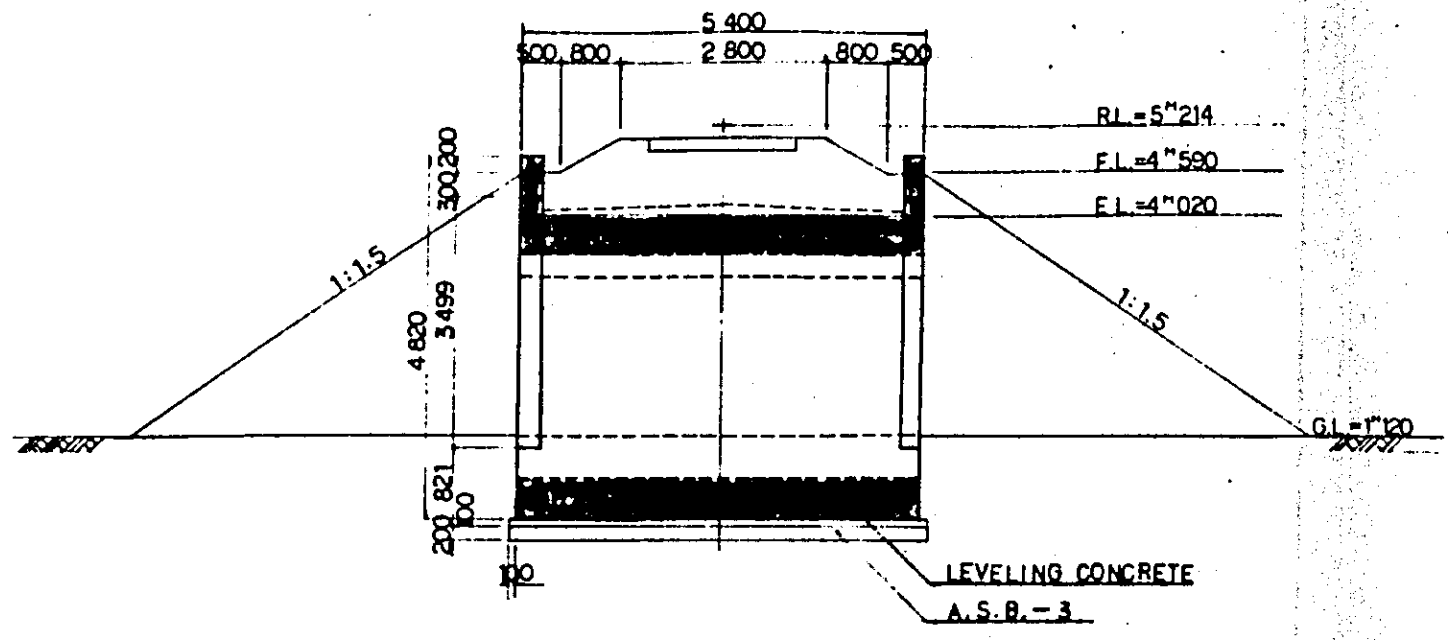
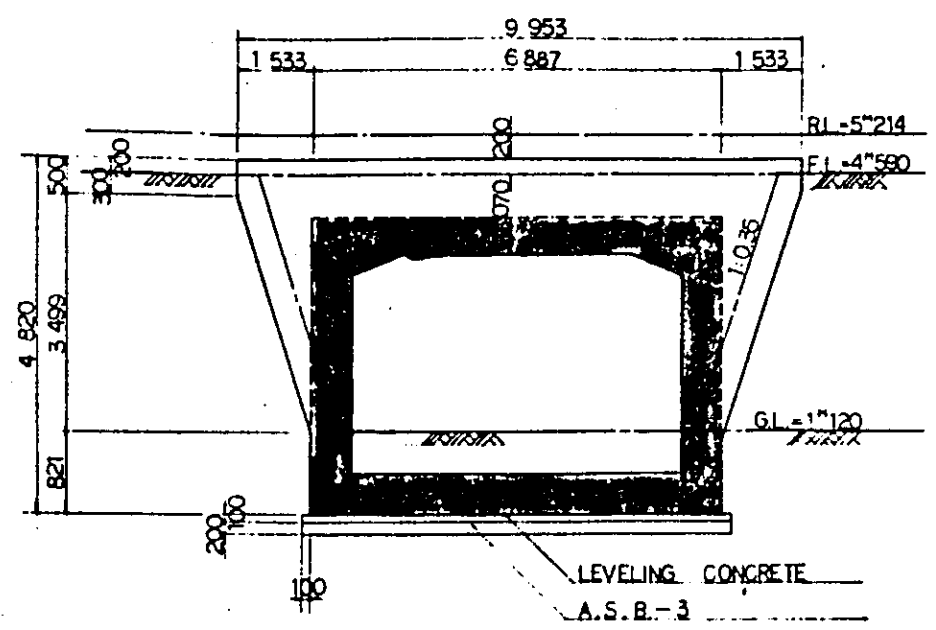


1 Configuration

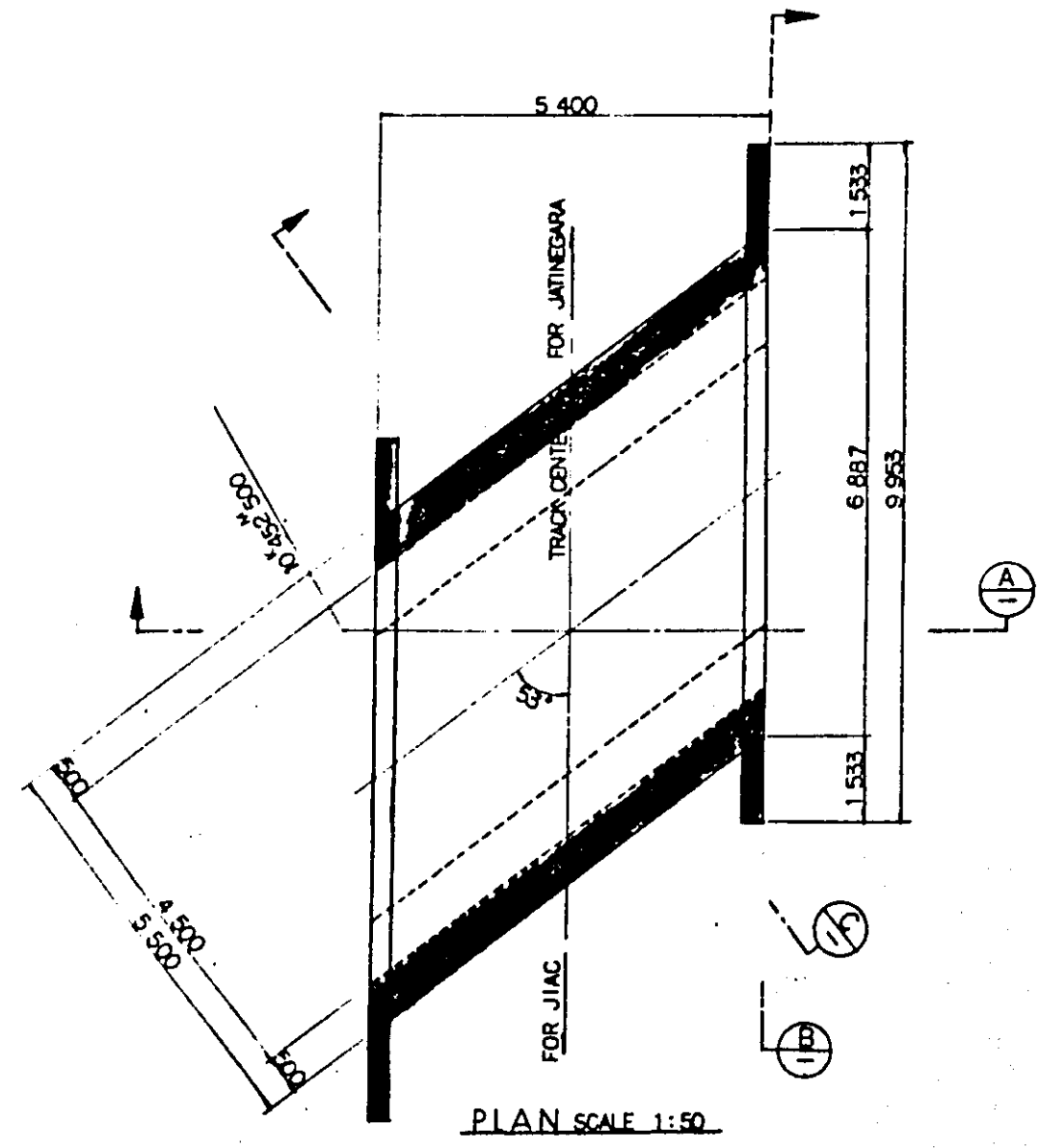


SECTION A
SCALE 1:50

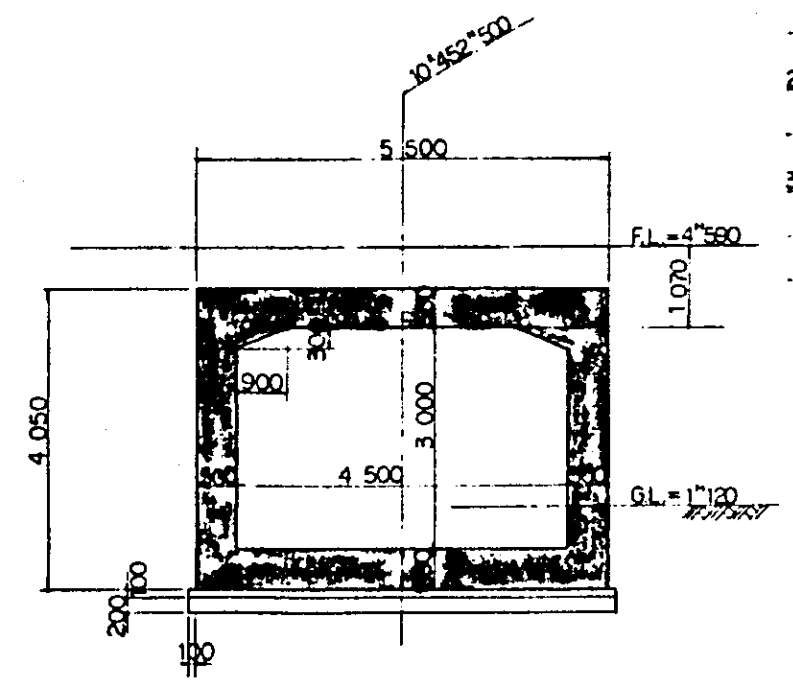


SECTION B
SCALE 1:50

- NOTES:
1. ALL DIMENSIONS ARE SHOWN IN MILLIMETERS UNLESS OTHERWISE INDICATED
 2. REFERENCE DRAWING FOR BAR ARRANGEMENT: CE-030
 3. GRADING CONCRETE SHALL BE SIMULTANEOUSLY PLACED WITH SLAB CONCRETE

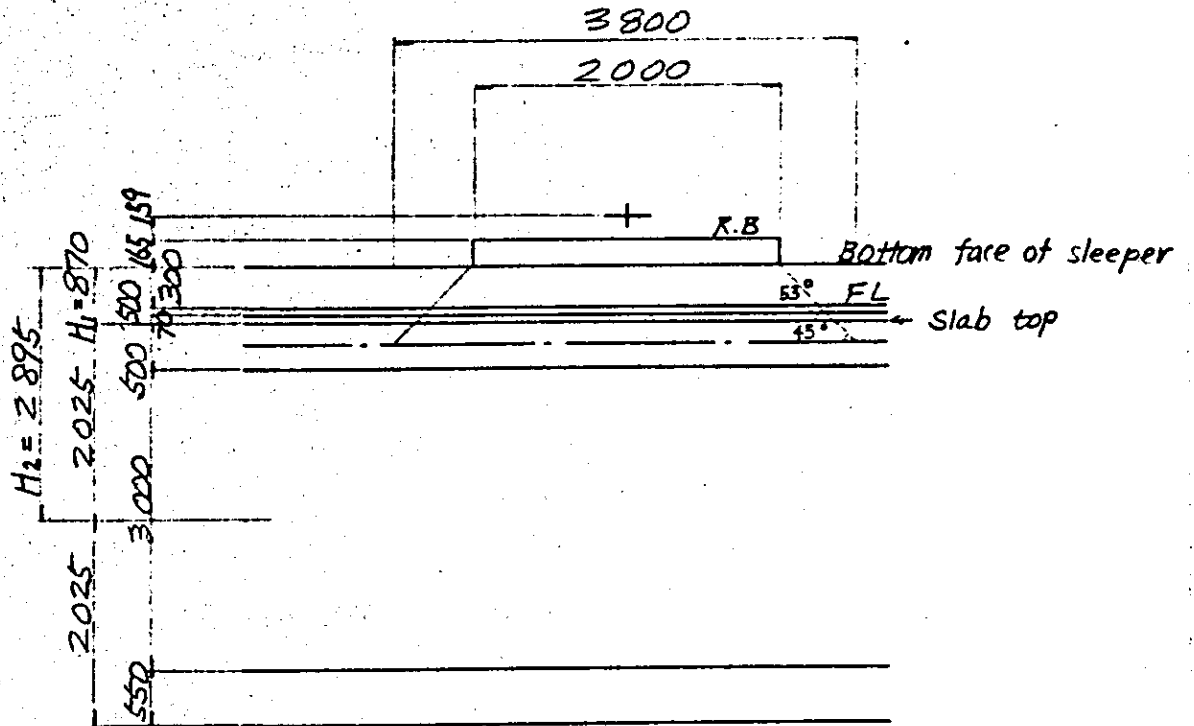


PLAN SCALE 1:50



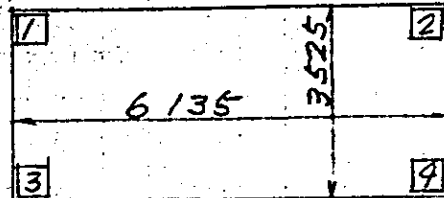
SECTION C
SCALE 1:50

2. Configuration and dimension: Surcharge earth load



3. cross section

1. Axis of Rahmen (Rigid frame)



2. Cross sectional area: Moment of inertia of the area

1) Top slab

$$A = 1.00 \times 0.50 = 0.500 \quad \text{m}^2$$

$$I = 1 / 12 \times 1.00 \times 0.50^3 = 0.01042 \quad \text{m}^4$$

2) Bottom slab

$$A = 1.00 \times 0.55 = 0.550 \quad \text{m}^2$$

$$I = 1 / 12 \times 1.00 \times 0.55^3 = 0.01386 \quad \text{m}^4$$

3) Side wall

$$A = 1.00 \times 0.50 = 0.500 \quad \text{m}^2$$

$$I = 1 / 12 \times 1.00 \times 0.50^3 = 0.01042 \quad \text{m}^4$$

4. Loads

Calculation is carried out based on the unit width of Rahmen (Rigid frame).

4.1 Dead load

Weight of track assembly

$$0.45 \text{ t / m} \times 3.80 \text{ m} = 0.12 \text{ t / m}^2$$

Track ballast

$$1.9 \text{ t / m}^2 \times 0.47 \text{ m} = 0.89 \text{ t / m}^2$$

(R.B. ~ F.L.)

Subgrade material

(upper layer) $1.9 \text{ t / m}^2 \times 0.50 \text{ m} = 0.95 \text{ t / m}^2$

Grading concrete $2.35 \text{ t / m}^2 \times 0.07 \text{ m} = 0.16 \text{ t / m}^2$

Weight of top slab $2.5 \text{ t / m}^2 \times 0.50 \text{ m} = 1.25 \text{ t / m}^2$

$$W_d = 3.37 \text{ t / m}^2$$

Weight of side wall

$$2.5 \text{ t / m}^2 \times 0.50 \text{ m} \times 3.525 = 4.41 \text{ t / m}$$

Weight of bottom slab

$$3.37 + 4.41 \times 2 \times 1 / 6.135 = 4.81 \text{ t / m}^2$$

4.2 Train load

Uniformly distributed load, equivalent to KS-16 loading

$\ell = 6.135 \text{ m}$ Surcharge earth (Sleeper bottom - Slab top)

is assumed as 0.87 m , then $P_m = 2.70 \text{ t / m}^2$

$$P_s = 2.70 \times 1.2 = 3.24 \text{ t / m}^2$$

4.3 Impact coefficient

$$i = i_0 \left\{ \frac{2.5 - H}{1.5} \right\}$$

$$= 0.47$$

$$H \leq 1.0 \text{ m} \quad i = i_0$$

$$H \geq 2.5 \text{ m} \quad i = 0$$

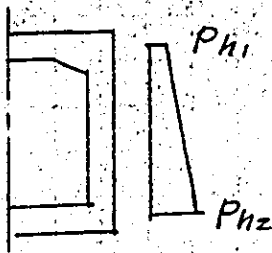
4.4 Earth pressure

1) Ordinary case, earth pressure due to dead load

Horizontal earth pressure acting at the depth of H

Bottom face of sleeper

$$P_h = K \cdot \gamma \cdot H$$



P_h : Horizontal earth pressure per unit area

γ : Unit weight of earth = 1.8 t / m³

H : Depth of earth (depth above slab top is the equivalent value)

K : Coefficient of static earth pressure = 0.5

$$K = 0.5$$

$$H_1 = 1 / 1.8 \times (3.37 - 1.25 - 0.16) = 1.09 \text{ m}$$

$$P_{h1} = K \cdot \gamma \cdot H_1 = 0.5 \times 1.8 \times (1.09 + 0.07 + 0.50 / 2) = 1.27 \text{ t / m}^2$$

$$P_{h2} = P_{h1} + K \gamma H_2 = 1.27 + 0.5 \times 1.8 \times 3.525 = 4.44 \text{ t / m}^2$$

$$K = 0.3$$

$$P_{h1} = 0.3 \times 1.8 \times (1.09 + 0.07 + 0.50 / 2) = 0.76 \text{ t / m}^2$$

$$P_{h2} = 0.76 + 0.3 \times 1.8 \times 3.525 = 2.66 \text{ t / m}^2$$

2) Horizontal earth pressure caused by train load

$$P_{he} = K \cdot P_{ve}$$

P_{he} : Effect of train load acting at the side.

