Basin map of the Khlong Luang river is shown in Fig. 2, and overall profile in Fig. 3 and Table 1.

2.1.2 Existing Flood Mitigation and Drainage Facilities

There exists Ban Bung dam in the upstream of the Huai Yang river which is a major tributary of the Khlong Luang river. The dam was constructed in 1958 for domestic, industrial and irrigation water supply purposes. The reservoir has a surcharge capacity of 1.0 x 10^6 m³ and active storage capacity of 0.4 x 10^6 m³.

In the lower reach of the Khlong Luang river, Phan Thong drainage canal was constructed for improvement of drainage condition. It connects the Khlong Nam Khen river with the Bang Pakong river. The canal is 7.8 km is length and two bays of the gate are installed in the lower end as shown in Fig. 4.

Along the highway Route-3, there have been constructed a large number of tidal gates by the RID in order to protect the cultivated land from the salt water intrusion and the backwater effect of the Bang Pakong river.

No other flood mitigation or protection works have been realized in the Khlong Luang river basin.

2.1.3 Channel Capacity

Channel capacity of the Khlong Luang river is calculated for a stretch between the Bang Pakong river and the proposed Khlong Luang damsite under the following conditions.

- (a) Channel flow calculation: by non-uniform flow formula using river profile surveyed in November, 1982. Table 2 gives the feature of the Khlong Luang river.
- (b) Manning's coefficient of roughness (n):

0.0 to 24.7 km : n = 0.03024.7 to 46.1 km : n = 0.035 The critical or bankful channel capacity is illustrated in Fig. 5 and is summarized below. It is considerably small compared with the magnitude of flood.

Stretch (km)	Average	Capacity	(m ³ /s)
0 - 10.4		63	
10.4 - 24.7		-23	
24.7 - 46.1	 e Les des	60	
		• •	

2.1.4 Flood Problems

As a results of the above analysis, the causes of flooding in the Khlong Luang river basin are characterized by the following:

- (a) In the middle and lower reaches, river courses are braided and channel capacities are thus remarkably reduced as revealed in sub-section 2.1.3.
- (b) In the lower reach, the flow is affected by tides through the Bang Pakong river which interfere the smooth gravity drainage of flood run-off of the Khlong Luang river.
- (c) Vertical bent of profile is observed near Phanat Nikhom as shown in Fig. 5. Channel capacity of the existing river channel also changes abruptly at this bent from $55 \text{ m}^3/\text{s}$ in the upstream to $22 \text{ m}^3/\text{s}$ in the downstream.

2.2 Rayong River

2.2.1 River System

The Rayong river flows southwards gathering its tributaries such as the Khlong Yai, Nong Pla Lai and Khlong Dok Krai rivers in the mountainous area. These rivers originate in Mt. Khao Liang Khwai (El. 179 m), Mt. Khao Chomphu (El. 725 m) and Mt. Khao Chak Kluai (El. 306 m) respectively. The paddy fields extend over the middle and lower reaches. In the lower reach, the Khlong Thap Ma river of which source is Mt. Khao Nang Yong (El. 419 m) joins with the main stream in the vicinity of Rayong city. After passing through Rayong city with meandering, it finally empties into the Gulf of Thailand. The Rayong river has a total length of 90 km from its source to rivermouth. Total drainage area amounts to 1,730 km².

The majority of the Rayong river basin lies in Rayong Province and a part of the basin in Chon Buri Province. The basin is bounded by the Prasae river basin in the east, the Khlong Luang river basin in the north and coastal river basins in the west.

About 17% of the basin, that is 295 km², is flat alluvial plain. The alluvial plain formed by the Nong Pla Lai, Khlong Dok Krai, and Khlong Yai rivers is approximately 13 m in elevation at Ban Khai weir. The gradient of river is 1/1,200 at the Ban Khai weir site and 1/4,000 near the highway Route-3.

Basin map of the Rayong river is shown in Fig. 6, and overall longitudinal profile of major channels are shown in Table 3 and Fig. 7.

2.2.2 Existing Flood Mitigation and Drainage Facilities

The Dok Krai dam, diversion channels and connecting channel have been constructed by RID as explained below.

Dok Krai Dam

The Dok Krai dam is located in the upper reach of the Khlong Dok Krai river which is one of the major tributaries of the Rayong river. It is situated at about 51 km upstream from the rivermouth of the Rayong river.

This dam was constructed in 1975 for domestic water supply, irrigation and flood control. The irrigation service area depending on the dam is about 4,800 ha. Total catchment area of the dam is 291 km 2 and its reservoir has a gross storage capacity of 70.8 x 10^6 m 3 . The dam has an effect in reduction of peak flood run-off by surcharge storage capacity of 20.0×10^6 m 3 .

Diversion Channels

Three diversion channels were constructed in 1962 for the purpose of draining flood water in the northern part of Rayong city. They are equipped with regulating gates to evacuate flood during the flood time and to prevent saline water intrusion during the dry season. Fig. 8 shows the location map of the diversion channels. The total design discharge of three diversion channels is 131 m³/sec and the cross-section for each channel is described below.

Divadaia			Cross-section				
Diversion Channel No.	Discharge	Length	Width	Height	Nos. of	Gate Sill	Remarks
NO.	(m ³ /s)	(km)	(m)	(m)	Gate	(El.m)	
				4			100
1	46	2.0	4.0	4.00	2	-1.0	West Side
2	25	1.6	2.25	2.25	3	0.6	East Side
3	60	1.6	6.0	4.00	2	-1.0	Center
1,							Marie Company

Connecting Channel

In addition to the three diversion channels, another channel of 500 m long was constructed at the same time, which connects the Rayong river with the Khlong Kha river in order to increase the capability of draining.

2.2.3 Channel Capacity

Channel capacity of rivers are calculated along the Rayong, Khlong Yai, Nong Pla Lai and Khlong Thap Ma rivers under the following conditions.

- (a) Channel flow calculation: by non-uniform flow formula for the main stem of Rayong river and uniform flow formula for the rest, using river profiles surveyed in 1981 for the Rayong river and in November 1982 for Khlong Yai and Khlong Thap Ma rivers. Tables 4 and 5 shows the features of the Rayong river and Khlong Yai and Khlong Thap Ma rivers respectively.
- (b) Manning's coefficient of roughness (n):
 - Rayong river

0.0 to 32.6 km : n = 0.030

32.6 to 47.7 km : n = 0.035

- Khlong Yai river : n = 0.035

- Khlong Thap Ma river : n = 0.030

The bankful channel capacity of each river is shown in Fig. 9 thru ll. The channel capacity is outlined as described below.

7	Stretch (km)	Average Capacity (m ³ /s)
(a)	Rayong River	
	0 - 23.1	50
	23.1 - 40.2	161
	40.2 - 47.7	29
(b)	Khlong Yai River	23
(c)	Khlong Thap Ma River	15

2.2.4 Flood Problems

The Rayong river forms a triangular shaped inundated area along the main river, getting wider toward downstream. At the lower end the river meanders considerably. Furthermore the river mouth is clogged by sand spit. The flooding in the lower Rayong river is also worsening the drainage of the tributaries. In order to solve these problems three floodways have been constructed on the meandering channel near the river mouth. The floodway, however, seems not successfully functioned due to clogging of the rivermouth.

2.3 Coastal Rivers

There are a number of small rivers in coastal area as shown in Fig. 12. These rivers are defined herein as coastal rivers. Most of coastal rivers have a catchment area less than 200 km² and major rivers in the coastal area are as follows.

River	Catchment Area (km²)
Khlong Yai Cheng	30.1
Huai Nong Khlong Kohk	20.4
Khlong Bang Prong	27.8
Huai Sukhrip	128.0
Khlong Bang Lamung	301.0
Huai Nong Pru	100.0
Huai Yai	119.0
Khlong Bang Phai	181.0
Khlong Phayun	31.8
Khlong Phala	18.8
Khlong Nam Tok	18.7
Khlong Huai Yai	120.0

There have been constructed four dams in the coastal area. They are Bang Phra, Map Prachan, Khlong Bang Phai and Phluta Luang dams. In addition, Nong Kho dam is under construction.

FLOOD DAMAGES

3.1 Historical Flood Events

No official report or information has been issued on the past floods in the Khlong Luang and Rayong river basins.

(1) Khlong Luang River Basin

The annual maximum daily discharges at Ban Mai station are shown in Table 6. The maximum discharge since 1965 was observed in Oct. 19, 1974. Recently, big floods were recorded on Sept. 19, 1978 and Sept. 23, 1981 which fall in the 4th and 5th order of magnitude in a 17-year's hydrological record respectively. The return periods of the 1974- and 1981-floods are estimated approximately at 18.0 and 3.6 years respectively by Thomas' method.

(2) Rayong River Basin

The annual maximum mean discharges at Ban Khai Weir are shown in Table 7. The maximum discharge since the year 1967 was observed on Oct. 20 in 1974. Recently, big floods were recorded on May 15, 1978, Oct. 24, 1980, and Sept. 23, 1981, respectively. They fall in the 2nd, 3rd and 7th order of magnitude respectively in a 15-year's hydrological record.

The return periods of the 1974- and 1981-floods are estimated approximately at 16.0 and 2.3 years respectively by Thomas' method.

(3) Seasonal Frequency of Floods

Almost all the big floods occurred in the months of September and October. Figs. 13 and 14 present the monthly frequencies of flood run-offs more than 25 m³/s at Ban Mai stream gauge and more than 70 m³/s at Ban Khai weir respectively. More than 80 % of floods concentrate in two months, September and October, in the both river basins.

3.2 Flood Damage Survey

In order to grasp the flooding conditions in the basin and gather the data required for the estimate of flood damages, flood damage surveys by the interview were conducted in the Khlong Luang and Rayong river basin by the Study Team.

The surveys were made at 31 sites in the Khlong Luang river basin and 27 sites in the Rayong river basin. Figs. 15 and 16 show the sampling points. Flooding data such as inundated depth and its duration were collected for the 1974-flood and 1981-flood which were the impressive events to the inhabitants.

(1) Khlong Luang River Basin

The results of survey are presented in Table 8 and Figs. 17 and 18. The inundated areas and land use are measured on the topographic maps (1/50,000) and shown in Tables 9 & 10. The inundated areas amounts to 58,300 ha for the 1974-flood and 44,400 ha for the 1981-flood of which breakdowns by land uses are as follows:

т Э тт	Inundated Area (ha)			
Land Use	1974-flood	1981-flood		
Paddy fields	40,100	30,400		
Uplands	2,100	1,700		
Village areas	1,700	1,500		
Other lands	14,400	10,800		
Total	58,300	44,400		

(2) Rayong River Basin

The results of survey are presented in Table 11 and Figs. 19 & 20. The inundated areas and land use are measured on the topographic maps (1/50,000) and shown in Tables 12 & 13. The inundated area amounts to 21,000 ha for the 1974-flood and 17,000 ha for the 1981-flood of which breakdowns by land uses are as follows:

		And the Control of th		
Land Use	Inundated Area (ha)			
nand ose	1974-flood	1981-flood		
Paddy fields	11,300	10,400		
Uplands	4,000	1,800		
Village area	1,600	1,200		
Other lands	4,100	3,600		
Total	21,000	17,000		
4				

3.3 Flood Damage Estimate

3.3.1 Flood Damage Categories

The flood damage is broadly classified into the following 4 categories.

- a. Paddy
- b. Upland crops
- c. House and household effects
- d. Livestock

The flood damages of these categories are estimated as a product of damageable value and damage ratio. The damageable value and damage ratio are estimated, in general, for respective inundated depths at every 0.5 m and, if necessary, for respective durations of inundation.

Other tangible damage and indirect damage are counted at 20% of the total values of the above items.

3.3.2 Damageable Value and Rate

The flood damage of each category is estimated based on the damageable value and damage rate as described hereunder.

(1) Damages to Paddy

The flood damage of paddy is the most predominant in the basin, since the paddy field occupies around 70% of the total inundated area.

Damageable Value

Damageable value of paddy is derived based on the agricultural statistics and economic unit price. Ratio between the local and improved varieties is assumed at 60:40. The damageable value is estimated as follows.

Unit	Khlong Luang River Basin	Rayon River Basin
t/ha	1.67	1.72
岁/ton		
	$8,430 \times 0.6$	8,360 x 0.6
	$8,010 \times 0.4$	$7,940 \times 0.4$
B/ha	13,760	14,090
	t/ha B/ton	### River Basin ### t/ha

The unit price is indicated in terms of economic price prospected to 1990.

Damage Ratio

Rates of reduction in yield due to inundation are based on the data prepared by Ministry of Agriculture, Forestry and Fishery, Japan which are summarized hereunder.

Depth		Durati	on (days)	e de la companya de l
(m)	0 to 1	1 to 4	4 to 7	More than 7
0 to 0.5	0.08	0.13	0.17	-
0.5 to 1.0		0.16	0.23	0.24
1.0 to 1.5	-	0.46	0.65	0.66
1.5 to 2.0	_		0.65	0.66

(2) Damages to Upland Crops

According to the agricultural statistical data cropping areas of upland crops are determined as follows.

	Cropping Ar	ea (%)
Crops	Khlong Luang	Rayong
·	River Basin	River Basin
Cassava	43.2	69.1
Sugarcane	40.7	14.8

Damageable Value

The damageable value of cassava is estimated by means of the same method as paddy as shown below. Since sugarcane is grown sufficienty high and harvested before September, it damage is thought to be negligible.

Description	Unit	Khlong Luang River Basin	Rayong River Basin
Unit yield	t/ha	13.2	16.8
Unit price	\(\begin{aligned} \begin{aligned} \text{ton} \end{aligned} \)	1,250	1,250
Planting area	8	43.2	69.1
Damageable value	₿/ha	7,130	14,510

Damage Ratio

Since the data on damage ratio of cassava are not available, damage ratio of sweet potato prepared by Ministry of Construction, Japan is adopted. It is presented below.

	5.3	and the second s	the state of the s		
Depth		Durati	on (days)	2 - 2	
(m)	1 to 2	3 to 4	5 to 6	Above 7	
0 to 0.5	0.11	0.30	0.50	0.50	
0.5 to 1.0	0.27	0.40	0.75	0.88	
Above 1.0	0.38	0.63	0.95	1.00	
			:		

(3) Damages to House and Household Effects

Damageable Value

The number of house in unit village area is estimated based on the house number in each district and village area measured on the topographic maps of scale 1/50,000. The Phanat Nikhom, Phan Thong and Ban Khai Districts are selected as representative areas for the estimate. The average

On the other hand, the amount of household effects is assumed broadly to be 80% of house (building) value based on the survey results and technical practice in similar countries in Southeast Asia. Accordingly, the unit value of house and household effects is estimated \$39,600/house as follows for both the Khlong Luang and Rayong river basins.

Description	Unit	Amount
lue of house	∄/house	22,000
lue of household effects	炒/house	17,600
mageable value	₿/house	39,600
mageable value	B/house	

Damage Ratio

The following damage ratios were established by adjusting the ratio prepared by Ministry of Construction, Japan, in due consideration that the most houses in the inundated areas are those with high floor about 1.0 m above the natural ground level.

Depth (m)	Damage Ratio
0 to 1.0	0.03
1.0 to 1.5	0.05
1.5 to 2.0	0.07
Above	0.11

(4) Damages to Livestocks

The buffalo, pig, chicken and duck are the major livestocks in the basin. Among these, pig, chicken and duck are presumed to be susceptible to flood damage. The livestock raising rate is as follows, according to the agricultural statistical data.

	Raising Rate	(head/house)
Livestock	Khlong Luang River Basin	Rayong River Basin
Pig	2.26	0.47
Chicken	45.60	2.62
Duck	34.70	0.64

Damageable Value

The damageable is estimated as tabulated below.

	Yradd your	Damageable Values (\$/house)		
Livestock	Unit Price (Ø/head)	Khlong Luang River Basin	Rayong River Basin	
P i g	2,316	5,230	1,090	
Chicken	24.25	1,110	54.8	
Duck	30.25	1,050	19.4	

Damage Ratio

Since no data on damage ratio of livestocks is available, the following damage ratios are assumed according to the information obtained in the inundated areas:

the state of the s					
Depth	Damage Ratio				
(m)	Pig	Chicken	Duck		
Below 0.5	0.3	0.5	0.3		
0.5 to 1.0	0.8	1.0	0.9		
Above 1.0	1.0	1.0	1.0		

3.3.3 Flood Damages

The flood damage of each catogory is estimated for the 1974- and 1981- floods based on flood damageable value, rate and inundation area.

The flood inundation areas by land uses are shown in Tables 9 and 10 for the Khlong Luang river basin and Tables 12 and 13 for the Rayong river basin. The flood damages are calculated for each category as shown in Tables 14 to 21 for the Khlong Luang river basin and Tables 22 to 29 for the Rayong river basin. The summary of the flood damage estimate is as follows.

n	Khlong Luang	Khlong Luang River Basin		Rayong River Basin		
Damage Category	1974-flood 1981-flood		1974-flood	1981-flood		
Paddy	173.5	95.8	48.6	34.2		
Uplands	3.4	1.8	15.1	6.4		
House and household effects	25.6	18.6	21.2	15.0		
Livestock	109.5	75.1	9.8	7.6		
Other tangible	62.4	38.3	18.9	12.6		
Total	374.4	229.6	113.6	75.8		
10001		227.0				

3.4 Annual Average Flood Damages

3.4.1 Flood Damage Curve

In order to facilitate the flood mitigation planning, the Khlong Luang and Rayong rivers are divided into 5 blocks and 6 blocks respectively as shown in Table 30. Flood damage curves are prepared for the respective blocks, distributing the flood damage in proportion to the extent of inundated areas of the respective blocks, and according to the return periods of floods as follows by Thomas' method:

D.1	Return Period (yr)			
River	1974-flood	1981-flood		
Khlong Luang	18.0	3.6		
Rayong	16.0	2.3		

The flood damage curves for each blocks are shown in Fig. 21 for the Khlong Luang river and Fig. 22 for the Rayong river.

3.4.2 Annual Average Flood Damage

By use of the damage curves and flood frequency curves of each blocks, the annual average flood damages are estimated as follows:

* .		(Unit: \$ 10 ⁶)
Inundation Block	Khlong Luang River Basin	Rayong River Basin
A	20.9	5.7
В	13.4	5.6
С	48.9	9.2
D	79.6 ^{/1}	16.9
${f E}$	157.7	36.2
. F	en de la companya de La companya de la co	16.9
G	-	56.6
Total	240.9	147.1
the state of the s		

 $\underline{/1}$: Excluded from total because of interior inundation

The flood frequency curves are constructed as shown in Fig. 23 for the Khlong Luang river basin and Fig. 24 for the Rayong river basin, based on the flood run-off analysis presented in Sectoral Report VII, Meteorology and Hydrology.

4. BASIC CRITERIA FOR FLOOD MITIGATION PLANNING

4.1 Approach to Planning

4.1.1 General Description

The flood damage situations in both the Khlong Luang and Rayong river basins are revealed through the analyses of channel capacity and flood damage survey as explained in Chapters 2 and 3 respectively. It could be expected that flood hazards to life and health will increase as population increase, and intesified use and occupancy of flood plain lands will result in increased property damage from future floods.

An appropriate flood protection measure will be essential for enhancement and prosperity of economic activity, conservation of land, increase of agricultural productivity and assurance of human life and health. Such protective measure, however, should be established in view of long-term perspective.

In order to regulate and drain the flood rum-off without causing damages, the following measures are taken in general:

(1) Channel improvement

The channel improvement which mainly aims at increasing flow capacity of channel is the principal and substantial flood mitigation measures. The followings are the major works included in the channel improvement:

- a. Widening and deepening of low water channel.
- b. Diking system to confine the flood run-off in the river area. Another function of the diking system is to clarify the river area to be conserved for flood control.
- c. Revetment, groyne and consolidation works to protect and maintain the river facilities like river channel and dikes.

(2) Cutoff channel

The cutoff channel in the meandering river is planned aiming at generally;

- a. Shortening the stretches subject to the channel improvement and, accordingly, to save the construction cost,
- Increasing channel capacity by steepening the river slope,
 and
- c. Preventing local scouring due to channel bent by transfering the channel course.

(3) Floodway or flood diversion channel

The floodway or flood diversion channel is planned generally in the case that;

- a. Channel improvement works and land acquisition are difficult in the lower reaches due to urban area, river port, etc.
- Construction cost of floodway gets lower than that of improvement of existing channel, and
- c. Improvement of existing channel is not technically and economically effective enough from the topographic features such as in low-lying areas.

(4) Dam reservoir and retarding basin

Function of these measures is to reduce the peak run-off, regulating and retarding the flood run-off artificially.

(5) Basin management

Regulating and guiding the land use in the basin, flood damages are mitigated in the following manners:

a. Conservation of natural flood retarding functions such as in swampy area and forest in the upper watershed. b. Reduction of the damageable properties in the flood prone area.

4.1.2 Methodology

A new water resources development is being contemplated to be carried out in both the Khlong Luang and Rayong river basins in order to cope with the domestic and industrial water demand and irrigation water demand.

In the Rayong river basin, Dok Krai dam has been constructed and Nong Pla Lai dam is being contemplated to be implemented within several years. The Khlong Yai dam is situated in the Khlong Yai river, which joins with the main stem of the Rayong river at about 3.5 km downstream from the proposed damsite and is expected to be implemented after Nong Pla Lai dam.

Also in the Khlong Luang river basin, New Ban Bung and Khlong Luang dams are being planned to be constructed in near future.

These dams will evidently contribute to flood damage reduction in the respective basin.

A basic flood control plan was initially elaborated. The plan aims at protecting the entire riparian lands for a standard project flood. The standard project flood has a recurrence interval of 50 years. The plan is formed by a combination of dams and river improvement works.

The magnitude of the river improvement works is dominated by flood control effect by reservoir. The flood control capability of reservoir varies with surcharge volume and spillway discharge capacity. A set of alternatives are therefore set up by employing the width of spillway as a parameter. An alternative with minimum cost is selected as the proposed basic flood control plan.

Economic viability of river improvement works is testified under condition that all contemplated dams should have been constructed. It is worked out for three different risk levels, namely, 10-year, 30-year and 50-year floods.

4.2 Run-off Simulation Model

The flood run-off along the river course is calculated by applying the run-off simulation model, which is explained in detail in Sectoral Report VII, Meteorology and Hydrology. The run-off simulation models are formulated for the Khlong Luang and Rayong river basins respectively as shown in Figs. 25 and 26.

The same models are adopted in evaluating the flood regulation effects by dams.

4.3 Criteria for Channel Design

The following criteria is employed for design of channels.

(1) Channel section

Compound section is adopted in principle, which is composed of low water channel and high water channel confined by dikes on the existing river banks.

(2) River width

The river width between both dikes is decided based on the following criteria recommended by Ministry of Contruction, Japan:

ı)
60
80
120
220
450

(3) Design river bed profile

The design river bed profile is set based on the average river bed of the existing channel.

(4) Low water channel section

The low water channel is excavated above the design river bed so as to get materials for dikes as well as to increase channel capacity. The excacation volume is thus nearly equivalent to embankment volume considering the swelling factor and some loss. The side slope of the channel is 1 on 3.

(5) Design high water level

The design high water level is decided based on the channel flow calculation for the designed section. Manning's coefficient of roughness for designed channel is taken at 0.03 to 0.035 for low water channel and 0.05 to 0.06 for high water channel depending on the river conditions.

(6) Dike section

The elevation of dike crown is decided above the design high water level adding some freeboard. The standard dike section based on the criteria by Ministry of Construction, Japan illustrated in Fig. 27.

5. PRELIMINARY FLOOD MITIGATION PLANNING FOR KHLONG LUANG RIVER

5.1 Formulation of Basic Flood Control Plan

5.1.1 Alternatives

In the Khlong Luang river basin, New Ban Bung and Khlong Luang multiple-purpose dams are being contemplated to be developed in near future. The basic flood control plan of the basin is constituted by a combination of these dams and river improvement works.

It is being anticipated that New Ban Bung dam is to be implemented within a couple of years. Its feasibility study has already been completed and its development scale has also been determined. The basic flood control plan is thus subject to flood regulation effect of Khlong Luang dam. Three alternatives are established as shown below for comparision.

