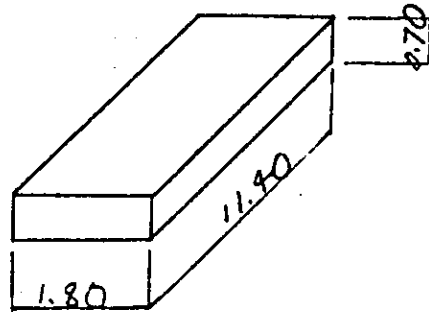
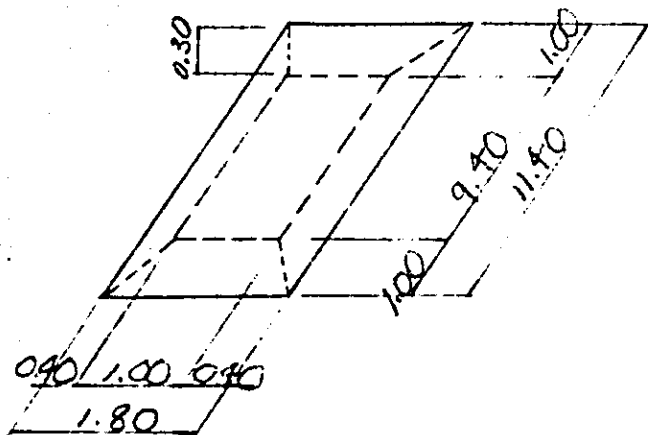


1) CONCRETE VOLUME

(1) BEAM



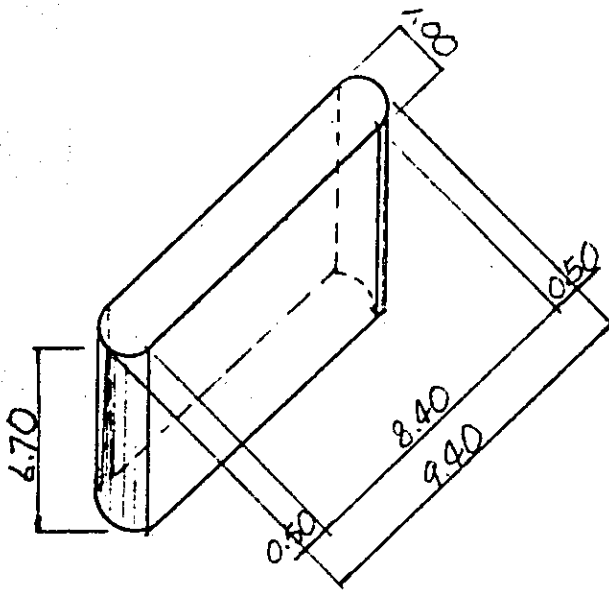
$$V_a = 1.80 \times 0.70 \times 11.40 = 14.364 \text{ m}^3$$



$$V_b = \frac{0.30}{6} \times \{ 1.00 \times 9.40 + 1.80 \times 11.40 + (1.00 + 1.80) \cdot (9.40 + 11.40) \} = 7.406 \text{ m}^3$$

$$\text{BEAM TOTAL} = 18.772 \text{ m}^3$$

(2) COLUMN



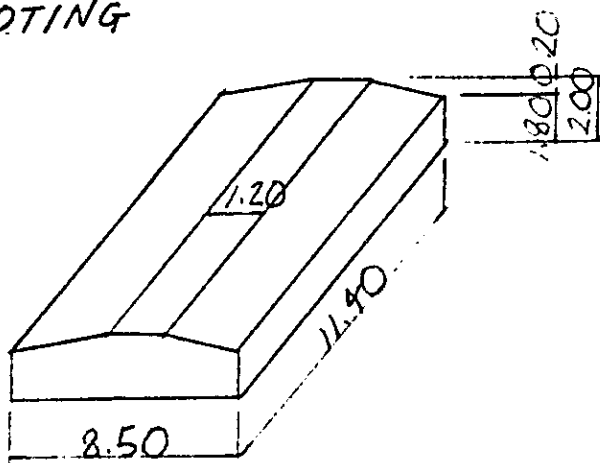
$$V_a = \frac{1}{4} \times \pi \times 1.00^2 \times 6.70 = 5.262 \text{ m}^3$$

$$V_b = 1.00 \times 8.40 \times 6.70 = 56.280$$

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$$\text{COLUMN TOTAL} = 61.542 \text{ m}^3$$

