

REPUBLIC OF INDONESIA

MINISTRY OF COMMUNICATIONS  
DIRECTORATE GENERAL OF LAND TRANSPORT  
AND INLAND WATERWAYS

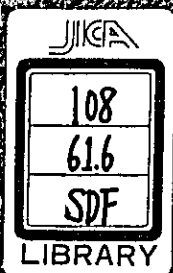
TENDER DOCUMENTS  
FOR  
NEW RAILWAY LINE FOR CENGKARENG AIRPORT  
CONSTRUCTION PROJECT

STRUCTURAL CALCULATION SHEETS

PACKAGE I CIVIL AND ARCHITECTURAL WORK

11 of 11

AUGUST 1984



JAPAN INTERNATIONAL COOPERATION AGENCY  
(JICA)



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国際協力事業団	
受入 月日 '84.11.19	108
登録No. 10883	616
	SDP

マイクロ  
フィルム作成

STRUCTURAL CALCULATION SHEETS  
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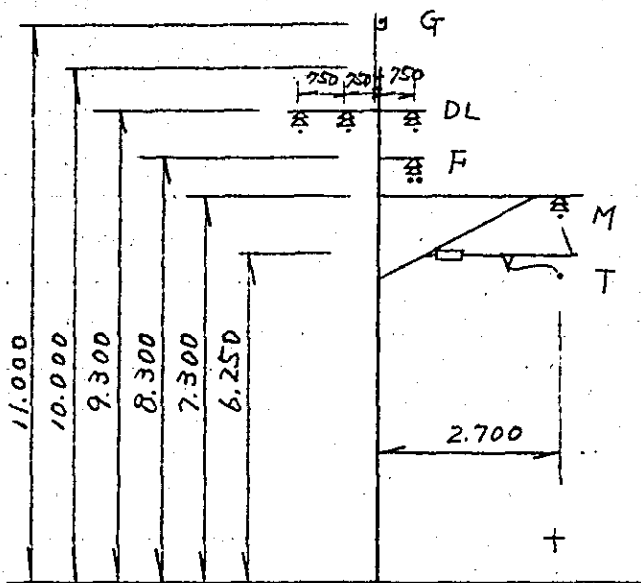
§§17. SUPPORTING STRUCTURE

FON

OVERHEAD CONTACT SYSTEM

# Moment Calculation of General Supporting Structure Between Stations

## Standard Supporting Structure



G: Ground Wire St 55 mm<sup>2</sup>

DL: Distribution Line  
OE-Cu 38 mm<sup>2</sup>

F: Feeder Wire Cu 300 mm<sup>2</sup>

M: Messenger Wire St 90 mm<sup>2</sup>

T: Trolley Wire Cu 110 mm<sup>2</sup>

### 1. Vertical Load

Item	Wire Classification	Unit Weight (kgf/m)	SPAN		
			60 M	50 M	40 M
Feeder Wire	Cu 300 mm <sup>2</sup> x 2	2.715	325.8	271.5	217.2
Messenger Wire	St 90 mm <sup>2</sup>	0.697	41.8	34.9	27.9
Trolley Wire	Cu 110 mm <sup>2</sup>	0.9877	59.3	49.4	39.5
Distribution Line	OE-Cu 38 mm <sup>2</sup> x 3	0.405	72.9	60.8	48.6
Ground Wire	St 55 mm <sup>2</sup>	0.446	26.8	22.3	17.8
Rigid Contilever			70		
Total Weight (kgf)			596.6	508.9	421.0

## 2. Horizontal Load

## 2-1 Wind Load

Item	Wire Classification	Unit Wind Load ( $\frac{\text{kgf}}{\text{m}}$ )	SPAN		
			60 M	50 M	40 M
Feeder Wire	Cu 300 $\text{mm}^2 \times 2$	$0.5625 \times 1.2$	40.5	33.8	27.0
Messenger Wire	St 90 $\text{mm}^2$	0.3	18.0	15.0	12.0
Trolley Wire	Cu 110 $\text{mm}^2$	0.3085	18.5	15.4	12.3
Distribution Line	OE-Cu 38 $\text{mm}^2 \times 3$	$0.295 \times 3$	53.1	44.3	35.4
Ground Wire	St 55 $\text{mm}^2$	0.24	14.4	12.0	9.6
Concrete Pole = $\frac{0.35^3 \times 20 \text{ kgf}}{7 \text{ kgf}} \times 10^3 = 70$			70.0	70.0	70.0
Total Wind Load (kgf)			212.4	188.4	164.2

