

V07

$$4 \times 8.0 + 3.0 = 35.000$$

CONCRETE VOLUME

(1) SLAB

$$V_a = 18.079 \text{ m}^3 \times \sqrt[3]{18.970} \times 34.970 = 33.327 \text{ m}^3$$

$$V_b = 0.279 \text{ m}^3 \times 1 = 0.279 \text{ m}^3$$

$$V_c = 0.288 \text{ m}^3 \times 3 = 0.864 \text{ m}^3$$

$$V_d = 0.127 \text{ m}^3 \times 1 = 0.127 \text{ m}^3$$

$$V_e = 10.956 \text{ m}^3 \times \sqrt[3]{18.970} \times 34.970 = 20.197 \text{ m}^3$$

$$\text{SLAB TOTAL} = 54.794 \text{ m}^3$$

(2) BEAM

1) LONGITUDINAL BEAM

$$V_a = 14.212 \text{ m}^3 \times \sqrt[3]{18.220} \times 34.220 = 26.692 \text{ m}^3$$

$$V_b = 0.144 \text{ m}^3 \times 9 \times 2 = 2.592 \text{ m}^3$$

$$\text{SUB TOTAL} = 29.284 \text{ m}^3$$

2) AT END OF VIADUCT, TRANSVERSE BEAM

END (1)

$$V = 4.908 \times 1$$

$$4.908 \text{ m}^3$$

3) AT END OF VIADUCT TRANSVERSE BEAM

END (2)

$$V = 0.585 \text{ m}^3 \times 1$$

$$0.585 \text{ m}^3$$

4) AT INTERMEDIATE OF VIADUCT TRANSVERSE BEAM

$$V_a = 0.936 \text{ m}^3 \times 4 = 3.744 \text{ m}^3$$

$$V_b = 0.006 \text{ m}^3 \times 4 = 0.024 \text{ m}^3$$

SUB TOTAL =

$$3.768 \text{ m}^3$$

5) BASE OF ELECTRIC POLE

$$V = 0.684 \text{ m}^3 \times 1$$

$$(0.684 \text{ m}^3)$$

6) ELECTRIC POLE

$$V = 0.358 \text{ m}^3 \times 1$$

(0.358 ^{m³})

5) + b) (ELECTRIC TOTAL = 1.042 ^{m³})

(1) BEAM TOTAL 38.545 ^{m³}

(2) BEAM TOTAL 39.587 ^{m³})

(3) COLUMN

1) AT END OF VIADUCT COLUMN.

$$V_a = 0.360 \text{ m}^2 \times (H_2 - 1.700) \times 2$$

7.058

3.858 ^{m³}

2) AT INTERMEDIATE OF VIADUCT COLUMN.

$$V_a = 0.360 \text{ m}^2 \times (H - 0.900) \times 8$$

(VARIES)
6.910

17.309 ^{m³}

COLUMN TOTAL 21.167 ^{m³}

(4) FOOTING

$$V_a = 10.752 \text{ m}^3 \times 5 = 53.760 \text{ m}^3$$

$$V_b = 2.880 \text{ m}^3 \times 5 = 14.400 \text{ m}^3$$

FOOTING TOTAL 68.160^{m³}

(5) BRACING BEAM

$$V_a = 1.572 \text{ m}^3 \times 2 = 3.144 \text{ m}^3$$

$$V_b = 1.680 \text{ m}^3 \times 2 \times 3 = 10.080 \text{ m}^3$$

BRACING BEAM TOTAL 13.224^{m³}

(4) + (5) TOTAL 81.384^{m³}

(6) CURB

$$V = 3.319 \text{ m}^3 \times \frac{1}{18.970} \times 34.970$$

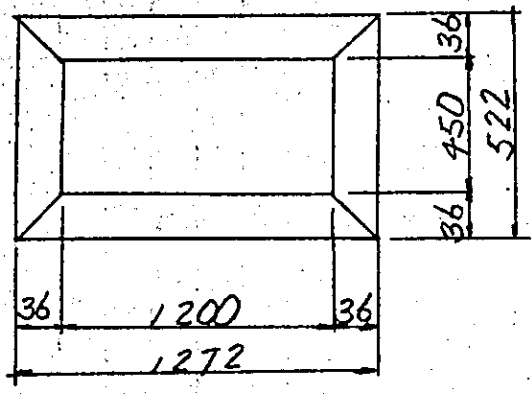
CURB TOTAL 6.118^{m³}

(7) GRADING CONCRETE

$$V = 6.640 \text{ m}^3 \times \frac{1}{18.970} \times 34.970$$

GRADING CONCRETE. TOTAL 12.240^{m³}

(8) FOUNDATION MORTAR



$h = 36$

$$V_{a'} = \{ 1.272 \times 0.522 + (1.272 + 1.200) \times 0.522 + 0.450 \} + 1.200 \times 0.450 \} \times \frac{1}{6} \times 0.036 = 0.022$$

$$V_a = V_{a'} \times 2 = 0.044 \text{ m}^3$$

FOUNDATION MORTAR TOTAL = 0.044 ^{m³}

(9) LEVELING CONCRETE

$$V_a = 1.700 \text{ m}^3 \times 5 = 8.500 \text{ m}^3$$

$$V_b = 0.300 \times 2 = 0.600 \text{ m}^3$$

$$V_c = 0.322 \times 2 \times 3 = 1.932 \text{ m}^3$$

LEVELING CONCRETE TOTAL 11.032 ^{m³}

(10) AGGREGATE SUB BASE

$$V_a = 3.400 \text{ m}^3 \times 5 = 17.000 \text{ m}^3$$

$$V_b = 0.601 \times 2 = 1.202 \text{ m}^3$$

$$V_c = 0.644 \times 2 \times 3 = 3.864 \text{ m}^3$$

AGGREGATE SUB BASE TOTAL 22.066 ^{m³}

(11) PILES

$$\phi = 350 \quad L = 11000 \quad n = 12 \times 4 = 48$$

$$A = \text{_____} \quad \text{A} \quad \text{NUMBER}$$

$$B = 11.0 \times 48 \quad \text{B} \quad 48$$

(12) EXCAVATION

1) FOOTING

$$V = 17.000 \text{ m}^2 \times 2.100 \times 5$$

178.500 ^{m³}

2) BRACING BEAM

$$V_a = 3.003 \text{ m}^2 \times 2.100 \times 2$$

= 12.613 ^{m³}

$$V_b = 3.220 \text{ m}^2 \times 2.100 \times 6$$

= 40.572 ^{m³}

SUB TOTAL =

53.185 ^{m³}

EXCAVATION TOTAL

231.685 ^{m³}

FORM AREA

(1) SLAB

$A_a = 12.998 \times 1$	=	12.998 ^{m²}
$A_b = 14.800 \times 3$	=	44.400
$A_c = 2.860 \times 1$	=	2.860
$A_d = 4.291 \times 1$	=	4.291
$A_e = 4.487 \times 3$	=	13.461
$A_f = 1.093 \times 1$	=	1.093
$A_g = 1.454 \times 5$	=	7.270
$A_a = 0.289 \times 4$	=	1.156 ^{m²}
$A_b = 1.269 \times 34.970 \times 2$	=	88.754
$-A_c = 0.952 \times 1$	=	⊖ 0.952
(POLE)		
$A_d = 1.862 \times 1$	=	1.862

(1) SLAB TOTAL 178.145^{m²}

(2) SLAB TOTAL POLE 177.193

(2) BEAM

1) LONGITUDINAL BEAM

$$Aa' = 7.799 \text{ m}^2 = 7.799 \text{ m}^2$$

$$Ab' = 4.332 \text{ " } = 4.332 \text{ "}$$

$$Ac' = 8.140 \cdot 3 = 24.420$$

$$Ad' = 4.518 \cdot 3 = 13.554$$

$$Ae' = 2.608 = 2.608$$

$$Af' = 1.557 = 1.557$$

$$Ag' = 0.330 = 0.330$$

$$Ah' = 0.240 \cdot 2 \cdot 9 = 4.320$$

$$A' = 58.920$$

$$A = A' \cdot 2$$

$$= 117.840 \text{ m}^2$$

$$117.840 \text{ m}^2$$

2) AT END OF VIADUCT. TRANSVERSE BEAM

AT END (1)

$$A = 16.295 \text{ m}^2 \cdot 1$$

$$= 16.295 \text{ m}^2$$

$$A_{\text{POLE}} = 16.115 \text{ m}^2 \cdot 1$$

$$(= 16.115 \text{ m}^2)$$

3) AT END OF VIADUCT TRANSVERSE BEAM

END (2)

$$A_a = 3.120 \times 1$$

$$= 3.120^{m^2}$$

$$3.120^{m^2}$$

4) AT INTERMEDIATE OF VIADUCT

TRANSVERSE BEAM

$$A_a = 2.600 \times 4$$

$$= 10.400^{m^2}$$

$$A_b = 0.020 \times 4$$

$$= 0.080$$

$$A_c = 1.609 \times 4$$

$$= 6.436$$

$$SUB TOTAL = 16.916^{m^2}$$

5) BASE OF ELECTRIC POLE

$$A = 3.643^{m^2} \times 1$$

$$(3.643^{m^2})$$

6) ELECTRIC POLE

$$A = 4.189^{m^2} \times 1$$

$$(4.189^{m^2})$$

5)+6) (ELECTRIC TOTAL

$$7.832^{m^2}$$

(1) BEAM TOTAL

$$154.171^{m^2}$$

(2) (BEAM TOTAL

$$162.003^{m^2}$$

(3) COLUMN

1) AT END OF VIADUCT COLUMN

$$A_a = 0.600 \times (H_2 \times 4 - 6.900) \times 2 = 25.598^{m^2}$$

(7.058)

