## Appendix 3.5.5

Base Year O&D Tables

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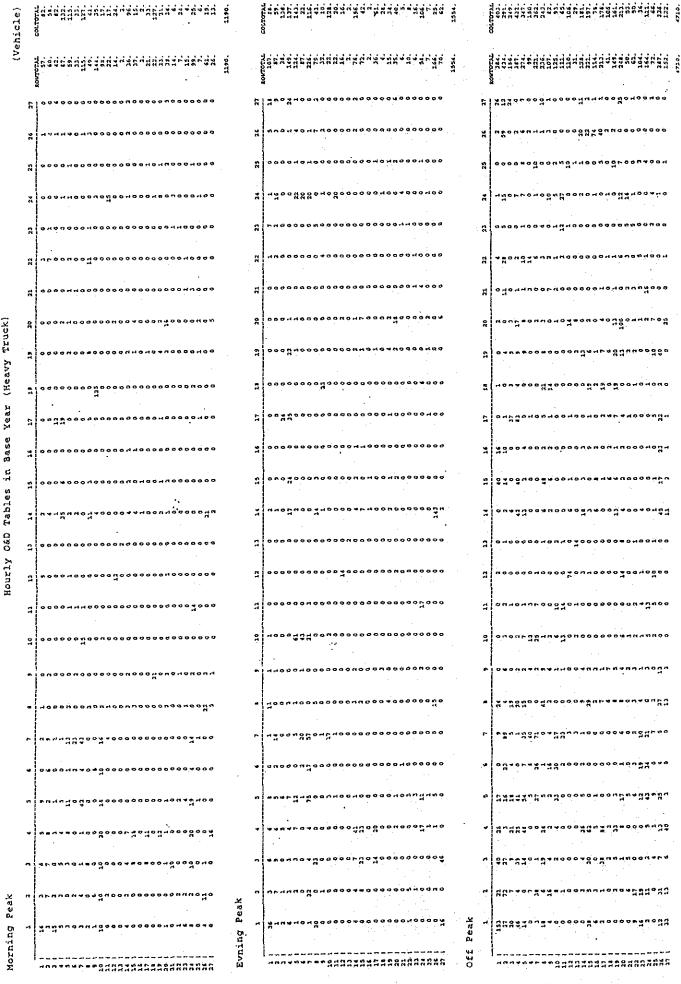
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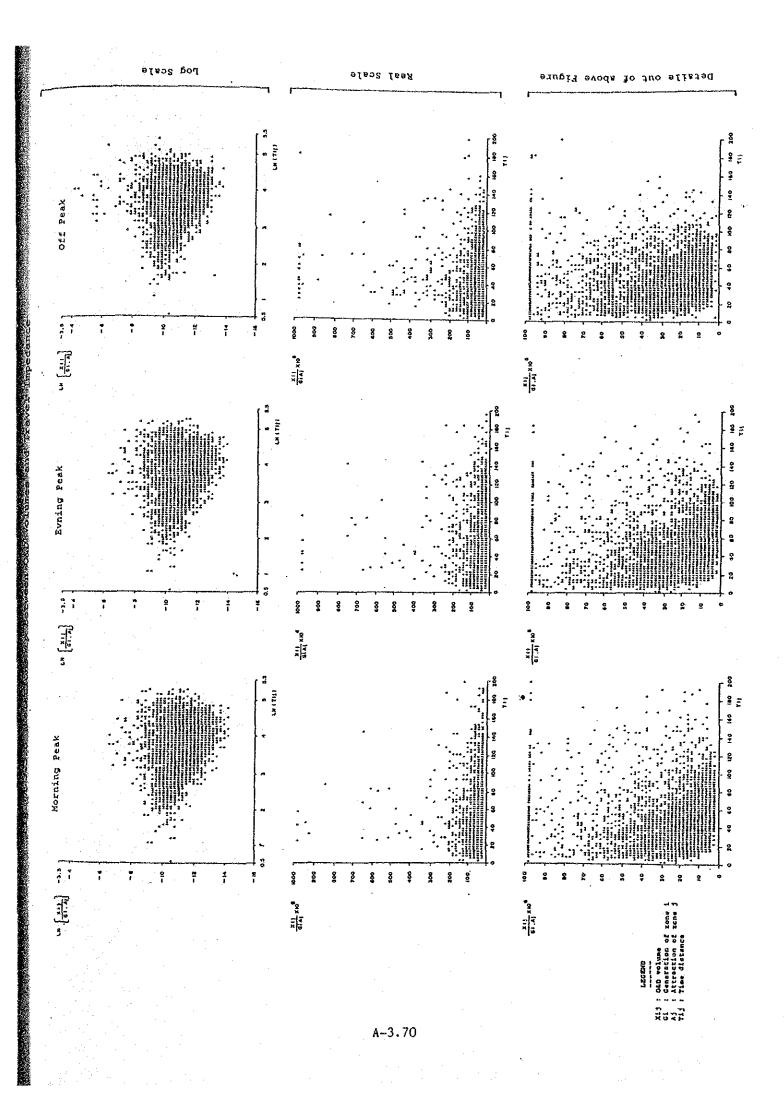
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Appendix 3.6.1

#### Travel Impedance and Trip Volume Relationship



#### Appendix 4.2.1

## Hourly O&D Tables (Base Case)

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Hourly O&D Tables in 2011 (Base case)

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Appendix 4.2.2

#### Hourly O&D Table in 1991, 2001 and 2011

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Hourly O&D Tables in 2011 (With Project case)

## Appendix 4.6.1 Vehicle Operating Cost

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#### Vehicle Operating Costs

Eight vehicle types were selected for the caculation of vehicle operating costs following the precedent of the JICA Feasibility Study on the Second Stage Expressway System in the Greater Bangkok, 1983, except that the category of taxis replaced the category of light buses. Representative vehicle makes were selected as the same as those by the aforementioned JICA study. The cost items were:

- fuel - oil - tyres

- maintenance (parts)

- maintenance (labor)

- overhead

- capital costs (depreciation and interest)

Except for the last item methods used in the JICA Study were applied with updating of prices at spring 1986 levels.

For capital costs methods used by STTR were applied. Depreciation was calculated on a straight-line basis and interest was calculated at 12% on the declining undepreciated balance. For motorcycles and cars part of depreciation was considered to be age-related instead of usage in terms of total kilometers. Interest rates for motorcycles and cars was set at 6% per annum and for pickups 8% instead of applying the full 12% sine a large portion of these vehicles are not business related. Basic assumptions for each vehicle type are shown in Table A.4.11.1.1.

As pointed out in the STTR Internal Working Paper No. 6, the effect of low congestion speed on the annual vehicle usage was considered important and the assumed relationship is shown in Table A.4.11:1:2.

Fuel and oil consumption data shown in the JICA Study, which were taken from actual experiments in similar conditions in Japan, were combined with spring 1986 prices to produce fuel and oil costs. Other costs were similarly adjusted from the JICA Study tables and STTR tables.

The resulting vehicle operating costs for each vehicle type are shown in Table A.4.11.1.3 through Table A.4.11.1.10 Crew costs not included as they are included in time values.

A-4.11

				$= e^{-\frac{1}{2} \left( \frac{1}{2} - \frac{1}{2} \right)^{\frac{1}{2}}}$	in de Co	s i An si	- AND AND	a at t	· .
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Henufacturer	Busukl		Corolin	Higan	Iousu	- "Illna"	FB 172	11 178	1.5
Snglne 0.0.	100	1300		. 1800	3300	5900	6600	8500	
Horket price (000 Hoht)	25.5	113.0	269.0	150.0	290.0	1065.0	645.0	810.0	1.1
Boonomic cost [000Buht]	11.1	165.7	- 144 9	118 8	213.0	924.4	. 110.9	620.4	•
Excluding tyram (000 Daht)	16.8	152.6	111.1	116.8	234.0	\$08.4	168.9	190 1	** * *
Vec depresiation X	80	85	100	100	100	100	100	100	$\mathcal{S}^{L_{1}}$
Use life (000 kms)	80	160	200	225	350	180	400	600	- 1 - L
Use dapreplation/000 kms	155	819	109	182	. 998	1893 -	892	881	
Senchmark speed {k/h}	30	10	30	30	28	20	30	26	11.1
Annual use at benchmark (000 kms)	10	12	100	28	78	60	56	61.	
Age deprinistion/00 kms at					,				
benohmark (Daht)	41	333	<u>_</u>			_		1 _ · ·	
Annual cooncale interest (Daht)	615	4677	8690	4810	14470	85830	12130	30109	•
Interest/000 kms at benchmark (Baht)	82	390	81	192	193.	694	402	598	
Life at benchmark (years)	8	13.3	2	9	. 4.7	8	113	9.8	

Table A.4.11.1.2 Annual Distances by Journey Speed

(000 kms)

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· .	•			1. 法法法法	с		n in Algoria Nga S		
Average Speed (k/h)	Hotor- oycla	Car	Taxi	Pick-Up	Hedium Bus	Heavy Bus	Trucks		eg bere
8	5.0	6.0	24.1	10.0	17,4	23.7	16.5	1. j. 1.	
	6.0	7.2	43.9	14.1	33.6	42.2	28.6		
15	7.0	8.4	61.0	17.5	48.9	61.8	37.8	1.1	
20	8.0	9.6	75.6	20.0	63,0	80.0	45.1	1111	10 A. A.
25	. 9.0	10.8	88.0	22.6	75.0	97.3	60.9		
30	10.0	12.0	100.0	26.0	88.8	113.9	55.0		1
35	11.0	13.2	108.6	26.5	101.1	129.6	60.0	1. 	11 J.
40	12.0	14.4	117.5	28.0	111.9	144.6	63.3	the point	
45	12.0	15.6	125.1	29.5	123.0	168.6	66.0	1. 1.	1
60	14.0	16.8	132.0	31.0	123.6	172.2	68.6		
66	15.0	18.0	138.3	32.6	123.8	172.2	71.0		S
60	16.0	18.0	143.9	34.0	123.6	172.2	73.1		
65	16.0	18.0	149.2	35.5	123.5	172.2	76.0	1.1	
70	16.0	18.0	164.1	37.0	123.6	172.2	76.4	1.14	1 A.
75	16.0	18.0	158.4	38,5	123.6	172.2	78.0		
80	16.0	18.0	162.4	40.0	123.6	172.2	79.2		
86	16.0	18.0	162.4	40.0					
.90		18.0	162.4	40.0		_	<u> </u>		
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Table A.4.11.1.3 Motorcycle VOC (Baht per 000 km)

Speed	k/h	Fuel	011	Tyras	Depreciaton (use)	Maintenance (parts)	Maintenance (labour)	Interest and age depre- oistion	Vehicle Operating Cost
. 5		440	28	16	165	B	. A. 17	196	860
10		406	24	16	165	8	. 17	165	791
15	*	328	19	18	165	1	17	133	686
20	~	280	16 .	18	165	7	17	116	517
25		245	15	16	165	1	17	103	568
30		219	16	16	165	7	19	93	522
35		205	15	16	165	1	- 19	85	513
40		194	15	17	165	5	20	78	496
45		185	16	18	168	6	21	71	482
50		182	16	18	165	6	22	66	475
65		177	18	19	165	6	23	62	470
60		175	18	19	165	7	24	57	465
65		174	19	20	165	9	25	67	468
70		172	21	20	165	8	25	57	458
75		173	22	21	165	8	26	67	472
			23	22	165	8	26	57	480
80		179		23	165	а а	26	67	486
85	•	184	23		165	. 0	26	67	496
90		191	24	25	- 100	0	20	57	

Table A.4.11.1.4	Private Car VOC
	(Baht per 000 km)

•	Speed k/h	Fuel	011	Tyres	Dipreciation (use)	Maintenance (parts)	Maintenanos (labour)	Interest and age depre- clation	Vehicle Operating Cost
۰.		~~~~~~							
	5	1121	80	70	619	80	43	1446	3469
	10	1023	60	70			43	1265	3095
		828	48		619	76			2721
· .	16			70	619	79	44	1033	
	20	706	42	70	619	87	44.	903	2471
	25	618	34	70	619	92	45	804	2282
	30	553	34	70	619	97	46	723	2142
	35	516	34	10	619	97	47	667	2040
÷.,	40	480	34	71	619	93	48	602	1955
1.1	45	467	38	12	519	89	50	557	1892
	50	459	40	73	619	89.	52	517	1849
Q	55	447	42	74			55	482	1811
					619	92		482	1814
	60	441	46	75	619	93	58		
	65	441	50	11	619	94	61	482	1824
	70	435	64	79	619	95	63	462	1827
$\sim 10$	75	441	56	82	619	96	64	482	1840
÷ .	80	463	58	85	619	97	55	482	1859
	86	465	60	90	619	98	66	482	1880
	90	482	64	96	619	100	68	482	1910

#### Table A.4.11.1.5 Taxi VOC

(Baht per 000 km)

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Speed k/h	Fuel	011	Tyres	Diprecistion (use)	Maintenance (parts)	Haintenance (labour)	Interest and age depre- clation	Vehicle Operating Cost
							************	**********
						32	361	2453
δ	1121	80	70	709	80		198	2167
10	1023	60	70	709	75	32		1910
15	828	48	70	709	19	33	143	
20	706	42	70	709	87	33	115	1762
25	618	34	70	709	92	34	99	1656
	553	34	70	709	97	35	87	1585
30			.70	709	97	35	. 80	1541
35	516	34		709	93	. 36	74	1505
40	488	34	71			37	69	1481
46	467	38	72	709	89		66	1475
50	459	40	13	709	89	39		1469
55	447	42	74	709	92	42	63	
60	441	46	76	109	93	. 44	60	1468
	441	50	17	709	93	45	68	1474
65		54	19	709	95	46	56	1474
70	435			709	96	48	55	1487
75	441	56	82		97	49	64	1505
80	453	68	85	709		50	53	1625
85	465	60	90	109	98	51	52	1553
90	482	64	95	709	100	<b>5</b> X		

Table A.4.11.1.6 Pick-up VOC

(Baht per 000 km)

Speed k/h	Fuel	011	Тугев	Diprecistion (use)	Haintenanoa (parts)	Maintenance (labour)	Interest and age depre- clation	Vehicle Operating Cost
5 10 16 20 25 30 35 40 45 55 60 55 60 65 70 76 80 85 90	$\begin{array}{c} 1315\\ 1201\\ 971\\ 828\\ 725\\ 648\\ 605\\ 573\\ 648\\ 638\\ 624\\ 617\\ 511\\ 517\\ 531\\ 645\\ 666\end{array}$	80 66 62 44 40 40 40 40 41 42 64 49 63 67 62 67 72	57 62 64 66 67 68 69 70 71 72 73 74 78 81 86 90 95	462 462 462 462 462 462 462 462 462 462	72 65 63 59 58 57 56 56 56 56 56 56 56 56 56 56 56 56 56	52 52 53 55 57 59 61 63 65 67 67 73 75 78 80 82	480 343 274 240 213 192 181 171 163 163 165 148 141 135 130 125 120 120 120	2518 2252 1938 1763 1625 1473 1404 1390 1374 1364 1364 1364 1362 1372 1392 1418 1461

# Table A.4.11.1.7 Midum Bus VOC (Baht per OOO km)

peed	k/h	Fuel		011	Tyres	Dipreciation (use)	Mainténance (parts)	Haintenance {labour}	Overheads	Interest age depre clation	Vehicle Operatin Cost
			,				***			******	
5	÷ .	1946		115	130	669	203	146	470	832	4611
10		1784		100	140	669	256	146	243	431	3778
15		1459		90	142	669	256	147	167	296	3226
20	1	1234		81	143	669	249	149	130	230	2885
25		1070		76	144	669	242	151	109	193	2654
30		944		76 .	145	669	235	155	92	163	2479
35		867		76	146	669	229	164	61	143	2375
40	- 1 - C	802	1.1	76	148	669	223	168	73	129	2288
45		785		78	150	669	219	178	66	118	2242
50	÷ .	730	- ÷	82	152	669	220	186	52	- 109	2210
55	1 H. 2 H - 1 H	714		85	155	669	. 221	194	62	109	2210
60		698		90	158	689	222	202	62	109	2210
65.		714	. 1	95	162	669	227	210	62	109	2248
70		747	•••	100	165	669	232	220	62	109	2305
75		783		105	171	669	238	232	62	109	2369
80	1	856		110	180	569	245	246	62	109	2477

Table A.4.11.1.3 Heavy Bus VOC (Baht per OOO km)

	· . ·	· · ·	х х			an an an star an star	ж. 1 1		
Speed k/ł	r Fuel	011	Tyres	Dipreciation (use)	Haintenance (parts)	Maintenance (labour)	Overheads	Interest age depre clation	Vehicle Operating Cost
		·							
5	3242	115	220	1893	542	233	1917	2343	10505
10	2972	100	240	1893	543	233	1077	1316	8374
15	2432	90	250	1893	544	233	735	899	7076
20	2058	81	255	1893	545	233	568	694	6327
25	1783	76	257	1893	546	241	467	571	5834
30	1573	76	258	1893	548	248	399	487	6482
35	1446	76	259	1893	550	258	351	428	5261
40	1338	76	261	1893	555	269	314	384	5090
45	1274	78	263	1893	561	282	287	350	4988
50	1215	82	266	1893	566	297	267	322	4906
55	1189	86	269	1893	572	310	264	322	4905
60	1163	90	273	1893	679	328	264		
65	1144	9.5	277	1893	582	342	264	322	4910
70	1244	100	282	1893	585	360		322	4919
75	1303	105	292				264	322	5050
80				1893	588	378	264	322	5145
¢ν	1409	110	305	1893	591	397	264	322	5291

Table A.4.11.1.9 Medium Truck VOC (Baht per 000 km)

ipeed k/h	Fuel	011	Tyres	Dipreciation (use)	Haintenance (parts)	Maintenance (labour)	Overheads	Interest age depre ciation	Vehicle Operating Cost
5	1804	120	220	982	367	145	767	1340	5645
10	1672	100	240	982	350	145	436	773 .	4608
15	1408	90	250	982	346	146	330	685	4045
20	1216	83	260	982	340	146	277	490	3704
25	1114	83	262	982	332	150	245	434	3512
30	990	83	264	982	338	154	227	402	3360
35	938	83	266	982	344	159	208	369	3259
40	877	83	269	982	366	164	197	349	3186
	849	86	272	982	370	174	189	335	3166
46		87		982	375	184	182	322	3139
50	822		275	962	382	194	175	311	3135
55	811	90	279	982	385	204	171	302	3141
60	811	93 87	283		386	214	167	295	3175
65	837		288	982					3211
70	863	102	293	982	395	224	163	289	
75	922	107	299	982	385	238	160	283	3283
80	1028	112	305	982	385	246	158	279	3406

A-4.14

speed k/h	Fuel	011	Tyres	Dipreciation (use)	Maintenance (parta)	Maintenance (labour)	Overheads	Interest age depre clation	Vehicle Operating Cost
5	2345	120	378	981	424	234	1040	1841	7363
10	2174	100	413	981	405	234	599	1062	5968
15	1830	90	430	981	399	234	453	804	5221
20	1581	83	- 447	981	393	235	381	673	4774
25	1448	83	450	. 981	384	242	337	596	4521
30	1288	83	454	981	391	249	312	552	4310
35	1219	83	457	981	389	257	286	507	4188
ιŐ	1140	83	462	981	410	265	271	480	4092
· 45	1104	85	468	981	428	285	260	460	4071
50	1070	87	473	981	434	298	250	442	1035
55	1054	90	480	981	442	314	242	427	4030
60	1054	93	486	981	445	330	235	415	4039 (
65	1087	97	495	981	445	346	229	405	4085
70	1121	102	504	- 981	445	362	224	397	4136
75	1199	107	514	981	445	380	220	389	4235
80	1338	112	524	981	445	398	217	383	4398

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## Table A.4.11.1.10 Heavy Truck VOC (Baht per 000 km)

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## Appendix 4.6.2 Time Values Per Vehicle

A-4.16

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#### Time Values Per Vehicle

Value of time losting while travelling has been estimated for trips of business purposes and trips of all other purposes separately. For the former average wage of crew and passengers differentiated by vehicle type were used but for the latter a same value was used for all vehicle types. The former could be considered reflecting the economic productivity of different types of person. However, the worth of non-productive time of a high wage earner should not be considered any more valuable than a low wage earner. The latter, therefore, is sometimes called the equity value.

Wages of drivers and assistants were estimated from the results of labor force survey carried out in 1984 by NSO and are shown in Table A.4.6.2.1Percentages of business trips were assumed for each vehicle type based on the JICA Second Stage Expressway Study. Business trips by cars were assumed to be done with professional drivers and business trips by pickups with 50% professional drivers. The equity value of time was assumed to be 25% of average wage Baht 5.25 per hour. Estimates and assumptions used are summarized in Table A.4.6.2.2.

Monthly Wage Cost Bt.	Hours/ Month	Cost/ Vehicle Hour Bt.
and the same are use and the same same are		400 444 was non syn 144 wat son son ann ann ann an an an an an an an an an
3000	240	12:5
3000	185	16.2
6350	280	22.7
4000	250	16.0
7500	200	37.5
6000	220	27.3
	Wage Cost Bt. 3000 3000 6350 4000 7500	Wage Cost Bt.Month30002403000185635028040002507500200

Table A.4.6.2.1 Wages of Vehicle Crew

Source: Special Report, SES Detailed Design.