(3) FOOTING



$$V_a = \frac{1}{2} \times (1.20 + 8.50) \times 0.20 \times 11.40 = 11.058 \text{ m}^3$$

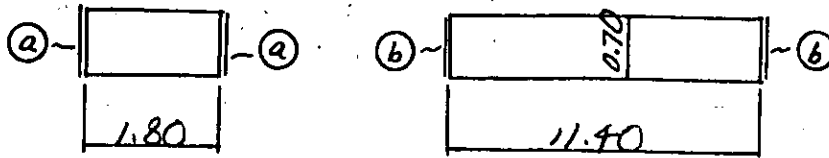
$$V_b = 8.50 \times 1.80 \times 11.40 = 174.420$$

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$$\text{FOOTING TOTAL} = 185.478 \text{ m}^3$$

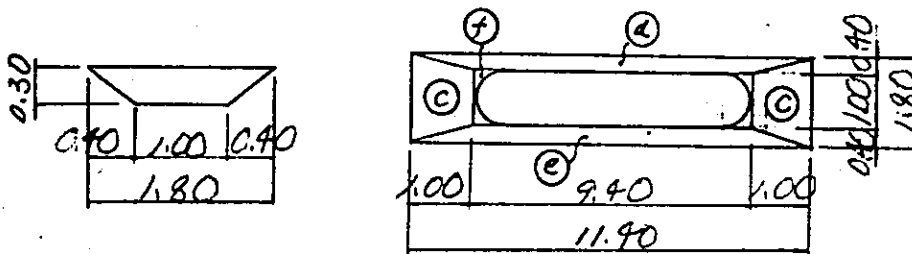
2) FORM AREA

(1) BEAM



$$A_a = 0.70 \times 11.40 \times 2 = 15.960 \text{ m}^2$$

$$A_b = 0.70 \times 1.80 \times 2 = 2.520$$



$$A_c = \frac{1}{2} \times (1.00 + 1.80) \times \sqrt{1.00^2 + 0.30^2} \times 2 = 2.923$$

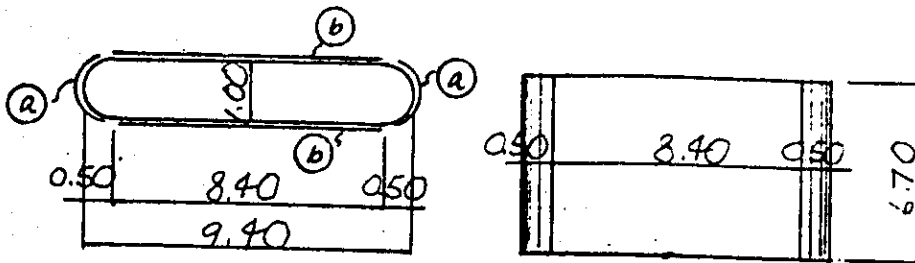
$$A_d = \frac{1}{2} \times (9.40 + 11.40) \times \sqrt{0.40^2 + 0.30^2} = 5.200$$

