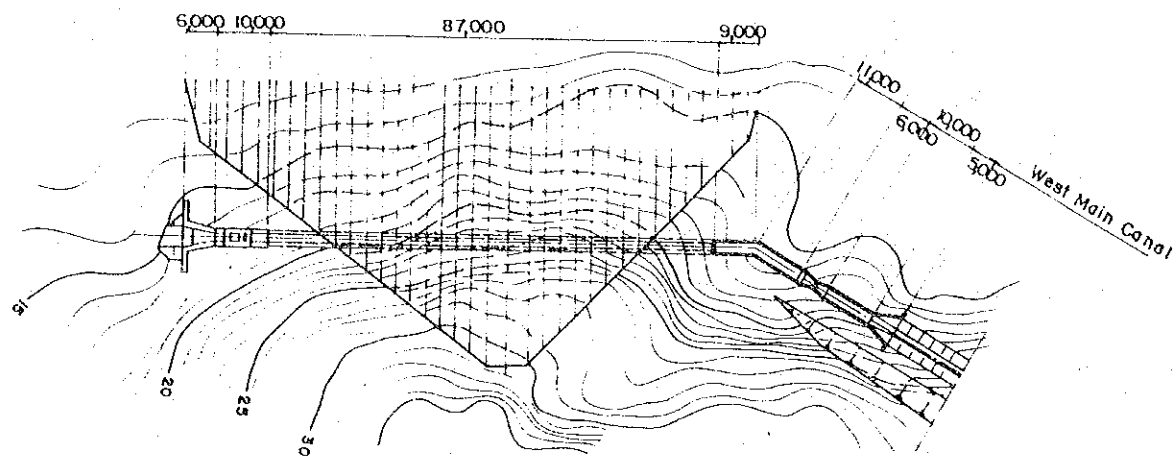
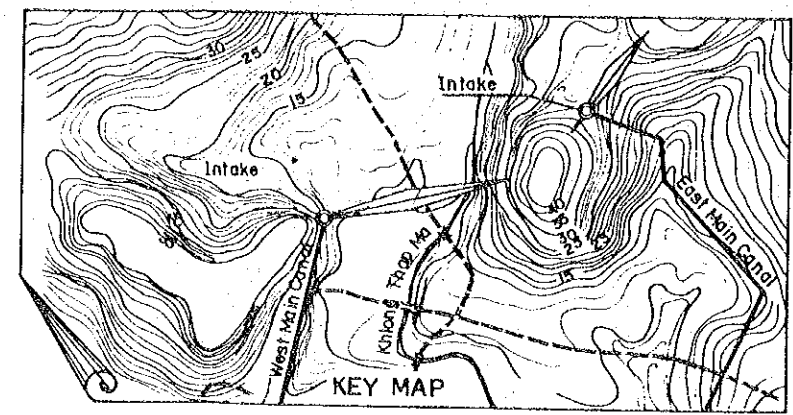


### Intake Facilities for West Main Canal

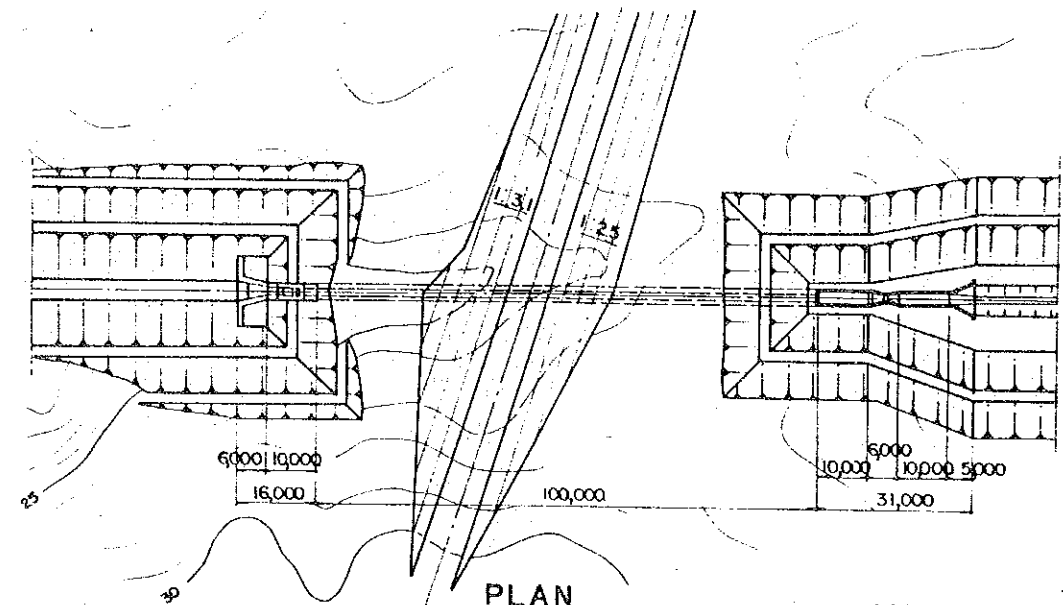


PLAN

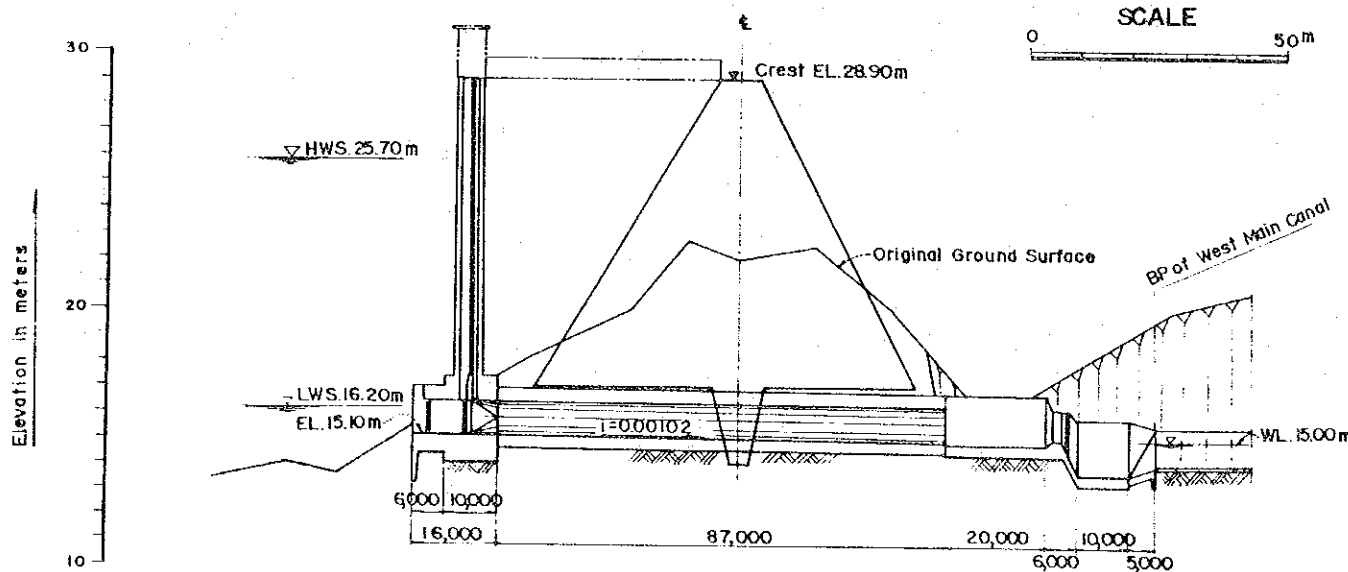


KEY MAP

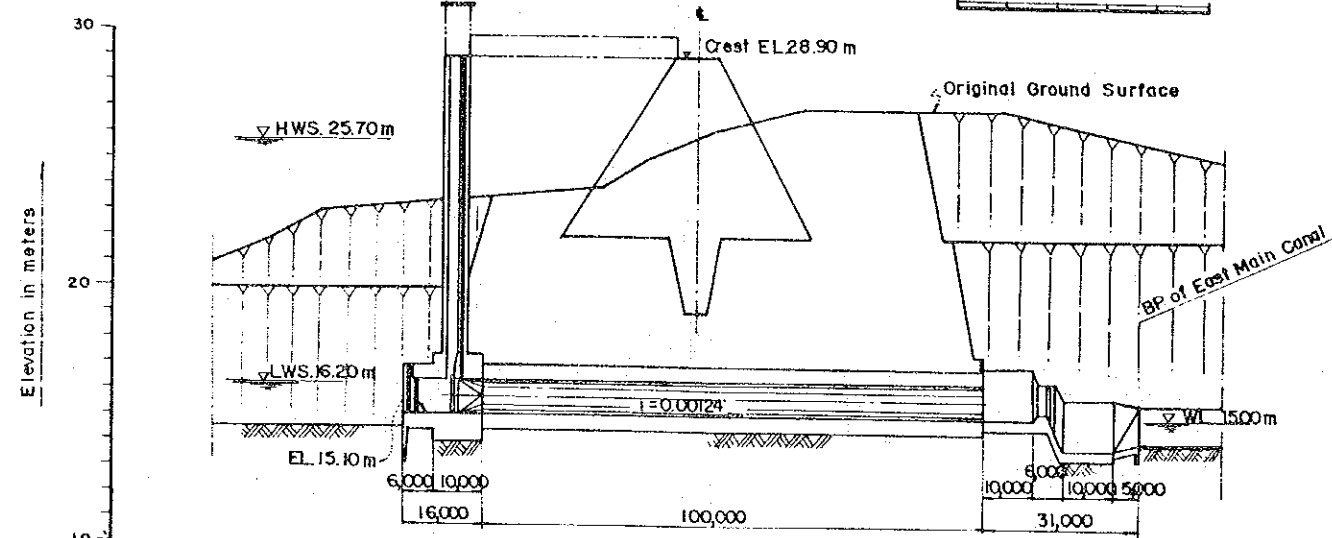
### Intake Facilities for East Main Canal



PLAN



PROFILE



PROFILE

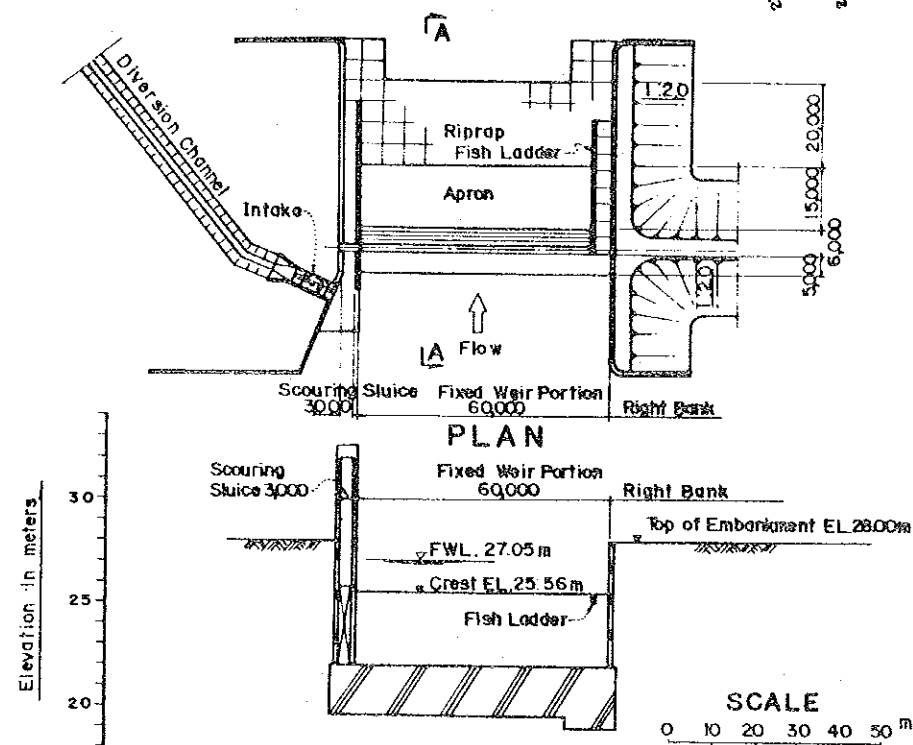
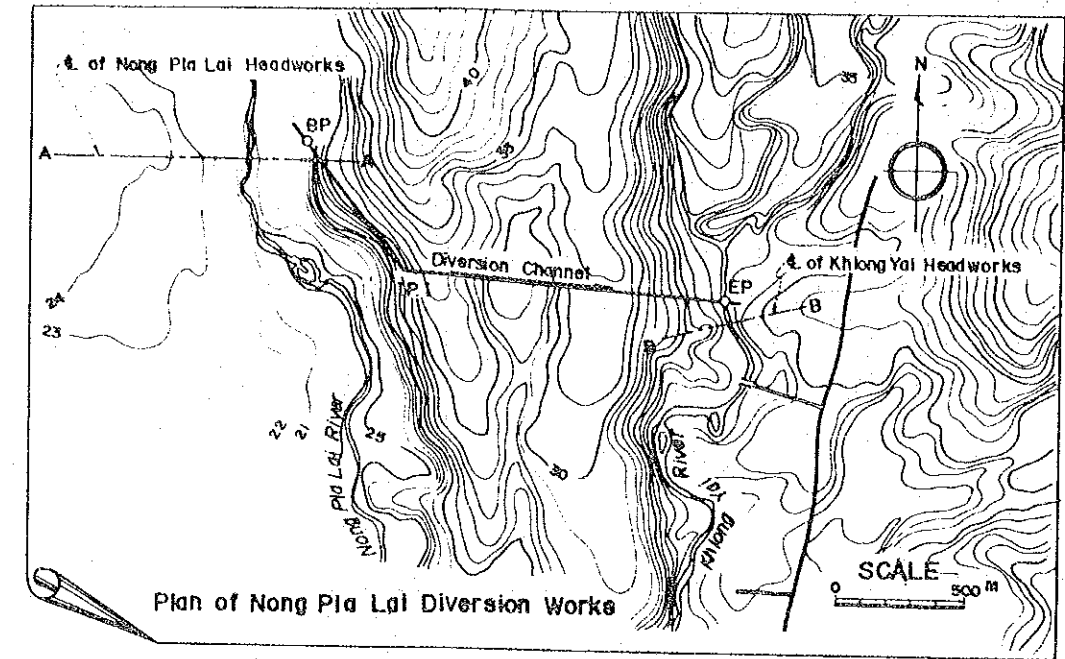
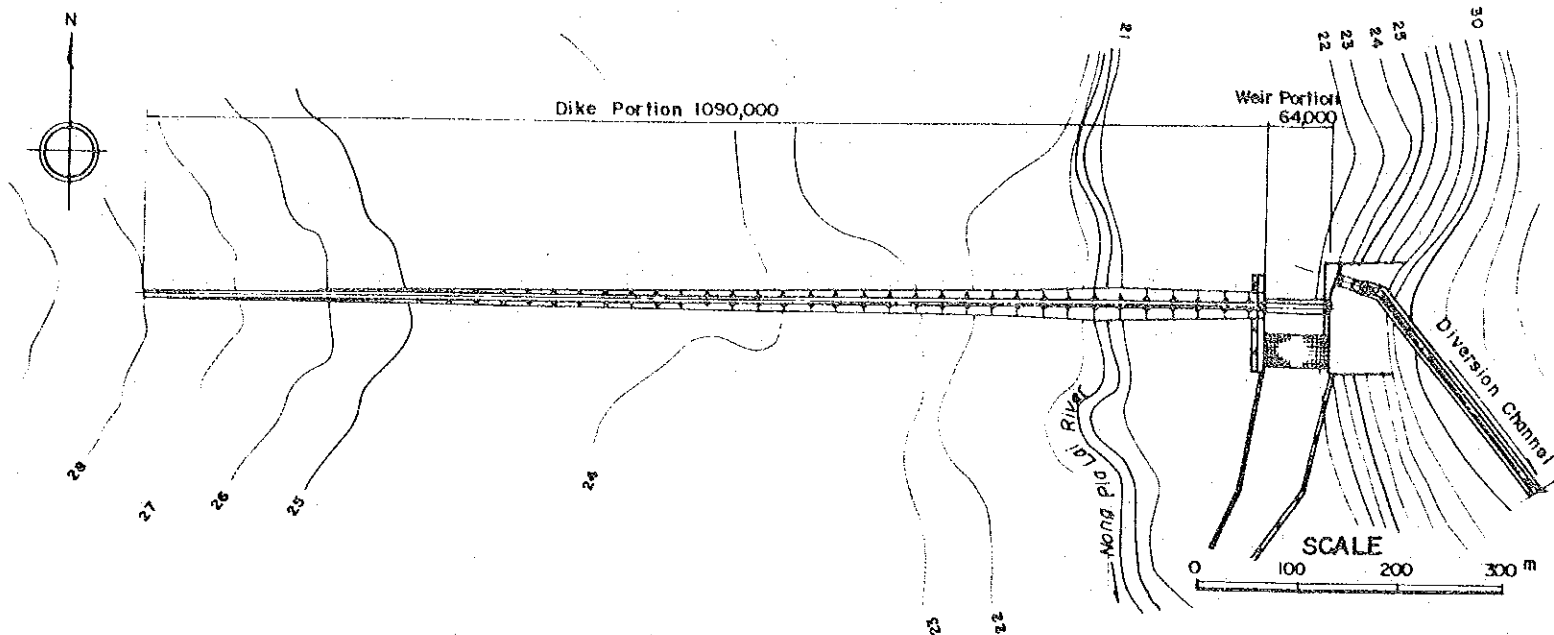
#### FEATURES OF INTAKE FACILITIES

