

Max. Hourly Rainfall of 1972 Flood

Sta. Cruz, Porac

Flood Hour	1972 June		1972 July		1972 Aug.	
	r (mm)	rt(mm/hr)	r (mm)	rt(mm/hr)	r (mm)	rt(mm/hr)
1 hr	42.5	42.5	37.4	374.4	24.0	24.0
2 hr	56.5	28.3	54.0	27.0	32.0	16.0
3 hr	60.0	20.0	62.2	22.4	43.0	14.3
6 hr	60.0	10.0	93.0	31.0	51.0	8.5

Therefore, the cause for the damage is attributed not to the flood caused by floods with a high intensity if hourly rainfall but to gradual increase of inundation to lowlands due to deficiency of discharge capacity of the Pasig-Potrero River channel at the stream together with successive in flow by long-term rainfall.

4-4 Area Mean Rainfall

Generally, the area mean rainfall is obtained by Tissen's method or Isohyetal method.

However, the good correlation if rainfall studied in Item 4-2 can conclude that the observed rainfall at the Porac station can presents the mean rainfall over the Pasig Potrero basin correctly.

4-5 Flood Records and Reports

Daily mean water level records at Cabetican Bacolor and daily rainfall records at Porac during five noticeable floods experienced in May of 1966, September of 1970, July of 1972, August of 1974 and May of 1976 are summarized in FIGURE I-18 as a hydrograph and hyetgraph.

The figures show that the hydrograph and the hyetograph in each flood correspond well to each other and their time lags between peak in the hydrograph and that in the hyetograph take place within a day in most cases. Futhermore, sharp depletion curves of hydrographs after the peak discharge is characterized as a nature of this river.

Other flood records in the Pasig Potrero River are also collected from flood reports at the Apalit office and shown in the Reference Material attached to the end of this report.

4-6 Water Balance

The water balance between annual discharge and annual rainfall is studied to check the reliability of discharge records at 4 stations Sachas Der Carmen, Valdez, HDA Dolores and Cabetican Bacolor.

Fig. I-18-1 Daily Rainfall and Water Stage in 1966 Flood

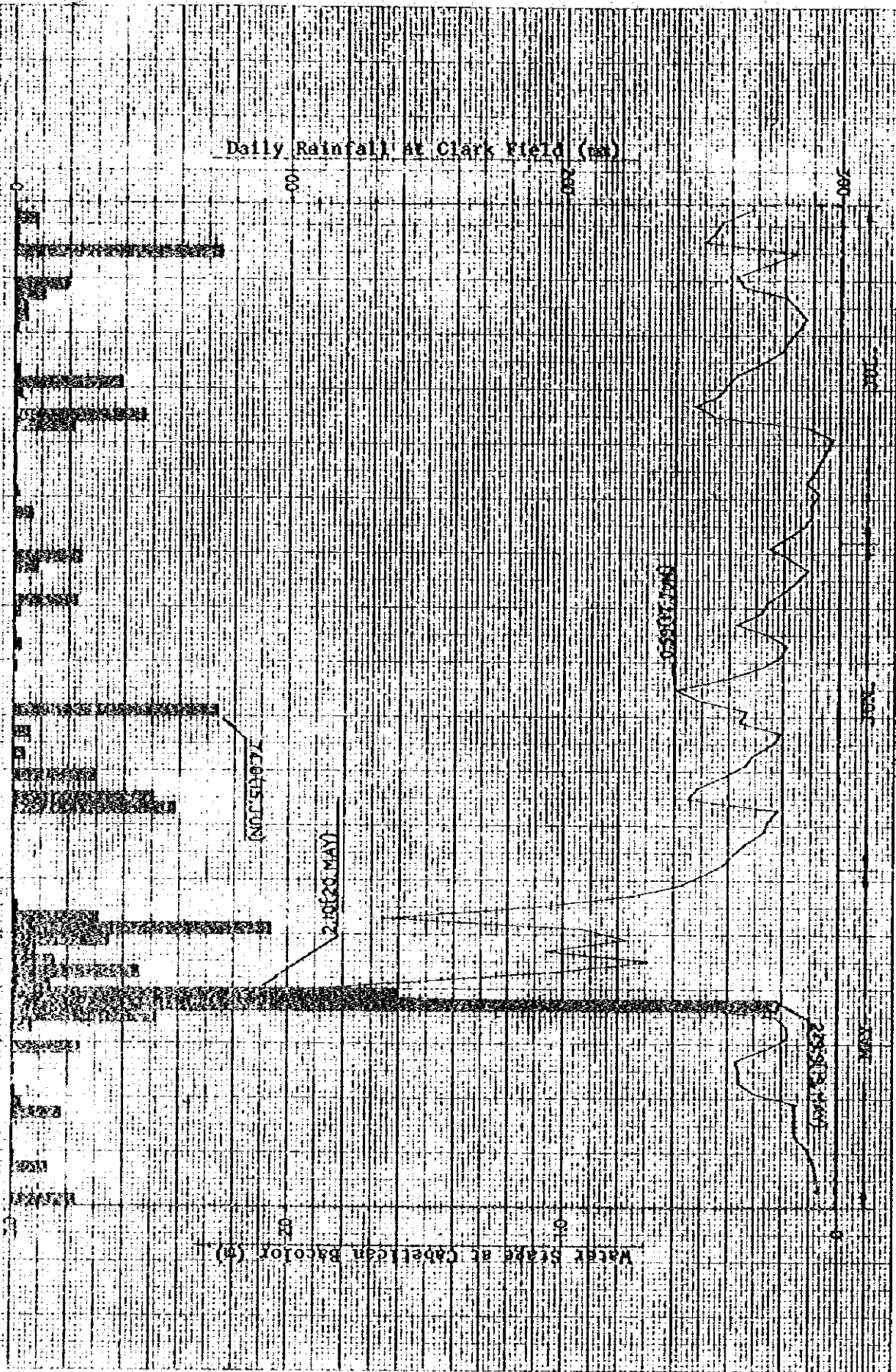


Fig. 1-18-2 Daily Rainfall and Water Stage in 1970 Flood

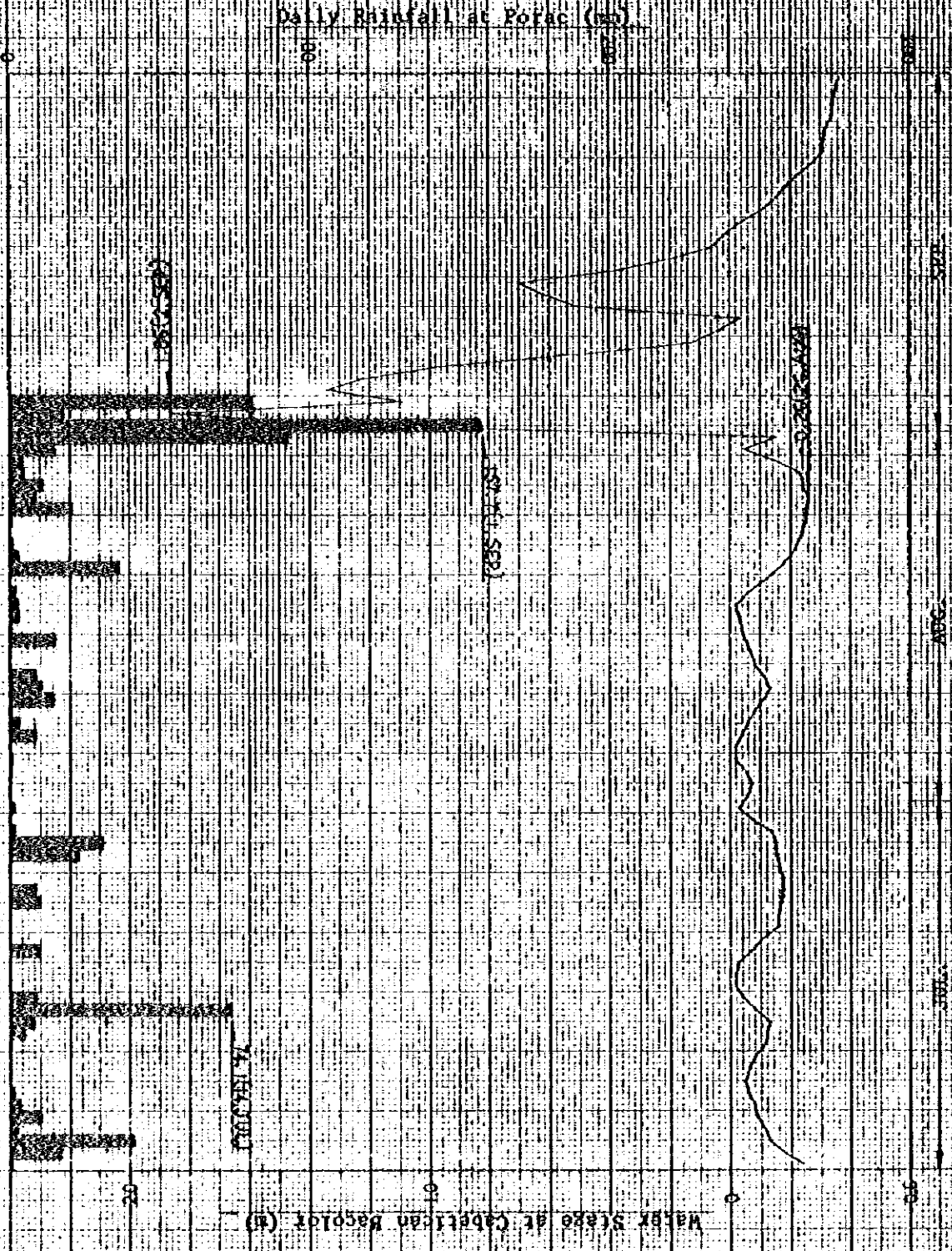
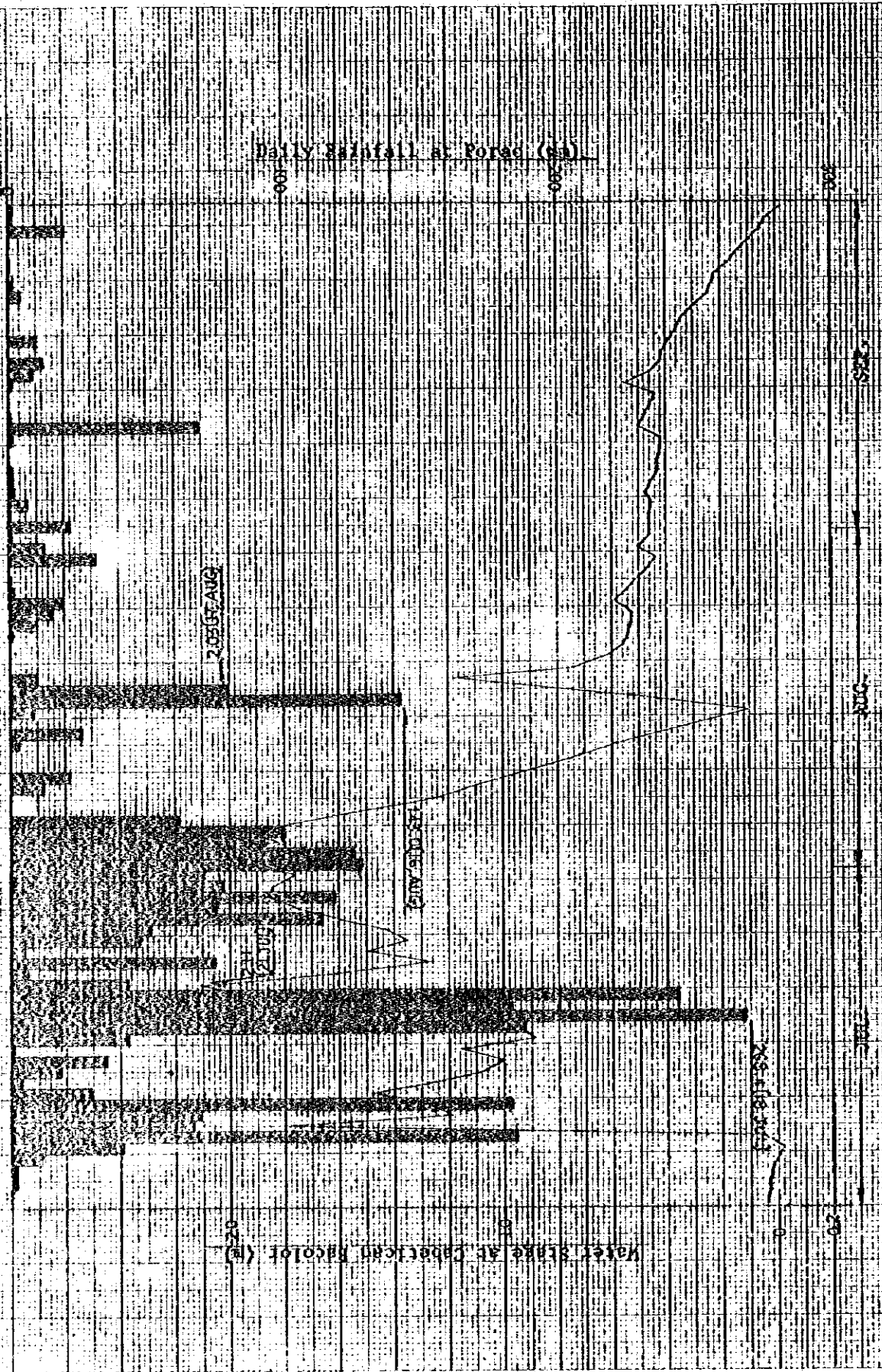


Fig. 1-18-3. Daily Rainfall and Water Stage in 1972 Flood



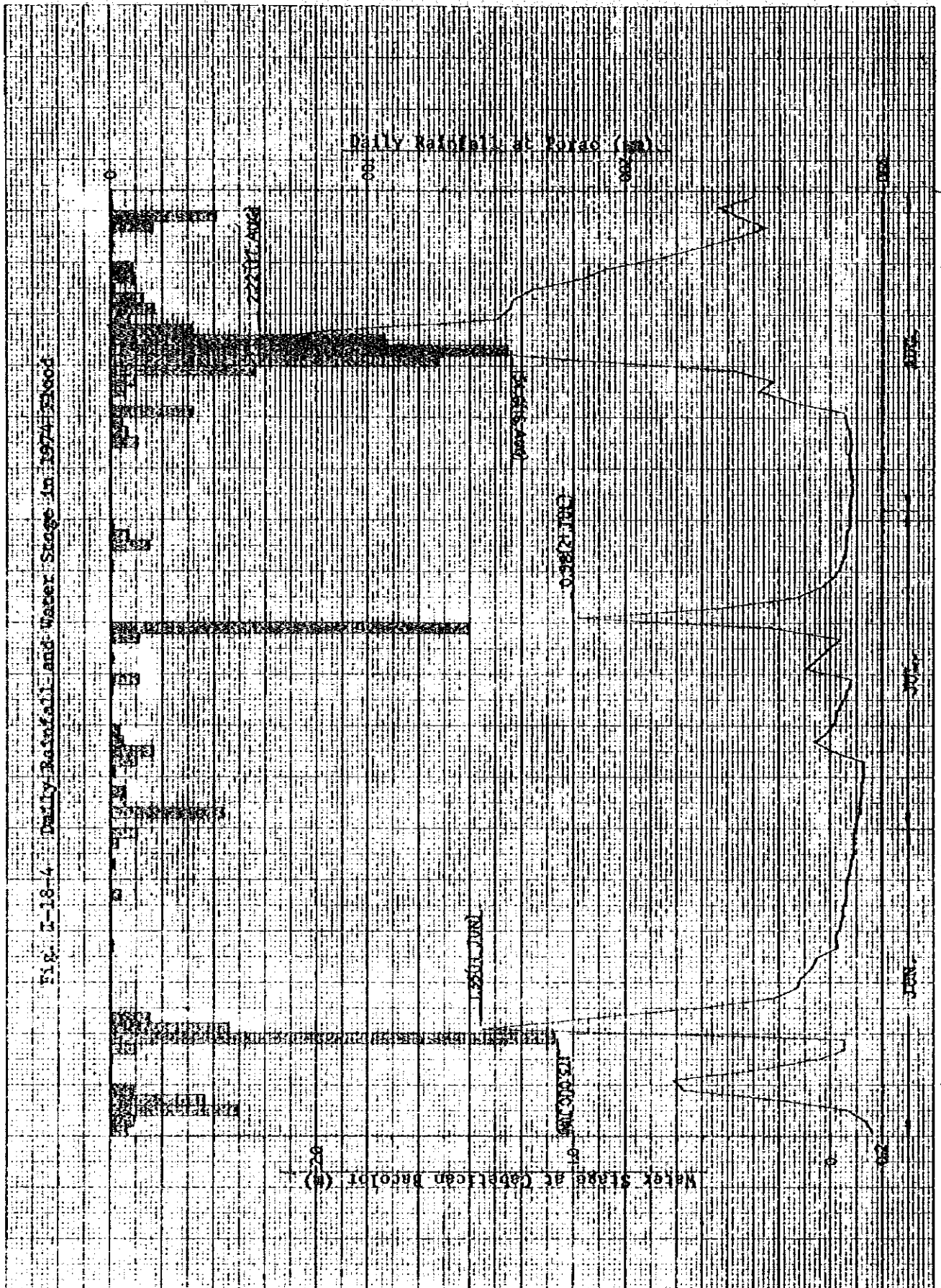


Fig. T-18-4 Daily Rainfall and Water Stage in 1974

Fig. II-18-5 Daily Rainfall and Water Stage in 1976 Flood

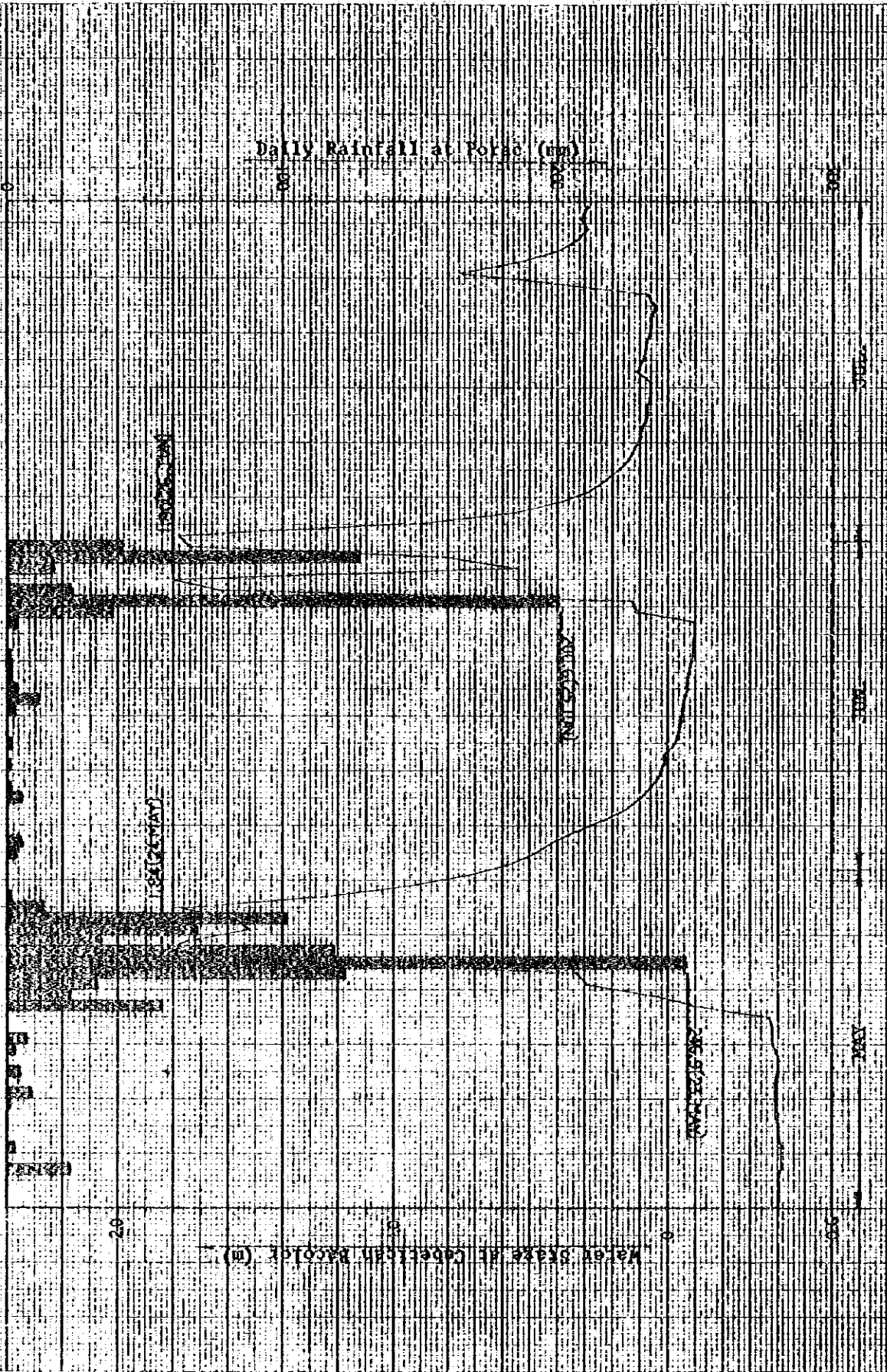


TABLE I-14 ANNUAL MAXIMUM WATER STAGE AND DISCHARGE

Item	Porac River						Pasig - Potrero River								
	Der Corman			Valdez			HDA Bdores			Cabetican Bacolor					
	Date	Hmax	Qmax	Date	Hmax	Qmax	Date	Hmax	Qmax	Date	Hmax	Qmax			
1977															
76										May 24	1.84				
75										Aug. 10	1.42				
74				Aug. 17	7:00	4.00	132,300			Aug. 17	17:00	2.24			
73				Oct. 17	7:00	2.55	21,750			Oct. 16	17:00	1.88			
72	May 31	12:00	3.44	(12,400)	Jul. 9	17:00	6.30	366,900	Jul. 25	7:00	2.00	8,860	Jul. 21	7:00	2.20
71	May 22	18:00	3.98	43,380	Jul. 26	7:00	3.15	56,000	Jul. 26	17:00	1.98	8,630	Jul. 24	17:00	1.54
70	Sep. 2	12:00	5.00	146,000	Sep. 2	12:00	3.50	82,600	Aug. 31	7:00	1.32	2,220	Sep. 2	7:00	2.00
69	Sep. 6	6:00	4.28	224,600	Aug. 6	5:00	2.98	44,640	Aug. 6	7:00	1.75	6,010	Aug. 7		1.34
68	Aug. 31	13:00	3.90	35,900	Aug. 28	17:00	2.86	37,360	Aug. 29	17:00	2.00	8,860	Aug. 29	18:00	1.92
67	Aug. 2	6:00	5.59	224,600	Jul. 30	7:00	4.20	152,700	Nov. 5	7:00	1.85	7,150	Jun. 8	18:00	2.12
66	Sep. 9	18:00	5.62	227,400	May 20	7:00	5.85	321,000	Nov. 21	17:00	1.60	4,300	May 20	12:00	2.16
65	Jul. 28	17:00	4.30	134,000	Jul. 14	3:00	5.20	254,700					Jul. 14	17:00	1.66
64	Jul. 1	12:00	4.81	124,100	Jun. 30	7:00	5.94	330,180							
63	Aug. 15	18:00	4.06	54,800	Jun. 28	17:00	5.97	333,240							
62	Jul. 24	18:00	6.80	400,000	Sep. 19	12:00	3.00	30,300							
61	Jul. 7	18:00	3.99	67,600	Jul. 7		4.22	141,480							
60	Aug. 13	17:00	3.86	128,500	Aug. 9	17:00	4.35	168,000							
59	Aug. 30	5:00	5.36	21,300	Aug. 6	12:00	2.93	23,160							
58	Jul. 14	7:00	4.70	6,930											
57	Aug. 16	5:00	4.30	5,450											

Hmax : Maximum Water Stage. Drainage Area : Der Carmen A= 111 Km, Valdez A= 118 Km²
 Qmax : Maximum Discharge. HDA Dolores A= 28 Km, Ca. Bacolor A= 242 Km²