25.598^{m²}

2) AT INTERMEDIATE OF VIADUCT COLUMN

$$A_a = 0.600 \times \overset{(VARIES)}{H} \times 4 - 3.900) \times 8 = 113.952^{m^2}$$

6.910

$$-A_b = \underset{POLE}{0.600} \times 0.550 = -0.330^{m^2} \quad (- \ominus 0.330)$$

113.952^{m²}

(113.622^{m²})

COLUMN TOTAL 139.550^{m²}

(COLUMN TOTAL (POLE) 139.220^{m²})

(4) FOOTING

$Aa = 5.680 \times 2$	=	11.360 ^{m²}
$Ab = 6.020 \times 2$	=	12.040
$Ac = 5.680 \times 3$	=	17.040
$Ad = 5.320 \times 3$	=	15.960

FOOTING TOTAL 56.400 ^{m²}

(5) BRACING BEAM

$Aa = 6.286 \times 2$	=	12.572 ^{m²}
$Ab = 6.720 \times 6$	=	40.320

BRACING TOTAL 52.892 ^{m²}

(6) CURB

$$A = 34.496 \times \frac{1}{18.970} \times 34.970$$

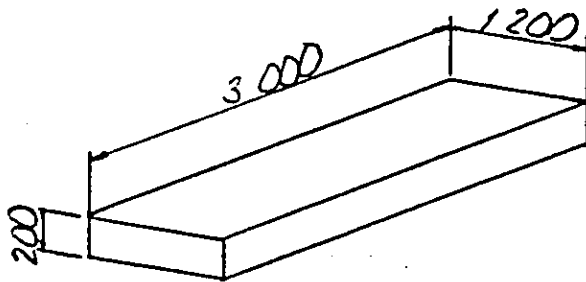
CURB TOTAL 63.591 ^{m²}

SIGNAL SPACE

$$L = 3000$$

CONCRETE VOLUME

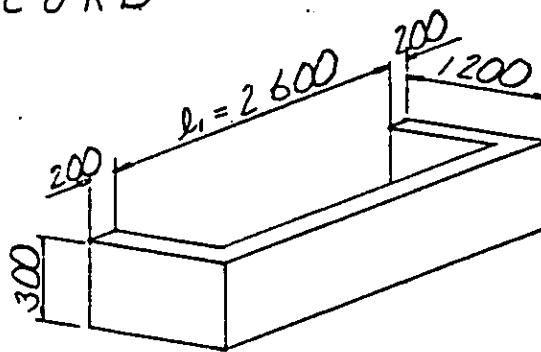
(1) SLAB



$$V = 1.200 \times 0.200 \times 3.000 = 0.720 \text{ m}^3$$

SLAB TOTAL 0.720 ^{m³}

(2) CURB

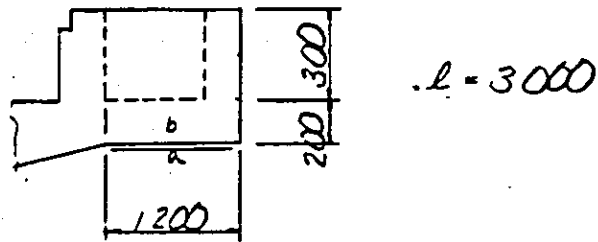


$$V = 0.200 \times 0.300 \times (1.200 \times 2 + 2.600) = 0.300 \text{ m}^3$$

CURB TOTAL 0.300 ^{m³}

FORM AREA

(1) SLAB



$$A_a = 1.200 \times 3.000$$

$$= 3.600 \text{ m}^2$$

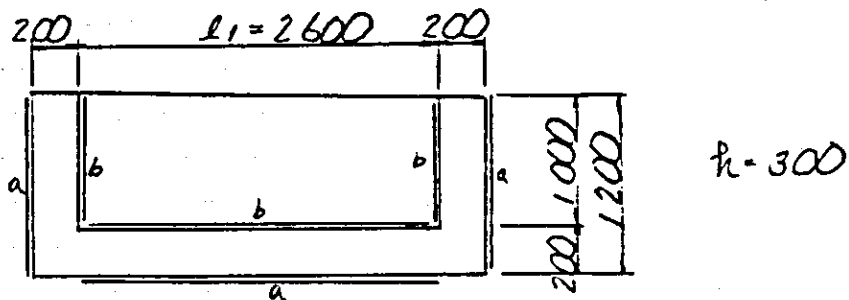
$$A_b = 0.200 \times 1.200 \times 2$$

$$= 0.480$$

SLAB TOTAL

$$4.080 \text{ m}^2$$

(2) CURB



$$A_a = 0.300 \times (1.200 \times 2 + 2.600)$$

$$= 1.500 \text{ m}^2$$

$$A_b = 0.300 \times (1.000 \times 2 + 2.600)$$

$$= 1.380$$

CURB TOTAL

$$2.880 \text{ m}^2$$

REINFORCING BAR

No. 158

REINF. NO.	DIA. (mm)	U. WEIGHT (kg/m)	LENGTH (mm)	NUMBER	WEIGHT (kg)	REMARKS
SIGNAL						
L = 3000						
SLAB						
S 31	D13	0.995	3840	25	95.5	—
32	"	"	1590	13	20.6	—
33	"	"	2930	11	32.1	—
				D13	148.2 ^{kg}	
				SLAB TOTAL	148.2 ^{kg}	
CURB						
W 31	D13	0.995	2930	4	11.7	—
32	"	"	1370	36	49.1	W
33	"	"	1460	8	11.6	J
				D13	72.4 ^{kg}	
				CURB TOTAL	72.4 ^{kg}	

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E

6. VIADUCT. V002

30 M 000

V002 ~ 003	2
V025 ~ 026	2
V049	1
V080 ~ 084	5
V099 ~ 100	2
V103 ~ 122	20

VIADUCT V 048. L = 30.000 (POLE)
H = 7.820

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (kg)	RATIO (kg/m ³)
SLAB	47.0	153.1	3676.1	78.2
BEAM	31.4	132.6	9735.1	310.0
COLUMN	19.9	131.4	5140.9	258.3
CURB	5.2	54.3	591.7	113.8
GRADING CONCRETE	10.5	—	—	—
TOTAL	113.0	464.5	18799.1	167.9
FOOTING	48.8	42.6	5463.2	112.0
BRACING BEAM	10.1	40.3	2731.5	270.4
TOTAL	58.9	82.9	8194.7	139.1

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	M ³	8.1	CLASS F
AGGREGATE SUB BASE	M ³	16.1	A.S.B. - U
EXCAVATION	M ³	145.4	
FOUNDATION MORTAR	M ³	—	$f_{ck} = 900 \text{ kg/cm}^2$
PILE	M x NUMBER	A - 8 x 48 B - 3 x 48	$\phi 350 - A$ $\phi 350 - B$

VIA DUCT V048 L = 30 000 (H = 7.820)
 (ELECTRIC POLE) (AT INTER)

SD 30 UNIT: K8

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	—	—	—	—	—	—	3676.1	—	3676.1
BEAM	—	—	6023.1	102.1	735.7	147.7	2726.5	—	9735.1
COLUMN	—	—	4492.0	—	—	—	648.9	—	5140.9
CURB	—	—	—	—	—	—	—	591.7	591.7
TOTAL	—	—	10515.1	102.1	735.7	147.7	7051.5	591.7	19143.8
FOOTING	2523.2	790.2	—	—	553.6	1150.0	446.0	—	5463.2
BRACING BEAM	—	—	1784.3	—	—	342.4	604.8	—	2731.5
TOTAL	2523.2	790.2	1784.3	—	553.6	1492.4	1050.8	—	8194.7

