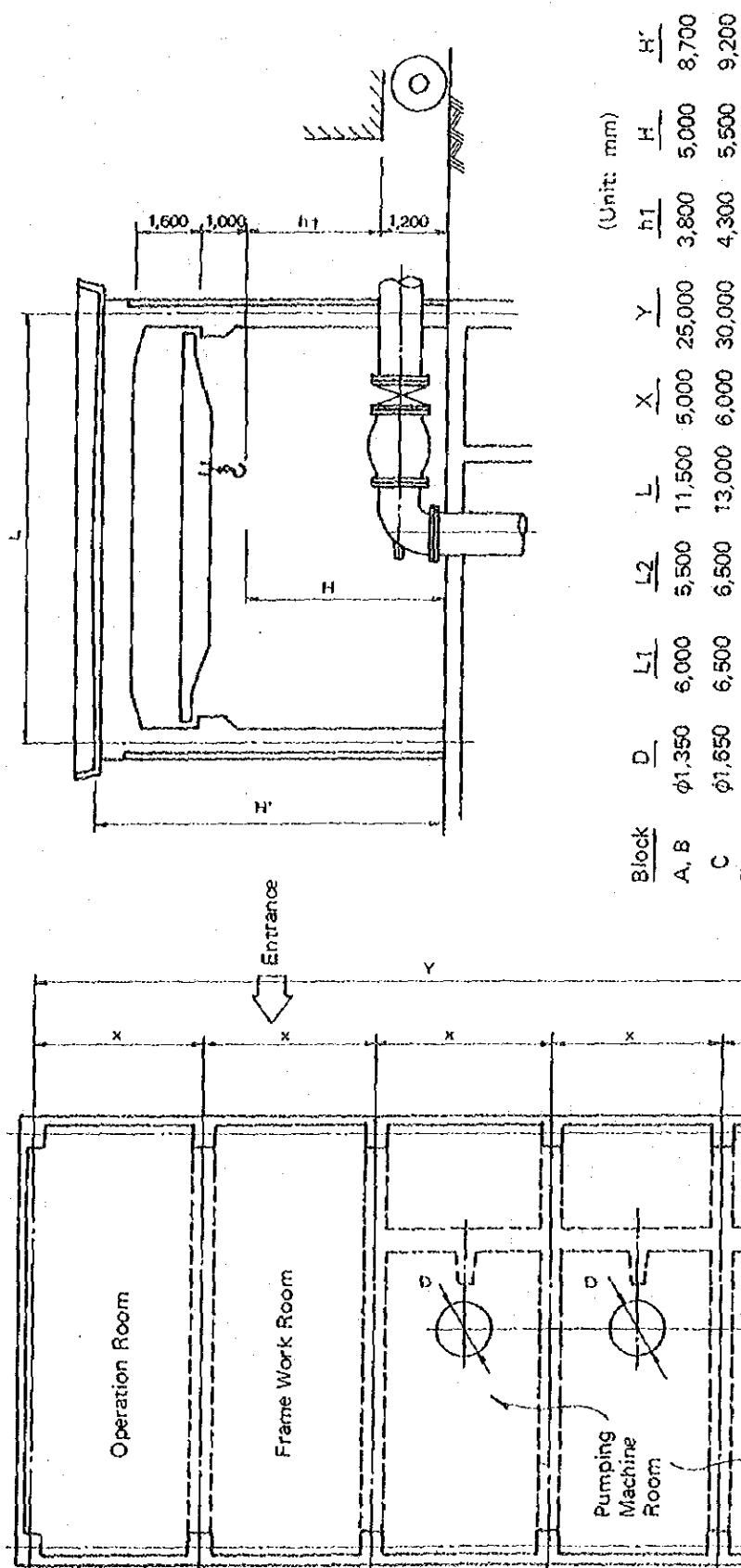
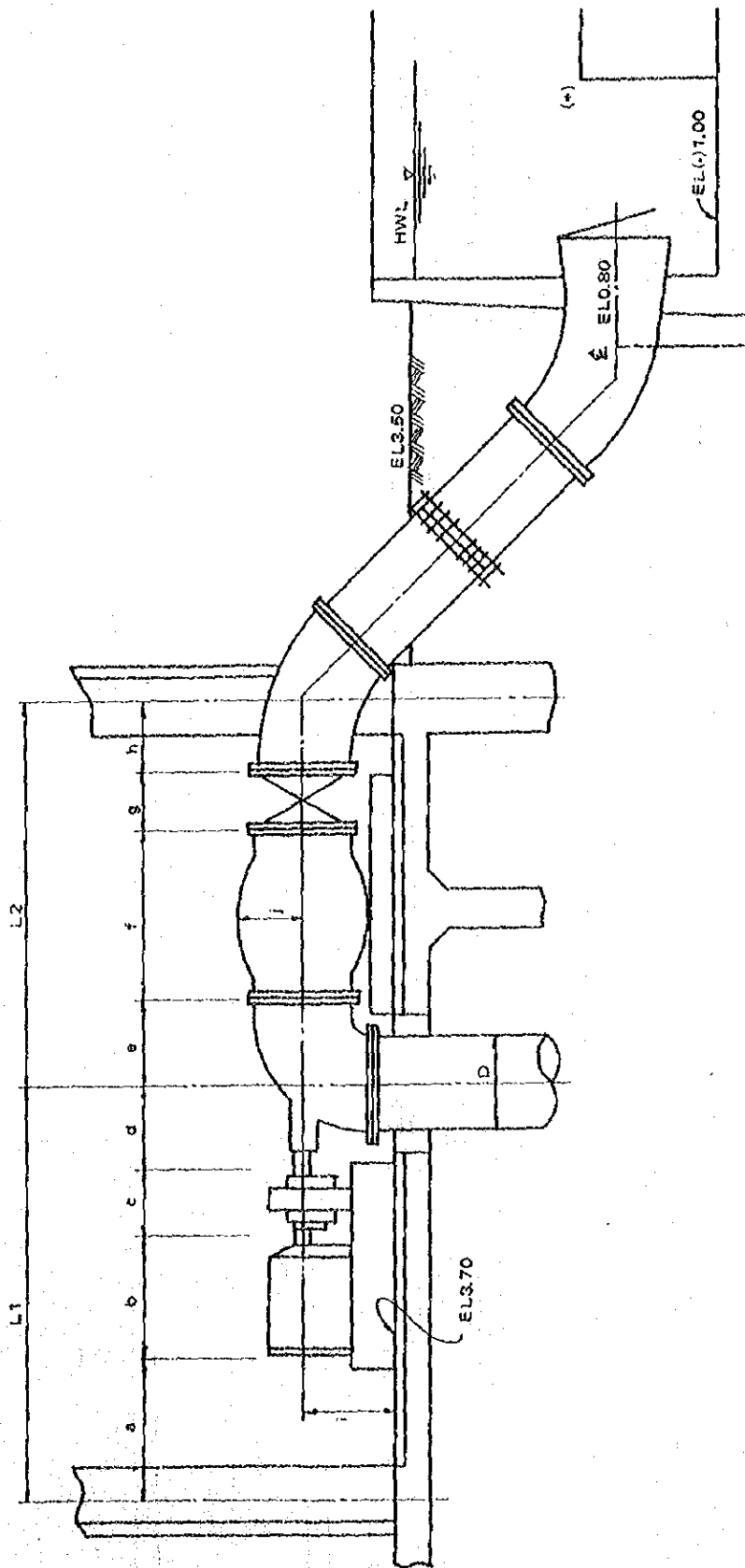


FIGURE A.4 - 45 DIMENSION OF PUMP



(Unit: mm)

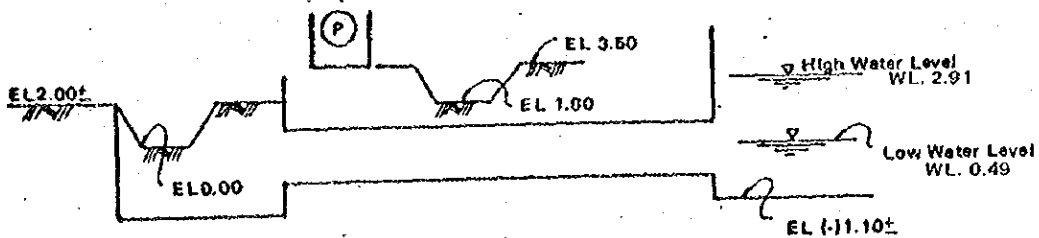
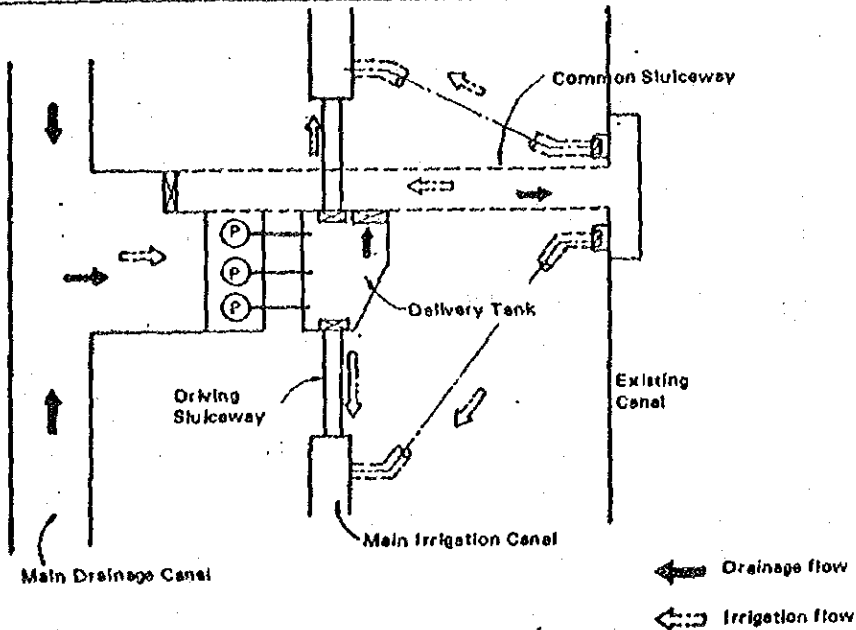
Block	D	L1	L2	a	b	c	d	e	f	g	h	i	j
A, B	φ1,350	6,000	5,500	1,850	1,800	1,000	1,350	1,200	2,160	1,300	840	1,300	1,100
C	φ1,650	6,500	6,500	1,800	2,000	1,100	1,600	1,500	2,500	1,400	1,100	1,400	1,200



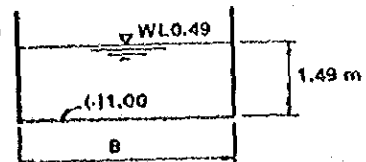
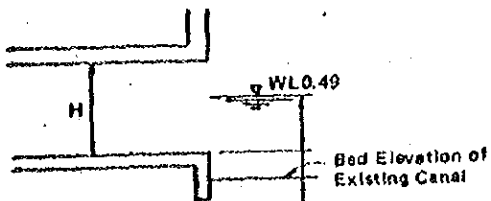
Hydraulic Calculation of Pumps

As already discussed previously, the proposed pumps have the dual purposes for irrigation and drainage, and their systems of water flow are illustrated as shown below:

Diagram of Water Flow for Irrigation and Drainage



Section of common sluiceway is as follows:

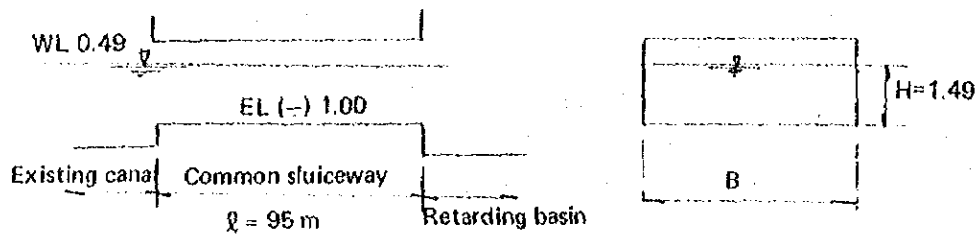


Diameter of pump	$\phi 1,350$	$\phi 1,650$
B m	4.40	5.40

(1) Decision of Suction Water Level

In case of irrigation water supply:

The suction water level in case of irrigation water supply is calculated by deducting the loss head caused by common sluiceway and screen provided in the suction tank from the low water level of 0.49 m.



Description	Unit	Block A	Block B	Block C
Design water requirements, Q	m <sup>3</sup> /s	4.96	4.14	7.82
Width of the common sluiceway, B	m	4.40	4.40	5.40
Cross-sectional area of flow, A	m <sup>2</sup>	6.56	6.56	8.05
Hydraulic radius, R	m	0.557	0.557	0.584
Velocity, V	m/s	0.756	0.631	0.971
Velocity head, hv	m	0.029	0.020	0.048
Hydraulic gradient, I		$2.8 \times 10^{-4}$	$2.0 \times 10^{-4}$	$4.3 \times 10^{-4}$
Loss head, h	m	0.071	0.049	0.113

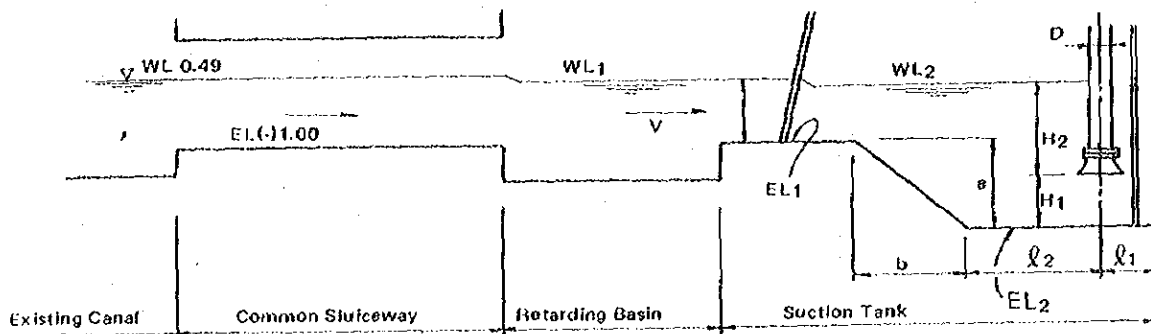
$$h = (f_e + f_{se})hv + I \cdot l$$

$f_e$  : coefficient of inlet loss 0.5  
 $f_{se}$  : coefficient of outlet loss 1.0

$$I = \frac{N^2 V^2}{R^{4/3}}$$

N : Roughness coefficient 0.015  
(Reinforced concrete)

The suction water level and the bed elevation of the suction tank are calculated as follows;



	Unit	Block A	Block B	Block C
Suction Water Level WL <sub>2</sub>	m	≅ 0.20	≅ 0.20	≅ 0.10
WL <sub>1</sub>	"	0.41	0.44	0.37
EL <sub>1</sub>	"	(-) 1.35	(-) 1.35	(-) 2.00
EL <sub>2</sub>	"	(-) 3.80	(-) 3.80	(-) 4.60
D	"	1.35	1.35	1.65
H <sub>1</sub>	"	1.35	1.35	1.65
H <sub>2</sub>	"	2.65	2.65	3.05
l <sub>1</sub>	"	2.05	2.05	2.50
l <sub>2</sub>	"	4.10	4.10	5.00
a	"	2.45	2.45	2.60
b	"	5.00	5.00	6.00
V	m/s	0.32	0.26	0.31

In case of drainage:

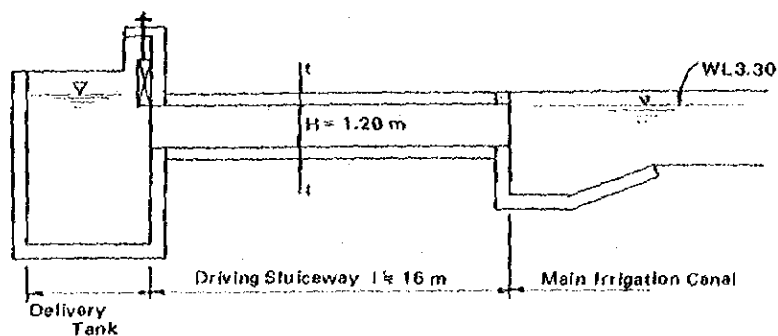
Assuming that the water level of the retarding basin is WL 1.50<sup>f</sup> in all blocks, the suction water level will be WL 1.20 after reducing the loss head of 0.30<sup>f</sup> m at the part of suction tank.

From the result of study, design bed elevation of suction tank and the depth of suction tube are decided based on the irrigation water supply.

(2) Decision of Delivery Water Level

In case of irrigation water supply:

Assuming that half of design irrigation water requirement is to be distributed from the suction tank to each main irrigation canal, the delivery water level will be roughly calculated as follows;



The water level of the delivery tank is calculated by adding the loss head caused by the driving sluiceway to the design water level of WL 3.30 for the main irrigation canal.

Description	Unit	Block A	Block B	Block C
Design water requirements, Q	m <sup>3</sup> /s	2.48	2.07	3.91
Cross-sectional width of the driving sluiceway, B	m	2.0	2.0	3.0
Cross-sectional area of flow, A	m <sup>2</sup>	2.4	2.4	3.6
Hydraulic radius, R	m	0.387	0.387	0.429
Velocity of flow, V	m/s	1.033	0.863	1.086
Velocity head, hv	m	0.054	0.038	0.060
Hydraulic gradient, I		8.5 × 10 <sup>-4</sup>	5.9 × 10 <sup>-4</sup>	6.2 × 10 <sup>-4</sup>
loss head, h	m	0.094	0.066	0.099
Delivery water level, HWL		≈ 3.40	≈ 3.40	≈ 3.40

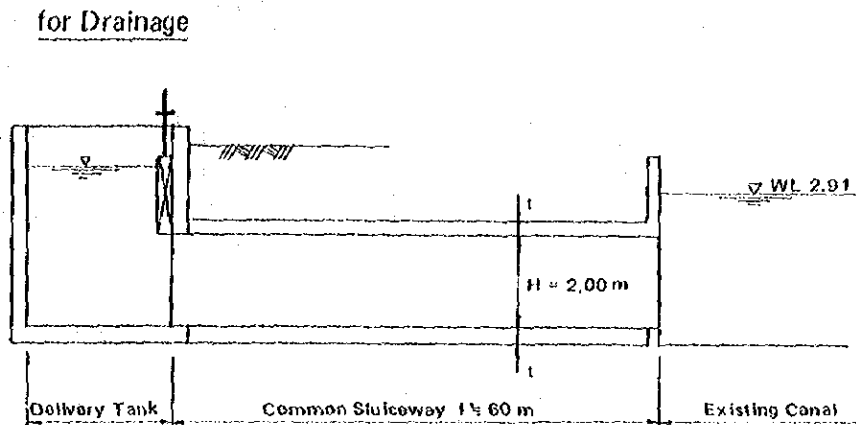
$$h = (f_e + f_{se})hv + I.l = 1.5hv + I.l$$

Roughness coefficient

$$I = \frac{N^2 V^2}{R^{4/3}}$$

N = 0.015

Incase of drainage:



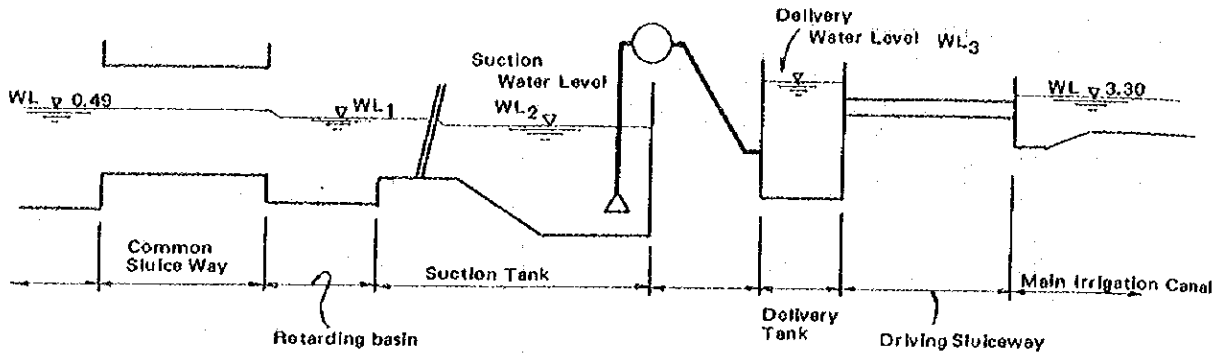
The delivery water level is calculated by adding the loss head caused by the common sluiceway to the design outer water level of WL 3.00<sup>±</sup>.

<u>Description</u>	<u>Unit</u>	<u>Block A</u>	<u>Block B</u>	<u>Block C</u>
Design drainage discharge, Q	m <sup>3</sup> /s	11.97	10.31	19.32
Cross-sectional width of the common sluiceway, B	m	4.4	4.4	5.4
Cross-sectional area of flow, A	m <sup>2</sup>	8.8	8.8	10.8
Hydraulic radius, R	m	0.688	0.688	0.730
Velocity of flow, V	m/s	1.360	1.172	1.789
Velocity head, hv	m	0.094	0.070	0.163
Hydraulic gradient, I		6.9x10 <sup>-4</sup>	5.1x10 <sup>-4</sup>	1.1x10 <sup>-3</sup>
Loss head, h	m	0.182	0.136	0.311
Delivery water level, HWL		≈ 3.10	≈ 3.10	≈ 3.10

Figure A.4-46 indicates the results of design suction and delivery water level.