1. West Main Canal		2. East Main Canal	
1) Intake Tower	20 x 1.3 x 13.9 m	1) Inlet	
Pressure Gate	1.3 x 1.3 m	Inlet Channel	270 m
Bed EL	EL. 15.1 m	Intake Tower	2.0 x 1.3 x 13.9
Design Intake Discharge	1.63 m <sup>3</sup> /sec	Pressure Gate	1.3 x 1.3 m
		Bed EL	EL. 15.1 m
		Design Intake Discharge	1.80 m <sup>3</sup> /sec
2) Conduit		2) Conduit	
Type	Open Channel Type	Type	Open Channel Type
Length	87.0 m	Length	100 m
Diameter	φ 1.3 m	Diameter	φ 1.3 m
Gradient	i = 0.00102	Gradient	i = 0.00124
3) Outlet		3) Outlet	
Length	41.0 m	Length	31.0 m
Parshall Flume	W = 4 ft Type	Parshall Flume	W = 4 ft Type
Head WL of Main Canal	WL. 15.0 m	Head WL of Main Canal	WL. 15.0 m

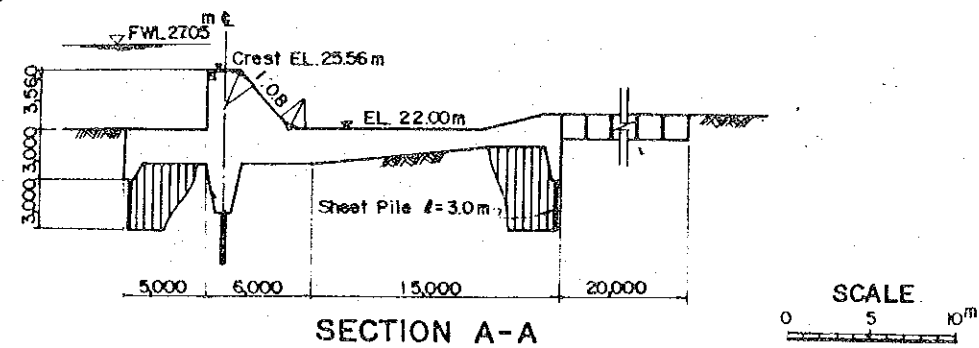
Fig. 9 Configurations of the Intake Structures for the Khlong Thap Ma Irrigation Scheme

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# NONG PLA LAI HEADWORKS



## ELAVATION



### FEATURES OF DIVERSION WORKS

#### 1) Headworks

##### 1.1 Fixed Weir Portion

Length	60.0 m
Height	3.56 m
Crest EL.	25.56 m
Design Flood Discharge	200 m <sup>3</sup> /sec
Design Flood Water Level—FWL	27.05 m

##### 1.2 Movable Weir Portion

Gate (BxH)	3,000 x 4,000
Nos.	1
Bed EL.	EL. 22.0 m

##### 1.3 Dike Portion

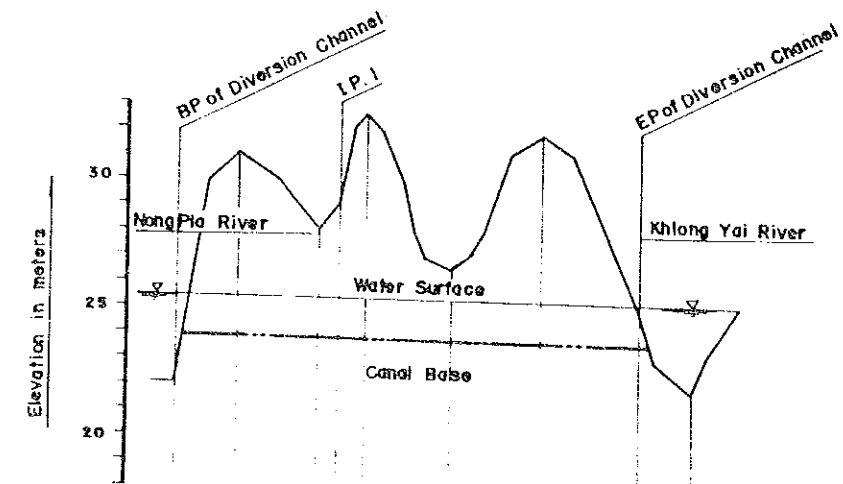
Length	1,090.0 m
Height	7.5 m
Slop	1:2.0
Crest EL.	EL. 28.0 m
Width	5.0 m

##### 1.4 Intake

Gate	2,500 x 2,000 x 2 Nos.
Design Intake Discharge	4.90 m <sup>3</sup> /sec
Design Intake Water Level—IWL	25.46 m
Bed EL.	23.96 m

#### 2) Diversion Channel

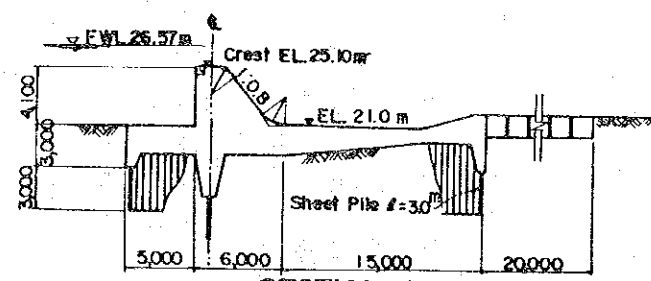
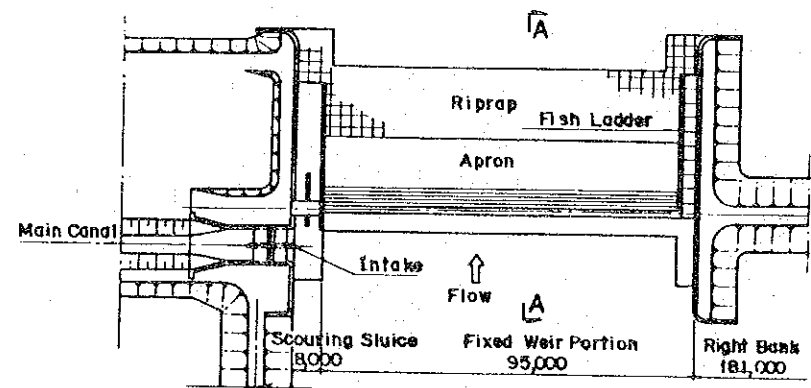
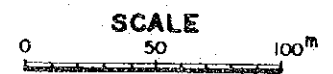
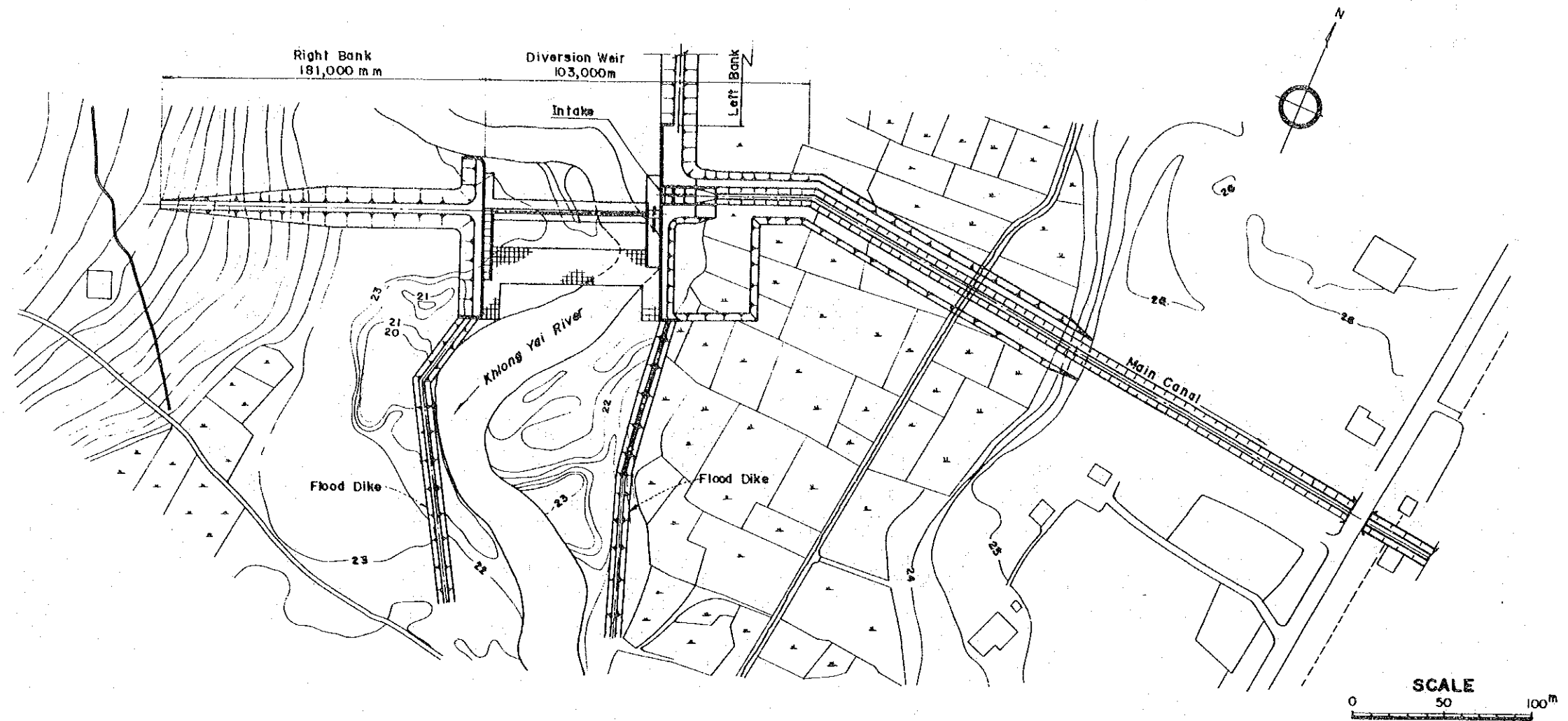
Canal Type	Concrete Lining
Gradient	1/5,000
Base Width	2.0 m
Height	2.1 m
Length	1,800.0 m
Design Discharge	4.90 m <sup>3</sup> /sec



No.	Reduced Distance	Ground Height	Canal Base	Water Level
BP	0.0	22.0	24.03	25.46
+230	230.0	31.0	23.98	25.41
No. 1	400.0	29.7	23.95	25.38
1150	550.0	28.0	23.92	25.35
IP. 1	630.0	29.0	23.90	25.33
+330	730.0	32.6	23.88	25.31
No. 2	800.0	31.7	23.87	25.30
+270	1,070.0	26.5	23.82	25.25
No. 3	1,200.0	28.2	23.79	25.22
+210	1,410.0	31.8	23.75	25.18
No. 4	1,600.0	29.4	23.71	25.14
EP	2,000.0	25.0	23.67	25.10
	2,100.0	21.6		25.10

Fig.10 General Feature of Nong Pla Lai Headworks and Diversion Channel

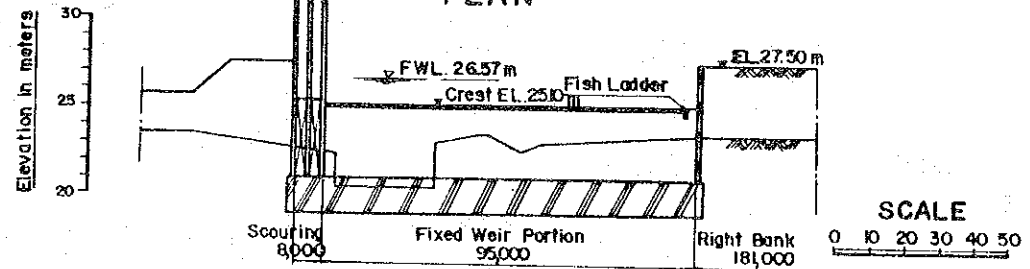
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SECTION A-A  
SCALE 0 5 10<sup>m</sup>

**FEATURES OF KHLONG YAI HEADWORKS**

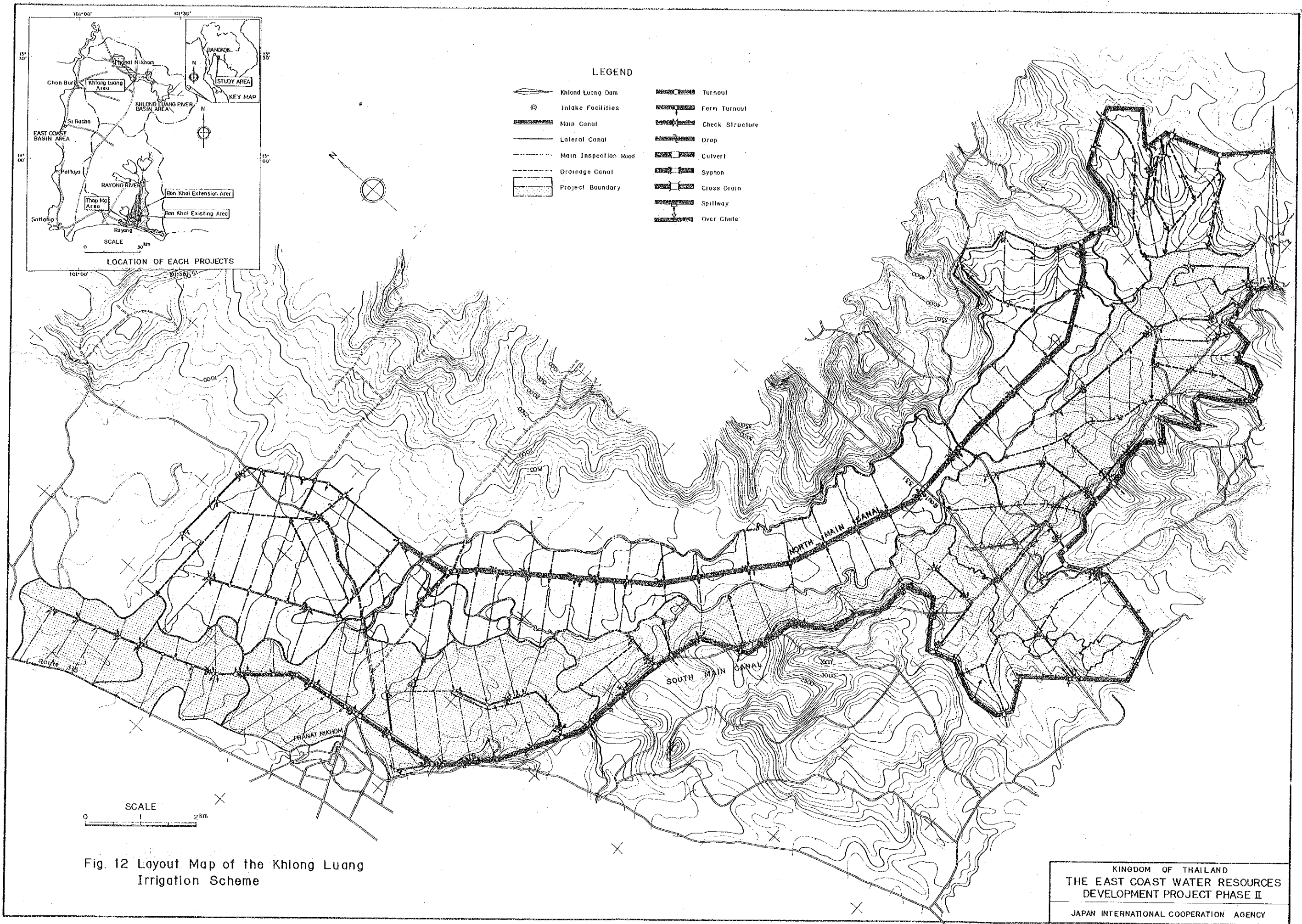
1. Fixed Weir (Flood Way Portion)
  - Length ————— 95.0 m
  - Height ————— 3.5 m
  - Crest EL. ————— EL. 24.5 m
  - Design Flood Discharge — 280 m<sup>3</sup>/sec
  - Design Flood Water Level — FWL. 25.98 m
2. Movable Weir Portion
  - Gate (B x H) ————— 3,000 x 4,400
  - Nos. ————— 2
  - Bed EL. ————— EL. 21.0 m
3. Dike Portion
  - 1) Right Bank
    - Length ————— 181 m
    - Height ————— 4.5 m
    - Slope ————— 1:2.0
    - Crest EL. ————— EL. 27.5 m
    - Width ————— 5.0 m
  - 2) Left Bank
    - Length ————— 700.0 m
    - Height ————— 4.5 m
    - Slope ————— 1:2.0
    - Crest EL. ————— EL. 27.5 m
    - Width ————— 5.0 m
4. Intake
  - Gate (B x H) ————— 4,000 x 3,000
  - Nos. ————— 2
  - Design Intake Discharge — 11.09 m<sup>3</sup>/sec
  - Design Intake Water Level — IWL 25.00 m
  - Bed EL. ————— EL. 23.00 m



