REPUBLIC OF INDONESIA

MINISTRY FOR COMMUNICATLONS
DIRECTORATE GENERAL OF LAND TRANSPORT
AND INLAND WATERWAYS

TENDER DOCUMENTS FOR NEW RAILWAY LINE FOR CENGKARENG AIRPORT CONSTRUCTION PROJECT

STRUCTURAL CALCULATION SHEETS:

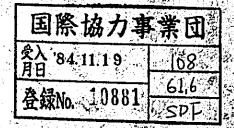
PACKAGE I CIVIL AND ARCHITECTURAL WORK

9 of 11

AUGUST 1984

JAPAN INTERNATIONAL COOPERATION AGENCY
(IICA)





アイ・クッロマイシェ作成

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SI GENERAL

- (1) NAME OF BUILDING
 - 9) AIRPORT TERMINAL STATION
 - 1) ROOH
 - i) BOOKING OFFICE
 - mi) TERMINAL BUILDING
 - b) KOTAINTAN STATION
 - 1) ROOF
 - ii) STATION BUILDING
 - C) SIGNAL CABIN
 - 1) TYPE (A)
 - ii) TYPE (B)
 - d) WATCHMAN'S BOX

(2) DESIGN OF BUILDINGS

1) AIRPORT TERMINAL STATION

AVROOF

IN STRUCTURE STEEL STRUCTURE

a mistery the state of the

iii) ROOF FINISH : ROOF TILE

DECK PLATE

W) METHOD RIGID FRAME

STRUCTURE

b) BOOKING OFFICE

1) STRUCTURE : REINFORCED CONCRETE

in STORY

III) ROOF FINISH : MORTAR

ASHALT WATER PROOF

MORTAR

IV) METHOD : RIGIO FRAME

STRUCTURE

C) TERMINAL BLD

1) STRUCTURE : REINHORCED CONCRETE

ii) STORY : 3

ROOF FINISH SAME AS BOOKING OFFICE

IU) WALL : BRICK AND CONCRETE.

WI METHOD : RIGID FRAME STRUCTURE

2) KOTAINTAN STATION

a) ROOT

STRUCTURE : STEEL STRUCTURE

117 STORY

111) ROOF FINISH : CORRUGATED ASBESTOS

CEMENT TILE

IV) METHOD : RIGID FRAME STRUCTURE

b) STATION BLD

I) STRUCTURE : REINFORCED CONCRETE

ii) STORY : Z

iii) ROOF FINSH: SAME AS BOOKING OFFICE

IV) WALL : BRICK

V) METHOD : RIGID FRAME STRUCTURE

3) SIGNAL CABIN TYPELA), TYPE (B)

1) STRUCTURE REINFORCED CONCRETE

11) STORY

111) ROOF FINISH : ROOF TILE

IV) WALL : MORTAR

V) HETHOD RIGID FRAME STRUCTURE

4) WATCHHAM'S BOX

:) STRUCTURE : REINFORCED CONCRETE
WALL STRUCTURE

ii) STORY

11) ROOF FINISH SAME AS BOOKING OFFICE

IV) WALL : MORTAR

V) METHOD : WALL STRUCTURE

(3) MATERIALS (JIS)

A) REINFORCE CONCRETE : FC-210

b) STEEL BAR SD30

c) STEEL: 5541 55C41

1) PILE (A)

4350

(4) METHOD OF DESIGN

A) EARTH QUAKE LOAD

THE REGULATION FOR FARTHQUAKE

RESISTANT DESIGN OF BUILDING

IN INDUNISIA

4) H<10m K0=-0.1

127102HC40 m Km = 1+0.05H

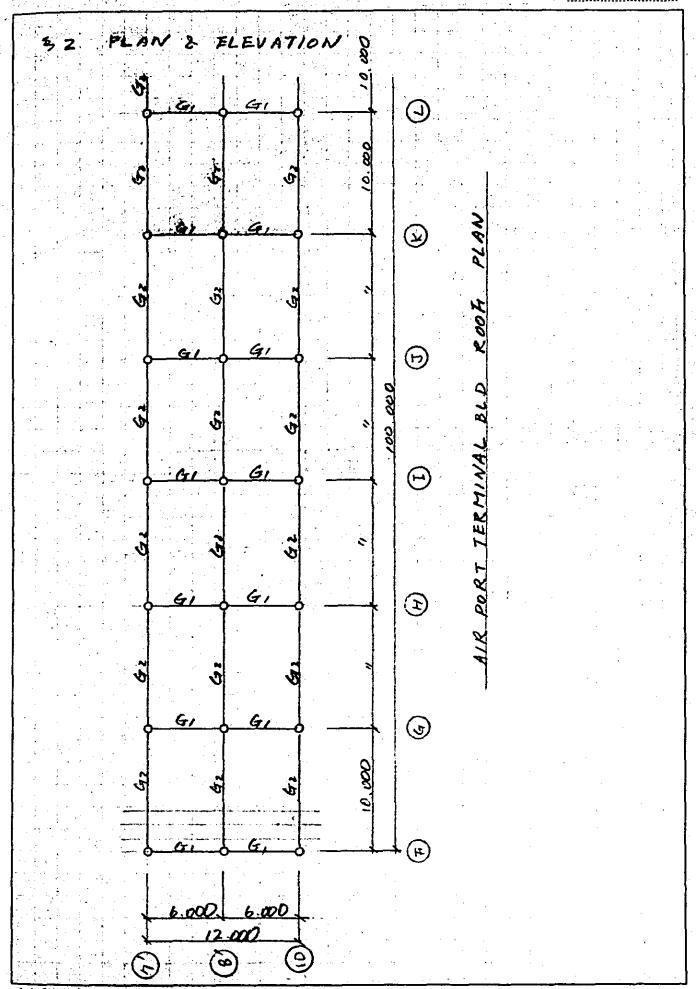
Ko = 1

Km = (1+0.05 H) Ko H

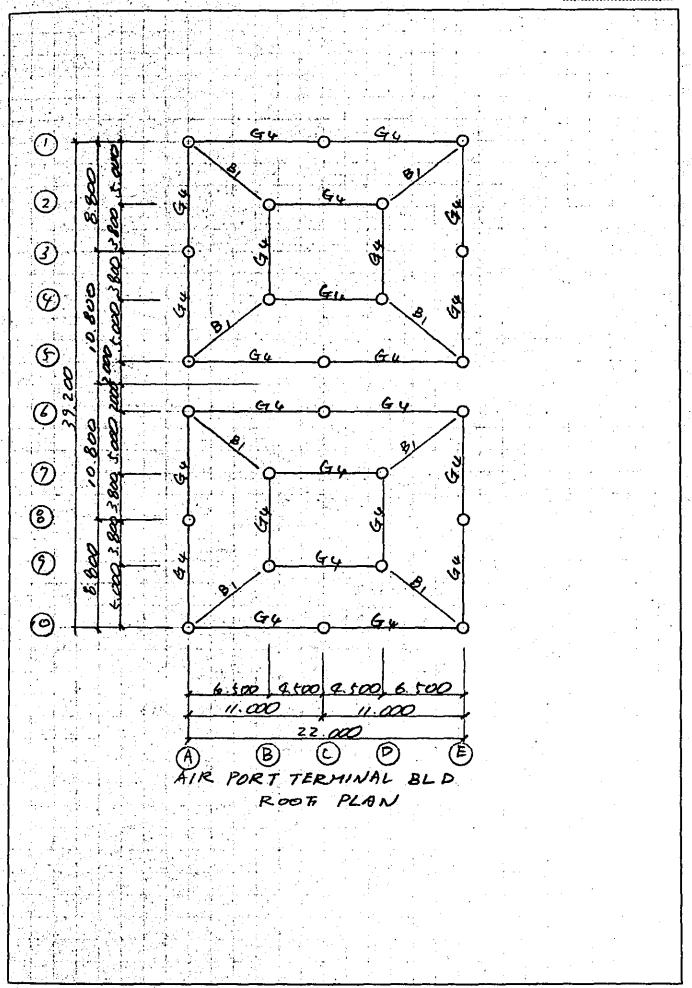
b) WIND LOAD.

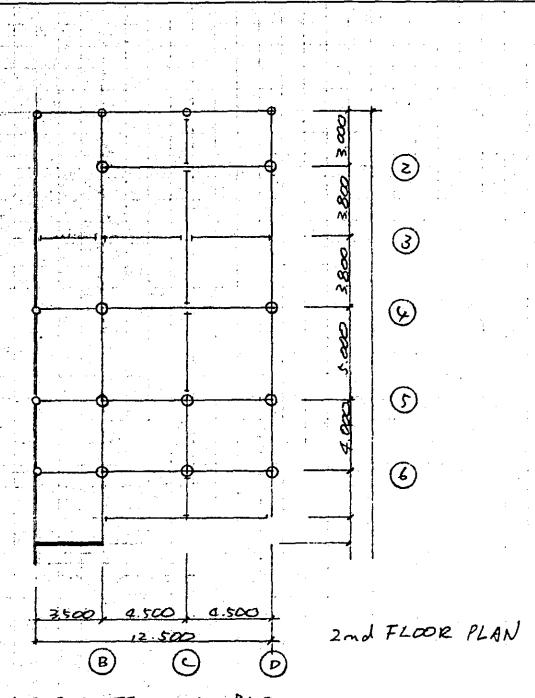
SAME a)