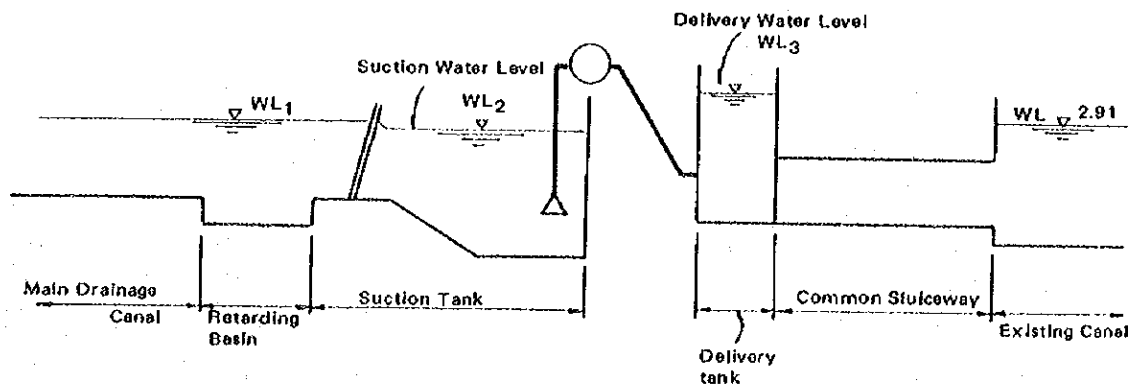
FIGURE A.4 - 46 DESIGN SUCTION AND DELIVERY WATER LEVEL

for Irrigation



Block	Delivery Water Level WL <sub>3</sub>	Suction Water Level WL <sub>2</sub>	WL <sub>1</sub>
A	3.40	0.20	0.41
B	3.40	0.20	0.44
C	3.40	0.10	0.37

for Drainage



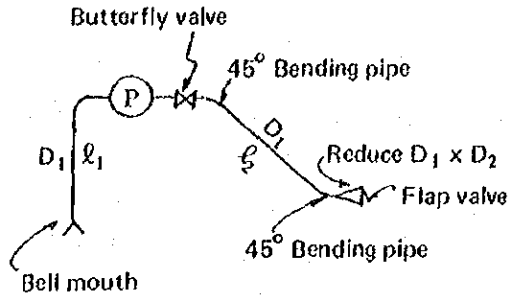
Block	Delivery Water Level WL <sub>3</sub>	Suction Water Level WL <sub>2</sub>	WL <sub>1</sub>
A	3.10	1.20	1.50
B	3.10	1.20	1.50
C	3.30	1.20	1.50

So, actual heads of pump (WL<sub>3</sub>-WL<sub>2</sub>) are calculated as follows;

	Block A	Block B	Block C
for Irrigation	3.20	3.20	3.30
for drainage	1.90	1.90	2.10

(3) Calculation of Total Head

The total head of pump can be calculated by adding the total loss head in the pipe to the actual head of pump. The loss heads in the pipe are calculated as follows;



	Block A, B	Block C
D <sub>1</sub> m	φ1.35	φ1.65
D <sub>2</sub> m	φ1.65	φ2.00
l <sub>1</sub> m	6.0	6.6
l <sub>2</sub> m	4.0	4.0

(1) Suction loss head (h<sub>e</sub>)

$$h_e = f_e \cdot V_1^2 / 2g \quad f_e: 0.3 \text{ (Bell mouth)}$$

(2) Bending loss head (h<sub>b</sub>)

$$h_b = n \cdot f_b \cdot V_1^2 / 2g \quad f_b: 0.13 \quad n: \text{Number of bending pipe}$$

(3) Reducer loss head (h<sub>gd</sub>)

$$h_{gd} = f_{gd} \cdot (V_1 - V_2)^2 / 2g \quad f_{gd}: 0.13$$

V<sub>2</sub>: Velocity of flow after reducing

(4) Valve loss head (h<sub>v</sub>)

$$h_v = f_v \cdot V_1^2 / 2g \quad f_v: 0.30 \text{ (Butterfly valve)}$$

(5) Delivery loss head (h<sub>se</sub>)

$$h_{se} = f_{se} \cdot V_2^2 / 2g \quad f_{se}: 1.5 \text{ (In case of flap Valve)}$$

(6) Friction loss head (h<sub>f</sub>)

$$h_f = f \cdot (l_1 + l_2) \cdot V_1^2 / 2g$$

F: 0.0141 (φ1,350)  
F: 0.0108 (φ1,650)

If pumps are planned for the purpose of drainage, total loss head for the irrigation will be nearly 130% of that in case of drainage, resulting in 70% of discharge in the drainage. Therefore, the loss heads in pipes in case of irrigation purpose are calculated based on the 70% of drainage discharge.

Items	Unit	Block A		Block B		Block C	
		Irriga- tion	Drainage	Irriga- tion	Drainage	Irriga- tion	Drainage
Lifting capacity, Q	m <sup>3</sup> /s	3.99	2.79	3.44	2.41	6.44	4.51
Diameter of pipe, D <sub>1</sub>	mm	φ1,350		φ1,350		φ1,650	
Diameter of pipe, D <sub>2</sub>	mm	φ1,650		φ1,650		φ1,800	
Velocity at D <sub>1</sub> , V <sub>1</sub>	m/s	2.787	1.949	2.403	1.684	3.012	2.109
Velocity at D <sub>2</sub> , V <sub>2</sub>	m/s	1.866	1.304	1.609	1.127	2.531	1.772
Velocity head of V <sub>1</sub> , hv <sub>1</sub>	m	0.396	0.194	0.295	0.145	0.463	0.227
Velocity head of V <sub>2</sub> , hv <sub>2</sub>	m	0.178	0.098	0.132	0.065	0.327	0.160
Suction loss head, h <sub>e</sub>	m	0.119	0.058	0.089	0.044	0.139	0.068
Bending loss head, h <sub>b</sub>	m	0.154	0.075	0.116	0.057	0.180	0.089
Valve loss head, h <sub>v</sub>	m	0.119	0.059	0.089	0.044	0.140	0.068
Reducer loss head, h <sub>gd</sub>	m	0.009	0.005	0.006	0.003	0.003	0.002
Friction loss head, h <sub>f</sub>	m	0.084	0.041	0.063	0.030	0.080	0.039
Delivery loss head, h <sub>sc</sub>	m	0.267	0.147	0.198	0.098	0.491	0.240
Total		0.752	0.385	0.561	0.276	1.033	0.506
		0.80	0.40	0.60	0.30	1.10	0.50

Therefore, the total loss head in each blocks are as follows;

	Block A		Block B		Block C	
	Irriga- tion	Drainage	Irriga- tion	Drainage	Irriga- tion	Drainage
Actual head	1.90	3.20	1.90	3.20	2.10	3.30
Loss head in pipe	0.80	0.40	0.60	0.30	1.10	0.50
Total (loss head)	2.70	3.60	2.50	3.50	3.20	3.80



Study on the Suction Performance of Pump (Cavitation)

If high vacuum is partly formed in water flow, the water vaporizes and fine bubble of vapor arises. This phenomenon called cavitation, which arises around the entrance of the impeller of pump, and causes noise and vibration, reduces the pumping efficiencies and the delivery discharge, and at last causes the stop of operation. In order to prevent from this phenomenon, the suction head and the type of pump must be decided considering that the pump is able to endure against the high pressure more than the required minimum pressure, that is, the reducing pressure at the entrance of the impeller plus the saturated steam pressure of water.

The study on cavitation was made for the horizontal axial flow pump and the horizontal mixed flow pump in each case of the operating for irrigation and/or drainage. In the study, following two values of Net Positive Suction Head (NPSH) are estimated;

NPSH which the pump requires ..... (hsv)

NPSH which the pump is able to avail ..... (Hsv)

In order that the pump does not cause the cavitation

Hsv > hsv is sufficient condition.

Calculation of hsv;

hsv is calculated by the following arithmetical formula.

$$hsv = \left( \frac{N \cdot \sqrt{Q}}{S} \right)^{4/3}$$
$$N = \frac{Ns \cdot H^{3/4}}{Q^{1/2}}$$

- where, Q : Delivery discharge (cu.m/min)  
S : Suction specific velocity  
Mixed flow pump  $\approx$  1,300, Axial flow pump  $\approx$  1,200  
N : Number of impeller turning (rpm)

- H : Total pump head  
 Ns: Specific velocity  
     Mixed flow pump: 900 - 1,000  
     Axial flow pump: 1,500 - 1,600

Block	Q(m <sup>3</sup> /min)	Q <sup>1/2</sup>	H(m)	H <sup>3/4</sup>	N(rpm)		hsv (m)	
					Axial flow pump	Mixed flow pump	Axial flow pump	Mixed flow pump
A	239	15.46	2.7	2.11	220	140	4.01	1.97
B	206	14.35	2.5	1.99	220	140	3.63	1.79
C	386	19.65	3.2	2.39	200	120	4.86	2.21

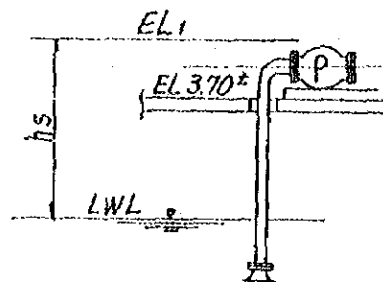
Calculation of Hsv:

Hsv is calculated by the following formula:

$$Hsv = Ha - hs - hv - ht - h = 9.5 - hs - ht$$

- Ha : Atmospheric pressure (10.33 m)  
 hs : Actual suction head (m)  
 hv : Saturated stream pressure (0.33 m)  
 ht : Suction loss head (m)  
 h : Allowance 0.5 (m)

Actual suction head indicates the height from the suction water level to the upper end of pump.



Item	Unit	Drainage			Irrigation		
		Block A	Block B	Block C	Block A	Block B	Block C
LWL	m	1.20	1.20	1.20	0.20	0.20	0.10
EL <sub>1</sub>	m	6.10	6.10	6.30	6.10	6.10	6.30
hs	m	4.90	4.90	5.10	5.90	5.90	6.20
ht	m	0.25	0.19	0.28	0.13	0.10	0.15
Hsv	m	4.35	4.41	4.12	3.47	3.50	3.15

Block	Horizontal axial flow pump					Horizontal mixed flow pump				
	hsv (m)	Drainage		Irrigation		hsv (m)	Drainage		Irrigation	
		Hsv	Judgment	Hsv	Judgment		Hsv	Judgment	Hsv	Judgment
A	4.01	4.35	P	3.47	I	1.97	4.35	P	3.47	P
B	3.63	4.41	P	3.50	I	1.79	4.41	P	3.50	P
C	4.86	4.12	I	3.15	I	2.21	4.12	P	3.15	P

Note: P; Possible      I; Impossible

From the table mentioned above, the axial flow pump is able to be operated for the purpose of drainage, but it can not be operated for the purpose of irrigation, therefore, the horizontal mixed flow pump are recommended in all blocks in the Project.



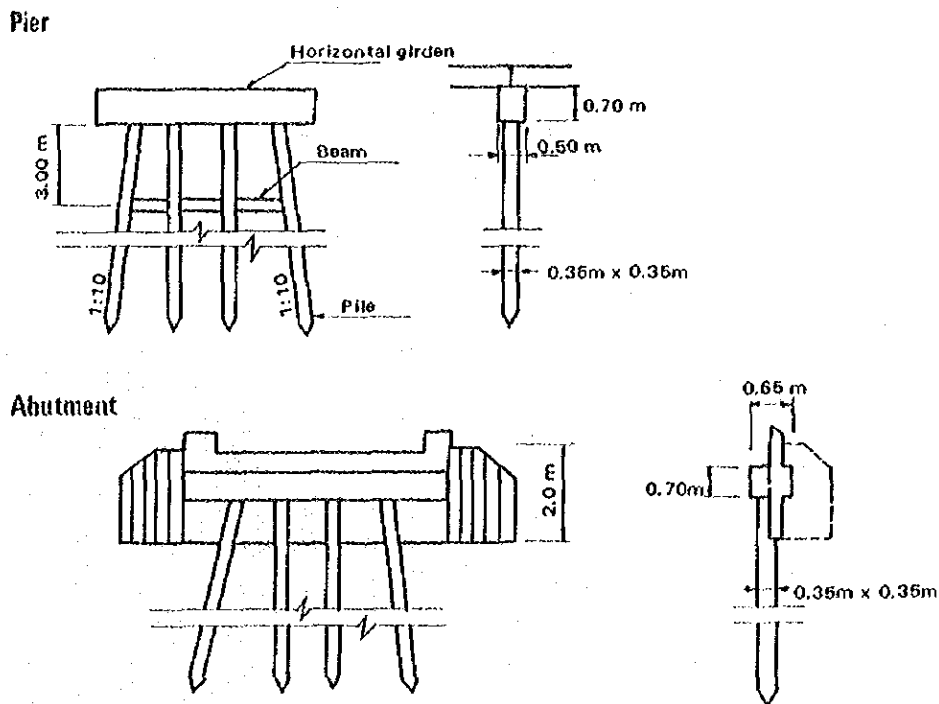
- Q : Delivery discharge (cu.m/min)  
H : Total pumping head (m)  
 $\alpha$  : Surplus coefficient 0.15  
 $\eta_p$  : Pumping efficiency  
 $\phi 1,350$   $\eta_P = 78 \%$   
 $\phi 1,650$   $\eta_P = 81 \%$   
 $\eta_g$  : Conduction efficiency in case of using the  
reduction gear, 0.95

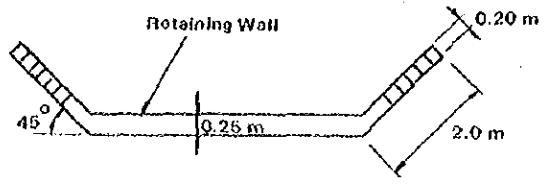
Block	Boredia- meter (mm)	Units	Delivery discharge per unit (m <sup>3</sup> /min)	Total Head	Required output	
					KW	PS
A	$\phi 1,350$	3	239	2.70	170	240
B	$\phi 1,350$	3	206	2.50	140	200
C	$\phi 1,650$	3	386	3.20	300	440

Typical Design of Bridge

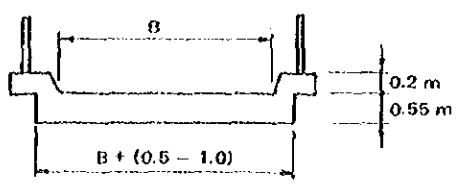
Bridges will be constructed across the existing canals, that is, Phraya Banlu, IL-2, Khud Mai and Nai Chat in the Project Area. Automobile can not pass to the existing wooden bridges except in a few bridges, but after completion of this Project, it is considered that the traffic will be increased owing to the development of road function, and therefore stable bridges, that is, steel bridges, prestressed concrete bridge and slab bridge will be necessary. Slab bridge is adopted in this planning from economical point of view. After completion of the Project, the existing canals will be used for the navigation canals, so the central portion of bridge should be high in order that a ship will be able to a through under the bridges.

Generally speaking, maximum span length of slab bridge is ten meters. So the length of it is planned to be eight meters in considering the safty and the span division. Typical designs are as follows, after making reference to the typical designs which belong to the Department of Highway, RID.

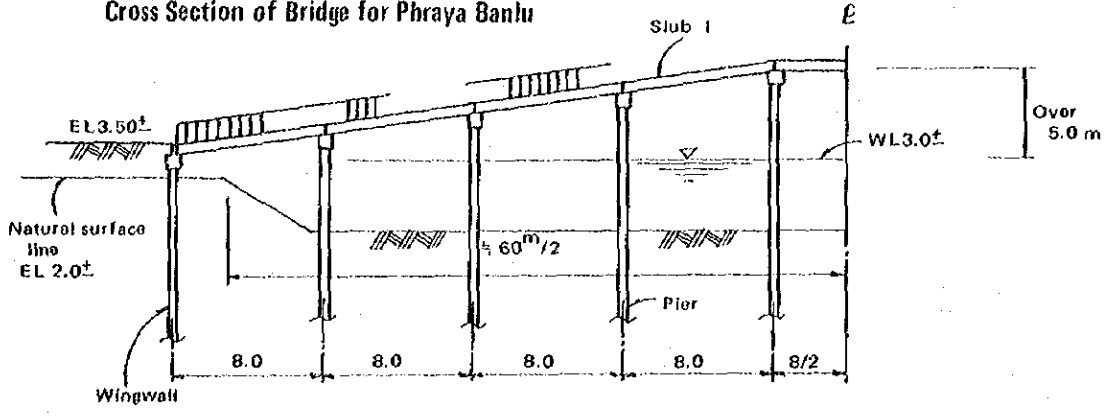




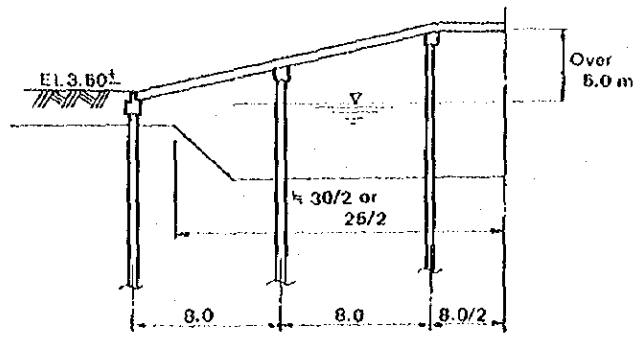
Slab



Cross Section of Bridge for Phraya Banlu



Cross Section of Bridge for IL-2, Khud Mai and Nai Chat



Estimation of Deduction Ratio

The deduction ratio after the project equipped with necessary facilities can be estimated by applying the following equation;

$$\text{Reduction Ratio (\%)} = \frac{\text{area to be reduced by on-farm facilities}}{\text{objective area for on-farm facilities}} \times 100$$

On farm facilities include the all irrigation and drainage canals and roads except the provincial roads, and of which reduction ratio of each block amounts as follows;

<u>Description</u>	<u>Block A</u>	<u>Block B</u>	<u>Block C</u>	<u>Total</u>
Area of on-farm facilities (ha) (a)	223	181	341	745
Objective area for on-farm facilities (ha) (b) <sup>1/</sup>	3,250	2,764	5,273	11,287
Reduction Ratio (%) (c)=(a)/(b)x100	6.9	6.5	6.5	6.6

Note:

	<u>Block A</u>	<u>Block B</u>	<u>Block C</u>
Gross area (1)	3,493 <sup>ha</sup>	3,057 <sup>ha</sup>	5,744 <sup>ha</sup>
Area of existing canal (2)	26	12	93
Area of public facilities (3) <sup>2/</sup>	121	125	198
River side land (4) <sup>3/</sup>	18	75	82
Dike (5)	50	43	62
Borrow pit (6)	24	22	32
Provincial road (7)	4	16	4

1/ : (b) = (1) - {(2) + (3) + (4) + (5) + (6) + (7)}

2/ : consists of homestead, school, supporting services, fish pond and animal husbandry.

3/ : areas between dikes to be provided along the Khlong Phraya Banlu, Khud Mai and Nai Chat.



Table A.4-64 Length and Area of Bed Width for Proposed Facilities (Block A)

Type	Length km	Dike		Borrow pit						Provincial road		Main road		Community road		On-farm road		Main irrigation canal		Lateral irrigation canal		
		U.W	R.A	U.W	B.P		M.D.C		L.D.C		U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A
		m/m	ha	m/m	km	ha	km	ha	km	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha
Dike (8m) (with M.I.C)	-	17.25	-	26.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
" (4m) (with M.I.C)	7.5	13.25	9.9	23.60	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
" (8m) (without M.I.C)	-	17.25	-	19.40	-	-	7.5	17.7	-	-	-	-	-	-	-	-	-	-	-	-	-	
" (4m) (without M.I.C)	29.9	13.25	39.6	16.80	14.0	23.5	1.5	2.5	14.4	24.2	-	-	-	-	-	-	-	-	-	-	-	
Main road (6m) (with L.I.C)	2.75	-	-	-	-	-	-	-	-	-	-	-	8.38	2.3	-	-	-	-	-	-	-	
" (with D.D)	10.45	-	-	-	-	-	-	-	-	-	-	-	8.50	8.9	-	-	-	-	-	-	4.30	
" (with L.D.C)	5.60	-	-	-	-	-	-	-	-	-	-	-	8.50	-	-	-	-	-	-	-	-	
" (with L.I.C and L.D.C)	1.30	-	-	-	-	-	-	-	-	-	-	-	8.88	1.2	-	-	-	-	-	-	3.30	
Community road (4.0m) (with M.I.C)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6.75	-	-	-	-	-	7.78	
" (with L.I.C)	25.05	-	-	-	-	-	-	-	-	-	-	-	-	-	6.38	16.0	-	-	-	-	4.30	
" (with D.D)	6.55	-	-	-	-	-	-	-	-	-	-	-	-	-	6.50	4.3	-	-	-	-	10.8	
" (with L.I.C and L.D.C)	1.0	-	-	-	-	-	-	-	-	-	-	-	-	-	6.88	0.7	-	-	-	-	3.30	
Leading drainage canal (with O.F.R)	9.75	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.3	
Existing provincial road (with M.I.C)	-	-	-	-	-	-	-	-	-	-	11.0	-	-	-	-	-	3.6x2	7.0	-	-	-	
" (with L.I.C)	4.15	-	-	-	-	-	-	-	-	-	10.5	4.4	-	-	-	-	-	-	-	-	10.10	
"	-	-	-	-	-	-	-	-	-	-	11.0	-	-	-	-	-	-	-	-	-	4.10	
Proposed provincial road (with M.I.C and M.D.C)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
" (with M.D.C)	-	-	-	-	-	-	-	-	-	-	12.0	-	-	-	-	-	-	-	-	-	8.10	
" (with M.I.C)	-	-	-	-	-	-	-	-	-	-	11.5	-	-	-	-	-	-	-	-	-	-	
Existing Dike (with L.D.C)	-	10.0	-	-	-	-	-	-	-	-	11.0	-	-	-	-	-	-	-	-	-	10.10	
Existing Road (with L.I.C)	-	-	-	-	-	-	-	-	-	-	-	-	9.25	-	-	-	-	-	-	-	4.30	
<b>Total</b>		<b>(37.4)</b>	<b>49.5</b>		<b>(14.0)</b>	<b>23.5</b>	<b>(9.0)</b>	<b>20.2</b>	<b>(14.4)</b>	<b>24.2</b>	<b>(4.15)</b>	<b>4.4</b>	<b>(14.5)</b>	<b>12.4</b>	<b>(32.6)</b>	<b>21.0</b>	<b>(19.50)</b>	<b>7.0</b>	<b>(7.50)</b>	<b>10.9</b>	<b>(34.25)</b>	<b>14.4</b>

Note: ( ) shows the total length of facilities

U.W : Unit width (m)  
R.A : Required area (ha)  
M.I.C : Main irrigation canal  
L.I.C : Lateral irrigation canal  
M.D.C : Main drainage canal  
L.D.C : Leading drainage canal  
D.D : Drainage ditch  
O.F.R : On-farm road

Length and Area of Bed Width for Proposed Facilities (Block A)

B.P	Borrow pit				Provincial road		Main road		Community road		On-farm road		Main irrigation canal		Lateral irrigation canal		Main drainage canal		Leading drainage canal		Drainage ditch		Total		Number of Unit Cost	
	R.A ha	M.D.C km	R.A ha	L.D.C km	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m		R.A ha
		7.5	17.7										14.50											57.85		D-1, MIC-1
													14.50	10.9										51.35	38.5	D-2, MIC-2
23.5	1.5	2.5	14.4	24.2																			36.65		D-3	
							8.38	2.3							4.30	1.2							30.05	89.8	D-4	
							8.50	8.9															12.68	3.5	MR-1, LIC-1	
							8.50															3.4x2	7.1	15.30	16.0	MR-2, DD-1
							8.88	1.2											5.4x2	3.0			19.30	3.0	MR-3, LDC-2	
									6.75						3.30	0.4			5.90	0.8			18.08	2.4	MR-4, LDC-3, LIC-3	
									6.38	16.0					4.30	10.8							14.53		CR-4, MIC-4	
									6.50	4.3												3.4x2	4.4	13.30	8.7	CR-1, LIC-2 CR-2, DD-2
									6.88	0.7					3.30	0.3			5.90	0.6			16.08	1.6	CR-3, LDC-4, LIC-4	
											3.6x2	7.0							7.00	6.8			14.20	13.8	OFR-1, LDC-1	
						11.0							10.10										21.10		PR-2, MIC-3	
						10.5	4.4							4.10	1.7								14.60	6.1	PR-1, LIC-5	
						11.0																	11.00		PR-5	
						12.0							8.10				12.05						32.15		PR-3, MIC-5, MDC-1	
						11.5											12.05						23.55		PR-4, MDC-2	
						11.0							10.10										21.10		PR-6, MIC-3	
																			5.0				15.00		D-5, LDC-5	
							9.25								4.30								13.55		MR-5, LIC-6	
23.5	(9.0)	20.2	(14.4)	24.2	(4.15)	4.4	(14.5)	12.4	(32.6)	21.0	(19.50)	7.0	(7.50)	10.9	(34.25)	14.4		(17.65)				11.2	(34.0)	11.5	210.2	

of facilities

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canal  
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Table A.4-65 Length and Area of Bed Width of Proposed Facilities (Block B)