Fig. I-19 MONTHLY MEAN DISCHARGE

Station	River	A (km ²)
1. Per. Carmelo	Porac	111.0
2. Valdey	Porac	118.0
3. SDA Dolores	Pasig-Poureko	28.0
4. Ca. Bacolor	Pasig-Poureko	242.0

Discharge (CFS)

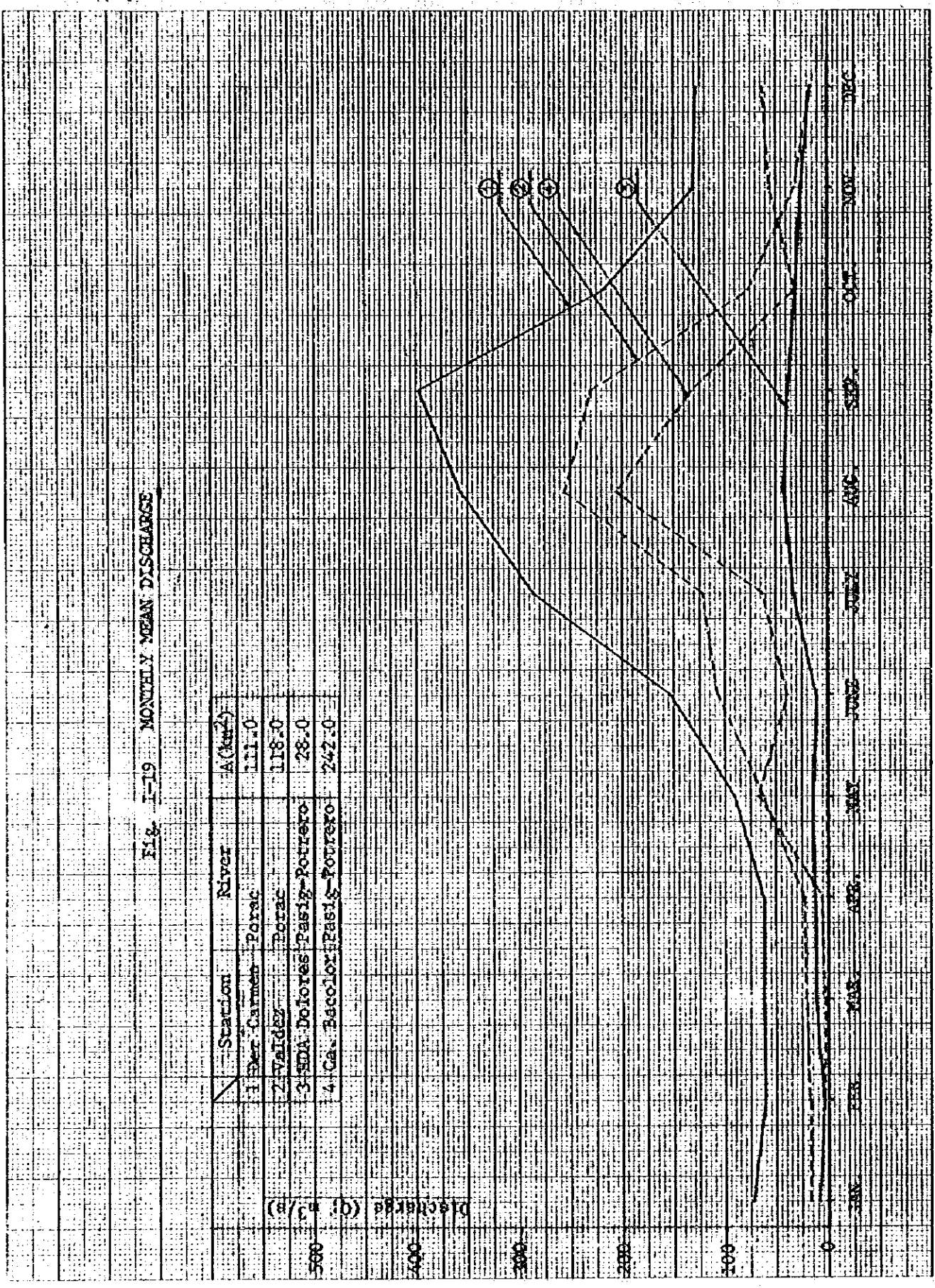


TABLE - I-5-1 ANNUAL MONTHLY TOTAL RUNOFF DISCHARGE (1)

DER CARMEN, PORAC

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
1977													
6													
5													
4													
3													
2	151.25	106.60	137.20	161.45	2580								
1	0.78	10.30	36.80	37.30	76.18	40.60	141.50	7.40	63.50	226.15	288.90	520.90	1250.21
0	91.50	26.75	23.45	68.80	47.70	23.29	260.56	314.07	967.86	382.88	247.20	537.40	2893.45
1969	68.50	122.75	127.00	177.90	167.30	206.60	453.96	592.63	564.88	388.69	20.10	16.45	2916.76
8	183.90	179.52	191.50	145.30	314.32	110.40	121.95	487.84	904.28	332.99	36.90	96.20	3106.10
7	57.25	28.15	28.35	23.90	27.20	564.61	266.40	1034.54	196.37	168.30	178.63	53.65	2627.35
6	76.60	67.45	73.15	69.60	399.25	265.55	216.40	390.20	1334.22	146.28	266.57	111.60	3416.87
5	73.45	58.75	55.56	38.08	50.75	134.95	521.80	174.05	148.75	212.30	131.50	91.50	1691.44
4	64.05	71.20	62.32	39.33	42.83	185.09	240.51	354.00	205.30	239.15	148.05	125.35	1777.18
3	66.65	48.50	44.22	34.57	33.78	198.99	170.05	234.93	244.11	154.95	77.10	84.80	1392.65
2	41.70	38.69	49.16	43.85	43.12	524.8	1128.91	95.15	292.55	92.50	82.40	77.37	2037.88
1	38.78	78.04	27.62	23.50	25.91	129.33	302.47	159.28	201.35	105.20	83.92	69.41	1244.81
0	23.65	25.13	30.97	30.97	27.41	150.35	48.15	979.59	328.30	437.35	126.46	90.25	(2267.61)
1959	47.36	27.70	30.30	23.06	52.76	44.84	52.66	121.98	153.17	104.26	89.68	82.60	830.37
8	54.03	14.36	23.96	14.77	16.14	39.92	63.61	49.71	53.91	69.97	57.32	58.90	516.60
TOTAL	1015.80	902.41	937.72	932.38	1350.45	214.70	3998.92	4995.37	5559.55	3060.97	1834.63	1816.38	28551.68
MEAN	72.56	60.16	62.51	62.16	90.03	153.35	285.63	356.8	397.11	218.64	131.05	129.74	2019.74

UNIT : m³/s

TABLE I-5-2 ANNUAL MONTHLY TOTAL RUNOFF DISCHARGE (2)

VALDEZ, PORAC

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
1977													
6													
5	9.42	3.64	8.97	8.14	8.38	95.75	12.67	61.95	86.30	68.55	59.20	11.60	434.57
4	10.00	2.46	4.57	7.24	10.88	71.76	12.04	196.15	10.00	34.82	26.60	11.00	397.60
3	9.10	7.56			5.38	4.95	6.53	8.25	6.43	59.18	7.58	7.29	(122.17)
2	7.31	8.03	9.37	9.29	11.28	9.56	128.68	240.69	42.89	7.20	7.55	7.69	489.54
1	75.22	19.68	28.80	21.29	23.18	26.81	104.04	12.19	12.66	79.77	25.20	28.73	456.27
0	30.72	36.50	31.20	26.24	25.39	31.87	30.05	28.32	123.30	23.72	36.42	40.00	465.92
1969	7.76	10.43	11.50	47.70	40.50	4.37	20.47	99.92	26.53	25.73	27.27	34.19	356.37
8	25.47	39.50	37.37	21.85	15.63	15.24	18.84	116.60	196.34	15.25	9.08	5.37	516.24
7	4.28	13.81	15.63	18.73	11.83	183.15	25.873	431.29	371.88	276.76	178.34	22.45	1796.88
6	42.15	51.67	54.75	44.39	802.74			531.32	670.90	26.28	38.39	11.11	(2273.73)
5	17.13	18.04	24.86	20.57	11.42	20.58	431.90	128.91	137.28	90.43	37.79	27.85	966.76
4	11.21	21.41	21.13	18.39	8.97	334.95	231.89	481.56	172.87	177.09	9.66	20.99	1510.06
3	17.71	18.39	19.23	19.74	21.32	693.14	179.03	364.29	170.46	55.31	17.80	6.31	2582.75
2	18.62	19.56	25.83	37.85	35.70	25.22							
1	17.85	20.11	20.24	19.85	18.69	81.68	63.451	35.457	255.78	95.84	12.23	16.99	(249.34)
0	6.20	12.67	15.74	14.28	8.34	91.89	64.72	981.68	288.03	192.91	26.60	13.37	1721.43
1959	6.61	9.35	8.93	9.24	6.97	3.80	5.91	48.32	119.36	33.46	20.20	6.13	279.38
8													
TOTAL	316.87	312.76	337.72	344.79	1066.56	1694.72	2139.81	4087.21	3696.13	1301.93	562.07	294.59	16175.16
MEAN	18.64	18.40	21.11	21.55	62.74	105.92	142.65	255.45	231.01	76.58	32.34	16.37	1002.76

UNIT : m³/s

TABLE I-15-3 ANNUAL MONTHLY TOTAL RUNOFF DISCHARGE (3)

HDA DOLORES, PASIG-POTRERO

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
1977													
6													
5													
4													
3													
2	8.50	7.09	7.45	6.88	8.67	11.60	25.35						
1	1.15	1.29	1.87	2.52	6.02	6.02	89.10	0.8	36.07	68.45	31.39	43.43	286.58
0	1.08	0.73	1.43	2.35	3.81	3.81	23.01	40.50	14.06	0.95	0.92	0.84	114.20
1969	5.02	2.22	2.36	2.47	3.64	3.64	13.95	28.22	9.22	2.47	1.14	1.01	74.73
8	11.92	9.04	16.03	25.49	36.63		17.23	55.87	96.94	41.52	27.77	4.86	(343.00)
7	26.31		31.68	29.33	28.29	28.29		95.12	38.99	38.40	54.68	29.37	(362.36)
6										23.74	38.75	29.19	
5													
4													
3													
2													
1													
0													
1959													
8													
TOTAL	53.69	20.37	60.92	69.24	87.06	43.72	168.54	210.51	195.27	180.53	154.15	10.47	1345.59
MEAN	8.95	4.07	10.14	11.54	14.51	10.93	33.71	42.10	39.05	30.09	25.77	17.45	248.31

UNIT : m³/s

TABLE I-15-1 ANNUAL MONTHLY TOTAL RUNOFF DISCHARGE (4)

CABETICAN BACOLOR, PASIG—POTRERO

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
1977													
6													
5													
4													
3													
2													
1													
0													
1969	5.50	2.42	0.60	0.46	2.95	1.84	77.92	164.5	60.32	28.47	12.20	5.66	362.84
8	16.95	7.46	4.44	14.33	7.91	7.44	36.82	200.69	145.84	56.48	2.78	1.56	502.70
7	1.33	1.40	2.53	2.60	7.98	119.72	65.96	521.04		30.01	126.45	21.49	(900.41)
6	6.12				287.24	40.58	30.48	104.19	273.43	9.78	90.45	60.77	(903.04)
5					26.27	27.77	104.98	35.78	58.13	34.74	52.11	106.12	(445.90)
4													
3													
2													
1													
0													
1959													
6													
TOTAL	29.9	11.28	7.57	17.39	332.25	197.55	316.16	1026.2	537.72	159.48	283.99	195.6	3114.89
MEAN	7.48	3.76	2.52	5.80	66.45	39.47	63.23	205.24	134.43	31.90	56.80	39.12	656.20

UNIT : m³/s

TENTATIVE WATER BALANCE OF THE UPPER PAMPANGA RIVER
ENTERING PROJECT AREA NEAR ALAYAT III.

	Q_n -CUSE ($10^9 m^3$)	CUSE ($10^9 m^3$)	Q_{nat} ($10^9 m^3$)	Q_{base} ($10^9 m^3$)	Q_{flood} ($10^9 m^3$)	Q_b/Q_f (%)	Precip. (M.M.)	R.O (M.M.)	$f = \frac{RO}{Prec.}$
1960	9.94	0.98	10.92	4.07	6.85	0.59	2277.	1960	0.74
1961	8.69	1.28	9.97	3.94	6.03	0.65	2175.	1540	0.71
1962	9.16	1.27	8.43	3.78	4.65	0.81	1781.	1303	0.73
1963	7.96	0.91	8.87	3.85	5.02	0.64	1755.	1371	0.78
1964	8.92	0.99	9.91	4.05	5.86	0.70	2369.	1531	0.65
1965	7.69	0.83	8.52	3.32	5.20	0.64	2024.	1318	0.65
1966	8.96	0.70	9.66	3.71	5.95	0.62	2564.	1497	0.58
1967	9.77	1.08	10.	3.97	6.88	0.58	1836.(?)	1680	0.92(?)
1968	6.40	1.40	7.80	3.06	4.74	0.64	1628.	1208	0.74
1969	4.92	0.81	5.73	3.10	2.33	1.52	1501.	885	0.59
1970	7.80	0.93	8.73	3.47	4.26	0.82	1305(?)	1350(?)	(?)
1971	10.45	0.60	11.05	1.23	6.82	0.62	2006.	1703	0.85
1972	12.61	0.94	13.55	5.35	8.20	0.65	2326(?)	2090(?)	0.90(?)
Average	8.47	0.98	9.45	3.86	5.59	0.69	2069.	1470	(?)

TABLE I-16-1 ANNUAL WATER BALANCE (1)

	RIVER : PORAC		Q : VALDEX, R : PORAC		REMARKS	
	①	②	③	④		⑤
	RUN-OFF-DISCHARGE ΣQ (m ³ /s)	RUN-OFF VOLUME $86,400 \Sigma Q$ (m ³)	RAINFALL ΣR (mm)	DRAINAGE AREA A (km ²)	RAINFALL VOLUME $A \Sigma R$ (m ³)	RUN-OFF COEFFICIENT $F = \frac{②}{⑤}$
1977				111		
76			(2345.0)	"	260.3 X 10 ⁶	
75			1417.3	"	157.3	
74			(2408.3)	"	267.3	
73			(1452.2)	"	161.1	
72			4092.0	"	454.2	
71	1250.21	108.0 X 10 ⁶	(1914.0)	"	2125	(0.508)
1970	2893.45	250.0	-	"		
69	2916.76	252.0	-	"		
68	3106.10	268.3		"		
67	2627.44	227.0		"		
66	3416.87	295.2		"		
65	1691.44	146.1		"		
64	1777.18	153.5		"		
63	1392.65	120.3		"		
62	2037.88	176.1		"		
61	1244.81	107.6		"		
1960	2267.61	195.9		"		
59	830.37	71.7		"		
58	516.60	44.6		"		
MEAN	2019.74	174.5 X 10 ⁶	2271.5		252.1 X 10 ⁶	0.69

* Q AND R ARE THE DISCHARGE OBSERVATION STATION AND THE RAINFALL OBSERVATION STATION.

** MEAN VALUE WAS ADOPTED FROM ANNUAL MONTHLY RUN-OFF DISCHARGE (TABLE I-15) AND ANNUAL MONTHLY RAINFALL (FIGURE I-15).

TABLE I-16-2 ANNUAL WATER BALANCE (2)

RIVER : PORAC

Q : VALDEX ; R : PORAC

	①		②		③		④		⑤		⑥	
	Q	Q	Q	Q	Q	Q	A	A	Q	Q	F	REMARKS
	(m ³ /s)	(m ³ /s)	86400 ΣQ	ΣQ	ZR (mm)	ZR (mm)	(km ²)	(km ²)	(m ³)	(m ³)	F = ②/⑤	
1977					(2345.0)	(2345.0)	118		276.7 X 10 ⁶			
76												
75	434.57		37.5 X 10 ⁶		1417.3				167.2		0.22	
74	397.58		34.3		(7408.3)				284.2		0.12	
73	(122.17)		10.6		(1452.2)				171.4		0.06	
72	489.54		42.2		(4092.0)				482.9		0.19	
71	458.27		39.6		(1914.0)				225.9		0.18	
1970												
69	356.37		30.7									
68	516.24		44.6									
67	1766.88		152.7									
66	(2273.73)		196.5									
65	966.76		83.5									
64	1510.06		130.5									
63	2564.36		221.6									
62	(249.34)		21.5									
61	1548.34		133.7									
1960	1721.43		148.7									
59	279.38		24.1									
58												
MEAN	1002.76		86.6 X 10 ⁶		2271.5				268.0 X 10 ⁶		0.32	

* Q AND R ARE THE DISCHARGE OBSERVATION STATION AND THE RAINFALL OBSERVATION STATION

MEAN VALUE WAS ADOPTED FROM ANNUAL MONTHLY RUN-OFF DISCHARGE (TABLE I-15) AND ANNUAL MONTHLY RAINFALL (FIGURE I-15).

TABLE I-16-3 ANNUAL WATER BALANCE (3)

RIVER: PASIG-POTORERO. Q: MDA DOLORES, R: PORAC

	(1)	(2)	(3)	(4)	(5)	(6)	REMARKS
	RUN OFF DISCHARGE ΣQ (m ³ /s)	RUN OFF VOLUME 86400ΣQ (m ³)	RAINFALL ΣR (mm)	DRAINAGE AREA A (km ²)	RAINFALL VOLUME AΣR (m ³)	RUN OFF COEFFICIENT F = (3)/(5)	
1977				28			
76			(2345. 0)	%	65.7 X10 ⁶		
75			1417. 3	%	39.7		
74			(2408. 3)	%	67.4		
73			(1452. 2)	%	40.7		
72			4092. 0	%	114.6		
71	286.58	24.8 X10 ⁶	(1914. 0)	%	53.6	0.46	
1970	114.20	9.9		%			
69	74.73	6.5		%			
68	(348. 00)	30.1		%			
67	(362. 36)	31.3		%			
66				%			
65				%			
64				%			
63				%			
62				%			
61				%			
1960				%			
59				%			
58				%			
MEAN	246.31	21.5 X10 ⁶	2271.5		63.6 X10 ⁶	0.34	

* Q AND R ARE THE DISCHARGE OBSERVATION STATION AND THE RAINFALL OBSERVATION STATION.

** MEAN VALUE WAS ADOPTED FROM ANNUAL MONTHLY RUN-OFF DISCHARGE (TABLE I-15) AND ANNUAL MONTHLY RAINFALL (FIGURE I-15).

TABLE I-15-4 ANNUAL WATER BALANCE (4)

RIVER: PASIG - POTORERO		Q. CABETICAN SACOLOR R. PORAC			
①	②	③	④	⑤	⑥
RUN OFF DISCHARGE ΣQ (m ³ /s)	RUN OFF VOLUME 86400 ΣQ (m ³)	RAINFALL ΣR (mm)	DRAINAGE AREA A (K.M ²)	RAINFALL VOLUME A ΣR (m ³)	RUN OFF COEFFICIENT F = ③/⑤
1977			242		
76		(2345.0)	"	567.5 X10 ⁶	
75		1417.3	"	343.0	
74		(2408.3)	"	582.7	
73		(1452.2)	"	351.4	
72		4092.0	"	990.3	
71		(1914.0)	"	463.2	
1970					
69	362.84				
	31.3 X10 ⁶				
68	502.70				
	44.3				
67	(900.04)				
	77.6				
66	(445.90)				
	38.5				
65					
64					
63					
62					
61					
1960					
59					
58					
MEAN	656.20	56.7	2271.5	549.7 X10	0.10

* Q AND R ARE THE DISCHARGE OBSERVATION STATION AND THE RAINFALL OBSERVATION STATION.

** MEAN VALUE WAS ADOPTED FROM ANNUAL MONTHLY RUN-OFF DISCHARGE (TABLE I-15) AND ANNUAL MONTHLY RAINFALL (FIGURE I-15)

Conversion from water stage records into discharge is done not by the method that water stage-discharge curves are adjusted according to the variation of a river-bed but by the method that the rating curve established initially is utilized as it is if the difference between the observed discharge and the calculated value from a rating curve is confined within $\pm 5\%$ (Area-Velocity Method).

The annual highest water stage and the annual maximum discharge thus calculated are given in TABLE I-14. However, conversion into discharge for this river with a high permeability and a serious variation of stream should be re-studied because the data observed at 7 A.M. on July 25, 1972, show $H_{max}=2.00$ m and $Q_{max}=8.86$ m³/s.

The monthly mean discharge duration curves at Der Darnen ($A = 111$ km³) Valdez ($A = 118$ km³), along the Porac River, HDA Dolores ($A = 28$ km³) and Cabetican Bacolor ($A = 242$ km²) along the Pasig-Potrero River show an adverse tendency that the discharge in the up-stream surpasses that in the down-stream. Balance between the annual total rainfall and run-off discharge is presented in the following table to clear up this tendency.

	① Runoff Volume 86400 Q (m ³)	② Rainfall Volume A R (m ³)	f ①/②	Remarks
Der Carmen	174.5 x 10 ⁶	252.1 x 10 ⁶	0.69	
Valdez	86.6 x 10 ⁶	268.0 x 10 ⁶	0.32	
HDA Dolores	21.5 x 10 ⁶	63.6 x 10 ⁶	0.34	
Cabetican Bacolor	56.7 x 10 ⁶	549.7 x 10 ⁶	0.10	

It is concluded that this tendency is caused by the higher permeability in the down reaches. Therefore, studies are carried out in Chapter II Hydrology, based on rainfall data, not water stage and discharge records.

4-7 Return Period of Rainfall

Annual maximum daily rainfall records (N days, $N = 1 - 7$) collected at seven stations are summarized in TABLE I-17. Based upon the records at Clark Field, Sta. Cruz Porac, San Agustín Arayat and Casinala Apalit stations, daily rainfall probability (Return Period of N-Day Rainfall, $N = 1, 3, 5$) has been estimated as given in the following table.

Return Period of N-Day Rainfall

Station	N-Days Rainfall	Return Period (T: Year)					
		2	5	10	20	50	100
Clark Air Base	R	140	220	275	330	410	470
	3R	240	410	540	690	900	1,050
	5R	300	520	700	890	1,180	1,400
Sta. Cruz Porac	R	150	230	280	340	410	470
	3R	270	430	560	700	900	1,050
	5R	320	550	720	920	1,200	1,400
Casinala Apalit	R	130	210	270	330	420	480
	3R	200	360	500	660	900	1,080
	5R	250	460	650	860	1,200	1,450
San Augustin Arayat	R	135	210	265	320	400	460
	3R	220	375	500	640	850	1,000
	5R	260	380	600	770	1,000	1,200

According to the table, rainfall values (especially 3-day rainfall and 5-day rainfall) at Clark Air Base fall below those at other three stations. Distribution of rainfall values plotted on the logarithmic probability paper shows that 5-day rainfall values do not exist in one line, though 3-day and 5-day rainfall values do (normal distribution), which indicates the following matters.

- i) The data have been collected at Clark Air Base for 16 years while less than 10 years at other stations. It is reasonable that rainfall probability based on the small number of statistical years be high.
- ii) 1-day and 3-day rainfall data present normal distribution while 5-day rainfall data do no. This phenomenon corresponds to the rainfall duration of less than 3 days mentioned in 4-3) Hourly Rainfall Distribution.

On the other hand, N-day maximum rainfall data during floods are listed up in the table below to study main floods statistically these years.