ELEVATION

Fig.11 General Feature of Khlong Yai Headworks

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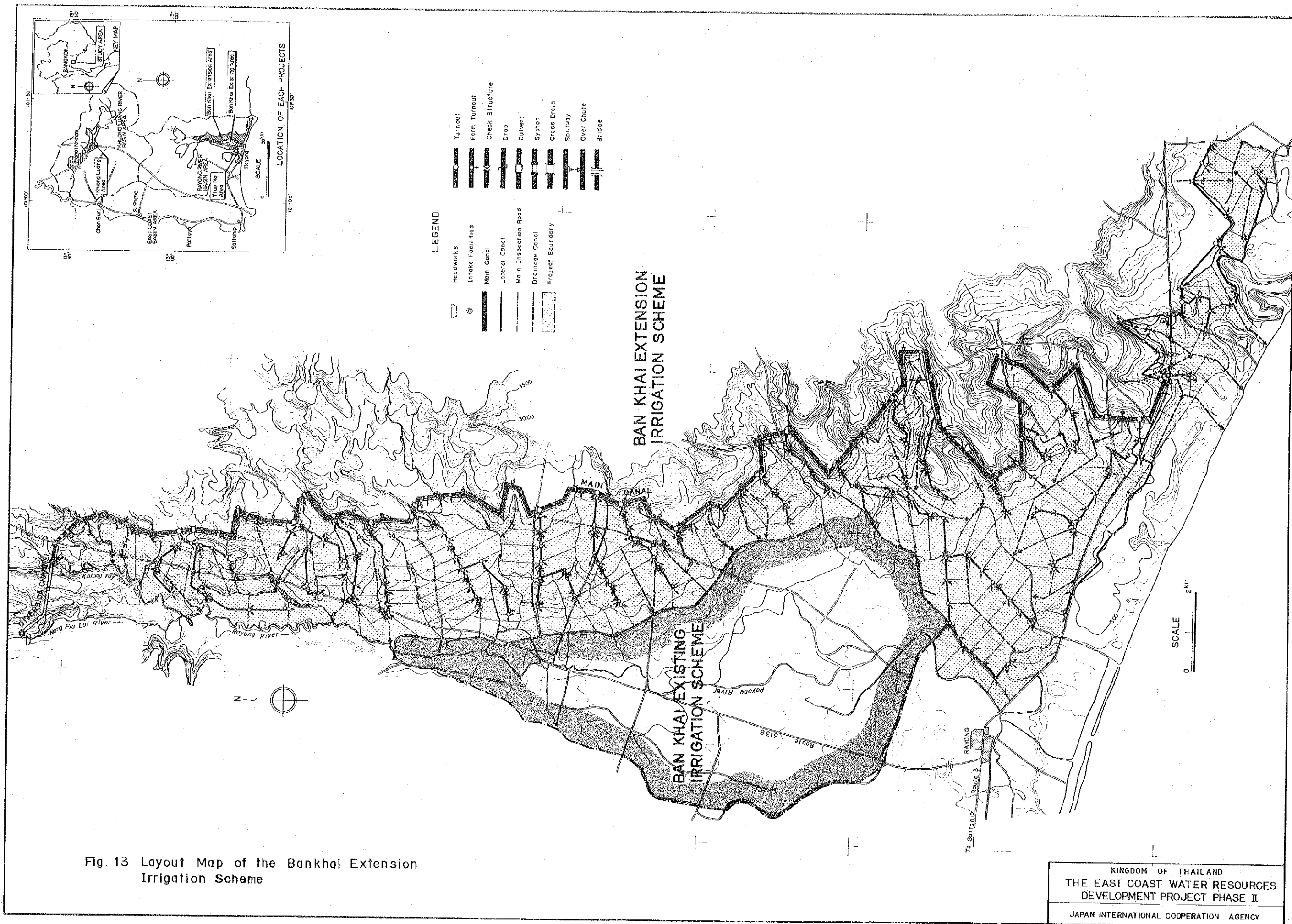


Fig. 13 Layout Map of the Bankhai Extension Irrigation Scheme

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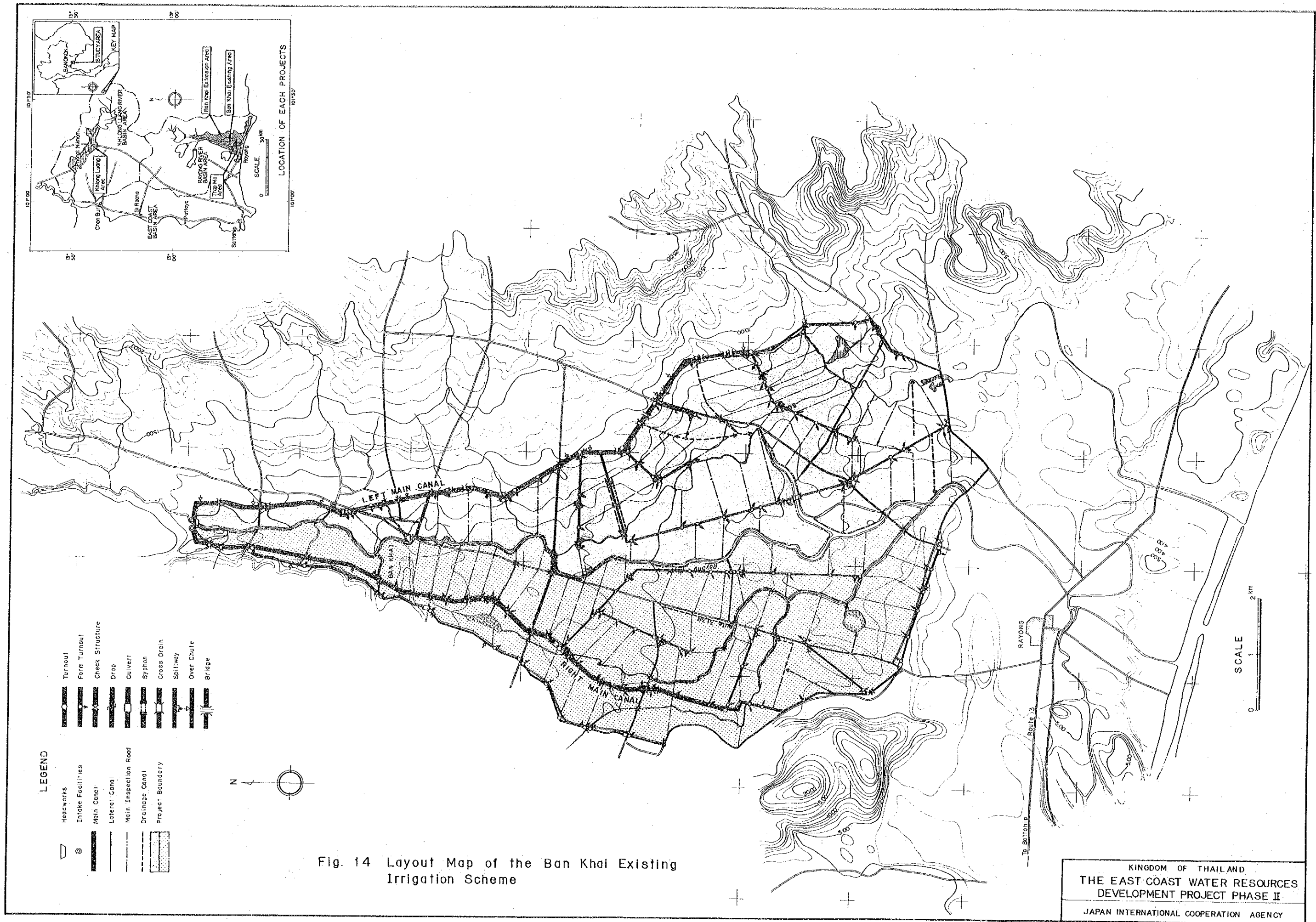


Fig. 14 Layout Map of the Ban Khai Existing Irrigation Scheme

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Fig. 15 Layout Map of the Khlong Thap Ma Irrigation Scheme

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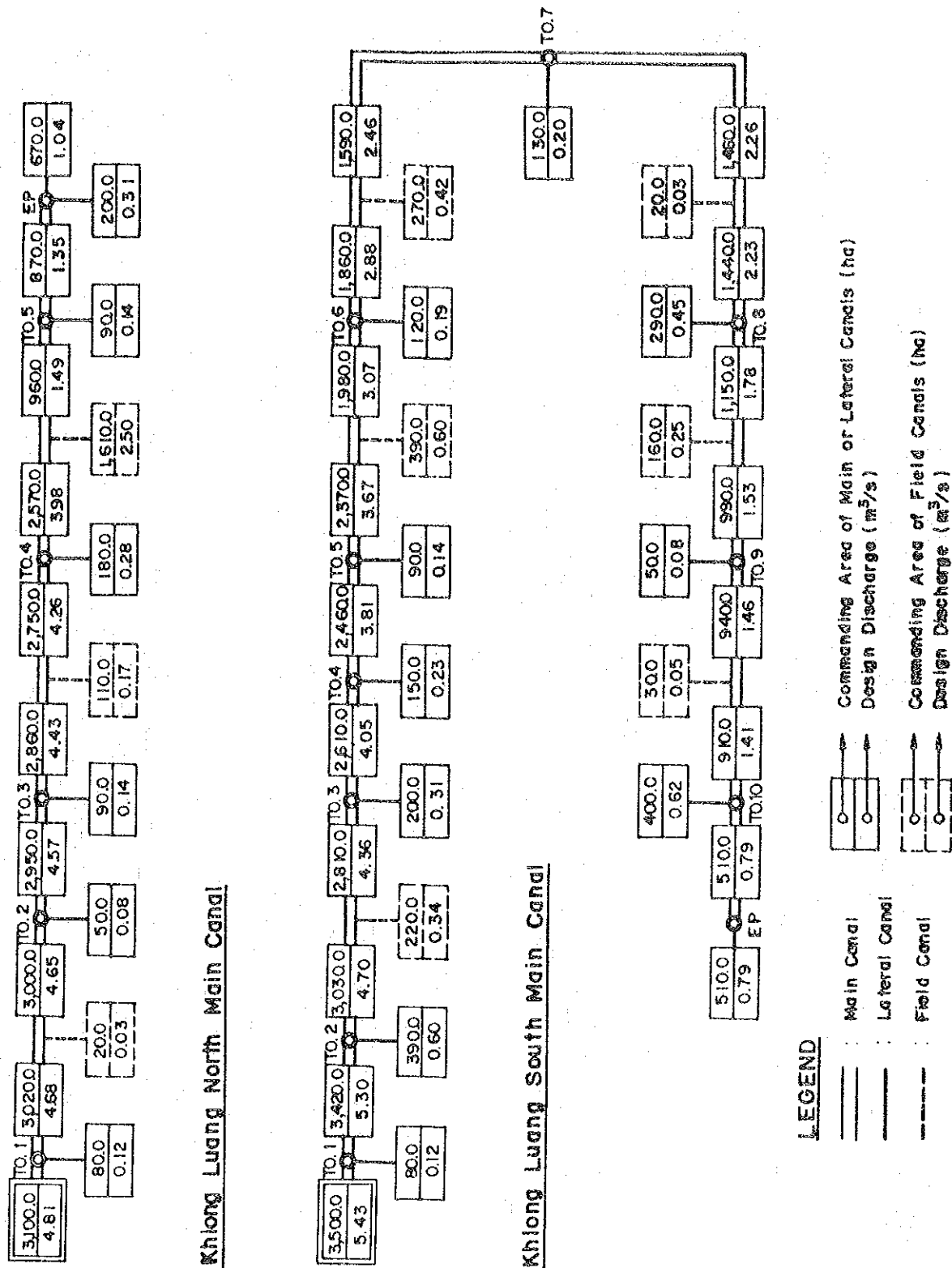