Structures		Unit		Alternatives		
Structures		OUTC	1	2	3	
New Ban Bung Dam						
High water level	÷	El.m	82.1	82.1	82.1	
Flood water level	100	El.m	84.3	84.3	84.3	
Surcharge volume		10 ⁶ m ³	7.8	7.8	7.8	
Spillway width		m	20.0	20.0	20.0	
Dam crest		El.m	86.3	86.3	86.3	
Khlong Luang Dam	•			1		
High water level		El.m	39.5	39.5	39.5	
Flood water level		El.m	40.6	40.5	40.4	
Surcharge volume		10^6 m 3	39.2	34.3	31.4	
Spillway width	•	m.	50.0	70.0	90.0	
Dam crest		El.m	42.6	42.5	42.4	
River Improvement Work	•			. :		
Length		km	47.0	47.0	47.0	
Earthwork		$10^{3} \mathrm{m}^{3}$	5,712	5,720	5,725	

The high water level of Khlong Luang reservoir has been determined from the viewpoint of conservation use as explained in Sectoral Report XI, Water Resources Engineering. The spillway and surcharge volume have been designed for a design flood with recurrence interval of 500-year.

The quantity of the river improvement works is resulted from preliminary design of channel improvement as explained in subsection 5.1.3.

5.1.2 Flood Regulation by Dams

The proposed reservoirs retain temporarily a substantial portion of flood run-off, resulting in reduction in a rate of peak discharge in the downstream reach from the damsites. The flood regulations by reservoirs were worked out for various probable floods based on the run-off simulation model. Tables 31 to 33 show the probable food run-offs under the present, with New Ban Bung and with Khlong Luang dam respectively.

Due to flood control effects by reservoirs, flood frequency curves naturally change. The modified flood frequency curves are constructed as a result of reservoir regulation effect. The modified curve is shown in Fig. 23 for the selected plan which has a spillway width of 70 m in Khlong Luang dam.

5.1.3 River Improvement Works

The distribution of flood discharge is determined for each alternative taking into account the flood regulation effects of New Ban Bung and Khlong Luang reservoirs and execution of river improvement works. It is shown in Table 34 and is illustrated in Fig. 28.

The preliminary layout design of river imprvement works was carried out in accordance with the criteria set forth in Section 4.3. The main stem of the Khlong Luang river is divided into 6 reaches as shown in Fig. 29. Channel factors applied for design of channel improvement works are shown in Table 35.

Based on the preliminary design, construction cost of the river imprvement works was estimated for the respective alternative as shown in Tables 36 to 38. The quantity of earth work, practically dike embankment work indicates only slight difference among the alternatives, since flood run-offs from the downstream area is so large compared to critical channel capacity.

5.1.4 Comparison of Construction Cost

The construction cost of the flood control measure is expressed as the sum of cost of Khlong Luang dam and cost of river improvement works. The construction cost of Khlong Luang dam attributable to flood control is assumed to be equivalent to the dam embankment cost above normal high water level and spillway cost. The comparison of construction cost is tabulated hereunder.

	÷ .	(Uni	t: 106)
		Alternatives	
Structures	1	2	3
Khlong Luang Dam (incremental only)	307.7	302.9	303.9
River improvement work	1,086.6	1,087.4	1,088.2
Total	1,394.3	1,390.3	1,392.1

The construction costs of three alternative are almost even. The Alternative 2 however, indicates the minimum cost. Further comparison was made from the view point of total construction cost of Khlong Luang dam as presented below.

	Spillway	Constr	Construction Cost (\$ 106)		
Alternative	Width(m)	Dam	Spillway	Total	
1	²³ 50	824,8	26.9	851.7	
2	70	815.8	31.1	846.9	
3	90	812.2	35.7	847.9	

The total construction cost is the lowest for spillway width of 70 m. Therefore, the Alternative 2 is adopted as the basic flood control plan.

5.1.5 Preliminary Design of Channel

The basic flood control plan for a 50-year probable flood is preliminarily formulated as described in the above. The outlines of the river improvement works are described herein.

The alignment of dikes is designed as smooth as possible confining the existing river channel with specified river width at minimum. The designed plan, profile and typical sections are shown in Figs. 30 through 32.

The revetment for low water channel is placed on the slope from the lowest river bed of the existing channel and high-water revetment on the dike slope up to design high water level.

As for the countermeasures to the tributaries and local drainage behind the dike, backwater levee or drainage sluices are considered. The backwater levees are provided for the tributaries. For the drainage of the area other than the tributaries, the drainage sluices are provied at the rate of one sluice (1.5 m \times 1.5 m) for 2.0 km 2 of drainage area.

The land areas to be acquired for channel improvement are those confined by the dikes excluding the existing channel areas.

5.2 Flood Regulation Effects by Dams

Although both the New Ban Bung and Khlong Luang dams have no specific flood control space other than surcharge volume in their reservoirs, they incidentally reduce flood stage in the downstream as indicated by the modified flood frequency curves in Fig. 23. The flood control benefits due to dam was measured as the difference between the annual damages under unregulated conditions of flooding and those with the reservoir in operation.

The flood control benefits are calculated block by block in accordance with development sequence of dam and based on the flood damage curves and modified flood frequency curves as shown in Tables 39 and 42. The summary is given below.

		<u> </u>			(Unit:	₿ 10 ⁶)
Condition			idual Dama			Total
		Block A	Block B	Block C	Block E	······································
1.	Unregulated	20.9	13.4	48.9	157.7	240.9
2.	N. Ban Bung	20.9	13.4	44.3	151.7	230.3
3.	N. Ban Bung + Khlong Luang	0.3	1.1	30.3	148.7	180.4

The annual flood control benefit of the Khlong Luang dam is assessed to be 10^{6} 49.9 x 10^{6} .

5.3 Appraisal of River Improvement Works

5.3.1 General Description

The basic flood control plan has been established for the main stem of the Khlong Luang river by means of a combination of New Ban Bung and Khlong Luang dams and river improvement works. The river improvement plan involved in the basic flood control plan is for a long-range objective and would be realized as a mean of final flood control resort, since existing channel capacity is not capable of flow down safely the flood discharge. It, however, would be planned to be executed progressively in due consideration of flood damage severities.

Taking the above circumstance into consideration, economic viability of river improvement works was examined. In the study, the river improvement works are assumed to be taken place upon completion of New Ban Bung and Khlong Luang dams and its economic viability is testified for three risk levels as follows.

of Flood (year)
10
30
50

5.3.2 Channel Design and Construction Cost Estimate

The design of the channel improvement is followed to the design criteria set forth in Section 4.3 and is based on the channel factors shown in Table 43 and discharge distribution shown in Fig. 33. The channel factors for Risk Level "3" are the same as those shown in Table 35.

The work quantity and construction cost are estimated for Risk Levels "1" and "2" as shown in Tables 44 and 45. Those for Risk Level "1" are the same as Table 37.

5.3.3 Economic Comparison

The residual damages after construction of New Ban Bung and Khlong Luang dams are counted as benefit of river improvement works. They are quoted from Tables 39 to 42 and are summarized below.

		(Unit:	≱ 10 ⁶ /yr)
Diomber		Risk Level	
Blocks	1	2	3
A	0.279	0.308	0.315
В	0.933	1.037	1.063
С	25.115	28.158	28.890
E	132.146	143.227	145.423
	.*		
Total	158.473	172.730	175.691

The construction cost including an interest during construction is converted into the annual equivalent cost at assumed discount rate of 8 % per annum. The interest during construction is calculated assuming construction period of 7 years. The useful life is assumed to be 50 years. The annual 0 & M cost is also assumed to be 0.5 % of the initial construction cost. The annual equivalent cost, 0 & M cost and benefit are tabulated below.

		(Unit:	ß 10 ⁶ /yr)
Description		Risk Levels	
Descripción	. 1	2	3
Construction Cost	1,192.2	1,268.9	1,331.0
Annual Equivalent Cost	97.4	103.7	108.7
Annual OM Cost	4.87	5.18	5.44
Annual Benefit	158.47	172.73	175.69
Annual Net Benefit	+56.20	+63.93	+61.55

As shown in the above, the river improvement works induce a positive economic return for all cases and appears to be most attractive for Risk Level "2".

It is expected, therefore, that the river improvement works would firstly be proceeded aiming at protecting the flood with the recurrence interval of 30 years. It is recommendable to carry out systematic statistical survey on flood damages so that economic and financial losses in the basin will realistically be clarified. Then the economic viability of the river improvement works should be reviewed.

6. PRELIMINARY FLOOD MITIGATION PLANNING FOR RAYONG RIVER

6.1 Formulation of Basic Flood Control Plan

6.1.1 Alternatives

There are one dam in operation, one dam under planning and two dams under study in the Rayong river basin as listed below.

Dams	Rivers	Catchment Area (km ²)	Status
Dok Krai	Khlong Dok Krai	291	Existing
Nong Pla Lai	Nong Pla Lai	408	Under planning
Khlong Yai	Khlong Yai	218	Under study
Khlong Thap Ma	Khlong Thap Ma	158	Under study

Upon completion of the above-listed 4 dams, out of 1,730 km² of the entire Rayong river basin, about 1,075 km² or 62 % would be kept under control. Thus these four dams certainly result in reduction in flood damage in the basin.

At present it is being expected that the three dams would be implemented in order of (i) Nong Pla Lai dam, (ii) Khlong Yai dam and (iii) Khlong Thap Ma dam, in compliance with the proposed development sequence in the long-term water supply.

The basic flood control plan of the Rayong river was established through two steps. The first step is directed to lay out the optimum plan with a combination of three dams (Dok Krai, Nong Pla Lai and Khlong Yai dams) and river improvement works. The second step is to work out the appropriate flood control plan for the Khlong Thap Ma river by a combination of Khlong Thap Ma dam and river improvement. The overall basic flood control plan of the Rayong river is given by the results of Steps "1" and "2".

For Step "1", three alternatives are established for their cost comparison as tabulated below.

					and the second section of the section of the second section of the
Structures		Unit		Alternatives 2	3
Dok Krai Dam					
Surcharge volume		10 ⁶ m ³	20.0	20.0	20.0
Spillway width		m	ø10.0	ø10.0	ø10.0
Nong Pla Lai Dam					
Surcharge volume		10^{6}m^{3}	43.5	43.5	43.5
Spillway width		m	120.0	120.0	120.0
Khlong Yai Dam	•		•		
High water level		El.m	47.5	47.5	47.5
Flood water level		El.m	48.9	48.8	48.7
Surcharge volume		10 m	18.6	16.9	15.6
Spillway width		m	50.0	70.0	90.0
River Improvement Work		7 .			
Length		km	47.3	47.3	47.3
Earth work		10^3m^3	3,880	3,910	3,950

The spillway width and surcharge volume of Nong Pla Lai dam is referred to the Phase I Study.

Also three alternatives are set up for Step "2" as shown below.

Ot the			Alternative	s
Structures	Unit	1	2	F - 3
Khlong Thap Ma Dam				
High water level	El.m	25.7	25.7	25.7
Flood water level	El.m	27.0	26.9	26.8
Surcharge volume	10^6m^3	15.2	13.5	12.8
Spillway width	m	30.0	50.0	70.0
Dam crest	El.m	29.0	28.9	28.8
River Improvement Work		·		
Length	km	10.5	10.5	10.5
Earth work	10^3m^3	982	995	1,003

The high water levels of Khlong Yai and Khlong Thap Ma reservoirs have been determined at El. 43.5 and El. 25.7 m respectively through the land and water resources development plan. The spillway is designed as a side-channel spillway without gate and its crest elevation is the same with the high water level. The surcharge volume and flood water level are decided by routing the 500-year inflow flood.

The requirement of the river improvement work was worked out subsequent to flood routing by reservoir and according to the flood run-off simulation model presented in Section 4.2. Preliminary design of channel improvement work is carried out based on the criteria recommended by Ministry of Construction, Japan.

6.1.2 Flood Regulation by Dams

The reservoir withhelds a portion of flood run-off for a while, resulting in reduction of a peak discharge in the downstream. The regulation effect however varies with width of spillway in case of non-gated spillway. The flood regulation by reservoir was worked out for various probable floods and for each alternative based on the run-off simulation model. The summaries of the calculations are shown in Tables 46 to 50.

Due to flood regulation by reservoirs, flood frequency curves naturally vary. The modified flood frequency curves are shown in Fig. 24 following to the development sequence of dams.

6.1.3 River Improvement Works

The distribution of flood discharge is preliminarily decided for each alternative of each Step, taking the flood regulation effects of dams. It is shown in Table 51 and Fig. 34 for a case of Dok Krai, Nong Pla Lai and Khlong Yai dmas and in Table 52 and Fig. 35 for a case of four dams, including Khlong Thap Ma dam. The effect of the Khlong Thap Ma dam is limited to the Khlong Thap Ma river and the extreme downstream reach of the Rayong river.

The preliminary layout design of the river improvement works was made for Steps "1" and "2" in accordance with the criteria set forth in Section 4.3 and discharge distributions. The main stems of the Rayong and Khlong

Thap Ma rivers are divided into 7 and 3 reaches respectively as shown in Fig. 36 for the preliminary design. Channel factors applied for the preliminary design are shown in Tables 53 and 54.

Based on the preliminary design, construction quantity and construction cost were estimated for each alternative of each Step. Tables 55 to 57 show the construction quantity and cost of the river improvement works of the Rayong river in case of 3 dams (Dok Krai, Nong Pla Lai and Nong Pla Lai dams) in operation. Tables 58 to 60 summarizes the construction quantity and cost of the river improvement works of the Khlong Thap Ma river only in case of Khlong Thap Ma dam in operation.

6.1.4 Comparison of Construction Cost

The same criteria as employed in the preceding sub-section 5.1.4 is applied for comparison of construction cost. The comparison of construction cost for Steps "1" and "2" is presented below.

			Const	ruction Cost (B	106)
	Description		Dam	River Impro. Works	Total
1.	Step "1"				No. of
	Alternative 1		298.5	1,187.2	1,485.7
	Alternative 2		295.0	1,190.4	1,485.4
	Alternative 3		298.7	1,191.2	1,489.9
2.	Step "2"	· · · · · · · · · · · · · · · · · · ·			
	Alternative 1		132.6	126.4	259.0
	Alternative 2		131.8	127.1	258.9
	Alternative 3		138.6	127.5	266.1

As shown in the above, the construction costs indicate only slight difference among the alternatives. Thus further comparison was made from the viewpoint of total construction cost of dam as tabulated below.

Dams	Spillway	Const	ruction Cost	(A 10e)
Dams	Width(m)	Dam	Spillway	Total
Khlong Yai	50	732.4	62.1	794.5
	70	718.4	72.6	791.0
	90	711.4	83.3	794.7
Khlong Thap Ma	30	314.2	43.9	358.1
	50	306.3	51.0	357.3
	70	302.9	61.2	364.1

Through the above two comparisons, the most appropriate basic flood control measures are considered to be Alternative 2 for both the Steps "1" and "2".

Accordingly, the discharge distribution in case of 4 dams (Dok Kari, Nong Pla Lai, Khlong Yai and Khlong Thap Ma dams) with the river improvement works is corresponded to the middle line in Fig. 35. The construction quantity and cost of the river improvement works are estimated for a case of 4 dams in operation as shown in Table 61.

6.1.5 Preliminary Design of Channel

The preliminary design of channel in case of 4 dams in operation is briefly described hereunder.

The dikes are aligned as smooth as possible along the existing river channel as shown in Fig. 37. The longitudinal profile is arranged almost the same slope with the existing channel as shown in Fig. 38. The typical cross section shown in Fig. 39 is laid out following the criteria set forth in Section 4.3.

The channel protection works and other associated works are the same as explained in the preceding sub-section 5.1.5.

6.2 Flood Regulation Effects by Dam

The flood control effect by dam is evaluated based on the modified flood frequency curves and flood damage curves. The evaluation was made for the following cases, taking into account the development sequence of dams.

Case 1 : Unregulated

Case 2 : Regulated by Dok Krai Dam

Case 3 : Regulated by Dok Krai and Nong Pla Lai Dams

Case 4 : Regulated by Dok Krai, Nong Pla Lai and Khlong Yai Dams

Case 5 : Regulated by Dok Krai, Nong Pla Lai, Khlong Yai and Khlong Thap Ma Dams

The flood damage reduction was calculated based on the above order and zone by zone as shown in Tables 62 and 68. The summary is given hereunder.

	· · · · · · · · · · · · · · · · · · ·	Re	sidual Da	amages ()	8 106/ye	ar)		
Cases	Block	Block	Block	Block	Block	Block	Block	Total
	A	В	C	D	E	F	G	
1	5.7	5.6	9.2	16.9	36.3	16.9	56.6	147.2
2	5.7	0.4	9.2	9.4	23.2	16.9	47.0	111.8
3	0.1	0.4	9.2	2.9	10.7	16.9	34.4	74.6
4	0.1	0.4	0.2	1.4	7.3	16.9	28.3	54.6
5	0.1	0.4	0.2	1.4	7.3	2.7	23.0	35.1
				41.1				

The average annual flood control benefit of the respective dam is calculated as follows.

· ·	
Dams	Flood Control Benefit (BlO ⁶ /yr)
Dok Krai	35.4
Nong Pla Lai	37.2
Khlong Yai	20.0
Khlong Thap Ma	19.5

6.3 Appraisal of River Improvement Works

6.3.1 General Description

The basic flood control plan of the Rayong river is preliminarily established by a combination of 4 dams and river improvement works as aforementioned. In accordance with the principles described in Section 5.3, the economic viability of the river improvement works was examined for the following cases and risk levels.

(1) Cases

Cases	Dams in Operation
1	Dok Krai, Nong Pla Lai and Khlong Yai
2	Dok Krai, Nong Pla Lai, Khlong Yai and Khlong Thap Ma
3/1	Khlong Thap Ma

/l : Khlong Thap Ma river only

(2) Risk Levels

Risk Level	Recurrence Interval of Flood (year)
1	10
2	30
3	50

6.3.2 Channel Design and Construction Cost Estimate

The discharge distribution by the respective case and risk level is shown in Figs. 40 and 41. For Risk Level "3", the discharge distribution is that shown in Figs. 34 and 35.

Preliminary channel design is dependent on channel factors presented in Tables 69 and 70, which are adopted for the Risk Levels "1" and "2". The channel factors for the Risk Level "3" are the same with Tables 53 and 54.

The construction quantity and cost of the river improvement works are estimated for Risk Levels "1" and "2" of each case as shown in Tables 71 to 76. The construction quantity and cost for the Risk Level "3" are the same with those of the basic flood control plan.

6.3.3 Economic Comparison

The residual flood damages are obtained case by case and risk by risk based on the modified flood frequency curves and flood damage curves as shown below.

4.			the state of the state of			and the second		
	Aver	age Ann	ual Res	idual [amages	(X 106)	/yr)	
Cases	Block	Block	Block	Block	Block	Block	Block	Total
	A	В	C	D	E	F	G	
Case 1						5.		
Risk Level 1	0.033	0.307	0.111	1.221	6.284	15.528	24.879	48.363
Risk Level 2	0.047	0.334	0.143	1.330	6.867	16.431	27.064	52.216
Risk Level 3	0.051	0.341	0.151	1.356	7.009	16.608	27.544	53.060
		e e e e e e e e e e e e				*		
Case 2	1 -		:					
Risk Level 1	0.033	0.307	0.111	1.221	6.284	2.475	20.110	30.541
Risk Level 2	0.047	0.334	0.143	1.330	6.867	2.639	21.914	33.270
Risk Level 3	0.051	0.341	0.151	1.356	7.009	2.676	22.340	33.924
Case 3								
Risk Level l		_	terior.	~	~	2.475		2.475
Risk Level 2	-	-		-		2.639	<u>-</u>	2.639
Risk Level 3	-			-		2.676		2.676
						1. 1		

By the same manner as the sub-section 5.3.3, the construction cost is converted into annual equivalent cost. The construction periods are assumed to be 7 years for Cases "1" and "2" and 3 years for Case "3" respectively.

(¥ 10⁶)

•					,,
Description	Construction Cost	Annual Equivalent Cost	OM Cost	Annual Benefit	Annual Net Benefit
Case 1					
Risk Level l	1,220.1	99.7	5.0	48.4	-56.3
Risk Level 2	1,403.3	114.6	5.7	52.2	-68.1
Risk Level 3	1,457.0	119.0	6.0	53.1	-71.9
Case 2					
Risk Level l	1,060.0	86.6	4.3	30.5	-60.4
Risk Level 2	1,136.4	92.8	4.6	33.3	-64.1
Risk Level 3	1,190.7	97.3	4.9	33.9	-68.3
Case 3		4			
Risk Level 1	127.1	10.4	0.6	2.5	-8.5
Risk Level 2	131.5	10.7	0.6	2.6	-8.7
Risk Level 3	139.3	11.4	0.6	2.7	-9.3

As shown in the above, the river improvement works are not economically attractive for all the cases studied. However, it is recommendable to execute the flood protective work in some local areas, where flood hazards are serious. It would also be recommended to conduct systematic flood damage statistical survey so that economic and financial losses in the basin will be realistically clarified.

REFERENCES

- 1. EAST COAST WATER RESOURCES DEVELOPMENT PROJECT; JICA, MAR. 1982.
- 2. WATER RESOURCES DEVELOPMENT IN THAILAND COMPLETED TO THE END OF 1979 AND UNDER CONSTRUCTION IN 1980; PROGRAM COORDINATION AND BUDGET DIVISION, RID, DEC. 1980.
- IRRIGATION PFOJECT MAPS IN EASTERN AREA; DESIGN DIVISION, RID, APR. 1976.
- 4. MANUAL FOR RIVER AND SABO WORKS IN JAPAN; INTERNATION ENGINEERING CONSULTANTS ASSOCIATION, JAPAN, 1977.