VIADUCT.
ELECTRIC POLE (AT INTER)

UNIT: KG

SD30

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB									
BEAM			112.2	102.1		21.3	109.1		344.7
COLUMN									
CURB									
TOTAL			112.2	102.1		21.3	109.1		344.7
FOOTING									
BRACING BEAM									
TOTAL									

VIADUCT

L = 30.000

REFER TO V078

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (kg)	RATIO (kg/m ³)
SLAB	47.0	154.0	3676.1	78.2
BEAM	30.4	124.8	9390.4	308.9
COLUMN	*	*	(617.9 ^{kg})	*
CURB	5.2	54.3	591.7	113.8
GRADING CONCRETE	10.5	—	—	—
TOTAL	*+93.1	*+333.1	*+13658.2	*
FOOTING	48.8	42.6	5463.2	112.0
BRACING BEAM	10.1	40.3	2731.5	270.4
TOTAL	58.9	82.9	8194.7	139.1

* REFER TO NO. 173

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	M ³	8.1	CLASS F
AGGREGATE SUB BASE	M ³	16.1	A.S.B. - 3
EXCAVATION	M ³	*	
FOUNDATION MORTAR	M ³	—	f _{ck} = 400 kg/cm ²
PILE	M x NUMBER	A - * B - *	Φ350 - A Φ350 - B

VIADUCT V048 L = 30 000

SD 30 UNIT: K8.

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	—	—	—	—	—	—	3676.1	—	3676.1
BEAM	—	—	3910.9	—	735.7	—	2617.4	—	9390.4
COLUMN	—	—	(539.9% _H)	—	—	—	(78.0% _H)	—	(617.9% _H)
CURB	—	—	—	—	—	—	—	591.7	591.7
TOTAL	—	—	3910.9 +(539.9% _H)	102.1	735.7	—	6293.5 +(78.0% _H)	591.7	13658.2 +(617.9% _H)
FOOTING	2523.2	790.2	—	—	553.6	1150.0	446.0	—	5463.2
BRACING BEAM	—	—	1784.3	—	—	342.4	604.8	—	2731.5
TOTAL	2523.2	790.2	1784.3	—	553.6	1492.4	1050.8	—	8194.7

NOTE; H = H + 0.50

VIADUCT V 002 L=30 000 (POLE)

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (kg)	RATIO (kg/m ³)
SLAB	47.0	153.1	3 676.1	78.2
BEAM	31.4	132.6	9 735.1	310.0
COLUMN	*	*	(617.9 kg/m)	
CURB	5.2	54.3	591.7	113.8
GRADING CONCRETE	10.5	—	—	—
TOTAL	*+94.1	*+340.0	*+14002.9	*
FOOTING	48.8	42.6	5 463.2	112.0
BRACING BEAM	10.1	40.3	2 731.5	270.4
TOTAL	58.9	82.9	8 194.7	139.1

* REFER TO NO. 173

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	M ³	8.1	CLASS F
AGGREGATE SUB BASE	M ³	16.1	A. S. B. - 3
EXCAVATION	M ³	*	
FOUNDATION MORTAR	M ³	—	f _{ck} = 700 kg/cm ²
PILE	M x NUMBER	A - * B - *	Φ350 - A Φ350 - B

VIADUCT V.048 L = 30 000
(ELECTRIC POLE) (AT INTER)

SD 30 UNIT: K8

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	—	—	—	—	—	—	3 676.1	—	3 676.1
BEAM	—	—	6 023.1	1 021	735.7	1 47.7	2 726.5	—	9 735.1
COLUMN	—	—	(539.9% _H)	—	—	—	(78.0% _H)	—	(617.9% _H)
CURB	—	—	—	—	—	—	—	591.7	591.7
TOTAL	—	—	6 023.1 +(539.9% _H)	1 021	735.7	1 47.7	6 402.6 +(78.0% _H)	591.7	14 002.9 +(617.9% _H)
FOOTING	2 523.2	790.2	—	—	553.6	1 150.0	446.0	—	5 463.2
BRACING BEAM	—	—	1 784.3	—	—	342.4	604.8	—	2 731.5
TOTAL	2 523.2	790.2	1 784.3	—	553.6	1 492.4	1 050.8	—	8 194.7

NOTE; H' = H + 0.50

SIGNAL SPACE

$$L = 7000$$

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (KG)	RATIO (KG/m ³)
SLAB	1.7	8.9	339.5	199.7
CURB	0.5	5.3	123.7	247.4
TOTAL	2.2	14.2	463.2	210.5

V 025 L - 30^m 14^x 702^m 000

VIADUCT. SIGNAL L = 7 000

SD30 UNIT: K8.

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	339.5	—	339.5
BEAM	—	—	—	—	—	—	—	—	—
COLUMN	—	—	—	—	—	—	—	—	—
CURB	—	—	—	—	—	—	123.7	—	123.7
TOTAL	—	—	—	—	—	—	463.2	—	463.2
FOOTING	—	—	—	—	—	—	—	—	—
BRACING BEAM	—	—	—	—	—	—	—	—	—
TOTAL	—	—	—	—	—	—	—	—	—

SIGNAL SPACE

$$L = 4.000$$

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (KG)	RATIO (KG/m ³)
SLAB	1.0	5.3	196.0	196.0
CURB	0.4	3.5	84.4	211.0
TOTAL	1.4	8.8	280.4	200.3

V081 L = 30^m 16^K 925^M 000

VIADUCT SIGNAL L = 4 000

SD30 UNIT: K8.

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	196.0	—	196.0
BEAM
COLUMN
CURB	84.4	—	84.4
TOTAL	280.4	—	280.4
FOOTING
BRACING BEAM
TOTAL

SIGNAL SPACE

$$L = 3000$$

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (KG)	RATIO (KG/m ³)
SLAB	0.7	4.1	148.2	211.7
CURB	0.3	2.9	72.4	241.3
TOTAL	1.0	7.0	220.6	220.6

V 115

L = 30^m18^k 239^m 000

VIADUCT SIGNAL L = 3.000

SD30 UNIT: kg

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	148.2	.	148.2
BEAM
COLUMN
CURB	72.4	.	72.4
TOTAL	220.6	.	220.6
FOOTING
BRACING BEAM
TOTAL

NOTES: $L = 30.000$

(1) COLUMN.

1) CONCRETE VOLUME, $V = 0.360 \times (H - 0.900) \times 8$

2) FORM. AREA $A = 0.600 \times (H \times 4 - 3.90) \times 8$

3) REINFORCING BAR $W = 617.9 \times (H + 0.50)$

(2) EXCAVAION. $V = 80.520 \times H$

(3) PC PILES $\phi = 350$ $A = M. \times 48$ $B = M. \times 48$

DIMENSION SCHEDULE NO. 1

	COLUMN				EXCAVAION		PC-PILE
	H	CONCRETE	FORM	REINFORC ING BAR	H	VOLUME	$\phi = 350$
V002	4.084	9.170	59.693	2832.5	1.80	144.9	A = B = 10.0 x 48
003	4.664	10.840	70.829	3190.8	"	"	B = 10.0 x 48
024	5.153	12.249	80.218	3493.0	"	"	B = 12.0 x 48
025	4.692	10.921	71.366	3208.1	"	"	B = 12.0 x 48
026	4.385	10.037	65.472	3018.4	"	"	B = 12.0 x 48
* 048	7.820	19.930	131.424	5140.9	1.806	145.4	A = 8.00 x 48 B = 8.00 x 48
049	"	"	"	"	"	"	"
080	6.786	16.952	111.571	4502.0	1.80	144.9	A = 13.0 x 48 B = 8.0 x 48
081	6.966	17.470	115.027	4613.2	"	"	"
082	7.146	17.988	118.483	4724.5	"	"	"

DIMENSION SCHEDULE NO 2

	COLUMN				EXCAVATION		PC PILE φ=350
	H	CONCRETE	FORM	REINFORCING BAR	H	VOLUME	
o V083	7.326	18.507	121.939	4835.7	1.80	144.9	A=14.0x48 B=8.0x48
o 084	7.506	19.025	125.395	4946.9	"	"	"
o 099	7.350	18.576	122.400	4850.5	1.806	145.4	A=11.0x48 B=8.0x48
o 100	"	"	"	"	"	"	"
o 103	6.868	17.188	113.146	4552.7	1.80	144.9	A=10.0x48 B=8.0x48
o 104	6.434	15.938	104.813	4284.5	"	"	"
o 105	5.984	14.642	96.173	4006.5	"	"	A=9.0x48 B=8.0x48
o 106	5.534	13.346	87.533	3728.4	"	"	"
o 107	5.064	11.992	78.509	3438.0	"	"	"
o 108	4.627	10.734	70.118	3168.0	"	"	"
o 109	4.400	10.080	65.760	3027.7	1.806	145.4	"
o 110	4.442	10.201	66.566	3053.7	1.80	144.9	"
o 113	4.979	11.748	76.877	3385.5	"	"	A=8.0x48 B=8.0x48
o 114	5.129	12.180	79.757	3478.2	"	"	"
o 115	5.279	12.612	82.637	3570.8	"	"	"
o 116	5.429	13.044	85.517	3663.5	"	"	"
o 117	5.579	13.476	88.397	3756.2	"	"	"
o 118	5.773	14.034	92.122	3876.1	"	"	"
o 119	6.082	14.924	98.054	4067.0	"	"	"