$$A_e = \text{''} = 5.200$$

$$A_f = (1 - \pi/4) \times 1.00^2 = 0.215$$

$$\text{BEAM TOTAL} = 32.018 \text{ m}^2$$

## (2) COLUMN



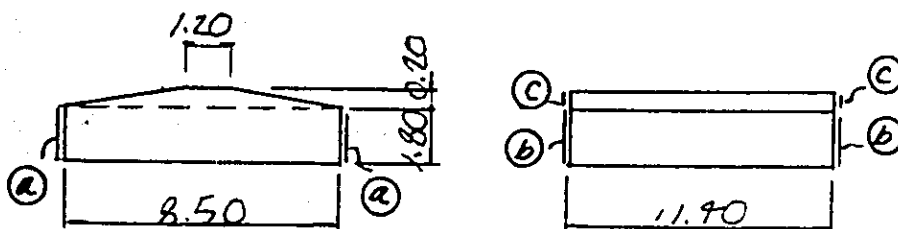
$$A_a = \pi \times 1.00 \times 6.70 = 21.049 \text{ m}^2$$

$$A_b = 8.40 \times 6.70 \times 2 = 112.560$$

COLUMN TOTAL =

133.609 m<sup>2</sup>

## (3) FOOTING



$$A_a = 1.80 \times 11.40 \times 2 = 41.040 \text{ m}^2$$

$$A_b = 1.80 \times 8.50 \times 2 = 30.600$$

$$A_c = \frac{1}{2} \times (1.20 + 8.50) \times 0.20 \times 2 = 1.940$$

FOOTING TOTAL =

73.580 m<sup>2</sup>

## 3) REINFORCING BAR

	V (m <sup>3</sup> )	R (kg/m <sup>3</sup> )	W = V · R (kg)
BEAM	18.8	82.971	1559.9
COLUMN	61.5	157.633	9694.4
FOOTING	185.5	65.383	12128.5

BEARING BASE      D16   489.6 kg

V — CONCRETE VOLUME

W — REINFORCING BAR

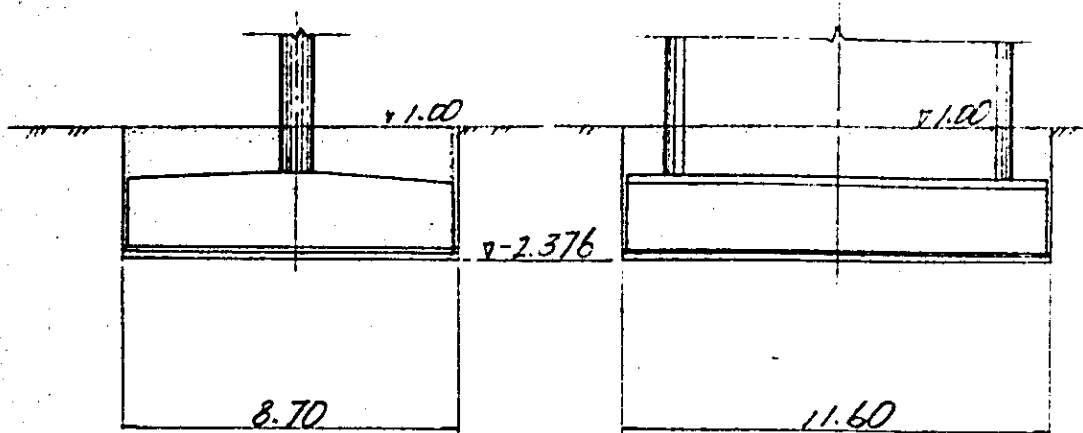
R =  $\frac{\text{REINFORCING BAR OF } P_{40}}{\text{CONCRETE VOLUME OF } P_{40}}$