TABLES

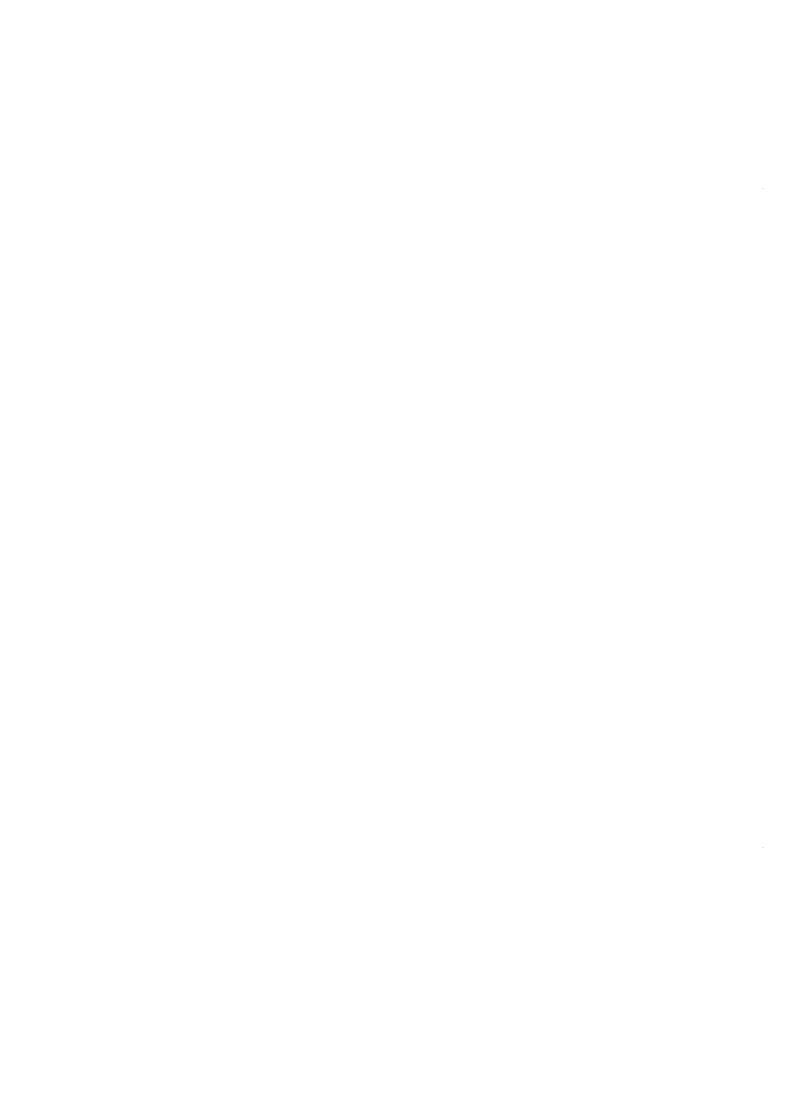


Table 1 OVERALL PROFILES OF KHLONG LUANG RIVER (1/3)

Accumulated Distance (km)	Elevation (El. m)	Remarks
(Kill)	(nr m)	
Khlong Luang R	iver	
0	-2.0	Confluence of Bang Pakong River
3.0	-1.0	Tide gate (End of drainage canal)
7.0	-0.5	Confluence of Huai Nong Takha River
10.5	*	Head of drainage canal
10.5	0.0	Confluence of Khlong Soet River
15.6		Confluence of Khlong Sala Daeng River
18.8	-	Bridge of Route No.315
29.2	10.0	Bridge
33.0	11.5	Confluence of Huai Yang River
41.3	-	Bridge of Route No.331
52.9	20.0	
55.0	-	Khlong Luang potential damsite
56.5	30.0	
61.0	34.0	Confluence of Khlong Wang Rakam River
66.8	40.0	
69.9	42.5	Confluence of Khlong Bung Duan River
77.5	50.0	
80.1	60.0	
85.5	70.0	
87.0	80.0	
87.6	90.0	
88.9	100.0	
89.5	120.0	
89.8	140.0	
Huai Nong Takha	a River	
0	-0.5	Confluence of drainage canal
6.5	9.8	Bridge of Route No.315
6.6	10.0	
10.1	20.0	
11.7	30.0	
14.0	40.0	
15.8	50.0	n
17.5	59.0	Bridge of Route No.3133
17.7	60.0	
18.8	70.0	
20.5	80.0 90.0	
21.7 23.0	100.0	
23.4	110.0	
23.5	120.0	
23.9	140.0	
24.0	160.0	
24.1	180.0	
	20010	

Table 1 OVERALL PROFILES OF KHLONG LUANG RIVER (2/3)

Accumulated Distance	Elevation	Remarks
(km)	(E1. m)	
24.2	200.0	
24.7	300.0	
25.0	400.0	
25.2	500.0	
25.5	600.0	•
Khlong Soet R	i <u>ver</u>	
0	0.0	Confluence of Khlong Nam Khem river
8.0	-	Bridge of Route No. 3133
11.5	10.0	bitage of Route Hotoros
13.5	20.0	
18.3	29.4	
	23.	
Huai Yang Riv	<i>i</i> er	
		Conclusion of White trans Disease
0	11.5	Confluence of Khlong Luang River
8.2	20.0	
14.5	30.0	
21.0	40.0	Dest 1- a
23.5	-	Bridge
24.5 27.3	50.0 60.0	
29.6	70.0	
30.0	70.0	Ban Bung Dam
32.5	80.0	
34.5	90.0	
36.0	100.0	
37.0	110 0	
38.6	120.0	
39.5	130.0	
39.8	140.0	
Khlong Wang R	iver	
0	34.0	Confluence of Khlong Luang River
5.2	40.0	, , , , , , , , , , , , , , , , , , ,
7.1	50.0	
10.9	60.0	
14.5	70.0	
16.0	80.0	
17.8	90.0	
19.0	100.0	
20.0	110.0	
	=,	
21.0	120.0	

Table 1 OVERALL PROFILES OF KHLONG LUANG RIVER (3/3)

Accumulated Distance (km)	Elevation (E1. m)	Remarks
Khlong Bung Du	an River	
Killong bung bu	att Kracr	
_		
0	42.5	Confluence of Khlong Luang River
0 4.3	42.5 50.0	Confluence of Khlong Luang River
0 4.3 6.7		Confluence of Khlong Luang River
· · ·	50.0	Confluence of Khlong Luang River
6.7	50.0 60.0	Confluence of Khlong Luang River

Table 2 FEATURE OF KHLONG LUANG RIVER

Section	Accumulated	River-bed	Ground	Ground Height		
No.	distance (km)	elevation (m)	Left (m)	Right (m)		
L - 31	0.00	0.03	4.44	5.68		
L - 30	1.08	1.14	4.86	4.96		
L - 29	2.68	1.39	5.38	5,43		
L - 28	4.08	2.43	5.20	5.53		
L - 27	5.48	2.61	5.62	5.96		
L - 26	7.18	1.99	5.27	5.52		
L - 25	8.23	1.43	5.66	5.55		
L - 24	9.38	1.57	5.49	5.70		
L-23	10.43	2.74	5.83	5.53		
L - 22	11.48	2.94	5.69	5.64		
L - 21	14.48	1.83	5.49	5.78		
L - 20	16.08	3.60	6.07	5.77		
L - 19	17.98	4.20	6.08	6.10		
L - 18	19.75	3.94	6.06	6.32		
ь - 17	21.38	4.13	7.96	7.39		
L - 16	23.48	4.10	7.32	7. 46		
L - 15	24.68	3.37	7.98	7.46 7.45		
L - 14	26.38	5.86	9.96	9.49		
L - 13	27.98	8.00	11.84	11.53		
L - 12						
	32,33	12.51	15.77	15.71		
L - 11 L - 10	33.66 35.08	14.35 14.95	17.35 18.24	17.45 18.10		
. *			4.			
T - 3	36.08	15.07	18.51	19.04		
L - 8	37.33	15.70	19.88	20.23		
L ~ 7	38.78	17.71	20.89	19.76		
L - 6	39.58	17.77	21.74	22.35		
L - 5	40.93	20.26	24.14	23.53		
L - 4	41.93	20.40	24.78	24.55		
L - 3	42.98	20.95	25.51	25.70		
L - 2	43.41	20.59	26.07	25.64		
L - 1	44.53	22.71	25.69	27.53		
L - 0	46.08	24.50	29.11	28.46		

Table 3 OVERALL LONGITUDINAL PROFILES
OF RAYONG RIVER (1/2)

Distance (km)	Elevation (El. m)		Remarks
Rayong River			
0.0	-4.83		Potname
2.1	-4.83 -1.93	:	Estuary Han Pak Nam bridge
3.7	-0.96	٠.	ban rak wam bildge
5.2	-0.93		
6.4	-1.02		
9.0	-0.88		, ·
10.8	~1.68	:	Bridge in Rayong city
11.2	-0.48	:	Downstream of 1st diversion channel
11.4	_	:	Confluence of Khlong Kha river
11.7	·	:	Highway Route-No. 3 bridge
11.9	-0.84		
13.4		:	Confluence of Khlong Thap Ma river
14.2	-0.18		
15.1	-	:	Wat Sa Pathum bridge
16.2	1.00		·
16.6	-	ŧ	Ban Don bridge
19.4	2.12	:	Ban Kao
20.4	2.73		
23.1	3.72		
23.7	-	:	Wooden bridge
24.2	-	ŧ	Confluence of Khlong Yai Lam
25.1	4.26		
27.1	5.00	:	Route-No. 3138 by-pass bridge
27.5		:	Wat Phai Lom bridge
29.1	6.20	.	Upstream of Khlong Nam Ngu
31.0	6.29	÷	Bridge
31.7 32.4	* 14		Confluence of Khlong Nam Yen Ban Khai weir
32.6	7.41		Wooden bridge
34.0	9.10	•	wooden bridge
34.3	5.10	:	Confluence of Khlong Bang Kradan
35.7	8.98	٠	Confidence of Autong bang Aradan
37.4	10.88	:	Bridge
40.2	16.00	•	wa wega
40.8	_	:	Confluence of Khlong Yai river
Nong Pla Lai Ri	iver		
42.3	14.28		
	- **	;	Confluence of Khlong Dok Krai river
42.4			Confidence of Ritory Dox Real fiver
43.0	15.64		Continence of Killing Dok Kill Tivel
43.0 45.1	21.00		CONTINUED OF RELEGIES
43.0 45.1 46.7	21.00 22.44		
43.0 45.1 46.7 47.7	21.00 22.44 22.18		Ban Nong Mapring gauging station
43.0 45.1 46.7 47.7 47.8	21.00 22.44 22.18	•	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng
43.0 45.1 46.7 47.7 47.8 48.8	21.00 22.44 22.18 26.16		Ban Nong Mapring gauging station
43.0 45.1 46.7 47.7 47.8 48.8 56.5	21.00 22.44 22.18 26.16 40.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5	21.00 22.44 22.18 26.16 40.00	•	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0	21.00 22.44 22.18 26.16 40.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5	21.00 22.44 22.18 	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5	21.00 22.44 22.18 26.16 40.00 50.00 70.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river
43.0 45.1 46.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river
43.0 45.1 46.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5 79.0	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river
43.0 45.1 46.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5 79.0 81.0	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river
43.0 45.1 46.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5 79.0 81.0 83.5	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00 100.00 110.00 120.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5 79.0 81.0 83.5 85.5	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00 100.00 110.00 120.00 140.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river
43.0 45.1 46.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5 79.0 81.0 83.5 85.5 87.5	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00 100.00 110.00 120.00 140.00 160.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river Route-No. 331 bridge
43.0 45.1 46.7 47.7 47.8 48.8 56.5 58.5 62.0 64.5 69.5 70.5 73.0 77.0 77.5 79.0 81.0 83.5 85.5	21.00 22.44 22.18 26.16 40.00 50.00 60.00 70.00 80.00 90.00 100.00 110.00 120.00 140.00	:	Ban Nong Mapring gauging station Confluence of Khlong Rawoeng Proposed Nong Pla Lai dam axis Confluence of Khlong Pluak Daeng river Confluence of Huai Prap river Route-No. 331 bridge

Table 3 OVERALL LONGITUDINAL PROFILES
OF RAYONG RIVER (2/2)

Accumulated Distance (km)	Elevation (El. m)		Remarks
Khlong Yai Riv	er		:
0.0	- , *	:	Confluence of Rayong river
2.0	20		
4.0	<u> </u>	:	Bridge
5.0	-	:	Confluence of Khlong Luai river
6.0	-	:	Bridge
9.5	-	:	Confluence of Khlong Ma Mui river
10.0	30		
10.5	-	:	Ban Pak Phraek gauging station
13.5		:	Khlong Yai potential damsite
15.0	40		and the second second
21.0	50	1	Ban Khlong Nam Dam (1)
25.0	60	2	Confluence of Khlong Map Khai Nao river
30.0	70		
31.5	80		
34.5	90		
35.0	100		and the second s
35.5	179	:	Mt. Khao Liang Khwai
Khlong Dok Kra	i River		
0.0			Confluence of Nong Pla Lai river
3.0	30	:	
8.5	=		Dok Krai Reservoir
· ·	_		
15.0	40	:	Confluence of Khlong Phlu river
16.0 23.0	60		
28.0	70		
31.0	80		
31.5	90		
36.5	100		* *** *
39.0	110	_	Bouts-No. 221 bridge
39.5	120	:	Route-No. 331 bridge
40.5	130		!
41.0	140		σ_{ij}
41.5	180		•
42.0	306		Mt. Khao Chak Kluai
42.0	300	•	HE. MIND CHAR KINAL
Khlong Thap Ma	River		
0.0	-	:	Confluence of Rayong river
0.7	, -	:	Bridge
2.5	~	:	Route-No. 3138 bridge
9.0	10		
10.5	-	1	Khlong Thap Ma potential damsite
11.5		•	
12.0	<u></u>	:	Confluence of Khlong Chang Tai river
12.5	20		
14.0	30		•
18.5	4u		
21.0	50 60		
22.5	60		
23.0	70		
29.0	80		
32.0	100		•
33.0	120		
33.5	140		

Table 4 FEATURE OF RAYONG RIVER

Section	Accumulated	River-bed	Ground	Ground Height		
No.	distance (km)	elevation (m)	Left (m)	Right (m)		
		 				
0	0.0	- 4.83	2.00	2.60		
2	2.1	- 1.93	2.70	1.40		
3	3.7	-0.96	1.30	2.00		
4	5.2	-0.93	1.00	1.02		
5	6.4	- 1.02	0.85	1.00		
6	9.0	-0.88	1.01	1.21		
7	10.8	-1.68	1.90	2.46		
8	11.2	-0.48	2,50	1.90		
9	11.9	-0.84	2.82	1.60		
10	14.2	-0.18	2.39	2.52		
. 11	16.2	1.00	3,13	3.14		
12	19.4	2.12	4.73	4.81		
13	20.4	2.73	5.04	5.12		
14	23.1	3.72	5.81	6.35		
15	25.1	4.26	7.40	7.70		
16	27.1	5.00	8.46	9,13		
17	29.1	6.20	10.22	9.97		
18	31.0	6.29	10.93	11.07		
- 19	32.6	7.41	12.86	12.84		
20	34.0	9.10	11.47	13.20		
21	35.7	8.98	14.32	13.49		
22	37.4	10.88	15.80	14.69		
23	40.2	16.00	20.36	20.48		
24	42.3	14.28	17.11	18.31		
25	43.0	15.64	20.02	19.42		
-26	45.1	21.00	24.79	24.70		
27	46.7	22.44	24.20	25.25		
28	47.7	22.18	25,62	25.86		

Table 5 FEATURE OF KHLONG YAT AND KHLONG THAP MA RIVERS

Section	Accumulated	River-bed	Ground	Ground Height		
No.	distance (km)	elevation (m)	Left (m)	Right (m)		
Khlong Yai	river					
Y - 12	0.00	13.49	17.21	16,81		
Y - 11	1.45	15.58	19.63	19.49		
Y - 10	2.60	16.11	20.54	21.12		
Y 9	3,85	17.77	21.04	19.68		
Y ~ 8	5.05	19.01	22.04	22.33		
Y - 7	6.40	20.18	22.90	24.45		
Y - 6	7.40	21.59	23.80	24.29		
Y - 5	9.00	23.32	26.05	25.24		
y - 4	10.45	25.83	28.82	28.81		
Y - 3	11.85	29.76	32.27	31.99		
Y - 2	13.10	30.23	33.82	33.73		
Y - 1	14.45	33.82	35.01	34.50		
		•				
Khlong Tha	p Ma river					
т - 12	0.00	-0.79	1.76	1.93		
T - 11	0.70	-0.72	1.64	1.22		
T - 10	1.75	- 0.35	1.92	1.15		
т - 9	2.70	0.06	2.73	1.45		
T - 8	3.50	0.35	1.80	1.63		
T - 7	4.45	1.29	2.48	2.69		
r - 6	5.20	1.72	3.61	3.86		
T - 5	6.85	2.88	4.91	4.03		
т - 4	7.75	3,39	5.68	5.07		
T - 3	8 .4 5	4.00	5.69	5.56		
T - 2	9.40	6.48	7.94	8.01		
T - 1	10.45	7.12	9.63	9.01		

Table 6 ANNUAL MAXIMUM DAILY
DISCHARGE AT BAN MAI STREAM GAUGE

Year	Date	Water Level (El. m)	Discharge (m³/sec)
1965	Oct. 10	23.70	13.9
1966	Sept. 19	25.24	25.6
1967	Oct. 3	25.45	39.0
1968	Aug. 6	25.12	25.8
1969	Sept. 22	25.83	105.0
1970	Jun. 29	25.20	34.0
1971	Oct. 12	25.79	95.0
1972	Sept. 9	25.54	52.2
1973	Sept. 19	25.67	71.0
1974	Oct. 19	26.56	350.0
1975	Oct. 4	25.67	71.0
1976	Sept. 15	25.54	47.8
1977	Sept. 2	24.45	16.7
1978	Sept. 19	25.72	81.0
1979	Oct. 3	25.11	25.2
1980	Sept. 24	25.58	53.9
1981	Sept. 23	25.68	73.0

Table 7 ANNUAL MAXIMUM DAILY
DISCHARGE AT BAN KHAI WEIR

		•		Discharge	(m³/sec)
Year	Date		Water Level (El. m)	Without Dok Krai dam	With Dok Krai dam
1967	Oct.	9	9.64	101.4	-
1968	Sept.	29	9.55	88.6	-er
1969	Sept.	22	9.75	109.7	- '
1970	Jun.	11	9.41	60.6	
1971	Oct.	13	9.64	94.6	
1972	Oct.	7	9.74	109.7	-
1973	Oct.	8	10.05	121.6	- -
1974	Oct.	20	10.72	156.2	
1975	Oct.	4	10.05	131.2	99.5
1976	Sept.	6	11.06	135.6	131.7
1977	Oct.	19	9.76	90.7	64.6
1978	May	15	10.22	145.3	109.2
1979	Oct.	4	9.80	68.3	62.2
1980	Oct.	24	10.44	135.0	133.8
1981	Sept.	23	10.18	121.2	125.7

Table 8 INUNDATION DEPTH AND DURATION OF PAST FLOOD
IN THE KHLONG LUANG RIVER BASIN
BY INTERVIEW SURVEY

/*				1974 Oct.	Flood	1981 Sep.		
Point/1 No.	River	District (Amphoe)	Village (Tambon)	Inundation Depth (m)	Duration (day)	Inundation Depth (m)	Duration (day)	Remarks
Khlong	Luang River Bas	sin	÷	٠			٠	
6	Khlong Luang	Phanat Nikhom	Tha Bun Mi	1.5	4		_	no flood in 1981
7	- do -	- do -	Ko Chan	2.0	2	Ī	 	no flood in 1981
8	- do -	- do -	- do -	1.9	3	0.6	0.5	110 T10001 T11 1501
1	- do -	- do -	Tha Bun Mi	1.0	0.5	-	_	no flood in 1981
3	- do -	- do -	- do -	2.0	3			no flood in 1981
	Khlong Kabo	do	- do -	0.3	2 hr.	-	_	no flood in 1981
	Khlong Luang	- do -	- do -	1.5	1	1.0	1	11000 111 1701
	Huai Krasang	- do - ·	- do -	3.0	2	1.0	1	*
	Khlong Luang	do	Na Roed	_	434	-	-	no flood
26	Huai Na Wang Hin	- do -	Nong Hiang	0.4	4	•	-	no flood in 1981
24	Khlong Luang	- do -	Wat Luang	1.4	7	0.5	7	
	Khlong Tan Dam	Ban Bung	Nong Irun	1.0	3	0.5	1	ut.
17	Huai Yang	Phanat Nikhom	Mon Nang	0.9	3	0.2	3	
56	Khlong Yai	Ban Bung	Nong Irun	1.5	. 2	1.0	2	te e a
	Huai Nong` Suang	Phanat Nikhom	Kut Ngong	1.1	3	en j <u>⇔</u> la job Lain	~	no flood in 1981
	Khlong Sala Daeng	- do -	Wat Bot	1.7	7	0.8	7	•
	Khlong Nam Khem	- do -	Phan Thong	2.0	60	.0.5	20	
	Khlong Phan Thong	- do -	Ban Nang	1.4	20	0.3	10	
27	Khlong Thanon	- do	Hua Thanon	(0.9)	(3)	0.5	4	1976 flood was the maximum in the past
21	Khlong Hua Sakae	- do -	Na Pradu	1.5	15	0.6	10	
55 🧬	Huai Sup	Ban Bung	Nong Bon	1.2	2	_	-	no flood in 1981
	Huai Mong Makhua	- do	Ban Bung	(8.0)	(3 hr.)	-	· · ·	no flood in 1981
57	Khlong Soet	Phanat Nikhom	Nong Khayai	(8.0)	(6)	~	-	no flood in 1981
22	- do -	Phan Thong	Map Pong	1.0	. 3	1.0	3 7.1	:
9	Huai Yang	Ban Bung	-	-	-	-	-	no flood
	Huai Nong Takha	- do -	Nong Samsak	(0.5)	(3)	<u></u>	-	no flo∞d in 1981
15	- do -	Phan Thong	Nong Hong	0.6	3	•	-	no flood in 1981
16	- do -	- do -	Nong Kakha	(0.3)	(1)		-	1976 flood was the maximum in the past
58	-	A.M. Chon Buri	Samnak Bok	(0.4)	(-1)	-	-	no flood in 1981
14	-	- đo -	Dan Hua Lo	0.8	2	0.5	2	
	Phan Thong Drainage Canal	Phan Thong	Bang Nang	(1.4)	(20)	-	-	no flood in 1981

 $[\]frac{1}{1}$: Refers to Fig. 15

Note: Figures in parentheses are estimated value.

Table 9 INUNDATED AREA FOR 1974 - FLOOD IN KHLONG LUANG RIVER BASIN

	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				(unit : ha)
Depth (m)	Paddy field	Orchard & plantation	Village area	Other lands	Total
0 to 0.5	8,490	1,790	110	3,160	13,550
0.5 to 1.0	14,270	260	1,140	6,830	22,500
1.0 to 1.5	13,650	80	320	3,390	17,440
1.5 to 2.0	3,500	·	150	960	4,610
above 2.0	180	•••	Park	40	220
Total	40,090	2,130	1,720	14,380	58,320

Inundated Duration : Paddy field

				100		(unit : ha)
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total
0 to 0.5	4,410	4,080	, il <u></u>	· •	.	8,490
0.5 to 1.0		10,920	540	1,780	1,030	14,270
1.0 to 1.5	-	7,120	2,960		3,570	13,650
1.5 to 2.0	-		680	1,020	1,800	3,500
above 2.0	· -	-		. -	180	180
Total	4,410	22,120	4,180	2,800	6,580	40,090

Inundated Duration : Uplands

a Seku As						(unit : ha)
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total
0 to 0.5	1,240	550	-	-	· · · · · · · · · · · · · · · · · · ·	1,790
0.5 to 1.0		260	·	• **		260
1.0 to 1.5		. 80		. 44	-	80
1.5 to 2.0	-	au.'				-
above 2.0	-	ands.			•••	- .
Total	1,240	890		<u>.</u> €		2,130

Table 10 INUNDATED AREA FOR 1981 - FLOOD IN KHLONG LUANG RIVER BASIN

		·		(ur	(unit : ha)		
Depth (m)	Paddy field	Orchard & plantation	Village area	Other lands	Total		
0 to 0.5	11,980	1,540	550	2,650	16,720		
0.5 to 1.0	15,780	110	930	7,430	24,250		
1.0 to 1.5	2,670	249	40	750	3,460		
1.5 to 2.0	***						
above 2.0	==	-	-				
Total	30,430	1,650	1,520	10,830	44,430		

Inundated Duration: Paddy field

					it : ha)	
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total
0 to 0.5	7,310	2,910	1,760			11,980
0.5 to 1.0		7,510	3,770	2,610	1,890	15,780
1.0 to 1.5	•	-	-	580	2,090	2,670
1.5 to 2.0	· <u>-</u>	•••	-	· •	-	<u></u>
above 2.0	· -	_	· · · · · · · · · · · · · · · · · · ·	-	, -	<u></u>
Total	7,310	10,420	5,530	3,190	3,980	30,430

Inundated Duration : Uplands

		4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4			(un i	t : ha)
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total
0 to 0.5	1,370	170		. .	-	1,540
0.5 to 1.0	<u>-</u>	90	20		_	110
1.0 to 1.5	· -	-	_	. -	-	
1.5 to 2.0	-	-			- .	· ·
above 2.0	-	, 	***	-	. ·	
Total	1,370	260	20	_		1,650

Table 11 INUNDATION DEPTH AND DURATION OF PAST FLOOD

IN THE RAYONG RIVER BASIN

BY INTERVIEW SURVEY

oint/1	.:/1 .:/1		1974 District Village Inundati			1981 Sep.		-	
No.	River	(Amphoe)	(Tambon)	Depth (m)	(day)	Depth (m)	(day)	Remarks	
Rayong	River Basin		***************************************						
28	Nong Pla Lai	Pluak Daeng	Mae Nam Khu		~		_	no flood since 1922	
29	- do -	- do -	- do -	- 		-	. =	near the Ban Nong Mapring gauge station	
51	Khlong Yai	- do -	Lahan	(0.5)	(3)	-	-		
52	- do -	Ban Khai	Nong Bua	(2.0)	(5)	0.5	5		
53	- do -	- do -	- do -	(1.0)	(4)	0.5	4		
54	Rayong	- do	- do -	1.0	6	1.0	3	1976 flood was the maxi- mum in the past	
50	- do -	- do -	- do -	0.6	7	0.5	2		
41	- do -	- do -	Nong Lalok	1.5	3	1.0	3	4	
49	Khlong Khao Suan	- do -	Nong Bua	(0.5)	(15)	0.5	- 5		
40	Rayong	- do -	- do -	1.0	15	0.9	3		
42	- do -	- do -	Nong Lalok	-	-	_	-	no flood since 1975	
47	Khlong Nam Ngu	Ban Khai	Bang But	-		<u>-</u>	· -	no flood	
48	- do -	- do -	→ do <i>-</i>	(0.6)	(3)	0.5	3	* .	
39	Rayong	- do -	Nong Lalok	0.8	5	0.7	5		
37	- do -	- do -	Ban Khai	2.0	. 3	1.0	3	en e	
38	- do -	- do -	Nong Lalok	0.7	5	0.6	5		
46	Khlong Yai Lam	- do	Chak Bok	0.7	7	0.6	2	1976 flood was the maxi- in the past	
45	- do -	- do -	- do -	0.9	5 -	-		no flood in 1981	
36	Rayong	~ do -	Ban Khai	1.1	3	1.0	3		
35	Khlong Kamaeng	~ do ~	Nong Saphan	(1.0)	(7)	0.8	. 3		
34	Khlong Lalok	- do -	Ta Khan	1.0	5	0.3	3		
44	Khlong Kha	A.M. Rayong	Choeng Noen	(1.1)	(7)	1.1	7.,		
30	Khlong Thap Ma	~ do -	Thap Ma	(0.3)	(1)	: 	.	no flood in 1981	
31	Khlong Map Kabak	- do -	Noen Phra	(0.3)	(1)			no flood in 1981	
33	Khlong Thap Ma	A.M. Rayong	Thap Ma	1.5	3. 	0.6	3		
32	- do -	- do -	Choeng Noen	1.5	7	1.0	7		
43	Rayong	- do -	- do -	2.0	30	0.5	20		

^{/1}: Refers to Fig. 16

Note: Figures in parentheses are estimated value.