(uniformly distributed load) (t / m²)

P_{ve} : Effect of train load in vertical direction

at the depth of the level of culvert

center (t / m²)

K : Coefficient of horizontal earth pressure

(generally assumed as 0.5)

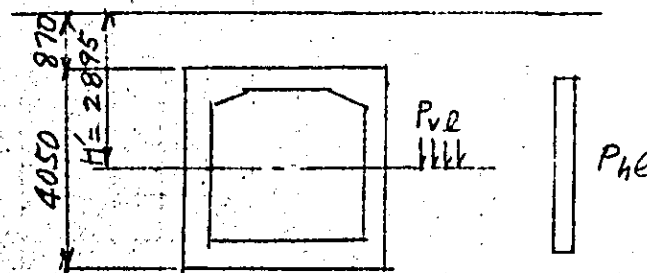
H' : Depth of the level of culvert center (m)

$$P_{he} = K \cdot P_{ve}$$

$$= 0.5 \times 1.90$$

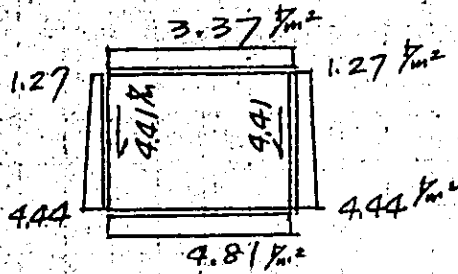
$$= 0.95 \text{ t / m}^2$$

Bottom face of sleeper

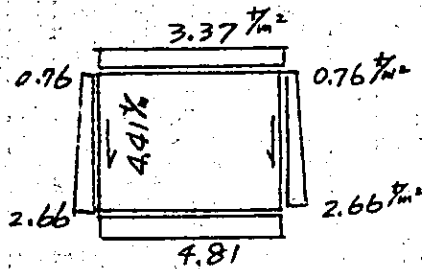


4.5 Loading condition

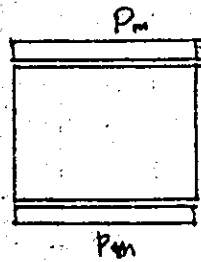
case1 Dead load + Earth pressure (0.5)



case2 Dead load + Earth pressure (0.3)



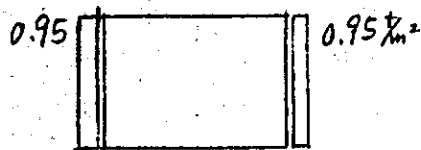
case3 Live load impact



$$P_m = 2.70 \times 1.47 = 3.97 \text{ t / m}^2$$

$$P_s = 3.24 \times 1.47 = 4.76 \text{ "}$$

case4 Earth pressure due to live load



Combination of loads

Case	Combination of loads
①	Dead load + Surcharge earth load + Earth pressure (K = 0.5)
②	Dead load + Surcharge earth load + Earth pressure (K = 0.3)
③	live load + Impact
④	live load, Earth pressure

Difine	Case
①	① ②
②	③
③	④

Combine	Difine	Coefficient of increased load
	①	1.00
	① + ②	1.00
	① + ③	1.00
	① + ② + ③	1.00

Pick up	Combine
1	1, 2, 3, 4

REACTION

LOAD	SUPPORT	X (TON)	Y (TON)	Z (TON-M)	SUPPORT	X (TON)	Y (TON)	Z (TON-M)
LOAD - 1	CASE 1 (SHI+DO (0.5))				CASE 2 (SHI+DO (0.3))			
	3	0.000	-0.001	0.000	3	0.000	-0.001	0.000
	4	0.000	-0.001	0.000	4	0.000	-0.001	0.000
LOAD - 3	CASE 3 (KATSU+SYOU)				CASE 4 (KATSU+DO)			
	3	0.000	0.000	0.000	3	0.000	0.000	0.000
	4	0.000	0.000	0.000	4	0.000	0.000	0.000

055

TITLE=C22

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DEFLECTION

LOAD	CASE	JOINT	X (MM)	Y (MM)	Z (MMRAD)	LOAD	CASE	JOINT	X (MM)	Y (MM)	Z (MMRAD)
LOAD - 1	CASE 1 (SHI+DO (0.5))	1	-0.005	-0.031	-0.331	LOAD - 2	CASE 2 (SHI+DO (0.3))	1	-0.004	-0.031	-0.375
		2	-0.022	-0.031	0.331			2	-0.013	-0.031	0.375
		3	0.000	0.000	0.360			3	0.000	0.000	0.405
		4	-0.027	0.000	-0.350			4	-0.017	0.000	-0.405
LOAD - 3	CASE 3 (KATSU+SYOU)	1	0.001	-0.032	-0.480	LOAD - 4	CASE 4 (KATSUDO)	1	0.000	0.000	0.038
		2	0.000	-0.032	0.450			2	-0.007	0.000	-0.038
		3	0.000	0.000	0.456			3	0.000	0.000	-0.036
		4	0.001	0.000	-0.456			4	-0.007	0.000	0.036

LOAD 1 CASE 1 (SHI+00 (0.5))
LOAD 2 CASE 2 (SHI+00 (0.3))

MEMBER	1 (1 - 2) G =	(M)	(N)	(Q)	(M)	(Q)	(N)
ITAN	0.000	-7.532	-7.532	10.337	-3.568	-3.568	-3.568
1	0.250	-5.053	-7.532	9.495	-3.568	-3.568	-3.568
2	0.650	-1.525	-3.667	8.147	-3.568	-3.568	-3.568
3	0.900	0.407	-1.525	7.304	-3.568	-3.568	-3.568
4	1.150	2.128	0.407	6.462	-3.568	-3.568	-3.568
5	2.067	6.636	5.690	3.370	-3.568	-3.568	-3.568
6	2.567	7.902	7.375	0.000	-3.568	-3.568	-3.568
7	3.067	8.323	8.323	0.000	-3.568	-3.568	-3.568
JTAN	6.135	-7.532	-7.532	-10.337	-3.568	-3.568	-3.568
MAX	3.067	9.323		0.000	-3.568	-3.568	-3.568

MEMBER	2 (3 - 4) G =	(M)	(N)	(Q)	(M)	(Q)	(N)
ITAN	0.000	9.411	9.411	-13.437	-6.496	-6.496	-6.496
1	0.250	6.175	6.175	-12.397	-6.496	-6.496	-6.496
2	0.525	2.932	2.932	-11.187	-6.496	-6.496	-6.496
3	1.568	-6.340	-6.340	-6.600	-6.496	-6.496	-6.496
4	2.067	-9.090	-9.090	-4.400	-6.496	-6.496	-6.496
5	2.567	-10.740	-10.740	-2.200	-6.496	-6.496	-6.496
6	3.067	-11.290	-11.290	0.000	-6.496	-6.496	-6.496
JTAN	6.135	9.411	9.411	13.437	-6.496	-6.496	-6.496
MAX	3.067	-11.290		0.000	-6.496	-6.496	-6.496

MEMBER	3 (1 - 3) G =	(M)	(N)	(Q)	(M)	(Q)	(N)
ITAN	0.000	7.532	7.532	-3.568	-10.337	-10.337	-10.337
1	0.250	6.682	6.682	-3.222	-10.561	-10.561	-10.561
2	0.550	5.787	5.787	-2.733	-10.330	-10.330	-10.330
3	0.650	5.523	5.523	-2.552	-10.920	-10.920	-10.920
4	1.300	4.297	4.297	-1.157	-11.502	-11.502	-11.502
5	1.800	4.042	4.042	0.175	-11.950	-11.950	-11.950
6	2.300	4.509	4.509	1.732	-12.398	-12.398	-12.398
7	3.000	6.591	6.591	4.289	-13.025	-13.025	-13.025
8	3.250	7.798	7.798	5.309	-13.249	-13.249	-13.249
JTAN	3.525	9.411	9.411	6.496	-13.496	-13.496	-13.496
MAX	1.800	4.042		0.175	-11.950	-11.950	-11.950

MEMBER	1 (1 - 2) G =	(M)	(N)	(Q)	(M)	(Q)	(N)
ITAN	0.000	-7.131	-7.131	10.337	-7.131	-7.131	-7.131
1	0.250	-4.652	-7.131	9.495	-7.131	-7.131	-7.131
2	0.650	-1.124	-3.266	8.147	-3.266	-3.266	-3.266
3	0.900	0.807	-1.124	7.304	-1.124	-1.124	-1.124
4	1.150	2.520	0.807	6.462	0.807	0.807	0.807
5	2.067	7.039	6.091	3.370	6.091	6.091	6.091
6	2.567	8.302	7.776	0.000	7.776	7.776	7.776
7	3.067	8.724	8.724	0.000	8.724	8.724	8.724
JTAN	6.135	-7.131	-7.131	-10.337	-7.131	-7.131	-7.131
MAX	3.067	8.724		0.000	-7.131	-7.131	-7.131

MEMBER	2 (3 - 4) G =	(M)	(N)	(Q)	(M)	(Q)	(N)
ITAN	0.000	8.860	8.860	-13.497	-4.062	-4.062	-4.062
1	0.250	5.623	5.623	-12.397	-4.062	-4.062	-4.062
2	0.525	2.380	2.380	-11.187	-4.062	-4.062	-4.062
3	1.568	-6.891	-6.891	-6.600	-4.062	-4.062	-4.062
4	2.067	-9.641	-9.641	-4.400	-4.062	-4.062	-4.062
5	2.567	-11.291	-11.291	-2.200	-4.062	-4.062	-4.062
6	3.067	-11.841	-11.841	0.000	-4.062	-4.062	-4.062
JTAN	6.135	8.860	8.860	13.497	-4.062	-4.062	-4.062
MAX	3.067	-11.841		0.000	-4.062	-4.062	-4.062

MEMBER	3 (1 - 3) G =	(M)	(N)	(Q)	(M)	(Q)	(N)
ITAN	0.000	7.131	7.131	-1.965	-10.337	-10.337	-10.337
1	0.250	6.665	6.665	-1.759	-10.561	-10.561	-10.561
2	0.550	6.180	6.180	-1.466	-10.830	-10.830	-10.830
3	0.650	6.039	6.039	-1.358	-10.920	-10.920	-10.920
4	1.300	5.416	5.416	-0.522	-11.502	-11.502	-11.502
5	1.800	5.349	5.349	0.276	-11.950	-11.950	-11.950
6	2.300	5.714	5.714	1.208	-12.398	-12.398	-12.398
7	3.000	7.081	7.081	2.740	-13.025	-13.025	-13.025
8	3.250	7.841	7.841	3.351	-13.249	-13.249	-13.249
JTAN	3.525	8.860	8.860	4.062	-13.496	-13.496	-13.496
MAX	1.800	5.330		-0.113	-11.741	-11.741	-11.741

TITLE..022

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LOAD 1 CASE 1 (SHI*00 (0.5))

LOAD 2 CASE 2 (SHI*00 (J.J))

-----L-----M-----N-----Q-----

== MEMBER 4 (2 - 4) C ==

MEMBER	4	2	4	C	==
JIAN	0.000	-7.532	3.566	-10.337	1.965
1	0.250	-6.682	3.222	-10.561	1.759
2	0.550	-5.787	2.733	-10.830	1.466
3	0.650	-5.523	2.552	-10.920	1.358
4	1.300	-4.297	1.157	-11.502	0.522
5	1.800	-4.042	-0.175	-11.950	-0.276
6	2.300	-4.509	-1.732	-12.398	-1.208
7	3.000	-6.591	-4.289	-13.025	-2.740
8	3.250	-7.790	-5.309	-13.249	-3.351
JIAN	3.525	-9.411	-6.496	-13.496	-4.062
MAX	1.800	-4.042	-0.175	-11.950	0.113

-----L-----M-----N-----Q-----

== MEMBER 4 (2 - 4) C ==

MEMBER	4	2	4	C	==
JIAN	0.000	-7.131	3.566	-10.337	1.965
1	0.250	-6.665	3.222	-10.561	1.759
2	0.550	-6.180	2.733	-10.830	1.466
3	0.650	-6.039	2.552	-10.920	1.358
4	1.300	-5.416	1.157	-11.502	0.522
5	1.800	-5.349	-0.276	-11.950	-0.276
6	2.300	-5.714	-1.732	-12.398	-1.208
7	3.000	-7.081	-4.289	-13.025	-2.740
8	3.250	-7.841	-5.309	-13.249	-3.351
JIAN	3.525	-8.860	-6.496	-13.496	-4.062
MAX	1.567	-5.330	-0.113	-11.741	0.113

LOAD 3 CASE 3 (KATSU+SYOU)

LOAD 4 CASE 4 (KATSUDO)

MEMBER	1	2	G	M	N	0	M	N	0
ITAN	0.000	-8.046	14.601	-0.327	-0.327	0.000	-0.347	0.000	-1.648
1	0.250	-9.046	13.411	-0.327	-0.327	0.250	-0.347	0.000	-1.648
2	0.650	-0.969	11.507	-0.327	-0.327	0.650	-0.347	0.000	-1.648
3	0.900	1.306	10.317	-0.327	-0.327	0.900	-0.347	0.000	-1.648
4	1.150	3.334	9.127	-0.327	-0.327	1.150	-0.347	0.000	-1.648
5	2.067	9.647	4.760	-0.327	-0.327	2.067	-0.347	0.000	-1.648
6	2.567	10.136	3.516	-0.327	-0.327	2.567	-0.347	0.000	-1.648
7	3.067	10.632	0.000	-0.327	-0.327	3.067	-0.347	0.000	-1.648
JTAN	6.135	-8.046	-14.601	-0.327	-0.327	6.135	-0.347	0.000	-1.648
MAX	3.067	10.632	0.000	-0.327	-0.327				

MEMBER	2	3	4	G	M	N	0	M	N	0
ITAN	0.000	6.894	-14.601	0.327	0.327	0.000	0.440	0.000	-1.701	
1	0.250	3.974	-13.411	0.327	0.327	0.250	0.440	0.000	-1.701	
2	0.525	1.048	-12.102	0.327	0.327	0.525	0.440	0.000	-1.701	
3	1.568	-7.317	-7.140	0.327	0.327	1.568	0.440	0.000	-1.701	
4	2.067	-9.799	-4.760	0.327	0.327	2.067	0.440	0.000	-1.701	
5	2.567	-11.287	-2.330	0.327	0.327	2.567	0.440	0.000	-1.701	
6	3.067	-11.784	0.000	0.327	0.327	3.067	0.440	0.000	-1.701	
JTAN	6.135	6.894	14.601	0.327	0.327	6.135	0.440	0.000	-1.701	
MAX	3.067	-11.794	0.000	0.327	0.327					

MEMBER	3	1	3	J	C	M	N	0	M	N	0
ITAN	0.000	8.046	-12.178	0.347	0.347	0.000	0.347	0.000	-1.648		
1	0.250	7.964	-12.178	0.347	0.347	0.250	-0.035	0.000	-1.648		
2	0.550	7.266	-12.178	0.347	0.347	0.550	-0.415	0.000	-1.648		
3	0.650	7.834	-12.178	0.347	0.347	0.650	-0.523	0.000	-1.648		
4	1.300	7.621	-12.178	0.347	0.347	1.300	-0.992	0.000	-1.648		
5	1.800	7.458	-12.178	0.347	0.347	1.800	-1.080	0.000	-1.648		
6	2.300	7.295	-12.178	0.347	0.347	2.300	0.537	0.000	-1.648		
7	3.000	7.066	-12.178	0.347	0.347	3.000	-0.322	0.000	-1.648		
8	3.250	6.984	-12.178	0.347	0.347	3.250	0.008	0.000	-1.648		
JTAN	3.525	6.894	-12.178	0.347	0.347	3.525	0.440	0.000	-1.648		
MAX	1.800	-1.060	-1.060	0.347	0.347						

TITLE..C22

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LOAD 3 CASE 3 (KATSU+SYOU)

LOAD 4 CASE 4 (KATSUDO)

-----L-----M-----Q-----N-----

= MEMBER 4 (2 - 4) C = =

MEMBER	4	2	4	C	=	Q	N
ITAN	0.000	-8.346	0.327	-12.178			
1	0.250	-7.964	0.327	-12.178			
2	0.550	-7.866	0.327	-12.178			
3	0.650	-7.834	0.327	-12.178			
4	1.300	-7.621	0.327	-12.178			
5	1.800	-7.458	0.327	-12.178			
6	2.300	-7.295	0.327	-12.178			
7	3.000	-7.066	0.327	-12.178			
8	3.250	-6.984	0.327	-12.178			
JTAN	3.525	-6.594	0.327	-12.178			

-----L-----M-----Q-----N-----

= MEMBER 4 (2 - 4) C = =

MEMBER	4	2	4	C	=	Q	N
ITAN	0.000	-0.347	1.648	0.000			
1	0.250	0.035	1.411	0.000			
2	0.550	0.415	1.126	0.000			
3	0.650	0.523	1.031	0.000			
4	1.300	0.992	0.413	0.000			
5	1.800	1.080	-0.062	0.000			
6	2.300	0.931	-0.537	0.000			
7	3.000	0.322	-1.202	0.000			
8	3.250	-0.008	-1.439	0.000			
JTAN	3.525	-0.440	-1.701	0.000			
MAX	1.800	1.080	-0.062	0.000			

FILE..C22

COMBINE 1

		MOMENT		MINIMUM		MAXIMUM		CASE	
		M	Q	N	N	M	Q	N	N
= MEMBER 1 (1 - 2) C =									
ITAN	0.000	-7.131	10.337	-1.965	(2)	-7.532	10.337	-3.568	(1)
1	0.250	-4.652	9.495	-1.965	(2)	-5.053	9.495	-3.568	(1)
2	0.650	-1.124	8.147	-1.965	(2)	-1.525	8.147	-3.568	(1)
3	0.900	0.807	7.304	-1.965	(2)	0.407	7.304	-3.568	(1)
4	1.150	2.528	6.462	-1.965	(2)	2.128	6.462	-3.568	(1)
5	2.067	7.039	3.370	-1.965	(2)	6.638	3.370	-3.568	(1)
6	2.567	8.302	1.685	-1.965	(2)	7.902	1.685	-3.568	(1)
7	3.067	8.724	0.000	-1.965	(2)	8.323	0.000	-3.568	(1)
JTAN	6.135	-7.131	-10.337	-1.965	(2)	-7.532	-10.337	-3.568	(1)
= MEMBER 2 (3 - 4) C =									
ITAN	0.000	9.411	-13.497	-6.496	(1)	8.860	-13.497	-4.062	(2)
1	0.250	6.175	-12.397	-6.496	(1)	5.623	-12.397	-4.062	(2)
2	0.525	2.932	-11.197	-6.496	(1)	2.380	-11.187	-4.062	(2)
3	1.568	-6.340	-6.600	-6.496	(1)	-6.891	-6.600	-4.062	(2)
4	2.067	-9.090	-4.400	-6.496	(1)	-9.641	-4.400	-4.062	(2)
5	2.567	-10.740	-2.200	-6.496	(1)	-11.291	-2.200	-4.062	(2)
6	3.067	-11.290	0.000	-6.496	(1)	-11.841	0.000	-4.062	(2)
JTAN	6.135	9.411	13.497	-6.496	(1)	8.860	13.497	-4.062	(2)
= MEMBER 3 (1 - 3) C =									
ITAN	0.000	7.532	-3.566	-10.337	(1)	7.131	-3.566	-10.337	(2)
1	0.250	6.682	-3.222	-10.561	(1)	6.665	-1.759	-10.561	(2)
2	0.550	6.180	-1.466	-10.830	(2)	5.787	-2.733	-10.830	(1)
3	0.650	6.039	-1.358	-10.920	(2)	5.523	-2.552	-10.920	(1)
4	1.300	5.416	-0.522	-11.502	(2)	4.297	-1.157	-11.502	(1)
5	1.800	5.349	0.276	-11.950	(2)	4.042	0.175	-11.950	(1)
6	2.300	5.714	1.208	-12.398	(2)	4.509	1.732	-12.398	(1)
7	3.000	7.081	2.740	-13.025	(2)	6.591	4.239	-13.025	(1)
8	3.250	7.841	3.351	-13.249	(2)	7.790	5.309	-13.249	(1)
JTAN	3.525	9.411	6.496	-13.496	(1)	8.860	4.062	-13.496	(2)
= MEMBER 4 (2 - 4) C =									
ITAN	0.000	-7.131	1.955	-10.337	(2)	-7.532	3.568	-10.337	(1)
1	0.250	-6.665	1.759	-10.561	(2)	-6.682	3.222	-10.561	(1)
2	0.550	-5.787	2.733	-10.830	(1)	-6.180	1.466	-10.830	(2)
3	0.650	-5.523	2.552	-10.920	(1)	-6.039	1.358	-10.920	(2)
4	1.300	-4.297	1.157	-11.502	(1)	-5.416	0.522	-11.502	(2)
5	1.800	-4.042	-0.175	-11.950	(1)	-5.349	-0.276	-11.950	(2)
6	2.300	-4.509	-1.732	-12.398	(1)	-5.714	-1.208	-12.398	(2)
7	3.000	-6.591	-4.239	-13.025	(1)	-7.381	-2.740	-13.025	(2)
8	3.250	-7.790	-5.309	-13.249	(1)	-7.341	-3.351	-13.249	(2)
JTAN	3.525	-8.860	-4.062	-13.496	(1)	-9.411	-6.496	-13.496	(2)