Note:

Hours include non-driving time

A-4.17

Table A.4.6.2.2 Time Values, 1986

Vehicle Type	Vehicle Occupancy	Business Trip X	Business Business Trip X Pass Hage (Baht/Hr)	Calculation	Êι
Motorcycle Cars Taxis Pickups Buses Trucks	1.2 2.0 driver + business pass. driver + 1.1 passengers driver + 1.5 passengers crew + 37.7 passengers crew	15 15 15 15 15 10 100	24441 2488.0 111.0 0.0 0 0 0 0 0 0 0 0 0	$ \begin{array}{l} (0.15 \times 12.5 + 0.85 \times 5.25) \times 1.2 \\ 0.15 \times (16.2 + 48.0) + 0.85 \times 2.0 \times 5.25 \\ (1.0 \times 22.7) + 1.1 \times (0.15 \times 48.0 + 0.85 \times 5.25) \\ (0.5 \times 16.0 + 0.5 \times 5.25) + 1.5 \times (0.1 \times 21.0 + 0.4 \times 48.0 + 0.5 \times 5.25) \\ (1.0 \times 37.5) + 37.7 \times (0.04 \times 21.0 + 0.96 \times 5.25) \end{array} $	(2)
			. •		

Tine Velue Per Ven. Hour (Baht/Hr)

7.6 18.5 35.5 259.3

Note: Pickup drivers 50% business, 50% non-business, Pickup passenger 10% assistant, 40% business, 50% non-business

Source: Special Report, SES Detailed Design

## Appendix 5.3.1

#### Compressive Strength Test of Existing Krungthep Bridge

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#### Compressive Strength Test (Schimdit Hammer NR-3)

No.	1 (Exterior, )	Upstrea	m )	Date	: May 15, 19	86	
Loca	tion :Bangkok	Side P	с	Angle	: 90 degree		
		· · · · · · · · · · · · · · · · · · ·	R - 3	Value			
1	53	6	57	11	52	16	49
2	53	7	56	12	51	17	54
3	55	8	55	13	51	18	51
4	52	9	55	14	50.5	19	50
5	54	10	57	15	51	20	50
R me	an = 52.5	·	·	Range	: 46.5 to 58	.5	
Nos	of Data within	Range	n = 20	Total	Ri = 1, + n	= 1,056	5.5
R av	erage = Total	R/n	= 52.8		·		
Comp	ressive Stren	ngth :	Fc = 520	kgf/d	cm <sup>2</sup>		

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No.	2			Date	: May 15,	1986	
	ation : Bangko	ok Side	PC	<u> </u>	e : 90 degre		
	<u>`</u>		R – V	alue			<u> </u>
1	51	6	50	11	51	16	50
2	47	7	51	12	51	17	53
3	55	8	54	13	48	18	49
4	54	9	52	14	56	. 19	50
5	53	10	53	15	51	20	51
R me	ean = 51		<u></u>	Rang	e : 45 to 5	7	
Nos	of Data withi	n Range	e n = 20	Tota	1 Ri = 1,	n = 1,03	0
Ra	verage = Total	R/n	= 51.5			· · ·	
Com	pressive Stren	qth : I	Fc = 505	kgf	/cm <sup>2</sup>		

No.	3	. •		Date	: May 15,	1986	
Loca	tion : Bangko	ok Side	PC	Angle	: 90 degr	ee	
· · · ·	ny==1==4=4 <sup>+</sup> ···· <b>F</b> •·· <b>·</b> F •···F •····F •····F •·····F •······ •····· •······ •········		R - 1	Value		<u></u>	
1	50	6	47	11	49	16	50
2	41	7	48	12	54	17	48
3	52	8	47	13	51	18	50
4	45	9	47	14	51	19	51
5	51	10	49	15	52	20	49
Rте	an = 49.5	_ <u></u>		Range	: 43.5 to	55.5	
Nos	of Data withi	n Range	n = 20	Totál	Ri = 1,+	n = 988	
R av	erage = Total	R/n	= 49.4	a <u></u>			
Comp	ressive Stre	ngth :	Fc = 450	kgf/a	<sub>cm</sub> 2		

	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
•	

No.	4			Date	: May 15,	1986	
Loca	tion : Bangko	k Side	PC	Angl	e : 90 degre	9e	
		- <b>-</b>	R – V	alue			
1	50	6	50	11	52	16	55
2	48	7	50	12	52	17	46
3	50	8	56	13	44	18	51
4	49	9	47	14	54	19	52
5	54	10	50	15	50	20	48
R me	an = 50	<u></u>	,	Rang	e: 44 to 50	5	
Nos	of Data withi	n Range	e n ⇔	Tota	l Ri = 1,+	n = 1,00	18
R av	erage = Total	R/n	= 50.4	-d			
	erage = Total ressive Strer			kaf	<u></u>		e de segue <del> </del>

Loca	ation : Bangk	ok Side :	PC	Angle	: 90 degr	<b>60</b>	
		ومسوابة المحف ليراكن والجرج والمستكفر	<u> </u>				
			R	Value			
1	50	6	49	11	49	16	50
2	53	7	50	12	53	17	50
3	55	8	53	13	52	18	52
4	52	9	52	14	55	19	52
5	53	10	50	15	52	20	57
Rme	ean = 52	a ya kanda ka managa ya nga ya Sano (	<u></u>	Range	: 45 to	58	·
Nos	of Data with	nin Range	e n = 20	Totàl	Ri = 1,	+n = 1,03	9
Ra	verage = Tota	al R/n	= 51.95	******	<u></u>		iii
Came	pressive Str		n- <u>196</u>	kqf/c			

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No.	Wall		н. 1	Date	: May 15,	1986	
Loca	tion : Bangk	ok Side	Pier	Angle	e : O degre	e	
	,	1994 <del>- ar ai</del> n an an an an an an an an a	R – V	alue			
1	36	6	36	11	38	16	32
2	37	7	35	12	34	17	38
3	34	8	36	13	42	18	37
4	39	9	42	14	40	19	35
5	32	10	38	15	44	20	36
Rmea	an = 36.5			Range	e : 30.5 to	42.5	
Nos	of Data with	in Range	en = 19	Total	Ri = 1,+	n = 697	
R ave	erage = Tota	1 R/n	<b>=</b> 36.7				
Comp	ressive Stre	ngth : H	°c ≃ 295	kgf/	′cm <sup>2</sup>		

Allowable Stress =  $295/3 = 98 \text{ kgf/cm}^2$ , Say 95 kgf/cm<sup>2</sup>

No.	1' (Exterior	, Downs	tream)	Date	: May 15,	1986	
Loca	tion : Thonbu	i Side	PC	Angle	: 90 degre	€	
	,		R -	Value	<u></u>	ng ang ang ang ang ang ang ang ang ang a	
1	52	- 6	52	11	52	16	51
2	52	7	50	12	51	17	56
3	50	8	44	13	55	18	45
A	44	9	51.	14	53	19	55
5	54	10	52	15	47	20	48
Rme	an $= 51.5$	-		Range	: 45.5 to	57.5	
Nos	of Data withi	n Range	e n = 19	Totál	Ri = 1,	⊦n = 970	
R av	erage = Total	R/n	= 51.0	-	****		
Comp	ressive Stre	nath :	Fc = 495	kgf/c	.m <sup>2</sup>		<u></u>

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	_						المحمد المراجع المراجع المراجع المحمد المراجع الم
No. 2	21			Date	e : May 15,	1986	
Locat	ion : Thon	buri Side	PC	Angl	e : 90 degre	e	
	<u> </u>		R – Va	alue			
1	55	6	51	11	50	16	54
2	47	7	49	12	43	17	51
3	48	8	49	13	48	18	38
4	52	9	42	14	45	19	48
5	42	10	45	15	45	20	50
R mea	ın <del>-</del> 47.5			Rang	je : 41.5 t0	53.5	
Nós o	f Data with	nín Range	n = 18	Tota	al Ri = 1,+	n ≈ 843	
R ave	erage = Tota	al R/n	<b>= 46.8</b>				
Compr	essive Str	ength : Fo	= 405	kgf	:/cm <sup>2</sup>		
	تكراء مستحدين وبعيان الترابية المناجبا سيبين الكتائة				. / 2		,

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Allowable Stress = 405/3 = 135 kgf/cm<sup>2</sup>, SAy 135 kgf/cm<sup>2</sup>

		······································			· · ·		
Loc	ation : Thon	buri Side	PC	Angle	: 90 degi	ree	
		· · · · · · · · · · · · · · · · · · ·	<u>R</u> –	Value	n ball Ransaman, an mar an		
1	41	6	51	11	44	16	51
2	49	7	51	12	53	17	52
3	45	. 8	50	13	56	18	55
4	43	9	52	14	51	19	47
5	51	10	48	15	50	20	53
Rm	iean = 51	· · · · · · · · · · · · · · · · · · ·		Range	: 45 to 5	7	······································
Nos	of Data wit	hin Range	n = 20	Total	Ri = 1,	+n ≖ 993	
Ra	verage = Tot	al R/n	= 49.6				
	pressive St	monath i l	- 450	kqf/	2	<del></del>	

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Locati	on : Thonbu			l	00.7		
		iri Side	PC	Angle	: 90 degr	-ee	
			R – V	alue	•		
1	55	6	48	11	56	16	54
2 .	51	7	54	12	53	17	56
3	43	8	42	13	47	18	54
4	54	9	47	14	52	19	55
5	52	10	55	15	54	20	53
R mean	. <del>≠</del> 53.5			Range	: 47.5 to	59.5	
Nos of	Data withi	.n Range	Total	Ri ≠ 1,+	•n = 856		
R aver	age = Total	. R/n	<b>=</b> 53.5	<u></u>			

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Loca	tion : Thon	buri Side		Angle	: 90 degree		
				<u></u>			
	• • •		R -	Value			
1	52	6	53	11	53	16	52
2	46	7	53	12	53	17	53
3	52	8	45	13	51	18	51
4	49	9	51	14	50	19	46
5	45	10	47	15	53	20	52
R me	an = 51	<b>L</b>		Range	: 45 to 57	4 - , <u></u>	
Nos	of Data with	nin Range	n = 20	Totàl	Ri = 1,+n	<u> </u>	7
R av	erage = Tota	al R/n	= 50.3	<b>.</b>			
()	ressive St	congth · F	c = 470	kaf/c	m <sup>2</sup>	<del>.</del>	

No.	Wall			Date	: May 15, 1	986	
Loca	ation : Thon	buri Side	Pier	Angl	e : O degree		u toki o sekala
·			R V	alue			
1	36	6	44	-11	45	16	47
2	38	7	43	12	40	17	45
3	35	8	40	13	40	18	40
4	39	9	38	14	40	19	44
5	35	10	50 .	15	42	20	42
R me	ean = 40		· · · · · · · · · · · · · · · · · · ·	Rang	e : 34 to 46	5	
Nos	of Data with	nin Range	n = 18	Tota	l Ri = 1,+1	n = 726	a an
R av	verage = Tota	al R/n	= 40.3	- <b>1</b>			
Comp	ressive Str	ength : Fo	<b>: =</b> 350	kgf	$/cm^2$		

No. Pier Column		Date	: May 16,	1986		
Location : Bascule Pie	er	Angle	: 0 degree			
	R	- Value				
1 49 6	47	11	45	16	37	
2 38 7	12	41	17	48		
3 37 8	39	13	39	18	39	
4 39 9	36	14	44	19	39	
5 38 10	41	15	44	20	45	
R mean = 39		Range	Range : 33 to 45			
Nos of Data within Ra	nge n = 17	Totàl	Total Ri = 1,+n = 678			

No.	Pier Column			Date	: May 16,	1986		
Loca	tion : Basc	ule Pier		Angl	Angle : 0 degree			
/=			R - 1	/alue			······································	
1	47	6	54	17	42	16	44	
2	50	7	44	12	50	17	50	
3	40	8	47	13	48	18	46	
4	÷ 50	9	51	14	48	19	27	
5	44	10	49	15	52	20.	50	
R · me	an = 48.5		······································	Range : 42.5 to 56.5				
Nos	of Data with	in Range	n = 18	Tota	l Ri = 1,+	n = 865		
R av	verage = Tota.	R/n	= 48.0					
Comp	ressive Stre	ngth : F	c = 495	kgf,	/cm <sup>2</sup>			

(Exterior,	Unation						
	opacrea	am)	Date	: May 16,	1986		
on : Thonbu	ri Side	e(Innerspan)	Angle	: 90 degre	e	1	
9 - 9 - 9 - 9 - 9 - 9 - 9 - 9 - 9 - 9 -		R - 1	/alue				
54	6	54	11	48	16	59	
56	7	52	12	54	17	52	
54	8	54	13	53	18	57	
55	9	54	14	51	19	50	
58	10	53	15	52	20	56	
= 54			Range	: 48 to 60			
Data withir	n Range	e n = 20	Total Ri = 1,+n = 1,076				
rage = Total	R/n	= 53.8					
essive Stren	ngth :	Fc = 525	kgf/o	<sub>cm</sub> 2			
	54 56 54 55 58 = 54 Data withir age = Total	54       6         56       7         54       8         55       9         58       10         =       54         Data within Range         age = Total R/n	R - V         54       6       54         56       7       52         54       8       54         55       9       54         58       10       53 $= 54$ 54         Data within Range n = 20       33         age = Total R/n       = 53.8	R - Value         54       6       54       11         56       7       52       12         54       8       54       13         55       9       54       14         58       10       53       15 $=$ 54       Range         Data within Range n = 20       Totál         age = Total R/n       = 53.8	R - Value $54$ $6$ $54$ $11$ $48$ $56$ $7$ $52$ $12$ $54$ $56$ $7$ $52$ $12$ $54$ $54$ $8$ $54$ $13$ $53$ $55$ $9$ $54$ $14$ $51$ $58$ $10$ $53$ $15$ $52$ $=$ $54$ Range : 48 to 60         Data within Range n = 20	R - Value $54$ $6$ $54$ $11$ $48$ $16$ $56$ $7$ $52$ $12$ $54$ $17$ $54$ $8$ $54$ $13$ $53$ $18$ $55$ $9$ $54$ $14$ $51$ $19$ $58$ $10$ $53$ $15$ $52$ $20$ $=$ $54$ Range : $48$ to $60$ Data within Range n = $20$ Total Ri = $1, \ldots + n = 1, 0$ Total R/n = $53.8$	

-	

No.	2 (Interior,	Upstre	am)	Date	: May 16	, 1986	
Loca	tion : Thonbu	ri Side	(Innerspan)	Angle	: 90 degi	ree	. <u>.</u>
		<u></u>	R – Va	alue	*		
V	39	6	32	11	55	16	49
2	50	7	50	12	46	17	50
3	49	8	50	13	52	18	49
4	53	9	47	14	50	19	53
5	47	10	49	15	50	20	51
R me	an = 50			Rạnge	: 44 to 5	56	
Nos	of Data within	n Range	n = 48	Total	Ri = 1,	+n = 900	
R av	erage = Total	R/n	= 50.0				
Comp	ressive Stren	gth : F	c = 460	kgf/	cm <sup>2</sup>		

r								
No.	Wall			Date	: May 16,	1986		
Loca	tion : Thonbu	ri Sid	: 0 degree	9				
			R - 1	Valuë			a, ito na fallinda industra a falla da	
1	34	6	41	H	34	18	34	
2	39	7	42	12	48	17	46	
3	35	40	13	41	18	36		
4	47 9 41				47	19	47	
5	43 :	10	42	15	45	28	49	
Rme	an = 41.5			Range	: 35.5 to	47.5 ·		
Nos	of Data within	n Range	e n = 14	Total Ri = 1,+ $n = 597$				
R av	erage = Total	R/n	= 42.6					
Compi	ressive Strem	ngth ;	Fc = 390	kgf/	cm <sup>2</sup>	<u> </u>		

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#### Appendix 5.3.2

## Alkalinity Test by 1% Phenolphialein of Existing Krungthep Bridge.

· · · ·

Date	Location	Depth of Concrete Neutralized
16 May '86	Bangkok Side PC-T,Exterior,Upstream	6 mm
ditto	Bangkok Side PC-T,Interior	5.5mm
ditto	Bangkok Side PC-T,Interior	5 mm
ditto	Bangkok Side PC-T,Interior	5 mm
ditto	Bangkok Side PC-T,Exterior,Downstream	5.5nm
ditto	Bier Wall, Bangkok Side	10.0mm
ditto	Bascule Bridge Pier	10.5mm
ditto	ditto	18.0mm
ditto	Thonburi Side PC-T,Exterior,Upstream	9.0mm
ditto	Thonburi Side PC-T, Interior	9.0mm
ditto	Thonburi Side PC-T, Interior	5.0mm
dítto	Thonburi Side PC-T, Interior	5.0mm
ditto	Thonburi Side PC-T,Exterior,Downstrea	m 5.0mm
ditto	Pier Wall, Thonburi Side	16.0mm
ditto	Pier Wall, Thonburi Side Off-Shore	18.0mm
	16 May '86 ditto ditto ditto ditto ditto ditto ditto ditto ditto ditto ditto ditto ditto ditto	InterLocation16 May '86Bangkok Side PC-T, Exterior, UpstreamdittoBangkok Side PC-T, InteriordittoBangkok Side PC-T, InteriordittoBangkok Side PC-T, InteriordittoBangkok Side PC-T, Exterior, DownstreamdittoBier Wall, Bangkok SidedittoBascule Bridge PierdittodittodittoThonburi Side PC-T, Exterior, UpstreamdittoThonburi Side PC-T, InteriordittoThonburi Side PC-T, Exterior, DownstreadittoThonburi Side PC-T, InteriordittoThonburi Side PC-T, Exterior, DownstreadittoThonburi Side PC-T, Exterior, DownstreadittoThonburi Side PC-T, Exterior, DownstreadittoPier Wall, Thonburi Side

## Alkalinity Test by 1% Phenolphthalein

#### Calculation of Constants k/R<sup>2</sup>

(1) PC T-Beam.

No.	x (cm)	$k/R^2(=30/x^2)$
1	0.6	
2	0.55	99.2
3	0.5	120.0
4	0.5	120.0
5.	0.55	99.2
9.	0.9	37.0
10	0.9	
11	0.5	120.0
12	0.5	120.0
13	<u>1.0</u>	30.0

Total

865.7

 $k/R^2$ (Average)  $\approx$  86.5 (Expected Residual Life as far as neutralization of concrete being concerned : T.

$$T = \frac{k}{R^2} \left\{ D^2 - d^2 \right\} = 36.5 \times (3.0^2 - 1.0^2)$$

= 690 years )

#### (2) Substructure

No.	x (cm)	$k/R^2$ ( = 30/x <sup>2</sup> )
6	1.0	30.0
7	1.05	27.2
8	1.8	9.3
14	1.6	11.7
15	1.8	9.3
	Total	87.5

 $k/R^2$  (average) = 17.5

(Expected Residual Life as far as neutralization of concrete being concerned : T

 $T = 17.5 (3.0^2 - 1.8^2) = 100 \text{ years}$ 

### Appendix 5.4.1

## Stresses in Steel Truss Girder of Existing Krungthep Bridge.

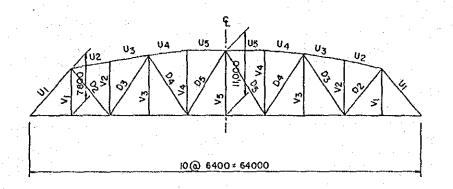
#### Superstructure

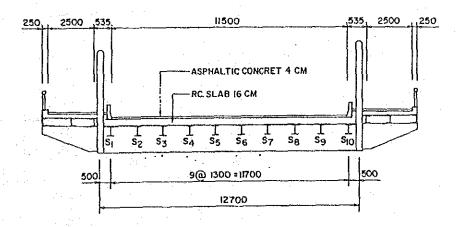
#### (i) Stresses in Steel Truss Girder

Structural calculation was made based on the service load design method (allowable stress design) for the three different live loads as specified in BSI, JRA and AASHTO, respectively.

The longitudinal and cross sections of the existing steel truss girder are as shown below, and the results of structural calculation are shown in Tables A.5.3.3.1 to A.5.3.3.3.

The results show that the existing steel truss girder has an enough capacity to carry either TL-20 or HS-20.





Steel Truss Girder

## Table A.5.3.3.1 SUMMARY OF STRESS IN STEEEL TRUSS GIRDER (SPAN = 64.0m) BSI Standard : HB-45 units &HA

:

		Dead Load	Stress	Live Loa	d Stress	Total	Allowable	Stress
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway	Stress	Stress	Excess
Carria String	ageway 1er	288*	38	. <del></del>	2,131	2,457	±1,400*	75.5%
Footpa	ath	211*	422	598	-	1,231	±1,400*	
	. ७ ०	-393*	52		-1,187	-1,632	-1,400*	16.6%
Inter- mediate Diaphragm	Gt	452*	60		1,365	1,877	1,400*	34.1%
	ଟ୍ଟ	-265*	-35		-1,287	-1,587	-1,400*	13.4%
End Dia- phragm	őt	304*	40		1,415	1,819	1,400*	29.9%
	UI	-621*	-40	-65	-444	-1,170	-1,139 *	2,7%
م	U2	-723*	44	-73	-502	-1,342	-1,309 *	2.5%
Main Truss Upper Chord	U3	-719*	-44	-72	-496	-1,331	-1313 *	1.4%
Main Uppei	U4	-756*	-49	-80	-535	-1,420	-1,295 *	9.7%
	· U5	-754*	49	-80	-532	-1,415	-1,296 *	9.2%
	Ľ1	700*	43	70	477	1,290	1,400 *	
τ <b>υ</b> .	L2	700*	43	70	477	1,200	1,400 *	
Main Truss Lower Chord	L3	821*	55	91	609`	1,576	1,400 *	12.6%
tain over	L4	821*	55	91	609	1,576	1,400 *	12.6%
21	LS	108*	60	99	651	1,718	1,400 *	22.7%
Member	D2	837*	49	84	626	1,596	1,400 *	14.0%
russ al Mei	D3	-516*	· -32	-67	~550	-1,165	-931 *	25.1%
Main Trus: Diagonal I	D4	433*	20	82	748	1,283	1,400 *	
žά	D5	-125*	-7	-67	-690	-889	876 *	1.5%
	VI	584*	34	60	884	1,562	1,400 *	11.6%
nber	V2	202*	12	20	143	377	1,400 *	
Main Truss Vercial Member	V3	584*	34	60	884	1,562	1,400 *	11.6%
ain T ercia	V4	2.58*	15	27	176	476	1,400 *	
ΣŠ	V5	584*	34	60	884	1,562	1,400 *	11.6%

: nD-45 units and Footpath = 3,485 K Pa  $(0.355 \text{ t.f/m}^2)$ 

Note: 1) The minus sign (-) shows the stress in compression.

 The figures with symbol\* are extracted from the previous study report (1982 March, by JICA).

## Table A.5.3.3.2 STUMMARY OF STRESS IN STEEEL TRUSS GIRDER (SPAN = 64.0m) JRA Standard : TL-20

4

$\wedge$		Dead Load	Stress	Live Lo	ad Stress	Total	Allowable	Stress
		Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway	Stress	Stress	Excess
string		288 *	38	_	904	1,230	±1,400 *	
footpo		211 *	422	590		1,223	±1,400 *	
Inter- mediate Diaphragm	्रद	-393 *	- 52		-681	-1,126	-1,400 *	
Inte medi Diapi	σt	452 <b>*</b>	60	-	786	1,298	1,400 *	
	oc.	256 *	- 35		-922	-1,222	-1,400 *	
end Dia- phragm	ď٤	304 *	40		1,056	1,400	1,400 *	
	U	-621 *	- 40	- 64	-146	871	-1,139 *	
ss ord	U2	-723 *	44	- 72	-161	-1,000	-1,309 *	
Main Truss Upper Chord	U3	-719 *	- 44	- 71	-159	- 993	-1,313 *	Nil
Main Uppe	U4	-756 *	- 49	- 79	-176	-1,060	-1,295 *	
	U5	-754 *	- 49	- 79	-176	-1,050	-1,296 *	
	LI	700 <b>*</b>	43	69	155	967	1,400 *	·····
ง ธุร	L2	700 \star	43	69	155	967	1,400 *	
Main Truss Lower Chord	L3	821 *	55	88	197	1,161	1,400 *	
Main Lovei	Ŀ4	821 \star	55	88	197	1,161	1,400.*	
	L5	908 \star	60	98	219	1,285	1,400 *	
amber	D2	837 *	49	82	209	1,171	1,400 *	
rues al Me	D3	~516 *	- 32	- 66	-170	~ 784	- 931 *	
Main Truss Diagonal Member	D4	433 ×	20	81	210	744	1,400 *	
ΣΩ	D5	-125 *	- 7	- 66	-177	- 375	- 876 *	
	V1	584 *	34	59	191	868	1,400 *	
mber	V2	202 ×	12	20	52	286	1,400 *	
Main Truss Vercial Member	V3	584 *	34	59	191	868	1,400 *	
ercu	V4	258 *	15	26	65	364	1,400 *	
·4 >	V5	584 *	34	59	191	868	1,400 *	- <u> </u>

Footpath = 3,430 K.Pa(0.350 t.f/m<sup>2</sup>)

Notea : 1)

 The minus sigh (-) shows the stress in compression.
 The figures with symbol \* are extracted from the previous study report (1982 March, by JICA)

### Table A.5.3.3.3 SUMMARY OF STRESS IN STEEL TRUSS GIRDER (SPAN = 64.0m) AASHTO Standard : HS-20 Footpath = 1,980 k.Pa (0.202 t.f/m<sup>2</sup>)

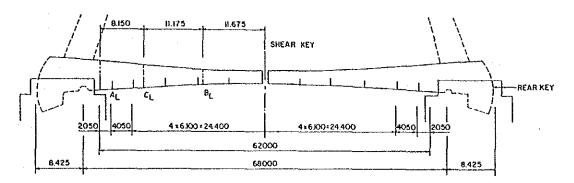
		Dead Load	Stress	Live Loa	d Stress	Total	Allowable	Stress
		Stress before Add. Stress Rehabili. by Rehabili.		Footpath Carriageway		Stress	Stress	Excess
Carria String		288*	38		790	1,116	±1,400*	
Footpa String	th	211*	422	598	-	1,231	±1,400*	
rag m	. oc	-393*	-52		-564	_1,009	-1,400*	
Inter- mediate Diaphragm	לנ	452*	60		651	1,163	1,400*	
	бc	-265*	-35		-658	-985	-1,400*	
End Dia- phragm	٥t	304*	40		754	1,098	1,400*	
	U1	-621*	-40	-37	-148	-846	-1,139*	
ກີບ ມີ	U2	-723*	-44	-41	-164	-972	-1,309*	Nil
Truss Chord	U3	-719*	-44	-41	-163	-967	-1,313*	
Main T Upper	 U4	-756*	-49	-46	-182	-1,033	-1,295*	
	U5	-754*	-45	-49	-45	-1,028	-1,296*	
	L1	700*	43	40	160	943	1,400*	:
	L2	700*	43	40	160	. 943	1,400*	
Truss r Chord	L3	821*	55	58	203 `	1,130	1,400*	
Main T Lower	L4	821*	55	58	203	1,130	1,400*	
	L5	908*	60	56	224	1,248	1,400*	
Member	D2	837*	49	47	216	1,149	1,400*	
al Me	D3	-516*	-32	-38	-180	-766	-931*	
Main Trus Diagonal	D4	433*	20	47	232	732	1,400*	
μ	D5	-125*	-7	-38	-203	-373	876*	
·	VI	584*	34	34	248	900	1,400*	
nber	V2	202*	12	11	47	272	1,400*	
russ 11 Mer	٧3	584*	34 •	34	248	900	1,400*	
Main Truss Vercial Member	V4	258	15	15	60	348	1,400*	
ΣÞ	V5	584*	34	34	248	900	1,400*	· · · · · · · · · · · · · · · · · · ·

The minus sign (-) shows the stress in compression. Notes : 1) 2)

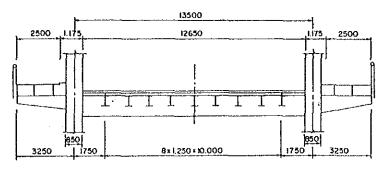
The figures with symbol \* are extracted from the previous study report (1982 March, by JICA)

## (ii) Stresses in Bascule Bridge

The longitudinal and cross sections of the existing steel bascule bridge are shown below, and the results of structural calculation are as shown in Tables A.5.3.3.4 to A.5.3.3.6 for the standard of BSI, JRA and AASHTO, respectively.