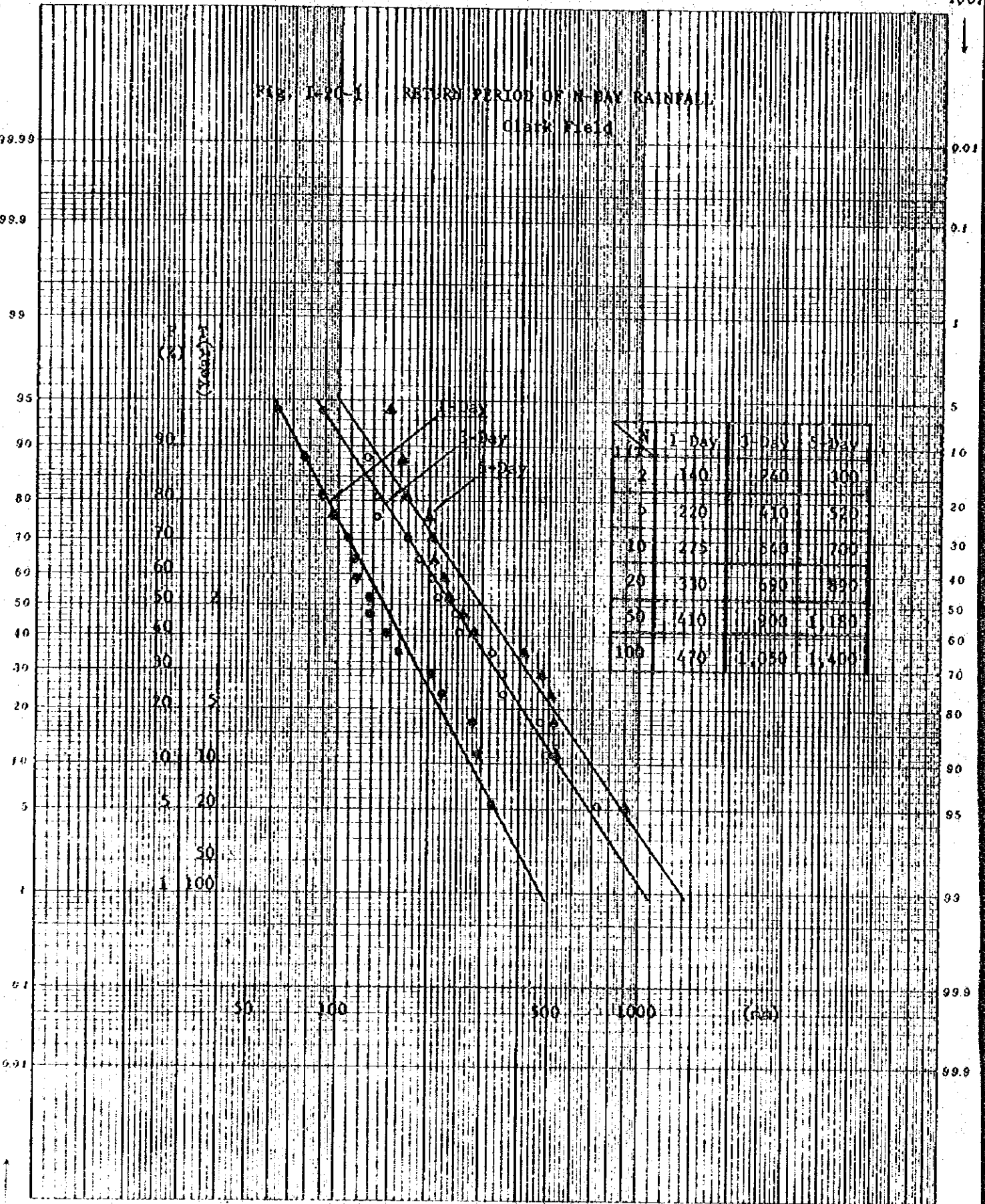
Maximum N-Day Rainfall in Main Floods

Flood	Station Rain	Clark Air Base		Sta. Cruz Porac		Casirala Apalit		San Augstin Arayat	
		Date	(mm)	Date	(mm)	Date	(mm)	Date	(mm)
1966 May	R	5/19	279.9						
	3R	5/18- 5/20	472.4						
	5R	5/18- 5/22	533.4						
	7R	5/18- 5/24	557.8						
1970 Sept.	R	9/1	222.0	9/1	157.7			9/1	312.4
	3R	8/31- 9/2	356.4	8/31- 9/2	269.1			8/30- 9/1	617.6
	5R	8/21- 9/4	511.3	8/30- 9/3	284.6			8/30- 9/3	696.7
	7R	8/30- 9/5	528.1	8/28- 9/3	376.1			8/30- 9/5	747.7
1972 July	R	7/19	291.6	7/18	269.4	7/18	282.0	7/17	337.7
	3R	7/18- 7/20	734.1	7/18- 7/20	697.2	7/17- 7/19	649.3	7/17- 7/19	833.0
	5R	7/17- 7/21	912.9	7/17- 7/21	927.2	7/16- 7/20	814.3	7/16- 7/20	1160.5
	7R	7/17- 7/23	938.8	7/17- 7/23	1007.7	7/17- 7/23	910.8	7/17- 7/23	1254.1
1974 Aug.	R			8/16	154.6	8/16	205.6	8/16	704.9
	3R			8/15- 8/17	389.1	8/15- 8/17	437.5	8/15- 8/17	1211.3
	5R			8/14- 8/18	477.8	8/13- 8/17	505.0	8/13- 8/17	1380.9
	7R			8/14- 8/20	502.8	8/13- 8/19	532.0	8/11- 8/17	1484.8
1976 May	R			5/23	246.9	5/22	212.0	5/23	167.9
	3R			5/22- 5/24	488.6	5/21- 5/23	413.0	5/22- 5/24	303.9
	5R			5/22- 5/26	591.1	5/20- 5/24	491.6	5/22- 5/26	427.0
	7R			5/21- 5/27	725.6	5/20- 5/26	597.6	5/21- 5/27	544.2

Since one continuous rainfall can be represented by 3-day rainfall, according to the study in 4-3), 3-day rainfall data at Clark Air Base with many statistical years are studied herein. The study of those data indicates that the flood in May of 1966 is 6.7-year probability, that in September of 1970 3.6-year probability and that is July of 1972 25-year probability.

A4 NO. 349 C

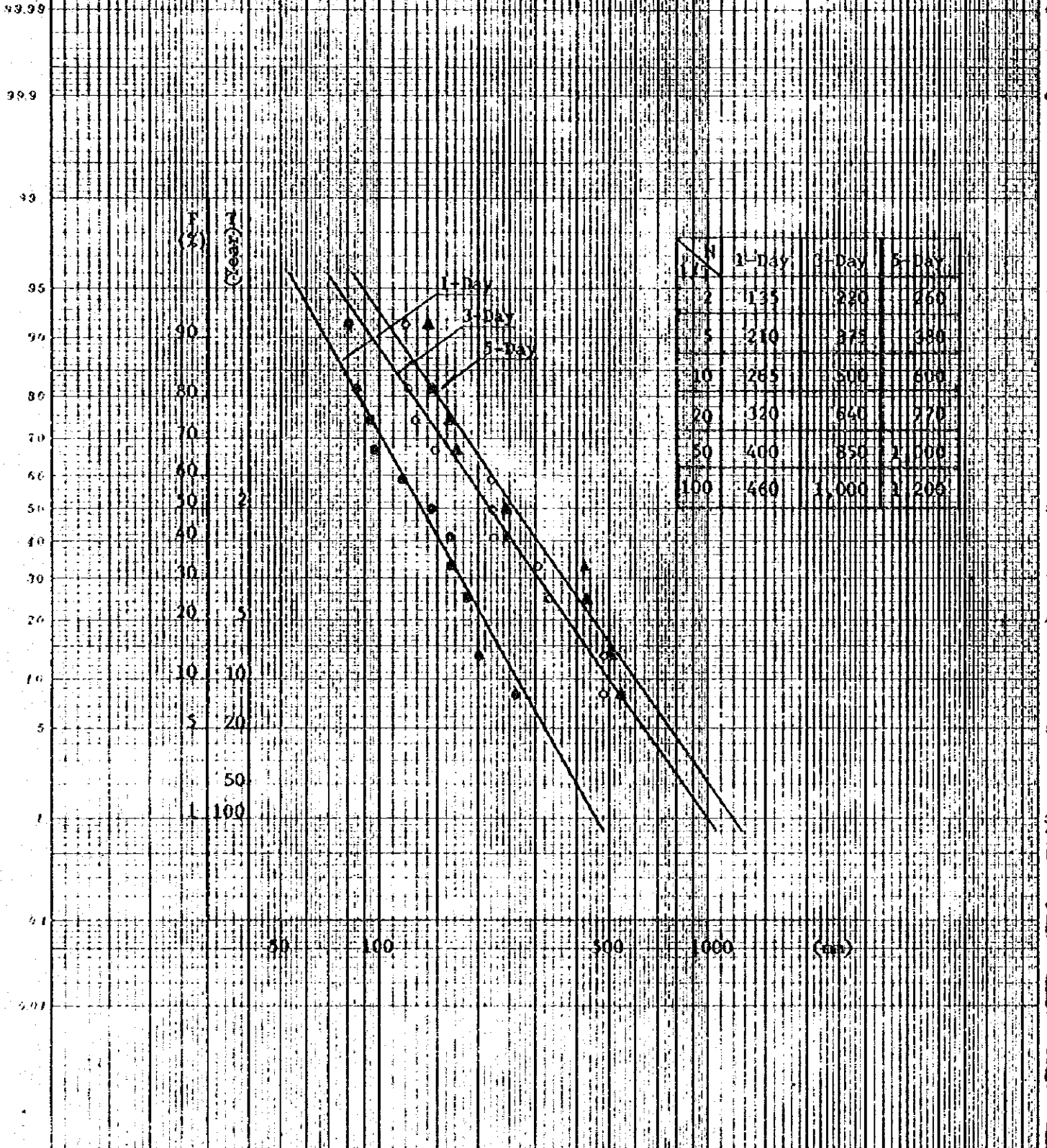
FIG. I-20-1 RETURN PERIOD OF N-DAY RAINFALL
Clark Field



log x

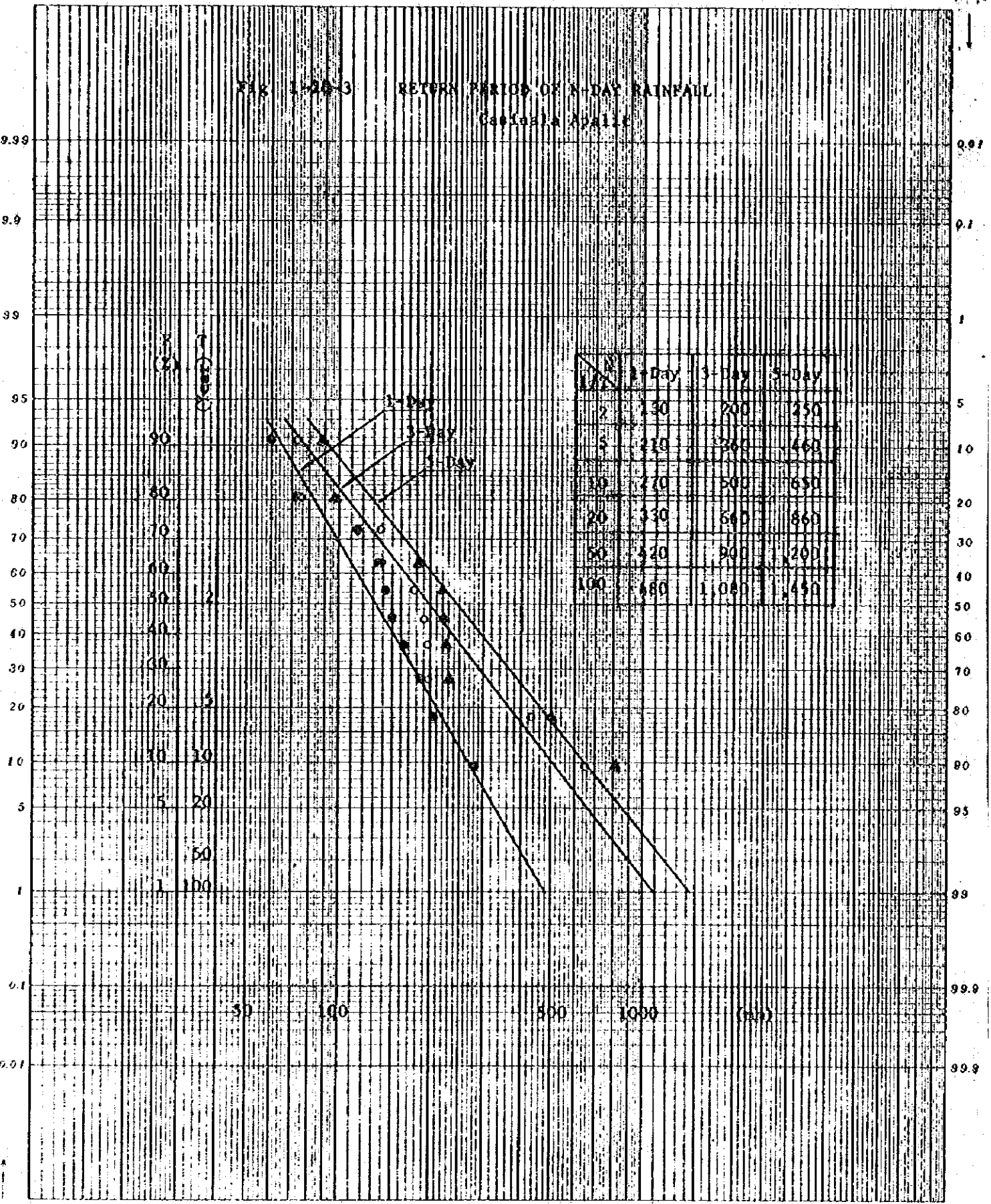
$$100F = 100 \times \int_{-\infty}^{\log x} u dx \quad ; \quad 100F = 100 \times \int_{\log}^{100} u dx \quad ; \quad u = \frac{1}{\sqrt{2\pi}} e^{-(\log x)^2/2}, \quad x > 0$$

Fig. 1-20-2: RETURN PERIOD OF N-DAY RAINFALL
San Augustin Arayat



$$100F = 100 \times \int_{\sigma}^{\log x} u \, dx \quad , \quad 100F = 100 \times \int_{\log \sigma}^{\log x} u \, dx \quad u \equiv \frac{1}{\sqrt{2\pi}} e^{-(\log x)^2/2}, \quad x > 0$$

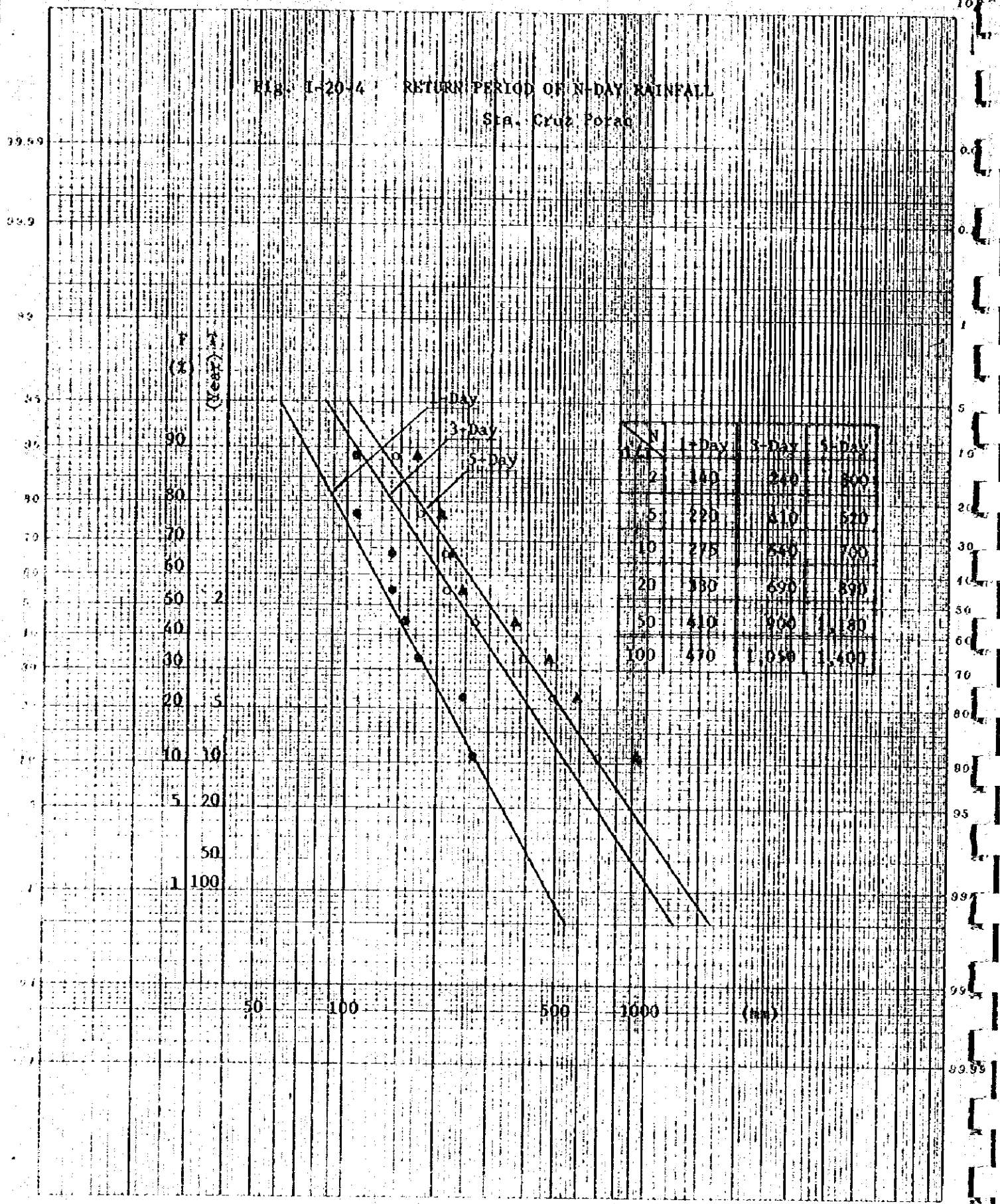
Fig. 1-20-3 RETURN PERIOD OF N-DAY RAINFALL
Columbia Basin



$\log x \rightarrow$
 $100F \equiv 100 \times \int_{-\infty}^{\log x} u \, ds$, $100F \equiv 100 \times \int_{\log}^{100} u \, dx$, $u \equiv \frac{1}{\sqrt{2\pi}} e^{-(\log x)^2/2}$, $x > 0$

A4 NO. 349 C

Fig. I-20-4 RETURN PERIOD OF N-DAY RAINFALL
Sta. Cruz Porac



$$1001 = 100 \int_{\log 1}^{\log x} u^{-1} du = 1001 - 100 \int_{\log 1}^{\log x} u^{-2} du = 1001 - 100 \left[\frac{1}{-1} u^{-1} \right]_{\log 1}^{\log x} = 1001 - 100 \left[-\frac{1}{u} \right]_{\log 1}^{\log x} = 1001 - 100 \left[-\frac{1}{\log x} + \frac{1}{\log 1} \right] = 1001 - 100 \left[-\frac{1}{\log x} + 1 \right] = 1001 - 100 + \frac{100}{\log x} = 901 + \frac{100}{\log x}$$

TABLE I-17-1 ANNUAL MAXIMUM DAILY RAINFALL

NO. ① CASINALA APALIT (mm)

	1 - DAY		2 - DAY		3 - DAY		4 - DAY		5 - DAY		6 - DAY		7 - DAY	
	DATE	R ₁	DATE	R ₂	DATE	R ₃	DATE	R ₄	DATE	R ₅	DATE	R ₆	DATE	R ₇
1977														
76	5/29	186.5	5/29-30	188.1	5/27-29	185.5	5/26-29	207.5	5/25-29	225.5	5/24-29	354.5	5/23-29	399.5
75	10/20	166.2	10/19-20	184.7	10/19-21	191.3	10/20-23	203.1	10/19-23	221.6	10/18-23	245.6	10/17-23	247.6
74	8/16	205.6	8/15-16	328.1	8/15-17	437.5	8/14-17	500.5	8/14-18	503.5	8/14-19	527.5	8/14-20	548.5
73	10/15	150.0	10/15-16	178.1	10/14-16	178.1	10/13-16	181.7	10/12-16	181.7	10/11-16	261.7	10/11-17	261.7
72	7/18	282.0	7/12-18	488.5	7/17-14	149.3	7/17-20	812.3	7/16-20	814.3	7/15-20	816.3	7/29 9/4	905.8
71	6/15	71.5	6/14-15	71.5	6/13-15	73.0	6/12-15	98.0	6/12-16	98.0	6/12-17	98.0	6/12-18	99.5
70	8/13	60.1	8/13-14	62.0	8/12-14	74.5	7/11-14	87.0	7/11-15	87.5	7/11-16	98.0	7/11-17	98.5
69	7/27	113.1	8/26-27	134.5	8/27-29	137.2	8/27-30	213.6	8/26-30	232.4	8/25-30	243.1	8/24-30	245.0
68	8/29	143.0	8/27-28	161.5	8/28-30	197.0	8/27-30	215.5	8/26-30	219.5	8/25-30	219.5	8/24-30	225.0
67	11/4	136.5	11/3-4	136.5	11/3-5	137.0	11/3-6	137.0	11/3-7	147.0	11/3-8	151.6	11/3-4	151.6

NO. ③ SAN AUGSTIN ARAYAT (mm)

	1 - DAY		2 - DAY		3 - DAY		4 - DAY		5 - DAY		6 - DAY		7 - DAY	
	DATE	R ₁	DATE	R ₂	DATE	R ₃	DATE	R ₄	DATE	R ₅	DATE	R ₆	DATE	R ₇
1977														
76	5/23	167.9	5/22-23	265.4	5/22-24	303.4	5/22-25	351.6	5/22-26	427.2	5/22-27	502.0	5/21-27	564.4
75	8/9	97.1	8/9-10	126.5	8/8-10	148.3	8/7-10	163.5	8/6-10	172.3	8/6-11	174.6	8/8-14	180.7
74	8/15	166.6	8/15-16	330.2	8/15-17	481.5	8/14-17	530.8	8/14-18	546.3	8/14-19	568.8	8/14-20	583.1
73	10/16	185.2	10/15-16	219.2	10/14-16	219.7	10/13-16	235.6	10/12-16	237.1	10/11-16	259.1	10/10-16	259.1
72	7/7	201.8	7/6-7	301.7	7/6-8	333.5	7/6-9	386.9	7/6-10	430.2	7/6-11	449.4	7/6-13	462.4
71	7/21	85.4	6/15-16	116.8	7/20-22	128.6	10/9-12	164.6	10/9-13	164.6	6/12-17	183.8	10/4-10	202.7
70	8/11	80.7	8/13-14	99.6	8/12-14	120.6	8/12-14	140.8	8/11-14	140.8	8/11-16	153.8	8/10-16	160.4
69	7/20	93.9	7/19-20	121.1	7/18-20	121.6	7/17-20	137.7	7/16-20	146.8	7/15-20	168.6	7/15-21	182.1
68	8/28	116.7	8/28-29	162.8	8/28-30	223.7	8/27-30	241.8	8/26-30	246.5	8/25-30	252.6	8/24-30	279.3
67	7/28	144.0	7/28-29	191.9	5/30 6/2	221.8	5/30 6/2	243.5	5/30 6/4	246.7	6/1-6	310.1	5/30 6/6	340.6
66	5/9	260.6	5/19-20	427.6	5/18-20	483.3	5/18-21	506.2	5/18-22	510.7	5/18-28	525.3	5/18-24	525.3

TABLE I-17-4 ANNUAL MAXIMUM DAILY RAINFALL

(INCH)

NO	1 - DAY		2 - DAY		3 - DAY		4 - DAY		5 - DAY		6 - DAY		7 - DAY	
	DATE	R ₁	DATE	R ₂	DATE	R ₃	DATE	R ₄	DATE	R ₅	DATE	R ₆	DATE	R ₇
1977														
76														
75														
74														
73														
72	7/19	11.48	7/18-19	21.46	7/18-20	28.89	7/18-21	32.42	7/17-21	35.93	7/16-21	36.26	7/27-9/2	37.28
71	10/12	4.46	10/11-12	6.27	7/18-12	7.35	7/9-12	7.50	10/9-13	7.87	10/7-12	8.00	10/6-12	10.22
70	9/1	8.74	8/31-9/1	12.14	8/31-9/2	14.03	8/31-9/3	17.12	8/31-9/4	20.13	8/31-9/5	20.67	8/30-9/5	20.79
69	8/6	4.24	8/5-6	4.67	8/4-6	5.32	8/3-6	8.83	8/3-7	8.90	8/3-8	11.96	8/2-8	12.57
68	8/28	12.77	8/28-29	17.34	8/28-30	19.35	8/27-30	19.66	8/25-30	21.19	8/26-31	22.45	8/26-9/1	22.70
67	7/29	5.68	7/29-30	8.90	7/29-31	9.94	6/1-4	8.51	6/4-8	10.19	6/3-8	12.83	6/2-8	14.10
66	5/19	11.02	5/19-20	16.52	5/18-20	18.5	5/18-21	19.19	5/18-22	21.00	5/18-23	21.62	5/18-24	21.98
65	7/14	4.54	7/13-14	6.74	7/12-14	8.18	7/12-15	8.18	7/12-16	8.26	7/12-17	9.19	7/12-18	9.19
64	12/15	3.79	12/14-15	4.48	8/21-23	4.89	8/21-24	4.93	8/21-25	6.62	8/20-25	6.88	8/19-25	7.36
63	6/28	6.29	6/27-28	8.90	6/27-29	8.50	6/27-30	9.17	6/27-7/1	9.17	6/27-7/2	10.21	6/27-7/3	12.86
62	7/20	5.03	7/19-20	9.88	7/19-21	12.83	3/19-22	13.91	7/19-23	10.041	7/19-24	19.26	7/18-24	19.96
61														
60	6/27	8.06	8/13-14	11.11	10/13-15	14.04	10/13-16	18.05	10/12-16	18.83	8/11-16	19.26	8/10-16	21.71
59	8/12	2.42	8/12-13	3.45	8/29-31	3.45	8/12-15	5.14	8/12-16	5.78	8/12-17	5.81	8/12-18	6.18
58	7/13	5.02	7/13-14	48.36	3/13-15	9.60	7/13-16	11.10	7/13-17	11.21	7/13-18	11.66	7/13-19	12.79
57	6/11	3.03	7/15-16	4.31	8/15-17	5.27	8/15-18	6.23	7/12-16	6.51	7/11-16	6.54	7/10-16	6.54
56	9/21	3.46	9/21-22	4.75	9/8-10	6.71	9/7-10	8.06	9/7-11	8.12	9/7-12	8.17	9/7-13	8.56

1 INCH = 2.54 cm.

4-8 Tide Record

The tide level is recorded at the southern part of Manila located at lat. $14^{\circ}34'30''N$ and long. $120^{\circ}57'40''E$. Hourly observation by the self-register tide level has been conducted since 1976. In the Bureau of Geodetic and Survey, the results of monthly extreme (highest and lowest) tide record are summarized in TABLE I-18. The highest tide level records of each year are given in the following table.

