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FOR
THE FEASIBILITY STUDY
ON
THE CONSTRUCTION OF DRY DOCK
IN
THE SOCIALIST REPUBLIC OF THE UNION OF BURMA

SUPPLEMENTARY DATA

- 1. SURVEYING
- 2. SOIL INVESTIGATION

MARCH,1984

JAPAN INTERNATIONAL COOPERATION AGENCY

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Boring Logs Soil Test Data Sheet Survey Map Calculation Sheets of Traversing

1. Objectives

Upon request by the Government of the Socialist Republic of the Union of Burma, a feasibility study for the construction of a dry dock was conducted by the Japan International Cooperation Agency (JICA). In order to obtain the data required to determine primary design costs for civil works and buildings, which form the basis for calculating the construction costs of this feasibility study, surveys and soil investigation were carried out between August and December 1982.

This document comprises supplementary data based on the results of the surveys and soil investigation undertaken for the feasibility study report. The surveys and soil investigation were conducted by the Waterways Department and the Construction Corporation (CC), respectively, under the guidance and supervision of the feasibility study team.

Surveys

2-1. Survey Area

The surveys were carried out at the proposed project site as designated by the Burma Dockyard Corporation (BDC), including in the area set aside for future expansion.

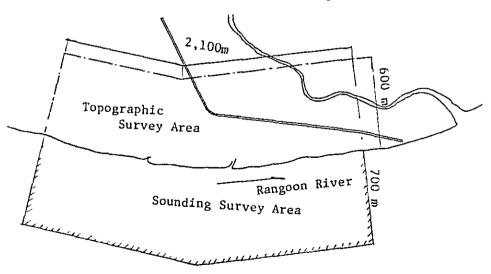


Fig. 2-1 Survey Area

2-2. Items to be Surveyed

- (1) Control Point Surveying and Traversing
- (2) Topographic Survey
- (3) Sounding

2-3. Datum Level

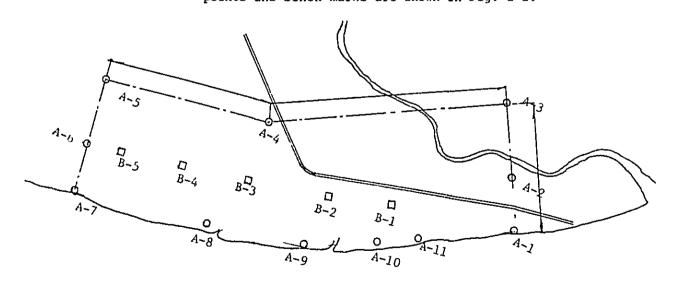
The Rangoon River has 4 tidal gauge points, one of which -- Chockey Point -- lies within the project site. The datum zero at Chockey Point is 0.216m lower than that for the Brooking Street Wharf Point in Rangoon Port.

This survey was performed with Chockey Point taken as datum 0.

2-4. Survey Methods

(1) Control Surveying and Traversing

Traverse points were established at eleven locations
around the periphery of the project area, and the angle
and distance between points were measured. In addition,
six bench marks were established on the small embankment
running north-south through the site. The traverse
points and bench marks are shown in Fig. 2-2.



A: Traverse Piont

B: Bench Mark

Fig. 2-2. Location of Traverse Points and Bench Marks



(2) Topographic Survey (Cross-Leveling)

Following the establishment of the traverse points, a topographic map was prepared based on measurements of elevations and respective distances to the base points. The following items were included in this map:

Offshore line

Road

Railway

Houses

Embankment

Paddy fields

Creeks

Concrete structures

Bridge

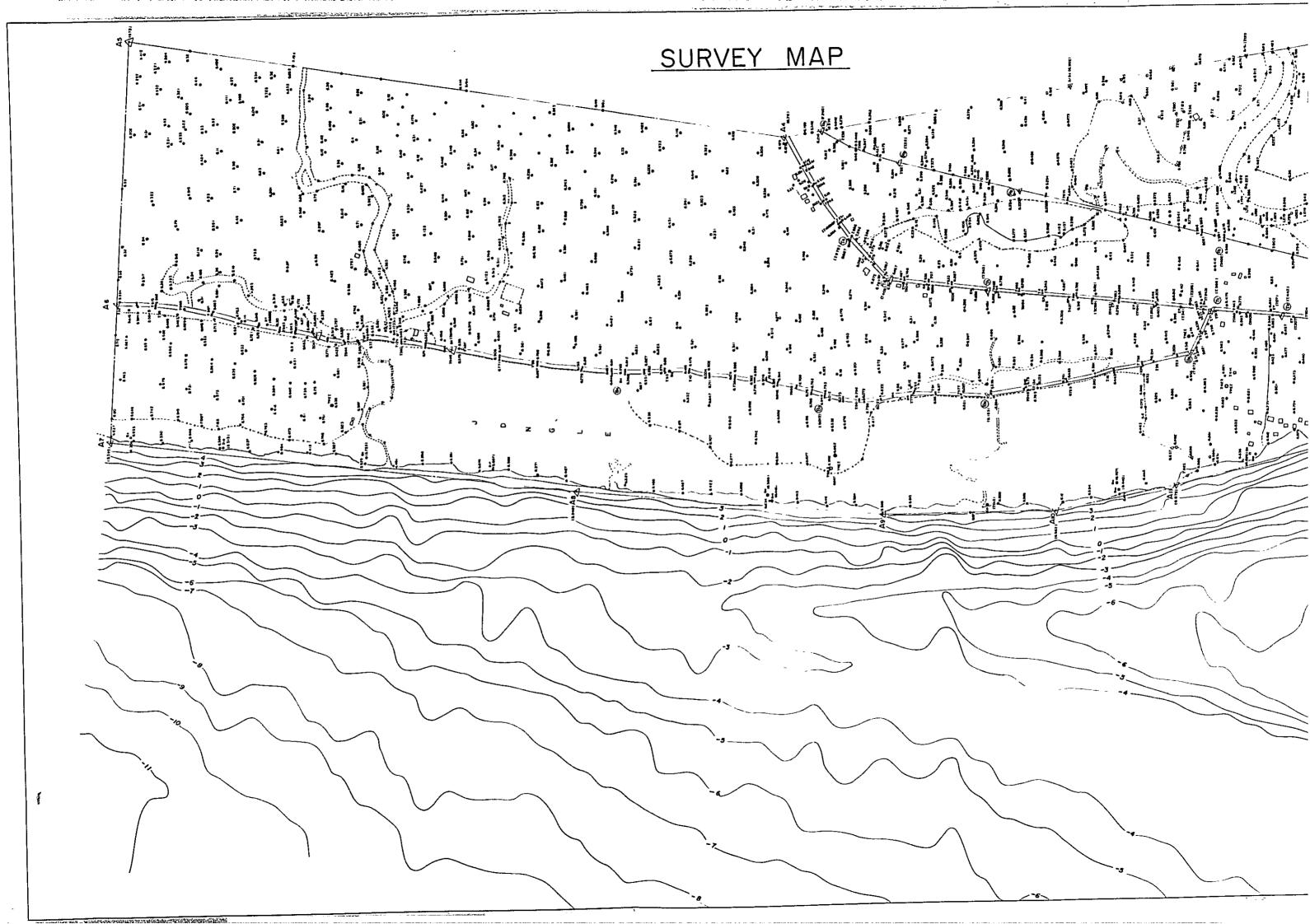
Traverse points, bench marks

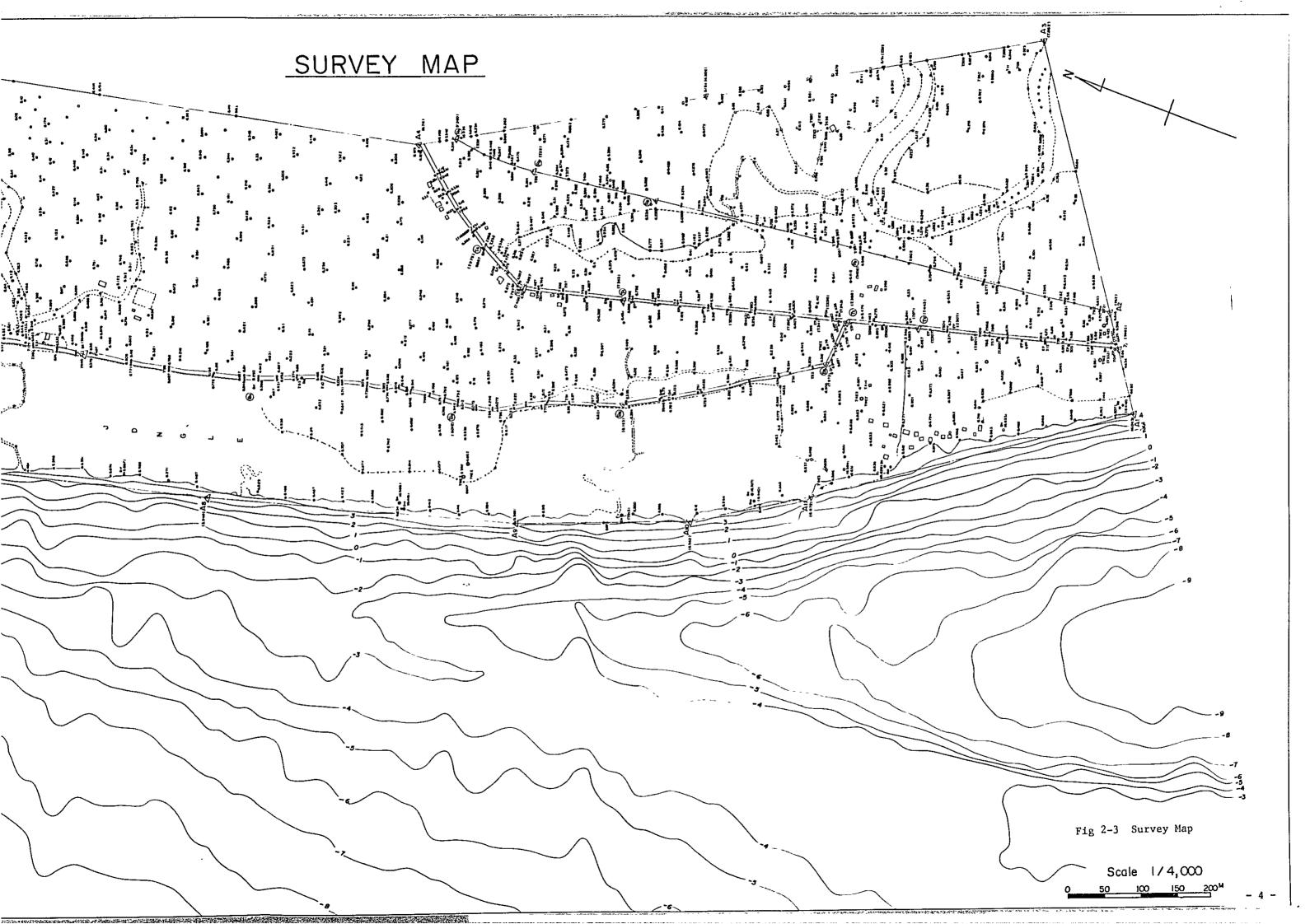
Boring and Dutch cone points

(3) Sounding

Sounding was performed using an echo sounder at 50m intervals along the coastline. The survey boat proceeded forward in accordance with guide points set up on the shore line, and measurement of distances was performed by the intersection method relying on two transit points on the shore.

A survey map based on the results of the topographic survey and sounding is given in Fig. 2-3.





3. Soil Investigation

3-1. Investigation Items and Quantities

The investigation items and quantities are listed in Table 3-1 below.

Table 3-1. Investigation Items and Quantities

Items	<u>Unit</u>	Quantity	
Boring	m	337	10 locations
Standard penetration test	time	322	-
Undisturbed sampling	time	19	·
Dutch cone sounding	m	525	19 locations
Laboratory soil tests			
Natural moisture content	time	300	
LL, PL	time	50	
Grain size analysis	time	50	
Bulk density	time	13	-
Unconfined compression	time	13	
Consolidation	time	10	

3-2. Investigation Site

Fig. 3-1 shows the location map for the soil investigation.

3-3. Investigation Methods

(1) Boring

Boring was conducted using 1 hand-feed boring machine and 1 hydraulic machine. The diameter of the bore holes was set at 66 to 88mm in places where standard penetration tests were performed, and 86 to 116mm in places where thin-walled sampling was performed.

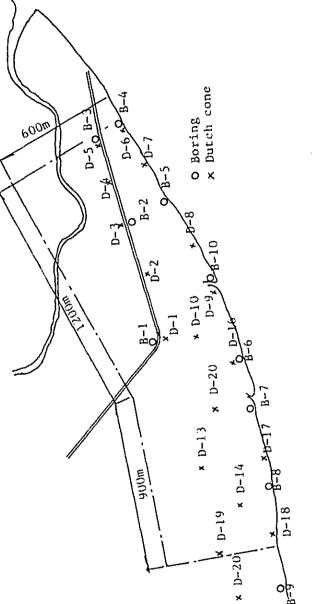


Fig 3-1 Location Map for the Soil Investigation

Bore holes No. 5 and No. 8, where thin-walled sampling was carried out, were drilled using the rotary method.

All other bore holes were drilled by the percussion method.

In principle, boring was conducted to a depth exceeding 5m until the hard bearing layer was encountered. However, at bore hole No. 5 an object thought to be stone was hit at a depth of 30m, rendering drilling impossible. Boring was halted at that depth.

(2) Standard Penetration Test (SPT)

In order to extract soil samples and study relative soil strength, the Standard Penetration Test was conducted at all boring points at intervals of lm.

The test method employed conformed to the standards given in ASTM-D-1586. The standard test method is as follows: the number of blows (N value) is measured so as to penetrate a sampler to a depth of 30cm by dropping a knocking hammer weighing 63.5kg from a height of 75cm. Measurement of the number of blows is taken at intervals of 15cm.

A portion of the samples obtained by the SPT were used for the physical tests, and the remainder were placed in sample bottles and stored at the CC Laboratory.

(3) Undisturbed Sampling

Thin-walled sampling is conducted in order to extract an undisturbed sample to serve as a specimen for mechanical test. A thin-walled sampler of fixed piston type, and a stainless sampling tube with an inside diameter of 75mm and a wall thickness of 1.5mm are used.

(4) Dutch Cone Penetration Test

The Dutch Cone Penetration test is performed as follows:

Cone shape: Angle 60°; end area

10cm , friction sleeve

attached

Measurement pitch: 10cm

Penetration speed: lcm/sec.