2-2 Tension ( Temperature 20°C  
Velocity of the Wind 20 m/sec )

Item	Wire Classification	Standard Tension (kgf)	SPAN		
			60 M	50 M	40 M
Feeder Wire	Cu 300 $\text{mm}^2$	1,200	1,320	1,360	1,430
Messenger Wire	St 90 $\text{mm}^2$	1,000	1,120	1,140	1,160
Trolley Wire	Cu 110 $\text{mm}^2$	900	900	900	900
Distribution Line	OE-Cu 38 $\text{mm}^2$	200	220	230	250
Ground Wire	St 55 $\text{mm}^2$	300	370	380	400

## 2-3 Cross Tension Load

$$C = \frac{ST}{R}$$

C: Cross Tension (kgf)

S: Span (M)

T: Tension (kgf)

R: Truck Curve Radius (M)

Item	Wire Classification	R=1.000		R=500
		60 M	50 M	40 M
Feeder Wire	Cu 300 mm <sup>2</sup> x 2	158.4	136.0	228.8
Messenger Wire	St 90 mm <sup>2</sup>	67.2	57.0	92.8
Trolley Wire	Cu 110 mm <sup>2</sup>	54.0	45.0	72.0
Distribution Line	OE-Cu 38 mm <sup>2</sup> x 3	39.6	34.5	60.0
Ground Wire	St 55 mm <sup>2</sup>	22.2	19.0	32.0
Total Cross Tension (kgf)		341.4	291.5	485.6
Total Wind Load (kgf)		212.4	188.4	164.2
Total Horizontal Load (kgf)		553.8	479.9	649.8

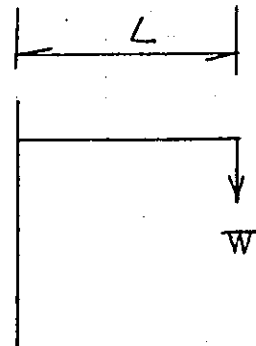
### 3. Moment

#### 3-1. By Weight

$$M_1 = WL$$

W: Weight (kgf)

L: Length (M)

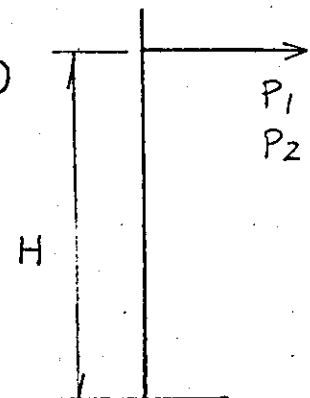


#### 3-2 By Wind pressure Load

$$M_2 = P_1 H$$

P<sub>1</sub>: Wind pressure Load (kgf)

H: Height (M)



#### 3-3 By Cross Tension

$$M_3 = P_2 \cdot H$$

P<sub>2</sub>: Cross Tension (kgf)

## 3-4 Total Moment

$$M = M_1 + M_2 + M_3 \quad (\text{kgf} - \text{M})$$

## 3-1 By Weight

Item	Wire Classification	Length (M)	SPAN		
			60 m	50 m	40 m
Feeder Wire	Cu 300 mm <sup>2</sup> x 2	0.75	244.4	203.6	162.9
Messenger Wire	St 90 mm <sup>2</sup>	2.7	112.9	94.2	75.3
Trolley Wire	Cu 110 mm <sup>2</sup>	2.7	160.1	133.4	106.7
Distribution Line	OE-Cu 38 mm <sup>2</sup>	1.5	-36.5	-30.9	-24.3
Ground Wire	St 55 mm <sup>2</sup>	0	0	0	0
Rigid Cantilever		2.7/2	94.5	94.5	94.5
Total	M <sub>1</sub> (kgf-M)		575.4	495.3	415.1