TABLE I-18 Highest Tide Record

Year	Date		Tide Height (Feet)	Year	Date		Tide Height (Feet)
	Month	Day			Month	Day	
1950	Aug.	13	10.9	1965	July	14	10.9
51	Aug.	16	11.1	66	Aug.	15	11.0
52	June	11	10.9	67	July	9	11.0
53	June	28	11.0	68	July	25	11.3
54	July	1	10.5	69	June	30	11.0
55	June	22	10.5	1970	Aug.	17	11.4
56	June	10	10.5	71	Oct.	11	11.4
57	June	29	10.2	72	July	12	12.0
58	July	17	10.1	73	July	1	11.3
59	Aug.	5	10.1	74	July	20	12.0
1960	Aug.	7	11.0	75	July	9	10.4
61	July	29	10.6				
62	Aug.	1	10.9				
63	July	21	10.6				
64	Aug.	7	11.7				

The table above shows that the highest tide level during 26 years from 1950 to 1975 is 12.0 feet (3.66 m) recorded on July 12 in 1972. According to the research of BPW, the flood trace survey (in August of 1960) near the estuary of the Pasig-Potrero River concluded that the highest water level is 13.632 m (3.16 m MSL). Although the height of 13.672 (3.29 m MSL) is employed to the river improvement plan, as far as the tide level is concerned, the tide level in June of 1972 is higher by one foot than that in the flood period in August of 1960. As causes for ponding in the down-stream

during a flood, the influence of backwater due to the tide level, the rainfall amount and the condition of drainage are considered. During a flood, however, the rainfall amount seems to be the main cause for the flood together with bad drainage condition in the down-stream.

The highest tide levels in 1960, 1966, 1970 and 1974 when floods occurred are so high, compared with those in other years with a difference in height confined within \pm one foot.

Hydrological factors in 1966 and 1972 are compared as follows.

Flood	①	②	(1) x (2)	Inundation Area	Inundated Water Depth
	Monthly Rainfall	Catchment Basin (km ²)			
1966 Aug.	1,038.1mm	125Km ²	129.75 x 106m ³	(20)Km ²	0.22m \pm 25cm
1972 July	2,274.5mm	125Km ²	284.31 x 106m ³	20 Km ²	0.47m \pm 50cm

The ponding area can be assumed to be approximately 20 km², which is the same area in 1966. Judging from the values in the table, values of ponding depth are 25 cm in 1966 and 50 cm in 1972 approximately, assuming that water spread in the ponding area equally, which coincide with the information obtained at the site.

On the other hand, since net-work water channels are developed due to the influence of back water, back water calculation used for ordinary water channels is not applicable in this case. Therefore, following the determination based on the existing maximum flood trace survey, H₀ = 3.50 m is adopted as a starting water level to the Pasig-Potrero River adjusting the starting water level of H₀ = 3.20 m which is determined by BPN from the flood trace survey in August of 1966 with H₀ = 30 cm.

This starting water level should be reviewed when the river improvement plan for the down-stream of Gua-Gua River is established.

III-5 Runoff Investigation

As mentioned before, the design high-water discharge in the Pasig-Potrero River has been determined at Q_p = 900 m³/S in 1964 by BPW. On the other hand, according to the relation* between discharge frequency and drainage area for Central Luzon (FIGURE) studied by BPW, discharge frequency "T" (year) at the base point of the Marcatian Bridge (A = 44 km²) is estimated at so much as 1,000 years approximately. A study on this value is carried out as follows with various data.

5-1 Review of BPW Formula

According to the runoff data of the Philippine stream from "Note on Intense Rainfall and Runoff in Central Luzon" by Villanueva and Delena, Philippine Engineering Record in December of 1939, the

TABLE 1-19 MONTHLY EXTREMES TIDE RECORD — HIGHEST TIDE —

	JAN	FEB	MAR	APR	MAY	JUN	JULY	AUG	SEP	OCT	NOV	DEC	REMARKS
1-94-6									6 99	14 96			
47			6 97	25 97	33 101	20 111	18 109	14 106	12 101	6 106	15 108	28 105	
48	25 103	22 97	22 92	28 95	13 99	12 103	9 105	3 104	1 111	23 96	20 99	19 99	
49	15 101	12 99	13 94	16 93	15 99	29 101							
1-95-0							27 109	14 109	20 102	2 102	13 103	10 103	
51	8 102	5 99	29 94	26 98	24 101	20 104	19 107	16 111	1 101	8 96	30 101	1 103	
52	27 99	22 96	22 92	29 95	13 102	11 109	9 108	5 109	3 104	23 99	19 100	19 102	
53	13 101	13 99	13 96	18 98	15 103	28 110	11 103	12 105	1 103	25 100	22 101	20 100	
54	2 100	3 98	3 95	8 96	5 102	2 103	1 105	15 104	9 101	18 102	12 103	10 105	
55	8 100	3 98	3 93	26 97	23 104	22 105	19 105	16 102	12 98	7 101	3 103	29 104	
56	27 98	24 96	20 92	16 98	13 101	10 105	7 105	6 104	3 105	25 99	20 101	18 101	
57	16 98	14 94	3 90	19 92	30 98	24 102	11 101	25 101	21 98	14 95	12 99	22 93	
58	7 92	4 92	5 87	8 91	6 95	3 99	17 101	15 96	13 97	7 94	13 98	11 96	
59	9 94	29 90	30 88	26 95	23 96	22 100	20 100	18 101	25 97	9 97	4 98	31 100	
1-96-0	29 96	25 91	23 88	17 93	28 100	12 105	9 103	7 110	10 100	25 95	22 99	18 101	
61	17 94	15 94	15 88	19 91	4 96	30 106	29 106	25 106	22 98	3 93	24 95	10 98	
62	7 99	6 93	5 91	25 93	22 96	20 100	20 108	1 109	13 100	18 95	15 100	12 98	
63	10 96	8 93	23 92	28 94	26 100	24 103	21 106	18 99	5 102	9 94	4 94	3 99	
64	1 98	25 92	2 39	18 95	15 101	12 103	10 105	7 117	4 103	24 101	20 102	20 103	
65	18 100	15 98	14 94	21 93	31 100	30 106	28 109	27 102	21 102	18 94	12 102	11 102	
66	3 102	5 100	7 97	28 96	29 101	21 104	19 107	15 110	13 109	9 98	29 103	13 105	
67	26 101	24 100	21 94	28 101	26 106	23 107	9 110	19 110	3 108	10 101	4 107	3 102	
68	2 101	14 97	22 96	18 99	15 103	13 108	26 113	8 109	5 111	25 103	23 104	21 106	
69	18 104	15 102	12 96	9 99	6 106	30 110		1 109	10 107	2 104	13 108	10 108	
1-97-0	9 106	6 105	5 100	23 102	23 108	20 111	20 111	7 114	11 112	18 107	15 110	15 108	
71	27 107	24 105	3 101	28 106	27 107	24 108	20 113	7 111	5 106	11 114	6 108	3 110	
72	1 110	15 103	21 100	18 107	15 111	13 111	12 120	6 113	4 105	27 104	23 107	21 107	
73	19 106	16 99	13 99	7 101	6 107	30 112	1 113	27 110	20 105	15 116	13 111	12 113	
74	9 110	8 106	6 104	27 105	24 111	21 114	20 120	17 118	3 105	16 110	7 113	30 104	
75	27 1012	26 939	25 945	28 991	27 1000	13 1049	10 1036	7 1055	5 1055	10 995	4 1006	4 1077	

LEFT SIDE; DATE, RIGHT SIDE; HIGHEST TIDE OBSERVED / : NO RECORD *; YEARLY EXTREMUM UNIT) FEET

TABLE I-20 MONTHLY EXTREMES TIDE RECORD -- LOWEST TIDE --

	JAN	FEB	MAR	APR	MAY	JUN	JULY	AUG	SEP	OCT	NOV	DEC	REMARKS
1946									21 5.1	17 5.2			
47			4.7	26 4.8	23 4.5	21 4.7	18 4.8	15 4.9	12 5.2	23 5.5	30 4.8	31 4.0	
48			4.4	18 4.9	25 4.7	9 4.5	8 4.4	4 4.4	29 5.1	25 5.0	18 4.6	19 4.1	
49			4.1	17 4.6	15 4.5	12 4.4							
1950									22 5.4	17 5.0	15 4.4	15 3.9	
51			4.1	25 4.4	23 4.1	21 4.0	18 4.3	15 5.3	12 4.9	8 5.1	30 4.3	30 3.9	
52			4.3	16 4.8	12 4.5	10 4.8	6 4.6	4 4.9	30 5.2	26 5.1	21 4.6	30 4.4	
53			4.0	17 4.9	15 5.2	13 5.0	25 4.7	20 5.3	4 5.6	23 5.0	22 4.5	21 4.6	
54			4.6	2 4.8	13 5.1	3 4.5	1 4.9	13 5.5	10 5.7	15 5.3	14 4.7	11 4.4	
55			4.1	2 4.9	23 5.0	20 4.6	20 5.0	16 5.2	11 5.6	20 5.7	30 5.0	30 4.4	
56			4.5	24 5.0	14 5.3	12 4.9	8 4.8	4 5.2	29 5.0	25 5.5	30 4.9	19 4.5	
57			4.4	13 4.6	13 5.2	17 5.0	3 4.8	7 5.3	30 5.6	24 5.0	24 4.4	22 4.5	
58			4.3	4 4.6	5 5.0	7 5.0	6 4.7	3 4.8	2 4.9	1 5.3	15 4.8	12 4.4	
59			4.4	6 4.6	30 5.1	26 4.9	25 4.6	22 4.9	20 4.9	17 5.7	3 5.0	31 4.5	
1960			4.5	26 5.0	22 5.4	16 5.3	14 5.0	10 5.0	9 4.9	6 5.9	4 5.5	28 5.5	
61			4.1	14 4.5	15 5.0	11 5.2	3 5.1	1 4.8	27 5.8	20 5.6	18 5.3	7 4.7	
62			4.6	6 4.2	3 4.6	1 5.2	24 5.1	20 5.1	2 5.2	14 5.3	11 5.5	10 5.4	
63			4.2	6 4.7	6 5.1	26 4.9	26 4.7	23 4.8	21 5.1	18 5.0	30 5.5	4 4.7	
64			4.1	4 4.6	25 4.8	16 4.9	15 4.8	12 4.1	10 4.7	3 5.5	3 5.4	25 5.4	
65			4.3	16 4.5	16 4.8	5 5.0	5 4.7	30 4.6	1 4.8	22 4.9	22 5.3	17 5.1	
66			4.1	4 4.4	4 4.8	26 5.1	8 4.9	21 4.8	16 5.3	10 5.8	20 5.2	14 5.1	
67			4.4	7 4.6	23 5.0	27 4.8	23 5.0	20 5.4	6 5.3	25 5.8	29 5.6	5 4.9	
68			4.2	25 4.6	12 5.0	17 4.6	4 5.2	12 4.5	10 4.8	7 5.1	3 5.7	22 4.6	
69			4.2	15 4.6	15 4.9	10 5.0	5 4.8	2 4.7	26 5.2	22 5.5	17 5.6	11 5.2	
1970			4.3	3 4.6	3 5.1	26 5.4	26 5.4	21 5.2	19 5.2	15 5.9	19 5.2	30 5.3	
71			5.1	5 5.3	5 5.7	26 5.4	26 5.2	11 5.2	9 5.3	17 5.8	3 5.8	27 5.0	
72			5.1	13 5.6	12 5.9	7 5.6	14 5.3	10 5.1	9 5.9	7 6.2	1 6.0	27 5.6	
73			5.0	15 5.1	5 5.7	5 5.0	2 5.4	25 5.3	21 5.4	19 5.4	30 5.4	14 5.7	
74			5.1	5 5.2	5 5.6	25 5.7	24 5.7	19 5.7	15 6.2	7 6.4	16 5.9	12 5.2	
75			5.2	23 5.8	9 5.47	26 5.68	26 5.98	23 5.49	8 5.85	3 5.35	2 4.10	25 5.04	

LEFT SIDE ; DATE, RIGHT SIDE : HIGHEST TIDE OBSERVED / ; NO RECORD * YEARLY EXTREMUM UNIT : FEET

TABLE I-21 OBSERVATION RECORD OF MAIN FLOODS

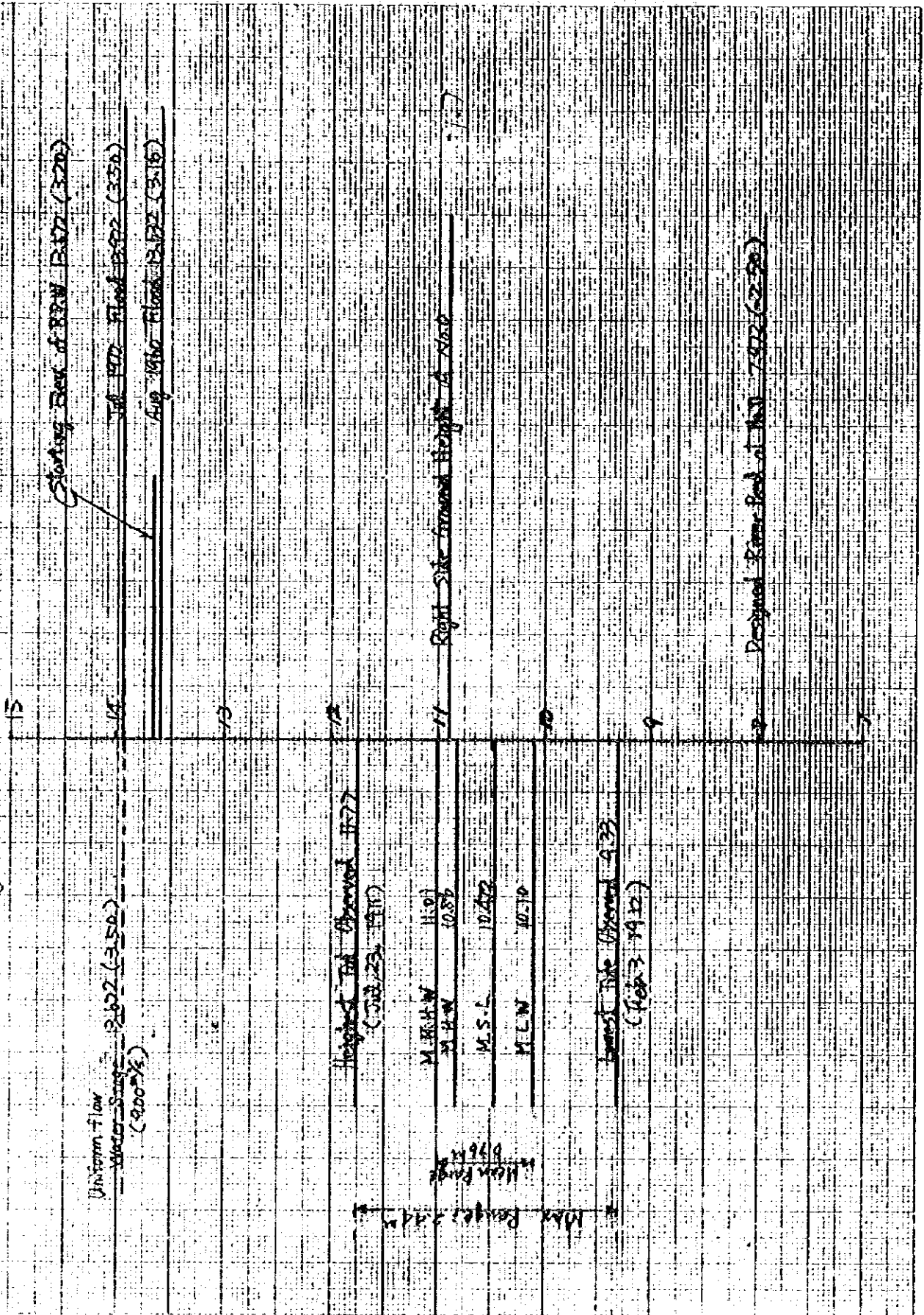
Flood	Flood Start*		Tide Record (feet)	Water Stage Record		Rain-fall of		Perac Monthly Total (mm)
	Bacolor (mm)	Calumpit (mm)		Calumpit (m)	Apalit (m)	Daily Max. (mm)	Flood Total (mm)	
1960 Aug	3.20**	4.94	11.0	Aug 15, 5.20	Aug 15, 5.11	Aug 14, 175.5 (Clark)	Jul 26-Aug 18, 922.0	Jun. 1972 Aug 1038.1 (Clark)
1966 May	3.15	3.97	10.1	May 20, 2.14	May 24, 4.22	May 19, 280.0 (Clark)	May 14-May 28, 753.4	May 813.8 (Clark)
1970 Sep.	/	/	11.2	Sep 2, 1.86	Sep 5, 4.31	Sep 1, 157.7	Aug 23-Sep 3, 417.6	Aug. 273.4 Sep. 257.7
1972 Jul	3.50***	5.33	12.0	Jul 21, Aug 1, 2.11	Aug 1, 5.86	Jul 18, 249.4	Jul 2-Aug 7, 2786.0	Jul. 2270.5 Aug. 916.6
1974 Aug.	/	/	12.0	Aug 17, 2.22	Aug 19, 5.40	Aug 10, 154.6	Aug 6-Aug 30, 671.7	Aug. 165.6
1976 May.	/	/	/	May 24, 1.84	May 27, 5.52	May 23, 246.9	May 17-Aug 25, 818.5	May 869.2

* Quoted from Flood Plain of the Pampanga River Basins by B.P.W. (c.f. Reference Material)

** 3.20 is the initial water stage of B.P.W. Design in 1964.

*** Right bank side ground height is about 3.0m and inundation depth is about 0.5m by reconnaissance survey.

Fig. I-21 INITIAL WATER STAGE



following equations are proposed as shown in FIGURE I-23.

- Eq. (1) $Q = 235 \cdot A / \sqrt{A+32}$ Extreme
- Eq. (2) $Q = 185 \cdot A / \sqrt{A+13}$ Rare
- Eq. (3) $Q = 85 \cdot A / \sqrt{A+11}$ Occasional
- Eq. (4) $Q = 50 \cdot A / \sqrt{A+9}$ Frequent

The design high-water discharge of 900 m³/S is established from Eq. (2), assuming a drainage area of 44 km² at the Mancatian Bridge.

The relation with specific discharge converted from Eq. (4) is presented in Fig.

Calculated Runoff Discharge ----- CASE A -----

	Mancatian	No.1	No.2	No.3	No.4	No.5
A (km ²)	44	23.3	16.8	14.3	9.2	4.3
Q (m ³ /s)	900	600	480	430	300	160

5-2 Based on Discharge Record

Discharge records serving as a base of "Regional Discharge Frequency-Discharge Area Relationship" (FIGURE I-22) are applicable only to a river basin with a drainage area of 100 km² or above. The specific discharge ($q = Q/A$) will increase reasonably in case of a smaller basin. Therefore, the relation between specific discharge and drainage area (A) based on the discharge records at obtained as shown in FIGURE . The discharge-frequency and discharge distribution thus obtained at the Mancatian Bridge as given in Fig.

Calculated Runoff Discharge ----- CASE B -----

	Mancatian	No.1	No.2	No.3	No.4	No.5
A (km ²)	44	23.3	16.8	14.3	9.2	4.3
Q (m ³ /s)	900	620	500	420	320	190

* "Regional Discharge Frequency-Drainage Area Relationship for central Luzon Basin" BPW Manila June 1964

5-3 Based on Rainfall Record

1) From Daily Rainfall Record

By the rainfall intensity equation (Dr. N. Monorobe's equation) in an arbitrary duration period in case that only daily rainfall is given, the discharge is obtained as follows.

$$t = \frac{R_{24}}{24} \left(\frac{24}{t}\right)^{2/3} \dots\dots\dots (5.1)$$

where; t: Mean rainfall intensity in a period of "t"(mm/hr)

t: Time of flood concentration (hr)

R₂₄: Rainfall in 24 hours (mm)

As to the Time of flood concentration, Ruchiha's Equation often used in Bayern district in Germany is utilized as follows.

$$t = L/W \dots\dots\dots 5.2$$

$$W_1 = 72(H/L)^{0.6} [km/hr] \dots\dots\dots 5.3$$

$$W_2 = 20(h/l)^{0.6} [m/sec] \dots\dots\dots 5.4$$

where; W₁, W₂: Time of flood concentration

L, l : Horizontal length from the estuary to the up-stream end

The results of the calculation are given in FIGURE I-31 .

Calculated Runoff Discharge ——— CASE C ———

	Mancatian	No.1	No.2	No.3	No.4	No.5
A (km ²)	44	23.3	16.8	14.3	9.2	4.3
Q (m ³ /s)	850	640	620	580	510	260

Judging from the above results, it is possible to estimate rainfall in a short period in Mancatian, No.1 Dam, No.2 Dam and No.3 Dam. However, rainfall in a short period in No.4 Dam and No.5 Dam which have a great discharge considering their small areas can be hardly estimated.