Capacity of proving ring 500kg, 2t

(5) Laboratory Soil Tests

(a) Physical Tests

Physical tests are conducted on the items summarized in Table 4, in accordance with the standards specified in JIS and ASTM.

Table 4. Laboratory Soil Test

<u>Item</u>	Standards for Test
Specific gravity test	JIS-A-1202
Natural water content test_	JIS-A-1203
Sieve analysis	JIS-A-1204
Liquid limit test	JIS-A-1205
Plastic limit test	JIS-A-1206

(b) Unconfined Compression Test

This test is performed in accordance with the method summarized below.

Testing method Conformity with JIS-A-1216

Type of testing apparatus Machine capable of controlling strain

Configuration and dimensions

of specimen Cylindrical shape, diameter:

5.0cm, height: 12.5cm

Rate of axial compression

loading l%/minute

Maximum strain Breaking strain +2% or 3%

Summarized test results The coefficient of deformation

is calculated by a strain corresponding to E50 = 1/2qu

(c) Consolidation Test

This test is performed in accordance with the method described as follows.

Testing method Conformity with JIS-A-1217

Type of consolidation box Fixed ring

Dimensions of specimen Diameter: 6.0cm, height: 2.0cm Loading pattern 0.1, 0.2, 0.4, 0.8, 1.6, 3.2,

6.4, 12.8 kg/cm²

Loading time 1 hour per load

3-4. Results of Soil Investigation

(1) Subsoil Conditions

The subsoil conditions in the project area are poor owing to the present of a soft clay deposit with a thickness of approximately 20 to 25m. In preparing a preliminary design, it will therefore be necessary to take this unfavorable foundation into consideration. It should also be noted that construction of a dockyard on such a poor foundation generally involves higher construction costs compared with dockyard construction on a favorable foundation.

A sketch of the subsoil structure in the project area is shown in Fig. 3-2.

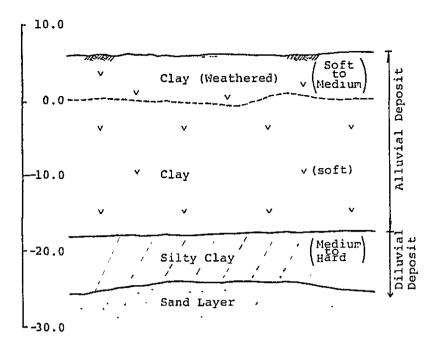


Fig. 3-2. A Sketch of Subsoil Structure

Fig. 3-3 shows a soil profile of the project area based on the results of boring and Dutch cone sounding. The subsoil of the project area comprises, from the top, a clay layer, silty clay layer and a sand layer.

(a) Clay layer

A clay layer deposit is found at a depth of 20 to 25m from the ground surface. The soil in this layer consists of clay featuring high plasticity and cohesiveness. A small amount of decayed plant material (humus) is present. A very thin layer of fine sand lies at the lower part of this layer. No shell fragments are detected. The soil layer is homogeneously composed of uniform clay, and the horizontal continuance of the soil layer is extremely good, thereby indicating that this is a marine or aqueous deposit layer from the alluvial era.

The uppermost portion of this layer is assumed to be a weathered layer to a depth of approximately 7m. While the material comprising this layer is identical to that of the lower portion, it is

relatively harder. Based on the Standard
Penetration Test (SPT), the N value of the
weathered layer is 2 to 5, while that of the lower
part is 0 to 4, indicating that it is soft or
medium soft.

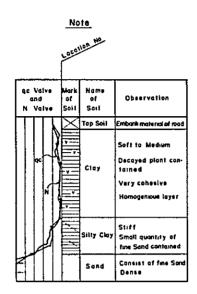
(b) Silty clay layer

A relatively hard clay soil layer is found below the clay layer described above, with a thickness ranging from 5 to 10m throughout the project area. A small amount of fine sand is mixed in this layer, and this soil layer is uniform. The color of this layer is gray or milkish white. The upper portion of the subsoil is reddish brown owing to the influence of weathering, thus indicating that this layer at one time formed the ground surface. The N value of this layer ranges from 10 to 20, which is relatively hard for a clay layer. However, it does not have sufficient bearing capacity to support a pile foundation of heavier structures.

(c) Sand layer

The sand layer exists at a depth of approximately 30m from the ground surface. This layer is composed of fine to medium sand, with a small amount of silt mixed in. The soil particles are uniform. The N value of this layer exceeds 50, thus indicating that this layer possesses sufficient bearing capacity to serve as a bearing layer for a pile foundation.





N valve Result of Standard Penetration Test

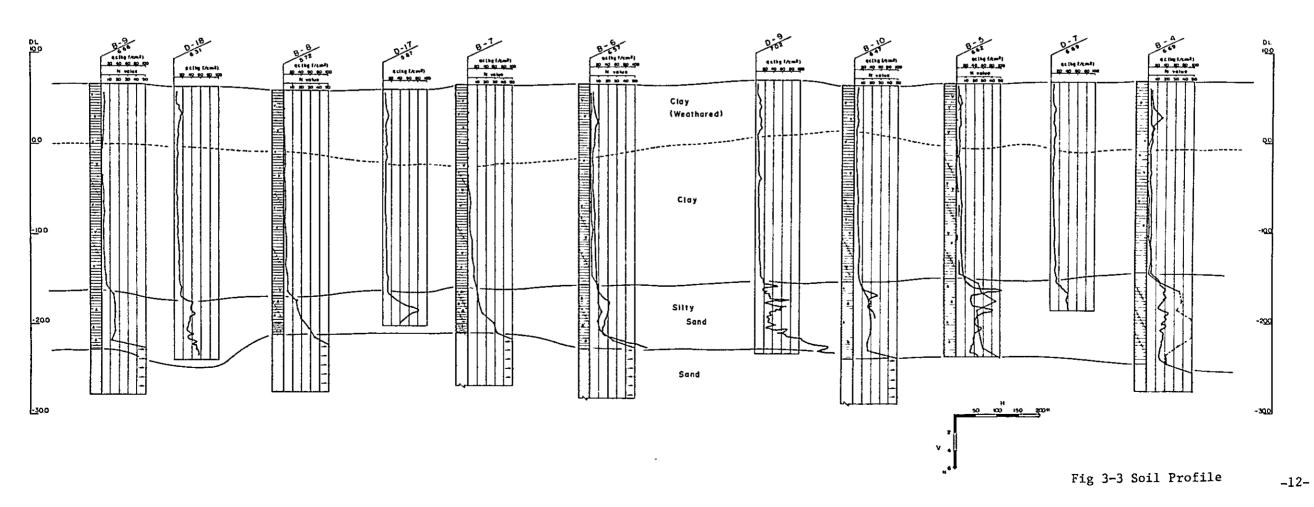
qc Result of Dutch Cone Sounding

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To Rangoon

River Coast Line



(2) Results of Soil Test

(1) Dutch Cone Sounding

Fig. 3-4 shows the depth distribution of the qc values for the clay soil layer. A wide spread exists in qc values in the weathered layer, i.e., the upper area between 0 and approx. 7m deep. The qc values range from 0.5 to 1.5kg/cm^2 .

In the area below a depth of 7m, the linear qc value increases with depth; the relationship between depth and qc value can be expressed using the formula qc = 2.8+0.35D.

A favorable correlation exists between the qc value and cohesion C: $qc = 7 \times c$. refers to the coefficient as determined by the shape of the cone being used. In the present test, a mantle cone was used with 2 = approx. 15. When 2 = approx is 15, the cohesion depth distribution can then be expressed by the formula:

$$C = 0.19 + 0.023D (C kg/cm2, Dm).$$

This cohesion distribution will be discussed further in the next article. The qc value of the sand layer exceeds 200kg/cm², thereby indicating that this layer offers sufficient bearing capacity to support a pile bearing foundation.

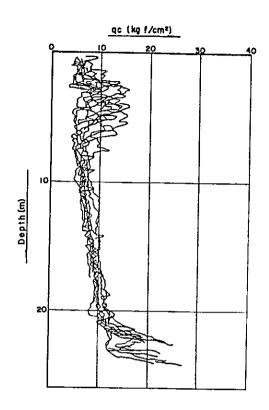


Fig. 3-4. Depth Distribution of qc

(2) Physical Tests

(a) Natural Moisture Content

Fig. 3-5 shows the depth distribution of the natural moisture content (Wn). The weathered upper portion of the clay layer has a Wn of 30 to 50%; the lower portion, 40 to 60%. In the lower silty clay layer, the Wn ranges from 20 to 30%. As a homogeneous clay layer, the spread is considerably large. The size of this spread is believed to be due to inaccuracy in measurement



resulting from non-uniformity in oven temperature during the drying of the test materials. The Wn of the clay layer is assumed to range between 50 and 60% except for the upper weathered portion; this Wn is relatively low compared with typical clay from the alluvial era. The reason for this low Wn is the disparity in minerals comprising the clay. The clay in the testing area is assumed to be kaolinite or some similar clay mineral.

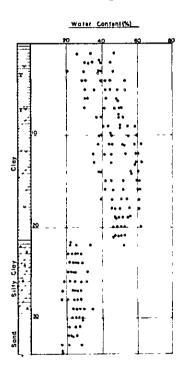


Fig. 3-5. Depth Distribution of Wn

(b) Consistency

Fig. 3-6 shows a plasticity chart based on the results of liquid limit (LL) and plastic limit (PL) tests. Because the measurement of the water content was inaccurate for the reason described above, it is possible that both WL and PL are measured too low. Based

on the test results, the portion of the plasticity chart above line A and with WL greater than 50 is categorized as CH (high-plasticity clay) according to unified classification.

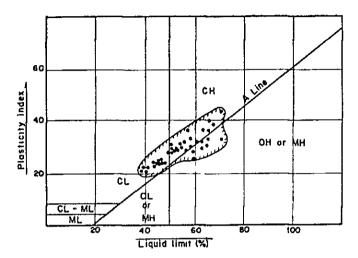


Fig. 3-6. Plasticity Chart

(c) Grain Size Distribution

The grain size distribution for soil in the various soil layers is shown in Fig. 3-7.

The major features of the grain size in each soil layer are as follows:

Clay: Contains almost no sand with particle size exceeding 74.

Uniform clay mixed with greater than 50% of clay below 5.

Silty clay: Contains approx. 10% fine sand and coarse sand. Shows almost identical distribution curve to that of above clay layer.

Sand: Consists of fine sand mixed with approx. 40% silt having particle size below 74.



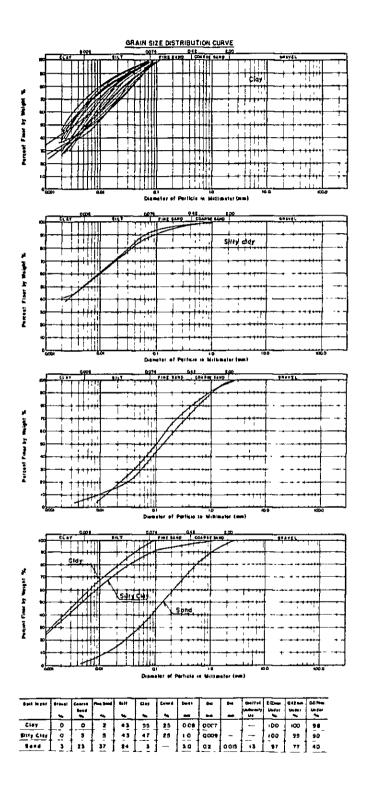
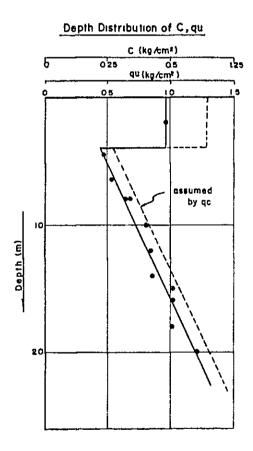


Fig. 3-7. Grain Size Distribution

(3) Unconfined Compression Test

Fig. 3-8 shows the depth distribution of compression strength (qu) based on the results of the unconfined compression test. In the figure, in addition to the qu scale the C scale when cohesion is C = 1/2 qu is shown.

Fig. 3-8. Depth Distribution of qu



Cohesion C increases linearly as depth increases, according to the formula C = 0.13 + 0.024D (C kg/cm², Dm). The distribution curve of C as assumed from the qc value of the above Dutch cone sounding is also shown.

The distribution curve of C derived from qu is almost identical in shape to that of C derived from qc; however, the value of C is approximately 0.05kg/cm² larger in the case of qc. This is due to the fact that there is no disturbance involved in the case of the qc value as the testing is performed in-site. On the other hand, with the qu the sampling is believed to be unavoidably disturbed due to sampling and trimming of the specimen, thereby resulting in a relatively small measurement for qu.

(4) Consolidation Tests

(a) e-log P curve

A comparative chart of the e-log P curve is shown in Fig. 3-10. The value curve in the normal consolidation area, i.e., the compression index (Cc), ranges from 0.4 to 0.6. However, as discussed below, because this soil has a large consolidation yield stress (Py), it is necessary to take Py into consideration when using Cc to calculate the consolidation settlement.

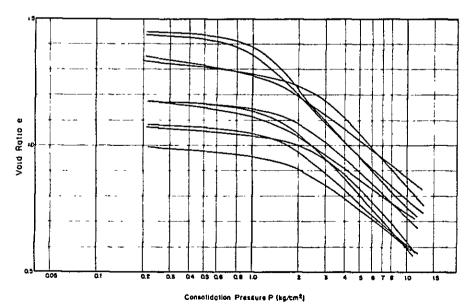


Fig. 3-10 Comparative Chart of e - log p Curve

(b) Coefficient of Consolidation (Cv)

Fig. 3-11 shows the comparative chart of Cv. The Cv for a consolidation yield stress (Py) of $1-2kg/cm^2$ is $0.07-0.2cm^2/min$, which is a normal value for a high-plasticity clay.

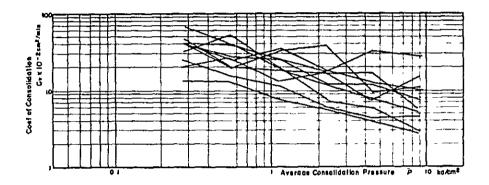


Fig. 3-11 Comparative Chart of Cv

(c) Coefficient of Volume Compressibility (Mv)

A comparative chart of Mv is shown in Fig. 3-12. The value of Mv for Py is $0.02-0.07 \text{cm}^2$, which is small for a high plasticity clay from the alluvial era. This indicates relatively small compressibility.