COMBINE 1

		SHEAR		MAXIMUM		SHEAR		MINIMUM		CASE			
		L	M	N	C	L	M	N	C	L	M	N	C
= MEMBER 1 (1 - 2) C =													
ITAN	0.000	-7.532	10.337	-3.568	(1)	-7.532	10.337	-3.568	(1)				
1	0.250	-5.053	9.495	-3.568	(1)	-5.053	9.495	-3.568	(1)				
2	0.650	-1.525	8.147	-3.568	(1)	-1.525	8.147	-3.568	(1)				
3	0.900	0.407	7.304	-3.568	(1)	0.407	7.304	-3.568	(1)				
4	1.150	2.128	6.462	-3.568	(1)	2.128	6.462	-3.568	(1)				
5	2.067	6.638	3.370	-3.568	(1)	6.638	3.370	-3.568	(1)				
6	2.567	7.902	1.685	-3.568	(1)	7.902	1.685	-3.568	(1)				
7	3.067	8.323	0.000	-3.568	(1)	8.323	0.000	-3.568	(1)				
JIAN	6.135	-7.532	-10.337	-3.568	(1)	-7.532	-10.337	-3.568	(1)				
= MEMBER 2 (3 - 4) C =													
ITAN	0.000	9.411	-13.497	-6.496	(1)	9.411	-13.497	-6.496	(1)				
1	0.250	6.175	-12.397	-6.496	(1)	6.175	-12.397	-6.496	(1)				
2	0.525	2.932	-11.187	-6.496	(1)	2.932	-11.187	-6.496	(1)				
3	1.568	-6.340	-6.600	-6.496	(1)	-6.340	-6.600	-6.496	(1)				
4	2.067	-9.090	-4.400	-6.496	(1)	-9.090	-4.400	-6.496	(1)				
5	2.567	-10.740	-2.200	-6.496	(1)	-10.740	-2.200	-6.496	(1)				
6	3.067	-11.290	0.000	-6.496	(1)	-11.290	0.000	-6.496	(1)				
JIAN	6.135	9.411	13.497	-6.496	(1)	9.411	13.497	-6.496	(1)				
= MEMBER 3 (1 - 3) C =													
ITAN	0.000	7.131	-1.965	-10.337	(2)	7.131	-1.965	-10.337	(1)				
1	0.250	6.665	-1.759	-10.561	(2)	6.665	-1.759	-10.561	(1)				
2	0.550	6.180	-1.466	-10.830	(2)	6.180	-1.466	-10.830	(1)				
3	0.650	6.039	-1.352	-10.920	(2)	6.039	-1.352	-10.920	(1)				
4	1.300	5.416	-0.922	-11.502	(2)	5.416	-0.922	-11.502	(1)				
5	1.800	5.349	0.276	-11.950	(2)	5.349	0.276	-11.950	(1)				
6	2.300	4.509	1.732	-12.398	(1)	4.509	1.732	-12.398	(2)				
7	3.000	6.591	4.289	-13.025	(1)	6.591	4.289	-13.025	(2)				
8	3.250	7.790	5.309	-13.249	(1)	7.790	5.309	-13.249	(2)				
JIAN	3.525	9.411	6.496	-13.496	(1)	9.411	6.496	-13.496	(2)				
= MEMBER 4 (2 - 4) C =													
ITAN	0.000	-7.532	3.568	-10.337	(1)	-7.532	3.568	-10.337	(2)				
1	0.250	-6.665	3.222	-10.561	(1)	-6.665	3.222	-10.561	(2)				
2	0.550	-5.787	2.733	-10.830	(1)	-5.787	2.733	-10.830	(2)				
3	0.650	-5.523	2.552	-10.920	(1)	-5.523	2.552	-10.920	(2)				
4	1.300	-4.297	1.157	-11.502	(1)	-4.297	1.157	-11.502	(2)				
5	1.800	-4.042	-0.175	-11.950	(1)	-4.042	-0.175	-11.950	(2)				
6	2.300	-5.714	1.208	-12.398	(1)	-5.714	1.208	-12.398	(2)				
7	3.000	-7.081	2.740	-13.025	(1)	-7.081	2.740	-13.025	(2)				
8	3.250	-7.841	3.351	-13.249	(1)	-7.841	3.351	-13.249	(2)				
JIAN	3.525	-8.860	4.062	-13.496	(1)	-8.860	4.062	-13.496	(2)				

TITLE..G22

COMBINE 1

		AXIAL		MINIMUM		CASE	
		H	N	H	N	H	N
= MEMBER 1 (1 - 2) G =							
ITAN	0.000	-7.131	10.337	-1.965	(2)	-7.532	10.337
1	0.250	-4.652	9.495	-1.965	(2)	-5.053	9.495
2	0.650	-1.124	8.147	-1.965	(2)	-1.525	8.147
3	0.900	0.807	7.304	-1.965	(2)	0.407	7.304
4	1.150	2.528	6.462	-1.965	(2)	2.128	6.462
5	2.067	7.039	3.370	-1.965	(2)	6.638	3.370
6	2.567	8.302	1.685	-1.965	(2)	7.902	1.685
7	3.067	8.724	0.000	-1.965	(2)	8.323	0.000
JTAN	6.135	-7.131	-10.337	-1.965	(2)	-7.532	-10.337
= MEMBER 2 (3 - 4) G =							
ITAN	0.000	8.860	-13.497	-4.062	(2)	9.411	-13.497
1	0.250	5.623	-12.397	-4.062	(2)	6.175	-12.397
2	0.525	2.380	-11.187	-4.062	(2)	2.932	-11.187
3	1.568	-6.891	-6.600	-4.062	(2)	-6.340	-6.600
4	2.067	-9.641	-4.400	-4.062	(2)	-9.090	-4.400
5	2.567	-11.291	-2.200	-4.062	(2)	-10.740	-2.200
6	3.067	-11.841	0.000	-4.062	(2)	-11.290	0.000
JTAN	6.135	8.860	13.497	-4.062	(2)	9.411	13.497
= MEMBER 3 (1 - 3) C =							
ITAN	0.000	7.532	-3.568	-10.337	(1)	7.532	-3.568
1	0.250	6.682	-3.222	-10.561	(1)	6.682	-3.222
2	0.550	5.787	-2.733	-10.830	(1)	5.787	-2.733
3	0.650	5.523	-2.552	-10.920	(1)	5.523	-2.552
4	1.300	4.297	-1.157	-11.502	(1)	4.297	-1.157
5	1.800	4.042	0.175	-11.950	(1)	4.042	0.175
6	2.300	4.509	1.732	-12.398	(1)	4.509	1.732
7	3.000	6.591	4.289	-13.025	(1)	6.591	4.289
8	3.250	7.790	5.309	-13.249	(1)	7.790	5.309
JTAN	3.525	9.411	6.496	-13.496	(1)	9.411	6.496
= MEMBER 4 (2 - 4) C =							
ITAN	0.000	-7.532	3.568	-10.337	(1)	-7.532	3.568
1	0.250	-6.682	3.222	-10.561	(1)	-6.682	3.222
2	0.550	-5.787	2.733	-10.830	(1)	-5.787	2.733
3	0.650	-5.523	2.552	-10.920	(1)	-5.523	2.552
4	1.300	-4.297	1.157	-11.502	(1)	-4.297	1.157
5	1.800	-4.042	-0.175	-11.950	(1)	-4.042	-0.175
6	2.300	-4.509	-1.732	-12.398	(1)	-4.509	-1.732
7	3.000	-6.591	-4.289	-13.025	(1)	-6.591	-4.289
8	3.250	-7.790	-5.309	-13.249	(1)	-7.790	-5.309
JTAN	3.525	-9.411	-6.496	-13.496	(1)	-9.411	-6.496

COMBINE 2

		MOMENT		MOMENT		MINIMUM		CASE	
L	M	Q	N	H	N	M	N	M	N
= MEMBER 1 (1 - 2) G =									
ITAN	0.000	-15.177	24.939	-2.292	(2 3)	-15.578	24.939	-3.894	(1 3)
1	0.250	-9.778	22.906	-2.292	(2 3)	-10.178	22.906	-3.894	(1 3)
2	0.650	-2.093	19.654	-2.292	(2 3)	-2.493	19.654	-3.894	(1 3)
3	0.900	2.114	17.622	-2.292	(2 3)	1.713	17.622	-3.894	(1 3)
4	1.150	5.882	15.589	-2.292	(2 3)	5.461	15.589	-3.894	(1 3)
5	2.067	15.686	8.130	-2.292	(2 3)	15.285	8.130	-3.894	(1 3)
6	2.567	18.438	4.065	-2.292	(2 3)	18.038	4.065	-3.894	(1 3)
7	3.067	19.356	0.000	-2.292	(2 3)	18.955	0.000	-3.894	(1 3)
JTAN	6.135	-15.177	-24.939	-2.292	(2 3)	-15.578	-24.939	-3.894	(1 3)
= MEMBER 2 (3 - 4) G =									
ITAN	0.000	16.306	-28.098	-6.170	(1 3)	15.754	-28.098	-3.736	(2 3)
1	0.250	10.149	-25.808	-6.170	(1 3)	9.597	-25.808	-3.736	(2 3)
2	0.525	3.980	-23.289	-6.170	(1 3)	3.428	-23.289	-3.736	(2 3)
3	1.568	-13.657	-13.740	-6.170	(1 3)	-14.209	-13.740	-3.736	(2 3)
4	2.067	-18.888	-9.160	-6.170	(1 3)	-19.440	-9.160	-3.736	(2 3)
5	2.567	-22.027	-4.580	-6.170	(1 3)	-22.879	-4.580	-3.736	(2 3)
6	3.067	-23.073	0.000	-6.170	(1 3)	-23.625	0.000	-3.736	(2 3)
JTAN	6.135	16.306	28.098	-6.170	(1 3)	15.754	28.098	-3.736	(2 3)
= MEMBER 3 (1 - 3) C =									
ITAN	0.000	15.578	-3.894	-22.515	(1 3)	15.177	-2.292	-22.515	(2 3)
1	0.250	14.629	-3.549	-22.739	(1 3)	14.629	-2.085	-22.739	(2 3)
2	0.550	14.047	-1.793	-23.008	(2 3)	13.653	-3.060	-23.008	(1 3)
3	0.650	13.873	-1.684	-23.098	(2 3)	13.356	-2.879	-23.098	(1 3)
4	1.300	13.037	-0.649	-23.680	(2 3)	11.918	-1.453	-23.680	(1 3)
5	1.800	12.807	-0.051	-24.128	(2 3)	11.500	-0.151	-24.128	(1 3)
6	2.300	13.009	0.682	-24.576	(2 3)	11.804	1.405	-24.576	(1 3)
7	3.000	14.146	2.413	-25.203	(2 3)	13.657	3.963	-25.203	(1 3)
8	3.250	14.825	3.024	-25.427	(2 3)	14.774	4.983	-25.427	(1 3)
JTAN	3.525	16.306	6.170	-25.674	(1 3)	15.754	3.736	-25.674	(2 3)
= MEMBER 4 (2 - 4) C =									
ITAN	0.000	-15.177	2.292	-22.515	(2 3)	-15.578	3.894	-22.515	(1 3)
1	0.250	-14.629	2.085	-22.739	(2 3)	-14.629	3.549	-22.739	(1 3)
2	0.550	-13.653	3.060	-23.008	(1 3)	-14.047	1.793	-23.008	(2 3)
3	0.650	-13.356	2.879	-23.098	(1 3)	-13.873	1.684	-23.098	(2 3)
4	1.300	-11.918	1.483	-23.680	(1 3)	-13.037	0.849	-23.680	(2 3)
5	1.800	-11.500	0.151	-24.128	(1 3)	-12.807	0.051	-24.128	(2 3)
6	2.300	-11.804	-1.405	-24.576	(1 3)	-13.009	-0.862	-24.576	(2 3)
7	3.000	-13.657	-3.963	-25.203	(1 3)	-14.146	-2.413	-25.203	(2 3)
8	3.250	-14.774	-4.983	-25.427	(1 3)	-14.825	-3.024	-25.427	(2 3)
JTAN	3.525	-15.754	-3.736	-25.674	(2 3)	-16.306	-6.170	-25.674	(1 3)

COMBINE 2

		SHEAR		MAXIMUM		MINIMUM		CASE	
		M	Q	N		M	Q	N	
MEMBER 1 (1 - 2) G =									
ITAN	0.000	-15.578	24.939	-3.894	(1 3)	-15.578	24.939	-3.894	(1 3)
1	0.250	-10.179	22.906	-3.894	(1 3)	-10.178	22.906	-3.894	(1 3)
2	0.650	-2.493	19.654	-3.894	(1 3)	-2.493	19.654	-3.894	(1 3)
3	0.900	1.713	17.622	-3.894	(1 3)	1.713	17.622	-3.894	(1 3)
4	1.150	5.461	15.589	-3.894	(1 3)	5.461	15.589	-3.894	(1 3)
5	2.067	15.205	8.130	-3.894	(1 3)	15.285	8.130	-3.894	(1 3)
6	2.567	18.038	4.065	-3.894	(1 3)	18.038	4.065	-3.894	(1 3)
7	3.067	18.955	0.000	-3.894	(1 3)	18.955	0.000	-3.894	(1 3)
JTAN	6.135	-15.578	-24.939	-3.894	(1 3)	-15.578	-24.939	-3.894	(1 3)
MEMBER 2 (3 - 4) G =									
ITAN	0.000	16.306	-23.098	-6.170	(1 3)	16.306	-23.098	-6.170	(1 3)
1	0.250	10.149	-25.808	-6.170	(1 3)	10.149	-25.808	-6.170	(1 3)
2	0.525	3.980	-23.299	-6.170	(1 3)	3.980	-23.289	-6.170	(1 3)
3	1.568	-13.657	-13.740	-6.170	(1 3)	-13.657	-13.740	-6.170	(1 3)
4	2.067	-18.888	-9.160	-6.170	(1 3)	-18.888	-9.160	-6.170	(1 3)
5	2.567	-22.027	-4.580	-6.170	(1 3)	-22.027	-4.580	-6.170	(1 3)
6	3.067	-23.073	0.000	-6.170	(1 3)	-23.073	0.000	-6.170	(1 3)
JTAN	6.135	16.306	23.098	-6.170	(1 3)	16.306	23.098	-6.170	(1 3)
MEMBER 3 (1 - 3) C =									
ITAN	0.000	15.177	-2.292	-22.515	(2 3)	15.578	-3.894	-22.515	(1 3)
1	0.250	14.629	-2.085	-22.739	(2 3)	14.646	-3.549	-22.739	(1 3)
2	0.550	14.047	-1.793	-23.008	(2 3)	13.653	-3.060	-23.008	(1 3)
3	0.650	13.873	-1.684	-23.098	(2 3)	13.356	-2.879	-23.098	(1 3)
4	1.300	13.037	-0.849	-23.680	(2 3)	11.918	-1.483	-23.680	(1 3)
5	1.800	12.807	-0.051	-24.128	(2 3)	11.500	-0.151	-24.128	(1 3)
6	2.300	11.804	1.435	-24.576	(1 3)	13.009	0.982	-24.576	(2 3)
7	3.000	13.657	3.963	-25.203	(1 3)	14.146	2.413	-25.203	(2 3)
8	3.250	14.774	4.983	-25.427	(1 3)	14.825	3.024	-25.427	(2 3)
JTAN	3.525	16.306	6.170	-25.674	(1 3)	15.754	3.736	-25.674	(2 3)
MEMBER 4 (2 - 4) C =									
ITAN	0.000	-15.578	3.894	-22.515	(1 3)	-15.177	2.292	-22.515	(2 3)
1	0.250	-14.646	3.549	-22.739	(1 3)	-14.629	2.085	-22.739	(2 3)
2	0.550	-13.653	3.060	-23.008	(1 3)	-14.047	1.793	-23.008	(2 3)
3	0.650	-13.356	2.879	-23.098	(1 3)	-13.873	1.684	-23.098	(2 3)
4	1.300	-11.918	1.483	-23.680	(1 3)	-13.037	0.849	-23.680	(2 3)
5	1.800	-11.500	0.151	-24.128	(1 3)	-12.807	0.051	-24.128	(2 3)
6	2.300	-13.009	-0.882	-24.576	(2 3)	-11.804	-1.405	-24.576	(1 3)
7	3.000	-14.146	-2.413	-25.203	(2 3)	-13.657	-3.963	-25.203	(1 3)
8	3.250	-14.825	-3.024	-25.427	(2 3)	-14.774	-4.983	-25.427	(1 3)
JTAN	3.525	-15.754	-3.736	-25.674	(2 3)	-16.306	-6.170	-25.674	(1 3)