Fig. 16 Irrigation Diagram for the Khlong Luang Irrigation Scheme

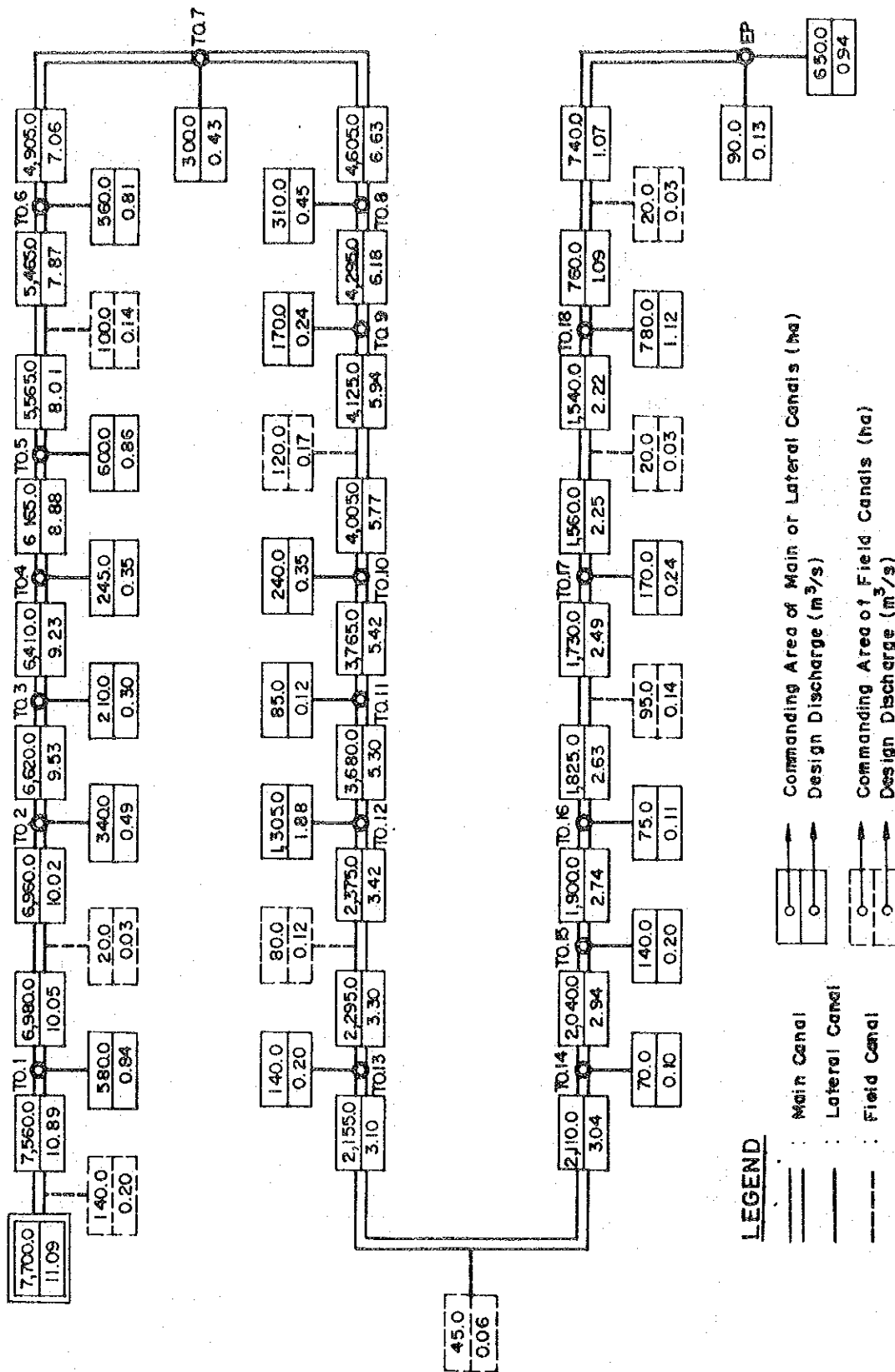


Fig. 17 Irrigation Diagram for the Ban Khai Extension Irrigation Scheme

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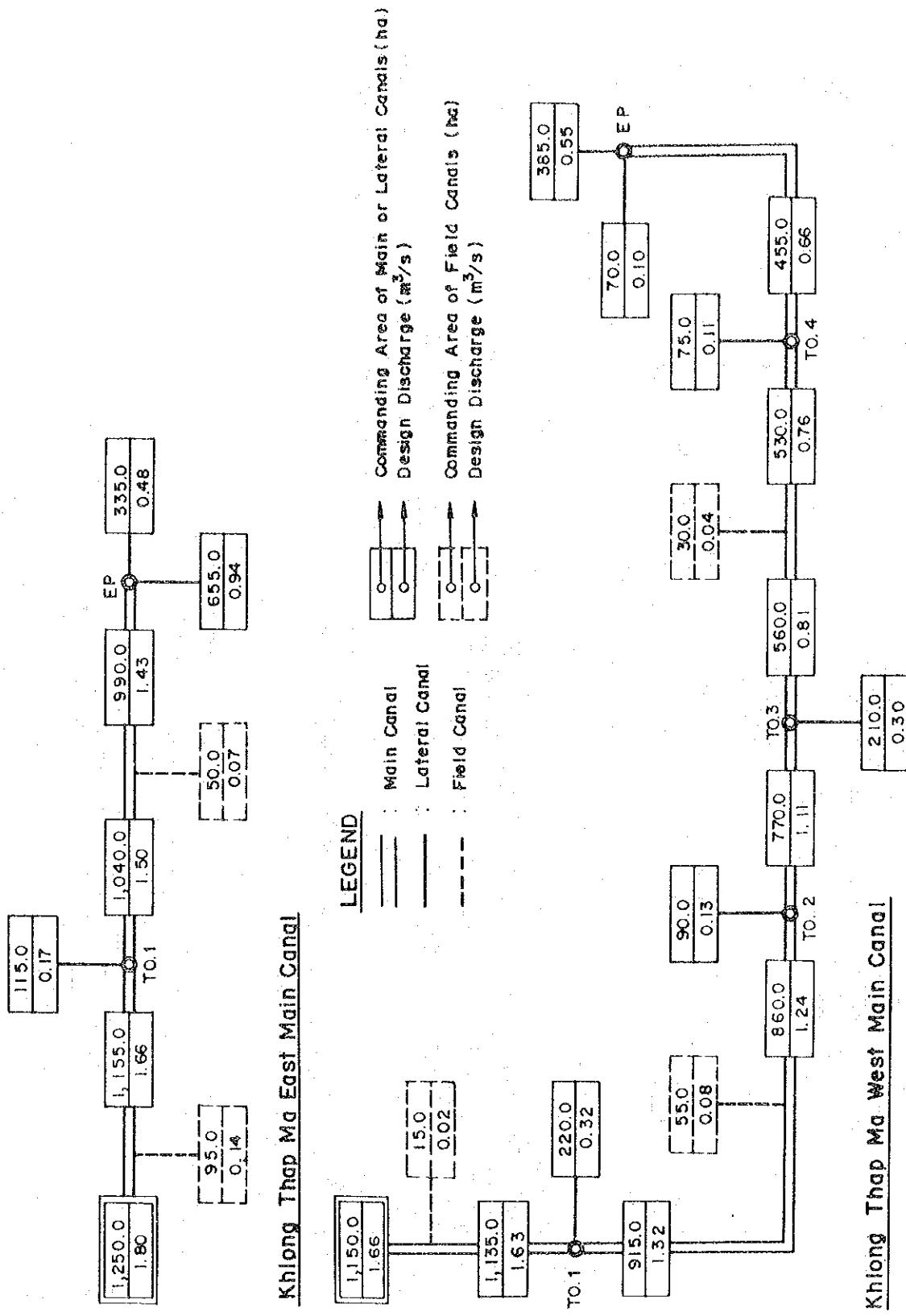
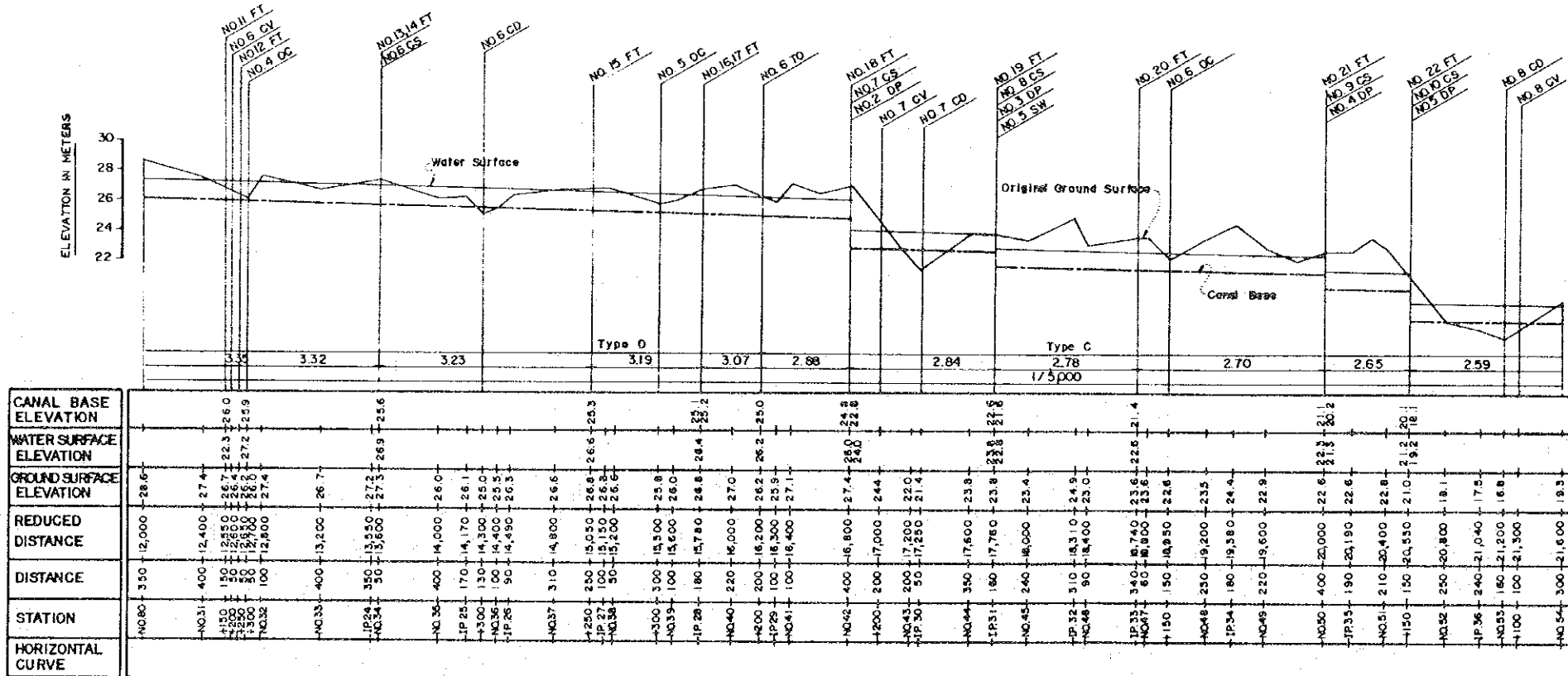
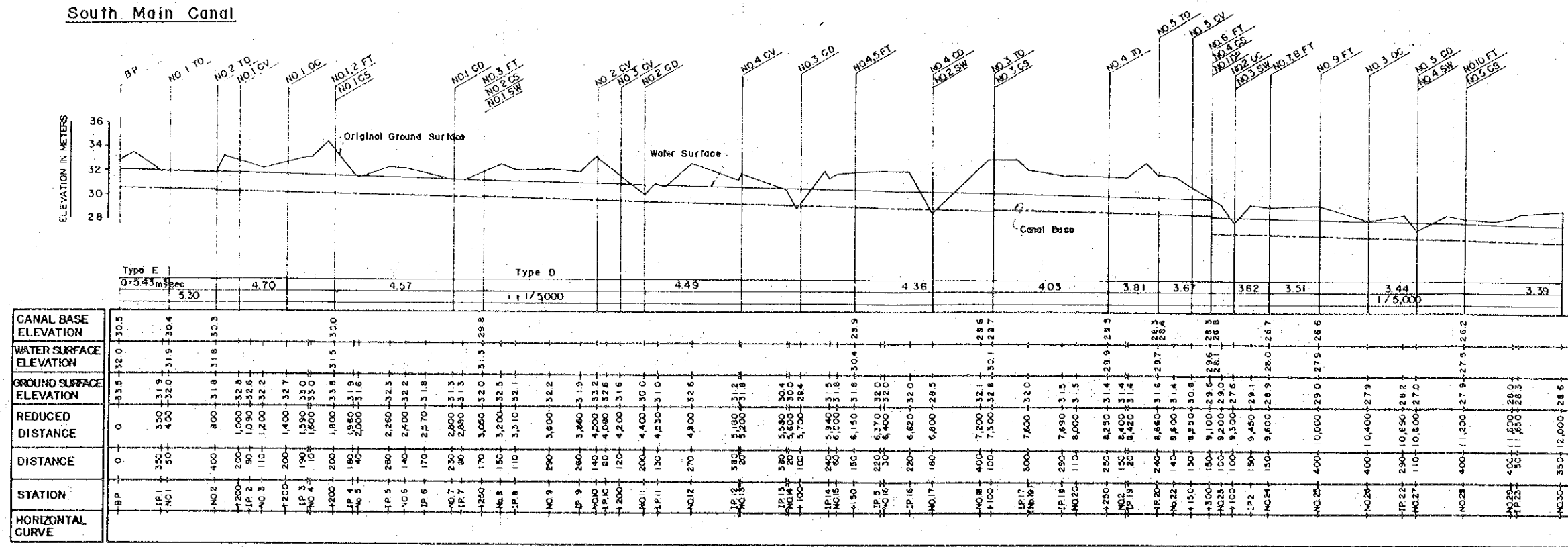


Fig. 19 Irrigation Diagram for the Khlong Thap Ma Irrigation Scheme

South Main Canal



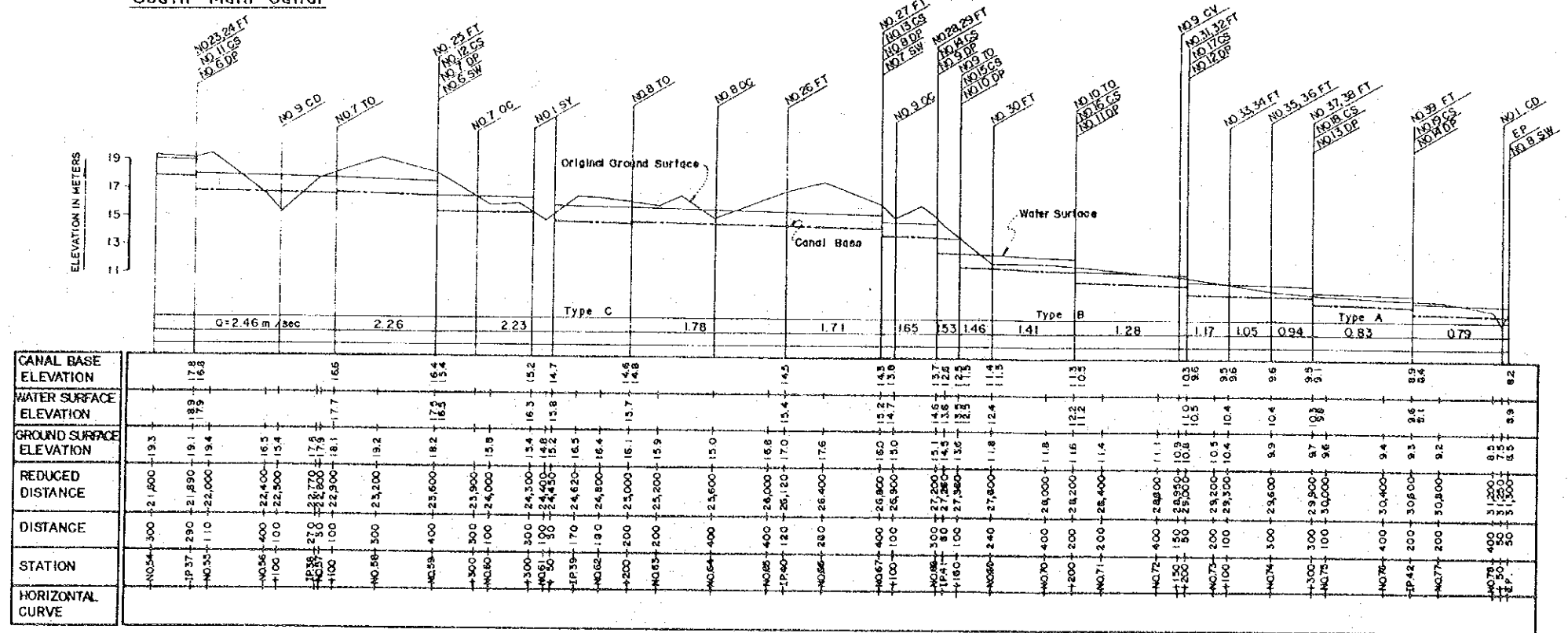
- ABBREVIATION
- TO Turnout
  - FT Farm Turnout
  - CS Check Structure
  - DP Drop Structure
  - SY Inverted Syphon
  - BO Bridge
  - CV Culvert
  - SW Spillway
  - OC Over Chute
  - CD Cross Drain
  - BP Beginning Point
  - IP Intersection Point
  - EP End Point

Fig. 20 (1) Longitudinal Profile of Main Canal for the Khlong Luang Irrigation Scheme



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South Main Canal



ABBREVIATION

TO	Turnout
FT	Farm Turnout
CS	Check Structure
DP	Drop Structure
SY	Inverted Syphon
BO	Bridge
CV	Culvert
SW	Spillway
OC	Over Chute
GD	Gross Drain
BP	Beginning Point
IP	Intersection Point
EP	End Point

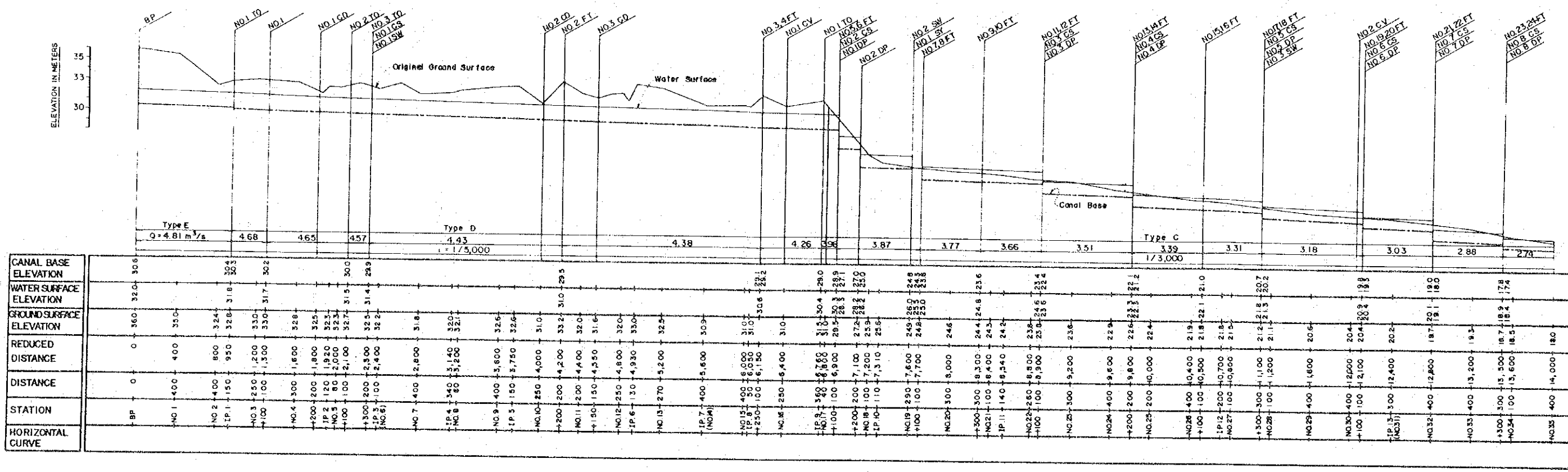
CANAL BASE ELEVATION	WATER SURFACE ELEVATION	GROUND SURFACE ELEVATION	REDUCED DISTANCE	DISTANCE	STATION	HORIZONTAL CURVE
18.2	18.5	19.3	21,500	300	NO.54	
18.3	18.6	19.1	21,890	290	IP.37	
18.4	18.7	19.4	22,000	110	NO.55	
18.5	18.8	16.5	22,400	400	NO.56	
18.6	18.9	15.4	22,500	100	IP.38	
18.7	19.0	17.8	23,200	300	NO.57	
18.8	19.1	17.7	23,900	100	IP.39	
18.9	19.2	19.2	23,200	300	NO.58	
19.0	19.3	18.2	23,600	400	NO.59	
19.1	19.4	15.9	23,900	300	NO.60	
19.2	19.5	15.9	24,000	100	NO.61	
19.3	19.6	16.3	24,300	300	IP.40	
19.4	19.7	14.8	24,450	150	NO.62	
19.5	19.8	15.2	24,620	170	IP.41	
19.6	19.9	16.5	24,800	180	NO.63	
19.7	20.0	16.4	25,000	200	NO.64	
19.8	20.1	15.7	25,200	200	NO.65	
19.9	20.2	15.9	25,600	400	NO.66	
20.0	20.3	15.0	26,000	400	NO.67	
20.1	20.4	16.8	26,000	400	IP.42	
20.2	20.5	17.0	25,120	120	NO.68	
20.3	20.6	17.6	26,400	280	NO.69	
20.4	20.7	16.0	26,800	400	IP.43	
20.5	20.8	15.0	26,900	100	NO.70	
20.6	20.9	14.6	27,200	300	NO.71	
20.7	21.0	12.5	27,280	80	IP.44	
20.8	21.1	13.5	27,360	100	NO.72	
20.9	21.2	11.8	27,800	240	NO.73	
21.0	21.3	11.8	28,000	400	NO.74	
21.1	21.4	11.5	28,200	200	IP.45	
21.2	21.5	11.4	28,400	200	NO.75	
21.3	21.6	11.1	28,800	400	NO.76	
21.4	21.7	10.8	29,050	150	IP.46	
21.5	21.8	10.5	29,300	100	NO.77	
21.6	21.9	10.5	29,200	200	NO.78	
21.7	22.0	10.4	29,300	100	IP.47	
21.8	22.1	10.4	29,600	300	NO.79	
21.9	22.2	9.4	30,400	400	NO.80	
22.0	22.3	9.3	30,800	200	IP.48	
22.1	22.4	9.2	30,800	200	NO.81	
22.2	22.5	8.8	31,200	400	NO.82	
22.3	22.6	8.5	31,500	300	IP.49	
22.4	22.7	8.3	31,800	300	NO.83	

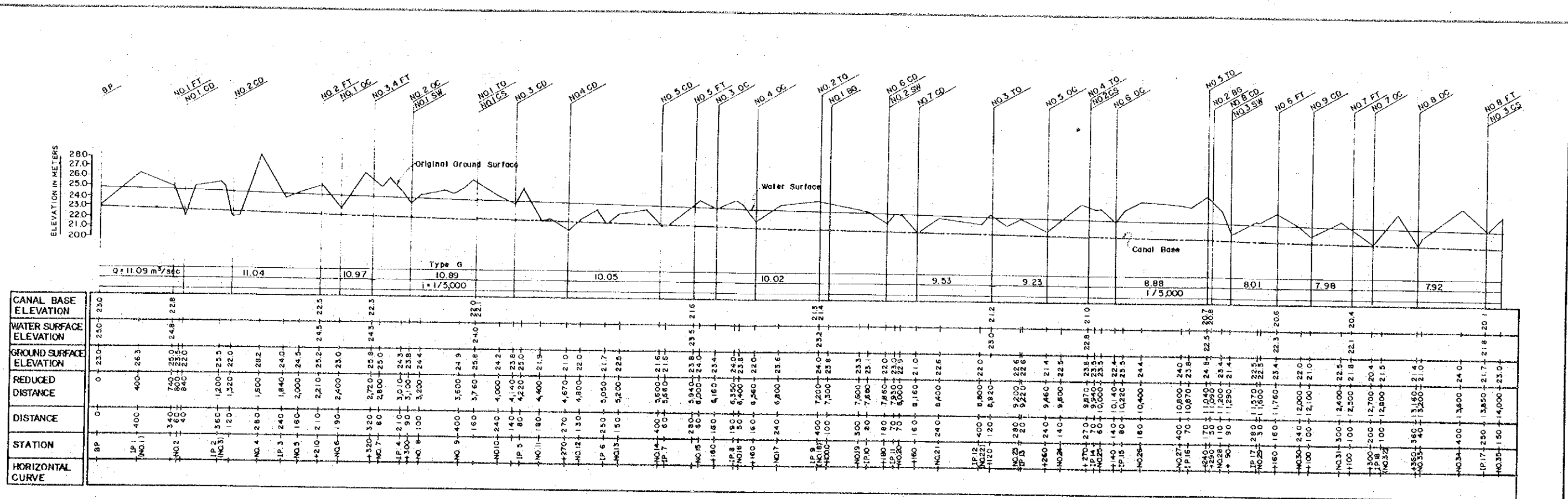
Fig. 20 (2) Longitudinal Profile of Main Canal for the Khlong Luang Irrigation Scheme



