

FIGURE 4B-12. DRAINAGE SYSTEM IN THE PADDY FIELD

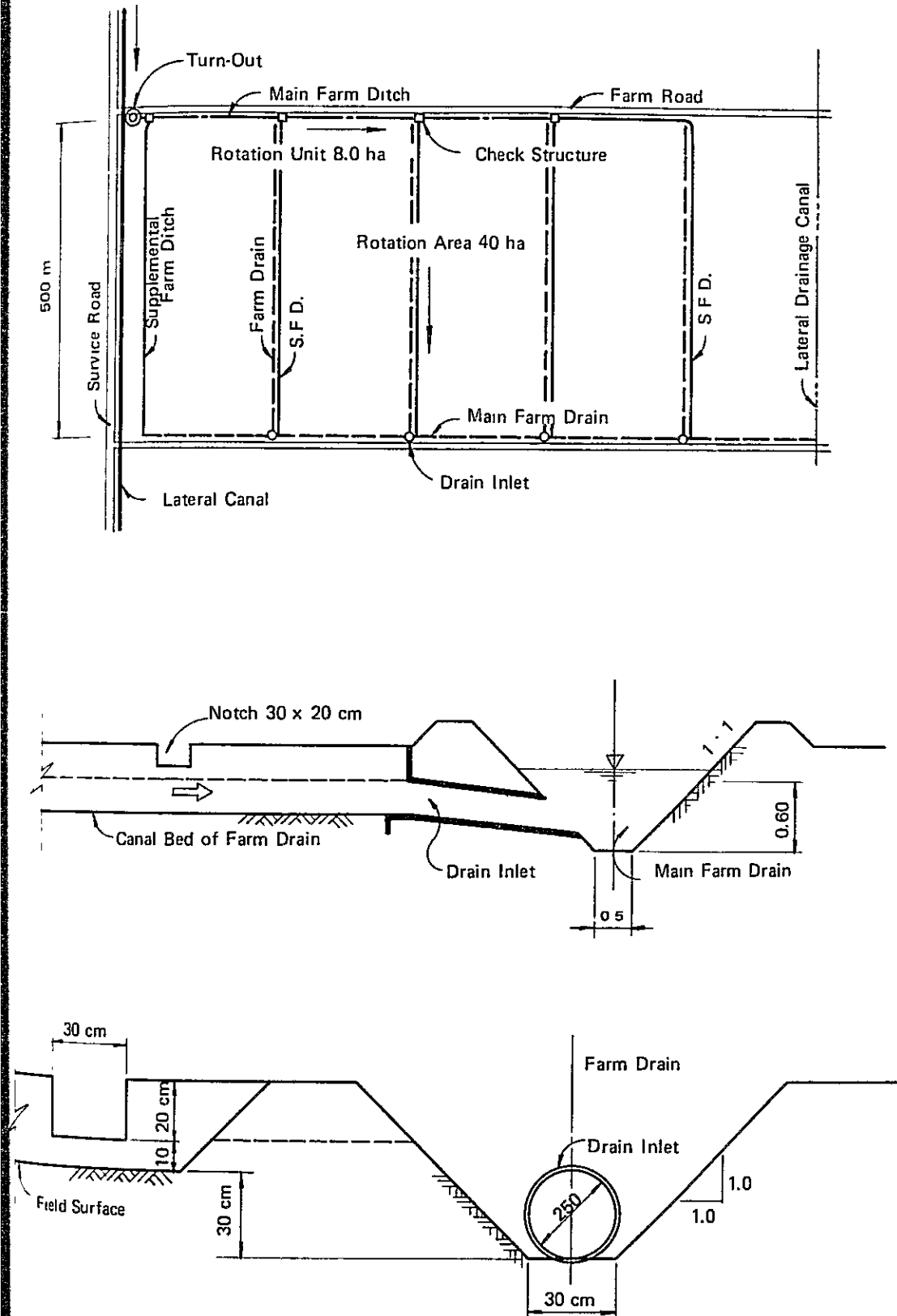


FIGURE 4B-13. DIAGRAM OF RUN-OFF CAPACITY IN PADDY FIELD

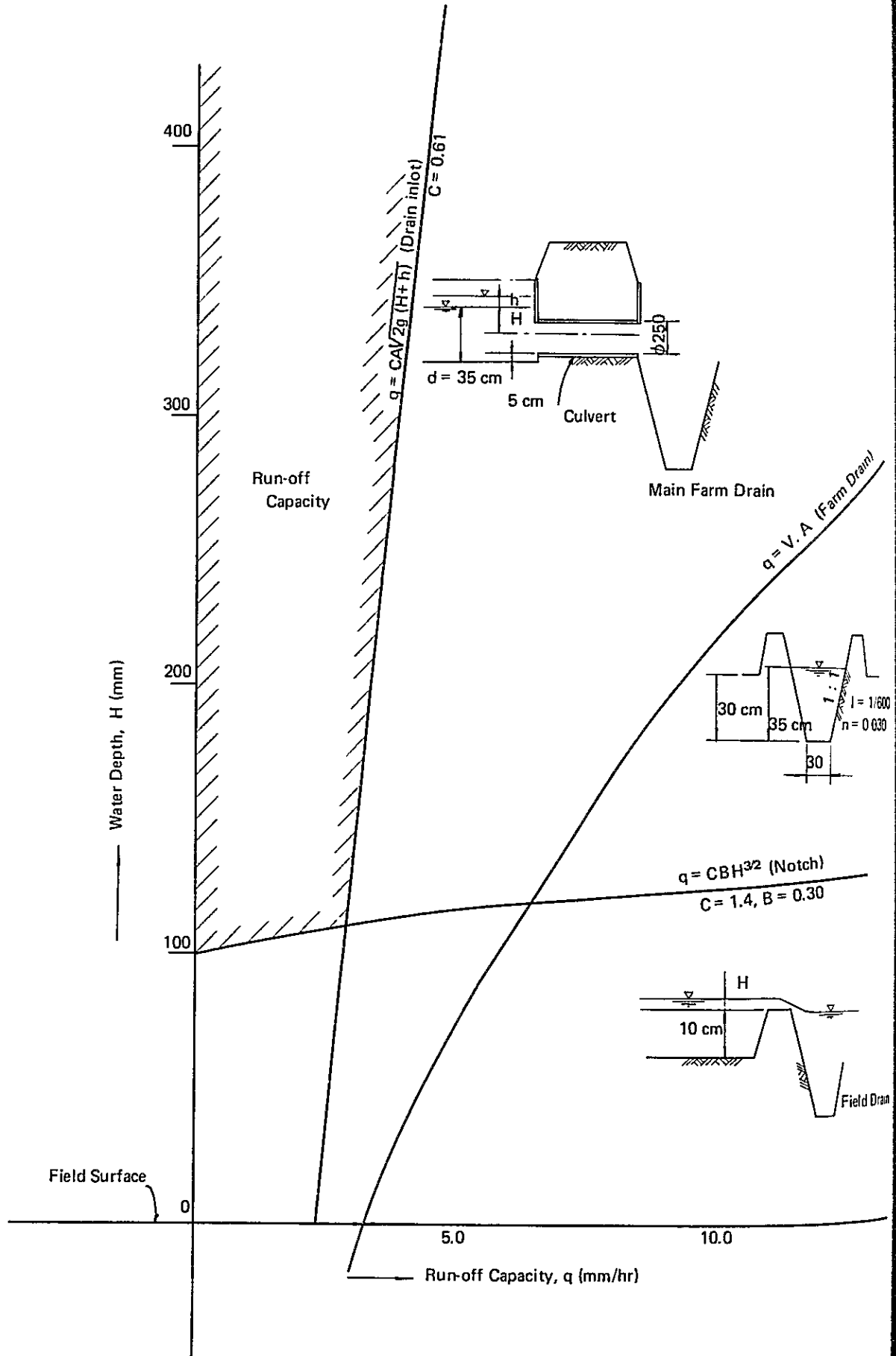
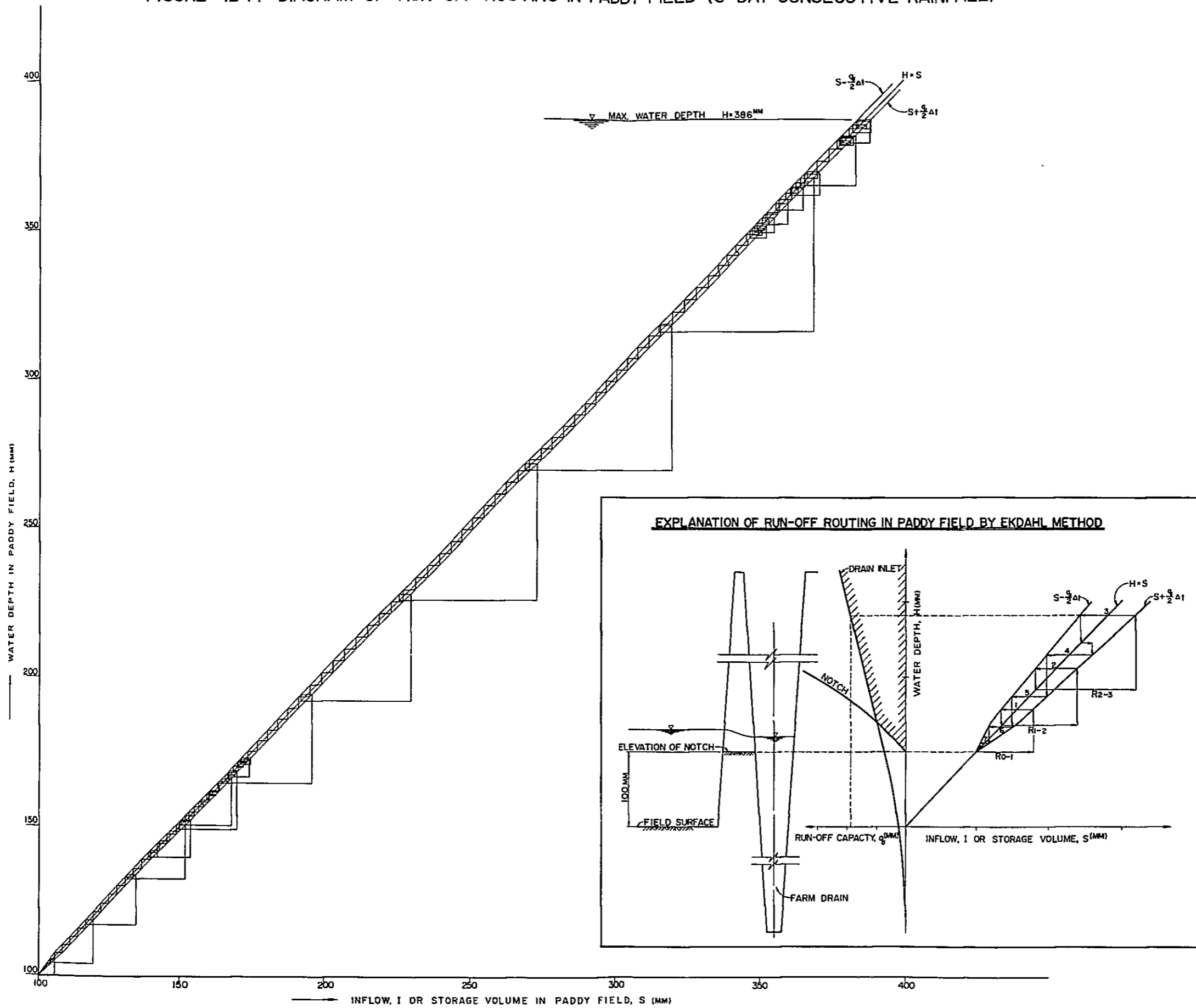


FIGURE 4B-14 DIAGRAM OF RUN-OFF ROUTING IN PADDY FIELD (3-DAY CONSECUTIVE RAINFALL)



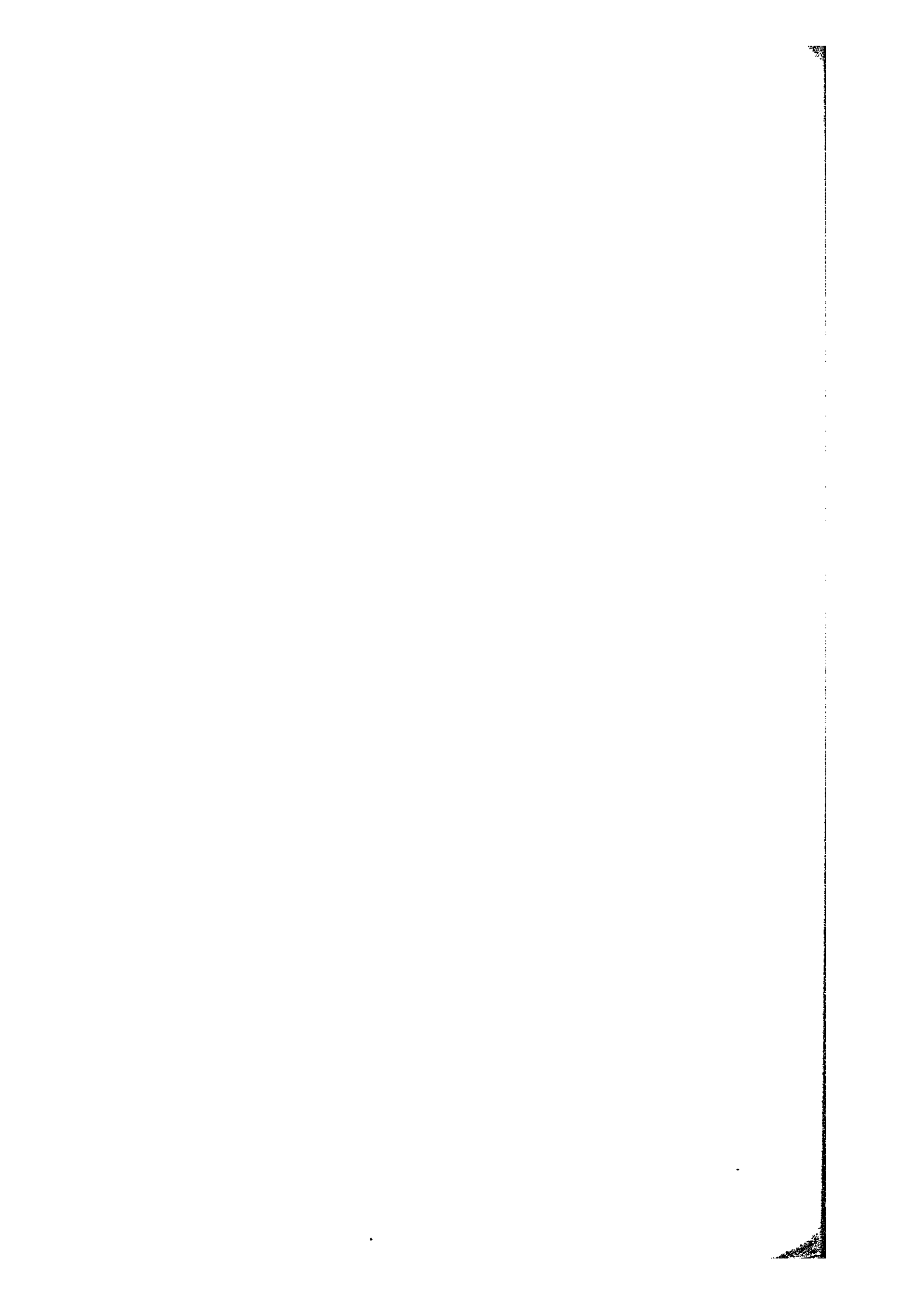


Table 4B-11 Result of Run-off Routing in Paddy Field (1)

Time	Hourly Rainfall (mm)	Effective Water Depth in Paddy Field ^{2/} (mm)	Run-off Discharge ^{3/} (mm ³ /cu.m/sec/50 km)	Time	Hourly Rainfall (mm)	Effective Water Depth in Paddy Field ^{2/} (mm)	Run-off Discharge ^{3/} (mm ³ /cu.m/sec/50 km)
1	1.2	0 1/	0	12	4.7	385.5	4.43
2	5.5	0	0	13	3.1	384.3	4.42
3	6.7	0	0	14	6.3	386.0	4.44
4	12.3	5.7	1.70	15	1.6	383.0	4.91
5	14.7	14.7	2.90	16	1.6	380.0	4.40
6	18.4	18.4	3.00	17	4.7	380.1	4.40
7	19.6	19.6	3.09	18	3.1	379.0	4.39
8	20.8	20.8	3.19	19	1.6	376.3	4.38
9	7.4	7.4	3.20	20	0	372.0	4.35
10	3.7	3.7	3.20	21	0	367.7	4.32
11	1.9	1.9	3.19	22	0	363.5	4.30
12	1.9	1.9	3.18	23	0	359.2	4.29
13	1.2	1.2	3.18	24	0	354.8	4.25
14	2.5	2.5	3.18	1	0.6	351.5	4.22
15	0.6	0.6	3.17	2	2.6	349.7	4.21
16	0.6	0.6	3.15	3	3.2	348.8	4.21
17	1.8	1.8	3.14	4	5.7	350.4	4.22
18	1.2	1.2	3.15	5	6.9	353.2	4.24
19	0.6	0.6	3.14	6	8.6	357.9	4.28
20	0	0	3.11	7	9.2	363.0	4.30
21	0	0	3.10	8	9.7	368.5	4.32
22	0	0	3.08	9	3.4	367.9	4.32
23	0	0	3.05	10	1.7	364.8	4.30
24	0	0	3.04	11	0.9	361.5	4.29
1	3.1	3.1	3.04	12	0.9	358.0	4.29
2	14.1	14.1	3.10	13	0.6	354.4	4.25
3	17.2	17.2	3.16	14	1.1	351.5	4.24
4	31.4	31.4	3.35	15	0.3	347.6	4.21
5	37.6	37.6	3.52	16	0.3	344.0	4.19
6	47.0	47.0	3.80	17	0.9	340.6	4.17
7	50.2	50.2	4.00	18	0.6	337.2	4.15
8	53.3	53.3	4.33	19	0.3	333.7	4.12
9	18.8	18.8	4.40	20	0	329.7	4.10
10	9.4	9.4	4.43	21	0	325.8	4.09
11	4.7	4.7	4.43	22	0	321.6	4.07

Note: 1/: Rainfall of 20 mm is neglected as initial losses in paddy fields
 height of notch : 100 m above field surface
 Water level : 80 mm

2/: Obtained from Figure

3/: Obtained from Figure

Table 4B-11. Result of Run-off Routing in Paddy Field (2)

Time	Hourly Rainfall (mm)	Effective Rainfall (mm)	Water Depth in Paddy Field (mm)	Run-off Discharge (mm)	Run-off Discharge (cu.m/sec/sq.km)	Time	Rainfall (mm)	Rainfall (mm)	in Paddy Field (mm)	Run-off Discharge (mm)	Run-off Discharge (cu.m/sec/sq.km)
23	0	0	317.6	4.05	1.125	9	-	-	185.6	3.29	0.914
24	0	0	313.8	4.01	1.113	10	-	-	182.0	3.26	0.906
1	-	-	310.0	3.99	1.108	11	-	-	178.5	3.24	0.900
2	-	-	306.4	3.96	1.100	12	-	-	175.0	3.23	0.897
3	-	-	302.4	3.95	1.097	13	-	-	171.8	3.20	0.889
4	-	-	298.5	3.92	1.089	14	-	-	168.5	3.19	0.886
5	-	-	294.8	3.90	1.083	15	-	-	165.0	3.16	0.878
6	-	-	291.1	3.89	1.081	16	-	-	161.5	3.15	0.875
7	-	-	287.4	3.88	1.077	17	-	-	158.2	3.12	0.867
8	-	-	283.5	3.84	1.066	18	-	-	154.8	3.10	0.861
9	-	-	279.8	3.82	1.061	19	-	-	151.7	3.10	0.858
10	-	-	276.0	3.80	1.055	20	-	-	148.4	3.09	0.858
11	-	-	272.3	3.79	1.053	21	-	-	145.0	3.07	0.853
12	-	-	268.5	3.75	1.042	22	-	-	143.8	3.05	0.847
13	-	-	264.7	3.74	1.039	23	-	-	139.0	3.03	0.842
14	-	-	260.9	3.70	1.028	24	-	-	135.8	3.00	0.833
15	-	-	257.0	3.69	1.025	1	-	-	133.0	2.99	0.831
16	-	-	253.0	3.67	1.019	2	-	-	130.3	2.98	0.828
17	-	-	249.9	3.65	1.014	3	-	-	127.2	2.96	0.822
18	-	-	245.0	3.62	1.005	4	-	-	124.3	2.94	0.817
19	-	-	241.0	3.60	1.000	5	-	-	121.3	2.92	0.811
20	-	-	237.0	3.60	1.000	6	-	-	118.6	2.90	0.806
21	-	-	233.0	3.56	0.989	7	-	-	115.7	2.90	0.806
22	-	-	229.0	3.54	0.983	8	-	-	113.0	2.88	0.800
23	-	-	225.0	3.51	0.975	9	-	-	110.2	2.86	0.794
24	-	-	221.0	3.50	0.972	10	-	-	107.3	2.86	0.794
1	-	-	217.1	3.50	0.972	11	-	-	105.4	2.80	0.639
2	-	-	213.3	3.48	0.966	12	-	-	103.7	1.60	0.444
3	-	-	209.1	3.40	0.944	13	-	-	102.7	1.00	0.278
4	-	-	205.2	3.39	0.942	14	-	-	102.0	0.90	0.250
5	-	-	201.2	3.37	0.946	15	-	-	101.3	0.50	0.139
6	-	-	197.2	3.36	0.933	16	-	-	101.3	0.30	0.083
7	-	-	193.4	3.34	0.930	17	-	-	101.7	0.20	0.056
8	-	-	189.4	3.30	0.911	18	-	-	101.4	0.10	0.037

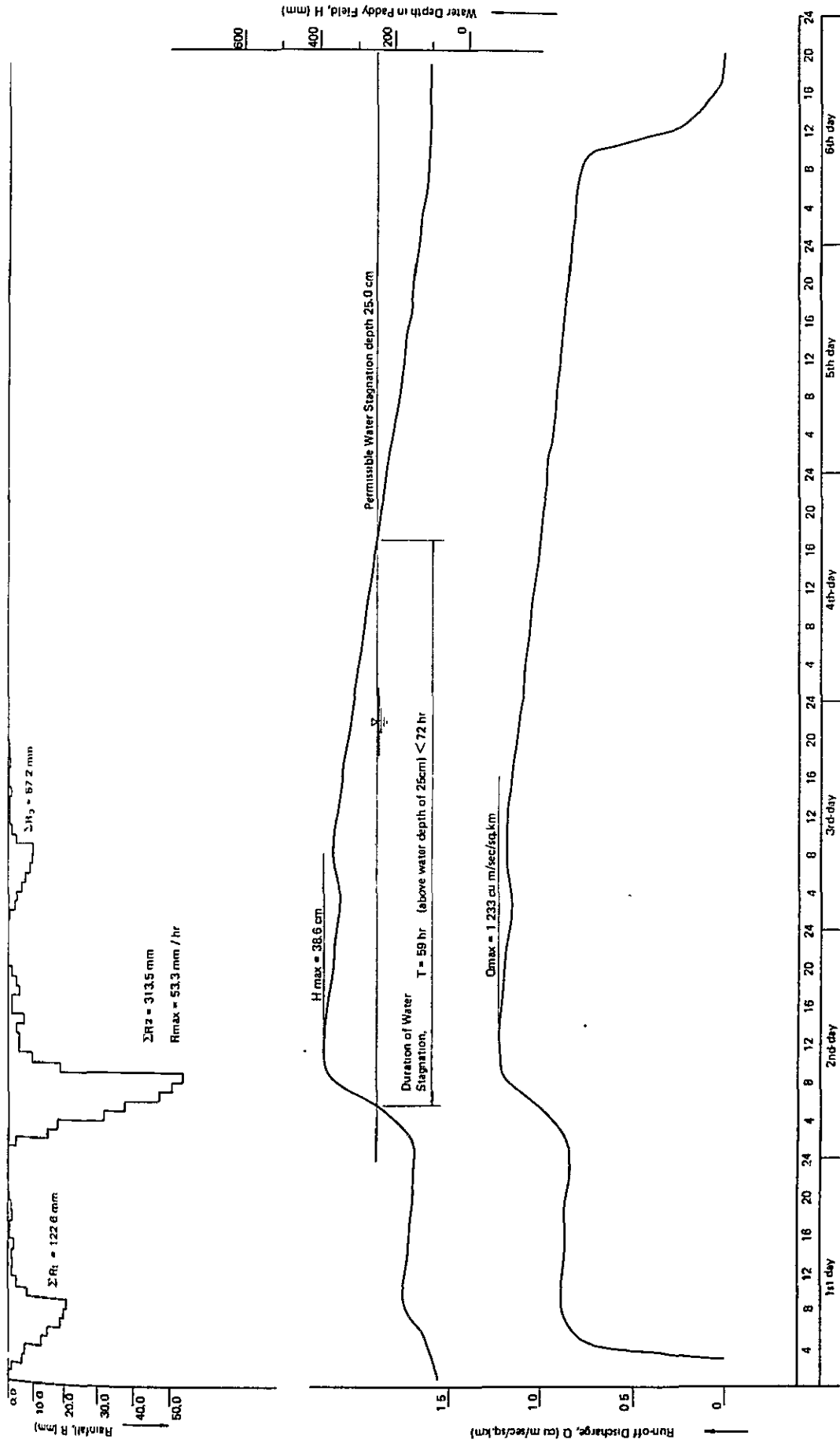
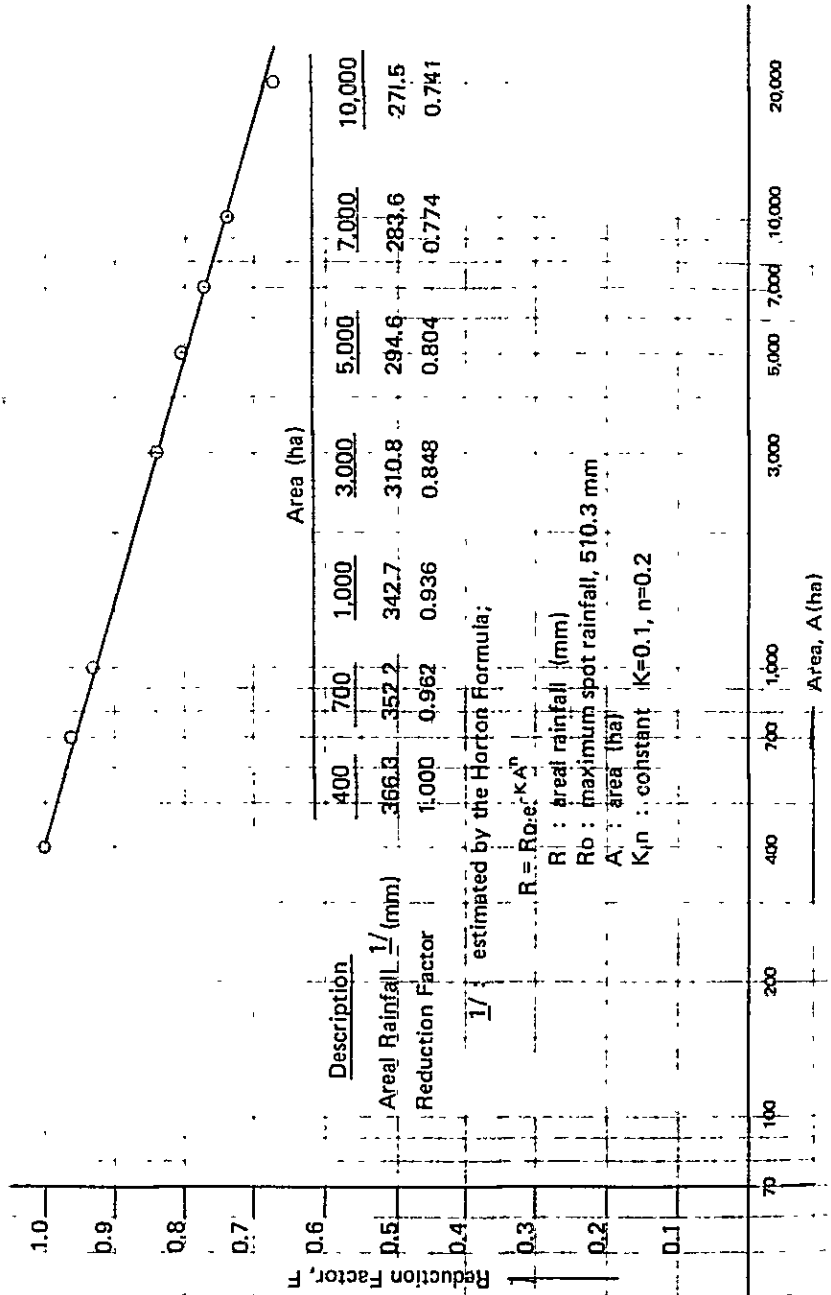


FIGURE 4B-16 AREA-REDUCTION FACTOR FOR DRAINAGE MODULUS



5. Drainage Discharge from Hilly Land ^{1/}

Drainage areas located in the hilly land are mostly less than 100 sq.km. The flood time of concentration for such a small drainage area will be less than one hour. Only hourly peak rainfall, therefore, was considered for drainage discharge analysis. Relationships between observed peak discharges from drainage areas less than 100 sq.km and hourly peak rainfall intensity was plotted as shown in Figure 4B-17. This shows the peak discharge and hourly rainfall increase in a rather constant rate. So, the run-off coefficient was estimated as $f = 0.269$ from the figure. This run-off coefficient was adopted to estimate the peak discharge from hourly rainfall.