P= 1/6 V2



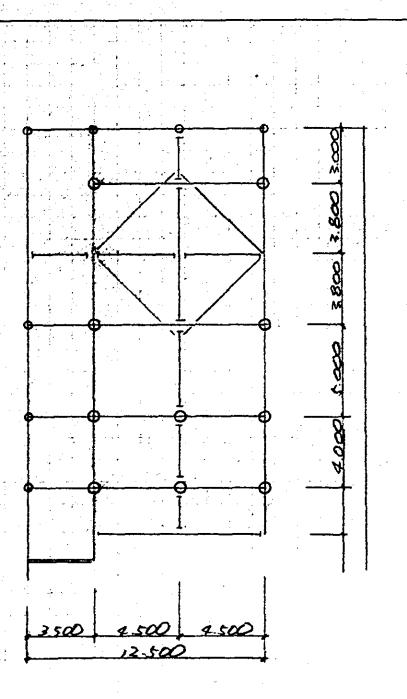
AIR TERMINAL BLP ROOF



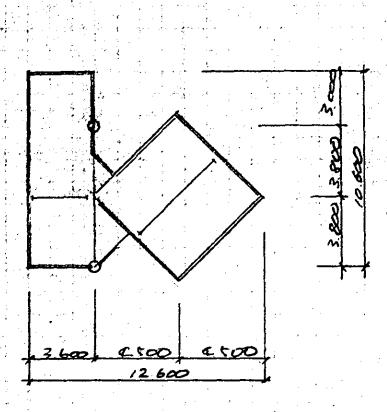


AIR PORT TERMINAL BLP

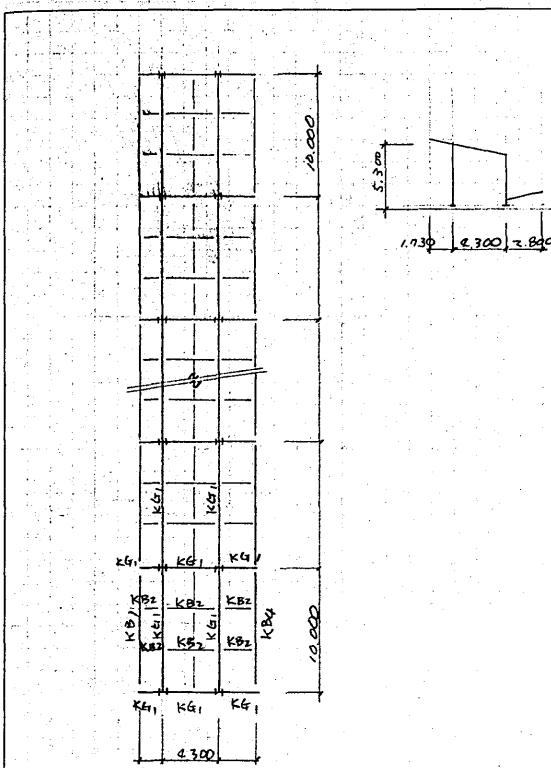
TERHINAL BLD



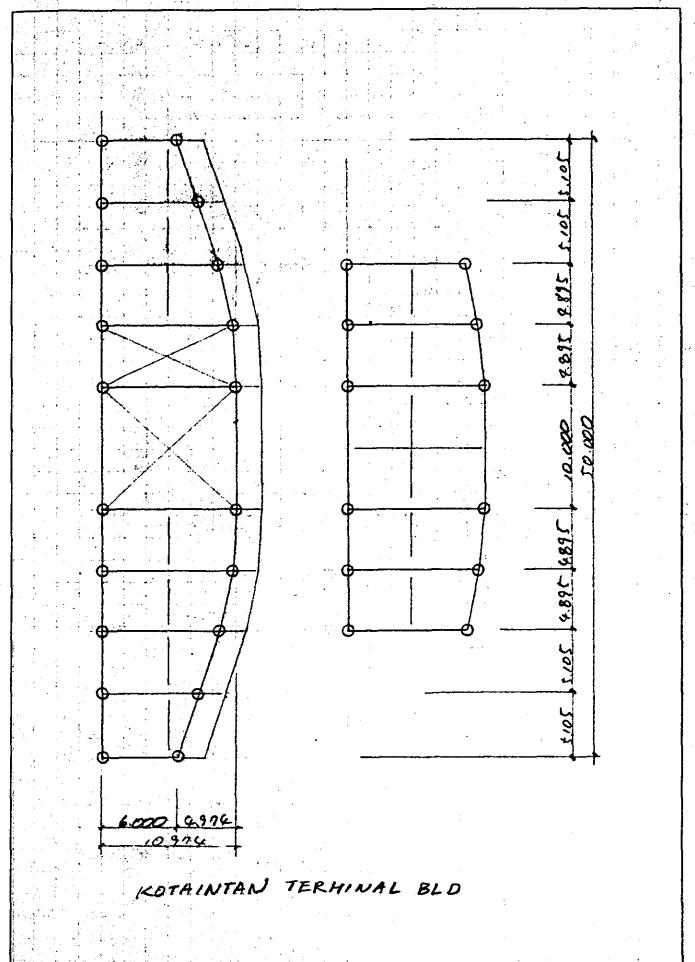
3-Yd FLOOR PLAN



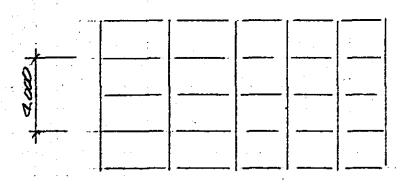
ROOF FLOOR PLAN



KOTA INTAN PLATFORM ROOF PLAN

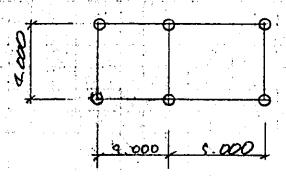


SIGNAL	CH	BIN	TYPE	A	
8				4.	
8			•		
8					
	_	7.00	×	8 000	
	· · · ·	·	' المساد	FLOOP	

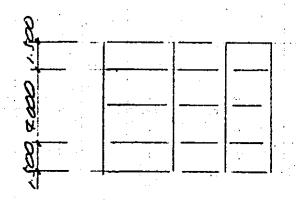


ROOF PLAN

SIGNAL CABIN TYPE (B)



2 nd FLOOR



ROOF PLAN

178 kg/m2. - 180 kg/m

DEHO LOAD ROOF CSTEEL BLD) CRC BLP)

MORTAL 1.90mm 189 kg/m²

ASPHALT 10 K2/m²

WATER PROOFI

HORTAR 1-20mm 92 K2/m²

RC SLAB 1-130 312

CEILING 10

673 - 580 Kg/m²

ROOFING HESHU 10 K2/m²

DECK PLATE

STEEL FRAME 50

(OFFICE)

TERRAZZO TILE 60 kg/m²

MORTAR @30mm q2

RC SLAB 312

CFILING 30

STAIR

TINISH 100
CONCRETE SLAB 720
CEILING 50

KOTA INTEN

ROOF

CORRUGATED ASBESTOS
CEHENT SLATE
SUB BEAM
STEEL FRAME

20 Fg/m² 10 40 10 Fg/m²

SIGNAL CABIN

ROOF

ROOFITILE 100 F3/m²
ROOFING MESHU 10
PLASTER 25
SUB BEAM 10
CEILING 20

165

WALL

40,30

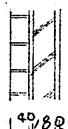
CONCRETE WALL, 1-150 360 12 BRICK t= 110 187 HORTAR FINISH 1-30 63

30,00

HORTAR FINISH \$=30.2 126 F3/m.
CONCRETE WALL \$=150 360
(490)



HORTAR FINISH 1-30,2 126 K2/m²
BRICK t=110 187
313
(320)



CONCRETE WALL t=180 432

BRICK t=10 187

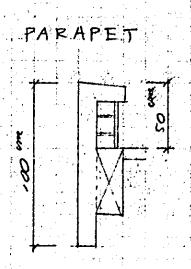
MORTAR FINISH t=30 63

682

(685)

43.44			i
A .		#1# ·	
		77.20	
	-		
3	Pit		30

BRICK 374 MORTAR FINISH t-30 x 2 126 5-00 Kg/m



CONCRETE WALL

BRICK MORTAR 1. 360 x 0.5 = 180 96 187 x 0.3 = 56 19 351 Kg/m (355)

10 150

355 Fd/m

LIVE LOAD

ROOF

50 Kg/mi

OFFICE

250 Kg/m2.

STAIR

300 leg/mi

TERRALE

200

TOTAL LOAP LOAD LOAD + LIVE LOAD)

- CIT AIRPORT TERMINAL STATION
 - a) PLAT FORM CSTEEL BLD)

ROOF 180+50-230 18/m2

b) RC BLP

ROOF \$80+50 = 630 =8/mi

OFFICE 440+250 = 690 Fa/m

STAIR 870 + 300 = 1170

TERRACE \$80+200 = 180

(2) KOTA INTAN STATION

A) PLAT FORM (STEEL BLD)

ROOF 70+f0 + 120 /3/mi.

b) RC BLP

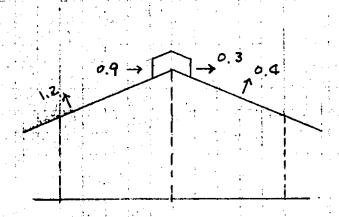
RUOF + 180 +50 = 630 19/~

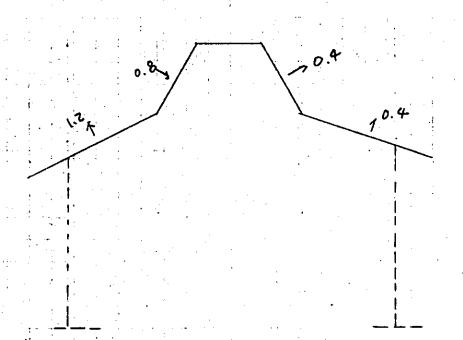
OFFICE 440+2 to = 690 K8/m2

CT SIGNAL CABIN

ROOF 165-10 = 215 Kg/m2

WIND LOAD

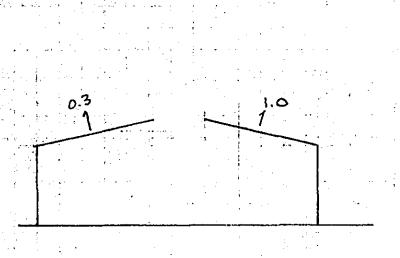




WIND PRESSURE

$$P = \frac{1}{16}V^2$$

 •	No	27
 		 · · · · ·
 		, .