COMBINE 2

		AXIAL		MAXIMUM		MINIMUM		CASE	
L	M	Q	N	H	D	N	CASE	H	D
= MEMBER 1 (1 - 2) G =									
ITAN	0.000	-15.177	24.939	-2.292	(2 3)	-15.578	24.939	-3.894	(1 3)
1	0.250	-9.778	22.906	-2.292	(2 3)	-10.178	22.906	-3.894	(1 3)
2	0.650	-2.093	19.654	-2.292	(2 3)	-2.493	19.654	-3.894	(1 3)
3	0.900	2.114	17.622	-2.292	(2 3)	1.713	17.622	-3.894	(1 3)
4	1.150	5.862	15.589	-2.292	(2 3)	5.461	15.589	-3.894	(1 3)
5	2.067	15.686	8.130	-2.292	(2 3)	15.285	8.130	-3.894	(1 3)
6	2.567	18.433	4.065	-2.292	(2 3)	18.038	4.065	-3.894	(1 3)
7	3.067	19.356	0.000	-2.292	(2 3)	18.955	0.000	-3.894	(1 3)
JTAN	6.135	-15.177	-24.939	-2.292	(2 3)	-15.578	-24.939	-3.894	(1 3)
= MEMBER 2 (3 - 4) G =									
ITAN	0.000	15.754	-28.098	-3.736	(2 3)	16.306	-28.098	-6.170	(1 3)
1	0.250	9.597	-25.808	-3.736	(2 3)	10.149	-25.808	-6.170	(1 3)
2	0.525	3.429	-23.289	-3.736	(2 3)	3.380	-23.289	-6.170	(1 3)
3	1.588	-14.209	-13.740	-3.736	(2 3)	-13.657	-13.740	-6.170	(1 3)
4	2.067	-19.440	-9.160	-3.736	(2 3)	-18.888	-9.160	-6.170	(1 3)
5	2.567	-22.579	-4.580	-3.736	(2 3)	-22.027	-4.580	-6.170	(1 3)
6	3.067	-23.625	0.000	-3.736	(2 3)	-23.073	0.000	-6.170	(1 3)
JTAN	6.135	15.754	28.098	-3.736	(2 3)	16.306	28.098	-6.170	(1 3)
= MEMBER 3 (1 - 3) C =									
ITAN	0.000	15.578	-3.894	-22.515	(1 3)	15.578	-3.894	-22.515	(1 3)
1	0.250	14.646	-3.549	-22.739	(1 3)	14.646	-3.549	-22.739	(1 3)
2	0.550	13.653	-3.060	-23.008	(1 3)	13.653	-3.060	-23.008	(1 3)
3	0.650	13.356	-2.879	-23.098	(1 3)	13.356	-2.879	-23.098	(1 3)
4	1.300	11.918	-1.483	-23.680	(1 3)	11.918	-1.483	-23.680	(1 3)
5	1.800	11.500	-0.151	-24.128	(1 3)	11.500	-0.151	-24.128	(1 3)
6	2.300	11.804	1.405	-24.576	(1 3)	11.804	1.405	-24.576	(1 3)
7	3.000	13.657	3.963	-25.203	(1 3)	13.657	3.963	-25.203	(1 3)
8	3.250	14.774	4.983	-25.427	(1 3)	14.774	4.983	-25.427	(1 3)
JTAN	3.525	16.306	6.170	-25.674	(1 3)	16.306	6.170	-25.674	(1 3)
= MEMBER 4 (2 - 4) C =									
ITAN	0.000	-15.578	3.694	-22.515	(1 3)	-15.578	3.694	-22.515	(1 3)
1	0.250	-14.646	3.549	-22.739	(1 3)	-14.646	3.549	-22.739	(1 3)
2	0.550	-13.653	3.060	-23.008	(1 3)	-13.653	3.060	-23.008	(1 3)
3	0.650	-13.356	2.879	-23.098	(1 3)	-13.356	2.879	-23.098	(1 3)
4	1.300	-11.918	1.483	-23.680	(1 3)	-11.918	1.483	-23.680	(1 3)
5	1.800	-11.500	0.151	-24.128	(1 3)	-11.500	0.151	-24.128	(1 3)
6	2.300	-11.804	-1.405	-24.576	(1 3)	-11.804	-1.405	-24.576	(1 3)
7	3.000	-13.657	-3.963	-25.203	(1 3)	-13.657	-3.963	-25.203	(1 3)
8	3.250	-14.774	-4.983	-25.427	(1 3)	-14.774	-4.983	-25.427	(1 3)
JTAN	3.525	-16.306	-6.170	-25.674	(1 3)	-16.306	-6.170	-25.674	(1 3)

TITLE..C22

COMBINE 3

		MEMBER 1 (1 - 2) C =		MEMBER 2 (3 - 4) C =		MEMBER 3 (1 - 3) C =		MEMBER 4 (2 - 4) C =		
		M	Q	M	Q	M	Q	M	Q	
		HOMENT		HOMENT		HOMENT		HOMENT		
		MAXIMUM		MAXIMUM		MAXIMUM		MAXIMUM		
		N		N		N		N		
		CASE		CASE		CASE		CASE		
ITAN	0.000	-7.479	10.337	-3.614	(2 4)	-7.479	10.337	-3.614	(2 4)	
1	0.250	-5.000	9.495	-3.614	(2 4)	-5.000	9.495	-3.614	(2 4)	
2	0.650	-1.471	8.147	-3.614	(2 4)	-1.471	8.147	-3.614	(2 4)	
3	0.900	0.460	7.304	-3.614	(2 4)	0.460	7.304	-3.614	(2 4)	
4	1.150	2.181	6.462	-3.614	(2 4)	2.181	6.462	-3.614	(2 4)	
5	2.067	6.691	3.370	-3.614	(2 4)	6.691	3.370	-3.614	(2 4)	
6	2.567	7.955	1.685	-3.614	(2 4)	7.955	1.685	-3.614	(2 4)	
7	3.067	8.376	0.000	-3.614	(2 4)	8.376	0.000	-3.614	(2 4)	
JTAN	6.135	-7.479	-10.337	-3.614	(2 4)	-7.479	-10.337	-3.614	(2 4)	
= MEMBER 2 (3 - 4) C =										
ITAN	0.000	9.851	-13.497	-8.197	(1 4)	9.851	-13.497	-8.197	(1 4)	
1	0.250	6.615	-12.397	-8.197	(1 4)	6.615	-12.397	-8.197	(1 4)	
2	0.525	3.372	-11.187	-8.197	(1 4)	3.372	-11.187	-8.197	(1 4)	
3	1.568	-5.900	-6.600	-8.197	(1 4)	-5.900	-6.600	-8.197	(1 4)	
4	2.067	-8.650	-4.400	-8.197	(1 4)	-8.650	-4.400	-8.197	(1 4)	
5	2.567	-10.300	-2.200	-8.197	(1 4)	-10.300	-2.200	-8.197	(1 4)	
6	3.067	-10.850	0.000	-8.197	(1 4)	-10.850	0.000	-8.197	(1 4)	
JTAN	6.135	9.851	13.497	-8.197	(1 4)	9.851	13.497	-8.197	(1 4)	
= MEMBER 3 (1 - 3) C =										
ITAN	0.000	7.879	-5.216	-10.337	(1 4)	7.879	-5.216	-10.337	(1 4)	
1	0.250	6.647	-4.633	-10.561	(1 4)	6.647	-4.633	-10.561	(1 4)	
2	0.550	5.765	-2.592	-10.830	(2 4)	5.765	-2.592	-10.830	(2 4)	
3	0.650	5.516	-2.382	-10.920	(2 4)	5.516	-2.382	-10.920	(2 4)	
4	1.300	4.423	-0.935	-11.502	(2 4)	4.423	-0.935	-11.502	(2 4)	
5	1.800	4.268	0.338	-11.950	(2 4)	4.268	0.338	-11.950	(2 4)	
6	2.300	4.794	1.745	-12.398	(2 4)	4.794	1.745	-12.398	(2 4)	
7	3.000	6.759	3.942	-13.025	(2 4)	6.759	3.942	-13.025	(2 4)	
8	3.250	7.849	4.791	-13.249	(2 4)	7.849	4.791	-13.249	(2 4)	
JTAN	3.525	9.851	8.197	-13.496	(1 4)	9.851	8.197	-13.496	(1 4)	
= MEMBER 4 (2 - 4) C =										
ITAN	0.000	-7.479	3.614	-10.337	(2 4)	-7.479	3.614	-10.337	(2 4)	
1	0.250	-6.630	3.169	-10.561	(2 4)	-6.630	3.169	-10.561	(2 4)	
2	0.550	-5.371	3.859	-10.830	(1 4)	-5.371	3.859	-10.830	(1 4)	
3	0.650	-4.999	3.533	-10.920	(1 4)	-4.999	3.533	-10.920	(1 4)	
4	1.300	-3.304	1.570	-11.502	(1 4)	-3.304	1.570	-11.502	(1 4)	
5	1.800	-2.962	-0.237	-11.950	(1 4)	-2.962	-0.237	-11.950	(1 4)	
6	2.300	-3.579	-2.269	-12.398	(1 4)	-3.579	-2.269	-12.398	(1 4)	
7	3.000	-6.269	-5.491	-13.025	(1 4)	-6.269	-5.491	-13.025	(1 4)	
8	3.250	-7.798	-6.749	-13.249	(1 4)	-7.798	-6.749	-13.249	(1 4)	
JTAN	3.525	-9.300	-5.763	-13.496	(2 4)	-9.300	-5.763	-13.496	(2 4)	

ITILE..C22

COMBINE 3

MEMBER	I	1	2	C	SHEAR		MAXIMUM		MINIMUM		CASE	
					M	N	M	N	M	N		
= MEMBER 1 (1 - 2) C =												
ITAN	0.000	-7.879	10.337	-5.216	(1 4)	-7.879	10.337	-5.216	(1 4)			
1	0.250	-5.400	9.495	-5.216	(1 4)	-5.400	9.495	-5.216	(1 4)			
2	0.650	-1.872	8.147	-5.216	(1 4)	-1.872	8.147	-5.216	(1 4)			
3	0.900	0.060	7.304	-5.216	(1 4)	0.060	7.304	-5.216	(1 4)			
4	1.150	1.780	6.462	-5.216	(1 4)	1.780	6.462	-5.216	(1 4)			
5	2.067	6.291	3.370	-5.216	(1 4)	6.291	3.370	-5.216	(1 4)			
6	2.567	7.555	1.685	-5.216	(1 4)	7.555	1.685	-5.216	(1 4)			
7	3.067	7.976	0.000	-5.216	(1 4)	7.976	0.000	-5.216	(1 4)			
JTAN	6.135	-7.879	-10.337	-5.216	(1 4)	-7.879	-10.337	-5.216	(1 4)			
= MEMBER 2 (3 - 4) C =												
ITAN	0.000	9.851	-13.497	-8.197	(1 4)	9.851	-13.497	-8.197	(1 4)			
1	0.250	6.615	-12.397	-8.197	(1 4)	6.615	-12.397	-8.197	(1 4)			
2	0.525	3.372	-11.187	-8.197	(1 4)	3.372	-11.187	-8.197	(1 4)			
3	1.568	-5.900	-6.600	-8.197	(1 4)	-5.900	-6.600	-8.197	(1 4)			
4	2.067	-8.650	-4.400	-8.197	(1 4)	-8.650	-4.400	-8.197	(1 4)			
5	2.567	-10.300	-2.200	-8.197	(1 4)	-10.300	-2.200	-8.197	(1 4)			
6	3.067	-10.950	0.000	-8.197	(1 4)	-10.950	0.000	-8.197	(1 4)			
JTAN	6.135	9.851	13.497	-8.197	(1 4)	9.851	13.497	-8.197	(1 4)			
= MEMBER 3 (1 - 3) C =												
ITAN	0.000	7.879	-3.614	-10.337	(2 4)	7.879	-3.614	-10.337	(1 4)			
1	0.250	6.630	-3.169	-10.561	(2 4)	6.647	-4.633	-10.561	(1 4)			
2	0.550	5.765	-2.592	-10.830	(2 4)	5.371	-3.859	-10.830	(1 4)			
3	0.650	5.516	-2.388	-10.920	(2 4)	4.999	-3.583	-10.920	(1 4)			
4	1.300	4.423	-0.935	-11.502	(2 4)	3.304	-1.570	-11.502	(1 4)			
5	1.800	4.268	0.338	-11.950	(2 4)	2.962	0.237	-11.950	(1 4)			
6	2.300	3.579	2.269	-12.398	(2 4)	4.784	1.745	-12.398	(2 4)			
7	3.000	6.269	5.491	-13.025	(1 4)	6.759	3.942	-13.025	(2 4)			
8	3.250	7.798	8.749	-13.249	(1 4)	7.349	4.791	-13.249	(2 4)			
JTAN	3.525	9.851	8.197	-13.496	(1 4)	9.300	5.763	-13.496	(2 4)			
= MEMBER 4 (2 - 4) C =												
ITAN	0.000	-7.879	5.216	-10.337	(1 4)	-7.479	3.614	-10.337	(2 4)			
1	0.250	-6.647	4.633	-10.561	(1 4)	-6.630	3.169	-10.561	(2 4)			
2	0.550	-5.371	3.859	-10.830	(1 4)	-5.765	2.592	-10.830	(2 4)			
3	0.650	-4.999	3.583	-10.920	(1 4)	-5.516	2.388	-10.920	(2 4)			
4	1.300	-3.304	1.570	-11.502	(1 4)	-4.423	0.935	-11.502	(2 4)			
5	1.800	-2.962	-0.237	-11.950	(1 4)	-4.268	-0.338	-11.950	(2 4)			
6	2.300	-4.784	-1.745	-12.398	(1 4)	-4.784	-1.745	-12.398	(2 4)			
7	3.000	-6.759	-3.942	-13.025	(2 4)	-6.269	-5.491	-13.025	(1 4)			
8	3.250	-7.849	-4.791	-13.249	(2 4)	-7.798	-6.749	-13.249	(1 4)			
JTAN	3.525	-9.300	-5.763	-13.496	(2 4)	-9.351	-8.197	-13.496	(1 4)			

COMBINE 3

		AXIAL		MAXIMUM		MINIMUM		CASE				
MEMBER	1	2	3	4	1	2	3	4	1	2	3	4
ITAN	0.000	-7.479	10.337	-3.614	(2 4)	-7.879	10.337	-5.216	(1 4)			
1	0.250	-5.000	9.495	-3.614	(2 4)	-5.400	9.495	-5.216	(1 4)			
2	0.650	-1.471	8.147	-3.614	(2 4)	-1.872	8.147	-5.216	(1 4)			
3	0.900	0.460	7.304	-3.614	(2 4)	0.060	7.304	-5.216	(1 4)			
4	1.150	2.181	6.462	-3.614	(2 4)	1.780	6.462	-5.216	(1 4)			
5	2.067	6.691	3.370	-3.614	(2 4)	6.291	3.370	-5.216	(1 4)			
6	2.567	7.955	1.685	-3.614	(2 4)	7.555	1.685	-5.216	(1 4)			
7	3.067	8.376	0.000	-3.614	(2 4)	7.976	0.000	-5.216	(1 4)			
JTAN	6.135	-7.479	-10.337	-3.614	(2 4)	-7.879	-10.337	-5.216	(1 4)			
ITAN	0.000	9.300	-13.497	-5.763	(2 4)	9.351	-13.497	-8.197	(1 4)			
1	0.250	6.063	-12.397	-5.763	(2 4)	6.615	-12.397	-8.197	(1 4)			
2	0.525	2.820	-11.187	-5.763	(2 4)	3.372	-11.187	-8.197	(1 4)			
3	1.568	-6.451	-6.600	-5.763	(2 4)	-5.900	-6.600	-8.197	(1 4)			
4	2.067	-9.201	-4.400	-5.763	(2 4)	-8.650	-4.400	-8.197	(1 4)			
5	2.567	-10.851	-2.200	-5.763	(2 4)	-10.300	-2.200	-8.197	(1 4)			
6	3.067	-11.401	0.000	-5.763	(2 4)	-10.850	0.000	-8.197	(1 4)			
JTAN	6.135	9.300	13.497	-5.763	(2 4)	9.851	13.497	-8.197	(1 4)			
ITAN	0.000	7.879	-5.216	-10.337	(1 4)	7.879	-5.216	-10.337	(1 4)			
1	0.250	6.647	-4.633	-10.561	(1 4)	6.647	-4.633	-10.561	(1 4)			
2	0.550	5.371	-3.859	-10.830	(1 4)	5.371	-3.859	-10.830	(1 4)			
3	0.650	4.999	-3.583	-10.920	(1 4)	4.999	-3.583	-10.920	(1 4)			
4	1.300	3.304	-1.570	-11.502	(1 4)	3.304	-1.570	-11.502	(1 4)			
5	1.800	2.962	0.237	-11.950	(1 4)	2.962	0.237	-11.950	(1 4)			
6	2.300	3.579	2.269	-12.398	(1 4)	3.579	2.269	-12.398	(1 4)			
7	3.000	6.269	5.491	-13.025	(1 4)	6.269	5.491	-13.025	(1 4)			
8	3.250	7.798	6.749	-13.249	(1 4)	7.798	6.749	-13.249	(1 4)			
JTAN	3.525	9.851	8.197	-13.496	(1 4)	9.851	8.197	-13.496	(1 4)			
ITAN	0.000	-7.879	5.216	-10.337	(1 4)	-7.879	5.216	-10.337	(1 4)			
1	0.250	-6.647	4.633	-10.561	(1 4)	-6.647	4.633	-10.561	(1 4)			
2	0.550	-5.371	3.859	-10.830	(1 4)	-5.371	3.859	-10.830	(1 4)			
3	0.650	-4.999	3.583	-10.920	(1 4)	-4.999	3.583	-10.920	(1 4)			
4	1.300	-3.304	1.570	-11.502	(1 4)	-3.304	1.570	-11.502	(1 4)			
5	1.800	-2.962	0.237	-11.950	(1 4)	-2.962	0.237	-11.950	(1 4)			
6	2.300	-3.579	2.269	-12.398	(1 4)	-3.579	2.269	-12.398	(1 4)			
7	3.000	-6.269	5.491	-13.025	(1 4)	-6.269	5.491	-13.025	(1 4)			
8	3.250	-7.798	6.749	-13.249	(1 4)	-7.798	6.749	-13.249	(1 4)			
JTAN	3.525	-9.851	8.197	-13.496	(1 4)	-9.851	8.197	-13.496	(1 4)			

COMBINE 4

		MEMBER 1 (1 - 2) G =				MEMBER 2 (3 - 4) G =				MEMBER 3 (1 - 3) C =				MEMBER 4 (2 - 4) C =													
		L		M		Q		N		L		M		Q		N		L		M		Q		N			
		CASE		CASE		CASE		CASE		CASE		CASE		CASE		CASE		CASE		CASE		CASE		CASE			
		MAXIMUM		MINIMUM		MAXIMUM		MINIMUM		MAXIMUM		MINIMUM		MAXIMUM		MINIMUM		MAXIMUM		MINIMUM		MAXIMUM		MINIMUM			
ITAN	0.000	-15.525	24.939	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
1	0.250	-10.125	22.906	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
2	0.650	-2.440	19.654	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
3	0.900	1.767	17.622	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
4	1.150	5.515	15.589	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
5	2.067	15.338	3.130	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
6	2.567	18.091	4.065	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
7	3.067	19.004	0.000	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
JTAN	6.135	-15.525	-24.939	-3.940	3.940	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	2	3	4	
= MEMBER 2 (3 - 4) G =																											
ITAN	0.000	16.746	-22.098	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
1	0.250	10.589	-25.202	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
2	0.525	4.420	-23.289	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
3	1.568	-13.217	-13.740	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
4	2.067	-18.448	-9.160	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
5	2.567	-21.587	-4.580	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
6	3.067	-22.633	0.000	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
JTAN	6.135	16.746	22.098	-7.870	7.870	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
= MEMBER 3 (1 - 3) C =																											
ITAN	0.000	15.925	-5.542	-22.515	22.515	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
1	0.250	14.611	-4.959	-22.739	22.739	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
2	0.550	13.631	-2.918	-23.008	23.008	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
3	0.650	13.349	-2.715	-23.098	23.098	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
4	1.300	12.045	-1.262	-23.680	23.680	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
5	1.800	11.726	0.011	-24.128	24.128	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
6	2.300	12.078	1.418	-24.576	24.576	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
7	3.000	13.824	3.615	-25.203	25.203	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
8	3.250	14.834	4.464	-25.427	25.427	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
JTAN	3.525	16.746	7.670	-25.674	25.674	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
= MEMBER 4 (2 - 4) C =																											
ITAN	0.000	-15.525	3.940	-22.515	22.515	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
1	0.250	-14.594	3.496	-22.739	22.739	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
2	0.550	-13.238	4.185	-23.008	23.008	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
3	0.650	-12.833	3.909	-23.098	23.098	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
4	1.300	-10.925	1.896	-23.680	23.680	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
5	1.800	-10.419	0.089	-24.128	24.128	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
6	2.300	-10.873	1.942	-24.576	24.576	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
7	3.000	-13.335	5.164	-25.203	25.203	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
8	3.250	-14.762	6.422	-25.427	25.427	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	
JTAN	3.525	-16.194	7.670	-25.674	25.674	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	1	3	4	