## 3-2 By Wind pressure Load

Item	Wire Classification	Height (M)	SPAN		
			60 M	50 M	40 M
Feeder Wire	Cu 300 mm <sup>2</sup> x 2	8.3	336.2	280.5	224.1
Messenger Wire	St 90 mm <sup>2</sup>	7.3	131.4	109.5	87.6
Trolley Wire	Cu 110 mm <sup>2</sup>	6.25	115.6	96.3	76.9
Distribution Line	OE-Cu 38 mm <sup>2</sup> x 3	9.3	493.8	412.0	229.2
Ground Wire	St 55 mm <sup>2</sup>	11.0	158.4	132.0	105.6
Concrete Pole		5.0	350.0	350.0	350.0
Total	M <sub>2</sub> (kgf-M)		1585.4	1380.3	1073.4



## 3-3 By Cross Tension

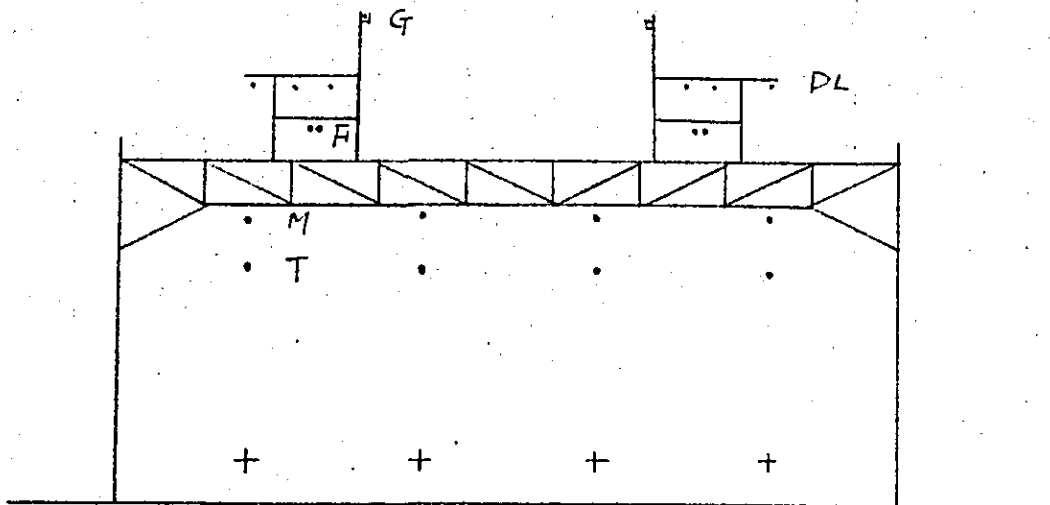
Item	Wire Classification	Height (M)	R=1.000		R=500
			60 M	50 M	40 M
Feeder Wire	Cu 300 <sup>mm<sup>2</sup></sup> x 2	8.3	1314.7	1128.8	1899.0
Messenger Wire	St 90 <sup>mm<sup>2</sup></sup>	7.3	490.6	416.1	677.4
Trolley Wire	Cu 110 <sup>mm<sup>2</sup></sup>	6.25	337.5	281.3	450.0
Distribution Line	OE-Cu 38 <sup>mm<sup>2</sup></sup> x 3	9.3	368.3	320.9	558.0
Ground Wire	St 55 <sup>mm<sup>2</sup></sup>	11.0	242.0	209.0	352.0
Total	M <sub>3</sub> (kgf-M)		2753.1	2356.1	3936.4

## 3-4 Total Moment (kgf-M)

Item	Straight Section			Curve Section	
	60 M	50 M	40 M	R=1000 60 M	R=500 40 M
Total Moment by Weight (M <sub>1</sub> )	575.4	495.3	415.1	575.4	415.1
Total Moment by Wind Load (M <sub>2</sub> )	1585.4	1380.3	1073.4	1585.4	1073.4
Total Moment by Cross Tension (M <sub>3</sub> )				2753.1	3936.4
Total Moment (Outside of Curve)	2160.8	1875.6	1488.5	4913.9	* 5424.9
Total Moment (Inside Curve)	1010.0	885.0	658.3	3763.1	4594.7
Concrete Pole	N 5.000	N 5.000	N 5.000	N 5.000	* N 6.500 N 5.000

\* This case is not concerned with this project design.