Type	Length km	Dike		U.W m/m	Borrow pit						Provincial road		Main road		Community road		On-farm road		Main irrigation canal		Lateral irrigation canal	
		U.W	R.A		B.P		M.D.C		L.D.C		U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R
		m/m	ha		km	ha	km	ha	km	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	
Dike (8m) (with M.I.C)	-	17.25		26.10															14.50			
" (4m) (with M.I.C)	9.45	13.25	12.5	23.60			9.45	22.3											14.50	13.7		
" (8m) (without M.I.C)	-	17.25		19.40																		
" (4m) (without M.I.C)	22.4	13.25	29.7	16.80	13.1	22.0	4.0	6.7	5.3	8.9												
Main road (6m) (with L.I.C)	2.4	-	-	-	-	-	-	-	-	-			8.38	2.0						4.30	1	
" (with D.D)	8.3	-	-	-	-	-	-	-	-	-			8.50	7.1								
" (with L.D.C)	5.6	-	-	-	-	-	-	-	-	-			8.50	4.8								
" (with L.I.C and L.D.C)	-	-	-	-	-	-	-	-	-	-			8.88							3.30		
Community road (4.0m) (with M.I.C)	-	-	-	-	-	-	-	-	-	-					6.75							
" (with L.I.C)	12.0	-	-	-	-	-	-	-	-	-					6.38	7.6				4.30	5	
" (with D.D)	-	-	-	-	-	-	-	-	-	-					6.50							
" (with L.I.C and L.D.C)	-	-	-	-	-	-	-	-	-	-												
Leading drainage canal (with O.F.R)	12.35	-	-	-	-	-	-	-	-	-					6.88					3.30		
Existing provincial road (with M.I.C)	2.65	-	-	-	-	-	-	-	-	-	11.0	2.9						3.6x2	8.9			
" (with L.I.C)	5.45	-	-	-	-	-	-	-	-	-	10.5	5.7								10.10	2.7	
"	6.50	-	-	-	-	-	-	-	-	-	11.0	7.2								4.10	2	
Proposed provincial road (with M.I.C and M.D.C)	-	-	-	-	-	-	-	-	-	-												
" (with M.D.C)	-	-	-	-	-	-	-	-	-	-	12.0									8.10		
" (with M.I.C)	-	-	-	-	-	-	-	-	-	-	11.5											
Existing Dikey (with L.D.C)	1.2	10.0	1.2	-	-	-	-	-	-	-	11.0									10.10		
Existing Road (with L.I.C)	-	-	-	-	-	-	-	-	-	-			9.25								4.30	
<b>Total</b>		<b>(33.05)</b>	<b>43.4</b>		<b>(13.1)</b>	<b>22.0</b>	<b>(13.45)</b>	<b>29.0</b>	<b>(5.3)</b>	<b>8.9</b>	<b>(14.6)</b>	<b>15.8</b>	<b>(16.3)</b>	<b>13.9</b>	<b>(12.0)</b>	<b>7.6</b>	<b>(24.7)</b>	<b>8.9</b>	<b>(12.10)</b>	<b>16.4</b>	<b>(19.85)</b>	<b>8</b>

Note: ( ) shows the total length of facilities

U.S : Unit width (m)  
R.A : Required area (ha)  
M.I.C : Main irrigation canal  
L.I.C : Lateral irrigation canal  
M.D.C : Main drainage canal  
L.D.C : Leading drainage canal  
D.D : Drainage ditch  
O.F.R : On-farm road

Area of Bed Width of Proposed Facilities (Block B)

Y pit C	L.D.C		Provincial road		Main road		Community road		On-farm road		Main irrigation canal		Lateral irrigation canal		Main drainage canal		Leading drainage canal		Drainage ditch		Total		Number of Unit Cost	
	R.A ha	L km	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m		R.A ha
22.3											14.50											57.85	-	D-1, MIC-1
											14.50	13.7										51.35	48.5	D-2, MIC-2
																						36.65	-	D-3
6.7	5.3	8.9																				30.05	67.3	D-4
					8.38	2.0							4.30	1.0								12.68	3.0	MR-1, LIC-1
					8.50	7.1													3.4x2	5.6		15.30	12.7	MR-2, DD-1
					8.50	4.8											5.4x2	3.0				19.30	7.8	MR-3, LDC-2
					8.88								3.30				5.90					18.08	-	MR-4, LDC-3, LIC-3
							6.75				7.78											14.53	-	CR-4, MIC-4
							6.38	7.6					4.30	5.2								10.68	12.8	CR-1, LIC-2
							6.50												3.4x2			13.30	-	CR-2, DD-2
							6.88						3.30				5.90					16.08	-	CR-3, LDC-4, LIC-4
									3.6x2	8.9							7.00	8.6				14.20	17.5	OFR-1, LDC-1
			11.0	2.9							10.10	2.7										21.10	5.6	PR-2, MIC-3
			10.5	5.7									4.10	2.3								14.60	8.0	PR-1, LIC-5
			11.0	7.2																		11.00	7.2	PR-5
			12.0								8.10				12.05							32.15	-	PR-3, MIC-5, MDC-1
			11.5												12.05							23.55	-	PR-4, MDC-2
			11.0								10.10											21.10	-	PR-6, MIC-3
																5.0	0.6					15.00	1.8	D-5, LDC-5
					9.25								4.30									13.55	-	MR-5, LIC-6
29.0	(5.3)	8.9	(14.6)	15.8	(16.3)	13.9	(12.0)	7.6	(24.7)	8.9	(12.10)	16.4	(19.85)	8.5		(19.15)	12.2	(16.6)	5.6			192.2		

Table A.4-66 Length and Area of Bed Width for Proposed Facilities (Block C)

Type	Length km	Dike			Borrow pit						Provincial road		Main road		Community road		On-farm road		i m/
		U.W m/m	R.A ha	U.W m/m	B.P		M.D.C		L.D.C		U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	U.W m/m	R.A ha	
					ℓ km	R.A ha	ℓ km	R.A ha	ℓ km	R.A ha									
Dike (8m) (with M.I.C)	7.0	17.25	12.1	26.10	6.5	17.0	0.5	1.3											14.
" (4m) (with M.I.C)	3.0	13.25	4.0	23.60			3.0	7.1											14.
" (8m) (without M.I.C)	6.6	17.25	11.4	19.40	0.4	0.8			6.2	12.0									
" (4m) (without M.I.C)	22.8	13.25	30.2	16.80	8.4	14.1	1.0	1.7	13.4	22.5									
Main road (6m) (with L.I.C)	5.0	-	-	-	-	-	-	-	-	-		8.38	4.2						
" (with D.D)	16.9	-	-	-	-	-	-	-	-	-		8.50	14.4						
" (with L.D.C)	-	-	-	-	-	-	-	-	-	-		8.50							
" (with L.I.C and L.D.C)	-	-	-	-	-	-	-	-	-	-		8.88							
Community road (4.0m) (with M.I.C)	3.2	-	-	-	-	-	-	-	-	-		-	-	6.75	2.2				7.5
" (with L.I.C)	28.2	-	-	-	-	-	-	-	-	-		-	-	6.38	18.0				
" (with D.D)	11.55	-	-	-	-	-	-	-	-	-		-	-	6.50	7.5				
" (with L.I.C and L.D.C)	11.55	-	-	-	-	-	-	-	-	-		-	-	6.88	8.0				
Leading drainage canal (with O.F.R)	15.20	-	-	-	-	-	-	-	-	-		-	-	-	-	3.6x2	10.9		
Existing Provincial road (with M.I.C)	-	-	-	-	-	-	-	-	-	-	11.0	-	-	-	-	-	-	-	10.1
" (with L.I.C)	-	-	-	-	-	-	-	-	-	-	10.5	-	-	-	-	-	-	-	
"	-	-	-	-	-	-	-	-	-	-	11.0	-	-	-	-	-	-	-	
Proposed provincial road (with M.I.C and M.D.C)																			
"	2.0	-	-	-	-	-	-	-	-	-	12.0	2.4	-	-	-	-	-	-	8.1
" (with M.D.C)	0.5	-	-	-	-	-	-	-	-	-	11.5	0.6	-	-	-	-	-	-	
" (with M.I.C)	1.3	-	-	-	-	-	-	-	-	-	11.0	1.4	-	-	-	-	-	-	10.1
Existing Dike (with L.D.C)	4.6	10.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Existing Road (with L.I.C)	3.0	-	-	-	-	-	-	-	-	-	-	9.25	2.8	-	-	-	-	-	
<b>Total</b>		<b>(44.0)</b>	<b>62.3</b>		<b>(15.3)</b>	<b>31.9</b>	<b>(4.5)</b>	<b>10.1</b>	<b>(19.6)</b>	<b>34.5</b>	<b>(3.8)</b>	<b>4.4</b>	<b>(24.9)</b>	<b>21.4</b>	<b>(54.5)</b>	<b>35.7</b>	<b>(30.4)</b>	<b>10.9</b>	<b>(16.5)</b>

Note: ( ) shows the total length of facilities

- U.W : Unit width (m)
- R.A : Required area (ha)
- M.I.C : Main irrigation canal
- L.I.C : Lateral irrigation canal
- M.D.C : Main drainage canal
- L.D.C : Leading drainage canal
- D.D : Drainage ditch
- O.F.R : On-farm road

Length and Area of Bed Width for Proposed Facilities (Block C)

Borrow pit				Provincial road		Main road		Community road		On-farm road		Main irrigation canal		Lateral irrigation canal		Main drainage canal		Leading drainage canal		Drainage ditch		Total		Number of Unit Cost
M.D.C		L.D.C		U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	U.W	R.A	
km	ha	km	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	m/m	ha	
0.5	1.3			-	-	-	-	-	-	-	-	14.50	10.1	-	-	-	-	-	-	-	-	57.85	40.5	D-1, MIC-1
3.0	7.1			-	-	-	-	-	-	-	-	14.50	4.3	-	-	-	-	-	-	-	-	51.35	15.4	D-2, MIC-2
		6.2	12.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	36.65	24.2	D-3
1.0	1.7	13.4	22.5	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	30.05	68.5	D-4
-	-	-	-	-	-	8.38	4.2	-	-	-	-	-	-	4.30	2.1	-	-	-	-	-	-	12.68	6.3	MR-1, LIC-1
-	-	-	-	-	-	8.50	14.4	-	-	-	-	-	-	-	-	-	-	-	3.4x2	11.5	-	15.30	25.9	MR-2, DD-1
-	-	-	-	-	-	8.50	-	-	-	-	-	-	-	-	-	-	-	5.4x2	-	-	-	19.30	-	MR-3, LDC-2
-	-	-	-	-	-	8.88	-	-	-	-	-	-	-	3.30	-	-	-	5.90	-	-	-	18.08	-	MR-4, LDC-3, LIC-3
-	-	-	-	-	-	-	-	6.75	2.2	-	-	7.78	2.5	-	-	-	-	-	-	-	-	14.53	4.7	CR-4, MIC-4
-	-	-	-	-	-	-	-	6.38	18.0	-	-	-	-	4.30	12.1	-	-	-	-	-	-	10.68	30.1	CR-1, LIC-2
-	-	-	-	-	-	-	-	6.50	7.5	-	-	-	-	-	-	-	-	-	-	3.4x2	7.9	13.30	15.4	CR-2, DD-2
-	-	-	-	-	-	-	-	6.88	8.0	-	-	-	-	3.30	3.8	-	-	5.90	6.8	-	-	16.08	18.6	CR-3, LDC-4, LIC-4
-	-	-	-	-	-	-	-	-	-	3.6x2	10.9	-	-	-	-	-	-	7.00	10.7	-	-	14.20	21.6	OFR-1, LDC-1
-	-	-	-	11.0	-	-	-	-	-	-	-	10.10	-	-	-	-	-	-	-	-	-	21.10	-	PR-2, MIC-3
-	-	-	-	10.5	-	-	-	-	-	-	-	-	-	4.10	-	-	-	-	-	-	-	14.60	-	PR-1, LIC-5
-	-	-	-	11.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	11.00	-	PR-5
-	-	-	-	12.0	2.4	-	-	-	-	-	-	8.10	1.6	-	-	12.05	2.4	-	-	-	-	32.15	6.4	PR-3, MIC-5, MDC-1
-	-	-	-	11.5	0.6	-	-	-	-	-	-	-	-	-	-	12.05	0.6	-	-	-	-	23.55	1.2	PR-4, MDC-2
-	-	-	-	11.0	1.4	-	-	-	-	-	-	10.10	1.3	-	-	-	-	-	-	-	-	21.10	2.7	PR-6, MIC-3
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5.0	2.3	-	-	15.00	6.9	D-5, LDC-5
-	-	-	-	-	-	9.25	2.8	-	-	-	-	-	-	4.30	1.3	-	-	-	-	-	-	13.55	4.1	MR-5, LIC-6
(4.5)	10.1	(19.6)	34.5	(3.8)	4.4	(24.9)	21.4	(54.5)	35.7	(30.4)	10.9	(16.50)	19.8	(47.75)	19.3	(2.5)	3.0	(31.35)	19.8	(56.9)	19.4		292.5	

ities

Table A.4-67 Investment Cost of the Project

Description	Foreign Currency		Local Currency		Total	
	(P'000)	(US\$'000)	(P'000)	(US\$'000)	(P'000)	(US\$'000)
1. Civil Works	94,020	( 4,701.0)	177,080	( 8,854.0)	271,100	(13,555.0)
1-1. Preparation	330	( 16.5)	18,580	( 929.0)	18,910	( 94.5)
1-2. Pumping Station	83,590	( 4,179.5)	31,760	( 1,588.0)	115,350	( 5,767.5)
1-3. Dike	1,340	( 67.0)	20,600	( 1,030.0)	21,940	( 1,097.0)
1-4. On-Farm Facilities	8,320	( 416.0)	101,500	( 5,075.0)	109,820	( 5,491.0)
1-5. Provincial Road	440	( 22.0)	4,640	( 232.0)	5,080	( 254.0)
2. Construction Equipment	133,100	( 6,655.0)	1,300	( 65.0)	134,400	( 6,720.0)
3. Agricultural Development	-	-	14,860	( 743.0)	14,860	( 743.0)
4. Project Administration & Facilities	1,280	( 64.0)	21,460	( 1,073.0)	22,740	( 1,137.0)
5. Consulting Services	22,700	( 1,135.0)	8,620	( 431.0)	31,320	( 1,566.0)
6. Land Acquisition	-	-	3,640	( 182.0)	3,640	( 182.0)
Sub-total (1 to 6)	251,100	(12,555.0)	226,960	(11,348.0)	478,060	(23,903.0)
7. Contingency	37,665	( 1,863.2)	34,045	( 1,702.3)	71,710	( 3,585.5)
Total	288,765	(14,438.2)	261,005	(13,050.3)	549,770	(27,488.5)
8. Price Escalation	82,269	( 4,113.5)	91,846	( 4,592.3)	174,115	( 8,705.8)
Grand Total	371,034	(18,551.7)	352,851	(17,642.6)	723,885	(36,194.3)

Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (P)	L.C. (P)	F.C. (P1,000)	L.C. (P1,000)	
<u>1. Civil Works</u>								
<u>1-1. Preparation</u>								
1-1-1.	Pumping Station							
	Dry works by pump $\phi 4"$	1,620	day	20	80	32.40	129.60	Pr-1 <sup>a/</sup>
	Setting & withdrawing of a concrete plant 0.5 m <sup>3</sup> .	3	L.S	2,000	23,000	6.00	69.00	Pr-2
	Retaining wall	90	m	200	290	18.00	26.10	Pr-3
	Temporary road	150	m	4	101	0.60	15.15	12
	Foothold	17,400	m <sup>3</sup>	-	40	-	696.00	Pr-4
	Curing concrete	11,600	m <sup>2</sup>	-	10	-	116.00	Pr-5
	Sub-total					57.00	1,051.85	
						≈ 60	1,050	
<u>1-1-2. Dike</u>								
	Temporary pier	16.0	m	-	6,000	-	960.00	Pr-6
	Foothold	7,000	m <sup>3</sup>	-	40	-	280.00	Pr-4
	Curing concrete	2,200	m <sup>2</sup>	-	10	-	22.00	Pr-5
	Temporary road	200	m	4	101	0.80	20.20	12
	Sub-total					0.80	1,282.20	
						≈ -	1,280	
<u>1-1-3. On-Farm</u>								
	Backfill of canal	116.0	km	1,880	18,750	218.08	2,175.00	Pr-7
	Sheet-piling cofferdam	1,400	m	-	600	-	840.00	Pr-8
	Temporary road	300	m	4	101	1.20	30.30	12
	Dry works by pump $\phi 4"$	2,400	day	20	80	48.00	192.00	Pr-1
	Foothold	9,500	m <sup>2</sup>	-	40	-	380.00	Pr-4
	Curing concrete	3,000	m <sup>2</sup>	-	10	-	30.00	Pr-5
	Sub-total					267.28	3,647.30	
						≈ 270	3,650	

Note: a/ : Number of unit cost (see Appendix 4-64)



Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (₪)	L.C. (₪)	F.C. (₪1,000)	L.C. (₪1,000)	
1-1-4.	Provincial Road							
	Temporary pier	210	m <sup>3</sup>	-	6,000	-	1,260.00	Pp-6
	Foothold	9,300	m <sup>2</sup>	-	40	-	372.00	Pp-4
	Curing concrete	3,000	m <sup>2</sup>	-	10	-	30.00	Pp-5
	Temporary road	150	m	4	101	0.60	15.15	12
	Sub-total					0.60	1,677.15	
							1,680	
1-1-5.	Other preparation							
	Field Office	1,000	m <sup>2</sup>	-	3,000	-	3,000.00	Pp-9
	Motor pool	5,000	m <sup>2</sup>	-	20	-	100.00	Pp-10
	Indirect labor cost	L.S		-		-	4,050.00	L.C x 3%
	Other	L.S		-		-	50.00	
	Sub-total						7,200.00	
							7,200	
1-1-6.	Overhead	L.S					3,720	L.C x 25%
	Total					330	18,580	
1-2.	Pumping Station							
1-2-1.	Block A							
	Pump φ1,350 mm	3	set			21,600	2,700	1-2-1-1 <sup>b</sup> / <sub>1</sub>
	Earthwork & other	L.S				800	4,400	L.C x 25%
	Overhead	L.S					1,780	
	Sub-total					22,400	8,880	

Note: <sup>b</sup>/<sub>1</sub> ; Number of construction cost of works (see Appendix 4-63)

Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (B)	L.C. (B)	F.C. (B1,000)	L.C. (B1,000)	
1-2-2.	Block B							
	Pump φ1,350 mm	3	set			21,600	2,700	
	Earthwork & other	L.S				800	4,400	1-2-2-1
	Overhead	L.S				-	1,780	L.C x 25%
	Sub-total					22,400	8,880	
1-2-3.	Block C							
	Pump φ1,650 mm	3	set			33,400	3,600	
	Earthwork & other	L.S				960	5,510	1-2-3-1
	Overhead	L.S				-	2,280	L.C x 25%
	Sub-total					34,360	11,390	
1-2-4.	Electric facility							
		L.S				4,430	2,610	
	Sub-total					4,430	2,610	
	Total					83,590	31,760	
1-3.	Dike							
1-3-1.	Block A							
	Dike, Type D-2	7,500	m	10	120	75.00	900.00	
	Dike, Type D-4	28,900	m	10	130	299.00	3,887.00	
	Bridge, Type Br-4	2	unit	50,200	204,500	100.40	409.00	
	Overhead	L.S				-	1,299.00	L.C x 25%
	Sub-total					474.40	6,495.00	
						480	6,490	
1-3-2.	Block B							
	Dike, Type D-2	9,450	m	10	120	94.50	1,134.00	
	Dike, Type D-4	22,400	m	10	130	224.00	2,912.00	

Item	Description	Quantity	Unit	Rate		Amount		Remarks	
				F.C. (£)	L.C. (£)	F.C. (£1,000)	L.C. (£1,000)		
	Dike, Type D-5	1,200	m	1	46	1.20	55.20		
	Overhead	L.S				-	1,025.00	L.C x 25%	
	Sub-total					319.70	5,126.20		
						= 320	5,130		
1-3-3. Block C									
	Dike, Type D-1	7,000	m	10	220	70.00	1,540.00		
	Dike, Type D-2	3,000	m	10	120	30.00	360.00		
	Dike, Type D-3	6,600	m	10	230	66.00	1,518.00		
	Dike, Type D-4	22,800	m	10	130	228.00	2,964.00		
	Dike, Type D-5	4,600	m	1	46	4.60	211.60		
	Bridge, Type Br-2	1	unit	93,600	386,300	93.60	386.30		
	Bridge, Type Br-4	1	unit	50,200	204,500	50.20	204.50		
	Overhead	L.S				-	1,796.00	L.C x 25%	
	Sub-total					542.40	8,980.40		
						= 540	8,980		
	Total					1,340	20,600		
1-4. On-Farm									
1-4-1. Block A									
	Survey	3,027	ha	-	60	-	181.62		
	Land clearing	3,027	ha	20	240	60.54	726.48	On-1	
	Land leveling	3,027	ha	480	3,740	1,452.96	11,320.98	On-2	
	Road	L.S				180.19	2,769.2	1-4-1-1	
	Canal & ditch	L.S				241.20	3,168.90	1-4-1-2	
	Terminal facilities	L.S				557.70	5,638.47	1-4-1-3	
	Overhead	L.S				-	5,951.00	L.C x 25%	
	Sub-total					2,492.59	29,756.05		
						= 2,490	29,760		

Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (B)	L.C. (B)	F.C. (B1,000)	L.C. (B1,000)	
<b>1-4-2. Block B</b>								
	Survey	2,583	ha	-	60	-	154.98	
	Land clearing	2,583	ha	20	240	51.66	619.92	On-1
	Land leveling	2,583	ha	480	3,740	1,239.84	9,660.42	On-2
	Road	L.S.				142.12	2,519.90	1-4-2-1
	Canal & ditch	L.S.				242.75	2,988.05	1-4-2-2
	Terminal facilities	L.S.				524.48	5,074.82	1-4-2-3
	Overhead	L.S.					5,254.00	L.C X 25%
	Sub-total					2,200.85	26,272.09	
						= 2,200	26,270	
<b>1-4-3. Block C</b>								
	Survey	4,932	ha	-	60	-	295.92	
	Land clearing	4,932	ha	20	240	98.64	1,183.68	On-1
	Land leveling	4,932	ha	480	3,740	2,367.36	18,445.68	On-2
	Road	L.S.				288.28	4,500.20	1-4-3-1
	Canal & ditch	L.S.				396.46	5,115.88	1-4-3-2
	Terminal facilities	L.S.				479.62	6,837.59	1-4-3-3
	Overhead	L.S.					9,094.00	L.C X 25%
	Sub-total					3,630.36	45,472.95	
						= 3,630	45,470	
	Total					8,920	101,500	
<b>1-5. Provincial Road</b>								
<b>1-5-1. Block A</b>								
	Road, Type PR-1	4,150	m	4	102	15.60	428.30	
	Overhead	L.S.					105.00	L.C X 25%
	Sub-total					16.60	528.30	
						= 20	530	

Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (₹)	L.C. (₹)	F.C. (₹1,000)	L.C. (₹1,000)	
1-5-2. Block B								
	Road, Type PR-1	5,450	m	4	102	21.80	555.90	
	Road, Type PR-2	2,650	m	4	103	10.50	272.95	
	Road, Type PR-5	6,500	m	4	103	26.00	669.50	
	Bridge, Type Br-1	1	unit	164,800	633,600	164.80	633.60	
	Bridge, Type Br-2	2	unit	93,500	386,300	187.20	772.00	
	Overhead	L.S				-	726.00	L.C x 25%
	Sub-total					410.40	3,630.55	
						₹ 410	3,630	
1-5-3. Block C								
	Road, Type PR-3	2,000	m	3	94	6.00	198.00	
	Road, Type PR-4	500	m	3	92	1.50	46.00	
	Road, Type PR-6	1,300	m	6	119	7.80	154.70	
	Overhead	L.S				-	97.00	L.C x 25%
	Sub-total					15.30	485.70	
						₹ 10	480	
	Total					440	4,640	

2. Construction Equipment

Equipments	Specification	No.	Unit		Amount	Remarks
			Price			
					(Unit: ¥'000)	
Bulldozer	15t 140ps	18	1,000		18,000	
Bulldozer	16t 140ps	11	1,050		11,550	Swampy type
Bulldozer	21t 200ps	11	1,600		17,600	
Backhoe	0.3m <sup>3</sup> 60ps	7	730		5,110	
Backhoe	0.6m <sup>3</sup> 100ps	5	1,450		7,250	
Dragline	0.6m <sup>3</sup>	2	90		180	Backhoe attachment
Fronted loader	1.6m <sup>3</sup> 125ps	1	770		770	
Scrape-dozer	6.4m <sup>3</sup> 183ps	22	1,850		40,700	
Motor grader	3.7m 125ps	1	1,150		1,150	
Tire roller	10t 40ps	6	350		2,100	
Dump truck	8t 196ps	3	310		930	
Water tank truck	6t	1	230		230	
Fuel truck	8t	2	380		760	
Agitator truck	1.6m <sup>3</sup> 135ps	2	250		500	
Agitator truck	3.0m <sup>3</sup> 195ps	2	420		840	
Mixing pland	0.5m <sup>3</sup> 30ps	1	580		580	
Pot mixer	0.3m <sup>3</sup> 7.5ps	3	60		180	
Diesel hammer	2.2t	1	690		690	Backhoe attachment
Diesel hammer	1.25t	1	410		410	Backhoe attachment
Vibrator	2.5ps	10	11		110	
Rammer	100kg	10	27		270	
Belt conveyor	7m 3ps	5	9		45	
Concrete conveyor	10m 3.5ps	2	90		180	
Pump	φ 4" 7ps	10	15		150	
Jeep		5	120		600	
Motor cycle	90cc	5	12		60	
Sub-total					110,945	
Spare parts		L.S.			22,155	20%
Transportation		L.S.			1,300	1%
Total					134,400	

Note: Required Numbers of construction equipments are referred to Appendix 4-65.