The obtained run-off coefficient might be on a high side, because the available peak discharges are mostly in the mountainous area. Normally run-off coefficient for small areas in hilly land ranges from 0.1 to 0.3. So, the obtained value of 0.269 is considered to be acceptable.

The estimation of peak discharge was made based upon the Rational formula;

$$Q = 0.2778.f.r_t.A$$

where f : run-off coefficient of 0.269

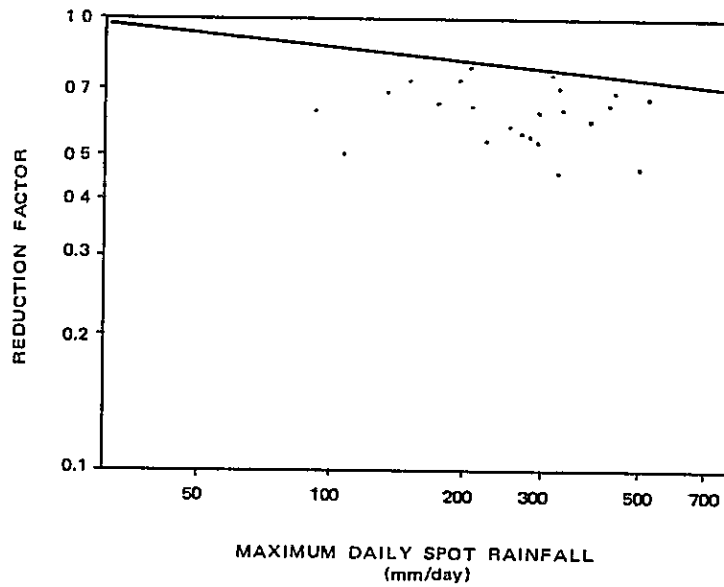
r_t : areal hourly peak rainfall intensity (mm/hr)

A : catchment area (sq.km)

In order to estimate the peak discharge, the peak rainfall as obtained from the reduction factor of daily rainfall (see figure 4B-18), was considered as average rainfall in the Project Area, and used in the Rational formula for areas smaller than 100 hectares. The areal hourly rainfall obtained from the relationship of spot hourly rainfall to areal hourly rainfall (see Figure 4B-19) was adopted to estimate the peak discharge for large areas up to 20,000 hectares using as basis for estimation of areal hourly rainfall obtained from Laoag and Vigan. The results are shown as follows;

^{1/} : reference was made to the study of NISIS-I, prepared by NIA, 1976.

Figure 4B-18. Reduction Factor of Areal Rainfall to Maximum Daily Spot Rainfall



Note: Arithmetic mean method was adopted to estimate a reduction factor of spot rainfall to the areal rainfall. The maximum rainfall and the reduction rate to the areal rainfall was plotted on the logarithmic paper.

Areal Hourly Rainfall of Each Return Period

<u>Return Period</u>	<u>Spot Peak Hourly Rainfall</u> (mm)	<u>Areal Hourly Rainfall</u>	
		<u>(1)</u> (mm)	<u>(2)</u> (mm)
5-year	53.3	41.0	35.5
10-year	66.2	49.7	40.0

- (1) : Obtained from the reduction factor of areal daily rainfall. (Fig. 4B-18)
 (2) : Obtained from the relation ship of spot hourly rainfall and (Fig. 4B-18) areal hourly rainfall.

Peak Discharge of Hilly Land Area

<u>Period</u>	<u>Drainage Area</u> (sq.km)	<u>Hourly Rainfall</u> (mm/hr)	<u>Peak Discharge</u> (cu.m/sec)	<u>Specific Discharge</u> (cu.m/sec/ha)
1,5	1.0	41.0	3.1	3.064
	200.0	35.5	530.5	2.653
1,10	1.0	49.7	3.7	3.714
	200.0	40.0	597.8	2.989

FIGURE 4B-17. RELATION OF PEAK HOURLY RAINFALL AND PEAK RUN-OFF
(LESS THAN 100 sq. KM)

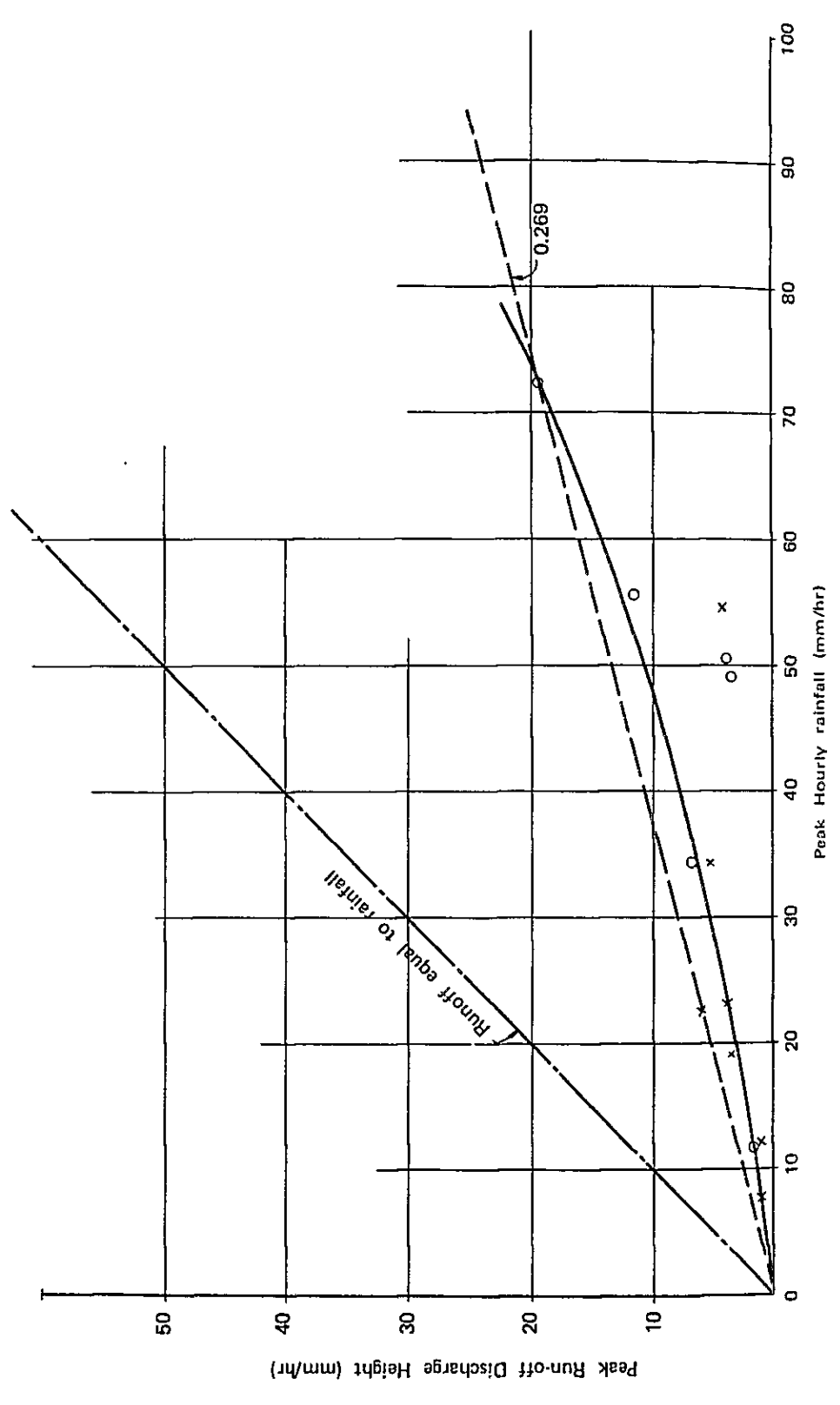
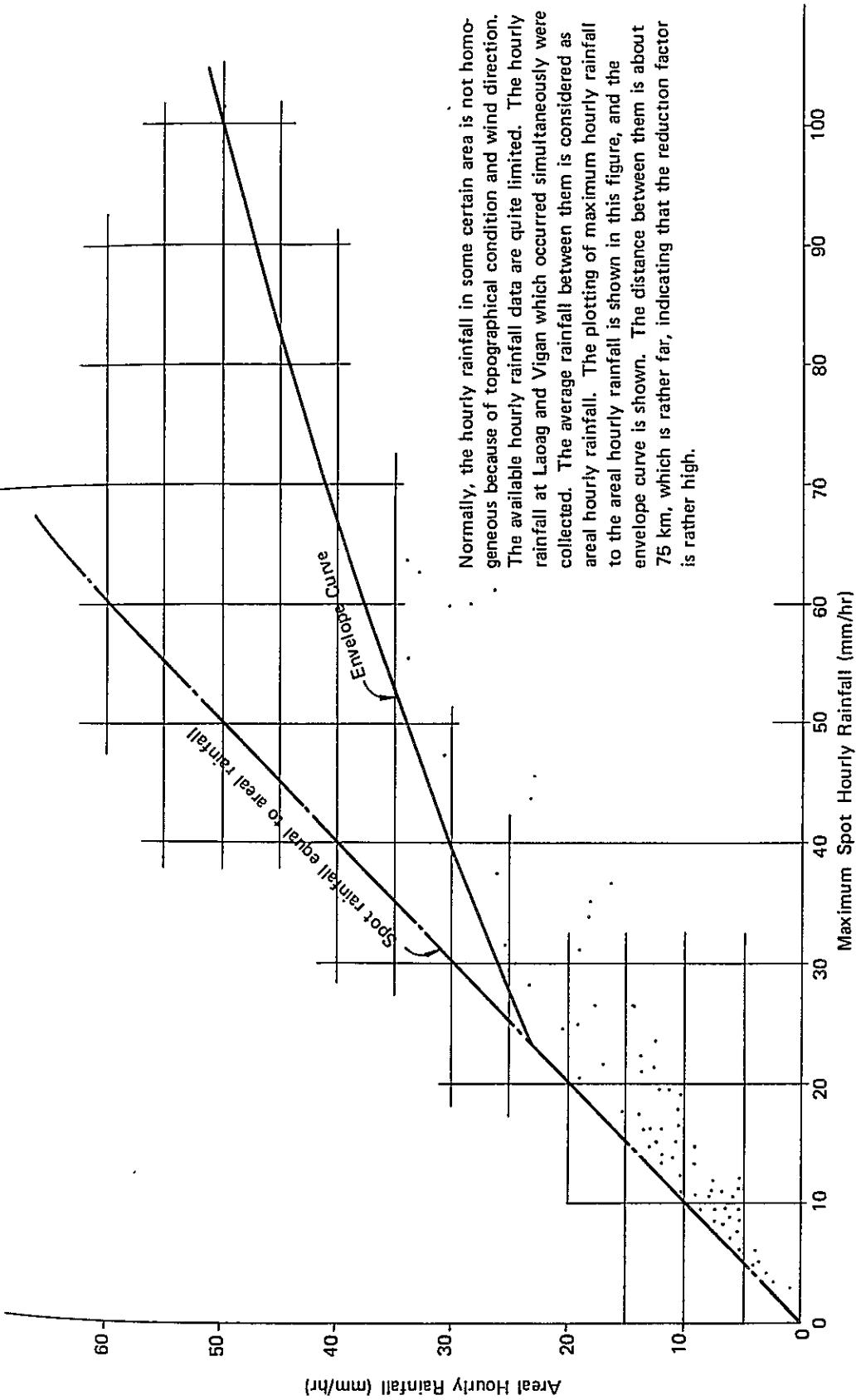


FIGURE 4B-1D. RELATION OF MAXIMUM SPOT HOURLY RAINFALL AND AREAL HOURLY RAINFALL



Normally, the hourly rainfall in some certain area is not homogeneous because of topographical condition and wind direction. The available hourly rainfall data are quite limited. The hourly rainfall at Laoag and Vigan which occurred simultaneously were collected. The average rainfall between them is considered as areal hourly rainfall. The plotting of maximum hourly rainfall to the areal hourly rainfall is shown in this figure, and the envelope curve is shown. The distance between them is about 75 km, which is rather far, indicating that the reduction factor is rather high.

Table 4B-12. Comparison of Low Water Level

Description	Unit	Low Water Level			
		EL 265.00 m	EL 275.00 m	EL 285.00 m	EL 295.00 m
Max. Discharge at Bonga P/S	cu.m/sec	28,225	28,225	28,225	28,225
Design Effective Head	m	146.26	150.82	155.40	161.22
Installed Capacity	KW	35,200	36,000	37,400	38,800
Annual Generation	GWh	154.83	159.66	164.51	170.67
Effective Output	KW	32,586	33,776	35,198	36,718
Construction Cost					
Bonga P/S ^{1/}	₱10 ³	527,241	538,325	566,564	584,127
Palsiguan Dam ^{2/}	₱10 ³	798,963	844,483	889,080	954,738
Tunnel	₱10 ³	160,140	140,330	138,330	138,330
Power Plant	₱10 ³	143,600	144,400	145,600	146,900
Annual Cost Ratio	%	15.43	15.43	15.43	15.43
Annual Cost Value of Bonga P/S (C)	₱10 ³ /Year	81,353	83,064	87,421	90,131
Annual Unit Capacity Value	₱/KW	700.0	700.0	700.0	700.0
Annual Unit Energy Value	₱/KWwh	0.429	0.429	0.429	0.429
Annual Capacity Value (B)	₱10 ³ /Year	82,599	85,288	88,156	91,599
Annual KW Value ^{3/}	₱10 ³ /Year	22,810	23,643	24,639	25,703
Annual KWh Value ^{4/}	₱10 ³ /Year	59,780	61,645	63,517	65,896
(C) - (B)		- 1,246	- 2,224	- 735	- 1,468

Note: ^{1/}: (Palsiguan dam cost + Tunnel cost) x 0.4 + Power plant cost

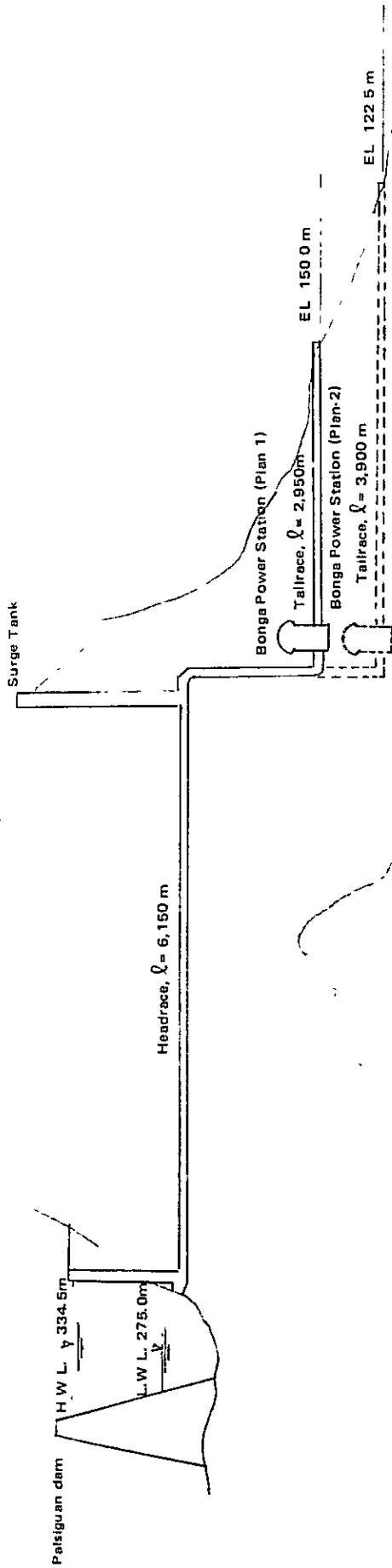
^{2/}: allocation ratio of hydropower

^{3/}: Total Palsiguan dam cost

^{4/}: Effective output x 700 ₱/KW

^{5/}: Annual generation (GWh) x 0.40 = 0.40 ₱/KWwh

Alternative - I (Without Nueva Era Dam)



Alternative - II (Upstream Nueva Era Dam)

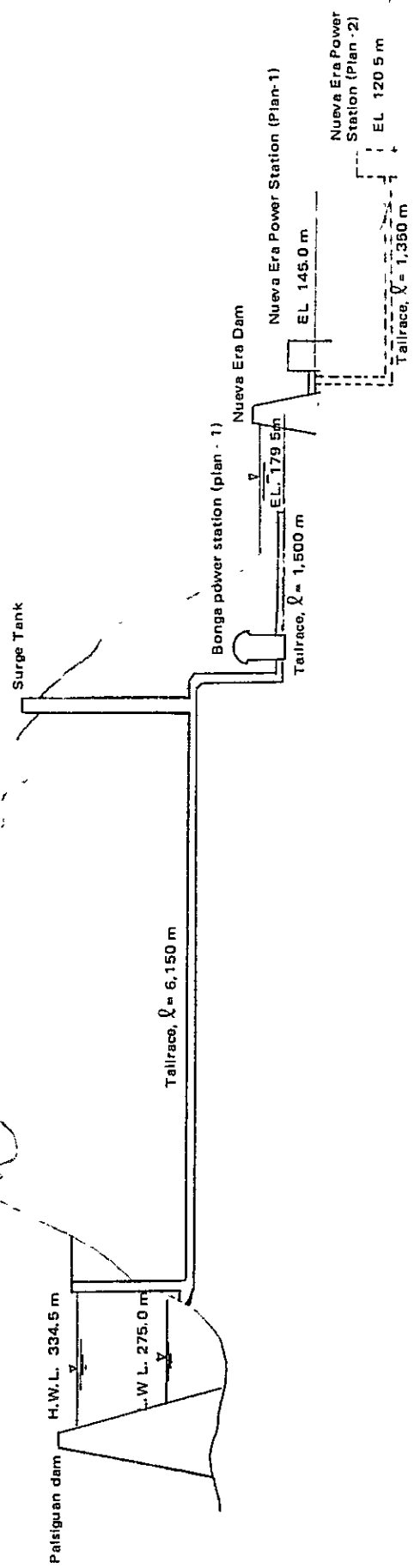


Table 4B-13 Comparison of Alternative Plans

1. Major Dimension

Item	Unit	Proposed	Alternative-I		Alternative-II	
		Plan	Plan-1	Plan-2	Plan-1	Plan-2
a. Bonga Power Station						
Full Water Level	m	334.50	334.50	334.50	334.50	334.50
Tail Water Level	m	150.00	150.00	122.50	179.50	179.50
Tailrace Length	km	2.95	2.95	3.90	1.50	1.50
Effective Head	m	150.82	150.82	177.32	119.82	119.82
Installed Capacity	KW	36,000	36,000	42,000	29,000	29,000
Firm Peak Capacity	KW	30,510	-	-	24,600	24,600
Annual Generation	MWh	159.660	159.660	185.300	123.400	123.400
Turbine	type	Deriaz	Francis	Francis	Deriaz	Deriaz
	set	1	1	1	1	1
b. Nueva Era Power Station						
High Water Level	m	150.00	-	-	179.50	179.50
Lower Water Level	m	148.50	-	-	177.50	177.50
Tail Water Level	m	120.50	-	-	145.00	120.50
Tailrace Length	km	-	-	-	-	1.55
Effective Head	m	27.92	-	-	32.32	55.00
Installed Capacity	KW	6,800	-	-	7,900	13,600
Annual Generation	MWh	39,540	-	-	54,700	73,200
Turbine	type	Kaplan	-	-	Kaplan	Kaplan
	set	1			1	1
c. Nueva Era Dam						
Catchment Area	sq.km	52.40	-	-	45.10	45.10
Dam Height	m	45.50	-	-	47.10	47.10
Crest Length	m	220.00	-	-	187.00	187.00
Dam Volume	cu.m	141,000	-	-	130,000	105,000
Diversion Channel		Open Channel 10mx7m	-	-	Tunnel ø8m	Tunnel ø8m
d. Diversion Dam		No Provision	Provided	Provided	Provided	No Provision

2 Evaluation of Alternative Plans

Item	Unit	Proposed Plan	Alternative-I		Alternative-II	
			Plan-1	Plan-2	Plan-1	Plan-2
a Installed Capacity						
Bonga Power Station	KW	36,000	36,000	42,000	29,000	29,000
Nueva Era Power Station	KW	6,800	-	-	7,900	13,600
Total		42,000	36,000	42,000	36,900	42,600
b Annual Generation						
Bonga Power Station	MWh	159.660	159.660	185.300	123.400	123.400
Nueva Era Power Station	MWh	39.540	-	-	45.700	73.200
Total		199.200	159.660	185.300	169.100	196.600
c Effective Output						
Bonga Power Station	KW	33,780	14,720	17,490	26,690	26,690
Nueva Era Power Station	KW	4,510	-	-	5,210	8,410
Total		38,290	14,720	17,490	31,900	35,100
5 Construction Cost						
Bonga Power Station	10 ³ P	144,400	171,700	198,000	113,600	113,600
Nueva Era Power Station	10 ³ P	40,900	-	-	46,000	75,200
Nueva Era Dam	10 ³ P	121,600	-	-	119,500	119,500
Diversion Dam	10 ³ P	-	10,000	10,000	10,000	-
Total		306,900	181,700	208,000	289,100	298,300
6. Unit Value						
Annual Cost Ratio	%	15.43	15.43	15.43	15.43	15.43
Annual Unit Capacity Value	P/KW	700	700	700	700	700
Annual Unit Energy Value	P/KWh	0.429	0.429	0.429	0.429	0.429
7. Annual Value						
kWh Value ^{1/}	10 ³ P	26,803	10,304	12,243	22,330	24,570
kWh Value ^{2/}	10 ³ P	76,911	61,645	71,544	65,290	75,907
Total (B)	10 ³ P	103,714	71,949	83,787	87,620	100,477
8 Annual Cost Value (C)						
	10 ³ P	47,355	28,036	32,094	44,608	46,027
9. (B) - (C)						
	10 ³ P	56,359	43,913	51,693	43,012	54,450
10. Use Ratio of Effective Output ^{3/}						
	%	0.281	0.134	0.138	0.275	0.261

Note: 1/: Effective output x 700P/KW

2/: Annual generation (KWh) x 0.90 x 0.429 P/KWh

3/: Effective output x 4hr x 365days/Annual Generation (KWh)