COMBINE 4

		MEMBER 1 (1 - 2) G =				MEMBER 2 (3 - 4) G =				MEMBER 3 (1 - 3) C =				MEMBER 4 (2 - 4) C =			
		M	O	N	CASE	M	O	N	CASE	M	O	N	CASE	M	O	N	CASE
				AXIAL	MAXIMUM			AXIAL	MINIMUM			AXIAL	MINIMUM			AXIAL	MINIMUM
ITAN	0.000	-15.925	24.939	-3.940	(2 3 4)	-15.925	24.939	-5.542	(1 3 4)	16.746	-28.098	-7.870	(1 3 4)	15.925	-5.542	-22.515	(1 3 4)
1	0.250	-10.125	22.906	-3.940	(2 3 4)	-10.125	22.906	-5.542	(1 3 4)	10.589	-25.908	-7.870	(1 3 4)	14.611	-4.959	-22.739	(1 3 4)
2	0.650	-2.440	19.654	-3.940	(2 3 4)	-2.440	19.654	-5.542	(1 3 4)	4.420	-23.289	-7.870	(1 3 4)	13.238	-4.185	-23.008	(1 3 4)
3	0.900	1.767	17.622	-3.940	(2 3 4)	1.767	17.622	-5.542	(1 3 4)	-13.217	-13.740	-7.870	(1 3 4)	12.833	-3.909	-23.098	(1 3 4)
4	1.150	5.515	15.589	-3.940	(2 3 4)	5.515	15.589	-5.542	(1 3 4)	-18.448	-9.160	-7.870	(1 3 4)	10.925	-1.896	-23.680	(1 3 4)
5	2.067	15.338	8.130	-3.940	(2 3 4)	15.338	8.130	-5.542	(1 3 4)	-21.587	-4.580	-7.870	(1 3 4)	10.419	-0.089	-24.128	(1 3 4)
6	2.567	18.091	4.065	-3.940	(2 3 4)	18.091	4.065	-5.542	(1 3 4)	-22.633	0.000	-7.870	(1 3 4)	10.873	1.942	-24.576	(1 3 4)
7	3.067	19.008	0.000	-3.940	(2 3 4)	19.008	0.000	-5.542	(1 3 4)	16.746	28.098	-7.870	(1 3 4)	13.335	5.164	-25.203	(1 3 4)
JTAN	6.135	-15.925	-24.939	-3.940	(2 3 4)	-15.925	-24.939	-5.542	(1 3 4)	16.746	28.098	-7.870	(1 3 4)	14.782	6.422	-25.427	(1 3 4)
MEMBER 2 (3 - 4) G =																	
ITAN	0.000	16.194	-28.098	-5.436	(2 3 4)	16.194	-28.098	-5.436	(2 3 4)	16.746	-28.098	-7.870	(1 3 4)	15.925	-5.542	-22.515	(1 3 4)
1	0.250	10.037	-25.606	-5.436	(2 3 4)	10.037	-25.606	-5.436	(2 3 4)	10.589	-25.908	-7.870	(1 3 4)	14.611	-4.959	-22.739	(1 3 4)
2	0.525	3.868	-23.289	-5.436	(2 3 4)	3.868	-23.289	-5.436	(2 3 4)	4.420	-23.289	-7.870	(1 3 4)	13.238	-4.185	-23.008	(1 3 4)
3	1.568	-13.769	-13.740	-5.436	(2 3 4)	-13.769	-13.740	-5.436	(2 3 4)	-13.217	-13.740	-7.870	(1 3 4)	12.833	-3.909	-23.098	(1 3 4)
4	2.067	-19.000	-9.160	-5.436	(2 3 4)	-19.000	-9.160	-5.436	(2 3 4)	-18.448	-9.160	-7.870	(1 3 4)	10.925	-1.896	-23.680	(1 3 4)
5	2.567	-22.139	-4.580	-5.436	(2 3 4)	-22.139	-4.580	-5.436	(2 3 4)	-21.587	-4.580	-7.870	(1 3 4)	10.419	-0.089	-24.128	(1 3 4)
6	3.067	-23.185	0.000	-5.436	(2 3 4)	-23.185	0.000	-5.436	(2 3 4)	-22.633	0.000	-7.870	(1 3 4)	10.873	1.942	-24.576	(1 3 4)
JTAN	6.135	16.194	28.098	-5.436	(2 3 4)	16.194	28.098	-5.436	(2 3 4)	16.746	28.098	-7.870	(1 3 4)	13.335	5.164	-25.203	(1 3 4)
MEMBER 3 (1 - 3) C =																	
ITAN	0.000	15.925	-5.542	-22.515	(1 3 4)	15.925	-5.542	-22.515	(1 3 4)	16.746	28.098	-7.870	(1 3 4)	15.925	-5.542	-22.515	(1 3 4)
1	0.250	14.611	-4.959	-22.739	(1 3 4)	14.611	-4.959	-22.739	(1 3 4)	10.589	-25.908	-7.870	(1 3 4)	14.611	-4.959	-22.739	(1 3 4)
2	0.550	13.238	-4.185	-23.008	(1 3 4)	13.238	-4.185	-23.008	(1 3 4)	4.420	-23.289	-7.870	(1 3 4)	13.238	-4.185	-23.008	(1 3 4)
3	0.650	12.833	-3.909	-23.098	(1 3 4)	12.833	-3.909	-23.098	(1 3 4)	4.420	-23.289	-7.870	(1 3 4)	12.833	-3.909	-23.098	(1 3 4)
4	1.300	10.925	-1.896	-23.680	(1 3 4)	10.925	-1.896	-23.680	(1 3 4)	-13.217	-13.740	-7.870	(1 3 4)	10.925	-1.896	-23.680	(1 3 4)
5	1.800	10.419	-0.089	-24.128	(1 3 4)	10.419	-0.089	-24.128	(1 3 4)	-18.448	-9.160	-7.870	(1 3 4)	10.419	-0.089	-24.128	(1 3 4)
6	2.300	10.873	1.942	-24.576	(1 3 4)	10.873	1.942	-24.576	(1 3 4)	-21.587	-4.580	-7.870	(1 3 4)	10.873	1.942	-24.576	(1 3 4)
7	3.000	13.335	5.164	-25.203	(1 3 4)	13.335	5.164	-25.203	(1 3 4)	-22.633	0.000	-7.870	(1 3 4)	13.335	5.164	-25.203	(1 3 4)
8	3.250	14.782	6.422	-25.427	(1 3 4)	14.782	6.422	-25.427	(1 3 4)	-22.633	0.000	-7.870	(1 3 4)	14.782	6.422	-25.427	(1 3 4)
JTAN	3.525	16.746	7.870	-25.674	(1 3 4)	16.746	7.870	-25.674	(1 3 4)	16.746	28.098	-7.870	(1 3 4)	16.746	7.870	-25.674	(1 3 4)
MEMBER 4 (2 - 4) C =																	
ITAN	0.000	-15.925	5.542	-22.515	(1 3 4)	-15.925	5.542	-22.515	(1 3 4)	16.746	28.098	-7.870	(1 3 4)	-15.925	5.542	-22.515	(1 3 4)
1	0.250	-14.611	4.959	-22.739	(1 3 4)	-14.611	4.959	-22.739	(1 3 4)	10.589	-25.908	-7.870	(1 3 4)	-14.611	4.959	-22.739	(1 3 4)
2	0.550	-13.238	4.185	-23.008	(1 3 4)	-13.238	4.185	-23.008	(1 3 4)	4.420	-23.289	-7.870	(1 3 4)	-13.238	4.185	-23.008	(1 3 4)
3	0.650	-12.833	3.909	-23.098	(1 3 4)	-12.833	3.909	-23.098	(1 3 4)	4.420	-23.289	-7.870	(1 3 4)	-12.833	3.909	-23.098	(1 3 4)
4	1.300	-10.925	1.896	-23.680	(1 3 4)	-10.925	1.896	-23.680	(1 3 4)	-13.217	-13.740	-7.870	(1 3 4)	-10.925	1.896	-23.680	(1 3 4)
5	1.800	-10.419	0.089	-24.128	(1 3 4)	-10.419	0.089	-24.128	(1 3 4)	-18.448	-9.160	-7.870	(1 3 4)	-10.419	0.089	-24.128	(1 3 4)
6	2.300	-10.873	-1.942	-24.576	(1 3 4)	-10.873	-1.942	-24.576	(1 3 4)	-21.587	-4.580	-7.870	(1 3 4)	-10.873	-1.942	-24.576	(1 3 4)
7	3.000	-13.335	-5.164	-25.203	(1 3 4)	-13.335	-5.164	-25.203	(1 3 4)	-22.633	0.000	-7.870	(1 3 4)	-13.335	-5.164	-25.203	(1 3 4)
8	3.250	-14.782	-6.422	-25.427	(1 3 4)	-14.782	-6.422	-25.427	(1 3 4)	-22.633	0.000	-7.870	(1 3 4)	-14.782	-6.422	-25.427	(1 3 4)
JTAN	3.525	-16.746	-7.870	-25.674	(1 3 4)	-16.746	-7.870	-25.674	(1 3 4)	16.746	28.098	-7.870	(1 3 4)	-16.746	-7.870	-25.674	(1 3 4)

TITLE..C22

PICK UP 1

		MOMENT		MINIMUM		MAXIMUM		COM		CASE	
		M	Q	N	N	M	Q	N	N	M	Q
= MEMBER 1 (1 - 2) G =											
ITAN	0.000	-7.131	10.337	-1.965	(1 ; 2)	-15.925	24.939	-5.542	(4 ; 1 ; 3 ; 4)		
1	0.250	-4.652	9.495	-1.965	(1 ; 2)	-10.526	22.906	-5.542	(4 ; 1 ; 3 ; 4)		
2	0.650	-1.124	8.147	-1.965	(1 ; 2)	-2.841	19.554	-5.542	(4 ; 1 ; 3 ; 4)		
3	0.900	2.114	17.622	-2.292	(2 ; 2 ; 3)	0.060	7.304	-5.216	(3 ; 1 ; 4)		
4	1.150	5.862	15.589	-2.292	(2 ; 2 ; 3)	1.780	6.462	-5.216	(3 ; 1 ; 4)		
5	2.067	15.686	8.130	-2.292	(2 ; 2 ; 3)	6.291	3.370	-5.216	(3 ; 1 ; 4)		
6	2.567	18.438	4.065	-2.292	(2 ; 2 ; 3)	7.555	1.685	-5.216	(3 ; 1 ; 4)		
7	3.067	19.356	0.000	-2.292	(2 ; 2 ; 3)	7.976	0.000	-5.216	(3 ; 1 ; 4)		
JTAN	6.135	-7.131	-10.337	-1.965	(1 ; 2)	-15.925	-24.939	-5.542	(4 ; 1 ; 3 ; 4)		
= MEMBER 2 (3 - 4) G =											
ITAN	0.000	16.746	-24.092	-7.870	(4 ; 1 ; 3 ; 4)	8.860	-13.497	-4.062	(1 ; 2)		
1	0.250	10.589	-25.808	-7.870	(4 ; 1 ; 3 ; 4)	5.623	-12.397	-4.062	(1 ; 2)		
2	0.525	4.420	-23.289	-7.870	(4 ; 1 ; 3 ; 4)	2.380	-11.187	-4.062	(1 ; 2)		
3	1.568	-5.900	-6.600	-8.197	(3 ; 1 ; 4)	-14.209	-13.740	-3.736	(2 ; 2 ; 3)		
4	2.067	-8.650	-4.400	-9.197	(3 ; 1 ; 4)	-19.440	-9.160	-3.736	(2 ; 2 ; 3)		
5	2.567	-10.300	-2.200	-8.197	(3 ; 1 ; 4)	-22.579	-4.580	-3.736	(2 ; 2 ; 3)		
6	3.067	-10.850	0.000	-8.197	(3 ; 1 ; 4)	-23.625	0.000	-3.736	(2 ; 2 ; 3)		
JTAN	6.135	16.746	24.092	-7.870	(4 ; 1 ; 3 ; 4)	8.860	13.497	-4.062	(1 ; 2)		
= MEMBER 3 (1 - 3) C =											
ITAN	0.000	15.925	-5.542	-22.515	(4 ; 1 ; 3 ; 4)	7.131	-1.965	-10.337	(1 ; 2)		
1	0.250	14.646	-3.549	-22.739	(2 ; 1 ; 3)	6.630	-3.169	-10.561	(3 ; 2 ; 4)		
2	0.550	14.042	-1.793	-23.008	(2 ; 2 ; 3)	5.371	-3.859	-10.830	(3 ; 1 ; 4)		
3	0.650	13.873	-1.684	-23.098	(2 ; 2 ; 3)	4.999	-4.999	-10.920	(3 ; 1 ; 4)		
4	1.300	13.037	-0.849	-23.680	(2 ; 2 ; 3)	3.304	-1.570	-11.502	(3 ; 1 ; 4)		
5	1.800	12.807	-0.051	-24.128	(2 ; 2 ; 3)	2.962	0.237	-11.950	(3 ; 1 ; 4)		
6	2.300	13.809	0.832	-24.576	(2 ; 2 ; 3)	3.579	2.269	-12.398	(3 ; 1 ; 4)		
7	3.000	14.146	2.413	-25.203	(2 ; 2 ; 3)	6.269	5.491	-13.025	(3 ; 1 ; 4)		
8	3.250	14.834	4.464	-25.427	(4 ; 2 ; 3 ; 4)	7.790	5.309	-13.249	(1 ; 1 ; 2)		
JTAN	3.525	16.746	7.870	-25.674	(4 ; 1 ; 3 ; 4)	8.860	4.062	-13.496	(1 ; 2)		
= MEMBER 4 (2 - 4) C =											
ITAN	0.000	-7.131	1.965	-10.337	(1 ; 2)	-15.925	5.542	-22.515	(4 ; 1 ; 3 ; 4)		
1	0.250	-6.630	3.169	-10.561	(3 ; 2 ; 4)	-14.646	3.549	-22.739	(2 ; 1 ; 3)		
2	0.550	-5.371	3.859	-10.830	(3 ; 1 ; 4)	-14.047	1.793	-23.008	(2 ; 2 ; 3)		
3	0.650	-4.999	3.583	-10.920	(3 ; 1 ; 4)	-13.873	1.684	-23.098	(2 ; 2 ; 3)		
4	1.300	-3.304	1.570	-11.502	(3 ; 1 ; 4)	-13.037	0.849	-23.680	(2 ; 2 ; 3)		
5	1.800	-2.962	-0.237	-11.950	(3 ; 1 ; 4)	-12.807	0.051	-24.128	(2 ; 2 ; 3)		
6	2.300	-3.579	-2.269	-12.398	(3 ; 1 ; 4)	-13.009	-0.882	-24.576	(2 ; 2 ; 3)		
7	3.000	-6.269	-5.491	-13.025	(3 ; 1 ; 4)	-14.146	-2.413	-25.203	(2 ; 2 ; 3)		
8	3.250	-7.790	-5.309	-13.249	(1 ; 1 ; 2)	-14.834	-4.464	-25.427	(4 ; 2 ; 3 ; 4)		
JTAN	3.525	-8.860	-4.062	-13.496	(1 ; 2)	-16.746	-7.870	-25.674	(4 ; 1 ; 3 ; 4)		

TITLE..G22

PICK UP 1

		SHEAR		MAXIMUM		MINIMUM		COM		CASE	
L	H	Q	N	H	N	Q	N	H	N	Q	N
MEMBER	1 (1 - 2)	G	=	MEMBER	1 (1 - 2)	G	=	MEMBER	1 (1 - 2)	G	=
ITAN	0.000	-15.578	24.939	-3.894	(2; 1)	3J	-7.532	10.337	-3.568	(1; 1)	1J
1	0.250	-10.178	22.906	-3.894	(2; 1)	3J	-5.053	9.495	-3.568	(1; 1)	1J
2	0.650	-2.493	*19.654	-3.894	(2; 1)	3J	-1.525	8.147	-3.568	(1; 1)	1J
3	0.900	1.713	17.622	-3.894	(2; 1)	3J	0.407	7.304	-3.568	(1; 1)	1J
4	1.150	5.461	15.589	-3.894	(2; 1)	3J	2.128	6.462	-3.568	(1; 1)	1J
5	2.067	15.285	8.130	-3.894	(2; 1)	3J	6.638	3.370	-3.568	(1; 1)	1J
6	2.567	18.038	4.065	-3.894	(2; 1)	3J	7.902	1.685	-3.568	(1; 1)	1J
7	3.067	8.323	0.000	-3.568	(1; 1)	1J	7.976	0.000	-5.216	(3; 1)	4J
JTAN	6.135	-7.532	-10.337	-3.568	(1; 1)	1J	-15.578	-24.939	-3.094	(2; 1)	3J
= = MEMBER 2 (3 - 4) G = =											
ITAN	0.000	9.411	-13.497	-6.496	(1; 1)	1J	16.306	-28.098	-6.170	(2; 1)	3J
1	0.250	6.175	-12.397	-6.496	(1; 1)	1J	10.149	-25.808	-6.170	(2; 1)	3J
2	0.525	2.932	-11.187	-6.496	(1; 1)	1J	3.980	*-23.269	-6.170	(2; 1)	3J
3	1.568	-6.340	-6.680	-6.496	(1; 1)	1J	-13.657	-13.740	-6.170	(2; 1)	3J
4	2.067	-9.090	-4.400	-6.496	(1; 1)	1J	-18.888	-9.160	-6.170	(2; 1)	3J
5	2.567	-10.740	-2.200	-6.496	(1; 1)	1J	-22.027	-4.580	-6.170	(2; 1)	3J
6	3.067	-11.290	0.000	-6.496	(1; 1)	1J	-10.850	0.000	-8.197	(3; 1)	4J
JTAN	6.135	16.306	28.098	-6.170	(2; 1)	3J	9.411	13.497	-6.496	(1; 1)	1J
= = MEMBER 3 (1 - 3) G = =											
ITAN	0.000	7.131	-1.965	-10.337	(1; 2)	2J	15.925	-5.542	-22.515	(4; 1)	3J
1	0.250	6.665	-1.759	-10.561	(1; 2)	2J	14.611	-4.959	-22.739	(4; 1)	3J
2	0.550	6.130	-1.466	-10.830	(1; 2)	2J	13.238	-4.185	-23.008	(4; 1)	3J
3	0.650	6.039	-1.358	-10.920	(1; 2)	2J	12.833	*-3.909	-23.098	(4; 1)	3J
4	1.300	5.416	-0.522	-11.502	(1; 2)	2J	10.925	-1.896	-23.660	(4; 1)	3J
5	1.800	4.268	0.336	-11.950	(3; 2)	4J	11.500	-0.151	-24.128	(2; 1)	3J
6	2.300	3.579	2.269	-12.398	(3; 1)	4J	13.009	0.882	-24.576	(2; 2)	3J
7	3.000	6.269	*5.491	-13.025	(3; 1)	4J	14.146	2.413	-25.203	(2; 2)	3J
8	3.250	7.798	6.749	-13.249	(3; 1)	4J	14.825	3.024	-25.427	(2; 2)	3J
JTAN	3.525	9.851	8.197	-13.496	(3; 1)	4J	15.754	3.736	-25.674	(2; 2)	3J
= = MEMBER 4 (2 - 4) G = =											
ITAN	0.000	-15.925	5.542	-22.515	(4; 1)	3J	-7.131	1.965	-10.337	(1; 2)	2J
1	0.250	-14.611	4.959	-22.739	(4; 1)	3J	-6.665	1.759	-10.561	(1; 2)	2J
2	0.550	-13.238	4.185	-23.008	(4; 1)	3J	-6.180	1.466	-10.830	(1; 2)	2J
3	0.650	-12.833	3.909	-23.098	(4; 1)	3J	-6.039	1.358	-10.920	(1; 2)	2J
4	1.300	-10.925	1.896	-23.680	(4; 1)	3J	-5.416	0.522	-11.502	(1; 2)	2J
5	1.800	-11.500	0.151	-24.128	(2; 1)	3J	-4.268	-0.338	-11.950	(3; 2)	4J
6	2.300	-13.009	-0.882	-24.576	(2; 2)	3J	-3.579	-2.269	-12.398	(3; 1)	4J
7	3.000	-14.146	-2.413	-25.203	(2; 2)	3J	-6.269	-5.491	-13.025	(3; 1)	4J
8	3.250	-14.825	-3.024	-25.427	(2; 2)	3J	-7.798	-6.749	-13.249	(3; 1)	4J
JTAN	3.525	-15.754	-3.736	-25.674	(2; 2)	3J	-9.851	-8.197	-13.496	(3; 1)	4J