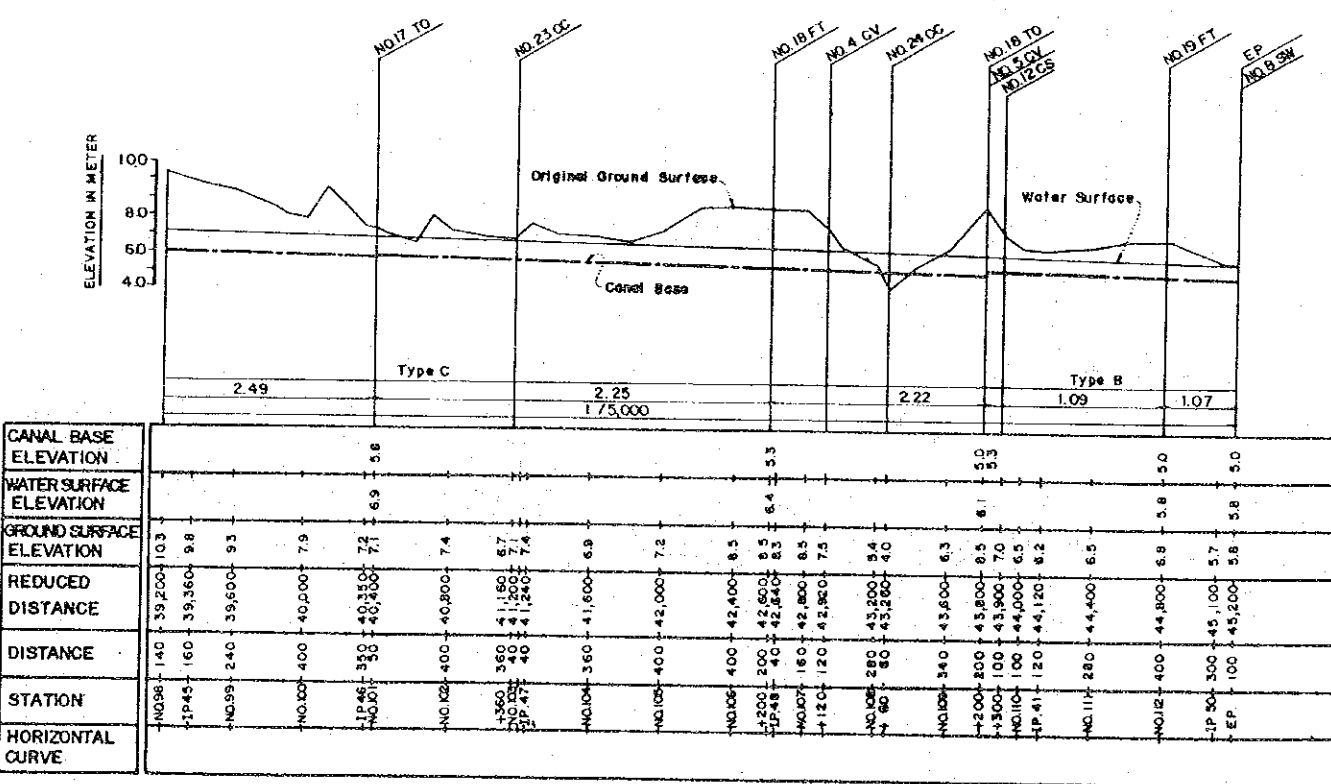
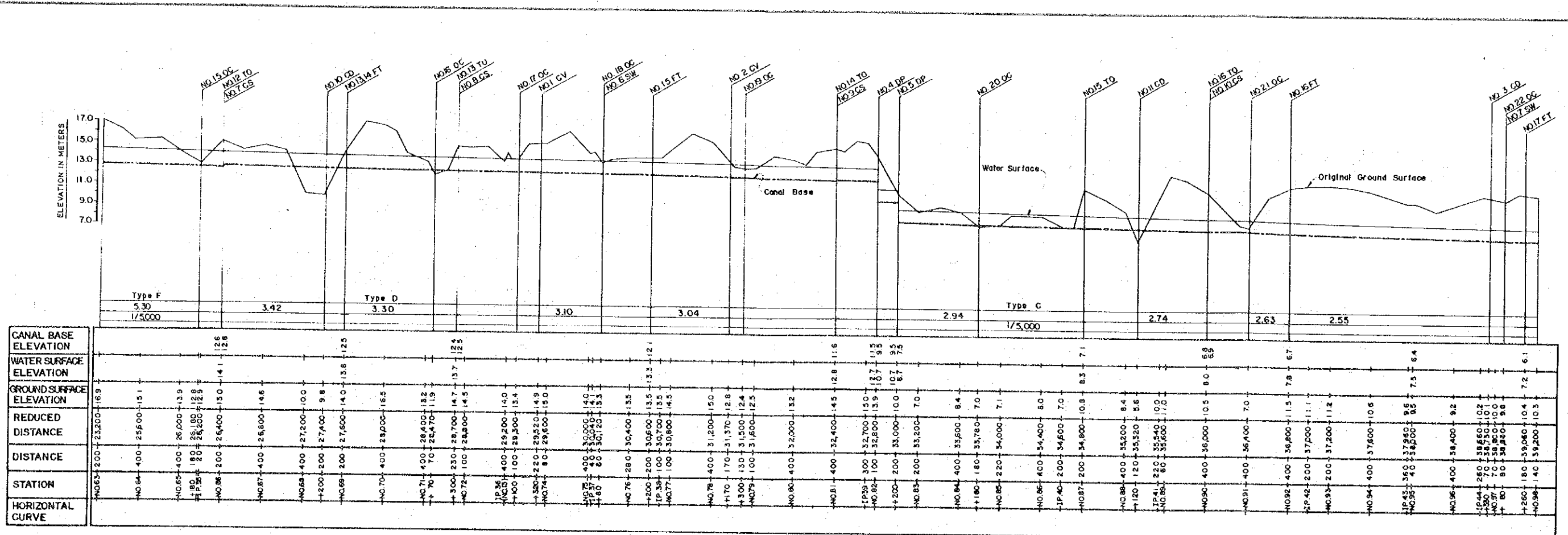
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North Main Canal



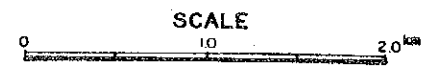






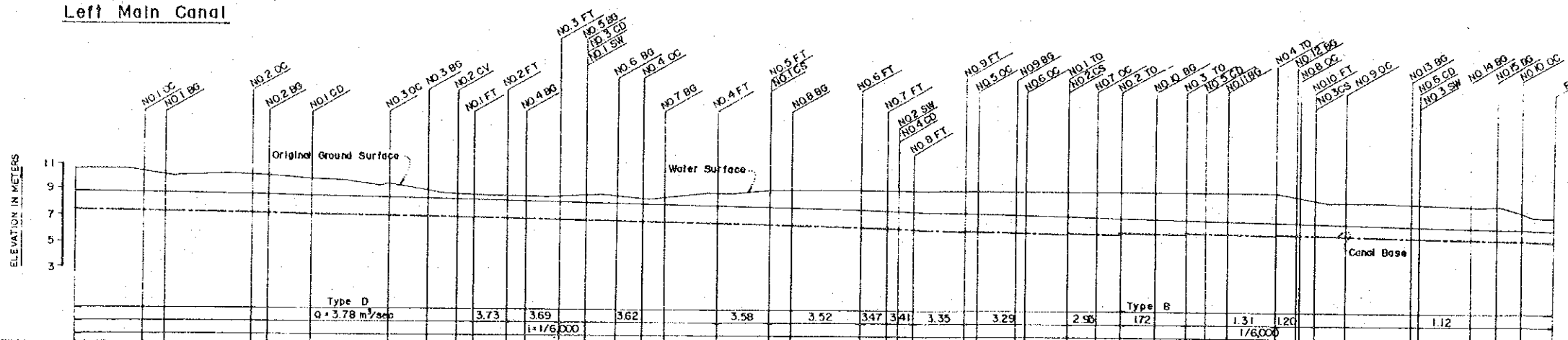
- ABBREVIATION
- TO : Turnout
  - ET : Farm Turnout
  - CS : Check Structure
  - DP : Drop Structure
  - SY : Inverted Syphon
  - BO : Bridge
  - CV : Culvert
  - SW : Spillway
  - OC : Over Chute
  - CD : Cross Drain
  - BP : Beginning Point
  - IP : Intersection Point
  - EP : End Point

Fig. 21 (2) Longitudinal Profile of Main Canal for the Ban Khai Extension Irrigation Scheme



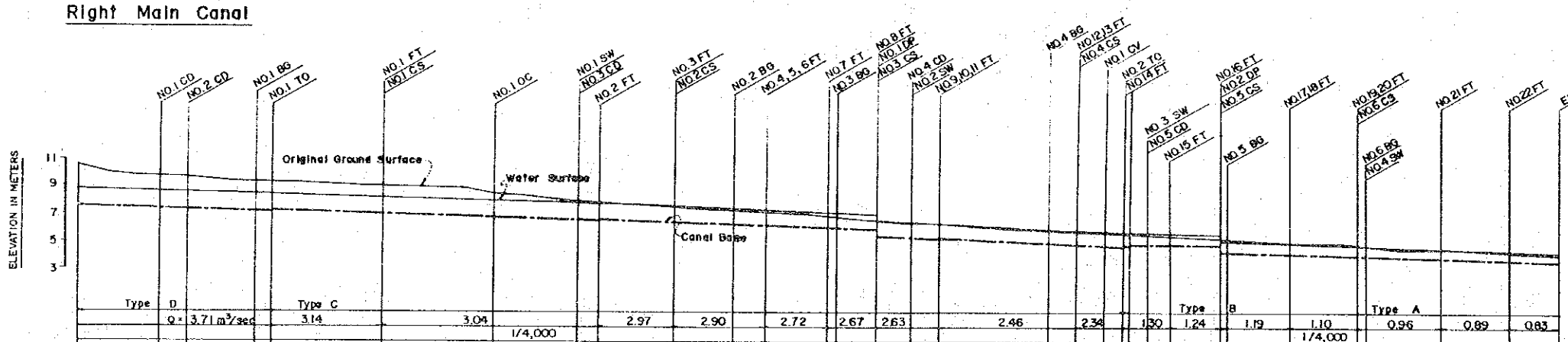
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Left Main Canal



CANAL BASE ELEVATION	WATER SURFACE ELEVATION	GROUND SURFACE ELEVATION	REDUCED DISTANCE	DISTANCE	STATION	HORIZONTAL CURVE
7.4	8.8	10.6	0	0	BP	
		10.5	400	400	IP 1	
		10.3	535	135	NO 1 BG	
		10.1	700	165	NO 2 BG	
		10.0	800	100	NO 1 CD	
		10.1	880	60	IP 2	
		10.2	1170	310	NO 2 BG	
		10.2	1200	30	NO 1 TO	
		10.1	1395	155	NO 1 BG	
		10.0	1470	115	NO 1 TO	
		10.0	1600	130	NO 4 BG	
		9.9	1800	200	NO 2 BG	
		9.7	2000	200	NO 5	
		9.7	2100	100	IP 4	
		9.5	2400	300	NO 6	
		9.0	2700	300	NO 3 OC	
		8.9	2800	100	NO 7	
		8.7	3050	130	IP 5	
		8.7	3200	150	NO 3	
		8.6	3300	100	NO 8	
		8.5	3440	140	NO 9	
		8.5	3700	100	NO 1	
		8.5	3900	200	NO 2	
		8.5	4000	100	NO 3	
		8.5	4130	130	IP 6	
		8.5	4340	210	NO 3 FT	
		8.4	4400	60	NO 11	
		8.5	4500	100	NO 1	
		8.8	4800	300	NO 12	
		8.8	4900	100	NO 1	
		8.8	5040	140	IP 7	
		8.9	5200	160	NO 13	
		8.9	5300	100	NO 1	
		8.9	5470	170	IP 8	
		9.0	5550	80	IP 9	
		9.0	5600	50	NO 14	
		9.0	6000	400	NO 15	
		9.1	6200	200	NO 2	
		9.1	6280	80	IP 9	
		9.1	6400	120	NO 16	
		9.1	6800	400	NO 17	
		9.1	7200	400	NO 18	
		9.0	7300	100	NO 1	
		9.0	7470	170	IP 10	
		9.0	7600	130	NO 19	
		9.0	7810	210	IP 11	
		9.0	7870	60	NO 20	
		9.0	8000	130	NO 2	
		9.0	8220	220	IP 11	
		9.0	8280	60	NO 21	
		9.0	8400	140	NO 2	
		9.0	8500	100	NO 3	
		9.0	8650	150	NO 22	
		9.0	8800	150	NO 2	
		9.0	9200	380	NO 23	
		8.8	9380	180	IP 12	
		8.8	9580	200	NO 24	
		8.4	9600	150	NO 24	
		8.5	9600	100	NO 24	
		8.5	9730	130	NO 13	
		8.5	10000	270	NO 25	
		8.1	10400	400	NO 26	
		8.1	10700	300	NO 27	
		8.0	10800	100	NO 27	
		8.0	10880	80	IP 13	
		8.0	10900	150	NO 28	
		7.2	11200	110	NO 28	
		7.2	11340	140	IP 14	
		7.2	11340	140	EP	

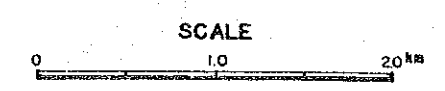
Right Main Canal



CANAL BASE ELEVATION	WATER SURFACE ELEVATION	GROUND SURFACE ELEVATION	REDUCED DISTANCE	DISTANCE	STATION	HORIZONTAL CURVE
7.5	8.8	10.3	0	0	BP	
		10.0	220	220	IP 1	
		9.8	400	180	NO 1	
		9.7	600	200	NO 2	
		9.7	800	200	IP 2	
		9.5	1200	400	NO 3	
		9.5	1280	80	IP 3	
		9.4	1400	120	NO 4	
		9.3	1600	200	NO 4	
		9.1	2000	400	NO 5	
		9.0	2200	200	NO 6	
		9.0	2400	200	IP 4	
		9.0	2530	130	NO 7	
		8.9	2800	270	IP 5	
		8.6	3200	400	NO 8	
		8.4	3500	300	NO 9	
		8.3	3730	150	NO 10	
		8.3	4000	250	IP 6	
		8.3	4010	10	NO 11	
		8.3	4300	290	NO 12	
		8.5	4400	100	IP 7	
		8.5	4480	80	NO 13	
		8.5	4720	260	NO 14	
		8.3	4800	80	NO 15	
		8.3	4930	150	NO 16	
		8.3	5200	250	IP 8	
		8.3	5220	20	NO 17	
		8.3	5400	180	NO 18	
		8.3	5485	65	NO 19	
		8.3	5600	135	NO 20	
		8.3	5750	150	NO 21	
		8.3	6000	250	NO 22	
		8.5	6200	190	IP 9	
		8.5	6400	200	NO 23	
		8.1	6800	280	IP 10	
		8.1	6900	140	NO 24	
		8.0	7000	200	NO 25	
		8.0	7200	200	IP 11	
		8.0	7210	10	NO 26	
		8.0	7420	210	NO 27	
		8.0	7500	80	NO 28	
		8.0	7650	150	NO 29	
		8.0	7780	130	NO 30	
		8.0	8000	220	NO 31	
		8.0	8210	210	IP 12	
		8.0	8300	90	NO 32	
		8.0	8350	50	NO 33	
		8.0	8400	50	NO 34	
		8.0	8600	200	IP 13	
		8.0	9200	370	NO 35	
		8.0	9300	100	NO 36	
		8.0	9560	260	IP 14	
		8.0	9680	120	NO 37	
		8.0	9800	120	NO 38	
		8.0	10000	200	NO 39	
		8.0	10400	400	NO 40	
		8.0	10760	360	EP	