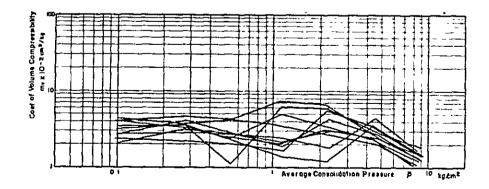
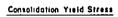


Fig. 3-12 Comparative Chart of Mv - 20 -

(d) Consolidation Yield Stress (Py)

The depth distribution of Py is shown in Fig. 3-13. The figure shows the effective overburden load (Po) line when the bulk density of the soil is calculated at 1.70. It is generally assumed that Po and Py coincide in the case of normal consolidated clay. However, the Py and Po coincide for clay in which a short aging period has elapsed after sediment; normally, Py is only slightly larger than Po. The clay in this layer shows a Py considerably larger than its Po, and the clay appears to be preconsolidated.

In view of the geographical conditions of the site, however, it is improbable that a surcharge has been loaded on the site subsoil in the past. For this reason, it is theorized that the cause for the Py greatly exceeding the Po is either a lengthy aging period after sediment or quasipreconsolidation effect due to secondary consolidation or chemicals in the clay minerals.



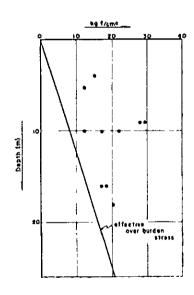


Fig. 3-13 Depth Distribution of Py

BORING LOGS

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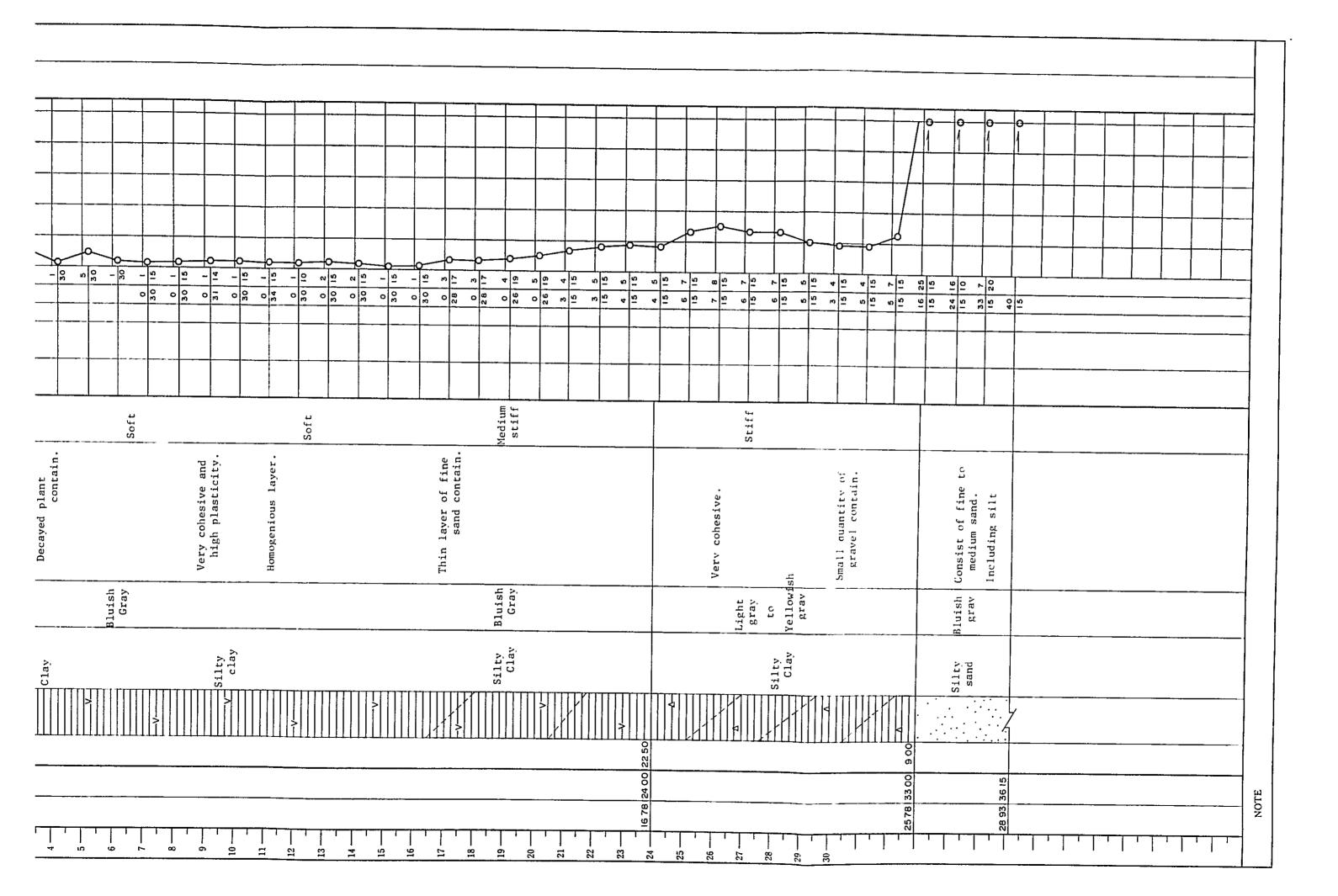
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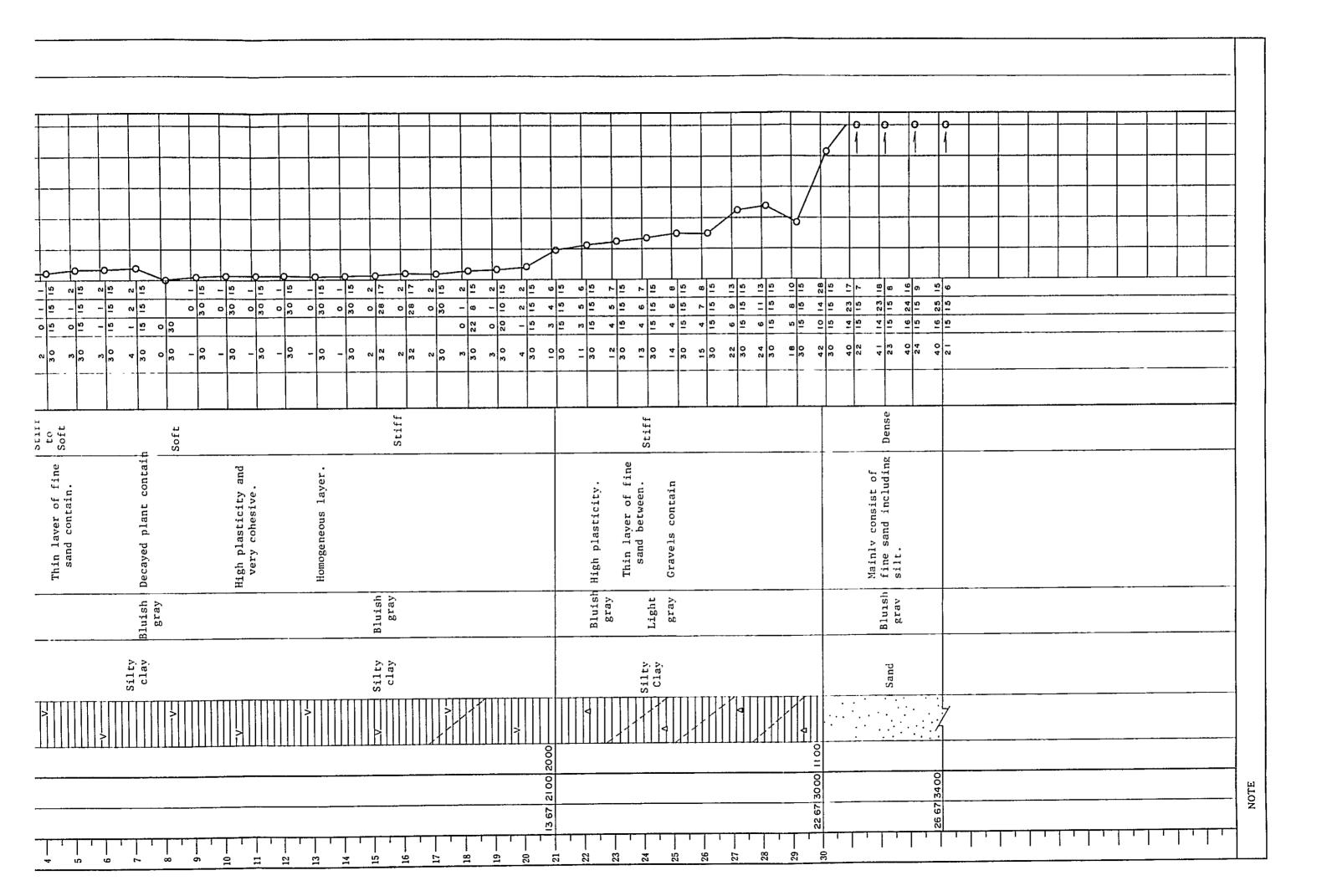
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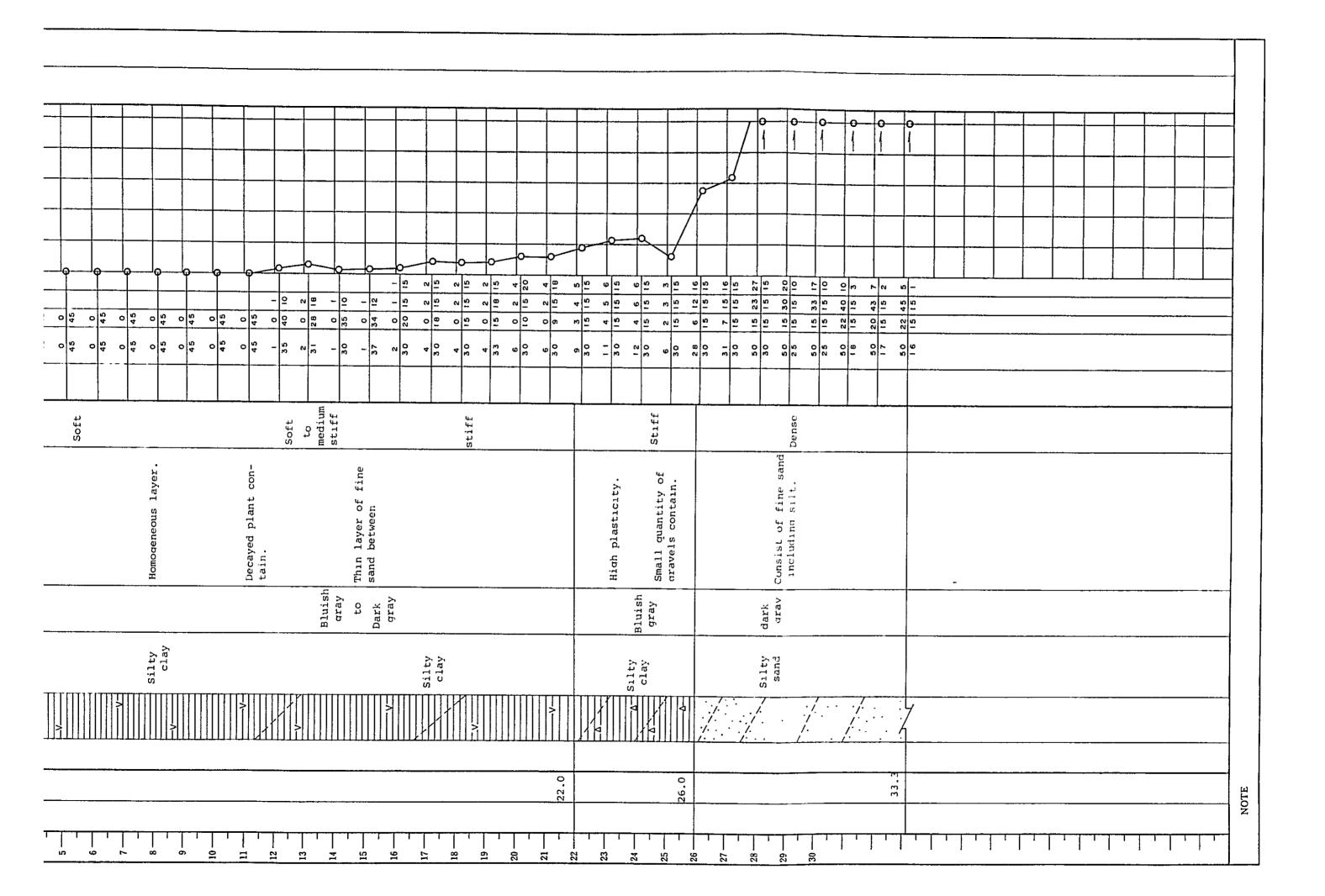
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High plasticity. Bluish
Bluish gray plant contain bluish gray high plasticity and very cohesive. Reddish Small gravels contain to Cohesive clay and the bluish gray area contain to bark gray cohesive clay consist of fine sand paray area consist of fine sand bark including silt.
Bluish gray plant contain bluish gray high plasticity and very cohesive. Thin layer of fine sand coutain to bark gray cohesive ciay Milkish High plasticity. Selluish gray contain to bark area contain to a bluish gray consist of fine sand paray consist of fine sand paray area including silt.
High plasticity. Bluish gray Decayed plant co Decayed plant co Decayed plant co to white Small gravels cout to Bluish gray Cohesive clay gray Gray Cohesive clay Consist of fine sincluding silt.
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BORING LOG	BORING METHOD RG	SAMPLING METHOD		OBSERVATION	Medium			High plasticity.	Very cohesive.			Decayed plant contain.		Soft						Thin layer of fine stiff	a)						Small quantity of gravels contain.	Stiff	High plasticity					hard ston,boring can not	
	ect			согов	Reddish Brown		Bluish	Gray					-				-	Gray							Reddish					Bluish	}		- -		
	Dry Dock Project	ON 6.62	II. TYPE	ZOIF OF NAME	Ř	Clay	<u> </u>			<u>.</u>				Silty	· · · · · · · · · · · · · · · · · · ·			4					Silty Clay		Re		S.† 1.	Sirly		— — —				Due to attach	
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1 17117	LOCATION	DATE		DEPTH											-									, , , , , , , , , , , , , , , , , , ,		<u> </u>			•				3 30 15		NOTE