Table 12 INUNDATED AREA FOR 1974 - FLOOD IN RAYONG RIVER BASIN

				(unit: ha)		
Depth (m)	Paddy field	Orchard & plantation	Village area	Other lands	Total	
0 to 0.5	3,710	3,430	1,080	1,940	10,160	
0.5 to 1.0	4,200	510	370	1,910	6,990	
1.0 to 1.5	2,530	20	190	200	2,940	
1.5 to 2.0	900	· <u>-</u>		-	900	
above 2.0		. 		-	-	
Total	11,340	3,960	1,640	4,050	20,990	

Inundated Duration : Paddy field

	·	<u> </u>		(unit : ha)			
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total	
0 to 0.5	2,630	1,080	_ :		_	3,710	
0.5 to 1.0	-	1,930	2,020	250	-	4,200	
1.0 to 1.5	 ,*		1,240	1,120	170	2,530	
1.5 to 2.0	**	-		180	720	900	
above 2.0	***	-	-	um.		-	
Total	2,630	3,010	3,260	1,550	890	11,340	

Inundated Duration: Uplands

					(unit : ha)	
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total
0 to 0.5	1,970	1,430	30		-	3,430
0.5 to 1.0		90	270	130	20	510
1.0 to 1.5			-	10	10	20
1.5 to 2.0	. 704		· -	-	_	_
above 2.0				-	-	****
Total	1,970	1,520	300	140	30	3,960

Table 13 INUNDATED AREA FOR 1981 - FLOOD IN RAYONG RIVER BASIN

					(unit : ha)
Depth (m)	Paddy field	Orchard & plantation	Village area	Other lands	Total
0 to 0.5	3,680	1,240	720	1,510	7,150
0.5 to 1.0	4,830	490	470	1,610	7,400
1.0 to 1.5	1,860	10	40	510	2,420
1.5 to 2.0	· .	· •	. 	•••	-
above 2.0			· 		
Total	10,370	1,740	1,230	3,630	16,970

Inundated Duration : Paddy field

18				(unit : ha)			
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total	
0 to 0.5	3,120	560			-	3,680	
0.5 to 1.0	· . P <u></u>	3,210	1,230	380	10	4,830	
1.0 to 1.5	er va	•••	1,260	370	230	1,860	
1.5 to 2.0	~	; 	-	-		***	
above 2.0	. ·	400		-	-	_	
Total	3,120	3,770	2,490	750	240	10,370	

Inundated Duration: Uplands

					(unit : ha)	
Duration	0 to 1	1 to 4	4 to 7	7 to 10	above 10	Total
0 to 0.5	780	30	_	- :	-	810
0.5 to 1.0	370	360	10	<u>-</u>		740
1.0 to 1.5	20		-		-	20
1.5 to 2.0	20	-	b==		; '	20
above 2.0	50	100	·	-	-	150
Total	1,240	490	. 10		-	1,740

Table 14 FLOOD DAMAGE OF PADDY FOR 1974 - FLOOD IN KHLONG LUANG RIVER BASIN

วามเกิดคำเก	Toundation Condition	Tumdation	Thit bride	Damadeable	Damage	Damade	
Duration (day)	Depth (m)	Area (ha)	(Æ/ha)	Value (# 103)	Ratio	Amount (\$ 103)	
to 1	0 to 0.5	4,410	13,760	60,700	0.08	4,900	
	0.5 to 1.0		ರ್	1	1		
	1.0 to 1.5	1	đo	•	í	T.	.*
	Sub, total	4,410		60,700		4,900	
to 4	0 to 0.5	4,080	13,760	56,100	0.13	7,300	
	40	10,920	ဗ္ဗ	150,300	0.16	24,000	
	1.0 to 1.5	7,120	ශ්ර	98,000	0.46	45,100	
	Sub total	22,120		304,400		76,400	
to 7	0 to 0.5		13,760	ı.	ï		
÷	ပ္ပ	540	တ္မ	7,400	0.23	1,700	
	q	2,960	ರಂ	40,700	0.23	9,400	
		680	độ	9,400	0.65	6,100	
	Sub total	4,180		57,500		17,200	
to 10	0 to 0.5	ì	13,760	1	ı	t	
	0.5 to 1.0	1,780	ф	24,500	0.24	2,900	
	ţ,	•	do	1		1	
	1.5 to 2.0	1,020	do do	14,000	0.66	9,200	
	Sub total	2,800		38,500		15,100	
above 10	0 to 0.5	I	13,760	1	1	ł	
-	0.5 to 1.0	1,030	භි	14,200	0.66	9,400	
	1.0 to 1.5	3,570	do .	49,100	99.0	32,400	
	t G	1,800	go	24,800	0.66	16,400	
	above 2.0	180	op G	2,500	99.0	1,700	
	Sub total	6,580		009'06		29,900	
Total	T e	40,090		551,700		173,500	

able 15 FLOOD DAMAGE OF CASSAVA FOR 1974 - FLOOD IN KHLONG LUANG RIVER BASIN

Inundation Condition	Condition	Inundation	Unit Price	Damageable	Damage	Damage
Duration (day)	Depth (m)	Area (ha)	(B/ha)	Value (\$ 103)	Ratio	Amount (\$ 103)
0 to 1	0 to 0.5	1,240	7,130	8,800	0.11	1,000
	0.5 to 1.0	1	g _O	t	1	1
	1.0 to 1.5	1	ශ්ර	t	1	1
	Sub total	1,240	\$ ¹	8,800		1,000
1 to 4	0 to 0.5	550	7,130	3,900	0.30	1,200
	0.5 to 1.0	260	တ္မ	1,900	0.40	800
	1.0 to 1.5	80	do	009	0.63	400
	Sub total	068		6,400		2,400
Total	-	2,130		15,200		3,400

Table 16 FLOOD DAMAGE OF HOUSE AND HOUSEHOLD EFFECTS FOR 1974 - FLOOD IN KHLONG LUANG RIVER BASIN

Inundation Depth (m)	Inundation Area (ha)	Nos. House (nos)	Unit Price (B/house)	Damageable Value (B 10 ³)	Damage Ratio	Damage Amount (# 103)
0 to 0.5	110	1,110	39,600	44.,000	0.03	7,300
0.5 to 1.0	1,140	11,510	တို	455,800	0.03	13,700
1.0 to 1.5	320	3,230	වි	127,900	0.05	6,400
1.5 to 2.0	150	1,510	ор	59,800	0.07	4,200
Total	1,720	17,360		687,500		25,600

Table 17 FLOOD DAMAGE OF LIVESTOCKS FOR 1974 - FLOOD IN KHLONG LUANG RIVER BASIN

[PIG]

Inundation Depth (m)	Inundation Area (ha)	Nos. of House (nos)	Unit Price (Æ/house)	Damageable Value (Ø 10 ³)	Damage Ratic	Damage Amount (# 10 ³)
0 to 0.5	110	1,110	5,230	5,800	0.30	1,700
ş	1,140	11,510	မွ	60,200	0.80	48,200
1.0 to 1.5	320	3,230	go	16,900	1,00	16,900
ဌ	150	1,510	do	7,900	do do	7,900
Total	1,720	17,360				74,700
[CHICKEN]						
, c	010	110	1.310	1.200	0.50	900
, r,	1.140	11,510) CC	12,700	00 - [12.700
8	320	3,230	do	3,500	op go	3,500
1.5 to 2.0	150	1,510	do	1,700	do	1,700
Total	1,720	17,360				18,500
[buck]						
0 to 0.5	110	1,110	1,050	1,200	0.30	400
0.5 to 1.0	1,140	11,510	op Op	12,100	06.0	10,900
ţ,	320	3,230	do	3,400	1.00	3,400
1.5 to 2.0	150	1,510	වි	1,600	op Op	1,600
Total	1,720	17,360				16,300
				GRAND TOTAL		109,500

Table 18 FLOOD DAMAGE OF PADDY FOR 1981 - FLOOD IN KHLONG LUANG RIVER BASIN

(£ 103)	0 0	0000	0010	0000		00
Damage Amount (B 10 ³)	8,000	5,200 16,500 21,700	4,100 12,000 _ _ 16,100	8,600 5,300 - 13,900	31,700 4,400 36,100	95,800
Damage Ratio	80 1 1	0.13	00.17	0.02	1 1 9 9 1	-
Damageable Value (Ø 103)	100,600	40,000 103,300 143,300	24,200 52,200 - - 76,400	35,900 8,000 - 43,900	- 48,000 6,700 54,700	418,900
Unit Frice (\$/ha)	13,760 do do	13,760 do do	13,760 do do	13,760 do do do	13,760 do do do	
 inundation Area (ha)	7,310	2,910 7,510 - 10,420	1,760 3,790 	2,610 580 3,190	3,490 490 1,980 1,980	30,450
Inundation Condition Duration Depth (day) (m)	0 to 0.5 0.5 to 1.0 1.0 to 1.5 Sub total	0 to 0.5 0.5 to 1.0 1.0 to 1.5 Sub total	0 to 0.5 0.5 to 1.0 1.0 to 1.5 1.5 to 2.0 Sub total	0 to 0.5 0.5 to 1.0 1.0 to 1.5 1.5 to 2.0 Sub total	0 to 0.5 0.5 to 1.0 1.0 to 1.5 1.5 to 2.0 above 2.0 Sub total	Ę
Inundation Duration (day)	0 to 1	۱ to 4	4 to 7	7 to 10	above 10	Total

Table 19 FLOOD DAMAGE OF CASSAVA FOR 1981 - FLOOD IN KHLONG LUANG RIVER BASIN

Inundation (Duration (dav)	Condition Depth (m)	Inundation Area (ha)	Unit Price (B/ha)	Damageable Value (B 10 ³)	Damage Ratio	Damage Amount $(\beta 10^3)$
0 to 1	0 to 0.5	1,370	7,130	008,6	0.11	1,100
	S	1	භ	1		. I
•	1.0 to 1.5	1.	op ,	1	. 1	
	Sub total	1,370		008'6		1,100
1 to 4	0 to 0,5	170	7,130	1,200	0.30	400
-	2	06	වි	009	0.40	200
- •	1.0 to 1.5	1	် ဝဉ်	1	1	
	Sub total	260		1,800		009
4 to 7	0 to 0,5	ı	7,130	ì	. 1	t
-	1,	20	. පි	100	0.75	100
•	1.0 to 1.5	1	වු	Ĭ	1	
•	1.5 to 2.0		op	t'.	ı	1
	Sub total	20		100		100
Total	ed.	1,650		11,700		008 T
	Et	Table 20 FLOOD DAM	DAMAGE OF HOUSE	FICOD DAMAGE OF HOUSE AND HOUSEHOLD BFFECTS	FECTS	
House	1981	T (40) #	ì	FEOUR IN MILLONG LURING KIVER PROIN	NT CHO	
Inundation Depth (m)	Inundation Area (ha)	Nos. of House (nos)	Unit Price (B/house)	Damageable Value (\$ 103)	Damage Ratio	Damage Amount (# 103)
C C +	R. C.	ν C	39.600	219 800	. 80 0	6.600
0 0	020	9,400	op op	372,200	0.03	11,200
	40	400	တို	15,800	0.05	800
ţ,	. f	•	ф	ľ	ŧ	ı
Total	1,520	15,350		607,800		18,600

Table 21 FLOOD DAMAGE OF LIVESTOCKS FOR 1981 - FLOOD IN KHLONG LUANG RIVER BASIN

[5Id]

			!			
Inundation Depth (m)	Inundation Area (ha)	Nos. of House (nos)	Unit Price (Ø/house)	Damageable Value (\$ 10 ³)	Damage Ratio	Damage Amount (# 10³)
0 to 0.5	550	5,550	5,230	29,000	0.30	8,700
0.5 to 1.0	930	9,400	ච	49,200	0.80	39,400
1.0 to 1.5	40	400	ф	2,100	1.00	2,100
1.5 to 2.0	1	•	op	·t		1
Total	1,520	15,350				50,200
[CHICKEN]						
4	r U	n n c	0,5	000	0	000
3 2	0000	000	O T TO	00%,0	000	3,100
1.0 to 1.5	40	400	g 9	400	0 0 0	400
ដ	1	. 1	do	1	1	1
Total	1,520	15,350				13,900
1, 240, 240, 1						
[DUCK]						
C C C	r L	ι η C	1 CRO	r SOO	08.0	005 6
	02.6	9,400	် တို့ တို့	006,6	0000	000
	40	400	တ္မ	400	1.00	004
ដ		1	ф	ı	ı	1
Total	1,520	15,350				11,000
				GRAND TOTAL	AI.	75,100

Table 22 FLOOD DAMAGE OF PADDY FOR 1974 - FLOOD IN RAYONG RIVER BASIN

O to 1 0 to 0. 1.0 to 1.	Depth			r clairen nicht		
to L	(m)	Area (ha)	(K/ha)	Value (Ø 10 ³)	Ratio	Amount (# 10)
to H	-					
	ដ	2,630	14,090	37,100	80.0	3,000
•	S	ì	go G	1	1	ı
	1.0 to 1.5	1	ල්	ı	1	1.
	ŭ	2,630		37,100		3,000
1 to 4	0 to 0.5	1,080	14,090	15,200	0.13	2,000
,	0.5 to 1.0	1,930	တ္မ	27,200	0.16	4,400
	1.0 to 1.5	1	වි		ı	. 1
	Sub total	3,010		42,400		6,400
4 to 7	0 to 0.5	. 1	14,090	t	I .	t
	0.5 to 1.0	2,020	ę	28,500	0.23	009'9
	1.0 to 1.5	1,240	တို	17,500	0.65	11,400
•	1.5 to 2.0	1	တ္မ	ı		1
	Sub total	3,260		46,000		18,000
7 to 10	0 to 0.5	1	14,090	ı	. 1	i
	5 to 1	250	ģ	3,500	0.24	800
. •	1.0 to 1.5	1,120	မှ	15,800	0.66	10,400
	ın	180	g G	2,500	0.66	1,700
	Sub total	1,550	ji.	21,800		12,900
above 10	0 to 0.5	. 1	14,090		ì	1
	0.5 to 1.0	.:	do		ı	l
	1.0 to 1.5	170	do	2,400	0.66	1,600
	1.5 to 2.0	720	ှင်	10,100	99.0	6,700
	above 2.0	1	တို	ı	ı	1
	Sub total	068		12,500		8,300
Total		11,340		159,800		48,600

Table 23 FLOOD DAMAGE OF CASSAVA FOR 1974 - FLOOD IN RAYONG RIVER BASIN

Inundation	Inundation Condition	Inundation	Unit Price	Damageable 3	Damage	Damage
Duration (day)	Depth (m)	Area (ha)	(B/ha)	value (\$ 10 ⁻)	Ratio	Amount (\$ 10)
ì				,		
0 to 7	0 40 0.5	1,970	14,510	28,600	0.11	3,100
	0.5 to 1.0	f .	đo	1	í	t .
•	1.0 to 1.5	1	වු	I,	1	1
	Sub total	1,970		28,600	٠.	3,100
1 to 4	0 to 0.5	1,430	14,510	20,700	0.30	6,200
	0.5 to 1.0	06	ф	1,300	0.40	200
:	1.0 to 1.5	1	අර	. I	ı	l
	Sub total	1,520		22,000		6,700
4 to 7	0 to 0.5	30	14,510	400	0.50	200
	0.5 to 1.0	270	අර	3,900	0.75	2,900
	1.0 to 1.5	i	တု	ı	1	1
	1.5 to 2.0		đọ	1		I
	Sub total	300 ::		4,300		3,100
7 to 10	0 to 0.5	1	14,510	•	1	i
	0.5 to 1.0	130	do	1,900	0.88	1,700
:	1.0 to 1.5	07	ග්	100	7.00	100
٠	1.5 to 2.0	1	ф	•	1	l
	Sub total	140	14	2,000		1,800
above 10	9	1	14,510	1	1	ı
	0.5 to 1.0	20	qo	300	0.88	300
	r-i	10	đo	100	1.00	100
	ខ្ព	1	do	1	ı	. 1
	above 2.0	. 1	do	1	ı	. 1
	Sub total	30		400		400
Total	al	3,960		57,300		15,100

Table 24 FLOOD DAMAGE OF HOUSE AND HOUSEHOLD EFFECTS FOR 1974 - FLOOD IN RAYONG RIVER BASIN

Inundation Depth (m)	Inundation Area (ha)	Nos. of House (nos)	Unit Price (B/house)	Damageable 3 Value (Ø 10 ³)	Damage Ratio	Damage Amount (\$ 10 ³)
0 to 0.5	1,080	10,910	39,600	432,000	0.03	13,000
0.5 to 1.0	370	3,740	do.	148,100	0.03	4,400
1.0 to 1.5	190	1,920	go	76,000	0.05	3,800
1.5 to 2.0	l		do do	ì	Í	
Total	1,640	16,570		656,100	:	21,200

Table 25 FLOOD DAMAGE OF LIVESTOCKS FOR 1974 - FLOOD IN RAYONG RIVER BASIN

[PIG]						
Inundation Depth (m)	Inundation Area (ha)	Nos. of House (nos)	Unit Price I (B/house) Va	Damageable Value (Ø 10 ³)	Damage Ratio	Damage Amount (\$ 10 ³)
0 to 0.5	1,080	10,910	1,090	11,900	0.30	3,600
t 0	370	3,740	චු	4,100	0.80	3,300
1.0 to 1.5	190	1,920	do do	2,100	1.00	2,100
Q (1	ι	. 1	တူ		ľ	i
Total	1,640	16,570				000,6
[CHICKEN]						
0 to 0.5	1,080	10,910	54.8	009	0.50	300
ţ,	370	3,740	ဝှာ	200	1.00	200
	190	1,920	မှ	100	ဝှု	100
to.	1	1	qo	.1	ı	•
Total	1,640	16,570				009
[Dilok1						
0 to 0.5	1,080	10,910	19.4	200	0.30	100
0.5 to 1.0	370	3,740	go.	100	06.0	100
	190	1,920	ු ම්	0	1.00	0
1.5 to 2.0	ı		do .	j	· •	ŧ
Total	1,640	16,570				200
				GRAND TOTAL		008,6

Table 26 FLOOD DAMAGE OF PADDY FOR 1981 - FLOOD IN RAYONG RIVER BASIN

	3 - 3 - 5			7.0000		
Duration (day)	Inundation Condition Duration Depth (day) (m)	Area (ha)	unit Frice (\$/ha)	value (\$ 10)	Ratio	Damaye 3
0 to 1	얺	3,120	14,090	44,000	80.0	3,500
	το το	1		ı		į
	1.0 to 1.5	0				1.
	Sub total	3,120		44,000		3,500
1 to 4	0 to 0.5	260		7,900	0.13	1,000
÷	0.5 to 1.0	3,210		45,200	0.16	7,200
	1.0 to 1.5	ı		1		ı
	Sub total	3,770		53,100		8,200
4 to 7	0 to 0.5	1				
	0.5 to 1.0	1,230		17,300	0.23	4,000
	1.0 to 1.5	1,260	:	17,800	0.65	11,600
	1.5 to 2.0	1		1		ı
-	Sub total	2,490		35,100		15,600
7 to 10	S	. I				 1
	0.5 to 1.0	380		5,400	0.24	1,300
٠	ů	370		5,200	99.0	3,400
	Q L					i
	Sub total	750		10,600		4,700
above 10	ដ			i		1
-	0.5 to 1.0	10		100	0.66	100
	0 (1	230		3,200	0.66	2,100
	5 to			, 1		ı
	above 2.0	1	:	1		ı
	Sub total	240		3,300		2,200
TOT	Total	10,370		146,100		34,200

Table 27 FLOOD DAMAGE OF CASSAVA FOR 1981 - FLOOD IN RAYONG RIVER BASIN

Inundation Duration (day)	Inundation Condition Duration Depth (day) (m)	Inundation Area (ha)	Unit Price (\$/ha)	Damageable 3 Value (Ø 10 ³)	Damage Ratio	Damage Amount (B 10 ³)
р С	0 to 0.5	780	14,510	11,300	0.11	1,200
	0.5 to 1.0	370	චූ	5,400	0.27	1,500
	1.0 to 1.5	20	တ ိ ့	300	0.38	100
	1.5 to 2.0	20	ဝှင	300	0.38	100
	above 2.0	50	do	200	0.38	300
	Sub total	1,240		18,000		3,200
			•			()
1 to 4	0 to 0.5	30	do	400	0.30	001
	0.5 to 1.0	360	ပ္မွာ	5,200	0.40	2,100
	1.0 to 1.5	ı	රු	1	I	1
	1.5 to 2.0	. 1	ටු	ı	1	ı
	above 2.0	100	đo	1,500	0.63	000
	Sub total	490		7,100		3,100
4 to 7	0 to 0.5		do	1	1	1 .
	0.5 to 1.0	10	do	100	0.75	100
	1.0 to 1.5	ı	do	1	1	I.
	Sub total	10		100		00T
				÷		1
TO	Total	1,740		25,200		6,400
						-

Table 28 FLOOD DAMAGE OF HOUSE AND HOUSEHOLD EFFECTS FOR 1981 - FLOOD IN RAYONG RIVER BASIN

Inundation	Inundation	Nos. of	Unit Price	Damageable 3	Damage	Damage
Depth (m)	Area (ha)	House (nos)	(B/house)	Value (% 10')	Ratio	Amount (\$ 10~)
0 to 0.5	720	7,270	39,600	287,900	0.03	8,600
0.5 to 1.0	470	4,750	තු	188,100	0.03	2,600
1.0 to 1.5	40	400	qo	15,800	0.05	800
1.5 to 2.0	Ī	ı	do Op	l	Í	1
Total	1,230	12,420		491,800		15,000

Table 29 FLOOD DAMAGE OF LIVESTOCKS FOR 1981 - FLOOD IN RAYONG RIVER BASIN

[PIG]						
Inundation Depth (m)	Inundation Area (ha)	Nos. of House (nos)	Unit Price (\$/house)	Damageable Value (岁 10 ³)	Damage Ratio	Damage Amount (Ø 10³)
0 to 0.5	720	7,270	1,090	7,900	0.30	2,400
0.5 to 1.0	470	4,750	දු -	5,200	0.80	4,200
1.0 to 1.5	04	400	ပ္တ	400	00. I	400 1
Total	1,230	12,420	· .			7,000
[CHICKEN]						
0 to 0.5	720	7,270	54.8	400	0.50	200
ន	470	4,750	ф	300	1.00	300
1.0 to 1.5	40	400	qo	0	ĝo	0
요.	3	1	රු	1	1	
Total	1,230	12,420				200
[DUCK]						
0 to 0.5	720	7,270	19.4	100	0,30	0
ន្ឋ	470	4,750	တ္	100	06.0	100
1.0 to 1.5	40	400	တ္မွ	0	1.00	0
1.5 to 2.0		r	do	1	ı	. \$
Total	1,230	12,420				100
				GRAND TOTAL		7,600