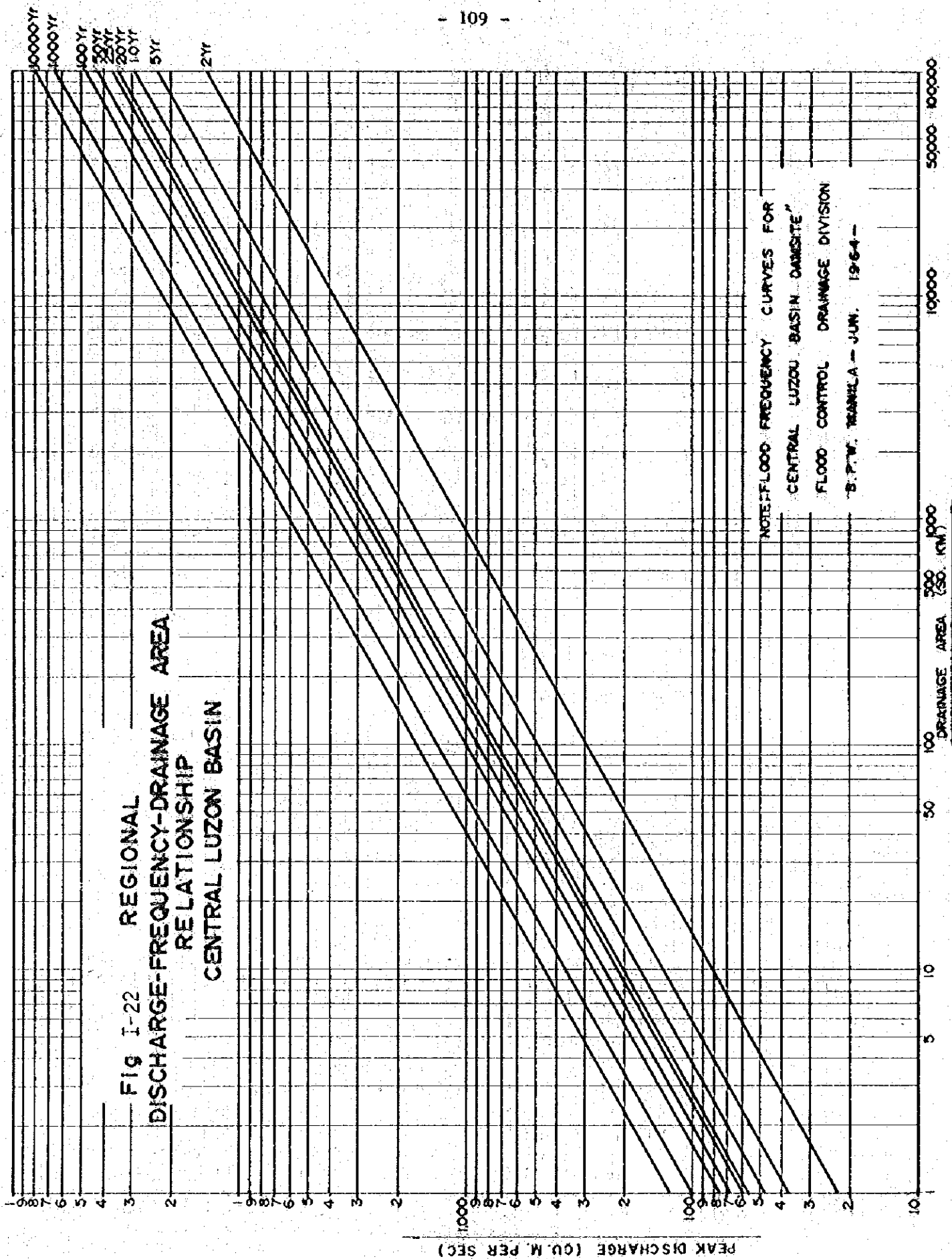
ii) From Hourly Rainfall intensity curves can be drawn from hourly rainfall records at Porac as shown in FIGURE I-33. Comparative study with rainfall intensity (refer to Extreme Value of Rainfall of Various Duration for Manila) is summarized in the following table.

		10 min	30 min	1 hr	3 hr	12 hr
1/T = 1/5	(1) Manila	168.7	111.9	74.1	35.9	14.9
	(2) Porac	125.0	90.0	68.0	39.0	15.0
	= (1)/(2)	1.34	1.24	1.1	0.92	0.99
1/T = 1/10	(1) Manila	194.6	129.3	84.3	41.8	17.8
	(2) Porac	148.0	105.0	80.0	48.0	19.0
	= (1)/(2)	1.31	1.23	1.1	0.88	0.94
1/T = 1/20	(1) Manila	219.3	146.0	94.0	47.4	20.6
	(2) Porac	171.0	121.0	94.0	58.0	23.0
	= (1)/(2)	1.28	1.21	1.0	0.81	0.90
1/T = 1/50	(1) Manila	251.4	167.7	106.7	54.6	24.1
	(2) Porac	196.0	141.0	110.0	69.0	28.0
	= (1)/(2)	1.28	1.18	0.96	0.79	0.87
1/T = 1/100	(1) Manila	275.4	183.9	116.2	60.0	26.8
	(2) Porac	218.4	158.6	124.0	78.0	35.0
	= (1)/(2)	1.26	1.16	0.94	0.79	0.75

As shown in the Table, rainfall intensity at Manila especially in a short period of less than one hour can be said to surpass that at Porac by 20% or 30%. This phenomenon is attributed to measurement method, rainfall pattern and topographic feature. It is naturally contemplated, however, that rainfall intensity at Porac located near the hilly area should surpass that at Manila.

The relation between elevation and rainfall is studied as shown in FIGURE I-34. Assuming an average elevation of 800 m in the Project Area, rainfall intensity in season all rainfall will increase by 50%, therefore, the rainfall intensity in the hilly area is estimated, though approximate, by multiplying rainfall intensity at Porac by 1.5. The results thus obtained are given in FIGURE

Calculated Runoff Discharge		CASE D				
	Mancatian	No.1	No.2	No.3	No.4	No.5
A (km ²)	44	23.3	16.8	14.3	9.2	4.3
Q (m ³ /s)	900	630	540	480	380	190



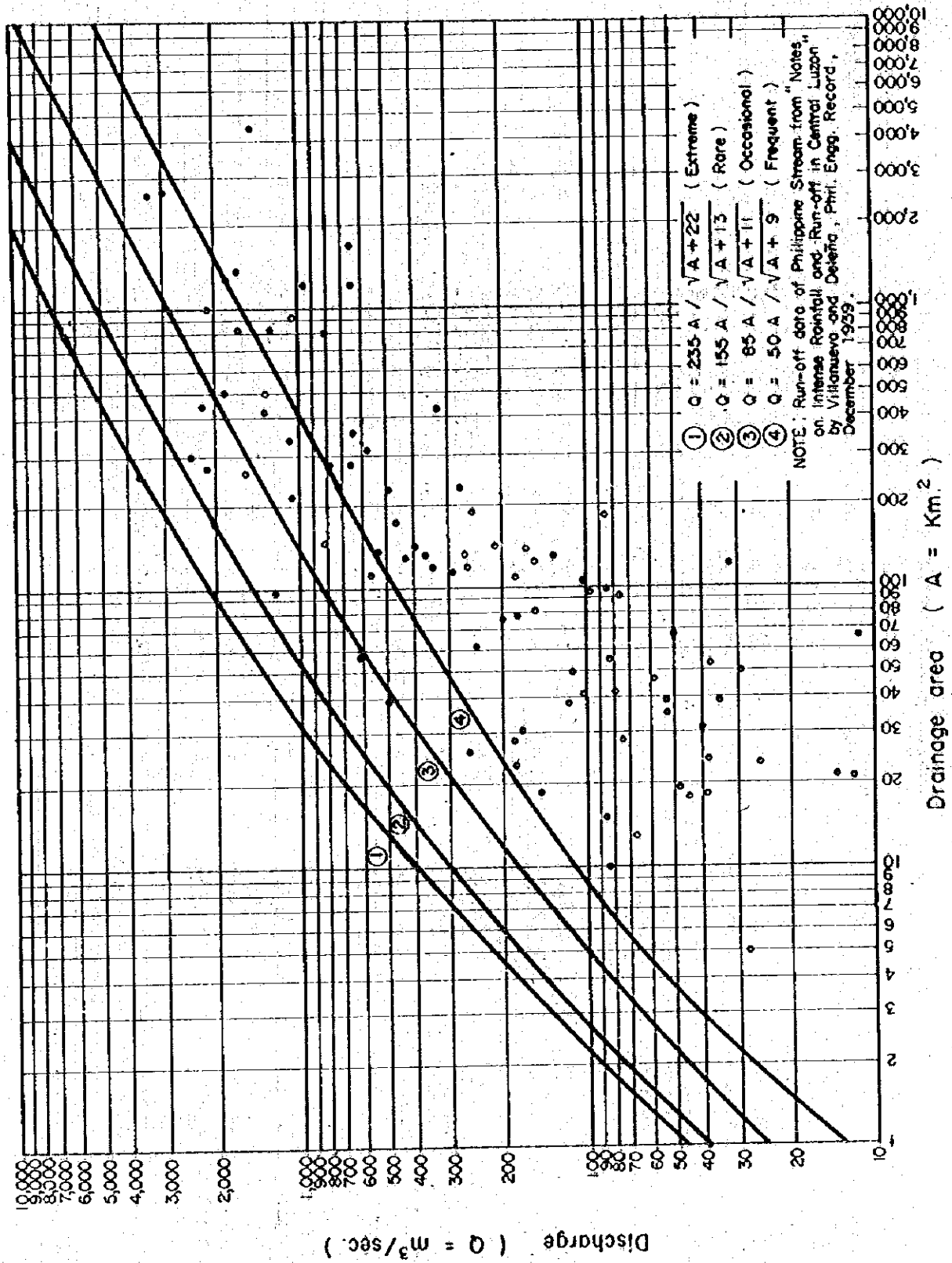


Figure I - 23 RELATION BETWEEN DRAINAGE AREA AND DISCHARGE IN THE PHILIPPINES

Table I-22-1 MAXIMUM DISCHARGE RECORD IN MAIN RIVERS (1)

RIVER BASIN	NAME OF RIVER	STATION	DRAINAGE AREA: MAX DISCHARGE A (km ²) (km ² (sq mi))	DATE OF OCCURRENCE	OBSERVATION PERIOD	Q_{max} / A	TYPE OF CLIMATE	REMARKS
1	LADAG	BONGA	534	9/6, 1947	1946-1965	8.22	/	
2	"	PRELACION	1355	9/30, 1962	1958-1965	8.37	"	
3	AERRA	PANG-OT	664	9/4, 1964	1957-1965	2.30	"	
4	"	BARANGANG	4813	8/31, 1962	1958-1965	2.23	"	
5	"	BIMAGCAY	2575	9/4, 1964	1958-1965	1.74	"	
6	"	LINESAD	120	7/21, 1962	1958-1965	7.93	"	
7	AGNO	ADAQAY	246	9/7, 1964	1959-1965	2.34	"	
8	"	SAN JOAQUE	1225	9/6, 1947	1945-1965	2.97	"	
9	"	STA MARVA	281	8/7, 1964	1958-1965	3.38	"	
10	"	CARMEN	2209	7/5, 1964	1945-1965	1.96	"	
11	"	POBLACION	2284	7/28, 1962	1945-1965	0.40	"	
12	"	PATLING	112	8/5, 1960	1958-1965	1.88	"	
13	"	PALOLUB	240	8/30, 1964	1964-1965	1.01	"	
14	"	BUISA	605	7/29, 1964	1960-1965	5.58	"	
15	"	TARLAC	872	6/8, 1961	1917-1965	2.17	"	
16	"	CAMILING	142	7/5, 1964	1964-1965	4.54	"	
17	"	POBLACION	289	7/6, 1964	1959-1965	4.57	"	
18	"	PACALAI	126	9/20, 1964	1957-1965	4.82	"	
19	"	DORONGAN	5134	9/2, 1963	1958-1965	2.50	"	
20	BALINGAGONG	BALINGAGONG	165	7/20, 1962	1959-1965	8.98	"	
21	NAYON	CULIBOLIS	128	7/14, 1957	1955-1965	14.81	"	
22	BUCAO	SAN JOAN	165	8/16, 1960	1955-1965	4.10	"	
23	STO TOMAS	DACAWAMAN	177	7/24, 1962	1947-1965	4.83	"	
24	GUAGUA	GUBLATN	128	9/30, 1964	1946-1965	2.93	"	
25	"	GUBERN BUDWAY	370	7/24, 1962	1958-1965	2.32	"	
26	"	POSAC	111	9/11, 1966	1910-1965	4.58	"	
27	"	VALVER	118	8/20, 1960	1958-1965	2.82	"	
28	PAMPANGA	CARRANGLAN	259	9/18, 1959	1958-1965	4.67	"	
29	"	PANTANGAN	253	7/3, 1965	1958-1965	4.60	"	

Table I-22-2 MAXIMUM DISCHARGE RECORD IN MAIN RIVERS (2)

RIVER BASIN	NAME OF RIVER	STATION	DRAINAGE AREA: A (km ²)	MAX. DISCHARGE Q _{max} (m ³ /s)	DATE OF OCCURRENCE	OBSERVATION PERIOD	Q = $\frac{Q_{max}}{A}$	TYPE OF CLIMATE	REMARKS
30	PAMPANGA	TANJUNGPURA	838	1412	8/4, 1963	1957-1965	1.68	"	
31	"	SANTUK	544	435	8/5, 1964	1957-1965	0.80	"	
32	"	CORUNEL	209	1224	1/5, 1964	1957-1965	1.73	"	
33	"	PALPANGA	205	1866	1/8, 1957	1957-1965	0.93	"	
34	"	CABU	143	221	9/2, 1963	1957-1965	1.90	"	
35	"	PAMPANGA	2441	2524	7/1, 1965	1964-1965	1.03	"	
36	"	SAN ANTON	2851	1674	2/22, 1962	1958-1965	0.59	"	
37	"	CHICO	149	584	9/30, 1964	1960-1965	3.78	"	
38	"	SUMACBAO	287	1414	1/23, 1960	1960-1965	4.93	"	
39	"	PENANGA	512	1265	9/11, 1946	1911-1965	2.47	"	
40	"	"	573	1304	1/12, 1957	1965-1965	2.43	"	
41	"	"	3467	1107	1/12, 1959	1958-1965	0.32	"	
42	"	PALWAG	284	461	1/13, 1957	1956-1965	1.63	"	
43	"	BENTUAN	208	886	8/5, 1961	1957-1965	4.26	"	
44	"	TALAVERA	261	294	8/9, 1964	1956-1965	3.43	"	
45	"	"	931	1179	1/6, 1959	1957-1965	2.73	"	
46	"	RIO CHICO	1177	530	8/16, 1960	1960-1965	0.45	"	
47	"	PALUA	148	192	6/27, 1960	1958-1965	1.30	"	
48	"	RIO CHICO	1246	444	8/9, 1964	1964-1965	0.36	"	
49	"	PAMPANGA	6487	2372	8/17, 1960	1946-1965	0.37	"	
50	"	MADUREI	102	462	9/23, 1955	1955-1965	4.53	"	
51	"	BALUNGA	204	850	9/2, 1961	1955-1965	4.17	"	
52	"	SAN MIGUEL	276	1051	1/13, 1960	1955-1965	4.10	"	
53	"	PAMPANGA	7468	1329	8/17, 1960	1958-1965	0.18	"	
54	"	MAASLEM	150	1399	1/13, 1960	1949-1965	9.33	"	
55	"	"	124	618	7/31, 1963	1950-1965	3.55	"	
56	"	ANGAI	629	3128	2/21, 1962	1956-1965	4.97	"	
57	"	"	959	1702	7/21, 1962	1909-1965	1.77	"	
58	LAGUNA	BALANAY	116	396	10/1, 1960	1958-1965	3.41	"	
59	LAGUNA	STA. CRUZ	103	298	10/28, 1953	1920-1965	2.89	"	

Table I-22-3 MAXIMUM DISCHARGE RECORD IN MAIN RIVERS (3)

RIVER BASIN	NAME OF RIVER	STATION	WRAHGE AREA A (km ²)	MAX DISCHARGE (m ³ /s)	DATE OF OCCURRENCE	OBSERVATION PERIOD	q _m (m ³ /s/A)	TYPE OF CLIMATE	REMARKS
10	LAGUNA LAKE	MARILUA	282	937	1/2, 1959	1958-1965	3.32	"	
11	"	"	494	2348	3/4, 1960	1958-1965	4.70	"	
12	MARAGONDON	MABALACAD	260	3495	6/20, 1964	1965-1965	13.40	"	
13	PALICO	BILARAN	158	1880	9/5, 1962	1960-1965	11.90	"	
14	TAL LAKE	PANSIHIT	644	63	9/6, 1962	1968-1965	0.10	"	
15	MALAGO	IDA CABANGAHAN	129	1801	1/14, 1964	1960-1965	13.96	"	
16	BAGO	MA-AO	423	2625	7/3, 1952	1949-1965	5.01	"	
17	BINALBAGAN	CADRE	380	296	8/6, 1962	1958-1965	0.85	"	
18	LILO	DAHILE	190	2480	12/1, 1956	1958-1965	1.78	"	
19	"	DANDAN	1453	2510	8/1, 1956	1958-1965	1.75	"	
20	"	HUKANGAN	392	249	1/20, 1965	1965	0.63	"	
21	"	PANGSOD	431	1391	4/25, 1955	1958-1965	3.04	"	
22	"	SAN JOAN	1947	1280	1/20, 1964	1958-1965	1.10	"	
23	"	CANDIAD	1959	1775	6/20, 1964	1958-1965	0.91	"	
24	BUKOP NORTE	GASGAS	73	1025		-21 Year	14.04	"	
25	"	BOLONG	40	475		15	10.78	"	
26	"	SIA MARIA	67	261		8	3.89	"	
27	LA UNION	MARAGAYAP	36	496		11	13.78	"	
28	PANGASTINAN	TAGAMOSING	53	214		7	4.03	"	
29	MT PROVINCE	BOXOD	48	125		6	2.81	3	
30	PANGASINAN	PAYA KAS	64	168		2	2.62	1	
31	ZAMBACES	BAGSIT	68	185		25	2.72	"	
32	BATAAN	BOLATE	16	90		21	5.62	"	
33	"	COED	26	148		10	4.95	"	
34	"	MIRAY	3	70		30	23.33	"	
35	"	PILAR	14	370		15	19.30	"	
36	PAMPALGA	CATHAMAN	22	289		11	11.78	"	
37	NEZA-EGEA	DIGMILA	52	191		6	2.92	"	
38	"	SANOR	89	277		8	3.11	"	
39	"	TABOATING	61	351		5	4.33	"	

Table I-22-4 MAXIMUM DISCHARGE RECORD IN MAIN RIVERS (4)

RIVER BASIN	NAME OF RIVER	STATION	BRANCHED AREA A (sq. mi.)	MAX PREVIOUS DISCHARGE (cfs)	DATE OF OCCURRENCE	OBSERVATION PERIOD	Q _{max} /A	TYPE OF CLIMATE	REMARKS
96	BULACAN	GARLANG	5	21	8.20	10	4.20	1	
97	LAGUNA	MARCA	35	41	1.17	11	1.17	1	
98	"	STA. MARIA	35	31	1.24	11	1.24	1	
99	"	MAVIN	45	180	3.11	11	3.11	1	
100	"	MARAPAN	86	351	8.28	15	8.28	1	
101	"	PAUTOK	85	91	1.07	11	1.07	1	
102	"	ARANGILAN	87	330	3.80	11	3.80	1	
103	CAVITE	ILANG-ILANG	60	489	8.15	11	8.15	1	
104	"	PANAYSAYAN	29	228	7.87	9	7.87	1	
105	BATANGAS	MOLINO	51	225	4.31	13	4.31	1	
106	"	DISACLI	25	12.5	0.50	8	0.50	1	
107	"	DUBACCAA	54	425	7.87	9	7.87	2	
108	"	IRIA	15	155	10.32	10	10.32	1	
109	"	DUMACA - A	74	574	7.75	20	7.75	1	
110	"	HIBANGA	5	38	7.60	9	7.60	1	
111	"	MORONG	12	97	8.08	9	8.08	1	
112	"	BULAKIN	10.5	57	5.43	7	5.43	1	
113	CAMARINES NORTE	DAET	80	468.8	-	11	5.84	1	
114	"	MATJEDON	28	232.0	-	15	8.53	1	
115	"	TALISAY	22	218	-	9	9.93	1	
116	ALBAY	UGSONG	11	159	-	11	14.35	1	
117	CAMARINES SUR	ANAYAN	17	319	-	11	18.75	1	
118	"	ASLONG	12	222	-	11	18.50	1	
119	"	HINGAYAN	23	101	-	11	4.39	1	
120	"	LAGUNDY	45	358	-	12	7.95	1	
121	"	TIGMAN	34	272	-	11	8.58	1	
122	SORSOGON	CUMADCAD	13	234	-	8	18.0	4	
123	"	CAWAYAN	15	28	-	12	5.20	1	
124	"	NAMUAI	10	216	-	12	21.60	1	
125	"	SAN FRANCISCO	36	302	-	12	8.38	1	

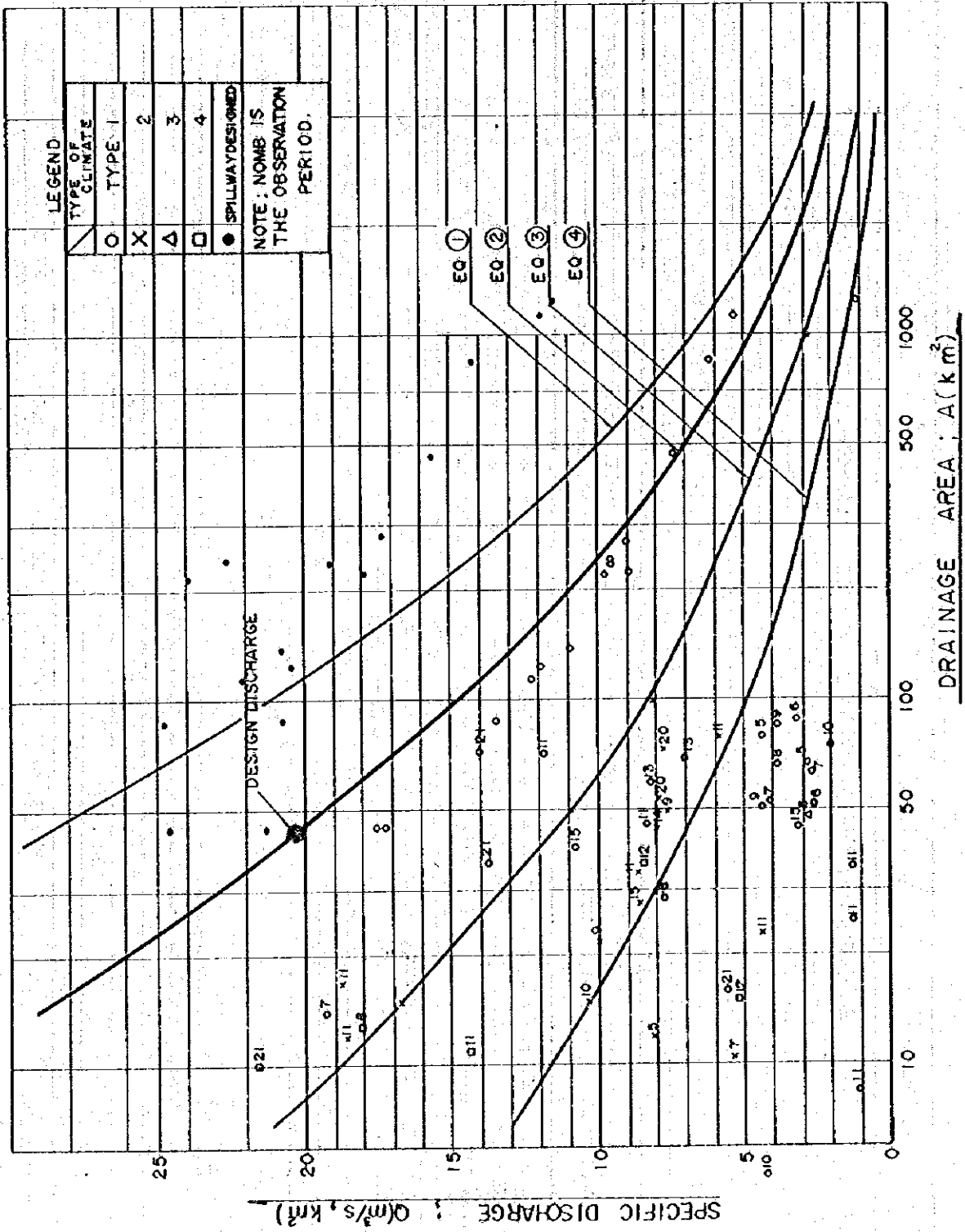
Table I-23 SPILLWAY DESIGN FLOOD PEAKS AND MAXIMUM OBSERVED FLOOD PEAKS

