

Appendix 3.5.5      Base Year O&D Tables



(vehicle)

CO-ORDINATE	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000	1001	1002	1003	1004	1005	1006	1007	1008	1009	1010	1011	1012	1013	1014	1015	1016	1017	1018	1019	1020	1021	1022	1023	1024	1025	1026	1027	1028	1029	1030	1031	1032	1033	1034	1035	1036	1037	1038	1039	1040	1041	1042	1043	1044	1045	1046	1047	1048	1049	1050	1051	1052	1053	1054	1055	1056	1057	1058	1059	1060	1061	1062	1063	1064	1065	1066	1067	1068	1069	1070	1071	1072	1073	1074	1075	1076	1077	1078	1079	1080	1081	1082	1083	1084	1085	1086	1087	1088	1089	1090	1091	1092	1093	1094	1095	1096	1097	1098	1099	1100	1101	1102	1103	1104	1105	1106	1107	1108	1109	1110	1111	1112	1113	1114	1115	1116	1117	1118	1119	1120	1121	1122	1123	1124	1125	1126	1127	1128	1129	1130	1131	1132	1133	1134	1135	1136	1137	1138	1139	1140	1141	1142	1143	1144	1145	1146	1147	1148	1149	1150	1151	1152	1153	1154	1155	1156	1157	1158	1159	1160	1161	1162	1163	1164	1165	1166	1167	1168	1169	1170	1171	1172	1173	1174	1175	1176	1177	1178	1179	1180	1181	1182	1183	1184	1185	1186	1187	1188	1189	1190	1191	1192	1193	1194	1195	1196	1197	1198	1199	1200	1201	1202	1203	1204	1205	1206	1207	1208	1209	1210	1211	1212	1213	1214	1215	1216	1217	1218	1219	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### Evening Peak

[illegible]

off peak

[illegible]

# Hourly O&D Tables in Base Year (Taxi)

Morning Peak

(Vehicle)	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	COUNT
1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
4	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
5	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
6	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
7	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
8	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
9	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
10	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
11	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
12	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
13	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
14	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
15	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
16	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
17	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
18	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
19	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
20	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
21	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
22	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
23	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
24	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
25	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
26	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
27	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1

Evening Peak

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	COUNT													
1	1020	1120	1220	1320	1420	1520	1620	1720	1820	1920	2020	2120	2220	2320	2420	2520	2620	2720	2820	2920	3020	3120	3220	3320	3420	3520	3620	3720	3820	3920	4020										
2	940	1040	1140	1240	1340	1440	1540	1640	1740	1840	1940	2040	2140	2240	2340	2440	2540	2640	2740	2840	2940	3040	3140	3240	3340	3440	3540	3640	3740	3840	3940	4040									
3	860	960	1060	1160	1260	1360	1460	1560	1660	1760	1860	1960	2060	2160	2260	2360	2460	2560	2660	2760	2860	2960	3060	3160	3260	3360	3460	3560	3660	3760	3860	3960	4060								
4	780	880	980	1080	1180	1280	1380	1480	1580	1680	1780	1880	1980	2080	2180	2280	2380	2480	2580	2680	2780	2880	2980	3080	3180	3280	3380	3480	3580	3680	3780	3880	3980	4080							
5	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000	3100	3200	3300	3400	3500	3600	3700	3800	3900	4000							
6	620	720	820	920	1020	1120	1220	1320	1420	1520	1620	1720	1820	1920	2020	2120	2220	2320	2420	2520	2620	2720	2820	2920	3020	3120	3220	3320	3420	3520	3620	3720	3820	3920	4020						
7	540	640	740	840	940	1040	1140	1240	1340	1440	1540	1640	1740	1840	1940	2040	2140	2240	2340	2440	2540	2640	2740	2840	2940	3040	3140	3240	3340	3440	3540	3640	3740	3840	3940	4040					
8	460	560	660	760	860	960	1060	1160	1260	1360	1460	1560	1660	1760	1860	1960	2060	2160	2260	2360	2460	2560	2660	2760	2860	2960	3060	3160	3260	3360	3460	3560	3660	3760	3860	3960	4060				
9	380	480	580	680	780	880	980	1080	1180	1280	1380	1480	1580	1680	1780	1880	1980	2080	2180	2280	2380	2480	2580	2680	2780	2880	2980	3080	3180	3280	3380	3480	3580	3680	3780	3880	3980	4080			
10	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000	3100	3200	3300	3400	3500	3600	3700	3800	3900	4000			
11	220	320	420	520	620	720	820	920	1020	1120	1220	1320	1420	1520	1620	1720	1820	1920	2020	2120	2220	2320	2420	2520	2620	2720	2820	2920	3020	3120	3220	3320	3420	3520	3620	3720	3820	3920	4020		
12	140	240	340	440	540	640	740	840	940	1040	1140	1240	1340	1440	1540	1640	1740	1840	1940	2040	2140	2240	2340	2440	2540	2640	2740	2840	2940	3040	3140	3240	3340	3440	3540	3640	3740	3840	3940	4040	
13	60	160	260	360	460	560	660	760	860	960	1060	1160	1260	1360	1460	1560	1660	1760	1860	1960	2060	2160	2260	2360	2460	2560	2660	2760	2860	2960	3060	3160	3260	3360	3460	3560	3660	3760	3860	3960	4060
14	40	140	240	340	440	540	640	740	840	940	1040	1140	1240	1340	1440	1540	1640	1740	1840	1940	2040	2140	2240	2340	2440	2540	2640	2740	2840	2940	3040	3140	3240	3340	3440	3540	3640	3740	3840	3940	4040
15	20	120	220	320	420	520	620	720	820	920	1020	1120	1220	1320	1420	1520	1620	1720	1820	1920	2020	2120	2220	2320	2420	2520	2620	2720	2820	2920	3020	3120	3220	3320	3420	3520	3620	3720	3820	3920	4020
16	10	100	200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000	3100	3200	3300	3400	3500	3600	3700	3800	3900	4000
17	5	90	190	290	390	490	590	690	790	890	990	1090	1190	1290	1390	1490	1590	1690	1790	1890	1990	2090	2190	2290	2390	2490	25														

## Morning Peak

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### Evening Peak

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### Evening Peak

NO	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	TOTAL
1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
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23	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
25	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
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27	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

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Morning Peak

Hourly O&D Tables in Base Year (Light Truck)

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1. What is the purpose of the document?  
 2. What are the main findings of the study?  
 3. What are the implications of the findings?  
 4. What are the limitations of the study?  
 5. What are the conclusions of the study?

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{vehicle}
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Hourly O&amp;D Tables in Base Year (Motor cycle)

**Morning Peak**

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{vehicle}
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	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	SUBTOTAL	TOTAL
417	144	322	67	103	43	35	264	48	3	35	30	36	74	13	10	42	22	6	113	6	113	6	0	0	0	0	0	0	354
418	144	322	166	559	147	187	459	307	273	106	28	28	144	13	15	42	22	13	126	141	32	322	42	3	0	0	0	0	2764
419	323	623	376	868	390	50	348	337	0	50	48	48	144	0	7	15	15	15	141	0	0	0	0	0	0	0	0	0	4531
420	3085	488	111	277	308	18	145	4	7	17	45	12	253	183	2	15	15	15	0	0	0	0	0	0	0	0	0	0	3075
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422	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
423	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
424	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
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426	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
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440	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
441	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
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473	147	712	61	127	270	137	119	5	14	45	21	12	253	253	0	15	0	0	44	0	0	0	0	0	0	0	0	0	2380
474	147	712	61	127	270	137	119	5	14																				

Evening Peak

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000
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Off Peak

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	COL TOTAL
1	1232	2823	848	362	286	155	119	859	384	223	125	59	21	0	4	1	27	43	258	12	42	12	0	0	0	0	0	640
2	1233	2824	117	384	289	207	191	211	112	172	67	27	22	0	0	1	7	43	19	0	32	12	0	0	0	0	0	276
3	1234	2825	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
4	1235	2826	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
5	1236	2827	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
6	1237	2828	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
7	1238	2829	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
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13	1244	2835	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
14	1245	2836	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
15	1246	2837	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
16	1247	2838	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
17	1248	2839	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
18	1249	2840	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
19	1250	2841	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
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23	1254	2845	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
24	1255	2846	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
25	1256	2847	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
26	1257	2848	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
27	1258	2849	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
28	1259	2850	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
29	1260	2851	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
30	1261	2852	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
31	1262	2853	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
32	1263	2854	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
33	1264	2855	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
34	1265	2856	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
35	1266	2857	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
36	1267	2858	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
37	1268	2859	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
38	1269	2860	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
39	1270	2861	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
40	1271	2862	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
41	1272	2863	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
42	1273	2864	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
43	1274	2865	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
44	1275	2866	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
45	1276	2867	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
46	1277	2868	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
47	1278	2869	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
48	1279	2870	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
49	1280	2871	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
50	1281	2872	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
51	1282	2873	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
52	1283	2874	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
53	1284	2875	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
54	1285	2876	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
55	1286	2877	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
56	1287	2878	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
57	1288	2879	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
58	1289	2880	124	384	290	207	191	211	112	172	66	26	21	0	0	1	7	43	19	0	31	11	0	0	0	0	0	276
59	1																											

# Daily O&D Tables in Base Year

## Passenger Car

(10 Vehicles)																													
Passenger Car	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	TOTAL	
1	3769	3122	1339	1323	1110	1019	864	810	69	69	137	112	14	38	38	21	66	60	31	304	185	104	18	13	80	43	7	14802.	
2	3123	2668	1378	1378	1136	1086	933	880	74	74	146	121	24	64	64	38	78	78	39	396	229	125	212	187	132	92	10	15097.	
3	2668	2178	1378	1378	1136	1086	933	880	511	511	107	86	33	84	84	46	105	105	54	526	304	165	116	84	54	36	19	15921.	
4	2178	1732	1378	1378	1136	1086	933	880	369	369	73	58	24	64	64	38	78	78	39	396	229	125	116	84	54	36	19	17113.	
5	1732	1446	1378	1378	1136	1086	933	880	255	255	51	41	16	44	44	25	59	59	27	250	152	88	70	41	205	14	23	20220.	
6	1446	1246	1378	1378	1136	1086	933	880	181	181	37	30	12	34	34	20	44	44	21	179	107	62	51	26	10	10	1	1	21262.
7	1246	1066	1378	1378	1136	1086	933	880	125	125	23	19	8	24	24	14	31	31	15	129	76	42	33	17	8	10	1	1	21262.
8	1066	933	1378	1378	1136	1086	933	880	89	89	15	12	5	17	17	10	21	21	10	89	51	28	22	10	5	2	1	1	21262.
9	933	810	1378	1378	1136	1086	933	880	64	64	11	9	4	13	13	8	16	16	7	64	38	21	16	7	3	1	1	1	21262.
10	810	732	1378	1378	1136	1086	933	880	49	49	8	7	3	10	10	6	12	12	5	49	28	16	11	5	2	1	1	1	21262.
11	732	668	1378	1378	1136	1086	933	880	33	33	5	4	2	7	7	4	9	9	4	33	19	11	8	4	2	1	1	1	21262.
12	668	610	1378	1378	1136	1086	933	880	23	23	4	3	2	5	5	3	7	7	3	23	13	8	6	4	2	1	1	1	21262.
13	610	564	1378	1378	1136	1086	933	880	17	17	3	2	1	4	4	3	5	5	2	17	9	5	4	3	2	1	1	1	21262.
14	564	526	1378	1378	1136	1086	933	880	12	12	2	1	1	3	3	2	4	4	2	12	6	4	3	2	1	1	1	1	21262.
15	526	499	1378	1378	1136	1086	933	880	9	9	1	1	1	1	1	1	1	1	1	9	5	3	2	1	1	1	1	1	21262.
16	499	478	1378	1378	1136	1086	933	880	7	7	1	1	1	1	1	1	1	1	1	7	4	3	2	1	1	1	1	1	21262.
17	478	459	1378	1378	1136	1086	933	880	5	5	1	1	1	1	1	1	1	1	1	5	3	2	1	1	1	1	1	1	21262.
18	459	441	1378	1378	1136	1086	933	880	4	4	1	1	1	1	1	1	1	1	1	4	3	2	1	1	1	1	1	1	21262.
19	441	424	1378	1378	1136	1086	933	880	3	3	1	1	1	1	1	1	1	1	1	3	2	1	1	1	1	1	1	1	21262.
20	424	408	1378	1378	1136	1086	933	880	2	2	1	1	1	1	1	1	1	1	1	2	1	1	1	1	1	1	1	1	21262.
21	408	392	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
22	392	377	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
23	377	362	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
24	362	347	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
25	347	332	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
26	332	317	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
27	317	302	1378	1378	1136	1086	933	880	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	21262.
COLTOTAL	14802.	15097.	15921.	17113.	20220.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.	21262.

## Taxi

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	TOTAL
1	1023	1699	1677	555	490	151	105	629	378	19	43	50	9	22	178	7	33	41	14	30	66	37	0	0	0	0	0	3789
2	3072	3449	1517	955	990	161	105	353	353	13	109	102	6	17	38	0	18	8	8	49	12	200	77	0	0	0	0	13130
3	3449	3131	2366	955	1504	555	353	284	284	10	89	82	4	13	32	0	14	4	4	34	14	97	0	0	0	0	3049	
4	3131	2366	1517	955	354	74	254	125	125	8	73	67	3	11	24	0	10	3	3	27	13	76	0	0	0	0	3789	
5	2366	1517	955	354	254	42	177	125	125	6	64	59	2	9	20	0	8	2	2	22	10	53	0	0	0	0	3582	
6	1517	955	354	254	42	177	125	125	125	5	53	49	1	7	16	0	6	1	1	16	9	33	0	0	0	0	3201	
7	955	354	254	42	177	125	125	125	125	4	43	40	1	6	12	0	5	1	1	12	7	24	0	0	0	0	3201	
8	354	254	42	177	125	125	125	125	125	3	37	34	1	5	10	0	4	1	1	10	6	22	0	0	0	0	3201	
9	254	42	177	125	125	125	125	125	125	2	28	26	1	4	9	0	3	1	1	9	5	17	0	0	0	0	3201	
10	42	177	125	125	125	125	125	125	125	1	22	20	1	3	7	0	2	1	1	7	4	14	0	0	0	0	3201	
11	177	125	125	125	125	125	125	125	125	1	15	14	1	2	5	0	1	1	1	5	3	12	0	0	0	0	3201	
12	125	125	125	125	125	125	125	125	125	1	11	10	1	1	4	0	1	1	1	4	2	10	0	0	0	0	3201	
13	125	125	125	125	125	125	125	125	125	1	8	8	1	1	3	0	1	1	1	3	2	8	0	0	0	0	3201	
14	125	125	125	125	125	125	125	125	125	1	6	6	1	1	2	0	1	1	1	2	1	6	0	0	0	0	3201	
15	125	125	125	125	125	125	125	125	125	1	5	5	1	1	1	0	1	1	1	1	1	4	0	0	0	0	3201	
16	125	125	125	125	125	125	125	125	125	1	4	4	1	1	1	0	1	1	1	1	1	3	0	0	0	0	3201	
17	125	125	125	125	125	125	125	125	125	1	3	3	1	1	1	0	1	1	1	1	1	2	0	0	0	0	3201	
18	125	125	125	125	125	125	125	125	125	1	2	2	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
19	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
20	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
21	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
22	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
23	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
24	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
25	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
26	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
27	125	125	125	125	125	125	125	125	125	1	1	1	1	1	1	0	1	1	1	1	1	1	0	0	0	0	3201	
COLTOTAL	13130	13470	14191	15097	15921	17113																					68095	

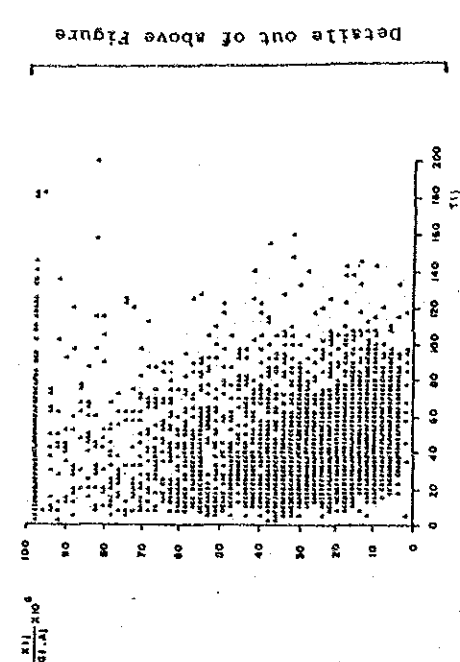
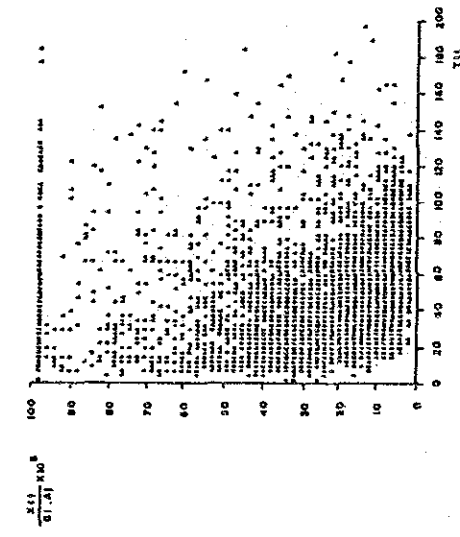
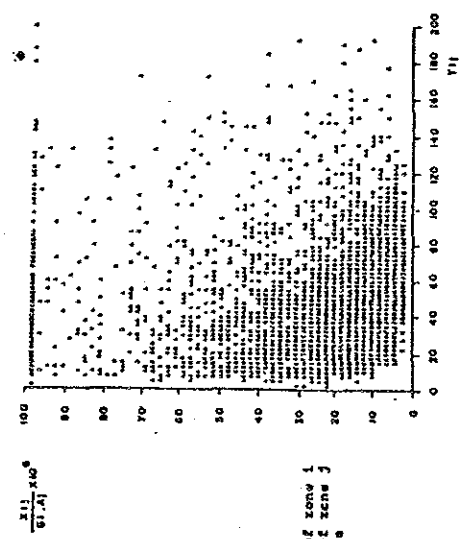
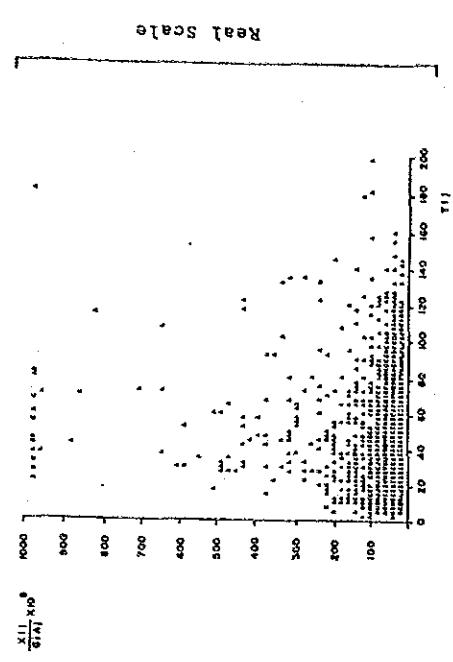
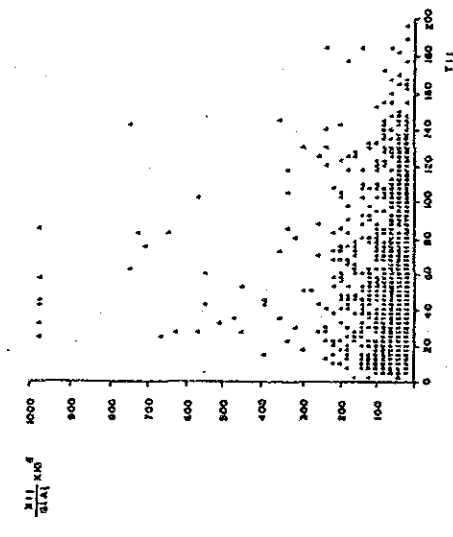
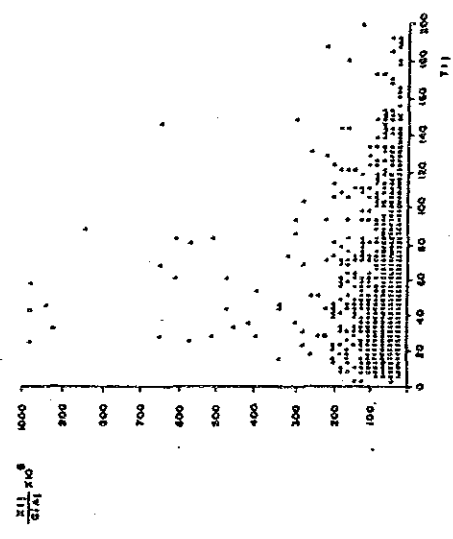
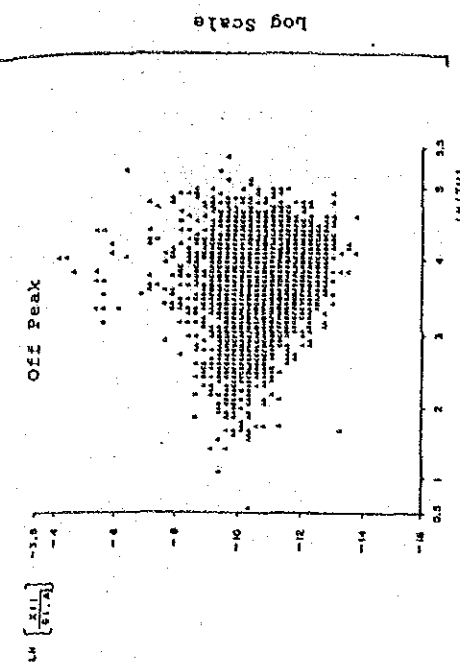
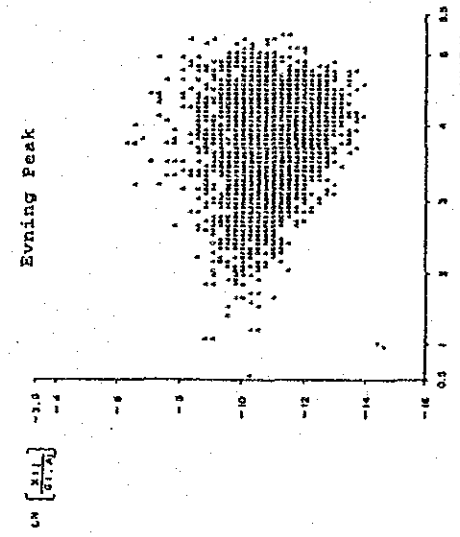
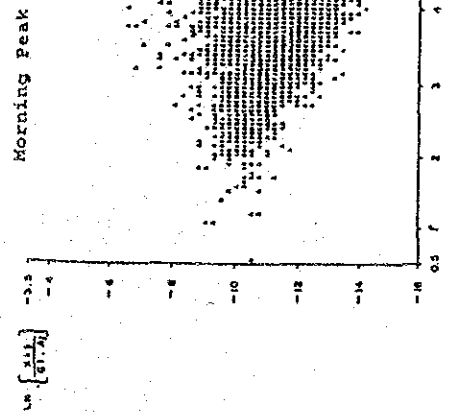


Daily O&D Tables in Base Year

Motorcycle	(10 Vehicles)																										
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27
1	2144	4463	1323	466	477	243	135	1403	710	42	300	102	38	1	30	13	104	77	231	34	82	22	6	5	1	0	0
2	1249	1770	3181	1764	1821	1038	162	1493	581	377	136	153	18	0	0	0	0	44	0	24	114	254	113	38	14	0	0
3	1334	3355	2722	2977	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
4	1422	4277	2727	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
5	1510	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
6	1600	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
7	1690	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
8	1780	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
9	1870	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
10	1960	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
11	2050	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
12	2140	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
13	2230	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
14	2320	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
15	2410	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
16	2500	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
17	2590	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
18	2680	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
19	2770	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
20	2860	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
21	2950	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
22	3040	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
23	3130	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
24	3220	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
25	3310	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
26	3400	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
27	3490	5122	3111	3202	3202	138	160	1428	243	23	152	154	46	0	3	9	30	49	50	176	114	0	0	0	11	1	0
TOTAL	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245	76245

Appendix 3.6.1      Travel Impedance and Trip Volume Relationship





LEGEND  
 X1j : Old volume  
 G1 : Generation of zone 1  
 A1 : Attraction of zone 1  
 T1j : Time distance





Appendix 4.2.1      Hourly O&D Tables (Base Case)