3. Agricultural Development

Description	Quantity	Unit	Rate		Amount		Remarks
			F.C. (B)	L.C. (B)	F.C. (B1,000)	L.C. (B1,000)	
AIRC Office	L.S				2,000		
Production unit Office	L.S				1,200		
Training center	L.S				2,000		
Seed center	L.S				2,000		
Motor pool	L.S				1,225		
Spare machine	L.S				1,461		
Assemble house & ware house	L.S				2,000		
Truck & motor bicycle	L.S				1,576		
Water supply	L.S				1,400		
Total					14,862		
					≐ 14,860		

4. Project Administration & Facilities

Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (B)	L.C. (E)	F.C. (B1,000)	L.C. (E1,000)	
<u>4-1. Pre-Engineering</u>								
	Surveying works							
	Topographical survey	10,000	ha		500		5,000	
	Rout surveying	153.05	km		5,000		762.25	
	Plain survey	L.S					400	
	Core-boring	L.S					300	
	Sub-total						6,462.25	
							6,470	
<u>4-2. Project Facilities</u>								
	Building	600	m <sup>2</sup>		3,000		1,800	
	Furniture	L.S					540	
	Office equipments	L.S					1,280	
	Sub-total						1,280	
							2,340	
<u>4-3. Overhead (Staff of project office)</u>								
Administration Division								
	University grad	2 x 72	man-month		4,000		576	
	Collage grad	3 x 72	man-month		3,000		648	
	High school grad	6 x 72	man-month		2,000		864	
	Others	5 x 72	man-month		1,500		540	
Engineering Division								
	University grad	5 x 12, 10 x 60	man-month		4,000		2,640	
	Collage grad	5 x 12, 10 x 60	man-month		3,000		1,980	
	High school grad	5 x 12, 20 x 60	man-month		2,000		2,520	
	Others	10 x 12, 30 x 60	man-month		1,500		2,880	
	Sub-total						12,648	
	Total						12,650	
							1,280	
							21,460	
Note:	1/ Rout surveying						114.45 km	
							36.1	
							2.5	
							153.05 km	



5. Consulting Services

Item	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (£)	L.C. (£)	F.C. (£1,000)	L.C. (£1,000)	
<u>5-1. Foreign Exchange Cost</u>								
	Final Design							
	Consultant remuneration	62	man-month	120,000		7,440		
	International travel expense	12	trip	15,500		186		
	Miscellaneous & communication	L.S				67		
	Sub-total					7,693		
						£ 7,690		
<u>Construction Supervision</u>								
	Consultant remuneration	120	man-month	120,000		14,400		
	International travel expense	18	trip	15,500		279		
	Miscellaneous & communication	L.S				333		
	Sub-total					15,012		
						£ 15,010		
<u>5-2. Local Currency Cost</u>								
	To be borne by the Government:							
	Local transportation, office accommodation and consultants per diem							
	Final Design							1,440
	Construction supervision							7,180
	Sub-total							8,620
	Total					22,700		8,620

6. Land Acquisition

<u>Description</u>	<u>Quantity</u>	<u>Unit</u>	<u>Rate</u>		<u>Amount</u>		<u>Remarks</u>
			<u>F.C. (P)</u>	<u>L.C. (P)</u>	<u>F.C. (P1,000)</u>	<u>L.C. (P1,000)</u>	
Block A Area of Dike	73.2	ha		15,600		1,141.92	
Block B Area of Dike	65.6	ha		15,600		1,023.36	
Block C Area of Dike	94.4	ha		15,600		1,472.64	
<b>Total</b>						<u>3,637.92</u>	
						<u>≠ 3,640</u>	

Table A.4-68 Construction Cost of Works

Earth Work & Other (pumping station)

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (₹)	L.C. (₹)	F.C. (₹1,000)	L.C. (₹1,000)	
1-2-1-1.	Block A							
(a)	Public Works							
	Earth works							
	Excavation	12,900	m <sup>3</sup>	1.0	5.9	12.90	76.11	EE-8
	Embankment	2,700	m <sup>3</sup>	0.7	10.2	1.89	27.54	EB-4
	Removal of excess soil	9,980	m <sup>3</sup>	0.2	1.1	1.986	10.923	EF-1
	Concrete works							
	Concrete	2,200	m <sup>3</sup>	70	450	154.00	990.00	CO-9
	Form works	3,700	m <sup>2</sup>	-	110	-	407.00	CO-12
	Reinforcement bar	160,000	kg	3.9	3.0	624.00	480.00	CO-2
	Foundation works							
	Pile 220 x 220 m/m	545	pile	10	1,070	5.45	583.15	CP-1
	Pipe work							
	Pipe φ700 mm	120	m	-	550	-	66.00	Pr-11
	Other	L.S					344.00	(F.C+L.C)x10%
	Sub-total					800.226	2,984.723	
(b)	Pumping Station							
	Building	312.5	m	-	3,000	-	937.50	Pr-9
	Sub-total					-	937.50	
(c)	Gate Works							
	Roller-gate 4.0 <sup>m</sup> x 4.0 <sup>m</sup>	1	gate	-	134,400	-	134.40	
	Roller-gate 4.0 <sup>m</sup> x 2.0 <sup>m</sup>	1	gate	-	67,200	-	67.20	
	Slush-gate 2.0 <sup>m</sup> x 1.6 <sup>m</sup>	2	gate	-	42,500	-	85.00	

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (B)	L.C. (B)	F.C. (B1,000)	L.C. (B1,000)	
	Slush-gate $\phi$ 700 mm	2	gate	-	4,400	-	8.80	
	Screen	90	m <sup>2</sup>	-	2,000	-	180.00	
	Sub-total						475.40	
	Total					800.226	4,397.623	
						= 800	4,400	same to
1-2-2-1.	Block B		L.S			800	4,400	Block A
	Total					800	4,400	
1-2-3-1.	Block C							
(a)	Public Works							
	Earth works							
	Excavation	16,300	m <sup>3</sup>	1.0	5.9	16.30	96.17	EE-8
	Embankment	6,000	m <sup>3</sup>	0.7	10.2	4.20	61.20	EB-4
	Removal of excess soil	9,700	m <sup>3</sup>	0.2	1.1	1.94	10.67	ET-1
	Concrete works							
	Concrete	2,700	m <sup>3</sup>	70	450	189.00	1,215.00	CO-9
	Form	4,200	m <sup>2</sup>	-	110	-	462.00	CO-12
	Reinforcement bar	190,000	kg	3.9	3.0	741.00	570.00	CO-2
	Foundation work							
	File 220 x 220 mm	669	pile	10	1,070	6.69	715.83	CP-1
	Pipe work							
	Pipe $\phi$ 700 mm	120	m	-	550	-	66.00	Pr-11
	Other						415.00	(F.C+L.C)x10%
	Sub-total					959.13	3,611.87	
(b)	Pumping Station Building	420	m <sup>2</sup>	-	3,000	-	1,260.00	Pr-9
	Sub-total					-	1,260.00	

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (₹)	L.C. (₹)	F.C. (₹1,000)	L.C. (₹1,000)	
(c)	Gate Works							
	Roller-gate 5.0 <sup>m</sup> x 5.0 <sup>m</sup>	1	gate	-	186,000	-	186.00	
	Roller-gate 5.0 <sup>m</sup> x 2.0 <sup>m</sup>	1	gate	-	93,600	-	93.60	
	Slush-gate 3.0 <sup>m</sup> x 1.6 <sup>m</sup>	2	gate	-	57,200	-	114.40	
	Slush-gate φ700 mm	2	gate	-	4,400	-	8.80	
	Screen	120	m <sup>2</sup>	-	2,000	-	240.00	
	Sub-total						642.80	
	Total				959.13	5,514.67		
					= 960	5,510		

Road (On-farm)

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (₹)	L.C. (₹)	F.C. (₹1,000)	L.C. (₹1,000)	
1-4-1-1.	Block A							
	Main Road, Type MR-1	2,750	m	4	101	11	277.75	
	Main Road, Type MR-2	10,450	m	2	83	20.9	867.35	
	Main Road, Type MR-4	1,300	m	2	95	2.6	123.50	
	Community Road, Type CR-1	25,050	m	2	19	50.1	475.95	
	Community Road, Type CR-2	6,550	m	-	5	-	32.75	
	Community Road, Type CR-3	1,000	m	1	7	1	7	
	On Farm Road, Type OFR-1	19,500	m	-	2	-	39.00	m/ha
	On Farm Road, Type On-3	94,590	m	1	10	94.59	945.90	31.25 x 3,027
	Total					180.19	2,769.2	
1-4-2-1.	Block B							
	Main Road, Type MR-1	2,400	m	4	101	9.6	242.40	
	Main Road, Type MR-2	8,300	m	2	83	16.6	688.90	
	Main Road, Type MR-3	5,600	m	2	90	11.2	504.00	
	Community Road, Type CR-1	12,000	m	2	19	24.00	228.00	
	On-Farm Road, Type OFR-1	24,700	m	-	2	-	49.40	m/ha
	On-Farm Road, Type On-3	80,720	m	1	10	80.72	807.20	31.25 x 2,583
	Total					142.12	2,519.90	

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (£)	L.C. (£)	F.C. (£1,000)	L.C. (£1,000)	
1-4-3-1. Block C								
	Main Road, Type MR-1	5,000	m	4	101	20	505	
	Main Road, Type MR-2	16,900	m	2	83	33.80	1,402.70	
	Main Road, Type MR-5	3,000	m	2	84	6.00	252.00	
	Community Road, Type CR-1	28,200	m	2	19	56.40	535.80	
	Community Road, Type CR-2	11,550	m	-	5	-	57.75	
	Community Road, Type CR-3	11,550	m	1	7	11.55	80.85	
	Community Road, Type CR-4	3,200	m	2	20	6.40	64.00	
	On-Farm Road, Type OFR-1	30,400	m	-	2	-	60.80	
	On-Farm Road, Type On-3	154,130	m	1	10	154.13	1,541.30	31.25 <sup>m/ha</sup> 4,932
Total						288.28	4,500.20	

Canal & Ditch (On-Farm)

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (£)	L.C. (£)	F.C. (£1,000)	L.C. (£1,000)	
1-4-1-2. Block A								
	Main Irrigation Canal, Type MIC-2	7,500	m	10	100	75	750	
	Lateral Irrigation Canal, Type LIC-1	2,750	m	-	15	-	41.25	
	Lateral Irrigation Canal, Type LIC-2	25,050	m	-	15	-	375.75	
	Lateral Irrigation Canal, Type LIC-3	1,300	m	-	15	-	19.50	
	Lateral Irrigation Canal, Type LIC-4	1,000	m	-	15	-	15.00	
	Lateral Irrigation Canal, Type LIC-5	4,150	m	1	24	4.15	99.60	
	Leading Drainage, Canal Type LDC-1	9,750	m	4	41	39	399.75	
	Leading Drainage, Type LDC-2	5,600	m	2	25	11.20	140.00	
	Leading Drainage, Type LDC-3	1,300	m	3	29	3.90	37.70	
	Leading Drainage, Type LDC-4	1,000	m	3	28	3	28.00	
	Drainage Ditch, Type DD-1	20,900	m	1	10	20.90	209.00	
	Drainage Ditch, Type DD-2	13,100	m	1	10	13.10	131.00	
	Drainage Ditch, Type On-4	70,950	m	1	13	70.95	922.35	23.44 <sup>m/ha</sup> 3,027
Total						241.20	3,168.90	

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (₹)	L.C. (₹)	F.C. (₹1,000)	L.C. (₹1,000)	
1-4-2-2. Block B								
	Main Irrigation Canal, Type MIC-2	9,450	m	10	100	94.50	945.00	
	Main Irrigation Canal, Type MIC-3	2,650	m	1	27	2.65	71.55	
	Lateral Irrigation Canal, Type LIC-1	2,400	m	-	15	-	36.00	
	Lateral Irrigation Canal, Type LIC-2	12,000	m	-	15	-	180.00	
	Lateral Irrigation Canal, Type LIC-5	5,450	m	1	24	5.45	130.80	
	Leading Drainage canal, Type LDC-1	12,350	m	4	41	49.40	506.35	
	Leading Drainage, Type LDC-2	5,600	m	2	25	11.20	140.00	
	Leading Drainage, Type LDC-5	1,200	m	2	21	2.40	25.20	
	Drainage Ditch, Type DD-1	16,600	m	1	10	16.60	166.00	23.44 m/ha X
	Drainage Ditch, Type On-4	60,550	m	1	13	60.55	787.15	23.44 m/ha X 2,583
	Total					<u>242.75</u>	<u>2,988.05</u>	
1-4-3-2. Block C								
	Main Irrigation Canal, Type MIC-1	7,000	m	10	100	70	700	
	Main Irrigation Canal, Type MIC-2	3,000	m	10	100	30	300	
	Main Irrigation Canal, Type MIC-4	3,200	m	-	17	-	54.40	
	Main Irrigation Canal, Type MIC-5	2,000	m	1	19	2	38.00	
	Main Irrigation Canal, Type MIC-3	1,300	m	1	27	1.30	35.10	
	Lateral Irrigation Canal, Type LIC-1	5,000	m	-	15	-	75.00	
	Lateral Irrigation Canal, Type LIC-2	28,200	m	-	15	-	423.00	
	Lateral Irrigation Canal, Type LIC-4	11,550	m	-	15	-	173.25	
	Lateral Irrigation Canal, Type LIC-6	3,000	m	-	15	-	45.00	
	Leading Drainage canal, Type LDC-1	15,200	m	4	41	60.8	623.20	
	Leading Drainage, Type LDC-4	11,550	m	3	28	34.65	323.40	
	Leading Drainage, Type LDC-5	4,600	m	2	21	9.20	96.60	
	Main Drainage canal, Type MDC-1	2,000	m	6	58	12.00	116.00	
	Main Drainage, Type MDC-2	500	m	8	82	4.00	41.00	
	Drainage Ditch, Type DD-1	33,800	m	1	10	33.80	338.00	
	Drainage Ditch, Type DD-2	23,100	m	1	10	23.10	231.00	23.44 m/ha X
	Drainage Ditch, Type On-4	115,610	m	1	13	115.61	1,502.93	23.44 m/ha X 4,932
	Total					<u>396.46</u>	<u>5,115.88</u>	

<u>Terminal Facilities (On-farm)</u>		<u>Quantity</u>	<u>Unit</u>	<u>Rate</u>		<u>Amount</u>		<u>Remarks</u>
<u>No.</u>	<u>Description</u>			<u>F.C.</u>	<u>L.C.</u>	<u>F.C.</u>	<u>L.C.</u>	
				<u>(₹)</u>	<u>(₹)</u>	<u>(₹1,000)</u>	<u>(₹1,000)</u>	
1-4-1-3.	Block A							
	Diversion box	18	unit	1,440	39,410	25.92	709.38	
	Check, Type CH-2	2	"	3,130	41,850	6.26	83.70	
	Check, Type CH-3	-	"	3,110	38,210	-	-	
	Culvert, Type A-1	1	"	36,900	242,900	36.90	242.90	
	Culvert, Type A-2	1	"	35,300	241,700	35.30	241.70	
	Culvert, Type B-2	-	"	3,130	29,050	-	-	
	Culvert, Type B-3	4	"	3,110	25,410	12.44	101.64	
	Culvert, Type C-1	-	"	590	10,090	-	-	
	Culvert, Type C-2	11	"	590	9,610	6.49	105.71	
	Culvert, Type C-3	6	"	580	9,120	3.48	54.72	
	Culvert, Type D-2	2	"	5,080	60,260	10.15	120.52	
	Culvert, Type D-3	8	"	5,030	48,460	40.24	387.68	
	Culvert, Type E-1	4	"	1,450	18,490	5.80	73.96	
	Culvert, Type E-2	12	"	1,430	16,600	17.16	199.20	
	Culvert, Type E-3	8	"	1,410	14,710	11.28	117.68	
	Culvert, Type F-1	1	"	142,200	595,000	142.20	595.00	
	Culvert, Type F-2	1	"	45,900	200,300	45.90	200.30	
	Bridge, Type Br-3	2	"	70,200	278,700	140.40	557.40	
	Turn out	158	"	100	560	15.8	88.48	4 unit/76.8 ha x 3,027 ha
	Farm inlet ø200 m/m	946	"	-	220	-	208.12	24 unit/76.8 ha x 3,027 ha
	Culvert (2) ø500 m/m Type-1	158	"	-	1,330	-	210.14	4 unit/76.8 ha x 3,027 ha
	Culvert (3) ø300 m/m Type-2	158	"	-	580	-	91.64	4 unit/76.8 ha x 3,027 ha
	Culvert (4) ø800 m/m Type-3	197	"	10	3,480	1.97	685.56	5 unit/76.8 ha x 3,027 ha
	Culvert (1) ø300 m/m Type-4	1,104	"	-	510	-	563.04	28 unit/76.8 ha x 3,027 ha
	Total					557.70	5,638.47	



No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (B)	L.C. (B)	F.C. (B1,000)	L.C. (B1,000)	
1-4-2-3. Block B								
	Division box	10	unit	1,440	39,410	14.40	394.10	
	Check, Type CH-1	-	"	3,150	45,490	-	-	
	Check, Type CH-2	2	"	3,180	41,850	6.26	83.70	
	Check, Type CH-3	-	"	3,110	38,210	-	-	
	Culvert, Type A-3	1	"	36,300	249,600	35.30	249.60	
	Culvert, Type A-4	1	"	46,900	328,700	46.90	328.70	
	Culvert, Type B-1	2	"	3,150	32,690	6.30	65.38	
	Culvert, Type B-2	-	"	3,130	29,050	-	-	
	Culvert, Type B-3	3	"	3,110	25,410	9.33	76.23	
	Culvert, Type C-1	5	"	590	10,090	2.95	50.45	
	Culvert, Type C-2	7	"	590	9,610	4.13	67.27	
	Culvert, Type C-3	2	"	580	9,120	1.16	18.24	
	Culvert, Type D-1	3	"	5,140	72,050	15.42	216.15	
	Culvert, Type D-2	2	"	5,080	60,260	10.16	120.52	
	Culvert, Type D-3	6	"	5,030	48,460	30.18	290.76	
	Culvert, Type E-1	6	"	1,450	18,490	8.70	110.94	
	Culvert, Type E-2	9	"	1,430	16,600	12.87	149.40	
	Culvert, Type E-3	3	"	1,410	14,710	4.23	44.13	
	Culvert, Type F-3	1	"	56,800	247,200	56.80	247.20	
	Culvert, Type F-4	1	"	102,800	424,900	102.80	424.90	
	Bridge, Type Br-3	2	"	70,200	278,700	140.40	557.40	
	Turn out	135	"	100	560	13.50	75.60	4 unit/76.8ha
	Farm inlet	808	"	-	220	-	177.76	24 unit/76.8ha
	Culvert (2), Type-1	135	"	-	1,330	-	179.55	4 unit/76.8ha
	Culvert (3), Type-2	135	"	-	580	-	78.90	4 unit/76.8ha
	Culvert (4), Type-3	169	"	10	3,480	1.69	588.12	5 unit/76.8ha
	Culvert (1), Type-4	942	"	-	510	-	480.42	28 unit/76.8ha
	<b>Total</b>					<b>524.48</b>	<b>5,074.82</b>	

No.	Description	Quantity	Unit	Rate		Amount		Remarks
				F.C. (£)	L.C. (£)	F.C. (£1,000)	L.C. (£1,000)	
1-4-3-3.	Block C							
	Diversion box	19	unit	1,440	39,410	27.36	748.79	
	Check, Type CH-1	-	"	3,150	45,490	-	-	
	Check, Type CH-2	4	"	3,130	41,850	12.52	167.40	
	Check, Type CH-3	-	"	3,110	38,210	-	-	
	Culvert, Type A-5	1	"	65,000	385,500	65.00	385.50	
	Culvert, Type B-1	1	"	3,150	32,690	3.15	32.69	
	Culvert, Type B-2	-	"	3,130	29,050	-	-	
	Culvert, Type B-3	10	"	3,110	25,410	31.10	254.10	
	Culvert, Type C-1	-	"	590	10,090	-	-	
	Culvert, Type C-2	11	"	590	9,610	6.49	105.71	
	Culvert, Type C-3	13	"	580	9,120	7.54	118.56	
	Culvert, Type D-1	1	"	5,140	72,050	5.14	72.05	
	Culvert, Type D-2	5	"	5,080	60,260	25.40	301.30	
	Culvert, Type D-3	8	"	5,030	48,460	40.24	387.68	
	Culvert, Type E-1	4	"	1,450	18,490	5.80	73.96	
	Culvert, Type E-2	14	"	1,430	16,600	20.02	232.40	
	Culvert, Type E-3	15	"	1,410	14,710	21.15	220.65	
	Culvert, Type F-5	1	"	109,600	449,500	109.60	449.50	
	Bridge, Type Br-3	1	"	70,200	278,700	70.20	278.70	
	Turn out	257	"	100	560	25.70	143.92	4 unit/76.8 ha, 4, 932 ha
	Farm inlet	1,542	"	-	220	-	339.24	24 unit/76.8 ha, 4, 932 ha
	Culvert (2), Type-1	257	"	-	1,330	-	341.81	4 unit/76.8 ha, 4, 932 ha
	Culvert (3), Type-2	257	"	-	580	-	149.06	4 unit/76.8 ha, 4, 932 ha
	Culvert (4), Type-3	321	"	10	3,480	3.21	1,117.08	5 unit/76.8 ha, 4, 932 ha
	Culvert (1), Type-4	1,799	"	-	510	-	917.49	28 unit/76.8 ha, 4, 932 ha
	Total					479.62	6,837.59	