ITILE..C22

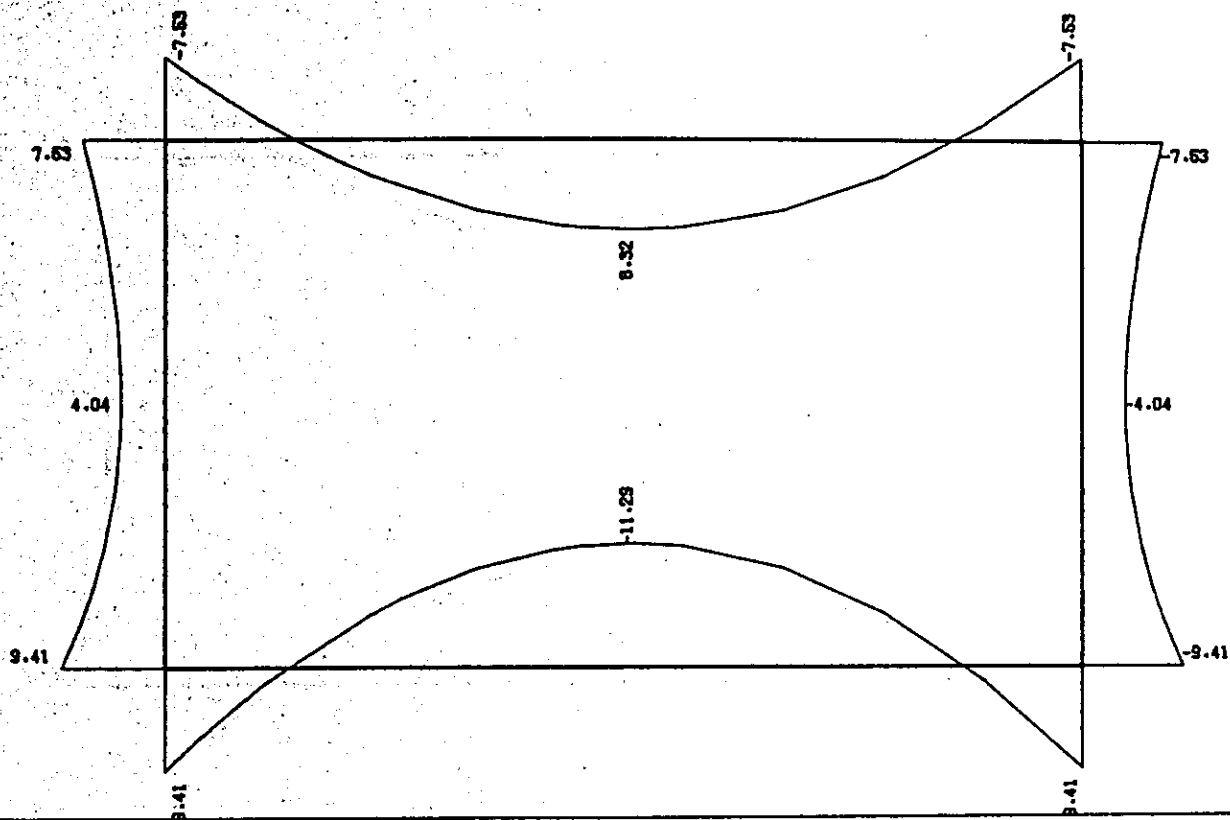
PICK UP 1

		AXIAL MAXIMUM				AXIAL MINIMUM			
		Q	M	COM	CASE	Q	M	COM	CASE
= MEMBER 1 (1 - 2) C =									
ITAN	0.000	-7.131	10.337	-1.965 (1 ; 2)		-15.925	24.939	-5.542 (4 ; 1)	3 4)
1	0.250	-4.652	9.495	-1.965 (1 ; 2)		-10.526	22.906	-5.542 (4 ; 1)	3 4)
2	0.650	-1.124	8.147	-1.965 (1 ; 2)		-2.841	19.654	-5.542 (4 ; 1)	3 4)
3	0.900	0.807	7.304	-1.965 (1 ; 2)		1.366	17.622	-5.542 (4 ; 1)	3 4)
4	1.150	2.528	6.462	-1.965 (1 ; 2)		5.114	15.589	-5.542 (4 ; 1)	3 4)
5	2.067	7.039	3.370	-1.965 (1 ; 2)		14.938	8.130	-5.542 (4 ; 1)	3 4)
6	2.567	8.302	1.685	-1.965 (1 ; 2)		17.690	4.065	-5.542 (4 ; 1)	3 4)
7	3.067	8.724	0.000	-1.965 (1 ; 2)		18.608	0.000	-5.542 (4 ; 1)	3 4)
JIAN	6.135	-7.131	-10.337	-1.965 (1 ; 2)		-15.925	-24.939	-5.542 (4 ; 1)	3 4)
= MEMBER 2 (3 - 4) C =									
ITAN	0.000	15.754	-28.098	-3.736 (2 ; 2)		9.851	-13.497	-8.197 (3 ; 1)	4)
1	0.250	9.597	-25.808	-3.736 (2 ; 2)		6.615	-12.397	-8.197 (3 ; 1)	4)
2	0.525	3.428	-23.289	-3.736 (2 ; 2)		3.372	-11.187	-8.197 (3 ; 1)	4)
3	1.568	-14.209	-13.740	-3.736 (2 ; 2)		-5.900	-6.600	-8.197 (3 ; 1)	4)
4	2.067	-19.440	-9.160	-3.736 (2 ; 2)		-8.650	-4.400	-8.197 (3 ; 1)	4)
5	2.567	-22.579	-4.580	-3.736 (2 ; 2)		-10.300	-2.200	-8.197 (3 ; 1)	4)
6	3.067	-23.625	0.000	-3.736 (2 ; 2)		-10.850	0.000	-8.197 (3 ; 1)	4)
JIAN	6.135	15.754	28.098	-3.736 (2 ; 2)		9.851	13.497	-8.197 (3 ; 1)	4)
= MEMBER 3 (1 - 3) C =									
ITAN	0.000	7.532	-3.566	-10.337 (1 ; 1)		-15.578	-3.894	-22.515 (2 ; 1)	3)
1	0.250	6.682	-3.222	-10.561 (1 ; 1)		14.646	-3.549	-22.739 (2 ; 1)	3)
2	0.550	5.787	-2.733	-10.830 (1 ; 1)		13.653	-3.060	-23.008 (2 ; 1)	3)
3	0.650	5.523	-2.552	-10.920 (1 ; 1)		13.356	-2.879	-23.098 (2 ; 1)	3)
4	1.300	4.297	-1.157	-11.502 (1 ; 1)		11.918	-1.483	-23.680 (2 ; 1)	3)
5	1.800	4.042	0.175	-11.950 (1 ; 1)		11.500	-0.151	-24.128 (2 ; 1)	3)
6	2.300	4.509	1.732	-12.398 (1 ; 1)		11.804	1.605	-24.576 (2 ; 1)	3)
7	3.000	6.591	4.289	-13.025 (1 ; 1)		13.657	3.983	-25.203 (2 ; 1)	3)
8	3.250	7.790	5.309	-13.249 (1 ; 1)		14.774	4.983	-25.427 (2 ; 1)	3)
JIAN	3.525	9.411	6.496	-13.496 (1 ; 1)		16.306	6.170	-25.674 (2 ; 1)	3)
= MEMBER 4 (2 - 4) C =									
ITAN	0.000	-7.532	3.566	-10.337 (1 ; 1)		-15.578	3.894	-22.515 (2 ; 1)	3)
1	0.250	-6.682	3.222	-10.561 (1 ; 1)		-14.646	3.549	-22.739 (2 ; 1)	3)
2	0.550	-5.787	2.733	-10.830 (1 ; 1)		-13.653	3.060	-23.008 (2 ; 1)	3)
3	0.650	-5.523	2.552	-10.920 (1 ; 1)		-13.356	2.879	-23.098 (2 ; 1)	3)
4	1.300	-4.297	1.157	-11.502 (1 ; 1)		-11.918	1.483	-23.680 (2 ; 1)	3)
5	1.800	-4.042	0.175	-11.950 (1 ; 1)		-11.500	0.151	-24.128 (2 ; 1)	3)
6	2.300	-4.509	1.732	-12.398 (1 ; 1)		-11.804	1.605	-24.576 (2 ; 1)	3)
7	3.000	-6.591	4.289	-13.025 (1 ; 1)		-13.657	3.983	-25.203 (2 ; 1)	3)
8	3.250	-7.790	5.309	-13.249 (1 ; 1)		-14.774	4.983	-25.427 (2 ; 1)	3)
JIAN	3.525	-9.411	6.496	-13.496 (1 ; 1)		-16.306	6.170	-25.674 (2 ; 1)	3)

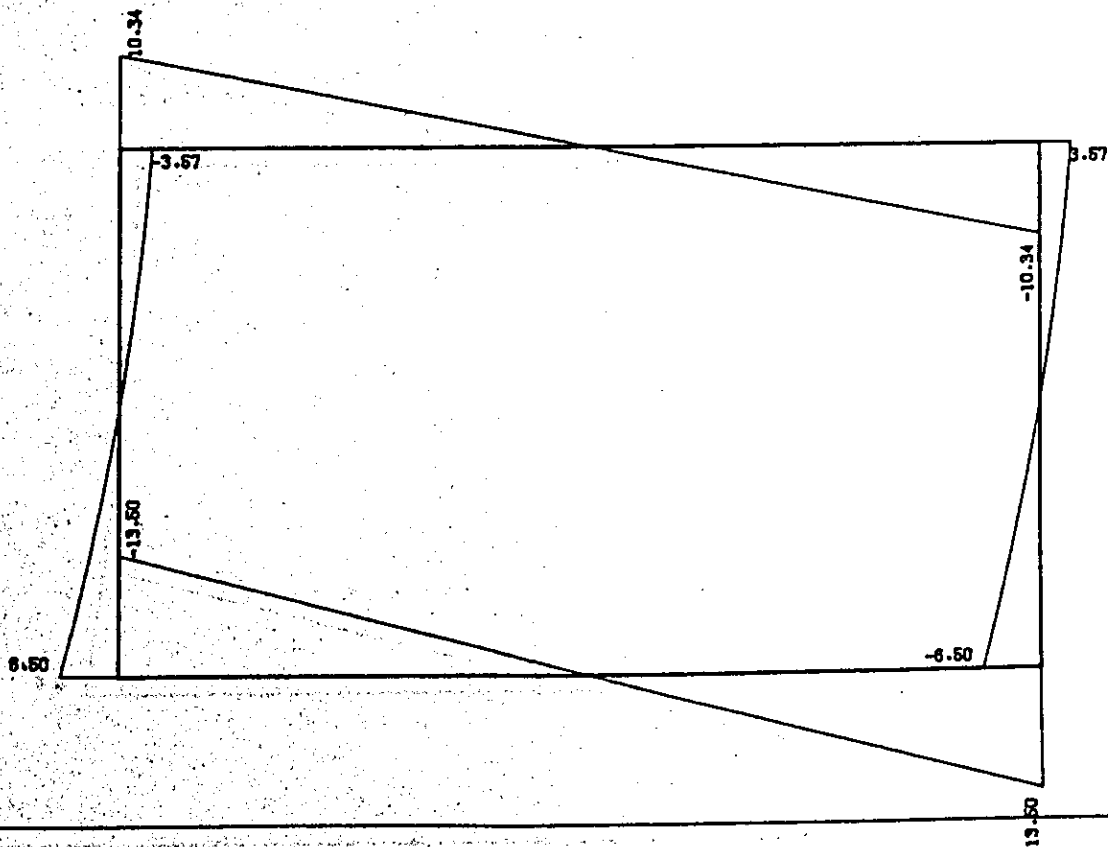
C22

CASE 1 (SHI+D0 (0.5))

BENDING MOMENT



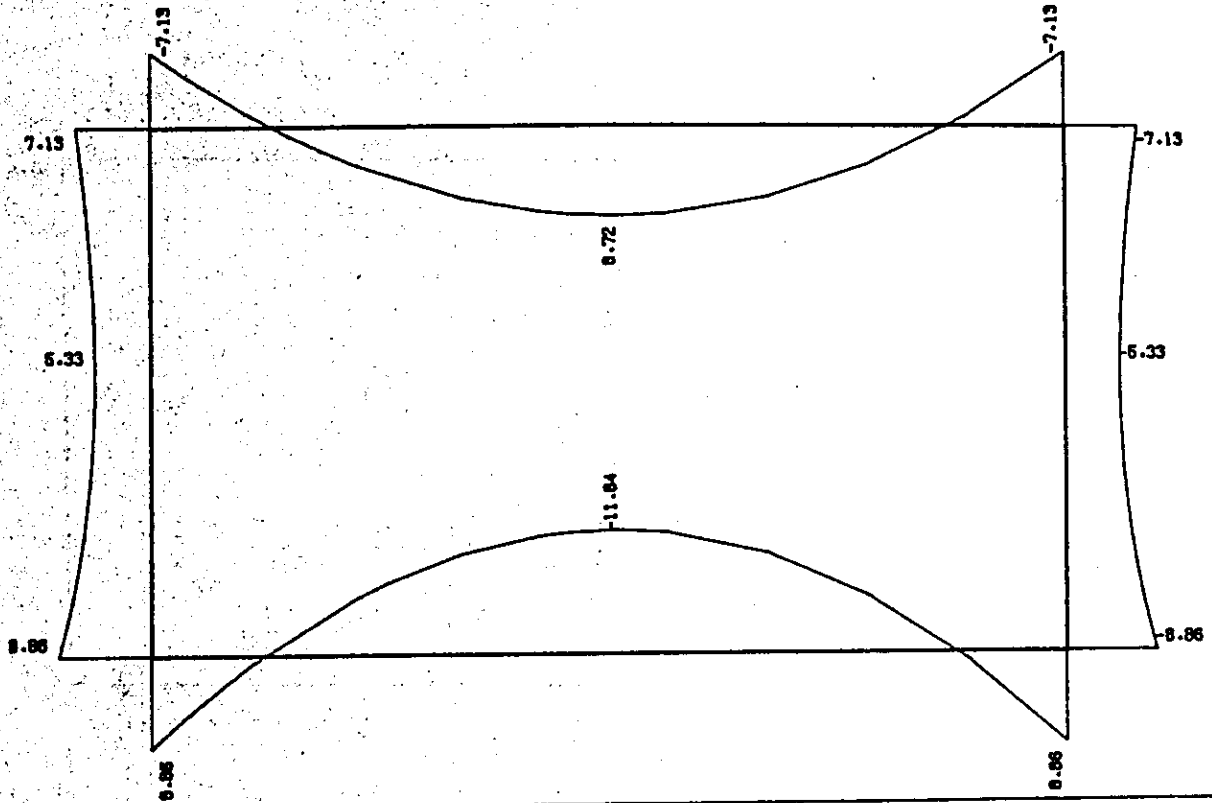
SHEARING FORCE



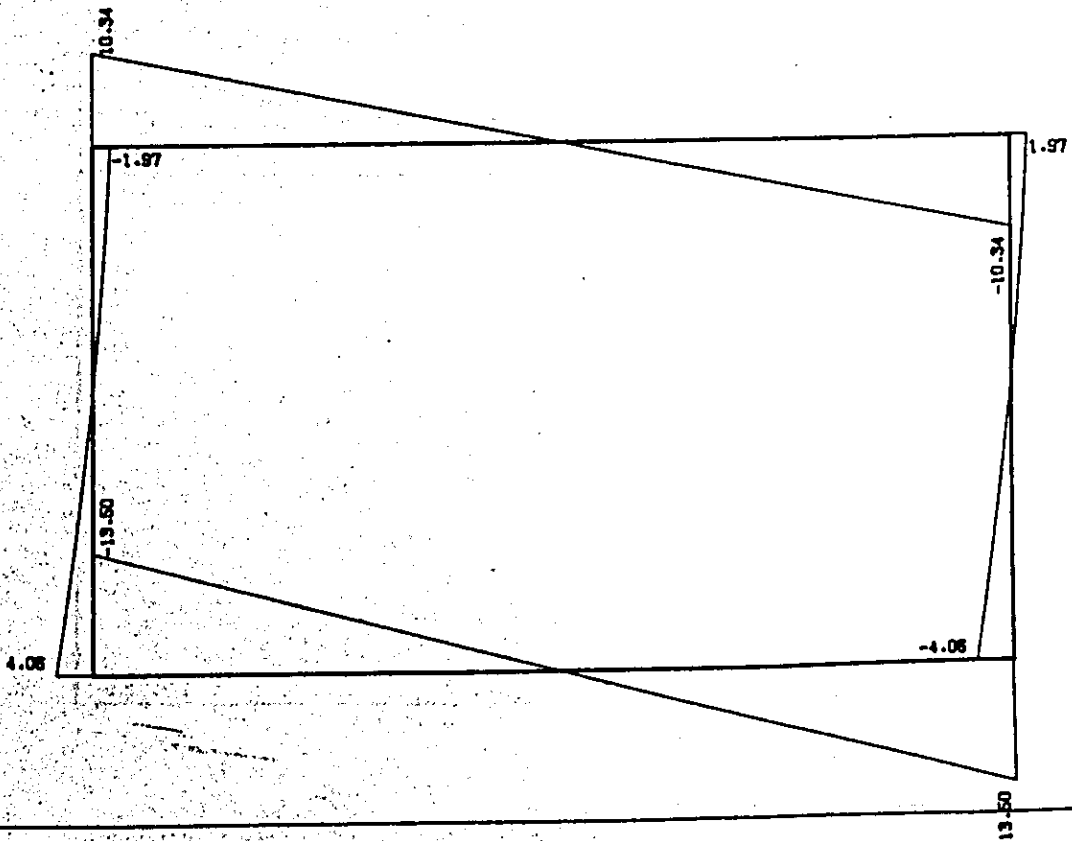
C22

CASE 2 (SHI+D0 (0.3))