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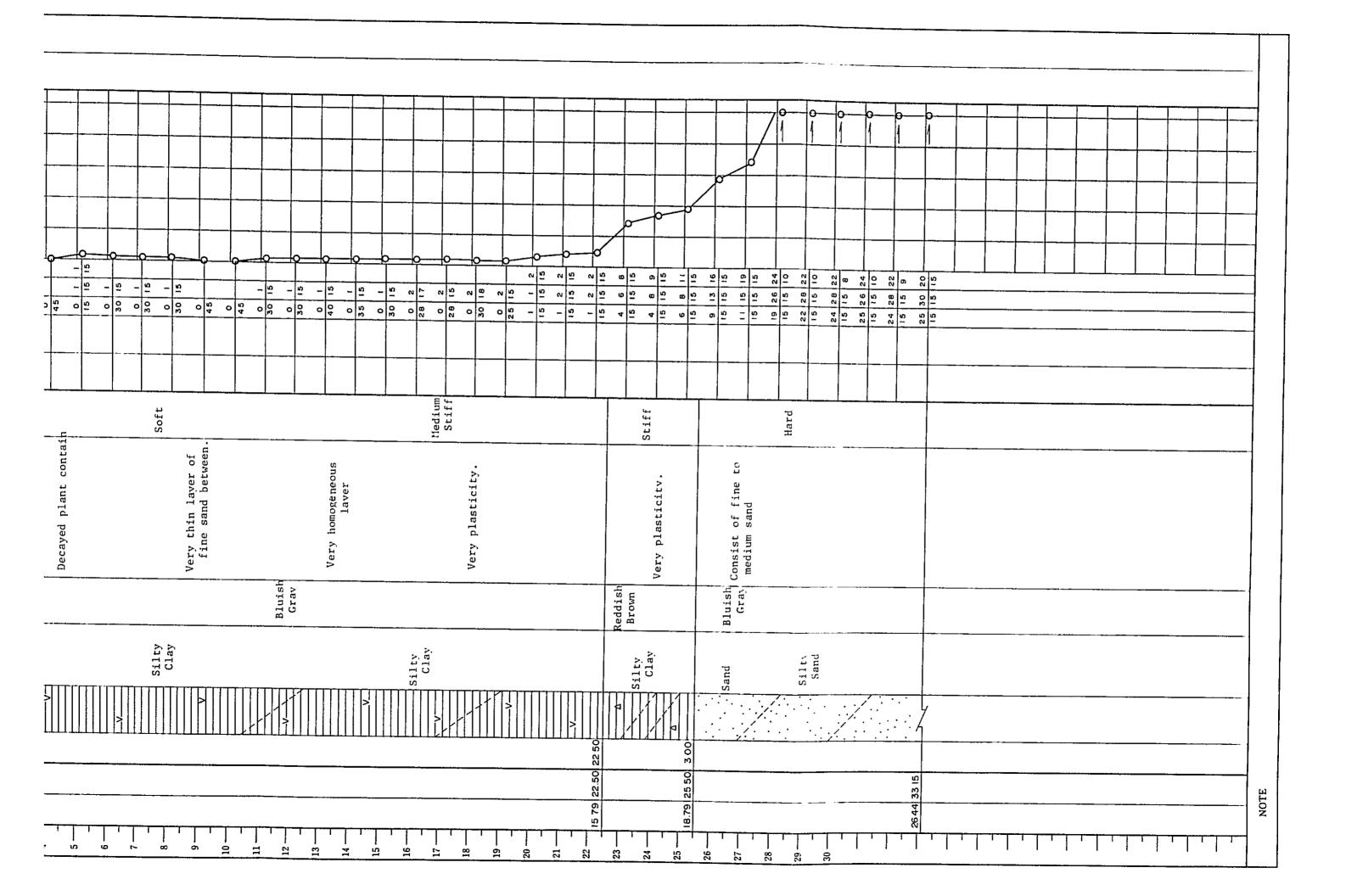
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	TIOS TO			>. v.			Silty				ty 1ay			73
1 -1 1	NAME F	Clay		Silty					 	 	Silty Clay		 	Sand .
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Soft	Soft	Medium Stiff	<u> </u>	Stiff
	Ü	High plasticitv.	Including small quar tity of fine sand.	Very cohesive. Thin layer of fine sand between. Small quantity of gravels contain medium sand.
		Bluish Gray	7	Rddish Brown Milkish White to Bluish Gray Gray
Silty		Silty		Silty Clay
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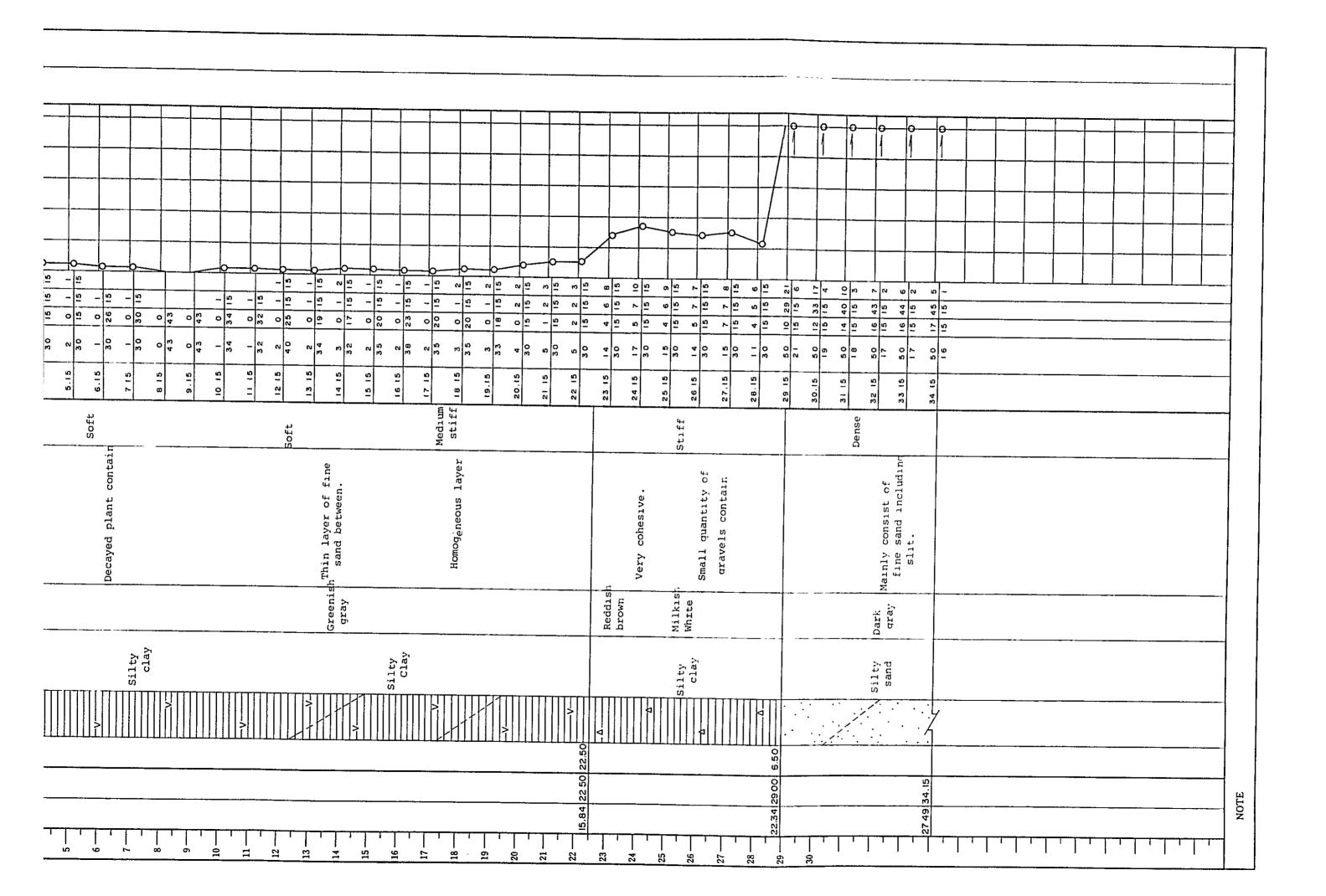
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	Percussi OD	A	CONSISTENC DENSITY OR RELATIVE				Soft							Soft	to med1um	stıff				stiff						Stiff					Demse			
907 5	BORING METHOD Per SAMPLING METHOD		OBSERVATION	-	Very cohesive and high plasticity					Homogeneous layer.			Decayed plant con-			Thin									High plasticity.	Small quantity	ontain)f f(1996 ga	4 5.14.			
	1 1	1	COL.OR	7. 7.	Bruish gray										Bluish	gray	Dark	gray	,						ί : :	gray				dark	· ·	-		
ard Project	e 2	TYPE	SOIL OF NAME		Clay				:	Silty clay									Sılty clay						Sılty	clay				Sılty				
Dry Dockyard	Thilawa	ELEVALION SOIL TYPE	SYMBOL SYMBOL			>	>		>	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\			- - - -		/			>		\ \ \ \ \ \] 		Δ_			;/. /	/	/. /	/	·/ 	·/ /· /-	- 1
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	SAMPLE NO PEPTTH(m) SAMPLING											
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Rotary	CONSISTENCY DENSITY OR RELATIVE	Soft	E	Soft			Medium			Stiff	Hard	
BORING LOG BORING METHOD R SAMPLING METHOD	OBSERVATION	Very cohesive and plasticity.	Decayed plant contai	Very thin laver of fine sand between.		Very homogeneous laver		Very plasticity.		Very plasticitv.	Consist of fine to medium sand	
	80.102	Reddish Brown Bluish Gray			Bluish					Reddish Brown	Bluish Gray	
ard Project	NAME E	Clay	TTTTTTTTTTTTTT	Silty	717 777 7777777		Siltv	01111PIII		Silty Clay	Sand	sang.
y Dockyard	SAVIBOT OF STATES (III)	<u> </u>	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	>		<u> </u>	<u> </u>					
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	Į	Percussi	Q	,	CONSISTENCY DENSITY OR CONSISTENCY	Medium stiff Soft Soft soft stiff stiff stiff		Stıff	Dense
	BORING LOG	BORING METHOD	SAMPLING METHOD		OBSERVATION	high plasticity. Decayed plant contain sand between. Homogeneous layer	_	Very cohesive. Small quantity of gravels contain	Mainly consist of fine sand including
į		1 1	ı		80.102	Reddish brown Dark gray Greenis	Reddis	Milkıs White	Dark gray
	rd Project	8		99.9 NC	JIVVIN	Clay Silty Clay	+++++++	Silty clay	Silty
	y Dockyard	Thilawa		ELEVATION SOIL	SAVIBOL CRAPHIC		δος	φ	
	E. P.	LOCATION		GROUND E	LAYER (m) THICKNESS OF		22 50 22.5	2900 6 5	
	TITIE	7007	DATE	GRO	ELEVATION(m)		15.84	22 34 26	
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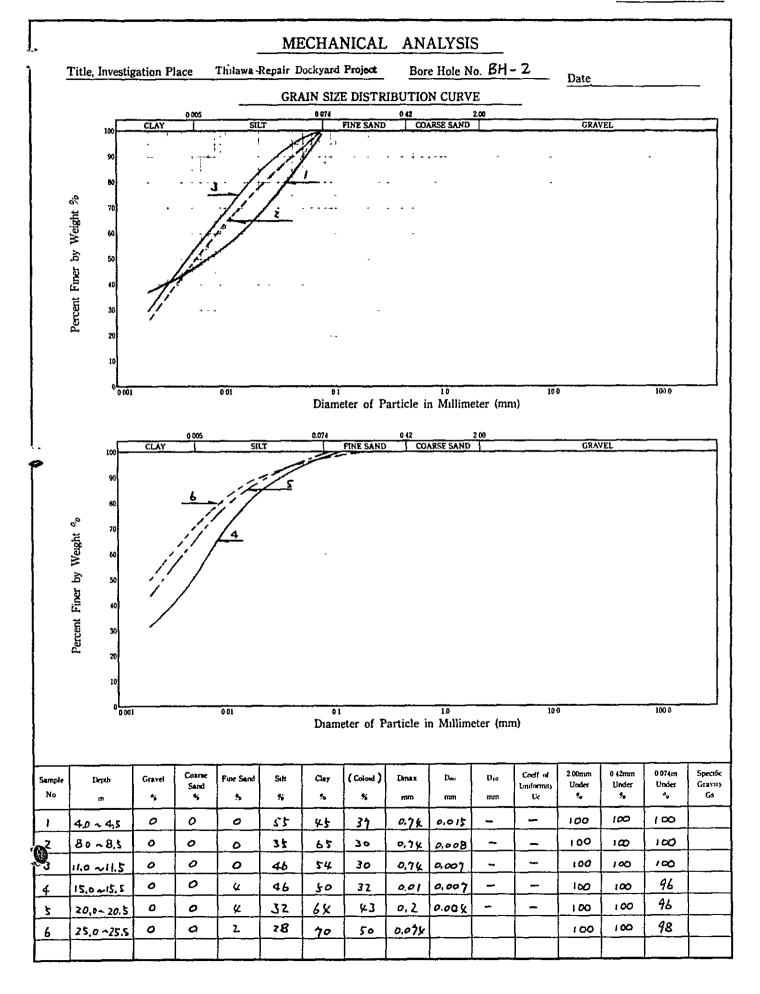