Hourly Q&D Tables in Base Year

Passenger Car Equivalent

Morning Peak

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000	1001	1002	1003	1004	1005	1006	1007	1008	1009	1010	1011	1012	1013	1014	1015	1016	1017	1018	1019	1020	1021	1022	1023	1024	1025	1026	1027	1028	1029	1030	1031	1032	1033	1034	1035	1036	1037	1038	1039	1040	1041	1042	1043	1044	1045	1046	1047	1048	1049	1050	1051	1052	1053	1054	1055	1056	1057	1058	1059	1060	1061	1062	1063	1064	1065	1066	1067	1068	1069	1070	1071	1072	1073	1074	1075	1076	1077	1078	1079	1080	1081	1082	1083	1084	1085	1086	1087	1088	1089	1090	1091	1092	1093	1094	1095	1096	1097	1098	1099	1100	1101	1102	1103	1104	1105	1106	1107	1108	1109	1110	1111	1112	1113	1114	1115	1116	1117	1118	1119	1120	1121	1122	1123	1124	1125	1126	1127	1128	1129	1130	1131	1132	1133	1134	1135	1136	1137	1138	1139	1140	1141	1142	1143	1144	1145	1146	1147	1148	1149	1150	1151	1152	1153	1154	1155	1156	1157	1158	1159	1160	1161	1162	1163	1164	1165	1166	1167	1168	1169	1170	1171	1172	1173	1174	1175	1176	1177	1178	1179	1180	1181	1182	1183	1184	1185	1186	1187	1188	1189	1190	1191	1192	1193	1194	1195	1196	1197	1198	1199	1200	1201	1202	1203	1204	1205	1206	1207	1208	1209	1210	1211	1212	1213	1214	1215	1216	1217	1218	1219	1220	1221	1222	1223	1224	1225	1226	1227	1228	1229	1230	1231	1232	1233	1234	1235	1236	1237	1238	1239	1240	1241	1242	1243	1244	1245	1246	1247	1248	1249	1250	1251	1252	1253	1254	1255	1256	1257	1258	1259	1260	1261	1262	1263	1264	1265	1266	1267	1268	1269	1270	1271	1272	1273	1274	1275	1276	1277	1278	1279	1280	1281	1282	1283	1284	1285	1286	1287	1288	1289	1290	1291	1292	1293	1294	1295	1296	1297	1298	1299	1300	1301	1302	1303	1304	1305	1306	1307	1308	1309	1310	1311	1312	1313	1314	1315	1316	1317	1318	1319	1320	1321	1322	1323	1324	1325	1326	1327	1328	1329	1330	1331	1332	1333	1334	1335	1336	1337	1338	1339	1340	1341	1342	1343	1344	1345	1346	1347	1348	1349	1350	1351	1352	1353	1354	1355	1356	1357	1358	1359	1360	1361	1362	1363	1364	1365	1366	1367	1368	1369	1370	1371	1372	1373	1374	1375	1376	1377	1378	1379	1380	1381	1382	1383	1384	1385	1386	1387	1388	1389	1390	1391	1392	1393	1394	1395	1396	1397	1398	1399	1400	1401	1402	1403	1404	1405	1406	1407	1408	1409	1410	1411	1412	1413	1414	1415	1416	1417	1418	1419	1420	1421	1422	1423	1424	1425	1426	1427	1428	1429	1430	1431	1432	1433	1434	1435	1436	1437	1438	1439	1440	1441	1442	1443	1444	1445	1446	1447	1448	1449	1450	1451	1452	1453	1454	1455	1456	1457	1458	1459	1460	1461	1462	1463	1464	1465	1466	1467	1468	1469	1470	1471	1472	1473	1474	1475	1476	1477	1478	1479	1480	1481
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## Hourly O&amp;D Tables in 1991 (Base case)

Morning Peak

Morning Peak		Hourly Car Volumes in 1934 (East Side)																										Passenger Car Equivalent		TOTAL																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
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### Evening Peak

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	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	TOTAL	
1	5143	4072	3087	1769	1392	410	560	1284	1069	111	223	133	129	74	344	21	124	129	377	301	210	126	12	37	133	1	53	3574	
2	2394	2325	2452	2785	1714	141	112	423	112	112	423	112	112	423	112	112	423	112	112	423	112	112	423	112	112	423	53	2324	
3	1292	1902	2850	1948	858	341	394	776	249	110	778	595	116	368	318	46	376	81	106	316	17	50	18	15	170	12	76	2732	
4	2359	1778	3315	1934	1988	745	1501	594	321	407	393	619	16	98	44	31	0	14	45	403	45	37	4	64	316	52	33	12503	
5	735	1761	137	322	694	763	565	135	143	208	323	149	13	23	7	0	13	1	1	401	45	37	4	64	316	52	33	18804	
6	1227	3778	1629	4711	1319	273	1913	136	436	436	1213	329	112	3	28	0	13	1	1	31	43	37	4	64	316	52	33	18771	
7	1445	1117	869	392	380	144	312	1128	403	145	775	31	15	713	609	12	241	312	342	131	27	11	1	1	1250	12	93	12489	
8	1445	1117	869	392	380	144	312	1128	403	145	775	31	15	713	609	12	241	312	342	131	27	11	1	1	1250	12	93	12489	
9	1445	1117	869	392	380	144	312	1128	403	145	775	31	15	713	609	12	241	312	342	131	27	11	1	1	1250	12	93	12489	
10	2399	750	376	133	548	142	1032	47	108	359	102	35	14	2	3	3	1	1	1	1	5	25	16	79	9	2	244	16397	
11	2399	750	376	133	548	142	1032	47	108	359	102	35	14	2	3	3	1	1	1	1	5	25	16	79	9	2	244	16397	
12	126	248	440	225	1299	135	294	82	38	521	107	304	10	21	34	8	6	40	19	11	1	1	1	1	1	1	1	3600	
13	118	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	5139
14	118	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	127	132	164	305	5139
15	55	43	38	56	25	3	1	93	90	33	2	7	0	208	10	125	12	116	103	31	50	3	0	4	73	16	1	3353	
16	17	177	199	332	408	18	5	0	212	170	2	0	1	2	33	1	36	336	44	52	1	0	5	3	28	169	8	2378	
17	18	115	41	83	13	1	22	91	410	2	2	2	2	2	16	1	1	37	12	4	1	0	0	0	30	1705	29	1091	
18	101	123	46	123	423	57	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	1705
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21	101	123	46	123	423	57	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	1705
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27	101	123	46	123	423	57	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	1705
28	101	123	46	123	423	57	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	1705
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53	101	123	46	123	423	57	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	16	186	1705
54	101	123	46	123	423	57	16	186	16</																				



**Morning Peak**

Evening peak

Peak  
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A-4:5

Appendix 4.2.2      Hourly O&D Table in 1991, 2001 and 2011





Hourly Q&amp;D Tables in 1991 (With Project case)

Morning Peak		Hourly O&D Tables in 1991 (With Project case)																								(Passenger Car Equivalent)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
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Hourly O&D Tables in 2001 (With Project case)

Morning Peak

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000	1001	1002	1003	1004	1005	1006	1007	1008	1009	1010	1011	1012	1013	1014	1015	1016	1017	1018	1019	1020	1021	1022	1023	1024	1025	1026	1027	1028	1029	1030	1031	1032	1033	1034	1035	1036	1037	1038	1039	1040	1041	1042	1043	1044	1045	1046	1047	1048	1049	1050	1051	1052	1053	1054	1055	1056	1057	1058	1059	1060	1061	1062	1063	1064	1065	1066	1067	1068	1069	1070	1071	1072	1073	1074	1075	1076	1077	1078	1079	1080	1081	1082	1083	1084	1085	1086	1087	1088	1089	1090	1091	1092	1093	1094	1095	1096	1097	1098	1099	1100	1101	1102	1103	1104	1105	1106	1107	1108	1109	1110	1111	1112	1113	1114	1115	1116	1117	1118	1119	1120	1121	1122	1123	1124	1125	1126	1127	1128	1129	1130	1131	1132	1133	1134	1135	1136	1137	1138	1139	1140	1141	1142	1143	1144	1145	1146	1147	1148	1149	1150	1151	1152	1153	1154	1155	1156	1157	1158	1159	1160	1161	1162	1163	1164	1165	1166	1167	1168	1169	1170	1171	1172	1173	1174	1175	1176	1177	1178	1179	1180	1181	1182	1183	1184	1185	1186	1187	1188	1189	1190	1191	1192	1193	1194	1195	1196	1197	1198	1199	1200	1201	1202	1203	1204	1205	1206	1207	1208	1209	1210	1211	1212	1213	1214	1215	1216	1217	1218	1219	1220	1221	1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(Passenger Car Equivalent)

Morning Peak		Hourly O&D Tables in 2011 (With Project Case)																								Total					
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	Total		
1	1	6283	5169	3275	1326	1307	397	644	259	814	213	267	124	112	121	374	77	324	113	160	273	212	134	122	123	124	125	126	127	32002	
2	2	4890	4351	2721	1116	1094	324	524	198	585	145	210	972	2285	147	458	107	416	71	149	321	47	123	123	123	123	123	123	123	77223	
3	3	4699	4377	2855	1774	2133	1573	1018	3260	560	22	381	2113	235	1640	884	202	768	379	19	291	64	113	0	13	1310	23	13	13	49177	
4	4	3616	4413	1708	1778	2605	1234	933	572	394	550	865	943	100	319	251	48	123	7	30	775	127	207	77	59	1859	108	10	10	29178	
5	5	3088	4019	1683	1533	1675	1213	1538	520	584	430	2213	238	131	131	32	52	123	24	117	255	37	0	0	0	0	0	0	0	25174	
6	6	2792	3644	1702	1432	1592	1202	1202	277	521	1946	1389	443	1262	122	104	32	123	24	117	255	37	0	0	0	0	0	0	0	23788	
7	7	2792	3644	1702	1432	1592	1202	1202	277	521	1946	1389	443	1262	122	104	32	123	24	117	255	37	0	0	0	0	0	0	0	23788	
8	8	2792	3644	1702	1432	1592	1202	1202	277	521	1946	1389	443	1262	122	104	32	123	24	117	255	37	0	0	0	0	0	0	0	23788	
9	9	2792	3644	1702	1432	1592	1202	1202	277	521	1946	1389	443	1262	122	104	32	123	24	117	255	37	0	0	0	0	0	0	0	23788	
10	10	2792	3644	1702	1432	1592	1202	1202	277	521	1946	1389	443	1262	122	104	32	123	24	117	255	37	0	0	0	0	0	0	0	23788	
11	11	1537	3388	915	555	2719	647	416	242	539	1447	928	934	154	103	27	76	34	31	13	1274	745	770	2394	415	59	123	611	0	27258	
12	12	1704	5667	3299	1202	3393	4540	6088	568	1145	555	8660	968	1986	54	189	146	132	47	0	344	792	229	2316	568	287	13	0	0	47758	
13	13	937	1714	3625	1846	6794	1446	688	279	502	108	1722	1438	136	482	23	0	0	0	0	0	111	36	8	111	238	216	0	0	17	
14	14	1642	364	522	131	273	43	1408	211	21	15	0	0	0	0	0	0	0	0	0	172	5	5	1	1	1	1	1	1	1156	
15	15	1660	319	853	765	324	11	34	1235	1599	17	236	138	0	750	332	27	26	110	324	139	110	34	94	7	4	216	34	0	91	9146
16	16	1213	711	272	283	288	8	21	485	377	19	163	0	212	83	271	60	176	26	111	323	282	0	16	8	143	143	0	0	9829	
17	17	887	127	532	484	37	50	3	236	203	3	21	151	19	13	66	11	12	16	0	0	12	0	0	0	0	0	0	0	3015	
18	18	124	34	170	142	5	4	121	156	872	5	20	61	7	1059	153	23	70	37	1644	210	73	28	6	46	23	29	29	29	5192	
19	19	1556	436	244	288	159	71	157	330	133	37	109	462	1714	163	112	59	14	3	1302	693	107	10	151	18	121	0	344	0	344	124343
20	20	1358	1348	382	108	55	458	161	466	2142	48	67	0	140	4	164	58	8	17	20	3	742	1235	245	228	0	0	10	10	10	1528
21	21	1394	1492	64	39	430	134	309	705	1233	199	117	11	406	31	230	20	0	0	0	0	1	1215	1215	372	115	0	0	0	0	8274
22	22	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
23	23	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
24	24	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
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41	41	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
42	42	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
43	43	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
44	44	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
45	45	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
46	46	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
47	47	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	274	14801
48	48	1231	1231	1231	273	273	273	408	448	1233	274	274	274	274	274	274	274	274	274	274	274	274									

Evning Peak

Evening Peak	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000
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Off Peak

Off Peak	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	KO TOTAL	COTOTAL		
1	10770	8498	6563	4566	3804	739	1455	2266	2436	224	563	305	13	113	599	137	235	310	460	642	427	716	24	33	204	182	151	43121	43882		
2	10468	19028	7847	3640	6256	4153	1667	3098	1917	2860	1741	124	113	399	487	279	410	582	779	1038	775	1375	81	78	276	224	171	46051	58841		
3	4925	7800	12878	7056	5412	1621	3258	2258	1501	1844	1044	24	777	437	119	708	187	79	1038	779	1038	45	102	0	38	490	24	23	35531	45515	
4	2112	3448	5344	2736	1637	1532	2065	1225	432	923	872	1375	30	216	94	73	57	34	67	955	585	9	134	571	116	61	61	32052	34816		
5	1440	4083	5113	782	1643	1165	190	411	231	1104	395	35	0	34	0	34	6	37	74	183	0	19	22	0	0	0	0	22932	20661		
6	1306	4558	1871	1074	1504	1307	2154	288	962	2307	3863	786	291	14	76	0	3	187	15	286	82	182	28	40	259	72	17	22665	17364		
7	3232	3800	1530	731	175	305	4758	3722	137	211	172	18	1343	1093	301	0	350	158	874	206	373	244	25	16	137	137	137	22737	27364		
8	3240	3504	1132	171	128	370	2159	1713	485	307	80	13	18	130	110	4	110	111	111	61	769	49	162	50	4	0	0	22737	27364		
9	924	275	339	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	138	22737	27364		
10	245	664	1073	578	272	336	477	159	121	136	351	978	29	55	47	68	6	0	0	0	56	223	103	408	64	0	0	5	18604	12700	
11	37	370	182	52	33	113	17	17	33	36	10	202	5	7	0	0	0	0	0	17	161	52	13	4	25	37	0	0	3310	10931	
12	15	233	59	243	41	136	62	64	1304	118	88	38	18	519	49	35	0	0	0	17	161	52	13	4	25	37	0	0	3310	10931	
13	15	774	354	569	434	97	23	51	911	1295	8	70	59	3	486	139	240	0	0	0	0	0	0	0	0	0	0	0	3310	10931	
14	16	104	95	21	70	51	33	0	436	1377	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3310	10931	
15	17	123	390	103	170	29	0	0	0	0	27	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3310	10931	
16	176	50	142	433	204	5	120	21	1042	3	21	64	0	0	16	38	359	0	72	6	9	18	0	0	0	0	0	0	3310	10931	
17	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
18	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
19	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
20	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
21	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
22	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
23	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
24	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
25	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
26	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
27	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
28	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
29	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
30	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
31	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
32	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
33	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
34	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
35	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
36	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
37	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
38	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
39	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
40	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
41	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
42	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
43	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
44	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
45	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
46	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
47	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
48	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
49	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28	23	0	11	31	13	13	3283	3890		
50	300	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320	320
51	708	530	747	713	304	5	3	39	312	111	0	43	13	184	155	38	87	253	416	305	28										



Appendix 4.6.1

Vehicle Operating Cost



### Vehicle Operating Costs

Eight vehicle types were selected for the calculation of vehicle operating costs following the precedent of the JICA Feasibility Study on the Second Stage Expressway System in the Greater Bangkok, 1983, except that the category of taxis replaced the category of light buses. Representative vehicle makes were selected as the same as those by the aforementioned JICA study. The cost items were:

- fuel
- oil
- tyres
- maintenance (parts)
- maintenance (labor)
- overhead
- capital costs (depreciation and interest)

Except for the last item methods used in the JICA Study were applied with updating of prices at spring 1986 levels.

For capital costs methods used by STTR were applied. Depreciation was calculated on a straight-line basis and interest was calculated at 12% on the declining undepreciated balance. For motorcycles and cars part of depreciation was considered to be age-related instead of usage in terms of total kilometers. Interest rates for motorcycles and cars was set at 6% per annum and for pickups 8% instead of applying the full 12% since a large portion of these vehicles are not business related. Basic assumptions for each vehicle type are shown in Table A.4.11.1.1.

As pointed out in the STTR Internal Working Paper No. 6, the effect of low congestion speed on the annual vehicle usage was considered important and the assumed relationship is shown in Table A.4.11.1.2.

Fuel and oil consumption data shown in the JICA Study, which were taken from actual experiments in similar conditions in Japan, were combined with spring 1986 prices to produce fuel and oil costs. Other costs were similarly adjusted from the JICA Study tables and STTR tables.

The resulting vehicle operating costs for each vehicle type are shown in Table A.4.11.1.3 through Table A.4.11.1.10. Crew costs not included as they are included in time values.

Table A.4.11.1.1 VOC Basic Data

Item	Motorcycle	Car	Taxi	Pick-Up	Medium Bus	Heavy Bus	Medium Truck	Heavy Truck
Manufacturer	Suzuki	Corolla	Corolla	Nissan	Isuzu	Hino	Hino	Hino
Engine C.C.	100	1300	1300	1800	3300	5900	5900	5900
Market price (000 Baht)	26.6	273.0	269.0	168.0	290.0	1065.0	445.0	310.0
Economic cost (000 Baht)	17.1	165.7	144.9	118.8	243.0	924.4	370.9	220.4
Excluding tyres (000 Baht)	16.8	162.6	141.7	115.6	234.0	908.4	366.9	490.4
Use depreciation X	80	88	100	100	100	100	100	100
Use life (000 kms)	80	160	200	226	360	180	400	800
Use depreciation/000 kms	188	819	709	492	889	1803	882	881
Benchmark speed (k/h)	30	30	30	30	28	20	30	28
Annual use at benchmark (000 kms)	10	12	100	28	78	80	88	81
Age depreciation/00 kms at benchmark (Baht)	41	333	-	-	-	-	-	-
Annual economic interest (Baht)	615	4677	8590	4810	14770	35830	22130	30408
Interest/000 kms at benchmark (Baht)	52	390	87	192	193	694	402	698
Life at benchmark (years)	8	13.3	2	9	4.7	8	7.3	9.8

Table A.4.11.1.2 Annual Distances by Journey Speed  
(000 kms)

Average Speed (k/h)	Motor-cycle	Car	Taxi	Pick-Up	Medium Bus	Heavy Bus	Trucks
5	5.0	6.0	24.1	10.0	17.4	23.7	16.5
10	6.0	7.2	43.9	14.1	33.6	42.2	28.6
15	7.0	8.4	61.0	17.5	48.9	61.8	37.8
20	8.0	9.6	75.6	20.0	63.0	80.0	45.1
25	9.0	10.8	88.0	22.5	75.0	97.3	50.9
30	10.0	12.0	100.0	25.0	88.8	113.3	55.0
35	11.0	13.2	108.6	26.5	101.1	129.8	60.0
40	12.0	14.4	117.5	28.0	111.9	144.6	63.3
45	13.0	15.6	125.1	29.5	123.0	158.6	66.0
50	14.0	16.8	132.0	31.0	123.6	172.2	68.8
55	15.0	18.0	138.3	32.5	123.6	172.2	71.0
60	16.0	18.0	143.9	34.0	123.6	172.2	73.1
65	16.0	18.0	149.2	35.5	123.6	172.2	76.0
70	16.0	18.0	154.1	37.0	123.6	172.2	78.4
75	16.0	18.0	158.4	38.5	123.6	172.2	78.0
80	16.0	18.0	162.4	40.0	123.6	172.2	79.2
85	16.0	18.0	162.4	40.0	-	-	-
90	16.0	18.0	162.4	40.0	-	-	-

Table A.4.11.1.3 Motorcycle VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Interest and age depreciation	Vehicle Operating Cost
5	440	28	18	165	8	17	188	860
10	406	24	16	165	8	17	155	791
15	328	19	18	165	7	17	133	686
20	280	16	18	165	7	17	116	617
25	245	16	16	165	7	17	103	568
30	219	16	16	165	7	19	93	522
35	205	16	18	165	7	19	85	513
40	194	16	17	165	6	20	78	496
45	185	16	18	165	6	21	71	482
50	182	16	18	165	6	22	66	475
55	177	18	19	165	6	23	62	470
60	175	18	19	165	7	24	57	465
65	174	19	20	165	8	25	57	468
70	172	21	20	165	8	25	57	468
75	173	22	21	165	8	26	57	472
80	179	23	22	165	8	26	57	480
85	184	23	23	165	8	26	57	486
90	191	24	25	165	8	26	57	498



Table A.4.11.1.4 Private Car VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Interest and age depre- ciation	Vehicle Operating Cost
5	1121	80	70	619	80	43	1446	3459
10	1023	80	70	619	75	43	1265	3095
15	828	48	70	619	79	44	1033	2721
20	706	42	70	619	87	44	903	2471
25	618	34	70	619	92	45	804	2282
30	553	34	70	619	97	46	723	2142
35	516	34	70	619	97	47	657	2040
40	480	34	71	619	93	48	602	1955
45	467	38	72	619	89	50	557	1892
50	459	40	73	619	89	52	517	1849
55	447	42	74	619	92	55	482	1811
60	441	46	75	619	93	58	482	1814
65	441	50	77	619	94	61	482	1824
70	435	54	79	619	95	63	482	1827
75	441	56	82	619	96	64	482	1840
80	463	58	85	619	97	65	482	1859
85	466	60	90	619	98	66	482	1880
90	482	64	95	619	100	68	482	1910

Table A.4.11.1.5 Taxi VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Interest and age depre- ciation	Vehicle Operating Cost
5	1121	80	70	709	80	32	361	2453
10	1023	80	70	709	75	32	198	2167
15	828	48	70	709	79	33	143	1910
20	706	42	70	709	87	33	115	1762
25	618	34	70	709	92	34	99	1656
30	553	34	70	709	97	35	87	1585
35	516	34	70	709	97	35	80	1541
40	488	34	71	709	93	36	74	1505
45	467	38	72	709	89	37	69	1481
50	459	40	73	709	89	39	66	1475
55	447	42	74	709	92	42	63	1469
60	441	46	75	709	93	44	60	1468
65	441	50	77	709	93	45	58	1474
70	435	54	79	709	95	46	56	1474
75	441	56	82	709	96	48	55	1487
80	463	58	85	709	97	49	54	1505
85	466	60	90	709	98	50	53	1525
90	482	64	95	709	100	51	52	1553

Table A.4.11.1.6 Pick-up VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Interest and age depre- ciation	Vehicle Operating Cost
5	1315	80	57	462	72	52	480	2518
10	1201	68	62	462	66	52	343	2252
15	971	62	64	462	63	52	274	1938
20	828	44	66	462	60	53	240	1753
25	726	40	67	462	59	55	213	1621
30	648	40	68	462	58	57	192	1525
35	605	40	69	462	57	59	181	1473
40	573	40	70	462	56	61	171	1433
45	548	41	71	462	55	63	163	1404
50	538	42	72	462	55	65	155	1390
55	524	44	73	462	55	67	148	1374
60	517	46	74	462	55	68	141	1364
65	517	49	76	462	55	70	135	1364
70	511	53	78	462	55	73	130	1362
75	517	57	81	462	55	75	125	1372
80	531	62	85	462	54	78	120	1392
85	545	67	90	462	54	80	120	1416
90	566	72	95	462	54	82	120	1451

Table A.4.11.1.7 Midum Bus VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Overheads	Interest age depre ciation	Vehicle Operating Cost
5	1946	115	130	669	303	146	470	832	4611
10	1784	100	140	669	256	146	243	431	3778
15	1459	90	142	669	256	147	167	296	3226
20	1234	81	143	669	249	149	130	230	2885
25	1070	76	144	669	242	151	109	193	2654
30	944	76	145	669	235	155	92	163	2479
35	867	76	146	669	229	164	61	143	2375
40	802	76	148	669	223	168	73	129	2288
45	765	78	150	669	219	178	66	118	2242
50	730	82	152	669	220	186	62	109	2210
55	714	85	155	669	221	194	62	109	2210
60	698	90	158	669	222	202	62	109	2210
65	714	95	162	669	227	210	62	109	2248
70	747	100	166	669	232	220	62	109	2305
75	783	105	171	669	238	232	62	109	2369
80	856	110	180	669	245	246	62	109	2477

Table A.4.11.1.3 Heavy Bus VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Overheads	Interest age depre ciation	Vehicle Operating Cost
5	3242	115	220	1893	542	233	1917	2343	10505
10	2972	100	240	1893	543	233	1077	1316	8374
15	2432	90	250	1893	544	233	735	899	7076
20	2058	81	255	1893	545	233	568	694	6327
25	1783	76	257	1893	546	241	467	571	5834
30	1573	76	258	1893	548	248	399	487	5482
35	1446	76	259	1893	550	258	351	428	5251
40	1338	76	261	1893	555	269	314	384	5090
45	1274	78	263	1893	561	282	287	350	4988
50	1216	82	266	1893	566	297	267	322	4906
55	1189	86	269	1893	572	310	264	322	4905
60	1163	90	273	1893	579	326	264	322	4910
65	1144	95	277	1893	582	342	264	322	4919
70	1244	100	282	1893	585	360	264	322	5050
75	1303	105	292	1893	588	378	264	322	5145
80	1409	110	305	1893	591	397	264	322	5291

Table A.4.11.1.9 Medium Truck VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Depreciation (use)	Maintenance (parts)	Maintenance (labour)	Overheads	Interest age depre ciation	Vehicle Operating Cost
5	1804	120	220	982	367	145	757	1340	5645
10	1672	100	240	982	350	145	438	773	4608
15	1408	90	260	982	346	145	330	685	4046
20	1216	83	260	982	340	146	277	490	3704
25	1114	83	262	982	332	150	245	434	3512
30	990	83	264	982	338	154	227	402	3360
35	938	83	266	982	344	159	208	369	3259
40	877	83	269	982	356	164	197	349	3186
45	849	85	272	982	370	174	189	335	3166
50	822	87	275	982	375	184	182	322	3139
55	811	90	279	982	382	194	176	311	3136
60	811	93	283	982	385	204	171	302	3141
65	837	97	288	982	386	214	167	295	3175
70	863	102	293	982	395	224	163	289	3211
75	922	107	299	982	385	235	160	283	3283
80	1028	112	305	982	385	246	158	279	3406

Table A.4.11.1.10 Heavy Truck VOC  
(Baht per 000 km)

Speed k/h	Fuel	Oil	Tyres	Dipreciation (use)	Maintenance (parts)	Maintenance (labour)	Overheads	Interest age depre ciation	Vehicle Operating Cost
5	2345	120	378	981	424	234	1040	1841	7363
10	2174	100	413	981	405	234	599	1062	5968
15	1830	90	430	981	399	234	453	804	5221
20	1581	83	447	981	393	235	381	673	4774
25	1448	83	450	981	384	242	337	596	4521
30	1288	83	454	981	391	249	312	552	4310
35	1219	83	457	981	389	257	286	507	4188
40	1140	83	462	981	410	265	271	480	4092
45	1104	85	468	981	428	285	260	460	4071
50	1070	87	473	981	434	298	250	442	4035
55	1054	90	480	981	442	314	242	427	4030
60	1054	93	486	981	445	330	235	415	4039
65	1087	97	495	981	445	346	229	405	4085
70	1121	102	504	981	445	362	224	397	4136
75	1199	107	514	981	445	380	220	389	4235
80	1338	112	524	981	445	398	217	383	4398



#### Appendix 4.6.2 Time Values Per Vehicle



### Time Values Per Vehicle

Value of time losting while travelling has been estimated for trips of business purposes and trips of all other purposes separately. For the former average wage of crew and passengers differentiated by vehicle type were used but for the latter a same value was used for all vehicle types. The former could be considered reflecting the economic productivity of different types of person. However, the worth of non-productive time of a high wage earner should not be considered any more valuable than a low wage earner. The latter, therefore, is sometimes called the equity value.