\$4	A	KIAL LO	DAP OF COLUMN			
		(1) AIR	PORT TERHINAL	•		
	;					
ier	/	POOA BEAM COLUMN	0.23(3.0+2.0)(1020)*109 0.1 (3.0)*107 0.1 2 10.0 0.1 × 4.5	12.31 0.32 1.0 0.45	14.0%	14.08
1 .	FOR BASE	FG	0.77 (10.0+3.0)	10.01	(0.01	24.09
			17, 14, 12, 13, 1 K, 1L, 1M 1N, 10, 1P, SAME			
2'4	1	ROOF BEAM LOLUMN	0.23 (6.0)(10.0) × 1.0 7 0.1 (10.0 + 6.0 × 1.0 7 7 0.1 7 9.0	14.77 1.64 0.9	17.31	ות.3).
	FOR BASE	FG	0.77 (10.0+6.0)	12.32	12.32	29.63
18		ROOFI BEAM COLUMN	0,23x(\$.\$47.07(4.\$)*1.0") 0.23(1.907(\$.207) *1.09 0.15(4.4+5.57 0.15(4.8)	8.31 2.43 1.49 1.49	13.90	13.70
	FOR BASE	FG	0.77 (4.4+ 5.5)	7.62	7.62	2132

						
===						
1'A	3	ROO F	0.63 × 1.8 × 3.4	3.86	;	
1 7		PARAPET	0.355 (1.8 + 3.4)	1.85		
»"• • "			0.36 *0.5 (1.843.4)	0.94		
		WALL	0.61 × 1.25 (1.8+3.4)	3.97		
		0 4 4 14	0.24 (1.8+3.4)	1.5 2		
		BEAM		0.66	12 60	2 6 2
	, ,	COLUMN	× 3.0	0.00	15.23	15.43
	2	STAIR		7.16.		
		BEAM	0.24 C1.8+ 3.4)	. 1.25		
		COLUMN	0.22 × 3.5	. o:ng .		
i,		WALL	0.6(>2.75 (1.8+3.4)	8.72	17.90	30,43
						•
	1	STAIR	7.16	7.16	٠.	
r sagar		BEAM	1.25	1.25		
		COLUMN	0.77	0.77	,	., .,
		WALL	0.61 x 3.0C1.8+3.4)	9.52	18.70	49.13
	d. 1	and an and				
					-,	
		h. Nagazina				
		FG.	0.77 (1.8+3.4)	4.71	4.7]	13.84
				,		
						·
3 A1	3	ROO FI	0.63 * 1.8 * 5.3	6.10		
		PARAPET	0.355 (5.3)	1.88		
		WALL	0.36×0.5(5.3)	0.45		
		W//CO	0.61 x 2.t (t.3)	8.08		,
			0.49 x 2.5 x 1.8	2.2/	1	
.			E CALL CAB & GO	1	1]
1		D-A-M	0.24.74.41.07	107	į.	, ,
		BEA M	0.74 (5.471.8)	1.73	21/1	3111
		BEA M COLUMN	0.24 (5.471.8)	0.66	21.61	21.61
		and the second s		1	21.61	21.61
		and the second s		1	21.61	21.67
		and the second s		1	21.61	21.61

No. 30

	2.00		<u>a engez e transa</u>	<u> </u>		,	······································
	3A1	2	SLAB	0-69 x 5.3 x 1.8	6.58		
	>/A	1	BEAM	1.93	1.73		
	, ₁ , , , , , , , , , , , , , , , , , , ,		WAL	0.66. 0.61 × 3.0 × 5.3	9.70		
				0.49 × 3.0 × 1.8	2.65	21.32	42.93
		d y c	6149		1 1 1 0	. ,	
		/	SLAB	6+8	6.58		
			LOLUMN	1.73+0.66+9.70+2.65	14.74	21.32	64.25
			WALCE			, , ,	
		,					
			FG	0.77 (5.3+1.8)	6.47	5.47	69.72
	2 () () () () () () () () () (
	4						:
					- 1.		
	4A'	3	ROOF PAKAPET	0.63×18×16 0.355 (1.8+1.8)	2.04 1.26		
		-	BEAM	0.24 (3.6)	0.86		
1			COLUMN		0.66		
		i,	WAL	0.36 × 0.5 × 3.6 0.61 × 3.0 × 3.6	0.65		
Ì	•			0.49 × 3.0 × 1.8	245	14.63	14.63
	1				_	·	
		2	SLAB:	0-69×1.8×1.8	2.24	·	
Ì			COLUMN	0.86	0.86	·	
l			ROOF	0.78,25x1.8	3.51		
			BEAM	0.24 (2.5)	0.60	·	
			PARAPET	0.355 (2.5)	0.89		
			WALL	0.36 × 0.5 × 2.5	0.45		
				0.61 x 4.4 x 3.D	8.05	. 4	
				0.49 x 3.0 x 1.8	265	19.91	34.54
	1						
× 10							

: 37						
4A'		SLAB BEAH COLUMN	0.24 (4.4+1.8)	6.49		
		WALL	0.61 x 4.4 x 3.0 0.49 x 3.0 x 1.8	8.05	18.42	t2.96
		FG	0.77 (4.4+1.8)	4.11		57.73
5A1	2	TERRACE PARA PET	0.315 , 4.7	632		
		BEAM COLUMN WALL	0.24 ×(4.5+1.8) 0.72 × 3.5 0.36 × 0.5 × 4.5 0.61 × 3.0 × 4.5 0.49 × 3.0 × 1.8	0.07 0.81 8.24 2.65	21.90	21,90
					20.70	21.10
		SLAB BEAM COLUMN WAL	0.61x3.0 x7.T	559 1.51 0.77 8.24		
			0.49 > 30 > 1.8	2.45	18.56	4046
		FG	0.77 > (4.5 +1.8)	4.85		45.3/
-	erverige into					1.00

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la.			. 1	1.
14.	 	1	~	~

						;
7B	2	TERRACE				
		PARAPET		2.15		
		BEAH	0.36 (2.5+1.8+2.25)	2.36		1
		WALL	0685×3.0×1.8	3.70		:
	-4-		0.36 x 0 t , (1.812.0+2.25)	(-09		
			0.6(73.0(2.0+2.0)	7.32	22.32	22.59
1.0						
	1	SLAP	0.69 (2.5 x 1.8 + 1.25 × 2.25)	5.05		
		BEAM	2.36	2.36		
		WALL	0.685 * 3.0 x 1.8	370		
			0.61,×3.0 (2.0+2.0)	7.32	18.43	40.75
					·	
-					: '	
4.		76	0.79 (4.4+1.8+4.5)	8'54	·	
		STEEL				
		COLUMN	19 219 49 W	1949	2773	68.48
	- 1 ×					
2B	3	ROOF	0.63 = 1.8 = 3.5-	3.86	1	
		PARAPET	0.355 (3.47	1.21		
		BEAM	0.24 (34)	0.82		
		COLUMN	0.95 × 3.5	3.3.3		
		WALL	0.61 × 3.0 (3.4)	6.22	11.44	15.44
		STEEL -		,	,	
		COLUMN	19.49		<u> </u>	
1 . 1						1
	2	STAIR	1.17 × 3.4 × 1.8	7.14		
		TERRACE	0.78 1.5 × 4.5	5.27		
			0.78x1.9x4.5	6.67	Í.	
		SLAB	0.69 1.9 12.25	2.95		
		ROOT	0.63x (1.9+3.8)/2 x2.25	4.04		}
		BEAM	0.36(3.18+3.18)	2.2.9		
		PARAPET	T = = =	1.13		
		WALL		5.82		[c = c]
	1 .		0.685×3.0×3.4	699	4232	157.76
		COLUMN				
	1			<u> </u>		I.C.A

	2B.	(SLAB	n, 16	7.16		
				0.69 × 1.9 × 2.25	4.6 6 2.9 5		
			BEAM COLUMN WALL		6.3 3.33 6.99	31.4	89.16
							6
			TG	0.77 (4.6+3.4)	6.08		95.24
	4B.	3	ROOF	063, (1.8+405)/2 > 1.8	3.32		
		al Cycle.	PARAPET BEAM	0.355 (1.8+3.18)	1.63	-	
	· · ·		COLUHN	0.85 × 3.5	333		
	en e		WALL	061 x 3.0 (1.8+3.18)	9.11		
			COLUMN	19.49	19:49	38.65	38.65
					4		
		2	SLAB	0.69 × 1.8 > 4 4	279		
			TERRALE	0.69 x 1.8 x 2.25	15.44		
			KOOF	063 (19+3.8)/2×2.25	404		
			BEAM	0.34 (3.18+3.18)	2.29		
			PARAPET		1.13		
	S. #		WALL	0.61 x 3.0 (3.18)	5.82	,	200-
			COLUMN	3.33	3.33	40.30	18,95
.,	, e,	/	SLAB	0,69 x 1.8 x 4.4	546		
į		10): 1	REAL	0.69 * (1.8 * 2.25 + 4.4 * 4.5)	1646		
	S	ने पर क्	COLUML	3.33	6.93		
			WALL	0.61 × 3.0 (1.8 + 1.8)	659	38,71	117.72
- 1			<u> </u>	l e e e e e e e e e e e e e e e e e e e	J		<u> </u>

			No.	34
48		FG	0.77 (6.3+1.87 45-13.15) 12.13	129.85
		· ·		
				·
,				
	,			:

No. 35

				·		
4 D	2	ROOF PARAPET BENFI WALL	0.63 × 4.5 ² /2 0.355 × 6.36 0.36 × (6.36 + 3.18) 0.61 × 3.0 (6.36)	638 226 3.43 11.64		
		PARAPET BEAM COLUMN WALL	0.00 × (2.5+6.3)/2 × 4.5 0.69 × 4.5²/2 0.355×6.3 0.36 (6.3+4.5+6.3/2+6.36) 3.33 0.61×3.0×6.3	15.44 699 2.24 1.31 3.33 11.53	70.55	70.55
	7	SLAB BEAM COLUMN WALL	055 (45+6.3)	19.56 5.94 3.33 11.53	40.36	110.91
		FG	0.77, (3.0763)	7.16		ુા 8.09