Table A.4-69 Unit Cost

## 1. Unit Cost of Major Structures

No.	Description	Unit	Unit Cost			Remarks
			F.C. (P)	L.C. (P)	Total (P)	
1.	Dike, Type D-1	m	10	220	230	B=8.00m
2.	" Type D-2	"	10	120	130	B=4.00m
3.	" Type D-3	m	10	230	240	B=8.00m
4.	" Type D-4	m	10	130	140	B=4.00m
5.	" Type D-5	m	1	46	47	
6.	Provincial Road, Type PR-1	m	4	102	106	
7.	" Type PR-2	m	4	103	107	
8.	" Type PR-3	m	3	94	97	
9.	" Type PR-4	m	3	92	95	
10.	" Type PR-5	m	4	103	107	
11.	" Type PR-6	m	6	119	125	
12.	Main Road, Type MR-1	m	4	101	105	
13.	" Type MR-2	m	2	83	85	
14.	" Type MR-3	m	2	90	92	
15.	" Type MR-4	m	2	95	97	
16.	" Type MR-5	m	2	84	86	
17.	Community Road, Type CR-1	m	2	19	21	
18.	" Type CR-2	m	-	5	5	
19.	" Type CR-3	m	1	7	8	
20.	" Type CR-4	m	2	20	22	
21.	On-Farm Road, Type OFR-1	m	-	2	2	
22.	Main Irrigation Canal, Type MIC-1	m	10	100	110	
23.	" Type MIC-2	m	10	100	110	
24.	" Type MIC-3	m	1	27	28	
25.	" Type MIC-4	m	-	17	17	
26.	" Type MIC-5	m	1	19	20	
27.	Lateral Irrigation Canal, Type LIC-1	m	-	15	15	
28.	" Type LIC-2	m	-	15	15	
29.	" Type LIC-3	m	-	15	15	
30.	" Type LIC-4	m	-	15	15	
31.	" Type LIC-5	m	1	24	25	
32.	" Type LIC-6	m	-	15	15	
33.	Leading Drainage Canal, Type LDC-1	m	4	41	45	
34.	" Type LDC-2	m	2	25	27	
35.	" Type LDC-3	m	3	29	32	
36.	" Type LDC-4	m	3	28	31	
37.	" Type LDC-5	m	2	21	23	
38.	Main Drainage, Type MDC-1	m	6	58	64	
39.	" Type MDC-2	m	8	82	90	

No.	Description	Unit	F.C. (P)	L.C. (P)	Total (P)	Remarks
40.	Drainage Ditch, Type DD-1	m	1	10	11	
41.	" Type DD-2	m	1	10	11	
42.	Bridge, Type Br-1	L.S.	164,800	633,600	798,400	B=8.00m
43.	" Type Br-2	"	93,600	386,300	479,900	B=8.00m
44.	" Type Br-3	"	70,200	278,700	348,900	B=6.00m
45.	" Type Br-4	"	50,200	204,500	254,700	B=4.00m
46.	Diversion box	L.S.	1,440	39,410	40,850	
47.	Check, Type CH-1	L.S.	3,150	45,490	48,640	
48.	" Type CH-2	"	3,130	41,850	44,980	
49.	" Type CH-3	"	3,110	38,210	41,320	
50.	Culvert, Type A-1	L.S.	36,900	242,900	279,800	
51.	" Type A-2	"	35,300	241,700	277,000	
52.	" Type A-3	"	36,300	249,600	285,900	
53.	" Type A-4	"	46,900	328,700	375,600	
54.	" Type A-5	"	65,000	385,500	450,500	
55.	" Type B-1	"	3,150	32,690	35,840	
56.	" Type B-2	"	3,130	29,050	32,180	
57.	" Type B-3	"	3,110	25,410	28,520	
58.	" Type C-1	"	590	10,090	10,680	
59.	" Type C-2	"	590	9,610	10,200	
60.	" Type C-3	"	580	9,120	9,700	
61.	" Type D-1	"	5,140	72,050	77,190	
62.	" Type D-2	"	5,080	60,260	65,340	
63.	" Type D-3	"	5,030	48,460	53,490	
64.	" Type E-1	"	1,450	18,490	19,940	
65.	" Type E-2	"	1,430	16,600	18,030	
66.	" Type E-3	"	1,410	14,710	16,120	
67.	" Type F-1	"	142,200	595,000	737,200	
68.	" Type F-2	"	45,900	200,300	246,200	
69.	" Type F-3	"	56,800	247,200	304,000	
70.	" Type F-4	"	102,800	424,900	527,700	
71.	" Type F-5	"	109,600	449,500	559,000	
72.	Turn out	L.S.	100	560	660	
73.	Inlet $\phi$ 200 mm	"	-	220	220	
74.	Culvert (2), Type-1	"	-	1,330	1,330	$\phi$ 500 mm
75.	Culvert (3), Type-2	"	-	580	580	$\phi$ 300 mm
76.	Culvert (4), Type-3	"	10	3,480	3,490	$\phi$ 800 mm
77.	Culvert (1), Type-4	"	-	510	510	$\phi$ 300 mm

No.	Description	Unit	Unit Cost			Remarks
			F.C. (₪)	L.C. (₪)	Total (₪)	
On-1	Land Clearing	ha	20	240	260	
On-2	Land Leveling	ha	480	3,740	4,220	
On-3	Farm Road & Ditch	m	1	10	11	
On-4	Drainage Ditch	"	1	13	14	
ES-1	Stripping	m <sup>3</sup>	0.6	3.8	4.4	D=50m
ES-2	"	"	0.5	3.3	3.8	D=40m
EE-1	Excavation	"	0.3	2.8	3.1	
EE-2	"	"	0.3	2.5	2.8	
EE-3	"	"	0.5	5.0	5.5	
EE-4	"	"	0.3	3.4	3.7	
EE-5	"	"	0.5	4.7	5.2	
EE-6	"	"	0.3	2.6	2.9	
EE-7	"	"	-	12.0	12.0	
EE-8	"	"	1.0	5.9	6.9	
EB-1	Embankment	"	0.2	1.9	2.1	
EB-2	"	"	0.2	2.7	2.9	
EB-3	"	"	-	12.0	12.0	
EB-4	"	"	0.7	10.2	10.7	
EP-1	Pavement	"	1.0	83.0	84.0	
ET-1	Removal of excess soil	"	0.2	1.1	1.3	
CO-1	Reinforcement bar	kg	3.9	3.1	7.0	
CO-2	"	"	3.9	3.0	6.9	
CO-3	Concrete material	m <sup>3</sup>	58.0	374.0	432.0	$\sigma_{28}=210 \text{ kg/m}^2$
CO-4	"	"	45.0	335.0	380.0	$\sigma_{28}=160 \text{ kg/m}^2$
CO-5	Concrete mixing	"	1.2	28.0	29.2	0.5m <sup>3</sup> Mixer
CO-6	"	"	1.3	29.8	31.1	0.3m <sup>3</sup> Mixer
CO-7	Placing concrete	"	0.6	24.2	24.8	0.3m <sup>3</sup> Mixer
CO-8	"	"	0.5	20.9	21.4	0.5m <sup>3</sup> Mixer
CO-9	Concrete	"	70	450	520	0.5m <sup>3</sup> Mixer
CO-10	"	"	70	450	520	$\sigma_{28}=210 \text{ kg/m}^2$ 0.3m <sup>2</sup> Mixer $\sigma_{28}=210 \text{ kg/m}^2$
CO-11	Form	m <sup>2</sup>	-	60	60	
CO-12	"	"	-	110	110	
CP-1	Concrete pile	pile	10	1,070	1,080	□200x200mm
CP-2	"	"	10	2,810	2,820	□350x350mm ℓ=12m
CP-3	"	"	10	3,470	3,480	□350x350mm ℓ=15m
CP-4	"	"	10	4,570	4,580	□350x350mm ℓ=20m
Pr-1	Drying works	day	20	80	100	φ4" pump
Pr-2	Setting & Withdrawing of a concrete plant	L.S.	2,000	23,000	25,000	0.5m <sup>3</sup> Mixer
Pr-3	Retaining wall.	m	200	290	490	
Pr-4	Foothold	m <sup>3</sup>	-	40	40	
Pr-5	Curing concrete	m <sup>2</sup>	-	10	10	
Pr-6	Temporary pier	m	-	6,000	6,000	

No.	Description	Unit	Unit Cost			Remarks
			F.C. (₱)	L.C. (₱)	Total (₱)	
Pr-7	Backfill of canal	km	1,880	18,750	20,630	
Pr-8	Sheet-piling cofferdam	m	-	600	600	
Pr-9	Field Office	m <sup>2</sup>	-	3,000	3,000	
Pr-10	Motor pool	"	-	20	20	
Pr-11	Concrete pipe	m	-	550	550	φ700 mm

2. Labor Cost

<u>Personnel</u>	<u>Unit</u>	<u>Cost</u>
		<u>₪</u>
Worker	day	40
Skilled worker	"	50
Foreman	"	65
Chief worker	"	100
Operator of vehicle	"	65
Asst. operator of vehicle	"	60
Operator of heavy equipment	"	90
Asst. operator of heavy equipment	"	60
Mason	"	40
Carpenter	"	40
Smith	"	50
Painter	"	80
Asphalt worker	"	80
Watchep	"	50
Head carpenter	"	100
Head smith	"	100
Head welder	"	100

Source: Land Reform Office, Royal Irrigation Department

## 3. Material Cost

Description	Unit	Cost (P)			Remarks
		F.C.	L.C.	Total	
Portland cement	ton	160.00	640.00	800.00	one bag 50 kgs
Sand (for concrete)	m <sup>3</sup>	-	70.00	70.00	
Gravel (for concrete)	"	-	130.00	130.00	
Unscreened gravel	"	-	120.00	120.00	
Deformed bar	t	4,314.00	2,876.00	7,190.00	SD30 φ16mm
Round bar	"	3,756.00	2,504.00	6,260.00	SR24 φ15mm
Miscellaneous steel	"	6,000.00	4,000.00	10,000.00	
Wood (for reform)	m <sup>2</sup>	-	150.00	150.00	
Gasolin	lit	1.80	1.80	3.60	
Lubricating oil	"	9.00	9.00	18.00	
Diesel fuel (for truck)	"	1.30	1.30	2.60	
Diesel fuel (for generator engine)	"	1.30	1.30	2.60	
Light gasolin	"	1.30	1.30	2.60	
Timber (soft)	m <sup>3</sup>	-	2,200.00	2,200.00	
" (hard)	"	-	4,000.00	4,000.00	
Brick	m <sup>2</sup>	-	40.00	40.00	
Masonry	m <sup>3</sup>	-	700.00	700.00	
Crushed stone	"	-	120.00	120.00	
Asphalt	t			3,000.00	
Laterite (pavement of road)	m <sup>3</sup>	-	80.00	80.00	from salaburi
Water stop	m	300.00	-	300.00	



4. Operation Cost of Heavy Equipment

Description	Purchase Price <sup>a/</sup> (1)	Life time (2)	Deprecia- tion Cost (3) = $\frac{(1-0.1) \times (1)}{(2)}$	Repair Cost		Fuel & Lubricant <sup>b/</sup> Cost (7)			Labor <sup>c/</sup> (Operator) (8)	Adminis- tration & Other (9)	Foreign		Local (10) = (7) + (8) + (9)	Remarks	
				Rate (4)	Parts Cost (5) = $\frac{(1) \times (4)}{(2)} \times 0.7$	Labor Cost (6) = $\frac{(1) \times (4)}{(2)} \times 0.3$	ℓ/h	F.C.			L.C.	Foreign (7)			Local (10)
Bulldozer 15 <sup>t</sup> 140 <sup>ps</sup>	1,000,000	6,500	138.5	1.10	118.5	50.8	11.0	18.6	18.6	26.3	15.4	18.6	111.1		
" 16 <sup>t</sup> 140 <sup>ps</sup>	1,050,000	6,500	145.4	1.20	136.7	58.2	11.0	18.6	18.6	26.3	16.2	18.6	119.3	Swampy type	
" 21 <sup>t</sup> 200 <sup>ps</sup>	1,600,000	7,800	184.6	1.15	165.1	70.8	20.0	33.8	33.8	26.3	20.5	33.8	151.4		
Backhoe 0.6 <sup>m<sup>3</sup></sup> 100 <sup>ps</sup>	1,450,000	7,200	181.3	0.95	133.9	57.4	8.0	13.5	13.5	26.3	20.1	13.5	117.3		
" 0.3 <sup>m<sup>3</sup></sup>	730,000	6,000	109.5	0.75	63.9	27.4	5.0	8.5	8.5	26.3	12.2	8.5	74.4		
Dragline 0.6 <sup>m<sup>3</sup></sup> 125 <sup>ps</sup>	90,000	7,200	11.3	0.95	8.3	3.7	-	-	-	-	-	-	3.7	Backhoe attachment	
Frontend loader 1.6 <sup>m<sup>3</sup></sup>	770,000	6,500	106.6	1.05	87.1	37.3	11.0	18.6	18.6	26.3	11.8	18.6	94.0		
Scrape-dozer 6.4 <sup>m<sup>3</sup></sup> 183 <sup>ps</sup>	1,850,000	6,500	256.2	1.00	199.2	85.4	19.0	32.1	32.1	26.3	28.5	32.1	172.3		
Motor-grader 3.7 <sup>m<sup>3</sup></sup> 125 <sup>ps</sup>	1,150,000	6,600	156.8	0.85	103.7	44.4	12.0	20.3	20.3	26.3	17.4	20.3	108.4		
Tire rollor 10 <sup>t</sup> 40 <sup>ps</sup>	350,000	7,000	45.0	0.85	29.8	12.8	4.0	6.8	6.8	26.3	5.0	6.8	50.9		
Water tank truck 6 <sup>t</sup>	230,000	6,000	34.5	0.90	24.2	10.4	5.0	8.5	8.5	11.4	3.8	8.5	34.1		
Dump truck 8 <sup>m<sup>3</sup></sup> 196 <sup>ps</sup>	310,000	6,000	46.5	0.80	28.9	12.4	7.0	11.8	11.8	11.4	5.2	11.8	40.8		
Agitator truck 3.0 <sup>m<sup>3</sup></sup> 195 <sup>ps</sup>	420,000	5,500	68.7	0.60	32.1	13.7	7.0	11.8	11.8	11.4	7.6	11.8	44.5		
" 1.6 <sup>m<sup>3</sup></sup> 135 <sup>ps</sup>	250,000	5,000	45.0	0.60	21.0	9.0	5.0	8.5	8.5	11.4	5.0	8.5	33.9		
Diesel hammer 2.2 <sup>t</sup>	690,000	6,000	103.5	0.95	76.5	32.8	-	-	-	-	-	-	32.8	Backhoe attachment	
Diesel hammer 1.25 <sup>t</sup> (95ps)	410,000	6,000	61.5	0.95	45.4	19.5	-	-	-	-	-	-	19.5	Backhoe attachment	

Note: a/ Heavy equipment purchase cost  
C.I.F Bangkok in Oct. 1976

b/ Fuel & Lubricant Cost  
Fuel consumption (ℓ/hr) x 1.30  $\frac{฿/ℓ}{฿/ℓ}$  x 1.3 (Light gasolin)  
Fuel consumption (ℓ/hr) x 1.80  $\frac{฿/ℓ}{฿/ℓ}$  x 1.15 (Gasolin)

c/ Labor  
Heavy equipment (Bulldozer, Backhoe, etc.)  
Operator:  $(90 \frac{฿}{hr} + 60 \frac{฿}{hr}) \times \frac{(\frac{8H}{8H} + \frac{4H}{8H} \times 1.5)}{10 \text{ hr}} = 26.3 \frac{฿}{H}$

Dump truck  
Operator:  $65 \frac{฿}{hr} \times \frac{(\frac{8H}{8H} + \frac{4H}{8H} \times 1.5)}{10 \text{ hr}} = 11.4 \frac{฿}{H}$

d/ Administration & other  
 $((1)/(2)) \times 0.1$

Decision of Required Number of Construction Equipments

The required number of construction equipments can be calculated based on the work volume, operation hours of equipment during the construction period.

Sample explanation for the estimation of the required number of equipments is given hereinafter in case of scrape dozer 6.4 cu.m;

Construction stage of max. work volume: 4th year

Required total operation hours in 4th year<sup>1/</sup>;

for dike and provincial road works: 2,191.9 hr

for on-farm works: 25,895.7 hr

Operation hours during construction period per unit:

for dike and provincial road works:

7 months x 25 days/mon. x 10 hr/day = 1,750 hr/unit

for on-farm works:

5 months x 25 days/mon. x 10 hr/day = 1,250 hr/unit

Required number of equipments;

for dike and provincial road works;

$2,191.9/1,750 = 1.25$

for on-farm works:

$25,895.7/1,250 = 20.72$

Total 21.97  $\approx$  22 units

<sup>1/</sup> : See Appendix 4-65.

Following figure shows the yearly required number of main construction equipments, which was estimated through same procedure mentioned above.

Equipment	Unit	Year				
		2nd	3rd	4th	5th	6th
Bulldozer 15 t	0					
	5					
	10					
	15			?		
	20			18 units		
Bulldozer 16 t	0					
	5					
	10			?		
	15			11 units		
Bulldozer 21 t	0					
	5					
	10			?		
	15			11 units		
Scrape-dozer 6.4m <sup>3</sup>	0					
	5					
	10					
	15					
	20			?		
	25			22 units		

Table A.4-70 Estimation of Total Operation Hours of Scrape Dozer 6.4 m<sup>3</sup>

Description	Works	Work Volume (m <sup>3</sup> )	Hourly Production (m <sup>3</sup> /hr)	Required Operation Hours (hour)
<u>1. Dike</u>				
Block A	Type D-2 Excavation	48,976.0	95.0	515.5
	Type D-4 Excavation	195,151.3 (62,657.3)	101.8	1,917.0 (615.5) <sup>a/</sup>
Block B	Type D-2 Excavation	61,702.5 (43,093.8)	95.0	649.0 (453.6)
	Type D-4 Excavation	146,195.0 (79,624.1)	101.8	1,436.1 (782.2)
Block C	Type D-1 Excavation	57,389.2	95.0	604.1
	Type D-2 Excavation	19,589.0	95.0	206.2
	Type D-3 Excavation	54,505.4	101.8	535.4
	Type D-4 Excavation	148,811.2	101.8	1,461.8
Sub-total				7,325.6 (1,851.3)
<u>2. On-Farm</u>				
Block A				
Land leveling	Excavation	732,534.0 (102,850.0)	57.0	12,851.5 (1,804.4)
	Excavation	1,059,450.0 (148,750.0)	86.4	12,262.2 (1,721.6)
Road	Type R-1 Excavation	5,619.4 (789.0)	101.8	55.2 (7.8)
	Type R-2 Excavation	2,504.3 (351.6)	101.8	24.6 (3.4)
	Type R-6 Excavation	37,452.2 (5,258.4)	101.8	367.9 (51.7)
Canal & ditch	Type MC-2 Excavation	33,915.0 (4,761.8)	95.0	357.0 (50.1)
	Type LI-1 Excavation	1,018.0 (142.9)	101.8	10.0 (1.4)
	Type LI-2 Excavation	9,263.8 (1,300.7)	101.8	91.0 (12.8)
	Type LI-5 Excavation	2,433.0 (341.6)	101.8	23.9 (3.4)

Note: <sup>a/</sup> shows the required operation hours in 4th year

<u>Description</u>	<u>Works</u>	<u>Work Volume</u> (m <sup>3</sup> )	<u>Hourly</u> <u>Production</u> (m <sup>3</sup> /hr)	<u>Required</u> <u>Operation</u> <u>Hours</u> hour
	Type DD-1 Excavation	1,349.7 (195.8)	101.8	13.7 (1.9)
	Type DD-2 Excavation	875.5 (122.9)	101.8	8.6 (1.2)
Block B				
Land leveling	Excavation	(625,084.8)	57.0	(10,966.4)
	Excavation	(904,055.0)	86.4	(10,463.6)
Road	Type R-1 Excavation	(4,906.8)	101.8	(48.2)
	Type R-2 Excavation	(1,985.1)	101.8	(19.5)
	Type R-6 Excavation	(17,937.2)	101.8	(176.2)
Canal & ditch	Type MC-2 Excavation	(42,731.0)	95.0	(449.8)
	Type MC-3 Excavation	(1,801.9)	101.8	(17.7)
	Type LI-1 Excavation	(885.7)	101.8	(8.7)
	Type LI-2 Excavation	(4,438.5)	101.8	(43.6)
	Type LI-5 Excavation	(3,196.5)	101.8	(31.4)
	Type DD-1 Excavation	(1,109.6)	101.8	(10.9)
Block C				
Land leveling	Excavation	1,193,545.8	57.0	20,939.4
	Excavation	1,726,211.5	86.4	19,979.3
Road	Type R-1 Excavation	10,220.7	101.8	100.4
	Type R-2 Excavation	4,051.6	101.8	39.8
	Type R-5 Excavation	2,107.3	101.8	20.7
	Type R-6 Excavation	42,165.6	101.8	414.2
	Type R-9 Excavation	4,947.5	101.8	48.6

<u>Description</u>	<u>Works</u>	<u>Work Volume</u> (m <sup>3</sup> )	<u>Hourly</u> <u>Production</u> (m <sup>3</sup> /hr)	<u>Required</u> <u>Operation</u> <u>Hours</u> hour
Canal & ditch	Type MC-1 Excavation	31,654.0	95.0	333.2
	Type MC-2 Excavation	13,566.0	95.0	142.8
	Type MC-4 Excavation	1,353.9	101.8	13.3
	Type MC-3 Excavation	885.7	101.8	8.7
	Type LI-1 Excavation	1,852.8	101.8	18.2
	Type LI-2 Excavation	10,424.3	101.8	102.4
	Type LI-6 Excavation	1,109.6	101.8	10.9
	Type DD-1 Excavation	2,260.0	101.8	22.2
	Type DD-2 Excavation	1,537.2	101.8	15.1
	Sub-total			
<u>3. Provincial Road</u>				
Block A	Type PR-1 Excavation	(7,502.7)	101.8	(73.7)
Block B	Type PR-1 Excavation	(9,854.2)	101.8	(96.8)
	Type PR-2 Excavation	(4,957.7)	101.8	(48.7)
	Type PR-5 Excavation	(12,358.5)	101.8	(121.4)
Block C	Type PR-6 Excavation	4,519.9	101.8	44.4
Sub-total				385.0 (340.6)
<u>Total</u>				<u>98,221.4</u> (28,087.6)

Residual Values of Construction Equipments

Residual values of the construction equipments can be calculated by the following equation;

$$\text{Residual values} = \text{Purchase Price} - \{ \text{Depreciation cost} + \text{Repair Cost} \}$$

Sample explanation for the residual values in case of scrape dozer 6.4 m<sup>3</sup> is given hereinafter.

$$\begin{aligned} \text{Purchase price} &= \text{K}1,850,000 \times 22^{\text{unit}} \times 1.2^{1/} \\ &= \text{K}48,840,000 \end{aligned}$$

$$\begin{aligned} \text{Depreciation cost} &= \text{Operation hours} \times \text{Depreciation cost/hours} \\ &= 98,221.4^{2/} \text{ hr} \times 256.2^{3/} \text{ K/hr} = \text{K}25,164,000 \end{aligned}$$

$$\begin{aligned} \text{Repair cost} &= \text{Operation hours} \times \text{Repair cost/hour} \\ &\quad \times (1.0 - 0.1)^{4/} \\ &= 98,221.4 \text{ hr} \times 199.2^{5/} \times 0.9 \\ &= \text{K}17,609,000 \end{aligned}$$

so that,

$$\begin{aligned} \text{Residual values} &= \text{K}48,840,000 - \\ &\quad ( \text{K}25,164,000 + \text{K}17,609,000 ) \\ &= \text{K}6,067,000 \end{aligned}$$

- Note: <sup>1/</sup> : 20% of spare parts are included (see Appendix 4-62, page 8)  
<sup>2/</sup> : See Appendix 4-65  
<sup>3/</sup> : See Appendix 4-64, page 7  
<sup>4/</sup> : 10% of residual values for spare parts are considered  
<sup>5/</sup> : See Appendix 4-64, page 7

Based on the above procedure for the estimation of the residual values of construction equipments, total residual values of the construction equipment required for the project are evaluated to be K25,460,000 as shown in Table A.4-71.