Wages of drivers and assistants were estimated from the results of labor force survey carried out in 1984 by NSO and are shown in Table A.4.6.2.1 Percentages of business trips were assumed for each vehicle type based on the JICA Second Stage Expressway Study. Business trips by cars were assumed to be done with professional drivers and business trips by pickups with 50% professional drivers. The equity value of time was assumed to be 25% of average wage Baht 5.25 per hour. Estimates and assumptions used are summarized in Table A.4.6.2.2.

Table A.4.6.2.1 Wages of Vehicle Crew

Vehicle Type	Monthly Wage Cost Bt.	Hours/ Month	Cost/ Vehicle Hour Bt.
Motorcycles	3000	240	12.5
Cars	3000	185	16.2
Taxis	6350	280	22.7
Pick-Ups	4000	250	16.0
Buses	7500	200	37.5
Trucks	6000	220	27.3

Source: Special Report, SES Detailed Design.

Note: Hours include non-driving time

Table A.4.6.2.2 Time Values, 1986

Vehicle Type	Vehicle Occupancy	Business Trip %	Business Pass. Wage (Baht/Hr)	Calculation	Time Value Per Veh. Hour (Baht/Hr)
Motorcycle	1.2	15	12.5	$(0.15 \times 12.5 + 0.85 \times 5.25) \times 1.2$	7.5
Cars	2.0 driver + business pass.	15	48.0	$0.15 \times (16.2 + 48.0) + 0.85 \times 2.0 \times 5.25$	18.5
Taxis	driver + 1.1 passengers	15	48.0	$(1.0 \times 22.7) + 1.1 \times (0.15 \times 48.0 + 0.85 \times 5.25)$	35.5
Pickups	driver + 1.5 passengers	see note	48.0	$(0.5 \times 16.0 + 0.5 \times 5.25) + 1.5 \times (0.1 \times 21.0 + 0.4 \times 48.0 + 0.5 \times 5.25)$	34.4
Buses	crew + 37.7 passengers	4	21.0	$(1.0 \times 37.5) + 37.7 \times (0.04 \times 21.0 + 0.96 \times 5.25)$	259.3
Trucks	crew	100	-		

Note: Pickup drivers 50% business, 50% non-business, Pickup passenger 10% assistant, 40% business, 50% non-business

Source: Special Report, SES Detailed Design



#### Appendix 5.3.1

#### Compressive Strength Test of Existing Krungthep Bridge



Compressive Strength Test  
(Schmidt Hammer NR-3)

No. 1 (Exterior, Upstream)				Date : May 15, 1986			
Location : Bangkok Side PC				Angle : 90 degree			
R - Value							
1	53	6	57	11	52	16	49
2	53	7	56	12	51	17	54
3	55	8	55	13	51	18	51
4	52	9	55	14	50.5	19	50
5	54	10	57	15	51	20	50
R mean = 52.5				Range : 46.5 to 58.5			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 1,056.5			
R average = Total R/n = 52.8							
Compressive Strength : Fc = 520 kgf/cm <sup>2</sup>							

No. 2				Date : May 15, 1986			
Location : Bangkok Side PC				Angle : 90 degree			
R - Value							
1	51	6	50	11	51	16	50
2	47	7	51	12	51	17	53
3	55	8	54	13	48	18	49
4	54	9	52	14	56	19	50
5	53	10	53	15	51	20	51
R mean = 51				Range : 45 to 57			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 1,030			
R average = Total R/n = 51.5							
Compressive Strength : Fc = 505      kgf/cm <sup>2</sup>							

No. 3				Date : May 15, 1986			
Location : Bangkok Side PC				Angle : 90 degree			
R - Value							
1	50	6	47	11	49	16	50
2	41	7	48	12	54	17	48
3	52	8	47	13	51	18	50
4	45	9	47	14	51	19	51
5	51	10	49	15	52	20	49
R mean = 49.5				Range : 43.5 to 55.5			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 988			
R average = Total R/n = 49.4							
Compressive Strength : Fc = 450 kgf/cm <sup>2</sup>							

No. 4				Date : May 15, 1986			
Location : Bangkok Side PC				Angle : 90 degree			
R - Value							
1	50	6	50	11	52	16	55
2	48	7	50	12	52	17	46
3	50	8	56	13	44	18	51
4	49	9	47	14	54	19	52
5	54	10	50	15	50	20	48
R mean = 50				Range : 44 to 56			
Nos of Data within Range n =				Total Ri = 1,...+n = 1,008			
R average = Total R/n = 50.4							
Compressive Strength : Fc = 465 kgf/cm <sup>2</sup>							

No. 5 (Exterior, Downstream)				Date : May 15, 1986			
Location : Bangkok Side PC				Angle : 90 degree			
R - Value							
1	50	6	49	11	49	16	50
2	53	7	50	12	53	17	50
3	55	8	53	13	52	18	52
4	52	9	52	14	55	19	52
5	53	10	50	15	52	20	57
R mean = 52				Range : 45 to 58			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 1,039			
R average = Total R/n = 51.95							
Compressive Strength : Fc = 496 kgf/cm <sup>2</sup>							

No. Wall				Date : May 15, 1986			
Location : Bangkok Side Pier				Angle : 0 degree			
R - Value							
1	36	6	36	11	38	16	32
2	37	7	35	12	34	17	38
3	34	8	36	13	42	18	37
4	39	9	42	14	40	19	35
5	32	10	38	15	44	20	36
R mean = 36.5				Range : 30.5 to 42.5			
Nos of Data within Range n = 19				Total Ri = 1,...+n = 697			
R average = Total R/n = 36.7							
Compressive Strength : Fc = 295 kgf/cm <sup>2</sup>							

Allowable Stress =  $295/3 = 98 \text{ kgf/cm}^2$ , Say  $95 \text{ kgf/cm}^2$

No. 1' (Exterior, Downstream)				Date : May 15, 1986			
Location : Thonburi Side PC				Angle : 90 degree			
R - Value							
1	52	6	52	11	52	16	51
2	52	7	50	12	51	17	56
3	50	8	44	13	55	18	45
4	44	9	51	14	53	19	55
5	54	10	52	15	47	20	48
R mean = 51.5				Range : 45.5 to 57.5			
Nos of Data within Range n = 19				Total Ri = 1,...+n = 970			
R average = Total R/n = 51.0							
Compressive Strength : Fc = 495 kgf/cm <sup>2</sup>							

No. 2'				Date : May 15, 1986			
Location : Thonburi Side PC				Angle : 90 degree			
R - Value							
1	55	6	51	11	50	16	54
2	47	7	49	12	43	17	51
3	48	8	49	13	48	18	38
4	52	9	42	14	45	19	48
5	42	10	45	15	45	20	50
R mean = 47.5				Range : 41.5 to 53.5			
Nos of Data within Range n = 18				Total Ri = 1,...+n = 843			
R average = Total R/n = 46.8							
Compressive Strength : Fc = 405 kgf/cm <sup>2</sup>							

Allowable Stress =  $405/3 = 135$  kgf/cm<sup>2</sup>, SAY 135 kgf/cm<sup>2</sup>

No. 3'				Date : May 15, 1986			
Location : Thonburi Side PC				Angle : 90 degree			
R - Value							
1	41	6	51	11	44	16	51
2	49	7	51	12	53	17	52
3	45	8	50	13	56	18	55
4	43	9	52	14	51	19	47
5	51	10	48	15	50	20	53
R mean = 51				Range : 45 to 57			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 993			
R average = Total R/n = 49.6							
Compressive Strength : Fc = 450 kgf/cm <sup>2</sup>							

No. 4'				Date : May 15, 1986			
Location : Thonburi Side PC				Angle : 90 degree			
R - Value							
1	55	6	48	11	56	16	54
2	51	7	54	12	53	17	56
<del>3</del>	43	<del>8</del>	42	<del>13</del>	47	18	54
4	54	<del>9</del>	47	14	52	19	55
5	52	10	55	15	54	20	53
R mean = 53.5				Range : 47.5 to 59.5			
Nos of Data within Range n = 16				Total Ri = 1,...+n = 856			
R average = Total R/n = 53.5							
Compressive Strength : Fc = 520 kgf/cm <sup>2</sup>							

No. 5'				Date : May 15, 1986			
Location : Thonburi Side				Angle : 90 degree			
R - Value							
1	52	6	53	11	53	16	52
2	46	7	53	12	53	17	53
3	52	8	45	13	51	18	51
4	49	9	51	14	50	19	46
5	45	10	47	15	53	20	52
R mean = 51				Range : 45 to 57			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 1,007			
R average = Total R/n = 50.3							
Compressive Strength : Fc = 470 kgf/cm <sup>2</sup>							

No. Wall				Date : May 15, 1986			
Location : Thonburi Side Pier				Angle : 0 degree			
R - Value							
1	36	6	44	11	45	16	47
2	38	7	43	12	40	17	45
3	35	8	40	13	40	18	40
4	39	9	38	14	40	19	44
5	35	10	50	15	42	20	42
R mean = 40				Range : 34 to 46			
Nos of Data within Range n = 18				Total Ri = 1,...+n = 726			
R average = Total R/n = 40.3							
Compressive Strength : Fc = 350 kgf/cm <sup>2</sup>							



No. Pier Column				Date : May 16, 1986			
Location : Bascule Pier				Angle : 0 degree			
R - Value							
1	49	6	47	11	45	16	37
2	38	7	39	12	41	17	48
3	37	8	39	13	39	18	39
4	39	9	36	14	44	19	39
5	38	10	41	15	44	20	45
R mean = 39				Range : 33 to 45			
Nos of Data within Range n = 17				Total Ri = 1,...+n = 678			
R average = Total R/n = 39.8							
Compressive Strength ; Fc = 345 kgf/cm <sup>2</sup>							

No. Pier Column				Date : May 16, 1986			
Location : Bascule Pier				Angle : 0 degree			
R - Value							
1	47	6	54	11	42	16	44
2	50	7	44	12	50	17	50
3	40	8	47	13	48	18	46
4	50	9	51	14	48	19	27
5	44	10	49	15	52	20	50
R mean. = 48.5				Range : 42.5 to 56.5			
Nos of Data within Range n = 18				Total Ri = 1,...+n = 865			
R average = Total R/n = 48.0							
Compressive Strength : Fc = 495 kgf/cm <sup>2</sup>							

No. 1 (Exterior, Upstream)				Date : May 16, 1986			
Location : Thonburi Side(Innerspan)				Angle : 90 degree			
R - Value							
1	54	6	54	11	48	16	59
2	56	7	52	12	54	17	52
3	54	8	54	13	53	18	57
4	55	9	54	14	51	19	50
5	58	10	53	15	52	20	56
R mean = 54				Range : 48 to 60			
Nos of Data within Range n = 20				Total Ri = 1,...+n = 1,076			
R average = Total R/n = 53.8							
Compressive Strength : Fc = 525 kgf/cm <sup>2</sup>							

No. 2 (Interior, Upstream)				Date : May 16, 1986			
Location : Thonburi Side(Innerspan)				Angle : 90 degree			
R - Value							
1	39	6	32	11	55	16	49
2	50	7	50	12	46	17	50
3	49	8	50	13	52	18	49
4	53	9	47	14	50	19	53
5	47	10	49	15	50	20	51
R mean = 50				Range : 44 to 56			
Nos of Data within Range n = 48				Total Ri = 1,...+n = 900			
R average = Total R/n = 50.0							
Compressive Strength : Fc = 460 kgf/cm <sup>2</sup>							

No. Wall				Date : May 16, 1986			
Location : Thonburi Side Pier (in River)				Angle : 0 degree			
R - Value							
1	34	6	41	11	34	16	34
2	39	7	42	12	48	17	46
3	35	8	40	13	41	18	36
4	47	9	41	14	47	19	47
5	43	10	42	15	45	20	49
R mean = 41.5				Range : 35.5 to 47.5			
Nos of Data within Range n = 14				Total Ri = 1,...+n = 597			
R average = Total R/n = 42.6							
Compressive Strength : Fc = 390 kgf/cm <sup>2</sup>							



**Appendix 5.3.2      Alkalinity Test by 1% Phenolphthalein  
of Existing Krungthep Bridge.**



Alkalinity Test by 1% Phenolphthalein

No.	Date	Location	Depth of Concrete Neutralized
1	16 May '86	Bangkok Side PC-T, Exterior, Upstream	6 mm
2	ditto	Bangkok Side PC-T, Interior	5.5mm
3	ditto	Bangkok Side PC-T, Interior	5 mm
4	ditto	Bangkok Side PC-T, Interior	5 mm
5	ditto	Bangkok Side PC-T, Exterior, Downstream	5.5mm
6	ditto	Pier Wall, Bangkok Side	10.0mm
7	ditto	Bascule Bridge Pier	10.5mm
8	ditto	ditto	18.0mm
9	ditto	Thonburi Side PC-T, Exterior, Upstream	9.0mm
10	ditto	Thonburi Side PC-T, Interior	9.0mm
11	ditto	Thonburi Side PC-T, Interior	5.0mm
12	ditto	Thonburi Side PC-T, Interior	5.0mm
13	ditto	Thonburi Side PC-T, Exterior, Downstream	5.0mm
14	ditto	Pier Wall, Thonburi Side	16.0mm
15	ditto	Pier Wall, Thonburi Side Off-Shore	18.0mm

### Calculation of Constants $k/R^2$

(1) PC T-Beam

No.	x (cm)	$k/R^2 ( = 30/x^2 )$
1	0.6	83.3
2	0.55	99.2
3	0.5	120.0
4	0.5	120.0
5	0.55	99.2
9	0.9	37.0
10	0.9	37.0
11	0.5	120.0
12	0.5	120.0
13	<u>1.0</u>	30.0
Total		865.7

$$k/R^2(\text{Average}) = 86.5$$

(Expected Residual Life as far as neutralization of concrete being concerned : T.

$$T = \frac{k}{R^2} \{ D^2 - d^2 \} = 86.5 \times (3.0^2 - 1.0^2)$$

$$= 690 \text{ years )}$$



(2) Substructure

No.	x (cm)	$k/R^2$ ( = $30/x^2$ )
6	1.0	30.0
7	1.05	27.2
8	1.8	9.3
14	1.6	11.7
15	1.8	9.3
Total		87.5

$$k/R^2 \text{ (average)} = 17.5$$

(Expected Residual Life as far as neutralization of  
concrete being concerned : T

$$T = 17.5 (3.0^2 - 1.8^2) = 100 \text{ years}$$



Appendix 5.4.1

Stresses in Steel Truss Girder of  
Existing Krungthep Bridge.



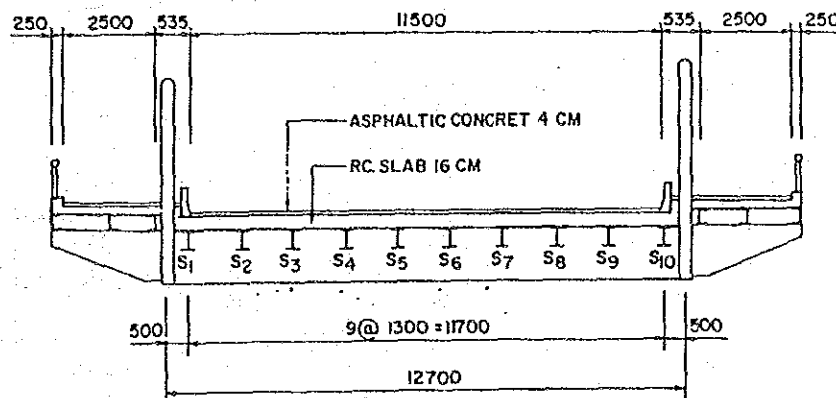
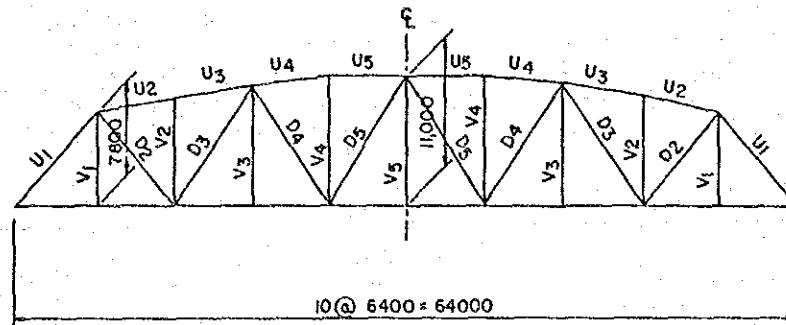
## Superstructure

### (i) Stresses in Steel Truss Girder

Structural calculation was made based on the service load design method (allowable stress design) for the three different live loads as specified in BSI, JRA and AASHTO, respectively.

The longitudinal and cross sections of the existing steel truss girder are as shown below, and the results of structural calculation are shown in Tables A.5.3.3.1 to A.5.3.3.3.

The results show that the existing steel truss girder has an enough capacity to carry either TL-20 or HS-20.



Steel Truss Girder

Table A.5.3.3.1 SUMMARY OF STRESS IN STEEL TRUSS GIRDER (SPAN = 64.0m)  
 BSI Standard : HB-45 units & HA  
 Footpath = 3,485 K Pa (0.355 t.f/m<sup>2</sup>)

		Dead Load Stress		Live Load Stress		Total Stress	Allowable Stress	Stress Excess
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway			
Carriageway Stringer		288*	38	-	2,131	2,457	±1,400 *	75.5%
Footpath Stringer		211*	422	598	-	1,231	±1,400 *	-
Inter-mediate Diaphragm	$\sigma_c$	-393*	-52	-	-1,187	-1,632	-1,400 *	16.6%
	$\sigma_t$	452*	60	-	1,365	1,877	1,400 *	34.1%
End Diaphragm	$\sigma_c$	-265*	-35	-	-1,287	-1,587	-1,400 *	13.4%
	$\sigma_t$	304*	40	-	1,415	1,819	1,400 *	29.9%
Main Truss Upper Chord	U1	-621*	-40	-65	-444	-1,170	-1,139 *	2.7%
	U2	-723*	-44	-73	-502	-1,342	-1,309 *	2.5%
	U3	-719*	-44	-72	-496	-1,331	-1313 *	1.4%
	U4	-756*	-49	-80	-535	-1,420	-1,295 *	9.7%
	U5	-754*	-49	-80	-532	-1,415	-1,296 *	9.2%
Main Truss Lower Chord	L1	700*	43	70	477	1,290	1,400 *	-
	L2	700*	43	70	477	1,200	1,400 *	-
	L3	821*	55	91	609	1,576	1,400 *	12.6%
	L4	821*	55	91	609	1,576	1,400 *	12.6%
	L5	108*	60	99	651	1,718	1,400 *	22.7%
Main Truss Diagonal Member	D2	837*	49	84	626	1,596	1,400 *	14.0%
	D3	-516*	-32	-67	-550	-1,165	-931 *	25.1%
	D4	433*	20	82	748	1,283	1,400 *	-
	D5	-125*	-7	-67	-690	-889	-876 *	1.5%
Main Truss Vertical Member	V1	584*	34	60	884	1,562	1,400 *	11.6%
	V2	202*	12	20	143	377	1,400 *	-
	V3	584*	34	60	884	1,562	1,400 *	11.6%
	V4	258*	15	27	176	476	1,400 *	-
	V5	584*	34	60	884	1,562	1,400 *	11.6%

- Note: 1) The minus sign (-) shows the stress in compression.  
 2) The figures with symbol\* are extracted from the previous study report (1982 March, by JICA).