DIMENSION SCHEDULE NO3

	COLUMN				EXCAVAION		PC PILE
	H	CONCRETE	FORM	REINFORCING BAR	H	VOLUME	$\phi = 350$
V 120	6.499	16.125	106.061	4324.7	1.80	144.9	A=8.0 x 48 B=8.0 x 48
121	6.949	17.421	114.701	4602.7	"	"	A=7.0 x 48 B=8.0 x 48
122	7.399	18.717	123.341	4880.8	"	"	"

$L = 30 \text{ m}$

CONCRETE VOLUM (V048)

(1) SLAB

$\Sigma V = 46.996 \text{ m}^3$

(2) BEAM

$\Sigma V = 30.376 \text{ m}^3$

($\Sigma V = 31.418$) POLE

(3) COLUMN

$\Sigma V = * \text{ REFER TO NO. 173}$

(4) FOOTING

$\Sigma V = 48.848 \text{ m}^3$

(5) BRACING BEAM

$\Sigma V = 10.080 \text{ m}^3$

(6) CURB

$$\Sigma V = 5.243 \text{ m}^3$$

(7) GRADING CONCRETE

$$\Sigma V = 10.486 \text{ m}^3$$

(8) BEARING BASE MORTAR

$$\Sigma V = 0.000 \text{ m}^3$$

(9) LEVELING CONCRETE

$$\Sigma V = 8.052 \text{ m}^3$$

(10) AGGREGATE SUB BASE

$$\Sigma V = 16.109 \text{ m}^3$$

(11) PILE

A = * REFER TO NO. 173

B = *

(2) EXCAVATION

$$\Sigma V = \times \text{ REFER TO NO. 173}$$

FORM AREA

(1) SLAB

$$\Sigma A = 154.031 \text{ m}^2$$

$$(\Sigma A = 153.079 \text{ m}^2) \text{ POLE}$$

(2) BEAM

$$\Sigma A = 124.764 \text{ m}^2$$

$$(\Sigma A = 132.596 \text{ m}^2) \text{ POLE}$$

(3) COLUMN

$$\Sigma A = \times \text{ REFER TO NO. 173}$$

(4) FOOTING

$$\Sigma A = 42.600 \text{ m}^2$$

(5) BRACING BEAM

$$\Sigma A = 40.320 \text{ m}^2$$

(6) CURB

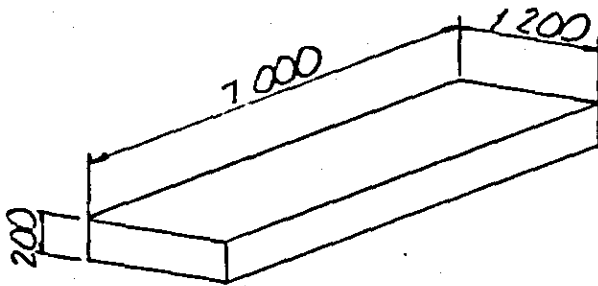
$$\Sigma A = 59.278 \text{ m}^2$$

SIGNAL SPACE

L = 7000

CONCRETE VOLUME

(1) SLAB



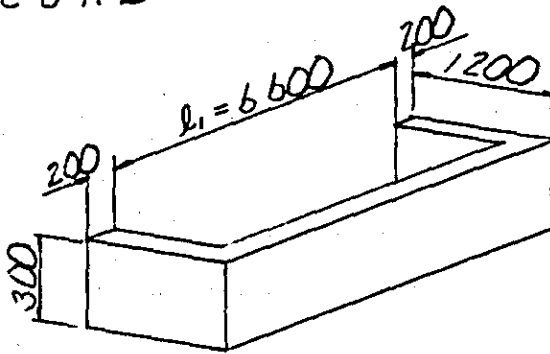
$$V = 1.200 \times 0.200 \times 7.000$$

$$= 1.680 \text{ m}^3$$

SLAB TOTAL

$$1.680 \text{ m}^3$$

(2) CURB



$$V = 0.200 \times 0.300 \times (1.200 \times 2 + 6.600)$$

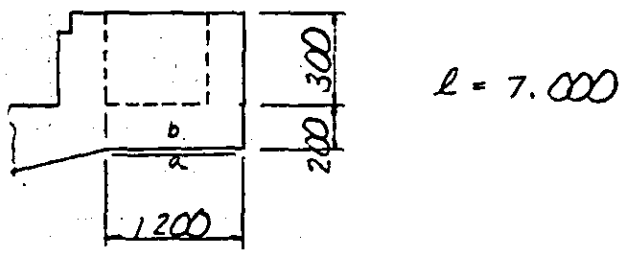
$$= 0.540 \text{ m}^3$$

CURB TOTAL

$$0.540 \text{ m}^3$$

FORM AREA

(1) SLAB

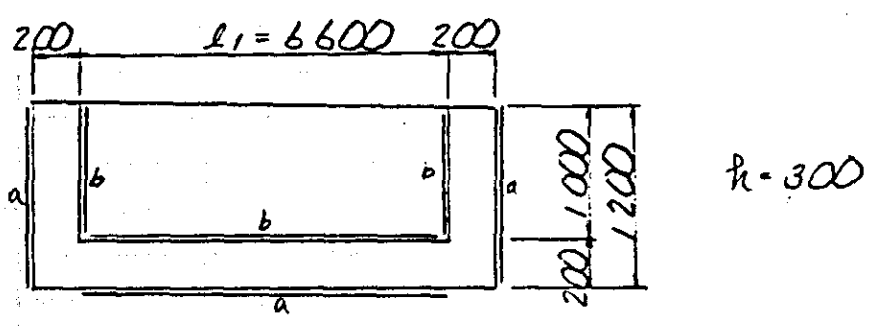


$$A_a = 1.200 \times 7.000 = 8.400 \text{ m}^2$$

$$A_b = 0.200 \times 1.200 \times 2 = 0.480$$

SLAB TOTAL 8.880 m²

(2) CURB



$$A_a = 0.300 \times (1.200 \times 2 + 6.600) = 2.700 \text{ m}^2$$

$$A_b = 0.300 \times (1.000 \times 2 + 6.600) = 2.580$$

CURB TOTAL 5.280 m²

REINFORCING BAR

No. 183

REINF. NO.	DIA. (mm)	U. WEIGHT (kg/m)	LENGTH (mm)	NUMBER	WEIGHT (kg)	REMARKS
SIGNAL						
L = 7000						
SLAB						
S 31	D13	0.995	3840	57	217.8	—
32	"	"	1590	29	45.9	—
33	"	"	6930	11	75.8	—
				D13	339.5 ^{kg}	
				SLAB TOTAL	339.5 ^{kg}	
CURB						
W 31	D13	0.995	6930	4	27.6	—
32	"	"	1370	62	84.5	□
33	"	"	1460	8	11.6	┘
				D13	123.7 ^{kg}	
				CURB TOTAL	123.7 ^{kg}	

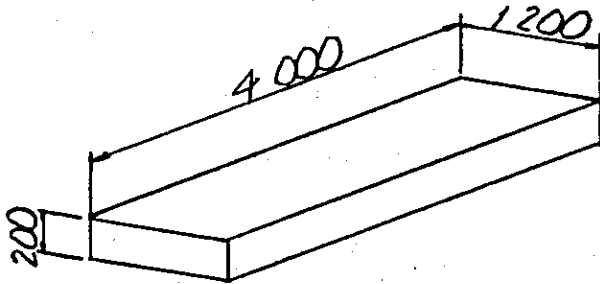
SIGNAL SPACE

$L = 4000$

5
11
17
23
29
35
41
47
53
59
65
71
77
83
89
95
101
107
113
119
125
131
137
143
149
155
161
167
173
179
185