Table 4B-14 Hydro Power Operation Study for 10 Year (1960 - 1969) - Bonga Power Station
Table 4B-15 Hydro Power Operation Study for 10 Year (1960 - 1969) - Nueva Era Power Station

Note	<u>Bonga Power Station</u>	
	Max Loss	13.85 m (28,225 m ³ /s)
	Mini Loss	4.23 (9.5 m ³ /s)
	Turbine Efficiency	d = H W L. - W.L.
	Efficiency (1)	= 0.000454 x d + 0.865 (28,225 m ³ /s)
	Efficiency (2)	= 0.00134 x d + 0.700 (9.50 m ³ /s)
	Generator Efficiency	0.975
	<u>Nueva Era Power Station</u>	
	Loss	= 0.0015 x Q ² + 0.30
	Turbine Efficiency	
	Discharge	Efficiency
	m ³ /s	m ³ /s
	27,273 - 25.00	0.890
	24.99 - 20.00	0.895
	19.99 - 17.50	0.892
	17.49 - 15.00	0.889
	14.99 - 12.50	0.882
	12.49 - 10.00	0.872
	9.99 - 9.00	0.850
	8.99 -	0.800
	Generator Efficiency	0.965
	H W.L.	: High Water Level (m)
	Gross H	: Gross Head (m)
	Losses	: Losses (m)
	E. Head	: Effective Head (m)
	P. Head	: Effective Head for Peak
	Req Dis	: Requirement Discharge (cu.m/sec)
	T Dis	: Turbine Discharge (cu.m/sec)
	Over F	: Over Flow (cu.m/sec)
	Effice	: Turbine Efficiency
	P. Effice	: Turbine Efficiency for Peak
	Peak Kw	: Peak Output (Kw)
	SWH	: SWH (m)

T.W.L=150.0

1961

RUNGA P/S

	H.W.L. (M)	GROSS H (M)	LOSSES (M)	F.H.FAD (M)	P.H.FAD (M)	REQ.DTS. (CM/S)	T.DIS. (CM/S)	OVFR F (CM/S)	EFFIC F	P.EFFI.	KW	I-F-K KW	KWH
NOV 1	333.157	183.157	12.978	170.179	169.307	12.149	12.149	0.0	0.481	0.481	17404.	36000.	4176960.
NOV 2	330.445	180.445	12.232	168.213	166.595	21.190	21.190	0.0	0.871	0.899	29605.	36000.	7119600.
NOV 3	327.023	177.023	12.830	164.193	161.173	23.335	23.335	0.0	0.876	0.889	32070.	36000.	7696800.
S.TOTAL	323.292	173.292	12.997	160.295	159.442	24.014	24.014	0.0	0.876	0.897	32214.	36000.	18993360.
DEC 1	319.170	169.170	13.091	156.079	155.320	24.413	24.413	0.0	0.875	0.885	31857.	36000.	7732560.
DEC 2	314.495	164.495	13.392	151.104	150.645	25.785	25.785	0.0	0.877	0.883	32649.	36000.	7642680.
DEC 3	309.371	159.371	13.789	145.592	145.521	27.855	27.855	0.0	0.880	0.891	34096.	36000.	8619136.
S.TOTAL	304.525	154.525	12.906	141.618	140.675	23.640	23.640	0.0	0.884	0.895	31316.	36000.	23397500.
JAN 1	292.159	142.159	10.161	131.998	128.309	16.073	16.073	0.0	0.875	0.883	31313.	36000.	6183200.
JAN 2	288.526	138.526	8.403	130.122	124.676	13.340	13.340	0.0	0.875	0.875	31313.	36000.	6633120.
JAN 3	285.734	135.734	7.962	127.772	121.884	12.794	12.794	0.0	0.875	0.875	31313.	36000.	6264432.
S.TOTAL	285.464	135.464	7.914	127.650	121.614	4.700	4.700	0.0	0.875	0.875	31313.	36000.	23093264.
FEB 1	285.115	135.115	7.814	127.301	121.265	4.700	4.700	0.0	0.875	0.875	31313.	36000.	3970990.
FEB 2	284.344	134.344	7.814	126.530	120.494	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1062240.
FEB 3	283.363	133.363	7.814	125.549	119.513	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1359120.
S.TOTAL	282.419	132.419	7.814	124.605	118.569	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1155544.
APR 1	281.511	131.511	7.814	123.697	117.661	4.700	4.700	0.0	0.875	0.875	31313.	36000.	3277444.
APR 2	280.807	130.807	7.814	122.993	116.357	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1041640.
APR 3	280.826	130.826	7.814	123.012	116.976	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1032720.
S.TOTAL	282.490	132.490	7.814	124.676	118.640	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1323840.
MAY 1	282.793	132.793	8.187	124.604	119.943	5.891	5.891	0.0	0.875	0.875	31313.	36000.	3373430.
MAY 2	277.739	127.739	10.430	117.309	113.889	16.593	16.593	0.0	0.875	0.875	31313.	36000.	1012440.
MAY 3	281.072	131.072	7.814	123.258	117.222	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1316784.
S.TOTAL	291.007	141.007	7.814	133.193	127.157	4.700	4.700	0.0	0.875	0.875	31313.	36000.	3170304.
JUN 1	311.204	161.204	7.814	153.390	147.354	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1306320.
JUN 2	316.730	166.730	7.814	158.516	152.880	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1316840.
JUN 3	319.901	169.901	7.814	162.097	156.251	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1306320.
S.TOTAL	323.281	173.281	7.814	165.467	159.431	4.700	4.700	0.0	0.875	0.875	31313.	36000.	3606720.
AUG 1	329.261	179.261	7.814	171.447	165.411	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1020240.
AUG 2	332.110	182.110	10.237	171.873	168.260	4.700	4.700	0.0	0.875	0.875	31313.	36000.	5933240.
AUG 3	331.465	181.465	11.570	169.685	167.815	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1116700.
S.TOTAL	331.122	181.122	12.383	168.519	167.272	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1116700.
SEP 1	333.586	183.586	10.419	173.167	169.736	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1424040.
SEP 2	334.008	184.008	12.125	171.853	170.158	4.700	4.700	0.0	0.875	0.875	31313.	36000.	560740.
SEP 3	333.900	183.900	11.946	172.554	170.651	4.700	4.700	0.0	0.875	0.875	31313.	36000.	715460.
S.TOTAL	333.845	183.845	11.490	172.554	170.651	4.700	4.700	0.0	0.875	0.875	31313.	36000.	434840.
OCT 1	333.900	183.900	11.946	172.554	170.651	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1711040.
OCT 2	333.900	183.900	11.946	172.554	170.651	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1424040.
OCT 3	333.900	183.900	11.946	172.554	170.651	4.700	4.700	0.0	0.875	0.875	31313.	36000.	560740.
S.TOTAL	333.900	183.900	11.946	172.554	170.651	4.700	4.700	0.0	0.875	0.875	31313.	36000.	1424040.

T.W.L=150.0													
1965													
DUNGA P/S													
	H.N.L. (M)	GROSS H. (M)	LOSSES (M)	E. HEAD (M)	P. HEAD (M)	RFQ. DIS. (CM/S)	T. DIS. (CM/S)	OVFR F. (CM/S)	P. EFFI.	KW	PEAK KW	KWH	
NOV 1	334.500	184.500	8.595	175.905	170.650	4.700	6.958	0.0	0.831	9718.	36000.	2332320.	
NOV 2	334.500	184.500	13.850	170.650	170.650	4.700	24.225	0.0	0.892	36000.	36000.	8640000.	
NOV 3	331.832	181.832	10.991	170.841	167.982	4.700	17.795	0.0	0.857	24994.	36000.	5374560.	
S.TOTAL												16346880.	
DEC 1	331.249	181.249	13.211	168.038	167.399	8.651	24.945	0.0	0.883	35366.	36000.	8587840.	
DEC 2	332.426	182.426	9.254	173.172	168.576	4.700	14.536	0.0	0.837	20152.	36000.	4631180.	
DEC 3	330.729	180.729	12.971	167.758	166.879	6.499	23.904	0.0	0.879	33680.	36000.	8891520.	
S.TOTAL												22211040.	
JAN 1	331.393	181.393	13.620	167.765	167.543	16.426	26.976	0.0	0.888	36000.	36000.	8640000.	
JAN 2	330.111	180.111	13.403	164.708	166.261	11.761	25.842	0.0	0.885	36000.	36000.	8640000.	
JAN 3	328.959	178.959	13.505	165.454	165.109	11.589	26.344	0.0	0.885	36990.	36000.	8640000.	
S.TOTAL												26734000.	
FEB 1	328.296	178.296	13.905	164.391	164.446	8.913	27.177	0.0	0.887	36000.	36000.	8640000.	
FEB 2	327.618	177.618	13.830	163.788	163.768	8.412	28.389	0.0	0.881	36000.	36000.	8640000.	
FEB 3	327.864	177.864	13.850	164.014	164.014	8.569	28.225	0.0	0.889	36000.	36000.	8640000.	
S.TOTAL												26734000.	
MAR 1	326.202	176.202	13.450	162.752	162.352	4.700	28.225	0.0	0.888	36000.	36000.	8640000.	
MAR 2	325.854	175.854	13.350	162.004	162.004	4.700	28.225	0.0	0.888	36000.	36000.	8640000.	
MAR 3	324.912	174.912	13.850	161.062	161.062	4.700	28.225	0.0	0.888	36000.	36000.	8640000.	
S.TOTAL												26734000.	
APR 1	324.857	174.857	13.850	161.007	161.007	4.700	28.225	0.0	0.888	36000.	36000.	8640000.	
APR 2	324.276	174.276	13.850	160.426	160.426	4.700	28.225	0.0	0.887	36000.	36000.	8640000.	
APR 3	324.021	174.021	13.604	160.418	160.171	4.700	26.847	0.0	0.894	36000.	36000.	8640000.	
S.TOTAL												26734000.	
MAY 1	323.760	173.760	13.160	160.600	159.910	4.700	25.634	0.0	0.881	34612.	36000.	8640000.	
MAY 2	323.065	173.065	13.505	159.560	159.215	4.700	26.344	0.0	0.873	35465.	36000.	8640000.	
MAY 3	322.553	172.553	13.454	159.097	158.703	4.700	26.100	0.0	0.882	34995.	36000.	8640000.	
S.TOTAL												26734000.	
JUN 1	326.034	176.034	11.190	164.844	162.184	4.700	14.240	0.0	0.855	24565.	36000.	8640000.	
JUN 2	328.820	178.820	9.953	168.867	164.970	4.700	15.692	0.0	0.842	21119.	36000.	8640000.	
JUN 3	332.335	182.335	9.871	172.464	168.485	4.700	15.039	0.0	0.840	23854.	36000.	8640000.	
S.TOTAL												26734000.	
JUL 1	330.702	180.702	12.993	167.709	166.852	4.700	23.997	0.0	0.880	31840.	36000.	8640000.	
JUL 2	331.635	181.635	11.436	170.199	167.785	4.700	18.379	0.0	0.862	26405.	36000.	8640000.	
JUL 3	331.192	181.192	11.775	169.417	167.342	4.700	19.798	0.0	0.881	27722.	36000.	8640000.	
S.TOTAL												26734000.	
AUG 1	330.906	180.906	12.712	168.194	167.056	4.700	22.877	0.0	0.870	32207.	36000.	8640000.	
AUG 2	330.755	180.755	12.822	167.833	166.705	4.700	23.703	0.0	0.879	33412.	36000.	8640000.	
AUG 3	330.350	180.350	12.993	167.357	166.500	4.700	23.997	0.0	0.880	33769.	36000.	8640000.	
S.TOTAL												26734000.	
SEP 1	331.563	181.563	11.535	169.028	167.713	4.700	19.274	0.0	0.864	27046.	36000.	8640000.	
SEP 2	332.531	182.531	11.109	171.422	168.681	6.939	18.071	0.0	0.959	25426.	36000.	8640000.	
SEP 3	333.165	183.165	10.083	173.082	169.315	4.700	15.329	0.0	0.847	22313.	36000.	8640000.	
S.TOTAL												26734000.	
OCT 1	333.586	183.586	11.981	171.605	169.731	4.700	11.584	0.0	0.870	16362.	36000.	8640000.	
OCT 2	334.008	184.008	9.338	174.670	173.150	4.700	7.214	0.0	0.830	11001.	36000.	8640000.	
OCT 3	334.500	184.500	8.118	176.382	170.650	4.700	5.687	0.0	0.825	7313.	36000.	8640000.	
S.TOTAL												26734000.	

	H.W.L. (M)	GROSS H. (M)	LOSS S. (M)	E-HEAD (M)	P-HEAD (M)	RFQ.DIS. (CM/S)	T-DIS. (CM/S)	OVFF F. (CM/S)	P.F.F.F.I.	KW	V.F.A.K. KW	KWH
NOV 1	332.195	182.195	11.313	170.882	168.345	18.567	18.567	0.0	0.891	26132.	16003.	62044.80.
NOV 2	328.364	178.364	13.650	164.714	164.514	27.094	27.094	0.0	0.887	30000.	56000.	60400.00.
NOV 3	325.583	175.583	12.973	162.610	161.733	23.912	23.912	0.0	0.877	32583.	56000.	78199.20.
S.TOTAL	322.638	172.638	13.203	159.435	158.788	24.907	24.907	0.0	0.887	33352.	36000.	227244.00.
DEC 1	318.114	168.114	13.484	154.630	154.264	26.239	26.239	0.0	0.880	34114.	36000.	83044.80.
DEC 2	314.203	164.203	11.962	152.241	150.393	20.345	20.345	0.0	0.857	25363.	35811.	81878.40.
DEC 3	309.374	159.374	13.135	146.039	145.524	25.516	25.516	0.0	0.874	31119.	34582.	69994.00.
S.TOTAL	304.553	154.553	12.415	142.138	140.703	21.803	21.803	0.0	0.857	25377.	33323.	78151.2.
JAN 1	296.730	146.730	13.574	133.152	132.580	26.719	26.719	0.0	0.871	29088.	31362.	213755.2.
JAN 2	292.083	142.083	10.287	131.794	128.233	16.317	16.317	0.0	0.817	16788.	30100.	40271.20.
JAN 3	288.403	138.403	8.204	130.199	124.553	13.088	13.088	0.0	0.781	12716.	29273.	33518.40.
S.TOTAL	285.417	135.417	8.030	127.387	121.567	12.875	12.875	0.0	0.776	12161.	28524.	84600.56.
FEB 1	284.217	134.217	7.928	126.289	120.367	5.089	5.089	0.0	0.773	4747.	28214.	155450.16.
FEB 2	283.033	133.033	7.814	125.219	119.183	4.700	4.700	0.0	0.770	4330.	27937.	11392.40.
FEB 3	281.685	131.685	7.814	123.871	117.035	4.700	4.700	0.0	0.768	4272.	27587.	13397.00.
S.TOTAL	280.378	130.378	7.814	122.564	116.528	4.700	4.700	0.0	0.767	4222.	27252.	33062.98.
APR 1	279.145	129.145	7.914	121.331	115.295	4.700	4.700	0.0	0.766	4174.	26503.	13132.40.
APR 2	277.820	127.820	7.814	120.014	113.978	4.700	4.700	0.0	0.764	4118.	26024.	10017.00.
APR 3	276.613	126.613	7.814	118.799	112.763	4.700	4.700	0.0	0.761	4071.	26341.	33033.00.
S.TOTAL	275.428	125.428	7.814	117.614	111.578	4.700	4.700	0.0	0.762	4025.	26034.	37706.00.
MAY 1	275.130	125.130	7.314	117.316	111.280	4.700	4.700	0.0	0.762	4015.	25964.	4660.00.
MAY 2	278.034	128.034	8.969	119.065	114.184	7.777	7.777	0.0	0.784	6937.	26873.	135996.00.
MAY 3	285.258	135.258	7.814	127.444	121.408	4.700	4.700	0.0	0.772	4418.	28491.	30030.00.
S.TOTAL	286.479	136.479	9.497	126.982	122.629	14.918	14.918	0.0	0.800	14880.	28773.	30330.00.
JUN 1	290.387	140.387	7.814	132.573	126.537	4.700	4.700	0.0	0.777	4626.	29761.	62304.00.
JUN 2	293.279	143.279	7.814	135.465	129.429	4.700	4.700	0.0	0.780	4745.	30474.	111384.00.
JUN 3	315.470	165.470	7.814	157.656	151.620	4.700	4.700	0.0	0.802	5678.	56003.	149809.2.
S.TOTAL	321.271	171.271	7.814	163.457	157.421	4.700	4.700	0.0	0.808	5931.	16003.	374903.00.
AUG 1	325.278	175.278	8.071	167.207	161.428	5.545	5.545	0.0	0.816	7229.	36001.	17234.00.
AUG 2	321.194	171.194	13.850	157.344	157.344	29.032	29.032	0.0	0.866	36000.	36001.	173496.00.
AUG 3	321.842	171.842	9.872	161.970	157.992	9.304	9.304	0.0	0.886	17023.	16003.	95040.00.
S.TOTAL	328.342	178.342	7.814	170.528	164.492	4.700	4.700	0.0	0.845	6241.	16003.	126674.00.
SEP 1	329.401	179.401	9.767	169.634	165.551	9.151	9.151	0.0	0.840	12459.	16003.	284016.00.
SEP 2	333.586	183.586	8.736	174.650	169.736	4.700	4.700	0.0	0.834	19578.	16003.	74735.20.
SEP 3	334.000	184.000	8.612	175.396	170.158	4.700	4.700	0.0	0.831	19941.	16003.	46916.00.
S.TOTAL	334.500	184.500	8.905	175.595	170.657	4.700	4.700	0.0	0.834	19941.	16003.	284016.00.

T.W.L=150.0

1969

80NGA P/S

NIEVA ERA P/S 1967

	H.W.L. (M)	T.W.L. (M)	GROSS.H (M)	LUSFS (M)	F.HFAD (M)	T.O.D.S. (CM/S)	OVER F. (CM/S)	FFFCF.	KW	KWH
NOV 1	149.514	119.259	30.255	0.660	29.595	15.489	0.0	0.889	3854.	924960.
NOV 2	149.417	119.010	30.407	0.543	29.864	12.723	0.0	0.882	3169.	760500.
NOV 3	149.576	119.418	30.158	0.746	29.412	17.247	0.0	0.889	4265.	1023600.
S.TOTAL										
DEC 1	149.348	118.836	30.512	0.474	30.038	10.783	0.0	0.872	2471.	2709120.
DEC 2	149.669	119.654	30.015	0.893	29.127	19.877	0.0	0.892	4813.	641040.
DEC 3	149.507	119.241	30.266	0.650	29.616	15.286	0.0	0.889	3806.	1171920.
S.TOTAL										
JAN 1	149.591	119.456	30.135	0.768	29.367	17.568	0.0	0.892	4377.	1004784.
JAN 2	149.590	119.454	30.136	0.767	29.369	17.654	0.0	0.892	4374.	2817744.
JAN 3	149.559	119.375	30.184	0.722	29.462	16.774	0.0	0.889	4155.	1950490.
S.TOTAL										
FEB 1	149.399	118.365	30.434	0.524	29.910	12.221	0.0	0.872	3014.	1049700.
FEB 2	149.352	118.845	30.507	0.478	30.029	10.884	0.0	0.872	2695.	3197160.
FEB 3	149.287	118.879	30.608	0.422	30.186	9.036	0.0	0.850	2193.	723360.
S.TOTAL										
MAR 1	149.250	118.585	30.665	0.396	30.269	8.070	0.0	0.800	1832.	433680.
MAR 2	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
MAR 3	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
S.TOTAL										
APR 1	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
APR 2	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
APR 3	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
S.TOTAL										
MAY 1	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
MAY 2	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
MAY 3	149.250	118.585	30.665	0.396	30.269	8.000	0.0	0.800	1832.	433680.
S.TOTAL										
JUN 1	149.664	119.643	30.021	0.885	29.136	19.752	0.0	0.892	4855.	136300F.
JUN 2	149.625	119.543	30.082	0.821	29.261	18.640	0.0	0.892	4601.	116520J.
JUN 3	149.935	120.335	29.600	1.429	28.171	27.440	0.0	0.890	6011.	1104240.
S.TOTAL										
JUL 1	150.000	120.500	29.500	1.505	27.915	29.273	2.158	0.893	6800.	1561440.
JUL 2	149.906	120.259	29.647	1.361	28.286	26.593	0.0	0.890	6331.	3630F0.
JUL 3	149.928	120.317	29.611	1.413	28.198	27.239	0.0	0.890	6465.	1632000.
S.TOTAL										
AUG 1	149.289	118.685	30.604	0.424	30.180	9.107	0.0	0.850	2209.	4858200.
AUG 2	149.292	118.693	30.594	0.427	30.172	9.193	0.0	0.850	2230.	580160.
AUG 3	149.318	118.760	30.558	0.448	30.110	9.940	0.0	0.850	2406.	535200.
S.TOTAL										
SEP 1	149.368	118.888	30.420	0.494	29.936	11.361	0.0	0.872	2809.	1700544.
SEP 2	149.278	118.656	30.622	0.416	30.206	8.784	0.0	0.869	2007.	674160.
SEP 3	149.523	119.283	30.240	0.672	29.568	15.753	0.0	0.869	3916.	481680.
S.TOTAL										
OCT 1	149.817	120.032	29.785	1.169	28.616	24.075	0.0	0.895	5831.	209F00.
OCT 2	150.000	120.500	29.500	1.585	27.915	29.273	0.0	0.893	6800.	1392440.
OCT 3	149.627	119.544	30.074	0.825	29.251	14.707	0.0	0.892	4614.	1632000.
S.TOTAL										

TOTAL

Sustainability Forecast of the Selected Major Crops

1. Method of Forecasting

Paddy rice, garlic and mungbean together with virginia tobacco and cotton have been selected as the major crops in the Project. In order to analyze their demand-supply balance at the three levels of the whole country, Ilocos region and Ilocos Norte province during the period of 1975 to 1979, the demand of these crops has been estimated base on the NEDA data on the projected population and food consumption per capita whereas the supply capacity estimation has been made based on the production data of BAEcon and the other government agencies.

The data of the projected population applied in forecasting are shown below;

Year	<u>Projected Population</u>		
	<u>Whole Country</u>	<u>Ilocos Region</u>	<u>Ilocos Norte Province</u>
1975	42,517	3,310	371
1976	43,841	3,374	380
1977	45,165	3,438	389
1978	45,500	3,499	397
1979	46,700	3,565	405
1980	49,137	3,631	414
1990	65,041	4,445	497

Source: Population Dimension of Planning, NEDA, 1975
(Medium Assumption)

To compute the demand of the selected major crops in from 1975 to 1979 and 1990, the food consumption per capita shown in the following table has been applied in the computation.

Consumption per Capita

Crop	Philippines			Ilocos Region		
	1975-1979	^{1/} I.Q.E.	^{2/} 1990 ^{3/}	1975-1979	^{1/} I.Q.E.	^{2/} 1990 ^{3/}
Rice & rice product	106.1kg	0.13%	112.9kg	133.8kg	0.02%	135.1kg
Garlic	0.4	0.48	6.5	0.3	0.23	0.4
Mungbean	1.3	0.48	1.6	1.8	0.34	2.1

Note: ^{1/} The data on the consumption per capita for the year from 1974 to 1976 in "the Regional Consumption Pattern for Major Foods" prepared by DA in 1976.

^{2/} Income-quantity elasticity in the report mentioned in ^{1/}

^{3/} Forecast from the assumed income increase rate at 50% from 1974-1976 to 1990, applying the above-mentioned I.Q.E.

B. Forecast of Marketability

1. Rice

The Philippines was an importer of rice for a long time although some surplus of rice was recorded in 1979 (See Table 4C-1) It is presumed that the average rice consumption per capita will continuously increase in future, too, judging from the small average consumption per capita at present as well as from the fact that reportedly about 20 percent of the total population lives on corn.

The Five-year Development Plan of the Government indicates that the total demand for rice in the year 1990 will increase to 140 percent of that as of 1978. This estimate has been made based on such a small consumption per capita as about 95 kg. Therefore, it is well anticipated that the actual demand for paddy rice will be more than the above-estimated in 1990.

In 1979 rice equivalent to 20 percent of the total demand in Ilocos region was short. The demand for rice in the region as of 1990 is forecast at 604 thousand tons. To meet the above-mentioned demand, the yield per unit area of paddy should increase to 3.1 ton/ha if the paddy cropping area is not expanded. In fact, the paddy cropping area in the region has not been expanded these 10 years:

Table 4C-1. Projected Balance on Demand and Supply, Rice
(Unit: thousand ton)

Item	Actual				Forecasted 1990
	1975	1976	1977	1978	
Philippines					
Supply (Paddy)	5,660.0	6,159.5	6,456.1	6,894.9	-
(Rice)	3,396.0	3,695.7	3,873.7	4,136.9	*7,329
Demand (Rice)	4,506.0	4,647.0	4,787.0	4,140.0	*5,766
Balance (Rice)	(-)1,110.0	(-)951.0	(-)913.0	(-)3.0	*1,563
Region I					
Supply (Paddy)	422.0	577.5	511.0	575.8	-
(Rice)	253.2	346.5	306.6	345.5	-
Demand (Rice)	443.5	452.1	460.7	468.9	596
Balance (Rice)	(-)190.3	(-)105.6	(-)154.1	(-)123.4	(-)94.5
Ilocos Norte					
Supply (Paddy)	50.9	72.4	83.6	83.3	-
(Rice)	30.5	43.4	50.2	50.0	-
Demand (Rice)	49.7	50.9	52.1	53.2	67
Balance (Rice)	(-)19.2	(-)7.5	(-)1.9	(-)3.2	(-)1.1

Source; Supply ---- BAEcon
----- Five-year Philippine Development Plan (1978-1982)
Note: Conversion rate from paddy to rice : 60%

On the contrary, it is the tendency that the paddy cropping area is decreasing in the region recently. It might be difficult to increase in a short time the present yield per unit area to the above-mentioned 3.1 ton/ha. Under the situations, Ilocos region will remain as one of the rice deficit area in the year of 1990.