$$R_B = \frac{1725.8}{20.8} = 82.971$$

$$R_C = \frac{9694.4}{61.5} = 157.633$$

$$R_F = \frac{12128.5}{185.5} = 65.383$$

## 4) EXCAVATION

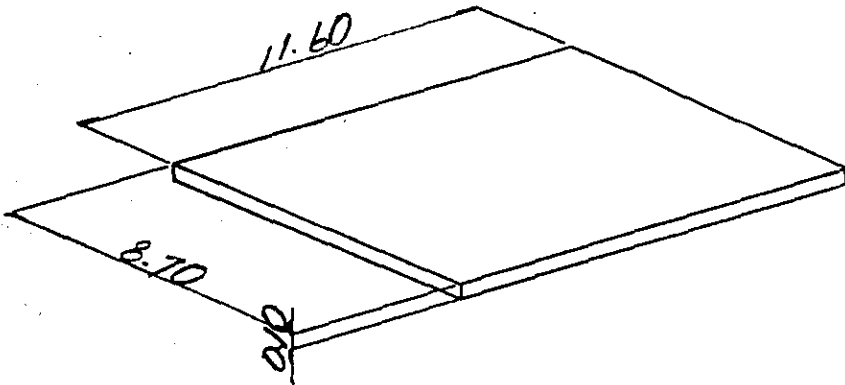


$$V_a = 8.70 \times 11.60 \times (1.00 + 2.376) = 340.706 \text{ m}^3$$

## 5) FOUNDATION MORTAR

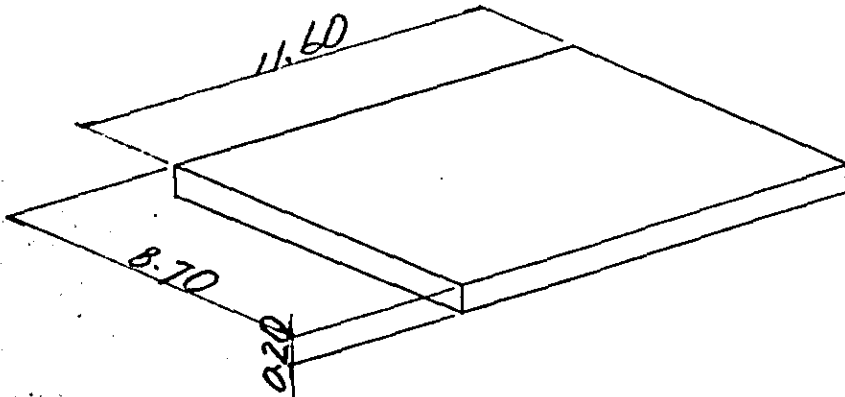
$$V_a = \frac{1}{2} \times (0.736 \times 0.536 + 0.70 \times 0.50) \times 0.018 \times 16 = 0.107 \text{ m}^3$$

## 6) LEVELING CONCRETE



$$V_a = 8.70 \times 11.60 \times 0.10 = 10.092 \text{ m}^3$$

## 7) AGGREGATE SUBBASE



$$V_a = 8.70 \times 11.60 \times 0.20 = 20.184 \text{ m}^3$$

## 8) PILE

$$\phi = 500$$

$$\text{TYPE-B } 13^m \times 72$$

51 PIER 50

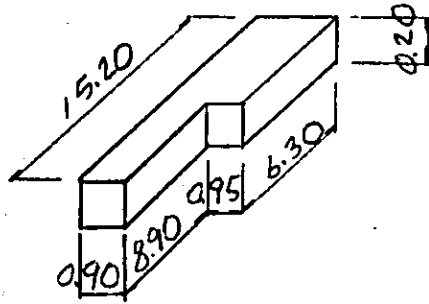
	CONCRETE VOLUME (m <sup>3</sup> )	FORM AREA (m <sup>2</sup> )	REINFORCING BAR (kg)	RATIO (kg/m <sup>3</sup> )
BEAM	29.8	48.3	2 542.7	85.3
COLUMN	87.7	170.7	13 022.7	148.5
TOTAL	117.5	219.0	15 565.4	132.5
FOOTING	134.3	62.3	7 660.9	57.0