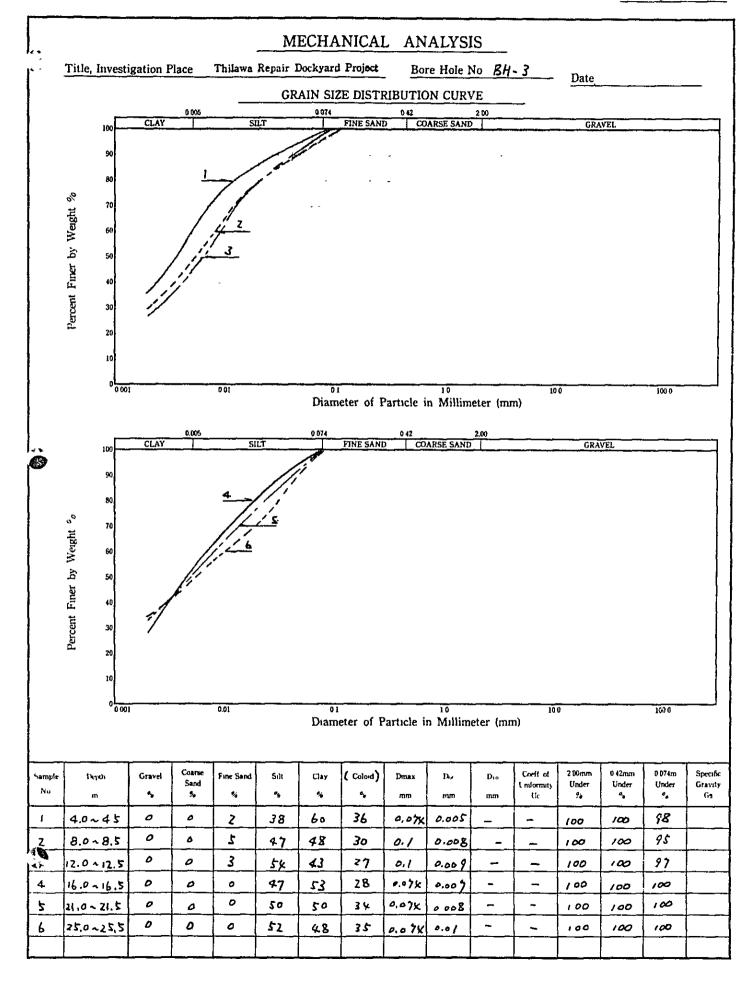
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f	%	DEPTH(m)														į							
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	tl t	согов	Reddish Brown	Bluish gray						Dark gray							Reddısh brown	Milkish white	to	b]ນາຊ	gray		Dark
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	Dry Dockyard V Thilawa 647 ELEVATION	GRAPHIC S		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\											>		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\				H		•
		LAYER (m)	<u> </u>				ווווק			}				}		<u> </u>	<u> </u>	<u> </u>				<u></u>	
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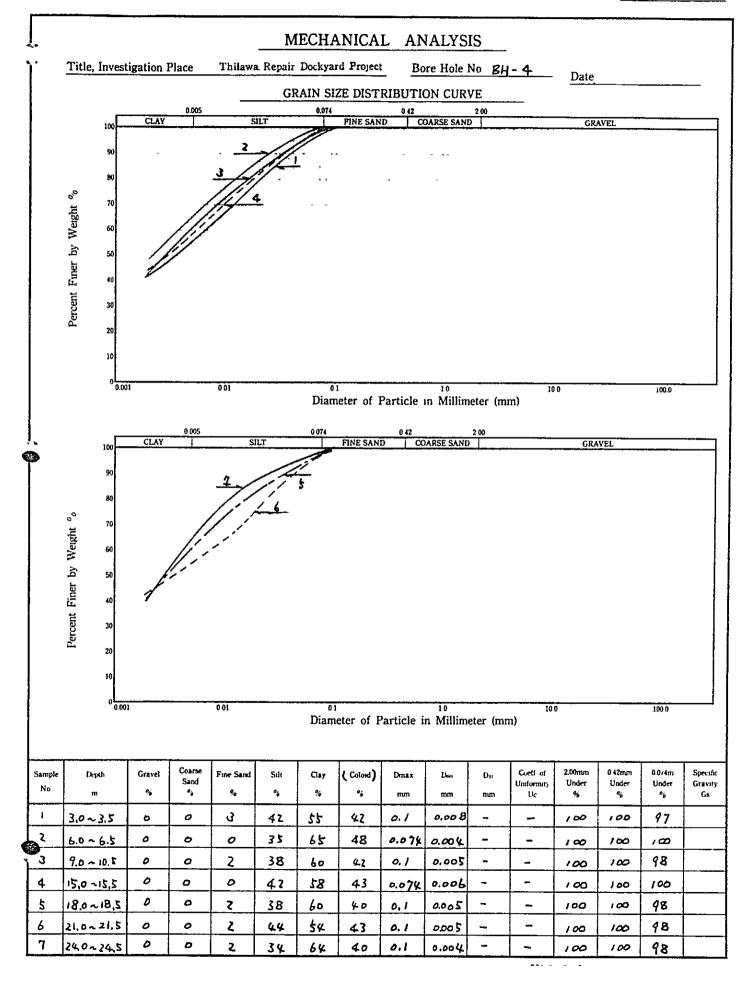
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			90 90 90 90 90 90 90 90 90 90 90 90 90 9	
Decayed plan contain.	l quantity of fine sand contain.	Consist of high plasticity clay. Small quantity of gravels and fine sand contain	Consist of fine to Dense medium sand includ- ing some gravels.	
Decay	Dark gray Small	Reddish brown Milkish white to to bluish gray	Dark	
Silty	21 50	S11ty Clay	8 50 Salty	
5 6 7 8 9 9	11 — 12 — 14 — 14 — 15 — 15 — 15 — 15 — 15 — 15	23 — 24 — 25 — 26 — 29 — 29 — 29 — 29 — 29 — 29 — 29	30 25.55 25.5	NOTE

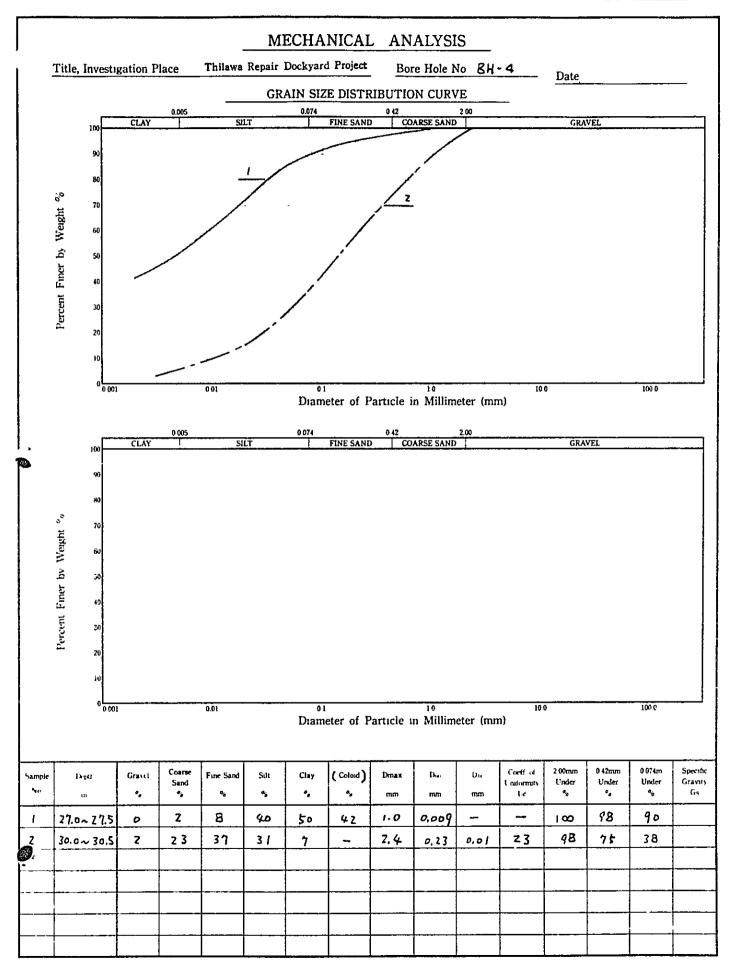
GRAIN SIZE ANALYSIS

MECHANICAL ANALYSIS Thilawa Repair Dockyar! Preject Title, Investigation Place Bore Hole No. BH-1 Date **GRAIN SIZE DISTRIBUTION CURVE** GRAVEL Percent Finer by Weight % 100 100 0 Diameter of Particle in Millimeter (mm) CLAY SILT COARSE SAND Percent Finer by Weight % 100 0 001 Diameter of Particle in Millimeter (mm) Coarse 0 42mm 0 074m Clay 2.00mm Gravel Fine Sand (Colord) Coeff of Sample Depth Silt Dmax D_{3D} Under Gravity Gs Undormity Under Under No 5 % % % ۶, 4 Ü¢ % 59 42 57 0.074 100 4.0 ~4.5 Хo 0.006 99 8.0 ~ 8.5 O 0 52 **ፉ**ን 30 0.074 0.014 100 0 0 97 53 27 0.1 44 0.014 100 4 99 0 0 23 0.074 46 33 0.01 100 100 44 5 0 6 89 20.0 ~ 20.5 ፋኔ 29 0,3 100 0.01 100 Φ 56 0 K K 0.42 0.09 0.02 4,5 100 56 6 24,0~ 24,5 100



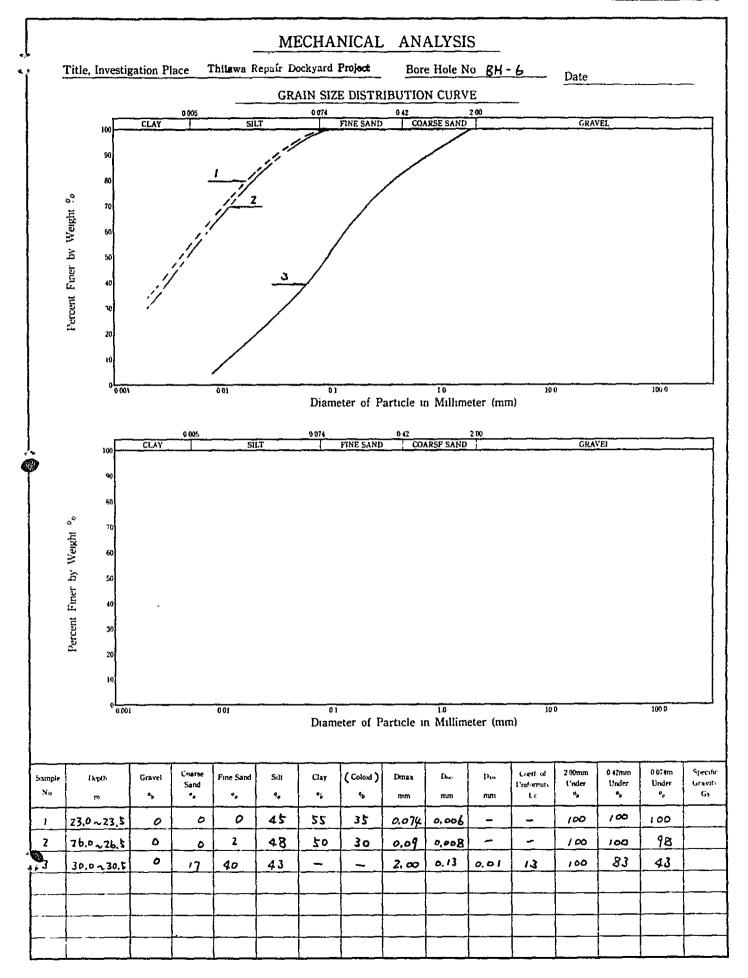






MECHANICAL ANALYSIS Thilawa Repair Docl yard Project Bore Hole No. BH - 5 Title, Investigation Place Date GRAIN SIZE DISTRIBUTION CURVE GRAVEL CLAY Percent Finer by Weight % 0 001 100 0 Diameter of Particle in Millimeter (mm) 0 42 2.60 COARSE SAND FINE SAND GRAVEL Percent Finer by Weight 0.01 Diameter of Particle in Millimeter (mm) 0 42mm 0 074m 2 00mm Specific Coeff of Die (Caloid) Dmax $D_{\rm co}$ Depth Fine Sand Silt Clay Sample Under Under Gravity Under Uniformity mm Ьc 43 57 44 0.074 0.006 100 100 0 3.0 ~ 3.3 ٥ 100 45 55 43 0.07x 100 0.008 O ø 17 45 0,09 0.009 100 8,5~95 0 3 52 40 100 96 48 0.15 100 12.0 -12.5 4 48 38 0.01 100 0 100 100 56 42 100 o 44 0.074 0.006 100 100 96 0 43 53 40 4 0.007 18.0~ 18.7 0.15 93 J 20 97 21.5 - 22.0 4 43 40 100 Z.∞ 0.009

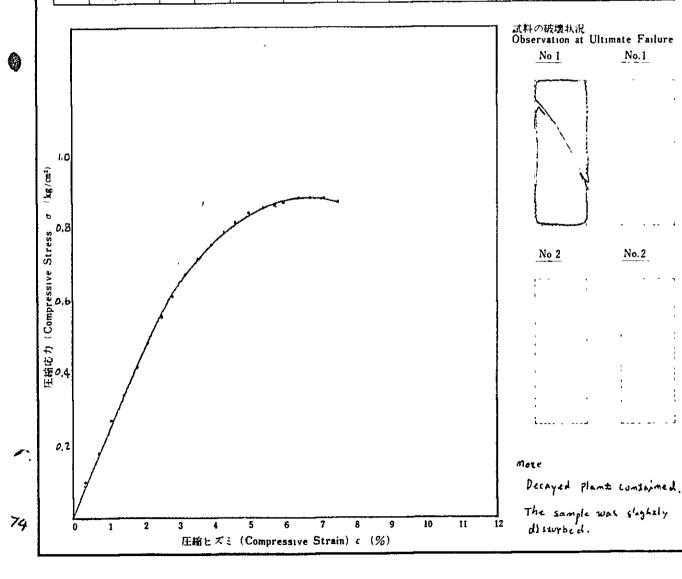
MECHANICAL ANALYSIS Thilawa Repair Dockyard Project Title, Investigation Place Bore Hole No BH ~6 Date GRAIN SIZE DISTRIBUTION CURVE COARSE SAND FINE SAND GRAVEL 100 Percent Finer by Weight % Diameter of Particle in Millimeter (mm) 0 42 20 COARSE SAND FINE SAND GRAVEL Percent Finer by Weight % 0 001 Diameter of Particle in Millimeter (mm) Charse cneff of 2.00mm 0.42mm 0 074m Specific hample Limpth Gravel Fine Sand Clay (Coloid) Dmax Den D_{10} Under Under Gravits Under Uniformity ۰, 9, a, 40 80 e, Gs mn шm lι 0 2 45 O 98 2,0 ~ 2,5 53 41 0.09 0.008 100 2 0 5.0 ~ 5.5 45 41 0.09 54 0.006 100 100 99 0 8.0 ~ 8.5 0 0 3 47 50 40 0,1 0.01 100 97 100 11.0 ~ 11.5 0 35 4 0 0 43 57 0.078 0.006 100 100 100 0 5 140214.5 48 30 0 5 Z 0.074 100 100 100 0.009 0 6 0 7.0 ~ 17.5 55 45 30 100 0 0.074 100 100 0.008 ٦ 0 200- 20.5 3 46 97 51 30 0.01 0.1 100