SIDE VIEW



CROSS SECTION

Bascule Bridge

#### Table A.5.3.3.4

.

SUMMARY OF STRESS IN BASCULE BRIDGE ( Span - 68.0m )

## BSI Standard : HB-45 units & HA

Footpath Loading = 4.715kPa(0.480t.f/m<sup>2</sup>)

kg.f/cm<sup>2</sup>

-					- · ·					
			Dead	Load	Stress	Live Loa	d Stress	Total	Allowable	Stress
			Stress be Rehabili.	1	Add. Stress by Rehabili.	Footpath Carriageway		Stress	Stress	Excess
	ciagewa inger	y	126	*	-		1,905	2,031	1,400*	45.1 %
1 5	6	:	-262	*			-1,243	-1,505	-1,400*	7.5 %
Dia- phragm	6 ا		322	*	-		1,529	1,851	1,400*	32.2 %
	 م	δt	364	.*	· _	82	734	1,180	1,400*	
-	Section	Ο <sub>c</sub>	-366	*	-	-82	-736	1,184	-1,142*	3.2 %
	Sec	T.	1	*	· _	17	167	267	800*	
	لى	() t	530	*		135	817	1,482	1,400*	5.9
Girder		0.	-536	*		-132	-798	-1,466	-1,162*	26.2 %
Main G	Section	T w	118	*		24	133	275	800*	-
Σ	ر ۲	θt	640	*	-	177	877	1,694	1,400*	21.0 %
	Section A	0 c	-646	*	~	-172	-853	-1,671	-1,132*	47.6%
	Sect	τ.,	89	*	-	25	112	226	800*	

Notes : 1) The minus sighn (-) shows the stress in compression.

 The figures with symbol \* are extracted from the previous study report (1982 March, by JICA ).

#### Table A.5.3.3.5 SUMMARY OF STRESS IN BASCULE BRIDGE (SPAN =68.0m)

#### JRA Standard : TL 20

Footpath Loading = 3.432 K.pa

$(or 0.350_2 tf/m^2)$
(Or 0.000/01/m /
1 61 4

					kgf/cm <sup>2</sup>						
Dead			Dead Load Stress		Live Loa	Live Load Stress		Allowable	Stress		
			Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway	Stress	Stress	Excess		
	riagewa inger	У	126*	-	· _	862	988	1,400 *	- ·		
- 5	6		-262*		· · · •	-709	-971	-1,400 *			
Dia- phragm	σt		322*	-	-	818	1,140	1,400 *			
	a J	∬t	364*	-	60	118	542	1,400 *			
	Section	Ο <sub>c</sub>	-366*	. –	-60	119	-545	-1,142 *			
	Sec	T.	83*	-	13	31	127	800 *	- -		
: 5	J.	θt	530*	-	99	240	869	1,400 *			
Girder	section C <sub>L</sub>	0.	-536*	-	-96	-234	-866	-1,162 *	-		
Main G	Sect	T w	118*	-	18	41	177	800 *	-		
.Σ.	ÅL	0 t	640*	-	129	305	1,074	1,400 *			
	ion /	6 c	-646*		-126	-297	-1,069	-1,132 *	<u></u>		
	Section	τw	89*		19	42	150	800 *	-		

Notes : 1)

The minus sign (-) shows the compressive stress.

The figures with symbol \* are extracted

from the previous study report (1982 March, by JICA).

Table A.5.3.3.6

SUMMARY OF STRESS IN BASCULE BRIDGE (SPAN = 68.0m)

AASHTO : HS-20 Footpath Loading = 2.544 K.pa

(or 0.259 tf/m<sup>2</sup>)

					2
•	1.	c	1.		4
ĸ	ന	T.	/c	٠m	

					· · · · · · · · · · · · · · · · · · ·			KGT/ Cill	
$\square$			Dead Load	Stress	Live Loa	d Stress	Total	Allowable	Stress
			Stress before Rehabili.	Add. Stress by Rehabili.	Footpath	Carriageway	Stress	Stress	Excess
	Carriageway Stringer		126*		-	753	879	1,400 *	nning Sol <del>a</del> y Solat
a - Gm	0.		-262*	-		-579	-841	-1,400 *	-
Dia- phragm	Ő t		322*	-	- 	668	990	1,400 *	
	в С	ſt	364*	-	44	198	606	1,400 *	- 1
	Section	0.	-366*	-	-44	-198	-564	-1,142 *	
	Sec	T.	83*	-	9	37	129	800 *	
	ت ري	() t	530*		73	259	862	1,400 *	-
Girder	Section (	Ô.c	-536*	-	-71	-253	-860	-1,162 *	-
Main G	Sect	τ.,	118*	-	13	43	174	800 *	
Σ	Р. С	σt	640*	-	95	315	1,050	1,400 *	-
	Section /	σc	646*	-	-93	-306	-1,045	-1,132 *	_
	Sect	T w	89*		14	42	145	800 ×	

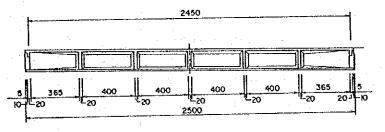
Notes : 1) 2) The minus sign (-) shows the compressive stress. The figures with symbol \* are extracted from the previous study report (1982 March, by JICA)

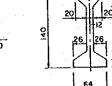
## (iii) <u>Stresses in PC Composite Beam</u>

The longitudinal and cross sections of the existing PC composite beams are shown below:

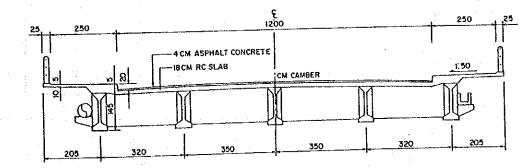
The results of stress calculation on the main beam shown in Table A.5.3.3.7.

The results show that the existing PC composite beams have an enough capacity to carry either TL-20 or HS-20 while HB-45 unit is not applicable.









CROSS SECTION

 $(\mathbf{x}_{i}) = (\mathbf{x}_{i}) + (\mathbf{$ 

#### PC Composite Beam

# Table A.5.3.3.7 SUMMARY OF STRESS IN PC COMPOSIT BEAM ( Span = 24.5m )

Bending	Axial	Stress	$(kg.f/cm^2)$
Moment(t.f.m)	Force(t,f)	Top Fiber	Bottom Fiber
124.5 *	220.0 *	46.4	-149.3
72.6 *		-63.9	50.2
27.0 *	-	-23.8	18.7
84.8 *	164.5 *	27.4	-106.0
113.1	-	-98.0	75.4
17.3		-1.5	7.7
-	-	-113.4	-103.3
542.2	-	-47.7	-241.3
209.4		-18.4	93.2
166.5	-	-14.6	74.1
_		-161.1	138.0
-	. –	-131.8	-10.1
-		-128.0	-29.2
-		-135.0	0
	124.5 * 72.6 * 27.0 * 84.8 * 113.1 17.3 - 542.2 209.4	Moment(t.f.m)       Force(t.f) $124.5$ $220.0$ $72.6$ - $27.0$ - $84.8$ $164.5$ $113.1$ - $17.3$ - $-$ - $542.2$ - $209.4$ -	Moment(t.f.m)Force(t.f)Top Fiber $124.5$ $220.0$ $46.4$ $72.6$ 63.9 $27.0$ 23.8 $84.8$ $164.5$ $27.4$ $113.1$ 98.0 $17.3$ 1.5113.4 $542.2$ 47.7 $209.4$ 18.4 $166.5$ 161.1128.0

Notes:

1) The minus sign(-) of stress shows that in compression.

 The figures with symbol \* are extracted from the previous study report (march 1982, by JICA).

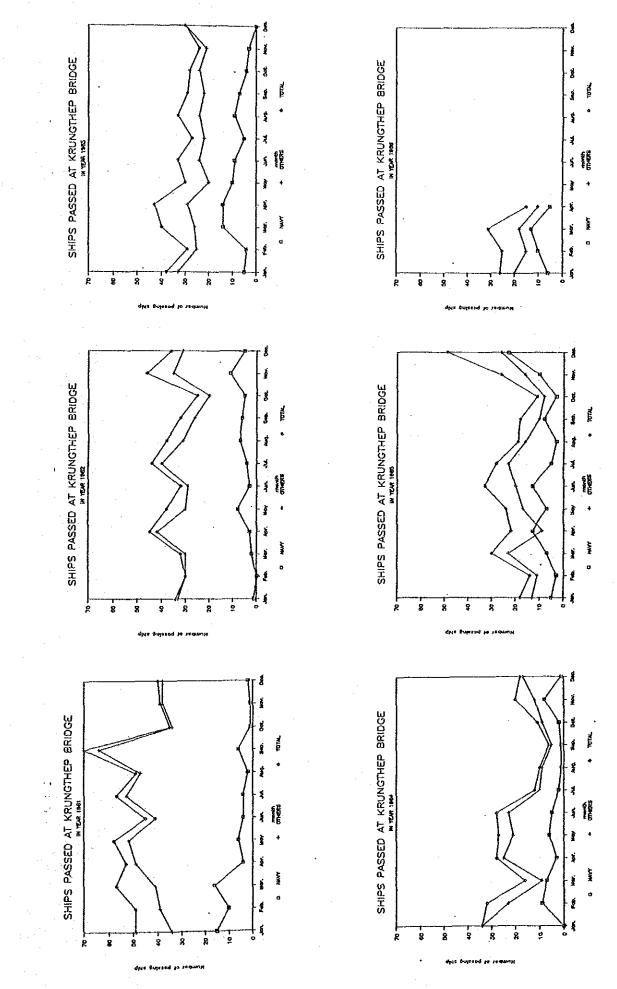
## Appendix 6.2.1

## Ships Passed at Krungthep Bridge

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A-6.1



A-6.2

Appendix 6.4.1

# Compensation Cost for River Facilities

# A-6.3

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### Compensation Costs for River Facilities

1) Bangkok Dock

As reported already in the Progress Report (I) May 1986, the compensation cost of the Bangkok Dock was estimated at about Baht 500 million with the following breakdown:

(a) Construction of New Dock

i) ii) iii)	New dock construcion cost M Baht 360 Land acquisition cost in downstream Infrastractures for Dock operation	Baht Baht Baht	360 20 45	million million million
(b)	Dismantling of Bangkok Dock and leveling works	Baht	75	million
	Total	Baht	500	million

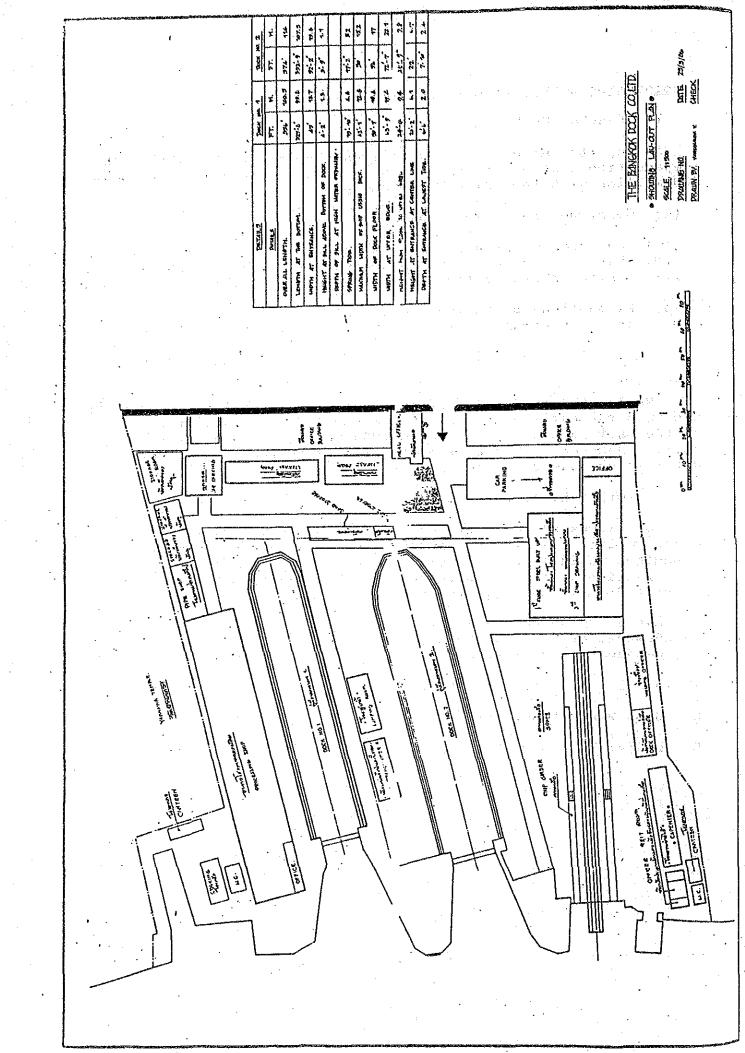
While the land value of the prime river front property along New Road was estited at the minimum about Baht 525 million based on the unit land price of Baht 60,000 per square wah. for 35,000 sq.m. However, this unit price will be reviewed later when the detailed evaluation is required.

#### 2) Other River Facilities

As discussed in Chapter 6, the Study Team has investigated other river facilities which are located upstream of the Krungthep bridge but downstream of the Sathorn bridge and compensation costs for their removal were approximately estimated at Baht 85 million as shown below:

Α.	Harin Ship Building Co., Ltd. Removal of 2-slipways only, headquaters will remain as it is.	Baht	40 million
в.	B.L.L. Shipyard Co., Ltd.	Baht	30 million
Ċ.	Show Wanakit Co., Ltd.	Baht	10 million
D.	Others,	Baht	5 million

A-6.4



A-6.5

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Appendix 8.3.1

# Movable Bridge Mechanisms

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#### Movable Bridge Mechanisms

There are three principal types of movable bridge, bascule, swing, and lift type. The following describes characteristic features of each type.

- 1) Bascule Type
  - Two types of bascule girders are usually used, such as single-leaf and double-leaf (same as Krungthep bridge) girders. In case of double-leaf girders, the longest spans constructed to date is 96.8 meters over the Bay of Cadiz, Spain. Large double-leaf girders have large deflection at the center key.
  - The size of substructure is normally large in order to provide chambers accommodating counter weights. Elimination of the chamber is possible, such as in the case of Bang Pakong bridge, but results in aesthetically poor appearance. Such large submerged structure causes a high degree of river flow disturbance.
  - The center key must be released and re-fixed at every opening operation. Together with the joint, this requires frequent maintenance.
  - The operation of this type requires double of power needed for the swing type. The length of time required for the opening and the closing operations is relatively small. Three kinds of mechanism are available, gear with electric power, gear with diesel power, and hydraulic system.

#### 2) Swing Type

- The largest of this type constructed to date is the one in the Suez canal with the total (both sides of the rotating center) length of 158.4 meters.
- When open, the girder is parallel to the navigation course, exposed to the direct collision with the vessel.
- The time required for opening and closing operations is longer than that for the bascule type, 30 to 40 minutes at a time.
- Maintenance costs are lower than the others.

#### 3) Lift Type

- The largest of this type constructed to date is the one in the James river in the U.S.A. with the total length of 126.6 meters.
- Heavy towers containing counter weights calls for the highest construction cost among the three types.

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- It limits the height of vessel.
- The time required for the opening and closing operations is shortest among the three, 15 to 20 minutes at a time.
- Mainteance costs are at the same level as the bascule type.

# Appendix 8.4.1

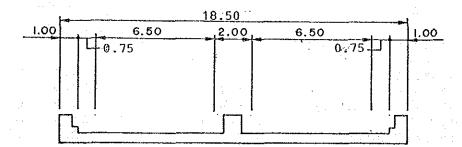
# Preliminary Cost Estimate

### Standard Cross Sections of New Bridges

Referring to the Geometric Design Standards of AASHTO, JRA and DOH, the Study Team determined the standard cross sections of the New Krungthep bridge as follows:

### 1) Twoway 4-lane bridge

- Carriage way	2 x 2 x 3.25	=	13.0	m
- Medium strip			2.0	m
- Side strip	$2 \times 0.75$	=	1.5	m
- Curb to hand hail	$2 \times 1.0$	=	2.0	m



2) Oneway 3-lane bridge

- ---Carriage way \_\_\_\_\_ Side strip \_
- Curb to Hand Rail

3	x	3.25	=
2	x	0.75	=
2	х	1.0	==

9.75

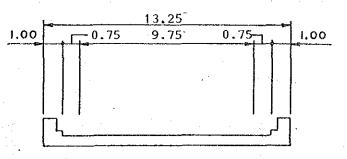
1.5

2.0

m

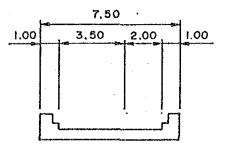
m

m



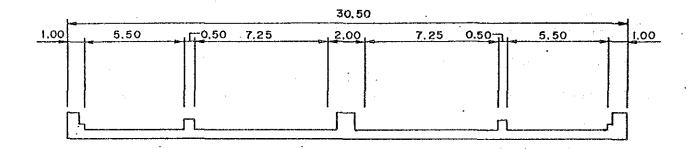
### 3) Rampway

<b>9</b> -01	Carriage way Side strip Curb to Hand Rail	1 x 3.5 1 x 2.0 2 x 1.0	*=	3.50 m 2.0 m 2.0 m
				A

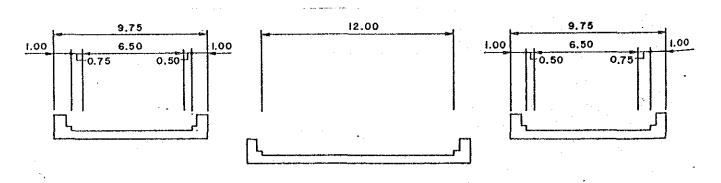


# 4) Twoway 4-lane with Rampways

 Medium strip	· · · · · · · · · · · · · · · · · · ·		2.0	m
Main carriage way	$2 \times 7.25$		14.5	m
Side separation strip	$2 \times 0.5$	=	1.0	m
Rampway	2 x 5.5	. =	11.0	m
Curb to Hand Rail	2 x 1.0	=	2.0	m



5) Separately idependent 2-2 lanes bridges



### Girder Depth

The girder depth is to decided in preliminary design to be commenced in the middle of November 1986. The Study Team roughly estimated the relationship between the girder depth and the bridge span by type of bridge as follows:

and the second second

1) PC girder by incremental launching method for Alternative D d/L =

0.06 - 0.05

L = 80 m d = 4.8 - 4.0 m

Where L = Span Length (m)d = Girder Depth (m)

2) Movable bridge for Alternative E

Bascule type d/L = 0.070

L = 85 m  $d = 5.95 \text{ m} \dots \text{E}-1 \text{ and } \text{E}-3$ d/L = 0.035Swing type L = 80 m  $d = 2.8 \text{ m} \dots E^{-2}$ 

3) Steel Box girder in side spans of Alternative E

$$d/L = 0.05$$
  
L = 65 m d = 3.25 m

4) PC Box girder by balanced cantilever election method.

d/L = 0.0165at crown d/L **≃** 0.06 at pier = 220 m d = 2.2 mL

#### Construction Methods

1) General Construction Methods

The requirements for the new bridge, necessitate the following general construction methods:

#### 이 사람은 것 같아요?

## Alternative D

Foundation works		Cast in situ concrete pile method such as reverse circulation drill method.
Temporary cofferedam	с С	Not required specially
Superstructures	•	Main bridge over the river can be constructed with PC continuous box girder by the incremental launching method.

The PC continuous box girder is selected based on the reasons listed below:

- a) PC girder with uniform girder depth is cheaper than steel box girder, which can keep NC of 7.5 m in main bridge spans.
- b) The segment casting yard is available in the adjacent land in the Bangkok side.

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<u>Alternavtive E</u>

Plan E-1 and E-2 can be constructed by the following general methods:

Foundation works :		Cast in situ concrete pile method such as reverse circulation drill method.
Temporary cofferdam :		In case of plan E-1, interlocked steel pipe type cofferdams are required for the construction of central two piers to support the bascule chambers.
Superstructures :	<u>.</u>	In case of plan E-1, in addition to the main movable bascule girders, the main side spans will be constructed by steel box girders.

Superstructures : In case of E-2, the main movable bridge is constructed with cable stayed steel girders of double span length of main navigational channel span of about 70 m. The plan E-3 which includes separate new bridges in both up and down stream sides is discussed in the succeeding Section.

#### Alternative G

The following are general methods for this type of bridge:

:

Foundation

Superstructure

Either types of concrete pile casted in situ or open caisson.

Temporary cofferdam :

In case of open caisson, the locations of main piers are situated close to each bank, so that double sheet pile type cofferdam filled with sand is applicable. In the case of concrete pile casted in situ, it is not required.

: PC box type by ballanced cantilever erection method is generally applicable. The effect of variable girder depth to NC is negligible due to the high piers.

2) Special Construction Method for Plan E-3

According to the request on the channel length of 60 m by Pilot Division of HD, the distance between the new separated bridges in up and down stream sides shall be kept in about 60 m applying a special construction method called "Close Construction", which allow it to be constructed closely to the existing structure within 20 m.

The plan is shown in Fig. A.8.4.1.1 The "Close Construction" is generally not recommended in order to keep the foundation base of the existing structure safe. However, the Study Team tried to make the advantages clear as summarized below:

Foundation works :

Bored steel pipe pile foundation is suitable, due to minimum disturbance to the existing foundation. remporary cofferdam

In addition to the interlocked steel pipe pile type cofferdam for large c hamber type pier, another independent large scale steel piles shall be driven in between the existing and new substructure to prevent the failure of existing foundation caused by new structure's construction.

After completion of new substructure, this steel piles shall be demolished to avoid ill effects to the scouring action.

Superstructure

Same as the plan E-1.

#### 3) Construction Periods

The key factor in determining the construction period of a bridge project is the concrete volume in the field, not in the factory, because the capacity of the batcher plant is the dominant factor.

:

In the case of Nonthaburi & Pathumthani Bridges Project, the construction period was 24 months with placing concrete of 26,000 cub.m.

The Wat Sai Steel Cable Stayed Bridge Project, it takes 36 months with the concrete volume of 32,000 cub.m in the field, and the Bangkok Approach Bridges Project, it takes 30 months for the placing of concrete of 31,000 cub.m in the site.

Therefore, it can be said that the maximum concrete volume per month is about 1,000 cub.m per one batcher plant.