Table A.5.3.3.2 SUMMARY OF STRESS IN STEEL TRUSS GIRDER (SPAN = 64.0m)

JRA Standard : TL-20

Footpath = 3,430 K.Pa(0.350 t.f/m<sup>2</sup>)

		Dead Load Stress		Live Load Stress		Total Stress	Allowable Stress	Stress Excess
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway			
Carriageway Stringer		288 *	38	-	904	1,230	±1,400 *	
Footpath Stringer		211 *	422	590	-	1,223	±1,400 *	
Inter-mediate Diaphragm	σ <sub>c</sub>	-393 *	- 52	-	-681	-1,126	-1,400 *	
	σ <sub>t</sub>	452 *	60	-	786	1,298	1,400 *	
End Dia-phragm	σ <sub>c</sub>	-256 *	- 35	-	-922	-1,222	-1,400 *	
	σ <sub>t</sub>	304 *	40	-	1,056	1,400	1,400 *	
Main Truss Upper Chord	U1	-621 *	- 40	- 64	-146	-871	-1,139 *	
	U2	-723 *	- 44	- 72	-161	-1,000	-1,309 *	
	U3	-719 *	- 44	- 71	-159	- 993	-1,313 *	Nil
	U4	-756 *	- 49	- 79	-176	-1,060	-1,295 *	
	U5	-754 *	- 49	- 79	-176	-1,050	-1,296 *	
Main Truss Lower Chord	L1	700 *	43	69	155	967	1,400 *	
	L2	700 *	43	69	155	967	1,400 *	
	L3	821 *	55	88	197	1,161	1,400 *	
	L4	821 *	55	88	197	1,161	1,400 *	
	L5	908 *	60	98	219	1,285	1,400 *	
Main Truss Diagonal Member	D2	837 *	49	82	209	1,171	1,400 *	
	D3	-516 *	- 32	- 66	-170	- 784	- 931 *	
	D4	433 *	20	81	210	744	1,400 *	
	D5	-125 *	- 7	- 66	-177	- 375	- 876 *	
Main Truss Vertical Member	V1	584 *	34	59	191	868	1,400 *	
	V2	202 *	12	20	52	286	1,400 *	
	V3	584 *	34	59	191	868	1,400 *	
	V4	258 *	15	26	65	364	1,400 *	
	V5	584 *	34	59	191	868	1,400 *	

- Notes : 1) The minus sign (-) shows the stress in compression.  
 2) The figures with symbol \* are extracted from the previous study report (1982 March, by JICA)

Table A.5.3.3.3 SUMMARY OF STRESS IN STEEL TRUSS GIRDER (SPAN = 64.0m)  
AASHTO Standard : HS-20  
Footpath = 1,980 k.Pa (0.202 t.f/m<sup>2</sup>)

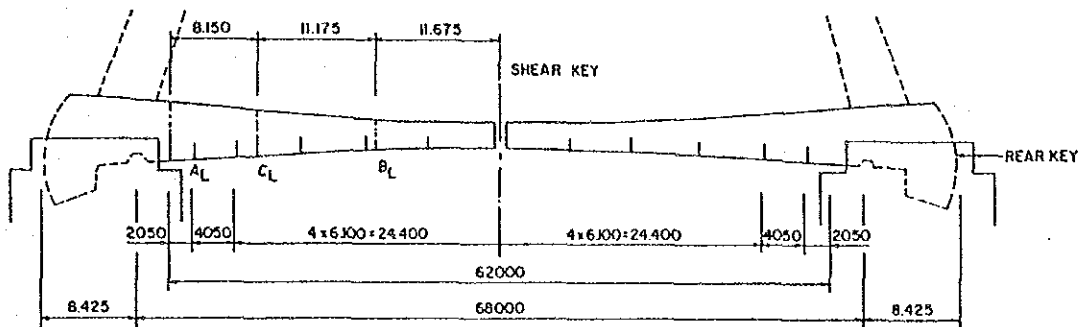
		Dead Load Stress		Live Load Stress		Total Stress	Allowable Stress	Stress Excess
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway			
Carriageway Stringer		288*	38	-	790	1,116	±1,400*	
Footpath Stringer		211*	422	598	-	1,231	±1,400*	
Inter-mediate Diaphragm	$\sigma_c$	-393*	-52	-	-564	-1,009	-1,400*	
	$\sigma_t$	452*	60	-	651	1,163	1,400*	
End Diaphragm	$\sigma_c$	-265*	-35	-	-658	-985	-1,400*	
	$\sigma_t$	304*	40	-	754	1,098	1,400*	
Main Truss Upper Chord	U1	-621*	-40	-37	-148	-846	-1,139*	
	U2	-723*	-44	-41	-164	-972	-1,309*	Nil
	U3	-719*	-44	-41	-163	-967	-1,313*	
	U4	-756*	-49	-46	-182	-1,033	-1,295*	
	U5	-754*	-45	-49	-45	-1,028	-1,296*	
Main Truss Lower Chord	L1	700*	43	40	160	943	1,400*	
	L2	700*	43	40	160	943	1,400*	
	L3	821*	55	58	203	1,130	1,400*	
	L4	821*	55	58	203	1,130	1,400*	
	L5	908*	60	56	224	1,248	1,400*	
Main Truss Diagonal Member	D2	837*	49	47	216	1,149	1,400*	
	D3	-516*	-32	-38	-180	-766	-931*	
	D4	433*	20	47	232	732	1,400*	
	D5	-125*	-7	-38	-203	-373	-876*	
Main Truss Vertical Member	V1	584*	34	34	248	900	1,400*	
	V2	202*	12	11	47	272	1,400*	
	V3	584*	34	34	248	900	1,400*	
	V4	258	15	15	60	348	1,400*	
	V5	584*	34	34	248	900	1,400*	

- Notes : 1) The minus sign (-) shows the stress in compression.  
2) The figures with symbol \* are extracted from the previous study report (1982 March, by JICA)

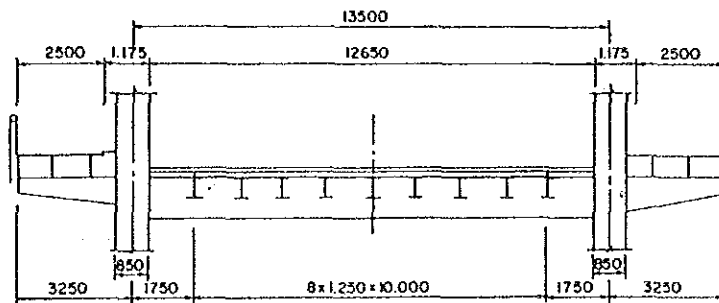


(ii) Stresses in Bascule Bridge

The longitudinal and cross sections of the existing steel bascule bridge are shown below, and the results of structural calculation are as shown in Tables A.5.3.3.4 to A.5.3.3.6 for the standard of BSI, JRA and AASHTO, respectively.



SIDE VIEW



CROSS SECTION

Bascule Bridge

Table A.5.3.3.4 . SUMMARY OF STRESS IN BASCULE BRIDGE ( Span - 68.0m )

BSI Standard : HB-45 units & HA

Footpath Loading = 4.715kPa(0.480t.f/m<sup>2</sup>)

kg.f/cm<sup>2</sup>

		Dead Load Stress			Live Load Stress		Total Stress	Allowable Stress	Stress Excess	
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway					
Carriageway Stringer		126	*	-	-	1,905	2,031	1,400*	45.1 %	
Dia-phragm	$\sigma_c$	-262	*	-	-	-1,243	-1,505	-1,400*	7.5 %	
	$\sigma_t$	322	*	-	-	1,529	1,851	1,400*	32.2 %	
Main Girder	Section B <sub>L</sub>	$\sigma_t$	364	*	-	82	734	1,180	1,400*	-
		$\sigma_c$	-366	*	-	-82	-736	1,184	-1,142*	3.2 %
		$\tau_w$	83	*	-	17	167	267	800*	-
	Section C <sub>L</sub>	$\sigma_t$	530	*	-	135	817	1,482	1,400*	5.9 %
		$\sigma_c$	-536	*	-	-132	-798	-1,466	-1,162*	26.2 %
		$\tau_w$	118	*	-	24	133	275	800*	-
	Section A <sub>L</sub>	$\sigma_t$	640	*	-	177	877	1,694	1,400*	21.0 %
		$\sigma_c$	-646	*	-	-172	-853	-1,671	-1,132*	47.6 %
		$\tau_w$	89	*	-	25	112	226	800*	-

Notes : 1) The minus sign (-) shows the stress in compression.

2) The figures with symbol \* are extracted from the previous study report ( 1982 March, by JICA ).

Table A.5.3.3.5 SUMMARY OF STRESS IN BASCULE BRIDGE (SPAN =68.0m)

JRA Standard : TL 20

Footpath Loading = 3.432 K.pa  
(or 0.350<sub>2</sub> tf/m<sup>2</sup>)  
kgf/cm

		Dead Load Stress		Live Load Stress		Total Stress	Allowable Stress	Stress Excess
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway			
Carriageway Stringer		126*	-	-	862	988	1,400 *	-
Dia-phragm	$\sigma_c$	-262*	-	-	-709	-971	-1,400 *	-
	$\sigma_t$	322*	-	-	818	1,140	1,400 *	-
Main Girder	Section B <sub>L</sub>	$\sigma_t$	-	60	118	542	1,400 *	-
		$\sigma_c$	-	-60	-119	-545	-1,142 *	-
		$\tau_w$	-	13	31	127	800 *	-
	Section C <sub>L</sub>	$\sigma_t$	-	99	240	869	1,400 *	-
		$\sigma_c$	-	-96	-234	-866	-1,162 *	-
		$\tau_w$	-	18	41	177	800 *	-
	Section A <sub>L</sub>	$\sigma_t$	-	129	305	1,074	1,400 *	-
		$\sigma_c$	-	-126	-297	-1,069	-1,132 *	-
		$\tau_w$	-	19	42	150	800 *	-

Notes : 1) The minus sign (-) shows the compressive stress.

2) The figures with symbol \* are extracted from the previous study report (1982 March, by JICA).

Table A.5.3.3.6

## SUMMARY OF STRESS IN BASCULE BRIDGE (SPAN = 68.0m)

AASHTO : HS-20

Footpath Loading = 2.544 K.pa  
(or 0.259 tf/m<sup>2</sup>)kgf/cm<sup>2</sup>

			Dead Load Stress		Live Load Stress		Total Stress	Allowable Stress	Stress Excess
			Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway			
Carriageway Stringer			126*	-	-	753	879	1,400 *	-
Dia-phragm	$\sigma_c$		-262*	-	-	-579	-841	-1,400 *	-
	$\sigma_t$		322*	-	-	668	990	1,400 *	-
Main Girder	Section B <sub>L</sub>	$\sigma_t$	364*	-	44	198	606	1,400 *	-
		$\sigma_c$	-366*	-	-44	-198	-564	-1,142 *	-
		$\tau_w$	83*	-	9	37	129	800 *	-
	Section C <sub>L</sub>	$\sigma_t$	530*	-	73	259	862	1,400 *	-
		$\sigma_c$	-536*	-	-71	-253	-860	-1,162 *	-
		$\tau_w$	118*	-	13	43	174	800 *	-
	Section A <sub>L</sub>	$\sigma_t$	640*	-	95	315	1,050	1,400 *	-
		$\sigma_c$	-646*	-	-93	-306	-1,045	-1,132 *	-
		$\tau_w$	89*	-	14	42	145	800 *	-

Notes : 1) The minus sign (-) shows the compressive stress.

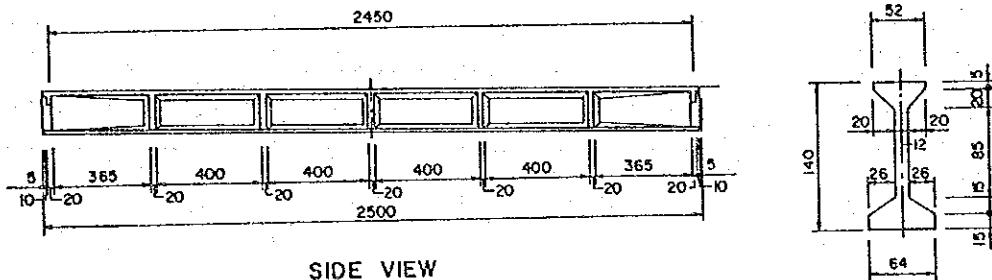
2) The figures with symbol \* are extracted from the previous study report (1982 March, by JICA)

(iii) Stresses in PC Composite Beam

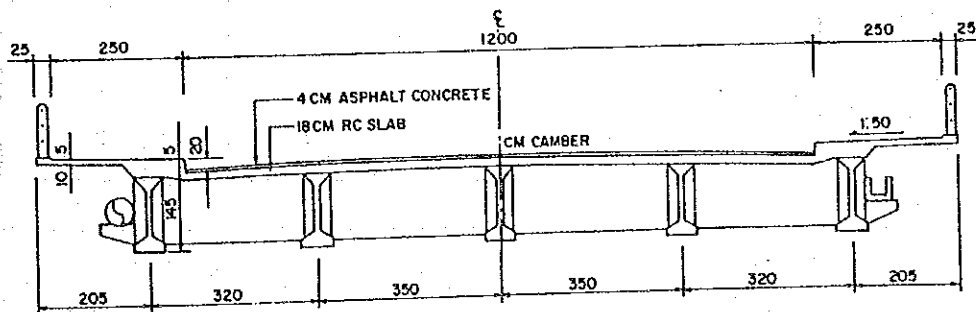
The longitudinal and cross sections of the existing PC composite beams are shown below:

The results of stress calculation on the main beam shown in Table A.5.3.3.7.

The results show that the existing PC composite beams have an enough capacity to carry either TL-20 or HS-20 while HB-45 unit is not applicable.



SIDE VIEW



CROSS SECTION

PC Composite Beam

Table A.5.3.3.7 SUMMARY OF STRESS IN PC COMPOSIT BEAM  
( Span = 24.5m )

	Bending Moment(t.f.m)	Axial Force(t.f)	Stress (kg.f/cm <sup>2</sup> )	
			Top Fiber	Bottom Fiber
1. Prestress (9 cables)	124.5 *	220.0 *	46.4	-149.3
2. Dead Weight of T-Beam	72.6 *	-	-63.9	50.2
3. Weight of Diaphragm	27.0 *	-	-23.8	18.7
4. Prestress (7 cables)	84.8 *	164.5 *	27.4	-106.0
5. Roadway Slab (18cm)	113.1	-	-98.0	75.4
6. Pavement ( 4cm)	17.3	-	-1.5	7.7
7. Subtotal (7=1+2+...+6)	-	-	-113.4	-103.3
8. BSI: HB-45unit	542.2	-	-47.7	-241.3
Live Load JRA: TL-20	209.4	-	-18.4	93.2
AASHTO: HS-20	166.5	-	-14.6	74.1
9. TOTAL BSI: HB-45unit	-	-	-161.1	138.0
JRA: TL-20	-	-	-131.8	-10.1
AASHTO HS-20	-	-	-128.0	-29.2
Allowable Stress	-	-	-135.0	0

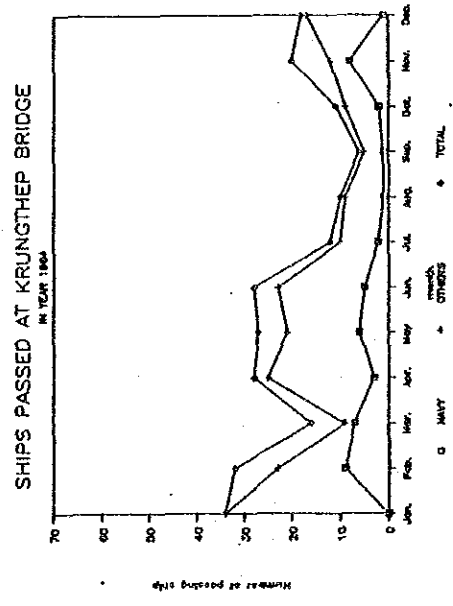
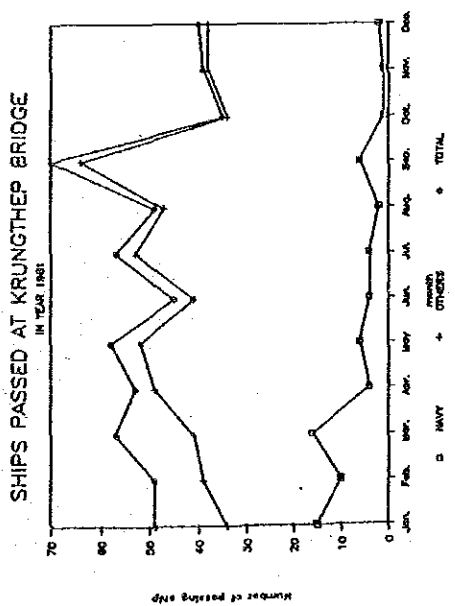
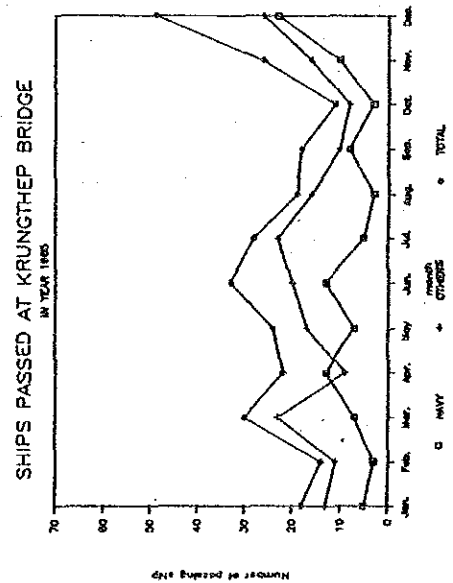
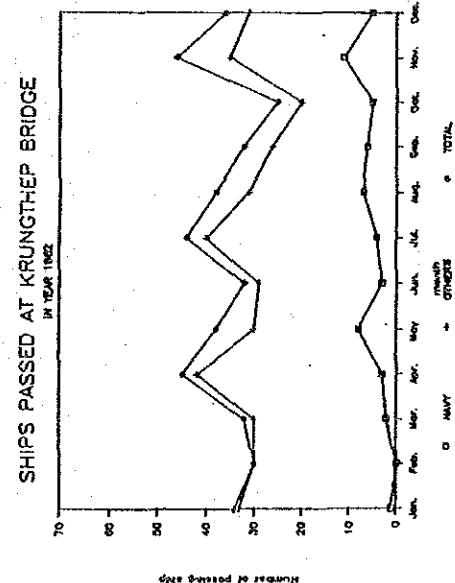
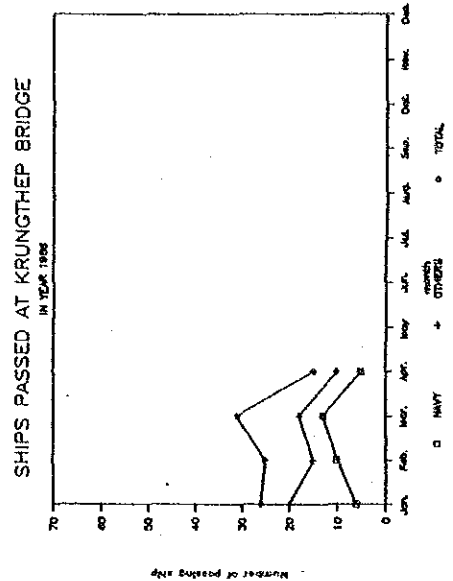
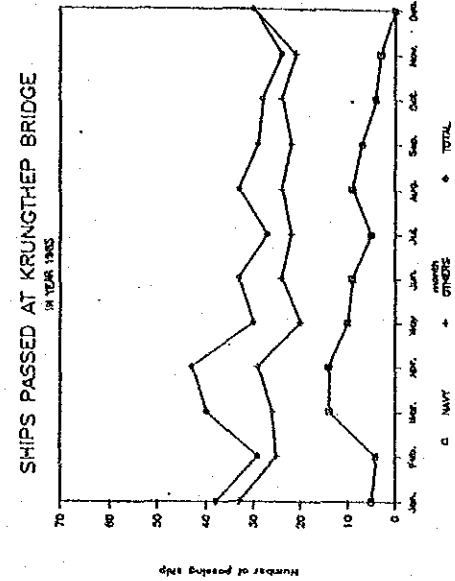
- Notes:
- 1) The minus sign(-) of stress shows that in compression.
  - 2) The figures with symbol \* are extracted from the previous study report (march 1982, by JICA).

#### Appendix 6.2.1

#### Ships Passed at Krungthep Bridge









#### Appendix 6.4.1

#### Compensation Cost for River Facilities



## Compensation Costs for River Facilities

### 1) Bangkok Dock

As reported already in the Progress Report (I) May 1986, the compensation cost of the Bangkok Dock was estimated at about Baht 500 million with the following breakdown:

#### (a) Construction of New Dock

i)	New dock construction cost M Baht 360	Baht	360	million
ii)	Land acquisition cost in downstream	Baht	20	million
iii)	Infrastructures for Dock operation	Baht	45	million

#### (b) Dismantling of Bangkok Dock and leveling works

Baht 75 million

Total Baht 500 million

While the land value of the prime river front property along New Road was estimated at the minimum about Baht 525 million based on the unit land price of Baht 60,000 per square wah. for 35,000 sq.m. However, this unit price will be reviewed later when the detailed evaluation is required.

### 2) Other River Facilities

As discussed in Chapter 6, the Study Team has investigated other river facilities which are located upstream of the Krungthep bridge but downstream of the Sathorn bridge and compensation costs for their removal were approximately estimated at Baht 85 million as shown below:

A.	Harin Ship Building Co., Ltd. Removal of 2-slipways only, headquarters will remain as it is.	Baht	40	million
B.	B.L.L. Shipyard Co., Ltd.	Baht	30	million
C.	Show Wanakit Co., Ltd.	Baht	10	million
D.	Others,	Baht	5	million



THE BANGKOK DOCK CO. LTD.

SHOWING LAY-OUT PLAN

SCALE 11500

**DRAWING NO.** **DATE** 25/5/00

DRAWN BY: VERONICA V. CHECK

### Appendix 8.3.1      Movable Bridge Mechanisms





### Movable Bridge Mechanisms

There are three principal types of movable bridge, bascule, swing, and lift type. The following describes characteristic features of each type.

#### 1) Bascule Type

- Two types of bascule girders are usually used, such as single-leaf and double-leaf (same as Krungthep bridge) girders. In case of double-leaf girders, the longest spans constructed to date is 96.8 meters over the Bay of Cadiz, Spain. Large double-leaf girders have large deflection at the center key.
- The size of substructure is normally large in order to provide chambers accommodating counter weights. Elimination of the chamber is possible, such as in the case of Bang Pakong bridge, but results in aesthetically poor appearance. Such large submerged structure causes a high degree of river flow disturbance.
- The center key must be released and re-fixed at every opening operation. Together with the joint, this requires frequent maintenance.
- The operation of this type requires double of power needed for the swing type. The length of time required for the opening and the closing operations is relatively small. Three kinds of mechanism are available, gear with electric power, gear with diesel power, and hydraulic system.

#### 2) Swing Type

- The largest of this type constructed to date is the one in the Suez canal with the total (both sides of the rotating center) length of 158.4 meters.
- When open, the girder is parallel to the navigation course, exposed to the direct collision with the vessel.
- The time required for opening and closing operations is longer than that for the bascule type, 30 to 40 minutes at a time.
- Maintenance costs are lower than the others.

### 3) Lift Type

- The largest of this type constructed to date is the one in the James river in the U.S.A. with the total length of 126.6 meters.
- Heavy towers containing counter weights calls for the highest construction cost among the three types.
- It limits the height of vessel.
- The time required for the opening and closing operations is shortest among the three, 15 to 20 minutes at a time.
- Maintenance costs are at the same level as the bascule type.

#### Appendix 8.4.1 Preliminary Cost Estimate

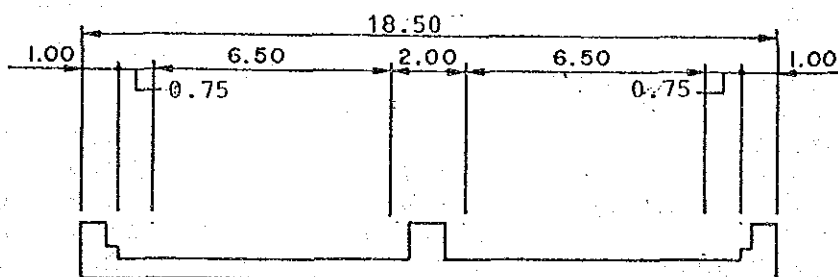


### Standard Cross Sections of New Bridges

Referring to the Geometric Design Standards of AASHTO, JRA and DOH, the Study Team determined the standard cross sections of the New Krungthep bridge as follows:

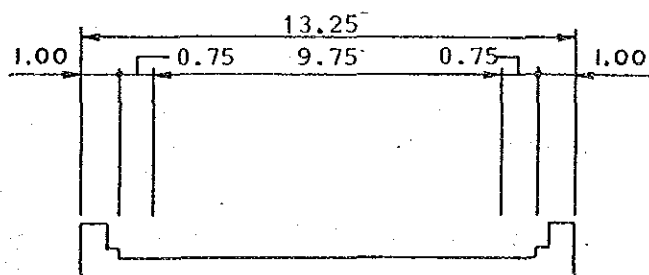
#### 1) Twoway 4-lane bridge

- Carriage way	2 x 2 x 3.25	=	13.0	m
- Medium strip			2.0	m
- Side strip	2 x 0.75	=	1.5	m
- Curb to hand hail	2 x 1.0	=	2.0	m



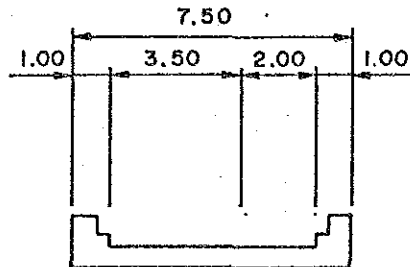
#### 2) Oneway 3-lane bridge

- Carriage way	3 x 3.25	=	9.75	m
- Side strip	2 x 0.75	=	1.5	m
- Curb to Hand Rail	2 x 1.0	=	2.0	m



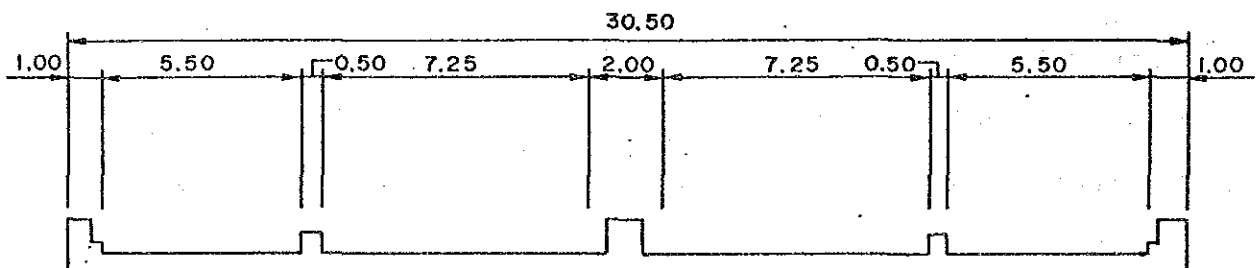
### 3) Rampway

- Carriage way	1 x 3.5	=	3.50	m
- Side strip	1 x 2.0	=	2.0	m
- Curb to Hand Rail	2 x 1.0	=	2.0	m

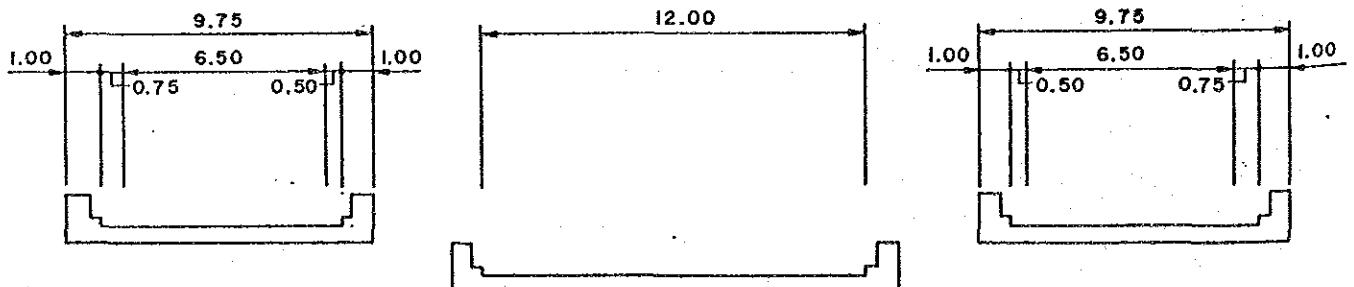


### 4) Twoway 4-lane with Rampways

- Medium strip			2.0	m
- Main carriage way	2 x 7.25	=	14.5	m
- Side separation strip	2 x 0.5	=	1.0	m
- Rampway	2 x 5.5	=	11.0	m
- Curb to Hand Rail	2 x 1.0	=	2.0	m



### 5) Separately independent 2-2 lanes bridges



### Girder Depth

The girder depth is to be decided in preliminary design to be commenced in the middle of November 1986. The Study Team roughly estimated the relationship between the girder depth and the bridge span by type of bridge as follows:

- 1) PC girder by incremental launching method for Alternative D

$$d/L = 0.06 - 0.05$$

$$L = 80 \text{ m} \quad d = 4.8 - 4.0 \text{ m}$$

Where

L = Span Length (m)

d = Girder Depth (m)

- 2) Movable bridge for Alternative E

Bascule type  $d/L = 0.070$

$$L = 85 \text{ m} \quad d = 5.95 \text{ m} \dots\dots E-1 \text{ and } E-3$$

Swing type  $d/L = 0.035$

$$L = 80 \text{ m} \quad d = 2.8 \text{ m} \dots\dots E-2$$

- 3) Steel Box girder in side spans of Alternative E

$$d/L = 0.05$$

$$L = 65 \text{ m} \quad d = 3.25 \text{ m}$$

- 4) PC Box girder by balanced cantilever erection method.

$$d/L = 0.0165 \quad \text{at crown}$$

$$d/L = 0.06 \quad \text{at pier}$$

$$L = 220 \text{ m} \quad d = 2.2 \text{ m}$$

### Construction Methods

- 1) General Construction Methods

The requirements for the new bridge, necessitate the following general construction methods:

#### Alternative D

- Foundation works : Cast in situ concrete pile method such as reverse circulation drill method.
- Temporary cofferdam : Not required specially
- Superstructures : Main bridge over the river can be constructed with PC continuous box girder by the incremental launching method.