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A 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		and the first services and the	andre green to the control of the co			
		y a a T	2.2 (11.2 (7.2 11.0)	12.18		
10		KOOTI BEAM	0.23 (11.0 × 4.5) × 1.0).	1.65		
		COLUMN		1.47	16.30	15.30
		COLUMN			15.30	, , , , ,
					5 i i	
	FOR	FG	8 77 ×(11.0 +2.0)	[0,0]	1001	25.3)
	BASE					
	2	ROOF	0.23 (1.75×2.0)	0.81		
		BEAM	0.1(1.15-2.0)	0.38		
, ·		COLUMN	1	0.63		
. 1		WALL	0 23 × 4.15 × 1.17 (1.75+3.8)/2			
		(RODFI)	0.23 x 4.15 x 1.17 (2.0+4.4)/2	3.63	8.55	8.55
	- :					
					1	
		0.05	A 32 - /h AF = (, F3 / x 2 F) AB	3.77		
		ROOF	0.23 × (7.05 + 4.5)/2 × 2.5 × 1.07	1	·	
1.		DEAL	0.23 (3.8+6.37/2 ×3.25 ×1.09)	404		
		COLUMN	0.15.645+387	1.88		1949
		COLUMN	0.15 * 12.5	110.8	10.94	
	1					
100		e de la companya de l				
						N.
						,
					}	
,						
						•
		Service State]

	ΚΟ Τ)	KATUIA	STATION			
					2	
9 H		ROOF PARAPET BEAM COLUHN WALL	0.56. (2.55-13.0.)	7.34 2.18 3.11 7.49	23.13	23.13
		F9 .	0.77 (2,65-13.0)	4.27	4,27	27.40
OH		ROOFI THRAPET BEAM COLUMN WALL	0.78 × 6.0 × 5.11 0.355 × 5.11 0.56 (6.0+5.11) 0.36, 5.11 0.86×3.5 0.50 × 2.7 × (2.55+4.0	23.91 1.81 6.22 1.84 3.01 8.84	41.63	45.43
		F-9	0.86 (5.55+4.0)	8.21		5 3.84
	2	ROOFI PARAPET SEAM COLUMN WALL	0.63 x 5.0 x 2.45 0.355 (\$.0-12.45) 0.56 (\$.0-12.45) 0.86x 3.0 0.50 x 2.2 (2.45+ 5.0)	7.72 2.64 4.17 2.58 8.70	Z\$.31	25.31
		SLAB BARAPET BEAM WALL	0.69 15.0 XC.0+2.0) 0.355 x 5.0 0.56 (5.0+5.0+2.0) 0.36 x 5.0 0.5 x 2.7 (5.0+5.0)	2415 1.78 6.72 1.80		

	<u>.</u>	Na			······································	38
*						
11.1	1	COLUMN	3.0)	3.01	t2.76	78.07
					, 44 ° ,	
		FG	0.86 (5.046.0)	8.6	8.6	86.67
(II	2	RODFI PARAPET BEAM COLUMN WALL	0.63 (\$.0×5.5) D.35 + x \$. D D.56 (\$.0 + \$.\$) 0.86 x \$. D 0.5 x 2.2 (\$.0)	1733 178 5.88 5.58 5.50	33.07	33.o T
		SLAB PARAPET BEAH COLUMN WALL	0.69×5.0×7.5 0.355×5.0 0.56 (5.0+5.5) 0.36×5.0 0.86×3.5 0.5×2.9×(5.0+5.5)	25.90 1.08 5.88 (.80 3.01 14.18	¢7.\$\$	85.62
		F G	0.86 (6.0 + 6.5)	9.03		94.65

Na. 39.

					***************************************	·····
117	2	ROOTI PARAPET	0.63 (5.79 × 7.45)	27.18 2.64		
			0.56(7.4575,79) 2.58 0.5 + 2.2 ×(2.57 5.79)	2.1.8 2.4.1		. \$.
		BEAH	0.66 (5.0/2+7.45/2)	3.49	52.42	¢2.42
	1	SLAB PARAPET		13.44 2.64		
		BEAM	0.56 (7.45+5.79+2.0) 0.56 x 7.45 3.01	8.53 4.17 3.0 J		
2		WALL	0. Cx2.7 x(2.5+5.79)	(1.19	42.98	95,40
					:	
	, ,		10.86 Ct.09+5.07	9.28	9.28	82.401
						·
		••				
] , [*				
				* ;		

EARTH QUAKE LOAD

AIR PORT TERMINAL STATION

PLATFORM

20071	0.23 / 17.0 × 104	-	406.64
BEAM	0,1 (1000 \$ 37)	=	30.00
	0.1 × 6.5 × 2 × 1]	=	14.30
COLUMN	0.1 (4.45×11×2)	Ξ	9.79
	0.1 × 6.45 × 11	=	765
SUBBEAM	0.05 x 17.0 x 104	B.	·88.4
		v =	5567B

TERMINAL BLD (ROOF)

KOOFI	0.23 + 4.0 ×3.5	:=	3.22
	0.23 x 4.84 (7.6+3.5)/2x2	=	1236
	0.23 + 4.63 (9.0+4.0)/2 *2	=	1384
	0.23 × 9.11 (7.6+17.6)/2 ×2	2	62.80
	0.23 , 7.83 (9.0 + 22.07/2 12	=	55.83
BEAH	0.15 (4.0 , 2 + 3 5x2 + 7.6x2+4.0x2		
	+ 17.0×2+22.0×2)	=	18.93
COLUMN	0.15 x 12.35 x 4	=	7.41
	0.15 x 9.85 x 8	. *	11.82
*			176.21

	TERMINAL	BLP	;
	3F	7	- 0
	ROOF	0.63 × 6.36	25.40
	Addition of the	0.63 × 3.8 × 2.25/z	7.69
		0.63 +1.9 × 2.0/2	= 1.20
		0.63 × 3.6 × 10.6	= 24.05
	PARA PET	1013 FT (10.6+3.6+3.6+ 10.6+6.36x4)	= 19.11
	BEAM	0.27 C10.6x2r3.6x3+6.36x4)	- 15.71
: * !	, , , , , , , , , , , , , , , , , , ,	: 0.4x x 6.36	2.80
	· ·	0.86×3.5	= 301
	WALL	0.61 v 3.0 (10.6+3.6+3.6+6.36×4	, –
		0.49,3.0,3.6/2	- 265
	WATER		= 3.00
	TANK		2
			₩~ 139.06 *
	-T.		
	<u> </u>	200 (20 0 2 1 2 2 2	= 721.0
	SLAP	0.18 (22.3) 12.6 +2.01 3.6)	= 224.8
3- 1	PARAPET) * 20.52
:	BEAM	0+B (8.4 × 2+3,9 × 4+2.0 × 2+3,4 × 3	= 52.2
		+4,4x3+6.6x3+3.0x3)	·
		034 (12.6+23.2+12.6+40+3.6xx	- = 34,42
	A	3.01	- 3.01
	COWNN	0.86×35×10/2	- 15.05
		• •	
	WALL	0.22×3.5×6/2	= 231
	,WALC	39.56+2.65	= 42.21 = 19.81
• · · · · · · · · · · · · · · · · · · ·		06(x2.8/2 (23.2)	
		0.61 x 3.0/2 (23.2+12.6+9.0+20)	= 13.0%
•		0.47x3.0/z (6.8+3.8+3.6x2)	-
1		0.685×3.0/2 ×3.6	= 3.700
			- 1.D.2 0.7%
			o = 473.93*
	T -		
- 		7.	
	SAME	2 Fr to	= 473.93
	at in the second second		

KOTAINTAN STATION

TERHINAL BLD

271

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ROOF 0.63 × 10.0 × 11.57
                                         72.89
      0.63 (1) 511+10,010/2 > 10,09 × 2
                                      = 130.18
PARAPET 0.355 (38.18 x2+10.01 x2)
                                        28.53
BEAH 0.64 (881+1037+10.0) x Z
                                       = 37.35
      6.64 (430+430+9.40+4.3+4.3) ×2= 34.05
                                          12.8D
       0.64 ×10.0 x Z
       0.55 × (9.79) × 2
                                       = (10,77)
                                         リンドに
COLUMN 10.864 × 3.0/2 (12)
                                      = 42.20
WALL 0.5 x2.2 (8.81+10.37) x2
        0.5x2.2 (4.30x2+9.4 +4.3x2)x2
```

444.84

117