B-5

土の一軸圧縮試験

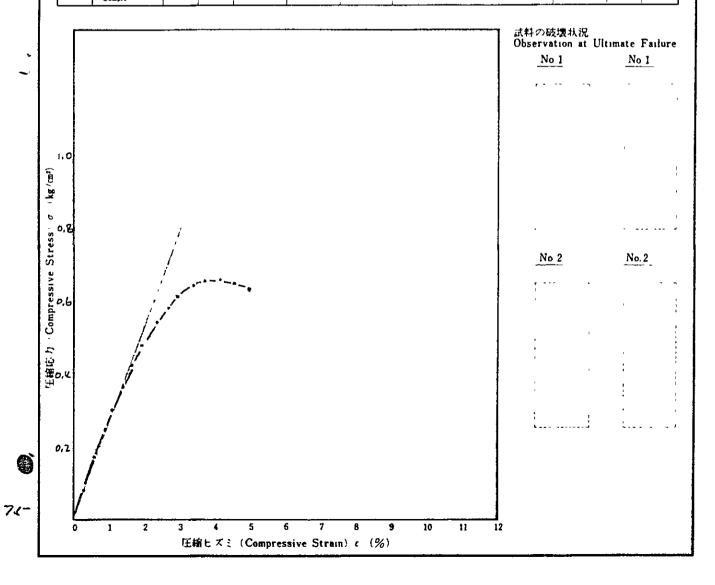
調佐名 Title, 1	·調養地点 Investigation I	Place	Thila	wa Repair	Dockyard l	Project	試験年月日 Date	! 	ή. 	<u>H</u>	11
	取位置(深度) ng Place, Dep	th	, 1	3.0 _m ~	18,6	<u>m</u>	上版名符 Soil Class	sification	clay		
武料番! Sample	s; No. Tw.	s -	6				土粒子のH Specific (.
に力制(Stress	即注: Control			ズミ制御法 rain Contr			圧縮速さ Compressi	ion Speed	<u></u>	%/min	mm/min
供えは	武林の状態	Demen Specie		3 * #	中心体种重量	間かま比	能和性	植竹杨佳生 Inconfined	破壊れる	64 14 25	報 被 tt
Specimen No	Specimen Condition	ւմ [†] Height Hica	pi (\$ Diameter #'cm	Moisture Content w **o	Wet Density	Void Ratio	Degree of Saturation S. 2	Compressive Strength qu kg m²	Failure Strain	Deformation Coefficient Eso by m ²	Sensitivity Ratio
1	At the selfs Undesturbed Sumple	14.0	7.5		1.73			0.88	6,5	<i>z</i> 3	!
1	Mary 1961 (1984) Remoided Sample		•				·	·			,
2	Undisturbed Sample						<u> </u>	` •			; ;======= 4
2	M (dg) / (RM) Remelded Samele						1	İ		!	



8-5

土の一軸圧縮試験

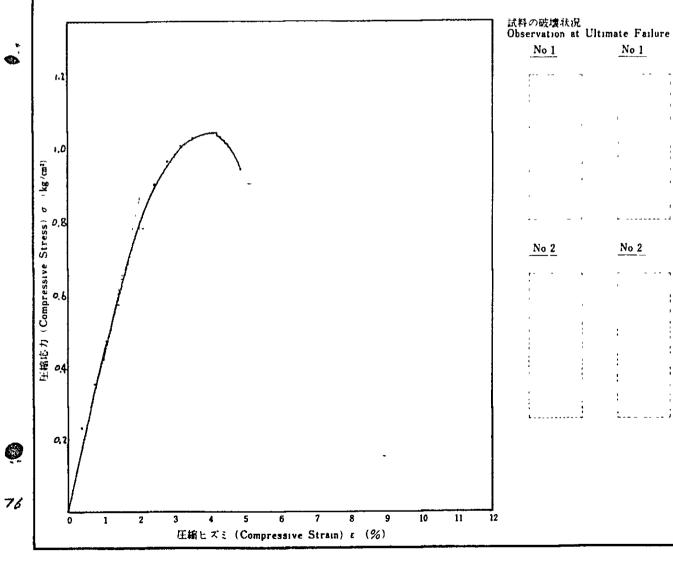
	· 調査地点 Investigation F	Place	Thiis	wa Repair	Dockyard	Project	試験年月日 Date	i 	។	<u> </u>	t1
	收位置(深度) ng Place, Dep	th	8	.50m~	9.20	m	土質名称 Soil Class	sification	clay	,	
試料器 Sample		TW	S - 10	·			土粒子の出 Specific (
に力制(Stress	即法 Control			ズミ制御法 rain Contr			圧縮速さ Compress:	ion Speed		%/min	ma/min
神太体 きょう	式料力状態		fk 1 å sion of	含水北	1 44 (21 株務市間	間平多比	超和性	触行新分寸 Unconfined	N III.	支护保护	観 腔 比
Speciales No	Specimen Condition	Height Height	fi († Diameter † c=	Morature Content	Wet Density	Void Ratio	Degree of Saturation	Compressive Strength	Failure Strain	Deformation Coefficient	Sensitivity Rati
1	M to ak 84 Undisturbed Sample	13.0	7.5		1.72	!	•	0.66	3.7	26	
1	Maring 1 of Pi Remolded Sample					T 	•			; 	
2	AL 2 G to 4K 94 Understurked Sample						† · · · · · · · · · · · · · · · · · · ·	-1			
2	朝日・北日・成年 Romelded Sample				-			·			



B-5.

土の一軸圧縮試験

	・調査地占 Investigation F	lace	Thi	lawa Repa	ir Dockyard	Project	試験年月日 Date		វា	<u> </u>	11
	限位置(深度) ng Place, Dep	th	7.5	5,0 m ~ 1	5.5	m	上質名称 Soil Class	infication	cla	Υ	
試料器 Sample		TWS	- v			·····	土粒子の比 Specific C	K iravity			
応力制徒 Stress	知法 Control			ズミ制御法 rain Contr	ol		圧縮速さ Compressi	on Speed	!	%/min	bb/min
**************************************	发料小柱型	ft L Demeni Specie		a k it	₩1× ΦΙΑ ΦΨ	州十年乾	12 41 2	帧+箱供注 Unconfined	破損! 7 七 7	4 + 4 22	銀粒比
Specimen No	Specimen Condition	Height Hem	afi i∳ Diameter ∮ca	Moisture Content w %	Wet Densits	Void Retio	Degree of Saturation	Compressive Strength	Fasture Strain	Deformation Coefficient Esa kg m²	Sensitivity Ratio
1	ALTICARM Undesturbed Sample	14,3	7. \$		1,79			1,04	4.2	44	
1	週 + 点 ・ 水料 Hemolded Sample					[L		 			
2	Al Carried Ph Undisturbed Sample							·	 		,
2	線・点したは的 Remolded Sample						<u> </u>				



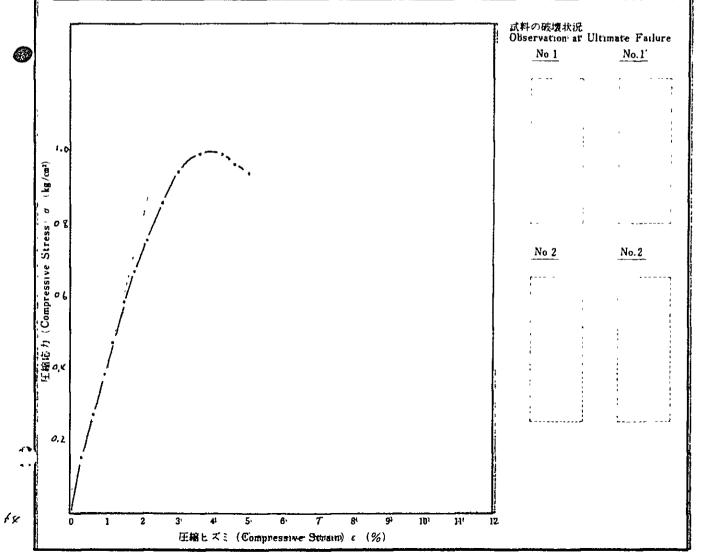
B-10

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土の一軸圧縮試験

調查名·調查地点 Title, Investigation Pla	ce Thilawa Repair Dockyard Project	試験年月日 Date	ти	Н
成料採取位置(深度) Sampling Place, Depth	2.00 m~ 2.50 m	土饭名称。 Soil Classification		
試料番号 Sample No. Tu	1 - 2 W	上粒子の比重 Specific Gravity		
吃力制御法 Strees Control	ヒズミ制御法 Strain Control	圧縮速さ Compression Speed	/ %/mi	ជ 🖦/ការក

供送体	大井の状態	序 JC 1 Dewens Specim		含水比	901以1本 村 6 単 以	問~年北	超和度	軸+新生生 Unronfined	病療的で ヒプ	金彩体数	號 触 北
Specimen Na	Specimen Condition	Height Height	et 15 Diameter é en	Moisture Content w %	Wet Density	Vaid Ratio	Degree of Saturation	Compressive Strength	Failure Strain	Deformation Coefficient Esa ligren?	Sensitivity Ratio
1	At the Land Ph Undisturbed Sample	13.4	7.5	44,5	1.82			0.98	3.7	42	
1	MTリス ・よか Remolded Sample										
2	ALT C. ACM Undistribed Sample										
2	部9 点した試料 Remelded' Sample										



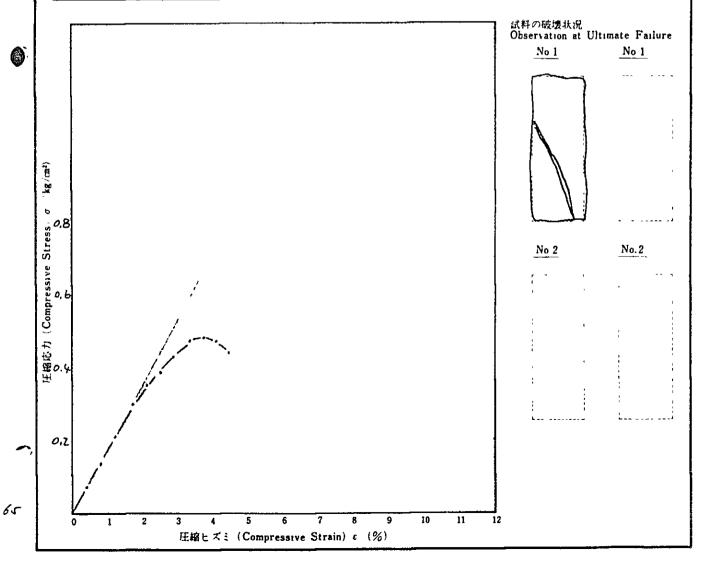
BH-10

土の一軸圧縮試験

調查名·調查地点 Title, Investigation Place	Thilawa Repair Dockyard Project	試験年月日 Date	η	H	- H
武料採收位置(深度) Sampling Place, Depth	4.00m~ 4.70 m	土代名称 Soil Classification	clay		
武料番号 Sample No TWS -	2	土粒子の比重 Specific Gravity			

吃力制御法 Stress Control	ヒズミ制御法 Strain Control	圧縮速さ Compression Speed	%/min	mm/min
	但是练上才 2 水 比 - 中心体的布量 - 間 ~ 年比	起 和 性 軸行編號 *	破壞的" 复形锑数	ge bér bt

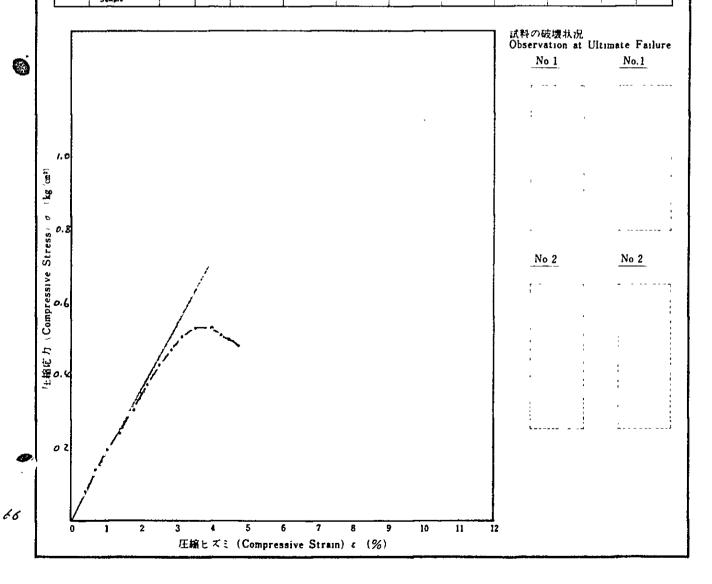
供成体	工作公司品		Marij Non of Hen	↑ 米比	401-1845年發	መጥቅድ	超 机性	植作新好性 Unconfined	破撞的" 生"	变形体数	gg be ht
Specimen No	Specimen Condition	,≨ 's Height H'en:	off of Diameter of cm	Mossiure Content	Wet Density	Void Ratio	Degree of Securation	Compressive Strength	Failure Strain	Deformation Coefficient Ess kg m²	Sensitivity Ratio
1	Al Constitution of the Underproperty of the Underpr	7.5	15.0	42,2	1.78			0.48	3,8	IB	
1	観り返して式件 Remoided Sample						' 				
2	51 3 5 18 84 Undinjurbed Sample						 	 		i 	
2	MLウ MLして X名 Remolded Semple							1			