	UNIT	QUANTITY	REMARKS
LEVELING CONCRETE	m <sup>3</sup>	8.2	CLASS F
AGGREGATE SUBBASE	m <sup>3</sup>	16.4	CLASS B-3
EXCAVATION	m <sup>3</sup>	341.8	
FOUNDATION MORTAR	m <sup>3</sup>	0.1	
PILE	m x NUMBER	13 x 45	Φ500-B

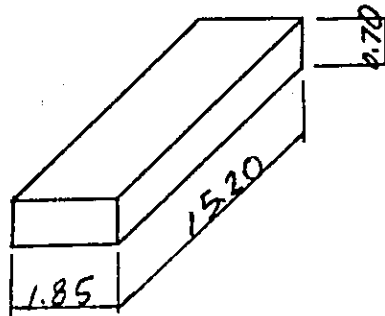


1) CONCRETE VOLUME

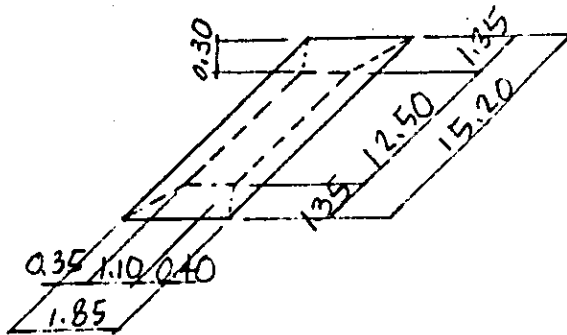
(1) BEAM



$$V_a = 0.20 \times (0.90 \times 8.90 + 1.85 \times 6.30) = 3.933 \text{ m}^3$$



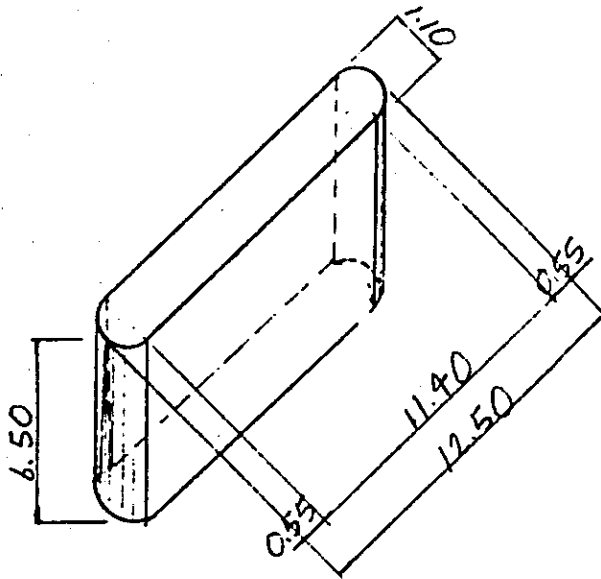
$$V_b = 1.85 \times 0.70 \times 15.20 = 19.684 \text{ m}^3$$



$$V_c = \frac{0.30}{6} \times \{ 1.10 \times 12.50 + 1.85 \times 15.20 + (1.10 + 1.85) \times (12.50 + 15.20) \} = 6.179 \text{ m}^3$$