BENDING MOMENT



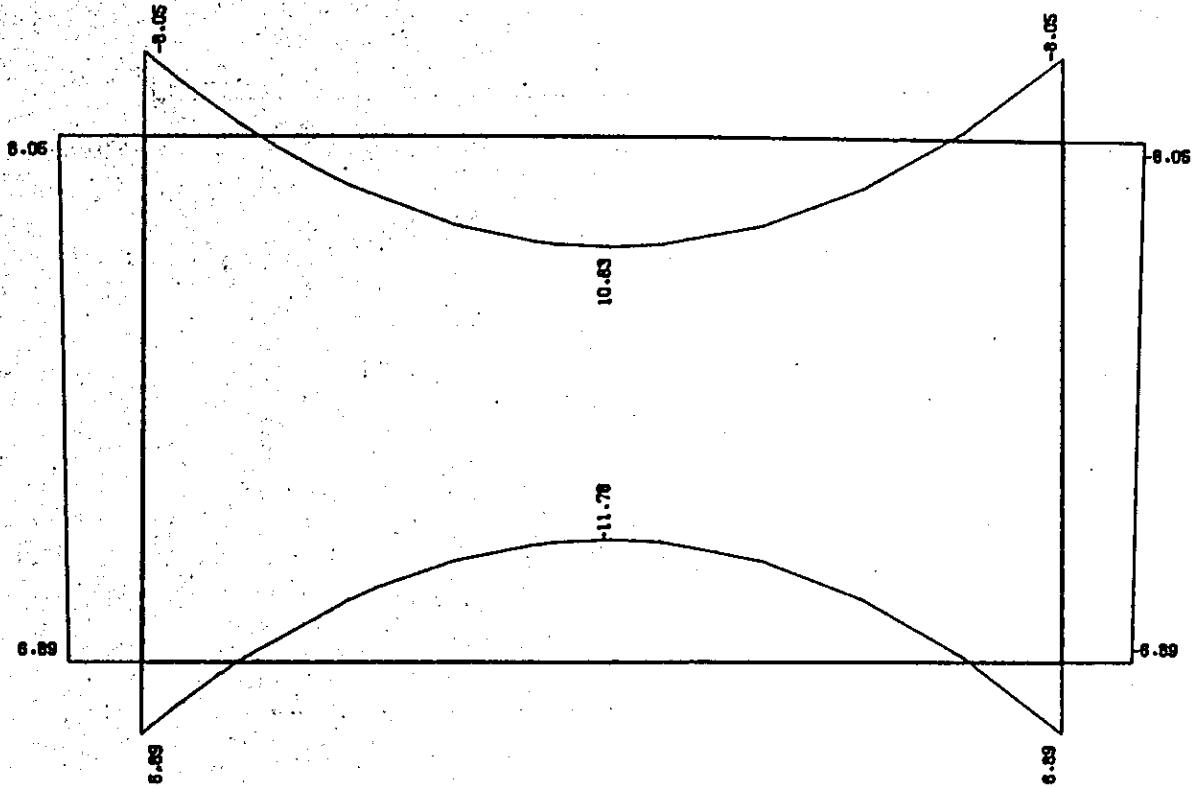
SHEARING FORCE



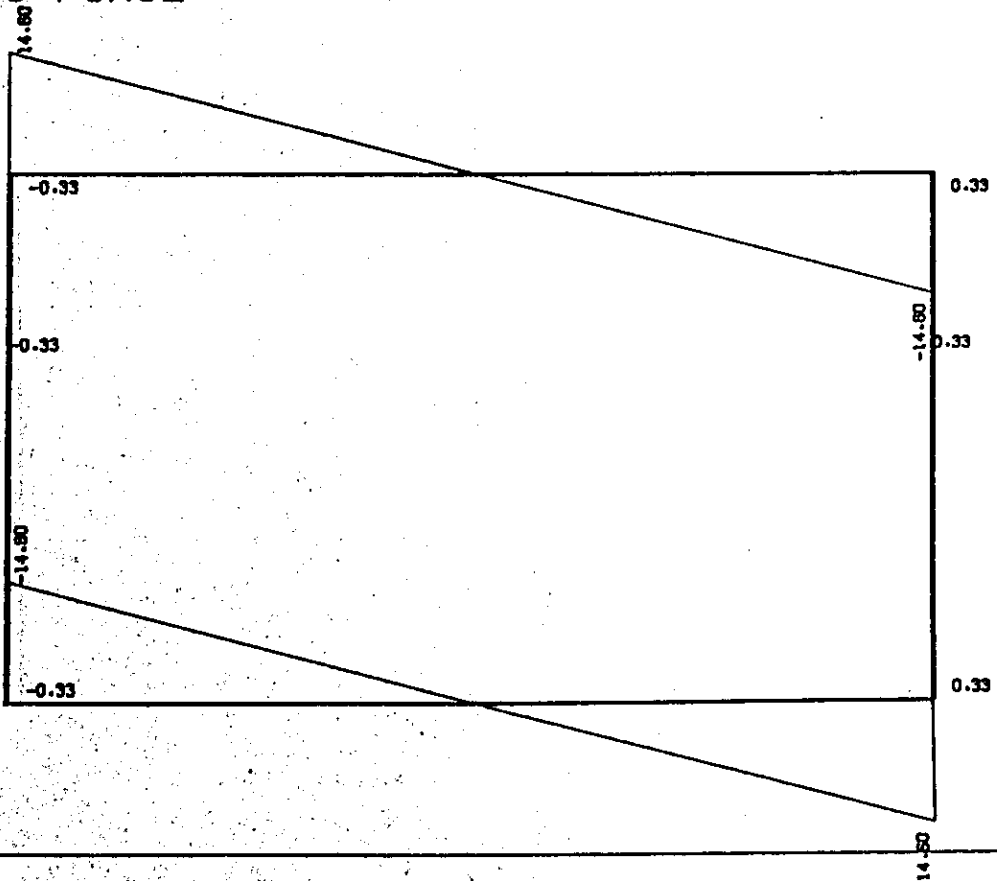
C22

CASE 3 (KATSU+SYOU)

BENDING MOMENT



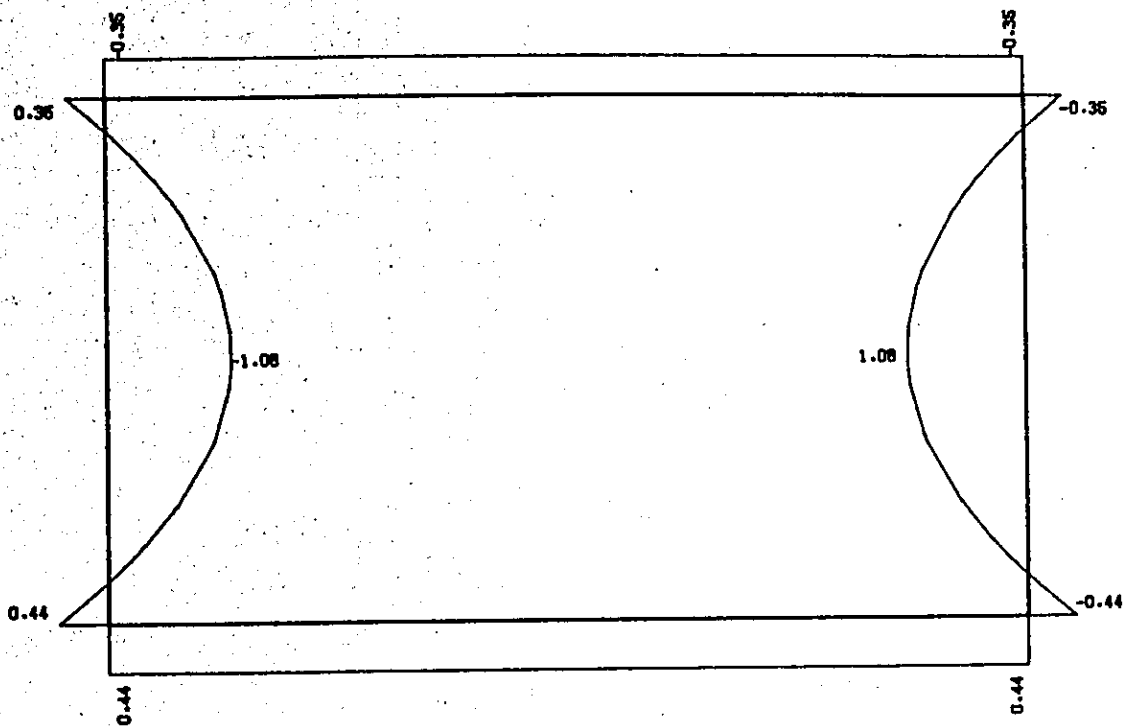
SHEARING FORCE



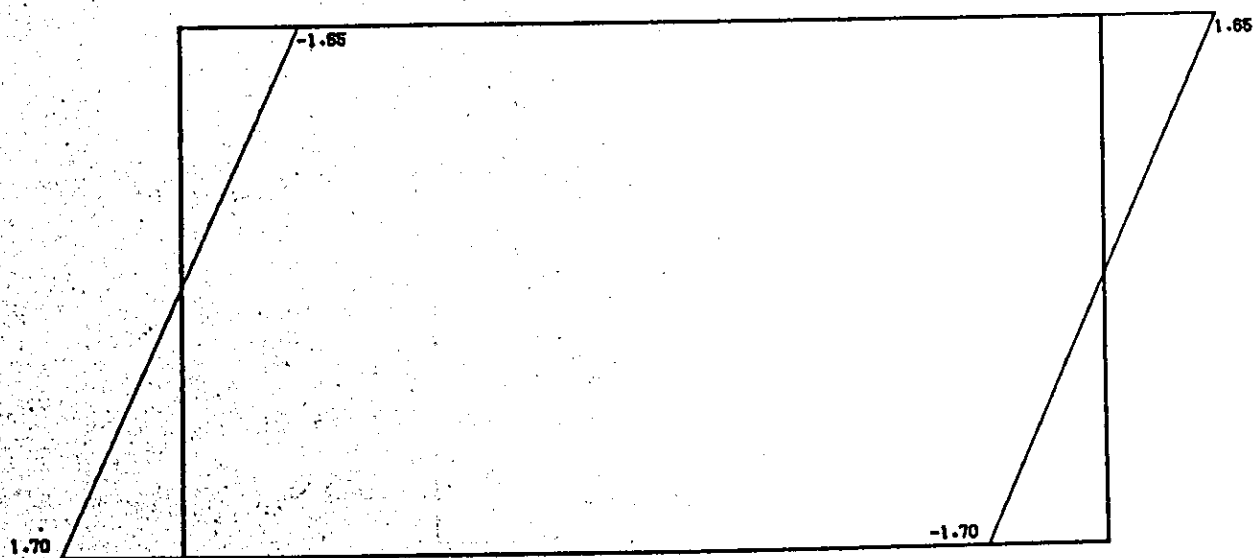
C22

CASE 4 (KATSUDO)

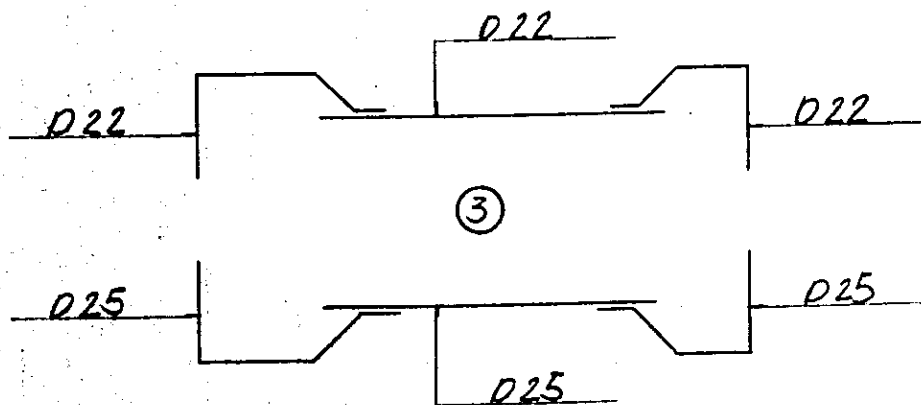
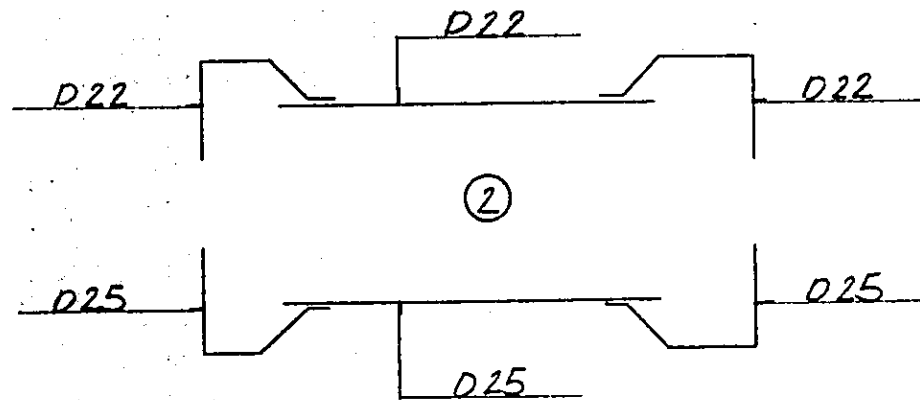
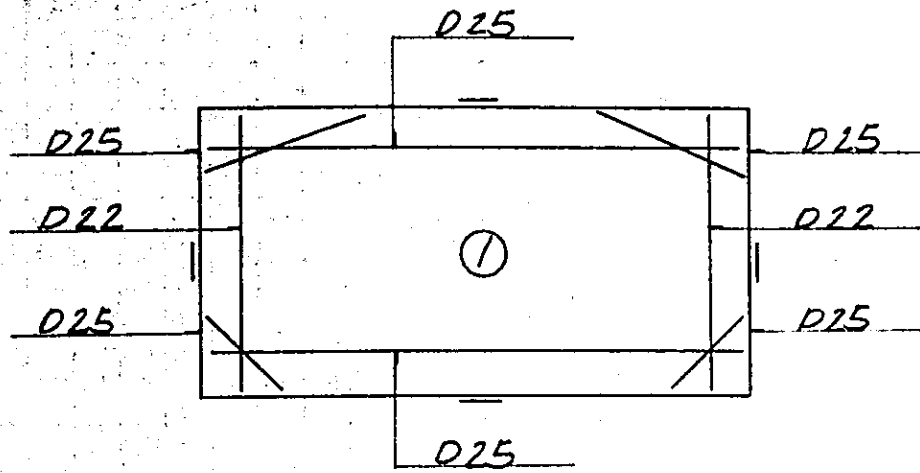
BENDING MOMENT



SHEARING FORCE



REINFORCEMENT FRAME



C22 CRACKING

STRESS

SGM CA = 0.000 (KG/M2)
 SGM SA = 0.000 (KG/M2)
 TAU A = 0.000 (KG/M2)

		NO. (1) 1-2	NO. (2) 1-2	NO. (3) 1-2
B	(CM)	100.00	100.00	100.00
H	(CM)	80.00	50.00	50.00
D	(CM)	73.00	43.00	43.00
D'	(CM)	0.00	7.00	7.00
D''	(CM)	7.00	7.00	7.00
		4.00 D-25	4.00 D-25	4.00 D-25
		4.00 D-22		4.00 D-22
AS	(CM2)	35.752	20.268	35.752
P		0.00489	0.00471	0.00831
			4.00 D-22	4.00 D-25
AS'	(CM2)	0.000	15.484	20.268
P'		0.00000	0.00360	0.00471
M	(TMM)	7.53	1.53	8.72
N	(T)	3.57	3.57	1.97
S	(T)	0.00	0.00	0.00
EO	(CM)	210.924	42.857	442.639
E	(CM)	243.924	60.857	460.639
E'	(CM)	170.924	24.857	424.639
E'/E		0.700	0.408	0.921
D'/D		0.000	0.162	0.162
D/E		0.299	0.706	0.093
N*E/E'*D2		1.634	1.175	4.907
K			0.422	0.374
LC			0.209	0.197
C		0.157		
BETA		26.909		
SGM C (KG/CM2)		10.36	5.61	24.89
SGM S (KG/CM2)		279.00	115.03	624.48
TAU (KG/CM2)		0.00	0.00	0.00

C22 CRACKING

STRESS

SGM CA = 0.000 (KG/M2)
 SGM SA = 0.000 (KG/M2)
 TAU A = 0.000 (KG/M2)

		NO. (4) 3-4	NO. (5) 3-4
B	(CM)	100.00	100.00
H	(CM)	55.00	55.00
D	(CM)	46.00	48.00
D'	(CM)	0.00	9.00
D''	(CM)	9.00	7.00
AS	(CM2)	8.00 D-25 40.536	8.00 D-25 40.536
P		0.00881	0.00844
AS'	(CM2)	0.000	4.00 D-25 20.268
P'		0.00000	0.00422
M	(T*M)	6.18	11.84
N	(T)	6.50	4.06
S	(T)	0.00	0.00
E0	(CM)	95.076	291.625
E	(CM)	113.576	312.125
E'	(CM)	67.576	273.125
E'/E		0.594	0.875
D'/D		0.000	0.187
D/E		0.405	0.153
N*E/B*D2		3.488	5.500
K			0.370
LC			0.196
C		0.176	
BETA		17.318	
SGM C (KG/CM2)		17.78	27.98
SGM S (KG/CM2)		308.02	655.74
TAU (KG/CM2)		0.00	0.00

C22. CRACKING

STRESS

SGM CA = 0.000 (KG/M2)
 SGM SA = 0.000 (KG/M2)
 TAU A = 0.000 (KG/M2)

		NO. (6) 1-3.2-4	NO. (7) 1-3.2-4	NO. (8) 1-3.2-4	NO. (9) 1-3.2-4
B	(CM)	100.00	100.00	100.00	100.00
H	(CM)	60.00	50.00	50.00	50.00
D	(CM)	53.00	43.00	43.00	43.00
D'	(CM)	0.00	7.00	7.00	0.00
D''	(CM)	7.00	7.00	7.00	7.00
		4.00 D-25	4.00 D-25	4.00 D-25	8.00 D-25
		4.00 D-22	4.00 D-22		
AS	(CM2)	35.752	35.752	20.268	40.536
P		0.00674	0.00831	0.00471	0.00942
			4.00 D-22	4.00 D-22	
AS'	(CM2)	0.000	15.484	15.484	0.000
P'		0.00000	0.00360	0.00360	0.00000
M	(T*M)	7.53	6.18	5.35	7.84
N	(T)	10.56	10.92	11.95	13.25
S	(T)	0.00	0.00	0.00	0.00
E0	(CM)	71.306	56.593	44.769	59.169
E	(CM)	94.306	74.593	62.769	77.169
E'	(CM)	41.306	38.593	26.769	34.169
E'/E		0.438	0.517	0.426	0.442
D'/D		0.000	0.162	0.162	0.000
D/E		0.561	0.576	0.683	0.557
N#E/B#D2		3.545	4.405	4.056	5.530
K			0.471	0.415	
LC			0.228	0.206	
C		0.193			0.211
BETA		17.769			14.492
SGM C (KG/CM2)		18.28	19.28	19.63	26.18
SGM S (KG/CM2)		324.83	323.83	413.45	379.49
TAU (KG/CM2)		0.00	0.00	0.00	0.00

C22 FATIGUE

STRESS

SGM CA = 0.000 (KG/M2)
 SGM SA = 0.000 (KG/M2)
 TAU A = 0.000 (KG/M2)