Ilocos Norte province recorded some surplus production of rice in 1979, however, the production trend indicates that the production in the province has a great yearly variation.

The demand for rice in the province as of 1990 has been forecast at 67 thousand tons. To meet the forecast demand plus the buffer stock equivalent to 15 percent of this forecast demand, the target production of rice in 1990 should be set up at 77 thousand tons, which is converted to the unhulled rice of 130 thousand tons. This volume of unhulled rice is equivalent to 150% of the averaged production in the province during the three year-period from 1977 to 1979. (The difference between the forecasted demand for the year of 1990 and the said present production is some 44 thousand tons.)

At least an area of about 10 thousand hectares of paddy harvesting area would be newly developed under on-going irrigation projects inclusive this Project. Under the situations, the above-mentioned target could be easily achieved if the yield per unit area slightly increases.

2. Virginia Tobacco

PVTA data shows that the average production of virginia tobacco at the whole country level was about 42 thousand tons during 1975 to 1978. Out of this production, about 10 thousand tons were exported as one of the major export goods obtaining foreign currencies. In the Five-year Development Plan of the Government the annual increase rate of virginia tobacco production is projected at 2.4 percent during 1978 to 1987.

Ilocos region and Ilocos Norte province produced virginia tobacco equivalent to 94% and 10% of the national production in 1977, respectively. The Presidential Degree on the zonificaion of virginia tobacco production area suggests that the cropping area of virginia tobacco will be continuously concentrated to Ilocos region.

The virginia tobacco production in the Project Area could increase at least up to 130 percent of the total production as of 1978 if 2.4 percent of annual increase rate is applied.

3. Garlic

The BAEcon data indicate that the garlic production in Ilocos region maintained its share of more than 75 percent of the total production in the whole country for the past ten years from 1970 to 1979. Garlic production area in the region concentrates in the Project Area and its neighborhood.

At the national level, the domestic demand of garlic in each of the last five years appears larger than the supply capacity as shown in the following table.

Table 4C-2 Demand and Supply Balance, Garlic

Item	(Unit:thousand ton)					Forecasted 1990
	Actual 1975	Actual 1976	Actual 1977	Actual 1978	Actual 1979	
Philippines Supply	16.0	15.2	16.0	16.8	14.0	
Demand	17.0	17.5	18.1	18.2	18.7	32.0
Balance	(-)1.0	(-)2.3	(-)2.1	(-)1.4	(-)4.7	
Region I Supply	11.8	11.9	12.4	13.6	9.6	
Demand	1.0	1.0	1.0	1.0	1.1	1.8
Balance	10.8	10.9	11.4	12.6	8.5	
Ilocos Norte Supply	9.9	10.0	10.5	11.5	7.4	
Demand	0.1	0.1	0.1	0.1	0.1	0.2
Balance	9.8	9.9	10.4	11.4	7.3	

Source: Supply----BAEcon

However, the "Garlic Projection and Marketing, Ilocos Region" prepared by DA in 1978 suggests that the Philippines has been an exporter of garlic these years, specially in 1978. A considerably large amount of demand than the supply capacity as previously described may be derived from the underestimate of the garlic production in the production data.

The demand of garlic at national level for the year of 1990 is forecast as much as 32 thousand tons, which corresponds to about 176 percent of the total demand in 1978. The production target in the Project Area for the year 1990 could be set up at 176 percent of the total production in 1978 since the marketing share of garlic in the Project Area will be easily maintained, at least, at present level.

4. Mungbean

A marked increase of mungbean production in the whole country has been registered since 1973 according to the BAEcon production data. The production share of mungbean in Ilocos region was expanded to a great extent from seven percent in 1973 to 45 percent in 1977 due to the increase of harvested area by five times. This sharp increase in mungbean production of the region made almost 100 percent contribution to the nation-wide increase of this crop during the said period.

The mungbean production data in the Ilocos Norte province also show a moderately large increase in production and harvested area as well since 1973. The harvested area was expanded in 1979 to 1.9 times of that in 1973 whereas the production in 1979 increased only to 1.4 times of that in 1973. That is, the increase of production per unit area is not observed during the period.

As seen in the following table 4C-3, a tremendous amount of deficit is found in the demand-supply balance of mungbean at the national level.

Table 4C-3 Demand and Supply Balance, Mungbean

(Unit:thousand ton)

Item		Actual					Forecasted
		1975	1976	1977	1978	1979	1990
Philippines	Supply	21.7	24.4	25.3	26.2	29.0	
	Demand	55.3	57.0	58.7	59.2	60.7	104.1
	Balance(-)	33.6	(-)32.6	(-)33.4	(-)33.0	(-)31.7	
Region I	Supply	6.0	10.8	11.3	N.A.	N.A.	
	Demand	6.0	6.1	6.2	6.3	6.4	9.3
	Balance	0.0	4.7	5.1	N.A.	N.A.	
Ilocos Norte	Supply	0.8	0.9	0.9	1.0	1.0	
	Demand	0.7	0.7	0.7	0.7	0.7	1.0
	Balance	0.1	0.2	0.2	0.3	0.3	

Source; Supply --- BAEcon

The "Commodity Situation Report, Mungbean" prepared by BAEcon in 1979 indicates that the Philippines imported mungbean of about 9 thousand tons during the 10 years from 1967 to 1976 whereas the country exported the product of less than one thousand tons during the same period. The report recommends to export mungbean since it is free from tariff duties.

The production target to meet the forecast demand in the year 1990 at the national level should be set up so much as 3.9 times of the total production in 1978. In this connection, the production in the Project Area could be at least as much as to 3.9 times of the production in 1978.

6. Cotton

Philippine Cotton Cooperation (PCC) is responsible for the development of cotton production in this country. The "National Cotton Production Development Program, 1979 to 1987" prepared by this organization aims to attain the self-sufficiency of cotton by 1987. The target production for this purpose is 138 thousand tons. The said program also suggests that some 146 thousand hectares are

identified as potential cotton production area, of which 32 thousand hectares (22 per cent of the total) and 4 thousand hectares (two %) belong to Ilocos region and Ilocos Norte province, respectively. For the identified area in Ilocos Norte, three thousand hectares are assumed to be located in paddy fields. Then, the potential cotton area in the Project Area is estimated at 1.3 thousand hectares based on the proportion of paddy field area in the Project Area to the total paddy field area in Ilocos Norte province.

Table 4C-4. Cropping Area by Cropping Pattern, Sub-project Area
(With Project, In Future)

Cropping Pattern (Wet Season + Dry Season)	(Unit:ha)								
	Cura	Nueve Era	Sub-total	Madupayas	Batac- Paoy	Pinili	Badoc- Sinait	Sub-total	Total
1 Paddy + Paddy	1,270	600	1,870	60	2,130	80	830	3,100	4,970
2 Paddy + Upland Crops									
(1) Paddy + Tobacco	40	30	70	80	1,150	280	550	2,060	2,130
(2) Paddy + Garlic	100	40	140	-	700	450	915	2,065	2,205
(3) Paddy + Garlic + Mungbeans	-	-	-	-	700	450	915	2,605	2,065
(4) Paddy + Cotton	-	-	-	20	510	140	360	1,030	1,030
(Sub-total)	(140)	(70)	(210)	(100)	(3,060)	(1,320)	(2,740)	(7,220)	(7,430)
Total (Physical Area)	<u>1,410</u>	<u>670</u>	<u>2,080</u>	<u>160</u>	<u>5,190</u>	<u>1,400</u>	<u>3,570</u>	<u>10,320</u>	<u>12,400</u>
(Cropping Area)	<u>2,820</u>	<u>1,340</u>	<u>4,160</u>	<u>320</u>	<u>11,080</u>	<u>3,250</u>	<u>8,055</u>	<u>23,245</u>	<u>26,865</u>
Cropping Intensity (%)	200	200	200	200	213	232	226	225	217

Result of Field Survey on Farmers' Intension for Improvement of Farm Management

A. Purpose and Method of the Survey

The propose of the survey is to know the needs of irrigation project for farm management in the Project Area and the field survey by interviewing some 70 sample farmers was made with prepared questionnaire in the Project Area, exclusive of Cura and Nueva Era Sub-project Areas from 18th to 28th, January, 1980.

The selected sample Barangays and sample farmers were, as a general rule, the same as those of the Farm Management Survey conducted by NIA in 1978. But some interviewees were unavoidably different from there of the NIA survey. The sample Barangays and number of farmers by Barangays are listed as follows;

Sample Barangays and Nos. of Sample Farmers

<u>No.</u>	<u>Municipality</u>	<u>Barangay</u>	<u>Nos. of Sample Farmers</u>
1	Batac	San Pedro	9
2	Paoay	Cabaguan	7
3	Paoay	San Roque	5
4	Paoay	Sideg	3
5	Paoay	Bacsil	3
6	Pinili	Dart	5
7	Pinili	Punitac	6
8	Pinili	Tartarabang	9
9	Badoc	Ar-arusip (A)	6
10	Sinait	Battog	4
11	Sinait	Duy-yayat	7
12	Sinait	Barikir	6
<u>Total</u>			<u>70</u>

5. Result of Survey

Farmers' responses to each question in the questionnaire are tabulated in Table 4C-5. Based on the Table 4C-5, farmers' attention for the improvement of their farm management and also the necessity of irrigation project are summarized as follows;

1. Outline of Farm Management of Sample Farmers

Average farm size per farm household is 1.2 hectares, including 1.1 hectares of paddy field and 0.1 hectare of upland field. Out of 1.1 hectares of paddy field, 0.2 hectares are irrigated by Communal Irrigation Systems and the remaining 0.9 hectares are rainfed. As for land tenure of the cultivated land, each half of 1.2 hectares is owned and tenanted.

The average cropping area by crops per farm household in this survey are as follows;

Cropping Area by Crop per Farm Household

<u>Crop</u>	<u>Area (ha)</u>	<u>Percentage (%)</u>
Paddy (Wet season)	1.1	52.4
Tobacco	0.3	14.3
Garlic	0.4	19.0
Mungo	0.1	4.8
Corn	0.1	4.8
Others	0.1	4.7
<u>Total</u>	<u>2.1</u>	<u>100.0</u>

The main cropping pattern by each sample farmer is shown as follows;

Main Cropping Pattern of Sample Farmers

<u>Cropping Pattern</u>	<u>Nos. of Farmers</u>	<u>Percentage (%)</u>
1. Paddy (P) only	3	4.3
2. P + Tobacco (T)	3	4.3
3. P + Garlic (G)	13	18.6
4. P + Corn (C)	1	1.4
5. P + T / G	17	24.4
6. P + T / G	1	1.4
7. P + T / Mung Bean (M)	1	1.4
8. P + G / M	4	5.7
9. P + G / O	1	1.4
10. P + G / C	3	4.3
11. P + M / C	1	1.4
12. P + T / G / M	1	1.4
13. P + T / G / C	9	12.9
14. P + T / G / O	3	4.3
15. P + T / M / C	1	1.4
16. P + G / M / C	1	1.4
17. P + T / G / C / O	6	8.6
18. P + T / G / M / C / O	1	1.4
<u>Total</u>	<u>70</u>	<u>100.0</u>

2. Major Source of Family Income in Future

All of sample farmers responded that they would continue their present farm management in future as well and the major source of their family income would come from their farm management.

3. Present Irrigation Condition

Seventy-seven percent of interviewees can not have enough irrigation water, especially during the dry season.

4. Flooding Damages in Past Five Years

Sixty percent of sample farmers have been suffered from flood damages two or three times on an average within past five years. Among the flood damages, the biggest damage experienced was such damage as scale of 0.3 hectares in the area with 1.7 meters flood depth and 3.3 days in duration.

5. Main Cropping Pattern in Future

Seventy-eight percent of sample farmers chose the cropping pattern of three croppings a year, for example, "Paddy + Garlic + Mung Bean", as the main cropping if they can have enough irrigation water throughout the year.

On the other hand, thirty two percent of sample farmers responded that they would choose such cropping pattern as "Paddy + Paddy" or "Paddy + Tobacco or Garlic + Paddy", both of which have two croppings of paddy. It may be one of the reason why so many farmers as thirty two percent of interviewees desire to plant paddy twice a year that these farmers want to increase rice production because their rice production is limited under small farm size at present.

Table 4C-5. Farmers' Response to Questionary

Main Item	Sub-Item	Unit	Total	Per Farm Household
I. General Information on Their Farm Management	I-1. Cultivated area:			
	- Paddy land, Owned	ha	36.6	0.5
	- Paddy land, Tenanted	ha	43.5	0.6
	- Upland, Owned	ha	6.6	0.1
	- Upland, Tenanted	ha	3.4	0.0
	I-2. Irrigated area by:			
	- Wet season Communal irri.systems	ha	12.6	0.2
	- Dry season	ha	-	-
	I-3. Irrigated area by:			
	- Wet season Pumps	ha	-	-
	- Dry season	ha	51.8	0.7
	I-4. Planted area by:			
- Paddy, Wet crop	ha	80.1	1.1	
- Paddy, Dry	ha	-	-	
- Tobacco	ha	19.4	0.3	
- Garlic	ha	29.0	0.4	
- Mungo	ha	4.8	0.1	
- Corn	ha	11.1	0.1	
- Others	ha	2.5	0.1	
- Total	ha	146.9	2.1	
I-5. Nos. of family members			420	6.0
I-6. Nos. of farm labor within family			200	2.9

(Cont'd)

Table 6. Farmer's Perception of the Future (Cont'd)

Main Item	Sub-Item	Unit	Total	Percent
II. Source of Family Income in Future	II-1. To continue farm management in future ; - Farm income only		68	97
	- Farm income + Others		2	3
	- Others/No response		0	0
	II-2. To stop farm management in future		0	0
	II-3. No response/other reply		0	0
III. Present Irrigation Condition (Availability of Irrigation Water)	III-1. Sufficient		16	23
	III-2. Deficient ; - In both wet & dry season		6	9
	- In wet season only		0	0
	- In dry season only		48	68
	III-3. No response/other reply		0	0
IV. Flooding Damages in Past Five Years	IV-1. Not suffered ;		28	40
	IV-2. Suffered ;		42	60
	- Nos. of times	time	2 - 3	
	- Damaged area (biggest flood)	ha	0.3	
	- Flood depth (biggest flood)	m	1.7	
	- Flood duration (biggest flood)	day	3.3	

(Cont'd)

Table 4C-5. Farmers' Response to Questionary (Cont'd)

Main Item	Sub-Item	Unit	Total	Percent
V. Main Cropping Pattern in Future (In case of Full Irrigation)	V-1. One cropping a year;		0	0
	V-2. Two cropping a year;	- Paddy + Paddy (P)	8	11
		- Paddy + Garlic (G)	5	8
		- Paddy + Tobacco(T)	0	0
		- Paddy + Other diversified crops	2	3
	V-3. Three cropping a year;		15	21
	- P + T/G + P	18	26	
	- P + T/G + Corn/Others	15	21	
	- P + T/G + Mungo (M)	7	10	
	- Others (Ex. P+P+M)	70	100	
VI. Needs of Agricultural Development Project	VI-1. Construction/improvement of irrigation facilities	%	70	100
	VI-2. Construction of drainage facilities	%	70	100
	VI-3. Construction/improvement of farm roads	%	70	100
	VI-4. Strengthening of extension activities	%	70	100
	VI-5. Expansion of farm credits such as M-99	%	70	100
	VI-6. Land exchange for grouping dispersed parcels	%	70	100

FIGURE 4C.1 FARM PRACTICES AND FARM INPUTS, PADDY RICE

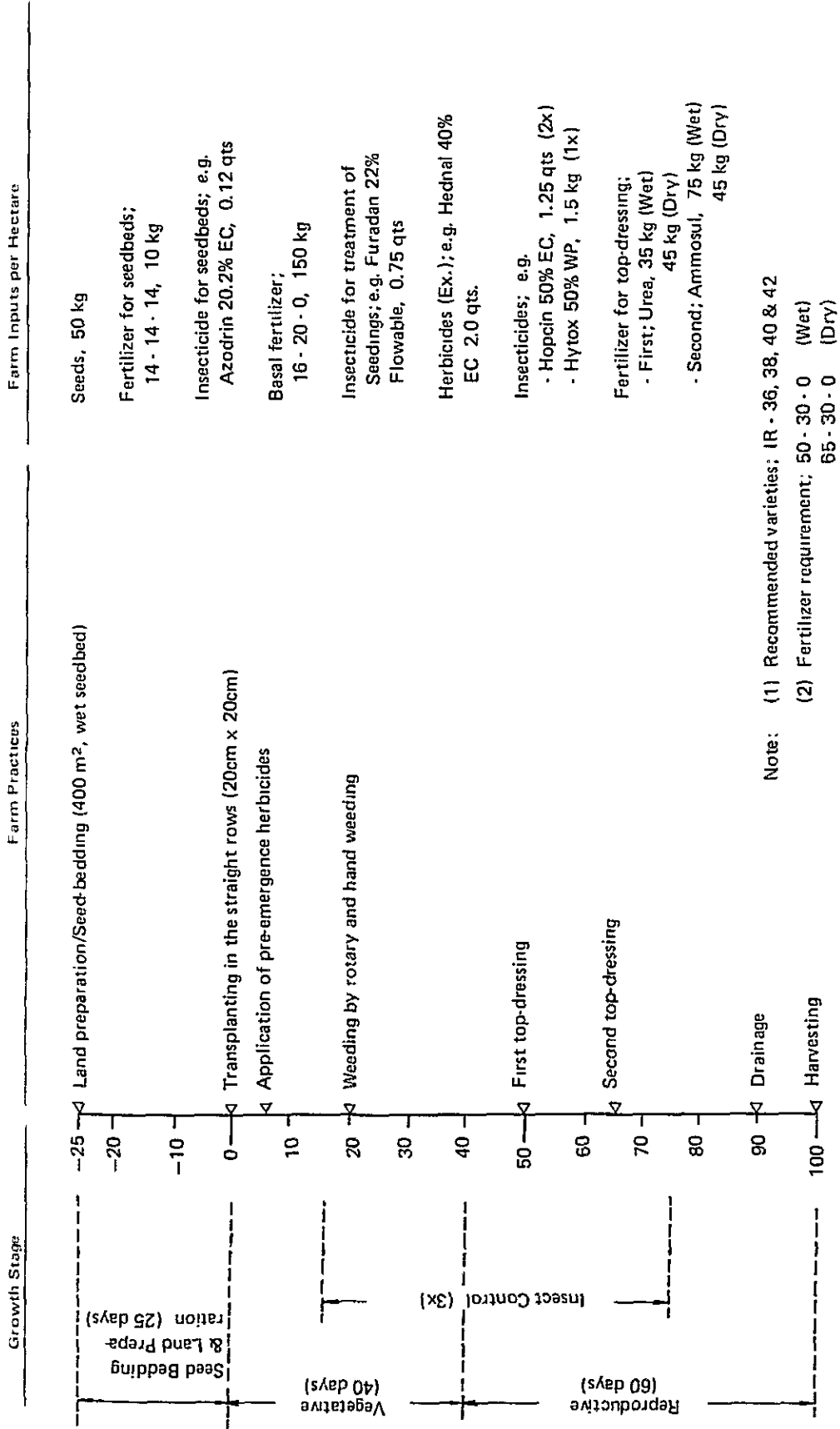
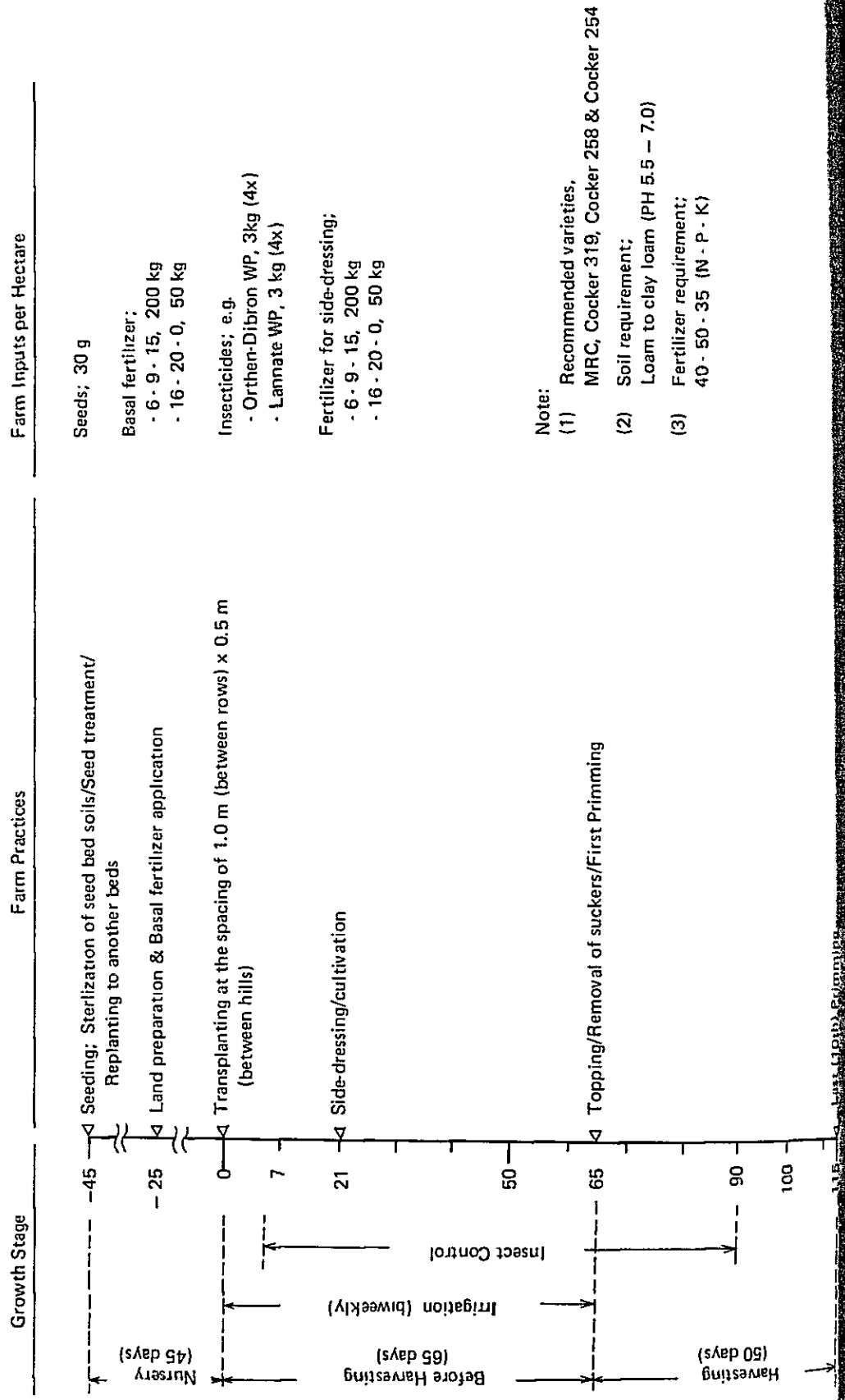