The PC continuous box girder is selected based on the reasons listed below:

- a) PC girder with uniform girder depth is cheaper than steel box girder, which can keep NC of 7.5 m in main bridge spans.
- b) The segment casting yard is available in the adjacent land in the Bangkok side.

#### Alternative E

Plan E-1 and E-2 can be constructed by the following general methods:

- Foundation works : Cast in situ concrete pile method such as reverse circulation drill method.
- Temporary cofferdam : In case of plan E-1, interlocked steel pipe type cofferdams are required for the construction of central two piers to support the bascule chambers.
- Superstructures : In case of plan E-1, in addition to the main movable bascule girders, the main side spans will be constructed by steel box girders.
- Superstructures : In case of E-2, the main movable bridge is constructed with cable stayed steel girders of double span length of main navigational channel span of about 70 m.



The plan E-3 which includes separate new bridges in both up and down stream sides is discussed in the succeeding Section.

#### Alternative G

The following are general methods for this type of bridge:

- |                     |   |   |
|---------------------|---|---|
| Foundation          | : | Either types of concrete pile casted in situ or open caisson.   |
| Temporary cofferdam | : | In case of open caisson, the locations of main piers are situated close to each bank, so that double sheet pile type cofferdam filled with sand is applicable. In the case of concrete pile casted in situ, it is not required. |
| Superstructure      | : | PC box type by ballanced cantilever erection method is generally applicable. The effect of variable girder depth to NC is negligible due to the high piers.   |

#### 2) Special Construction Method for Plan E-3

According to the request on the channel length of 60 m by Pilot Division of HD, the distance between the new separated bridges in up and down stream sides shall be kept in about 60 m applying a special construction method called "Close Construction", which allow it to be constructed closely to the existing structure within 20 m.

The plan is shown in Fig. A.8.4.1.1 The "Close Construction" is generally not recommended in order to keep the foundation base of the existing structure safe. However, the Study Team tried to make the advantages clear as summarized below:

- |                  |   |  |
|------------------|---|--|
| Foundation works | : | Bored steel pipe pile foundation is suitable, due to minimum disturbance to the existing foundation. |
|------------------|---|--|

Temporary cofferdam : In addition to the interlocked steel pipe pile type cofferdam for large chamber type pier, another independent large scale steel piles shall be driven in between the existing and new substructure to prevent the failure of existing foundation caused by new structure's construction.

After completion of new substructure, this steel piles shall be demolished to avoid ill effects to the scouring action.

Superstructure : Same as the plan E-1.

### 3) Construction Periods

The key factor in determining the construction period of a bridge project is the concrete volume in the field, not in the factory, because the capacity of the batcher plant is the dominant factor.

In the case of Nonthaburi & Pathumthani Bridges Project, the construction period was 24 months with placing concrete of 26,000 cub.m.

The Wat Sai Steel Cable Stayed Bridge Project, it takes 36 months with the concrete volume of 32,000 cub.m in the field, and the Bangkok Approach Bridges Project, it takes 30 months for the placing of concrete of 31,000 cub.m in the site.

Therefore, it can be said that the maximum concrete volume per month is about 1,000 cub.m per one batcher plant.

The construction periods of each Alternative could be estimated by the above concept as below:

<u>Alternative</u>	<u>Concrete Volume</u>	<u>Construction Periods</u>
D	25,000 cub.m	24 months
E	20,000 cub.m	24 months
G	75,000 cub.m	36 months

The above concrete volumes were approximately estimated by counting the volume to be placed in the site only. In the case of the Alternative G, two (s) batcher plants shall be operated from both Bangkok and Thonburi sides unless otherwise divided into two packaged projects.

In the case of Alternative E, cofferdam works in the river and steel fabrication works in the factory were taken into consideration.

### Land Acquisition and Other Compensation

#### 1) Land Acquisition

Land acquisition has been a big problem for the Government as elsewhere in the world. The legal basis for the Government to acquire the right of way for public infrastructure projects has been strengthened in recent years but the enforcement sometimes still is very difficult. Therefore, the degree of difficulty in land acquisition was assessed for the evaluation of each alternative as shown in main volume.

#### 2) Compensation for Moving River Facilities

At the moment there is no legal grounds to impose private operators of shipyards, slipways and jetties to move downstream so that a low fixed bridge could be built. Properties within the right of way could be expropriated by law, at least in principle, and the Bangkok Dock, which is a government enterprise, could be moved by a cabinet decision upon request by PWD. Private operators, however, can not be forced to move for the sake of a public infrastructure project located in distance unless a new law be enacted. Under such circumstances negotiations with the private operators could be easily prolonged.

### Project Costs

Preliminary construction cost estimates were first presented in Table 7.3.1 in page 7-15 of the Progress Report (II). Further refinements were made reflecting subsequent considerations.

## 1 Construction Costs

### 1) Bridge Cost Estimate

Taking into account more detailed considerations PC box girder by launching method of the Alternative D, steel box girder for side spans of the Alternative E and bascule type bridge of Alternative E, bridge construction costs were re-estimated as shown below:

a. PC Box girder by balanced cantilevered method with high piers	Baht 50,000 per sq.m
b. PC Box girder by launching method with low piers	Baht 45,000 per sq.m
c. Approach PC bridge to the high main bridge	Baht 25,000 per sq.m
d. Approach PC bridge to the low main bridge	Baht 20,000 per sq.m
e. Approach ramp RC bridge	Baht 12,000 per sq.m
f. Movable swing steel bridge	Baht 115,000 per sq.m
g. Movable bascule steel bridge	Baht 125,000 per sq.m
h. Middle span main steel box girder	Baht 70,000 per sq.m

### 2) Other Cost Estimates

Other unit costs such as those for road works, miscellaneous work, contingency and engineering were kept unchanged at the level reported in the Progress Report (II). However, costs for asphalt pavement for bridge surface and concrete pavement for road surface were newly added.

### 3) Costs for Scouring Protection

Placing of stones on the river bed is an effective protection method against scouring. The cost for the work was estimated at 30 to 50 million Baht even in the case of Alternatives D or E. However, this work could be avoided by designing substructures while neglecting the supporting power of soil above the scour level. Therefore, in this cost estimation, the protection cost against scouring was ignored considering that it could be included in the miscellaneous works cost.

### 4) Cost of Strengthening the Existing Bridge

In case of the plan D-3, the cost of strengthening the existing bridge was calculated as summarized below:

- Dismantling of structural members and reinforcing of steel truss bridge	Baht 86 million
- Reinforcing of bascule bridge	Baht 4 million
- Dismantling of PC approach bridge	Baht 3 million
	-----
Total	Baht 93 million

The cost of reconstructing PC approach bridge was estimated with the proposed plan.

## 2. Compensation Cost

### 1) Land Acquisition and Building Removal

Based on the results of the building conditions survey, the Study Team calculated the lands area to be acquired and counted the number of buildings by floor spaces.

It is not possible to avoid either of the two Chinese Shrines at the foot of the Krungthep bridge on the Thonburi side except in the cases of the plan G-1, and E-2.

## 2) Compensation Cost for River Facilities

For the case of Alternative D, the Study Team estimated compensation costs for moving the river facilities including Bangkok Dock as follows:

- Compensation cost for Bangkok Dock Baht 500 million
- Compensation cost for other river facilities such as Harin 2 slipway, BLL slipway Shaw Wanakit and others Baht 85 million
- Revenue by selling the land of the existing Bangkok Dock Baht 525 million

The Study Team interviewed the General Manager of Supakarn condominium project to confirm the unit land price of the existing Bangkok Dock site. Mr. Somphot Piyaoui said, if the land can be sold, price would be between the maximum of Baht 80,000 per sq.wa and the minimum of Baht 60,000 per sq.wa, seeing the Dock from his office. His lowest estimate of Baht 60,000 per sq.wa was just the same with that estimated by the Study Team.

It is likely, therefore that the sale of the land of the existing Bangkok Dock would more than compensate the cost of moving it downstream. However, such a sale may take place only several years or longer after the moving, whereas the moving must be preceded by the preparation and the construction at the new site. This gap in time can not be determined. Thus, for the sake of keeping the estimates conservative the amount only realizable by the sale of the existing site was not included in the financial and economic evaluation in this Report.

## 3. Traffic Loss

In the case of Alternative E, two (2) kinds of movable bridge were examined of their operating costs. Their operating costs and economic losses while bridge opening operations were estimated based on the general information and the data obtained from PWD and were reported in the reported in the Progress Report (II) as shown in the next page.

Preliminary cost estimates were calculated for the following items for relevant alternatives.

- Loss to traffic due to opening of movable bridge
- Loss to traffic during construction period

For the above items computer simulation runs were carried out for the target year of 1991 against a network without the Krungthep bridge. The difference in the total cost of vehicle operation and time value during a half hour period in off-peak hours between the cases of with and without the bridge was taken as the loss to traffic per day. The difference in the total cost for a two year period was taken as the loss to traffic. Estimates are shown in Table below.

Estimation of Loss Due to Closure of Existing Bridge, 1991

		Total Veh.-hrs. (Veh.-Hr/Hr)	(Baht/Hr)	Total Cost (Baht/Hr)
Morning Peak	with Bridge	99072	7867418	
	<u>without Bridge</u>	<u>99574</u>	<u>7884609</u>	
	A. Difference	502	17191	48466
Off-peak	with Bridge	57612	6001414	
	<u>without Bridge</u>	<u>59352</u>	<u>6016112</u>	
	B. Difference	1740	14698	118924

Total closure for 1 year

(5 x A + 15 x B) x 365

740 million

(VOC only 112 million)

Half hour closure every day during off-peak

VOC only 2.68

Time 19.02

Total 21.70 million

Note: Value of time 62.3 Baht/Hr/PCU (peak hour)  
59.9 Baht/Hr/PCU (off-peak)

Cost of vehicle operation and time values were developed in terms of economic cost.





Appendix 9.2.1      Wind Probability



# PROBABILITY ANALYSIS OF WIND VELOCITY

## Annual Maximum Wind Velocity Records

Year	Velocity (knot)	Direction
1951	27	S
2	56	E
3	43	ENE
4	40	WSW
5	43	NW
6	45	NNW
7	51	SW
8	44	SSW
9	52	E
1960	35	ESE
1	48	ENE
2	52	ESE
3	41	W
4	40	SE
5	41	S
6	40	W
7	42	NNW
8	37	ESE
9	36	WSW

Year	Velocity (knot)	Direction
1970	41	N
1	33	WSW
2	42	W
3	25	W
4	27	NW
5	40	NE
6	32	SE
7	28	SW
8	32	NE
9	36	SW
1980	42	WSW
1	32	WSW
2	40	SW
3	38	ESE
4	35	NNE
5	32	W

Probability analysis was made by the Study Team using a) Gumbel's method, b) Order Probability method and c) Gumbel-Weibulls' method. The results are summarized below:

Return Period (Year)	Maximum Wind Velocity (knot)		
	a)Gumbel	b)Order Probability	c)Gumbel-Weibull
5	45.3	45.6	45.1
10	50.1	49.6	49.0
30	57.4	54.8	54.2
50	60.8	56.9	56.3
100	65.3	59.7	58.9

The maximum value for 100 year probability was obtained by Gumbel's method (above a) ). Hence, the design wind velocity  $V_D$  is determined as:

$$V_{100} = 65.3 \text{ knots} = 120.9 \text{ km/h} = 75.2 \text{ mph, say } 80 \text{ mph}$$

$$V_D = 80 \text{ mph ( equivalent to } 130 \text{ km/h ) --- same as Sathorn Br.}$$



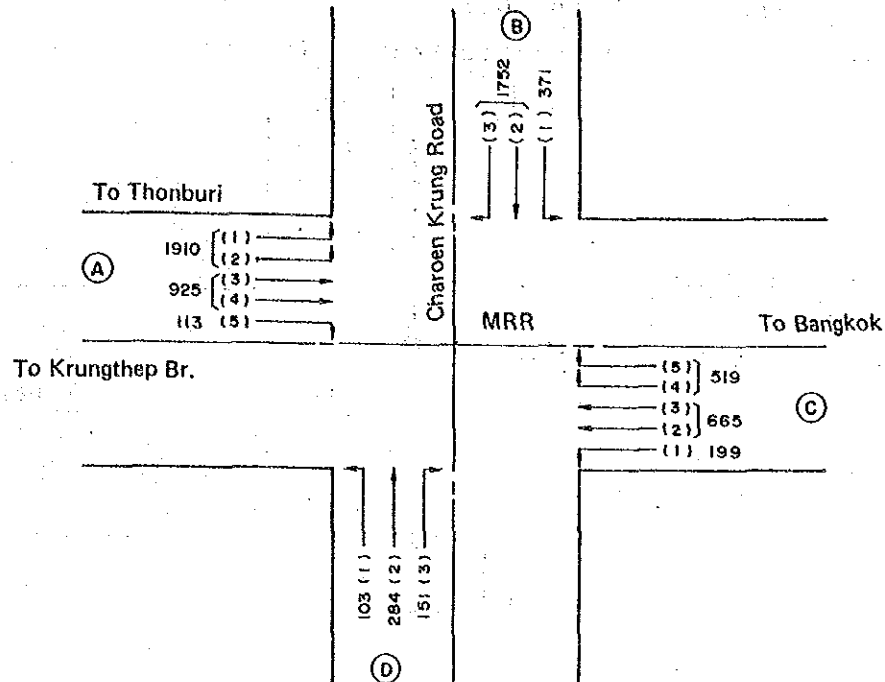
Appendix 9.4.1

Calculation of Saturation Degree for  
Examination of Intersection Type



Traffic Saturation Degree of Intersections  
Intersection "A" in 2001

a) Forecasted Traffic Volume (PCU/H)



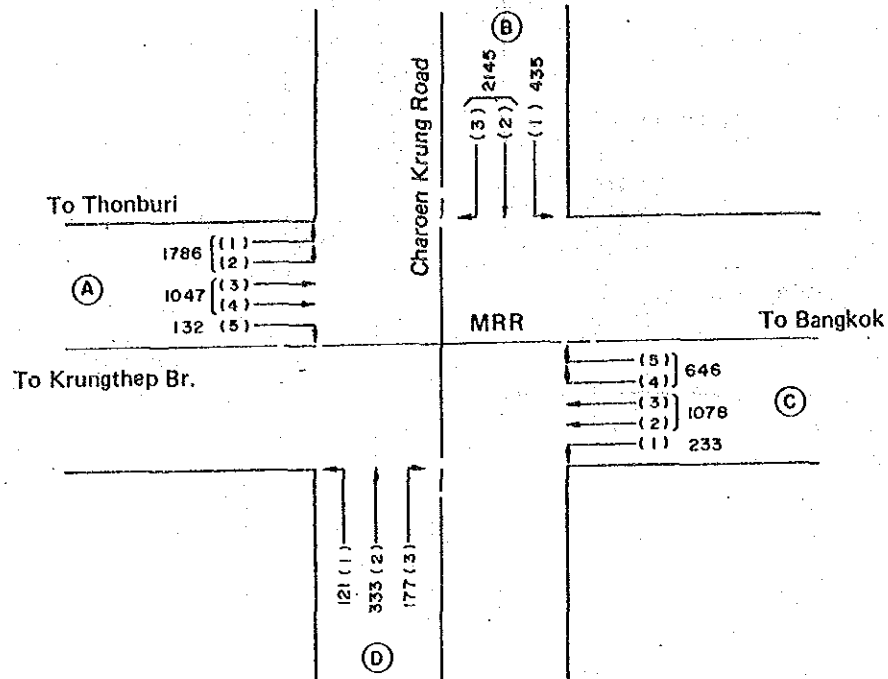
b) Calculation of Traffic Saturation Degree

Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1)	2000	0.48	
	(2)	2000	0.48	
	(3)	2200	0.21	
	(4)	2200	0.21	
	(5)	2000	0.06	
B	(1)	2000	0.19	
	(2)	2000	0.44	
	(3)	2000	0.44	
C	(1)	2000	0.10	Phase1 0.48
	(2)	2200	0.15	Phase2 0.13
	(3)	2200	0.15	Phase3 0.44
	(4)	2000	0.13	Phase4 0.13
	(5)	2000	0.13	
				1.18
D	(1)	2000	0.05	
	(2)	2200	0.13	
	(3)	2000	0.08	
Total			1.18 > 1.0	

# Intersection "A" in 2011

## a) Forecasted Traffic Volume (PCU/H)



## b) Calculation of Traffic Saturation Degree

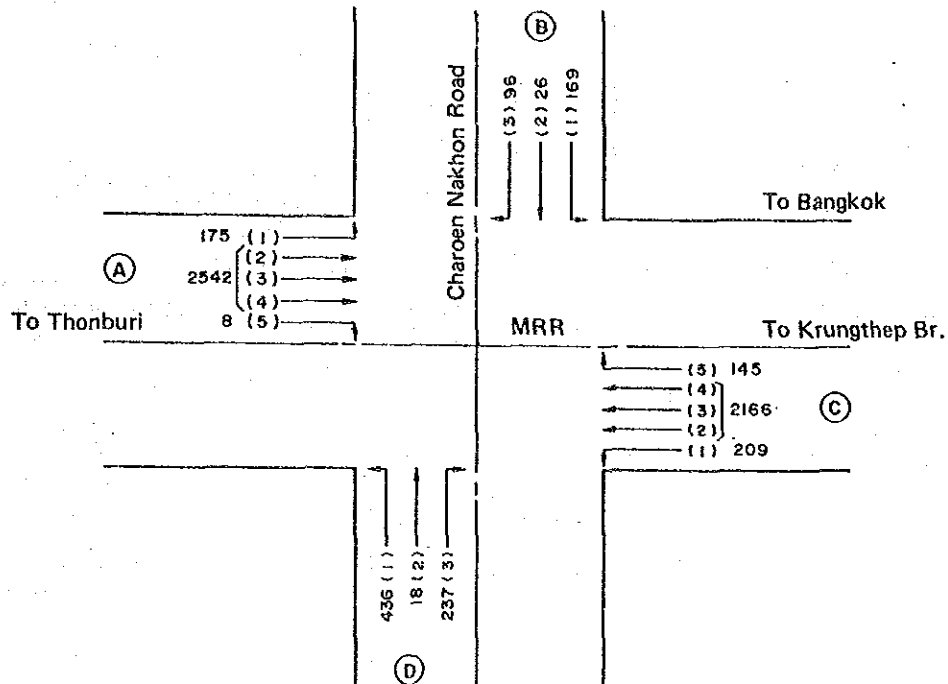
### Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1)	893	0.45	
	(2)	893	0.45	
	(3)	524	0.24	
	(4)	524	0.24	
	(5)	132	0.07	
B	(1)	435	0.23	
	(2)	1073	0.55	
	(3)	1073	0.55	
C	(1)	233	0.12	Phase1 0.45
	(2)	539	0.25	Phase1 0.16
	(3)	539	0.25	Phase3 0.55
	(4)	323	0.16	Phase4 0.15
	(5)	323	0.16	
				1.31
D	(1)	121	0.06	
	(2)	333	0.15	
	(3)	177	0.09	
Total				1.31 > 1.0



# Intersection "B" in 2001

## a) Forecasted Traffic Volume (PCU/H)



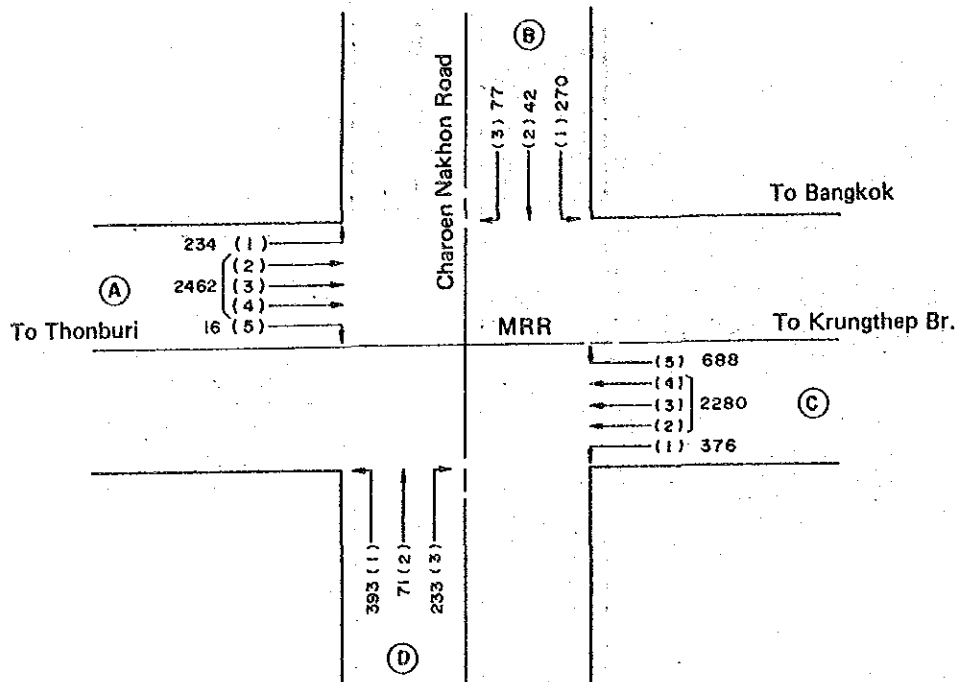
## b) Calculation of Traffic Saturation Degree

### Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1)	175	0.09	
	(2)	847	0.39	
	(3)	847	0.39	
	(4)	847	0.39	
	(5)	8	0.01	
B	(1)	169	0.08	
	(2)	26	0.01	
	(3)	96	0.05	
C	(1)	209	0.10	Phase1 0.39
	(2)	722	0.33	Phase2 0.07
	(3)	722	0.33	Phase3 0.08
	(4)	722	0.33	Phase4 0.22
	(5)	145	0.07	
				0.76
D	(1)	436	0.22	
	(2)	18	0.01	
	(3)	237	0.12	
Total				0.76 < 1.0