The construction periods of each Alternative could be estimated by the above concept as below:

<u>Alternative</u>	<u>Concrete Volume</u>	<u>Construction Periods</u>
D	25,000 cub.m	24 months
E	20,000 cub.m	24 months
G	75,000 cub.m	36 months

The above concrete volumes were approximately estimated by counting the volume to be placed in the site only. In the case of the Alternative G, two (s) batcher plants shall be operated from both Bangkok and Thonburi sides unless otherwise devided into two packaged projects.

In the case of Alternative E, cofferdam works in the river and steel fabrication works in the factory were taken into consideration.

#### Land Acquisition and Other Compensation

1) Land Acquisition

Land acquisition has been a big problem for the Government as elsewhere in the world. The legal basis for the Government to acquire the right of way for public infrastructure projects has been strengthened in recent years but the enforcement sometimes still is very difficult. Therefore, the degree of difficulty in land acquisition was assessed for the evaluation of each alternative as shown in main volume.

#### 2) Compensation for Moving River Facilities

At the moment there is no legal grounds to impose private operators of shipyards, slipways and jetties to move downstream so that a low fixed bridge could be built. Properties within the right of way could be expropriated by law, at least in principle, and the Bangkok Dock, which is a government enterprise, could be moved by a cabinet decision upon request by PWD. Private operators, however, can not be forced to move for the sake of a public infrastructure project located in distance unless a new law be enacted. Under such circumstances negotiations with the private operators could be easily prolonged.

#### Project Costs

Preliminary construction cost estimates were first presented in Table 7.3.1 in page 7-15 of the Progress Report (II). Further refinements were made reflecting subsequent considerations.

- 1 Construction Costs
- 1) Bridge Cost Estitmate

Taking into account more detailed considerations PC box girder by launching method of the Alternative D, steel box girder for side spans of the Alternative E and bascule type bridge of Alternative E, bridge construction costs were re-estimated as shown below:

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PC Box girder by balanced the state the second a. cantilevered method with Baht 50,000 per sq.m high piers PC Box girder by launching b. per sq.m Baht 45,000 method with low piers Approach PC bridge to the C. 25,000 Baht per sq.m high main bridge Approach PC bridge to the d. 20,000 Baht per sq.m low main bridge Approach ramp RC bridge 12,000 per sq.m Baht e. Baht 115,000 per sq.m Movable swing steel bridge £. Baht 125,000 Movable bascule steel bridge per sq.m g. Middle span main steel box h. 70,000 per sq.m Baht girðer

#### 2) Other Cost Estimates

Other unit costs such as those for road works, miscellaneous work, contigency and engineering were kept unchanged at the level reported in the Progress Report (II). However, costs for asphalt pavement for bridge surface and concrete pavement for road surface were newly added.

#### 3) Costs for Scouring Protection

Placing of stones on the river bed is an effective protection method against scouring. The cost for the work was estimated at 30 to 50 million Baht even in the case of Alternatives D or E. However, this work could be avoided by designing substructures while neglecting the supporting power of soil above the scour level. Therefore, in this cost estimation, the protection cost against scouring was ignored considering that it could be included in the miscellaneous works cost.

4) Cost of Strengthening the Existing Bridge

In case of the plan D-3, the cost of strengthening the existing bridge was calculated as summarized below:

-			structural members and steel truss bridge	Baht	86	million
· <b>—</b>	Reinforcing	of	bascule bridge	Baht	4	million
	Dismantling	of	PC approach bridge	Baht	3	million

Total Baht 93 million

The cost of reconstructing PC approach bridge was estimated with the proposed plan.

2. Compensation Cost

1) Land Acquisition and Building Removal

Based on the results of the building conditions survey, the Study Team calculated the lands area to be acquired and counted the number of buildings by floor spaces.

It is not possible to avoid either of the two Chinese Shrines at the foot of the Krungthep bridge on the Thonburi side except in the cases of the plan G-1, and E-2.

# 2) Compensation Cost for River Facilities

For the case of Alternative D, the Study Team estimated compensation costs for moving the river facilities including Bangkok Dock as follows:

- Compensation cost for Bangkok Dock Baht 500 million
  - Compensation cost for other river facilities such as Harin 2 slipway, BLL slipway Shaw Wanakit and others
  - Revenue by selling the land of the existing Bangkok Dock Baht 525 million

Baht

85

million

The Study Team interviewed the General Manager of Supakarn condominium project to confirm the unit land price of the existing Bangkok Dock site. Mr. Somphot Piyaoui said, if the land can be sold, price would be between the maximum of Baht 80,000 per sq.wa and the minimum of Baht 60,000 per sq.wa, seeing the Dock from his office. His lowest estimate of Baht 60,000 per sq.wa was just the same with that estimated by the Study Team.

is likely, therefore that the sale of the land of the It existing Bangkok Dock would more than compensate the cost of moving it downstream. However, such a sale may take place only several years or longer after the moving, whereas the moving must be preceeded by the preparation and the construction at the new This gap in time can not be determined. Thus, for the site. sake of keeping the estimates conservative the amount only realizable by the sale of the existing site was not included in the financial and economic evaluation in this Report.

3. Traffic Loss

In the case of Alternative E, two (2) kinds of movable bridge were examined of their operating costs. Their operating costs and economic losses while bridge opening operations were estimated based on the general information and the data obtained from PWD and were reported in the reported in the Progress Report (II) as shown in the next page. Preliminary cost estimates were calculated for the following items for relevant alternatives.

- Loss to traffic due to opening of movable bridge
- Loss to traffic during construction period

For the above items computer simulation runs were carried out for the target year of 1991 against a network without the Krungthep bridge. The difference in the total cost of vehicle operation and time value during a half hour period in off-peak hours between the cases of with and without the bridge was taken as the loss to traffic per day. The difference in the total cost for a two year period was taken as the loss to traffic. Estimates are shown in Table below.

Estimation of Loss Due to Closure of Existing Bridge, 1991

		Total <u>Vehhrs.</u> (VehHr/Hr)	(Baht/Hr)	Total <u>Cost</u> (Baht/Hr)
Morning Peak	with Bridge without Bridge	99072 99574	7867418 <u>7884609</u>	
	A. Difference	502	17191	48466
Off-peak	with Bridge without Bridge	57612 59352	6001414 6016112	
	B. Difference	1740	14698	118924

Total closure for 1 year

(5 x A + 15 x B) x 365

740 million

(VOC only 112 million)

Half hour closure every day during off-peak

VOC only2.68Time19.02Total21.70 million

Note: Value of time 62.3 Baht/Hr/PCU (peak hour) 59.9 Baht/Hr/PCU (off-peak)

Cost of vehicle operation and time values were developed in terms of economic cost.

Appendix 9.2.1

Wind Probability

257

# PROBABILITY ANALYSIS OF WIND VELOCITY

Year	Velocity (knot)	Direction
1951	27	S
2	56	Б
3	43	ENE
	40	WSW
5	43	NW
6	45	NNW
7	51	SW
8	44	SSW
9	52	E
1960	35	ESE
1 1.	48	ENE
2	52	ESE
23	41	W
4	40	SE
5	41	S
6	40	W
7	42	NNW
8	37	ESE
9	36	WSW

Annual Max	imum Wind	Velocity	Records
------------	-----------	----------	---------

Year	Velocity (knot)	Direction
1970	41	N
1	33	WSW
2	42	W
3	25	W
4	27	NW
5	40	NE
6	32	SE
7	28	SW '
8	32	NE
9	36	SW
1980	42	WSW
1 1	32	WSW
2	40	SW
3	38	ESE
4	35	NNE
5	32	W

Probability analysis was made by the Study Team using a) Gumbel's method,b) Order Probability method and c) Gumbel-Weibulls' method. The results are summarized below:

Return Period		Maximum Wind Velocity	/ (knot)
(Year)	a)Gumbel	b)Order Probability	c)Gumbel-Weibull
5	45.3	45.6	45.1
10	50.1	49.6	49.0
30	57.4	54.8	54.2
50	60.8	56.9	56.3
100	65.3	59.7	58.9
	1	}	· ·

The maximum value for 100 year probability was obtained by Gumbel's method (above a) ). Hence, the design wind velocity Vd is determined as:

 $V_{100} = 65.3$  knots = 120.9 km/h = 75.2 mph, say 80 mph  $V_D = 80$  mph ( equivalent to 130 km/h ) --- same as Sathorn Br.

A-9.2

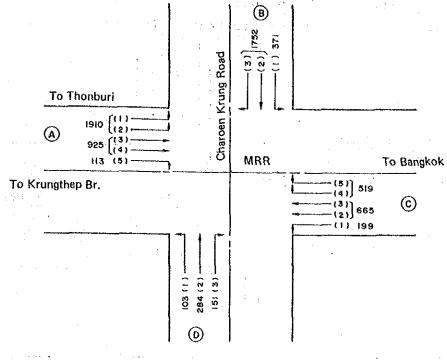
# Appendix 9.4.1

# Caculation of Saturation Degree for Examination of Intersection Type

A-9.3

Traffic Saturation Degree of Intersections Intersection"A " in 2001

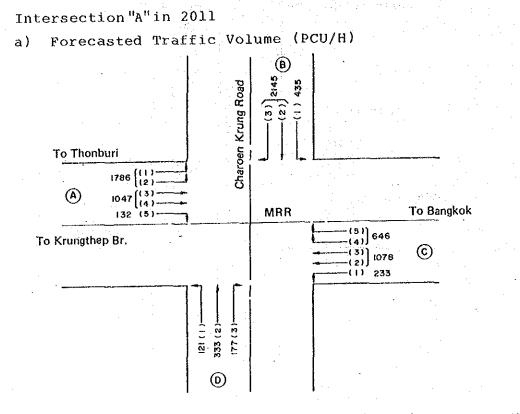
a) Forecasted Traffic Volume (PCU/H)



b) Calculation of Traffic Saturation Degree

	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maxin Satur Degre	ation
—- А	ſ(1)	2000	955	0.48		
	<b>(2)</b>	2000	955	0.48	-	
	U (3)	2200	463	0.21		
	~ <sup>(4)</sup>	2200	463	0.21		
<u>.</u>	(2) (5)	2000	113	0.06		
6 6	ſ(1)	2000	371	0.19		
	(3) (2)	2000	876	0.44		
	∼L(3)	2000	876	0.44		
c	 ۲(1)	2000	199	0.10	Phase1	0.48
	$(\mathbf{\hat{n}})$ (2)	2200	333	0.15	Phase2	0,13
	Ŭ (3)	2200	333	0.15	Phase3	0.44
	G(4)	2000	260	0.13	Phase4	0.13
	C(5)	2000	260	0.13		1.18
D	(1)	2000	103	0.05	-	C
	a (2)	2200	284	0.13	· · ·	
	U (3)	2000	151	0.08		
	otal				1.18 >	1.0

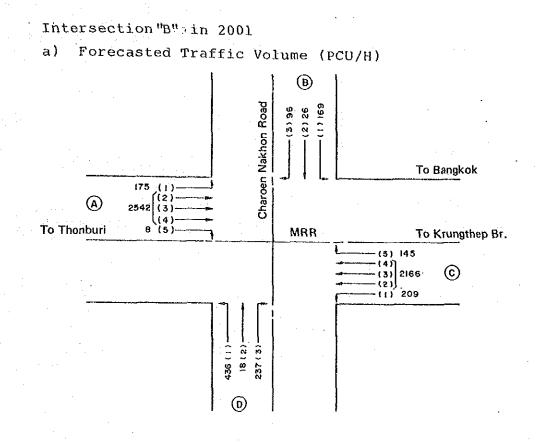
A-9.4



Calculation of Traffic Saturation Degree b;

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturat Degree	- 14	ration
Α ((1)	2000	893	0.45	·	
$\bigcirc$ (2)	2000	893	0.45		<i>i</i> .
$\cup$ (3)	2200	524	0.24		
L(4)	2200	524	0.24		1
(2) (5)	2000	132	0.07		× .
β (1)	1930	435	0.23		· · ·
(3)(2)	2000	1073	0.55		
(3)	2000	1073	0.55	a fit	ma e t
; (1)	2000	233	0.12	Phase1	0.4
(1) (2)	2200	539	0.25	Phase1	
(13)	2200	539	0.25	Phase3	
G(4)	2000	323	0.16	Phase4	
©((5)	2000	323	0.16		
· ~ • <b>~</b> ~					1.31
(1)	2000	121	0.06		
(4) (2)	2200	333	0.15		an an an an an
= L(3)	2000	177	0.09		Ξγ.
Total				1.31	> 1.0

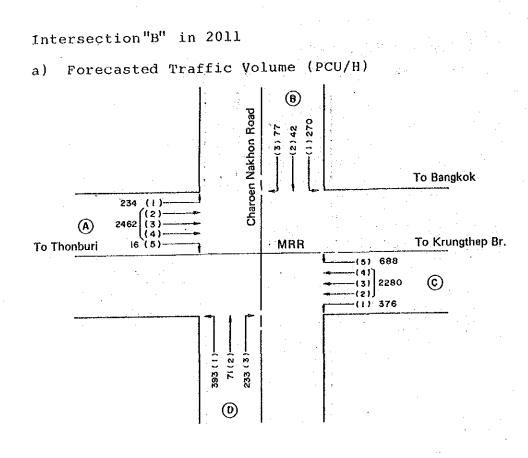
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b) Calculation of Traffic Saturation Degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	
$ \begin{array}{c}                                     $	2000 2200 2200 2200 2200 2000	175 847 847 847 847 8	0.09 0.39 0.39 0.39 0.39 0.01	
$\mathfrak{T}_{(2)}^{\mathfrak{B}}$	2000 2200 2000	169 26 96	0.08 0.01 0.05	
$ \begin{array}{c} c \\ (1) \\ (2) \\ (3) \\ (4) \\ (5) \end{array} $	2000 2200 2200 2200 2200 2000	209 722 722 722 722 145	0.10 0.33 0.33 0.33 0.33 0.07	Phase1 0.39 Phase2 0.07 Phase3 0.08 Phase4 0.22
$\Phi \left( \begin{array}{c} 1 \\ 2 \\ 3 \end{array} \right)$	2000 2200 2000	436 18 237	0.22 0.01 0.12	- 0.76
Total				0.76 < 1.0

apacity and saturation degree

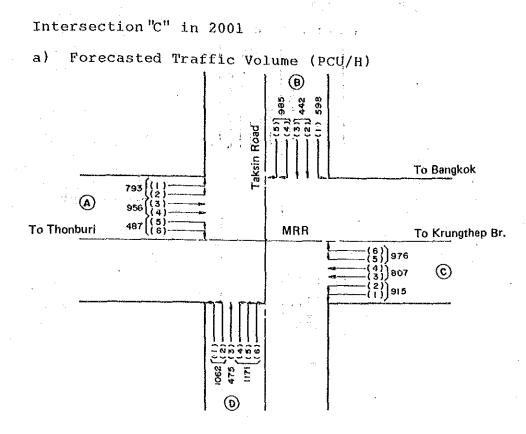


b) Calculation of Traffic Saturation Degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
$ \begin{array}{c}     (1) \\     (2) \\     (3) \\     (4) \\     (2) \\     (5) \end{array} $	2000 2200 2200 2200 2200 2000	234 821 821 821 821 16	0.12 0.37 0.37 0.37 0.37 0.01	
$ \overset{(1)}{=} \overset{(1)}{\overset{(2)}{=}} \overset{(2)}{\overset{(3)}{=}} $	2000 2200 2000	270 42 77	0.14 0.02 0.04	_
$ \begin{array}{c} (1) \\ (2) \\ (3) \\ (4) \\ (2) \\ (5) \end{array} $	2000 2200 2200 2200 2200 2000	376 760 760 760 760 688	0.19 0.35 0.35 0.35 0.35 0.34	Phase1 0.37 Phase2 0.36 Phase3 0.20 Phase4 0.16
$ \begin{array}{c}                                     $	2000 2200 2000	393 71 233	0.20 0.03 0.14	- 1.05
Total				1.05 > 1.0

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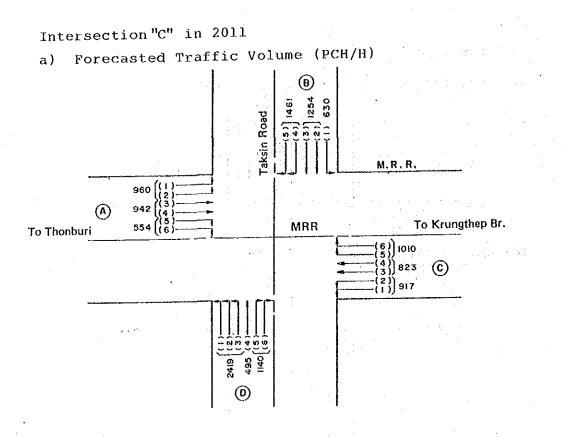


b' Calculation of Traffic Saturation Degree

	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
 A	r(1)	2000	397	0.20	
	(2)	2000	397	0.20	
	(3)	2200	478	0.22	
	$\cup$ (4)	2200	478	0.22	
	(5)	2000	244	0.12	
	L(6)	2000	244	0.12	
в	ſ(1)	2000	299	0.15	-
	(3)(2)	2000	299	0.15	
		2200	442	0.20	
	Q(4)	2000	493	0.25	
	<b>(4)</b> ((5)	2000	493	0.25	_
 C	r(1)	2000	458	0.23	Phase1 0.22
•	(2)	2000	458	0.23	Phase2 0.25
	~ (1)	2200	404	0.18	Phase3 0.22
	(2) (4)	2200	404	0.18	Phase4 0.27
	(5)	2000	488	0.25	
	L(6)	2000	488	0.25	0.96
 D		2000	531	0.27	
·	<b>(12)</b>	2000	531	0.27	
	· (3)	2200	475	0.22	*
	alin	2200	390	0.20	
	3 3	2000	390	0.20	
	- L(6)	2000	390	0.20	
T	otal	****			0.96 < 1.0

Capacity and saturation degree

A-9.8



b) Calculation of Traffic Saturation Degree

Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
r(1)	2000	480	0.24	
(2)	2000	480	0.24	
$\bigcup$ (3)	2200	471	0.21	
L(4)	2200	471	0.21	
Q (5)	2000	277	0.14	
@L(6)	2000	277	0.14	-
 (1)	2000	630	0.32	
(3) (2)	2200	627	0.29	
	2200	627	0.29	1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -
$\alpha$	2000	. 731	0.37	
<b>(4)</b> (5)	2000	731	0.37	_
r(1)	2000	459	0.23	- Phase1 0.24
(2)	2000	459	0.23	Phase2 0.25
$\mathbb{O}[\tilde{\omega}]$	2200	412	0.19	Phase3 0.40
(4)	2200	412	0.19	Phase4 0.3
C ((5)	2000	505	0.25	
$\mathbb{O}[6]$	2000	505	0.25	1.20
۲(1)	2000	806	0.40	••• • • • • •
- 1105	2000	806	0.40	
(3)	2000	806	0.40	
(14)	2200	495	0.23	· · · ·
$\sim (5)$	2000	570	0.23	
$(4)_{(6)}$	2000	570	0.23	•
otal				1.26 > 1.0

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anacity and saturation degree

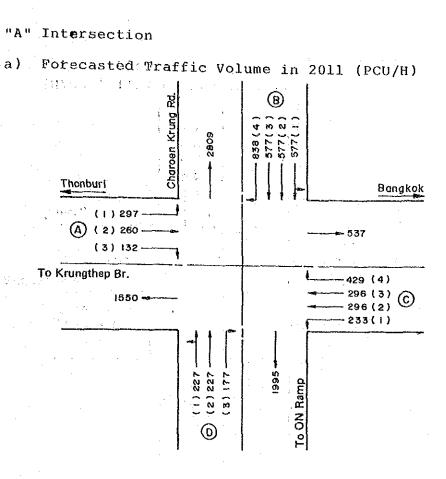
A-9.9

Appendix 9.4.2

#### Calculation of Saturation Degree for Examination of At-Grade Intersection (A, B and C Intersection),

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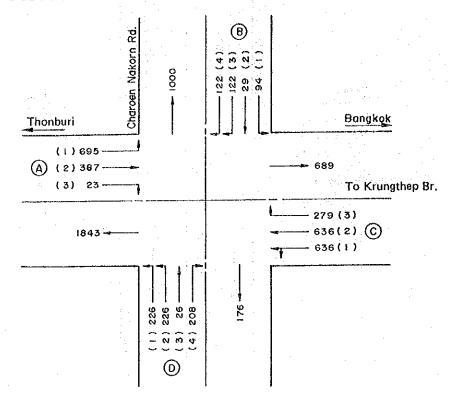


# b) Calculation of Traffic Saturation Degree

. 5	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A		ation
 А	(1)	2000	297	Free		
	(2)	2200 2000	260 132	0.12 0.06		
 B	(1)	2105	577	0.27		
	(2)	2105	577	0.27		
	(3)	2105	577	0.27		
	(4)	2000	838	0.42		
с.	(1)	2000	233	0.11	Phase1	0.12
•	(2)	2200	296	0.13	Phase2	0.23
	(3)	2200	296	0.13	Phase3	0.43
	(4)	2000	429	0.21	Phase4	0.13
 D	(1)	2035	227	0.11		0.86
D	(2)	2035	227	0.11		
	(3)	2000	177	0.09		
	tal				0.86 <	0.90

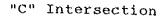
#### "B" Intersection

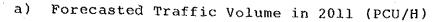
a) Forecasted Traffic Volume in 2011 (PCU/H)

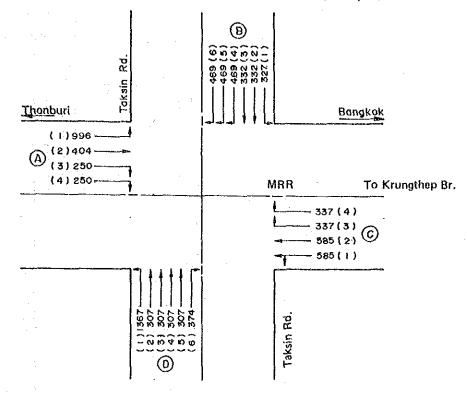


#### b) Calculation of Traffic Saturation Degree

	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maxim Satur Degre	ation
A.	(1) (2)	2000 2200	695 387	Free 0.18	~ ~ ~ ~ ~ ~ ~ ~ ~ ~	· <b></b>
	(3)	2000	23	0.01	· · ·,	
3	(1)	2000	94	0.05		
	(2)	2200	29	0.01	· . ·	
	(3)	2000	122	0.06	$\{a_{ij},a_{ij}\}_{i \in I} \in \mathbb{N}$	
	(4)	2000	122	0.06		
;	(1)	2145	636	0.30	Phase1	0.30
	(2)	2145	636	0.30	Phase2	0.14
	(3)	2000	279	0.14	Phase3 Phase4	0.11
)	<b>{1}</b>	2000	226	0.11		
	(2)	2000	226	0.11		0.65
	(3)	2200	26	0.01		
	(4)	2000	208	0.10	- X	<sup>1</sup>
7	otal			·	0.65 <	0.90



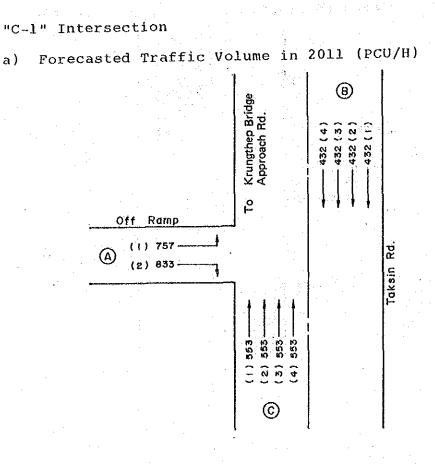




#### b) Calculation of Traffic Saturation Degree

:	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
 A	(1)	2000	996	Free	
	(2)	2200	404	0.18	
	(3)	2000	250	0.13	
	(4)	2000	250	0.13	
в.	(1)	2000	327	0.16	
-	(2)	2200	332	0.15	
	(3)	2200	332	0.15	
	(4)	2000	469	0.23	
	(5)	2000	469	0.23	
	(6)	2000	469	0.23	
с	(1)	2035	585	0.29	Phasel 0.29
Ť	(2)	2035	585	0.29	Phase2 0.17
	(3)	2000	337	0.17	Phase3 0.14
۰.	(4)	2000	337	0.17	Phase4 0.23
D	(1)	2000	1367	Free	0.83
	(2)	2200	307	0.14	
	(3)	2200	307	0,14	
	(4)	2200	307	0.14	
	(5)	2200	307	0.14	
	(6)	2000	374	0.19	
 TC	otal	~			0.83 < 0.90