BEAM TOTAL = 29.796 m<sup>3</sup>

## (2) COLUMN



$$V_a = \frac{1}{4} \times \pi \times 1.10^2 \times 6.50$$

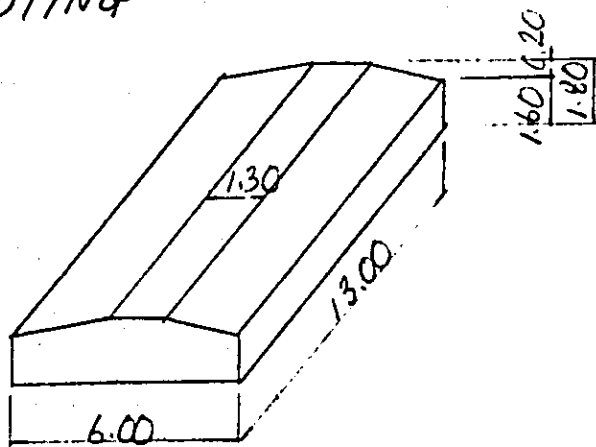
$$= 6.177 \text{ m}^3$$

$$V_b = 1.10 \times 11.40 \times 6.50$$

$$= 81.510 \text{ m}^3$$

$$\text{COLUMN TOTAL} = 87.687 \text{ m}^3$$

## (3) FOOTING



$$V_a = \frac{1}{2} \times (1.30 + 6.00) \times 0.20 \times 13.00$$

$$= 9.490 \text{ m}^3$$

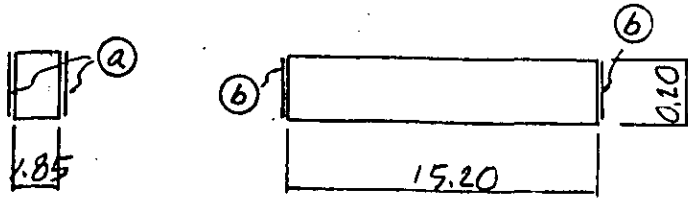
$$V_b = 6.00 \times 1.60 \times 13.00$$

$$= 124.800 \text{ m}^3$$

$$\text{FOOTING TOTAL} = 134.290 \text{ m}^3$$

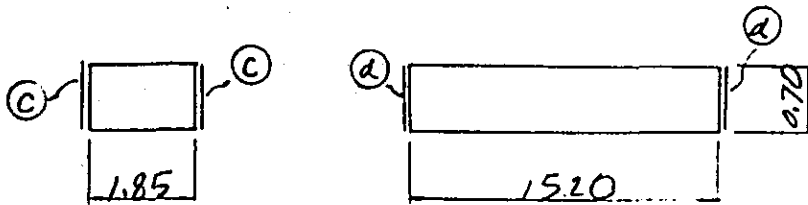
2) FORM AREA

(1) BEAM



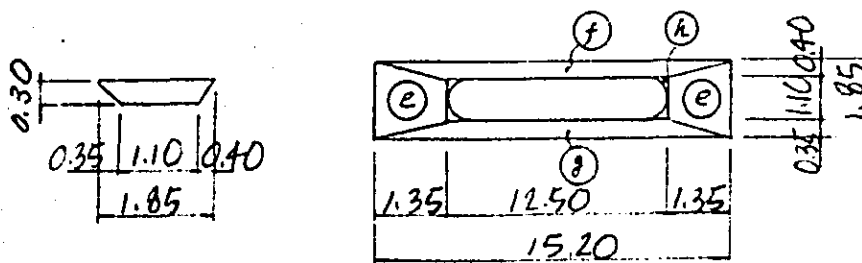
$$A_e = 0.20 \times 15.20 \times 2 = 6.080 \text{ m}^2$$

$$A_b = 0.20 \times 1.85 \times 2 = 0.740$$



$$A_c = 0.70 \times 15.20 \times 2 = 21.280$$

$$A_d = 0.70 \times 1.85 \times 2 = 2.590$$



$$A_e = \frac{1}{2} \times (1.10 + 1.85) \times \sqrt{1.35^2 + 0.30^2} \times 2 = 4.080$$