No.	DAM SITE	DRAINAGE AREA A(km ²)	DESIGN DISCHARGE Q _d (m ³ /s)	Q _D = Q _D /A	OBSERVED MAX. DISCHARGE [*] Q _{max} (m ³ /s)	q = Q _{max} /A	r = Q _d /Q _{max}
1	Pantabangan	845	12,045	14.3	5,170	6.1	2.33
2	Mt. Puncan	242	5,480	22.6	2,300	9.5	2.38
3	Papaya	124	2,535	20.4	1,478	11.9	1.72
4	Mt. Balintingon	228	4,065	17.8	2,024	8.9	1.83
5	Ligaya	477	7,450	15.6	3,500	7.3	2.80
6	Kalangan	89	2,215	24.8	1,200	13.5	1.85
7	Bulu	45.2	365	21.3	781	17.2	1.04
8	San Roque	1,221	13,940	11.4	1,350	1.1	2.20
9	Tayum	1,155	13,760	11.8	6,140	5.3	2.24
10	Lubas	89	1,850	20.7	1,200	13.5	1.54
11	San Nicolas	275	4,750	17.3	2,470	8.9	1.92
12	Sapinit	240	4,530	18.8	2,200	9.5	1.88
13	Balog-Balog	282	5,390	19.1	2,510	8.9	2.15
14	O'donnell	139	2,890	20.8	1,500	10.8	1.83
15	Bangat	44.5	1,095	24.6	778	17.5	1.41
16	Camiling	221	5,270	23.9	2,160	2.4	2.44
17	Pila	117	2,580	22.1	1,430	12.2	1.81

MAX. PEAK RATIO: r max = 2.44

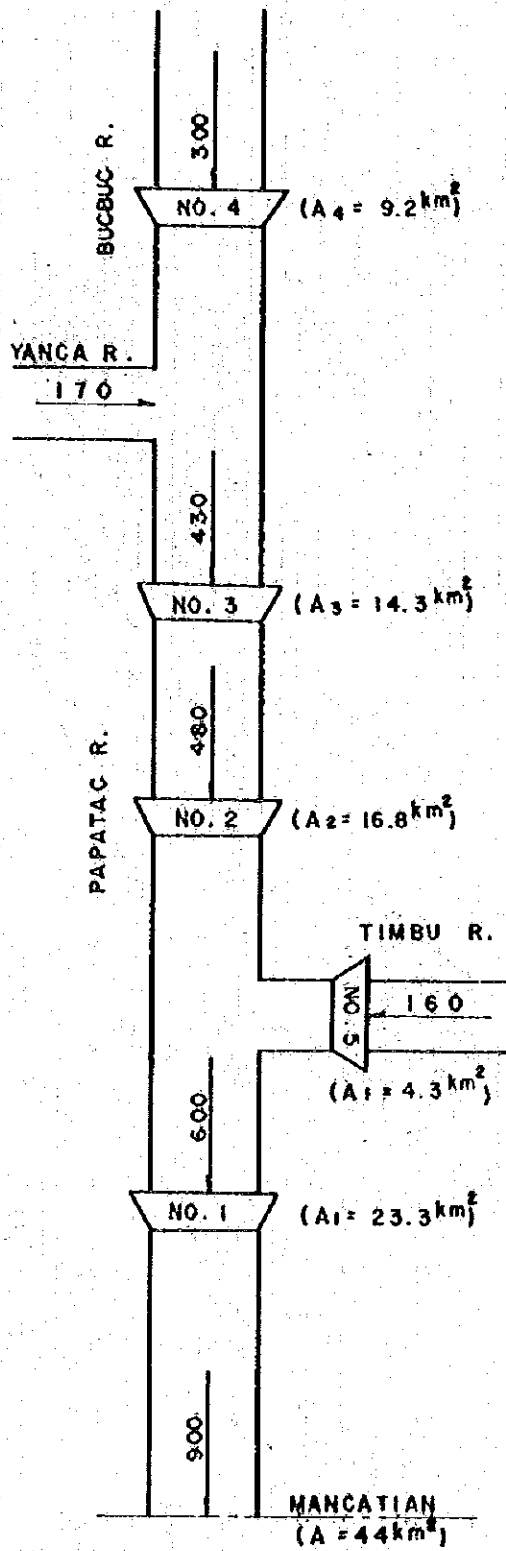
* Obtained from "Envelope Curve of Peaks V.S Drainage Area for Central Luzon.

Fig 1 24 DRAINAGE AREA(A) V.S SPECIFIC DISCHARGE (Q)



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Fig-II-25 CALCULATED RUNOFF DISCHARGE. — CASE A —



	A (km^2)	$155 A$	$\sqrt{A + 13}$	Q (m^3/s)	Q (m^3/s)	q ($\text{m}^3/\text{s}/\text{km}^2$)	REMARKS
NO. 1 DAM	23.3	3642	6.04	602.9	600	25.5	
2	16.8	2604	5.46	476.9	480	28.5	
3	14.3	2216	5.22	424.6	430	30.0	
4	9.2	1426	4.71	302.2	300	32.6	
5	4.3	666	4.19	160.2	160	37.2	
YANCA	4.6	713	4.19	169.9	170	36.9	
MANCATIAN	44.0	6820	7.35	903.0	900	20.4	

Fig. I-26 SPECIFIC DISCHARGE(Q) V.S DRAINAGE AREA (A)

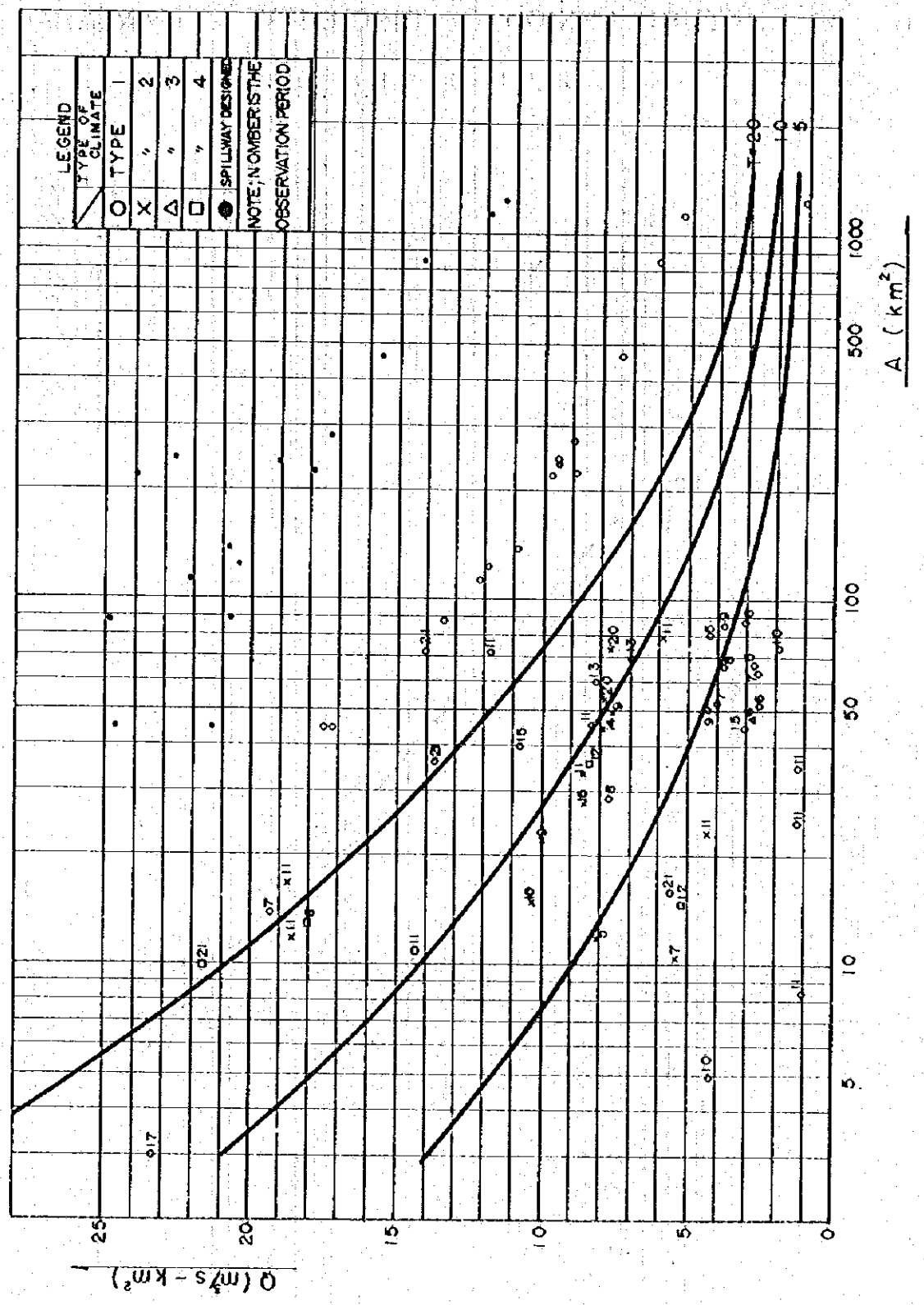


Fig. I-27 RETURN PERIOD OF DISCHARGE AT MANCIAN BRIDGE

FROM SPECIFIC DISCHARGE

1/T	Q _r
1/2	145
1/5	285
1/10	410
1/20	550
1/50	790
1/80	900
1/100	970
1/200	1200
1/500	1600

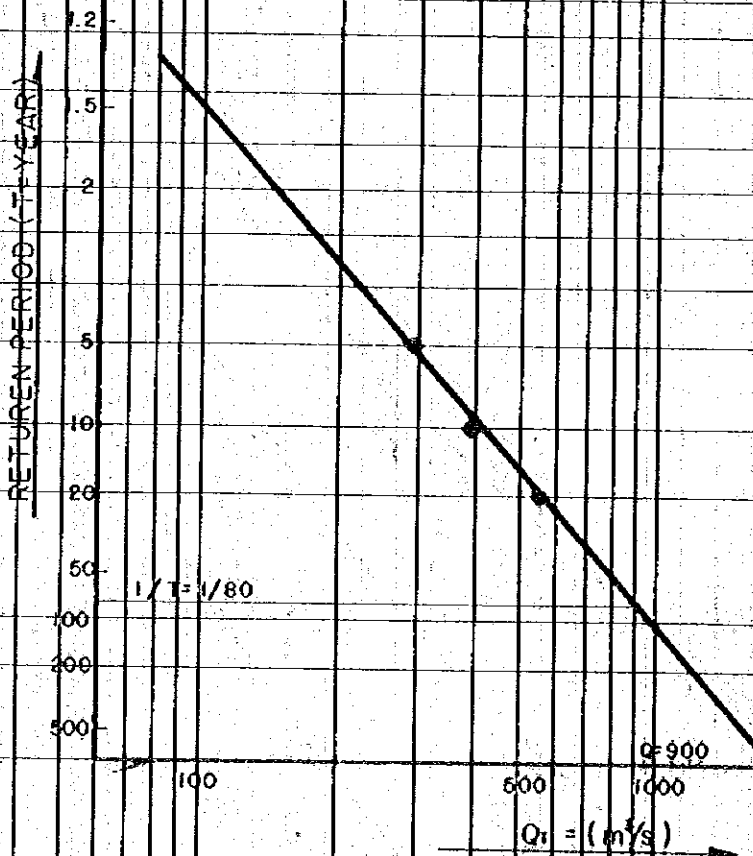


Fig. I-28 CALCULATED RUNOFF DISCHARGE - CASE B-

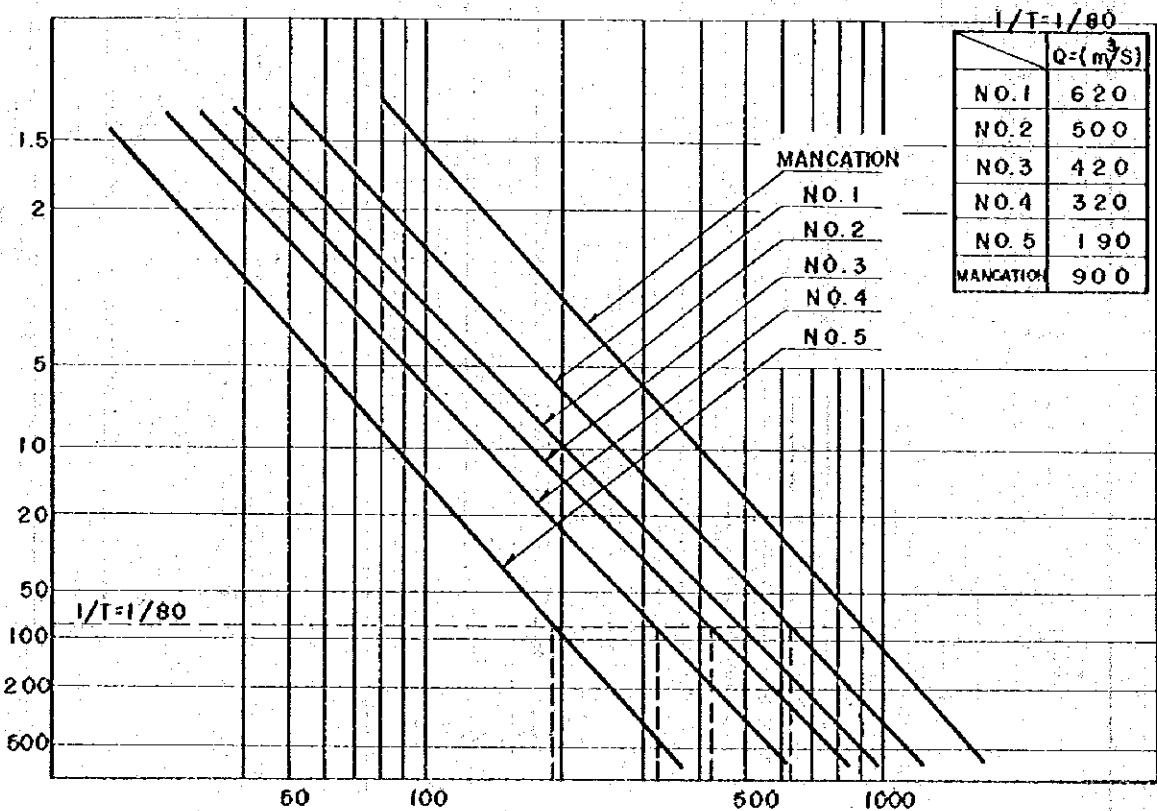
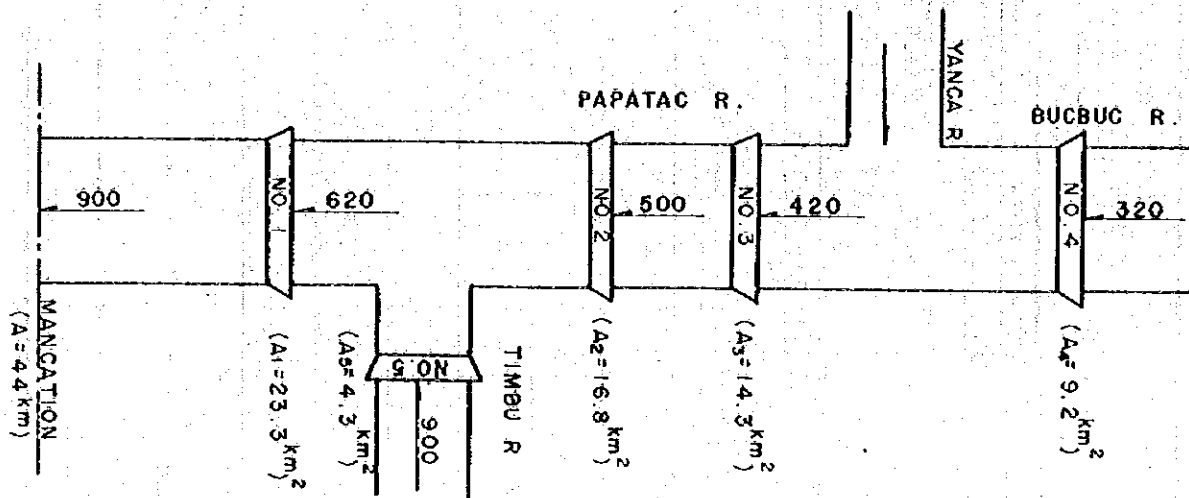
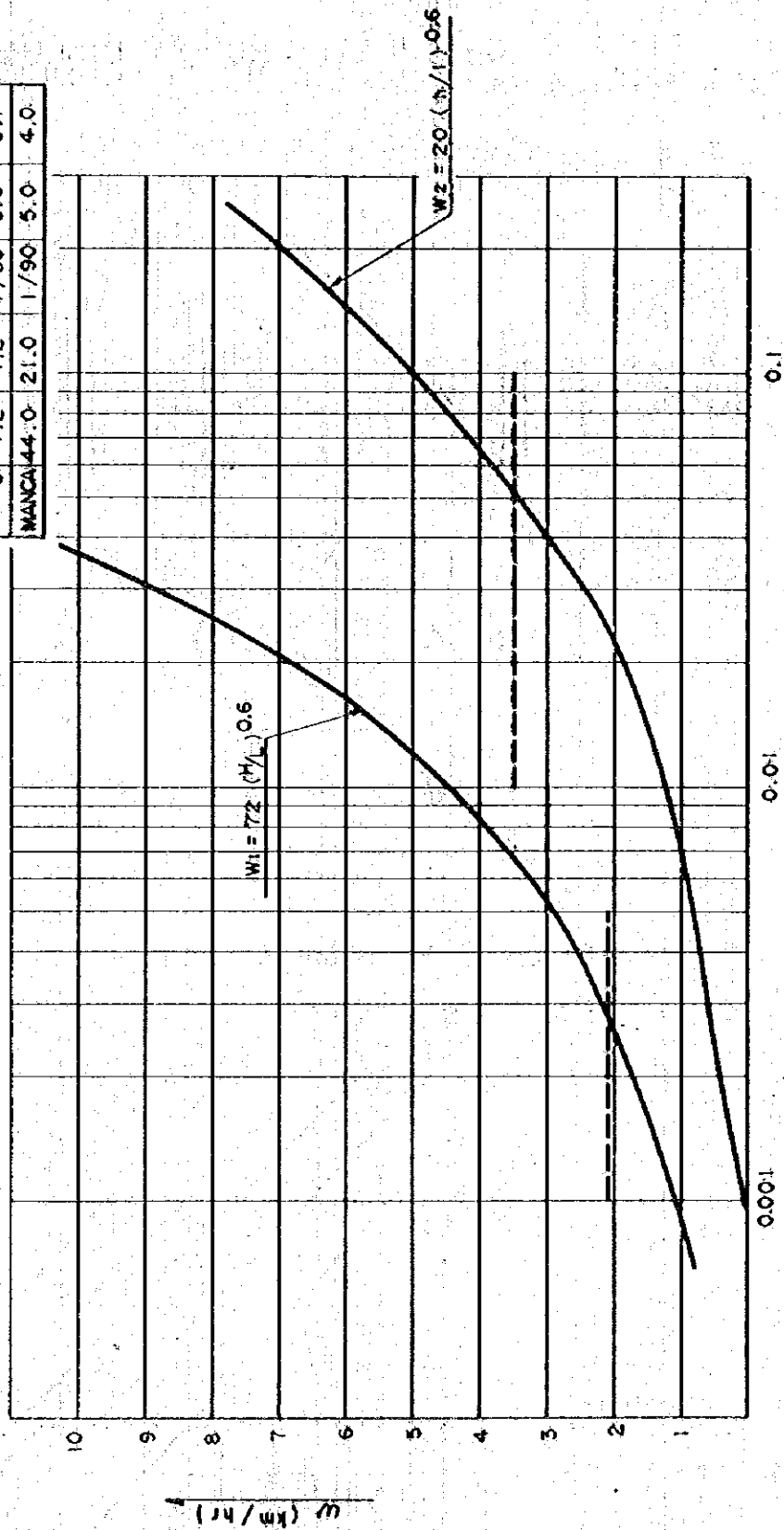


Fig-I-29 FLOOD PROPAGATE VELOCITY(W) V-S RIVER BED GRADIENT (L)

	A (km)	L (km)	H / L	W (k/h)	T (hr)
NO. 1	23.5	13.2	1/80	5.5	2.4
2	16.8	11.0	1/20	6.0	1.5
3	14.3	8.1	1/60	6.5	1.3
4	9.2	5.9	1/45	7.3	0.8
5	4.2	4.2	1/50	6.6	0.7
MANCA	44.0	21.0	1/90	5.0	4.0

— ; RUCHIHA
 - - - ; CRAVEN



L

Fig I-30 DAILY RAINFALL (R) V S RAINFALL INTENSITY (r_T)

$$r_T = \frac{R}{24} \left(\frac{24}{T} \right)^{2/3}$$

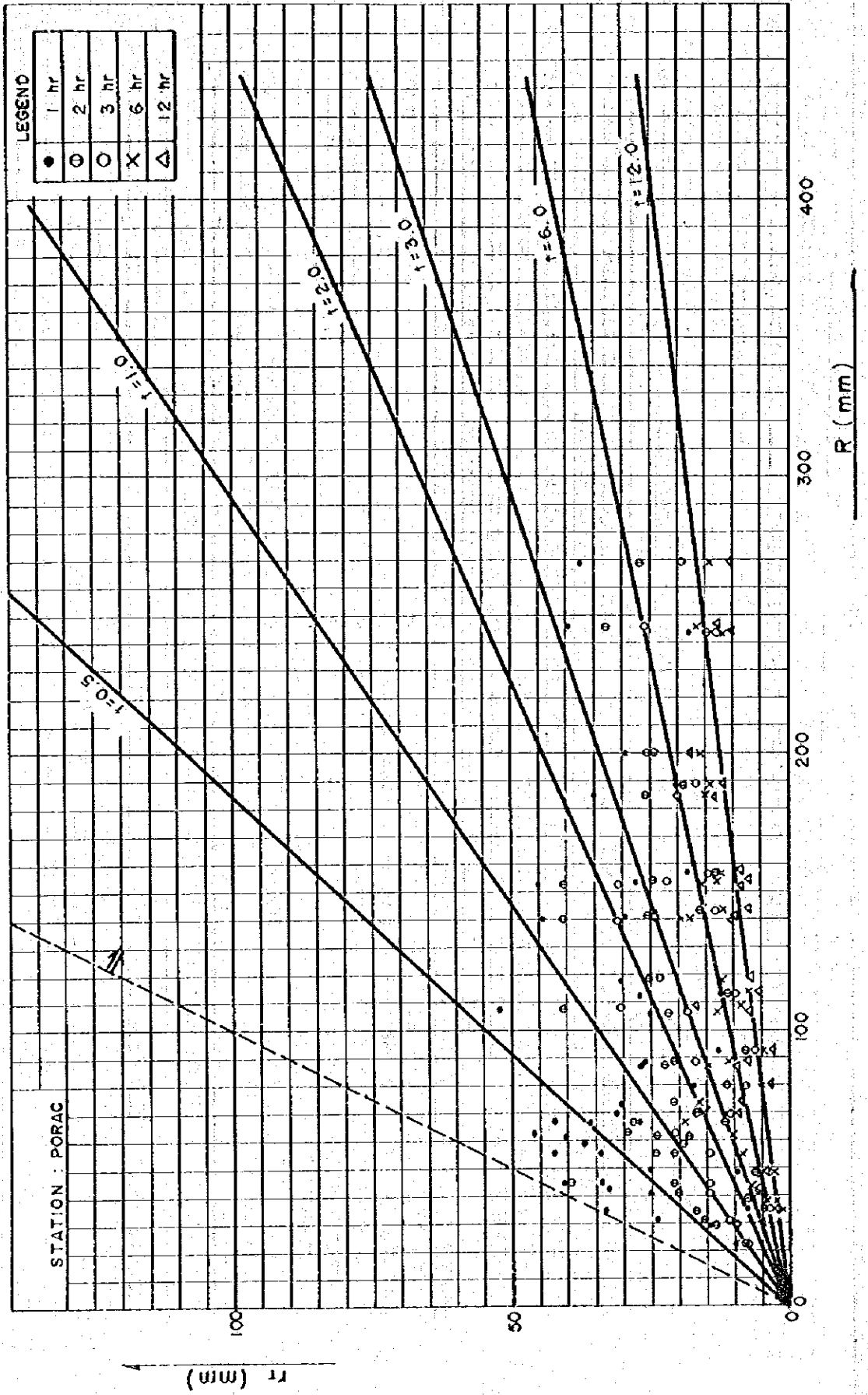


Fig. I-31 CALCULATED RUNOFF DISCHARGE. — CASE C —

	A (km ²)	T (hr)	R/24	(24/T) ^{2/3}	RT	RT (=1.5RT)	Q = $\frac{1}{36}$ RT · A (m ³ /S)
NO. 1	23.5	2.4	18.75	4.64	87	130.5	637
2	16.8	1.5	"	6.34	118	177.0	618
3	14.3	1.3	"	6.98	131	196.0	585
4	9.2	0.8	"	9.65	181	271.0	510
5	4.3	0.7	"	10.35	195	292.0	261
MANCATION	44.0	4.0	"	3.30	611	91.6	844