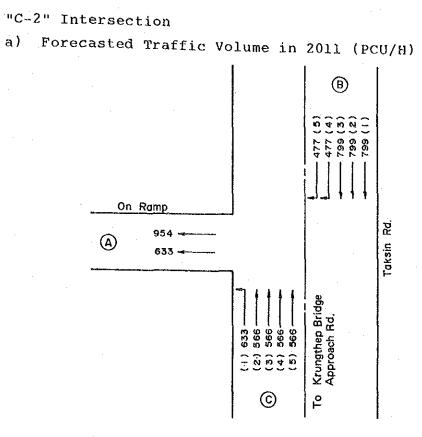
Capacity and saturation degree



a)

#### Calculation of Traffic Saturation Degree b)

. 5	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	
A	(1) (2)	2000 2000	757 833	0.38 0.42	
B	(1) (2) (3) (4)	2200 2200 2200 2200 2200	432 432 432 432 432	0.20 0.20 0.20 0.20 0.20	Phase1 0.42 Phase2 0.25 0.67
C	(1) (2) (3) (4)	2200 2200 2200 2200 2200	553 553 553 553 553	0.25 0.25 0.25 0.25 0.25	



#### b) Calculation of Traffic Saturation Degree

	Section	Capacity(A) V/H	Future Traffic Volume(B) V/H	Saturation Degree B/A	Maximum Saturation Degree
B	(1)	2200	799	0.36	
	(2)	2200	799	0.36	
	(3)	.2200	799	0.36	
	(4)	2000	477	0.24	· .
	(5)	2000	477	0.24	
с	(1)	2000	633	Free	Phasel 0.36
	. (2)	2200	566	0.26	Phase2 0.20
	(3)	2200	566	0.26	~~~~~~~~~~~~
	(4)	2200	566	0.26	0.62
	(5)	2200	566	0.26	
1	'otal				0.62 < 0.90

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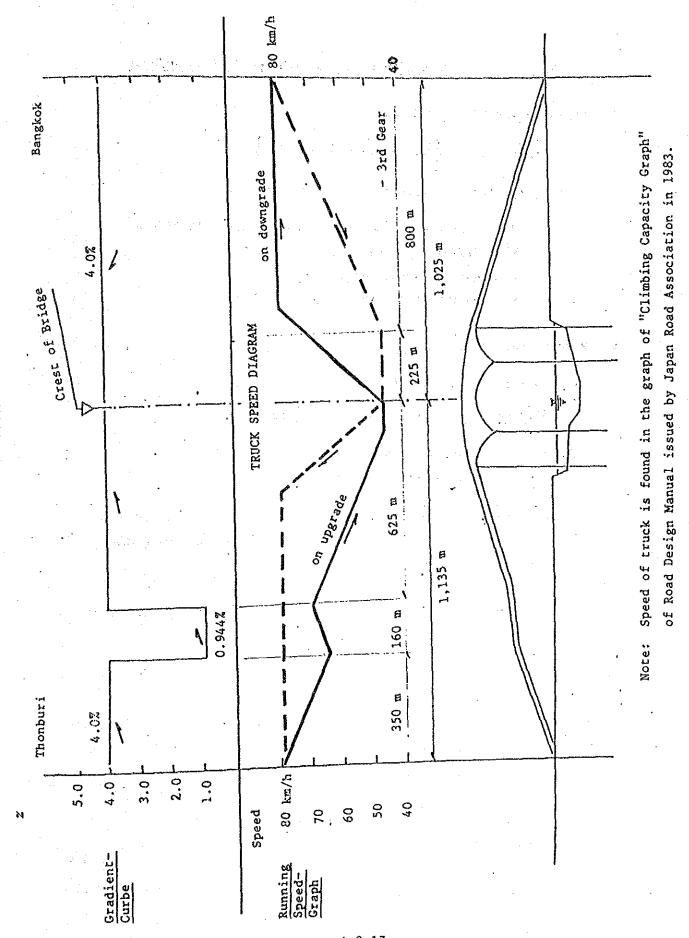
a)

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Appendix 9.4.3 Running Speed of Truck on upgrade of 4.0%

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Theoretical Possibility to Climb with Speed of 45 km/hour  
A. Engine power (T) by 3rd gear  

$$T = \frac{270}{V}$$
. H. E  $\left\{ 1.2 - 1.3 \left( \frac{V}{V} - 0.6 \right)^2 \right\}$   
 $= \frac{54.2 \text{ kg/ton}}{V}$  (power per loading ton)  
where, H : Engine horse power per loading ton: 13 PS/ton  
E : Efficiency of engine power, 0.7\*  
v : Running speed, 45 km/hour  
V : Maximum speed by 3rd gear, 45 km/hour  
B. Running resistance (R) on 4.0% grade  
R = 10 + 10 i + 0.0015 v<sup>2</sup>  
 $= \frac{53.2 \text{ kg/ton}}{V}$  (power per loading ton)  
where, i : Gradient %, 4.0  
v : Running speed, 45 km/hour  
Based on the above calculation, the engine power (T) by 3rd gear  
(54.2 kg/ton) exceeds the running resistance (R) on 4.0% grade  
(53.2 kg/ton): T > R.  
Note: 1. The efficiency of engine power for 3rd gear is adopted at 0.7

considering the conditions of trucks is Thailand.

2. Engine hourse power per loading ton in Japan:

11 ton truck, 13.2 - 15.0 PS/ton
8 ton truck, 13.1 - 13.4 PS/ton
6 ton truck, 12.0 - 14.4 PS/ton

4 ton truck, 16.8 - 18.2 PS/ton

3. 1.0 PS = 75 kgf.m/s

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Appendix 9.4.4 Examination of the Distance among Intersections "C", "C-1" and "C-2"

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# Examination of the Distance among Intersections

Distances among intersections were examined from the following points:

- The layout of three (3) intersections which consist of the Taksin Intersection is shown in sheet No. 1/47 in Drawing Volume, the distance from C-1 to C being 215 m, and from C to C-2 165 m, respectively;
- The detailed layout of the At-Grade Intersection "C" is shown in sheet No. 19/47 in Drawing Volume, there are clearly shown the number of lane by directions and the length of storage lanes for right turning within the total length of 100 m toward C-1 from C, and toward C-2 from C respectively. The scale of above intersection was determined based on the storage lane-length required, with its details are shown hereinafter; and
- On the other hand, the examination on weaving length required on the Taksin road was neglected because the effect of traffic from/into ramps is to be controlled by signals as same as the case of the road without ramps.

#### Details of the Storage Length required

The traffic data by directions at the C-intersection has been indicated in Appendix 9.4.2, and using these traffic volumes the length of storage lane required at morning peak period were examined by the following formulas:

#### $L = 1.5 \times N \times S$

where,	L :	Length of storage lane required
	N° :	Number of cars flowing during a cycle time from Green
·		signal to the next Green signal
· · · · · · · ·	S .:	Length occupied by a car in the storage lane, 7.0 m

#### N =: qc/ 3600

where,

q : Traffic volume per hour per lane at morning peak C : Cycle time of the Green required

c ≧ 0.9	T	1	(0.9 - R)	
te est to			-1 less time out	na - Salayan Din yaara ugu aara (fe
			Clearance loss time est	
х ізт. 	R	:	Saturation flow rate es	timated, 0:65 when the descent of

The cycle time is, therefore, calculated to be about 40 sec, the storage lane lengths required based on these formulas are less than the actual distances as shown in the Table below, therefore, no trouble in traffic flows at these intersections will happen.

		194 - Contra 194 - C	
·	Table Storage Lane	Length Required	and the second second second
	······································		e de la companya de l

n en la companya de la placementa de la décargementa de

Flow		affic by ection	Number of Lane	q (PCU/h)	N (NOS)	1.5xS (m)	Ĺ (m)	Distance (m)
	L:	327	1	327	4	10.5	42	
B	C:	664	2	332	4	10.5	42	< 165
:	R:	1,407	3	469	5	10.5	53	
	L:	1,367	1	1,367	15	10.5	158*	
D	· C:	1,228	. 4	307	4	10.5	42	< 215
	R:	374	1	374	4 .	10.5	42	ating and
	 L:			· · · ·	· · · · · · · · · · · · · · · · · · ·			
Е	C:	1,848	4	462	5	10.5	53	< 215
	R:		-	-	-	•7		
. <u> </u>	L:	633	1	633	7	10.5	74	
H	•C:	2,264	3	754	8	10.5	84	< 165
	R:	**	. <b>***</b>	1 - 1 - 12 <b>-</b>				
	L:	757	2	379		10.5	42	· · · · · · · · · · · · · · · · · · ·
F	C:	-				· · · ·		∕∵′< 70
	R:	833	2	417	<b>5</b>	10.5	53	4014-2

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a she water we apply

Note -

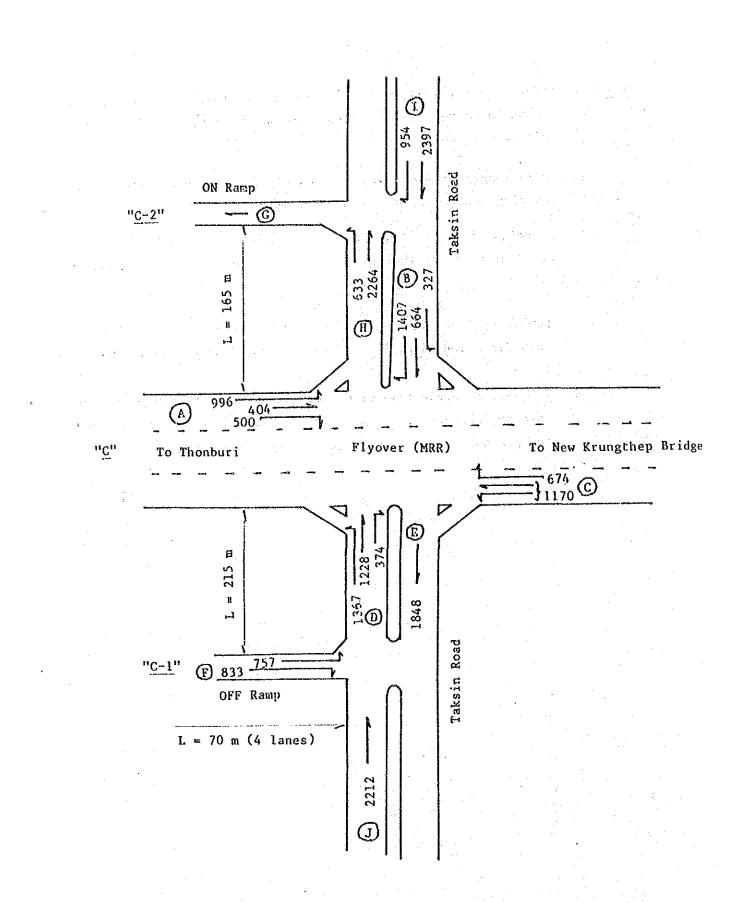
L: Left turning traffic flow

C: Straight traffic flow

R: Right turning traffic flow

- Note: 1. Traffic volumes on the Taksin road shown above are already accumulated volumes of inflow from the Off Ramp of C-1 or the Middle Ring Road.
  - \*2. More longer or full Green cycle time will be given to the left turning traffic at D-flow by channelization.
  - 3. A sketch of these flows is attached in the next page, however, other details are the subjects to the future Detailed Design.
  - Number of lanes for straging on the OFF Rump is 2 lanes for each direction with 70 m length from the edge of Taksin road.

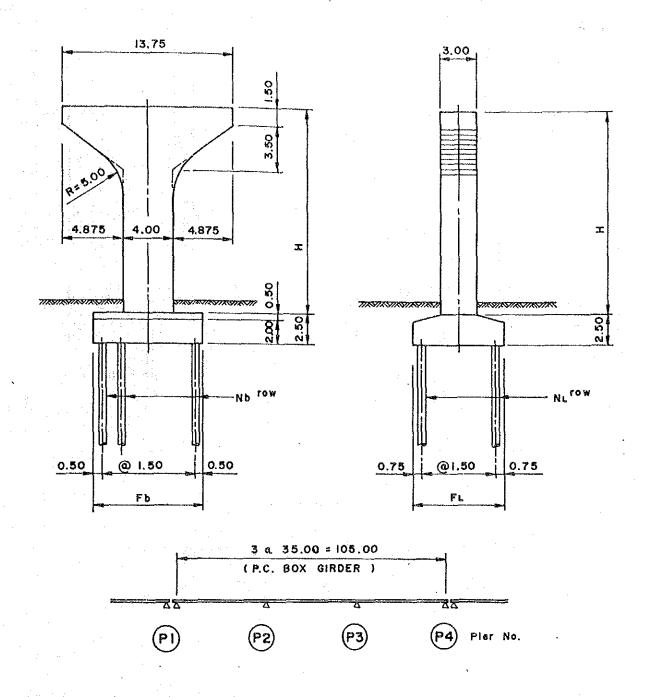
5. A theorized cycle time of 40 sec discussed in this paper may be able to increase up to max. 90 sec, if required, provided that the present intersection plan is kept as proposed.



Turning Movement (2011) (Morning Peak Hour)

# Appendix 9.5.1

Relationship Between Height of Pier and Number of Pile

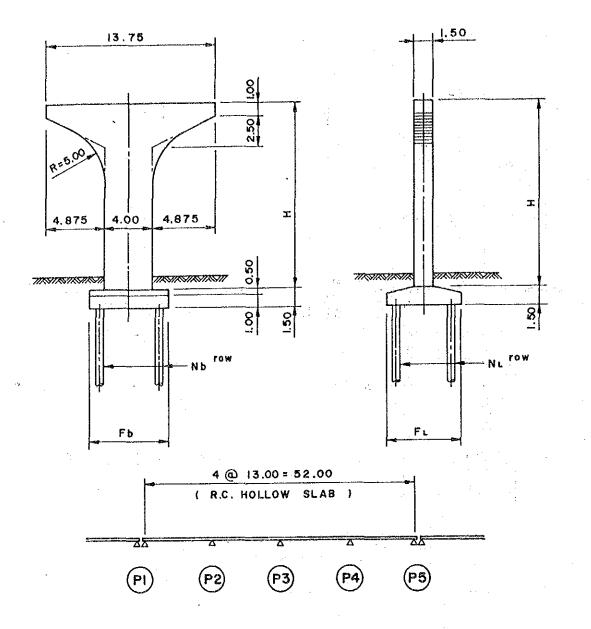


NOTE: N = NOS OF PILE

Pier NO.	Pler Height (m)	10		25 	30
(PI),(P4)	FbxFL <sup>(m)</sup>	7.50 x 6.00	7.50 x 7.50	9.00 x 7.50	•
	NEX NL TOW)	5 x 4	5 x 5	6 x 5	
	N. A. A.	20	25	30	
9	FD X FL	7.50 x 7.50	9.00 x 7.	50 9.00 x	9.00
P2),(P3)	NDX NL	5 x 5	6 x 5	6 x	6
	N	25	30		

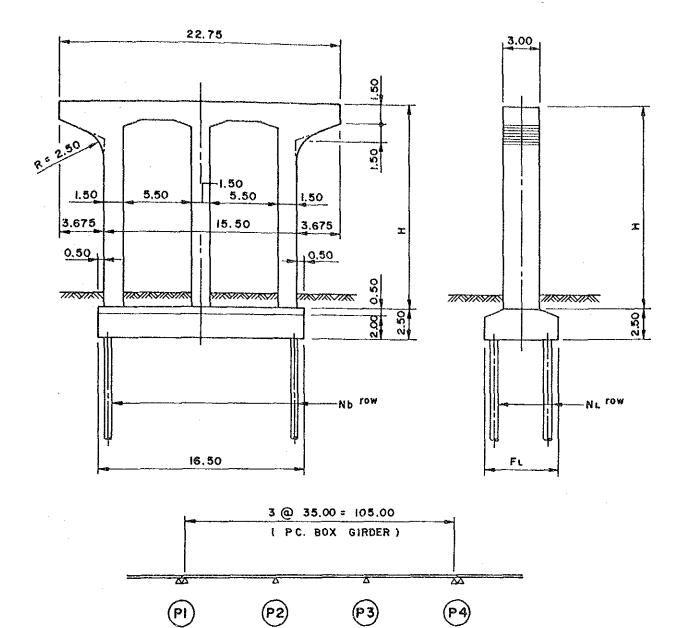
PIERS FOR PC BOX GIRDER





ler No.	Pier Height (m)	5	و و و و و و و و و و و و و و و و و و و	, 10		<u>1</u>	15
PI),(P5)-	FbxFL <sup>(m)</sup>	6.00 x 3.00	6.00 x 4.	50		6.00 x 4	.50
	ND X NL (row)	4 x 2	4 x 3			4 x 3	<b>.</b>
	N	8	10		<u> </u>	12	
P2 P3	Fb x FL	6.00	) x 4.50		6,00	) x 4.50	
P4)	NB X NL	4	x 3		4	x 3	
<u>ש</u> ן ש	N		10		• .	12	

# PIERS FOR RC. HOLLOW SLAB



NOTE : N = NOS OF PILE.

Pier No.	Pier Height (m)	10	15 1		2	0	25
			ee	P		·	
(PI),(P4)	16.50 x F⊥ <sup>(m)</sup>	16.50 x 4.50	16.50x4.50	16.50	x 6.00	16.50	x 6.00
	NE X NL(row)	10 x 3	li x 3	9 ;	x 4	10	x 4
	N	30	33	3	6	4	0
			• • • • • • • • • • • • • • • • • • •		·		9
(P2) (P3)	16.50 x FL	16.50 x 6.00	16.50 x (	6.00	16,50	0 x 6.00	16,50×6.00
6063	ND X NL	9 x 4	10 x	4	11	x 4	10 x 5
	N	36	40			44	50