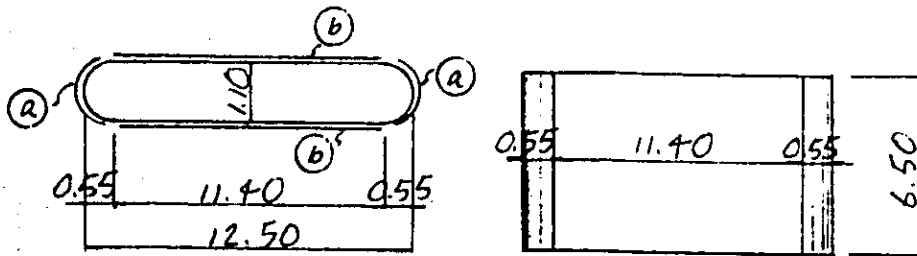
$$A_f = \frac{1}{2} \times (12.50 + 15.20) \times \sqrt{0.40^2 + 0.30^2} = 6.925$$

$$A_g = \frac{1}{2} \times (12.50 + 15.20) \times \sqrt{0.35^2 + 0.30^2} = 6.385$$

$$A_h = (1 - \pi/4) \times 1.10^2 = 0.260$$

BEAM TOTAL = 48.340 m<sup>2</sup>

## (2) COLUMN.

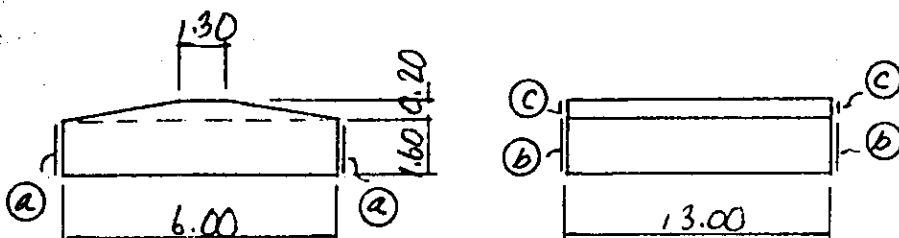


$$A_a = \pi \times 1.10 \times 6.50 = 22.462 \text{ m}^2$$

$$A_b = 11.40 \times 6.50 \times 2 = 148.200 \text{ m}^2$$

$$\text{COLUMN TOTAL} = 170.662 \text{ m}^2$$

## (3) FOOTING



$$A_a = 1.60 \times 13.00 \times 2 = 41.600 \text{ m}^2$$

$$A_b = 1.60 \times 6.00 \times 2 = 19.200 \text{ m}^2$$

$$A_c = \frac{1}{2} \times (1.30 + 6.00) \times 0.20 \times 2 = 1.460 \text{ m}^2$$

$$\text{FOOTING TOTAL} = 62.260 \text{ m}^2$$

## 3) REINFORCING BAR

	V (m <sup>3</sup> )	R (kg/m <sup>3</sup> )	W = V · R (kg)
BEAM	29.8	67.165	2001.5
COLUMN	87.7	148.491	13022.7
FOOTING	134.3	57.043	7660.9

BEARING BASE      016 541.2<sup>kg</sup>

V — CONCRETE VOLUME

W — REINFORCING BAR

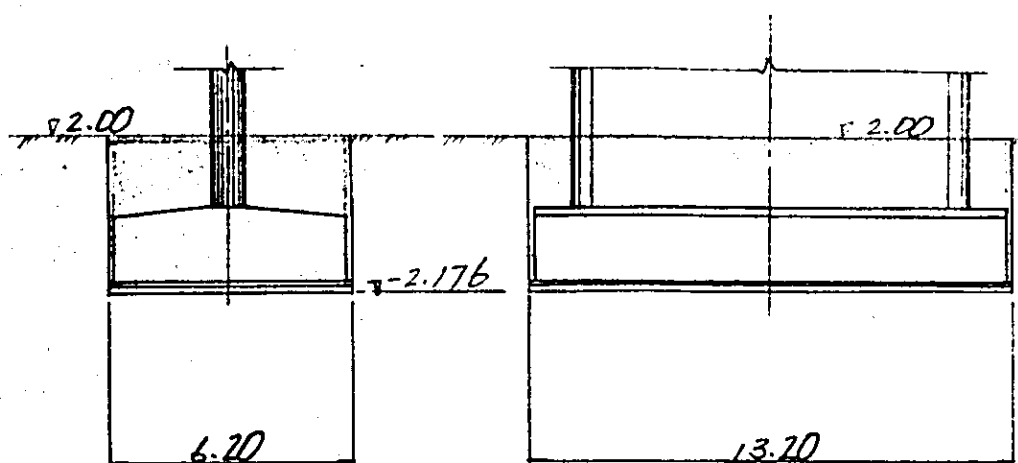
R =  $\frac{\text{REINFORCING BAR OF } \phi 48}{\text{CONCRETE VOLUME OF } \phi 48}$