Inside of Station (V-Truss Beam)



G: Ground Wire St 55mm<sup>2</sup> x 2

DL: Distribution Line OE-Cu 38mm<sup>2</sup> x 3 x 2

F: Feeder Wire Cu 300mm<sup>2</sup> x 2 x 2

M: Messenger Wire St 90mm<sup>2</sup> x 4

T: Trolley Wire Cu 110mm<sup>2</sup> x 4

Span 60 M R=1000

(1) Moment at Weight

$$MA_1 = MD_1 = \frac{wl^2}{12(K+2)}$$

$$MB_1 = MC_1 = -2MA_1$$

(2) Moment at Horizontal Load

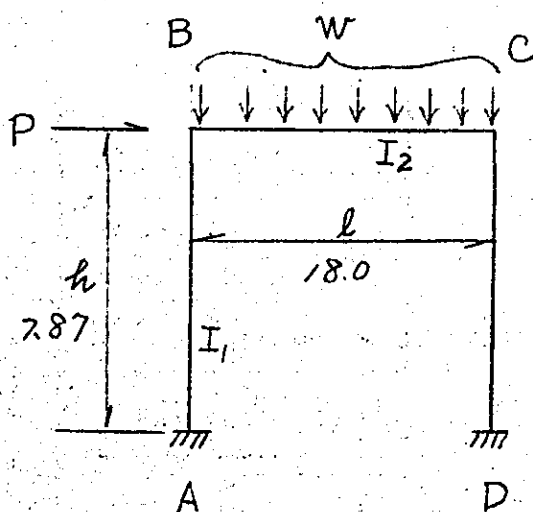
$$MA_2 = -MD_2 = -\frac{ph}{2} \cdot \frac{3K+1}{6K+1}$$

$$MB_2 = -MC_2 = \frac{ph}{2} \cdot \frac{3K}{6K+1}$$

$$MA = MA_1 + MA_2 \quad MC = MC_1 + MC_2$$

$$MB = MB_1 + MB_2 \quad MD = MD_1 + MD_2$$

(3) Total Moment



$$K = \frac{I_2}{I_1} \cdot \frac{h}{l}$$

$I_1$ : Inertia Moment of Pole ( $\text{cm}^4$ )

$I_2$ : Inertia Moment of Beam ( $\text{cm}^4$ )

$W$ : Vertical Load Unit Weight ( $\text{kgf}$ )

$l$ : Beam Length ( $\text{M}$ )

$h$ : Horizontal Load Height ( $\text{M}$ )

$P$ : Horizontal Load

### Inertia Moment

Concrete Pole  $N5.000 = 9.880 \text{ cm}^4$

$N6.500 = 10.196 \text{ cm}^4$

$N7.500 = 10.526 \text{ cm}^4$

V-Truss Beam  $L75 \times 75 \times 6 = 62.149 \text{ cm}^4$

$L75 \times 75 \times 9 = 91.100 \text{ cm}^4$

### 1. Vertical Road (Span 60 M)

Item	Quantity	Weight (kgf)
Feeder Wire $\text{Cu } 300 \text{ mm}^2$	4	651.6
Messenger Wire $\text{St } 90 \text{ mm}^2$	4	167.2
Trolley Wire $\text{Cu } 110 \text{ mm}^2$	4	237.2
Distribution Line $\text{OE-Cu } 38 \text{ mm}^2$	6	145.8
Ground Wire $\text{St } 55 \text{ mm}^2$	2	53.6
V-Truss Beam $L75 \times 75 \times 9$	1	(18M) 1001.0
Total Weight (kgf)		2,256.4