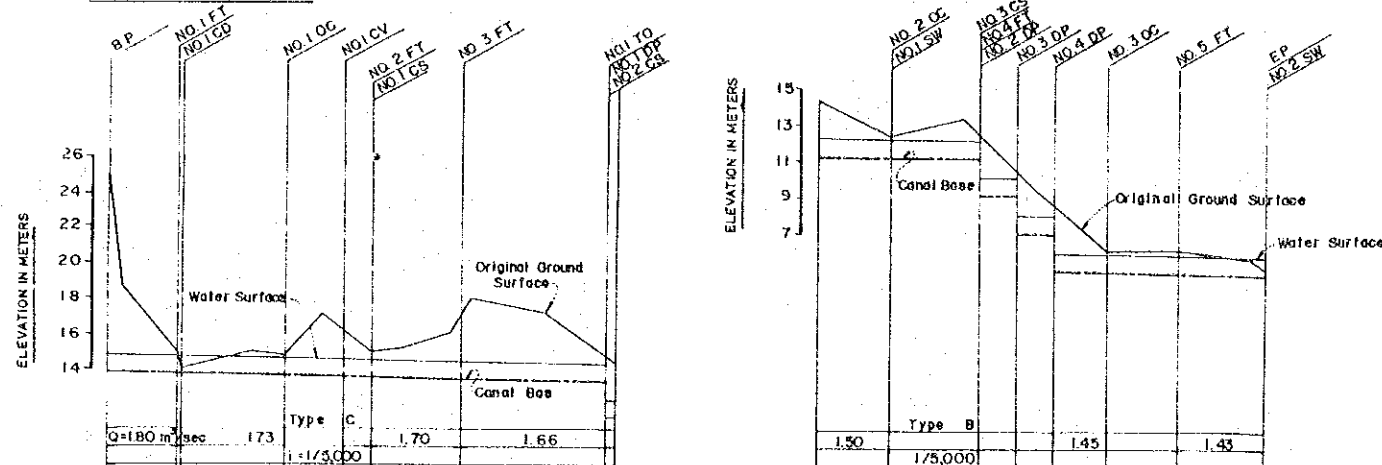
- ABBREVIATION
- TO : Turnout
  - FT : Farm Turnout
  - CS : Check Structure
  - DP : Drop Structure
  - SY : Inverted Syphon
  - BG : Bridge
  - CV : Culvert
  - SW : Spillway
  - OC : Over Chute
  - CD : Cross Drain
  - BP : Beginning Point
  - IP : Intersection Point
  - EP : End Point

Fig. 22 Longitudinal Profile of Main Canal for the Ban Khai Existing Irrigation Scheme



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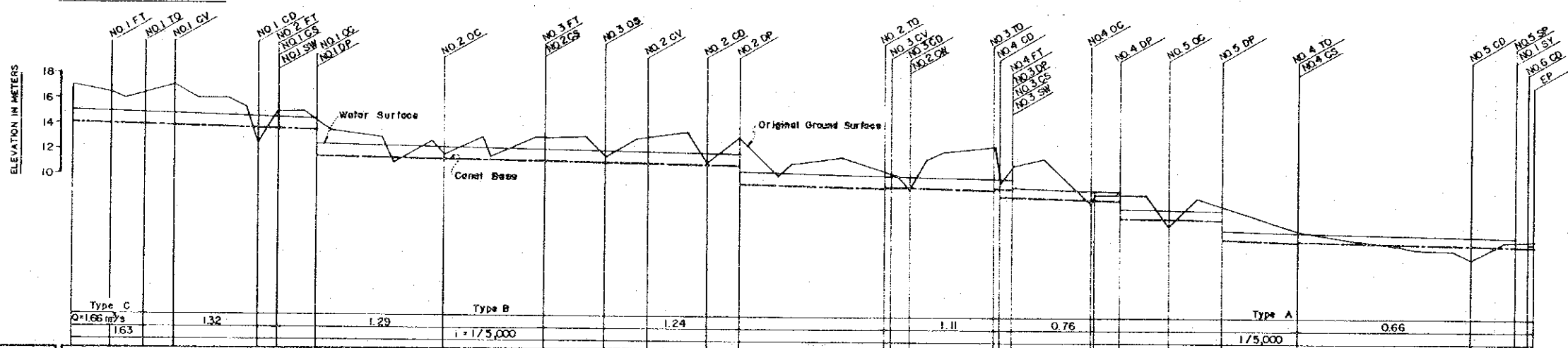
East Main Canal



STATION	REDUCED DISTANCE	GROUND SURFACE ELEVATION	WATER SURFACE ELEVATION	CANAL BASE ELEVATION
BP	0	18.5	15.0	14.1
IP 1	80	18.5	15.0	14.1
NO 1 FT	280	15.1	14.9	14.0
NO 1 CV	400	14.0	14.9	14.0
NO 2-400	800	15.0		
IP 3	980	14.8		
NO 3-220	1,200	17.0		
IP 4	1,310	16.2		
NO 4-150	1,460	15.0	14.6	13.7
NO 4-140	1,600	15.2		
NO 5	3,600	18.0	14.5	13.6
NO 5-200	3,800	18.0		
NO 6-400	2,400	17.2		
NO 7-300	2,750	14.7	14.3	13.4
NO 7-50	2,800	14.4	12.3	11.3

STATION	REDUCED DISTANCE	GROUND SURFACE ELEVATION	WATER SURFACE ELEVATION	CANAL BASE ELEVATION
NO 7-90	2,800	14.4	12.3	11.3
IP 6	3,200	12.4		
NO 9-400	3,600	13.3		
IP 10	3,700	12.4	11.1	
NO 10-200	3,900	10.4	10.0	9.0
NO 10-100	4,000	8.3	8.8	7.0
IP 11	4,100	8.3	7.9	6.9
NO 11-300	4,400	6.0	5.8	4.8
IP 12	4,800	6.0	5.7	4.7
NO 13-400	5,200	5.5	5.0	4.6
IP 13	5,270	5.0	5.6	

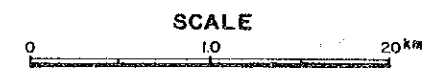
West Main Canal



STATION	REDUCED DISTANCE	GROUND SURFACE ELEVATION	WATER SURFACE ELEVATION	CANAL BASE ELEVATION
BP	0	12.0	13.0	14.1
NO 1-300	300	16.5	14.9	14.0
NO 1-100	400	16.0		
NO 1-150	550	16.4	14.8	13.9
NO 2-250	600	17.0		
IP 1	950	16.0		
NO 3-250	1,200	16.0		
IP 2	1,350	15.2		
NO 3-100	1,450	12.5		
NO 4-150	1,600	13.0	14.5	13.6
IP 3	1,800	13.0		
NO 5-100	1,800	14.2	14.4	13.5
NO 5-100	2,000	13.3	12.4	11.5
NO 6-400	2,400	13.0		
IP 4	2,740	11.0		
NO 7-360	2,800	12.7		
IP 5	2,900	11.8		
NO 8-300	3,200	13.0		
IP 6	3,250	11.5		
NO 9-350	3,600	13.0		
IP 7	3,650	13.0	12.0	11.1
NO 10-350	4,000	13.0		
IP 8	4,150	11.5		
NO 11-250	4,400	13.0		
IP 9	4,500	13.1		
NO 12-300	4,800	13.5		
IP 10	4,950	11.0		
NO 13-250	5,200	13.0	11.7	11.0
IP 11	5,500	10.5	10.2	9.5
NO 14-100	5,600	11.0		
NO 15-400	6,000	11.5		
NO 16-350	6,350	10.4	9.2	
IP 12	6,400	10.3	10.0	9.2
NO 17-40	6,440	10.2		
IP 13	6,590	9.0		
IP 14	6,710	11.5		
NO 18-180	6,800	12.0		
NO 19-260	7,600	11.5		
IP 15	7,200	12.5	9.8	8.9
IP 16	7,230	9.5	9.7	9.5
IP 17	7,540	11.0	9.7	9.5
NO 20-200	8,200	8.5	8.9	8.4
IP 18	8,400	8.5	7.4	6.5
IP 19	8,580	6.0		
NO 22-220	8,800	6.2		
NO 23-200	9,000	7.6	7.3	6.2
IP 20	9,200	7.0		
NO 24-100	9,600	3.7	3.5	2.9
IP 21	10,000	5.0		
NO 26-400	10,400	4.5		
IP 22	11,520	4.3		
NO 27-280	10,800	4.2		
IP 23	10,900	3.5		
NO 28-280	11,200	4.8		
IP 24	11,300	4.8	4.2	4.7
NO 29-100	11,500	4.8		
IP 25	11,450	4.8	4.9	4.7
NO 30-50	11,450	3.0		

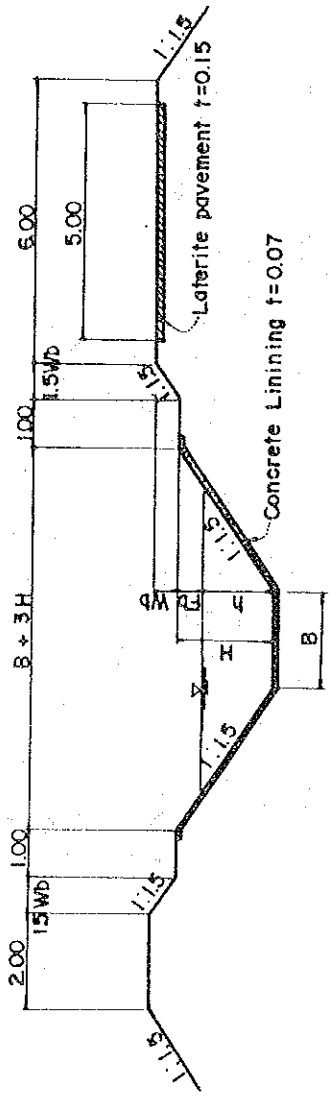
- ABBREVIATION
- TO : Turnout
  - FT : Farm Turnout
  - CS : Check Structure
  - DP : Drop Structure
  - SY : Inverted Siphon
  - BR : Bridge
  - CV : Culvert
  - SW : Spillway
  - OC : Over Chute
  - CD : Cross Drain
  - BP : Beginning Point
  - IP : Intersection Point
  - EP : End Point

Fig. 23 Longitudinal Profile of Main Canal for the Thap Ma Irrigation Scheme

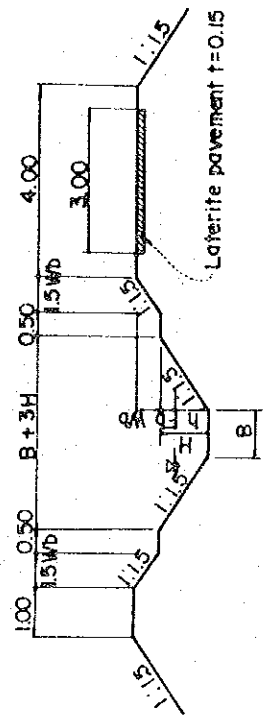


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MAIN CANAL & MAIN INSPECTION ROAD



LATERAL CANAL & LATERAL INSPECTION ROAD

(Unit : m)

Canal Type	B	H	Wb
A	1.0	1.0	0.2
B	1.0	1.2	0.2
C	1.5	1.5	0.3
D	1.5	1.8	0.3
E	2.0	1.8	0.3
F	2.0	2.0	0.5
G	2.5	2.3	0.5

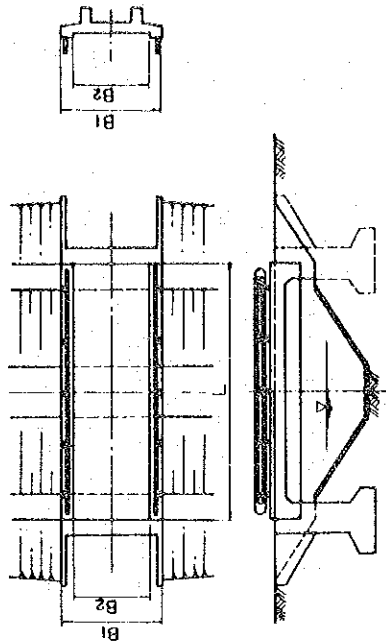
(Unit : m)

Canal Type	B	H	Wb
A	0.5	0.6	0.2
B	0.6	0.7	0.2
C	0.8	0.9	0.2
D	1.0	1.0	0.3
E	1.2	1.2	0.3
F	1.4	1.4	0.3

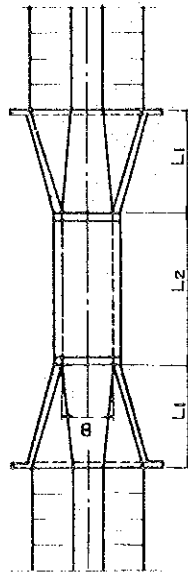
Fig. 24 Typical Cross Section of Canal and Inspection Road

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 DEVELOPMENT PROJECT PHASE II  
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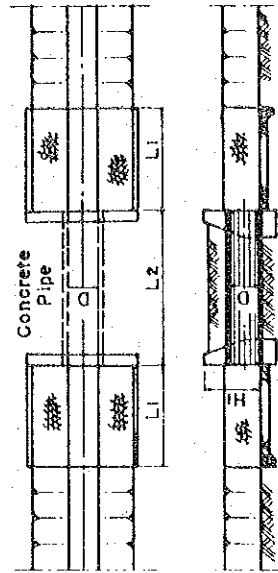
BRIDGE



CULVERT, TYPE - A



CULVERT, TYPE - B



DIMENSIONS OF BRIDGES (Unit : mm)

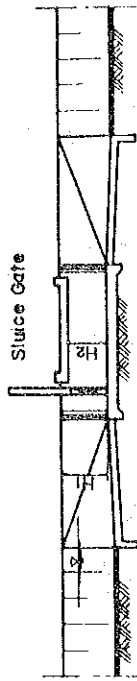
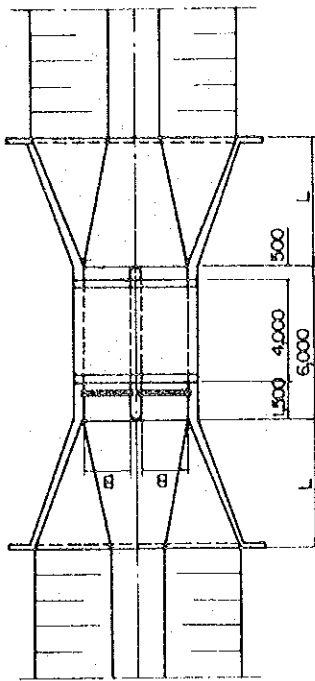
TYPE	Discharge (m <sup>3</sup> /sec)	L	B1	B2
A - 1	15.0 ~ 12.0	12,000	4,000	3,000
A - 2	12.0 ~ 8.0	11,000	~ 6,000	~ 5,000
A - 3	8.0 ~ 5.0	10,000	~ 6,000	~ 5,000

DIMENSIONS OF CULVERTS (Unit : mm)

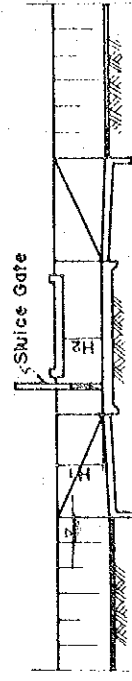
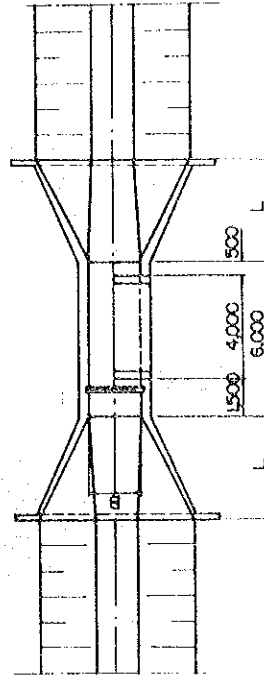
TYPE	Discharge (m <sup>3</sup> /sec)	L	H	B	D
A - 1	5.00 ~ 4.00	4,000	2,250	2,000	—
A - 2	4.00 ~ 3.20	∗	2,050	1,800	—
A - 3	3.20 ~ 2.50	∗	1,850	1,600	—
A - 4	2.50 ~ 1.80	∗	1,600	1,400	—
A - 5	1.80 ~ 1.30	∗	1,400	1,200	—
A - 6	1.30 ~ 1.00	∗	1,200	1,000	—
B - 1	1.00 ~ 0.83	3,000	1,900	—	1,000
B - 2	0.83 ~ 0.64	∗	1,800	—	900
B - 3	0.64 ~ 0.50	∗	1,700	—	800
B - 4	0.50 ~ 0.36	∗	1,600	—	700
B - 5	0.36 ~ 0.26	∗	1,500	—	600
B - 6	0.26 ~ 0.16	∗	1,400	—	500
B - 7	0.16 ~ 0.09	∗	1,300	—	400
B - 8	0.09 ~	∗	1,200	—	300

Fig. 25 Typical Dimensions of Bridges and Culverts

TYPE - A



TYPE - B



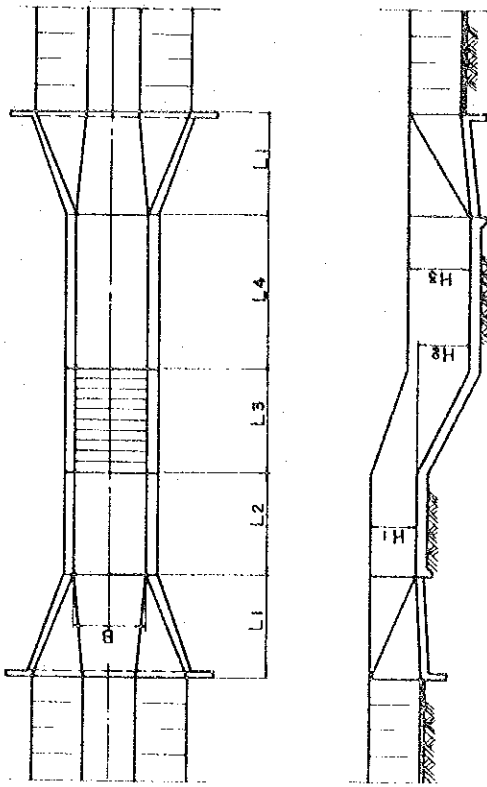
DIMENSION OF CHECK STRUCTURES

(Unit: mm)