Table A.4-71 Residual Values of Construction Equipments and Provisional Facilities

Equipments	Purchase Cost (A) 1,000£	Operation Hour (hr) hour	Depreciation Cost		Repair Cost		Total (B)+(C) 1,000£
			Cost/hr £	Cost (B) 1,000£	Cost/hr £	Cost (C) 1,000£	
Bulldozer	21,500	82,569.3	138.5	11,436	106.7	8,810	20,246
"	13,860	49,864.7	145.4	7,250	123.0	6,133	13,383
"	21,120	43,564.6	184.6	8,042	148.6	6,474	14,516
Scrapedozer	48,840	98,221.4	256.2	25,164	179.3	17,611	42,775
Tire Roller	2,520	32,273.6	45.0	1,452	26.8	865	2,317
Motor-grader	1,380	485.2	156.8	76	93.3	45	121
Watertruck	276	485.2	34.5	17	21.8	11	28
Backhoe	8,700	13,784.0	181.3	2,499	120.5	1,611	4,160
"	6,132	29,491.5	109.5	3,229	57.5	1,696	4,925
Fronted Loader	924	578.0	106.6	62	78.4	45	107
Dump Truck	1,116	1,503.5	46.5	70	26.0	39	109
Disel Hammer	492	1,865.4	61.5	116	40.9	77	193
"	828	253.4	103.5	26	68.8	17	43
Mixer	696	1,420.0	130.5	185	54.8	78	263
"	216	390.9(day)	90.0	35	44.1	17	52
Total	128,700			59,659		43,579	103,238

Residual values of equipments = (A) - {(B) + (C)}  
 = £128,700,000 - £103,238,000  
 = £25,462,000 ≈ £25,460,000

Residual values of provisional facility = £1,800,000 x 0.3  
 = £540,000

Total residual value = £25,460,000 + £540,000  
 = £26,000,000



Table A.4-72 Disbursement Schedule

(Unit: ¥'000)

Discription	Total			1st Year(Oct.'77-Sep.'78)			2nd year(Oct.'78-Sep.'79)			3rd year(Oct.'79-Sep.'80)			4th year(Oct.'80-Sep.'81)			5th year(Oct.'81-Sep.'82)			6th year(Oct.'82-Sep.'83)		
	F.C.	L.C.	T	F.C.	L.C.	T	F.C.	L.C.	T	F.C.	L.C.	T	F.C.	L.C.	T	F.C.	L.C.	T	F.C.	L.C.	T
1. Civil Works	94,020	177,080	271,100				26,850	16,118	42,968	25,086	44,611	69,697	37,778	56,745	94,523	2,126	30,468	32,594	2,180	29,138	31,138
1-1. Preparation	330	18,580	18,910	-	-	-	20	4,628	4,648	87	3,385	3,472	97	5,981	5,988	63	1,949	2,012	63	2,727	2,790
1-2. Pumping Station	83,590	31,760	115,350				26,830	11,490	38,320	22,400	8,880	31,280	34,360	11,390	45,750						
1-3. Dike	1,340	20,600	21,940							459	6,764	7,223	341	4,856	5,197	249	5,442	5,691	291	3,538	3,829
1-4. On-farm	8,320	101,500	109,820							2,140	25,582	27,722	2,550	30,448	32,998	1,804	22,597	24,401	1,826	22,873	24,699
1-5. Provincial Road	440	4,640	5,080										430	4,160	4,590	10	480	490			
2. Construction Equipment	133,100	1,300	134,400				9,276	100	9,376	101,669	1,000	102,669	7,385	67	7,452	7,385	67	7,452	7,385	66	7,451
3. Agricultural Development	-	14,860	14,860		2,200	2,200		3,640	3,640		3,290	3,290		980	980		980	980		3,770	3,770
4. Project Administration & Facility	1,280	21,460	22,740	1,280	9,530	10,810		2,386	2,386		2,386	2,386		2,386	2,386		2,386	2,386		2,386	2,386
5. Consulting Services	22,700	8,620	31,320	7,690	1,440	9,130	3,002	1,436	4,438	3,002	1,436	4,438	3,002	1,436	4,438	3,002	1,436	4,438	3,002	1,436	4,438
6. Land Acquisition	-	3,640	3,640	-	910	910		910	910		910	910		910	910		910	910			
Sub-total	251,100	226,960	478,060	8,970	1,4080	23,050	39,128	24,590	63,718	129,757	53,633	183,390	48,165	62,524	110,689	12,513	35,337	47,850	12,567	36,796	49,363
7. Contingency	37,665	34,045	71,710	1,346	2,113	3,459	5,869	3,689	9,558	19,464	8,045	27,509	7,224	9,379	16,603	1,877	5,300	7,177	1,885	5,519	7,404
Total	288,765	261,005	549,770	10,316	16,193	26,509	44,997	28,279	73,276	149,221	61,678	210,899	55,389	71,903	127,292	14,390	40,637	55,027	14,452	42,315	56,767
8. Price Escalation	82,269	91,846	174,115	825	1,295	2,120	7,487	4,706	12,193	38,754	16,019	54,773	19,967	25,920	45,887	6,754	19,072	25,826	8,482	24,834	33,316
Grand Total	371,034	352,851	723,885	11,141	17,488	28,629	52,484	32,985	85,469	187,975	77,697	265,672	75,356	97,823	173,179	21,144	59,709	80,853	22,934	67,149	90,083

## Construction Method of the Works

### 1. Outline

Construction works in the Project Area consist of the following works:

1. Pumping station
2. Dike
3. Land consolidation
4. Provincial road

Project Area is located in the low-lying area with an elevation of averaged two meters above mean sea level and it is submerged in the wet season. The construction of dike and pumping station will be proceeded prior to the construction of land consolidation taking account the flooding irrigation in the wet season. Construction period will be five years, of which Block A will be started first, then Block B and lastly Block C. It will be possible to use the existing canals and provincial road in order that the construction materials will be transported to Project Area. And, in order to transport the heavy equipments, it would be effective to use the ship which comes from Bangkok passing through Chao Phya river and goes through the existing canals, Khlóng Phraya Banlu, in the Project Area. As regards the soil texture, silty clay (or CH) covers the ground surface in the Project Area, therefore crawler type construction equipments are more effective for these works and swampy-type heavy equipment would also be partly required. It is assumed in this planning that daily operation hours of heavy equipments would be ten (10) hours which is generally adopted in Thailand. Outline of construction method for principal works are as follows.

### 2. Workable days and Bulk factor of soil

#### Workable days

Workable days for civil works are influenced by climate, especially rainfall. And in order to calculate the workable days, the daily rainfall data for the last ten years (1966 - 1975, observation station, Sing Ha Nat Office of RID) were analyzed based on the standards below.

a) Concrete works

Below 10 mm/day ----- Possible to work  
Over 10.1 mm/day ----- Impossible to work

b) Other works (Excavation, Embankment, and Others)

Below 5 mm/day ----- Possible to work  
5.1 mm/day - 20.0 mm/day ----- One restday  
20.1 mm/day - 50.0 mm/day ----- Two restdays  
Over 50.1 mm/day ----- Three restdays

Monthly Workable Days

<u>Works</u>	<u>Jan.</u>	<u>Feb.</u>	<u>Mar.</u>	<u>Apr.</u>	<u>May</u>	<u>Jun.</u>	<u>Jul.</u>	<u>Aug.</u>	<u>Sep.</u>	<u>Oct.</u>	
Concrete	30	27	29	28	26	25	25	24	20	24	
Others	30	27	28	26	23	21	22	19	15	20	
											<u>Annual mean</u>
							<u>Nov.</u>	<u>Dec.</u>	<u>Total</u>		
							28	30	316		26days
							27	29	287		24days

The above-mentioned table indicates the annual workable days after analyzing the rainfall data, but flooding irrigation method has been adopted in the Project Area, therefore, annual workable days will be decided taking into account the water level in the Project Area and the growing period of paddy. Figure A.5-1 indicates the relationships.

After these analyses, workable periods for civil works are as follows:

Pumping station works ----- From the beginning of January to the end of August (Eight months/year)

Dike and provincial road works -- From the beginning of January to the end of July (Seven months/year)

On-farm works ----- Twentieth of January - Twentieth of June (Five months/year)

And also, monthly mean workable days are as follows:

Concrete works ----- Twenty seven (27) days/month

Other works ----- Twenty five (25) days/month

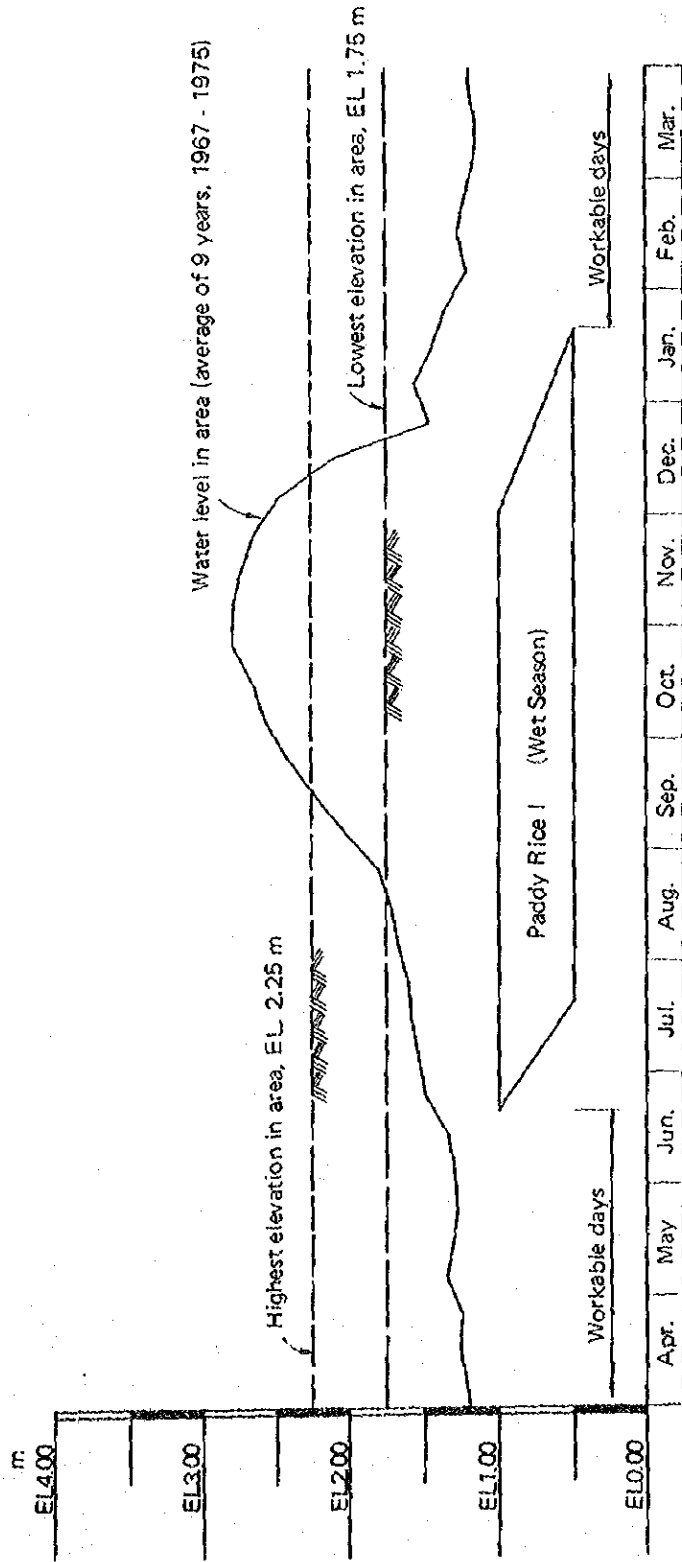
Bulk factor of soil

Bulk factor of soil for construction of civil works were decided follows:

Classification of Soil texture	Bulk Factor of Soil			
	Original condition of soil	Bulk factor of soil		
		P	L	B
Clayey soil	P	1.00	1.43	0.90
	L	0.70	1.00	0.63
	B	1.11	1.59	1.00
Sandy soil	P	1.00	1.25	0.90
	L	0.80	1.00	0.72
	B	1.11	1.39	1.00
Gravel & Sand	P	1.00	1.18	1.08
	L	0.85	1.00	0.91
	B	0.93	1.09	1.00

Note: P: in place condition  
L: in bank condition  
B: in loose condition

FIGURE A.5-1 WORKABLE PERIOD OF THE PROJECT UNDER PRESENT CONDITIONS



### 3. Construction method of works

#### (a) Pumping station works

Pumping station are to be constructed in each blocks. Pumping station works must be started earlier than the other works, with Block A at first, then Block B and finally Block C.

#### Excavation works

The following equipments are required for excavation works.

- Bulldozer with capacities of 15 ton
- Fronted loader with capacities of 1.6 cu.m
- Backhoe with capacities of 0.6 cu.m
- Dump truck with capacities of 8 ton

Materials excavated by the above mentioned equipments are deposited at nearby and, which would used for backfill or embankment materials. And also excess soil are leveled in field for the land consolidation works.

#### Works of concrete pile

The foundation at construction site is weak of N-values 1-2 to the depth of ten meters, therefore concrete piles are required in order to bear the pumping stations. Concrete pile are driven by 1.25 ton diesel pile hammer attached to the universal excavator from which the backhoe is taken off. Daily (10 hours/day) production of piling is about 13 pieces.

#### Concrete works

Concrete mixing works are accomplished by 0.5 cu.m portable batching plant which is set at job site, then, mixed concrete is transferred and placed by agitator truck and concrete conveyer. Daily mixing capacity is about 3.0 cu.m by using the 0.5 cu.m portable batching plant .

(b) Dike works

Stripping of surface soil

The work belongs to embankment works of dike and is composed of stripping the vegetations and the organic soil lying on the depth of 20 cm. This work is required in order that the connection of embankment portion and foundation will be in good condition, and in order to prevent from water seepage from the abutment portion. The surface soil is stripped by 15 ton bulldozer or swampy type 16 ton bulldozer, and it is hauled and embanked at nearby place. Then, the soil is leveled in land consolidated area.

Excavation works

Planning of excavation works is indicated in Type A of Figure A.5-2. Excavated materials from the borrow pit portion are transferred to the embankment portion. The upper layer is excavated by 15 ton bulldozer and the lower layer is excavated by 6.4 cu.m scrape dozer, and afterward excavated materials are hauled and transferred to the embankment portion.

Embankment works

Non-pavement embankment portion:

Transferred embankment materials are spread by 15 ton bulldozer or swampy type 16 ton bulldozer and afterwards compacted by tire roller with capacities of 8-20 ton

Pavement embankment portion:

Principal main roads are paved at depth of 15 cm on the surface by the good materials contained with many gravel and sand, so called a laterite, which is transferred from the mountainous area. In this planning, the laterite is transported from Salaburi located in the northern part of about 100 km from the Project Area. Transported laterite is spread by 15 ton bulldozer and afterward compacted by tire roller with capacities of 8-20 ton.

(c) On-farm works

On-farm works will be divided into following works.

- (i) Road and canal works
- (ii) Land leveling works
- (iii) Related structure works

Road and canal works

Planning of road and canal works are indicated in Type B and Type C of Figure A.5-2.

Type B:

Excavated materials from the canal portion are transferred to the embankment portion. Methods of excavation works are as follows: the excavated materials by 0.6 cu.m backhoe are hauled to the embankment portion and embanked by 15 ton bulldozer or swampy type 16 ton bulldozer

Type C:

Embankment materials, which are equally excavated from surrounding paddy fields, are transferred to the embankment portion. Excavation works are conducted as follows, embankment materials are excavated by 15 ton bulldozer or swampy-type 16 ton bulldozer and afterward hauled or transferred to embankment portion.

Construction methods of embankment works are same as methods adopted to dike works.

Land levelling works

(1) Land clearing works

There are many small hills in which trees and shrubs grow in the Project Area, therefore, before the execution of land consolidation works, stripping the obstructions (land clearing works) are required in the Project. And also it is considered that these obstructions will be cleared by bulldozer. Therefore, in this planning, 15 ton and/or 21 ton bulldozer are used for this purpose.



(2) Land leveling works

As regards soil texture in the Project Area, which was previously described, silty clay reaches to the depth from 10 m to 15 m. Namely, clear classifications between surface soil and sub-surface soil can not be observed. Averaged volumes of land levelling works are calculated in sample areas as follows:

Land adjustment: 242 cu.m/ha (hauling distance 197 m)

Land leveling: 520 cu.m/ha (hauling distance 65 m)

Method of land adjustment and land leveling works are: two third of these works are accomplished by 6.4 cu.m scrape dozer and one third of these works are accomplished by 21 ton bulldozer and 15 ton bulldozer or swampy type 16 ton bulldozer. After hauling the soil, in order to remove fluctuation and level the field surface, bulldozer are used along the width of plot throughout the field, and also along the length of run throughout the field.

(3) Farm Road and Ditch works

Road works:

Embankment materials are gathered from vicinity of proposed farm roads, and embanked by 15 ton bulldozer and/or swampy type 16 ton bulldozer. And afterward, these are spready by the bulldozer and compacted by tire roller having 8 - 20 ton capacities.

Ditch works:

Ditches are excavated by backhoe with capacities of 0.3 cu.m

Related structure works

In this planning, concrete required for related structures are mixed by the potable concrete mixer set at jobe site, and mixed concrete are transferred and placed by man power using wheelbarrows or by belt conveyer.

Daily mixing capacities of 0.3 cu.m potable concrete mixer will be considered to 20 cu.m/day.

(d) Provincial road works

Construction methods for provincial roads conform to Type B of Figure A.5-2.

FIGURE A. 5-2  
CONSTRUCTION METHOD OF DIKES, ROADS AND CANALS

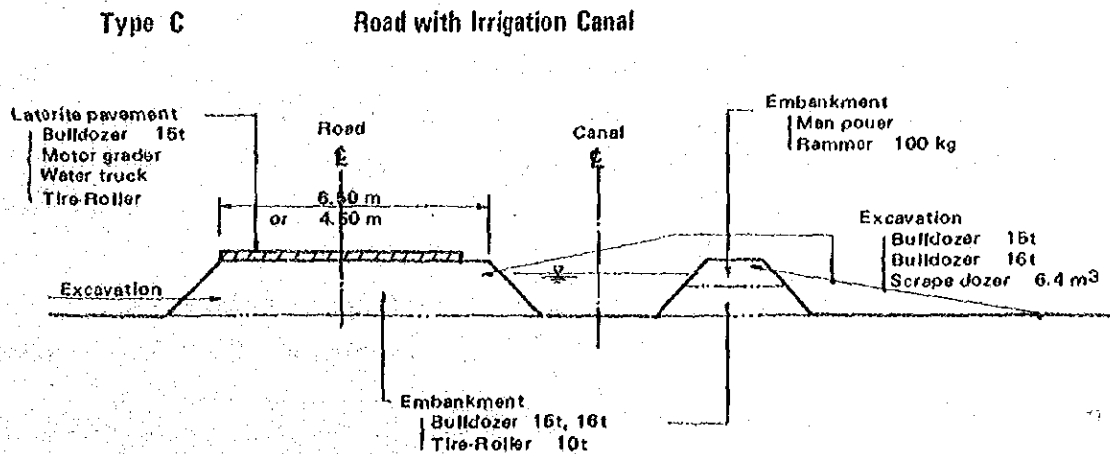
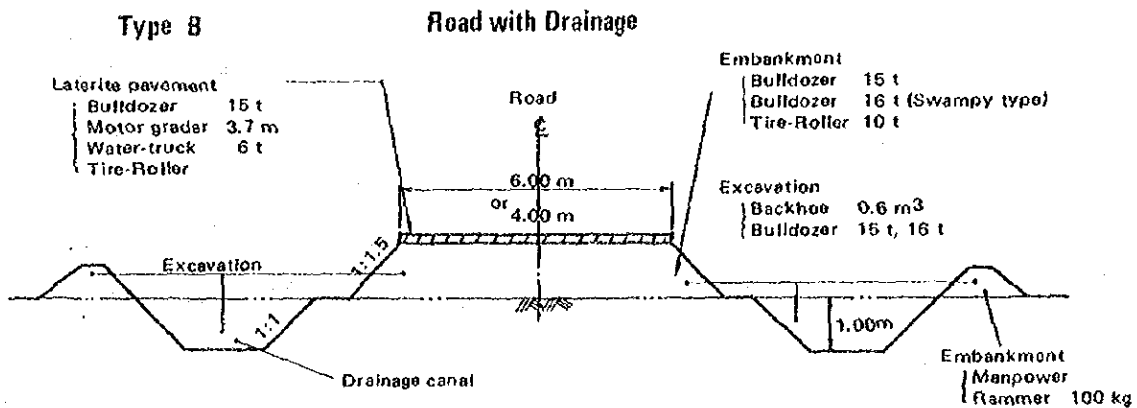
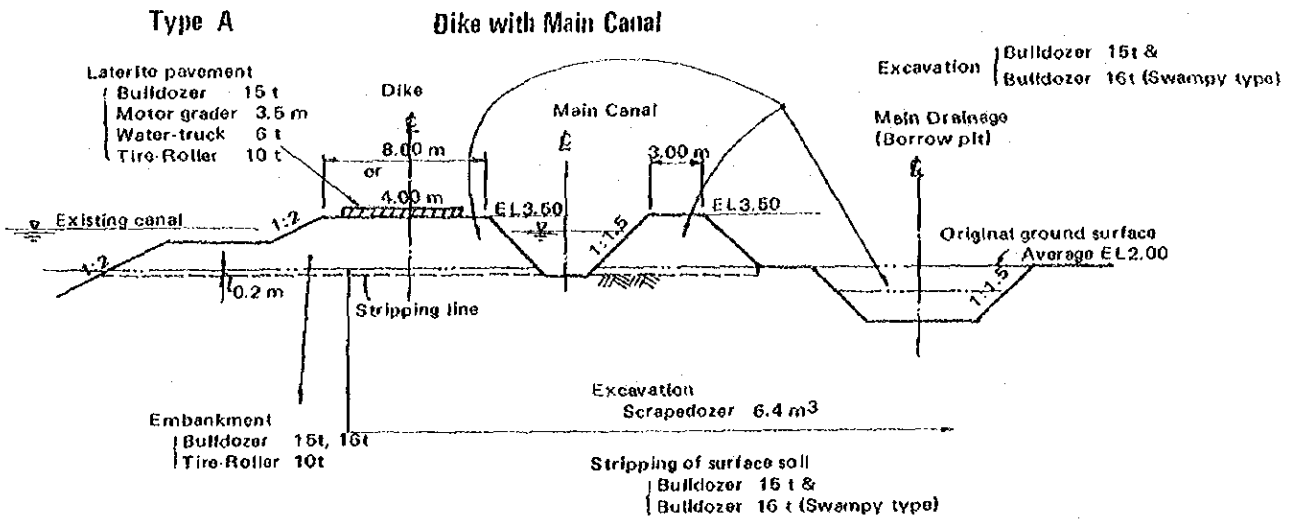
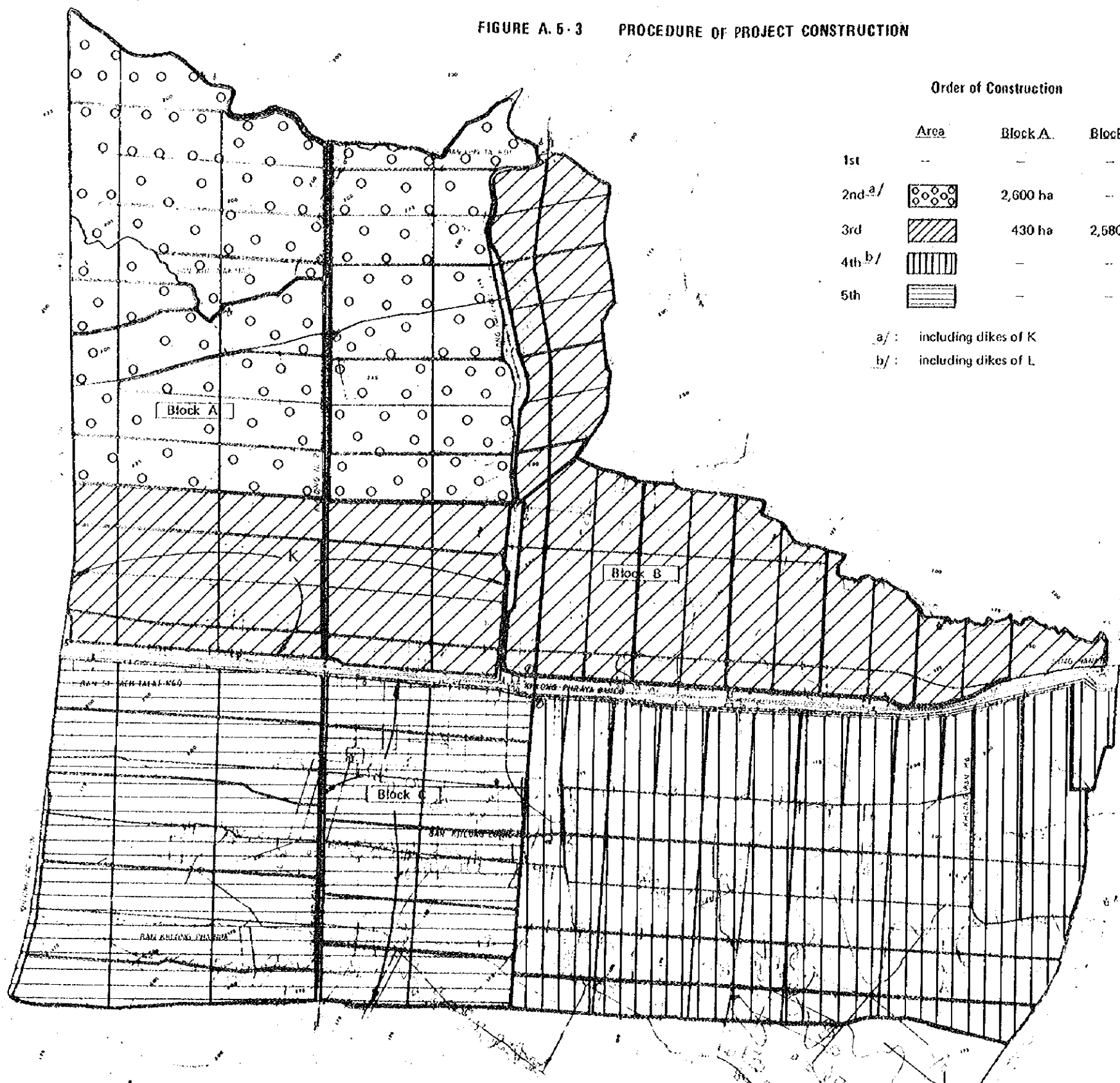