Unit Weight  $w = \frac{2.256^4}{18} = 125.3 \text{ kgf}$

2. Horizontal Load (Span 60M. R=1000)

Item	Load Classification	Quantity	Load (kgf)
Feeder Wire Cu300mm <sup>2</sup>	Wind	4	85.0
	Cross Tension		332.6
Messenger Wire St90mm <sup>2</sup>	Wind	4	22.3
	Cross Tension		250.0
Trolley Wire Cu110mm <sup>2</sup>	Wind	4	19.5
	Cross Tension		170.6
Distribution Line OE-Cu38mm <sup>2</sup>	Wind	6	125.4
	Cross Tension		93.4
Ground Wire St55mm <sup>2</sup>	Wind	2	40.2
	Cross Tension		62.2
Concrete Pole		2	72.0
Total Horizontal Load (kgf)			1273.2

$$K = \frac{I_2}{I_1} \cdot \frac{h}{l} = \frac{91.100}{9.880} \cdot \frac{7.87}{18} = 4.03$$

(1) Moment by Weight

$$MA_1 = MD_1 = \frac{wl^2}{12(K+2)} = \frac{125.3 \times 18^2}{12(4.03+2)} = 561$$

$$MB_1 = MC_1 = -2MA = -2 \times 561 = -1122$$

(2) Moment by Horizontal Load

$$MA_2 = -MD_2 = -\frac{Ph}{2} \cdot \frac{3K+1}{6K+1} = -\frac{1273.2 \times 7.87}{2} \cdot \frac{3 \times 4.03 + 1}{6 \times 4.03 + 1}$$

$$= -2.639$$

$$M_{B2} = -M_{C2} = \frac{Ph}{2} \cdot \frac{3K}{6K+1} = \frac{1.273 \cdot 2 \times 7.87}{2} \cdot \frac{3 \times 4.03}{6 \times 4.03 + 1}$$

$$= 2.394$$

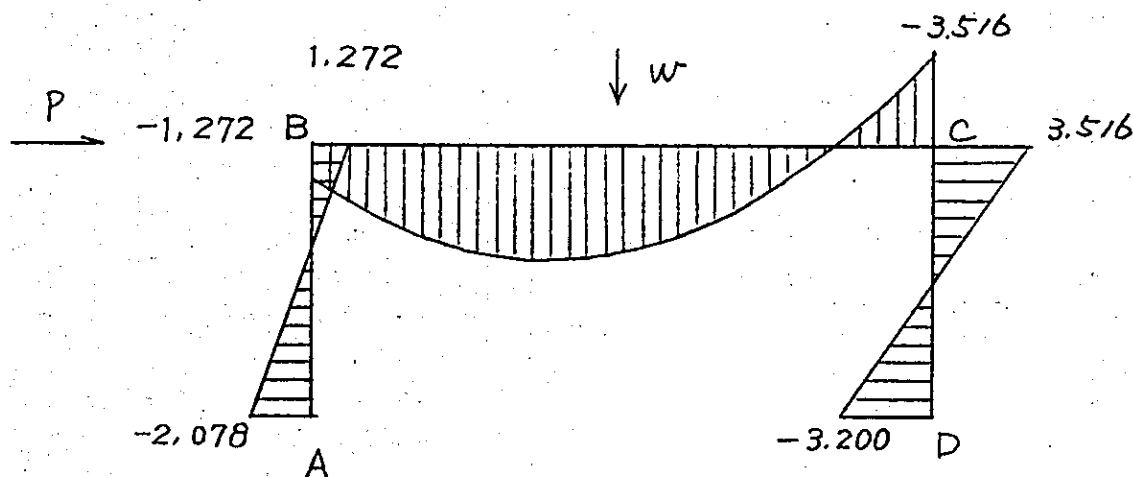
## (3) Total Moment

$$M_A = M_{A1} + M_{A2} = 561 - 2.639 = -2.078 \text{ Kgf-M}$$

$$M_B = M_{B1} + M_{B2} = -1.122 + 2.394 = 1.272 \text{ Kgf-M}$$

$$M_C = M_{C1} + M_{C2} = -1.122 - 2.394 = -3.516 \text{ Kgf-M}$$

$$M_D = M_{D1} + M_{D2} = 561 + 2.639 = 3.200 \text{ Kgf-M}$$



The designed bending moment of 5000 N concrete poles is 5000 kgf·m. Therefore the strength of 5000 N concrete poles is enough to the above loads.