# PIERS NEAR INTERCHANGES



→ 19 (4) (10 (19)<sup>3</sup>) (10)<sup>3</sup> (10)<sup>3</sup>

#### Appendix 9.7.1 Data on Cost Estimates

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1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1     1 <th></th> <th>Contractors</th> <th></th> <th></th> <th><b>*4</b></th> <th>-</th> <th>U S</th> <th><b>A</b></th> <th>14 14 1</th> <th><b>5</b>4</th> <th></th> <th>Lovest</th> <th>Average</th> <th>Rounded</th>		Contractors			<b>*4</b>	-	U S	<b>A</b>	14 14 1	<b>5</b> 4		Lovest	Average	Rounded
1. Labour Costs         1. Costs														777
Unit Baht per day         Di Stilled Operatur         200         132         130         140         150         100         100         150           2. Matrial Casts         1         Structur         200         150         150         100         100         100         100         150.11           2. Matrial Casts         1         Structur         200         150         150         100         100         100         100         100         100         100         253.6           1. Bakt part ton         1         Structur         200         150         150         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100	Unit: Bah		-	Jur	140	108	15	08	20	140	10	10	5	
2. Naterial Costs       20       250       130       130       150       190       151.4         3. Expert worker       200       150       150       1700       100       100       100       100       151.4         1. Strett worker       1. Strett worker       200       150       150       1750       1750       1700       100       131.4         1. Strett worker       200       150       150       1750       1750       1750       1700       200       100       100       100       131.6       1100       100       100       1100       100       1100       100       1100       100       1100       100       1100       100       1100       100       1100       100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100       1100 <td></td> <td></td> <td></td> <td>Inoc</td> <td>240</td> <td>132</td> <td>130</td> <td>140</td> <td>150</td> <td>250</td> <td>DOI</td> <td></td> <td>163</td> <td></td>				Inoc	240	132	130	140	150	250	DOI		163	
2. Kiterial Costs       3       50       15       200       100       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       200       100       100       200       200       200       200       200       200       200       200       200       200       200       100       100       200       200       200       200       200       100       100       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5       201.5 <td< td=""><td></td><td></td><td></td><td></td><td>200</td><td>250</td><td>120</td><td>130</td><td>120</td><td>150</td><td>06</td><td></td><td>-1 -1</td><td></td></td<>					200	250	120	130	120	150	06		-1 -1	
2. Material Costs       50       150       150       500       120       120       120       121.63       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.457       1.452       1.452       1.475       1.475       1.475       1.475       1.475       1.475       1					240	360	175	200	120	250	100		206.4	
2. Barterial Costs       3       Distribution       200       150       150       100       100       100       203.45         1. Barty per tons       1. Barty per tons       1. Barty per tons       1. Stand Interenting cement       1.100       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501       1.501					600	360	175	200	200	200	120		307.9	
2. Material Costs a) Portland cement 1,630 1,659 1,730 1,630 1,706 - 1,571 1,500 1,703 138.0 1,706 1,700 1,706 2,138 1,500 1,706 - 1,571 1,500 1,703 138.0 1,706 1,706 1,700 1,700 1,703 138.0 1,706 1,700 1,706 1,700 1,703 138.0 1,700 1,700 1,700 1,701 1,500 1,703 1,700 1,701 1,500 1,703 1,700 1,703 1,700 1,703 1,700 1,703 1,700 1,703 1,700 1,703 1,700 1,703 1,700 1,703 1,700 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,701 1,700 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1,7000 1				ker .	260	360	155	250	150	200	100	100	253.5	
Unit of cement a steel       b) Rapid hardening cement       1,760       2,158       1,500       1,750       1,740.8       1         i       Bahk per ton       concrete(130 Mg/sq.cm)       1,060       1,035       1,000       1,125       1,001       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,010       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,035       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011       1,011 <td></td> <td>Costs</td> <td></td> <td>ament</td> <td>1.630</td> <td>1.669</td> <td>5</td> <td></td> <td>1.603</td> <td>ı</td> <td>.45</td> <td></td> <td>1,619.8</td> <td>, स</td>		Costs		ament	1.630	1.669	5		1.603	ı	.45		1,619.8	, स
<pre>: Baht per ton c) Said Inft of aggregate 110 250 128 150 1.001 1.031 1.001 1.031 1.01 1.001 1.035.0 1.035 Unit of aggregate concrete(130 K/sq.cm) 1.000 1.031 1.001 1.031 1.015 1.035 1.035.1.115 i Baht per cu.m f) PC concrete(130 K/sq.cm) 1.100 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.101 1.201 1.175 1.1015.1 1.175 1.1010 1.101 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201 1.201</pre>	Unit of				1,760	2,158	• •	•	1.706	1	1.571		1 740 8	-
Unit of aygregate,       170       250       130       135       175       -       180       270       190.0         Unit of aygregate,       0       Concrete(300 kg/sq.cm)       1,100       1,150       1,125       -       1,135       1,005       1,125       1,210       1,135       1,135       1,205       1,135       1,205       1,135       1,205       1,135       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,205       1,215       1,205       1,205       1,205       1,205       1,215       1,205       1,215       1,205       1,215       1,205       21,205       21,105       1,122       1,215       1,250       21,105       1,122       1,250       21,105       21,505       21,505       21,505       21,505       21,505       21,505       21,505       21,505       21,505       21,505       21,550		ht per ton			140	250		•	130	I	150		158.0	
Unit of aggregate, e) Concrete(180 kg/sq.cm) 1,050 1,085 1,100 1,125 - 1,015 1,125 1,125 1,115 1,200 1,205 1,115 1,200 1,200 1,201 1,201 1,125 1,115 1,205 1,115 1,200 1,200 1,201 1,210 1,125 1,125 1,115 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 2,2150 2 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 1,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,200 10,20	·	·	_	Jredate	170	250	180	185	175	ł	180		190.0	• •
Concrete & Yood       f) Concrete (240 Kg/sq.cm)       1,100       1,100       1,120       1,120       -1,135       1,125       1,115.8       1,230       1,210       -1,135       1,210       1,126       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,125       1,112       1,125       1,112       1,125       1,112       1,125       1,112       1,125       1,125       1,125       1,125       1,125       1,125       1,125<	Unit of a	garegate,	-	30 kg/sg.cm)	1,050	1,000	1,030	1.050	1.035	ł	1,045		1,035.0	r.
<pre>: Baht per cu.m g FC concrete(300 kg/sq.cm) 1,200 1,115 1,200 1,230 1,230 1,220 1,1175 1,205.8 1 1) FC concrete(300 kg/sq.cm) 1,200 1,2100 0,240 7,450 9,2900 9,212 1) FC concrete(300 kg/sq.cm) 1,200 10,240 7,450 9,240 7,450 9,232.9 9 1) FC wire 21,000 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,200 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,500 21,0</pre>	concret	e & vood	-	40 kg/sg.cm)	1.100	1.085	1.100	1.150	1.125	Í	1.135		1,115.8	
h) FC concrete(350 Kg/sq.cm)       1,250       1,250       1,290       1,300       1,250       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,270       1,300       1,250       1,250       2,2750.0       21,000       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,501       21,501       21,501       21,501       21,501       21,501       21,501 <t< td=""><td>100</td><td>ht per cu.m</td><td></td><td>= (300 kg/sg.cm)</td><td>1,200</td><td>1,175</td><td>1.200</td><td>1.230</td><td>1.210</td><td>1</td><td>1.220</td><td></td><td>1,205.8</td><td></td></t<>	100	ht per cu.m		= (300 kg/sg.cm)	1,200	1,175	1.200	1.230	1.210	1	1.220		1,205.8	
1       Found ber       7,300       9,100       10,000       10,340       7,450       9,232.30       9,500       1,450       9,232.30       9,500       1,450       9,232.30       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,501       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500       21,511       12,500				e(350 kg/sg.cm)	1,250	1 2 2 2 2	1.270	1.300	1.290	ı	1 300		1.277.5	
j) peformed bar       7,900       8,940       7,650       9,960       7,650       9,960       7,650       9,960       7,650       9,960       7,650       9,960       7,650       9,960       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,550.0       21,551.1       12,500       12,550.1       12,550.1       12,550.1       12,551.1       12,500       12,550.1       21,11.1       12,500       12,550.0       21,200       12,550.1       21,20       21,551.1       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,500       12,651.1       12,500       12,651.1       12,500       12,500       12,500			Rou		7,800	9.100	10,200	10,000	10.340	7.450	9.740		9.232.9	9
X)       PC wire       21,000       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,500       21,550       21,000       21,550       21,500       21,550       21,500       21,550       21,500       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,550       21,74       4,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,550       4,410       1,500       4,610       5,500       4,500       5,500       4,500       5,500	з				7.900	8.940	10.000	11.120	10.160	7.650	960		9.390.0	0
I) FC strand $  10,900$ $21,500$ $ 21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,500$ $21,511$ $12,200$ $21,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $12,500$ $4,191$ $4,200$ $4,200$ $4,200$ $4,200$ $4,200$ $4,200$ $4,200$ $4,200$ $4,200$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $25,100$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$ $5,500$					21,000	21,500	21 500	1 6	21.500	•	21.500		21,550.0	~
m)       PC bar       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       10,860       10,903       10,903       11,000       9,400       10,242.8       10,111       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12       12					21,000	21.500	21.500	1 60	21.500	I	21.500		21 550 0	212
Noci for for stand at the form of t			С G		1	·	10.900	i Ön	10.860	ı	I		10.903_3	10.9
o) Steel pipe       13,000       14,000       12,657.1       12         p) Wood for formworks       4,900       13,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       9,000       14,000       1,500       4,190       1,500       4,410       1,500       4,410       1,500       4,410       1,500       4,410       1,500       4,410       1,500       4,410       1,500       4,410       1,500       4,420.8       4,120       1,200       4,120       1,200       4,120       1,200       4,120       1,500       4,120       1,500       4,120       1,500       4,120       1,200       1,200       1,200       1,200       1,200       1,200       1,200       1,200       1,200       1,200				ed steel		10,000	10,700	ā	11,000		10,000	00	10.242.8	10.2
p) Wood for formworks       4,900       1,500       4,550       4,790       -       4,410       1,500       4,191.7       4,         q) Wood for scafolding       5,100       1,500       4,550       5,295       -       4,940       1,500       4,191.7       4,         Puel Costs       a) Gasoline       8.20       9.50       9.50       4,550       5,295       -       4,940       1,500       4,480.8       4,         Unit: Baht per litre       b) Dissel fuel       6.10       6.50       6.50       6.50       5,200       -       26.10       5.30       26.00       5,200       5,200       5,200       5,200       5,200       5,200       4,400       5.30       4,400       5.30       4,400.8       5.30       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,200       5,000       5,200       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000       5,000 </td <td>•</td> <td></td> <td></td> <td></td> <td>Ìm</td> <td>14.000</td> <td>12,600</td> <td>Ö</td> <td>13,000</td> <td></td> <td>14.000</td> <td>200</td> <td>12.657.1</td> <td>12.6</td>	•				Ìm	14.000	12,600	Ö	13,000		14.000	200	12.657.1	12.6
Tuel Costs       a) Wood for scafolding       5,100       1,500       4,550       5,295       -       4,940       1,500       4,480.8       4,         Tuel Costs       a) Gasoline       8.20       9.50       9.50       9.50       5,295       -       4,940       1,500       4,480.8       4,         Unit: Baht per litre       b) Diesel fuel       6.10       6.50       6.50       6.50       6.30       -       8.20       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06       9.06				OTEVOLKS	-	1,500	5,000	ഹ	4.790	•	4.410	000	4,191.7	4
Twel Costs       a) Gasoline       8.20       9.50       9.50       9.50       -       8.20       9.06         Unit: Baht per litre       b) Diesel fuel       6.10       6.50       6.50       6.50       6.30       -       8.20       9.06         C) Mechanical oil       27.30       25.00       4.50       6.50       6.50       6.50       6.50       7.30       26.10       26.10       6.36         Machine Costs       a) Buildozer 15 ton       6,000       4,000       5,000       5,300       5,200       4,800       4,000       5,000       4,000       5,000       4,000       5,000       4,150.0       4,150.0       4,150.0       4,150.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       4,100.0       5,500       4,100.0       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,500       4,100.0       5,000       4,000			Wood for	cafolding		1,500	5,500	ഹ	. <b>.</b>	t	4,940	000	4,480.8	4.4
Unit: Baht per litre       b) Diesel fuel       6.10       6.50       6.50       6.50       6.30       -       6.30       6.10       6.36         C) Mechanical oil       27.30       25.00       -       -       -       -       26.10       25.00       26.13       2         Machine Costs       a) Bulldozer 15 ton       6,000       4,000       5,000       5,200       -       4,800       4,000       5,000       4,000       5,200       26.10       25.00       26.13       2         Vnit: Baht per day       b) Wheel loader 1.4 cu.m       4,300       3,600       4,500       4,500       4,000       -       4,000       2,000       4,000       -       2,000       4,1000       -       2,000       4,1000       -       2,000       4,1000       -       2,000       4,1000       -       2,000       4,1000       -       2,000       4,1000       -       2,200       1,7300       1,733.3       1         d) Dump truck 8 ton       2,000       4,000       6,500       1,600       -       2,000       4,000       -       2,000       4,000       -       2,000       1,7300       1,733.3       1         e) Wheel crane 25 ton       5,000		r v				9.50	ŝ	្រុ	ŝ	1	<u></u>	8.20	0	ۍ د
c) Mechanical oil       27.30       25.00       -       -       -       26.10       25.00       26.13       2         Machine Costs       a) Bulldozer 15 ton       6,000       4,000       5,000       5,300       5,200       -       4,800       4,000       5,050.0       5         Unit: Baht per day       b) Wheel loader 1.4 cu.m       4,300       3,600       4,500       4,500       -       4,000       5,000       4,1000       5         C) Excavator 0.6 cu.m       4,500       2,500       4,500       5,000       4,400       -       3,790       2,500       4,100.0         d) Dump truck 8 ton       2,000       1,300       1,500       1,800       1,600       -       2,000       5,583.3       5         e) Wheel crane 25 ton       5,000       4,000       6,500       7,000       -       5,583.3       5	Unit:	t per				6.50	i n	ĽЧ,	<u>ا</u>	1	<u></u>	6.10	3	6
<pre>Machine Costs a) Bulldozer 15 ton 6,000 4,000 5,000 5,300 5,300 5,200 - 4,800 4,000 5,050.0 5 Unit: Baht per day b) Wheel loader 1.4 cu.m 4,300 3,600 4,500 4,500 4,000 - 4,000 3,600 4,150.0 4 c) Excavator 0.6 cu.m 4,500 2,500 4,500 5,000 4,400 - 3,700 2,500 4,100.0 4 d) Dump truck 8 ton 2,000 1,300 1,500 1,800 1,600 - 2,200 1,300 1,733.3 3 e) Wheel crane 25 ton 5,000 4,000 6,500 7,000 - 5,000 4,000 5,583.3 5</pre>				oil	r.	25.00				1	6.1	<u>ທ</u>	6.1	26.
Unit: Baht per day b) Wheel loader 1.4 cu.m 4,300 3,600 4,500 4,500 4,000 - 4,000 3,600 4,150.0 4 c) Excavator 0.6 cu.m 4,500 2,500 4,500 5,000 4,400 - 3,700 2,500 4,100.0 4 d) Dump truck 8 ton 2,000 1,300 1,500 1,800 1,600 - 5,000 4,000 5,583.3 3 e) Wheel crane 25 ton 5,000 4,000 6,500 7,000 - 5,000 4,000 5,583.3 5		24 C		15 100	6 000	000	5 000	200	5 200		4 800	A 000	5 050 0	C u
C) Excavator 0.6 cu.m 4,500 2,500 4,500 5,000 4,400 - 3,700 2,500 4,100.0 4 d) Dump truck 8 ton 2,000 1,300 1,500 1,800 1,600 - 2,200 1,300 1,733.3 1 e) Wheel crane 25 ton 5,000 4,000 6,500 7,000 - 5,000 4,000 5,583.3 5	Unit:	ير و		er 1.4 cu.m	4,300	3,600	4.500	4.500	4,000	F	4,000	3,600	4.150.0	
Dump truck 8 ton 2,000 1,300 1,500 1,800 1,600 - 2,200 1,300 1,733.3 1 Wheel crane 25 ton 5,000 4,000 6,000 6,500 7,000 - 5,000 4,000 5,583.3 5				0.6 cu.m	4.500	2.500	4.500	5,000	4.400	1	3.700	2.500	4.100.0	-
Wheel crane 25 ton 5,000 4,000 6,000 6,500 7,000 - 5,000 4,000 5,583.3 5				8 ton	2,000	1.300	1.500	1.800	1,600	I	2.200	1,300	1.733.3	
				e 25 ton	5,000	4,000	6,000	6,500	7,000	1	5,000	4,000	5,583.3	1 20
					• ,	•		•	•			•		
	Source: The	The Study Team							-	.e				

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Table A 9.7.2 Analysis of Market Prices of Construction Equipment

				ـــــــــــــــــــــــــــــــــــــ
Machine \ Price Source	Contractors Price in BKK Baht/day	Rental Charge in BKK Baht/8hrs.	Japan Depreciati Average Charge Baht/day	charge ru
Bulldozer 15 ton	5,050	4,800	4,982	3,848
Wheel loader 1.4 cu.m	4,150	3,600	3,646	2,769
Excavater 0.6 cu.m	4,100	4,000	4,787	3,692
Dump truck 8 ton	1,730	1,500	1,764	1,365
Wheel crane 25 ton	5,580	7,000	8,463	5,538
Construction 2. Average daily	Table 1984" Machanizatio machine cha	issued by Jap n Association. rge	an in the second s	
3. Houry charge = $P \ge \frac{1/2}{Y}$				
$M \Rightarrow M$	epreciation	cost ratio ost ratio at s	ite	
d	na neau vill	•••	-	

- Y = Life year of machine C = Control and managing cost ratio per year ÷
  - H = Standard operation hours per year

Table A 9.7.3 Custom Duty and Business Tax

Imported Goods		om Duty			ax
	Rate o CIF x %	f Duty 1 Unit Bht.		_	Rate of Tax
			Profit %		*
Portland cement	10	Ton 80		ید بد شرح می می در بد بد می می . چن	
(Cement clinker)	-	-	11.0	1.5	1.8
(Petroleum crude)	25	-	ي ڪري ۽ در	_	<u></u>
Petroleum	-	L 0.01	Value*	9.0	19.4
Diesel		L 0.01	Value*	. 6	12.9
Lubricant oil	30	L 0.86	Value*	5.0	14.0
Asphalt		100kgs 1.0	11.0	5.0	6.0
(Soap)	-	~	11.0	9.0	10.8
(Explosive, Blasting)	30	_	11.0	9.0	14.0
*Wood	10	· 🕳	13.5	9.0	12.1
(Pig iron, cast iron)	6	kg 0.2	6.5	1.5	1.8
Angles & shapes	-	kg 0.4	11.0	9.0	10.8
Sheets&plates of iron or steel	-	·	11.0	9.0	10.8
*Bars of iron or steel	8	kg 0.75	5.5	9.0	11.1
Steel wire	25	kg 0.90	11.0	9.0	14.1
Machinally	30	· · ·	16.0	9.0	14.6
*(Passenger car)	80	-	Value*	40.0	155.2
Works trucks	15	-	11.0	5.0	6.9
(Ships&vessels,more than 250ton					-

Source: Custom Tariff of Thailand (Feb.1986)

Note:

 Value in the above tabel means market price of properties (rate of profit: 100% or more)

- 2. Duty amount is calculated by CIF x Rate(%) or duty (Baht)
  - 3. Wood, Bar or Rod of iron and passenger car are belong to the import banned items unless otherwise special licence or tax be made.

		سنبة جاند ومدر بدين وين الم	
Related Items	Foreign Portion	Local Portion	
ست میں دیک میں ایک مند ہیں ہیں ہیں ایک سار ایک ہیں میں میں میں ایک میں میں میں میں میں میں میں میں ایک میں ایک دی		يين محد منه بعد عند عنه الدو جن جند جند	
Saw mill,wood Limestone,Rock Gravel, Sand quarring	2% 14% 5%	98% 86% 95%	
Petroleum refineres, Diesel,Gasolin, and Asphalt	89%	11%	
Cement	5%	95%	a da ser a compositiva de la compositi En la compositiva de l En la compositiva de l
Concrete	2%	98%	
Iron & steel (pigiron ingot, shaped section)	69%	31%	AN UNDER STATE AND AN UNDER STATE AND AN UNDER STATE AND
Secondary steel product (angle bar, wire,tube pipe, PC tendon)	28%	72%	
Stucturl metal product (bridge, tanks,building)	45%	55%	
Special industrial machinery & appliances	40%	60%	
Moto vehicle assembly	42%	58%	
Electricity	7%	93%	
Land transport	5%	95%	
Ocean & water transport	4-6%	96-94%	

# Table A 9.7.4 foreign & Local Component

Source: NESDB

# Table A 9.7.5 Market Prices and Cost Components

	Items	Average of Market	Cost Foreign	Comone	nts ocal	Economio Cost
		Price		Net		Ratio
}	Labour costs (Baht per day)					یانی سے بند سے بند ہے۔ را
	Common labour	100		100.0	***	100
	Skilled labour(carpenter etc.)	160	•			100.0
	Car driver(dum truck etc.)	150	-		-	100.0
	Operater(bulldozer etc.)	210	-		11.0	
	Foreman	310	-	294.0	16.0	
	Expert worker (mechanics etc.)	250	-			94.8
)	Material Costs (Baht)					
	Portland cement per ton	1,620	81.0	1,531	8.0	99.5
	Rapid hardening cement (t)	1,740	87.0			99.5
	Sand per cu.m	160	8.0	152	-	100.0
	Crushed aggregate per cu.m	190	27	163	· -	100.0
	Concrete(180kg/sq.m) per cu.m	1,040	21	1,019	-	100.0
	Concrete(240kg/sq.m) per cu.m	1,120	22	1,098	-	24 6 0 1 -
	Concrete(300kg/sq.m) per cu.m	1,210	24	1,186	-	
	Concrete(350kg/sq.m) per cu.m	1,280	26	1,254		100.0
۰.	Interlocking block each	3.50	0.07		0.31	
	Concrete block 0.4x0.4 each	8.00	0.16			
	Curb & Gutter per m.	135.00	2.70	120.2	12.10	91.0
	Curb stone per m.	42.00			3.80	
	RC pile 0.22x0.22 per m.	477.00	77.10			96.8
	PC pile 0 0.60 per m.	1,000	20	890 1,645	90	91.0
	Round bar per ton	9,230		1,645		86.8 86.8
	Deformed bar per ton	9,390 21,550		13,159		89.1
	PC wire per ton	21,550		13,159	2,359	89.1
	PC strand per ton	10,900	7,521	1 9/2	1,437	86.8
	PC bar per ton U.I.H shaped steel per ton	10,240		1,824		86,8
	Steel pipe per ton	12,660			383.0	97.0
	Wood for formworks (cu.m)	4,190		4,106		100.0
	Wood for scafolding (cu.m)	4,480		4,390		100.0
	Asphalt concrete per ton	1,200*	120		80	
	Steel bridge structure (t)	36,500*	14,600			
	Cast iron structure (t)	225,000*		57,640		94.6
)	Fuel Costs (Baht per litres)					
-	Gasoline	9.10	6.80	1.0	1.30	85.7
	Dieselfuel	6.40	4.80	0.7	0.90	85.9
•	Mechanical oil	26.10	19.50	2.8	3.80	85.4
)	Machine Costs (Baht per day)		0 770	1 022	1 120	75.5
	Bulldozer (15 ton)	5,050	2,778	1,033	1,239 1,018	
÷	Wheel loader (1.4 cu.m)	4,150	2,283	849	•	75.5 75.5
	Excavator (0.6 cu.m)	4,100	2,255	839 844.0	1,006 159.0	/5.5 90.8
	Dump truck (8 ton)	1,730 5,580	727 3,069	1,142	1,369	90.8 75.5
	Wheel.crane (25 ton)	5,550	5,002		2,303	10.0

Note: r),s),t) these prices obtained from the recent contract documents Note.

A-9.33

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# Appendix 9.7.2

#### New Krungthep Bridge Construction Project Financial/Economic Construction Cost

A-9.34

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	Table A.9.7.6 (1)	New Kr	Krungthep Br	idge Constr	uction Pro	ហ្គុំ ភូមិ ភូមិ ភូមិ		•	•
		Financ	ial/Econom	ic Construc	tion Cost	Table	rijer Stefan		· · · · · · · · · · · · · · · · · · ·
						. '			
(New R	Krungthep Bridge Main & Approach)						       		
Item No.	. Work Item	Unit	Quantity	22	Compon	ent (%	•	nancial	Econom
* * * * *	·····································		• • • •	CDI.C		Г	Тах	uno	<u>п</u> .
	₽ T # \$ \$ # T = 6 ± 7 ± 6 T ± 5 ± 6 T = 0 ± 0 ± 0 ± 0 ± 0 ± 0 ± 0 ± 0 ± 0 ± 0			ri i Na i			1         		
		F	42.	•	tin Star	ì	ł	2 • • • • • • • •	•
ร 1 ย	Cast-in-place RC pile 2.0 m dia	E	, 9 10	20	5.7 5	÷	6	12,03	25,00
B-3	Precast PC pile 60 cm dia	E	. 37	j)	2.5 7	1	۰.	, 819,06	5,912,00
B-2		•	6 6 7	20	1.0	5	ŝ	482,42	403,60
B-7	Concrete of Pile cap P29 & P30		10,	Ó.	4.6 7	ທີ	ы. 1	,601,45	,916,00
9-9 B	Concrete of Substructure	E. 10	0	50	00 m	8	m,	948,86	19,00
о 1 Д	Formwork for Substructure	E 09	0,0	រក ភូមិ រ		-1		2,804,59	2,529,00
φ, ·	Re-bar for Substructure	ron L	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	$\sim 1$		23 4		, 4CU , 4CU	
	Stone Fitching against Scouring		, п - ( 0	ין כ קייי ה			•	C. KO K. + 4 A C C O O O O O	00 191
	4 8 0 6	ミ・プク	n 0 1 0		יים ער יינג	•	, c	1 ROJ 24	6 466 00
	79		0.0 0.0 7 -	ט ה ייי ט ש נ		R 00	•	1.075.80	8.399.00
	PC Terdor for Main Bridge		2 6	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	10	10	0.8	64.757.166	59,576,000
B-24	so active row iter. Saras Guardrail Double Leaf	) E	44	ំហ ហេ	7.1. 7		, i	288.62	279,00
101 101 101	Expansion Joint, Rubber	Ę	ŀΜ	23	ິ ເຜີ	4	N	31,92	89,00
9	Bearing, Cast iron	ton	60	ŪU,	7.6. 3	2	٠	,834,02	39,00
B-29	Concrete curb & Railing	E	യ	10 10	2.8 7	-		05,34	, 526, 00
n	Concrete Median Strip	E	442	Ö.	1.8	8		269,17	247,00
- E.	Bridge Wearing Course	ແ ບັນ		5	5	5	•	49.,64	00, 50
	Sub total	           			9.2 6		9.8	314,282,539	283,474,000
•.		i				-			
	2. Anoroach Bridge		0   +	1. 01					
с 1 Д	Precast PC pile 60 cm dia	E	35,55	١n	2.5	7.	÷.	,430.24	0.58,00
B-2	Bridge Excavation	cu.m	6	មា ស	1.0	o.	ം	4,285,57	3,587,00
B-6.	Concrete of Substructure	u no	ດ ຄ	O '	ი 	8	÷.,	9,176,78	213,00
8 - B	Formwork for Substructure	50°	0 0 0 0 0	មារ ហេរ ប	-1 < 	÷,	n'a	21 226	
ດ   ແ	,	ton.	20°	n d		α (	N C		
EL-A	σ	น. มาว		Sic		n r		A, 004 A,	
ชา - 1 - 1 - 1 - 1	RC CONCIELE ION HOLLOWED BILGGE		~ U 0 0		4 a	1.0	- - -		2.642.00
0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			1 0 1	  	0 00 	3 a	N OC	00 000 0	5.912.00
107 107	rt remann for re mon birge Guardrail double Leaf	1 2 2	) (N	ាំព រំល		5	• •	927.26	898,00
ν 1 1 1 α	Rynansion Joint Rubber	8	) (C) ( (C) ( (C)	23	с, С	4	R	,042,60	,653,00
	Bearing Neoprene	Each	322	11,671	69.6 3	0.4	12.2	3,758,062	3,299,000
9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	oncrete Curb &	E	.8	1,81	2.8	2	٠	,157,44	, 904,00
B-30	Median	E	г, <u>4</u> 2	0	~; •	8	٠	864,78	793,00
	Bridge Wearing Course	sq. B	<b>,</b> 60	ຫ		2	• 1	4 6 - 4 8 	

- · · · · · Continued to 3. Interchange & Access Road...