# Intersection "B" in 2011

## a) Forecasted Traffic Volume (PCU/H)



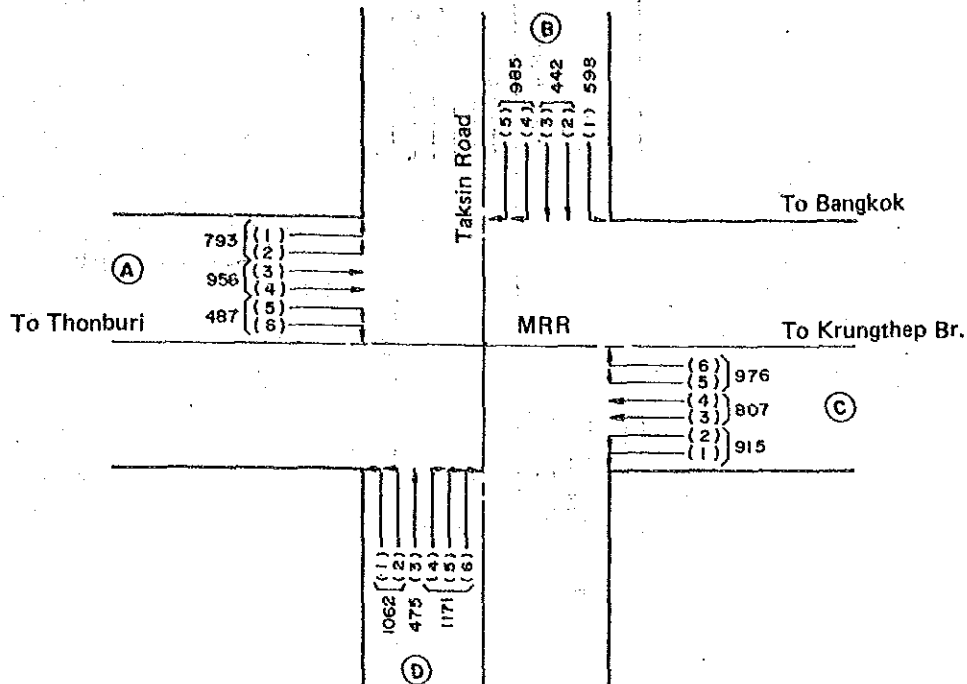
## b) Calculation of Traffic Saturation Degree

Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree		
A	(1)	2000	234	0.12		
	(2)	2200	821	0.37		
	(3)	2200	821	0.37		
	(4)	2200	821	0.37		
	(5)	2000	16	0.01		
B	(1)	2000	270	0.14		
	(2)	2200	42	0.02		
	(3)	2000	77	0.04		
C	(1)	2000	376	0.19	Phase1	0.37
	(2)	2200	760	0.35	Phase2	0.34
	(3)	2200	760	0.35	Phase3	0.20
	(4)	2200	760	0.35	Phase4	0.14
	(5)	2000	688	0.34		
1.05						
D	(1)	2000	393	0.20		
	(2)	2200	71	0.03		
	(3)	2000	233	0.14		
Total						1.05 > 1.0

# Intersection "C" in 2001

## a) Forecasted Traffic Volume (PCU/H)



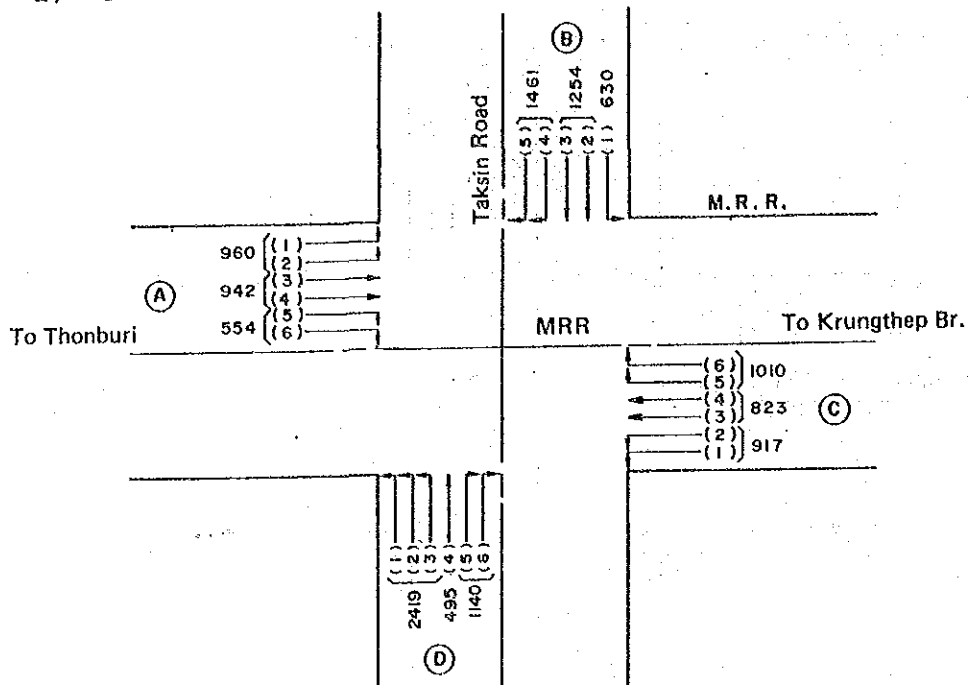
## b) Calculation of Traffic Saturation Degree

### Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A ①	(1) 2000	397	0.20	
	(2) 2000	397	0.20	
	(3) 2200	478	0.22	
	(4) 2200	478	0.22	
	(5) 2000	244	0.12	
	(6) 2000	244	0.12	
B ③ ④	(1) 2000	299	0.15	
	(2) 2000	299	0.15	
	(3) 2200	442	0.20	
	(4) 2000	493	0.25	
	(5) 2000	493	0.25	
C ②	(1) 2000	458	0.23	Phase1 0.22
	(2) 2000	458	0.23	Phase2 0.25
	(3) 2200	404	0.18	Phase3 0.22
	(4) 2200	404	0.18	Phase4 0.27
	(5) 2000	488	0.25	
	(6) 2000	488	0.25	0.96
D ④ ③	(1) 2000	531	0.27	
	(2) 2000	531	0.27	
	(3) 2200	475	0.22	
	(4) 2200	390	0.20	
	(5) 2000	390	0.20	
	(6) 2000	390	0.20	
Total				0.96 < 1.0

# Intersection "C" in 2011

## a) Forecasted Traffic Volume (PCH/H)



## b) Calculation of Traffic Saturation Degree

### Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1) 2000	480	0.24	
	(2) 2000	480	0.24	
	(3) 2200	471	0.21	
	(4) 2200	471	0.21	
	(5) 2000	277	0.14	
	(6) 2000	277	0.14	
B	(1) 2000	630	0.32	
	(2) 2200	627	0.29	
	(3) 2200	627	0.29	
	(4) 2000	731	0.37	
	(5) 2000	731	0.37	
C	(1) 2000	459	0.23	Phase1 0.24
	(2) 2000	459	0.23	Phase2 0.25
	(3) 2200	412	0.19	Phase3 0.40
	(4) 2200	412	0.19	Phase4 0.37
	(5) 2000	505	0.25	1.26
	(6) 2000	505	0.25	
D	(1) 2000	806	0.40	
	(2) 2000	806	0.40	
	(3) 2000	806	0.40	
	(4) 2200	495	0.23	
	(5) 2000	570	0.23	
	(6) 2000	570	0.23	
Total			1.26	1.0

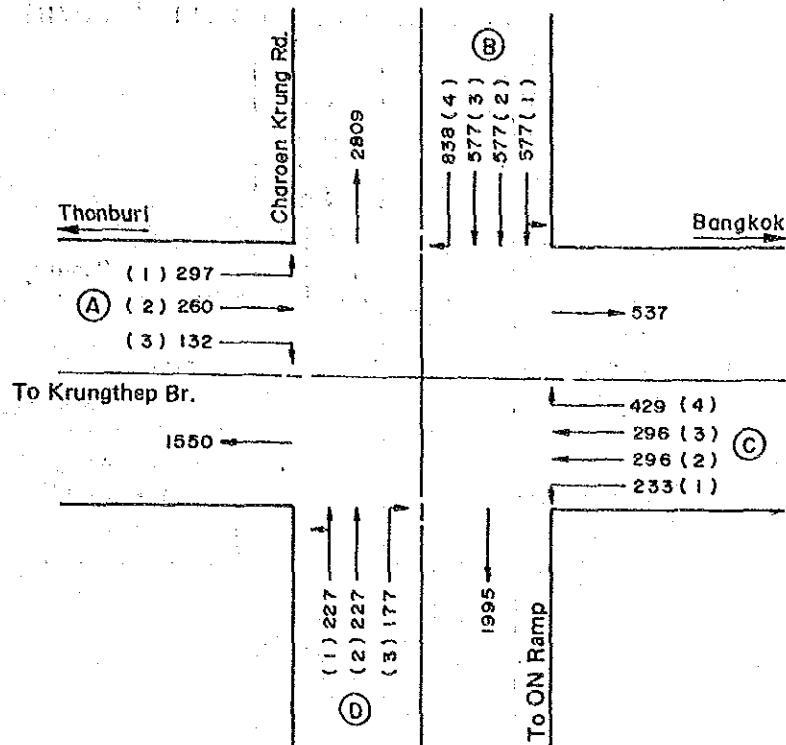
Appendix 9.4.2

Calculation of Saturation Degree for Examination  
of At-Grade Intersection (A, B and C Intersection).



# "A" Intersection

## a) Forecasted Traffic Volume in 2011 (PCU/H)



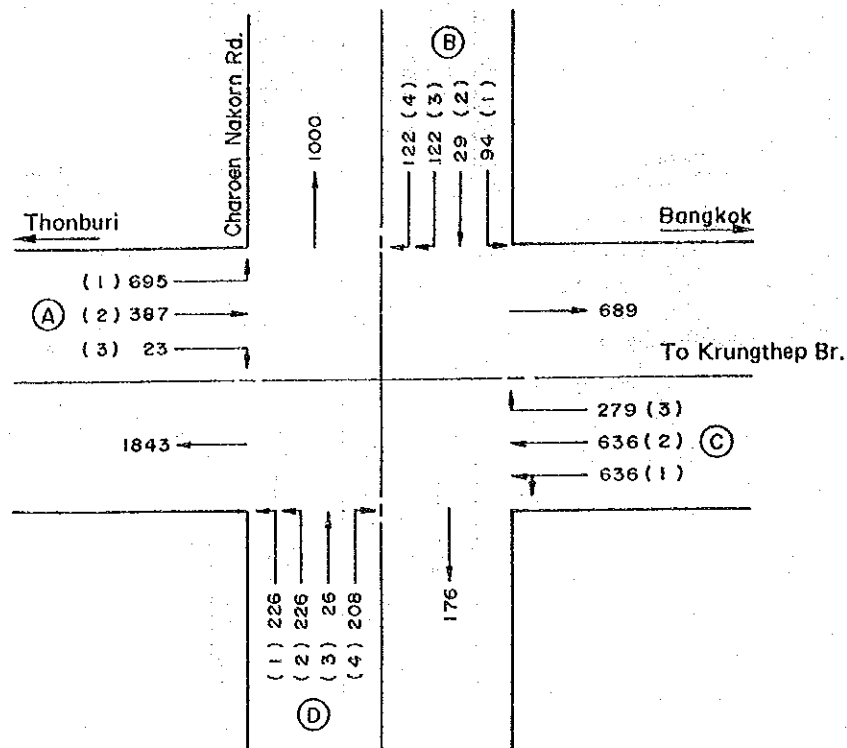
## b) Calculation of Traffic Saturation Degree

### Capacity and saturation degree

Section		Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1)	2000	297	Free	
	(2)	2200	260	0.12	
	(3)	2000	132	0.06	
B	(1)	2105	577	0.27	
	(2)	2105	577	0.27	
	(3)	2105	577	0.27	
	(4)	2000	838	0.42	
C	(1)	2000	233	0.11	Phase1 0.12
	(2)	2200	296	0.13	Phase2 0.21
	(3)	2200	296	0.13	Phase3 0.42
	(4)	2000	429	0.21	Phase4 0.11
D	(1)	2035	227	0.11	0.86
	(2)	2035	227	0.11	
	(3)	2000	177	0.09	
Total					0.86 < 0.90

## "B" Intersection

### a) Forecasted Traffic Volume in 2011 (PCU/H)



### b) Calculation of Traffic Saturation Degree

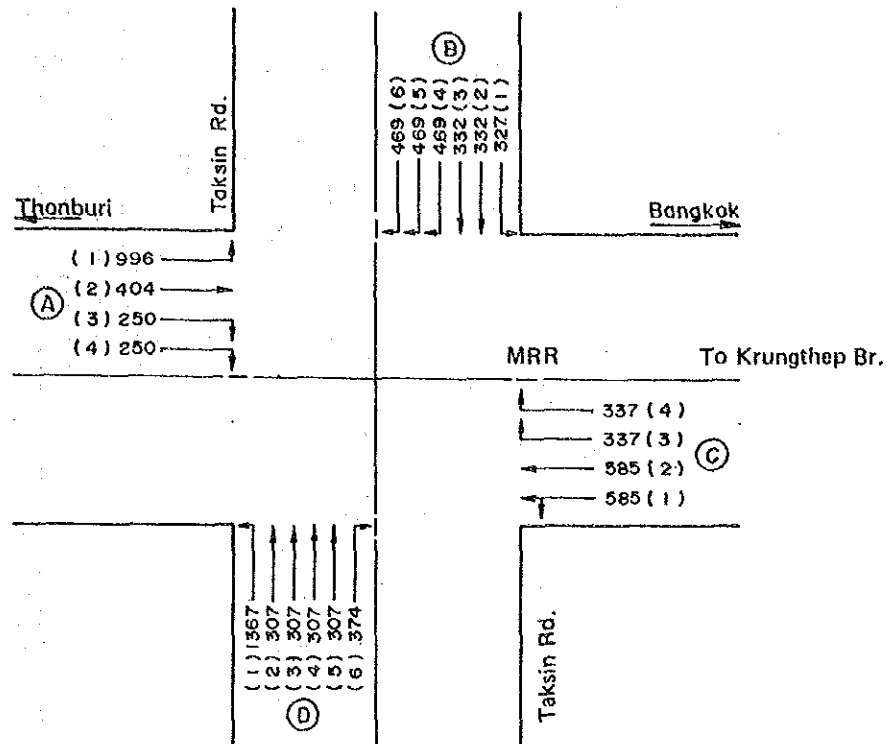
#### Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1) 2000	695	Free	
	(2) 2200	387	0.18	
	(3) 2000	23	0.01	
B	(1) 2000	94	0.05	
	(2) 2200	29	0.01	
	(3) 2000	122	0.06	
	(4) 2000	122	0.06	
C	(1) 2145	636	0.30	Phase1 0.30
	(2) 2145	636	0.30	Phase2 0.14
	(3) 2000	279	0.14	Phase3 0.11
				Phase4 0.10
D	(1) 2000	226	0.11	
	(2) 2000	226	0.11	
	(3) 2200	26	0.01	
	(4) 2000	208	0.10	
Total			0.65 < 0.90	



# "C" Intersection

## a) Forecasted Traffic Volume in 2011 (PCU/H)



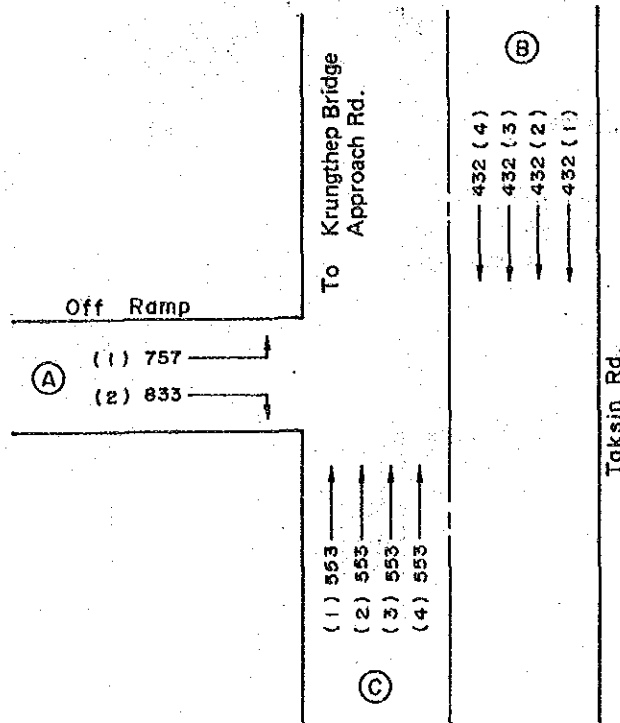
## b) Calculation of Traffic Saturation Degree

### Capacity and saturation degree

Section		Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1)	2000	996	Free	
	(2)	2200	404	0.18	
	(3)	2000	250	0.13	
	(4)	2000	250	0.13	
B	(1)	2000	327	0.16	
	(2)	2200	332	0.15	
	(3)	2200	332	0.15	
	(4)	2000	469	0.23	
	(5)	2000	469	0.23	
	(6)	2000	469	0.23	
C	(1)	2035	585	0.29	Phase1 0.29
	(2)	2035	585	0.29	Phase2 0.17
	(3)	2000	337	0.17	Phase3 0.14
	(4)	2000	337	0.17	Phase4 0.23
D	(1)	2000	1367	Free	0.83
	(2)	2200	307	0.14	
	(3)	2200	307	0.14	
	(4)	2200	307	0.14	
	(5)	2200	307	0.14	
	(6)	2000	374	0.19	
Total				0.83 < 0.90	

"C-1" Intersection

a) Forecasted Traffic Volume in 2011 (PCU/H)



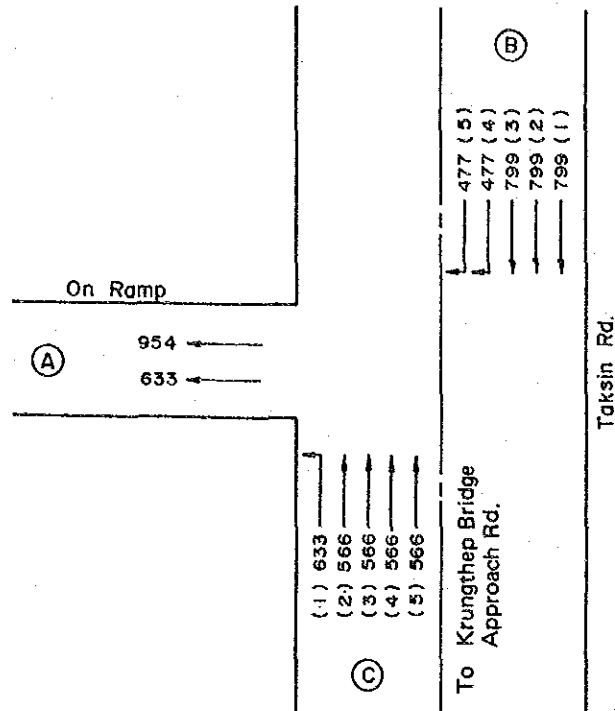
b) Calculation of Traffic Saturation Degree

Capacity and saturation degree

Section		Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
A	(1)	2000	757	0.38	Phase1 0.42 Phase2 0.25 0.67
	(2)	2000	833	0.42	
B	(1)	2200	432	0.20	
	(2)	2200	432	0.20	
	(3)	2200	432	0.20	
	(4)	2200	432	0.20	
C	(1)	2200	553	0.25	
	(2)	2200	553	0.25	
	(3)	2200	553	0.25	
	(4)	2200	553	0.25	
Total					0.67 < 0.90

# "C-2" Intersection

## a) Forecasted Traffic Volume in 2011 (PCU/H)



## b) Calculation of Traffic Saturation Degree

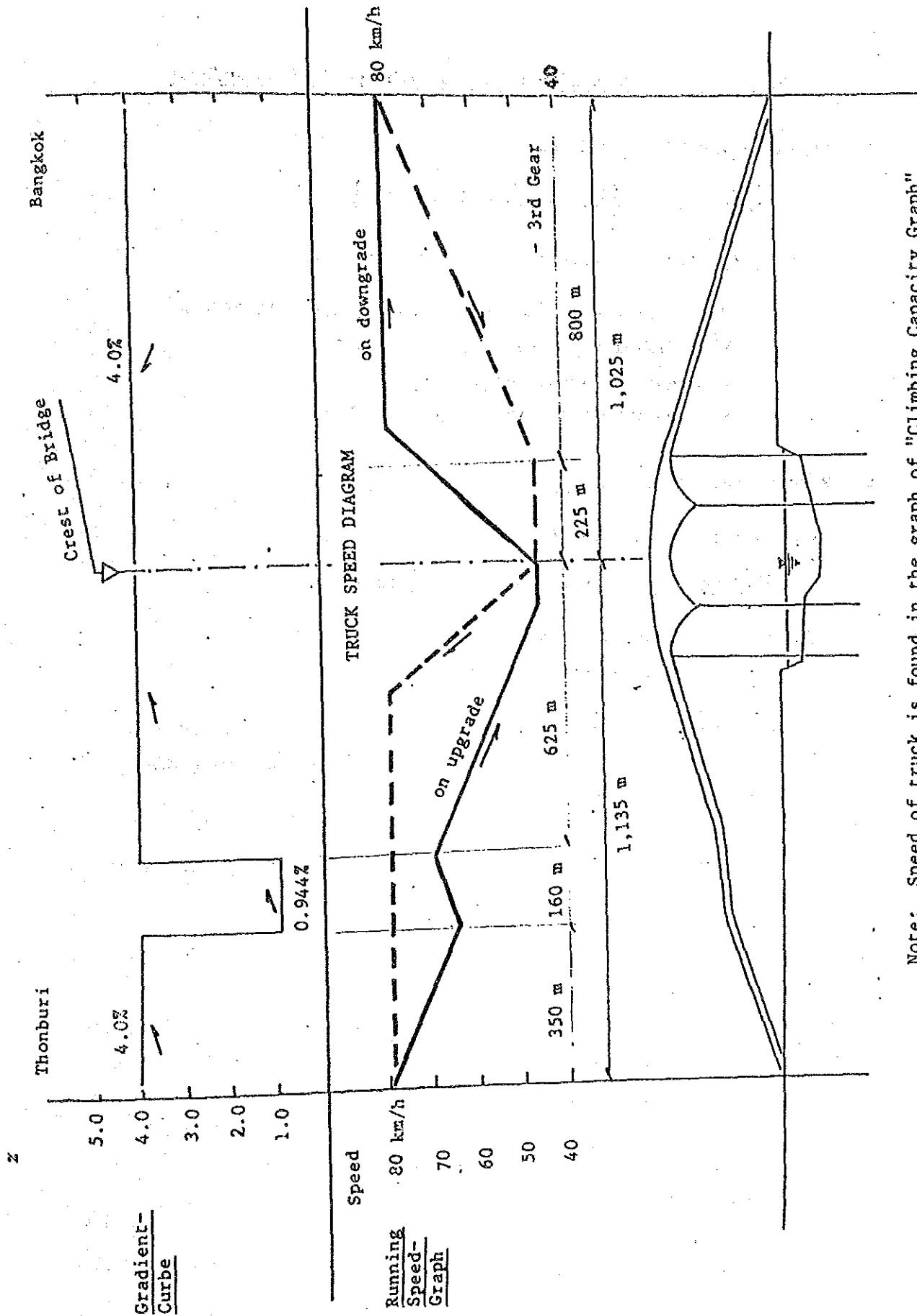
### Capacity and saturation degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
B	(1) 2200	799	0.36	
	(2) 2200	799	0.36	
	(3) 2200	799	0.36	
	(4) 2000	477	0.24	
	(5) 2000	477	0.24	
C	(1) 2000	633	Free	Phase1 0.36
	(2) 2200	566	0.26	Phase2 0.26
	(3) 2200	566	0.26	
	(4) 2200	566	0.26	0.62
	(5) 2200	566	0.26	
Total				0.62 < 0.90



#### Appendix 9.4.3 Running Speed of Truck on upgrade of 4.0%





Note: Speed of truck is found in the graph of "Climbing Capacity Graph" of Road Design Manual issued by Japan Road Association in 1983.

Theoretical Possibility to Climb with Speed of 45 km/hour

A. Engine power (T) by 3rd gear

$$T = \frac{270}{V} \cdot H \cdot E \left\{ 1.2 - 1.3 \left( \frac{v}{V} - 0.6 \right)^2 \right\}$$
$$= \underline{54.2 \text{ kg/ton}} \quad (\text{power per loading ton})$$

where, H : Engine horse power per loading ton: 13 PS/ton

E : Efficiency of engine power, 0.7\*

v : Running speed, 45 km/hour

V : Maximum speed by 3rd gear, 45 km/hour

B. Running resistance (R) on 4.0% grade

$$R = 10 + 10 i + 0.0015 v^2$$
$$= \underline{53.2 \text{ kg/ton}} \quad (\text{power per loading ton})$$

where, i : Gradient %, 4.0

v : Running speed, 45 km/hour

Based on the above calculation, the engine power (T) by 3rd gear (54.2 kg/ton) exceeds the running resistance (R) on 4.0% grade (53.2 kg/ton): T > R.