$RT = \frac{R}{24} \left(\frac{24}{T} \right)$, R $\frac{24}{60} = 450$ mm. (AT PORAC STATION)

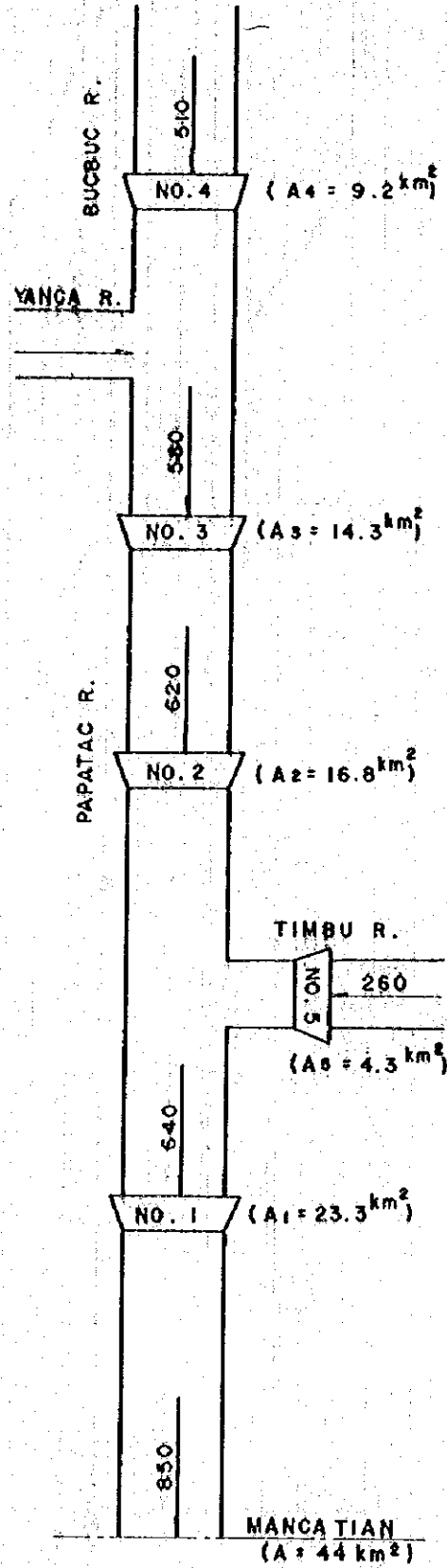


Table 24-1 DAILY RAINFALL V.S HOURLY RAINFALL INTENSITY

Flood	Daily Rainfall		1 hr		2 hr		3 hr		6 hr		12 hr		Remarks
	R (mm)	r1'	r1	r2'	r2	r3'	r3	r6'	r6	r12'	r12		
1969 July 20	56.8	24.0		41.9	20.0	43.9	14.3	51.6	8.6				
" 27	36.2	7.7		8.9	4.5	9.5	3.2	18.3	3.1	20.1	1.68		
" 28	38.9	12.8		14.8	7.4	14.8	4.9	23.3	4.0	24.3	2.02		
Sep 6	108.4	52.8		80.3	40.2	90.1	30.3	102.9	17.1	105.9	8.8		
Nov 23	31.8	29.2		30.8	15.4	31.3	10.4						
" 25	39.7	32.2		38.2	19.1	38.2	12.7	38.2	6.37	39.7	3.1		
1970 Jun 3	34.5	33.0		34.5	17.3								
" 13	47.6	9.8		14.0	7.0	20.6	7.0	27.3	4.5	31.5	2.6		
Jul 14	74.1	30.6		41.9	21.0	49.8	16.7	52.6	8.8	64.3	5.3		
" 28	31.0	28.5		29.0	15.0	29.5	9.8	50.5	5.1	31.5	2.63		
Aug 20	36.5	28.0		36.0	18.0					36.5	3.04		
" 31	93.2	13.1		16.1	8.1	17.6	6.0	26.3	4.4	37.9	3.15		
Sep 1	157.7	18.6		27.0	13.5	42.9	14.5	74.5	12.6	106.6	8.9		
1971 Jun 15	71.1	12.1		14.1	7.1	21.2	7.1	36.4	6.4	54.0	4.5		
" 16	89.7	26.2		42.4	21.2	51.5	17.2	68.6	11.4	88.7	7.4		
Jul 20	88.1	26.9		47.3	23.6	53.4	14.5	59.9	9.8	80.1	6.7		
" 21	44.2	9.9		18.3	9.2	21.3	7.1	33.8	5.6	41.7	3.5		
" 25	141.7	29.3		51.5	25.8	72.7	24.2	107.0	58.0	130.1	10.9		
" 30	45.5	40.5		44.0	22.0								
Sep 10	30.0	14.7		28.0	14.0	28.5	9.5	30.0	5.0				
" 24	63.4	46.8		58.0	29.1	63.4	21.1						
1972 Jun 24	67.0	42.5		56.5	28.3	57.0	19.0						
" 25	38.2	10.6		18.6	9.3	29.2	9.1	35.1	6.0	38.2	3.2		
Jul 7	185.0	135.0		53.0	26.5	60.0	20.0	89.0	15.0	174.5	14.5		
" 8	65.4	16.0		22.6	11.3	26.6	8.5	50.3	13.2				
" 9	69.4	31.7		33.7	16.9	33.7	11.2	66.2	11.0	88.0	7.3		
" 10	183.0	35.5		39.5	19.8	56.5	13.5	94.5	16.0	115.0	9.6		
" 17	188.5	20.5		38.0	19.0	52.0	17.0	83.0	14.0	148.5	12.1		
" 18	269.4	37.4		54.0	27.0	57.5	19.2	86.8	14.5	126.6	10.6		

r': Cumulative Hourly Rainfall, r: Hourly Rainfall Intensity, R: Daily Rainfall.

Table I-24-2 DAILY RAINFALL V-S HOURLY RAINFALL INTENSITY

FLOOD	R(mm)	1 hr		2 hr		3 hr		6 hr		12 hr		REMARK
		r'	r	r'	r	r'	r	r'	r	r'	r	
1972 JUL 19	244.0	18.0	30.0	15.0	40.0	13.3	79.0	13.0	130.0	10.8	10.8	
" 23	74.5	10.5	11.5	5.7	12.5	4.2	36.5	6.1	46.5	3.9	3.9	
" 27	114.0	12.0	22.0	11.0	29.0	9.7	44.5	7.4	65.5	5.5	5.5	
" 28	74.5	11.0	15.5	16.0	17.0	6.3	28.5	4.8	47.0	3.9	3.9	
" 29	118.5	30.3	51.0	25.5	67.2	23.1	73.1	12.1	80.2	6.7	6.7	
AUG 16	143.0	24.0	32.0	16.0	39.0	13.0	73.0	12.1	92.0	7.7	7.7	
1973 JUN 20	56.6	42.8	49.7	24.9								
JUL 5	39.0	14.5										
" 14	80.5	17.5	22.0	11.0	24.0	8.0	30.0	5.0	42.5	3.5	3.5	
" 15	73.0	14.5	10.5	8.3	18.0	6.3	23.5	4.0	63.0	5.3	5.3	
AUG 2	49.6	25.4	32.5	6.3	36.5	12.3	45.1	7.5				
" 3	107.5	25.0	44.0	22.0	54.0	18.0	86.0	13.0	87.5	7.3	7.3	
SEP 10	41.5	25.0	41.0	20.5	41.5	14.0						
OCT 8	84.9	20.9	22.8	11.4	27.1	9.0	39.6	6.6	58.7	4.9	4.9	
1974 APR 23	67.0	27.0	44.5	12.3	45.0	15.0						
" 25	70.0	46.6	54.4	27.2	55.9	18.6	56.4	9.4	70.0	5.8	5.8	
JUN 10	173.0	40.0	63.0	31.5	79.0	29.8	118.0	19.3	141.0	11.8	11.8	
JUL 2	43.5	33.0	10.5	5.3								
" 20	140.0	44.0	81.0	40.5	92.0	31.0	113.5	19.0	124	10.3	10.3	
AUG 15	154.6	28.0	49.0	24.5	66.0	22.0	79.5	13.3	90.5	7.5	7.5	
SEP 24	23.5	60.0	82.0	41.0	83.0	28.0						
OCT 17	113.5	27.0	34.5	7.9	39.0	13.0	50.0	8.3	87.0	7.3	7.3	
NOV 28	153.5	45.0	82.0	41.0	80.5	30.2	94.5	16.0	49.5	8.3	8.3	
1975 MAY 24	43.4	33.8	42.4	21.2	42.4	14.1	43.4	7.3				
AUG 26	60.6	40.9	48.5	24.3	55.1	18.7	64.1	10.0				
1976 MAY 23	246.9	39.4	67.2	33.6	78.4	26.1	99.6	16.6	21.1	17.6	17.6	
JUN 25	200.6	28.8	51.0	25.5	73.2	24.4	98.8	16.5	166.2	13.8	13.8	

PORAC

Fig 1-52 DAILY RAINFALL (R) V.S RAINFALL INTENSITY (IT)

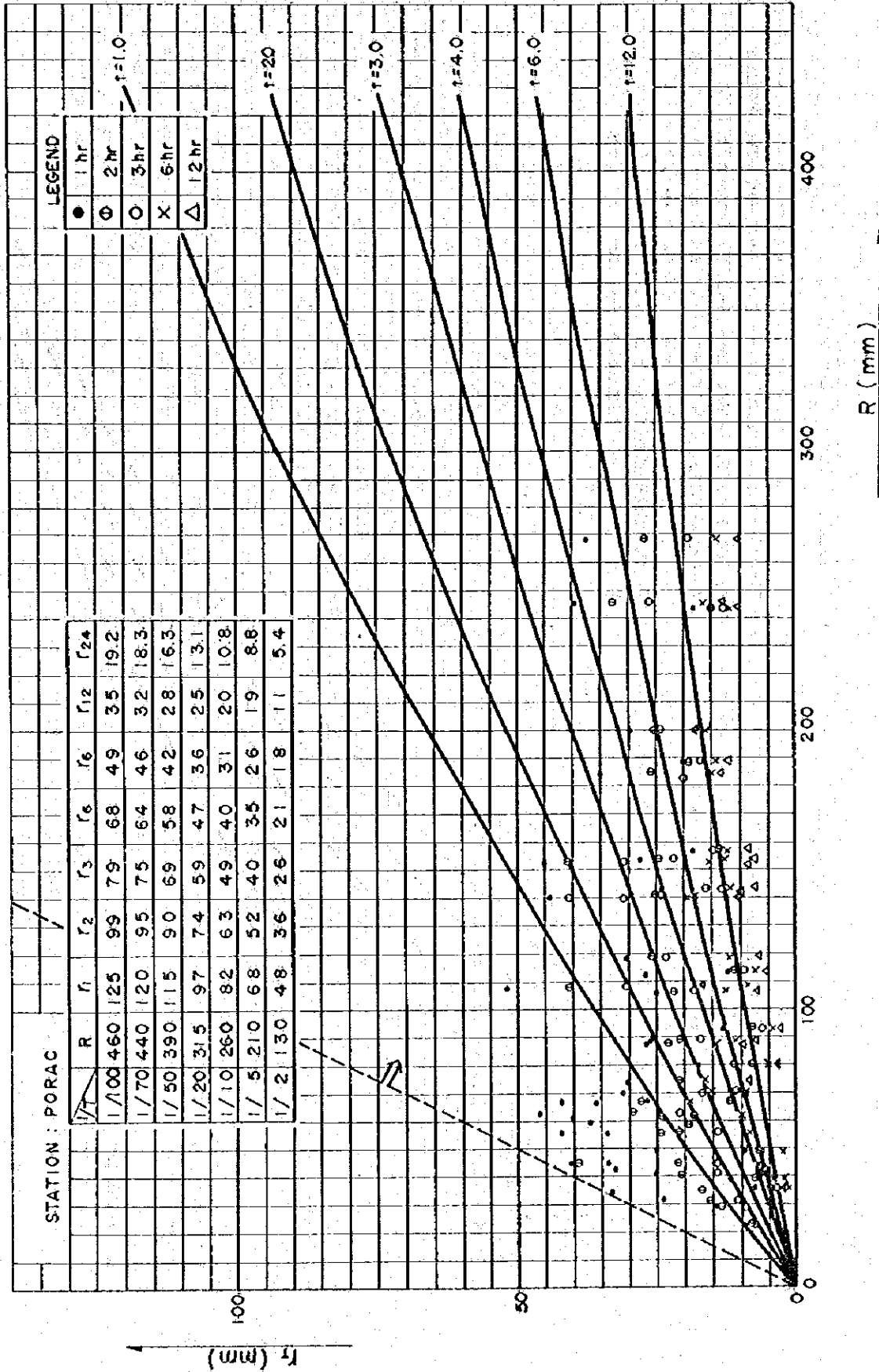
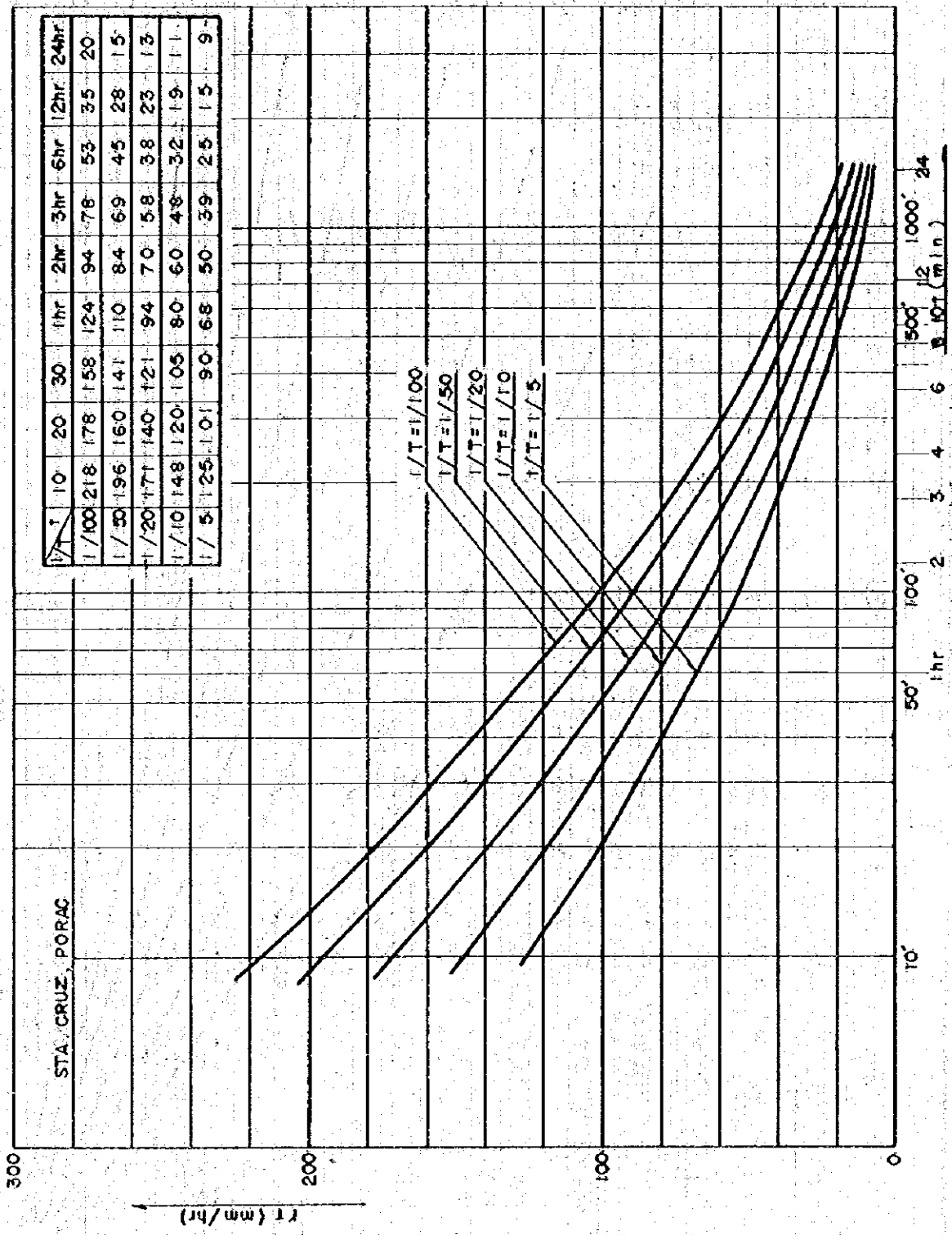


Fig-I-33 RAINFALL INTENSITY DURATION CURVE.(rt v.s.t)



10' 50' 100' 150' 200' 300' 400' 500' 600' 800' 1000' 24

hr (min)

300

200

100

0

rt (mm/hr)

STA. CRUZ, PORAC

$1/T = 1/100$
 $1/T = 1/50$
 $1/T = 1/20$
 $1/T = 1/10$
 $1/T = 1/5$

EXTREME VALUES OF RAINFALL OF VARIOUS DURATIONS

FOR

MANILA (PORT AREA)

COMPUTED EXTREME VALUES (IN MM) OF PRECIPITATION

RETURN PERIOD (YEARS)	5 MINS	10 MINS	15 MINS	30 MINS	60 MINS	2 HRS	3 HRS	6 HRS	12 HRS	24 HRS
2	13.2	21.6	28.2	42.8	58.7	72.7	81.5	102.8	126.2	162.4
5	16.9	28.1	37.0	55.9	74.1	95.4	107.8	139.8	178.7	237.8
10	19.4	32.4	42.9	64.7	84.3	110.4	125.3	164.2	213.4	287.8
15	20.8	34.9	46.1	69.6	90.0	118.9	135.2	178.0	232.9	315.9
20	21.8	36.6	48.4	73.0	94.0	124.8	142.1	187.6	246.7	335.6
25	22.5	37.9	50.2	75.7	97.1	129.4	147.4	195.1	257.2	350.8
50	24.9	41.9	55.7	83.8	106.7	143.4	163.8	218.0	289.8	397.6
100	27.2	45.9	61.1	92.0	116.2	157.4	180.0	240.7	322.1	444.1

INTENSITY (IN MM/HR) OF THE COMPUTED MAXIMUM PRECIPITATION

RETURN PERIOD (YEARS)	5 MINS	10 MINS	15 MINS	30 MINS	60 MINS	2 HRS	3 HRS	6 HRS	12 HRS	24 HRS
2	158.0	129.8	113.0	85.5	58.7	36.3	27.2	17.1	10.5	6.8
5	203.0	168.7	148.2	111.9	74.1	47.7	35.9	23.3	14.9	9.9
10	232.8	194.6	171.4	129.3	84.3	55.2	41.8	27.4	17.8	12.0
15	249.6	209.1	184.6	139.1	90.0	59.4	45.1	29.7	19.4	13.2
20	261.4	219.3	193.7	146.0	94.0	62.4	47.4	31.3	20.6	14.0
25	270.4	227.2	200.8	151.3	97.1	64.7	49.1	32.5	21.4	14.6
50	298.3	251.4	222.6	167.7	106.7	71.7	54.6	36.3	24.1	16.6
100	326.0	275.4	244.3	183.9	116.2	78.7	60.0	40.1	26.8	18.5

Fig. I-34 ELEVATION V-S MEAN SEASONAL (MAY TO OCT) RAINFALL.

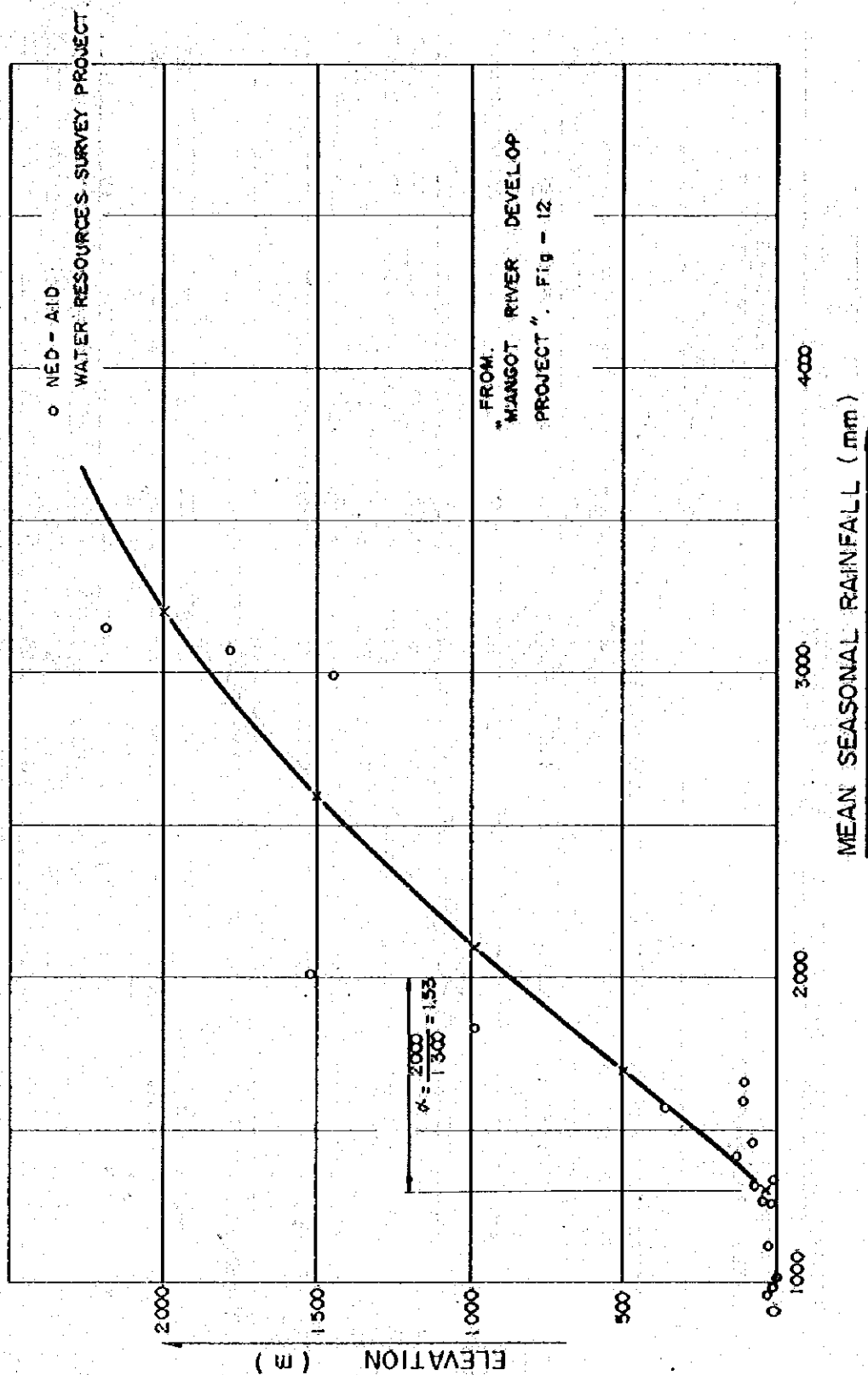


Table I-25 ELEVATION VS MEAN SEASONAL (JUNE TO SEP.) RAINFALL

NO	Stations	Mean Seasonal Rainfall (mm)	Elevation (m)	Year of Period	NO of Year Recorded
1	BONGABON, N. ECJA	1320	70	1928 ~ 1934 1936 ~ 1939	11
2	MUNOZ, N. ECJA	1270	50	1945 ~ 1950	16
3	PENARANDA RIS POB N.E	988	15	1945 ~ 1961	15
4	SAN JOSE REF. SU JOSE N.E	1635	106	1945 ~ 1958	13
5	CABANATUAN CITY, N. ECJA	1155	30	1935 ~ 1960	15
6	BAGUIO CITY, MT. PROVINCE	3000	1450	1902 ~ 1934 1936 ~ 1939	32
7	SUGARMILL CABIAO N. ECJA	1115	10	1938 ~ 1934 1936 ~ 1938	10
8	AMBUKLAO PRAJ. MT. PROY	1340	995	1949 ~ 1960	12
9	SAN ISLARO, NUEVA ECJA	1268	20	1903 ~ 1906 1908 ~ 1932	29
10	CAMILING, TARAC	1340	15	1921 ~ 1931	11
11	O'DONNELL RIS. SU. MIGUEL, TARAC	1415	130	1949 ~ 1962	11
12	ABOAY KABAYAN MT. PRAY	1585	370	1950 ~ 1962	12
13	PENARANDA, RIS SU JOSEF NE	963	30	1945 ~ 1960	15
14	SANTON RIS SU VICEUTE, LOUR NE	1460	80	1958 ~ 1962	3
15	PALPALAN, MT. PROVINCE	3090	1785	1922 ~ 1932	10
16	ATOK, MT. PROVINCE	3150	2290	1921 ~ 1932 1930 ~ 1951	24
17	BNGUET MINES, MT. PROVINCE	2015	1520	1947 ~ 1961	15
18	TARAVERA RIS HEADWORKS NE.	1600	105	1946 ~ 1960	15

* "NEO-AID WATER RESOURCES SURVEY PROJECT" CENTRAL LUZON BAINN HYDROLOGY, FLOOD CONTROL AND DRAINAGE DIVISION B. P. W. JULY 1964.