Table 30 STRETCH OF INUNDATION BLOCK

Biver		Channel			strettch
				Upstream	Downstream
Khlong Luang		Ø		Khlong Luang damsite	confluence of K. Kabo river
		Ø	Pad	Khlong Kabo river	confluence of Huai Yang river
		υ		Huai Yang river	road No. 315
		Ω		area extends to south	southernward of road No. 315
		ध्य	н	road No. 315	river mouth
Rayong	÷	æ	. 24	Nong Pla Lai damsite	confluence of K. Yai river
		ca	I	Dok Krai damsite	confluence of Nong Pla Lairiver
		Ŋ		Khlong Yai damsite	confluence of Nong Pla Lairiver
		: О	i ∡ i	K. Yai river	Ban Khai weir
		Œ	щ	Ban Khai weir	Khlong Yai Lam river
		ſΣų	i×4	K. Thap Ma damsite	confluence of Rayong river
		ტ	p.e.¢	K. Yai Lam river	Rayong bridge

Table 31 PROBABLE FLOOD RUNOFF OF KHLONG LUANG RIVER UNDER PRESENT CONDITION $^{\prime}$

						-		(Unit : m3/s)	(8)	
Point					Return	period				
No. /2	Location	2-yr	5-yr	10-yr	30-yr	50-yr	100-yr	200-yr	500-yr	
. 1	Inflow of Khlong Luang damsite	567	729	858	1,015	1,101	1,224	1,346	1,462	
rl	Outflow of Khlong Luang damsite	567	729	858	1,015	101,1	1,224	1,346	1,462	
44	Upstream of confluence of Khlong Kabo river	176	210	238	273	292	319	347	381	
7	Upstream of confluence of Huai Yang river	141	166	188	213	227	247	266	290	
. \$	Inflow of Ban Bung damsite	159	202	228	265	283	306	332	365	
ω	Outflow of Ban Bung damsite	1.59	202	228	265	283	306	332	365	
10	Huai Yang river	499	629	729	850	806	986	1,058	1,160	
13	Panat Nikhom	202	250	289	344	361	65 88 87 87	426	452	
8	River mouth	285	361	411	493	520	565	607	80 60	

/1 : With existing Ban Bung dam

/2 : Refer to Fig. 25

Table 32 PROBABLE FLOOD RUNOFF OF KHLONG LUANG RIVER WITH BAN BUNG DAM

(Unit: m^3/s)

Point					Return period	riod			
No. /1	Location	2-yr	2-Yr	10-yr	30-yr	50-yr	100-yr	200-yr	500-yr
ı	Inflow of Khlong Luang damsite	567	729	858	1,015	10τ'τ	1,224	1,346	1,462
н	Outflow of Khlong Luang damsite	567	729	858	1,015	1,101	1,224	1,346	1,462
≂! *	Upstream of confluence of Khlong Kabo river	176	210	238	273	292	319	347	381
7	Upstream of confluence of Hual Yang river	141	166	188	213	227	347	266	290
t	Inflow of Ban Bung damsite	159	202	228	265	283	306	332	365
3 0	Outflow of Ban Bung damsite	더	ម	17	22	23	26	28	31
10	Huai Yang river	464	615	681	795	850	924	ო თ თ	1,085
13	Panat Nikhom	198	245	283	338	354	382	418	444
18	River mouth	279	353	402	483	0. 0. 0.	554	595	645

11: Refer to Fig. 25

Table 33 PROBABLE FLOOD RUNOFF OF KHLONG LUANG RIVER WITH BAN BUNG DAM AND KHLONG LUANG DAM $^{oldsymbol{L}}$

				• .		•	(Unit	(Unit: m^3/s)		•
Point	7 (100)				Return	Period				ŀ
No. 22	170,000,000	2-yr	5-γ κ	10-yr	30-yr	50-Yr	100-yr	200-yr	200-yr	1 -
	Inflow of Khlony Luang damsite	567	729	858	1,015	1,101	1,224	1,346	1,462	ı
-1	Outflow of Khlong Luang damsite	3.7	ហ	୍ଷ	86	9	107	120	139	
4 ,	Upstream of confluence of Khlong Kabo river	62	70	75	\odol	96	109	121	68 T	
	Upstream of confluence of Hual Yang river	80	П б	101	112	118	124	132	142	:
, 1	Inflow of Ban Bung damsite	159	202	228	265	283	306	8 8 8 8	365	
00	Outflow of Ban Bung damsite	77	iS L	17	22	23	56	78	31	
70	Huai Yang river	464	615	681	795	850	924	699	1,085	
13	Panat Nikhom	174	21.5	239	274	295	319	340	370	
а 8	River mouth	278	35.2	400	481	507	552	593	645	
!		The second secon								

21 : With spillway width 70 m

42 : Refer to Fig. 25

PROBABLE FLOOD RUNOFF OF KHLONG LUANG RIVER WITH BAN BUNG DAM, KHLONG LUANG DAM^{4, 2} AND RIVER IMPROVEMENT Table 34

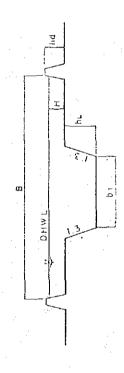
							inu)	(Unit: m^3/s)	
Point					Return	Return Period			
No. 12	LOCA CLON	2-yr.	5-yr	10-yr	30-yr	50-yr	100-yr	200-yr	500-yr
ı	Inflow of Khlong Luang demsite	567	729	858	1,015	1,101	1,224	1,346	1,462
H	Outflow of Khlong Luang damsite	37	55	<u>რ</u> დ	80	ω π	107	120	139
4	Upstream of confluence of Khlong Kabo river	125	161	187	223	237	262 2	297	H E E
7	Upstream of confluence of Huai Yang river	218	277	331	8 8 8	423	467	520	580
1	Inflow of Ban Bung damsite	159	202	228	265	283	306	332	365
; co	Outflow of Ban Bung damsite	11	12	17	22	23	26	28	HE.
10	Huai Yang river	464	615	631	795	850	924	60 60	1,085
13	Panat Nikhom	681	891	1,010	1,101	1,272	1,391	1,510	1,670
ω H	River mouth	681	891	1,010	1,101	1,272	1,391	1,510	1,670

11: With spillway width 70 m

.23 : Refer to Fig. 25

Table 35 CHANNEL FACTOR FOR CHANNEL DESIGN FOR KHLONG LUANG RIVER

Alternative		Stretches	Length (Km)	Design Discharge Q (m³/sec)	Gradient	River Width B (m)	Low Water C. Width bl(m)	Low Water C. Depth hl (m)	Water Depth of High Water C. H (m)	Dike Height nd (m)	
	н	0.0-12.2	11.5	1,300	1/5,000	150	71.7	4.0	9.8	4. 8.	ı
	II	12.2-22.5	89	1,300	1/5,000	150	71.7	2.5	4,	8.0	
	III	22.5-29.5	6.0	1,300	1/1,400	150	71.7	2.2	ເຄ• ຕ	4.5	
) V.	ΣV	29.5-38.0	8.5	430	1/1,100	100	14.6	3.4	8.1	2.6	
	۸	38.0-44.2	6.0	240	1/1,100	80	7.5	4.0	1.1	ų. 9.	
	T.	44.2-47.0	2.5	80	1/1,100	80	1.8	4.2	0	0.4	
	н	0.0-12.2	11.5	1,300	1/5,000	150	71.7	4.0	3.6	4.6	1
	ĭ	12.2-22.5	8.5	1,300	1/5,000	150	71.7	2.5	8-4	5.8	
2	III	22.5-29.5	6.0	1,300	1/1,400	150	71.7	2.2	ນ •	4. N.	
	ΙΛ	29.5-38.0	ω Ω	430	1/1,100	100	14.6	3.4	1.8	2.6	
	>	38.0-44.2	0.9	240	1/1,100	50	7.5	0.4	1.1	٠ و.	
	VI	44.2-47.0	8.5	95	1/1,100	50	2.1	4.2	0.1	0.7	
	Н	0.0-12.2	11.5	1,300	1/5,000	150	71.7	4.0	3.6	4.6	
	Ħ	12.2-22.5	8.5	1,300	1/5,000	150	71.7	2.5	8.4	5.8	
ო	III	22.5-29.5	6.0	1,300	1/1,400	150	71.7	2.2	3.5		
	ΔÏ	29.5-38.0	8.5	430	1/1,100	100	14.6	6. 4.	αο -1	2.6	
	٥	38.0-44.2	0.9	240	1/1,100	20	7.5	4.0	1.1	თ i	
	Ĭ	44.2-47.0	2.5	120	1/1,100	50	2.7	4.	e • 0	6.0	



MORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS, ALTERNATIVE 1, KHLONG LUANG RIVER Table 36

dc2 - 00	Remarks																				<i>y</i>		1,086,600,000
OC7# + ¢T	Amount	× 103		18,670	133,090	51,040	3,564		27,160	4,026	5,750		3,984	1,694	36,400	266,708	യ്	313,916	155,725	469,641	70,446	81,013	621,100 \$ 1,0
۲	Unit Cost				23.3	11.0	7.1		485.0	366.0	230,000	5. ⁷⁷	23.3	11.0	6,500	ē	1	:	347,600	1.1			
1001	Amount	× 103	l	016	6,168	2,691	241		898	121	545		185	89	2,100	13,008	1,391	15,309	1	15,309	2,296	2,635	20,240
1	Unit Cost		•	ı	1.08	0.58	0.48		15.5	11.0	21,800		1.08	0.58	375		ı		I .				
	Quantity		-	ı	5,712,000	4,640,000	502,000		56,000	11,000	25		171,000	154,000	5,600		i		448		× E S S S S		
	Unit		:	, S	E	е ш	щ3		m ²	m ₂	nos.		۳. ظ	E	m ²		ក. ស		ha		n (A+B)		
	Item		A. Main clvil works	I. Preparation (9. \times 7%)	2. Excavation	3. Embankment	4. Disposal	5. Revetment	- Low water channel	- High water channel	6. Drainage sluice	7. Backwater levee	- Excavation	- Embankment	8. Bridge	9. Sub-total (2 ∿ 8)	10. Miscellaneous (1+9) \times 10%	Total	B. Acquisition and Compensation	ъ. + в.	C. Eng. service and Administration	D. Contingency (A+B+C) × 15%	Grand Total

Note: Excluding dam cost

WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS, ALTERNATIVE 2, KHLONG LUANG RIVER Table 37

						# \$T	¥250 = 23B
Item	Unit	Quantity	Unit Cost Am	US\$) Amount	Unit Cost	(B) Amount	Remarks
				er e		,	
A. Main civil works) OT X		201 x	-
1. Preparation $(9. \times 78)$	Ľ.S	ı	î	911		18,682	
2. Excavation	ខ្ព	5,718,000	1.08	6,175	23.3	133,229	
3. Embankment	e E	4,644,000	0.58	2,694	11.0	51,084	
4. Disposal	e H	502,000	0.48	241	7.1	3,564	
5. Revetment							
- Low water channel	2 H	56,000	15.5	868	485.0	27,160	
- High water channel	H 2	TI,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos.	25	21,800	545	230,000	5,750	
7. Backwater levee							
- Excavation	ខ្ព	171,000	1.08	185	23.3	3,984	-
- Embankment	£ #	154,000	0.58	68	31.0	1,694	
8. Bridge	m ₂	5,600	375	2,100	6,500	36,400	
9. Sub-total (2 v 8)			-	13,018		266,891	
10. Miscellaneous (1+9) x 10%	S.	1	í	1,391	1	28,562	
Total			:	15,320	. *	314,135	
B. Acquisition and Compensation	ha	448	ŧ	I	347,600	155,725	
A. + B.				15,320		469,860	
C. Eng. service and Administration	(A+B)	x 15%		2,300		70,480	
D. Contingency (A+B+C) × 15%				2,640		81,050	
Grand Total				20,260		621,390 F	¥1,087,370,000

WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS, ALTERNATIVE 3, KHLONG LUANG RIVER Table 38

Item			נ	(7100)		(,0,)	
	That	4 . 1	ر ب	(20)	יי	a .	7. c # 0.0
	7	אממזירדרא	Unit Cost	Amount.	Unit Cost	Amount	L'EMBALNS.
0 7 7 6 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				× 103		× 103	
Dyonatation (O < 79)	υ -	:		(0	
. Freparacion (9. x /%)	n c	ŀ	1	276		T8, 698	
. Excavation	e E	5,725,000	1.08	6,183	23.3	133,393	
). Embankment	m E	4,650,000	0.58	2,697	11.0	51,150	
. Disposal	e E	502,000	0.48	241	7.1	η,	•
. Revetment							
- Low water channel	ш ²	56,000	15.5	898	485.0	27,160	
- High water channel	m ₂	11,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos.	25	21,800	545	230,000	5,750	
7. Backwater levee		:					
- Excavation	E	171,000	1.08	185	23,3	3,984	
- Embankment	e E	154,000	0.58	89	11.0	1,694	
8. Bridge	m ₂	5,600	375	2,100	6,500	36,400	
9. Sub-total (2 ∿ 8)				13,029		267,121	
10. Miscellaneous (1+9) x 10%	ა 1	ı	. 1	1,394		28,582	
Total				15,335		314,401	
B. Acquisition and Compensation	Ъа	448	1	1	347,600	155,725	
A. + B.				15,335		470,126	
C. Eng. service and Administration	n (A+B)	× 15%		2,300		70,519	
D. Contingency (A+B+C) × 15%				2,645		81,095	
Grand Total				20,280		621,740	Ø 1,088,180,000
	٠.						

Table 39 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR KHLONG LUANG RIVER, BLOCK A

Flo	ood scharge	Expected Frequency Event per	Flood Damages	Events per Year within Interval	Average Damage per Interval	Average Annual Flood	Accumulated Average Annual
	,,	100 years (%)	(8 10 ⁶)		(8 10 ⁶)	Damages (# 10°)	Damages (B LO ⁶)
1.	Unregul	ated					
	127	300.0	1.8	1.000	3 300	2.204	2,300
	140	200.0	2.8	1.000	2.300	2.300	* *
	162	100.0	6.1	1.000	4.450	4.450	6.750
	176	66.7	9.1	0.333	7.600	2.531	9.281
	210	22.2	18.2	0.445	13.650	6.074	15.355
	238	10.5	23.8	0.117	21.000	2.457	17.812
	273	3.3	30.6	0.072	27.200	1.958	19.770
1.4	292	2.0	33.4	0.013	32.000	0.416	20.186
	319	1.0	36.8	0.010	35.100	0.351	20.537
	347	0.5	40.1	0.005	38.450	0.192	20.729
	381	0.2	43.8	0.003	41.950	0.126	20.855
	٠.	÷	. 1			:	
2.	Regulate	ed by Khlong	Luang Dam				
	50	300.0	0.0	1.000	0.050	0.050	0.050
	52	200.0	0.1	1.000	0.010	0.100	0.150
	58	100.0	0.1	0.333	0.100	0.033	0,183
	62	66.7	0.1	0.445	0.150	0.067	0.250
-	70	22.2	0.2	0.117	0.250	0.029	0.279
	75	10.5	0.3	0.072	0.400	0.029	0.308
	88	3.3	0.5	0.013	0.550	0.007	0.315
	96	2.0	0.6	0.010	0.750	0.008	0.323
	109	1.0	0.9		1.100	0.008	
	121	0.5	1.3	0.005			0.329
	139	0.2	2.7	0.003	2.000	0.006	0.335

Table 40 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR KHLONG LUANG RIVER, BLOCK B

Flood Dischare	Expected ge Frequency Event per 100 years (%)	Flood Damages (B 10 ⁶)	Events per Year within Interval	Average Damage per Interval (Ø 10 ⁶)	Average Annual Flood Damages (Ø 10 ⁶)	Accumulated Average Annual Damages (B 10 ⁶)
					: '	
l. Unr	egulated					
102		1.2	1.000	1.500	1.500	1.500
112		1.8 3.4	1.000	2.600	2.600	4.100
141		6.1	0.333	4.750	1.582	5.682
166	22.2	12.5	0.445	9.300	4.139	9.821
188		15.8	0.117	14.150	1.656	11.477
			0.072	17.700	1.274	12.751
213	:	19.6	0.013	20.250	0.263	13.014
227		20.9	0.010	21.550	0.216	13.230
247	1.0	22.2	0.005	22.750	0,114	13.344
266	0.5	23.3	0.003	23.750	0.071	13.415
290	0.2	24.2				
2. Reg	ulated by Khlong	Luang Dam			a t	
. 61	300.0	0.1		0.150	0.170	0.150
66	200.0	0.2	1.000	0.150	0.150	0.150
74	100.0	0.3	1.000	0.250	0.250	0.400
80	66.7	0.5	0.333	0.400	0.133	0.533
91	22.2	0.8	0.445	0.650	0.289	0.822
101	•	1.1	0.117	0.950	0.111	0.933
			0.072	1.450	0.104	1.037
112	٠	1.8	0.013	2.000	0.026	1.063
118		2.2	0.010	2.500	0.025	1.088
124	1.0	2.8	0.005	3.300	0.017	1.105
132	0.5	3.8	0.003	5,200	0.016	1.121
142	0.2	6.6	0.000			_,

Table 41 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR KHLONG LUANG RIVER, BLOCK C (1/2)

Flo	od charge	Expected Frequency Event per 100 years (%)	Flood Damages (# 10 ⁶)	Events per Year within Interval	Average Damage per Interval (ß 10 ⁶)	Average Annual Flood Damages (Ø 10)	Accumulated Average Annual Damages (Ø 10 ⁶)
1.	Unregul	ated					
	135	300.0	4.5	1.000	6.000	6.000	6.000
	153	200.0	7.5	1			
-	184	100.0	14.0	1.000	10.750	10.750	16.750
	202	66.7	18.0	0.333	16.000	5.328	22.078
				0.445	27.600	12.282	34.360
	250	22.2	37.2	0.117	49.500	5.792	40.152
	289	10.5	61.8	•		•	
	344	3.3	88.2	0.072	75,000	5.400	45.552
	361	2.0	100.2	0.013	94.200	1.225	46.777
				0.010	107.700	1.077	47.854
	389	1.0	115.2	0.005	122.400	0.612	48.466
	426	0.5	129.6				a de la companya de La companya de la co
	452	0.2	148.2	0.003	138.900	0.417	48.883
	e e						
2.	Regulat	ed by New Ban	Bung Dam		4.97.3		
	127	300.0	4.0				•
	146	200.0	6.0	1.000	5.000	5.000	5.000
	٠.	•		1.000	9.000	9.000	14.000
	177	100.0	12.0	0.333	14.900	4,962	18.962
	198	66.7	17.8	0.445			
	245	22.2	36.0	0.445	26.900	11.971	30.933
	283	10.5	52.3	0.117	44.150	5.166	36.099
				0.072	69.250	4.986	41.085
	339	3.3	86.2	0.013	91.700	1.192	42.277
	354	2.0	97.2				••
	382	1.0	110.5	0.010	103.850	1.039	43.316
	418	0.5	126.7	0.005	118.600	0.593	43.909
		1		0.003	131.000	0.393	44.302
	444	0.2	135.3				*.

(cont'd)

Table 41 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR KHLONG LUANG RIVER, BLOCK C (2/2)

	Expected Frequency Event per 100 years (%) L by New Ban 300.0 200.0 100.0 66.7	3.5 4.5 8.5	Events per Year within Interval hlong Luang Dam 1.000	Average Damage per Interval (# 10 ⁶) s 4.000 6.500	Average Annual Flood Damages (E 10 ⁶) 4.000 6.500	Accumulated Average Annual Damages (B 10 ⁶)
Regulated 122 135 159 174 215	Event per 100 years (%) 1 by New Ban 300.0 200.0 100.0	(B 10 ⁶) Bung and Kl 3.5 4.5 8.5	Interval hlong Luang Dam 1.000 1.000	per Interval (# 10 ⁶) s 4.000	Flood Damages (E 10 ⁶)	Annual Damages (½ 10 ⁶)
122 135 159 174 215	(%) 1 by New Ban 300.0 200.0 100.0	3.5 4.5 8.5	1.000	4.000	(½ 10 ⁶)	(½ 10 ⁶)
122 135 159 174 215	300.0 200.0 100.0	3.5 4.5 8.5	1.000	4.000		
122 135 159 174 215	300.0 200.0 100.0	3.5 4.5 8.5	1.000	4.000		
135 159 174 215	200.0	4.5 8.5	1.000			
135 159 174 215	200.0	4.5 8.5	1.000			
159 174 215	100.0	8.5	1.000			
174 215				6.500	6.500	10.500
174 215						
215	66.7			10.150	3 300	13 000
215		11.8	0.333	10.150	3.380	13.880
	~~.	11,0	0.445	17.700	7.877	21.757
239	22.2	23.6				
. 439	10.5	33.8	0.117	28.700	3.358	25.115
	10.2	33.8	0.072	42.250	3.042	28.157
274	3,3	50.7	0.0.			20.20
			0.013	56.350	0.733	28.890
295	2.0	62.0	0.010	69,400	0.604	20 504
319	1.0	76.8	0.010	09.400	0.694	29.584
		The state of the s	0.005	81.900	0.410	29.994
340	0.5	87.0	- 444			
370	0.2	105.7	0.003	96.350	0.289	30.283
570	. 0.2	193.7				

Table 42 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR KHLONG LUANG RIVER, BLOCK E (1/2)