Note: 1. The efficiency of engine power for 3rd gear is adopted at 0.7 considering the conditions of trucks is Thailand.

2. Engine hourse power per loading ton in Japan:

11 ton truck, 13.2 - 15.0 PS/ton

8 ton truck, 13.1 - 13.4 PS/ton

6 ton truck, 12.0 - 14.4 PS/ton

4 ton truck, 16.8 - 18.2 PS/ton

3. 1.0 PS = 75 kgf.m/s



Appendix 9.4.4 Examination of the Distance among  
Intersections "C", "C-1" and "C-2"



### Examination of the Distance among Intersections

Distances among intersections were examined from the following points:

- The layout of three (3) intersections which consist of the Taksin Intersection is shown in sheet No. 1/47 in Drawing Volume, the distance from C-1 to C being 215 m, and from C to C-2 165 m, respectively;
- The detailed layout of the At-Grade Intersection "C" is shown in sheet No. 19/47 in Drawing Volume, there are clearly shown the number of lane by directions and the length of storage lanes for right turning within the total length of 100 m toward C-1 from C, and toward C-2 from C respectively. The scale of above intersection was determined based on the storage lane-length required, with its details are shown hereinafter; and
- On the other hand, the examination on weaving length required on the Taksin road was neglected because the effect of traffic from/into ramps is to be controlled by signals as same as the case of the road without ramps.

### Details of the Storage Length required

The traffic data by directions at the C-intersection has been indicated in Appendix 9.4.2, and using these traffic volumes the length of storage lane required at morning peak period were examined by the following formulas:

$$L = 1.5 \times N \times S$$

where, L : Length of storage lane required

N : Number of cars flowing during a cycle time from Green signal to the next Green signal

S : Length occupied by a car in the storage lane, 7.0 m

$$N = qc / 3600$$

where, q : Traffic volume per hour per lane at morning peak

C : Cycle time of the Green required

$$C \geq 0.9 T / (0.9 - R)$$

where, T : Clearance loss time estimated, 10 sec.

R : Saturation flow rate estimated, 0.65.

The cycle time is, therefore, calculated to be about 40 sec, the storage lane lengths required based on these formulas are less than the actual distances as shown in the Table below, therefore, no trouble in traffic flows at these intersections will happen.

Table Storage Lane Length Required

Flow	Traffic by Direction	Number of Lane	q (PCU/h)	N (NOS)	1.5xS (m)	L (m)	Distance (m)
B	L: 327	1	327	4	10.5	42	
	C: 664	2	332	4	10.5	42	< 165
	R: 1,407	3	469	5	10.5	53	
D	L: 1,367	1	1,367	15	10.5	158*	
	C: 1,228	4	307	4	10.5	42	< 215
	R: 374	1	374	4	10.5	42	
E	L: -	-	-	-	-	-	
	C: 1,848	4	462	5	10.5	53	< 215
	R: -	-	-	-	-	-	
H	L: 633	1	633	7	10.5	74	
	C: 2,264	3	754	8	10.5	84	< 165
	R: -	-	-	-	-	-	
F	L: 757	2	379	4	10.5	42	
	C: -	-	-	-	-	-	< 70
	R: 833	2	417	5	10.5	53	

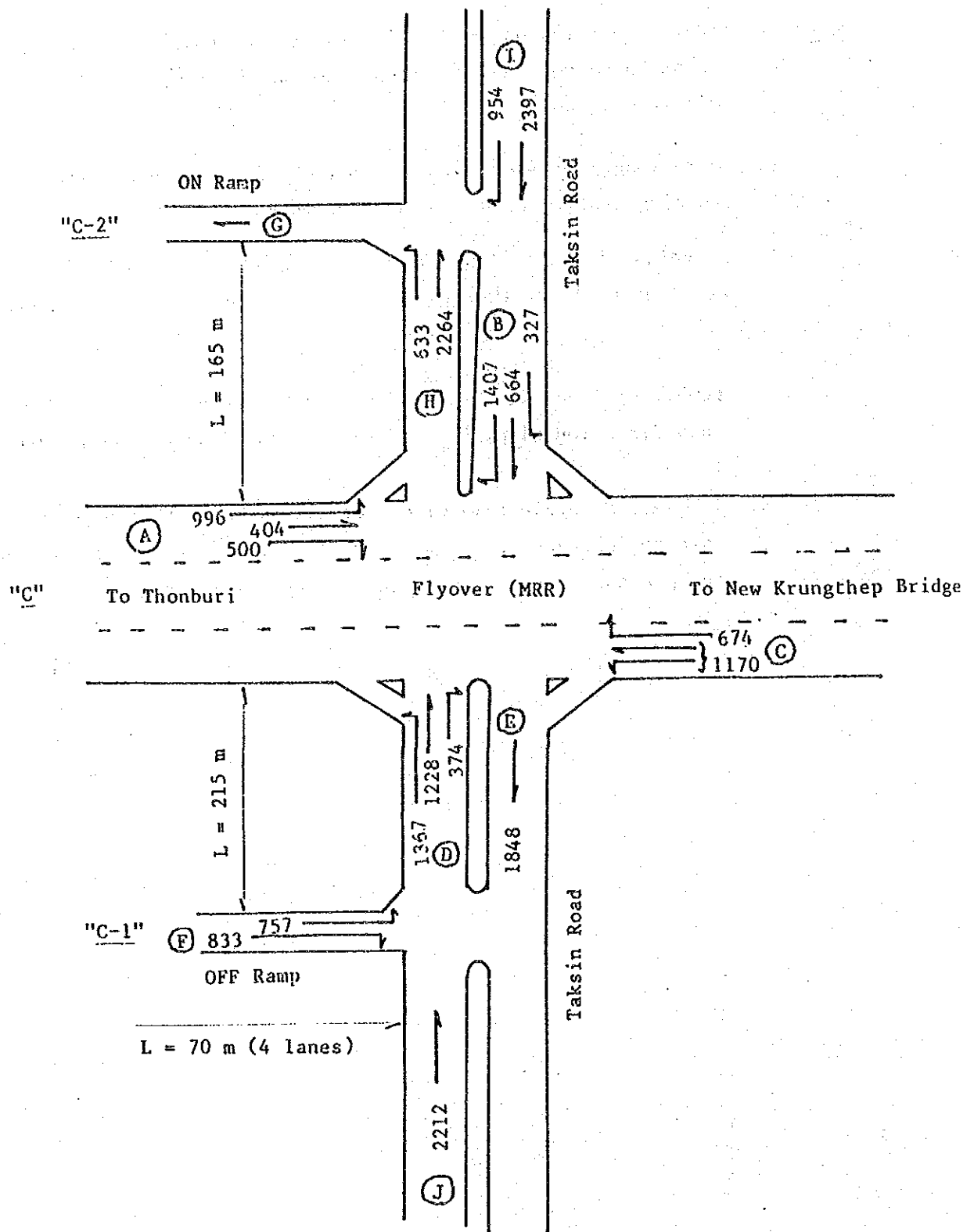
Note

L: Left turning traffic flow

C: Straight traffic flow

R: Right turning traffic flow

- Note:
1. Traffic volumes on the Taksin road shown above are already accumulated volumes of inflow from the Off Ramp of C-1 or the Middle Ring Road.
  - \*2. More longer or full Green cycle time will be given to the left turning traffic at D-flow by channelization.
  3. A sketch of these flows is attached in the next page, however, other details are the subjects to the future Detailed Design.
  4. Number of lanes for straging on the OFF Rump is 2 lanes for each direction with 70 m length from the edge of Taksin road.
  5. A theorized cycle time of 40 sec discussed in this paper may be able to increase up to max. 90 sec, if required, provided that the present intersection plan is kept as proposed.



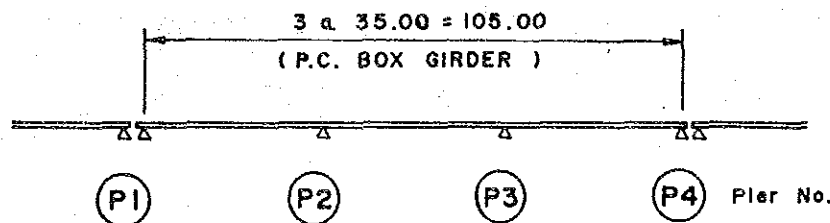
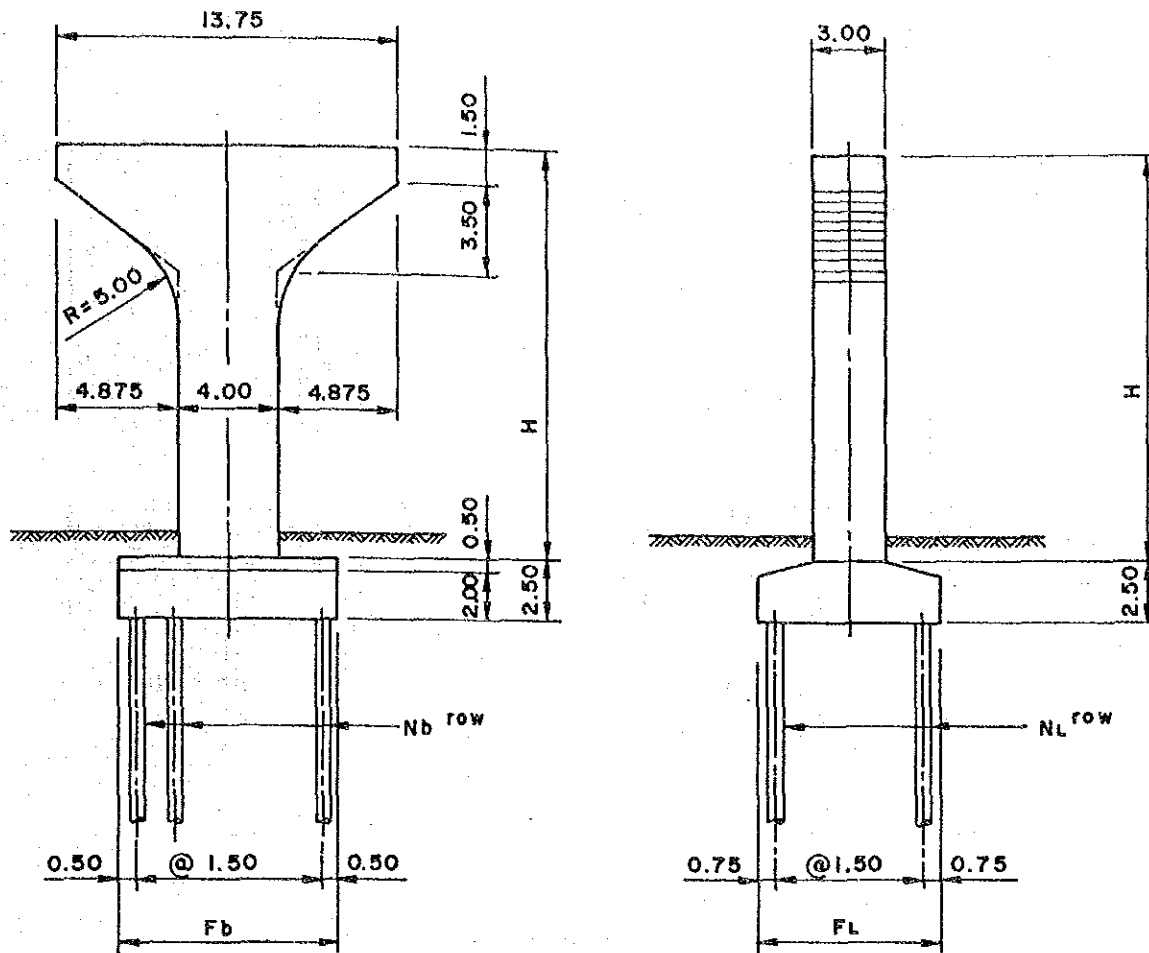
Turning Movement (2011)  
(Morning Peak Hour)

#### Appendix 9.5.1

Relationship Between Height of Pier  
and Number of Pile



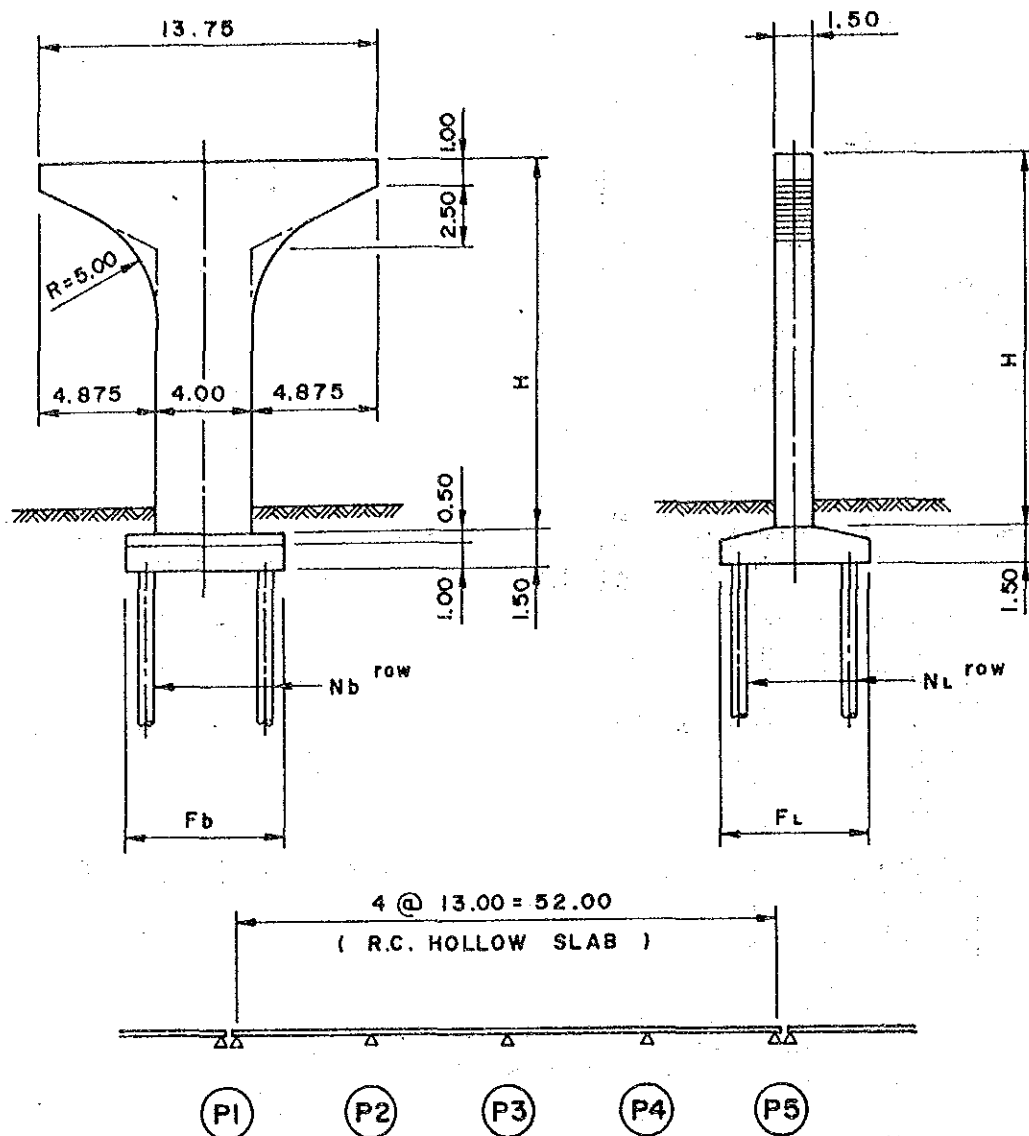




NOTE : N = NOS OF PILE

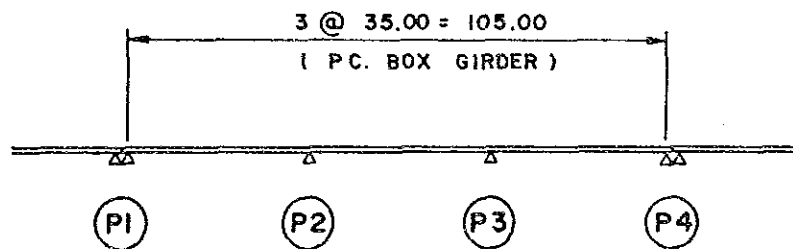
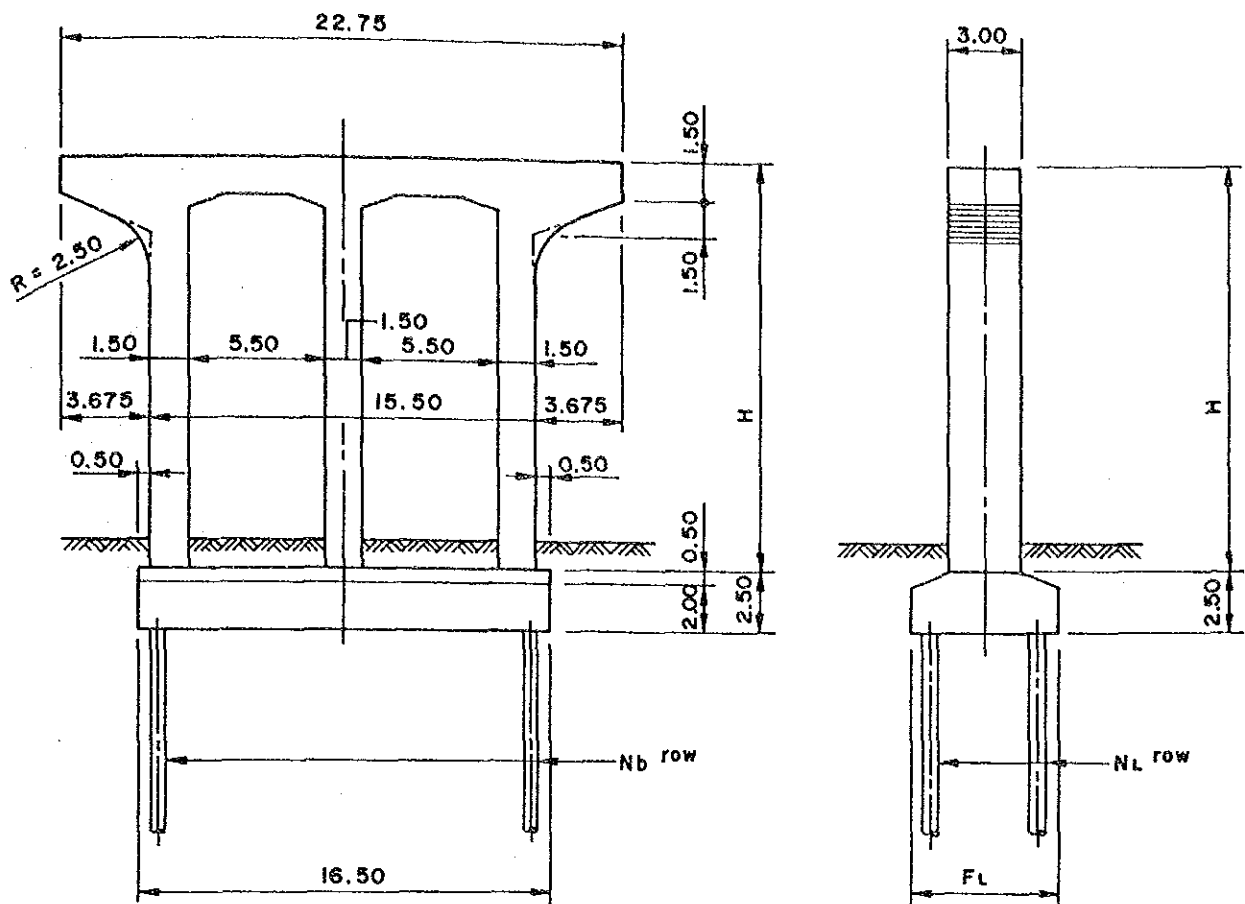
Pier Height (m)		10	15	20	25	30
Pier NO.						
(P1, P4)	Fb x FL (m)	7.50 x 6.00	7.50 x 7.50	9.00 x 7.50		
	Nb x Nl (row)	5 x 4	5 x 5	6 x 5		
	N	20	25	30		
(P2, P3)	Fb x FL	7.50 x 7.50	9.00 x 7.50	9.00 x 9.00		
	Nb x Nl	5 x 5	6 x 5	6 x 6		
	N	25	30	36		

## PIERS FOR PC BOX GIRDER



Pier Height (m)		5	10	15
Pier No.				
(P1), (P5)	$F_b \times F_L$ (m)	6.00 x 3.00	6.00 x 4.50	6.00 x 4.50
	$N_b \times N_L$ (row)	4 x 2	4 x 3	4 x 3
	N	8	10	12
(P2), (P3), (P4)	$F_b \times F_L$	6.00 x 4.50	6.00 x 4.50	6.00 x 4.50
	$N_b \times N_L$	4 x 3	4 x 3	4 x 3
	N	10	12	12

## PIERS FOR RC. HOLLOW SLAB



NOTE : N = NOS OF PILE.

Pier Height (m)		10	15	20	25
Pier No.					
(P1), (P4)	16.50 x FL (m)	16.50 x 4.50	16.50 x 4.50	16.50 x 6.00	16.50 x 6.00
	Nb x NL (row)	10 x 3	11 x 3	9 x 4	10 x 4
	N	30	33	36	40
(P2), (P3)	16.50 x FL	16.50 x 6.00	16.50 x 6.00	16.50 x 6.00	16.50 x 6.00
	Nb x NL	9 x 4	10 x 4	11 x 4	10 x 5
	N	36	40	44	50

## PIERS NEAR INTERCHANGES



Appendix 9.7.1 Data on Cost Estimates



Table A 9.7.1 Results of Averaged Market Prince Levels Survey

Item \ Contractors	A	B	C	D	E	F	G	Lowest Price	Average Price	Rounded Price
<b>1. Labour Costs</b>										
Unit: Baht per day										
a) Common labour	140	108	75	80	70	140	70	70	97.6	100
b) Skilled labour	240	132	130	140	150	250	100	100	163.1	160
c) Car driver	200	250	120	130	120	150	90	90	151.4	150
d) Operater	240	360	175	200	120	250	100	100	206.4	210
e) Foreman	600	360	175	200	200	500	120	120	307.9	310
f) Expert worker	260	360	155	250	150	500	100	100	253.6	250
<b>2. Material Costs</b>										
Unit of cement & steel										
: Baht per ton										
a) Portland cement	1,630	1,669	1,730	1,630	1,603	-	1,457	1,457	1,619.8	1,620
b) Rapid hardening cement	1,760	2,158	1,500	1,750	1,706	-	1,571	1,500	1,740.8	1,740
c) Sand	140	250	128	150	130	-	150	128	158.0	160
d) Crushed aggregate	170	250	180	185	175	-	180	170	190.0	190
e) Concrete (180 kg/sq.cm)	1,050	1,000	1,030	1,050	1,035	-	1,045	1,000	1,035.0	1,040
f) Concrete (240 kg/sq.cm)	1,100	1,085	1,100	1,150	1,125	-	1,135	1,085	1,115.8	1,120
g) PC concrete (300 kg/sq.cm)	1,200	1,175	1,200	1,230	1,210	-	1,220	1,175	1,205.8	1,210
h) PC concrete (350 kg/sq.cm)	1,250	1,255	1,270	1,300	1,290	-	1,300	1,250	1,277.5	1,280
i) Round bar	7,800	9,100	10,200	10,000	10,340	7,450	9,740	7,450	9,232.9	9,230
j) Deformed bar	7,900	8,940	10,000	11,120	10,160	7,650	9,960	7,650	9,390.0	9,390
k) PC wire	21,000	21,500	21,500	22,300	21,500	-	21,500	21,000	21,550.0	21,550
l) PC strand	21,000	21,500	21,500	22,300	21,500	-	21,500	21,000	21,550.0	21,550
m) PC bar	-	-	10,900	10,950	10,860	-	-	10,860	10,903.3	10,900
n) U.I.H shaped steel	9,600	10,000	10,700	11,000	11,000	9,400	10,000	9,400	10,242.8	10,240
o) Steel pipe	13,000	14,000	12,600	13,000	13,000	9,000	14,000	9,000	12,657.1	12,660
p) Wood for formworks	4,900	1,500	5,000	4,550	4,790	-	4,410	1,500	4,191.7	4,190
q) Wood for scaffolding	5,100	1,500	5,500	4,550	5,295	-	4,940	1,500	4,480.8	4,480
<b>3. Fuel Costs</b>										
Unit: Baht per litre										
a) Gasoline	8.20	9.50	9.50	9.50	9.50	-	8.20	8.20	9.06	9.10
b) Diesel fuel	6.10	6.50	6.50	6.50	6.30	-	6.30	6.10	6.36	6.40
c) Mechanical oil	27.30	25.00	-	-	-	-	26.10	25.00	26.13	26.10
<b>4. Machine Costs</b>										
Unit: Baht per day										
a) Bulldozer 15 ton	6,000	4,000	5,000	5,300	5,200	-	4,800	4,000	5,050.0	5,050
b) Wheel loader 1.4 cu.m	4,300	3,600	4,500	4,500	4,000	-	4,000	3,600	4,150.0	4,150
c) Excavator 0.6 cu.m	4,500	2,500	4,500	5,000	4,400	-	3,700	2,500	4,100.0	4,100
d) Dump truck 8 ton	2,000	1,300	1,500	1,800	1,600	-	2,200	1,300	1,733.3	1,730
e) Wheel crane 25 ton	5,000	4,000	6,000	6,500	7,000	-	5,000	4,000	5,583.3	5,580

Source: The Study Team

Table A 9.7.2 Analysis of Market Prices of Construction Equipment

Machine \ Price Source	Contractors Price	Rental Charge	Japanese Depreciation Cost	
	in BKK Baht/day	in BKK Baht/8hrs.	Average Charge Baht/day	Charge in Operation Baht/8hours
Bulldozer 15 ton	5,050	4,800	4,982	3,848
Wheel loader 1.4 cu.m	4,150	3,600	3,646	2,769
Excavater 0.6 cu.m	4,100	4,000	4,787	3,692
Dump truck 8 ton	1,730	1,500	1,764	1,365
Wheel crane 25 ton	5,580	7,000	8,463	5,538

Note: 1. Japanese depreciation cost is based on "Depreciation Table 1984" issued by Japan Construction Machanization Association.