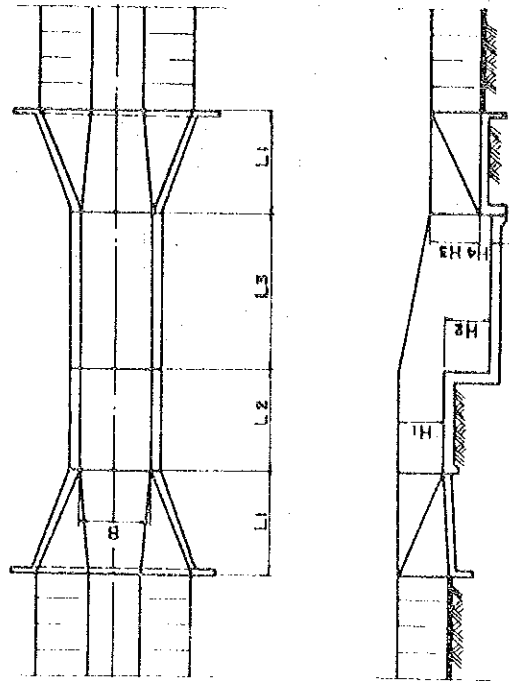
TYPE	Discharge (m <sup>3</sup> /sec)	L	B	H1	H2	Sluice Gate
A-1	8.0~6.4	5,000	1,800	2,000	1,800	1,800 x 1,600
A-2	6.4~5.0	5,000	1,600	1,800	1,600	1,600 x 1,400
B-1	5.0~4.0	4,000	2,000	2,200	2,000	2,000 x 1,700
B-2	4.0~3.2	4,000	1,800	2,000	1,800	1,800 x 1,600
B-3	3.2~2.5	4,000	1,600	1,800	1,600	1,600 x 1,400
B-4	2.5~1.8	4,000	1,400	1,600	1,400	1,400 x 1,300
B-5	1.8~1.3	4,000	1,200	1,400	1,200	1,200 x 1,100
B-6	1.3~1.0	4,000	1,000	1,200	1,000	1,000 x 900
B-7	1.0~0.6	3,000	800	1,200	1,000	800 x 800
B-8	0.6~0.4	3,000	600	1,000	800	600 x 500
B-9	0.4~0.25	3,000	500	1,000	800	500 x 500
B-10	0.25~	3,000	400	800	600	400 x 500

Fig. 26 Typical Dimension of Check Structures

TYPE - A (Inclined Drops)



TYPE - B (Vertical Drops)



DIMENSIONS OF DROPS

(Unit: mm)

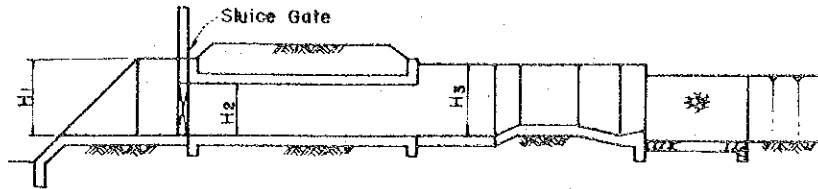
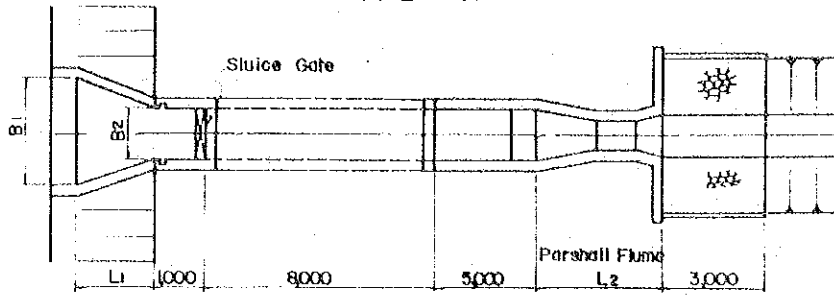
TYPE	Discharge (m <sup>3</sup> /sec)	L	L1	L3	L4	B	H1	H2	H3	H4
A-1	8.0 ~ 6.4	5,000	5,000	4,000 ~ 9,000	6,000	3,600	2,000	2,000 ~ 4,500	2,500	-
A-2	6.4 ~ 5.0	5,000	5,000	4,000 ~ 9,000	6,000	3,200	2,000	2,000 ~ 4,500	2,500	-
A-3	5.0 ~ 4.0	4,000	4,000	4,000 ~ 9,000	6,000	2,000	2,000	2,000 ~ 4,500	2,500	-
A-4	4.0 ~ 3.2	4,000	4,000	4,000 ~ 9,000	6,000	1,800	2,000	2,000 ~ 4,500	2,500	-
A-5	3.2 ~ 2.5	4,000	4,000	4,000 ~ 9,000	5,000	1,600	1,800	2,000 ~ 4,500	2,300	-
A-6	2.5 ~ 2.0	4,000	4,000	4,000 ~ 9,000	5,000	1,400	1,600	2,000 ~ 4,500	2,100	-
B-1	8.0 ~ 6.4	5,000	5,000	3,500 ~ 4,500	-	3,600	2,000	900 ~ 1,400	2,000	400
B-2	6.4 ~ 5.0	5,000	5,000	3,500 ~ 4,500	-	3,200	2,000	900 ~ 1,400	2,000	400
B-3	5.0 ~ 4.0	4,000	4,000	3,500 ~ 4,500	-	2,000	2,000	900 ~ 1,400	2,000	400
B-4	4.0 ~ 3.2	4,000	4,000	3,500 ~ 4,500	-	1,800	2,000	900 ~ 1,400	2,000	400
B-5	3.2 ~ 2.5	4,000	4,000	3,500 ~ 4,500	-	1,600	1,800	900 ~ 1,400	1,800	400
B-6	2.5 ~ 2.0	4,000	4,000	3,500 ~ 4,500	-	1,400	1,600	900 ~ 1,400	1,600	400
B-7	2.0 ~ 1.3	4,000	4,000	3,500	-	1,200	1,400	800 ~ 2,300	1,400	500
B-8	1.3 ~ 1.0	4,000	4,000	3,500	-	1,000	1,200	800 ~ 2,300	1,200	300
B-9	1.0 ~ 0.6	3,000	3,000	3,000	-	800	1,200	800 ~ 2,300	1,200	300
B-10	0.6 ~ 0.4	3,000	3,000	3,000	-	600	1,000	800 ~ 2,300	1,000	300
B-11	0.4 ~ 0.25	3,000	3,000	2,500	-	500	1,000	800 ~ 2,300	1,000	300
B-12	0.25 ~	3,000	3,000	2,500	-	400	800	800 ~ 2,300	800	300

Fig. 27 Typical Dimensions of Drops

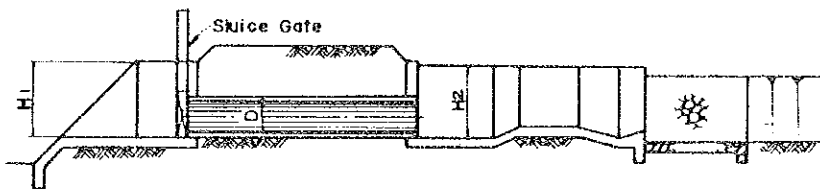
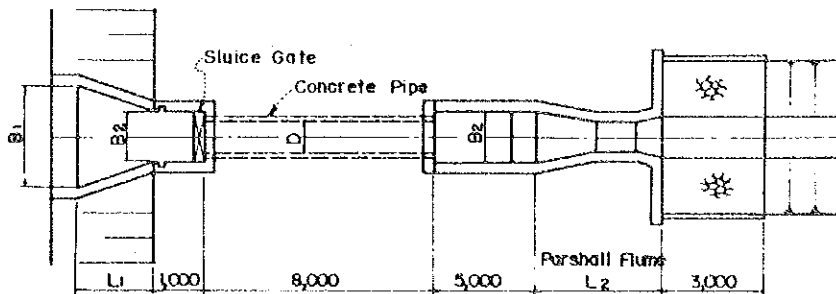
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TYPE - A



TYPE - B



DIMENSION OF TURNOUTS

(Unit : mm)

TYPE	Discharge (m <sup>3</sup> /sec)	L1	L2	B1	B2	D	H1	H2	H3	Sluice Gate	Parshall Flume Type
A-1	2.50 ~ 1.80	2,350	3,467	2,400	1,400	—	1,900	1,400	1,800	1,400x1,400	5 ft
A-2	1.80 ~ 1.30	2,530	3,318	2,200	1,200	—	1,700	1,200	1,600	1,200x1,200	4 ft
A-3	1.30 ~ 1.00	2,250	3,169	2,000	1,000	—	1,500	1,000	1,400	1,000x1,000	3 ft
B-1	1.00 ~ 0.83	2,250	3,169	2,200	1,200	1,000	1,500	1,400	—	1,000x1,000	3 ft
B-2	0.83 ~ 0.64	2,100	3,019	2,100	1,100	900	1,400	1,300	—	900 x 900	2 ft
B-3	0.64 ~ 0.50	1,950	2,943	2,000	1,000	800	1,300	1,200	—	800 x 800	1ft 6in
B-4	0.50 ~ 0.36	1,800	2,943	1,900	900	700	1,200	1,100	—	700 x 700	1ft 6in
B-5	0.36 ~ 0.26	1,650	2,867	1,800	800	600	1,100	1,000	—	600 x 600	1 ft
B-6	0.26 ~ 0.16	1,500	1,625	1,700	700	500	1,000	1,000	—	500 x 500	9 in
B-7	0.16 ~ 0.09	1,350	1,625	1,600	600	400	900	1,000	—	400 x 400	9 in
B-8	0.09 ~	1,200	1,525	1,500	500	300	800	1,000	—	300 x 300	6 in

Fig. 28 Typical Dimensions of Turnouts

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# DIMENSION OF PARSHALL FLUMES

(Unit : mm)

TYPE	Discharge (m <sup>3</sup> /sec)	W	A	B	C	D	E	F	G	K	N	X	Y
5 ft.	2.42 ~ 0.045	1,524	1,981	1,943	1,829	2,302	914	610	914	76	229	51	76
4 ft.	1.92 ~ 0.037	1,219	1,829	1,794	1,524	1,937	914	610	914	76	229	51	76
3 ft.	1.43 ~ 0.017	914	1,676	1,645	1,219	1,572	914	610	914	76	229	51	76
2 ft.	0.94 ~ 0.012	610	1,524	1,495	914	1,206	914	610	914	76	229	51	76
1 ft. 6 in	0.70 ~ 0.004	457	1,448	1,419	762	1,025	914	610	914	76	229	51	76
1 ft.	0.46 ~ 0.003	305	1,372	1,343	610	844	914	610	914	76	229	51	76
9 in	0.28 ~ 0.003	229	879	863	381	575	762	305	457	76	114	51	76
6 in	0.11 ~ 0.001	152	792	610	397	397	610	305	610	76	114	51	76

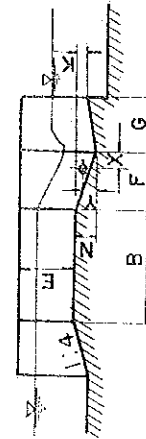
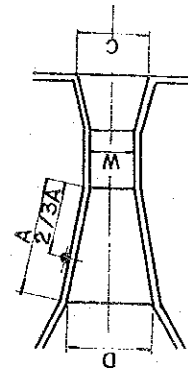
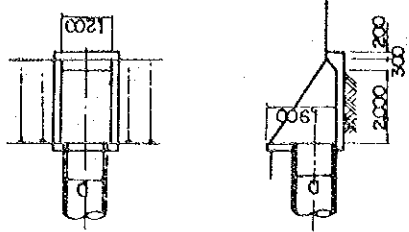
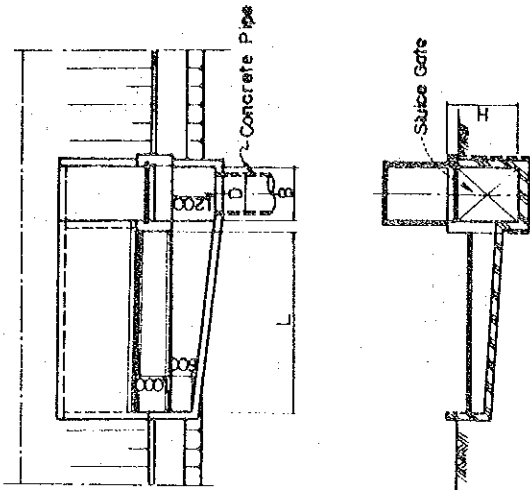


Fig. 29 Typical Dimensions of Parshall Flumes

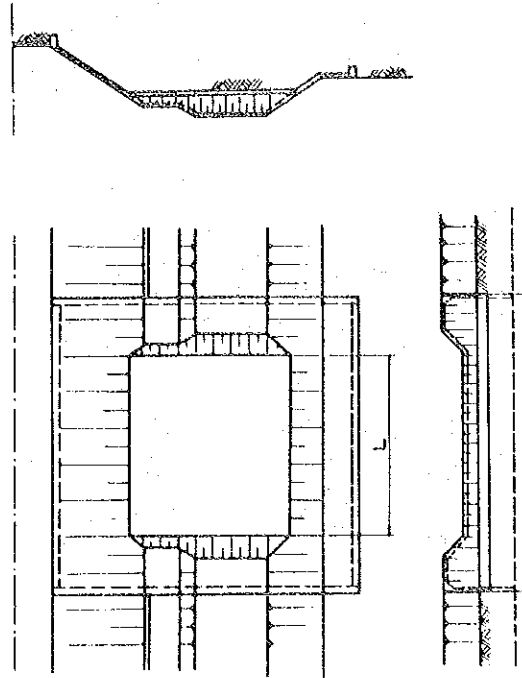
OUTLET



SPILLWAY



OVER CHUTE



DIMENSIONS OF SPILLWAYS

(Unit: mm)

TYPE	Discharge (m <sup>3</sup> /sec)	L	B	H	D	Sluice Gate
A-1	2.18 ~	15,000	1,800	2,000	1,000	1,800 x 1,800
A-2	1.74 ~	12,000	1,800	2,000	1,000	1,800 x 1,800
A-3	1.45 ~	10,000	1,600	1,800	1,000	1,600 x 1,600
A-4	1.16 ~	8,000	1,400	1,800	800	1,400 x 1,400
A-5	0.73 ~	5,000	1,200	1,800	800	1,200 x 1,200
A-6	0.44 ~	3,000	1,000	1,800	800	1,000 x 1,000

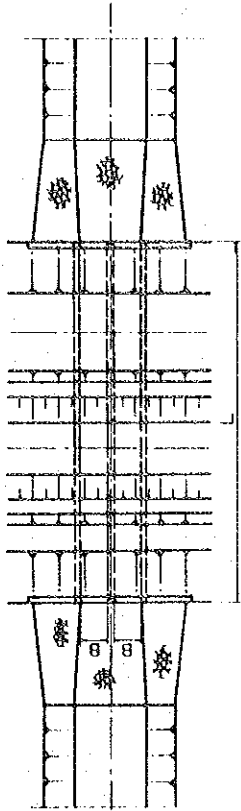
DIMENSIONS OF OVER CHUTES

TYPE	Discharge (m <sup>3</sup> /sec)	L (mm)
A-1	1.60 ~	10,000
A-2	1.28 ~	8,000
A-3	0.80 ~	5,000
A-4	0.48 ~	3,000

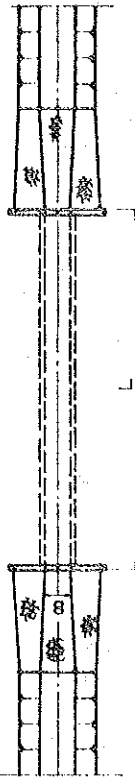
Fig. 30 Typical Dimensions of Spillway and Over Chute

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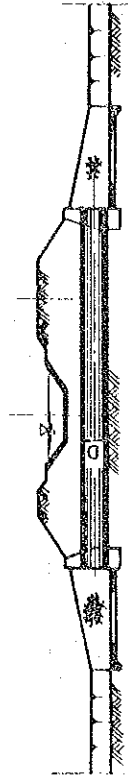
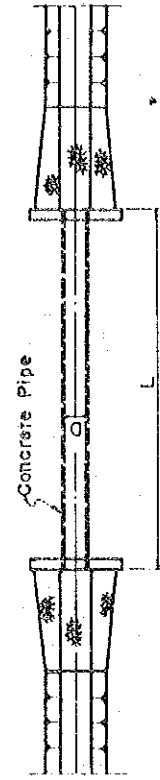
TYPE - A



TYPE - B



TYPE - C



**DIMENSIONS OF CROSS DRAINS (Unit: mm)**

TYPE	Discharge (m <sup>3</sup> /sec)	L	B	H	D
A - 1	16.0~12.0	20,000 ~ 30,000	2,000	2,000	—
A - 2	12.0~8.0	•	1,500	2,000	—
B - 1	8.0~6.5	•	2,000	2,000	—
B - 2	6.5~5.0	•	1,800	1,800	—
B - 3	5.0~4.0	•	1,600	1,600	—
B - 4	4.0~3.0	•	1,400	1,400	—
B - 5	3.0~2.0	•	1,200	1,200	—
B - 6	2.0~1.6	•	1,000	1,000	—
C - 1	1.6~1.0	•	—	—	1,000
C - 2	1.0~0.6	•	—	—	800
C - 3	0.6~	•	—	—	600

Fig. 31 Typical Dimensions of Cross Drains

KINGDOM OF THAILAND  
**THE EAST COAST WATER RESOURCES  
 DEVELOPMENT PROJECT PHASE II**  
 JAPAN INTERNATIONAL COOPERATION AGENCY