Plood Expected Flood Events per Vear within Damage Proquency Damages Vear within Enterval Der faterval Plood Annual Alvarage Annual Annual Annual Plood Plood Annual Plood Plood Annual Plood				•	*			
1. Unregulated 181			Frequency Event per 100 years	Damages	Year within	Damage per Interval	Annual Flood Damages	Average Annual Damages
181 300.0 15.5 1.000 20.750 20.750 20.750 20.750 209 200.0 26.0 1.000 38.750 38.750 59.500 256 100.0 51.5 0.333 63.250 21.062 80.562 285 66.7 75.0 0.445 99.900 44.456 125.018 361 22.2 124.8 0.117 135.300 15.830 140.848 411 10.5 145.8 0.072 156.900 11.297 152.145 493 3.3 168.0 0.013 171.900 2.235 154.380 560 2.0 175.8 0.010 180.300 1.803 156.183 667 0.5 192.0 0.005 188.400 0.942 157.125 658 0.2 200.4 0.003 196.200 0.589 157.714 2. Regulated by New Ban Bung Dam 1.000 20.000 20.000 20.000 20.000 252 100.0 48.5 0.333 60.300 20.080 76.330		· · · · · · · · · · · · · · · · · · ·		() 10 /		(p 10)	(B 19 7	<u>(p 20 1</u>
181 300.0 15.5 1.000 20.750 20.750 20.750 20.750 209 200.0 26.0 1.000 38.750 38.750 59.500 256 100.0 51.5 0.333 63.250 21.062 80.562 285 66.7 75.0 0.445 99.900 44.456 125.018 361 22.2 124.8 0.117 135.300 15.830 140.848 411 10.5 145.8 0.072 156.900 11.297 152.145 493 3.3 168.0 0.013 171.900 2.235 154.380 560 2.0 175.8 0.010 180.300 1.803 156.183 667 0.5 192.0 0.005 188.400 0.942 157.125 658 0.2 200.4 0.003 196.200 0.589 157.714 2. Regulated by New Ban Bung Dam 1.000 20.000 20.000 20.000 20.000 252 100.0 48.5 0.333 60.300 20.080 76.330	1.	Unrequl	ated					•
1.000 20.750 20.855 20.00 20.000 20.200 20.000 20.000 20.000 20.000 20.000 20.000 20.000 20.255 20.					4 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -			
209		181	300.0	15.5	1.000	20.750	20.750	20.750
256		209	200.0	26.0		* * * *	·	
285 66.7 75.0	:	256	100.0	51.5	1.000	38.750	38.750	59.500
14.456 125.018 361 22.2 124.8 0.117 135.300 15.830 140.848 141 10.5 145.8 0.072 156.900 11.297 152.145 1493 3.3 168.0 0.013 171.900 2.235 154.380 156.183 15					0.333	63.250	21.062	80.562
361		285	66.7	75.0	0.445	99.900	44 456	125 038
411 10.5 145.8 493 3.3 168.0 520 2.0 175.8 565 1.0 184.8 607 0.5 192.0 658 0.2 200.4 2. Regulated by New Ban Bung Dam 179 300.0 16.0 205 200.0 24.0 1.000 36.250 36.250 56.250 252 100.0 48.5 279 66.7 72.1 353 22.2 122.6 402 10.5 142.1 483 3.3 166.0 509 2.0 172.1 500 0.003 194.850 500 0.003 169.000 500 0.003 169.000 500 0.003 1000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 20.000 500 0.000 0.000 500 0.000 0.000 500 0.0000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000 500 0.000 0.000 0.000		361	22.2	124.8				e - 9
156.900 11.297 152,145 154,145 156,143 156,143 156,143 156,143 156,143 156,143 156,143 156,143 156,143 156,143 156,145 157,125 156,000 156,000 157,125 157,1		41.1	10.5	145.9	0.117	135.300	15.830	140.848
171.900 2.235 154.380 520 2.0 175.8 0.010 180.300 1.803 156.183 157.125		411	* *	742.0	0.072	156.900	11.297	152,145
520		493	3.3	168.0	0.013	171 000	2.225	 2.54. 200
565		520	2.0	175.8	0.013	171.900	2.235	154.380
0.005 188.400 0.942 157.125 0.003 196.200 0.589 157.714 2. Regulated by New Ban Bung Dam 179 300.0 16.0 1.000 20.000 20.000 20.000 205 200.0 24.0 1.000 36.250 36.250 56.250 252 100.0 48.5 0.333 60.300 20.080 76.330 279 66.7 72.1 0.445 97.350 43.321 119.651 353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		566	1.0	104.0	0.010	180.300	1.803	156.183
607		565	1.0	184.8	0.005	188.400	0.942	157.125
2. Regulated by New Ban Bung Dam 179		607	0.5	192.0			•	
2. Regulated by New Ban Bung Dam 179		658	0.2	200.4	0.003	196.200	0.589	157.714
179 300.0 16.0 1.000 20.000 20.000 20.000 205 200.0 24.0 1.000 36.250 36.250 56.250 252 100.0 48.5 0.333 60.300 20.080 76.330 279 66.7 72.1 0.445 97.350 43.321 119.651 353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9								· ·
179 300.0 16.0 1.000 20.000 20.000 20.000 205 200.0 24.0 1.000 36.250 36.250 56.250 252 100.0 48.5 0.333 60.300 20.080 76.330 279 66.7 72.1 0.445 97.350 43.321 119.651 353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9	2.	Regulat	ed hy New Ban	Bung Dam		. *		
1,000 20,000 20,000 20,000 20,000 1,000 36,250 36,250 56,250 252 100.0 48.5 0.333 60,300 20,080 76,330 279 66.7 72.1 0.445 97,350 43,321 119,651 353 22.2 122.6 0.117 132,350 15,485 135,136 402 10.5 142.1 0.072 154,050 11,092 146,228 483 3.3 166.0 0.013 169,050 2.198 148,426 509 2.0 172.1 0.010 177,600 1.776 150,202 554 1.0 183.1 0.005 187,000 0.935 151,137 595 0.5 190.9 0.003 194,850 0.585 151,722			21 11011 2011	David Davi				
205 200.0 24.0 252 100.0 48.5 279 66.7 72.1 353 22.2 122.6 402 10.5 142.1 483 3.3 166.0 509 2.0 172.1 554 1.0 183.1 595 0.5 190.9 1000 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.250 36.300 20.080 76.330 76.330 37.350 43.321 119.651 15.485 135.136 135.136 10.072 154.050 11.092 146.228 148.426 2.198 148.426 37.760 1.776 150.202 38.760 190.9 190.9 39.770 190.9 190.9 40.217 140.228 43.321 119.651 43.321 119.651 <tr< td=""><td></td><td>179</td><td>300.0</td><td>16.0</td><td>1 000</td><td></td><td>20, 200</td><td>20.000</td></tr<>		179	300.0	16.0	1 000		20, 200	20.000
252 100.0 48.5 0.333 60.300 20.080 76.330 279 66.7 72.1 0.445 97.350 43.321 119.651 353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		205	200.0	24.0	1.000	20.000	20.000	20.000
279 66.7 72.1 0.445 97.350 43.321 119.651 353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		252	100.0	40 5	1,000	36.250	36.250	56.250
279 66.7 72.1 0.445 97.350 43.321 119.651 353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		252	100.0	48.5	0,333	60.300	20.080	76.330
353 22.2 122.6 0.117 132.350 15.485 135.136 402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		279	66.7	72.1				
402 10.5 142.1 0.072 154.050 11.092 146.228 483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		353	22.2	122.6	0.445	97.350	43.321	119.651
483 3.3 166.0 0.072 154.050 11.092 146.228 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722					0.117	132.350	15.485	135.136
483 3.3 166.0 0.013 169.050 2.198 148.426 509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		402	10.5	142.1	0.072	154 050	11.002	146 229
509 2.0 172.1 0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		483	3.3	166.0	0.072	154.050	11.092	140.220
0.010 177.600 1.776 150.202 554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		FÓO	2.0	170.1	0.013	169.050	2.198	148.426
554 1.0 183.1 0.005 187.000 0.935 151.137 595 0.5 190.9 0.003 194.850 0.585 151.722		JU9 .	2.0	116,1	0.010	177.600	1.776	150.202
595 0.5 190.9 0.003 194.850 0.585 151.722	٠. '	554	1.0	183.1	0.005	107.000		
0.003 194.850 0.585 151.722		595	0.5	190.9	0.005	187.000	0.935	151.137
645 0.2 198.8					0.003	194.850	0.585	151.722
		645	0.2	198.8			•	

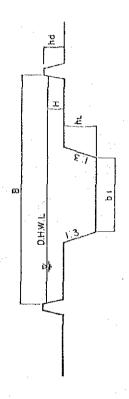
(cont'd)

Table 42 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR KHLONG LUANG RIVER, BLOCK E (2/2)

Flood	Expected	Flood	Events per	Average	Average	Accumulated
Discharge	Frequency	Damages	Year within Interval	Damage per Interval	Annual Flood	Average Annual
	Event per 100 years		fuccival	ber inferagi	Damages	Damages
	(%)	(B 10 ⁶)		(\$ 10 ⁶)	(B 10 ⁶)	() 10 ⁶)
3 Pogula	ted by New Par	Bung and M	hlong Luang Dam		•	
. S. Reguia	ced by New Bal	Dung and K	infong buang ban	12		
176	300.0	14.5				
	•		1.000	19.000	19.000	19.000
203	200.0	23.5				
0.10	100.0	46.0	1.000	34.750	34.750	53.750
248	100.0	46.0	0.333	59.000	19.647	73.397
278	66.7	72.0	0.333	39.000	19.047	13.391
. 270	33.7	, 0	0.445	97.250	43.276	116.673
352	22.2	122.5			45.	
			0.117	132.250	15.473	132.146
400	10.5	142.0				
481	3.3	165.8	0.072	153.900	11.081	143.227
401	3.3	103.0	0.013	168.900	2.196	145.423
507	2.0	172.0	0.015	100.300		143
			0.010	177.500	1.775	147.198
552	1.0	183.0				127
			0.005	186.900	0.935	148.133
593	0.5	190.8	0.000	104.000	0.504	140.313
645	0.2	198.8	0.003	194.800	0.584	148.717
040	0.2	190.0				1

2 , KHLONG LUANG RIVER CHANNEL FACTORS FOR CHANNEL DESIGN, RISK LEVELS Table 43

01	Stretches	Οł	н	j. i	щ	a Q	hl	Ħ	'nđ
•		(m^3/sec)	(Km)	ı	(田)	(田)	(m)	(四)	(E)
	30-yr (Risk Level :	2)							
Н	0.0 - 12.2	1,150	11.5	1/5,000	150	66.5	4.0	3.4	7.4
H	12.2 - 22.5	1,150	ω	1/5,000	150	66.5	2.5	4.5	ນ ນັ
H	22.5 - 29.5	1,150	0.9	1/1,400	150	66.5	2.2	3.3	4.3
<u>}</u>	29.5 - 38.0	400	8 5	1/1,100	100	14.6	3,4	₩.	2.6
⊳	38.0 - 44.2	230	0	1/1,100	O In	7.5	4.0	H	on m
ΛI	44.2 - 47.0	06	2.5	1/1,100	50	8.4	4.2	0	0
	10-yr (Risk Level :	1)							
H	0.0 - 12.2	1,050	11.5	1/5,000	150	61.5	0.4	E3	4.2
II	12.2 - 22.5	1,050	ω Ω	1/5,000	150	61.5	2.5	4. E	m m
III	22.5 - 29.5	1,050	0.9	1/1,400	150	เก เา เง	2.2	3.1	7
A	29.5 - 38.0	340	8	1/1,100	100	12.6	м Ч	: 9 :H	2.4
>	38.0 - 44.2	190	0.9	1/1,100	20	ស	4.0	6.0	
ŢΛ	44.2 - 47.0	70	2.5	1/1,100	50		4.2	0.0	0.6
7		0/	4.0	77,100	00	۳.۲	4.	7	



WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS FOR RISK LEVEL 1, KHLONG LUANG RIVER Table 44

\$1=\\\\\250=\\\\\23

++++++++++++++++++++++++++++++++++++++	7, 5	, i	F.C (US\$)	(\$8)	L.C (B)	(8)	To the second of
reall	7.TUO	Zuanti ty	Unit Cost	Amount	Unit Cost	Amount	Reliaers
			s".	× 103		× 103	
A. Main Civil Works) -		2	
1. Preparation (9 × 7 %)	r.s	1	í	798		16,346	
2. Excavation	e E	4,780,000	1.08	5,162	23.3	111,374	
3. Embankment	ខ្ល	3,890,000	0.58	2.256	11.0	42,790	
4. Disposal	в 3	412,000	0.48	198	7.1	2,925	
5. Revetment	.:		• •	:	٠		
- Low water channel	m ₂	56,000	15.5	898	485.0	27,160	
- High water channel	m2	11,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos.	25	21,800	545	230,000	5,750	
7. Backwater levee						-	
- Excavation	e H	93,000	1.08	100	23,3	2,167	
- Embankment	ສ	84,000	0.58	49	11.0	924	
8. Bridge	m ²	5,600	375	2,100	6,500	36,400	
9. Sub-total (2 ~ 8)				11,399		233,516	
10. Miscellaneous $(1+9) \times 10$ %	ក ស	, , , , , , , , , , , , , , , , , , ,	ı	1,223	1	24,984	
Total				13,420		274,846	
B. Acquisition and Compensation	ha	440	l	1 .	347,600	152,944	
Total (A + B)		.*		13,420		427,790	
C. Eng. Service and Administration				2,010		64,170	·
(A + B) x 15 &							
D. Contingency (A + B + C) × 15 %			:	2,320		73,790	
Grand Total				17,750		565,750	¥ 974,000,000

WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS FOR RISK LEVEL 2, KHLONG LUANG RIVER Table 45

\$1=¥250=展23

***	4		F.C (US\$)	5\$)	L.C (B)	(対	2 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d
יי רביוו	י דיי	Zagai car C.X	Unit Cost	Amount	Unit Cost	Amount	Melilarks
				٠ د د		د	
A. Main Civil Works) -1 ×) OT K	
1. Preparation (9 × 7%)	ນ	1	ı	861		17,465	
2. Excavation	е П	5,297,000	1.08	5,721	23.3	123,420	
3. Embankment	ന (ജ	4,286,000	0.58	2,486	11.0	47,146	
4. Disposal	ຕິເສ	481,000	0.48	231	7.1	3,415	
5. Revetment	5.14						
- Low water channel	m ²	56,000	15.5	868	485.0	27,160	
- High water channel	m ₂	11,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos.	25	21,800	543	230,000	5,750	
7. Backwater levee	•						
- Excavation	<u>بر</u>	143,000	1.08	154	23.3	3,332	
- Embankment	e E	129,000	0.58	75	11.0	1,419	
8. Bridge	m ²	5,600	375	2,100	6,500	36,400	-
9. Sub-total (2 ∿ 8)				12,301		252,068	
10. Miscellaneous (1+9) ×10%	L.S	i	1	1,318	1 	26,952	
Total		-		14,480		296,485	
B. Acquisition and Compensation	ръ	444	1	1	347,600	154,335	
Total (A + B)	ų.			14,480		450,820	
C. Eng. Service and Administration (A + B) × 15 %				2,170		67,620	
D. Contingency (A + B + C) × 15 %				2,500		77.770	
							-
Grand Total	ē	, affir		19,150		596,210 #	1,036,660,000

Table 46 PROBABLE FLOOD RUNOFF OF RAYONG RIVER UNREGULATED

Point				-	Return Period	Period			
No. 21	Location	2-yr	5-Yr	10-yr	30-yr	20-yr	100-YE	200-YE	500-yr
4	Khlong Dok Krai river	247	306	344	417	443	492	530	283
α	Nong Pla Lai river	278	343	8 8 2	467	496	552	.5 89	649
1	Inflow of Khlong Yai damsite	507	637	718	885	932	1,039	1,120	1,226
v v	Outflow of Khlong Yai damsite	507	637	718	885	932	1,039	1,120	1,226
7	Upstream of confluence of Khlong Luai river	181	221	247	299	314	348	373	408
- t	Ban Krai weir	609	732	811	996	1,021	1,120	1,193	1,290
14	Upstream of confluence of Khlong Thap Ma river	531	632	969	825	869	о го го	1,016	1,122
·.1	Inflow of Khlong Thap Ma damsite	377	480	542	658	669	771	840	918
16	Outflow of Khlong Thap Ma damsite	377	480	542	658	669	771	840	918
6	Khlong Thap Ma river	159	197	222	263	278	301	322	354
. 21	Rayong bridge	396	4 58	495 3	563	584	626	658	9 8 9

11 : Refer to Fig. 32

Table 47 PROBABLE FLOOD RUNOFF OF RAYONG RIVER, REGULATED BY DOK KRAI DAM

 $(unit: m^3/s)$

Point No. /1 4 Khlq 2 Nonc	Location Khlong Dok Krai river	2-vr			Return	$\alpha_{\rm I}$			
	ver	2-vr			3	50-477			
	ong Dok Krai river q Pla Lai river	7	5-yr	10-yr	30-yr	30-7 E	100-yr	200-yr	500-yr
	d Pla Lai river	92	106	115	133	139	152	162	180
		278	343	385	467	496	552	ი დ დ	649
	Inflow of Khlong Yai damsite	507	637	718	88 5	932	1,039	1,120	1,226
6 Out:	Outflow of Khlong Yai damsite	507	637	718	88 83	83.0	1,039	1,120	1,226
7 Ups Khl	Upstream of confluence of Khlong Luai river	181	221	227	2000	314	348	373	408
11 Ban	Ban Khai weir	493	590	644	753	791	862	915	980
14 Ups Khl	Upstream of confluence of Khlong Thap Ma river	452	534	583	677	709	774	819	895
- Inf	Inflow of Khlong Thap Ma damsite	377	480	542	658	669	771	840	918
16 Out	Outflow of Khlong Thap Ma damsite	373	480	542	658	ტ ტ	771	840	816
19 Khl	Khlong Thap Ma river	159	197	222	263	278	301	322	354
21 Ray	Rayong bridge	371	425	459	523	541	579	809	640

/1 : Refer to Fig. 32

Table 48 PROBABLE FLOOD RUNOFF OF RAYONG RIVER, REGULATED BY DOK KRAI AND NONG PLA LAI DAMS

		:					(Unit	(Onit : m~/s)	
Point	Location	2017	} !	77.70		Period	100-	200-477	i C
7 7 OM		2 _ Z Z	ユメエ	7 X L O T	- λ-Oc	4.Z-0;	- 4X_00T	マメーシウラ	300-YE
•		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1							
₫'	Khlong Dok Krai river	9 2	106	115	133	139	152	797	T80
7	Nong Pla Lai river	χ ω	109	126	160	172	195	211	239
1	Inflow of Khlong Yai damsite	507	637	718	88 885	932	1,039	1,120	1,226
w	Outflow of Khlong Yai damsite	507	637	718	885	932	1,039	1,120	1,226
7	Upstream of confluence of Khlong Luai river	181	221	247	299.	314	348	373	408
	Ban Khai weir	347	424	456	522	544	592	629	670
14	Upstream of confluence of Khlong Thap Ma river	352	419	4 የር የ	218	9 6 9 9	583	616	664
t	Inflow of Khlong Thap Ma damsite	3.7.7	480	542	658	669	771	840	918
70	Outflow of Khlong Thap Ma damsite	377	480	542	658	669	771	840	918
19	Khlong Thap Ma river	 68 H	197	222	263	278	301	322	354
2	Rayong bridge	339	386	418	480	495	530	557	59 3

21 : Refer to Fig. 32

Table 49 PROBABLE FLOOD RUNOFF OF RAYONG RIVER, REGULATED BY DOK KRAI, NONG PLA LAI AND KHLONG YAI¹ DAMS

							(Unit	(Unit: m^3/s)	. •
Point				14	Return Period	riod			
No. 12	Location	2-yr	5-yr	10-yr	30-yr	50-YK	1001	Z00-VZ	500-yr
ধ্য	Khlong Dok Krai river	9 2	1.06	115	133	139	152	162	180
7	Nong Pla Lai river	83	109	126	160	172	195	211	239
ı	Inflow of Knlong Yai damsite	507	637	718	885	932	1,039	1,120	1,226
vo	Outflow of Khlong Yai damsite	57	78	6	121	130	152	168	190
7	Upstream of confluence of Khlong Luai river	4. 4.	61	7.1	98	<u>س</u> در	102	109	121
1.1	Ban Khai weir	287	333	359	412	432	453	477	503
14	Upstream of confluence of Khlong Thap Ma river	317	373	404	466	488	529	53 52 4	009
1	Inflow of Khlong Thap Ma damsite	377.	480	542	658	669	773	840	818
16	Outflow of Khlong Thap Ma damsite	377	480	542	658	900	771	840	918
61	Khlong Thap Ma river	159	197	222	263	278	301	322	354
21 2	Rayong bridge	322	362	392	449	466	4. 0.	523	560

11: With spillway width 70 m

12 : Refer to Fig. 32

Table 50 PROBABLE FLOOD RUNOFF OF RAYONG RIVER, REGULATED BY DOK KRAI, NONG PLA LAI, KHLONG YAI AND KHLONG THAP MA DAMS

		- - !	•				(Unit	: m /s)	
Point				ř	Return Period	riod			
No. 12	Location	2-yr	5-yr	10-yr	30-yr	50-yr	100-yr.	200-yr	500-yr
4	Khlong Dok Krai river	92	106	115	133	139	152	162	180
	Nong Pla Lai river	83	109	126	160	172	195	211	239
1	Inflow of Khlong Yai damsite	507	637	718	882	932	1,039	1,120	1,226
9	Outflow of Khlong Yai damsite	57	78	92	121	130	152	168	190
<i>c</i>	Upstream of confluence of Khlong Luai river	7 7	19	71	88	T o	102	0 0	121
11	Ban Khai weir	287	ლ ლ	359	412	432	453	477	503
14	Upstream of confluence of Khlong Thap Ma river	317	373	404	466	488	529	554	009
I	Inflow of Khlong Thap Ma damsite	377	480	542	658	669	771	840	918
16	Outflow of Khlong Thap Ma damsite	9 6	20	ю 9	77	84	. 97	108	122
- 19	Khlong Thap Ma river	71,	83	91	104	112	120	128	140
21	Rayong bridge	307	339	367	420	436	474	503	545
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1.4		10		E	32.	

Dok Krai dam, Nong Pla Lai dam, Khlong Yai dam with spillway width 70m and Khlong Thap Ma dam with spillway width 50m 77:

: Refer to Fig. 32

Table 51 PROBABLE FLOOD RUNOFF OF RAYONG RIVER WITH DOK KRAI, NONG PLA LAI, KHLONG YAI DAMS¹ AND RIVER IMPROVEMENT

							(Unit	: m ³ /s)	
Point			, 1 'S	μ.	Recurn Pe	Period			
No. 22	Location	2-yr	2 <u>7</u> 2	70-7x	30-7x	20-YE	100-yr	200-yr	500-yr
4	Khlong Dok Krai river	159	201	227	275	292	325	350	386
0	Nong Pla Lai river	68	122	143	183	197	224	243	274
1	Inflow of Khlong Yai damsite	507	637	718	885	932	1,039	1,120	1,226
Ø	Outflow of Khlong Yai damsite	5.6	77	91	121	130	152	168	061
σ ι .	Upstream of confluence of Rayong river	277	354	თ თ თ	484	521	580	625	069
דר	Ban Khai weir	370	470	530	6.44	685	763	822	016
14	Upstream of confluence of Khlong Thap Ma river		753	847	1,029	1,095	1,219	1,304	1,435
ı	Inflow of Khlong Thap Ma damsite	377	480	542	65 58 89	669	771	840	918
16	Outflow of Khlong Thap Ma damsite	37.7	480	542	658	669	771	840	918
. 6t	Khlong Thap Ma river	411	537	607	737	793	872	9 8 8	1,050
. 2	Rayong bridge	1,329	1,745	2,009	2,412	2,626	2,903	3,135	3,490

^{21 :} With spillway width 70m

^{12 :} Refer to Fig. 32

Table 52 PROBABLE FLOOD RUNOFF OF RAYONG RIVER WITH ALL DAMS²¹ AND RIVER IMPROVEMENT

							ין אַנוּט י	(פון : אַדוּה)	
Point No. 42	Location	2-yr	2-yr	10-yr	Return Period 30-yr 50-y	Period 50-yr	100-yr	200-yr	200-yr
7	Khlong Dok Krai river	159	201	227	275	292	325	350	386
7	Nong Pla Lai river	Ø 8	122	143	183	197	224	243	274
ı	Inflow of Khlong Yai damsite	507	637	718	885	932	1,039	1,120	1,226
w	Outflow of Khlong Yai damsite	56	77	д С	121	130	152	168	190
Q	Upstream of confluence of Rayong river	277	354	399	484	521	580	625	069
7.7	Ban Khai river	370	470	530	644	169	763	822	910
14	Upstream of confluence of Khlong Thap Ma river	588	753	847	1,029	1,095	1,219	1,304	1,435
1	Inflow of Khlong Thap Ma damsite	377	480	542	658	ტ ტ	771	840	918
16	Outflow of Khlong Thap Ma damsite	9e	20	59	77	84	97	108	122
61	Khlong Thap Ma river	137	186	219	266	296	322	350	385
77	Rayong bridge	1,057	1,353	1,522	1,849	1,999	2,226	2,381	2,640

^{11 :} Dok Krai dam, Nong Pla Lai dam, Khlong Yai dam with spillway width 70m and Khlong Thap Ma dam with spillway width 50m

^{22:} Refer to Fig. 32

Table 53 CHANNEL FACTORS FOR CHANNEL DESIGN FOR RAYONG RIVER

	÷									
Alternatives		Stretches	Length	Design Discharge	Gradient	River Width	Low Water C. Width	Low Water C. Depth	Water Depth of High Water C.	Dike Height
			(Km)	Q (m³/sec)	ы	ж (m)	b ₁ (m)	h] (m)	н (ш)	hđ (m)
	· H	0.00-13.40	3.6	2,650	1/2,500	300	204.0	4.0	F. T.	2.9
	1	13.40-32.55	15.5	1,100	1/2,500	150	63.0	2.6	3.2	4.2
-	III	32,55-40.85	5.2	700	1/1,800	100	42.1	3.6	2.3	3.3
-4		40.85-42.80	1.2	520	1/900	100	16.1	0.4	1.7	2.7
		42.80-47.00	2.4	300	1/900	100	10.8	4.2	6.0	1.7
-		47.00-50.70	2.8	110	1/900	000	7.8	3.0	0.4	1.0
	VII	50.70-55.90	5.2	110	1/520	ဝမ္	7.8	2.5	o.s	1.1

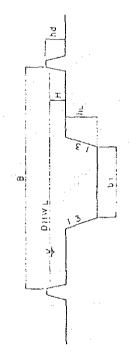
	н	0.00-13.40	9. n	2,650	1/2,500	300	204.0	4.0	1.7	6.2
	II	13,40-32,55	15.5	1,100	1/2,500	150	63.0	2,6	3.2	4.2
ŗ	HHH	32.55-40.85	5.2	700	1/1,800	1.00	42.1	3.6	2.3	3.3
N	ΔI	40.85-42.80	1.2	530	1/900	100	16.1	6.0	1.7	2.7
	•	42.80-47.00	2.4	310	1/900	100	11.0	5,2	3.0	1.8
	ĬΛ	47.00-50.70	2.8	140	1/900	50	4.6	3.0	0.7	7.3
	VII	50.70-55.90	5.2	140	1/520	20	4.0	2.5	0.7	1.3
:	н	0.00-13.40	3,6	2,650	1/2,500	300	204.0	0.4	1.7	2. ق
		13.40-32.55	15.5	1,100	1/2,500	150	63.0	2.6	3.2	4.2
r		32.55-40.85	52	700	1/1,800	100	42.1	3.6	2.3	3.3
า		40.85-42.80	1.2	540	1/900	100	16.4	0.4	1.7	2.7
	٥	42.80-47.00	2 4	320	1/900	100	11.2	4.2	7.0	8.1
	IA	47.00-50.70	2.8	160	1/900	50	10.0	3.0	6.0	3.5
	IIA	50,70-55.90	5.2	160	1/520	20	10.0	2.5	6.0	₽. ₽.