FIGURE 4C-2. FARM PRACTICES AND FARM INPUTS, VIRGINIA TOBACCO



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FIGURE 4C-3 FARM PRACTICES AND FARM INPUTS, GARLIC

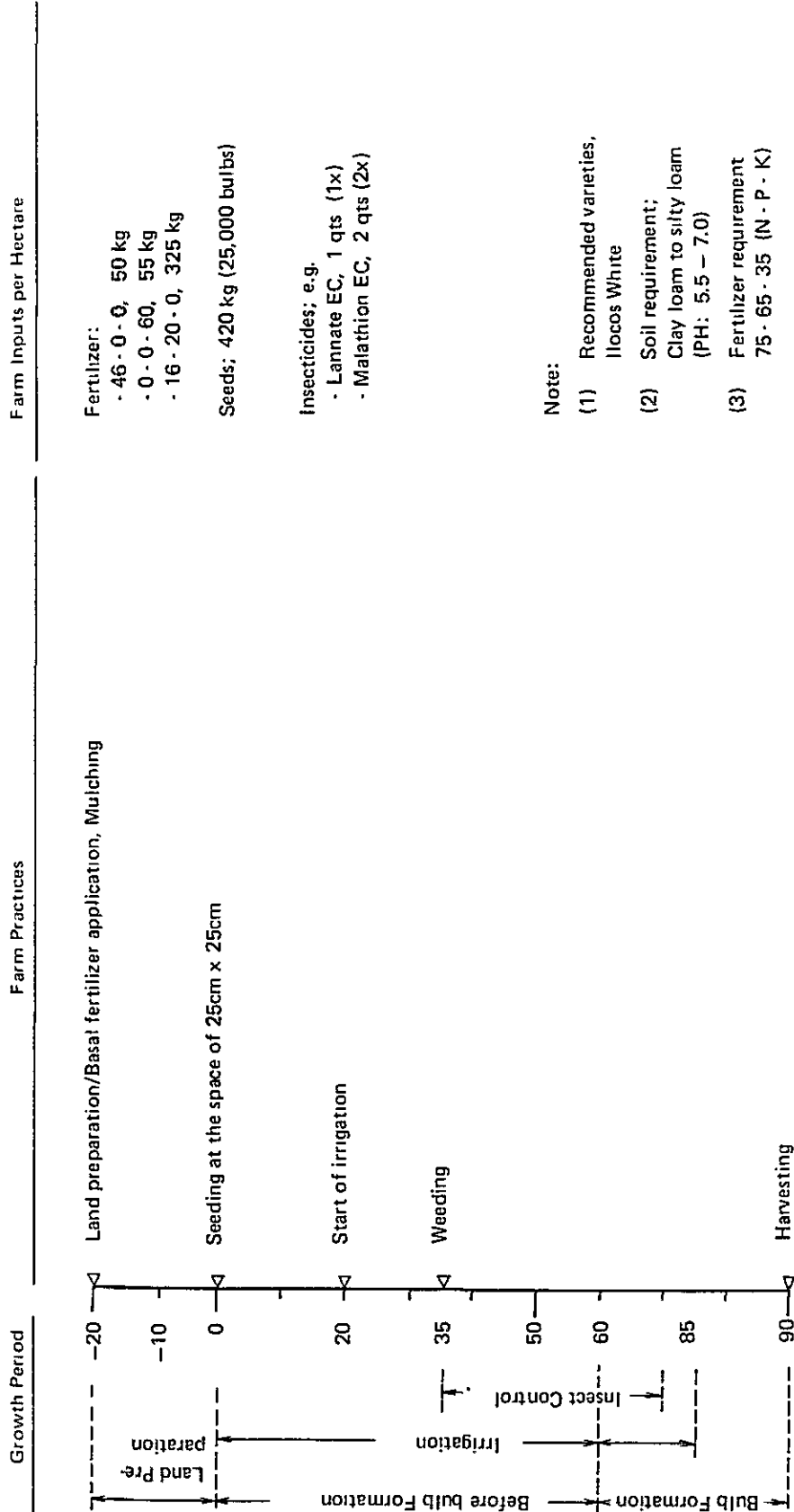


FIGURE 4C-4. FARM PRACTICES AND FARM INPUTS, MUNGBEAN

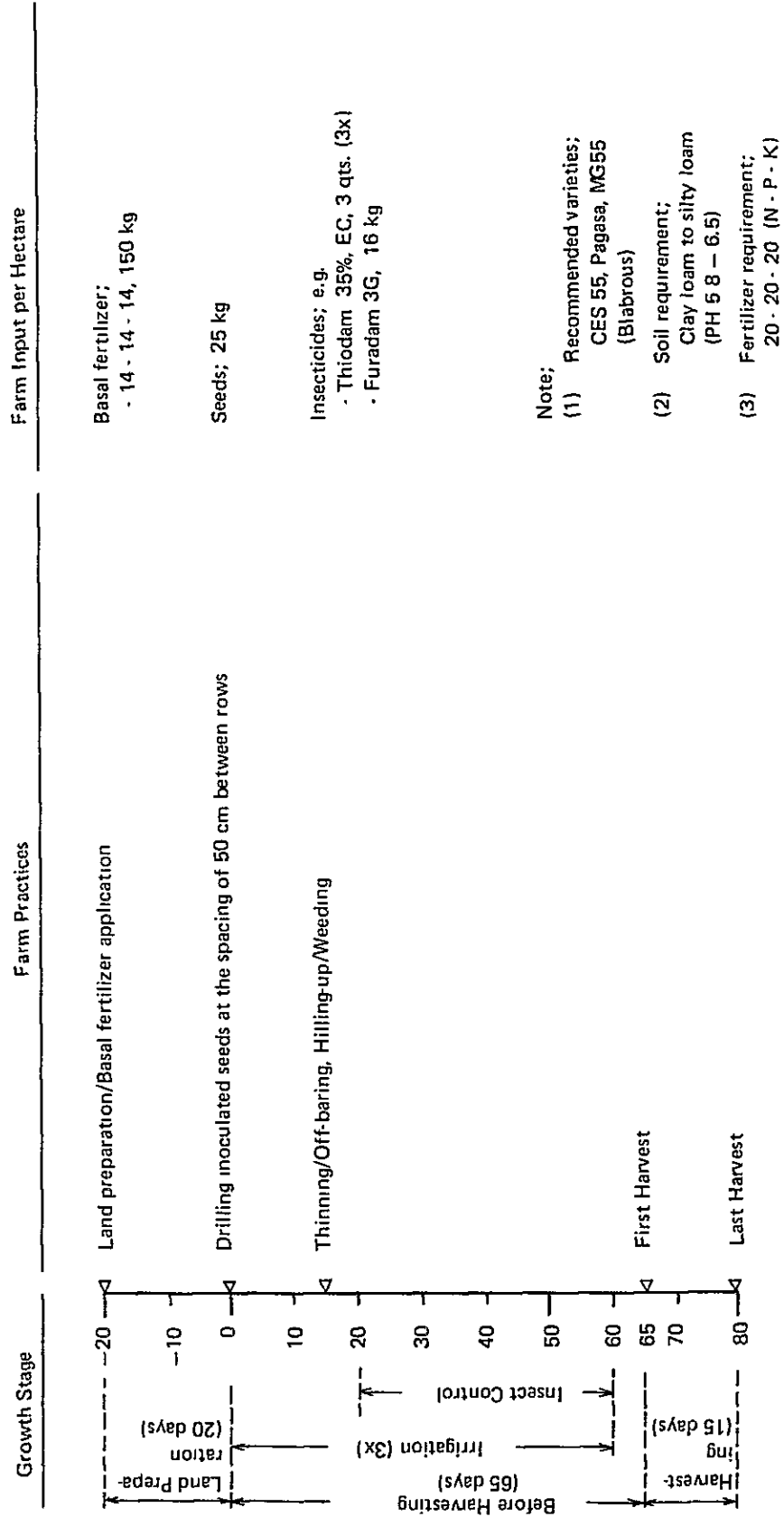


FIGURE 4C 5. FARM PRACTICES AND FARM INPUTS, COTTON

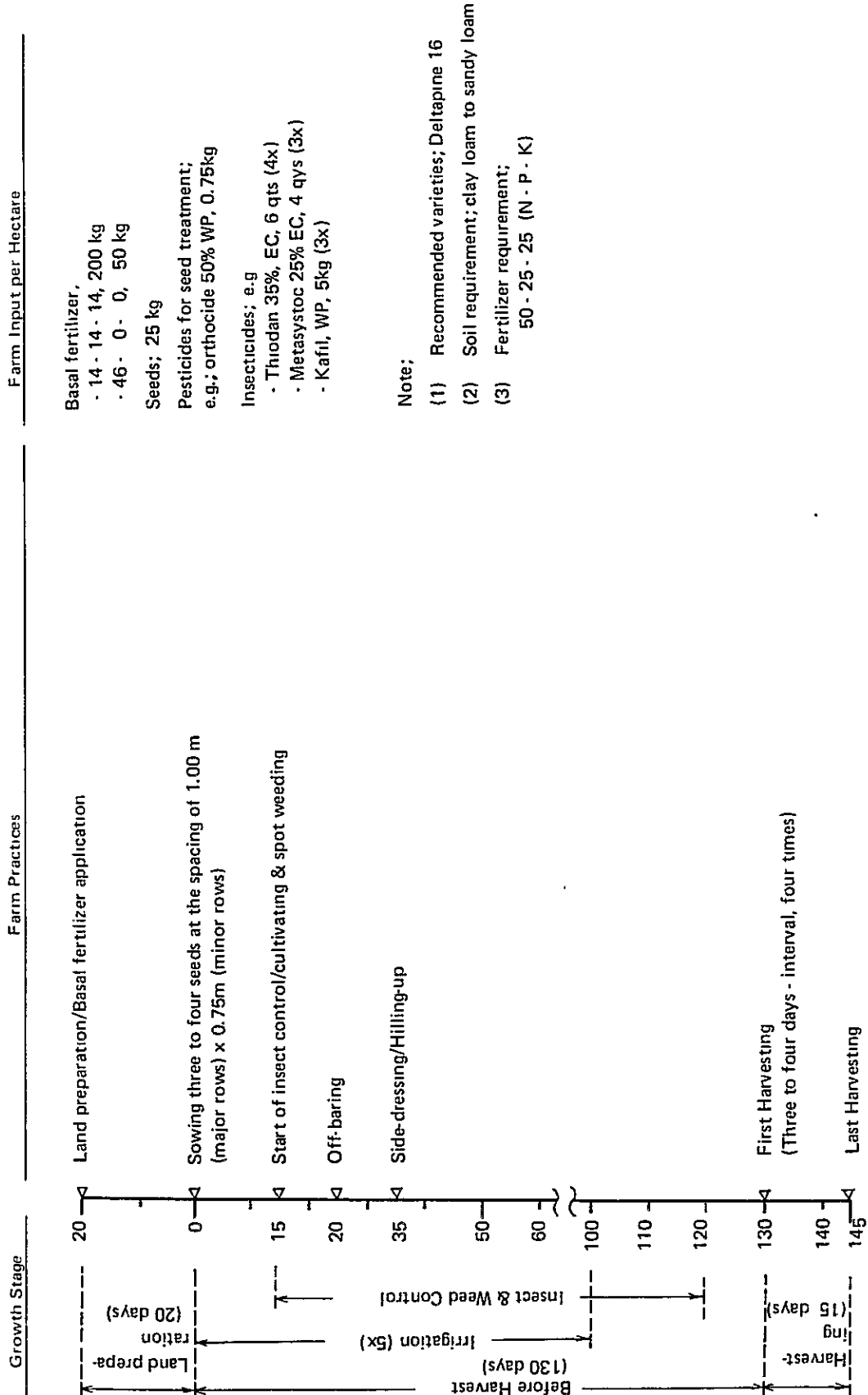


Table 4C-6. Total Amount of Input Materials, with Project, in Future

Input Materials	Unit	Paddy, Wet (12,400 ha)		Paddy, Dry (4,970 ha)		Tobacco (2,130 ha)		Garlic (4,270 ha)		Mung Bean (2,065 ha)		Cotton (1,030 ha)		Ground Total
		Per ha	Total	Per ha	Total	Per ha	Total	Per ha	Total	Per ha	Total	Per ha	Total	
1. Seeds	ton	50 kg	620	50 kg	249	0.03 kg	0	420 kg	1,793	25 kg	52	25 kg	26	Paddy 869 Tobacco 0 Garlic 1,793 Mung Bean 52 Cotton 26
2. Fertilizer														
- Urea (45-0-0)	ton	35 kg	434	45 kg	224	-	-	50 kg	214	-	-	50 kg	52	924
- Ammosul (21-0-0)		75 kg	930	100 kg	497	100 kg	213	-	-	-	-	-	-	1,640
- Potassium Chloride (0-0-60)		-	-	-	-	-	-	55 kg	235	-	-	-	-	235
- Compound (16-20-0)		150 kg	1,860	150 kg	746	-	-	325 kg	1,388	-	-	-	-	3,994
- Compound (14-14-14)		10 kg	124	-	-	-	-	-	-	150 kg	310	200 kg	206	640
- Compound (6-9-15)		-	-	-	-	400 kg	852	-	-	-	-	-	-	852
Total														8,285
3. Insecticides														
- Liquid	x100 qt.	2.1 qt.	260	2.1 qt.	104	6.0	128	3.0 qt.	128	30 qt.	62	10.0 qt.	103	785
- Water Soluble Powder	ton	1.5 kg	19	1.5 kg	7	3.0	6	-	-	-	-	5.8 kg	6	38
- Granular	kg	-	-	-	-	-	-	-	-	17 kg	35	-	-	35
4. Herbicides														
- Liquid	x100 qt.	2.0 qt.	248	2.0 qt.	99	-	-	-	-	-	-	-	-	347
- Water Soluble Powder	kg	-	-	-	-	-	-	-	-	-	-	-	-	-

Note: (1) The figures in the parentheses show the total amount of input materials with the project.

Target Yield of Major Crops, "With Project"

1. Paddy

a. Experimental Yield

Following experimental data on paddy yield at each amount of nitrogen were used to have the quadratic equation for the regression between yield (Y) and amount of applied nitrogen (X).

Paddy Yield at Each Level of Applied Nitrogen

(unit:ton/ha)

Season	Nitrogen Application (kg/ha)				
	0	30	90	120	150
Wet	3.3	3.9	4.5	4.2	-
Dry	3.5	-	4.7	5.3	5.4

Source: BPI, Average yield of the latest 6 HYVs at three national experiment stations (Maligaya Rice Research and Training Center, Bicol Rice and Corn Experiment Station and Visaya Rice Experiment Station)

The quadratic equations for above data are shown as follows;

$$\text{Wet season crop: } y = 3.28 + 0.025x - 0.00014x^2$$

$$\text{Dry season crop: } y = 3.50 + 0.029x - 0.00011x^2$$

(The curves for both equations are shown in Figure 4C-6.)

b. Attained Yield in the Project and Its Vicinity

The attained yield under Masagana 99 (1976-1979) in the Project municipalities is 3.6 ton/ha for wet season crop and 3.5 ton/ha for dry season as shown in Table 3D-16, Appendix 3D-1.

Almost all the same level of average yields are attained in the existing national irrigation system areas which are located near to the Project Area as shown below:

Attained Yield in Existing National Irrigation
System Areas (1977)

<u>Area</u>	<u>Yield</u> (cavan/ha)	<u>Area Coverage of HYVs</u>
Laoag-Vintar	70	more than 70%
Dingras	70	more than 75%
Cura	40	less than 50%
Bonga Bump #2	73	more than 70%
Mean	<u>71</u> (3.6 ton/ha)	

Note: (1) The yields refer to the average yields for wet and dry season crops

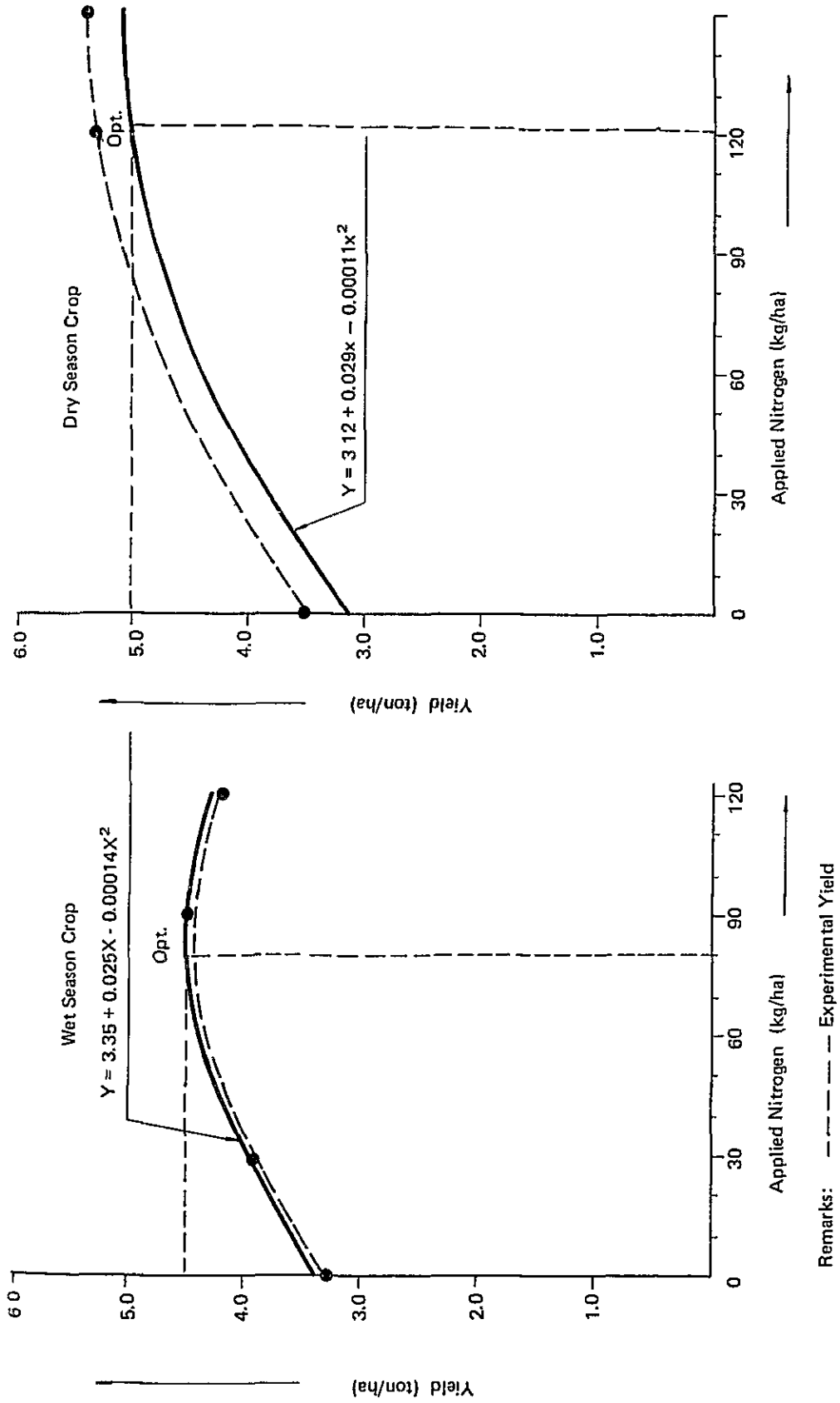
(2) The mean yield is exclusive of the yield in Cura area because of small area coverage of HYVs.

The attained yield in the foundation seed farm at BPI Dingras Experimental Station (1977) was 3.8 ton/ha for wet season crop and 3.5 ton/ha for dry season crop. But the station has attained more high yield of 4.4 ton/ha for wet season crop a few years ago.

The irrigation conditions in the existing national irrigation system areas need rehabilitation under NISIP because of the inadequate irrigation water amount and on-farm facilities. Also, at BPI Dingras experiment station, the irrigation water supply during dry season is limited very much because the irrigation water supply to the station is made by one of the existing national irrigation systems. These unfavorable irrigation conditions are considered to be the main reason why the comparatively low yield in the existing national irrigation system areas and also at BPI Dingras Experimental Station.

On the other hand, the yield of 4.41 ton/ha was attained at 70 kg of applied nitrogen according to the yield record of FAO/NFAC/JICA Fertilizer Trial (1978-1979) in three Project municipalities (See Table 3D-18, Appendix 3D-1).

FIGURE 4C-6. EXPERIMENTAL AND PREDICTED YIELD OF PADDY



It is considered that the proposed irrigation development of the Project will improve the irrigation conditions to attain high yield, at least such level of attained yield of 4.41 ton/ha at 70 kg of applied nitrogen for wet season paddy rice by FAO/HFAC/JICA Fertilizer Trial in the Project municipalities, as shown previously. The following equation is assumed to represent the quadratic equation for the regression between potential yield (y) and amount of applied nitrogen (x) in the Project, which is produced by substituting the above yield data into the said equation for the experimental yield data of wet season crop:

$$y = 3.35 + 0.025x - 0.00014x^2$$

For dry season crop, such kind of yield data for the substitution is not available. Therefore, the assumed potential yield of 4.78 ton/ha at 70 kg of applied nitrogen is used to have the equation as follows:

The assumed potential yield for dry season crop:

$$\frac{4.41 \text{ ton/ha (potential yield of the wet season crop)} \times 4.7 \text{ ton}}{4.5 \text{ ton}} = 4.61 \text{ ton/ha}$$

where; 4.7 tons and 4.5 tons are the above-mentioned experimental yields at 90 kg of applied nitrogen for wet season crop.

Thus the new equation with regard to potential yield for dry season crop in the Project Area is as follows:

$$y = 3.12 + 0.029x - 0.00011x^2$$

3. Yield at Optimum Amount of Nitrogen Application

Optimum amount of nitrogen application and their yields are computed as follows:

Wet season crop:

$$\text{Opt. nitrogen (kg/ha)} = \frac{0.025P_y - P_n}{2(0.00014 \times P_y)} = 82 \text{ kg}$$

Yield at 82 kg of the nitrogen application = 4.5 ton/ha

Dry season crop:

$$\text{Opt. nitrogen (kg/ha)} = \frac{0.029P_y - P_n}{2(0.00011 \times P_y)} = 122 \text{ kg}$$

Yield at 122 kg of the nitrogen application = 5.0 ton/ha

Where $P_y = \text{P } 2,090$ (Paddy price per ton)

$P_n = \text{P } 4.5$ (Nitrogen price per kg)

(Projected prices for the year of 1990 at the values in 1980)

4. Estimated Yield at Different Land Class

The yields at optimum amount of nitrogen application are regarded as the potential yield after Project. Hence, the yield at different land classes are estimated as follows:

Target Yield of Paddy

<u>Land Class</u>	<u>Range of Productivity (%)</u>	<u>Average Productivity Rating (%)</u>	<u>Target Yield (ton/ha)</u>	
			<u>Wet Season</u>	<u>Dry Season</u>
Potential	100	100	4.5	5.0
1R	90 - 100	95	4.3	4.7
2R	80 - 90	85	3.8	4.2
3R	70 - 80	75	3.4	3.8

B. Upland Crops

1. Tobacco

The potential yields of virginia tobacco varieties to be introduced in the Project are estimated at 2.0 ton/ha, based on the following Table.

Potential Yields of Selected Varieties

<u>Variety</u>	<u>Growth^{1/} Period (day)</u>	<u>No. of Harvested Leaves</u>	<u>Potential^{2/} Yield (ton/ha)</u>
NC Blight Yellow	80 - 90	29	2.08
Coker 254	90 - 100	26	2.27
Coker P-11-1	N.A.	N.A.	2.05
MRS-3	90 - 100	25	1.89
Golden Harvest	80 - 90	26	1.91
Average			<u>2.04</u>

Note: 1/ : From transplanting to flowering

2/ : Data source : "Three Years of Cooperative Research on Philippine Virginia Tobacco, UFLA. PVTA Research & Training Program, 1973"

The PVTA compact farm that was organized in San Pedro, Batac by the farmer participants to the "UPLB-PVTA applied package technology commercial trials for virginia tobacco" attained about 1.6 tons of average yield in 1977-78, according to "the Philippine Virginia Tobacco Industry : In Search for Effective Technology for Development and Transfer in the Farmland prepared by Adolf C. Necesito in 1979"

The yield was attained by applying the PVTA-UPLB and Taiwan technology having the fertilizer recommendations of 30-60-90 and 64-64-128 (N-P-K in kg per hectare) respectively. Referring to

The relationship between the attained yield and the soil conditions in the compact farm, the target yield for second class land is set up at 1.7 tons per hectare.

The target yields by land class are decided in the basis of above mentioned potential yield and the target yield for second class land as follows;

Target Yield of Virginia Tobacco

<u>Land Class</u>	<u>Range of Productivity (%)</u>	<u>Average Productivity Rating (%)</u>	<u>Target Yield (ton/ha)</u>
Potential	100	100	2.0
First	90 - 100	95	1.9
Second	80 - 90	85	1.7
Third	70 - 80	75	1.5

2. Garlic

The potential yield of garlic is estimated at 3.0 ton/ha from the experimental data on the garlic planted in October and November at BII Dingras Experimental Station.

The high-yield farmers, one third of sample farmers in "the Survey on Production and Marketing, Ilocos Region prepared by DA" had about 2.3 tons of average yield per hectare.

Following target yields by land class are decided in the basis of the above-mentioned potential yield and the yield of high-yield farmers.

Target Yield of Garlic

<u>Land Class</u>	<u>Range of Productivity (%)</u>	<u>Average Productivity Rating (%)</u>	<u>Target Yield (ton/ha)</u>
Potential	100	100	3.0
First	90 - 100	95	2.9
Second	80 - 90	85	2.6
Third	70 - 80	75	2.3

3. Mungbeans

The potential yield of mungbeans is estimated at 1.3 tob/ha from the following studied yield of improved varieties.

Studied Yield of Mungbeans

<u>Variety</u>	<u>Studied Yield (ton/ha)</u>	<u>Maturity to days to 1st Priming (day)</u>
MG 50-10A(G)	1.0 - 1.3	60 - 65
MD 15-2	1.0 - 1.3	60 - 65
E.G. Glabrous	1.0 - 1.3	60 - 65
CES 55	1.0 - 1.5	65 - 68
CES 87	1.0 - 1.5	65 - 68
CES 1D-21	1.0 - 1.2	65 - 68

Source: Philippine Recommendation on Mungbeans, PCARR

According to the yield data taken from "the Angat-Magat Integrated Agricultural Development Project Applied Research Phase one Report", the yield of about 1.2 ton/ha was attained for the wet season crop in 1974 on farmers' field of Magat Project Area. (used variety; CES-55)

The target yields by land class are decided as shown in the following Table, assuming the potential yield and the yield for the first class land as 1.3 ton and 1.2 ton per hectare respectively.

Target Yield of Mungbean

<u>Land Class</u>	<u>Range of Productivity (%)</u>	<u>Average Productivity Rating (%)</u>	<u>Target Yield (ton/ha)</u>
Potential	100	100	1.3
First	90 - 100	95	1.2
Second	80 - 90	85	1.1
Third	70 - 80	75	1.0

4. Cotton

The estimated potential yield is 2.9 tons/ha (seed cotton) from the following experimental data.

Experimental Yield of Cotton

(Unit: ton/ha, seed cotton)

<u>Fertilizer Level (kg/ha)</u>	<u>Plant Density (plant/ha) & Planting Time</u>		
	<u>83,882 (Oct. 1st)</u>	<u>93,393 (Nov. 1st)</u>	<u>Mean</u>
120 - 25 - 25	2.4	2.4	2.4
90 - 25 - 25	2.9	2.9	2.9
90 - 0 - 0	1.8	1.0	1.4
50 - 25 - 25	1.2	2.0	1.6
Control	1.1	1.5	1.3

Source: PCC Annual Report (1977 - 1978)
Philippine Cotton Cooperation through Cotton
Development and Research Institute

Notes: (1) Experimental location : Pangasinan (Irrigated)
(2) Variety : Deltapine

The target yields by land class are decided as shown in the following Table, based on the above mentioned potential.