$$R_B = \frac{1591.8}{23.7} = 67.165$$

$$R_C = \frac{11775.3}{79.3} = 148.491$$

$$R_F = \frac{7660.9}{134.3} = 57.043$$

## 4) EXCAVATION



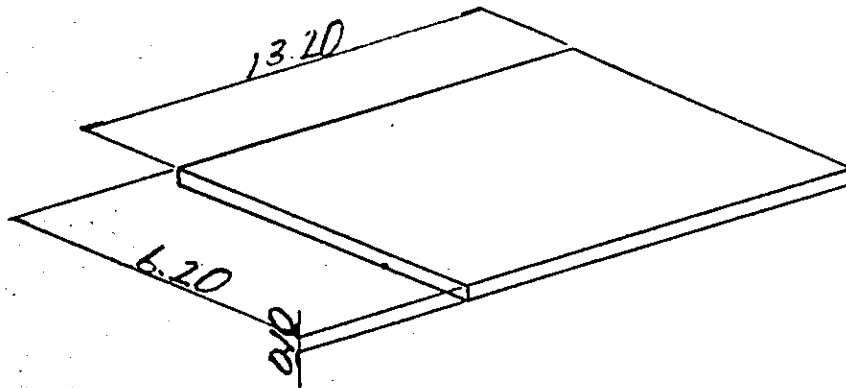
$$V_a = 6.20 \times 13.20 \times (2.00 + 2.176) = 341.767 \text{ m}^3$$

## 5) FOUNDATION MORTAR

$$V_a = \frac{1}{2} \times (0.736 \times 0.536 + 0.70 \times 0.50) \times 0.018 \times 12$$

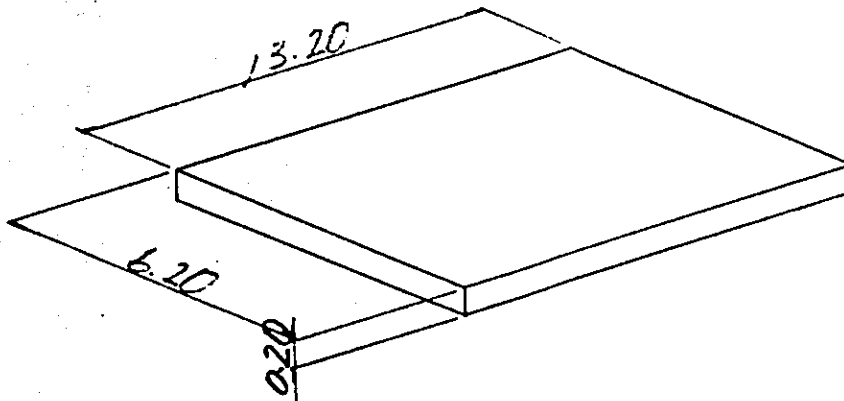
$$+ \frac{1}{2} \times (0.736 \times 0.688 + 0.70 \times 0.65) \times 0.018 \times 4 = 0.115 \text{ m}^3$$

6) LEVELING CONCRETE



$$V_a = 6.20 \times 13.20 \times 0.10 = 8.187 \text{ m}^3$$

7) AGGREGATE SUBBASE



$$V_a = 6.20 \times 13.20 \times 0.20 = 16.368 \text{ m}^3$$

8) PILE

$\phi = 500$

TYPE - B  $13^m \times 75$