		NO. (1) 1-2	NO. (2) 1-2	NO. (3) 1-2
B	(CM)	100.00	100.00	100.00
H	(CM)	80.00	50.00	50.00
D	(CM)	73.00	43.00	43.00
D'	(CM)	0.00	7.00	7.00
D''	(CM)	7.00	7.00	7.00
		4.00 D-25	4.00 D-25	4.00 D-25
		4.00 D-22		4.00 D-22
AS	(CM2)	35.752	20.268	35.752
P		0.00487	0.00471	0.00831
			4.00 D-22	4.00 D-25
AS'	(CM2)	0.000	15.484	20.268
P'		0.00000	0.00360	0.00471
M	(T*M)	15.93	2.84	19.36
N	(T)	5.54	5.22	2.29
S	(T)	0.00	0.00	0.00
E0	(CM)	287.545	54.406	845.414
E	(CM)	320.545	72.406	863.414
E'	(CM)	247.545	36.406	827.414
E'/E		0.772	0.502	0.958
D'/D		0.000	0.162	0.162
D/E		0.227	0.593	0.049
N*E/B*D2		3.332	2.044	10.693
K			0.390	0.368
LC			0.196	0.194
C		0.153		
BETA		28.265		
SGM C (KG/CM2)		21.73	10.41	54.95
SGM S (KG/CM2)		614.36	243.66	1413.97
TAU (KG/CM2)		0.00	0.00	0.00

C22 FATIGUE

STRESS

SGM CA = 0.000 (KG/M2)
 SGM SA = 0.000 (KG/M2)
 TAU A = 0.000 (KG/M2)

		NO. (4) 3-4	NO. (3) 3-4
B	(CM)	100.00	100.00
H	(CM)	55.00	55.00
D	(CM)	46.00	48.00
D'	(CM)	0.00	9.00
D''	(CM)	9.00	7.00
		8.00 D-25	8.00 D-25
AS	(CM2)	40.536	40.536
F		0.00881	0.00844
			4.00 D-25
AS'	(CM2)	0.000	20.268
F'		0.00000	0.00422
M	(Γ*M)	10.59	23.63
N	(T)	7.87	3.74
S	(T)	0.00	0.00
EO	(CM)	134.561	631.818
E	(CM)	153.061	652.318
E'	(CM)	107.061	613.318
E'/E		0.699	0.940
D'/D		0.000	0.187
D/E		0.300	0.073
N*E/B*D2		5.692	10.588
K			0.379
LC			0.191
C		0.189	
BETA		18.757	
SGM C (KG/CM2)		30.07	55.26
SGM S (KG/CM2)		564.18	1357.62
TAU (KG/CM2)		0.00	0.00

C22 FATIGUE

STRESS

SGM CA = 0.000 (KG/M2)
 SGM SA = 0.000 (KG/M2)
 TAU A = 0.000 (KG/M2)

		NO. (6) 1-3.2-4	NO. (7) 1-3.2-4	NO. (8) 1-3.2-4	NO. (9) 1-3.2-4
B	(CM)	100.00	100.00	100.00	100.00
H	(CM)	60.00	50.00	50.00	50.00
D	(CM)	53.00	43.00	43.00	43.00
D'	(CM)	0.00	7.00	7.00	0.00
D''	(CM)	7.00	7.00	7.00	7.00
		4.00 D-25 4.00 D-22	4.00 D-25 4.00 D-22	4.00 D-25	8.00 D-25
AS	(CM ²)	35.752	35.752	20.268	40.536
P		0.00674	0.00331	0.00471	0.00942
			4.00 D-22	4.00 D-22	
AS'	(CM ²)	0.000	15.484	15.484	0.000
P'		0.00000	0.00360	0.00360	0.00000
M	(T*MM)	15.93	14.05	12.81	14.83
N	(T)	22.74	23.10	24.13	25.43
S	(T)	0.00	0.00	0.00	0.00
EO	(CM)	70.052	60.822	53.087	58.316
E	(CM)	93.052	78.822	71.087	76.316
E'	(CM)	40.052	42.822	35.087	33.316
E'/E		0.430	0.543	0.493	0.436
D'/D		0.000	0.162	0.162	0.000
D/E		0.569	0.545	0.604	0.563
N*E./B*D ²		7.533	9.847	9.277	10.496
K			0.464	0.393	
LC			0.225	0.197	
C		0.194			0.211
BETA		17.636			14.404
SGM C (KG/CM ²)		38.71	43.67	46.98	49.58
SGM S (KG/CM ²)		682.70	756.74	1086.28	714.19
TAU (KG/CM ²)		0.00	0.00	0.00	0.00

Stress calculation

Stress calculation of slab

1-2 (1)

(for slab calculation)

Section	Standard strength for design $\sigma_{ck} = 240 \text{ kg/cm}^2$ Re-bar SD 30 $AS = 35.80 \text{ cm}^2$ (4 - D $\frac{25}{22}$) $AS' = 0$ " (- D)		
	Analysis of cracking	Analysis of fatigue	Analysis of registing power
Bending moment	$M_d = 7.53 \text{ t m}$	$M_{d \ell i} = 15.93 \text{ t m}$	$M = 15.93 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 10.4 \text{ kg/cm}^2$ $\sigma_s = 279 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 21.7 \text{ kg/cm}^2$ $\sigma_s = 614 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 21.7 \text{ kg/cm}^2$ $\sigma_s = 614 \text{ "}$ $\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{614 - 279}{614} = 0.54 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

2. Allowable stress of re-bar in terms of fatigue

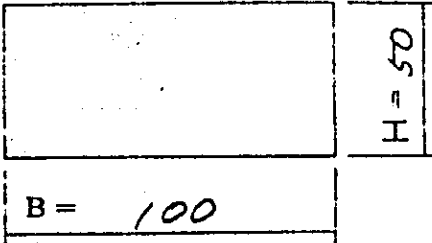
Span for fatigue analysis KS-16 $l = 4.35 \text{ m}$ $\sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 279 + (1 - 279 / 5000) \times 1800 = 1980 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation
Stress calculation of slab

1-2 (2)

(for slab calculation)

Section	Standard strength for design $\sigma_{ck} = 240 \text{ kg/cm}^2$ Re-bar SD 30 $A_s = 20.30 \text{ cm}^2 (4-D25)$ $A_s' = 15.50 \text{ cm}^2 (4-D22)$		
			
	Analysis of cracking	Analysis of fatigue	Analysis of registering power
Bending moment	$M_d = 1.53 \text{ t m}$	$M_{d \ell i} = 2.84 \text{ t m}$	$M = 2.84 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 5.6 \text{ kg/cm}^2$ $\sigma_s = 115 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 10.4 \text{ kg/cm}^2$ $\sigma_s = 244 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 10.4 \text{ kg/cm}^2$ $\sigma_s = 244 \text{ "}$ $\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{244 - 115}{244} = 0.53 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis $K_s-16 \quad \ell = 6.135 \quad \sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 115 + (1 - 115 / 5000) \times 1800 = 1870 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

1-2 (3)

(for slab calculation)

Section	Standard strength for design $\sigma_{ck} = 240 \text{ kg/cm}^2$		
	Re-bar SD 30 AS = 35.80 cm ² (4 - D 25) AS' = 20.30 cm ² (4 - D 25)		
	<div style="border: 1px solid black; padding: 5px; display: inline-block;"> B = 100 </div>		H = 50
	Analysis of cracking	Analysis of fatigue	Analysis of registering power
Bending moment	$M_d = 8.72 \text{ t m}$	$M_{d \ell i} = 19.36 \text{ t m}$	$M = 19.36 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 24.9 \text{ kg/cm}^2$ $\sigma_s = 624 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 55.0 \text{ kg/cm}^2$ $\sigma_s = 1414 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 55 \text{ kg/cm}^2$ $\sigma_s = 1414 \text{ "}$ $\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static/Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{1414 - 624}{1414} = 0.56 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis Ks-16 $\ell = 6.135 \text{ m}$ $\sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 624 + (1 - 624 / 5000) \times 1800 = 2210 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

3-4 (4)

(for slab calculation)

Section	Standard strength		
	for design $\sigma_{ck} = 240 \text{ kg/cm}^2$		
		$H = 55$	Re-bar SD 30
			$A_s = 40.50 \text{ cm}^2 (8 - D25)$
	$B = 100$		$A_s' = 0 (- D)$
	Analysis of cracking	Analysis of fatigue	Analysis of registering power
Bending moment	$M_d = 6.18 \text{ t m}$	$M_{d \ell i} = 10.59 \text{ t m}$	$M = 10.59 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 17.8 \text{ kg/cm}^2$	$\sigma_c = 30.1 \text{ kg/cm}^2$	$\sigma_c = 30.1 \text{ kg/cm}^2$
	$\sigma_s = 308 \text{ "}$	$\sigma_s = 564 \text{ "}$	$\sigma_s = 564 \text{ "}$
	$\tau = \text{---} \text{ "}$	$\tau = \text{---} \text{ "}$	$\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$	$\sigma_{ca} = 90 \text{ kg/cm}^2$	$\sigma_{ca} = 90 \text{ kg/cm}^2$
	$\sigma_{sa} = 1000 \text{ "}$	$\sigma_{sa} = 1800 \text{ "}$	$\sigma_{sa} = 1800 \text{ "}$
	$\tau_a = \text{---} \text{ "}$	$\tau_a = \text{---} \text{ "}$	$\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{564 - 308}{564} = 0.45 > 0.25$$

 $(\alpha \geq 0.25 \rightarrow \text{Dynamic } \sigma_{sa} = 1000 \text{ kg/cm}^2,$ $\alpha \leq 0.25 \rightarrow \text{Static } \sigma_{sa} = 1200 \text{ kg/cm}^2)$

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis ks-16 $\ell = 6.135$ $\sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 308 + (1 - 308 / 5000) \times 1800 = 2000 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

3-4 (5)

(for slab calculation)

Section	Standard strength		
	for design $\sigma_{ck} = 240 \text{ kg/cm}^2$		
		$H = 55$	Re-bar SD 30
			$A_s = 40.50 \text{ cm}^2$ (8-D 25)
	$B = 100$		$A_s' = 20.304$ (4-D 25)
	Analysis of cracking	Analysis of fatigue	Analysis of registering power
Bending moment	$M_d = 11.84 \text{ t m}$	$M_{d \ell i} = 23.63 \text{ t m}$	$M = 23.63 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 28.0 \text{ kg/cm}^2$ $\sigma_s = 656 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 55.3 \text{ kg/cm}^2$ $\sigma_s = 1358 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 55.3 \text{ kg/cm}^2$ $\sigma_s = 1358 \text{ "}$ $\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{1358 - 656}{1358} = 0.52 > 0.25$$

 $(\alpha \geq 0.25 \rightarrow \text{Dynamic } \sigma_{sa} = 1000 \text{ kg/cm}^2,$ $\alpha \leq 0.25 \rightarrow \text{Static } \sigma_{sa} = 1200 \text{ kg/cm}^2)$

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis $k_s - 16$ $\ell = 6.135$ $\sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 656 + (1 - 656 / 5000) \times 1800 = 2220 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

1-3, 2-4 (6)

(for slab calculation)

Section	Standard strength		
	for design $\sigma_{ck} = 240 \text{ kg/cm}^2$		
		Re-bar SD 30	
		$A_s = 35.80 \text{ cm}^2$ (4 - D $\frac{25}{22}$)	
	$B = 100$	$A_s' = 0$ (- D)	
	Analysis of cracking	Analysis of fatigue	Analysis of registing power
Bending moment	$M_d = 7.53 \text{ t m}$	$M_{d \ell i} = 15.93 \text{ t m}$	$M = 15.93 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 18.3 \text{ kg/cm}^2$ $\sigma_s = 325 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 38.7 \text{ kg/cm}^2$ $\sigma_s = 683 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 38.7 \text{ kg/cm}^2$ $\sigma_s = 683 \text{ "}$ $\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{683 - 325}{683} = 0.52 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis $k_s - 16 \quad \ell = 6.135 \quad \sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{min} + (1 - \sigma_{min} / 5000) \times \sigma_{rao} \\ &= 325 + (1 - 325 / 5000) \times 1800 = 2010 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

1-3, 2-4 (7)

(for slab calculation)

Section	Standard strength		
	for design $\sigma_{ck} = 240 \text{ kg/cm}^2$		
		Re-bar SD 30	
		$A_s = 35,80 \text{ cm}^2$ (4 - D $\frac{25}{22}$)	
	$B = 100$	$A_s' = 15,50 \text{ cm}^2$ (4 - D22)	
	Analysis of cracking	Analysis of fatigue	Analysis of registing power
Bending moment	$M_d = 6.18 \text{ t m}$	$M_{d \ell i} = 14,05 \text{ t m}$	$M = 14,05 \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 19,3 \text{ kg/cm}^2$	$\sigma_c = 43,7 \text{ kg/cm}^2$	$\sigma_c = 43,7 \text{ kg/cm}^2$
	$\sigma_s = 324 \text{ "}$	$\sigma_s = 757 \text{ "}$	$\sigma_s = 757 \text{ "}$
	$\tau = \text{---} \text{ "}$	$\tau = \text{---} \text{ "}$	$\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$	$\sigma_{ca} = 90 \text{ kg/cm}^2$	$\sigma_{ca} = 90 \text{ kg/cm}^2$
	$\sigma_{sa} = 1000 \text{ "}$	$\sigma_{sa} = 1800 \text{ "}$	$\sigma_{sa} = 1800 \text{ "}$
	$\tau_a = \text{---} \text{ "}$	$\tau_a = \text{---} \text{ "}$	$\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{757 - 324}{757} = 0.57 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis $k_s - 16 \quad \ell = 6,135 \quad \sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 324 + (1 - 324 / 5000) \times 1800 = 2010 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

1-3, 2-4 (8)

(for slab calculation)

Section	Standard strength		
	for design $\sigma_{ck} = 240 \text{ kg/cm}^2$		
			Re-bar SD 30
			$A_s = 20.30 \text{ cm}^2$ (4-D 25)
			$A_s' = 15.50 \text{ cm}^2$ (4-D 22)
	$B = 100$	$H = 50$	
	Analysis of cracking	Analysis of fatigue	Analysis of registering power
Bending moment	$M_d = 5.35 \text{ t m}$	$M_{d \ell i} = 12.81 \text{ t m}$	$M = \text{---} \text{ t m}$
Shearing force	$S_d = \text{---} \text{ t}$	$S_{d \ell i} = \text{---} \text{ t}$	$S = \text{---} \text{ t}$
Stress	$\sigma_c = 19.6 \text{ kg/cm}^2$ $\sigma_s = 413 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 47.0 \text{ kg/cm}^2$ $\sigma_s = 1086 \text{ "}$ $\tau = \text{---} \text{ "}$	$\sigma_c = 47.0 \text{ kg/cm}^2$ $\sigma_s = 1086 \text{ "}$ $\tau = \text{---} \text{ "}$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ "}$ $\tau_a = \text{---} \text{ "}$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{1086 - 413}{1086} = 0.62 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis $\ell_{s-16} = 6.135 \text{ m}$ $\sigma_{rao} = 1800 \text{ kg/cm}^2$

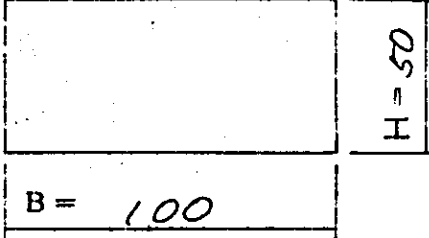
$$\begin{aligned} \sigma_{sa} &= \sigma_{\min} + (1 - \sigma_{\min} / 5000) \times \sigma_{rao} \\ &= 413 + (1 - 413 / 5000) \times 1800 = 2060 \text{ kg/cm}^2 \end{aligned}$$

Stress calculation

Stress calculation of slab

1-3, 2-4 (9)

(for slab calculation)

Section	Standard strength for design $\sigma_{ck} = 240 \text{ kg/cm}^2$ Re-bar SD 30 $A_s = 40.50 \text{ cm}^2 (8-D25)$ $A_s' = 0 (- D)$		
	 $B = 100$ $H = 50$		
	Analysis of cracking	Analysis of fatigue	Analysis of registering power
Bending moment	$M_d = 7.84 \text{ t m}$	$M_{d \ell i} = 14.83 \text{ t m}$	$M = \text{ t m}$
Shearing force	$S_d = \text{ t}$	$S_{d \ell i} = \text{ t}$	$S = \text{ t}$
Stress	$\sigma_c = 26.2 \text{ kg/cm}^2$ $\sigma_s = 379 \text{ kg/cm}^2$ $\tau = \text{ kg/cm}^2$	$\sigma_c = 50.0 \text{ kg/cm}^2$ $\sigma_s = 714 \text{ kg/cm}^2$ $\tau = \text{ kg/cm}^2$	$\sigma_c = 50.0 \text{ kg/cm}^2$ $\sigma_s = 714 \text{ kg/cm}^2$ $\tau = \text{ kg/cm}^2$
Allowable stress	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1000 \text{ kg/cm}^2$ $\tau_a = \text{ kg/cm}^2$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ kg/cm}^2$ $\tau_a = \text{ kg/cm}^2$	$\sigma_{ca} = 90 \text{ kg/cm}^2$ $\sigma_{sa} = 1800 \text{ kg/cm}^2$ $\tau_a = \text{ kg/cm}^2$

1. Allowable stress applied for analysis of cracking

Static / Dynamic distinction

$$\alpha = \frac{\sigma_{\ell i}}{\sigma_d + \sigma_{\ell i}} = \frac{714 - 379}{714} = 0.47 > 0.25$$

($\alpha \geq 0.25 \rightarrow$ Dynamic $\sigma_{sa} = 1000 \text{ kg/cm}^2$,

$\alpha \leq 0.25 \rightarrow$ Static $\sigma_{sa} = 1200 \text{ kg/cm}^2$)

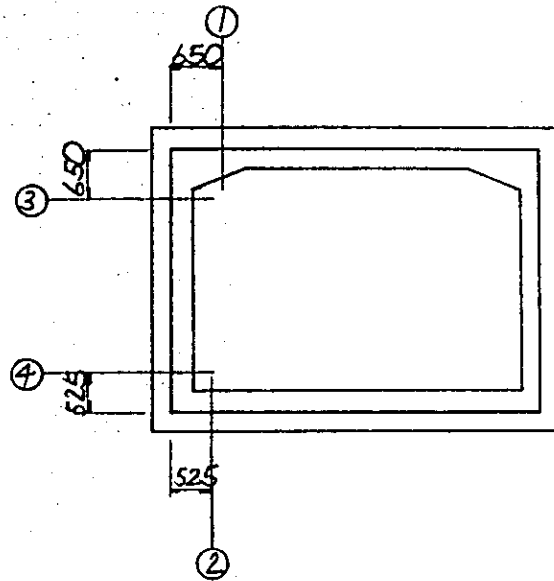
2. Allowable stress of re-bar in terms of fatigue

Span for fatigue analysis $k_s - 16 \quad \ell = 6.735 \quad \sigma_{rao} = 1800 \text{ kg/cm}^2$

$$\begin{aligned} \sigma_{sa} &= \sigma_{min} + (1 - \sigma_{min} / 5000) \times \sigma_{rao} \\ &= 379 + (1 - 379 / 5000) \times 1800 = 2040 \text{ kg/cm}^2 \end{aligned}$$

Calculation of shearing force

Sections of calculation of shearing stress



$$\tau = \frac{S}{b \cdot d}$$

b = 100 cm

h = Thickness of member

d' = Thickness of concrete cover of tension bar

d = Effective height

Sections of calculation	h cm	d' cm	d = h-d' cm	S kg	τ kg/cm ²	ρ %	τ_a kg/cm ²
1	50	7	43	19650	4.6	0.76	6.2
2	55	9	46	23290	5.1	0.76	6.2
3	50	7	43	3910	0.9	0.76	6.2
4	50	7	43	5490	1.3	0.76	6.2