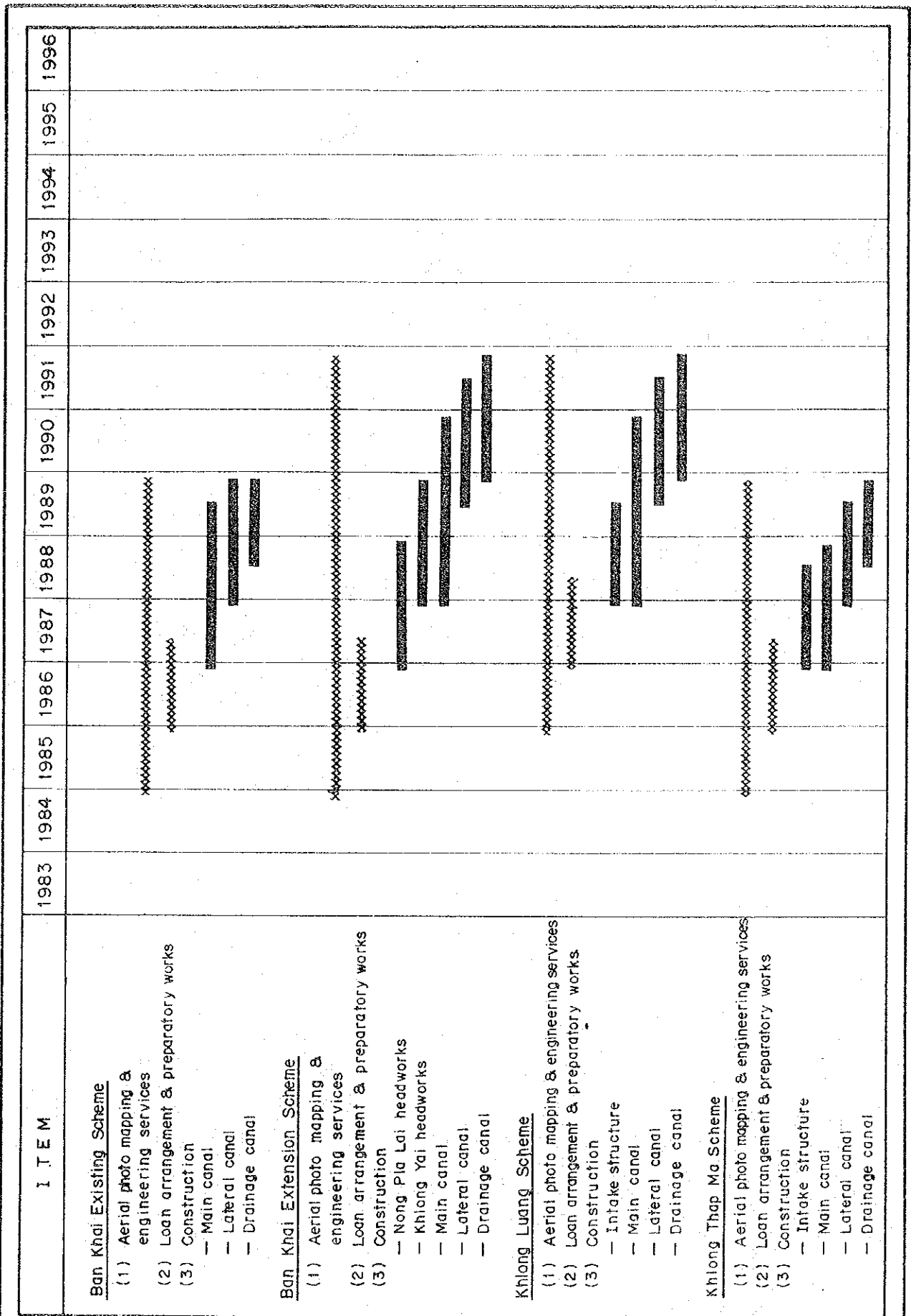


Fig. 32 Construction Time Schedule

KINGDOM OF THAILAND  
 THE EAST COAST WATER RESOURCES  
 DEVELOPMENT PROJECT PHASE II  
 JAPAN INTERNATIONAL COOPERATION AGENCY



**SECTORAL REPORT IV**  
**DOMESTIC AND INDUSTRIAL WATER DEMAND**





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## 1. INTRODUCTION

This sectoral report presents the result of water demand projection for domestic and industrial use until the Target Year of 2001. The result will be used as the basis of the water demand and supply balance study together with the agricultural water demand. Total water demand, will be compared with the available water resources and where deficit arises, development plan of water resources and water conveyance system is proposed.

With reference to the future water demand projection, two previous studies are available, namely East Coast Water Resources Development Project Phase I (hereafter called Phase I Study) and interim report of Eastern Seaboard Study (hereafter called ESS). In order to avoid the confusion among studies of similar kinds, the present study has adopted the methodology and results of ESS to a fairly good extent in agreement with RID. ESS has been chosen because the study was conducted more recently when compared with Phase I Study and was able to take policy amendments into account.

However, some parts of the results in ESS had to be amended so as to be used as the basis of the water balance study. The study area is divided into 10 zones for the purpose of the water balance study and accordingly projected population in ESS is rearranged to present the figure by each zone.

Projected water demand for domestic and industrial use is also adjusted and complemented so as to present the projection by each zone.

## 2. PROJECTED SOCIO-ECONOMIC FRAMEWORK

### 2.1 Industrial Development

The Fifth National Economic and Social Development Plan (hereafter called the Fifth National Plan) spells out the balanced economic development of all the regions of Thailand as its one of the major targets. Eastern Seaboard, which comprises three provinces, Chon Buri, Rayong and Chachoengsao was selected as the new industrial development centers in view of its good locational and economic factors. This region is located in the vicinity of Bangkok and North Eastern region which is the supplier of raw materials and labour forces to the region. It is also provided with relatively good infrastructural facilities. This region is planned to become new growth centers and the location of future basic industries. With implementation of the Eastern Seaboard Development, such targets as diffusion of growth from Bangkok, generation of new employment opportunities and creation of new gateway for the North Eastern region, are expected to be attained.

Since Eastern Seaboard Development owes much to the investment by private sector, several measures to be taken are under consideration to attract private investors, among which are provision of investment incentives and development of such infrastructural facilities as water resources and transportation networks.

Planned industrial development centers in the Study Area are, Chon Buri, Laem Chabang, Rayong, Map Ta Phut and Sattahip. Although Chachongsao is also one of the industrial centers selected by ESS, it is not included in the present study since it is located outside of the Study Area. The types of industries planned to be developed and development schedule at each center are summarized hereunder.

Industrial Center	Type of Industry	Starting Year	Year of full Operation
Chon Buri	Small Urban Industry	Existing	-
Laem Chabang	Industrial Estate	1985	2001
	Port	1996	2001
Si Racha	Industrial Estate	Existing	1990
Sattahip	Naval Base	1984	1990
	Port	1984	1984
Map Ta Phut	Gas separation/Petro-chemical	1983	1983
	Soda Ash	1985	1985
	Fertilizer	1982	1984
	Industrial Estate	1984	2001
Rayong	Agro-processing	Existing	-

## 2.2 Population

### 2.2.1 Population by Urban and Rural Areas

Future population of the Study Area is projected until the year 2001 based on the administrative boundaries in the Sectoral Report VI. I (Socio-Economy). The projected population is divided into urban and rural areas in this report. Urban area is the municipal areas and sanitary districts whose population are projected to exceed 5,000 by the year 2001 and rural areas the rest of the study area.

The standard of 5,000 is adopted in accordance with the Fifth Five Year Plan of Provincial Water Works Authority (PWWA) which plans to supply relatively developed areas with the population of more than 5,000 with piped water by large scale water works.

Projected population for urban and rural areas is summarized as follows:

PROJECTED POPULATION BY URBAN AND RURAL AREAS

	1981	1986	1991	1996	2001
Urban	358	419	501	603	738
Rural	570	619	659	669	645
Total	928	1,038	1,160	1,272	1,383

Unit: 10<sup>3</sup>

2.2.2 Demand Center and Zoning

There are 13 urban areas in the study area, out of which 6 are specified as development areas (hereafter called DA) and the rest as non-development areas (hereafter called NDA). DA will be new industrial centers of Eastern Seaboard as discussed in the precedent section. Since population is concentrated in urban areas, they are treated as demand centers. As shown in Fig. 1, the Study Area is divided into ten zones for the purpose of water balance study considering such factors as the location of demand centers, river basin boundaries, and the location of existing, planned and proposed water works. Zone 1 and Zone 10 include sub-zone 1-1 and 10-1 respectively. Accordingly future population and water demand is projected by each zone.

Name of demand centers and population by each zone are summarized in the table below.



NAME OF DEMAND CENTER

Zone		Name
1		Phanat Oikhom Than Bun Mi Nong Tamlung Bang Bung
2	DA	Chon Buri
3		
4	DA	* Laem Chabang
5	DA	Pattaya
6		Huai Yai
7	DA	Sa-tahip
8		Ban Chang
9	DA	Map Ta Phut
10	DA	Rayong Ban Khai

\* Hereafter Laem Chabang includes Si Racha

Historical and projected population of the Study Area is given in Table 1 and Fig. 2 and is presented by zone in Table 2 and Fig. 3.

### 3. WATER DEMAND PROJECTION

#### 3.1 Basis of the Study

This chapter presents the results of domestic and industrial water demand projection. Domestic water demand is divided into urban and rural water demand. Urban water demand is composed of the amount for residential and commercial use.

Projected demand in this report is consumer demand, which takes into account the water loss and unaccounted loss between consumer and water works. It is the amount of water to be supplied by water works. Loss between intakes and water works and in the purification process is determined to be 10 % of consumer demand and included in source demand.

Methodology for projecting the urban water demand refers to ESS. Such basic figures as consumption per capita and service factor are compared with the recorded figure collected by the Study Team and used in the projection.

Since ESS does not refer to NDA and rural water demand, projection for these areas was done by the Study Team. The consumption per capita is determined with reference the ESS.

Industrial water demand is projected in ESS. It is adopted in the present study with some adjustments based on data on industrial water supply by existing source facilities.

#### 3.2 Domestic Water Demand Projection

##### 3.2.1 Methodology

Domestic water demand is calculated by the following equation.

$$DWD = (CPC \times Pn \times SF) \times 365 \times 10^{-6}$$

where, DWD : Domestic Water Demand ( $10^6 \text{ m}^3/\text{yr}$ )  
CPC : Consumption Per Capita ( $\text{m}^3/\text{d}$ )  
Pn : Population  
SF : Service Factor

(1) Consumption Per Capita

Consumption per capita is determined to be 180 lit./d for urban areas and 140 lit./d for rural areas. 180 lit. for urban areas can be divided into 140 lit. and 40 lit. The former is consumed for domestic use and the latter is the allowance for commerce and light industry of which amount are too small to be treated separately. This allowance is not included in the water demand in rural areas since it is considered to be negligibly small.

Detail of 180 lit./d and past record of water consumption per capita are shown in Tables 3 and 4 respectively.

Based on the definition of consumer demand, unaccounted loss in the distribution system is taken into consideration and the following figures are determined as consumption per capita.

Unit: lit./d/c.

Year	Urban		Rural
	DA	NDA	
1981	360	225	175
1991	323	225	175
2001	276	225	175

Source: ESS and PWWA

The figures for DA are taken from ESS and the one for NDA and rural areas are determined based on the collected data. 180 lit./d for NDA and 140 lit./d for rural areas are adjusted to the figures above with 25% of loss in the distribution systems. Data on water loss is presented for recent five years in Table 5.

(2) Service Factor

Service factor is determined to be the ratio of the population supplied with piped water to the total population in the area.

Service factor is determined for each area as follows:

(Unit: %)			
Year	Urban		Rural
	DA	NDA	
1981	53	53	3
1991	60	60	15
2001	100	70	30

Source: ESS and PWWA

Service factor of urban areas in the Study Area for recent five years is summarized as follows:

(Unit: %)				
1977	1978	1979	1980	1981
50.0	51.7	54.4	52.8	53.6

Source: PWWA

Service factor grew at the annual rate of 1.8 % in recent five years.

Future growth is determined with reference to ESS for DA, while the one for NDA is determined at lower level in view of more concentrated development in DA.

Service factor for rural areas is determined based on the estimated service factor at present and PWWA's program for rural water supply projects in the Fifth National Plan. Since sufficient data is not available for the present service factor in rural areas, it is estimated considering such factors as plant capacity, assumed consumption per capita in rural areas and rural population. (Refer to Sectoral Report IX, Water Transmission Engineering).

For rural water supply, small scale projects have been implemented by the government for sanitary districts and residential area with population between 1,500 and 5,000. At the end of the Fourth National Plan, 663 systems were completed supplying water to 324 villages.

In the Fifth National Plan, the program to construct and expand water works systems are planned to be implemented. In the five years 250 water works are planned to be constructed with the government budget of  $1,004.62 \times 10^6$  Baht and local finance of  $133.0 \times 10^6$  Baht as well as development and expansion of existing water works with  $401.90 \times 10^6$  Baht from central government and  $57.0 \times 10^6$  Baht from local area.

Service factor for rural area is determined until the year 2001 on the basis that the government will continue to make effort on expanding water supply networks in rural areas.

### (3) Population

In the precedent section, future population is projected for each demand center and rural area. These figures are applied to the domestic water demand projection.

#### 3.2.2 Projected Water Demand

Domestic water demand is projected for each demand center and rural area for the year of 1991 and 2001 based on the methodology explained in the precedent subsection. For intermediate years 1986 and 1996, figures are obtained based on the figures of 1981, 1991 and 2001. That is to say, average amount of water demand in two years, 1981 and 1991 for instance, is adjusted to take into account the projected population growth rate. This is summarized by the following equation:

$$D_{86} = (D_{81} + D_{91}) / 2 \times P_{86} / (P_{81} + P_{91}) / 2$$

where,

- $D_{86}$  = Water demand in 1986
- $D_{81}$  = Water demand in 1981
- $D_{91}$  = Water demand in 1991
- $P_{86}$  = Population in 1986
- $P_{81}$  = Population in 1981
- $P_{91}$  = Population in 1991

Historical and projected domestic water demand of the Study Area is summarized in the table below and presented in Table 6 and Fig. 4. Water demand by each zone is presented in Table 7 and Fig. 5.

Area	(Unit: $10^6 \text{ m}^3/\text{yr}$ )				
	1981	1986	1991	1996	2001
Urban	19.0	26.5	34.7	51.1	71.0
Rural	0.8	3.6	6.4	9.8	12.2
Total	19.8	30.1	41.1	60.9	83.2

### 3.3 Industrial Water Demand Projection

#### 3.3.1 Water Use in Industry

In industrial activities water is used for such various purposes as raw material cleaning, boiler, cooling and controlling of the temperature and humidity.

Amount of water used varies considerably depending on such factors as purpose of used water, kind and scale of industry, degree of recycling and water tariff. The tables below present cases of Japan that amount of water consumed is different to a great extent depending on the kind and scale of industry.

Kind of Industry	Amount of Water per Production ( $\text{m}^3/\text{d}/10^8 \text{ Yen}$ )
Food	48.4
Textile	94.6
Pulp	386.6
Petroleum	151.2
Leather	11.0

(Unit: 10<sup>3</sup>/d)

No. of Employee	Water Used Per Worker
30 - 49	3.6
50 - 99	5.5
100 - 199	9.5
200 - 299	15.3
300 - 499	28.7
500 - 999	43.4
1000 -	123.8

For industrial water demand projection, there are three kinds of methodology in general: 1) Per capita method 2) Percentage of domestic water demand method 3) Per acreage method. Although, "3) Per acreage method" seems to be theoretically most justifiable, it can be applied only when sufficient data is available for kinds and area of industry.

### 3.3.2 Projected Water Demand

In the present study, industrial water demand is projected with reference to ESS and is adjusted based on the collected data on industrial water supply by existing water source facilities.

Both Phase I Study and ESS put forward the industrial water demand projection and their figures favorably compare with each other as shown in Table 8. In Phase I Study, projection was made based on previous reports and above-mentioned methodology; "3) per acreage method", for Laem Chabang and Rayong area. It is assumed in the study that the industrial area is 320 ha and 448 ha for Laem Chabang and Rayong respectively. Water requirement per hectare is assumed to be 90 ton.

The present study, however, adopts the figure of ESS considering that the ESS was conducted more recently and therefore could incorporate the recent policy amendments into the study. Sponge iron project is no more under consideration and the location of soda ash project was changed from Sattahip to Map Ta Phut. Industrial water demand projected by ESS is presented in Table 9.

Some adjustments is made to those figures based on the data on water supplied by existing source facilities. At present, there are several enterprises and facilities which are taking water directly from such existing reservoirs as Ban Phra, Map Prachan, Ban Bung and Dok Krai. Amount of water which should be guaranteed even in future is determined for these users based on the past record of water supply and is added to the demand projected by ESS at each zone. Water to be supplied by existing source facilities is presented in Table 10.

Total industrial water demand until the year 2001 is summarized in the table below.

(unit: $10^6$ m <sup>3</sup> /yr)				
1981	1986	1991	1996	2001
9.2	51.3	64.5	70.2	80.3

Historical and projected industrial water demand of the Study Area is presented in Table 11 and Fig. 6.

#### 3.4 Total Water Demand

Historical and projected water demand for domestic and industrial use is presented in Table 12 and Fig. 7. As shown below total water demand is projected to increase more than five fold in 20 years from  $28.7 \times 10^6$  m<sup>3</sup>/yr in 1981 to  $163.5 \times 10^6$  m<sup>3</sup>/yr in 2001. After consumer demand in this report is adjusted to source demand, it is used as the basis of the water supply and demand balance study together with agricultural water demand.

(Unit: $10^6$ m <sup>3</sup> /yr)				
1981	1986	1991	1996	2001
29.0	81.4	105.6	131.1	163.5



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