** ELEVATION ARE OBTAINED FROM AMONG MAPS AND WEATHER PURAN SCIENTIFIC PAPER, NO. 407 "ANNUAL CHIMATOLOGICAL SERIES".

Table I-26 RUN - OFF DISCHARGE.

Moncollan

t/T	A (km ²)	t (hr)	r (mm/h)	r' (mm/h)	$Q_p = \frac{1}{3.6} i. r. A$	
1/100	44.0	4.0	68	102	$Q = \frac{1}{3.6} \times 0.75 \times 102 \times 44 = 935$	940
1/50	"	"	57	85.5	$Q = \frac{1}{3.6} \times 0.75 \times 85.5 \times 44 = 783$	780
1/20	"	"	45	67.5	$Q = \frac{1}{3.6} \times 0.75 \times 67.5 \times 44 = 618$	600
1/10	"	"	40	60.0	$Q = \frac{1}{3.6} \times 0.75 \times 60 \times 44 = 511$	510
1/5	"	"	32	48.0	$Q = \frac{1}{3.6} \times 0.75 \times 48 \times 44 = 409$	410

NO. 1 DAM

t/T	A (km ²)	t (hr)	r (mm/h)	r' (mm/h)	Q_p	
1/100	23.5	2.4	86	129	$Q = \frac{1}{3.6} \times 0.75 \times 129 \times 23.5 = 631$	630
1/50	"	"	76	114	$Q = \frac{1}{3.6} \times 0.75 \times 114 \times 23.5 = 557$	560
1/20	"	"	62	93	$Q = \frac{1}{3.6} \times 0.75 \times 93 \times 23.5 = 458$	460
1/10	"	"	52	78	$Q = \frac{1}{3.6} \times 0.75 \times 78 \times 23.5 = 381$	380
1/5	"	"	44	66	$Q = \frac{1}{3.6} \times 0.75 \times 66 \times 23.5 = 322$	320

NO. 2

t/T	A (km ²)	t (hr)	r (mm/h)	r' (mm/h)	Q_p	
1/100	16.8	1.5	105	157.5	$Q = \frac{1}{3.6} \times 0.75 \times 157.5 \times 16.8 = 651.3$	660
1/50	"	"	93	140	$Q = \frac{1}{3.6} \times 0.75 \times 140 \times 16.8 = 490$	490
1/20	"	"	80	120	$Q = \frac{1}{3.6} \times 0.75 \times 120 \times 16.8 = 420$	420
1/10	"	"	68	102	$Q = \frac{1}{3.6} \times 0.75 \times 102 \times 16.8 = 352$	352
1/5	"	"	56	84	$Q = \frac{1}{3.6} \times 0.75 \times 84 \times 16.8 = 280$	280

NO. 3

t/T	A (km ²)	t (hr)	r (mm/h)	r' (mm/h)	Q_p	
1/100	14.3	1.3	112	168	$Q = \frac{1}{3.6} \times 0.75 \times 168 \times 14.3 = 498$	500
1/50	"	"	98	147	$Q = \frac{1}{3.6} \times 0.75 \times 147 \times 14.3 = 436$	440
1/20	"	"	84	126	$Q = \frac{1}{3.6} \times 0.75 \times 126 \times 14.3 = 374$	370
1/10	"	"	71	106	$Q = \frac{1}{3.6} \times 0.75 \times 106 \times 14.3 = 315$	310
1/5	"	"	60	90	$Q = \frac{1}{3.6} \times 0.75 \times 90 \times 14.3 = 267$	260

NO. 4

t/T	A (km ²)	t (hr)	r (mm/h)	r' (mm/h)	Q_p	
1/100	9.2	0.8	135	195	$Q = \frac{1}{3.6} \times 0.75 \times 195 \times 9.2 = 374$	370
1/50	"	"	120	180	$Q = \frac{1}{3.6} \times 0.75 \times 180 \times 9.2 = 345$	345
1/20	"	"	102	153	$Q = \frac{1}{3.6} \times 0.75 \times 153 \times 9.2 = 293$	290
1/10	"	"	88	132	$Q = \frac{1}{3.6} \times 0.75 \times 132 \times 9.2 = 253$	250
1/5	"	"	72	108	$Q = \frac{1}{3.6} \times 0.75 \times 108 \times 9.2 = 207$	200

NO. 5

t/T	A (km ²)	t (hr)	r (mm/h)	r' (mm/h)	Q_p	
1/100	4.3	0.7	140	210	$Q = \frac{1}{3.6} \times 0.75 \times 210 \times 4.3 = 186$	190
1/50	"	"	126	189	$Q = \frac{1}{3.6} \times 0.75 \times 189 \times 4.3 = 168$	170
1/20	"	"	106	159	$Q = \frac{1}{3.6} \times 0.75 \times 159 \times 4.3 = 141$	140
1/10	"	"	92	138	$Q = \frac{1}{3.6} \times 0.75 \times 138 \times 4.3 = 122$	120
1/5	"	"	77	116	$Q = \frac{1}{3.6} \times 0.75 \times 116 \times 4.3 = 102$	100

Fig. I-35 RETURN PERIOD OF RUN-OFF DISCHARGE AT MANCATIAN B.
 FROM RAINFALL DURATION INTENSITY CURVE

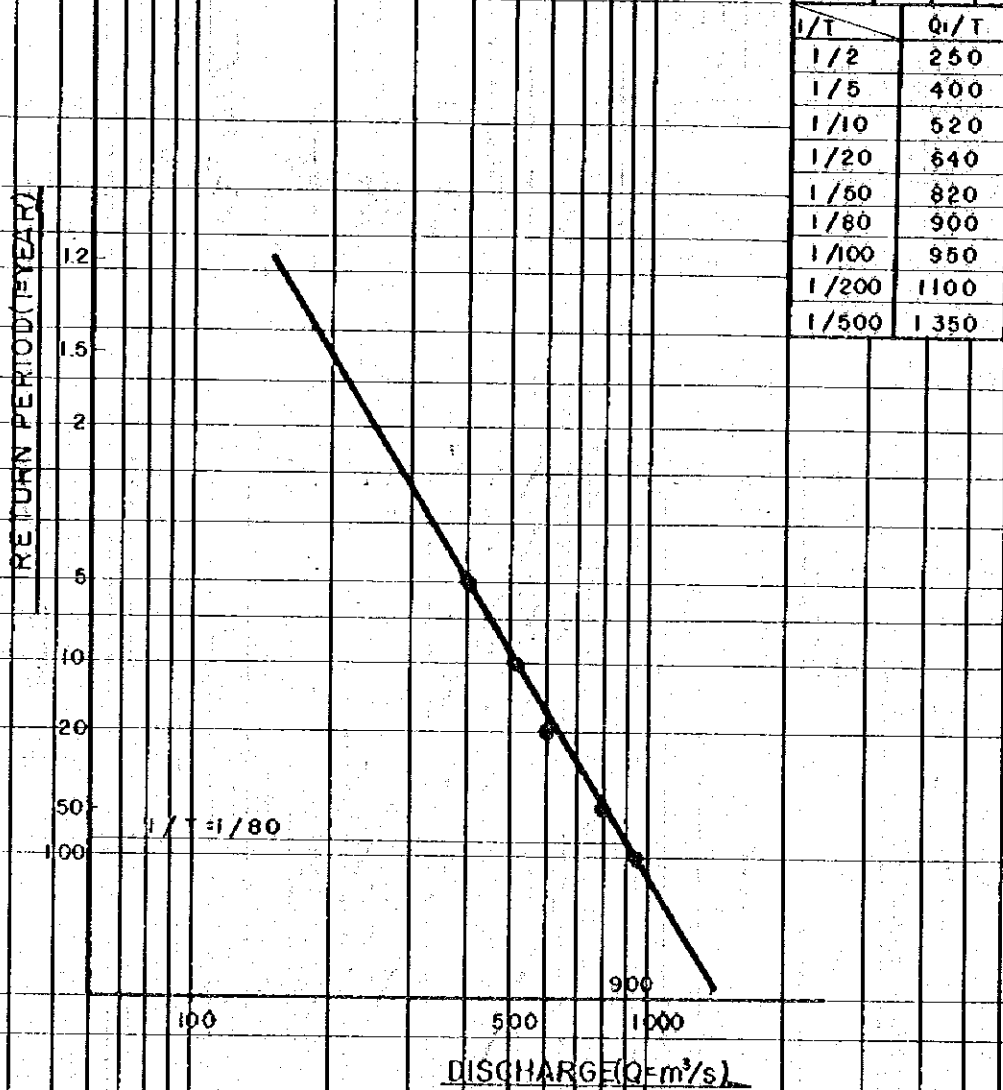


Fig. I-36 CALCULATED RUNOFF DISCHARGE — CASE D —

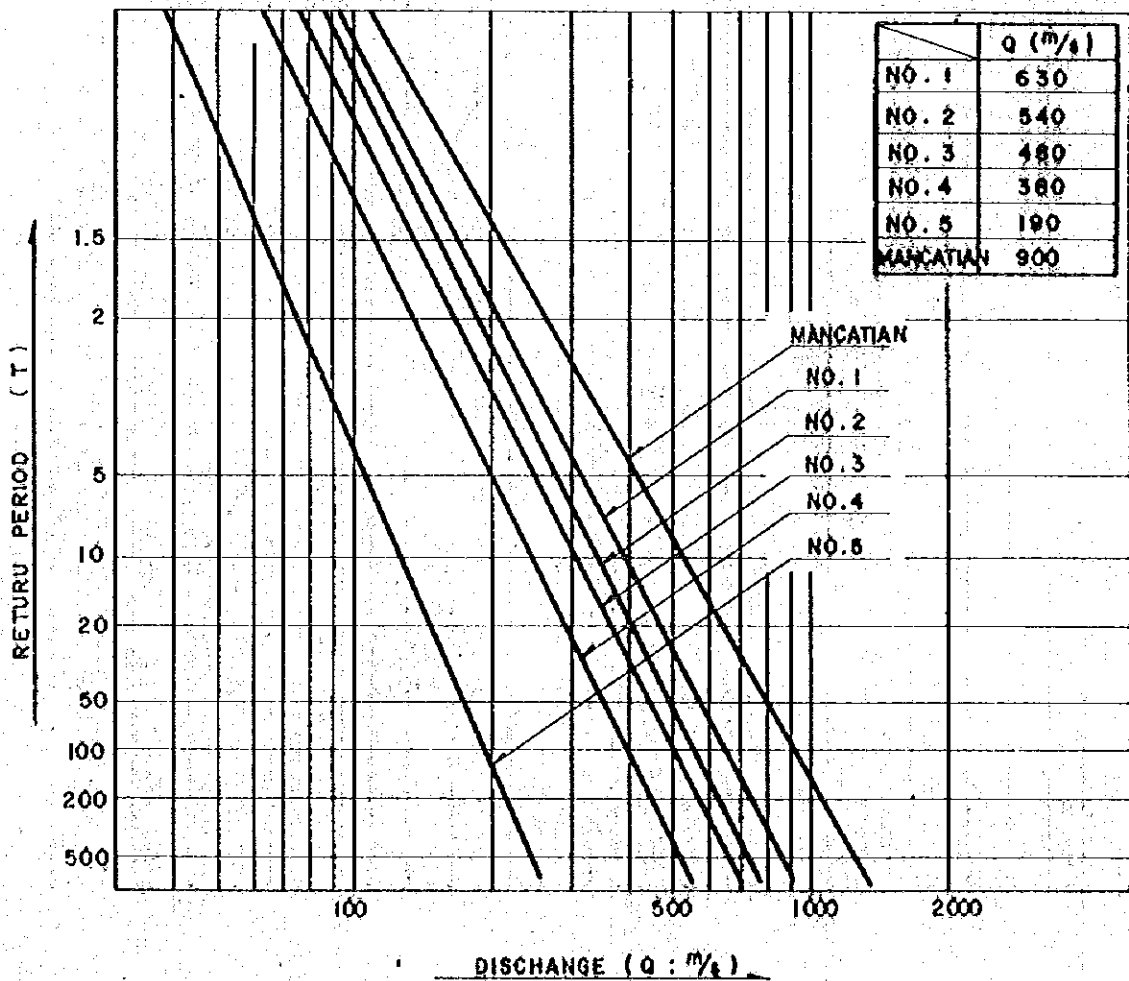
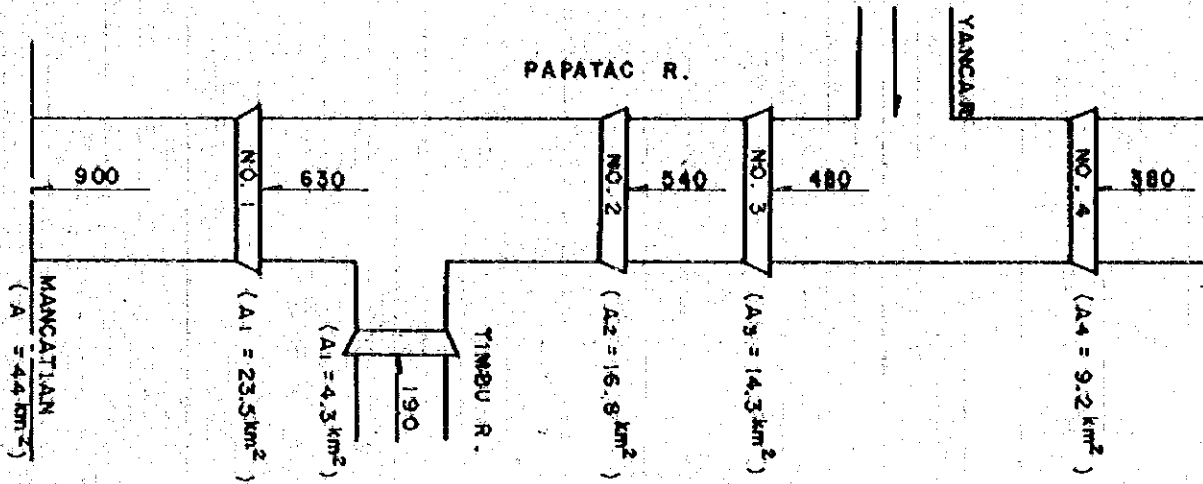
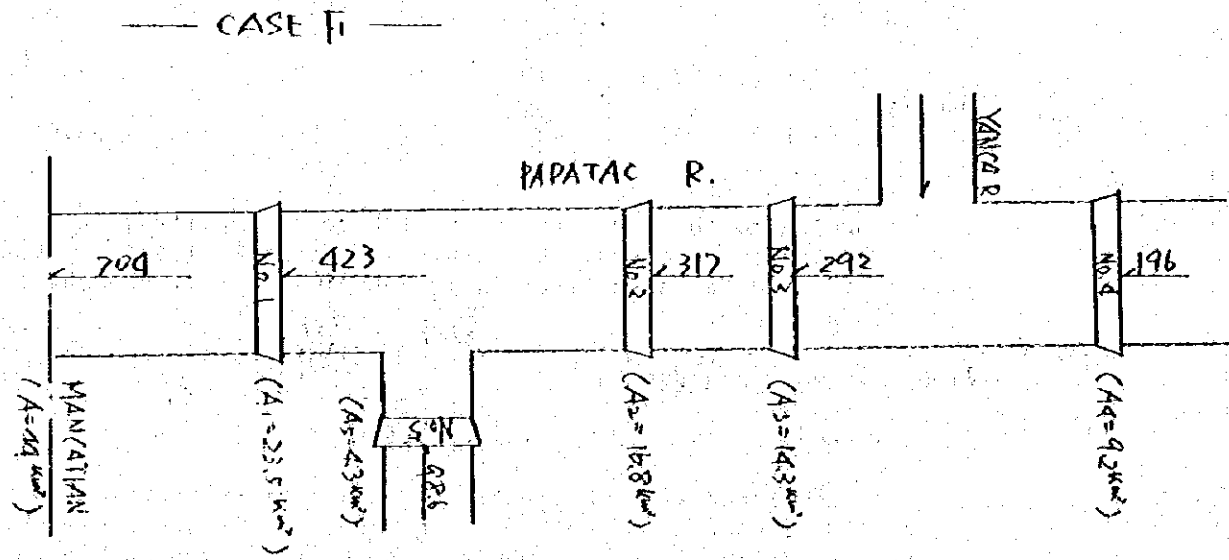
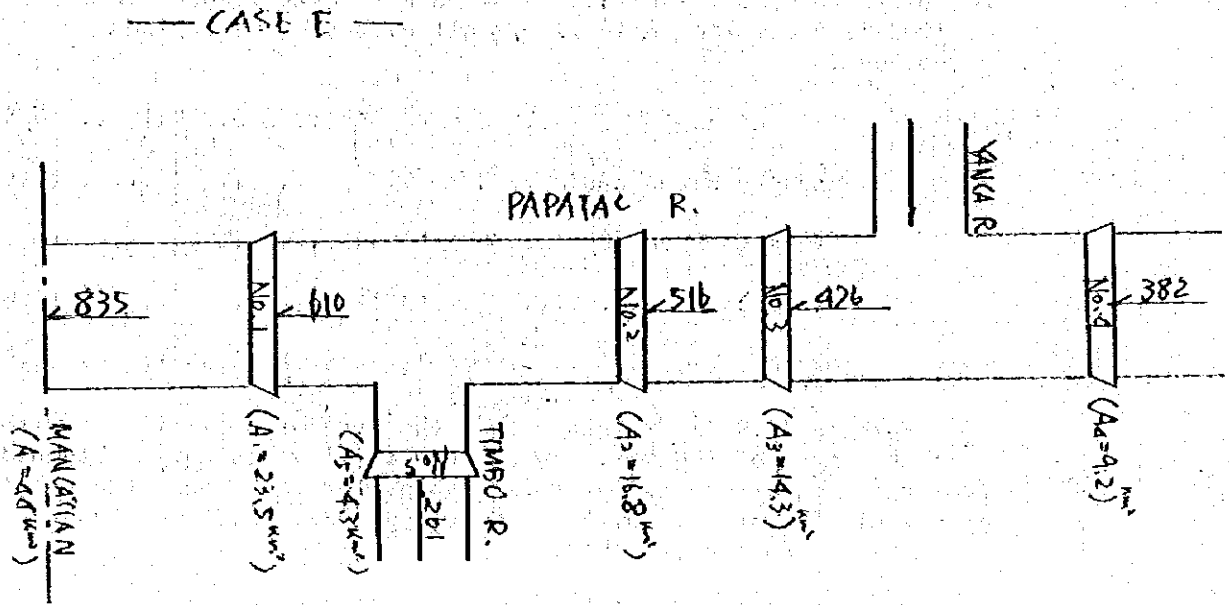


Fig. I - 37 CALCULATED RUNOFF DISCHARGE



5-4 Based on the Formula Proposed in Pampanga Basin

According to M.M. Obradovich* below-mentioned the calculation formulars for peak discharge based on discharge records in a period of 1908-1965 covering main rivers all over the Philippines are recommended.

All Philippine $Q = 27.5A (1.2A-0.05-1) \dots (5.5)$

Cagayan River Basin .. $Q = 269A \dots (5.6)$

Pampanga River Basin.. $Q = 126A \dots (5.7)$

Mindanao $Q = 134A \dots (5.8)$

Luzon $Q = 22.5A (1.1A-0.05-1) \dots (5.9)$

Formula (5.9) is resulted from the data of small rivers with a catchment area of 100 km². Though probability discharge obtained from these equations is not so reliable, for reference, the results of calculation of Eq. (5.7) — Case E and Eq. (5.9) — Case F are given in the following table.

Calculated Runoff Discharge ——— CASE E and CASE F ———

		Mancatian No.1	No.2	No.3	No.4	No.5	
A (km ²)		44	23.3	16.8	14.3	9.2	4.3
Q(m ³ /s)	CASE E	835	610	516	476	382	261
	CASE F	704	423	317	292	196	98.6

According to the above table, Case F reads smaller values compared with other cases. The results of Equation (5.7) — Case E utilized for the Pampanga River Basin are similar to those obtained from rainfall data.

III-6 Design Discharge

6-1 Comparison of Runoff Investigation

Results of runoff investigation in III-5 are summarized in TABLE , CASE B, CASE C and CASE D among the cases in the table have been statistically asured. However, Case C is not reasonable to be adopted to a small river basin with a short time of flood concentration. And Case B is the specific discharge based on discharge data

* [Envelope Curves For Peak Discharges in the Philippines] by Dipl. Eng. M.M. OBRADOVICH, Technical series No.18, WMO/UNDP Project, January, 1973.

all over the Philippines. Therefore, Case D based on rainfall records at Porac shall be used as a design high-water discharge.

6-2 Determination of Design Discharge

Design high-water discharge (probability: $T = 80$) at each station to which CASED is adopted is presented in the table below. Double section shall be adopted to Pasig Potreró River having a serious variation of flow regime to protect and maintain the levees and the channel. This design high-water discharge is estimated at $120 \text{ m}^3/\text{s}$ ($T = 1.1$).

Design Discharge Distribution

	Mancatian	No.1 -DAM	No.2 -DAM	No.3 -DAM	No.4 -DAM	No.5 DAM
A (km^2)	44	23.5	16.8	14.3	9.2	4.3
Q (m^3/s)	900	630	540	480	380	190

Table I-27 COMPARISON OF CALCULATED RUNOFF DISCHARGE

	CASE A	CASE B	CASE C	CASE D	CASE E	CASE F	CASE G
NO.1 DAM	600	620	640	630	610	423	700
NO.2	480	500	620	540	516	317	
NO.3	430	420	580	480	476	292	
NO.4	300	320	(510)	380	382	196	
NO.5	160	190	(280)	190	261	986	
MANCATION	900	900	850	900	835	704	

CASE A : FROM EQ 2, $Q = 155A/\sqrt{A+13}$

CASE B : FROM SPECIFIC DISCHARGE DATA (1/T = 1/80)

CASE C : FROM RAINFALL DATE (BY USING DR MONONOBE EQ $n = \frac{R}{24} (\frac{24}{T})^{2/3}$)

CASE D : " (1/T=1/80)

CASE E : RUNOFF EQ. $Q = 126A^{\sqrt{A}}$ (PANPANGA BASIN $A > 100 \text{ km}^2$) BY M. M. OBRADOVICH

CASE F : " $Q = 22.5A^{(1.14-0.001)}$ ($A < 100 \text{ km}^2$)

CASE G : DR.ENDO MEMO

Fig I-58 DESIGN DISCHARGE DISTRIBUTION