```
0.78 (85+11.91)/2×1051x2
                                            167.32
          0.78 x 2.2 x 9.79 x 2
                                             33.60
                                             17.16
          0.78 x 2..2, x 10.0
                                             145.78
          0.69 x (10.01 + 11.57)/2 x9.79x2
SLAB
                                            = 31.79
          0.355 (10.51+8.5+10.76+10.0+5.0)x2
PARAPET. .
          37.35+34.05+12.80+10.77
                                             94.97
BEAM
          0.64 (4.4 +7.11+45122) 22
                                             26.28
           0.64 x 2.0 x 10
                                             12.8
           0.36 x(10.77+10.0+5.0) x2
                                             18.55
           0.5tx(9.79+10.21) x 2
                                             22.00
                                             15.55
COLUMN
           15.75
          0.864×3.5/2 (20)
                                             30.24
          0.5x2.8 (4.51+4.61+4.51+4.30x4
 WALL
            + 7.10 +8.81+ 89+t.0+t.5+50) x2=99.45
```

715.49

SIGNAL CABIN

TYPEA

2 Tr

ROOTI, 0215 x (8.0 x 9.0 x 1.17 = 40.75 BEAM 0.43 x (5.0 x 2 + 6.0 x 3) = 20.64 WALL, 0.49 x 3.85 /2 (6.0+) t.0 3x2 = 39.62 0.458 x 6.0 x 3.85/2 = 6.44 COLUMN 0.60 x 3.85 /2 x 4, = 4.62

112.07

I.T.

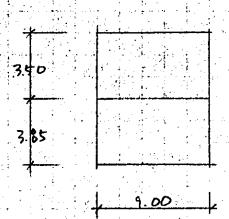
SLAB $0.69 \times 6.0 \times 15.0$ = 62.1BEAM $0.36 (150 + 15.0 + 4.0 \times 3)$ = 15.12COLUMN 4.62 = 4.62 $0.60 \times 4.5/2 \times 6$ = 8.10WALL 39.62 + 6.44 = 46.06 $0.50 \times 3.90/2 (30.0 + 4.0 \times 3)$ = 40.95

176.95

46. DESIGN OF STRESS.

TERMINAL BLD

DLINE



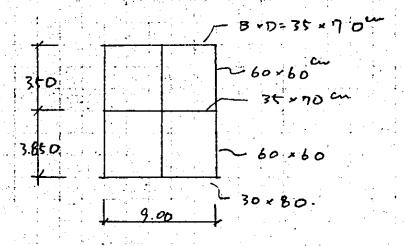
BEAM B . D = 40 . 70

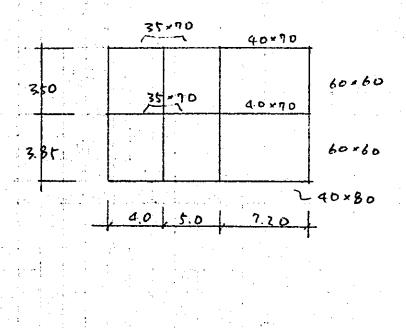
COLUMN BXD = 60 x60

BEAM B.D: 40 x70

COLUMN B = 60 + 60

FG 40 × 80



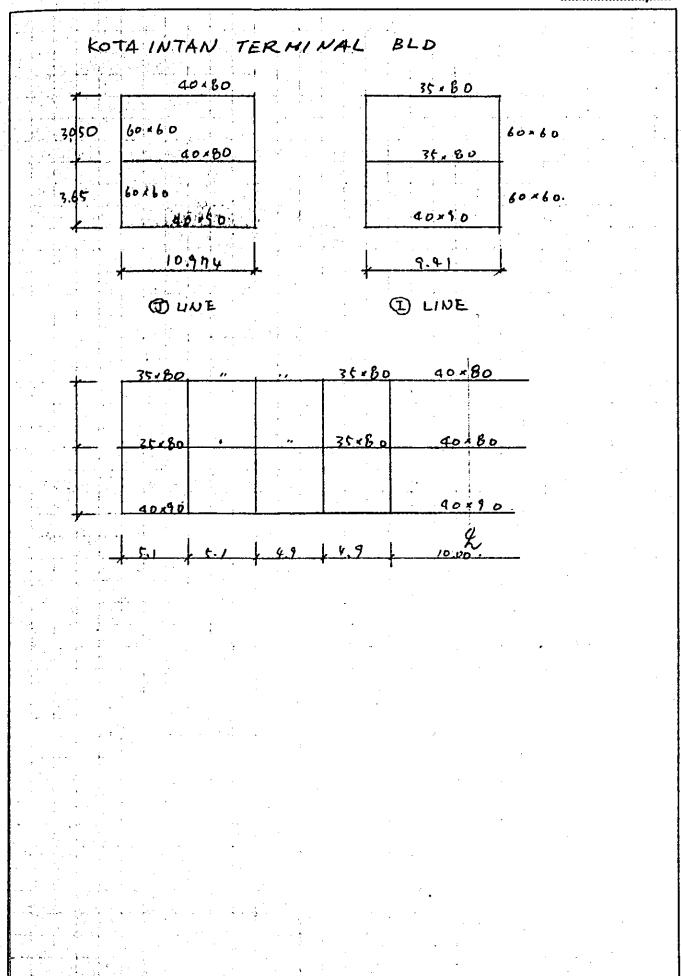


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		HIR PO		TERHIN	AL BLI	D TER	HINAL	BLD	4
		al a Art of the same of the same of	2 E	1	Φ	φ×I	Q	长	
	CBEAL	() 40 ;	יחט	11.43 × 10	2.0	22.87×1	05 9.0 7.2		' I
		35 4	20	16.60	7.0	70,00,11			44
,							4.0		00
٠		ere.					<i>ن.</i> د	4	.00
,	(F4)	4 0 x	80	17.07 ×10	1,0	ואף ס.רו	9.0	1.9	0
					1		7.2		³
		30. A	6 6 0	12:00	١,٥	12.80	4.5		84 .20
							5.0		16
						•			
	•		254			4.44 441			
	3	90 30	1		3	09			
		<u> </u>	2.74		-	444 44.4	<u>-</u>		,
	7	85 2.8	1		2	.61			
٠,	7.	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1.90	<u>, i</u>		2.84 2.84			
		·	9.00	<u> </u>	<u></u>	4.50 4.50	_ 	·.	
		· (4	DLINE	· ·		1 LINE	· .		
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		<u>, </u>	0	4.0 3.1	8				
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ı	3.	85 28	,						
ŀ	· · · · · · · · · · · · · · · · · · ·	1 .	.2	2.56 2.5	37				
		<u>k 4</u>	•	D 7.7	0		•		
		•	D L	INE	-	•			
									.
Ì	na tanàna ao ao ao amin'ny faritr'i Nordan- Nordan-Paritr'i Nordan-Paritr'i No	į							į

3454	78.95	v	70.57	ZN= 184.04 W = 184.04 × 0.1 = 18.40 t.
3414 7837 70.57 $\pm N^{2}$ (84.04 $\pm 18.40^{4}$ $\pm 19.40^{4}$ $\pm 19.40^{$				
	4) L	/ N E		
21,9	ን ୫5ኖ	3.0	78.55	
40.46	ጠኝባኈ	6.0	110.51	
	(G) L	WE		
	: 11 T			139.06 × 0.1 × 0.64 = 8.9 8.9
			- 20	
	-			473.93 20.1 20.64 = 30.33 69.56
	(D)L	NI.		

	Fig. 1	·		4.5	No		
		1					:
							•
	₽	ROOF	0.63	v 10.12	= 6	38	
30/1 AP		1		6.367	° 7.	· ·	-
		BEAH		5 Test ()	: 2:		
7.2/2		and the second second		t , 6.36			-
} 		WAL	०५५,	2.4 × 6.36		and the second second	:-
9.00					18	3.2,8	
						$\frac{1}{2}$ $\frac{1}$	-
formación de la composición de la comp La composición de la		W	W	C	14°	₽ ;	
(3,49)	1	•	1876	20.57	41.1	9,14	
	TERRALE	0.78		7.5	11.4	4.35	
		o.78		16.1	28.4	• • •	:
,	ne 814			3.71	5.57		•
	BEAH	0.75			4.5		
to the second of the second		0.36	2.0	2.75		1.0	
	•			50.1	91.0	25.3	
		į		,			:
		•					
(2nd)		ω	10	د	Ho	•	
	SLAB	. 0.69	••	6.6	10.1	388	
		0.69		14.2	25.2	7,3	
	BEAM	0.55		3.71	1-5	2.5	
	D Pri			7.25	4 5		. •
	•	0.36				1.0	
	•			26.8	45.3	147	
	en e		•	•			•
				0		•	
	(3rd)	ω	, · •	_ د	_ H. >	Q	
	ROUF	0.63	-	1.5	2.4	1.6	
	V.	•		1.5			
450	2 BEAM	0.55		o.		1.5	
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				7.	, •	7. 4	
•						·	
•	_		-				
	(zmd)	₩	্য	•	· Mo		
•	Roof	0.69		1.6	2.6	1.7	
				1.1	2.6	1.7	
a de la companya del companya de la companya del companya de la co	BEAM	0.55	- ——	0.9	1,4	1.2	 .
	- 	- -		4.7	: 6.6		
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		. :			Section 1997		•

			<i>.</i>		·	NU	
		(3/1)	$\boldsymbol{\omega}$	W	<u>c</u>	Ho	Ω
45/2/		RUOT	0.78		6.3	9.9	4.3
		BEAM	0.55		2.4	3.6	2.0
	7.20	PAPA.	0.755		1.5	2.3	1.3
1			**		10.2	15.8	ካ. 6
		(2md)	w .	100	د	to	ର
		56 4 B	0.69		4.6	8.8	3. B
		BEAM	0.55	· · · · · · · · · · · · · · · · · · ·	2.4	3.6	7.0
		WAL	0.61-1.7	, t/m	74	11.7	6,2
			į	•	15.4	23.4	11.9
							· · · · · · · · · · · · · · · · · · ·
			4				•
<u></u>		(311)	$\boldsymbol{\omega}$	W	د	Mo	ି କ
4.+/2 /		ROOF	0.78		2.5	4.0	2.4
	5.0	BEAM	0.55		1.1	1.7	1.4
* -		PARA	0.355		ባ. ሃ	1,1	09
					4.4	68	67
			'				
			٠		• • •		
		(2md)	ω	-	ر	Ho	C
		SLAB	0.69	. •	2.2	3.5	2.,
		BEAM	0.55		1.1.	1.7	1.4
		WALL	1.71_		3.5	5-1	4.3
					6.9	10.6	ካ.ሄ
	•	•			•		
6							



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- 61		ī	4	Q x Z	l	h.
(BEAM)	vox Bo	17,07 2105	2.0	34.1	10.97	3.11
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		14.9 3 "05		-0.4	4 4 1	3,17
	35 x 80	11443	7.0	24.6	(+T)	•
			.1.5	22.4	5-1	4.39
					4.9	4.57
· .	. San Line and		: :	•	•	**************************************
1	40×90	74.3 ×105	1.0	24.3	10.57	2.22
	-		•		9.4)	2.58
					5.1	4.76
				:	4.9	4.96
•	•		•		10.0	2 43
	100	•	-			- 73

(COLUMN) 60,60	T= (0.8	R=3.05	k=3.54
	en e	3.65	2.96

				Na
	3/11		₹. iŋ	
			7. 1	
3.05	1 [3.\$		
	3.11	<u> </u>	3.10	
3.65 .2.	16	29	6 : : :	• • • • • • • • • • • • • • • • • • • •
	32.32		258	
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	10.979	, [.	9.41	
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	4.39 4.39	457 45	7 2.56	
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	3.54			oten er en
305	1			e. ·
<u> </u>	4.39 4.39	457 45	7 2.16	
				•
3.65	2.96.			
	4.76 4.76	496 4.51	2.43	
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			4	
	4.0 5.0	490 490	10.00	
	+ 5.10 × 5.10	k. Tadda-fan-in-	4	· · · · · · · · · · · · · · · · · · ·