Target Yield of Cotton

<u>Land Class</u>	<u>Range of Productivity (%)</u>	<u>Average Productivity Rating (%)</u>	<u>Target Yield (ton/ha)</u>
Potential	100	100	2.9
First	90 - 100	95	2.8
Second	80 - 90	85	2.5
Third	70 - 80	75	2.2

C. Projected Total Production with Project

1. Paddy

The aggregate projected production after full implementation of the Project is 74,492 ton, 51,916 tons of which are wet season harvest and the balance of 22,576 tons are dry season harvest. The weighted average yield at each area of different land class is 4.2 tons/ha and 4.5 tons/ha for wet and dry season crop respectively. (See Table 4C-7)

2. Upland Crops

The total projected production and the weighted average yield at each area of different land class for each selected upland crops are as follows;

Total Projected Upland Crops Production

<u>Crop</u>	<u>Weighted Average Yield (ton/ha)</u>
Tobacco	1.7
Garlic	2.6
Mungbean	1.1
Cotton	2.5

Table 4C-7. Paddy, Rice, and Upland Crops Production by Land Class, With IR(2), 3R, and Upland Crops Production in 1991, Future

Season	1R or 1R(2)		2R or 2R(2)		3R or 3R(3)		Total					
	Area (ha)	Yield (ton/ha)	Area (ha)	Yield (ton/ha)	Area (ha)	Yield (ton/ha)	Area (ha)	Yield (ton/ha)				
Wet	9,920	4.3	42,656	2,070	3.8	7,866	410	3.4	1,394	12,400	(4.2)	51,916
Dry	3,660	4.7	17,202	980	4.2	4,116	330	3.8	1,254	4,970	(4.5)	22,576

Crop	1R(2) or 2R(2)		3R(3)		Total				
	Area (ha)	Yield (ton/ha)	Area (ha)	Yield (ton/ha)	Area (ha)	Yield (ton/ha)			
(1) Tobacco	2,130	1.7	3,621	-	-	2,130	(1.7)	3,621	
(2) Garlic	4,190	2.6	10,894	80	2.3	184	4,170	(2.6)	11,078
(3) Mungbean	2,065	1.1	2,272	-	-	2,065	(1.1)	2,272	
(4) Cotton	1,030	2.5	2,575	-	-	1,030	(2.5)	2,575	
Total	9,415		80			9,495			

2. Upland Crops

Farm Mechanization and Land Requirement

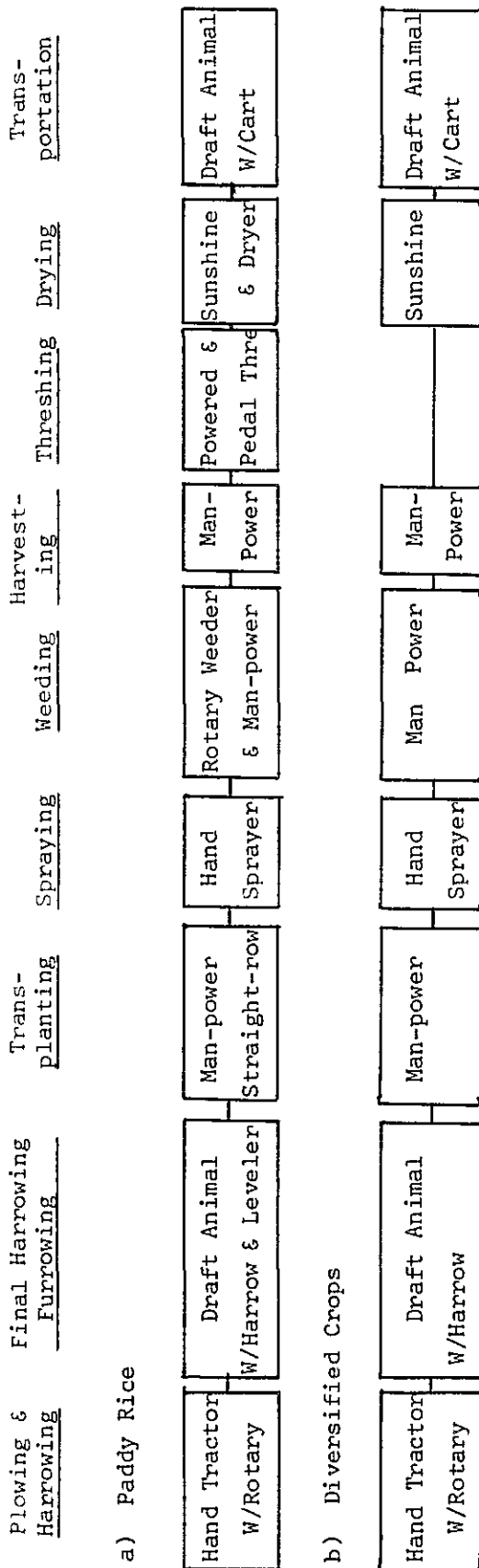
A. Proposed Farm Mechanization Plan

The farm operation systems as shown in Figure 4C-7 will be applied for the scheduled farming after completion of the Project. These systems require mechanization in such major operations as land preparation, threshing and drying. The selected machines and the assumed area coverages of mechanization area by selected machine are as follows;

Selected Farm Machinery and Assumed Area Coverage of Farm Mechanization

<u>Operation</u>	<u>Selected Machine</u>	<u>Area Coverage (%)</u>
Land preparation	- Hand tractor (7-8 HP diesel)	40
Threshing	- Powered thresher (7-8 HP diesel, throw-in type)	50
	- Pedal thresher	50
Drying	- Dryer (flat-bed type, 2.0 ton bin)	50

As for all selected machines, the Philippines-made or the imported ones are available in the Philippines. The traditional way of operation for land preparation by draft animal and drying by sunshine will be employed in the areas where the above-mentioned mechanization will not cover. The farm operation efficiency are calculated for the operation of land preparation, threshing, drying and transportation in both cases of using machineries and draft animals as shown in Table 4C-8.



Note: (1) Area coverage of mechanization for the operations of plowing and harrowing by hand tractor is estimated at 40 percent of cultivated area in both cases of paddy rice and diversified crops.

(2) Area coverage of mechanization for threshing by the powered thresher and pedal thresher is estimated at 50 percent of cultivated area respectively.

Table 4C-8. Efficiency of Farm Operation

Operation	Machinery/ Animal	(1) Ope. Width (m)	(2) Ope. Speed (km/hr.)	(3)= Theoretic Ope. Capacity (ha/hr.)	(4) Efficiency in Field (%)	(5)=(3)x(4) Ope. Capacity in Field (ha/hr.)	(6) Ope. Efficiency (%)	(7)=(5)x(6) Actual Ope. Capacity (ha/hr.)	(8) Hours per ha (hr./ha)	(9) Ope. Times (time)	(10)=(8)x(9) Hours per ha (hr./day)	(11) Ope. Hours per day	(12)= (10)+(11) Days per day
a) Plowing	-Carabao w/plow -Hand tractor w/rotary	0.12 0.55	2.2 1.5	0.026 0.083	84 80	0.022 0.066	80 80	0.018 0.053	55.6 18.9	1 1	55.6 18.9	8 8	7.0 2.4
b) Harrowing	-Carabao w/harrow -Hand tractor w/rotary	1.0 0.55	2.0 2.0	0.200 0.110	80 80	0.160 0.088	65 80	0.104 0.070	9.6 14.3	2 1	19.2 14.3	8 8	2.4 1.8
c) Final Harrowing	-Carabao w/harrow & leveler	0.7	2.0	0.140	80	0.112	65	0.073	13.7	2	27.4	8	3.4
	-Hand tractor w/ harrow & leveler	1.4	3.0	0.420	80	0.336	80	0.269	3.7	2	7.4	8	0.9
d) Threshing	-Powered thresher	-	-	0.298 (1.25ton/hr.)	80	0.238	75	0.179	5.6	1	5.6	6	0.9
	-Pedal thresher	-	-	0.070 (0.32ton/hr.)	80	0.056 (0.25ton)	75	0.042 (0.19ton)	24.0	1	24.0	6	4.0
e) Drying	-Dryer (Flat-bed type, 2.0 tons bin)	-	-	0.119 (0.5ton/hr.)	-	-	80	0.095 (0.4ton/hr.)	10.5	1	10.5	16	0.7
f) Transportation	-Carabao w/ cart	-	2.5	0.1 ton	80	0.16 ton	80	0.1 ton	-	-	-	-	-

3. Number of Required Machinery per Compact Farm

It is considered that the required units of machinery will be introduced in each compact farm or in each two compact farms for the collective use of machinery in order to minimize the machinery costs. The required units of the machinery per compact farm (average size is 40 ha) are computed as follows;

1. Hand tractor

The type of hand tractor having P.T.O. for the attachment of rotary will be chosen in most of the Project Area because of big area coverage of upland crops. One unit of the hand tractor is required for land preparation work in 16 ha (40 percent of compact farm area) in each compact farm in this case as follows;

Efficiency: 4.2 (2.4 + 1.8) days/ha/unit (see Table 4C-8)

Possible operation days per crop season: 67 days (Lag period +
land preparation period +
Overtime work)

Number of required units: $4.2 \text{ days/ha} \times 16 \text{ ha} \div 67 \text{ days} \doteq \text{one unit}$

2. Thresher

One unit of powered thresher and five units of pedal threshers will cover each half area of compact farm as follows;

Powered Thresher

Efficiency: 0.9 days/ha/unit (See Table 4C-8)

Assumed possible operation days per crop season: $35 \text{ days} \times 0.8 \frac{1}{2}$
= 28 days

Number of required units: $0.9 \text{ days/ha} \times 20 \text{ ha} \div 28 \text{ days} \doteq \text{one unit}$

Pedal Thresher

Efficiency: 4.0 days/ha/unit (See Table 4C-8)

Possible operation days per crop season: 28 days

Number of required units: $4.0 \text{ days/ha} \times 20 \div 28 \text{ days} \div 0.6^{2/} = \text{five units}$

Remarks 1/: assumed rate of possible operation days to full operation days

2/: assumed reduction rate of operation efficiency

3. Dryer

One unit of the selected drier has the capacity of drying up 2.0 tons of threshed paddy in moisture content from 26% to 14% and will take eight hours. But it is considered that the traditional way of sunshine-drying will be employed at least in the half stage of drying in moisture content from 26% to 14%. In this case, the dryer has the capacity to dry up 2.0 ton with four hours of the drying time. Then, one unit of the drier is able to cover two compact farms as follows;

Efficiency: $4.2 \text{ ton/ha (unit yield of wet season crop)} \div 2.0 \text{ tons/unit} \times \text{three rotations} = 0.7 \text{ days/ha/unit}$ (See Table 4C-8)

Possible operation days per crop season: $28 \text{ days}^{1/}$

Number of required units: $0.7 \text{ day/ha/unit} \times 40 \text{ ha} \div 28 \text{ days} \div \text{one unit}$

Remarks 1/: 35×0.8 (assumed rate of possible operation days)

4. Working Carabao

The operation of land preparation in non-mechanization area (24 per compact farm ha) will be performed by animal power (carabao or cattle). The required heads of draft animal per compact farm is computed as follows:

Efficiency: Plowing:	7.0 animal-days/ha
1st harrowing:	2.4 animal-days/ha
2nd harrowing:	2.4 animal-days/ha
3rd harrowing:	3.4 animal-days/ha
Total	<u>15.2 animal-days/ha</u>

Estimated possible operation days per crop season = 55 days

Required heads of working carabao: 15.3 animal-days/ha x 24 ha
÷ 67 days ÷ 6 heads

2. Labor Requirement and Machinery Cost

The labor requirements for paddy rice and concerned diversified crops are estimated as follows:

<u>Crop</u>	<u>Labor Requirement per ha.</u>	
	<u>Man-day</u>	<u>Machinery or animal day</u>
Paddy Rice	103.1	20.2
Tobacco	273.5	28.8
Garlic	165.0	22.5
Mung Beans	86.3	22.3
Cotton	139.3	37.3

Remarks: The detailed labor requirements by crop and operation are indicated in Table 4C-9 to 4C-13.

The machinery cost of paddy rice cultivation is calculated at 395 per ha. The cost for the diversified crops is computed at 395 per ha because of no machinery cost for threshing and drying. (See Table 4C-14).

Table 4C-9. Labor Requirement of Paddy Cultivation with Project
in Future

(Unit: day/ha.)

<u>Operation</u>	<u>Man-day</u>	<u>Machinery or</u>		<u>Remarks</u>
		<u>Animal-day</u>		
1. Seed-bedding				
a) Land preparation/Sowing	1.5	0.5		Same as the item 2
b) Care of seedlings	1.5			
Sub-total	(3.0)	(0.5)		
2. Land Preparation				
a) Plowing	5.2	5.2		Animal(60%), Machin-(40%)
b) Harrowing	3.6	3.6		-do-
c) Final harrowing/Leveling	2.6	2.6		Animal(100%)
d) Repair of dikes	2.7			
Sub-total	(14.1)	(11.4)		
3. Transplanting				
a) Pulling/Delivery of seedlings	7.5	0.5		
b) Transplanting	20.0			
Sub-total	(27.5)	(0.5)		
4. Fertilizer Application				
a) Basal fertilizer	1.0	0.2		
b) Top dressing (2x)	1.0	0.2		
Sub-total	(2.0)	(0.4)		
5. Spraying				
a) Insecticides (3x)	3.0			
b) Herbicides (1x)	1.0			
Sub-total	(4.0)			
6. Weeding				
a) 1st weeding (2x)	7.0			
b) 2nd weeding	6.0			
Sub-total	(13.0)			
7. Irrigation/Drainage	5.0			
8. Harvesting				
a) Cutting/Bundling	16.0			
b) Hauling/Piling	3.0	2.0		
c) Threshing	8.5	2.6		Powered Thresher(50%) & Pedal Thresher (50%)
Sub-total	(27.5)	(4.6)		
9. Post Harvesting				
a) Drying	3.5	2.5		Drier(50%) & sunshine (50%)
b) Sacking	2.0			
c) Piling/Delivery	1.5	0.3		
sub-total	(7.0)	(2.8)		
<u>Total</u>	<u>103.1</u>	<u>20.2</u>		

Table 4C-10. Labor Requirement of Tobacco Cultivation with Project in Future

<u>Operation</u>	<u>Man-day</u>	<u>Machinery or</u>		<u>Remarks</u>
		<u>Animal-day</u>		
1. Seed-bedding				
a) Land preparation/Sowing	4.0	0.5		
b) Care of seedling	22.2			
Sub-total	(26.2)	(0.5)		
2. Land Preparation				
a) Plowing	9.4	9.4		Animal(60%), Machine(40%)
b) Harrowing	4.2	4.2		-do-
c) Leveling/Furrowing	2.7	2.7		Animal(100%)
Sub-total	(16.3)	(16.3)		
3. Transplanting				
a) Preparing of seedlings	5.0			
b) Transplanting/Replanting	30.0			
Sub-total	35.0			
4. Fertilizer Application				
a) Basal fertilizer	4.0			
b) Side-dressing	4.0			
Sub-total	8.0			
5. Weeding/Hilling-up	8.8	8.0		Animal(100%)
6. Mowing (8x)	8.0			
7. Irrigation (5x)	10.0			
8. Topping/Sucker control	10.0			
9. Harvesting (8x)				
a) Printing/Hauling	60.0	4.0		Animal(100%)
b) Sorting/Sticking	40.0			
Sub-total	(100.0)	(4.0)		
10. Post harvesting				
a) Curing	30.0			
b) Grading/Bundling	20.0			
c) Packing/Storing	2.0			
Sub-total	(52.0)			
<u>Total</u>	<u>273.5</u>	<u>28.8</u>		

Table 4C-11. Labor Requirement of Garlic Cultivation with Project in Future

<u>Operation</u>	<u>Man-day</u>	<u>Machinery or Animal-day</u>	<u>Remarks</u>
1. Land Preparation			
a) Plowing	9.4	9.4	Animal(60%), Machinery(40%)
b) Harrowing	4.2	4.2	-do-
c) Leveling/Furrowing	2.7	2.7	Animal(100%)
d) Mulching	8.7	4.0	Animal(100%)
Sub-total	(25.0)	(20.3)	
2. Planting			
a) Preparing of seeds	10.0		
b) Planting	35.0		
Sub-total	45.0		
3. Fertilizer Application	2.0	0.2	Animal(100%)
4. Thinning	2.0		
5. Spraying (3x)	8.0		
6. Irrigation (4x)	8.0		
7. Harvesting			
a) Pulling/Gathering	23.0		
b) Hauling	2.0	2.0	Animal(100%)
Sub-total	(25.0)	(2.0)	
8. Post Harvesting			
a) Drying	5.0		
b) Cleaning/Cutting of roots	15.0		
c) Classifying/Grading	10.0		
d) Bundling/Trimming	10.0		
Sub-total	(40.0)		
9. Removal of hay (rice straw)	10.0		
<u>Total</u>	<u>165.0</u>	<u>22.5</u>	

Table 4C-12. Labor Requirement of Mungbeans Cultivation with Project
in Future

<u>Operation</u>	<u>Man-day</u>	Machinery or <u>Animal-day</u>	<u>Remarks</u>
1. Land Preparation			
a) Plowing	9.4	9.4	Animal(60%), Machine(40%)
b) Harrowing	4.2	4.2	-do-
c) Leveling/Furrowing	2.7	2.7	Animal(100%)
Sub-total	(16.3)	(16.3)	
2. Planting			
a) Seeding/Covering	4.0		
b) Thinning	1.0		
Sub-total	(5.0)		
3. Fertilizing	2.0		
4. Cultivation/Weeding			
a) Off-baring	2.0	2.0	Animal(100%)
b) Hilling-up	2.0	2.0	Animal(100%)
c) Weeding	10.0		
Sub-total	(14.0)	(4.0)	
5. Spraying (3x)	4.0		
6. Irrigation (3x)	6.0		
7. Harvesting			
a) Picking (4x)	25.0		
b) Hauling	2.0	2.0	Animal(100%)
8. Threshing/Drying/Others	12.0		
<u>Total</u>	<u>86.3</u>	<u>22.3</u>	

Table 4C-13. Labor Requirement of Cotton Cultivation with Project in Future

<u>Operation</u>	<u>Man-day</u>	<u>Machinery or Animal-day</u>	<u>Remarks</u>
1. Land Preparation			
a) Plowing	9.4	9.4	Animal(60%), Machine(40%)
b) Harrowing	4.2	4.2	-do-
c) Leveling/Furrowing	2.7	2.7	Animal(100%)
Sub-total	(16.3)	(16.3)	
2. Planting/Thinning			
a) Seeding/Covering	20.0		
b) Re-planting	2.0		
c) Thinning	1.0		
Sub-total	(23.0)		
3. Fertilizing			
a) Basal fertilizer	4.0		
b) Side dressing	4.0		
Sub-total	(8.0)		
4. Cultivation			
a) Off-baring/Hilling-up	18.0	18.0	Animal(100%)
5. Spraying (10x)	20.0		
6. Irrigation (5x)	10.0		
7. Harvesting/Drying			
a) Picking (4x)	30.0		
b) Hauling	3.0	3.0	Animal(100%)
c) Cleaning	3.0		
d) Drying	4.0		
Sub-total	(40.0)		
8. Packaging/Delivering	4.0		
<u>Total</u>	<u>139.3</u>	<u>37.3</u>	

Table 4C-14. Farm Machinery Cost

1. Fixed Cost

Machinery	Purchasing Price (₱)	Durable Period (Year)	Depreciation Cost ^{1/} (₱/year)	Repair Cost (₱/year)	Other Fixed Cost ^{2/} (₱/year)	Total Cost (₱/year)	Coverage per unit (ha)	Total cost per hectare (₱)
Hand tractor	23,000	5	4,140	1,840 (8%)	230	6,210	Wet 16 Dry 19	177
Thresher	19,425 ^{3/}	8	2,185	583 (3%)	194	2,962	Wet 20 Dry 12	93
Pedal thresher	650	6	98	-	7	105	Wet 4 Dry 0	26
Dryer	8,250	8	928	412 (5%)	83	1,423	Wet 40 Dry 12	27
<u>Total</u>								<u>323</u>

Note: 1/ Computed as $(1) \times 0.9 \div (2)$

2/ Computed as $(1) \times 0.01$

3/ Price without engine because the engine of hand tractor can be used for thresher.

2. Variable Cost

<u>Operation</u>	<u>Machinery</u>	(1) Ope. hours per ha (hr/ha)	(2) Fuel Con- sumption (ℓ/hr)	(3) Fuel (ℓ)	(4) Unit Cost (₱/ℓ)	(5) Cost of Fuel (₱)	(6)=(5)x1.3 Cost in- clusive of lubricant (₱)
Land preparation for seedbed	Hand tractor with rotary	1.7	D 2.0	3.4	2.4	8.2	10.7
Land preparation	Hand tractor with rotary	33.2	D 2.0	66.4	2.4	159.4	207.2
Threshing	Powered thresher	5.6	D 2.0	11.2	2.4	26.9	35.0
Drying	Drier	10.5	G+L 0.75 K 1.5	G+W 7.9 K 15.8	4.3 2.4	34.0 37.9	44.2 49.3
<u>Total</u>							<u>346.4</u>

Note: G: Gasoline, L: Lubricant Oil, K: Kerosin

3. Machinery Cost per Hectare

Paddy : ₱323 (fixed cost) + ₱346 (variable cost) = 669
 Diversified Crops: ₱177 (fixed cost) + ₱218 (variable cost) = ₱395

FIGURE 4C-8. FARM LABOR BALANCE
(WITH PROJECT, IN FUTURE)

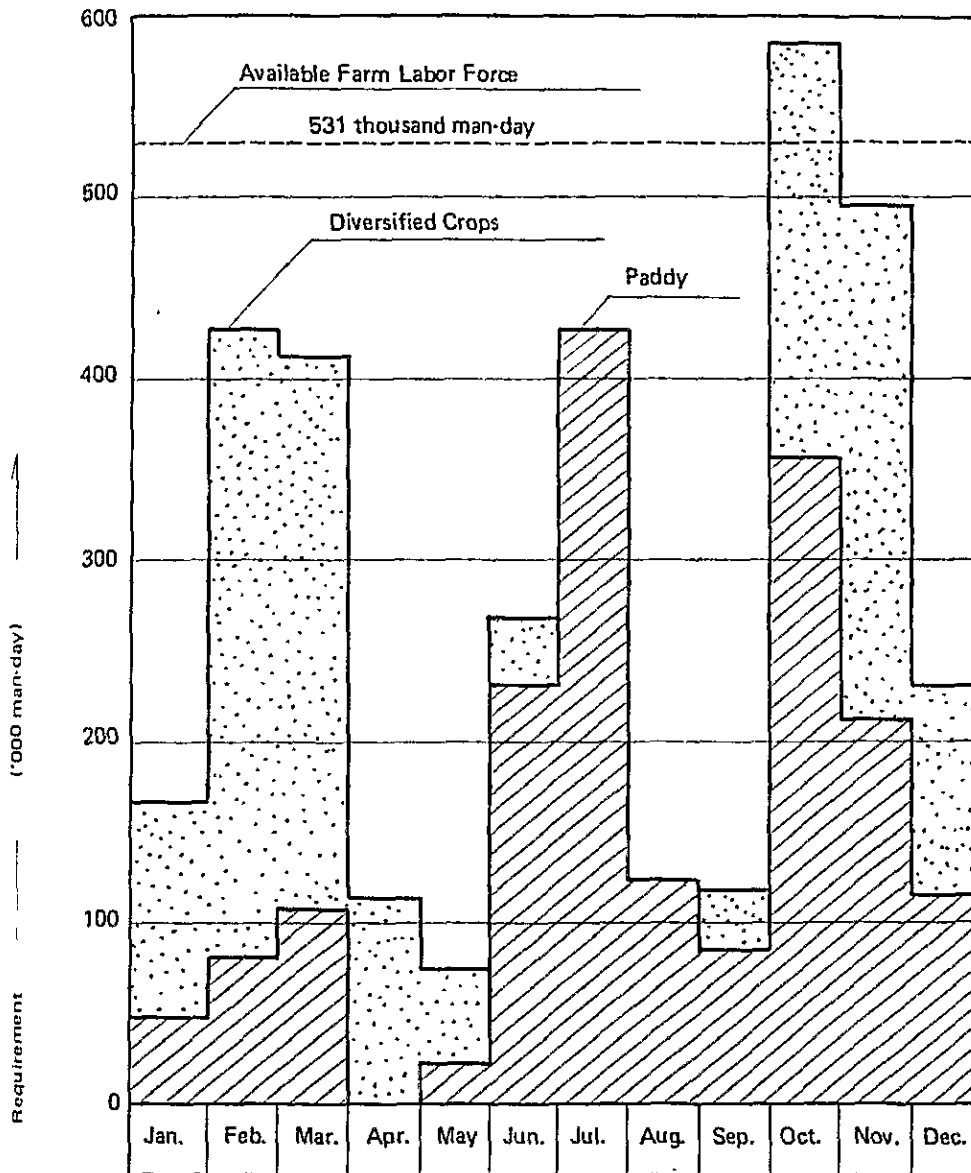


Table 4C-15. Agricultural Development Cost

Items	Amount	Unit Price (P)	Cost (P '000)	Remarks
1. Preparation of cadastral maps				
(a) Production of cadastral maps	2,440 ha	32	78	Siniot(1,770ha), Nueva Era(670ha) only
(b) Preparation of cadastres (sub-total)	12,400.ha	22	272 (350)	Whole service area
2. Arrangement for establishment of FIAs	106	1,400	150	
3. Construction of FAI Offices	106	7,200	770	
4. Extension of FIA Federation offices	1	20,000	20	
5. Others			150	
Total			<u>1,440</u>	
6. Operation Cost				
(a) FIA Offices	106	14,500/year	2,690	P 1,540,000/year x 1.75(1st to 3rd year)
(b) FIA Federation Offices	lump sum		210	P 70,000/year x 3 years
Total			<u>2,900</u>	
Ground Total			<u>4,340</u>	