BH - 10

土の一軸圧縮試験

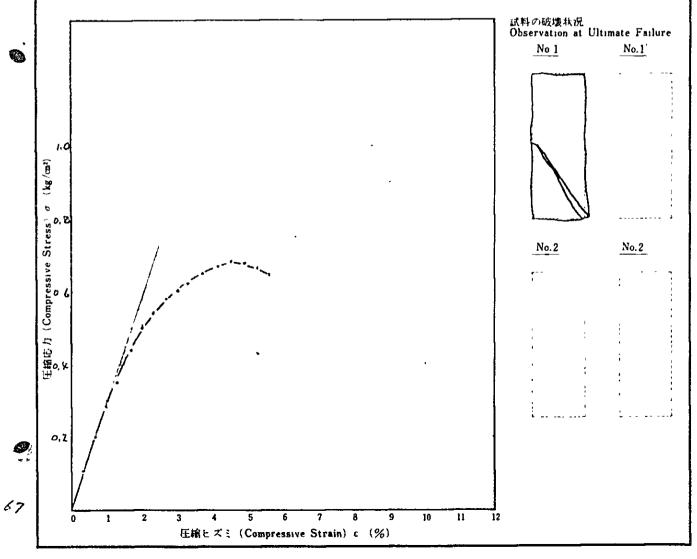
	· 調養地占 Investigation I	Place	Thiles	wa Repair	Dockyard I	Project	試験年月日 Date	l 	4	<i>J</i>]	H
	取位置(深度) ng Place, Dep	th	6.0	o m∼	6-78	m	土饭名称 Soil Class	sification			
武科器 Sample							上粒子のH Specific C				
吃力制的 Stress	那法 Control			ズミ制御法 rain Contr	ol		圧縮速さ Compressi	ion Speed		%/min	mm/min
供式体	武器の状態	Demen Specia		* * ±		間 ヤ * 北	整有线	触け締分さ Unconfined	破壊的の	电平体路	能 收 比
Specimen No	Specimen Condition	△ 3 Height Hice	jK (≸ Diameter ∮'en	Moisture Content w %	Wet Density	Void Ratio	Degree of Saturation S %	Compressive Strength	Failure Strain	Deformation Coefficient Ess kg m²	Senactivity Ratio
ì	N of 94 Undisturbed Sample	15.0	7.5	46,9	1.75		1	0,53	3.5	18	
1	間つ以上する人件 Remolded Sumple			*			† · ·	¶ ▶ } !			- 1
2	ALT TO ARM Undisturbed Sample						1				
2	親・成しア状態 Remolded Sample						T	*	-		



8H - 10

土の一軸圧縮試験

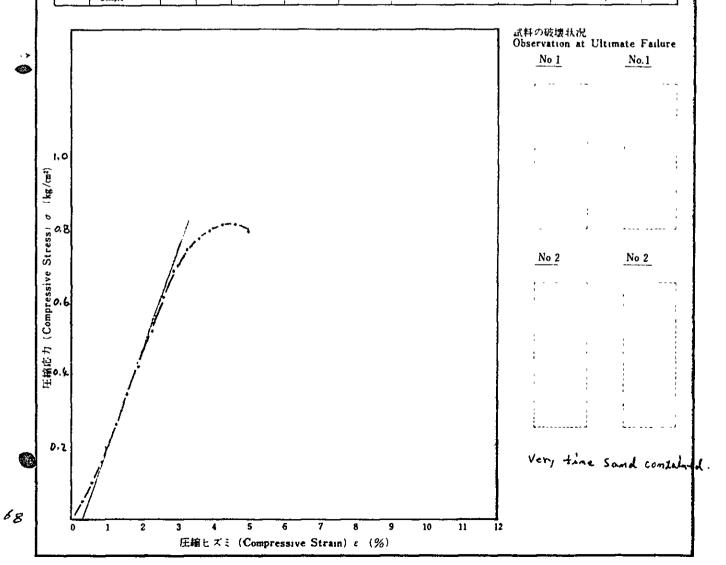
調查名 Title, 1	· 調査地方 Investigation F	lace	Thilas	va Repair-	Dockyard I	Project	試験年月日 Date	 -	η	Н	- 11
	取位置(深度) ng Place, Depi	th	8.0	⊘ m~	8.78	m	土質名称 Soil Class	sification	Clay		
式料指 Sample		TW:	s - 4				土粒子の比 Specific C				
応力制i Stress	即法 Control			ズミ制御法 rain Contr			圧縮速き Compressi	on Speed		%/min	mm/min
保えは	水井の井堂		Lt 41 pion of	含水比	- 中心体育等量 -	問マキ北	复和境	Mili 描写 *	破壊がで	意见证数	ing into the
Specimen No	Specimen Condition	Height Hick	∮ (f Diameter ∮ cm	Moisture Content	Wet Density	Void Ratio	Degree of Saturation	Compressive Strength	Failure Strain	Deformation Coefficient Ese kg m2	Sensitivity Ratio
1	At W. C. at M. Undisturbed Sample	15.0	7.5		1,81		-	0.68	4.5	30	1 1
1	雑)返して成料 Remolded Sample						1				
2	AL A C . A #4 Undistanted Sample							1			
2	練)あした式料 Remaided Sample		1				j				





土の一軸圧縮試験

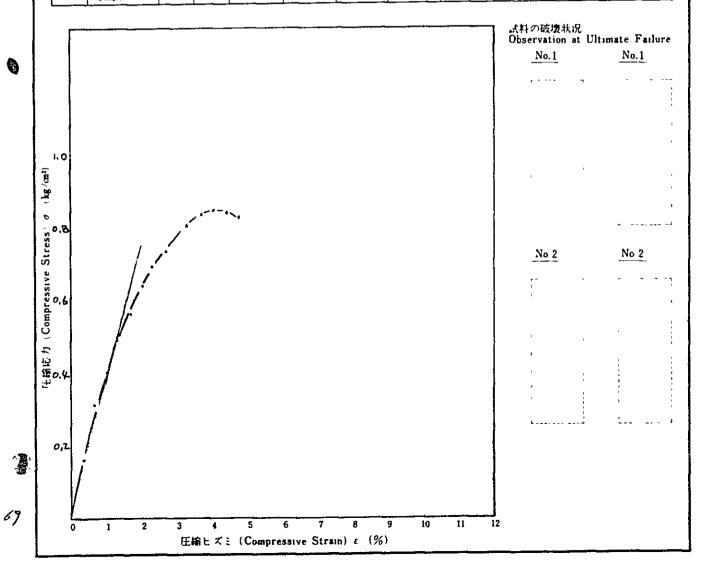
	·調査地点 Investigation I	Place_	Thile	wa Repair	Dockyard	Project	試験年月日 Date	i 	Ą	Н	Н
	取位置(深度) ng Place, Dep	th	10.	0 m~	10.7	m	土質名称 Soil Clas	sification			
式料品 Sample	ÿ No T∨	v <u>S</u> -	5				土粒子の出 Specific(
応力制 Stress	即法 Control			ズミ制御法 rain Contr			圧縮速さ Compress	ion Speed		%/min	ww/min
非太郎 と 番	よれの共働	is L		2 * H	Ψta tk# Φ ¥	□□→★比	起机度	軸i 板守 *	城境のな	克形体数	ing the Ht.
Specimen No	Specimen Condition	Height Hicm	Ø 1≸ Dismeter ∮ cu	Moisture Content	Wet Denaity	Void Ratio	Degree of Saturation	Compressive Strength	Failure Strain	Deformation Coefficient Esu ag m²	Sensitivity Ratio
1	N C M Undesturbed Sample	15.2	7.5	37,7	1.83	:	 	0.81	4,2	35	
1	間り返して成的 Remoided Sample							4 	 :	 	
2	AL TELES. Undisturbed Sample						*	1			
2	Miling F 1594 Remoided						-†		 --		



BH - 10

土の一軸圧縮試験

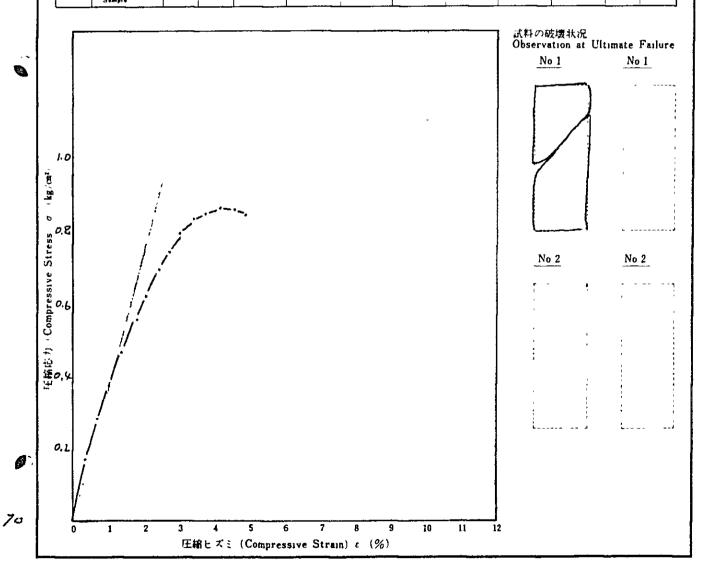
調查名 Title,	·調査地市 Investigation P	Thile	wa Repair	Project	試験年月日 Date		4	<u> </u>	11		
武料採 Sampli	取位置(深度) ng Place, Dept	th	/ 2	. o m -	12,8		土質名称 Soil Class	sification			
太料番 Sample); No.	Т	ws-	6			土粒子のH Specific C				
に力制(Stress	和法 Control		는 : Sti	ズミ制御法 rain Contr	ol		圧縮速さ Compressi	on Speed		%/min	mm/min
(株式体	水井の北佐		(\$ 1 d) sion of nen	ar 水 比	电视体的重量	图》年此	te # 12	★ 体络结节 Inconfined	板堆料 "	复影谱数	穀 鹼 比
Specimen No	Specimen Condition	Height Height	# (f Diameter # cm	Morsture Content	Wet Density	Lord Ratio	Degree of Saturation S %	Compressive Strength	Fulure Strain	Deformation Coefficient Ese kg =2	Sensitivity Ratio
1	file in the Control of Sample	15,0	7.5	45	1.73			28.0	4,0	40	
1	Marin Marin よく時 Remolded Sample					 	· + -	<u></u>	·		
2	AL 3 S. of M Undisturbed Sample					 	. L .,	.	; ' 		!
2	M 11 to 1 7 1894 Remaided Sample] -				ļ		<u></u>	 	.



BH- 10

土の一軸圧縮試験

	· 調査地点 Investigation I	Place	Thila	wa Repair	Dockyard	Project	試験年月1 Date	 	ή 	H	H
	取位置(裸度) ng Place, Dep	th	14,	00 m ~	14.78	m	土質名称 Soil Class	sification			
成料 街 Sample		ws	- 7				Lite f のH Specific C				
吃力制 Stress	知法 Control	<u>-</u>		ズミ制御法 rain Contr			圧縮速さ Compress:	on Speed		%/min	mm/mjn
ብንዱሳት ኛ ሕ	よれの状態	Demen Specia		含水比	44分体标准证	川ザキ比	, Ke to 15	触り 指注 ***********************************	破壊的の	电影绎数	级敏壮
Specimen Na	Specimen Condition	tà 3 Height Hices	jn (f Diameter ∮tm	Moisture Content	Wet Densits	Void Ratio	Degree of Saturation	Compressive Strength	Failure Strain	Deformation Coefficient Ese kg cm ²	Sensitivity Ratio
1	Ri E C . A 64 Undiscurbed Sample	14.9	7.5	.,	1.72	 		0.86	٧,2	37	
1	離・点して式料 Remolded Sample										
2	AL C. IK M. I Vadraterbed Sample										
2	親リカリア以外 Remolded Sample			·			 	·			1



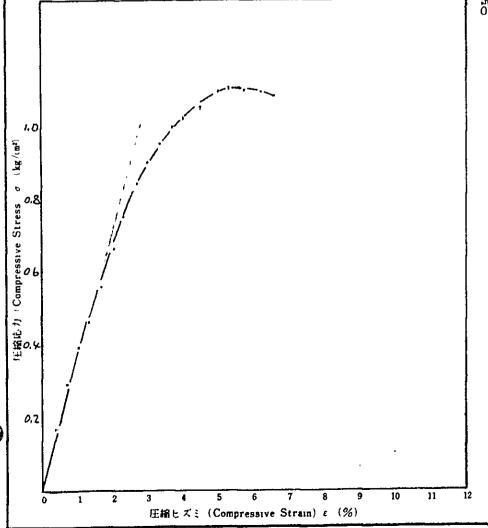
B - 10

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土の一軸圧縮試験

UNCONFINED COMPRESSION TEST

	· 調査地占 Investigation F)lace	Thiley	va Repair	Dockyard P	roject	試験年月日 Date		গ	11	Н
	取位置(深度) ng Place, Dept	th	16.	О м~	16.7	m	土質名称 Soil Class	sification	clay		
式料番 Sample		TWS	- 8				土粒子の比 Specific C				
ご力制で Stress	即法 Control			ズミ制御仏 rain Contr			圧縮速さ Compressi	on Speed		%/min	ma/min
IRAIL L	おおなけた	it A Demeni Speciu		2 * tt	明白体的 多量	同ヤキ此	1 12 利 2	抽作新货 3	級技術プ と	支充保险	gap bei st
Specimen No	Specimen Condition	ià S Height Hicm	∦ 45 Diameter ∮ co	Maisture Content	Wet Density	Void Ratio	Degree of Saturation S 'b	Compressive Strength	Failure Strain	Deformation Coefficient Eso kg m²	Sensitivity Rati
1	RL W A 95 Undisturbed Sample	15.0	1.5		1.74			1,10	5,5	37	
1	雑り力、7元件 Remolded Sample						<u> </u>	i	<u> </u>	 	
2	gr v] 	· 	 			;
2	脚:出して試料 Remoleted Sample					! 	 	1		<u> </u>	