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	CKT)					
P		ິພ່	w	·ر	440	0
	(ROOF)	0.63	1.73	14,4	21.6	8.65
2,75		0 63	3.94	6 Q	9.9	and the second s
	PARA			3.0		1.8
10.0	BEAM		1.		8.0	the state of the s
**************************************	DEAM	0.64			8.0	
		0.64			51.9	
				5 (± 6 .)		17.2
					3 - 5 - 6 - 10 - 10 - 10 - 10 - 10 - 10 - 10	
And the second second						C
	(2F)	W	M	د.	Ho	
	•	0.64		5.3	8.0	3.2
	WALL	1.13 */-	-	٩.५		5:65
		. • •		14.8	3 2 . 1	8.85
		•		· · · · · · · · · · · · · · · · · · ·		
				.,		
	CRAI	w	M		No	Σ /
	. ROOF	0.63		2.2	3.5	2.1
	BEAM	0.64		1.14	2.1.	1.6
5.10	PAR4	0.355	1	0.8	1,2	0.1
			٠	4.4	6.0	4.6
	. •					
	; . ;		•	: · = · · · · ·		4 - 4 - 4
and the second of the second o	31.	W	Ŵ	ر	No	Q
	(ZA) SLAS	5 0.69		2.4	3.8	7.7
		1 0.64		1.5	2.2	١. 8
		- 1 <u>-13</u>		2.4	3.1	2,9
	• -					,
	÷			6.3	9.7	6.9
	;		•	6.3	9.7	6.9
	; •			6.3	9.7	6.9
	; •			6.3	9.7	6.9
	; 			6-3	9.7	6.9
	; • • • • • • • • • • • • • • • • • • •			6-3	9.7	69
				6.3	9.7	6.9
				6-3	9.7	69
				6.3	9.7	6.9
				6.3	9.7	69
				6.3	9.7	6.9
				6.3	9.7	69
				6.3	9.7	6.9
				6.3	9.7	6.9

		And the second second				No.	<u> </u>	
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9 3					•			
					•			
		CR FT)	W	70	. <u>.</u>	Mo	Q	
,				ν,	(0.4	17.4	5.4	
5.0/	,人人	ROOF.					\$.7 \$.2	
+	- - X -		0.63		10.6		3.0	٠
53/	i Y Y	BEAM			4.7	7.1		
7			0.36	1.8	2.1	4,2		
	فالمعالم المراجع	<u>k</u> .			279	46.5	14.9	
ا في المحمد الم				÷ .				
		(ZF)	w	or	_	Mo.	, O	
		SLAB	0.69		\mathfrak{u}, Ψ	19.1	6.0	
		12	0.69		11.6	19.5	6.0).
		BFAM		1	4. J	7.1	3,0	>
			0.36		2.1			
			·	, ,	29.9			
				100	- 1 - 1			
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+	-	_ ckti)	w	TÚ)	· •	Ho	Q	٠.
5.97	'	1 '		•		51.9	17.2	
* *		ROUF	0.63		31.6	19. z	18	
4.1	12 YLY	<u> </u>	0 63		11.7	·		
					43.3	71.2	73.0	
	10.0	+		· ·		-	٠.	٠
	:						C	
*		(24)	W	49	, C	Ha	2	٠
4.9	, Y Y	SLAB	0.63		11.7	19.3"	5.8	
<u> </u>		BEAM	0.64	v	4.3	8.0	3.2	
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37 DESIGN OF	SUB R	FAM	(RC)	* ************************************		•
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P	P :	3.01	(WATER	TANK)	· · · · · · · · · · · · · · · · · · ·
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	BEAM	0.44	3.e 7.5	2.2	1. 12.	-
	TANK	7 <u>=30</u> 7	9.9	4.8	2.7	
	0.60=0	6+9.	9 = 5.94	tm.		
H0-0.4C-						
	<i>B</i> ,	D + 35	* 60	d = 4	5 cm 6.375 cm	• · · · · · · · · · · · · · · · · ·
(END)	at so	94/		=	cunz	
		(2 x	.46375))		
(CENTER)	at-	1234	(2.067)	= 13.	30 am	•
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		3./	
		Con	0
	1.6C= 2.2Z	B > D = 30 x50	o d= 43 en
			d = 31.625

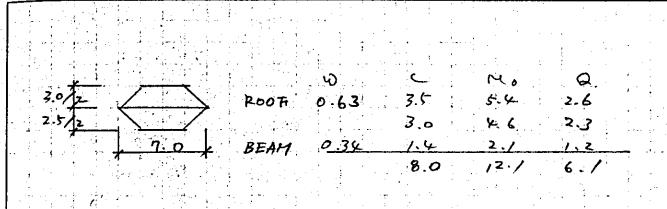
(CENTER)

(ALL SECTION)

DA = 37.62 (x 30 x 1.0)

7.9 t

STP D10A@200



10.6c. 4.8+ m

Mo-0.4C= 89

B x D=30x60 d=53 cm d=46.375

(END) at - 480/ (2×46.375) = 5.18 cm=

(CENTER)
at: 890/ 20,46375)

(END) (CENTER)

OA- 9.7

STP P100@200

$$P = 0.63 \times 5.0 \times 5.0 = 15.75 \text{ T}$$
 $BEAMPI = .0.643 \times (5.045.0) = 6.437$

22.18

J = 63.875

(CENTER)

at - 2750/(2×63.875)

STP. DIDD @ 200



TERRAS 078

2.54 40

2. U

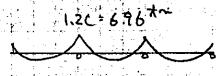
2.14.4

2 4

BEAM 0.34

0.71 1.1

09



Mo-0.4C- 6.88

B = D= 30 -60

d = 46.375

CEND) at

at = 696/2 ×46.375 = 7.50 cm²

(CENTER) at = 688/2x46.375 = 7.42

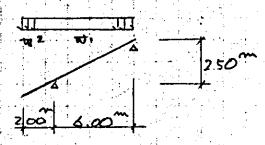
QA=9.7

Bor

003-022 pog 3-022 0003-022 pog 5-022

STP DIOD@ 200

38 BEAM O



Q' = 6.00 x 1.083 = 6.5 m 2/2=2.0 × 1.003 = 2.17 m

LOA D

DEAD LOAD 130 kg/m

LIVE LOAD 50 Fa/m2

COMBINE LOAD

130+50 = 180 kg/m2.

BEAM PICH @ 1.000

w/= 0.18 */m2 x 1.0 = 0.18 */m

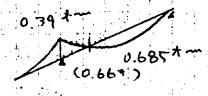
 $W_1 = 0.18 \times 6.5 = 1.17^{+}$ $W_2 = 0.18 \times 2.17 = 0.39^{+}$

LONG SPAN

C = 1.17 x 6.0 /12 + 0.585 * m Mo= . /8 = 0.88 *~ Q = 1.17/2 = 0.59 + Q = 1.17/2

CANTI BEAM.

C= 0.39 x 2.0 /2 = 0.39 * 0 = 0.39 t



uas - \$ 165.2 x.4.0

PEFLECTION

$$\Delta = \frac{5WL^3}{386.EI} = \frac{5(1.17)(600)^3}{384(2100)(668)} = 2.38cm$$

$$\frac{\Delta}{\ell} = \frac{1}{2\tau_1} < \frac{1}{2\tau_0}$$
 or

CANTI BEAM DEFLECTON

$$\Delta = \frac{\varpi L^3}{8EI} = \frac{0.39(200)^3}{8(2100)(658)} = 0.28 \text{ cm}$$

COMBINED LOAD 110 F8/m2

W . O.11 * x 3.33 = 0.37 */m

H mox = 0.37 x 4132/8 = 0.86 +m

Q = 0.37 x4.3/2 = 0.80 \$

USE - H-175 x 90 x 5 v 8 16=2.38

ln=430/2=215 cm

>= lo/ib= 215/2.38 = 90.3

fb= 1.6 t/cm2

Co/fb = 86/134 × 1.6 = 0.39 < 1.0

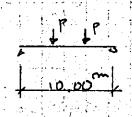
PEFLECTION

\$ = 5 LO.37)(430) 4 (10-2)

384 ×2100 × 1210

= 0.65 cm

8/2= 1/663 c1/250 OK



$$\Delta = \frac{73PL^3}{648EL} + \frac{+WL4}{384EI}$$

$$= \frac{23(0.32)(1000)^3}{648(2100)(3540)} + \frac{5(0.03)(1000)^4(10^2)}{384(2100)(3540)}$$

$$\frac{\Delta}{L} = \frac{1}{48\eta} \quad \frac{1}{250}$$

\$ 9

DESIGN OF FOUNDATION .

USEPILE \$350 (P.C PILE)

BEARING CAPACITY OF PILE

$$Ra + \frac{30}{3} \times N \times AP$$

$$= \frac{30}{3} \times 50 \times 0.0962$$

= 48.08+

. 16.5t - 46.0t

DESIGN OF FOOTING

2 PILES

P= 46.0 t M= 46.0 x 0.15 = 6.9 tm Q= 46.0 x

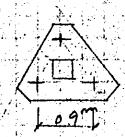
D=80 d=90 j=61.25

at = 690/2 ×61.7+ = 5.63

BAR 5-019

M/ad = 0.21

QA= 38.6 x2= 77.2 T



3 PILES

BOZ 5-D19

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AIR PORT TERMINAL STATION

LY* 2.50 LY* 7.00m PAMDA* 2.00 u UAL: 2. 7.71 t (cm) =13.00cm D1 (cm) *10.00cm J1 Tmin =10.84cm D2 (cm) *9.00cm J1 MX1* 7.717 * 0.072 * 0.609 tm MY2* 7.717 * 0.052 * 0.401 tm MY2* 7.717 * 0.052 * 0.401 tm MY3* 7.717 * 0.052 * 0.10 tm O - 2.205 * 0.570 * 1.140 tm CX: U-BAP	0. w(t/m2)= 0.63t/m2 L/= 4.00 LY= 7.00m RAbiDA= 1.75 w;t/m2)= 0.63t/m2 U*L/= 2.52 U*LX^2= 10.08 U*D U*	n ate 2.293cm2 Mile 10.080 + 0.075 = 0.761 tm ste 4.348cm2 n ate 2.293cm2 Mile 10.080 + 0.050 = 0.504 tm ste 2.880cm2 m ate 2.058cm2 Myle 10.080 + 0.042 = 0.423 tm ste 2.688cm2 m ate 1.372cm2 Myle 10.080 + 0.048 = 0.282 tm ste 1.792cm2 n ate 1.372cm2 O = 2.520 + 0.350 = 1.310 t	(Y) U-BAR 3 D-BAR 3 D-BAR 3		1 w(t/m2) = 0.63t/m2	tm
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	÷		/m2)* 0.69t/m2 8.750cm 7.975cm	M 14 M	; ,	(G) (
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		-	4.70m Rabiba 1. U+LY2 11.59 1D1(cm) = 10.00cm	86.93	3.00	