CONCRETE VOLUME

(1) SLAB



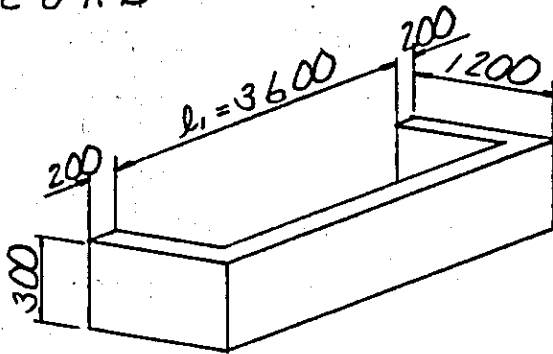
$$V = 1.200 \times 0.200 \times 4.000$$

$$= 0.960 \text{ m}^3$$

SLAB TOTAL

$$0.960 \text{ m}^3$$

(2) CURB



$$V = 0.200 \times 0.300 \times (1.200 \times 2 + 3.600)$$

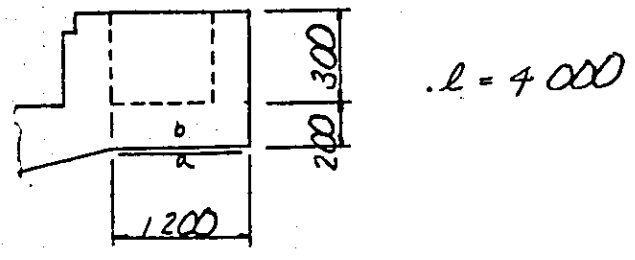
$$= 0.360 \text{ m}^3$$

CURB TOTAL

$$0.360 \text{ m}^3$$

FORM AREA

(1) SLAB



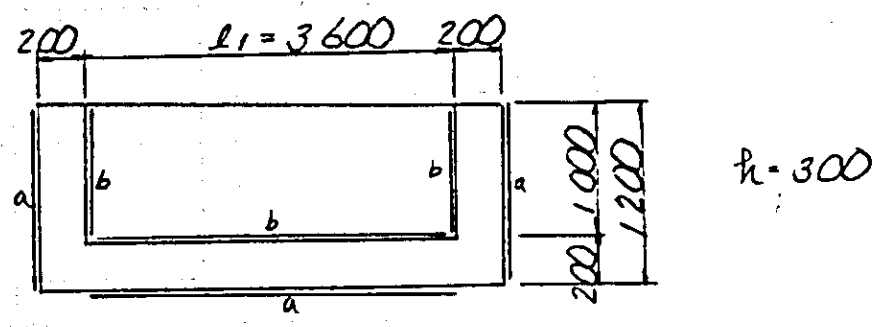
$$A_a = 1.200 \times 4.000 = 4.800 \text{ m}^2$$

$$A_b = 0.200 \times 1.200 \times 2 = 0.480 \text{ m}^2$$

SLAB TOTAL

4.800^{m²}
 0.480^{m²}
 5.280^{m²}

(2) CURB



$$A_a = 0.300 \times (1.200 \times 2 + 3.600) = 1.800 \text{ m}^2$$

$$A_b = 0.300 \times (1.000 \times 2 + 3.600) = 1.680 \text{ m}^2$$

CURB TOTAL

1.800^{m²}
 1.680^{m²}
 3.480^{m²}

REINFORCING BAR

No. 187

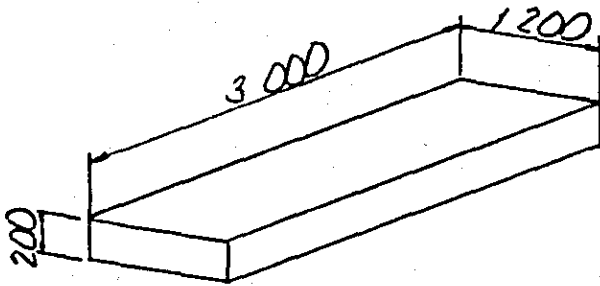
REINF. NO.	DIA. (mm)	U. WEIGHT (kg/m)	LENGTH (mm)	NUMBER	WEIGHT (kg)	REMARKS
SIGNAL						
L = 4000						
SLAB						
S 31	D13	0.995	3840	33	126.1	—
32	"	"	1590	17	26.9	—
33	"	"	3930	11	43.0	—
					D13	196.0 ^{kg}
SLAB TOTAL					196.0 ^{kg}	
CURB						
W 31	D13	0.995	3930	4	15.6	—
32	"	"	1370	42	57.2	□
33	"	"	1460	8	11.6	┘
					D13	84.4 ^{kg}
CURB TOTAL					84.4 ^{kg}	

SIGNAL SPACE

$$L = 3000$$

CONCRETE VOLUME

(1) SLAB



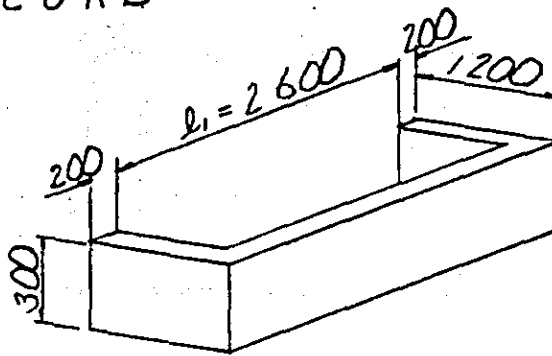
$$V = 1.200 \times 0.200 \times 3.000$$

$$= 0.720 \text{ m}^3$$

SLAB TOTAL

$$0.720 \text{ m}^3$$

(2) CURB



$$V = 0.200 \times 0.300 \times (1.200 \times 2 + 2.600)$$

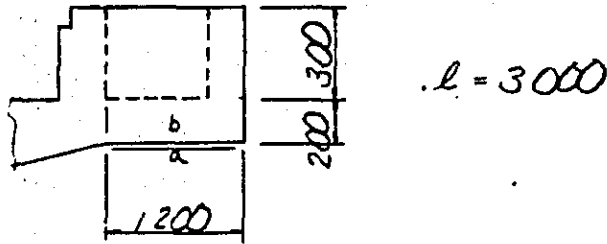
$$= 0.300 \text{ m}^3$$

CURB TOTAL

$$0.300 \text{ m}^3$$

FORM AREA

(1) SLAB



$$A_a = 1.200 \times 3.000$$

$$= 3.600 \text{ m}^2$$

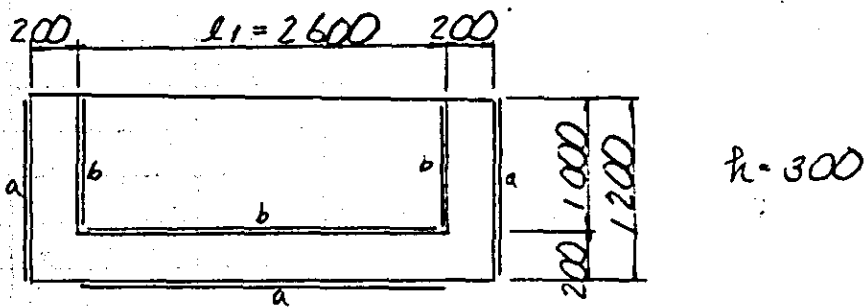
$$A_b = 0.200 \times 1.200 \times 2$$

$$= 0.480$$

SLAB TOTAL

$$4.080 \text{ m}^2$$

(2) CURB



$$A_a = 0.300 \times (1.200 \times 2 + 2.600)$$

$$= 1.500 \text{ m}^2$$

$$A_b = 0.300 \times (1.000 \times 2 + 2.600)$$

$$= 1.380$$

CURB TOTAL

$$2.880 \text{ m}^2$$

REINFORCING BAR

No. 191

REINF. NO.	DIA. (mm)	U. WEIGHT (kg/m)	LENGTH (mm)	NUMBER	WEIGHT (kg)	REMARKS
SIGNAL						
L = 3000						
SLAB						
S 31	D 13	0.995	3840	25	95.5	→
32	"	"	1590	13	20.6	—
33	"	"	2930	11	32.1	—
				D 13	148.2 ^{kg}	
				SLAB TOTAL	148.2 ^{kg}	
CURB						
W 31	D 13	0.995	2930	4	11.7	—
32	"	"	1370	36	49.1	□
33	"	"	1460	8	11.6	↓
				D 13	72.4 ^{kg}	
				CURB TOTAL	72.4 ^{kg}	

7. VIADUCT. V004

38 M 000

V004	V005	V006	V009	V012
V015	V016	V019 ~	V023	
V029 ~	V045	V050 ~	V072	
V075 ~	V079	V123 ~	V126	

VIADUCT V004 L = 38.000

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (kg)	RATIO (kg/m ³)
SLAB	59.5	195.1	4 618.7	27.6
BEAM	38.1	156.2	11 783.9	309.3
COLUMN	16.1	105.3	4 529.3	281.3
CURB	6.6	68.8	749.1	113.5
GRADING CONCRETE	13.3	—	—	—
TOTAL	133.6	525.4	21 681.1	162.3
FOOTING	61.1	52.9	6 829.0	111.8
BRACING BEAM	13.4	53.8	3 522.4	262.9
TOTAL	74.5	106.7	10 351.4	138.9