2. Average daily machine charge

$$= P \times \left( \frac{D + M}{Y} + C \right) \times \frac{1}{H} \times 6 \text{ hours}$$

3. Hourly charge in operation

$$= P \times \frac{1/2 D + M}{Y} \times \frac{1}{H}$$

where: P = Present machine price

D = Depreciation cost ratio

M = Maintenance cost ratio at site and head office

Y = Life year of machine

C = Control and managing cost ratio per year

H = Standard operation hours per year



Table A 9.7.3 Custom Duty and Business Tax

Imported Goods	Custom Duty		Business Tax		
	Rate of Duty 1 CIF x %	Unit Bht.	Rate of Standard Profit %	Rate of Busines Tax %	Rate of Tax %
Portland cement	10	Ton 80	-	-	-
(Cement clinker)	-	-	11.0	1.5	1.8
(Petroleum crude)	25	-	-	-	-
Petroleum	-	L 0.01	Value*	9.0	19.4
Diesel	-	L 0.01	Value*	6	12.9
Lubricant oil	30	L 0.86	Value*	5.0	14.0
Asphalt	-	100kgs 1.0	11.0	5.0	6.0
(Soap)	-	-	11.0	9.0	10.8
(Explosive, Blasting)	30	-	11.0	9.0	14.0
**Wood	10	-	13.5	9.0	12.1
(Pig iron, cast iron)	6	kg 0.2	6.5	1.5	1.8
Angles & shapes	-	kg 0.4	11.0	9.0	10.8
Sheets&plates of iron or steel	-	-	11.0	9.0	10.8
**Bars of iron or steel	8	kg 0.75	5.5	9.0	11.1
Steel wire	25	kg 0.90	11.0	9.0	14.1
Machinally	30	-	16.0	9.0	14.6
**(Passenger car)	80	-	Value*	40.0	155.2
Works trucks	15	-	11.0	5.0	6.9
(Ships&vessels, more than 250ton)	30	-	-	-	-

Source: Custom Tariff of Thailand (Feb.1986)

- Note:
1. Value in the above tabel means market price of properties (rate of profit: 100% or more)
  2. Duty amount is calculated by CIF x Rate(%) or duty (Baht)
  3. Wood, Bar or Rod of iron and passenger car are belong to the import banned items unless otherwise special licence or tax be made.

Table A 9.7.4 foreign & Local Component

Related Items	Foreign Portion	Local Portion
Saw mill, wood	2%	98%
Limestone, Rock	14%	86%
Gravel, Sand quarrying	5%	95%
Petroleum refineries, Diesel, Gasolin, and Asphalt	89%	11%
Cement	5%	95%
Concrete	2%	98%
Iron & steel (pig iron ingot, shaped section)	69%	31%
Secondary steel product (angle bar, wire, tube pipe, PC tendon)	28%	72%
Structural metal product (bridge, tanks, building)	45%	55%
Special industrial machinery & appliances	40%	60%
Motor vehicle assembly	42%	58%
Electricity	7%	93%
Land transport	5%	95%
Ocean & water transport	4-6%	96-94%

Source: NESDB

Table A 9.7.5 Market Prices and Cost Components

Items	Average of Market Price	Cost Components			Economic Cost Ratio
		Foreign	Local	Tax	
			Net		
a) Labour costs (Baht per day)					
Common labour	100	-	100.0	-	100
Skilled labour (carpenter etc.)	160	-	160.0	-	100.0
Car driver (dum truck etc.)	150	-	150.0	-	100.0
Operator (bulldozer etc.)	210	-	199.0	11.0	94.8
Foreman	310	-	294.0	16.0	94.8
Expert worker (mechanics etc.)	250	-	237.0	13.0	94.8
2) Material Costs (Baht)					
Portland cement per ton	1,620	81.0	1,531	8.0	99.5
Rapid hardening cement (t)	1,740	87.0	1,644	9	99.5
Sand per cu.m	160	8.0	152	-	100.0
Crushed aggregate per cu.m	190	27	163	-	100.0
Concrete (180kg/sq.m) per cu.m	1,040	21	1,019	-	100.0
Concrete (240kg/sq.m) per cu.m	1,120	22	1,098	-	100.0
Concrete (300kg/sq.m) per cu.m	1,210	24	1,186	-	100.0
Concrete (350kg/sq.m) per cu.m	1,280	26	1,254	-	100.0
Interlocking block each	3.50	0.07	3.12	0.31	91.0
Concrete block 0.4x0.4 each	8.00	0.16	7.12	0.72	91.0
Curb & Gutter per m.	135.00	2.70	120.2	12.10	91.0
Curb stone per m.	42.00	0.80	37.40	3.80	91.0
RC pile 0.22x0.22 per m.	477.00	77.10	384.40	15.50	96.8
PC pile 0.60 per m.	1,000	20	890	90	91.0
Round bar per ton	9,230	6,369	1,645	1,216	86.8
Deformed bar per ton	9,390	6,479	1,674	1,237	86.8
PC wire per ton	21,550	6,034	13,159	2,359	89.1
PC strand per ton	21,550	6,034	13,159	2,359	89.1
PC bar per ton	10,900	7,521	1,942	1,437	86.8
U.I.H shaped steel per ton	10,240	7,066	1,824	1,350	86.8
Steel pipe per ton	12,660	3,545	8,732	383.0	97.0
Wood for formworks (cu.m)	4,190	84	4,106	-	100.0
Wood for scaffolding (cu.m)	4,480	90	4,390	-	100.0
Asphalt concrete per ton	1,200*	120	1,000	80	93.3
Steel bridge structure (t)	36,500*	14,600	19,923	1,977	94.6
Cast iron structure (t)	225,000*	155,250	57,640	12,110	94.6
3) Fuel Costs (Baht per litres)					
Gasoline	9.10	6.80	1.0	1.30	85.7
Diesel fuel	6.40	4.80	0.7	0.90	85.9
Mechanical oil	26.10	19.50	2.8	3.80	85.4
4) Machine Costs (Baht per day)					
Bulldozer (15 ton)	5,050	2,778	1,033	1,239	75.5
Wheel loader (1.4 cu.m)	4,150	2,283	849	1,018	75.5
Excavator (0.6 cu.m)	4,100	2,255	839	1,006	75.5
Dump truck (8 ton)	1,730	727	844.0	159.0	90.8
Wheel crane (25 ton)	5,580	3,069	1,142	1,369	75.5

Note: r), s), t) these prices obtained from the recent contract documents



**Appendix 9.7.2**

**New Krungthep Bridge Construction Project  
Financial/Economic Construction Cost**



Table A-9.7.6 (1) New Krungthep Bridge Construction Project  
Financial/Economic Construction Cost Table

(New Krungthep Bridge Main & Approach)

Item No.	Work Item	Unit	Quantity	Financial Unit Price	Component F	Component L	Tax (%)	Financial Amount	Economic Amount
<b>1. Main Bridge Work</b>									
B-5	Cast-in-place RC pile 2.0 m dia	m	442.0	23,922	45.7	54.3	10.3	101,812,032	91,325,000
B-3	Precast PC pile 60 cm dia	m	4,256	1,559	22.5	77.5	13.3	6,819,066	5,912,000
B-2	Bridge Excavation	cu.m	4,374	251	51.0	49.0	16.3	482,422	403,000
B-6	Concrete of Pile cap P29 & P30	cu.m	1,922	2,104	24.6	75.4	5.9	11,601,456	10,916,000
B-7	Concrete of Substructure	cu.m	5,514	1,508	11.3	88.7	3.3	6,948,864	6,719,000
B-8	Formwork for Substructure	sq.m	4,608	551	58.1	41.9	9.8	2,804,590	2,529,000
B-9	Re-bar for Substructure	ton	5,090	12,577	62.0	38.0	12.6	23,405,797	20,456,000
B-10	Stone Pitching against Scouring	cu.m	1,861	403	35.2	64.8	6.7	10,282,142	9,593,000
B-11	Precast Curtain Wall of P29 & P30	sq.m	25,514	1,372	37.8	62.2	5.5	812,224	767,000
B-12	PC Concrete for Main Bridge	cu.m	13,888	3,730	35.6	64.4	10.3	51,802,240	46,466,000
B-15	Re-bar for Superstructure	ton	1,607	13,115	61.8	38.2	12.7	21,075,805	18,399,000
B-16	PC Tendon for Main Bridge	ton	1,266	51,151	20.7	79.3	8.0	64,757,166	59,576,000
B-24	Guardrail, Double Leaf	m	442	653	27.1	72.9	3.1	288,626	279,000
B-25	Expansion Joint, Rubber	m	36	9,220	65.8	34.2	12.8	331,920	289,000
B-27	Bearing, Cast iron	ton	60	130,567	67.6	32.4	12.7	7,834,020	6,839,000
B-29	Concrete curb & Railing	m	884	1,816	22.8	77.2	4.9	1,605,344	1,526,000
B-30	Concrete Median Strip	m	442	609	31.8	68.2	8.2	269,178	247,000
R-8	Bridge Wearing Course	sq.m	6,851	197	20.3	79.7	8.6	1,349,647	1,233,000
Sub total					39.2	60.8	9.8	314,282,539	283,474,000
<b>2. Approach Bridge</b>									
B-3	Precast PC pile 60 cm dia	m	787 + 635 = 1,422	1,559	22.5	77.5	13.3	55,430,245	48,058,000
B-2	Bridge Excavation	cu.m	35,555	251	51.0	49.0	16.3	4,285,574	3,587,000
B-6	Concrete of Substructure	cu.m	17,074	1,508	11.3	88.7	3.3	29,176,784	28,213,000
B-8	Formwork for Substructure	sq.m	19,348	551	58.1	41.9	9.8	10,924,126	9,853,000
B-9	Re-bar for Substructure	ton	19,826	12,577	62.0	38.0	12.6	38,837,776	33,944,000
B-13	PC Concrete for PC Box Bridge	cu.m	3,088	5,030	40.1	59.9	12.4	92,094,270	80,674,000
B-14	RC Concrete for Hollowed Bridge	cu.m	18,309	4,272	38.2	61.8	10.1	16,549,728	14,878,000
B-15	Re-bar for Superstructure	ton	3,874	13,115	61.8	38.2	12.7	37,390,865	32,642,000
B-17	PC Tendon for PC Box Bridge	ton	2,851	54,242	21.8	78.2	8.5	93,892,902	85,912,000
B-24	Guardrail, double Leaf	m	1,731	653	27.1	72.9	3.1	927,260	898,000
B-25	Expansion Joint, Rubber	m	1,420	9,220	65.8	34.2	12.8	3,042,600	2,653,000
B-28	Bearing, Neoprene	Each	330	11,671	69.6	30.4	12.2	3,758,062	3,299,000
B-29	Concrete Curb & Railing	m	322	1,816	22.8	77.2	4.9	5,157,440	4,904,000
B-30	Concrete Median Strip	m	2,840	609	31.8	68.2	8.2	864,780	793,000
R-8	Bridge Wearing Course	sq.m	1,420	197	20.3	79.7	8.6	5,043,988	4,610,000
Subtotal					35.7	64.3	10.7	397,376,400	354,918,000

Continued to 3. Interchange & Access Road...

Table A 9.7.6 (2) New Krungthep Bridge Construction Project  
Financial/Economic Construction Cost Table

(New Krungthep Bridge Main & Approach)

Item No.	Work Item	Unit	Quantity	Financial Unit Price	Component F	Component L	Tax	Financial Amount	Economic Amount
3. Interchange & Access Road									
126 + 229 = 355									
B-3	Precast PC Pile 60 cm dia	m	9,326	1,559	22.5	77.5	13.3	14,539,234	12,605,000
B-4	Precast RC Pile 2 x 22 sq.cm	m	9,600	856	32.6	67.4	11.3	8,217,600	7,289,000
B-1	Structure Excavation	cu.m	4,589	197	44.2	55.8	17.8	904,033	743,000
R-3	Embankment, Sand	cu.m	13,500	283	46.8	53.2	12.0	3,362,000	3,362,000
B-6	Concrete of Substructure	cu.m	1,357	1,508	11.3	88.7	3.3	2,046,356	1,978,000
B-20	RC Concrete for Road Structure	cu.m	3,623	1,477	10.6	89.4	2.9	5,351,171	5,195,000
B-8	Formwork for Substructure	sq.m	14,613	551	58.1	41.9	9.8	8,051,763	7,262,000
B-9	Re-bar for Substructure	ton	846	12,577	62.0	38.0	12.6	10,640,142	9,299,000
B-13	PC Concrete for PC Box Bridge	cu.m	1,704	5,030	40.1	59.9	12.4	8,571,120	7,508,000
B-17	PC Tendon for PC Box Bridge	ton	160	54,242	21.8	78.2	8.5	8,678,720	7,941,000
B-14	RC Concrete for Hollowed Bridge	cu.m	1,060	4,272	38.2	61.8	10.1	4,528,320	4,070,000
B-15	Re-bar of Superstructure	ton	415	13,115	61.8	38.2	12.7	5,442,725	4,751,000
B-25	Expansion Joint, Rubber	m	49	9,220	65.8	34.2	12.8	451,780	393,000
B-28	Bearing, Neoprene	Each	62	11,671	69.6	30.4	12.2	723,602	635,000
B-29	Concrete Curb & Railing	m	1,350	1,816	22.8	77.2	4.9	2,451,600	2,331,000
R-8	Bridge Wearing Course	sq.m	4,471	197	20.3	79.7	8.6	880,787	805,000
B-21	Heightening Girder of Exist. Bridge	sq.m	1,253	4,389	45.8	54.2	11.4	5,499,417	4,872,000
B-22	Widening of Exist. Bridge	sq.m	1,003	8,721	30.3	69.7	10.7	8,790,768	7,850,000
R-7	Frontage/Rampway Pavement	sq.m	11,492	448	16.5	83.5	4.5	5,148,416	4,916,000
R-9	Sidewalk Block	sq.m	12,840	201	12.9	87.1	10.0	2,580,840	2,322,000
R-10	Island Block	sq.m	16,675	193	30.1	69.9	8.8	3,218,275	2,935,000
R-11	Curb & Gutter	m	3,024	409	21.3	78.7	7.6	1,236,816	1,142,000
R-12	Curb Stone	m	4,118	111	15.3	84.7	7.7	457,098	421,000
R-19	Drainage Pipe O 0.60	m	520	3,514	8.3	91.7	2.0	1,827,280	1,790,000
R-20	Drainage Ditch U-0.5 x 0.5	m	948	1,746	8.6	91.4	2.0	1,655,208	1,622,000
R-22	Drainage manhole	Each	40	12,453	14.4	85.6	3.2	498,120	482,000
R-23	Traffic Signs	Each	5	33,354	28.0	72.0	2.9	166,770	161,000
R-24	Lane Marking	sq.m	2,235	70.0	67.7	32.3	13.1	156,450	135,000
R-25	Traffic Signals	Each	9	208,535	27.8	72.2	2.9	1,876,815	1,822,000
R-26	Bridge Lighting Pole	Each	155	28,806	30.8	69.2	5.3	4,464,930	4,228,000
R-2	Removal of Existings	cu.m	3,750	824	44.2	55.8	17.3	3,090,000	2,555,000
Subtotal					34.6	65.4	10.0	125,966,656	113,421,000
4. Temporary Works									
T-1	Itemized Cost Total (1 + 2 + 3)	L.S	1		36.8	63.2	10.2	837,625,595	751,813,000
Direct Cost Total					47.2	52.8	15.5	233,000,000	196,885,000
H-1	5. Over Head	L.S	1		39.1	60.9	8.9	1,070,625,595	948,698,000
Total Construction Cost					40.5	59.5	16.9	1,385,074,345	1,151,202,000
Physical Contingency					40.5	59.5	16.9	114,925,655	95,503,000
Total					40.5	59.5	16.9	1,500,000,000	1,246,705,000



Table A 9.7.7 Land Acquisition and Compensation  
for New Krungthep Bridge

(Unit: Baht, October 1986 prices)

Location & Area	Financial Cost		Component (%) F L Tax	Economic Cost
1) Land Acquisition				
Thonburi Side West of Taksin Road (1,623.5 sq.wah)	76,710,375	-	100 4.8	73,057,500
Near Chinese Shrine (299.5 sq.wah)	6,289,500	-	100 4.8	5,990,000
Bangkok Side Near Customs Training Center (414.3 sq.wah)	10,874,062	-	100 4.8	10,356,250
Along Charoen Krung Road (1,064.8 sq.wah)	44,719,500	-	100 4.8	42,590,000
South of River Side Road (582.3 sq.wah)	21,397,687	-	100 4.8	20,378,750
Contingency (about 15%)	23,008,876	-	100 1.7	22,627,500
Subtotal	183,000,000	-	100 4.2	175,000,000
2) Building&Housing Compensation				
Thonburi Side West of Taksin Rd. (1,534 sq.m)	5,489,925	-	100 4.8	5,228,500
Near Shrine (456 sq.m)	1,436,400	-	100 4.8	1,368,000
Bangkok Side Near Training Center (1,094 sq.m)	3,843,000	-	100 4.8	3,660,000
South of River Side Rd. (1,348 sq.m)	4,668,195	-	100 4.8	4,445,900
Along Charoen Krung Rd. (7,754 sq.m)	28,348,950	-	100 4.8	26,999,000
Contingency (about 25%)	8,213,530	-	100 4.8	8,298,600
Subtotal	52,000,000	-	100 4.0	50,000,000
Total	235,000,000	-	100 4.1	225,000,000

Table A 9.7.8 Annual Maintenance Cost  
for Cement Concrete Surface Road

(Unit: Baht, October 1986 prices)

Work Items	Financial Cost	Component (%) F      L      Tax			Economic Cost
-----					
Estimate per 10 km for a year					
1) Daily maintenance					
Slope grass cutting & transportation	170,460	14.5	85.5	4.8	162,360
2) Seasonal maintenance					
after flood period					
Drainage work & lane marking	89,620	30.6	69.4	8.3	82,220
Total per 10 km	260,080	20.0	80.0	6.0	244,580
Total per km	26,008	20.0	80.0	6.0	24,458

Table A 9.7.9 Annual Maintenance Cost  
for Viaduct or Bridge

(Unit: Baht, October 1986 prices)

Work Items	Financial Cost	Component (%)			Economic Cost
		F	L	Tax	
Estimate per 1.0 km elevated road for a year					
1) Daily Inspection	105,660	23.4	76.6	7.2	98,100
2) Annual Maintenance					
Repair of pavement	11,600	27.5	72.5	9.3	10,520
Repair of lighting pole, Concrete curb & railing, median strip, and guard rail	20,858	26.6	73.4	4.9	19,828
Replace of expansion joint	32,428	67.5	32.5	12.1	28,488
3) Electricity Charge	29,500	7.0	83.0	10.0	27,000
Total per km	200,046	27.7	72.3	8.1	183,936

Table A 9.7.10 Annual Maintenance Cost  
for Main PC River Crossing Bridge

(Unit: Baht, October 1986 prices)

Unit: Baht, October 1988 prices

Work Items	Financial Cost	Component (%)			Economic Cost
		F	L	Tax	
Estimate per 1.0 km main bridge for a year					
1) Annual Maintenance Cost of Viaduct Road (Table A 9.7.9)	200,046	27.7	72.3	8.1	183,936
2) Additional Maintenance					
Periodical inspection	50,000	-	100.0	2.6	48,700
Repair of bearing, girder concrete surface, pier curtain wall	186,835	40.4	59.6	7.2	170,922
Stone pitching	63,267	33.7	66.3	6.6	59,073
Total per km	500,148	33.2	66.8	7.5	403,558

#### Appendix 9.8.1      Cost and Benefit Stream of New Krungthep Bridge



# ECONOMIC EVALUATION

New Krungthep Bridge Project (Basic Case)

Discount Rate (%) = 12.0

UNIT : million Baht

YEAR	COST	BENEFIT	DISCOUNTED	
			COST	BENEFIT
0	0.	0.	0.	0.
1	266.	0.	238.	0.
2	444.	0.	354.	0.
3	449.	0.	320.	0.
4	447.	0.	284.	0.
5	1.	144.	0.	82.
6	1.	228.	0.	116.
7	1.	312.	0.	141.
8	1.	397.	0.	160.
9	1.	481.	0.	173.
10	1.	565.	0.	182.
11	1.	650.	0.	187.
12	1.	734.	0.	188.
13	1.	818.	0.	187.
14	1.	903.	0.	185.
15	1.	987.	0.	180.
16	1.	900.	0.	147.
17	1.	814.	0.	119.
18	1.	727.	0.	95.
19	1.	640.	0.	74.
20	1.	554.	0.	57.
21	1.	467.	0.	43.
22	1.	381.	0.	31.
23	1.	294.	0.	22.
24	1.	207.	0.	14.
25	-910.	121.	-54.	7.

NET PRESENT VALUE = 1247.

B/C RATIO = 2.09

IRR= 20.71

# ECONOMIC EVALUATION

New Krungthep Bridge Project (Cost up 15%)

Discount Rate (%) = 12.0

UNIT : million Baht

YEAR	COST	BENEFIT	DISCOUNTED	
			COST	BENEFIT
0	0.	0.	0.	0.
1	306.	0.	273.	0.
2	511.	0.	407.	0.
3	516.	0.	367.	0.
4	514.	0.	327.	0.
5	1.	144.	0.	82.
6	1.	228.	0.	116.
7	1.	312.	0.	141.
8	1.	397.	0.	160.
9	1.	481.	0.	173.
10	1.	565.	0.	182.
11	1.	650.	0.	187.
12	1.	734.	0.	188.
13	1.	818.	0.	187.
14	1.	903.	0.	185.
15	1.	987.	0.	180.
16	1.	900.	0.	147.
17	1.	814.	0.	119.
18	1.	727.	0.	95.
19	1.	640.	0.	74.
20	1.	554.	0.	57.
21	1.	467.	0.	43.
22	1.	381.	0.	31.
23	1.	294.	0.	22.
24	1.	207.	0.	14.
25	-1046.	121.	-62.	7.

NET PRESENT VALUE = 1075.

B/C RATIO = 1.82

IRR= 18.86



# ECONOMIC EVALUATION

New Krungthep Bridge Project (Benefit less 15%)

Discount Rate (%) = 12.0

UNIT : million Baht

YEAR	COST	BENEFIT	DISCOUNTED	
			COST	BENEFIT
0	0.	0.	0.	0.
1	266.	0.	238.	0.
2	444.	0.	354.	0.
3	449.	0.	320.	0.
4	447.	0.	284.	0.
5	1.	122.	0.	69.
6	1.	194.	0.	98.
7	1.	265.	0.	120.
8	1.	337.	0.	136.
9	1.	409.	0.	147.
10	1.	480.	0.	155.
11	1.	553.	0.	159.
12	1.	624.	0.	160.
13	1.	695.	0.	159.
14	1.	768.	0.	157.
15	1.	839.	0.	153.
16	1.	765.	0.	125.
17	1.	692.	0.	101.
18	1.	618.	0.	80.
19	1.	544.	0.	63.
20	1.	471.	0.	49.
21	1.	397.	0.	37.
22	1.	324.	0.	27.
23	1.	250.	0.	18.
24	1.	176.	0.	12.
25	-910.	103.	-54.	6.

NET PRESENT VALUE = 888.  
B/C RATIO = 1.78

IRR= 18.57