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<u> 3</u>	7		50m FAMDA: 1.2 X12: 15.79 D1(cm):12.00cm D2(cm):11.00cm	, 62, 63, 64, 64, 64, 64, 64, 64, 64, 64, 64, 64	1.8.5	(Y) U-BAR
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	KOTAINTAN STATION	1	LX* 4.50 LY* 5.50n FAMDA* 1.22 U*LX* 3.51 U*LX*2* 15.79 	MX1= 15.795 + 0.058 = 0.930 tm MX2= 15.795 + 0.039 = 0.626 tm MY1= 15.795 + 0.042 = 0.663 tm	ÇW.	A S
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		J	X	MX1**	, .	(X) U-BAR
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1 20	. 00cm	0.930	0.620	0.663	0.442	.825	(Y) U-BAR	
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V* 5.50% FAMDA	D1 (cm) *12, 00cm	* 0.058	. 620.0	0.042	. 028	0.320		
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PLANE FRANE ANALYSIS

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AIF TEFMINAL STATION

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1984, 1.10

DATE

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DENICO - A3 Ver 4.02

by Tokyo Denco Co., Ltd.

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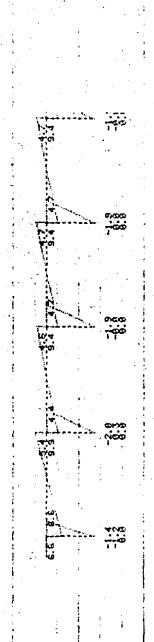
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		•	7	0.0000		-12,56000		
			m	0.0000	-15.70000	0.0000		
			•	00000	-15.70000	0.00000		
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			9	0.00000	15,70000-	00000		
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F-X(t)	1.42000	1.42000	1: 42000	1.42000	1.42000	1.42000	1.42000	1.42000	1.42000.	1.42000	1.42000	
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PLANE FPANE ANALYSIS

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by Tokyo Denco Co., Ltd

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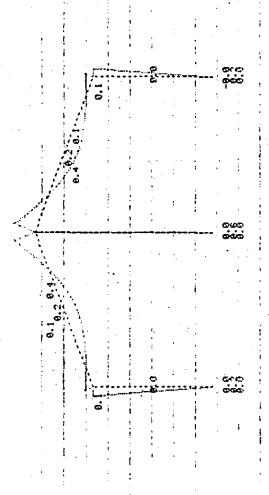
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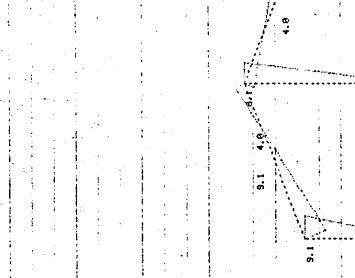


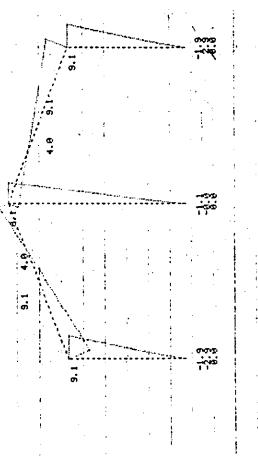
* PLANE FPAME ANALYSIS LOAD NO 2

.... MONENT

SCALE 17 100

1. ...





FLANE FRAME ANALYSIS

TITLE AIR TEPMINAL STATION TERMINAL BLD

naTE 1984, 1, 10

DENCO - A3 Ver 4.02 by Tokyo Penco Co. Ltd.

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PLAVE FPAME ANALYSIS LOAD NO 2

MONENT

SCALE 1/ 200

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PLANE FRAME ANALYSIS ...

TITLE KOTAINTAN PLAT

DATE 1984, 1,10

.... DENCO - A3 Ver 4,02 --

..... by Tokye Dence Co. Ltd.

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7 - COMPINATION DATA TABLE

- C'NO L'INO VALVE

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/L E-4* (A/A/) 1	4,350E+001 3,652E+001 2,241E+001 5,67EE+001			AXIAL-31ti	0,170				•		A. IAL-J(t)	0, 000 0, 000				
E+1+(1/10)/L	5.096E+004 4.276E+004 3.454E+004 B.752E+004			AVIAL-1(1) 6	-0.170	· · · · ·					AYIAL-1(1) A	0.000				
k (cm+3)	24.26 20.37 16.44 41.67		(d)	SHEAP-3(1)	-0.340		1 1 (W)	-0.62825 0.53023 0.09802		ER) *****	SHEAR-J(t)	0.547		· (1)	1026 1791 1234	
e -	1,000 1,000 0,193 0,184		ENT JOINT LOAD (MEMBEP)	BEND-J(tm)SHEAR-1(t)	0.860 0.340	LOAD (JOINT)	M-Z(tm)			ENT JOINT LOAD (MEMBER)	BEND-J(tm)SHEAP-I(t)	-0.5147	LOAD (JOINT)	M-Z(tm)	0.40026 -0.33791 -0.06234	
800	0,000 -0,000 0,991 -0,982	1	JALENT JOINT		0.098	JALENT JOINT	F-Y(t)	-0,87664 -1,22261 -0,34597	2,44522	ALENT JOINT		-0.400	EQUIVALENT JOINT LOAD	F-Y(t)	0.53750 0.75000 0.21250	1,50000
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JOINT DISFLACEMENT	0.0000000	-0.000000 -0.0017008 -0.0046691		(t)	H26 0,739E6	0000 2.44522	MENPER FORCE - (NEMPER AXIS)	(ENTEP(tm)·BEND-3(tm)	0.117 0.234 0.472 0.822 -0.147 -0.000	•	JOINT DISPLACEMENT	D15P-Y(cm)	0.000000 0.000000 0.0002551 0.0038029	0,0655496	SUPPORT	t) (+) / /	\$070 -0.11099 \$070 -1.38900	249 -1.50000	
LOND CASE NO 1	0.0000000	0.0891683 0.0896782 0.0896782	LOAD CASE NO 1 SUFFOR		2 -0.04426	0.0000		- JmBEND-I (tm) C	3 6.000 4 0.000 4 0.10c		LOAD CASE NO 2	DISP-X (cm)	0.0000000 0.0000000 -0.4643946 -0.46439307	-0.4772535	ON HO		2 0.15070	0.29249	
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***** . LOAD CASE-NO 2 . MEMBER FORCE (NEMPER AXIS) ***

ATAL-11+1	0.110	00:7	-0. 117	0.000
A) IAL-1(1)	-0.110	-1.389	6.117	-0.000
BEND-J(tm)SHEAP-1(t) SHEAR-J(t) A)JAL-1(t) AVJAL-J)+	0.141	0,150	-0.059	-0.000
(1) 1-dv3HS	-0.141	-0.150	-0.136	-0.432
BEND-J'tm	-0.630	-0.798	1.172	000.0
CENTER 1 tm	-0.315	-0.100	-0.329	2000
Im - Jm BEND-1(tm) CENTER(tm)				-0.374
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			***	ROTATION (rad)	0.000000	0.0003016	0,0001883		H-Z(tm)	0,00000	
	VALUE	00.1	JOINT DISPLACEMENT	DISP-Y(cm)	0.0000000	-0.0014456	0.0346029	SUPPORT REACTION	F-Y(t)	0.62887 0.31635	0, 94522
***** MIX CASE NO	C. NO L. NO		MIX CASE NO 1. JOINT	DISP-X(cm)	0.000000	-0.3752262	-0, 3757524 -0, 3827436	MIX CASE NO :-1 SUPPOR	J. NO F-X(t)	2 0.10443	0.29249
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	AXIAL-311	-0.628	-0. HIS	0000	,
	AXIAL-1(1)	0.628	0,316	0.127	
**************************************	M.NO Im - Jm BEND-I(tm) CENTER(tm) BEND-3(tm)SHEAR-I(t) . SHEAR-3(t) AXIAL-1(t) AXIAL-3(t)	-0.1860.186	0.100	0.000	- !
MEMBER AX19)	(tm) SHEAR-1		90.00	0.000	
ER FORCE (tm) BEND-3	-0.827	1	•	
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11X CASE -NO.	I-GNO-I	000 0	0.000	0.214	
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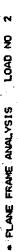
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- PLANE FRANE ANALYSIS MOMENT

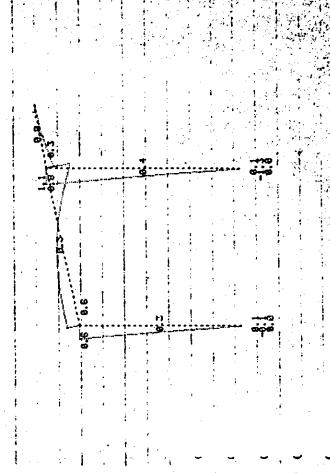
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• PLANE FRANE ANALYSI MONENT SCALE 1/		7	<u>දෙන්</u> අත්ත
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PLANE FRANC ANALYBIS	KOTAINTAN PLAT IND	SIGN	DENCO - A3 Ver 4.02 by Tokyo Denco CoLtd.	

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			Mirad/tm. rad/10) 0.0000 0.0000 0.0000 0.0000 0.0000	(cm*2) (cm*4) 92,18 3650,00 -46,78 7210,00
12 6 6 7 11 11 CASE 1 DATA 2	1 0 0 (w)	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000	Git/cmiz) - A(cmiz) 810.0 92.18 810.0 46.78
NAMEE OF JOINTS NAMEER OF SUPPORTS NAMEER OF MATERIAL NAMEER OF LOAD CASE NAMEER OF LOAD DATA NAMEER OF MIX LOAD CA NAMEER OF MIX LOAD CA NAMEER OF MIX LOAD CA	COTPUT OPTION SHEAF DEFORMATION COMPINATION OPTION COOPDINATE OF JOINTS	10.000 2 20.000 3 30.000 5 40.000 6 40.000 7 6.000 7 7 7.000 8 7.000 1 7.000 1 7.000 1 7.000	0F 301NT	MATERIAL TABLE 2100.0 2100.0