FIGURE A. 5-3 PROCEDURE OF PROJECT CONSTRUCTION



Order of Construction			
Area	Block A.	Block B.	Block C.
1st	—	—	—
2nd <sup>a/</sup>	2,600 ha	—	—
3rd	430 ha	2,580 ha	—
4th <sup>b/</sup>	—	—	2,450 ha
5th	—	—	2,480 ha

a/ : including dikes of K  
b/ : including dikes of L

KAGE NAM CHAO PHRAYA

SCALE

LEGEND

- BOUNDARY OF PROJECT AREA
- CANAL AND DIKE
- VILLAGE
- BOUNDARY OF TANBON (VILLAGE)
- SCHOOL
- WAT

Operation and Maintenance Costs

## 1. Maintenance costs (¥'000)

## 1-1. Irrigation, drainage and road systems

Laterite pavement of main road and dike

$$0.18\text{m}^3/\text{m} \times 78.3\text{km} \times 84\text{¥}/\text{m}^3 = 1,184$$

$$0.18\text{m}^3/\text{m} \times 13.6\text{km} \times 84\text{¥}/\text{m}^3 = 206$$

Excavation

$$0.15\text{m} \times 4.0\text{m} \times 343\text{km} \times 13\text{¥}/\text{m}^3 = 2,675$$

## 1-2. Building facilities

$$10,425,000 \times 4\% = 417$$

## 1-3. Others 900

Total 5,382

## 2. Personnel Cost

<u>Personnel</u>	<u>Yearly Expenses</u> (¥ '000)	<u>Number</u>	<u>Amount</u> (¥ '000)
2-1. Director			
General Director	12	1	12
Vice General Director	6	1	6
Director	3	8	24
Deputy Director	3	2	6
2-2. Man Staff			
General Manager	72	1	* 72
Division Chief	50	7	*350
Clerk - 1st Class	30	10	300
- 2nd Class	25	12	300
- 3rd Class	15	9	135
Expert	45	11	*495
2-3. Woman Staff			
Woman Clerk - 1st Class	10	10	100
- 2nd Class	96	15	144
2-4. Others			
Common Irrigator	3	29	97
Leader of Management Group	3	29	97
Leader of Farming Group	1	136	136
Hired Labor	(1,917man x ¥30 x 12mos) 690		
Total		<u>281</u>	<u>2,964</u>

## 3. Running cost (1,000 ₪)

## 3-1. Operation cost of pumps

0.5 ₪/kwh x 170kw x 1,842hr x 3units = 470

0.5 ₪/kwh x 140kw x 1,802hr x 3units = 378

0.5 ₪/kwh x 300kw x 1,817hr x 3units = 818

3-2. Others 1,917

Total 3,583

## 4. Depreciation cost of vehicles and equipments

## 4-1. Vehicles

Truck 2 x ₪260,000 x 20% depn. = 104

Pickup Truck 2 x ₪100,000 x 20% " = 40

Motor Bicycle 24 x ₪ 12,000 x 20% " = 58

## 4-2. Wireless phone

Main station (ALRC) 1x2x₪2,000x10% depn.= 1

Sub-station (Production unit) 4x2x₪2,000x10% " = 2

Sub-station (Management group) 29x2x₪2,000x10% " =12

## 4-3. Office equipment

Typewriter 7 x ₪10,000 x 20% depn. = 14

Calculation machine 11 x ₪ 1,500 x 20% " = 3

Copying machine 5 x ₪20,000 x 20% " = 20

Total 254

Grand Total (1 to 4) 12,200  
(12,183)

Terms of Reference for the Consultant's Services

1. Objectives

The purpose of the consultant's services is to assist the Government in the effective implementation of the Irrigated Agricultural Development Project in the West Bank Tract of Greater Chao Phya.

2. Specific Terms of Reference

The consultants will provide a team to undertake the followings of the consultant's services.

- (a) To prepare detailed design, cost estimates, specifications and tender documents for civil works and for procurement of operation and maintenance equipments, construction machineries, construction materials and other goods and instruments necessary for the project;
- (b) To assist ALRO in the supervision of construction works under the project.
- (c) To assist and advise the Project Manager in preparing monthly construction schedule and work records;
- (d) To assist the relevant Government agencies to prepare agri-institutional establishment program which will include provision for:
  - (i) effective education of farmers in the project area through intensive demonstration and other means to enable them to adopt new cropping systems, diversify crops, use improved varieties of crops and improve cultivation practices;
  - (ii) strengthening of existing farmer's organizations and establishment of new organizations, of local farmers for the effective channelling of agricultural services;
  - (iii) adequate supply of agricultural credit and production requisites to these farmers as required for the recommended system of intensive cropping; and

- (e) To train local counterpart personnel in all phases of project activities.

### 3. Expertise

- (a) Senior Irrigation Engineer with sufficient experience in the planning, design, and operation and maintenance of irrigation and drainage system and with sufficient seniority to function as team leader
- (b) Design Engineer with sufficient experience in the planning, design and construction of the pumps, road and dike, canal systems and on-farm facilities
- (c) Equipment Engineer with experience in management and organization of operation and maintenance of construction equipments;
- (d) Engineering Geologist with sufficient experience in the geological and soil mechanical investigation for the major structures such as canal structures, pumping station, bridge and etc.
- (e) Agronomist with sufficient experience in the crop and soil management under paddy irrigation and upland crops at the farm level as well as in agricultural supporting services for irrigated agriculture.
- (f) Agri-institutional Expert with broad experience in the agricultural supporting services for irrigated agriculture
- (g) Economist with sufficient experience in the establishment of farm budgets, marketing and credit services and in the evaluation of economic and financial viability of the irrigated agricultural development project.
- (h) Topographical Surveyor with sufficient experience in the planning execution and supervision of the topographical surveying works.



4. Services to be provided by the Government

The Government will provide the following for carrying out the Consultant's services.

- (a) All available documents, drawing, maps, statistics, data and other information related to the Irrigated Agricultural Development Project in the West Bank Tract.
- (b) Suitable full-time counterparts personnel, including engineers, technicians and professionals, as required for the project; and
- (c) To exempt the Consultants from (or bear the cost of) any taxes, duties, fees, levies and other impositions imposed under its laws and regulations in the respect of;
  - (i) any payment made to the consultants in connection with the carrying out their services;
  - (ii) any equipment and materials and supplies brought into the territories of the Government for the purpose of carrying out the services; and
  - (iii) any property brought by the members of the Consultants for their personnel use and consumption.

Figure A.5-4 shows the proposed schedule for the Consultant's services.



Table A. 6-1 Structure of Paddy Price per Ton

1. World market price of milled rice in 1985 in constant end-1974 prices, Bangkok fob.	US\$222	IBRD forecast in constant end-1974 prices 5% broken US\$ 285/MT 25% broken US\$ 206/MT Assumption: 20% of 5% broken and 80% of 25% broken Weighted average price: US\$221.8 Weighted price index of "Total food" 1973 : 100 64 1974 : 157 or 100 1976 : 147 94 Source: "Price forecast for main primary commodities" July 1975. IBRD
2. World market price of milled rice in 1985 in constant 1976 prices	US\$209	
3. Price of milled rice in Baht	฿4,174	
4. Commission for middleman and exporter	฿ -417	
5. Transport & handling	฿ -295	Transport and handling Mill to boat 40 Freight by boat 35 Handling 20 Ware house 200 10/sack/month 2 month
6. Rice price at rice mill	฿3,462	
7. Transport from farm to mill	฿ -40	
8. Milling charge	฿ -150	฿100/ton Paddy, ฿100÷0.66 = ฿150/ton milled rice
9. Farm gate price of milled rice	฿3,272	
10. Farm gate price of paddy excluding by-products	฿2,160	Recovery rate: 0.66
11. By-products	฿ 196	$0.7 \frac{1/}{x} (1 - 0.94) \frac{2/}{}$ ; $0.66 = 0.06$ $3,272 \times 0.06 = 196$ 1/: Recovery rate of hulled rice from paddy in weight 2/: Recovery rate of milled rice from hulled rice in weight
12. Farm gate price of paddy	฿2,356 ฿2,360	

Farm Labor Analysis

1. Labor Supply

Because almost all of the farm lands have been planted to rice, seasonal fluctuation of farm labor requirement has been high, namely, during the seasons of transplanting and harvesting of rice, relatively large sized farmers have suffered from scarcity of hired labor, resulting in introducing them from outside the Project Area.

On the other hand, in the slack seasons, even large sized farmers have little opportunity of employment.

In future, however, when the double cropping of rice and horticulture, animal husbandry and inland fishery are introduced through the provision of facilities for flood control and irrigation, and on-farm facilities, the said fluctuation of farm labor requirement is expected to be diminished.

But a substantial increase in absolute labor requirement will cause severe scarcity of labor forces in near future if the growth rate of population will remain at low of 1.9% as it is.

To meet the increase in farm labor requirement, workable population should be increased by preventing emigration and by U-turn of ones who have deserted the villages of the Project Area.

Furthermore, according to the proposed cropping pattern, the peak month of labor demand will be shifted from June at present to February, March, August and November in future.

Present conditions of labor requirement in the neighboring area are similar to those in the Project Area and are not expected to change simultaneously with the Project Area, therefore, many of the workers of these areas would be expected to commute to the Project Area on a daily basis.

In this context, the available labor force for the farming in the Project Area including casual migration was estimated, as shown in the Tables A.6-2, on the assumption that the workable heads per family are three, workable days per month are 25 and available labor force which commutes to the Project Area is 40 per cent of those inside.

A part of this potential labor force, for instance, of well-off farmers, in fact, does not participate in manual farm work, but it can be compensated by a part of non-farmers' labor force which is estimated at 20 per cent of total labor force in the Project Area.

## 2. Existing Wage Rate

It is difficult to obtain reliable data on the existing wage rates for farm labor, because of the following reasons;

- Most of the farm labor is provided by the farm family.
- An exchange of labor among neighbors and relatives is common.
- Land preparation, transplanting, weeding, harvesting etc. are paid by the job.
- Therefore, an employment on a daily basis is limited.

So far as the information obtained by field survey, the current wage rates are likely to be 15 to 18 Baht for male and 10 to 12 Baht for female on transplanting of rice, and about 20 Baht for male and 10 to 12 Baht for female on harvesting of rice.

In the case of permanent labor in orchards, employees are paid 15 Baht for male and 12 Baht for female per day.

## 3. Economic Costs of Farm Labor

It is postulated that the marginal opportunity cost of farm labor in the Project Area can be approximated by an S-shaped curve (Figure A. 6-2). The marginal opportunity costs are positive at all levels of labor demand and increase as more labor is employed in farm work.

The increase is slow initially, reflecting the scarcity of alternative productive employment, but becomes more rapid as the labor supply becomes fully used.

At the employment level corresponding to full employment of available family labor in the Project Area and surrounding areas (Table A. 6-2), the marginal opportunity costs are assumed equal to the market wage rates. If the demand for farm labor were to increase beyond this level, as would generally happen under the with project conditions the opportunity cost would continued to rise, though at a slower pace. At a level of about 30 per cent about full employment it is postulated that the opportunity cost curve would flatten out, at that cost the available labor pool would be drastically enlarged, as laborers from many other areas could be diverted to farm work in the Project Area.

It is, therefore, reasonable to approximate the S-shaped curve by three straight line segments. (Figure A.6-1)

Three points determine the position of the curve. Point A represents the minimum opportunity cost of farm labor which is judged to be four Baht per man-day, equal to economic values of alternative employment (casual non-farm labor, fishing, house repairs, etc.) plus the cost to supply the additional food requirements of strenuous farm work with the Project. Point B indicates that at full employment of available labor in the Project Area and in neighboring areas, the opportunity cost would equal the market wage rate of 15 Baht per man-day. The horizontal segment to the right of Point C indicates that at 20 Baht per man-day as many hired laborers as needed would be available for farm labor in the Project Area.

The curve in Figure A.6-2 was drawn by idealizing figure. The monthly opportunity cost may be read directly from the curve at the corresponding level of labor demand.

#### 4. Labor Demand

Based on the results of field survey and information from extension workers, labor requirement per hectare and per production unit were estimated as shown in the tables A.6-3 and A.6-4.

Total farm labor requirements at present and in future with Project were computed on the basis of the above tables and the present and projected cropping patterns, and the results were shown in Tables A.6-5 and A.6-6.

#### 5. Yearly Change in Labor Demand

Labor demand in future was assumed that the demand level would remain as it was with the situations of without Project, while with the with-Project-situations, the demand would increase corresponding to the intensification and diversification of farming through the implementation of the Project in the Area.

Estimated labor demand are tabulated in Tables A.6-7 and A.6-8.

#### 6. Ratios of Labor Demand to Available Labor Force

Monthly farm labor demand as percentage of available labor force was derived from Tables A.6-2, A.6-7 and A.6-8, as shown in Table A.6-9 and A.6-10.

#### 7. Wage Rate (Opportunity Cost of Labor per Man-day)

The monthly marginal opportunity costs of labor per man-day were read from the curve in Figure A.6-2 at the corresponding level of labor demand and shown in Tables A.6-11 and A.6-12.

#### 8. Total Labor Costs

The total labor costs were computed by multiplying labor demand by wage rates and the results were tabulated in Tables A.6-13 and A.6-14. The differences between total labor costs without project and the ones with project were applied to Table A.6-16.

Table A.6-2 Available Farm Labor Force in Future

Year	Without Project			With Project		
	Population Growth Rate %	Family Size	No. of Farm Families	Population Growth Rate %	Family Size	No. of Farm Families
1976	1.9	6.34	1,610	1.9	6.34	1,610
1977	1.9	6.25	1,660	2.1	6.25	1,670
1978	1.9	6.16	1,720	2.3	6.16	1,730
1979	1.9	6.08	1,770	2.5	6.08	1,800
1980	1.9	5.99	1,840	2.8	5.99	1,870
1981	1.9	5.90	1,900	3.0	5.90	1,970
1982	1.9	5.83	1,960	3.0	5.83	2,040
1983	1.9	5.74	2,030	3.0	5.74	2,140
1984	1.9	5.66	2,100	3.0	5.66	2,230
1985	1.9	5.58	2,170	2.8	5.58	2,330
1986	1.9	5.50	2,240	2.6	5.50	2,420
1987	1.9	5.50	2,280	2.5	5.50	2,480
1988	1.9	5.50	2,330	2.5	5.50	2,550

Note: 1/ Average size of families 1970  
Changwat Ayutthaya: 5.4 persons  
Project area : 5.34 persons

Available Farm Labor Force per Month including Surrounding Areas

Year	Without Project		Total	With Project		Total
	Inside	Outside		Inside	Outside	
1980	138	14	152	140	21	161
1981	143	14	157	148	25	173
1982	147	13	160	153	31	184
1983	152	13	163	161	34	195
1984	158	12	170	167	45	212
1985	163	12	175	175	56	231
1986	168	11	179	182	65	248
1987	171	11	182	186	73	259
1988	174	10	184	191	76	267

(Unit: 1,000 man-days)



Table A.6-3 Monthly Labor Requirement per Hectare by Crop at Present

Per	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	(Unit: Man-day)			Total
										Oct.	Nov.	Dec.	
Rice													
Wet, BC	31.6	8.1	0	1.3	4.8	5.9	5.8	1.0	0	0	0	19.9	78.4
Wet, TP	0	0	0	8.7	28.8	17.6	8.0	37.0	23.9	0	0	0	124.0
Dry, TP	0	9.7	20.6	22.2	6.0	33.4	28.7	3.4	0	0	0	0	124.0
Upland Crops	0	0	6.0	6.0	6.0	6.0	6.0	6.0	0	0	0	0	36.0
Citrus	28.0	28.0	28.0	12.0	12.0	28.0	28.0	28.0	12.0	12.0	12.0	12.0	240.0

Table A.6-4 Monthly Labor Requirement per Hectare or Production Unit by Crop in Future with Project

Per	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	(Unit: Man-day)			Total
										Oct.	Nov.	Dec.	
Rice (Total)	9.0	23.5	20.4	7.0	16.3	19.6	16.8	25.0	15.1	7.5	19.8	18.2	198.2
Wet, BC	-	-	-	-	-	0.6	12.9	25.0	15.1	7.5	19.8	18.2	99.1
Wet, TP	9.0	23.5	20.4	7.0	16.3	19.0	3.9	-	-	-	-	-	99.1
Vegetables	30	30	30	30	30	30	30	30	30	30	30	30	360
Citrus	30	30	30	30	30	30	30	30	30	30	30	30	360
Animal	15	15	15	15	15	15	15	15	15	15	15	15	180
Fish	15	15	15	15	15	15	15	15	15	15	15	15	180

Note: PU; Production Unit

Table A.6-5 Monthly Labor Requirement by Crop at Present

Area (ha)	(Unit: 1,000 Man-days)												
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
Rice (Total)	69	41	49	71	76	124	96	76	42	0	0	44	688
Wet, BC	69	18	0	3	11	13	13	2	0	0	0	44	172
Wet, TP	0	0	0	15	51	31	14	66	42	0	0	0	220
Dry, TP	0	23	49	53	14	80	69	8	0	0	0	0	296
Upland Crops	0	0	1	1	1	1	1	1	0	0	0	0	6
Citrus	7	7	7	3	3	7	7	7	3	3	3	3	60
Total	76	48	57	75	80	132	104	84	45	3	3	47	754

Table A.6-6 Monthly Labor Requirement by Crop in Future with Project

Area & PU	(Unit: 1,000 Man-days)												
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	Total
Rice (Total)	85	224	195	67	156	187	160	239	144	72	189	174	1,893
Wet, TP	0	0	0	0	0	6	123	239	144	72	189	174	947
Dry, TP	85	224	195	67	156	181	37	0	0	0	0	0	947
Vegetables	500	15	15	15	15	15	15	15	15	15	15	15	180
Citrus	500	15	15	15	15	15	15	15	15	15	15	15	180
Animal	250	4	4	4	4	4	4	4	4	4	4	4	48
Fish	250	4	4	4	4	4	4	4	4	4	4	4	48
Total	124	262	233	105	194	225	198	277	182	110	227	212	2,349

Note: BC; Broadcast TP; Transplanted PU; Production Unit

Table A-6-7 Total Labor Demand by Month without Project

Year	(Unit: 1,000 Man-days)												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1980 to 1988	76	48	57	75	80	132	104	84	45	3	3	47	754

Note: It is assumed that the labor demand for farming in the Project area is constant in the conditions of without-Project

Table A-6-8 Total Labor Demand by Month with Project

Year	AL	(Unit: 1,000 Man-days)												Total
		Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1980	161	76 (41)	48 (41)	57 (49)	75 (49)	80	132	104	84	45	3	3	47	754
1981	173	79 (29)	61 (29)	69 (29)	77 (29)	88	138	111	96	55	12	16	57	859
1982	184	84 (28)	84 (28)	85 (28)	80 (28)	99	148	120	117	68	21	41	73	940
1983	195	91 (28)	116 (28)	113 (28)	85 (28)	118	163	135	148	89	38	77	95	1,268
1984	212	101 (28)	160 (28)	150 (28)	91 (28)	140	181	154	186	116	61	122	125	1,587
1985	231	111 (28)	203 (28)	184 (28)	97 (28)	163	201	173	225	144	83	169	156	1,909
1986	248	118 (28)	237 (28)	212 (28)	101 (28)	180	214	187	254	164	99	202	179	2,147
1987	259	122 (28)	254 (28)	227 (28)	104 (28)	189	221	195	267	175	108	218	205	2,285
1988	267	124 (28)	262 (28)	233 (28)	105 (28)	194	225	198	227	182	110	227	212	2,349