354,918,000 

397,376,400

10.7

64.3

35.7

Subtotal'

Table A 9.7.6 (2) New Krungthep Bridge Construction Project

# Financial/Economic Construction Cost Table

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Approach)
હ
Main
Bridge
Krungthep
(New

Term No.         Work frem         Unit         Quantity         Finnetial         Component         (%)         Finnetial         Component         (%)           District mays         A core at a manual and a manual	A B B B B B B B B B B B B B B B B B B B	(New Krungtnep Bridge Main & Approach)								
Price         Price <th< th=""><th></th><th>WORK IT</th><th>т.</th><th>uantit</th><th>inancia <sup>1114</sup>t</th><th>uodmo</th><th>ent (</th><th>) ( E</th><th>inancia</th><th>conomi</th></th<>		WORK IT	т.	uantit	inancia <sup>1114</sup> t	uodmo	ent (	) ( E	inancia	conomi
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$					- 0 - 1 - 1 - 1	4	 1	4 1		
m         9,326         1,559         22.5         77.5         13.3         14,539,234         12,500         7,283           u.m         1,550         1577         10.5         57.4         11.3         924,003         7,283           u.m         1,550         1577         10.6         88.7         11.3         924,003         7,283           u.m         1,550         1577         10.6         88.7         17.5         51.25           u.m         1,550         1,577         10.6         88.75         51.95         7.283           u.m         1,561         12.577         62.0         12.6         10.6401142         7.283           u.m         1,650         12.6         12.6         12.6         7.941         7.941           u.m         1,650         12.6         12.6         12.6         4.751         5.125           u.m         1,620         13.1167         53.23         12.0         4.578         4.751         5.459         4.751           u.m         1,560         5,120         12.6         12.6         10.6401142         7.962         2.512           u.m         1,560         5,120         12.6 <td< td=""><td>           </td><td>. Interchange &amp; Access Roa</td><td>Ē</td><td>26 + 22</td><td>355</td><td>1 1 1 1 1 1</td><td></td><td></td><td></td><td></td></td<>	         	. Interchange & Access Roa	Ē	26 + 22	355	1 1 1 1 1 1				
m       9,600       856       32.6       67.4       11.3       8,217,600       7,283,         u.m       1,550       1,571       10.5       32.5       17.8       904,033       743,         u.m       1,550       283       46.8       53.5       17.8       3,621,763       5,195,         u.m       1,551       581.1       41.9       9.8       0640,142       2,929,         u.m       1,704       5,195,       14.73       2.9       3,551,763       7,522,939         u.m       1,704       5,195,       12.74       2.9       3,514112       3,229,940,33       7,520,299,39,393         u.m       1,600       5,125,1       58.1       41.9       9.8       751,720       7,500       7,503         u.m       1,050       5,126,1       51.9       10.1       5,494,416       7,510         u.m       1,253       4.12       8.7       3.12.1       4.528,320       7,313         u.m       1,253       4.12       2,451,460       2,451,160       2,451,160       2,451,160       2,451,160       2,451,160       2,451,160       2,451,160       2,451,160       2,451,160       2,451,1720       4,751,630       4,528,320	<b>т</b> 1 Д	recast PC Pile 60 cm di	e	, 32		~	.*	'n	4,539,23	2,605,00
u.m 1,559 197 44.2 55.8 17.8 904,033 743. u.m 1,550 157 62.0 38.7 3.3 2.964,356 1.978 u.m 1,704 5.277 62.0 38.7 3.3 2.964,356 1.978 u.m 1,704 5.2242 21.8 14.9 9.8 0.051.763 7.269, u.m 1,704 5.2242 21.8 10.1 3.8.7 1220 7.941, 1.660 447 12.2 12.8 4.7 7.508 u.m 1,950 1.151 65.8 34.2 12.8 8.678,720 7.941, 1.610 54,2142 9.20 65.8 34.2 12.8 8.678,720 7.941, 1.610 54,2142 13.120 7.508 u.m 1,950 1.511 65.8 34.2 12.8 8.678,720 7.941, 1.621 11,512 65.8 34.2 12.8 8.678,720 4.751, 0.1 1,950 1.511 65.8 34.2 12.8 8.678,720 4.751, 0.1 1,950 1.511 65.8 34.2 12.8 8.678,720 4.751, 0.1 1,950 1.511 65.8 34.2 12.8 8.678,720 4.751, 0.1 1,922 65.8 34.2 12.8 10.1 7.935,602 2.331, 0.1 1,922 43 11. 12.8 4.7 7.2 4.9 7.26 4.751, 0.1 11,402 1.137 30.3 65.7 10.7 8.790,768 7.870,800 0.1 12,402 1.11 15.3 84.7 7.7 4.970,768 7.487,935 0.1 12,403 1.711 10.0 2,580,840 2.922,031,922 0.1 12,403 1.711 15.3 84.7 7.7 4.97,098 4.916,62 0.1 12,403 10.1 25.9 87.1 10.0 2,580,840 2.923,203 0.1 12,403 1.713 16.5 1.1,222 1.739 0.1 12,403 1.731 10.0 2,580,840 2.932,202 0.1 12,433 1.731 10.0 2,580,840 2.932,203 0.1 12,433 1.731 1.735,315 1.739 0.1 12,433 1.73 32.3 1.73 0.1 12,433 1.73 32.3 1.1,44 2.0 1.657,203 1.652,203 0.1 12,432 2.14 8.8 31.7 10.0 2,590,840 2.925,366,656 113,421 0.1 15,33 8.7 1.735,366,656 113,421 4.225 0.1 12,44,955,955 2.948,698 0.1 12,93 2.035,535 2.1 1.0 2.25,956,656 113,421 4.225,000,000 1000 1.651,820 0.1 25,966,656 113,422 2.11,44,955 9.2,955 9.48,698 0.1 2.0 125,966,555 113,423 1.1,135,700 1.255,966,555 113,421 1.656,550 1.13,421 1.255 0.1 12,44,955,955 2.1 1.1 15.1 2.21 1.1 1.255,956,555 2.1 1.1 1.25,100 0.1 1.44,955 595 2.1 1.44,955 9.2,155 9.48,698 0.1 11,44,955 595 2.1 1.9 10,00 000 000 000 000 000 000 000 000 0	1.	recast RC File 2 x 22 sq.	E	, 60	.~	~	. *	ч.	,217,60	,289,00
um 13,500 283 46.8 53.2 12.0 3,360,356 1,972 um 1,357 1,377 10.6 89.4 2.9 5,511,171 5,195, 0.1 1,60 5,500 40.1 59.9 12.6 8,511,120 7,508, 1,60 14,232 21.8 78.2 8.5 10.1 44 8,511,120 7,508, 0.1 1,060 4,232 21.8 78.2 8.5 10.1 4,528,320 6,50 3,262, 0.1 1,060 4,232 21.8 78.2 8.5 12.8 7720 4,070 7,941, 0.1 1,060 4,232 21.8 78.2 12.7 5,442,725 4,751, 0.1 1,060 4,232 21.8 78.2 12.7 5,442,725 4,751, 0.1 1,000 65.8 377 2 4.5 2,457 60 2,321,42 0.1 1,100 8,721 65.6 51.8 33.5 11.4 5,799,417 4,872,323,523,50 0.1 1,142 20.1 12.9 87.1 10.0 7,557 4,770 1,421 0.1 1,003 4,65 65.8 377 2 4.5 2,457 60 2,322,322 0.1 1,746 8.6 31.1 0,0 3,235 4,790 1,621 0.1 1,492 4,916 12.9 87.1 10.0 3,240,416 4,916 0.1 1,492 4,916 12.9 87.1 10.0 3,240,417 4,872,993 0.1 1,402 12,9 87.1 10.0 3,240,416 4,916 1,421 0.1 1,492 13,35 54.2 11.4 5,493,416 4,916 1,422 0.1 11,492 4,916 12.9 87.1 10.0 1,255,816 1,422 0.1 12,423 13.1 10.0 1,528 2,912 1,422 0.1 12,424 18.6 31.4 2.0 1,655,293 1,422 0.1 12,735 22.8 77.2 2.9 1,56,815 1,142 2,235,912 0.1 12,746 8.6 31.2 1,233 1,427 2,03 1,625,595 1,142 1,235 0.1 12,735 22.8 77.2 2.9 1,570 2,555 1,142 1,225 0.1 15,450 12,435 1,427 1,021 1,525 1,422 1,422 1,427 1,022 1,525 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,421 1,422 1,421 1,422 1,421 1,422 1,421 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,422 1,42	· 1 -	tructure Excavat		4,58	~	4		ŗ.	904.03	743,00
um 1.357 1.568 11.3 88.7 3.3 2.946,356 1.978, um 1.704 513 1.568 1.41.9 9.8 8.051.711 5.1958, um 1.704 5.21.8 78.1 2.8 8.678,720 7.509, um 1.660 54,242 21.8 78.2 8.5 8.678,720 7.503, ach 1.471 53.1 10.1 55.8 12.7 5.4570 7.503, ach 1.253 8.721 53.1 11.4 12.2 7.725 4.771, ach 1.253 8.721 53.1 10.0 8.6 5.8 30,137 4.872, ach 1.280 12.1816 53.1 10.0 8.6 5.407417 4.872, ach 1.280 12.1816 53.1 10.0 8.6 5.407415 4.791, ach 1.280 12.1816 53.1 10.0 8.6 3.20,758 7.7850 ach 1.280 12.1816 53.1 10.0 1.657,700 1.622, ach 2.235 10.1 12.1 2.0 1.655,203 1.623,223 ach 2.225 10.3 8.1 10.0 1.655,203 1.623,223 ach 2.225 10.3 30.1 69.9 8.8 3.218,2750 2.935, ach 2.235 14.4 85.6 3.2 1.677 0.151 ach 2.235 14.4 85.6 3.2 1.672,123 1.623,120 1.655,120 ach 2.235 14.4 85.6 3.2 1.672,123 1.65,770 1.52 ach 2.235 14.4 85.6 3.2 1.655,113,423 ach 2.235 14.4 85.6 1.1 1.655,120 1.655,133 1.65,770 ach 2.235 14.4 85.6 1.1 1.655,120 1.52 ach 2.535 14.4 85.6 1.1 1.655,120 1.52 ach 2.535 14.4 85.6 1.1 1.655,120 1.55 ach 2.535 14.4 85.6 1.1 1.655,120 1.55 ach 2.55 14.4 85.6 1.1 1.655,120 1.55 ach 2.55 14.4 85.6 1.1 1.655,100,100 2.555 ach 2.55 14.4 85.6 1.1 1.655,100,100 2.555 ach 2.55 14.4 85.6 1.1 1.655,100,100 2.555 ach 2.55 14.4 85.6 1.1 1.655,100,000 0.056,111,42,750 2.025,505 about 40.5 5.9.5 16.9 1.124,245,750 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.022,505 2.	1	mbankment, Sand	•	3,50	3 8			n,	,820,50	,362,00
u.m $3.613$ 1,477 10.6 89.4 2.9 5,511,171 5,125, u.m $1.612$ 1,477 10.6 89.4 2.9 5,511,171 5,126, u.m $1.704$ 5,5030 40.1 59.9 12.6 10,640,142 9,299, u.m $1.704$ 5,5030 40.1 59.9 12.6 10,640,142 9,509, acch 1.600 4,272 38.2 61.8 38.2 12.1 4,558,320 4,7751 9,791, acch 1.950 51.8 38.2 12.1 4,558,720 4,775 5,791, acch 1.953 4,771 5,12 12.2 7,791,700 3,915 acm 1.002 4,372 36.5 57.2 12.8 8,674,725 4,772 8,05, acm 1.033 8,739 45.8 57.2 12.8 8,7499,417 4,872, acm 1.032 8,739 45.8 57.2 11.4 8,7499,417 4,872, acm 1.032 8,739 45.8 57.2 11.4 8,7499,417 4,872, acm 1.032 8,739 45.8 57.2 11.4 8,7499,417 4,872, acm 11,032 8,739 45.8 57.2 11.4 8,7499,417 4,872, acm 11,032 8,739 45.8 57.2 11.4 8,7499,417 4,872, acm 12,647 10.7 8,799,417 4,872, acm 12,647 10.7 10.7 8,799,417 4,872, acm 12,647 10.7 10.7 8,799,417 4,872, acm 12,647 10.0 125,830,781 4,820,730 1,522, acm 12,647 10.0 125,831 14.4 85.6 2,321,1722 1,722 acm 2,235 14.4 85.6 3.2 14.5 1,225,932 1,720 acm 2,235 27.8 72.2 2.9 1,877,280 1,522 acm 2,755 21.8 13.1 14.6 5,559 1,720 1,225 acm 2,756 44.2 25.8 17.3 14.4 85.6 1,13,421 acm 2,235 27.8 72.2 2.9 1,670,655,595 1,13,421 acm 2,255 21.8 13.1 160.9 125,555 11,421,222 about 40.5 59.5 16.9 1,13,421 1,222 about 40.5 59.5 16.9 1,134,21720 2,025,595 1,134,202 47.8 7,50 202,504 1,245 1,131,222 about 40.5 59.5 16.9 1,134,27720 2,022,504 1,134,202 about 40.5 59.5 16.9 1,134,245 1,131,222 about 40.5 59.5 16.9 1,250,500 1,264,555 103 4,124,750 202,500 100,000 1,264,555 103 4,122,555 103 4,1222 1,221,222 about 40.5 59.5 16.9 1,144,750 202,564,555 103 4,124,770 1055,555 104 4,125 155 113,421,720 1055 104 4,124,755 105,104 4,125 155 1	1	oncrete of Substructure		ູ່	1,50	i			,046,35	,978,00
qr.m       14,613       2551       55.1       41.9       9.8       8,571,1220       7,503         u.m       1,704       5,033       40.1       59.9       12.6       640,142       9,293         u.m       1,704       5,033       40.1       59.9       12.6       8,571,1220       7,503         u.m       1,704       5,033       40.1       59.9       12.7       8,571,1220       7,503         u.m       1,605       54.27       38.2       10.1       4,528,720       7,503         ach       1,150       1,971       5,422,725       4,751       393         ach       1,191       22.8       77.2       4,970       5,432,725       4,770         ach       1,253       12.7       11.4       5,432,725       4,770       5,331         ach       1,253       11.6       57.2       11.4       5,432,755       2,322         ach       1,253       11.6       57.2       2,342       1,523       3,232         ach       12,343       16.5       87.7       10.7       8,514,8416       7,932       4,912         ach       12,343       11.1       15.3       34.7       7.7	1.1	Road Structur	•	3,62	1,47	ò	<b>_</b>		,351,17	,195,00
on 1846 12.577 52.0 33.0 12.6 10,640,142 9,299, U.m 1704 54,242 21.8 12.1 4.521,720 7,941, 0.n 166 14.272 51.8 12.1 4.521,720 7,941, 4.15 13.115 11.671 52.8 34.2 12.8 4.51,780 55.5 ach 1,350 11.671 52.8 34.2 12.8 4.5 4.5 72.0 4,770, 7.350 4.771 8.6 54.5 30.5 4.5 602 3.314 7.350 4.772 8.5 54.5 72.0 7,9416 65.5 305 7.147 1.003 8.721 0.7 8.6 5.4 10.7 8.8 775 5,143,416 4,916 7.142 1.735 1.7 7.6 1.236,816 1,914 2,916 7.142 7.8 8.6 91.4 2.0 1.655,208 1,780 4,916 7.142 7.8 8.6 91.4 2.0 1.655,208 1,780 4,916 7.142 7.8 8.6 91.4 2.0 1.655,208 1,780 4,916 7.142 7.5 3 4,55 31.7 7.6 1,236,816 1,742 4,916 7.142 7,800 409 21.3 78.7 7.6 1,236,816 1,742 4,916 7.142 1,723 4,916 7,70 1,625,208 1,742 4,231 7.142 111 8.5 31.7 7.6 1,236,816 1,742 4,916 7.142 1,622 1,732 1,44 8.6 91.4 2.0 1,655,208 1,720 4,222 7.142 1,142 1,144 8.6 91.4 2.0 1,655,208 1,720 4,222 7.15 2,935 2,935 2,732 1,73 1,66 1,700 1,621 4,916 7.142 1,523 1,521 1,722 1,521 1,822 1,13,421 7.15 2,335 2,133 1,11 1,855 2,133,150 1,525 1,813 1,822 7.15 2,235 1,732 1,122 1,525 1,13,421 1,822 7.15 2,332,000,000 1,655,595 148,669 7.15 2,035 1,13,421 1,00 1,25,965,655 11,42,420 2,555 1,813 7.5 40.5 591 1,734 1,655 595 148,698 7.5 about 40.5 591 1,00 1,25,965,655 11,42,420 2,555 1,134,220 4,245 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,822 1,13,421 1,200 1,000 1,965 5,955 1,13,42 1,202 1,13,421 1,202 1,13,421 1,202 1,151,200 1,000 1,25,965 5,55 1,13,42 1,500,000 0,196 1,13,421 1,202 1,142 1,202 1,13,421 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,13,421 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142 1,202 1,142		bstructur		4,61	ഗ		- 1	<b>ო</b>	8,051,76	, 262, 00
u.m 1,704 5,020 40.1 59.9 12.4 8,571,120 7,508, ach 1,060 54,242 218 58.2 61.8 10.1 5,442,725 4,751, ach 415 15,120 561.8 33.2 12.7 5,442,725 4,751, ach 4,150 1,815 551.8 30.4 12.2 723,602 6,33, ar 1,350 1,815 22.8 77.2 4.9 2,451,600 2,331, ar 1,1003 8,72 11.4 5,481,600 2,322,6 ar 11,003 8,72 11.4 5,481,600 2,922,9 ar 1,253 4,389 45.8 54.2 11.4 5,481,600 2,923,9 ar 1,253 4,389 45.8 54.2 11.4 5,481,600 2,923,9 ar 1,253 111 15.3 84.7 7.7 1,872,098 1,921,7 ar 1,1003 112,9 87.1 10.0 2,580,840 2,923,921,7 ar 1,1003 112,9 87.1 10.0 1,827,280 1,622,923,921 ar 1,1003 11,75 8.5 11.4 7,7 1,877,098 1,921,7 ar 1,128 40 21.3 789.7 7.7 1,877,098 1,421,7 ar 1,128 40 21.3 789.7 7.7 1,877,098 1,421,7 ar 1,128 40 21.3 789.7 1,877,098 1,421,7 ar 1,128 40 21.3 789.7 1,877,098 1,421,7 ar 1,128 1,732 14,415 1,732 1,742,1 ar 1,128 1,732 14,912 1,732 1,421,7 ar 1,128 1,732 14,415 1,732 1,421,421 ar 1,128 1,752 1,555 1,131 1,521,700 2,555 1,13,421 1,134,712 1,151,700 2,555 1,13,421 1,134,712 1,151,700 2,555 1,13,421 1,134,712 1,151,700 2,555 1,134 1,525,555 1,134,45 1,151,700 2,555 1,134 1,52 1,51,200 1,000 1,24,845 150 1,134,455 1,151,200 ar 1,24,6,775 1,255,555 1,134,455 1,151,200 ar 1,24,6,775 1,255,555 1,134,455 1,151,200 1,200 1,200 1,246,775 1,251,200 1,200 1,246,775 1,255,555 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255 1,255	δ I	tructure	<b>-</b>	00	5.01	d.	<u>.</u>	તં	0,640,14	, 299, 00
on 100 54,242 21.8 78.2 8.5 8,678,720 7,941, 745 13,115 61.8 38.2 12.7 5,445,725 4,757, 749, 1,552 11,615 51.8 34.2 12.7 5,445,725 4,757, 74,471 1,552 11,655 34.2 12.3 7,550 533, 74,471 1,605 55.8 34.2 12.3 7,550 533, 74,471 1,452 51.8 77.2 4,5 5,495,768 7,855,665 6,916 7,855,600 2,333,555 6,757 1,007 75,495,758 6,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,916 7,91	1	PC Box Bridg	д		ທີ່ ຕິ	ò	<u> </u>	3	,571,12	, 508, 00
u.m 1,060 4,272 38.2 61.8 10.1 4,522,320 4,070, ach 1,350 1,315 61.8 34.2 12.3 5,442,725 4,751, ach 1,253 1,15 61.8 34.2 12.2 7,24,602 2,331, q.m 4,471 8,65 8 34.2 12.2 7,245,602 2,331, q.m 1,492 201 12.9 87.1 10.0 2,580,840 2,322 q.m 11,492 201 12.9 87.1 10.0 2,580,840 2,322 q.m 11,492 201 12.9 87.1 10.0 2,580,840 2,322 q.m 12,840 201 12.9 87.1 10.0 2,580,840 2,322 q.m 2,013 30.1 68.9 8.6 31.7 7.7 457,098 1,622 ach 4,916 3,231 4,4 85.6 3.2 4,99,120 1,622 ach 2,235 1,7146 8.6 31.4 4,51 1,223 1,492 ach 2,235 1,7146 8.6 31.4 4,71 1,827,098 1,720 4,118 2,233 1,7146 8.6 31.4 1,644,910 1,652 1,923 1,623 2,23 14,4 85.6 1,13,421 ach 2,235 1,713 15.3 84.7 7.7 457,098 1,623 1,623 1,7146 8.6 31.4 2.0 1,652,599 1,628 1,622 2,23 ach 2,235 14 4,230 1,659 1,13,421 1,664,910 1,658 1,7146 8.6 31.2 1,60,9 1,628 1,628 1,691 1,2,555 1,151,202 ach 2,559 1,151,202 1,202 ach 2,559 1,151,202 1,222 2,555 1,000 1,246,831 1,556,703 1,1446,715 1,151,202 1,255 3,144,442,700 1,126,255,595 948,698 1,557,955 1,151,202 1,252,505 1,151,202 1,255 3,144,442,755 1,151,202 1,252,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,255,505 1,151,202 1,25	1	C Box Bridge	g	9 . 	4,24		<u>~</u>		,678,72	,941,00
on       415       13,115       61.8       38.2       12.7 $5,442,725$ $4,751$ $451,600$ $393,393$ ach       1,350       1,671 $55.8$ $34.2$ $12.3$ $451,600$ $2,331,600$ $553,393,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,702$ $533,392$ $533,702$ $533,102$ $533,102$ $4,381$ $12,253$ $4,75,763$ $4,87,753$ $532,226$ $532,114$ $8,723,222$ $65,77$ $7,690,768$ $7,870,768$ $7,870,768$ $7,870,768$ $7,870,768$ $7,142$ m $3,024$ $112,645$ $30,11$ $69,9$ $8.8$ $3,218,275$ $2,322,6316,416$ $2,322,6316,416$ $2,322,6316,416$ $2,322,6316,416$ $2,142,232$ m $3,1024$ $12,746$ $8,73$ $36,102$ $2,323,326,316$ $1,722$ m $3,1024$ $12,746$ $12,732$ $24,76,720$ $1,722$ m $3,133$ $12,732$ $23,732,720$ $1,722$ $33,752,220$	Ff.	Kollowed Bridg	д	00	10	÷.	. i.	ċ.	, 528, 32	,070,00
m       451,780       535,72       12.8       451,780       535,733         m       1,350       1,816       23,451,600       339,453       45.8       34.2       12.2       723,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,600       535,148       456,776       456,776       456,776       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       452,756       1,742         m       2,026       12,455       14,746       2,726       1,655,7268       1,742       464,930       4,625       1,732,423       4,225	-1 ' 	structur	0	ч,				сі і	, 442, 72	,751,00
acn       1,550       1,511       54.5       54.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5       55.5	°∎+ 	dany',	ទ	54	51	۰. ۱		n' i		
1,350 $1,316$ $1,316$ $1,316$ $22.8$ $77.2$ $4.9$ $2,451,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,331,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,321,500$ $2,1420$ $2,1420$ $2,1420$ $2,1420$ $2,1420$ $2,1420$ $2,1420$ $2,1420$ $2,1420$ $2,1240$ $2,1240$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,124$ $2,$	ñ.	че	ช ช	ا ۵	19'T	<u>.</u>	<u>.</u>	ni -	723,60	635,00
q.m       4,471       197       20.3       79.7       8.6       880,787       880,787       880,787         q.m       1,253       4,389       45.8       53.5       4.5       5,148,416       4,916         q.m       11,492       8,790,768       7,850       8,711       10.0       2,188,416       4,916         q.m       12,640       201       12.9       87.1       10.0       2,138,416       4,916         q.m       15,675       12.9       87.1       10.0       2,132,375       2,935         q.m       5,035       111       15.3       84.7       7.7       4,916       2,935         q.m       5,035       11,746       8.3       91.7       2.0       1,655,203       1,142         m       5,118       1,746       8.5       91.4       2.0       1,655       1,1720         m       5,235       14,45       8.6       91.4       2.70       1,655,203       1,655,450       1,655,450       1,656,450       1,656,450       1,656,450       1,656,770       1,655,203       1,656,770       1,656,770       1,656,770       1,656,770       1,656,770       1,656,770       1,656,770       1,623       1,623       1,	C1 1	Raili		ຕູ ເມ	ω, 	o.			,451,60	,331,00
q:m       1,253       4,389       45.8       54.2       11.4       5,499,417       4,812         q:m       11,403       8,721       30.3       55.7       10.7       8,790,768       7,850         q:m       12,840       201       12.9       87.1       10.0       2,325       232         m       12,840       201       12.9       87.1       10.0       2,326       346       4,916       2,323         m       3,024       12,453       14.1       57.098       81.5       91.7       7.6       1,2368       1,790       2,325         m       4,116       8.5       91.7       7.6       1,2368       12,445       1,790         m       2,026       12,453       14.4       85.6       91.4       2.0       1,827,280       1,790         m       2,133       234       45.7       2.0       1,827,280       1,790       4223         m       2,144       8.5       91.7       2.0       1,827,280       1,790       4223         m       2,235       27.8       72.2       2.96,790       1,625,708       1,790         m       2,53       32.3       12,442       72.2 <td>1</td> <td>Course</td> <td>D'</td> <td>147</td> <td></td> <td>o'</td> <td>n,</td> <td>ŵ</td> <td>8,80,78</td> <td>805,00</td>	1	Course	D'	147		o'	n,	ŵ	8,80,78	805,00
q:m       1,003       8,721       30.3       69.7       10.7       8,790,768       7,850         q:m       11,492       2048       12.5       83.5       4.5       5,148,416       2,925         q:m       12,840       203.5       8.1       10.7       5,148,416       2,925         q:m       12,845       12.5       83.5       8.8       3,218,275       2,935         m       3,024       409       21.3       78.7       7.6       1,236,846       2,325         m       5,148       111       8.5       34.7       7.7       457,098       1,421         m       5,146       8.6       91.4       2.0       1,256,846       1,720         acch       40       12,453       14.4       85.6       3.2       4,827,208       1,422         acch       2,335       14.4       85.6       3.2       1,525       1,720       1,526         acch       2,735       13.14       85.7       2.0       1,566,815       1,525       1,525         acch       2,735       13.1       1.657       3.136,442,930       1,555       1,525       1,525       1,525       1,525       1,525       1,5		der of Exist.	ບ ທ	, 10 10	ε Έ	ഗ			,49.9.4	,872,00
q:m       11,492       448       16.5       83.5       4.5       5,148,416       4,916         q:m       12,840       201       12.9       87.1       10.0       2,580,840       2,322         q:m       16,75       201       12.9       87.1       10.0       2,580,840       2,322         m       3,024       111       15.3       84.7       7.7       457,098       421         m       4,118       111       15.3       84.7       7.7       457,098       421         m       520       1,746       8.5       91.7       2.0       1,826,815       1,442         m       40       12,453       14.4       85.6       3.2       452       421         ach       2,235       14.4       85.6       3.2       1,655,203       1,655,450       1,625         ach       2,750       12,666,656       13,700       161       125       422         ach       2,750       12,696,656       1,822       1,625,703       1,655,703       1,625,703       1,61       1,223         ach       2,750       125,496,6656       13,626,700       1,626,700       1,61,823       1,223         <	1	با	υ.	0 0 1	12	<u>.</u>	м. М	o.	,790,76	,850,00
q:m       12,840       201       12.9       87.1       10.0       2,580,840       2,322,935         m       3,024       409       30.1       69.9       8.8       3,218,275       2,935         m       4,118       111       15.3       84.7       7.7       457,098       1,423         m       520       3,514       8.3       91.7       2.0       1,657       2,935         m       4,118       1,144       8.3       91.7       2.0       1,657,098       1,622         m       948       12,453       8.6       91.4       2.0       1,626       421         m       2,135       27.8       91.7       2.0       1,627       425,708       1,622         ach       2,3335       27.8       72.0       2.9       1,626,450       1,720         q:m       2,235       13,144       872.2       2,321       1,525       1,626,450       1,525         ach       155       28,806       57.8       72.2       1,525,450       1,525       1,525         u.m       3,750       824       44.2       55.3       1,666,656       1,344       42.255         u.m       3,755<	· I	Y Pavenen	ΰ	1,49	<b>ST</b>	ف		4	,148,41	,916,00
q:m       16,675       193       30.1       69.9       8.8       3,218,275       2,935         m       3,024       409       21.3       78.7       7.6       1,236,816       1,142         m       520       3,514       8.6       91.4       2.0       1,857,098       1,740         m       948       1,746       8.6       91.4       2.0       1,857,098       1,790         m       948       12,453       14.4       85.6       3.2       498,120       1,655,208       1,622         ach       2,235       37,354       28.0       77.7       1,877,280       1,522         ach       2,235       14.4       85.6       3.2       1,876,450       1,522         ach       2,730       12,451       1,667,930       1,622       1,323         ach       2,750       17.3       3,090,000       2,133       4,223         u.m       3,750       824       44.2       55.3       4,264,930       4,284,930       4,223         u.m       3,750       824       44.2       55.3       1,876,815       1,876,815       1,876,815       1,876,815         u.m       3,750       828	1	Sidewalk Block	ч.	20		ณ่า		o'	, 580, 84	, 322, 0(
m       3.024       409       21.3       7.8.7       7.6       1.236,816       1.142         m       4.118       111       15.3       84.7       7.7       457,098       1.421         m       948       1.746       8.5       91.4       2.0       1.655,208       1.622         ach       520       3.7445       8.6       91.4       85.6       3.2       4.87,098       1.730         ach       57       33,354       28.0       72.0       1.655,208       1.622         ach       2,235       33,354       28.0       72.0       2.98,805       3.432         ach       2,235       27.8       7.1 $1.766,770$ 161         ach       2,235       33,354       28.0       72.0       1.876,815       1.822         ach       2,555       33,356       55.3       1.733       3.990,000       2.555         u.u.m       3,750       82.4       44.2       55.3       1.876,815       1.876,815       1.822         ach       1.55       2.33,000,000       125,966,656       1.23,425       1.33,425       1.13,425         u.u.m       3,750       8.9       1.000       1.2	1	Island Block	5	6,67	<b>D</b> E 9	<b>.</b>	<b>ო</b> .		218 2	, 935, 0(
w       4.118       111       15.3       84.7       7.7       457,098       452,1798         m       520       1,746       8.3       91.7       2.0       1,827,280       1,623         ach       5       33,354       28.0       72.0       1,827,280       1,623         ach       2       33,354       28.0       72.0       2.9       166,770       161         q.m       2,235       70.0       67.7       32.3       13.1       1.827,280       1,623         q.m       2,235       37,05       34,46,930       438,120       1,625,450       135,625,450         ach       2,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       833,65,595       1,325,956,656       13,42,750       2,555         u.m       3,750       1,60.7       125,966,656       113,421       125,255         u.m       3,755       3,44,48,750       200,000	1	Curb & Gutter	E.				ю. 1		236.8	,142,00
m       520       1,324       8.5       91.4       2.0       1,65,208       1,622         ach       40       12,453       14.4       8.5       91.4       2.0       1,65,208       1,622         ach       40       12,453       14.4       8.6       91.4       2.0       1,65,208       1,622         ach       40       12,453       14.4       8.6       91.4       2.0       1,65,208       1,622         ach       155       9208       535       27.8       72.2       2.9       1,876,815       1,822         ach       155       208       50.5       30.6       30.8       69.2       5.3       4,464,930       4,223         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555       1,822         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555       13,421         ach       46.66       55.3       4,464,930       4,223       4,223         u.m       3,750       824       44.2       52.8       17.3       3,090,000       2,555       54.5         ach       1       36.8	닡	•	ផ	ц,	HI	ດ່	d.,		0 . t c d	421.0
m $745$ $1,450$ $6.5$ $71.4$ $85.6$ $3.2$ $495,770$ $482$ ach5 $30,354$ $28.4$ $85.6$ $3.2$ $498,120$ $482$ ach $155$ $30,354$ $27.8$ $72.2$ $2.9$ $1,876,815$ $1,822$ ach $155$ $208,535$ $27.8$ $72.2$ $2.9$ $1,876,815$ $1,822$ ach $155$ $208,535$ $27.8$ $72.2$ $2.9$ $1,876,815$ $1,822$ ach $155$ $208,535$ $30.6$ $30.8$ $69.2$ $5.3$ $4,464,930$ $4,223$ $3,750$ $824$ $44.2$ $55.8$ $17.3$ $3,090,000$ $2,555$ $3,750$ $824$ $44.2$ $55.8$ $17.3$ $3,090,000$ $2,555$ $3,750$ $824$ $44.2$ $55.8$ $17.3$ $3,090,000$ $2,751,813$ $3,750$ $824$ $44.2$ $55.8$ $10.0$ $125,966,656$ $113,421$ $36.8$ $63.2$ $10.0$ $125,966,656$ $113,421$ $35.1$ $47.2$ $52.8$ $10.0$ $125,966,656$ $555$ $4,750$ $203,764,555$ $948,698$ $45.5$ $54.5$ $337,625,595$ $948,698$ $45.5$ $54.5$ $337,625,595$ $948,698$ $45.5$ $54.5$ $54.5$ $57.2$ $202,504$ $45.5$ $54.5$ $54.5$ $55.5$ $94.8,698$ $40.5$ $59.5$ $16.9$ $124,925,655$ $94.8,592$ $40.5$ $59.5$ $16.9$ <td>-10 1</td> <td>Fipe O U.5U</td> <td><b>E</b> 1</td> <td>¥."</td> <td>0.1</td> <td></td> <td></td> <td></td> <td>7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7</td> <td>2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2</td>	-10 1	Fipe O U.5U	<b>E</b> 1	¥."	0.1				7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
ach       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *       *	∩≀ ( 1	Ditch U-0.5 X U.	ឝ ្	ক ৰ			-i u		7 000 ×	0, 220,
ach       2,235       70.0       67.7       32.3       13.1       1.876,450       135         ach       155       28,806       67.7       32.3       13.1       1.876,815       1.822         ach       155       28,806       67.7       32.3       13.1       1.876,815       1.822         ach       155       28,806       67.7       32.3       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       837,625,595       751,813       42,223         u.m       36.8       63.2       10.0       125,966,655       751,813         u.m       37.5       53.1       1337,625,595       948,698         u.m       40.5       54.5       355,074,345       1131,202         u.m       40.5       59.5       14.925,655       95,503         u.m       40.5       59.5       14	N. (	uusu	ø	⊅. I	3 F	di c	o c		- , t 0 \ 5 \	
q.m       2.235       27.8       72.2       2.9       1,876,815       1,822         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         34.6       65.4       10.0       125,966,656       113,421         36.8       63.2       10.2       837,625,595       751,813          37.6       55.595       751,813          37.6       55.595       751,813          37.6       55.595       948,698          39.1       60.9       8.9       1,070,625,595       948,698          39.1       60.9       8.9       1,070,625,595       948,698          39.1       60.9       8.9       1,070,625,595       948,698          39.1       60.9       8.9       1,070,625,595       948,698          39.1       60.9       59.5       14,448,750       202,503         <	Ni I	ubr:	Q		້ເ	ά	N C	N C		
ach       155       28,806       30.8       69.2       5.3       4,464,930       4,228         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         u.m       3,750       824       44.2       55.8       17.3       3,090,000       2,555         34.6       65.4       10.0       125,966,656       113,421         35.8       63.2       10.2       837,625,595       751,813         36.8       63.2       10.2       837,625,595       751,813         36.8       63.2       10.2       837,625,595       948,698         37.5       314,448,750       202,504       948,698         *       7.5       35.6       14,448,750       202,504         *       7.5       59.5       16.9       114,925,655       95,503		e Marking	υ.	N	ji g	- 1	νíc	n (		
u.m 3,750 20,000 2,555 17.3 3,090,000 2,555 u.m 3,750 824 44.2 55.8 17.3 3,090,000 2,555 34.6 65.4 10.0 125,966,656 113,421 36.8 63.2 10.2 837,625,595 751,813 47.2 52.8 15.5 233,000,000 196,885 39.1 60.9 8.9 1,070,625,595 948,698 about 40.5 59.5 16.9 1,385,074,345 1,151,202 * 7.5 40.5 59.5 16.9 1,385,074,345 1,151,202 40.5 59.5 16.9 1,385,074,345 1,151,202 59.5 16.9 1,500,000 1,246,705	Ni ( 1	reffic Signals	ថ្ម			- c	νic		0.010.	, 0 0 0 0 0 0 0 0 0 0 0 0
34.6       65.4       10.0       125,966,656       113,421         36.8       63.2       10.2       837,625,595       751,813         36.8       63.2       10.2       837,625,595       751,813         36.8       63.2       10.2       837,625,595       751,813         37.655       54.5       52.8       15.5       948,698         39.1       60.9       8.9       1,070,625,595       948,698         39.1       60.9       8.9       1,070,625,595       948,698         39.1       60.9       8.9       1,070,625,595       948,698         39.1       60.9       8.9       1,070,625,595       948,698         39.1       60.5       59.5       16.9       1,134,925,655       95,503         %       7.5       59.5       16.9       1,14,925,655       95,503	N 0	ridge Lighting For	ស្ដ	4 6	0 0 0	5 M	n ur	1 0		2 IC 2 IC 2 IC
34.6       65.4       10.0       125,966,656       113,421          36.8       63.2       10.2       837,625,595       751,813          37.6       52.8       15.5       233,000,000       196,885          39.1       60.9       8.9       1,070,625,595       948,698          39.1       60.9       8.9       1,070,625,595       948,698          39.1       60.5       54.5       35.6       314,448,750       202,504          40.5       59.5       16.9       1,385,074,345       1,151,202         %       7.5       40.5       59.5       16.9       1,14,925,655       95,503	11					n i	51	- i	· · · · · · ·	
S       1      S       35.8       63.2       10.2       837,625,595       751,813        S       1       47.2       52.8       15.5       233,000,000       196,885        S       1       47.2       52.8       15.5       233,000,000       196,885        S       1       -       45.5       52.5       948,698        S       1       -       45.5       54.5       35.6       314,448,750       202,504        S       1       -       45.5       54.5       35.6       314,448,750       202,504        S       1       -       45.5       59.5       16.9       1,385,074,345       1,151,202          7.5       59.5       16.9       1,14,925,655       95,503          7.5       59.5       16.9       1,500,000,000       1,246,705	·	ubtot				×.#	່ ທ	0.	25,966,65	13,42
36.8       63.2       10.2       837,625,595       751,813         47.2       52.8       15.5       233,000,000       196,885         10.5       15.5       233,000,000       196,885         39.1       60.9       8.9       1,070,625,595       948,698         11.5       1       -       45.5       54.5       35.6       14,448,750       202,504         *       7.5       40.5       59.5       16.9       1,385,074,345       1,151,202         *       7.5       40.5       59.5       16.9       1,385,074,345       1,151,202		· · · · · · · · · · · · · · · · · · ·				,				
S     1     47.2     52.8     15.5     233,000,000     196,885       39.1     60.9     8.9     1,070,625,595     948,698      S     1     -     45.5     54.5     35.6     314,448,750     202,504       %     7.5     40.5     59.5     16.9     1.488,750     202,504       %     7.5     40.5     59.5     16.9     114,925,655     95,503						6	m		37,625,59	51,813,00
39.1       60.9       8.9       1,070,625,595       948,698         39.1       60.9       8.9       1,070,625,595       948,698         35.6       314,448,750       202,504         about       40.5       59.5       16.9       114,925,655       95,503         %       7.5       40.5       59.5       16.9       114,925,655       95,503	ri I Er	. Temporary Works	г. s	H	• -	7.		ហ	33,000,00	96,885,0
S 1.070,625,595 948,698 S 1 - 45.5 54.5 35.6 314,448,750 202,504 about 40.5 59.5 16.9 1,385,074,345 1,151,202 % 7.5 40.5 59.5 16.9 1,4,925,655 95,503				******		1	1.	1 -		
		Cost Tot	1 3			თა		ີ. ເວັນ	.070,625,59	48,698
about     40.5     59.5     16.9     1,385,074,345     1,151,202       %     7.5     40.5     59.5     16.9     114,925,655     95,503		очет не	- i			i l	4 I	-	14,9440,10	00' 40C' 20
200,000 1,360,000 1,366,70 1,360,000 1,366,70		tal Construction Co vsical Contingency	æ	noq.		00	<u>.</u>	6 0	,385,074,34 114,925,65	,151,202,0 95,503,0
		- · · · · · · · · · · · · · · · · · · ·		•	•		Ċ	ų		201 201
		. Total				5Ì	., н I			