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	M ³	10.2	CLASS F
AGGREGATE SUB BASE	M ³	20.5	A.S.B. - 3
EXCAVATION	M ³	184.1	
FOUNDATION MORTAR	M ³	—	f _{ck} = 400 kg/cm ²
PILE	M x NUMBER	A - — B - 10.0 x 60	Φ350 - A Φ350 - B

VIA DUCT V004 L-38.00 (H-5364)

SD30 UNIT: Kg

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	4618.7	.	4618.7
BEAM	.	.	7459.9	.	879.7	151.7	3292.6	.	11783.9
COLUMN	.	.	3957.6	.	.	.	571.7	.	4529.3
CURB	749.1	749.1
TOTAL	.	.	11417.5	.	879.7	151.7	8483.0	749.1	21681.0
FOOTING	3154.0	988.0	.	.	692.0	1437.5	557.5	.	6829.0
BRACING BEAM	.	.	2293.8	.	.	442.3	786.3	.	3522.4
TOTAL	3154.0	988.0	2293.8	.	692.0	1879.8	1343.8	.	10351.4

NOTE ; 0 COLUMN . 772.4 $\frac{1}{2}$ x (H + 0.50) = TOTAL WEIGHT.

$2D.H = H + 0.50$

VIADUCT L = 38.000

REFER TO V004

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (kg)	RATIO (kg/m ³)
SLAB	59.5	195.1	4618.7	77.6
BEAM	38.1	156.2	11783.9	309.3
COLUMN	*	*	(772.4/m)	*
CURB	6.6	68.8	749.1	113.5
GRADING CONCRETE	13.3	—	—	—
TOTAL	* + 117.5	* + 420.1	* + 17151.7	*
FOOTING	61.1	52.9	6829.0	111.8
BRACING BEAM	13.4	53.8	3522.4	262.9
TOTAL	74.5	106.7	10351.4	138.9

* REFER TO NO. 202

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	M ³	10.2	CLASS F
AGGREGATE SUB BASE	M ³	20.5	A.S.B. - 3
EXCAVATION	M ³	*	
FOUNDATION MORTAR	M ³	—	f _{ck} = 400 kg/cm ²
PILE	M x NUMBER	A - * B - *	Φ350 - A Φ350 - B

VIA DUCT V004 L-38.000

SD30 UNIT: kg

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	4618.7	.	4618.7
BEAM	.	.	7459.9	.	879.7	151.7	3292.6	.	11783.9
COLUMN	.	.	(674.9% ^{MY} / _{4MH})	.	.	.	(97.5% ^{MY} / _{4MH})	.	(772.4% ^{MY} / _{4MH})
CURB	749.1	749.1
TOTAL	.	.	7459.9 + 674.9% ^{MY} / _{4MH}	.	879.7	151.7	7911.3 +(97.5% ^{MY} / _{4MH})	749.1	17151.7 +(772.4% ^{MY} / _{4MH})
FOOTING	3154.0	988.0	.	.	692.0	1437.5	557.5	.	6829.0
BRACING BEAM	.	.	2293.8	.	.	442.3	706.3	.	3522.4
TOTAL	3154.0	988.0	2293.8	.	692.0	1879.8	1343.8	.	10351.4

NOTE; COLUMN (1) 772.4%^{MY}/_{4MH} x (H+0.50) = TOTAL WEIGHT

2) H² = H+0.50

VIADUCT V005 L = 38.000 (POLE)

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (kg)	RATIO (kg/m ³)
SLAB	59.5	194.1	9 618.7	77.6
BEAM	39.2	164.0	12 128.6	309.4
COLUMN	*	*	* (772.4 ^{kg/m})	*
CURB	6.6	68.8	749.1	113.5
GRADING CONCRETE	13.3	—	—	—
TOTAL	* + 118.6	* + 427.0	* + 17 496.4	*
FOOTING	61.1	52.9	6 829.0	111.8
BRACING BEAM	13.4	53.8	3 522.4	262.9
TOTAL	74.5	106.7	10 351.4	138.9

* REFER TO NO. 202

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	M ³	10.2	CLASS F
AGGREGATE SUB BASE	M ³	20.5	A.S.B. - 3
EXCAVATION	M ³	*	
FOUNDATION MORTAR	M ³	—	f _{ck} = 400 kg/cm ²
P I L E	M x NUMBER	A - * B - *	Φ350 - A Φ350 - B

VIA DUCT V004 L=38.000
(ELECTRIC. POLE)(AT. INTER)

SD 30 UNIT: K8

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	4618.7	.	4618.7
BEAM	.	.	7572.1	102.1	879.7	173.0	3401.7	.	12128.6
COLUMN	.	.	(674.9% ^{M/H})	.	.	.	(97.5% ^{M/H})	.	(772.4% ^{M/H})
CURB	749.1	749.1
TOTAL	.	.	7572.1 (674.9% ^{M/H})	102.1 (9.1% ^{M/H})	879.7	173.0	8020.4 (97.5% ^{M/H})	749.1	17496.4 (772.4% ^{M/H})
FOOTING	3154.0	988.0	.	.	692.0	1437.5	557.5	.	6829.0
BRACING BEAM	.	.	2293.8	.	.	442.3	786.3	.	3522.4
TOTAL	3154.0	988.0	2293.8	.	692.0	1879.8	1343.8	.	10351.4

NOTE ; 1) COLUMN. 772.4%^{M/H} x (H+0.50) = TOTAL WEIGHT

2) $H' = H + 0.50$

VIADUCT.
ELECTRIC POLE (AT INTER)

SD30 UNIT: K8.

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB
BEAM	.	.	112.2	102.1	.	21.3	109.1	.	344.7
COLUMN
CURB
TOTAL	.	.	112.2	102.1	.	21.3	109.1	.	344.7
FOOTING
BRACING BEAM
TOTAL

SIGNAL SPACE

$$L = 7000$$

	CONCRETE VOLUME (m ³)	FORM AREA (m ²)	REINFORCING BAR (KG)	RATIO (KG/m ³)
SLAB	1.7	8.9	339.5	199.7
CURB	0.5	5.3	123.7	247.4
TOTAL	2.2	14.2	463.2	210.5

V 042 L = 38^m 15^k 405^m 000
 V 062 L = 38^m 16^k 199^m 000
 V 068 L = 38^m 16^k 419^m 000
 V 126 L = 38^m 18^k 601^m 000

VIADUCT SIGNAL L = 7.000

SD 30 UNIT: KG

	D32	D29	D25	D22	D19	D16	D13	D10	TOTAL
SLAB	339.5	—	339.5
BEAM
COLUMN
CURB	123.7	—	123.7
TOTAL	463.2	—	463.2
FOOTING
BRACING BEAM
TOTAL

NOTES: $L = 38.000$

(1) COLUMN.

1) CONCRETE VOLUME. $V = 0.360 \times (H - 0.900) \times 10$ ^{VARIES}

2) FORM. AREA $A = 0.600 \times (H \times 4 - 3.90) \times 10$ ^{VARIES}

3) REINFORCING BAR $W = 772.4 \frac{\text{kg}}{\text{m}} \times (H - 0.50)$ ^{VARIES}

(2) EXCAVAION. $V = 102.260 \times H$ ^{VARIES}

(3) PC PILES $\phi = 350$ $A = M. \times 60$ $B = M. \times 60$

DIMENSION SCHEDULE NO. 1.