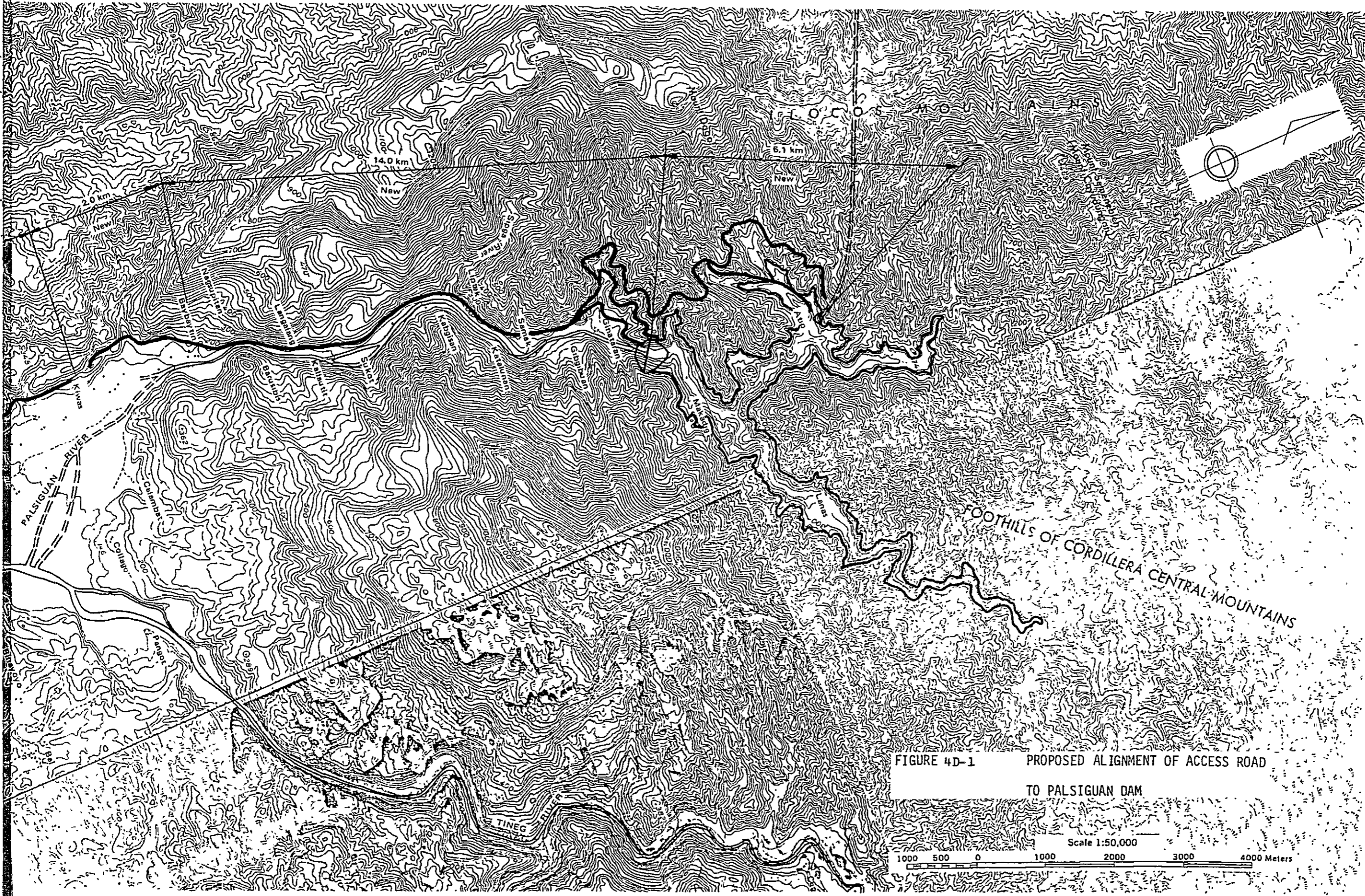


FIGURE 4D-1 PROPOSED ALIGNMENT OF ACCESS ROAD TO PALSIGUAN DAM

Scale 1:50,000
1000 500 0 1000 2000 3000 4000 Meters

Table 4D-1 Dam Type Comparison Chart for Palsiguan Dam

Dam Type	Earth and Rock Fill	Concrete Gravity	Concrete Arch
1) Dam height (m)		140 ~ 145	
2) Dam volume (cu.m)	8,900,000	2,100,000 ^{1/}	500,000 ^{2/} - 600,000 ^{3/}
3) -ditto- (ratio)	100	24	6 ~ 7
4) Unit cost ratio for Dam construction	100	600	1,500
5) Cost index 3) x 4)	10,000	14,400	9,000 ~ 10,500
6) Required compressive strength of bed rock (kg/sq.cm)	250 ~	350 ~	600 ~
7) Present compressive strength of bed rock (kg/sq.cm)	good	300 ~ 600 (average 418)	impossible
8) Judgement 6) < 7)		need more excavation	
9) Required elastic wave velocity of bed rock (km/sec)	1 ~	3 ~	4 ~
10) Present elastic wave velocity of bed rock (km/sec)		2.0 ~ 2.5 ~ 3.4	
11) Judgement 9) < 10)	good	need more excavation	impossible
12) Construction material	earth and rock; exist	gravel; exist	gravel; exist
13) Concrete mixing plant	small	huge	big
Final judgement	suitable	not economical	impossible

Note 1/ estimated at normal condition,

2/ -ditto- ,Dome type

3/ -ditto- ,Constant angle type

Table 4D-2 Probable Earthquake Accelerations at Palsiguan Damsite

Ranking	Date	Epicenter		Inten- sity (m)	Distance from Palsiguan Dam (km)	Acceleration	
		Latitude	Longitude			(Gal) ^{1/}	Average
4	3-19-31	18.3	120.2	6.9	72.9	121.8	112.9
		18.0	121.5	6.9	83.7	104.0	
7	10-28-31	17.5	121.5	6.25	98.1	84.1	84.1
12	1-14-32	18.3	120.2	6.5	72.9	85.9	66.0
		18.0	120.0	6.0	77.5	44.1	
		18.5	120.25	6.5	86.0	68.1	
14	2-14-34	17.5	119.0	7.6	186.4	51.7	51.7
3	3-16-37	18.0	121.0	6.5	33.0	174.9	127.0
		18.0	120.0	6.5	77.5	79.2	
6	3-23-38	18.2	119.7	7.0	113.9	73.8	85.3
		18.0	119.5	7.0	129.8	59.1	
		18.0	120.0	7.0	77.5	123.1	
1	12-29-49	17.5	121.5	7.2	92.2	116.8	183.5
		18.0	121.0	7.2	33.0	250.3	
10	1-3-50	18.0	121.5	6.5	83.7	70.9	70.9
5	6-11-57	18.0	121.5	6.7	83.7	86.9	86.9
16	9-30-62	18.0	121.0	5.0	32.9	45.7	45.7
17	3-14-63	18.3	120.5	5.5	53.3	43.4	43.4
8	8-26-70	18.02	120.5	5.5	30.4	83.9	83.9
15	4-29-71	18.21	120.85	5.3	40.3	50.4	46.2
		18.25	120.72	5.2	42.2	42.0	
9	5-22-72	17.7	120.7	5.0	19.1	75.5	75.5
2	2-29-72	17.8	120.6	5.5	14.9	131.3	131.3
13	3-6-73	18.1	120.71	5.1	27.0	62.9	62.9
11	9-21-76	17.98	120.82	5.1	16.2	91.4	66.5
		17.95	120.83	4.2	14.2	41.6	

^{1/} Okamoto's Formula: $\text{Log}_{10} \frac{\text{Galmax}}{640} = \frac{D + 40}{100} (-7.604 + 1.7244M - 0.103M^2)$

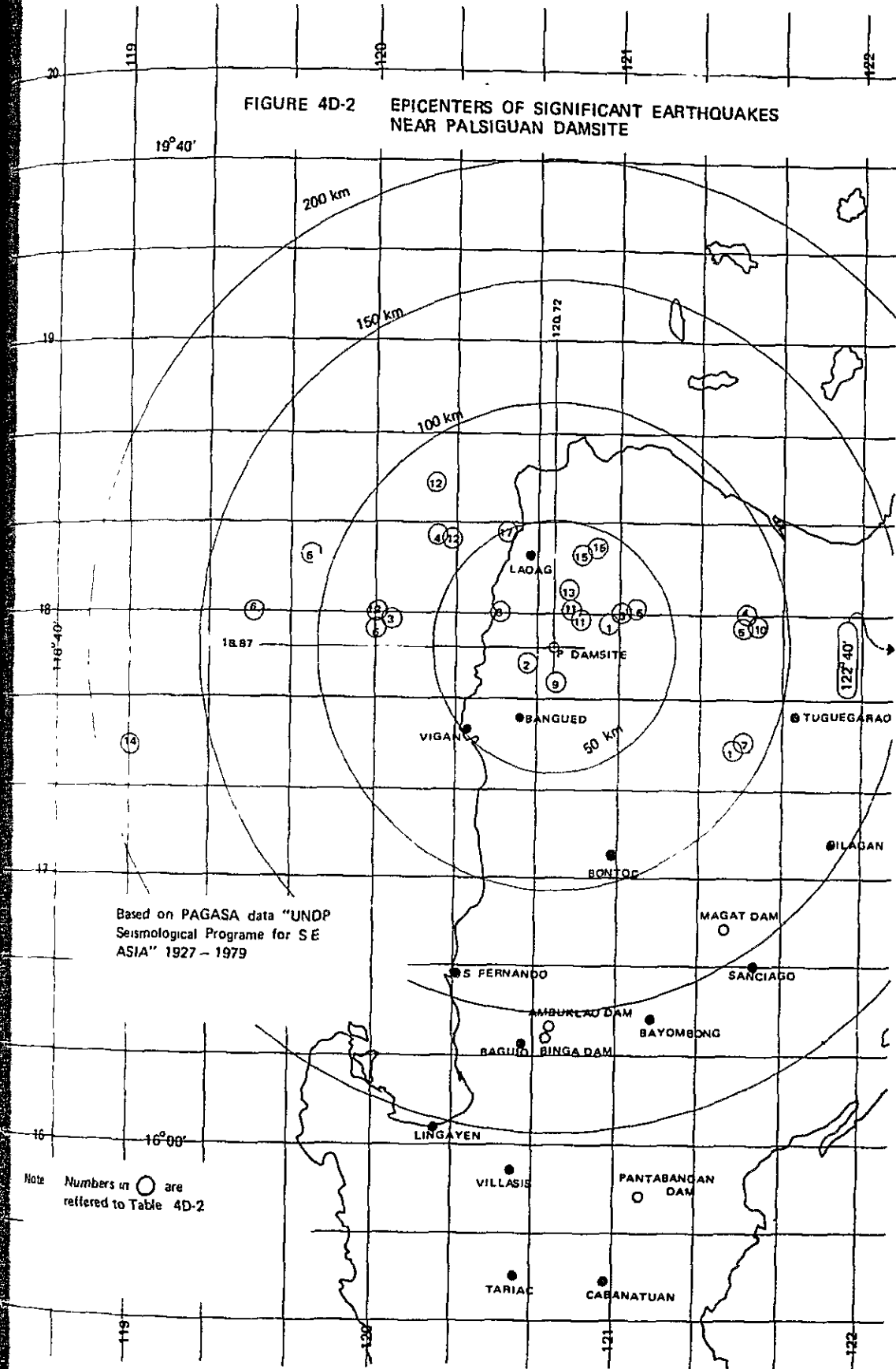
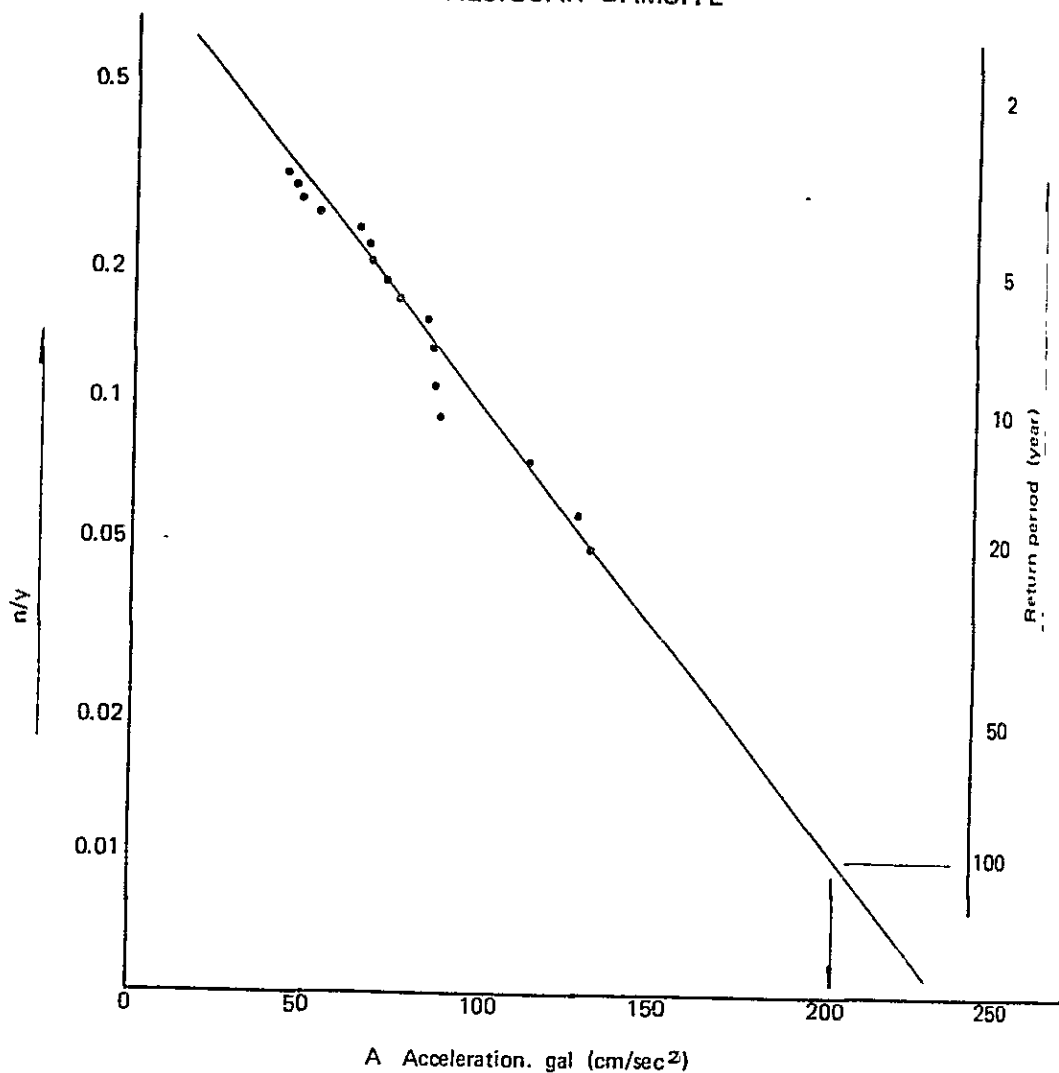


FIGURE 4D-3 PROBABLE EARTHQUAKE ACCELERATION CURVE AT PALSIGUAN DAMSITE



Based on PAGASA data "UNDP Seismological Programme for Southeast ASIA" 1927 - 1979, and Okamoto's Formula;

$$\log_{10} \frac{\text{gal max}}{640} = \frac{D + 40}{100} (-7.604 + 1.7224M - 0.1036 M^2)$$

- gal: Acceleration at the damsite
- D: Distance between damsite and epicenter
- M: Magnitude at epicenter
- n: Chronological order according to acceleration
- y: Number of years of records = 53

Seismic forth, $K = \text{gal}/980$

Design of Dam and Appurtenant Structures

1. Freeboard and Crest Elevation of Dam

Freeboard is the difference between the crest elevation and the full water surface level in a reservoir and is shown in the following equation in consideration of various factors according to the Design Criteria for Dams which was published (and revised on July 1978) by Japanese National Committee on Large Dams.

$$H_f \geq h_w + h_e + h_a + h_i$$

- where,
- H_f: freeboard of dam (m)
 - h_w: height of wave due to wind (m)
 - h_e: height of wave due to earthquake (m)
 - h_a: rise of water level due to unexpected accident in operating spillway gates, standard value h_a=0.5 m is adopted
 - h_i: addition of allowance according to type and importance of dam, standard value h_i=1.0 m is adopted for fill type and zero for concrete type.

1.1 Height of Wave due to Wind

Height of wave due to the wind is considered to be caused by deep water wave, and then, the height of significant wave is adopted based on S.M.B. (Sherdrup-Munk-Breschneider) method which is derived from factors such as fetch and wind speed. On the other hand, since uprushing height varies considerably with embankment slope and roughness of slope, height of significant wave should be adjusted adequately with Saville method to obtain height of wave due to the wind with consideration of uprushing height as well.

The calculation results with various slope, fetch and wind speed are shown in the Figures 4D-4.

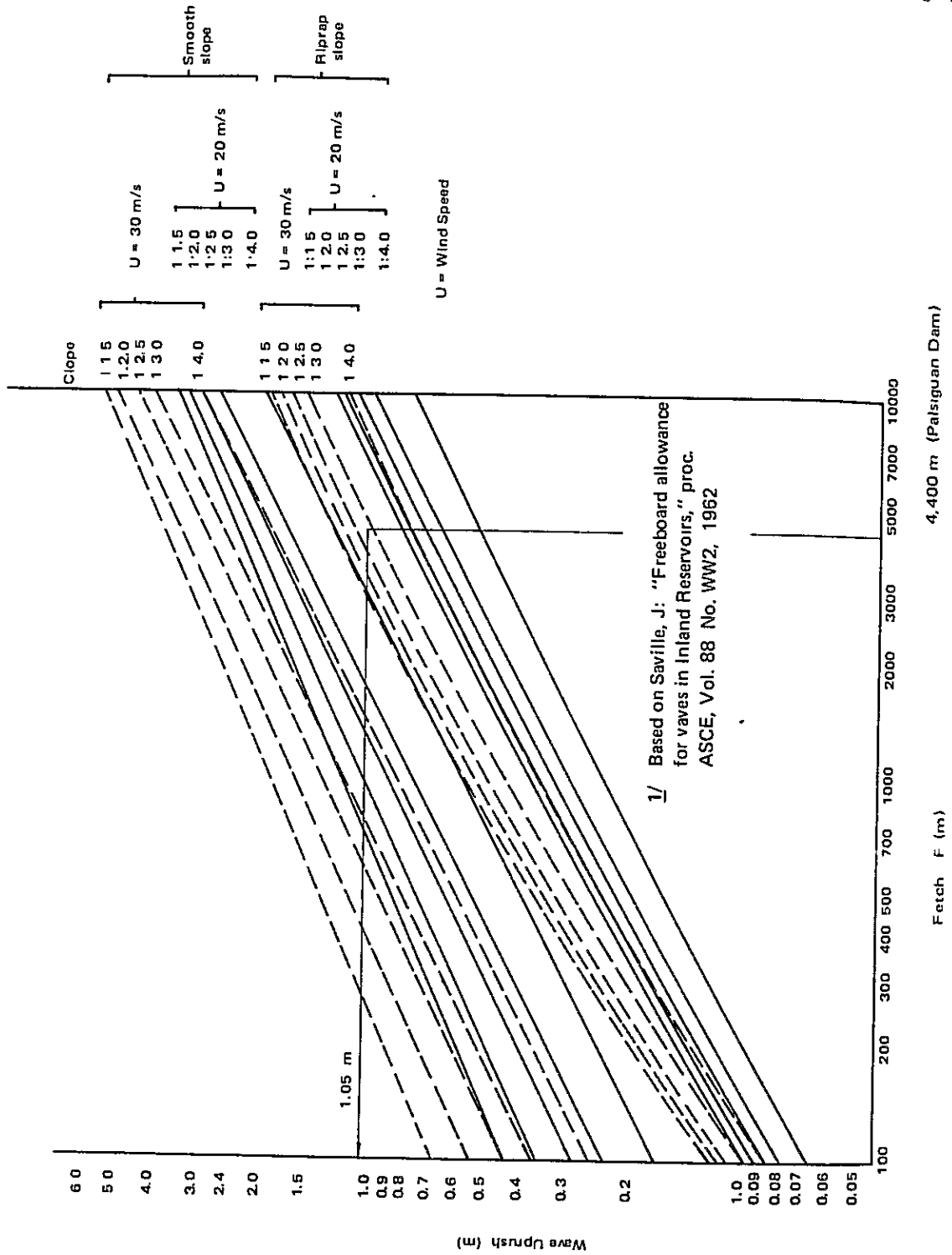


FIGURE 4D.4 WAVE UPRUSH ONTO THE SLOPE 1/

In order to obtain the height of wave due to wind in the Palsiguan damsite, the wind speed of 30 meters per second in 10 minutes on an average is to be adopted taking into account the observed data of maximum instantaneous wind speed in Laoag (observed maximum value is 40 m/sec in July 1965) and the following conditions.

- The maximum instantaneous wind speed does not last for the blow time (usually 10 minutes) which is required for bringing about the wave due to wind.
- In many case, the wind direction does not accord with the maximum fetch direction.
- The wind speed in the Palsiguan damsite decrease by the topographical and vegetative conditions.

c) Height of Wave due to Earthquake

The height of wave due to earthquake can be obtained by Sato's formula, as follows;

$$h_e = \frac{K\tau}{2\pi} \cdot \sqrt{g \cdot H_o}$$

he: height of wave at upstream face of the dam due to the earthquake.

K : horizontal seismic coefficient. (K=0.2)

τ : period of seismic wave in second. (usually, τ is 1.0 second adopted)

g : gravitational acceleration ($g=9.8 \text{ m/sec}^2$)

H_o : depth of reservoir water (m)

c) Freeboard

Estimated freeboard of the Palsiguan dam is shown in the following table. The upstream surface of Palsiguan dam is formed with rockzone by the materials obtained from quarry site, therefore, the "hipran slope" was adopted as the height of wave due to wind.

$\frac{F}{(m)}$	$\frac{hw}{(m)}$	$\frac{Ho}{(m)}$	$\frac{he}{(m)}$	$\frac{ha}{(m)}$	$\frac{hi}{(m)}$	$\frac{Hf}{(m)}$
4,400	1.05	130	1.15	0.50	1.00	$3.70 \leq 4.00$

d) Dam Crest Elevation

The Palsiguan dam will be constructed not only for irrigation but also for hydro-electric power, and its corresponding water surface elevation are tabulated as follows;

<u>Water level</u>	<u>Storage capacity</u> (x 10 ³ cu.m)	<u>Water surface elevation</u> (EL m)	<u>Area of water surface</u> (x 10 ³ sq.m)
Full water	232,000	334.5	5,040
Dead water	43,000	275.0	1,560

From the above table, the Palsiguan dam crest elevation without extra bank can be obtained by adding the freeboard to full water surface;

$$\text{Dam crest elevation EL. } 334.50 + 4.00 = \text{EL. } 338.50 \text{ m}$$

2. Surface slope stability

In case that the dam body will be constructed by the material with less cohesion, the critical slip circle approaches to the surface of dam body. In this case, the factor of safety $\frac{1}{F}$ can be obtained from the following formula;

$$\text{For upstream slope} \quad F.S = \frac{(1 - K \cdot \frac{\gamma_{sat}}{\gamma_{sub}} \cdot \tan \alpha)}{K \cdot \frac{\gamma_{sat}}{\gamma_{sub}} + \tan \alpha} \cdot \tan \phi$$

$$\text{For downstream slope} \quad F.S = \frac{1 - K \cdot \tan \alpha}{K + \tan \alpha} \cdot \tan \phi$$

where, F.S : factor of safety
 K : seismic coefficient (K=0.2 see Figure 4D-3, Appendix 4D-3)
 γ_{sat} : saturated density of outer shell material ($\gamma_{sat}=2.21$ ton/cu.m ^{2/})
 γ_{sub} : submerged density of outer shell material ($\gamma_{sub}=\gamma_{sat} - 1= 1.21$ ton/cu.m)
 α : tangential value of slope
 ϕ : angle of internal friction of outer shell material ($\phi=45$, see Appendix 3B-4)

1/ The factor of safety is in conformity with the Design for Dams established by Japanese National Committee on Large Dams is to say, it must not be less than 1.2 in any case.

2/
$$\gamma_{sat} = \frac{G_s + e}{1 + e} = 2.21$$

where, G_s (Specific gravity of the rock material) = 2.7 reduced as 10% allowance from the Test result; 2.95, See Appendix 3B-4.

e (Void ratio of the rock embankment) = 0.4

3. Spillway

a) Design Flood Discharge

The peak discharge (inflow) and design flood discharge of the Haisiguan dam are shown in the following table.

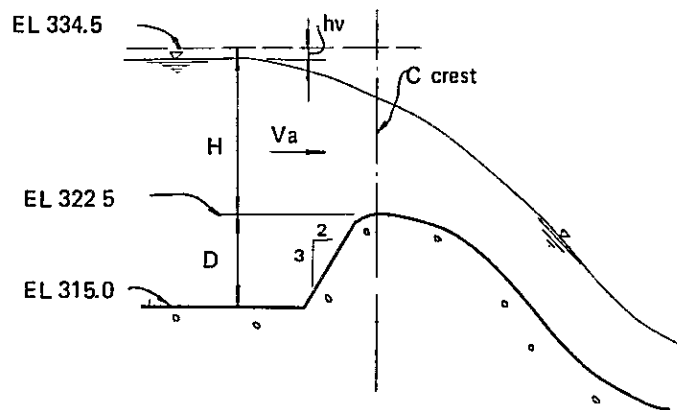
<u>Dam type</u>	<u>Catchment area</u> (sq.km)	<u>Max. Peak Discharge</u> (cu.m/sec)	<u>Design Flood Discharge</u> (cu.m/sec)
Earth and rockfill	153	3,070	3,070

Note; (See Chapter III.B.2)

b) Overflow Head and Width

In general, open type spillway should be adopted to the fill type dam from view points of nonresistance against overtopping from unexpected flood and hydraulic characteristic of itself. It is considered that the gate type spillway is more suitable to be adopted

than the ungated spillway from the view points of design discharge and topographical feature. In the gated spillway, at least two gates or more should be provided for the purpose of diversification of risk by gate control.