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* ***** - - PPOPERTIES OF NEWERPS - - *****

E-A+1A/A51 "	5.652E+001 5:652E+001	4,652E+002 4,652E+002 4,652E+003 9,852E+003 9,813E+001 9,813E+001	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	(* * * * * * * * * * * * * * * * * * *	୍ର ବ ବ ବ
E+1+(1/19)/L		446E+004 446E+004 5146E+004 514E+004 514E+004 514E+004		AVIAL-ICO AXIAL-JOS	0,000 0,000 0,000 0,000 0,000 0,000 0,000 0,000 0,000
· K (cm*3) E	6.88	6.88 6.88 7.7.7 7.21	7.21	R) ***** SHEAR-J(t)	-2.350 -2.350 -2.350 -2.350 -2.350 -2.350 -2.350 -2.350 0000 00000 00000 00000 00000 00000 0000
COS.		0.000 0.000 0.000 1.000 1.000 0.000 0.000		EQUIVALENT JOINT LOAD (NEMBER) CENTER(tm) BEND-J(tm)SHEAR-1(t) 8	875 3.916 2.350 875 3.916 2.350 875 3.916 2.350 875 3.916 2.350 875 3.916 2.350 875 3.916 2.350 877 3.916 2.350 877 3.916 2.350 877 3.916 2.350 62.3500 64.7000 64.7000 64.7000 64.7000 64.7000 64.7000 64.7000 62.35000
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	F-X(1)	0.47000	0.47000	0.47000	0.47000	0.47000	0.47000	2, 87000
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						N -1(4) Ast& -1(4)	10.7 44 64 10.0 10.0 10.0	646 0466 15.00 10.00	
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JOINT DISPLACEMENT	0,000000	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	-0.012/218 -0.012/218 -0.0138164 -0.0056321	ג קד ה	5.04632 7.4,64636 7.4,64636 7.4,64636 7.64632 7.05711 7.05711	MEMBER FORCE (MEMBER FORCE (MEMBER FORCE)	0.334	0.036 0.073 0.167 0.334 0.865 1.730 2.679 -4.659 1.816 -3.791 2.010 -3.864 1.816 -4.325 2.679 1.730	
LOAD CASE-NO -1	00000000	0,000000 0,0000000 0,0000000 0,0074177 0,0074177	1116 1116 1771	-X:E	2 -0.032634 2 -0.06303 3 -0.01387 4 -0.01387 5 -0.04305 5 -0.32654	LOAD CASE NO 1	0,000	10 0,000 0,01 11 0,000 0,1 12 0,000 0,1 12 0,000 0,1 13 864 2,0 10 3,864 2,0 11 3,791 1,18 12 4,659 2,0	
2.7	₩ ()	3 + 10 + 0 F cc	:	· · · · · · · · · · · · · · · · · · ·	ማርዛ ነት ነጋ ሲ	# 1 # C	0 m	48.47.40.51 48.48.48.51	

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	D15P-Y(cm)	0.000000	0.000000	0.000000	0,000000	0.000000	0.000000	£666000 0 ·····	-0.0003259	0.0000722	-0.0000732	0.0003259	-0.0009a93		-SUPPORT_REACTION	F~Y(t)	-0, 3e500	0.11905		0.02574	-0.11905	0.36500	0.00000
•	DISP-X(Cm)	0.000000	0.0000000	0.000000	0.0000000	0.000000	0.000000	4.0190297	4.0182187	4.0179365	4.0179365	4,0182187	4.0100207		C)	Faxity	-0.39032	-0.52195	-0.49778	10.40111	10.02198	-0.39082	-2, B2000
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317-7eExp	392.0	-0.113	0.026	-0.036	0.119	-0.365	-0.00-	-0.02	0.000	0.027	0.010	
AX14(-1(t)	-0.365	0.110	-0.020	0.020	-0.110	0.365	0.079	0.027	000.0	-0.027	-0.023	
SHEAR-JCt	-04.390	155.0-	267.0-	10 7.0-	-0.521	-0.330	0.365	0.245	0.272	0.243	0.505	
CENTER (4m) BEND-J (4m) SHEAR-I (4)	0.390	0.521	264.0	0.497	0.521	0,390	-0.365	-0.245	-0.272	-0.245	-0.365	
BEND-Jetm	2.06B	2.766	2, 637	2.637	2.760	2.068	-1.581	-1.274	-1.363	-1.185	-2.068	
CENTERITE	1,034	1.393	1.318	1.318	1.383	7,0:-	0.243	-0.044	0.000	0.044	-0.243	
REND-1 (***)	0.000	0.000	000.0	0.00	0000	0,000	-2.058	-1.185	-1.363	-1.274	-1.581	
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MEMBER FORCE (MEMBER AXIS)

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			·	AXIAL-1(t) AXIAL-1(t)	1.692 - 1.692 5.165 - 5.165 4.619 - 4.619 4.927 - 4.927 2.427 - 2.427 0.400 - 0.400 0.201 - 0.201
	***** ROTATION (r&d)	0.0000000 0.0000000 0.0000000 0.0000000 0.000000	M-Z(tm) 0.00000 0.00000 0.00000 0.00000 0.00000	**************************************	0.063 -0.063 1. 0.585 -0.585 5. 0.483 -0.483 4. 0.511 -0.511 4. 0.458 -0.458 4. 0.716 -0.716 2. 1.692 3.007 0. 2.157 2.542 0.
ASE NO 1 L.NO VALUE 1 1.00 2 1.00	NT DISPLACEMENT DISP-Y(cm)	0.0000000 0.0000000 0.0000000 0.0000000 0.000000	SUPPORT REACTION F-Y(t) B 1.69210 5.16538 4 4.67330 9 4.92726 5 2.42211	1 1	0.338 3.100 2.238 2.711 3.240 5.240
C. NO L. NO	X CASE NO 1 JOINT DISF-X(cm)	0.000000 0.000000 0.000000 0.000000 0.000000	3.NO F-X(t) 3.NO F-X(t) 2 -0.585.0 3 -0.48384 4 -0.51159 5 -0.71686	MIX CASE NO 1 MEMBER FEIND-I(tm) CENTER(tm)	0.000 0.000 0.000 0.000 0.000 1.282 0.000 1.216 0.000 1.216 0.000 1.216 3.140 1.772
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AXIAL-1(t) A(1AL	
EAR-1(t) SHEAR-J(t) 2.077 - 2.622 2.050 2.649 2.277 2.422	
CENTER (tm) BEND-J(tm) SHEAR-1(t) 2.010 -5.227 2.037 -1.861 -5.510 2.050 2.436 -3.799 2.436	
2.501 2.910 2.910 2.916 3.079 2.556	
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SCALE 1/ 200



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7 COMBINATION DATA TABLE

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	E+A+(A/An)	3, 651E+002 1, 335E+003 6, 452E+003	5. 241E+002 3. 457E+003 3. 457E+003		AL - 3 (t)	-0.192 -0.170 0.370						AKINE-1(t)	0,000 0,000 0,000			
:	E+1+(1/10)/L	4.279E+004 1.364E+005 2.560E+004	9,955E+004 5,955E+004 6,035E+004		AYIM-ICO AKIM	0.192 -0	,		·			AKTAL-TOD AKT	0.000 0.000 0.000			
	n (ca*3)	000 74.48	184 47,46 193 18,73 197 28,74	LOAD (NEWBER)	(tm) SHEAR-I(t) SHEAR-J(t)	1,020 -1,020 0,860 -0,860 1,680 -1,680	רטשט (למנאד)	M-Z (tm)	-0.29407 -0.33418 -0.62825 -0.70080		LOAD (MEMBER)	tm) SHEAR-1(t) SHEAR-1(t)	-0.216 0.215 -0.547 0.547 0.107 -0.107	LOAD (JOINT) *****	0.06224 0.33791	* · · · · · · · · · · · · · · · · · · ·
	•	0.000	000	EGUITVALENT JOINT LO	CENTEP(tm) BEND-J(tm)	0.294 0.628 0.799	EQUIUM ENT. JOINT LA	F-Y(t)	-1,03791 -1,91455 -0,87644 -1,71386 -1,71386	-7,25683	EGUTVALENT JOINT LO	BEND-3(-0.062 -0.400 0.050	Ę	0.21250	
		5.300 1.450	1,729 4,383 2,856	· ·	ta)	0.441		F-Y(1)	0,00000 0,00000 0,00000 0,00000 0,00000	0,00000	2 EQUIV	(tm) CENTER(tm)	-0.093 -0.600 0.07e	r Equiv	6.04000 0.14675	A the second sec
o section	NO THE JA	- 77 E	m 4 0	LOAD CASE NO	Jm BEND-I(tm)	5 -0.294 5 -0.628 7 -0.799	LOAD CASE NO	ev.t.	M4001	· · · · · · · · · · · · · · · · · · ·	LOAD CASE NO	Jm BEND-I(tm)	4 0.062 5 0.400 7 0.050	LOAD CASE NO	0. N	
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CENTER(tm) BEND-J(tm)SHEAR-I(t)	0.722	-0.387	-0.429	0.000	-0.986	3, 574
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SCALE 1/ 75

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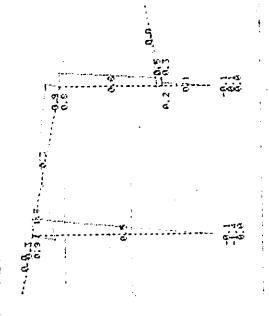
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STALE 1/ 75



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