	COLUMN				EXCAVAION		PC: PILE
	H	CONCRETE	FORM	REINFORC ING BAR	H	VOLUME	$\phi = 350$
* V004	5.364	16.070	105.336	4529.4	1.80	184.1	A = B = 10.0 x 60
o 005	6.124	18.806	123.576	5116.4	"	"	B = 10.0 x 60
o 006	6.450	19.980	131.400	5368.2	"	"	B = 11.0 x 60
o 009	7.060	22.176	146.040	5839.3	"	"	B = 11.0 x 60
o 012	"	"	"	"	"	"	B = 11.0 x 60
o 015	7.322	23.119	152.328	6041.7	1.80	"	B = 11.0 x 60
o 016	7.436	23.530	155.064	6129.8	"	"	B = 11.0 x 60
o 019	7.176	22.594	148.824	5928.9	"	"	B = 12.0 x 60
o 020	7.268	22.925	151.032	6000.0	"	"	B = 12.0 x 60
8 o 021	6.804	21.254	139.896	5641.6	"	"	B = 12.0 x 60

DIMENSION SCHEDULE NO 2

	COLUMN				EXCAVAION		PC PILE φ=350
	H	CONCRETE	FORM	REINFORC ING BAR	H	VOLUME	
o V022	6.234	19.202	126.216	5201.3	1.80	184.1	A = x B = 12 x 60
o 023	5.662	17.143	112.488	4759.5	1.80	"	B = 12.0 x 60
o 029	4.720	13.752	89.880	4031.9	1.806	184.7	"
o 030	"	"	"	"	"	"	B = 13.0 x 60
o 031	"	"	"	"	"	"	"
o 032	"	"	"	"	"	"	"
o 033	"	"	"	"	"	"	"
o 034	"	"	"	"	"	"	B = 14.0 x 60
o 035	"	"	"	"	"	"	"
o 036	"	"	"	"	"	"	"
o 037	"	"	"	"	"	"	"
o 038	"	"	"	"	"	"	"
o 039	"	"	"	"	"	"	"
o 040	4.712	13.723	89.688	4025.7	1.80	184.1	B = 14.0 x 60
o 041	5.006	14.782	96.744	4252.8	"	"	"
o 042	6.168	18.965	124.632	5150.4	"	"	"
o 043	6.700	20.880	137.200	5561.3	"	"	"
o 044	7.232	22.795	150.168	5972.2	"	"	"
o 045	7.746	24.646	162.504	6369.2	"	"	"

DIMENSION SCHEDULE NO 3

	COLUMN				EXCAVAION		PC PILE φ=350
	H	CONCRETE	FORM	REINFORC ING BAR	H	VOLUME	
V050	7.723	24.563	161.952	6351.4	1.80	184.1	A=8.0 x 60 B=8.0 x 60
051	7.499	23.756	156.576	6178.4	"	"	"
052	7.233	22.799	150.192	5973.0	"	"	"
053	6.967	21.841	143.808	5767.5	"	"	"
054	6.701	20.884	137.424	5562.1	"	"	"
055	6.644	20.678	136.056	5518.0	"	"	"
056	6.532	20.275	133.368	5431.5	"	"	A=9.0 x 60 B=8.0 x 60
057	6.550	20.340	133.800	5445.4	1.806	184.7	"
058	"	"	"	"	"	"	"
059	"	"	"	"	"	"	"
060	"	"	"	"	"	"	"
061	"	"	"	"	"	"	"
062	"	"	"	"	"	"	A=10.0 x 60 B=8.0 x 60
063	"	"	"	"	"	"	"
064	"	"	"	"	"	"	"
065	"	"	"	"	"	"	"
066	"	"	"	"	"	"	"
067	"	"	"	"	"	"	"
068	"	"	"	"	"	"	A=11.0 x 60 B=8.0 x 60

DIMENSION SCHEDULE NO 4

	COLUMN				EXCAVATION		PC PILE Φ=350
	H	CONCRETE	FORM	REINFORCING BAR	H	VOLUME	
069	6.550	20.340	133.800	5445.4	1.806	184.7	A=11.0x60 B=8.0x60
070	6.350	19.620	129.000	5290.9	"	"	"
071	"	"	"	"	"	"	"
072	"	"	"	"	"	"	A=11.0x60 B=8.0x60
075	"	"	"	"	"	"	"
076	"	"	"	"	"	"	"
077	6.150	18.900	124.200	5136.5	"	"	A=13.0x60 B=8.0x60
078	6.306	19.462	127.944	5257.0	1.80	184.1	"
079	6.582	20.455	134.568	5470.1	"	"	"
123	7.909	25.232	166.416	6495.1	1.80	"	A=7.0x60 B=8.0x60
124	8.469	27.248	179.856	6541.5	"	"	"
125	8.844	28.598	188.856	7217.3	"	"	"
126	9.150	29.700	196.500	7453.7	1.806	184.7	"

V04

$$3.0 + 4 \times 8.0 + 3.0 = 38.000$$

CONCRETE VOLUME

(1) SLAB

$V_a = 3.800 \times (0.250 \times 37.960 + 0.150 \cdot 0.100 \cdot 2) =$	36.176 m^3
$V_b = 17.302 \text{ m}^3 \cdot \frac{1}{2} 29.960 \times 37.960$	$= 21.922 \text{ m}^3$
$V_c = 0.127 \cdot 2$	$= 0.254 \text{ m}^3$
$V_d = 0.288 \times 4$	$= 1.152 \text{ m}^3$

SLAB TOTAL

59.504 ^{m³}

(2) BEAM

1) LONGITUDINAL BEAM

$V_a = 23.134 \text{ m}^3 \cdot \frac{1}{2} 29.660 \times 37.660$	$= 29.374 \text{ m}^3$
$V_b = 0.144 \cdot 10 \cdot 2$	$= 2.880 \text{ m}^3$

32.254 ^{m³}

2) AT END OF VIADUCT TRANSVERSE BEAM

END (2)

$$V_a = 0.585 \cdot 2$$

= 1.170 ^{m³}

1.170 ^{m³}

3) AT INTERMEDIATE OF VIADUCT

TRANSVERSE BEAM

$$V_a = 0.936 \times 5 = 4.680 \text{ m}^3$$

$$V_b = 0.003 \times 2 \times 5 = 0.030 \text{ m}^3$$

$$\text{SUB TOTAL} = 4.710 \text{ m}^3$$

4) BASE OF ELECTRIC POLE

$$V = 0.684 \text{ m}^3 \times 1 = 0.684 \text{ m}^3$$

$$(0.684 \text{ m}^3)$$

5) ELECTRIC POLE

$$V = 0.358 \text{ m}^3 \times 1 = 0.358 \text{ m}^3$$

$$(0.358 \text{ m}^3)$$

$$4) + 5) \text{ ELECTRIC TOTAL } (1.042 \text{ m}^3)$$

$$\text{BEAM TOTAL } 38.134 \text{ m}^3$$

$$(\text{BEAM TOTAL } 39.176 \text{ m}^3 \\ \text{(POLE)})$$

(3) COLUMN

1) AT END OF VIADUCT COLUMN

$$V = \frac{0.360 \text{ m}^2 \text{ VARIES}}{5.364} \times (H - 0.900) \times 4 = 6.428 \text{ m}^3$$

$$6.428 \text{ m}^3$$

2) AT INTERMEDIATE OF VIADUCT COLUMN

$$V = \frac{0.360 \text{ m}^2 \text{ VARIES}}{5.364} \times (H - 0.900) \times 6 = 9.642 \text{ m}^3$$

$$9.642 \text{ m}^3$$

COLUMN TOTAL

$$16.070 \text{ m}^3$$

(4) FOOTING

$$V_a = 9.632 \text{ m}^3 \times 5 = 48.160 \text{ m}^3$$

$$V_b = 2.580 \text{ m}^3 \times 5 = 12.900 \text{ m}^3$$

FOOTING TOTAL

$$61.060 \text{ m}^3$$

(5) BRACING BEAM

$$V = 3.360 \text{ m}^3 \times 4 = 13.440 \text{ m}^3$$

BRACING TOTAL

$$13.440 \text{ m}^3$$

(6) CURB

$$V = 5.243^{\text{m}^3} \times \sqrt{29.960} \times 37.960 = 6.643^{\text{m}^3}$$

CURB TOTAL 6.643^{m^3}

(7) GRADING CONCRETE

$$V = 0.070 \times 5.000 \times 37.960 = 13.286^{\text{m}^3}$$

GRADING CONCRETE TOTAL 13.286^{m^3}

(9) LEVELING CONCRETE

$$V_a = 1.530^{\text{m}^3} \times 5 = 7.650^{\text{m}^3}$$

$$V_b = 0.644 \times 4 = 2.576^{\text{m}^3}$$

LEVELING CONCRETE TOTAL 10.226^{m^3}

(10) AGGREGATE SUB BASE

$$V_a = 3.060^{\text{m}^3} \times 5 = 15.300^{\text{m}^3}$$

$$V_b = 1.288 \times 4 = 5.152^{\text{m}^3}$$

AGGREGATE SUB BASE TOTAL 20.452^{m^3}

(11) PILE

$$\phi = 350 \quad l = 10.000 \text{ m} = 60$$

$$A = \text{—————}$$

$$B = 10.00 \times 60$$

$$A \text{ —————}$$

$$B = 60$$

(12) EXCAVATION

1) FOOTING

$$V_a = 15.300 \text{ m}^2 \cdot 1.800 \text{ m} \cdot 5 \text{ m} = 137.700 \text{ m}^3$$

$$= 137.700 \text{ m}^3$$

$$137.700 \text{ m}^3$$

2) BRACING BEAM

$$V_a = 6.440 \text{ m}^2 \cdot 1.800 \text{ m} \cdot 4 \text{ m} = 46.368 \text{ m}^3$$

$$= 46.368 \text{ m}^3$$

$$46.368 \text{ m}^3$$

EXCAVATION TOTAL

$$184.068 \text{ m}^3$$

FORM AREA

(1) SLAB

SLAB (1) Aa = 2.860 ^m × 2	=	5.720 ^{m²}
Ab = 14.800 × 4	=	59.200 ^{m²}
Ac = 4.487 ^m × 4	=	17.948 ^{m²}
Ad = 1.093 ^m × 2	=	2.186 ^{m²}
Ae = 1.454 ^m × 6	=	8.724 ^{m²}
SLAB (2) Aa = 0.289 × 4	=	1.156 ^{m²}
Ab = 1.269 × 37.960 × 2	=	96.342 ^{m²}
-Ac = 0.952 × 1 POLE	=	⊖ 0.952 ^{m²}
Ad = 1.900 × 2	=	3.800 ^{m²}