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for New Krungthep Bridge	•	Table A 9.7.7	Land Acquisition and Compensation for New Krungthep Bridge
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(Unit: Baht, October 1986 prices)

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	Location & Area	Financial Cost	Com F	ponent L	(%) Tax	Economic Cost
.)	Land Acquisition					الله منه شرو چو ها الله من الله عنه من جو هو هو.
	Thonburi Side					
	West of Taksin Road					
	(1,623.5 sq.wah)	76,710,375	-	100	4.8	73,057,50
	Near Chinese Shrine				-	
	(299.5 sq.wah)	6,289,500		100	4.8	5,990,00
	Bangkok Side					
	Near Customs Training Center					
	(414.3 sq.wah)	10,874,062	-	100	4.8	10,356,25
	Along Charoen Krung Road					
	(1,064.8 sq.wah)	44,719,500	-	100	4.8	42,590,00
	South of River Side Road					
	(582.3 sq.wah)	21,397,687	~	100	4.8	20,378,75
	Contingency (about 15%)	23,008,876	-	100	1.7	22,627,50
	Subtotal	183,000,000		100	4.2	175,000,00
	Building&Housing Compensation			، جند کلو سے وجہ بند کہ ک		
	Thonburi Side					
	West of Taksin Rd. (1,534 sq.m	) 5,489,925		100	4.8	5,228,500
	Near Shrine (456 sq.m)	1,436,400		100	4.8	1,368,000
	Bangkok Side					
	Near Training Center					
	(1,094 sq.m)	3,843,000		100	4.8	3,660,000
	South of River Side Rd.	1				
	(1,348 sq.m) Along Charoen Krung Rd.	4,668,195	-	100	4.8	4,445,900
	(7,754 sq.m)	28,348,950	-	100	4.8	26,999,000
	Contingency (about 25%)	8,213,530	-	100		
	Subtotal	52,000,000		100	4.0	50,000,000
•~	Total	235,000,000		100	4.1	225,000,000

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Work Items	Financial Cost		onent L		Economic Cost
Estimate per 10 km for a year			L COL BUD COM AND SHE	,,	
) Daily maintenance Slope grass cutting & transportation	170,460	14.5	85.5	4.8	162,360
2) Seasonal maintenance after flood period Drainage work			•		
& lane marking	89,620	30.6	69.4	8.3	82,220
Total per 10 km Total per km	260,080 26,008		80.0 80.0		244,580

#### Table A 9.7.8 Annual Maintenance Cost for Cement Concrete Surface Road

#### Table A 9.7.9 Annual Maintenance Cost for Viaduct or Bridge

(Unit: Baht, October 1986 prices)

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Work Items	Financial	Comp			
	Cost	F	L	Tax	Cost
Estimate per 1.0 km elevated road for a year					
1) Daily Inspection	105,660	23.4	76.6	7.2	98,100
<ol> <li>Annual Maintenance Repair of pavement Repair of lighting pole, Concrete curb &amp; railing, median strip, and</li> </ol>	11,600	27.5	72.5	9.3	10,520
guard rail	20,858	26.6	73.4	4.9	19,828
Replace of expansion joint	32,428	67.5	32.5	12.1	28,488
3) Electricity Charge	29,500	7.0	83.0	10.0	27,000
Total per km	200,046	27.7	72.3	8.1	183,936

# Table A 9.7.10 Annual Maintenance Cost for Main PC River Crossing Bridge

(Unit: Baht, October 1986 price				/0.1	
Work Items F	inancial Cost	Comp F	onent L	(%) Tax	Economic Cost
stimate per 1.0 km main bridge for a year					
) Annual Maintenance Cost of Viaduct Road (Table A 9.7.9)	200,046	27.7	72.3	8.1	183,936
) Additional Maintenance		1 . *			n an Arta an 1970. An t-airtean Arta
Periodical inspection	50,000	-			48,700
Repair of bearing,					
girder concrete surface, pier curtain wall	186,835	40.4	59.6	7.2	170,922
Stone pitching	63,267	33.7	66.3	6.6	59,073
Total per km	500,148	33.2	66.8	7.5	403,558

Appendix 9.8.1	Cost	and	Benefit	Stream	of	New	Krungthep	Bridge	

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#### ECONOMIC EVALUATION

New Krungthep Bridge Project (Basic Case)

Discount Rate (%) = 12.0

UNIT : million Baht

· · ·			DISCOUNTED		
YEAR	COST	BENEFIT	COST	BENEFIT	
0	0,	0.		0,	
1	266.	0.	238,	0.	
2	444.	0.	354.	0.	
3	449.	.0.	320.	0.	
4	447.	0.	284.	0.	
5	1.	144.	0.	82.	
6	1.	228.	0.	116.	
7	1.	312.	0.	141.	
8	1.	397.	0.	160.	
9	1.	481.	Ο.	173.	
10	1.	565.	0.	182.	
11	1.	650.	0.	187.	
12	1.	734.	0.	188.	
13	1.	818.	0.	187.	
14	1.	903.	0.	185.	
15	1.	987.	0.	180.	
16	1.	900.	Ο.	147.	
17	1.	814.	0.	119.	
18	1.	727.	0.	95.	
19	1.	640.	0.	74.	
20	1.	554.	0.	57.	
21	. 1.	467.	Ο.	43.	
22	1.	381.	0.	31.	
23	1.	294.	0.	22.	
24	1.	207.	0.	14.	
25	-910.	121.	-54.	7.	
NET PRE B/C RAT	SENT VA		1247.		

IRR= 20.71

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ECONOMIC EVALUATION

New Krungthep Bridge Project (Cost up 15%)

Discount Rate (%) = 12.0

UNIT : million Baht

· .		a Na sa ta	DISCO	UNTED
YEAR	COST	BENEFIT	COST	BENEFIT
0	0.	0.	0.	0.
1	306.	0.	273.	0.
2	511.	0.	407.	0.
3	516.		367.	··· 0 •
4	514.	0.	327.	0.
5	:1.	144.	Ο.	82.
- 6	1.	228.	0.	116
7	1.	312.	0.	141.
8	1.	397.	0.	160.
9	1.	481.	0.	173.
10	1.	565.	0.	182.
11	1.	650.	0,	187.
12	1.	734	0.	188.
13	1.	818.	0.	187.
14	. 1.	903.	.0.	185.
15	1.	987.	0.	180.
16	1.	900.	0.	147.
17	1.	814.	.0.	119.
18	1.	727.	0.	95
19	1.	640.	<u>,</u> 0,	71.
20	1.	554.	0.	57.
21	.1 .	467.	:0.	43.
22	. 1.	381.	0.	31.
23	1.	294.	0.	22.
24	· · · · <u>1 ·</u> ·	207.	0.	14
25	-1046.	121.	-62.	7.
			میں ہوئے نہی تھر سرامی افرار آسار ہو	
		LUE =	1075.	
B/C RA	TIO =	1.82	an a	

IRR= 18.86

#### ECONOMIC EVALUATION

New Krungthep Bridge Project (Benefit less 15%)

Discount Rate (%) = 12.0

UNIT : million Baht

· .			DISC	OUNTED
YEAR	COST	BENEFIT	COST	BENEFIT
0	0.	0.	0.	0.
. 1	266.	0.	238.	0.
2	444.	0.	354.	0.
3	449.	0.	320.	0.
4	447.	0.	284.	0.
5	1.	122.	0.	69.
6	1.	194.	0.	98.
7	1.	265.	0.	120,
8	1.	337.	Ο.	136.
9	1.	409.	Ο.	147.
10	1.	480.	Ο.	155.
11	1.	553.	0.	159.
12	1.	624.	Ο.	160.
13	1.	695.	0.	159.
14	1.	768.	Ο.	157.
15	-1.	839.	Ο.	153.
16	1.	765.	0.	125.
17	1.	692.	Ο.	101.
18	1.	618.	0.	80.
19	1.	544.	0.	63.
20	1.	471.	Ο.	49.
21	1.	397.	· 0.	37.
22	1.	324.	0.	27.
23	1.	250,	Ο.	18.
24	1.	176.	0.	12.
25	-910.	103.	-54.	6,
NET PR B/C RA	ESENT VA TIO =	LUE = 1.78	888.	

IRR= 18.57