AL : Available Labor Forces

Note: The figures in parentheses indicate the labor required for construction works of the Project, and are excluded from ones outside

Table A.6-9 Monthly Farm Labor Demand as Percentage of Potential Full Employment Without Project

Year	AL	(Unit: %)												
		Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1980	152	50	32	38	49	53	87	68	55	30	2	2	2	31
1981	157	48	31	36	48	51	84	66	54	29	2	2	2	30
1982	160	48	30	36	47	50	83	65	53	28	2	2	2	29
1983	163	47	29	35	46	49	81	64	51	28	2	2	2	29
1984	170	45	28	34	44	47	78	61	49	26	2	2	2	28
1985	175	43	27	33	43	46	75	59	48	26	2	2	2	27
1986	179	42	27	32	42	45	74	58	47	25	2	2	2	26
1987	182	42	26	31	41	44	73	57	46	25	2	2	2	26
1988	184	41	26	31	41	43	72	57	46	24	2	2	2	26

Table A.6-10 Monthly Farm Labor Demand as Percentage of Potential Full Employment With Project

Year	AL	(Unit: %)												
		Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1980	161	73	56	61	73	50	82	65	52	28	2	2	2	29
1981	173	74	64	68	73	51	80	64	56	32	7	9	9	33
1982	184	61	61	61	59	54	80	65	64	37	11	22	22	40
1983	195	61	74	72	58	61	84	69	76	46	19	39	39	49
1984	212	48	75	71	43	66	85	73	88	55	29	58	58	59
1985	231	48	88	80	42	71	87	75	97	62	36	73	73	68
1986	248	48	96	85	41	73	86	75	102	66	40	81	81	72
1987	259	47	98	88	40	73	85	75	103	58	42	84	84	79
1988	267	46	98	87	39	73	84	74	104	58	41	85	85	79

FIGURE A.6 - 1. TYPICAL OPPORTUNITY COST CURVES FOR FARM LABOR

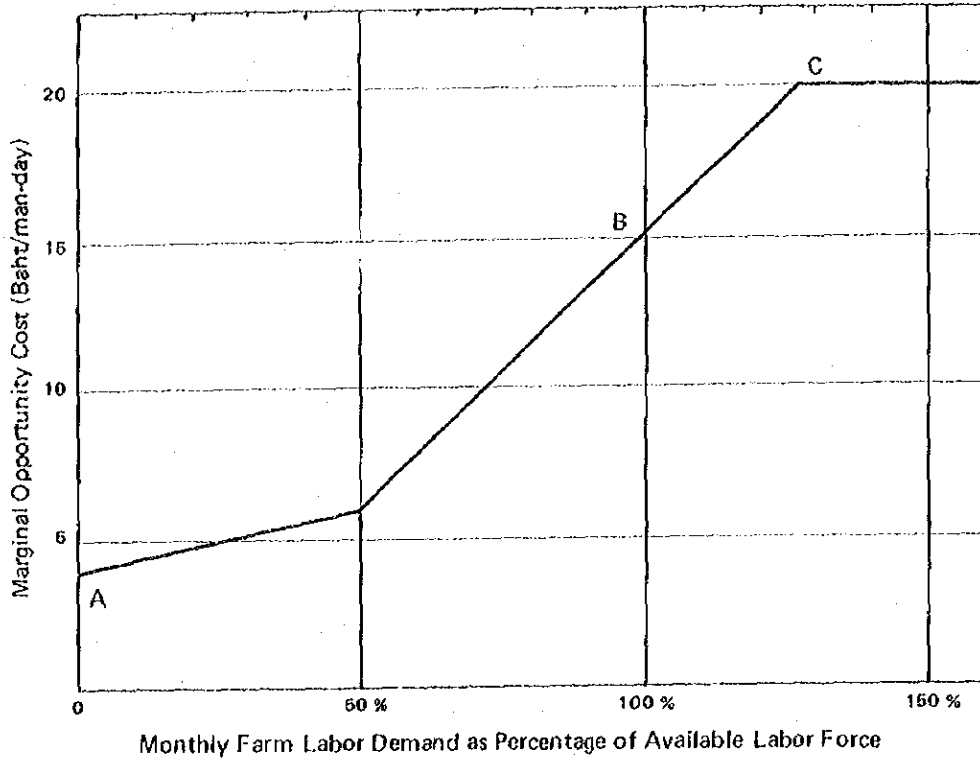


FIGURE A.6 - 2. OPPORTUNITY COST CURVES FOR FARM LABOR

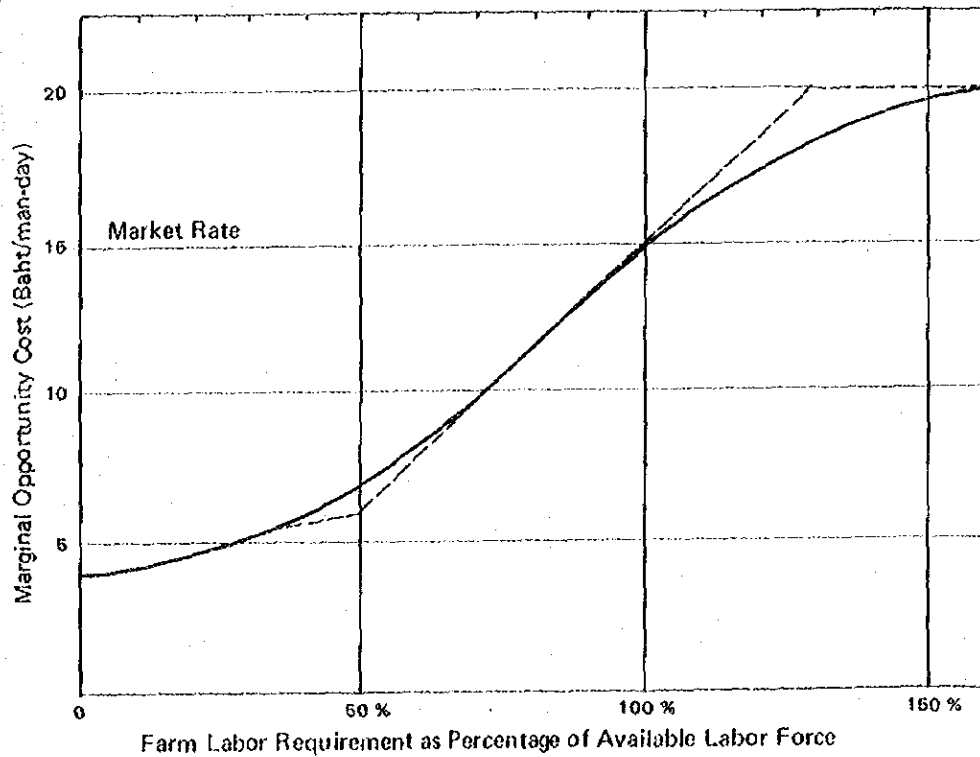


Table A.6-11 Wage Rate by Month without Project

Year	(Unit: Baht/Man-day)											
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
1980	7	5	6	7	8	13	10	8	5	4	4	5
1981	7	5	6	7	7	12	9	8	5	4	4	5
1982	7	5	6	7	7	12	9	8	5	4	4	5
1983	7	5	6	7	7	12	9	7	5	4	4	5
1984	7	5	6	7	7	11	9	7	5	4	4	5
1985	6	5	6	6	7	11	8	7	5	4	4	5
1986	6	5	5	6	7	11	8	7	5	4	4	5
1987	6	5	5	6	7	10	8	7	5	4	4	5
1988	6	5	5	6	6	10	8	7	5	4	4	5

Table A.6-12 Wage Rate by Month with Project

Year	(Unit: 1,000 Man-day)											
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
1980	10	8	9	10	7	12	9	7	5	4	4	5
1981	10	9	10	10	7	12	9	8	5	4	4	6
1982	9	9	9	8	8	12	9	9	6	4	5	6
1983	9	10	10	8	9	12	10	11	7	5	6	7
1984	7	11	10	6	9	12	10	13	8	5	8	8
1985	7	13	12	6	10	13	11	15	9	6	10	10
1986	7	14	12	6	10	13	11	15	9	6	12	10
1987	7	15	13	6	10	12	11	15	10	6	12	11
1988	7	15	13	6	10	12	11	15	10	6	12	11

Table A.6-13 Total Labor Costs by Month without Project

Year	(Unit: 1,000 Baht)												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1980	532	240	342	525	640	1,716	1,040	672	225	12	12	235	6,191
1981	532	240	342	525	560	1,584	936	672	225	12	12	235	5,875
1982	532	240	342	525	560	1,584	936	672	225	12	12	235	5,875
1983	532	240	342	525	560	1,584	936	588	225	12	12	235	5,791
1984	532	240	342	525	560	1,452	936	588	225	12	12	235	5,659
1985	456	240	342	450	560	1,452	832	588	225	12	12	235	5,404
1986	456	240	285	450	560	1,452	832	588	225	12	12	235	5,347
1987	456	240	285	450	560	1,320	832	588	225	12	12	235	5,215
1988	456	240	285	450	480	1,320	832	588	225	12	12	235	5,135

Table A.6-14 Total Labor Costs by Month with Project

Year	(Unit: 1,000 Baht)												Total
	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.	
1980	760	384	513	750	560	1,584	936	588	225	12	12	188	6,512
1981	790	549	690	770	616	1,656	999	768	275	48	54	342	7,567
1982	756	756	765	640	792	1,776	1,080	1,053	408	84	205	438	8,753
1983	819	1,160	1,130	680	1,062	1,956	1,350	1,628	623	190	462	665	11,725
1984	707	1,760	1,500	546	1,260	2,172	1,540	2,418	928	305	976	1,000	15,212
1985	777	2,639	2,208	582	1,630	2,613	1,903	3,375	1,296	516	1,690	1,560	20,789
1986	826	3,318	2,544	606	1,800	2,782	2,057	3,810	1,476	594	2,424	1,790	24,027
1987	854	3,810	2,951	624	1,890	2,652	2,145	4,005	1,750	648	2,616	2,255	26,200
1988	868	3,930	3,029	630	1,940	2,700	2,178	4,155	1,820	660	2,724	2,332	26,966

Table A.6-15 Net Production Value before Labor by Crop at Present and at Full Development

Item	Unit	R i c e							V e g e t a b l e s					
		Without Project			With Project				Without Project	With Project				
		Wet Season		Dry Season	Total	Wet Season	Dry Season	Total	Coconut <sup>2/</sup>	Chinese Cabbage	Cabbage	Cauli-flower	Total	Average
BC	TP	TP	(Avg.)	TP	TP	(Avg.)								
Yield	ton/ha	1.6	2.2	3.0	(2.294)	(4.2)	4.7	(4.45)		14.6	15.2	9.0	38.8	12.93
Price	Baht/ton	2,360	2,360	2,360	(2,360)	2,360	2,360	(2,360)		1,200	1,600	2,500	-	1,658
Value	Baht/ha	3,776	5,192	7,080	(5,414)	9,912	11,092	(10,502)	10,000	17,520	24,320	22,500	64,340	21,447
Cash Cost <sup>1/</sup>	Baht/ha	1,689	2,510	2,510	(2,227)	3,042	3,210	(3,126)	0	6,572	9,183	7,995	23,750	7,917
NPV	Baht/ha	2,087	2,682	4,570	(3,187)	6,870	7,882	(7,376)	10,000	10,948	15,137	14,505	40,590	13,530
Area	Ha	2,190	1,773	2,390	6,353	9,542	9,542	19,084	106	500	500	500	1,500	500
Total NPV	(฿ '000)	4,571	4,755	10,922	20,248	65,554	75,210	140,764	1,060	5,474	7,569	7,253	20,295	6,765
Incremental NPV	(฿ '000)					140,764 - 20,248 = 120,516							20,295 - 1,060 = 19,235	

<sup>1/</sup>: Labor costs are not included in the cash cost.

<sup>2/</sup>: It is assumed that other crop than rice is coconut, yield: 5 tons, price: 2 Baht/kg

BC: Broadcast

TP: Transplanted



Net Production Value before Labor by Crop at Present and at Full Development (cont.)

Item	Unit	C i t r u s				Item	Unit	A n i m a l					F i s h					
		Without Project	With Project					Swine	Chicken	Ducks	Total	Average	Carp	Ciclid	Catfish	Total	Average	
			Existing	Expanding	Total (Avg.)													
Yield	Ton/ha	15	30	30	(30)	Products		8,080	900kg(Meat) 108,000eggs	675kg(Meat) 67,500eggs	-	-	3.0	3.0	7.5	13.5	4.5	
Price	Baht/ton	3,000	3,000	3,000	(3,000)	Price	Baht/egg	13	12 0.95/egg	10 1.0/egg	-	-	8	8	12	-	10.2	
Value	Baht/ha	45,000	90,000	90,000	(90,000)	Value	Baht/PU	105,040	113,400	74,250	292,690	97,560	24,000	24,000	90,000	138,000	46,000	
Cash Cost	Baht/ha	18,930	27,040	27,040	(27,040)	Cash Cost	Baht/PU	87,600	90,615	43,900	222,115	74,040	14,076	13,060	80,719	107,850	35,950	
NPV	Baht/ha	26,070	62,960	62,960	(62,960)	NPV	Baht/PU	17,440	22,785	30,350	70,575	23,530	9,930	10,940	9,281	30,150	10,050	
Area	Ha	240	240	260	500	Number of PU		83	84	83	250	83.3	83	83	84	250	83.3	
Total NPV	1,000Baht	6,257	15,110	16,370	31,480	Total NPV	1,000Baht	1,448	1,914	2,519	5,881	1,960	824	908	780	2,512	837	
Incremental NPV	1,000Baht		31,480 - 6,257 = 25,223			Incremental NPV	1,000Baht		5,881					2,512				

Note: PU - Production unit

Table A.6-16 Economic Cost and Benefit Stream

(unit: million Baht)

Year	C o s t			I n c r e m e n t a l P r o d u c t i o n B e n e f i t		
	Construction Costs	Operation & Maintenance Costs	Sub-total	Net Production Value before Labor		Project Benefits
				Labor <sup>1/</sup>	Labor Costs	
1978	25.5	0	25.5	0	0	-25.5
1979	70.6	0	70.6	0	0	-70.6
1980	190.0	1.8	191.8	-8.4	0.3	-200.5
1981	118.5	6.6	125.1	0.2	1.7	-126.6
1982	49.9	9.0	58.9	14.8	2.9	-47.0
1983	50.3	10.9	61.2	40.9	5.9	-26.2
1984	-12.53/	12.2	-0.3	84.5	9.6	75.2
1985	0	12.2	12.2	129.3	15.4	101.7
1986	0	12.2	12.2	148.8	18.7	117.9
1987	0	12.2	12.2	165.2	21.0	132.0
1988	0	12.2	12.2	173.2	21.8	139.2

Notes: 1/ Operation and maintenance costs indicate all the costs for the Land Reform Cooperative who is proposed to conduct the operation and maintenance of irrigation and drainage facilities and also the supporting services.

2/ Costs for construction of orchards and fish-ponds, and net value of production which will be lost during construction works (dry season) are counted in the farm management costs.

3/ Residual values of equipment and provisional facilities for the project construction.

Table A.6-17 Crop Budgets per Hectare without Project

(unit: Baht)

Item	R i c e			Citrus
	Wet Season		Dry Season	
	BC	TP	TP	
1. Gross Income (Baht)	3,520	4,840	6,600	45,000
Yield (ton)	1.6	2.2	3.0	15.0
Price (¥/kg)	2.2	2.2	2.2	3.0
2. Production Costs excl. Labor Cost	1,899	2,677	2,677	24,543
Seeds	312	108	108	240 <sup>1/</sup>
Fertilizers	0	658	658	5,684
Agro-chemicals	385	581	581	10,364
Draft Animals	90	90	90	700 <sup>2/</sup>
Equipment	787	845	845	170
Others	79	114	114	858
Interest	169	240	240	6,486
Land Tax	41	41	41	41
3. NPV before Labor	1,621	2,163	3,923	20,457
4. Hired Labor				
5. Net Crop Income				

<sup>1/</sup>: Costs for recruiting trees<sup>2/</sup>: Costs for fuel

Table A. 6-18 Crop Budgets per Hectare (Field Crops)

Item	R i c e			Vegetables				Citrus
	Wet Season	Dry Season	Total	Chinese Cabbage	Cabbage	Cauli-flower	Total	
1. Gross Income (Baht)	9,240	10,340	19,580	17,520	24,320	22,500	64,340	90,000
Yield (ton)	4.2	4.7	-	14.6	15.2	9.0	(12.14)	30
Price (B/kg)	2.2	2.2	-	1.2	1.6	2.5	5.3	3
2. Production Costs excl'd. Labor Cost	3,214	3,387	6,601	6,769	9,610	8,126	24,505	33,531
Seeds	104	104	208	315	898	776	1,989	340
Fertilizers	878	1,020	1,898	3,028	3,918	3,523	10,469	8,120
Agro-chemicals	763	763	1,526	1,966	2,638	2,051	66,550	14,805
Equipment	1,063	1,063	2,126	637	996	791	2,424	1,240
Others	140	148	288	297	423	357	1,077	1,225
Interest	225	248	473	499	710	600	1,809	7,719
Land Tax	41	41	82	27	27	28	82	82
3. NPV before Labor	6,026	6,953	12,979	10,751	14,710	14,374	39,835	62,130

Table A.6-18 (Cont.) Crop Budgets per Production Unit (Animal and Fish)

(Unit: Baht)

Item	Animals				Fish			
	Swine	Chicken	Ducks	Average	Carp	Tilapia	Catfish	Average
1. Gross Income (Baht)	105,040	113,400	74,250	97,560	24,000	24,000	90,000	46,000
Yield	8,080	108,000eggs 900head	67,500eggs 675heads		3.0tons	3.0	7.5	
Price	13	฿0.95/egg ฿ 12/head	฿1/egg ฿10/head		8 ฿/kg	8	12	
2. Production Costs excl. labor Costs	96,405	101,236	49,187	82,260	14,755	13,729	84,598	37,690
Feed	84,000	73,000	36,500	64,500	9,200	9,200	63,870	27,420
Recruiting	-	12,940	5,190	6,040	Fingerlings 3,200	"-" 2,240	"-" 12,000	5,810
Vaccination	1,800	2,050	1,500	1,780	-	-	-	-
Facilities	1,800	4,000	1,500	2,430	1,000	1,000	1,000	1,000
Others	4,380	4,600	2,235	3,730	670	622	3,844	1,715
Interest	4,415	4,636	2,252	3,770	675	657	3,874	1,735
Land Tax	10	10	10	10	10	10	10	10
3. NPV before Labor	8,635	12,164	25,063	15,300	9,245	9,644	5,402	8,310

Table A.6-19 Farm Budgets (Without Project)

Item	3 ha			4 ha			6 ha		
	Rice			Rice			Rice		
	Wet Season BC	Dry Season TP	Wet Season BC	Dry Season TP	Wet Season BC	Dry Season TP	Wet Season BC	Dry Season TP	
Gross Income	10,560	14,520	14,080	19,360	26,400	21,120	29,040	39,600	
Production Costs	6,000	9,140	8,440	13,185	12,690	13,775	21,760	21,175	
Costs excl. Labor Costs	5,700	8,030	7,600	10,710	10,710	11,390	16,060	16,060	
Hired Labor	300	1,110	840	2,475	1,980	2,385	5,700	5,115	
Net Income	4,560	5,380	5,640	6,175	13,710	7,345	7,280	18,425	

Table A.6-20 Farm Budgets (With Project)

Item	3 ha			4 ha			6 ha		
	Rice			Rice			Rice		
	Wet Season BC	Dry Season TP	Wet Season BC	Dry Season TP	Wet Season BC	Dry Season TP	Wet Season BC	Dry Season TP	
Gross Income	58,740	103,500	129,160	202,300	78,320	126,080	148,740	117,480	
Production Costs	19,943	38,027	85,080	142,173	27,534	46,728	55,754	45,846	
Costs excl. Labor Costs	19,803	37,707	84,760	139,753	26,404	44,308	53,334	39,606	
Hired Labor	140	320	320	2,420	1,130	2,420	2,420	6,240	
Net Income	38,797	65,473	44,080	60,127	50,786	76,352	92,986	71,634	

Table A.6-21 Farmers' Ability to Pay Water Charge and Investment Costs

(unit: Baht)

Item	3 ha			4 ha			6 ha
	2 ha Rice 1 ha Vege- tables	2 ha Rice 1 ha Citrus	3 ha Rice 1 PU Animals 1 PU Fish	3 ha Rice 1 ha Vege- tables	3 ha Rice 1 ha Citrus	3 ha Rice 1 ha Citrus	6 ha Rice
Net income	38,797	65,473	60,127	50,786	76,352	92,352	71,634
Amount for tax exemption	38,797	25,865	38,797	50,786	38,797	38,797	71,634
Taxable amount	0	39,608	21,330	0	37,555	53,555	0
Tax rate	-	0.08	0.07	-	0.08	0.10	-
Individual income tax	-	3,169	1,493	-	3,004	5,356	-
Amount after deduction of income tax (a)	38,797	62,304	58,634	50,786	73,348	86,996	71,634
Water Charge (OX costs) (b)	3,450	3,450	3,634	4,600	4,600	4,600	6,900
(b)/(a) x 100	9	5	6	9	6	5	10
Disposable Income (c)=(a)-(b)	35,347	58,854	55,000	46,186	68,748	82,396	64,734
FC Amortisation (d)	8,448	8,448	8,899	11,264	11,264	11,264	16,896
Amount left to individual farmer after deduction of FC repayment	26,899	50,406	46,101	34,922	57,484	71,132	47,838
(d)/(c) x 100	24	14	16	24	16	14	25

Table A.6-22 Contribution to Rice Export by Project

## 1. Incremental Paddy Production by Project

## (a) Present Production

1.6ton/ha x 2,190ha = 3,504ton	(Wet season broadcasting)
2.2ton/ha x 1,773ha = 3,900ton	(Wet season transplanting)
3.0ton/ha x 2,390ha = 7,170ton	(Dry season transplanting)
Total	<u>14,574ton</u>

## (b) Production with Project

4.2ton/ha x 9,542ha = 40,076ton	(Wet season transplanting)
4.7ton/ha x 9,542ha = 44,847ton	(Dry season transplanting)
Total	<u>84,923ton</u>

(c) Increment = (b) - (a) = 70,349 ton

## 2. Increasing Rice Consumption in Project Area

## (a) Present Consumption

10,200 (Population at present) x 197kg<sup>1/</sup> = 2,009 ton

## (b) Future Consumption

14,000 (Population in future) x 197kg<sup>1/</sup> = 2,758 ton

(c) Increasing Paddy Consumption = (b) - (a) = 749 ton

1/ 130kg ÷ 0.66 (Milling rate)

3. Contribution Amount to Export in Paddy = 1 - 2

= 69,600

4. Contribution Amount to Export of Milled Rice = 45,936 ton/year

## 5. Saving of Foreign Currency

45,936ton x 6,152¥/ton = ¥282.6 million : US\$14.1 million