SLAB TOTAL 195.076^{m²}

(SLAB TOTAL POLE 194.124^{m²})

(2) BEAM

1) LONGITUDINAL BEAM

$$A_a = 7.400 \cdot 0.550 \cdot 2 \cdot 4 \cdot 2 = 65.120 \text{ m}^2$$

$$A_b = (1.265 \cdot 2 + 5.000) \cdot 0.600 \cdot 4 \cdot 2 = 36.144 \text{ m}^2$$

$$A_c = 5.216 \text{ m}^2 \cdot 2 = 10.432 \text{ m}^2$$

$$A_d = 3.114 \cdot 2 = 6.228 \text{ m}^2$$

$$A_e = 0.660 \cdot 2 = 1.320 \text{ m}^2$$

$$A_f = 1.200 \cdot 0.400 \cdot \frac{1}{2} \cdot 2 \cdot 10 \cdot 2 = 9.600 \text{ m}^2$$

$$2) \quad 0.000 \text{ m}^2 \quad \text{SUB TOTAL} = 128.844 \text{ m}^2$$

3) AT END OF VIADUCT TRANSVERSE BEAM

END (2)

$$A_a = 6.240 \text{ m}^2 = 6.240 \text{ m}^2$$

$$6.240 \text{ m}^2$$

4) AT INTERMEDIATE OF VIADUCT

TRANSVERSE BEAM

$$A_a = 2.600 \text{ m}^2 \cdot 5 = 13.000 \text{ m}^2$$

$$A_b = 0.020 \cdot 5 = 0.100 \text{ m}^2$$

$$A_c = 1.609 \cdot 5 = 8.045 \text{ m}^2$$

$$\text{SUB TOTAL} = 21.145 \text{ m}^2$$

5) BASE OF ELECTRIC POLE

$$A_a = 3.643^{m^2} = 3.643^{m^2}$$

(3.643^{m²})

6) ELECTRIC POLE

$$A = 4.189^{m^2} = 4.189^{m^2}$$

(4.189^{m²})

(5) + 6) - ELECTRIC TOTAL (7.832^{m²})

BEAM TOTAL 156.229^{m²}

(BEAM TOTAL (POLE) 164.061^{m²})

(3) COLUMN

1) AT END OF VIADUCT. COLUMN

$$A_a = 0.600 \cdot \frac{(H_{2.6} \times 4 - 3.900)}{5.364} \times 4$$

$$= 42.134 \text{ m}^2$$

$$42.134 \text{ m}^2$$

2) AT INTERMEDIATE OF VIADUCT. COLUMN

$$A_a = 0.600 \cdot \frac{(H_4 \times 4 - 3.900)}{5.364} \times 6$$

$$= 63.202 \text{ m}^2$$

$$63.202 \text{ m}^2$$

COLUMN TOTAL

$$105.336 \text{ m}^2$$

(4) FOOTING

$$A_a = 5.680 \times 2$$

$$= 11.360 \text{ m}^2$$

$$A_b = 5.320 \times 2$$

$$= 10.640$$

$$A_c = 5.680 \times 3$$

$$= 17.040$$

$$A_d = 4.620 \times 3$$

$$= 13.860$$

FOOTING TOTAL

$$52.900 \text{ m}^2$$

(5) BRACING BEAM

$$A_a = 6.720 \times 4 = 26.880 \text{ m}^2$$

$$A_b = 6.720 \times 4 = 26.880 \text{ m}^2$$

BRACING TOTAL

53.760 ^{m²}

(6) CURB

$$A = 54.278 \times \frac{1}{2} \times 29.960 \times 37.960 = 68.771 \text{ m}^2$$

CURB TOTAL

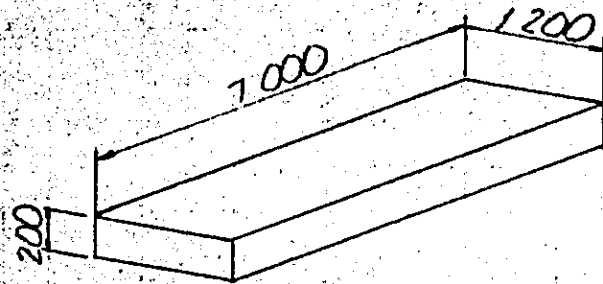
68.771 ^{m²}

SIGNAL SPACE

$$L = 7000$$

CONCRETE VOLUME

(1) SLAB



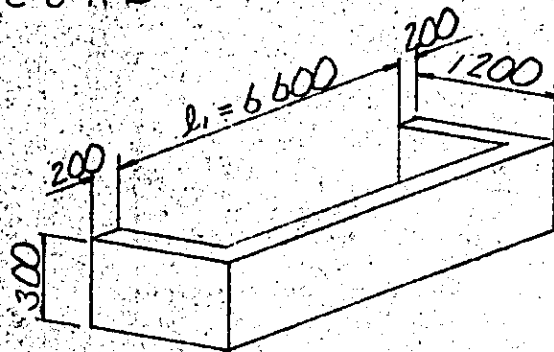
$$V = 1.200 \times 0.200 \times 7.000$$

$$= 1.680 \text{ m}^3$$

SLAB TOTAL

$$1.680 \text{ m}^3$$

(2) CURB



$$V = 0.200 \times 0.300 \times (1.200 \times 2 + 6.600)$$

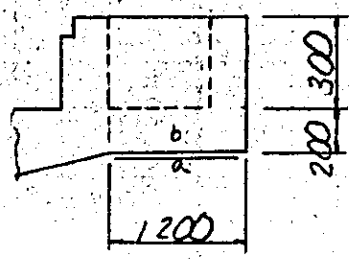
$$= 0.540 \text{ m}^3$$

CURB TOTAL

$$0.540 \text{ m}^3$$

FORM AREA

(1) SLAB



$l = 7.000$

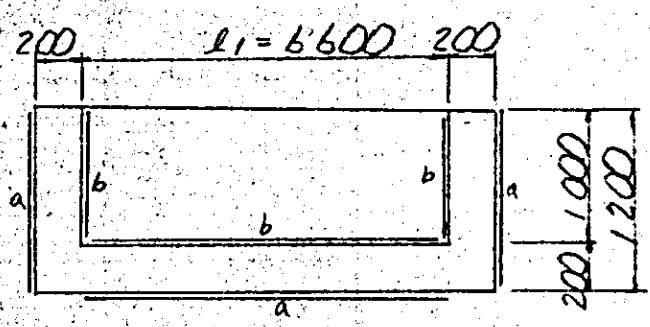
$A_a = 1.200 \times 7.000 = 8.400 \text{ m}^2$

$A_b = 0.200 \times 1.200 \times 2 = 0.480$

SLAB TOTAL

8.880 m^2

(2) CURB



$h = 300$

$A_a = 0.300 \times (1.200 \times 2 + 6.600) = 2.700 \text{ m}^2$

$A_b = 0.300 \times (1.000 \times 2 + 6.600) = 2.580$

CURB TOTAL

5.280 m^2

REINFORCING BAR

No. 219

REINF. NO.	DIA. (mm)	U. WEIGHT (kg/m)	LENGTH (mm)	NUMBER	WEIGHT (kg)	REMARKS
SIGNAL						
L = 7000						
SLAB						
B. 31	D. 13	0.995	3840	57	217.8	—
L. 32	"	"	1590	29	45.9	—
33	"	"	6930	11	75.8	—
					D. 13	339.5 ^{kg}
SLAB TOTAL						339.5 ^{kg}
CURB						
W. 31	D. 13	0.995	6930	4	27.6	—
32	"	"	1370	62	84.5	□
33	"	"	1460	8	11.6	┘
					D. 13	123.7 ^{kg}
CURB TOTAL						123.7 ^{kg}