The discharge coefficient of complete overflow on the weir has a close relation with a shape of weir. On assumption that D/H is 0.51, the most effective upstream slope of weir is 3 vertical to 2 horizontal and coefficient of discharge is 2.14. Length of weir, e.g. width of spillway can be obtained from the following equation, considering the contraction by piers.

$$L = \frac{Qd}{CH^{3/2}} + N \cdot b + 2N \cdot K_p \cdot H = \frac{Qd}{2.14 \cdot H^{3/2}} + (b + 2K_p \cdot H)N$$

where, L : length of weir (width of spillway) (m)
H : overflow head (m)
b : width of pier (b=2.50m, place for lifting unit)
Kp : coefficient of contraction on pier.
(Kp=0 at the design discharge)
N : number of pier

The length of weir for various overflow heads in case of the Palsiguan dam spillway is estimated in the following table.

Overflow head (m)	Number of pier	$\frac{Qd}{CH^{3/2}} + 2N.Kp.H$ (m)	Gate size (m)	Length of weir (m)
H = 11.0	N = 2	39.3	3 - ^(a) 11.5 x ^(b) 13.1	44.3
H = 12.0	N = 2'	34.5	3' - 12.5 x 11.5	39.5
H = 13.0	N = 2	30.6	3 - 13.5 x 10.2	35.6

Note: (a) height of gate leaf
(b) width of gate leaf

c) Hydraulic Dimensions

The water depth on chute section for the Palsiguan dam is shown in the following table.

Elevation (EL m)	Water Depth ^{1/} d (m)	Velocity V(m/sec)	Froude Number Fr
322.50	8.0	11.1	1.3
300.00	3.2	24.5	4.4
275.00	2.5	31.4	6.4
250.00	2.2	35.1	7.5
230.00	2.2	35.9	7.8

^{1/} calculated by Manning's roughness : n=0.014

From the flip-bucket which is installed at EL 230 m ~ 235 m. The water jet shall fly through following points.

Distance from the Flip (m)	Elevation ^{1/} (EL-m)
0	235.0
20	244.3
40	249.0
60	249.1
80	244.7
100	235.7
140	204.0 = river bed

^{1/} calculated by energy loss of 10%

4. Diversion Works

The tunnel type diversion facilities are planned for the Palsiguan dam due to the topographical condition and the diversion facilities will be used for the outlet facilities from the reservoir after completion of the dam.

a) Design Flood Discharge

The design flood discharge for diversion facilities is as follows.

<u>Dam type</u>	<u>Catchment Area</u> (sq.km)	<u>Design Flood Discharge</u> <u>for Diversion Facilities</u> (cu.m/sec)
Earth and rockfill	153	950

The open flow type tunnel has been proposed so that the dam construction will not hinder the transportation of pulpwood by the Palsiguan river flow.

Necessary tunnel sizes according to the ration of water dept. to tunnel diameter is as follow;

<u>Depth/Diameter</u>	<u>Tunnel Diameter</u> ^{1/} (m)	<u>Clearance</u> (m)	<u>Velocity</u> ^{1/} (m/sec)
0.70	10.16	3.05	14.6
0.75	9.86	2.47	14.5
0.80	9.62	1.92	14.3
0.85	9.45	1.42	14.1
0.90	9.34	0.93	13.8

1/ conditions : Q = 950 cu.m/sec
Slope = 7/740
Manning's n = 0.014

Sizes of pulpwood, which is measured at Baybaytin on February 4th, 1980 are varied as follows;

	<u>Normal</u>	<u>Longest</u>	<u>Thickest</u>
	(cm)	(cm)	(cm)
Diameter	40	25	85
Length	150	180	145

In order to keep the pulpwood transportation, the tunnel clearance should be nearly two meters, so the tunnel diameter of 9.6 meters is adopted with the following hydraulic dimensions:

<u>Tunnel Diameter</u>	<u>Slope</u>	<u>Manning's Roughness</u>	<u>Depth</u>	<u>Velocity</u>
(m)			(m)	(m/sec)
9.60	7/740	0.014	7.74	14.3

b) Crest Elevation of Cofferdam

The necessary elevation of coffer dam can be obtained from the necessary head for tunnel flow as shown belows;

<u>Velocity</u>	<u>Velocity Head</u>	<u>Necessary Head</u>
(m/sec)	(m)	at Tunnel Front ^{1/}
		(m)
14.3	10.4	15.6

<u>Necessary Elevation</u>	<u>Free board</u>	<u>Top of Cofferdam</u>
at Tunnel Front ^{2/}	(m)	(EL-m)
(m)		
222.6	2.4	225.0

^{1/} 150% of tunnel velocity head

^{2/} Invert of tunnel entrance = EL 207.0 m

Table 4D-3 Stability Analysis of Gravity Dam

Downstream Slope M=0.88
 Upstream Slope N1=0.0
 Distance from Crest to Beginning Point of Fillet HF=27.00
 Slope of Fillet N2=0.20

Condition

Full water Earthquake, K=0.15
 KA=1 KB=1 KC=1 K=0.150

X	T	SN	SH	SM	SU	SD	N
10.500	9.240	130.094	-79.727	-106.067	6.626	21.533	22.493
15.500	13.640	246.454	-185.164	-369.060	6.167	29.970	14.591
20.500	18.040	410.114	-338.827	-984.760	4.578	40.889	10.794
25.500	22.440	621.074	-540.272	-2009.042	2.785	52.569	8.626
30.500	27.540	904.081	-769.643	-3901.125	2.758	62.898	7.423
35.500	32.940	1258.380	-1087.983	-6194.941	3.946	72.459	6.606
40.500	38.340	1675.730	-1435.213	-9377.719	5.430	81.985	5.976
45.500	43.740	2156.131	-1831.206	-13459.426	7.084	91.505	5.477

Full water Earthquake, K=0.0
 KA=1 KB=1 KC=2 K=0.0

X	T	SN	SH	SM	SU	SD	N
10.500	9.240	130.094	-42.821	48.649	17.498	10.661	41.879
15.500	13.640	246.454	-113.771	51.809	19.739	16.398	23.746
20.500	18.040	410.114	-222.221	-98.253	20.922	24.545	16.458
25.500	22.440	621.074	-368.170	-484.978	21.898	33.456	12.638
30.500	27.540	904.081	-551.620	-1176.438	23.521	42.135	10.626
35.500	32.940	1258.380	-772.570	-2191.448	26.084	50.320	9.303
40.500	38.340	1675.730	-1031.020	-3579.922	29.095	58.319	8.319
45.500	43.740	2156.131	-1326.970	-5395.230	32.374	66.214	7.558

KA=1 KB=2 KC=1 K=0.075

X	T	SN	SH	SM	SU	SD	N
10.500	9.240	143.911	-20.100	144.904	25.758	5.392	89.904
15.500	13.640	262.779	-77.231	324.624	29.734	8.796	35.193
20.500	18.040	428.947	-177.599	409.282	31.323	16.232	20.699
25.500	22.440	642.415	-320.767	287.998	32.060	25.197	14.595
30.500	27.540	926.333	-506.730	-161.443	32.359	34.913	11.611
35.500	32.940	1280.861	-735.972	-979.735	33.467	44.302	9.797
40.500	38.340	1698.439	-1008.431	-2235.391	35.175	53.424	8.528
45.500	43.740	2179.067	-1324.031	-4010.159	37.242	62.395	7.592

KA=1 KB=2 KC=2 K=0.0

X	T	SN	SH	SM	SU	SD	N
10.500	9.240	143.911	-7.538	199.361	29.585	1.565	239.740
15.500	13.640	262.779	-49.988	476.052	34.518	3.913	54.373
20.500	18.040	428.947	-129.938	745.697	37.526	10.030	28.292
25.500	22.440	642.415	-247.387	924.859	39.648	17.608	18.924
30.500	27.540	926.333	-402.337	917.478	40.894	26.378	14.623
35.500	32.940	1280.861	-594.787	710.729	42.815	34.955	12.122
40.500	38.340	1698.439	-824.737	264.906	45.381	43.218	10.427
45.500	43.740	2179.067	-1092.187	-473.352	48.334	51.303	9.204

Empty Earthquake, K=0.075
 KA=1 KB=3 KC=1 K=0.075

X	T	SN	SH	SM	SU	SD	N
10.500	9.240	150.698	11.302	277.047	35.779	-3.160	160.488
15.500	13.640	285.118	21.384	827.067	47.576	-5.769	128.149
20.500	18.040	471.238	35.343	1789.781	59.119	-6.875	105.210
25.500	22.440	709.058	53.179	3298.316	70.898	-7.703	89.288
30.500	27.540	1001.457	75.109	5174.844	77.301	-4.574	79.333
35.500	32.940	1356.777	101.758	7610.523	83.274	-0.895	71.601
40.500	38.340	1775.547	133.166	10849.355	90.595	2.026	65.157
45.500	43.740	2257.767	169.332	15022.941	98.732	4.504	59.829

Empty Earthquake, K=0.0
 KA=1 KB=3 KC=2 K=0.0

X	T	SN	SH	SM	SU	SD	N
10.500	9.240	150.698	0.0	224.002	32.051	0.567	0.0
15.500	13.640	285.118	0.0	693.921	43.282	-1.475	0.0
20.500	18.040	471.238	0.0	1516.435	54.080	-1.836	0.0
25.500	22.440	709.058	0.0	2805.280	65.024	-1.828	0.0
30.500	27.540	1001.457	0.0	4362.996	70.879	1.849	0.0
35.500	32.940	1356.777	0.0	6358.492	76.380	6.029	0.0
40.500	38.340	1775.547	0.0	9011.992	83.095	9.526	0.0
45.500	43.740	2257.767	0.0	12431.316	90.604	12.632	0.0

IN PUT DATA NO.1

HIGH-WATER-LEVEL	2.000
LOW-WATER-LEVEL	7.700
ELEVATION OF SEDIMENT	7.700
UNIT-WEIGHT OF CONCRETE	2.350
UNIT-WEIGHT OF SEDIMENT	1.000
COEFFICIENT OF SEDIMENT-PRESSURE	0.500
COEFFICIENT OF UP-LIFT	0.200
COEFFICIENT OF SLIDE	1.000
ALLOWABLE SHEARING-STRESS	180.000

Note: X: Dam height (m)
 T: Bottom width of dam (m)
 SN: Vertical force (ton)
 SH: Horizontal force (ton)
 SM: Moment (ton.m)
 SU: Vertical stress at upstream toe (ton/sq.m)
 SD: Vertical stress at downstream toe (ton/sq.m)
 N: Safety factor for sliding shearing friction

3. Diversion Dams

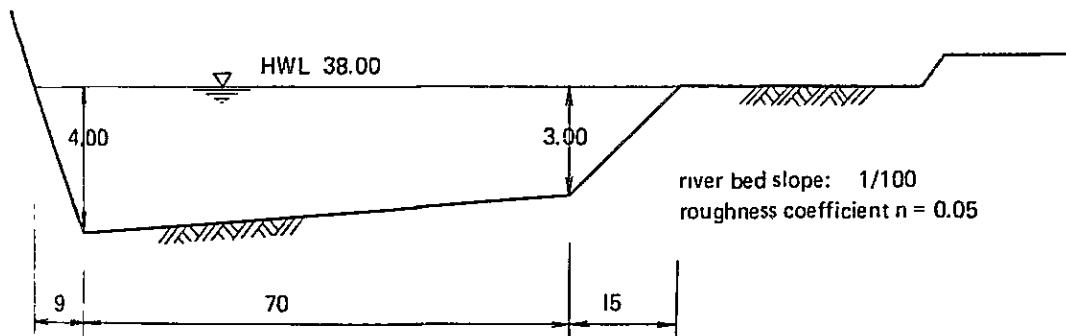
Two diversion dams, Madupayas and Tibangran diversion dams, are proposed in the project. The subsequent paragraph gives the hydraulic calculation for determining major dimensions of the facilities in case of Tibangran diversion dam.

1. Design Conditions

Intake discharge:	$Q = 7.71 \text{ cu.m/sec}$
High-water discharge:	$QF = 950 \text{ cu.m/sec}$
Intake water surface:	NWS. 36.50 m
Dam bed elevation:	EL. 34.15 m
Dam crest elevation:	$NWS. 36.50 + 0.15 = 36.65 \text{ m}$
Weir type:	Floating type

2. Present High-Water Level

Present high-water level can be estimated depending on the present typical cross section of river at the proposed site as shown below:



Now, when high-water level is assumed at WL 38.0 m, total discharge of the section is estimated at 1,188 cu.m/sec, by applying the Manning's formula.

$$A_1 = 4 \times 9 \times 1/2 = 18 \text{ sq.m}$$

$$A_2 = (4 + 3) \times 1/2 \times 70 = 245$$

$$A_3 = 3 \times 15 \times 1/2 = 22.5$$

$$\Sigma A = 285.5 \text{ sq.m}$$

$$P = 9.8 + 70 + 15.3 = 95.1 \text{ m}$$

$$R = 285.5/95.1 = 3.00, R^{2/3} = 2.08$$

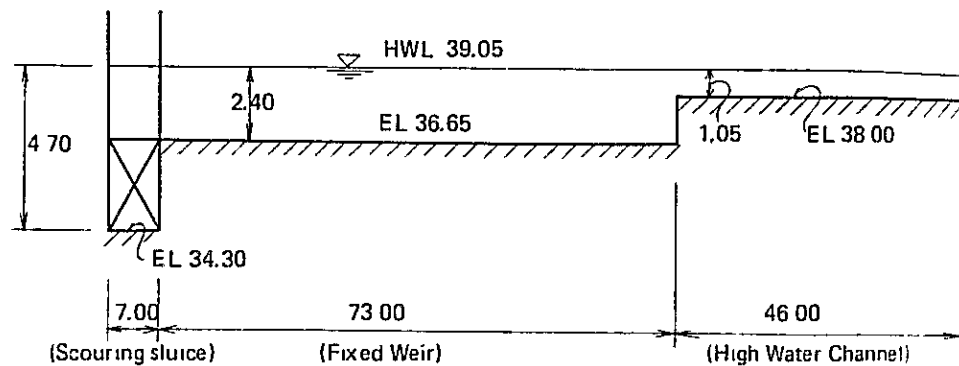
$$V = \frac{1}{0.05} \times 2.08 \times \left(\frac{1}{100}\right)^{1/2} = 4.16 \text{ m/sec}$$

$$Q = 285.5 \times 4.16 = 1,188 > 950 \text{ cu.m/sec}$$

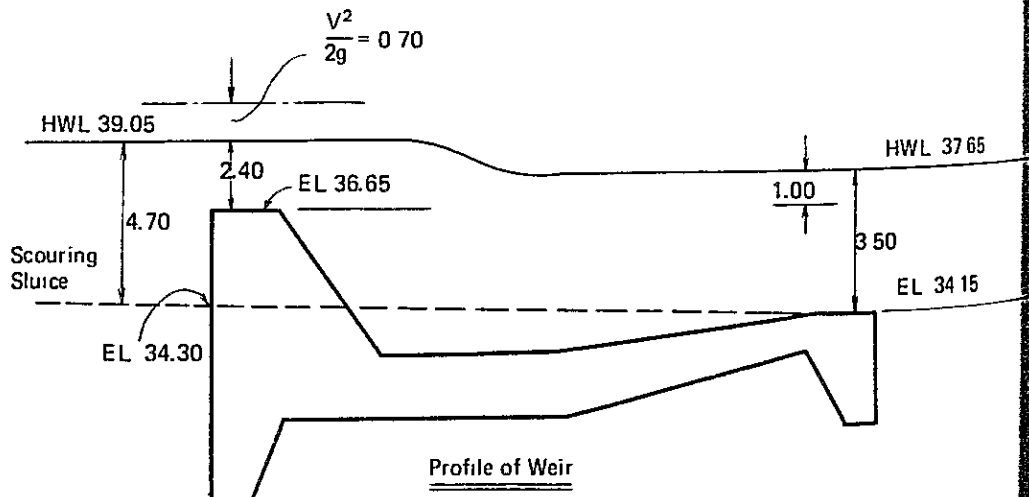
So, high-water level is decided at WL. 38.0 m.

3. Future High Water Level

Future high water level after completion of the diversion dam, can be estimated by using the proposed section of weir.



Cross Section of Weir



Profile of Weir

Approaching velocity:

$$A_1 = 7.00 \times 4.70 = 32.9 \text{ sq.m}$$

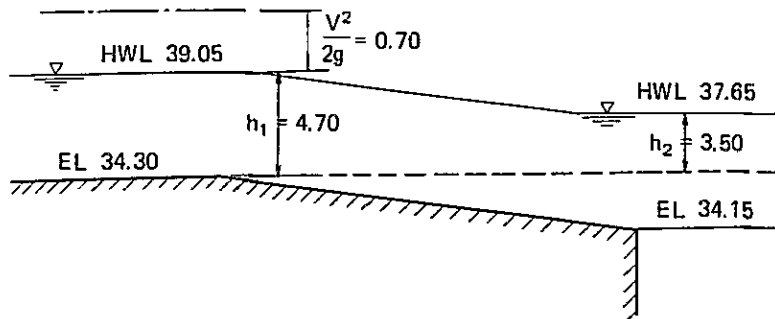
$$A_2 = 73.00 \times 2.40 = 175.2$$

$$A_3 = 46.00 \times 1.05 = 48.3$$

$$\Sigma A = 256.4 \text{ sq.m}$$

$$V = \frac{0.50}{256.4} = 3.71 \text{ m/sec} \quad \frac{V^2}{2g} = \frac{3.71^2}{19.6} = 0.70 \text{ m}$$

Spring Sluice



$$h_2 = 3.50 < \frac{2}{3} h_1 = \frac{2}{3} \times 5.40 = 3.60 \text{ (Complete overflow)}$$

$$Q_m = 1.70 L_m \left(h_1 + \frac{V^2}{2g} \right)^{3/2}$$

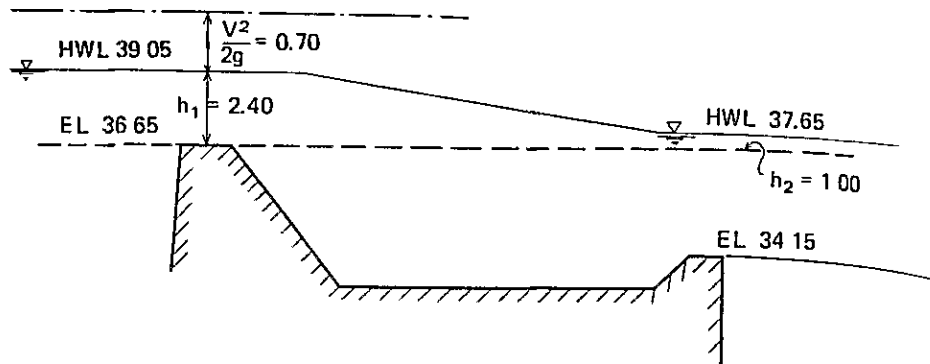
$$= 1.70 \times 6.62 \times (4.70 + 0.70)^{3/2}$$

$$= 141 \text{ cu.m/sec}$$

where; $b_0 = 6 - 0.04 n h_1$

$$= 7.00 - 0.04 \times 2 \times 4.70 = 6.62$$

Fixed Weir

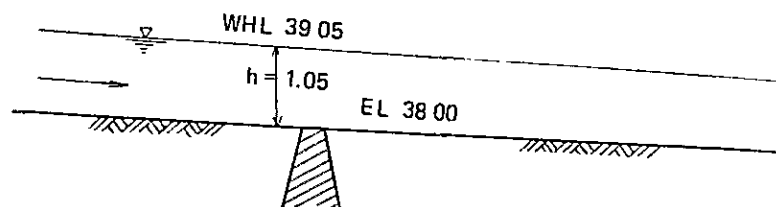


$$h_2 = 1.00 < \frac{2}{3} h_1 = \frac{2}{3} \times 3.10 = 2.07 \text{ (Complete overflow)}$$

$$Q_f = 1.7 l \left(h_1 + \frac{V^2}{2g} \right)^{3/2}$$

$$= 1.7 \times 73.00 \times (2.40 + 0.70)^{3/2} = 677 \text{ cu.m/sec}$$

High-Water Channel



$$Q_{f'} = A.V$$

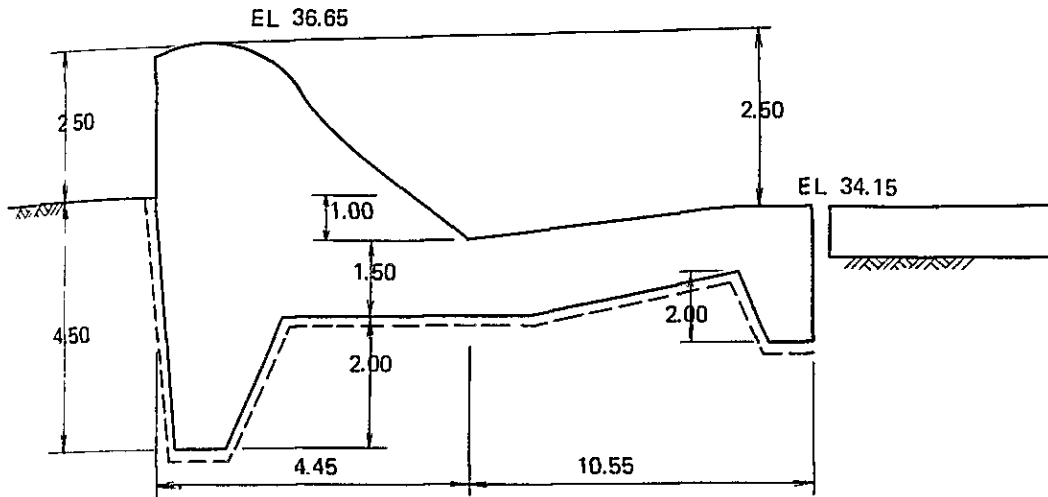
$$= 46.00 \times 1.05 \times 3.71 = 179 \text{ cu.m}$$

Total discharge;

$$\Sigma Q = Q_m + Q_f + Q_{f'} = 141 + 677 + 179 = 997 > 950 \text{ cu.m/sec}$$

Therefore, back water shall be determined at HWL 39.05 m as assumed.

4. Design of Fixed Weir



a) Length of Fore-apron

By Bligh Method;

$$\begin{aligned} Lf_1 &= 0.6 \cdot C \sqrt{D_1} \\ &= 0.6 \times 9 \times \sqrt{2.50} = 8.53 < 10.55 \text{ m} \\ \text{Length} &= 10.55 \text{ m} \end{aligned}$$

b) Slipping (Creep Length)

By Bligh Method;

$$\begin{aligned} S &= C \cdot \Delta h \\ &= 9.0 \times 2.50 = 22.50 \text{ m} \\ S' &= 4.50 + 2.00 + 15.00 + 2.00 = 23.50 \text{ m} \\ S &< S' \quad \text{O.K} \end{aligned}$$

By Lane Method;

$$\begin{aligned} L &= C \cdot \Delta h \\ &= 3.5 \times 2.50 = 8.75 \\ L' &= (4.50 + 2.00 + 2.00) + 1/3 \times 15.00 = 13.50 \text{ m} \\ L &< L' \quad \text{O.K} \end{aligned}$$

c) Thickness of Fore-apron

$$T_a = \frac{4}{3} \times \frac{\Delta h - hf}{r - 1}$$

$$= \frac{4}{3} \times \frac{2.50 - 1.16}{2.3 - 1} = 1.37 < 1.50 \text{ m}$$

$$hf = \frac{\Delta h}{S} \times S' = \frac{2.50}{23.50} \times (4.50 + 2.00 + 4.45) = 1.16 \text{ m}$$

Thickness = 1.50 m

d) Apron Protection Works

By Bligh Method;

$$l = 0.67 C \sqrt{D_1 q}$$

$$l_{fr} = l - l_{F_1}$$

$$l = 0.67 \times 9 \times \sqrt{2.50 \times 9.3} = 46.22 \text{ m}$$

$$\text{where; } q = \frac{677}{73} = 9.3$$

$$l_{fr} = 29.08 - 10.55 = 18.53 \text{ m}$$

Length = 20.00 m

5. Design of Scouring Sluice

a) Elevation

Elevation of scouring sluice shall be determined based on the averaged river bed slope of the stream center line after construction of diversion dam. Accordingly, the elevation was calculated at El 34.17 based on the averaged river bed slope derived from the elevation at the 100 m upstream point and the 150 m lowerstream point of the dam site.

b) Design Discharge for Flushing Sediment

In order to flush away sediment even during irrigation period, the normal water discharge in irrigation periods ($Q_0 = 13.0 \text{ cu.m/second}$) is adopted.

c) Required Water Velocity for Flushing Sediment

$$V_c = 1.5 C \sqrt{d}$$

where; V_c : required velocity (m/sec)

C : coefficient by sand/gravel condition 4.5

d : max. particle size of sediment (m)

$$V_c = 1.5 \times 4.5 \times \sqrt{0.15} = 2.61 \text{ m/sec}$$

d) Width

The width should be determined to secure the required velocity mentioned above to lead the water course and to flow driftwood, etc.

$$L_{mc} = \frac{Q_0}{q}$$

$$q = \frac{V_c^3}{g}$$

where; L_{mc} : width (m)

q : traction discharge per unit width (cu.m/sec/m)

g : acceleration of gravity (m/sq.sec)

Q_0 : design discharge for flushing away sediment (cu.m/sec)

V_c : required water velocity for flushing away sediment (m/sec)

$$q = \frac{2.61^3}{9.8} = 1.81 \text{ cu.m/sec/m}$$

$$L_{mc} = \frac{13}{1.81} = 7.18 \text{ m}$$

$$\text{width} = 7.00 \text{ m} \times 1 \text{ set}$$

e) Slope

The slope is determined to secure the critical velocity in the scouring sluice canal, setting the control point on the upstream end of this canal as follows:

$$I_c \geq \frac{n^2 g}{hc^{1/3}}$$