Note: In case of 3 dams in operation.



Table 54 CHANNEL FACTORS FOR CHANNEL DESIGN FOR KHLONG THAP MA RIVER

Alternatives		Stretches	Length (Km)	Design Discharge Q (m³/sec)	Gradient I	River Width B (m)	Low Water C. Width bl (m)	Low Water C. Depth hl (m)	Water Depth of High Water C. H (m)	Dike Height hd (m)
	н	0.00-13.40	3.6	2,000	1/2,500	250	145.0	4.0	1.8	2.8
	Ħ	0.00- 3.50	ۍ د	290	1/4,500	100	31.5	1.0	4.2	5.0
r -l	III	3.50-8.00	4.5	9	1/1,500	20	7,8	1.8	. 8.0	1.4
	ΔI	8.00-11.05	ιη · Ε	09	1/600	900	6.4	2.0	E + O	6.0
	 	0.00-13.40	3 6	2,000	1/2,500	250	145.0	4.0	80	2.8
, (II	0.00- 3.50	<u>د</u>	300	1/4,500	100	32.0	1.0	4.2	5.0
7	HH	3.50- 8.00	4.5	85	1/1,500	50	11.4	1.8	6 0	1.5
	IV	8.00-11.05	S .	89	1/600	50	8	2.0	9.0	1.2
- Andrews	н	0.00-13.40	3.6	2,000	1/2,500	250	145.0	4.0	1.8	2.8
r	Ħ	0.00-3.50	e C	310	1/4,500	100	33.0	1.0	4.2	5.0
ก	III	3.50- 8.00	5.4	110	1/1,500	20	14.1	1.8	1.1	1.7
	IΛ	8.00-11.05	3.5	110	1/600	50	ഗ	2.0	0.7	1.3



WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS OF RAYONG RIVER, ALTERNATIVE 1 Table 55

	:					2₹ = T\$	50 = 第23	
Item	Unit	Ouantity	D.	(\$SD)			Remarks	
		3	Unit Cost	Amount	Unit Cost	Amount		· 1
A STATE OF THE PROPERTY OF THE				×103				
At MALIE CIVIL WOLNS			•					
1. Preparation (9×78)	,	1	1	1,002	1	20,283		
2. Excavation		,860,000	1.08	6,329	23.3	136,538		
3. Embankment		3,880,000	0.58	2,251	11.0	42,680		
4. Disposal	33	,560,000	0.48	749	7.1	11,076		
5. Revetment								
- Low water channel	m ₂	54,000	15.5	837	485	26,190		
- High water channel	т Т	11,000	11.0	121	366	4,026		
6. Drainage sluice	sou	20	21,800	436	230,000	4,600		
7. Backwater levee	•							
- Excavation	E (459,000	1.08	406	23.3	10,695		
- Embankment	e E	413,000	0.58	240	11.0	4,543		
8. Bridge		7,600	375.0	2,850	6,500.0	49,400		
9. Sub-total (2 ∿ 8)				14,309		289,748		
10. Miscellaneous $(1 + 9) \times 10$ %				1,532		•		
Total		•		16,843		341,035		
B. Acquisition and Compensation	ha	487	ı		009,740	169,282		
Sub-Total				16,843		510,317		
C. Eng. service and Administration	n (A+B)	× 15%		2,527		76,548		
D. Contingence (A+B+C) \times 15%				2,906		88,030		
Grand Total				22,276	·	674,895	B 1,187,243,000	. 0
								ı

WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS OF RAYONG RIVER, ALTERNATIVE 2Table 56

					\$1 = 苯5	¥250 = \$23
\$4.0 apr	} .	F.C (US\$)	3\$)	L.C (B	(S)	D. 27.70
man T	OILL QUALLELY	Unit Cost	Amount	Unit Cost	Amount	Velilatas
	-		×103			
A. Main Civil Works			٠			
1. Preparation $(9 \times 7\%)$	l	1	1,005	ı	20,348	
2. Excavation	m ³ 5,880,000	1.08	6,350	23.3	137,004	
3. Embankment	m ³ 3,910,000	0.58	2,268	11.0	43,010	
4. Disposal	m ³ 1,580,000	0.48	759	7.1	11,218	
5. Revetment						
- Low water channel	m ² 54,000	15.5	837	485	26,190	
- High water channel	m ² 11,000	11.0	121	366	4,026	
6. Drainage sluice	nos 20	21,800	436	230,000	4,600	
7. Backwater levee						
- Excavation	m ³ 459,000	1.08	496	23.3	10,695	
- Embankment	m ³ 413,000	0.58	240	11.0	4,543	
8. Bridge	7,600	375.0	2,850	6,500.0	49,400	
9. Sub-total (2 ∿ 8)			14,357		290,686	
10. Miscellaneous $(1 + 9) \times$	10%		1,536		31,103	
Total			16,898		342,137	
B. Acquisition and Compensation	ion ha 487	1		347,600	169,282	
Sub-Total			16,898		511,419	
C. Eng. service and Administration	ration (A+B) × 15%		2,535		76,711	
D. Contingence (A+B+C) × 15%			2,917		88,220	
Grand Total			22,350		676,350	¥1,190,400,000
						:

WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPRVEMENT WORKS OF RAYONG RIVER, ALTERNATIVE 3 Table 57

					\$1 = \$250	50 = \$23
Item	Unit Ouantity	F.	()	ĺ		Remarks
		Unit Cost	Amount	Unit Cost	Amoun t	
0.7.2.0.0 M			×IO3			
A. HALI CLVII WOLKS		-				
1. Preparation (9 × 7%)	ţ	î	1,010		20,438	-
2. Excavation	m ³ 5,910,000	1.08	6,383	23.3	137,703	
3. Embankment		0.58	2,291	11.0	43,450	
4. Disposal	m ³ 1,600,000	0.48	768	7.7	11,360	
5. Revetment						
- Low water channel	m ² 54,000	15.5	837	485	26,190	
- High water channel	m ² 11,000	11.0	121	366	4,026	
6. Drainage sluice	nos. 20	21,800	436	230,000	4,600	
7. Backwater levee	.0	٠.				
- Excavation		1.08	496	23.3	10,695	
- Embankment	m ³ 413,000	0.58	240	11.0	4,543	
8. Bridge	7,600	375.0	2,850	6,500.0	49,400	
9. Sub-total (2 ν 8)			14,422		291,967	
10. Miscellaneous (1 + 9) × 10%			1,544		31,241	
Total			16,976	:	343,646	
B. Acquisition and Compensation	ha 487	i		347,600	169,282	
Sub-Total			16,976		512,928	
C. Eng. service and Administration	n (A+B) × 15%		2,547	·	76,940	
D. Contingence (A+B+C) \times 15%			2,929		88,481	
Grand Total	ar '		22,452		678,349	Z 1,194,745,000
· · · · · · · · · · · · · · · · · · ·						÷ 1.2

WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPRIEMENT WORKS OF KHLONG THAP MA RIVER, ALTERNATIVE I Table 58

¥250 = B23	Bomerka	Exclication									-												¥ 126,361,000
SI IS	(B)	Amount	× 103	2,082	5,732	10,802	0		4,753	586	920		2,144	905	3,900	29,739	3,183	35,004	20,856	55,860	8,379	9,636	73,875
	I.C (B	Unit Cost			23,3	11.0	7.1		485.0	366.0	230,000		23.3	11.0	6,500		1		347,600				
	(US\$)	Amount	× 103	103	266	570	0		152	18	87		<u>თ</u>	48	225	1,465	157	1,725	. 1	1,725	259	298	2,282
	F.C (1	Unit Cost		ſ	1.08	0.58	0.48		15.5	11.0	21,800		1.08	0.58	375		1		•				
) 1 + 0 = 1.0	אמשוו רד רא		1 1	246,000	982,000	0		008,6	1,600	4,		92,000	82,000	009		1		09		< 15%		
÷.	۲. ۲.	OTT C		S.	e E	E	e H		m ²	m ²	nos.		₈ ۾	⊞3.	m2		N. N		ha		n (A+B) x		
	4 → ↓ L	ד הפזוו	Men nivil monka		2. Excavation	3. Embankment	4. Disposal	5. Revetment	- Low water channel	- High water channel	6. Drainage sluice	7. Backwater levee	- Excavation	- Embankment	8. Bridge	9. Sub-total (2 1 8)	10. Miscellaneous (1+9) x 10%	Total	B. Acquisition and Compensation	Å. + B.	C. Eng. service and Administration	D. Contingency (A+B+C) × 15%	Grand Total

Table 59 WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS OF KHLONG THAP MA RIVER, ALTERNATIVE 2

			. •			** T.S.	¥250 = 蔣23
	4 ; 5 ;	1 1 1 1 1 1 1 1 1	D.F	(\$SD)	O.J	(選)	0 0 1 1 1 1
ד רפוון	Onto	Vadicity	Unit Cost	Amount	Unit Cost	Amount	nemarns
-				× 103		× 103	
A. Main civil works							
1. Preparation (9. × 7%)	 S	1 1	i	103		2,097	
2. Excavation	e H	249,000	1.08	269	23.3	5,802	
3, Embankment	E E	995,000	0.58	577	11.0	10,945	
4. Disposal	Н	0	0.48	0	7.1	0	
5. Revetment							
- Low water channel	ш ₂	9,800	15.5	152	485.0	4,753	
- High water channel	m ²	1,600	11.0	13	366.0	586	
6. Drainage sluice	nos.	4	21,800	87	230,000	920	
7. Backwater levee				1.,			
- Excavation	ж ж	92,000	1.08	თ	23.3	2,144	
- Embankment	E H	82,000	0.58	48	11.0	905	
8. Bridge	m2	009	375	225	6,500	3,900	
9. Sub-total (2 ∿ 8)				1,475		29,952	
10. Miscellaneous $(1+9) \times 10$ %	S. J	ŧ	ŀ	158	1	3,205	
Total	;			1,736		35,254	
B. Acquisition and Compensation	'na	09	l.	1	347,600	20,856	
A. + B.				1,736		56,110	
C. Eng. service and Administration	n (A+B)	× 15%		260		8,417	
D. Contingency (A+B+C) × 15%				303		6,680	
Grand Total				2,299		74,207	R 127,084,000
	: .i						

Table 60 WORK QUANTITY AND CONSTRUCTION COST FOR RIVER IMPROVEMENT WORKS OF KHLONG THAP MA RIVER, ALTERNATIVE 3

e reformable commenter de la commente de la commen			- 1	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		\$1 =	¥250 = \$23
	Unit	Quantity	F.C (Unit Cost	(US\$) Amount	L.C Unit Cost	(A) Amount	Remarks
				× 103		× 103	
Preparation (9. × 7%)	υ. Ω	1	ŧ	104		2,107	
	H 3	251,000	1.08	272	23.3	5,849	
	H	1,003,000	0.58	582	11.0	11,033	
	m ₃	0	0.48	Ö	7.1	0	
- Low water channel	m ₂	008,6	15.5	152	485.0	4,753	
- High water channel	m2	1,600	11.0	Η Θ	366.0	586	
6. Drainage sluice	noe.	ব	21,800	87	230,000	920	
Backwater levee	•						
	e E	92,000	1.08	66	23.3	2,144	
	e E	82,000	0.58	84	11.0	902	
	m2	009	375	225	6,500	3,900	
Sub-total (2 v 8)				1,483		30,087	
Miscellaneous $(1+9) \times 10$ %	S	ì	F	159	ι	3,220	
i i				1,746		35,414	
B. Acquisition and Compensation	ក្ន	09	t,	•	347,600	20,856	:
	. e			1,746		56,270	
C. Eng. service and Administration	(A+B)	X R R R		262		8,441	
D. Contingency (A+B+C) × 15%				302		9,707	
				2,310		74,418	B 127,548,000

Table 61 WORK QUANTITY AND CONSTRUCTION COST FOR OVERALL BASIC FLOOD CONTROL PLAN OF RAYONG RIVER

						* = TS	¥250 = 好23
-	4 7 7 4 4	4 4 5 4 5 6	F.C (US\$	uss)	I.C	(A)	2000000
Lem	OILE	Vualitaty	Unit Cost	Amount	Unit Cost	Amount	rematrs
				× 103		x 103	
A. Main civil works			: -				
1. Preparation $(9. \times 78)$	L N	î	1	823		16,768	
2. Excavation	₈ ه	4,251,000	1.08	4,591	23.3	99,048	
3. Embankment	변	3,557,000	0.58	Q	11.0	39,127	-
4. Disposal	ខ្ព	269,000	0.48	129	7.1	1,910	
5. Revetment			:				
- Low water channel	щ 2	54,000	15.5	837	485.0	26,190	
- High water channel	m ²	11,000	0.44	121	366.0	4,026	
6. Drainage sluice	nos.	20	21,800	436	230,000	4,600	
7. Backwater levee			1				:
- Excavation	E E	459,000	1.08	496	23.3	10,695	
- Embankment	E	413,000	0.58	240	11.0	4,543	
8. Bridge	m^2	7,600	375	2,850	6,500	49,400	
9. Sub-total (2 ∿ 8)				11,763		239,539	-
10. Miscellaneous $(1+9) \times 10$ %	S.J	i.	ì	1,254	ı	25,637	
Total	٠			13,840		281,944	
B. Acquisition and Compensation	na	389	1	ı	347,600	135,216	
A. + B.				13,840		417,160	
C. Eng. service and Administration	(A+B)	х ж ж		2,080		62,570	
D, Contingency (A+B+C) × 15%		•		2,390		71,960	
Grand Total				18,310		551,690	¥ 972,820,000
						·	

Table 62 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK A

Flood Discharge	Expected Frequency	Flood Damages	Events per Year within	Average Damage	Average Annual	Accumulated Average
prscharge	Event per	Jamages	Interval	per Interval	Flood	Annual
	100 years (%)	(B 10 ⁶)	 	(ø 10 ⁶)	Damages (8 10)	Damages (B 10 ⁶)
. Unregu	lated					
188	300.0	0.53	1.000	0.665	0.665	0,665
212	200.0	0.80	1.000	1.320	1.320	1.985
253	100.0	1.84	0.333	2.510	0.836	2.821
278	66.7	3.18	0.445	3.925	1.747	4.568
343	22.2	4.67	0.117	4.920	0.576	5,144
385	10.5	5.17	0.072	5.405	0.389	5.533
467	3.3	5.64	0.013	5.710	0.074	5.607
496	2.0	5.78	0.010	5.880	0.059	5,666
552	1.0	5.98	0.005	6.045	0.030	5.696
589	0.5	6.11	0.003	6.170	0.019	5.715
649	0.2	6.23			0.013	3
. Regula	ted by Nong Pl	a Tai Dame				
					1 -	
70	120.0	0.0	0.200	0.005	0.001	0.001
74	100.0	0.01	0.333	0.015	0.005	0.006
.83	66.7	0.02	0.445	0.040	0.018	0.024
109	22.2	0.06	0.117	0.080	0.009	0.033
126	10.5	0.10	0.072	0.190	0.014	0.047
160	3.3	0.28	0.013	0.325	0.004	0.051
172	2.0	0.37	0.010	0.485	0.005	0.056
195	1.0	0.60	0.005	0.700	0.004	0.060
211	0.5	0.80	0.003	1.125	0.003	0,063
239	0.2	1.45		. – …		•

Table 63 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK B

				1			English to the second
Flo	ood scharge	Expected Prequency Event per 100 years (%)	Flood Damages (% 10 ⁶)	Events per Year within Interval	Average Damage per Interval (8 10 ⁶)	Average Annual Flood Damages (Ø 10°)	Accumulated Average Annual Damages (B 10 ⁶)
	2 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -				•		
1.	Unregul	ated					Ž.÷
	164	300.0	0.76	1.000	0.920	0.920	0.920
	187	200.0	1.08				
	223	100.0	1.83	1.000	1.455	1.455	2.375
	247	66.7	2.74	0.333	2.285	0.761	3.136
				0.445	3.345	1.489	4.625
	306	22,2	3.95	0.117	4.085	0.478	5.103
	344	10.5	4.22	0.072	4.330	0.312	5.415
	417	3.3	4.44	0.013	4.500	0.059	5.474
	443	2.0	4.56		4	•	
	492	1.0	4.67	0.010	4.615	0.046	5.520
	530	0.5	4.76	0.005	4.715	0.024	5.544
	589	0.2	4.81	0.003	4.785	0.014	5.558
:		V	1.01				
2	Regulat	ed by Dok Kra	i Dam	:			
	76	300.0	0.04			•	
	80	200.0	0.07	1.000	0.055	0.055	0.055
				1.000	0.095	0.095	0.150
	. 88	100.0	0.12	0.333	0.130	0.043	0.193
	92	66.7	0.14	0.445	0.185	0.082	0.275
	106	22.2	0.23	0.117	0.270	0.032	0.307
	115	10.5	0.31				
	133	3.3	0.44	0.072	0.375	. 0.027	0.334
	139	2.0	0.56	0.013	0.500	0.007	0.341
	152	1.0	0.63	0.010	0.595	0.006	0.347
	4	1.00		0.005	0.690	0.003	0.350
	162	0.5	0.75	0.003	0.855	0.003	0.353
	180	0.2	0.96				

Table 64 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK C

Flo	ood scharge	Expected Frequency Event per 100 years	Flood Damages	Events per Year within Interval	Average Damage per Interval	Average Annual Flood	Accumulated Average Annual Damages
		(%)	(# 10 ⁶)		(a 1.9 ⁶)	Damages (\$ 10°)	(8 10 ⁶)
i.	Unregul	ated				:	
	- 127	300.0	1.40	41 2 mag		en e	; ; ;
	142	200.0	1.83	1.000	1.615	1.615	1.615
	166	100.0	3.15	1.000	2.490	2.490	4.105
	181	66.7	4.42	0.333	3.785	1.260	5.365
	221	22.2	6.22	0.445	5.320	2.367	7.732
	247	10.5	6.79	0.117	6.505	0.761	8.493
	299	3.3	7.38	0.072	7.085	0.510	9.003
		2.0	7.56	0.013	7.470	0.097	9.100
	314		•	0.010	7.660	0.077	9.177
	348	1.0	7.76	0.005	7.840	0.039	9.216
	373	. 0.5	7.92	0.003	8.005	0.024	9.240
	408	0.2	8, 09	:			
2.	Regulat	ed by Khlong	Yai Dam				
	40	94.0	0.00			1.0	
	44	66.7	0.07	0.273	0.035	0.010	0.010
	61	22.2	0.23	0.445	0.150	0.067	0.077
	71	10.5	0.35	0.117	0.290	0.034	0.111
	86	3.3	0.54	0.072	0.445	0.032	0.143
	91	2.0	0.63	0.013	0.585	0.008	0.151
	102	1.0	0.83	0.010	0.730	0.007	0.158
	102	0.5	1.02	0.005	0.925	0.005	0.163
	1	ta di di		0.003	1.190	0.004	0.167
	121	0.2	1.36			4	

Table 65 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK D (1/2)

Flood Discharge	Expected Frequency Event per 100 years (%)	Flood Camages (B 10 ⁶)	Events per Year within Interval	Average Damage per Interval (3 10 ⁶)	Average Annual Flood Damages (B 10 ⁶)	Accumulated Average Annual Damages (B 10 ⁶)
l. Unregul	ated					
443	300.0	2.40	1.000	2.850	2.850	2.850
485	200.0	3,30	*	4.570	* •	7 500
564	100.0	6.00	1.000	4.650	4.650	7,500
609	66.7	7.98	0.333	6.990	2.328	9.828
609	00.7	7.90	0.445	9.630	4.285	14.113
732	22.2	11.28	0.117	11.850	1.386	15.499
811	10.5	12.42				
966	3.3	13.62	0.072	13.020	0.937	16.436
•			0.013	13.770	0.179	16.615
1,021	2.0	13.92	0.010	14.160	0.142	16.757
1,120	1.0	14.40				
1,193	0.5	14.70	0.005	14.550	0.073	16.830
	. •		0.003	14.910	0.045	16.875
1,290	0.2	15.12	٠			
3				- 10 m		
2. Regulat	ed by Dok Kra	11 Dam	*			
370	300.0	1.30	1.000	1.525	1.525	1.525
403	200.0	1.75	1.000	1.525	4.4	
460	100.0	2.75	1.000	2.250	2.250	3.775
			0.333	3.125	1.041	4.816
493	66.7	3.50	0.445	5.450	2,425	7.241
590	22.2	7.40				
644	10.5	9.37	0.117	8.385	0.981	8.222
			0.072	10.525	0.758	8.980
753	3.3	11.68	0.013	11.865	0.154	9.134
791	2.0	12.05				
862	1.0	12.89	0.010	12.470	0,125	9.259
•			0.005	13.120	0.066	9.325
915	0.5	13.35	0.003	13.540	0.041	9.366
980	0.2	13.73		•		

(cont'd)

Table 65 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK D (2/2)

Flo	od charge	Expected Frequency Event per 100 years (%)	Flood Damages (# 10 ⁶)	Events per Year within Interval	Average Damage per Interval (\$ 10 ⁶)	Average Annual Flood Damages (B 10)	Accumulated Average Annual Damages (N 10 ⁶)
3.	Regulat	ed by Dok Kra	i and Nong	Pla Lai Dams			
	258	300.0	0.40	1.000	0.475	0.475	0.475
	282	200.0	0.55	1.000	0.675	0.675	1.150
	323	100.0	0.80	0.333	0.935	0.311	1.461
	347	66.7	1.07			•	
	424	22.2	2.08	0.445	1.575	0.701	2.162
	456	10.5	2.62	0.117	2.350	0.275	2.437
	522	3.3	4.38	0.072	3,500	0.252	2.689
	544.	2.0	5.20	0.013	4.790	0.062	2.751
	592	1.0	7.44	0.010	6.320	0.063	2.814
	629		144	0.005	8.160	0.041	2.855
	670	0.5	8.88	0.003	9.480	0.028	2.883
4.	Regulat	ed by Dok Kra	i, Nong Pla	Lai and Khlong	Yai Dams		
	209	300.0	0.20				
	228	200.0	0.30	1.000	0.250	0.250	0.250
	257	100.0	0.40	1.000	0.350	0.350	0.600
	287	66.7	0.58	0.333	0.490	0.163	0.763
	333	22.2	0.92	0.445	0.750	0.334	1.097
			:	0.117	1.060	0.124	1.221
	359	10.5	1.20	0.072	1.515	0.109	1.330
	412	3.3	1.83	0.013	2.010	0.026	1.356
	432	2.0	2.19	0.010	2.375	0.024	1.380
	453	1.0	2.56	0.005	2.810	0.014	1.394
	471	0.5	3.06	0.003	3.385	0.010	1.404
	503	0.2	3.71			3.310	

Table 66 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK E (1/2)

		14					
Flood Discharge	Expected Frequency Event per 100 years (%)	Flood Camages (B 10 ⁶)	Events per Year within Interval	Average Damage per Interval (% 10 ⁶)	Average Annual Flood Damages (# 10)	Accumulated Average Annual Damages (B 10 ⁶)	
1. Unregu	lated			. •			
392	300.0	5.2	1.000	6.350	6.350	6.350	
429	200.0	7.5	1.000	9.900	9.900	16.250	
493	100.0	12.3					
531	66.7	16.6	0.333	14.450	4.812	21.062	
632	22.2	24.4	0.445	20.500	9.123	30.185	
			0.117	25.800	3.019	33.204	
696	10.5	27.2	0.072	28.700	2.066	35,270	
825	3.3	30.2	0.013	30.700	0.399	35.669	
869	2.0	31.2			•		
955	110	32.3	0.010	31.750	0.318	35.987	
1,016	0.5	33.1	0.005	32.700	0.164	36.151	
-			0.002	33.650	0.101	36.252	
1,122	0.2	34.2					
2. Regula	ted by Dok Kr						
	ted by lok Kr	ar Dam					
348	300.0	3.7	1.000	4.100	4.100	4.100	
375	200.0	4.5					
423	100.0	7.2	1.000	5.850	5.850	9.950	
452	66.7	8.8	0.333	8.000	2.664	12.614	
	:	. *	0.445	12.900	5.741	18,355	
534	22.2	17.0	0.117	19.200	2.246	20.601	
583	10.5	21.4	0.072	23.900	1.721	22.322	
677	3,3	26.4	•		1		
709	2.0	27.5	0.013	26.950	0.350	22.672	
: 774	1.0	: 20 - 3	0.010	28.400	0.284	22.956	
774	1.0	29.3	0.005	29.700	0.149	23.105	
819	0.5	30.1	0.003	30.800	0.092	23.197	
895	0.2	31.5	2.000				

(cont'd)

Table 66 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK E (2/2)

Floo	od charge	Expected Frequency Event per	Flood Damages	Events per Year within Interval	Average Damage per Interval	Average Annual Flood	Accumulated Average Annual
		100 years (%)	(8 10 ⁶)		(# 10 ⁶)	Damages (B 10°)	Damages (B 10 ⁶)
							•
3.	Regulat	ed by Dok Kra	i and Nong	Pla Lai Dams		v."	
	273	300.0	1.7	1.000	1.950	1.950	1.950
	295	200.0	2.2	1.000	2.750	2.750	4.700
	332	100.0	3.3	0.333	3.550	1.182	5.882
	352	66.7	3.8	0.445	5.400	2.403	8.285
	419	22.2	7.0	0.117	8.100	0.948	9.233
	455	10.5	9.2	0.072	12.100	0.871	10.104
	518	3.3	15.0	0.013	16.300	0.212	10.316
	539	2.0	17.6	0.010	19.500	0.195	10.511
	583	1.0	21.4	0.005	22.500	0.113	10.624
	616	0.5	23.6	0.003	24.800	0.074	10.698
	664	0.2	26.0		• .		
1.	Regulat	ed D <u>ok Krai,</u>	Nong Pla La	i and Khlong Ya	i Dams	1. P	
	245	300.0	1.2			: *	
	264	200.0	1.4	1.000	1.300	1.300	1.300
	299	100.0	2,3	1.000	1.850	1.850	3.150
	317	66.7	2.8	0.333	2.550	0.849	3.999
	373	22.2	4.6	0.445	3.700	1.647	5.646
	404	10.5	6.3	0.117	5.450	0.638	6.284
	466	3.3	9.9	0.072	8.100	0.583	6.867
	488	2.0	12.0	0.013	10.950	0.142	7.009
	529	1.0	16.4	0.010	14.200	0.142	7.151
		1 11		0.005	17.700	0.089	7.240
	554	0.5	19.0		and the second s		

Table 67 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK F

Flo	od scharge	Expected Frequency	Flood Damages	Events per Year within	Average Damage	Average Annual	Accumulated Average
DIS	charge	Event per 100 years		Interval	per Interval	Flood Damages	Annual Damages
		(%)	(3 10 ⁶)		(Ø 10 ⁶)	(B 10 ⁶)	(8 10 ⁶)
						•	
1.	Unregul	ated		1			
	107	300.0	2.80	•			
				1.000	3.250	3.250	3.250
	121	200.0	3.70	1.000	4.850	4.850	8.100
	145	100.0	6.00		* 4		
	159	66.7	7.26	0.333	6.630	2.208	10.308
	139	.00.7	7.20	0.445	8.820	3.925	14.233
	197	22.2	10.38	0.110	11.070	1 205	15 500
	222	10.5	11.76	0.117	11.070	1.295	15.528
				0.072	12.540	0.903	16.431
	263	3.3	13.32	0.013	13.590	0.177	16.608
	278	2.0	13.86	0,013	13.370	0.177	10.000
			14.50	0.010	14.190	0.142	16.750
	301	1.0	14.52	0.005	14.850	0.074	16.824
	322	0.5	15.18				
	354	0.2	15.84	0.003	15.510	0.047	16.871
	334	:	13.04	i.			
			m			-	
2.	Regulat	ed by Khlong	Thap Ma Dam				
	52	300.0	0.25		4		
	58	200.0	0.60	1.000	0.425	0.425	0.425
	, 30	200.0	.0.00	1.000	0.725	0.725	1.150
	66	100.0	0.85				
	71	66.7	1.10	0.333	1.550	0.516	1,666
		: .		0.445	1.350	0.601	2.267
	83	22.2	1.60	0.117	1.775	0.208	2,475
	91	10.5	1.95	0.117	1.775	0.200	2,475
	10 by 10 by			0.072	2.275	0.164	2.639
	104	3.3	2.60	0.013	2.850	0.037	2.676
	112	2.0	3.10				
	120	1.0	2.65	0.010	3.375	0.034	2.710
	120	1.0	3.65	0.005	3.950	0.020	2.730
	128	0.5	4.25				0.74
	140	0.2	5.35	0.003	4.800	0.014	2.744
		J. 2	J. J.J				

Table 68 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK G (1/3)

Flo	od charge	Expected Frequency Event per 100 years (%)	Flood Damages (B 19 ⁶)	Events per Year within Interval	Average Damage per Interval (# 10 ⁶)	Average Annual Flood Damages (B 10°)	Accumulated Average Annual Damages (B 10 ⁶)
1.	Unregul	ated					
			0.0				
	307	300.0	9.3	1.000	10.500	10.500	10.500
	332	200.0	11.7	1.000	15.600	15.600	26.100
	372	100.0	19.5				
	396	66.7	25.7	0.333	22.600	7.526	33.626
		22,2	37.1	0.445	31.400	13.973	47.599
	458			0.117	38.950	4.557	52.156
	495	10.5	40.8	0.072	42.350	3.049	55.205
	563	3.3	43.9	4			
	584	2.0	44.8	0.013	44.350	0.577	55.782
*	626	1.0	45.7	0.010	45.300	0.453	56.235
				0.005	46.050	0.230	56.465
	658	0.5	46.4	0.003	46.600	0.140	56.605
	689	0.2	46.8				
					e e		
2.	Regulat	ed by Dok Kra	i Dam				4
	292	300.0	7.7				
	313	200.0	10.0	1.000	8.850	8.850	8.850
				1.000	12.600	12.600	21.450
	350	100.0	15.2	0.333	17.250	5.744	27.194
	371	66.7	19.3	0.445	25.900	11.526	38.720
	425	22.2	32.5		•		
	459	10.5	37.3	0.117	34.900	4.083	42.803
	523	3.3	42.3	0.072	39.800	2.866	45.669
			•	0.013	42.750	0.556	46.225
	541	2.0	43.2	0.010	43.950	0.440	46.665
	579	1.0	44.7	0.005	45.150	0.226	46.891
	608	0.5	45.6			4.	
	640	0.2	46.3	0.003	45.950	0.138	47.029

(cont d)

Table 68 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK G (2/3)

loo	d harge	Expected Frequency Event per 100 years	Flood Damages	Events per Year within Interval	Average Damage per Interval	Average Annual Flood Damages	Accumulated Average Annual Damages
	- :	(3)	(8 10 ⁶)		(\$ 10 ⁶)	(3 10°)	(8 10 ⁶)
· .	Regulat	ed by Dok Kra	i and Nong	Pla Lai Dams		* .	
	265	300.0	5.3	1.000	6.200	6.200	6.200
	285	200.0	7.1	1.000	8.900	8.900	15.100
	318	100.0	10.7	0.333	12.150	4.046	19.146
	339	66.7	13.6				
	386	22.2	23.5	0.445	18.550	8.255	27.401
	418	10.5	31.3	0.117	27.400	3.206	30.607
	480	3.3	39.3	0.072	35.300	2.542	33.149
	495	2.0	40.8	0.013	40.050	0.521	33.670
			100	0.010	41.800	0.418	34.088
	530	1.0	42.8	0.005	43.200	0.216	34.304
	557	0.5	43.6	0.003	44.400	0.133	34.437
	593	0.2	45.2				
	Regulat	ed by Dok Kra	i, Nong Pla	Lai and Khlong	Yai Dams		
	255	300.0	4.5		5.050	5.050	5.250
	274	200.0	6.0	1.000	5.250	5.250	5.250
	304	100.0	9.1	1.000	7.550	7.550	12.800
	322	66.7	11.0	0.333	10.050	3.347	16.147
	362	22.2	17.2	0.445	14.100	6.275	22.422
	fire and			0.117	21.000	2.457	24.879
	392	10.5	24.8	0.072	30,350	2.185	27.064
	449	3,3	35.9	0.013	36.950	0.480	27.544
	466	2.0	38.0	0.010	39.500	0.395	27.939
	499	1.0	41.0	0.005	41.650	0.208	28,147
	523	0.5	42.3	0.003	43.050	0.129	28.276
				0.003	43.030	U. 123	40.410

(cont'd)

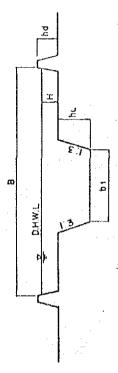
Table 68 RESIDUAL AVERAGE ANNUAL FLOOD DAMAGE FOR RAYONG RIVER, BLOCK G (3/3)

Flood	Expected	Flood	Events per	Average	Average	Accumulated
Discharge	Frequency	Damages	Year within	Damage	Annual	Average
	Event per	•	Interval	per Interval	Flood	Annual
	100 years	6.		(g 10 ⁶)	Damages	Damages
	(3)	(½ 10 ⁶)		(8 10)	(B 10°)	(8 10 ⁶)
	to a second					
. Regula	ted by Dok Kra	i, Nong Pla	Lai, Khlong Yai	and Khlong Tha	p Ma Dams	
248	300.0	2,3				
240	200.0	2,5	1.000	3.800	3.800	3,800
265	200.0	5.3	1.0.77		.*	
			1.000	6.500	6.500	10,300
292	100.0	7.7	1 to 1			
202	n		0.333	8.500	2.831	13.131
307	66.7	9.3	0.445	11.450	5.095	18.226
339	22.2	13.6	0.442	11.430	3.033	20.220
			0.117	16.100	1.884	20.110
367	10.5	18.6	•			
			0.072	25.050	1.804	21.914
420	3.3	31.5	0.013		0.426	22.340
436	2.0	34.1	0.013	32.800	0.426	22.340
430	2.0	34.1	0.010	36.500	0.365	22.705
474	1.0	28.9				
4	A 114		0.005	40.050	0.200	22.905
503	0.5	41.2		1820	.1-1.11	
C 1.5			0.003	42.350	0.127	23.032
545	0.2	43.5				

CHANNEL FACTORS FOR CHANNEL DESIGN OF RAYONG RIVER FOR CASE 1 (with K. Yai dam) Table 69

				t		-	;	-1
	Q	μì	H	മ്പ	r p	겁		ជ្រុ
(H ₃ /	/sec)	(Km)		(m)	(m)	(H)	(m)	(田)
				<u> </u> -				
7	450	٠.	1/2500	300	201.0		٦. د	
H	. 050		1/2500	150	58.0		3.2	
	650	•	1/1800	100	39.7	•	2.1	•
	490		1/900	001	13.6	•	1.7	
	280		1/900	100	10.5		8 O	
	130	•	1/900	50	9.4		9.0	
-	L30	•	1/520	20	ο 4		0.7	•
7	40	3.5	1/4500	200	0.06	1.0	4.2	5,2
w	60		1/1500	100	53.4		2.6	•
Ğ	. 09	3.0	1/600	100	34.0		ர ப	•
2,05	0		1/2500					
.:	0		1/2500	150	37.0		3.2	•
IU	30		1/1800	100	36.4		8	•
4	0		1/900	001	12.0		7.5	•
,	230		1/900	100	ഗ		9 0	•
·	95	2.8	1/900	20	7.4	3.0	0.4	J. 0
	95		1/520	20	7.4	•	4.0	•
	510		1/4500	200	74.0		4.2	
	550		1/1500	100	44.5		2.6	
	550		1/600	100	28.3		1.9	•

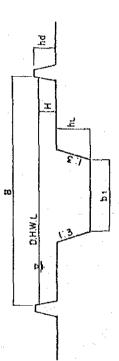
* ...K. Thap Ma River Note



CHANNEL FACTORS FOR CHANNEL DESIGN OF RAYONG RIVER FOR CASES 2 AND 3 (with K. Yai and Thap Ma dams) Table 70

	ļ									_						3.	1.34	-			2.0			
pq	Œ			4.2	•	2.5				5.0					4.2	•	2.3		•		0.0	•	• 1	
н	(田)		1.7	3.2	2.1	1.7	0	9.0	0.7	4.2	6.0	0.5		1.7	3.2	9	1.5	9-0	0.4	4.0	4.2	0.7	0.4	
Ţų.	(m)		4.0	2.6	9,6	4.0	4.2	3.0	2.5	о. Н	1.8	2,0			2.6	3.6		4.2			1.0		•	
p ¹	(田)		136.0	58.0	39.7	13.6	10.5	9.4	4.	28.0	11.4	7.6		110.0	37.0	36.4	12.0	5 6	7.4	7.4	22.0	٠. د.	6.8	
Я	(田)		250	150	100	100	100	50	50	100	50	20		250	150	100	100	100	50	50	100	20	50	
н	i		1/2,500	1/2,500	1/1,800	1/900	1/900	1/900	1/520	1/4,500	1/1,500	1/600		1/2,500	1/2,500	1/1,800	1/900	1/900	1/900	1/520	1/4,500	1/1,500	1/600	
ᆈ	(Km)		3.6	15.5	5.2	1.2	2.4	2.8	5.2	ი გ.	<u>4</u> .	м			15.5				2.8		ស	4 .0	3.5	
Ø	(m3/sec)		1,850	1,050	650	490	280	130	130	270	80	08		ıΩ	850	m	0	230	ტ ე	-95	220	09	60	7 6
)- 13.40	- 32.55	- 40.85	- 42.80	- 47.00	- 50.70	- 55.90	3.50	8.00	- 11.05)- 13.40	- 32,55	- 40.85	- 42.80	- 47.00	- 50,70	- 55.90	- 3.50	8.00	- 11.05	77
Stretches		30-yr	0.0 (New)	13.40	32.55	40.85	42.80	47.00	50.70	00.00	3.50	8.00	10-yr	0.0 (New)-	13.40	32.55	40.85	42.80	47.00	50.70	00.00	3.50	8.00	
St		ထိ	Н	II.	III	ΔI	٥	IA	VII	VIII*	****	*		1-4	HH	III	ΔI	٥	HD	NII	*IIIA	*× H	××	;

Note * ... K. Thap Ma River



WORK QUANTITY AND CONSTRUCTION COST OF RIVER IMPROVEMENT WORKS OF RAYONG RIVER, CASE 1 AND RISK LEVEL 1 Table 71

Remarks \$ 1 = \(\frac{4}{2}\) 23 4,400 2,493 4,026 4,600 25,018 × 103 16,367 26,190 10,346 49,400 233,809 167,196 442,390 95,064 37,290 275,194 66,359 76,313 Amount 9 485.0 366.0 23.3 23.3 7.1 Unit Cost i, 230,000 6,500 347,600 1,967 11,499 2,335 436 2,031 x 10³ 805 169 837 480 2,850 1,231 13,535 Amount 13,535 F.C (US\$) 1.08 0.58 Unit Cost 1.08 0.48 15.5 21,800 4,080,000 54,000 20 400,000 481 351,000 444,000 7,600 Quantity Unit e E E, S ក្នុង nos. 33 m 27 E 2 ц Eng. Service and Administration Miscellaneous (1+9) × 10% Contingency (A + B + C) × 15 % B. Acquisition and Compensation - High water channel Preparation $(9 \times 7 \%)$ - Low water channel Sub-total (2 ~ 8) Drainage sluice Backwater levee Main Civil Works Total (A + B) - Excavation - Embankment Embankment Excavation $(A + B) \times 15 %$ Revetment Disposal Item Bridge Q+ 5. യ് ത്

Ø 996,785,000

585,062

17,901

Grand Total

å

ပံ

ď,

WORK QUANTITY AND CONSTRUCTION COST OF RIVER IMPROVEMENT WORKS OF RAYONG RIVER, CASE 1 AND RISK LEVEL 2 Table 72

Item	Unit	Quantity	16.1	(\$8)	D.	(82)	Remarks
			UNIT COST	Amount	Unit Cost	Amount	
Main Civil Works				× 103		x 103	
Drenaration (9 x 7 %)	۲-	1	1	9		יי ני ני	
The state of the s	3 ec	1. C) () ()	c c		
EXCAVATION	, « ≅	000,056,6	ο . - Ι	5/2/5	23.3	178,849	
. Embankment	ສ໌	3,780,000	0.58	2,193	11.0	41,580	
. Disposal	Ë	1,372,000	0.48	629	7.1	9,742	
. Revetment							
- Low water channel	m ²	54,000	15.5	837	485.0	26,190	-
- High water channel	щ2	11,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos	20	21,800	436	230,000	4,600	
. Backwater levee							
- Excavation	ლ [450,000	1.08	486	23,3	10,485	
- Embankment	e E	405,000	0.58	235	11.0	4,455	
8. Bridge	щ ₂	7,600	375	2,850	6,500	49,400	
9. Sub-total (2 ~ 8)				13,790		279,327	
10. Miscellaneous (1+9) ×10%	ы. Э	•	1	1,476	1	29,888	
Total		e e		16,232		328,768	
Acquisition and Compensation	'nа	474	1	ı	347,600	164,763	
Total (A + B)	:			16,232		493,531	
Eng. Service and Administration (A + B) ×15%				2,435		74,030	
D. Contingency (A + B + C) × 15%				2,801		85,135	
Grand Total				21,468		652,696	¥ 1,146,460,000

WORK QUANTITY AND CONSTRUCTION COST OF RIVER IMPROVEMENT WORKS OF RAYONG RIVER, CASE 2 AND RISK LEVEL 1 Table 73

\$ 1 = ¥ 250 = 第 23

	4.47.	1 1 1 1 1 1	F.C (US\$)	3\$)	I.C (B	3)	
דכמוו	OIII C	אַרים ריי ראַ	Unit Cost	Amount	Unit Cost	Amount	кепаткѕ
				x 103		× 103	
A. Main Civil Works	:			2) 	
1. Preparation (9 × 7%)	S.T	ı	!	717		14,530	
2. Excavation	m ₃	3,340,000	1.08	3,607	23.3	77,822	
3. Embankment	E H	2,421,000	0.58	1,404	11.0	26,631	
4. Disposal	က ရ	585,000	0.48	281	7.1	4,154	
5. Revetment	. (
- Low water channel	m ²	54,000	15.5	837	485.0	26,190	
- High water channel	п 2	11,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos.	20	21,800	436	230,000	4,600	
7. Backwater levee	,						
- Excavation	E H	444,000	1.08	480	23.3	10,345	
- Embankment	e E	400,000	0.58	232	0.11	4,400	
8. Bridge	m2	7,600	375	2,850	6,500	49,400	
9. Sub-total (2 ∿ 8)				10,248		207,568	
10. Miscellaneous (1+9) × 10 %	អ ស	ı	1	1,095	ı	22,212	
Total				12,060		244,310	
				÷			
B. Acquisition and Compensation	ha	383	ı	i	347,600	133,130	
Total (A + B)	A		. :	12,060		377,440	
C. Eng. Service and Administration	4		:	1.810	:	56.620	
$(A + B) \times 15 $	ja Ja	· ·	:				<i>.</i>
			л.				
D. Contingency (A + B + C) \times 15 %		-		2,080		65,110	
Grand Total				15,950		499,170	¥ 866,020,000

Table 74 WORK QUANTITY AND CONSTRUCTION COST OF RIVER IMPROVEMENT WORKS OF RAYONG RIVER, CASE 2 AND RISK LEVEL 2

\$1= \pm 250 = \pm 23

			(2011) 7 0	(۵)	(A) U 1	(2)	
Item	Unit	Quantity	Unit Cost	Amount	Unit Cost	Amount	Remarks
	: .			x 103		x 103	
A. Main Civil Works				1) H	
1. Preparation (9 × 7%)	ง เป	Í.	1	779		15,868	
2. Excavation	e E	3,832,000	1.08	4,139	23.3	89,286	
3. Embankment	e E	3,462,000	0.58	2,008	11.0	38,082	
4. Disposal	E III	23,000	0.48	11	7.1	163	
5. Revetment						-	
- Low water channel	m ²	54,000	15.5	837	485.0	26,190	
- High water channel	m ²	11,000	11.0	121	366.0	4,026	
6. Drainage sluice	nos.	20	21,800	436	230,000	4,600	
7. Backwater levee	,	.:					
- Excavation	က (H	450,000	1.08	486	23.3	10,485	
- Embankment	e H	405,000	0.58	235	11.0	4,455	
8. Bridge	m ₂	7,600	375	2,850	6,500	49,400	-
9. Sub-total (2 ∿ 8)				11,123		226,687	
10. Miscellaneous $(1+9) \times 10$ %	L.S	1	1	1,188	ţ	24,255	
Total				13,090		266,810	
B. Acquisition and Compensation	, ਸੂਬ	386	1	ı	347,600	134,174	
Total (A + B)				13,090		400,984	
C. Eng. Service and Administration (A + B) × 15 %				1,960		60,146	
D. Contingency (A + B + C) \times 15 %		·		2,260		69,170	
Grand Total				17,310		530,300	¥ 928,430,000

WORK QUANTITY AND CONSTRUCTION COST OF RIVER IMPROVEMENT WORKS OF KHIONG THAP MA RIVER, CASE 3 AND RISK LEVER 1 Table 75

				-	-	S. II	¥250 = JS 23
Item	Unit	t Quantity	F.C (((USS)	Unit Cost	(B) Amount	Remarks
				× 303		× 103	
A. Main civil works							
1. Preparation (9. x 7%)	L.S		i.	95		1,867	
2. Excavation	E	200,000	1.08	216	23.3	4,660	
3. Embankment	e E	800,000	0.58	464	11.0	8,800	
4. Disposal	m ³	0	0.48	0	7.1	0	
5. Revetment							
- Low water channel	m ²	008'6	15.5	152	485.0	4,753	
- High water channel	m ²	1,600	11.0	18	366,0	586	:
6. Drainage sluice	nos	4	21,800	87	230,000	920	
7. Backwater levee			-				
- Excavation	e E	92,000	1.08	<u>თ</u>	23.3	2,144	
- Embankment	m3	82,000	0.58	48	11.0	902	
8. Bridge	m ²	909	375	225	6,500	3,900	
	-			1,309		26,665	
	8 L.S	í	, i	140	1	2,853	
Total				1,541		31,385	
B. Acquisition and Compensation	n ha	09	l,	i	347,600	20,856	
A. + B.				1,541	:	52,241	
C. Eng. service and Administration	tion (A+B	B) x 15%		232		7,836	
D. Contingency (A+B+C) x 15%				266		9,012	
Grand Total				2,039	٠.	680,69	W 115,986,000
				4			

WORK QUANTITY AND CONSTRUCTION COST OF RIVER IMPROVEMENT WORKS OF KHLONG THAP MA RIVER, CASE 3 AND RISK LEVEL 2 Table 76

				-		.¥ = ₹\$	¥250 = B 23
7 + 7	1 , 41,	4	P. C. (C	(\$\$0)	1.0	. (対)	
LECH	י לחט	guan cu cy	Unit Cost	Amount	Unit Cost	Amount	השנוומדאי
		i.		x 103		× 103	
A. Main CIVIL WORKS			-				
1. Preparation (9. x 7%)	ប្ត	1	1	96		1,950	
2. Excavation	E III	218,000	1.08	236	23.3	5,080	
3. Embankment	E	870,000	0.58	505	11.0	9,570	
4. Disposal	E	0	0.48	0	7.1	0	
5. Revetment							19
- Low water channel	m ²	98,000	15.5	152	485.0	4,753	
- High water channel	m ²	1,600	11.0	18	366.0	586	
6. Drainage sluice	nos.	4	21,800	87	230,000	920	
7. Backwater levee							
- Excavation	E E	92,000	1.08	<u>ი</u>	23.3	2,144	
- Embankment	m ₃	82,000	0.58	48	11.0	902	
8. Bridge	m2	009	375	225	6,500	3,900	
9. Sub-total (2 ∿ 8)			:	1,370		27,855	
10. Miscellaneous (1+9) × 10%	S L	ı	1	147	ı	2,980	
				1,613	÷	32,785	
B. Acquisition and Compensation	Ц	09	t,		347,600	20,856	
A. + B.				1,613		53,641	
C. Eng. service and Administration	m (A+B)	× 15%		242		8,046	
D. Contingency (A+B+C) × 15%	:			279		9,253	
Grand Total			.*	2,134		70,940	Z 120,022,000