試料の破壊状況 Observation at Ultimate Failure No.1 No.1

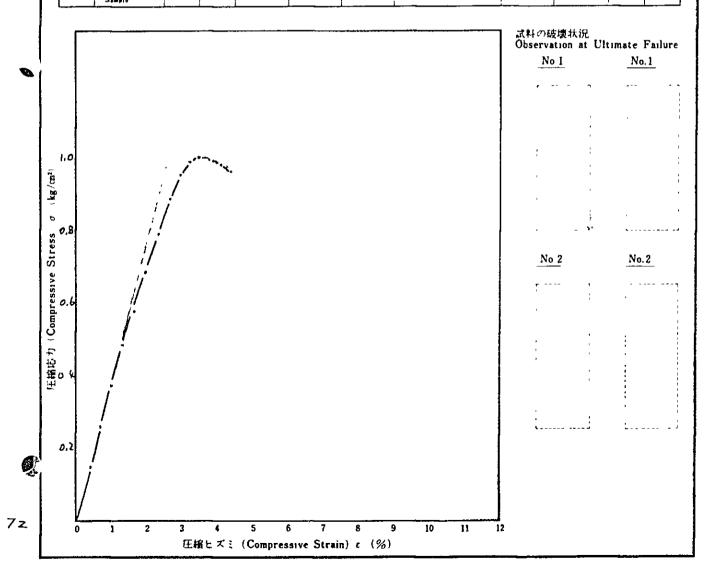
Decayed wood compained

8-10

土の一軸圧縮試験

UNCONFINED COMPRESSION TEST

	· 調介地占 Investigation I	Place	Thila	wa Repuir	Dockyard	Project	試験年月日 Date	l 	9	月	_ !!
	取位置(深度) ing Place, Dep	th	18.0	m ~	18.8	m	主貨名称 Soil Class	sification		., _ = -	
式料 指 Sample		rws	- 9				上粒子の出 Specific C				
応力制 Stress	间上 Control			ズミ制御法 ain Contr			圧縮速さ Compressi	on Speed		%/mm	mm/min
ብሊቱ ራ ላ	大井の北京	(# "K Demen Specin		27 水北	中の株件事業	間~年比	起 和 性	他用紙分字 Unconfined	破壊いり	生形体数	袋 植 北
Specimen No	Specimen Condition	Height	# cf Diameter # cm	Moisture Content	Wet Density	Vord Ratio	Degree of Saturation	Compressive Strength	Failure Strain	Deformation Coefficient Esa kg m²	Sensitivity Ratio
1	At v 5 i at 95 Underturbed Sample	145	7.5		1.73		:	1,00	3,5	34	
1	親ロルロフル(終 Remolded Sample				1	· -		1			
2	AL 2 5 A 44 Undisturbed Sample										
·	1 2777777777	r	7	-	T	! -	,				i]



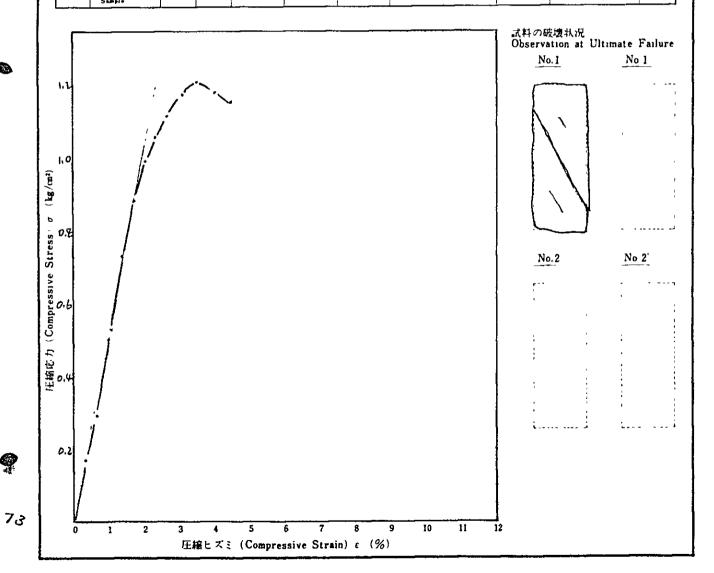
BH-10

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土の一軸圧縮試験

UNCONFINED COMPRESSION TEST

	·調査地点 Investigation F	lace	Thilay	wa Repair	Dockyard I	Project	試験年月日 Date		íI	H	11
	取位置(深度) ng Place, Dept	.h	20	, o m ~	20.7	m	士質名称 Soil Class	ification			
武科语 Sample			TWS	- 10			土粒子の比 Specific C		cla	<u>/</u>	
応力制行 Stress	和法 Control			ズミ制御法 ain Contr	ol		圧縮速さ Compressi	on Speed	9	%/min	## / min
排 式体 备 门	以料の状態	(R .A ! Demens Species	lo not	計 永 比	ም12 (ፍትት ብ ዝ	切でも比	120 €1 2	植生杨孝 ^士 Laconfined	放填料が	医乳体粉	錠 鹼 比
Specimen No	Specimen Condition	Δ = Height H'cm :	n (f Diameter p'cu	Mossture Content	Wel Density	Void Ratio	Degree of Saturation	Compressive Strength q. kg and	Failure Strain	Deformation Coefficient Ess kg m²	Sensitivity Rati
1	Al 1 C at 84 Undisturbed	, է ,	7.6		1.71	!			7 Ł	6.7	

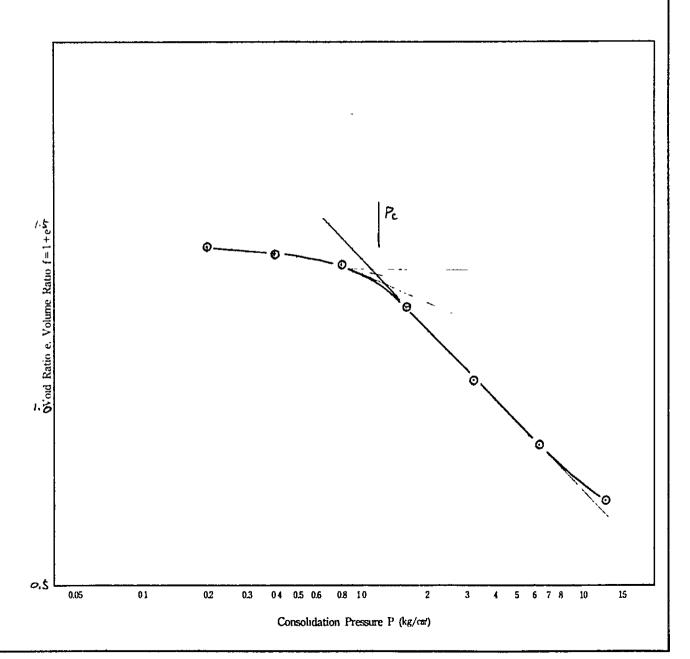


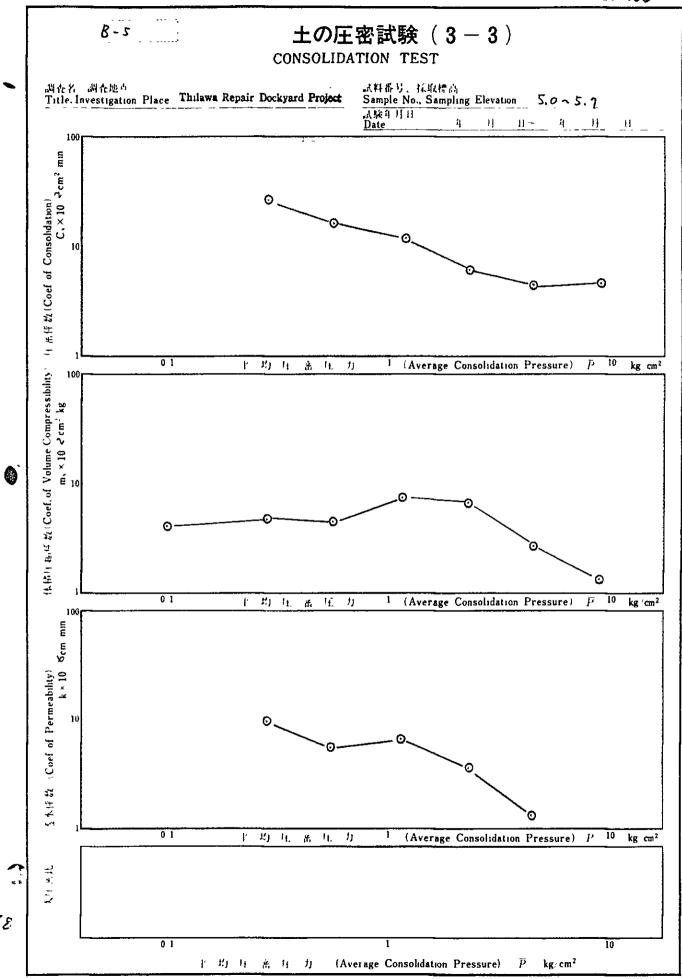
CONSOLIDATION TEST

Title, Investigation Place Thilaws Repair Dockyard Project

Sample No Sampling Elevation BH-5, $D=5.0 \sim 5.7 m$

	Diameter	D	6,00	C=	L.	Befor Test	₩o	55.¥	%	Liguid	LL	%	Compression Index
_	<u> </u>	_			Asstua Henst				_	Limit			-
mension	Height	211	2.00	CR	Mon	After Test	Wf		%	Specific Gravity	Ge		Cc. 0.64.
ectinen D	Section Area	٨	28.26		תב מל מטובר	Befor Test	Sm	100	%	Instal Void Ratio	e _o	1, 44	Consolidation Yield Stress
<i>.</i> 5	Volume	v	26.52	œ	Degri	After Test	Srf		%	Initial Volume Ratio	f.	z, 44	Pr 1, 2 kg or Pr

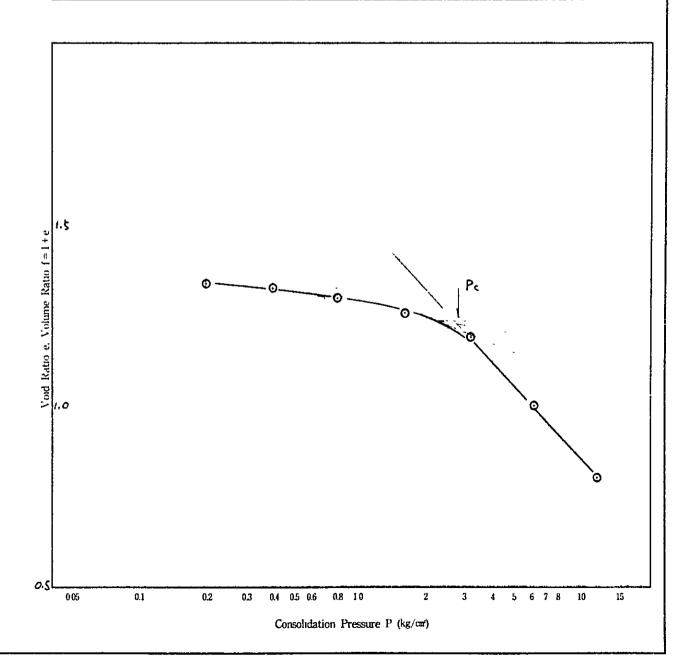


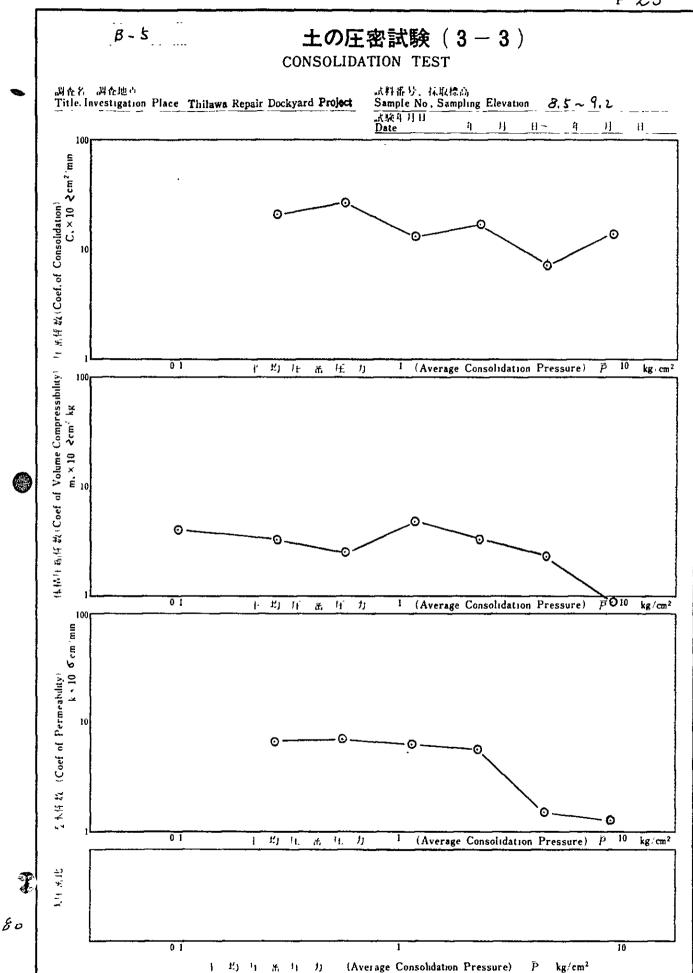


Title, Investigation Place Tullawa Repair Dockyard Project Date

Sample No Sampling Elevation B-5, 8.5 ~ 9, 2

	Diameter	D.	6.	œ	c	£ _	Befor Test	Wo	39.1 %	Liguid Limit	LL	4,	Compression Index
Mensuki	Height	211		00	CM -	Morstur	After Test	Wf	%	Specific Gravity	Gs		c 0.65
(I comean D	Section Area	A	28	.26	æ	e of	Befor Test	Sno	%	Initial Void Ratio	e _o	1.35	Consolidation Yield Stress
) ×	Volume	v	56	52	œ	Degre	After Text	Srf	%	Instial Volume Ratio	f o	2,35	Ps 2.8 ₩ ₩





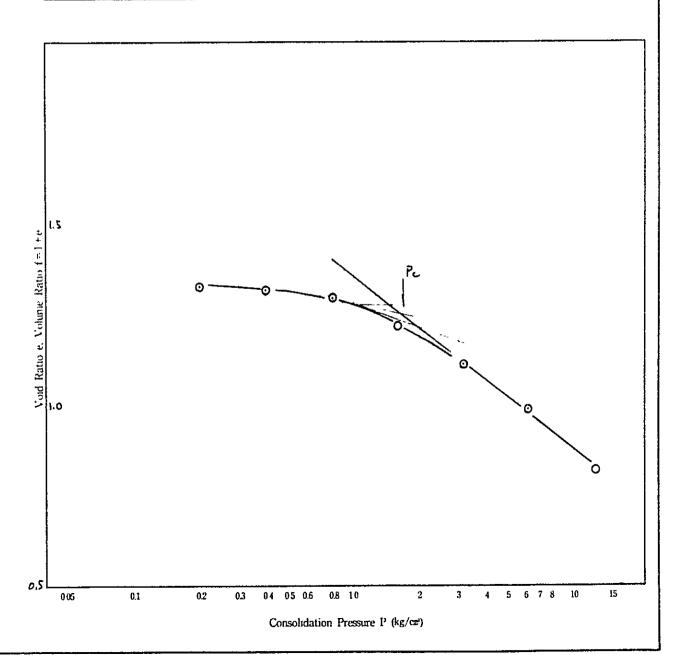


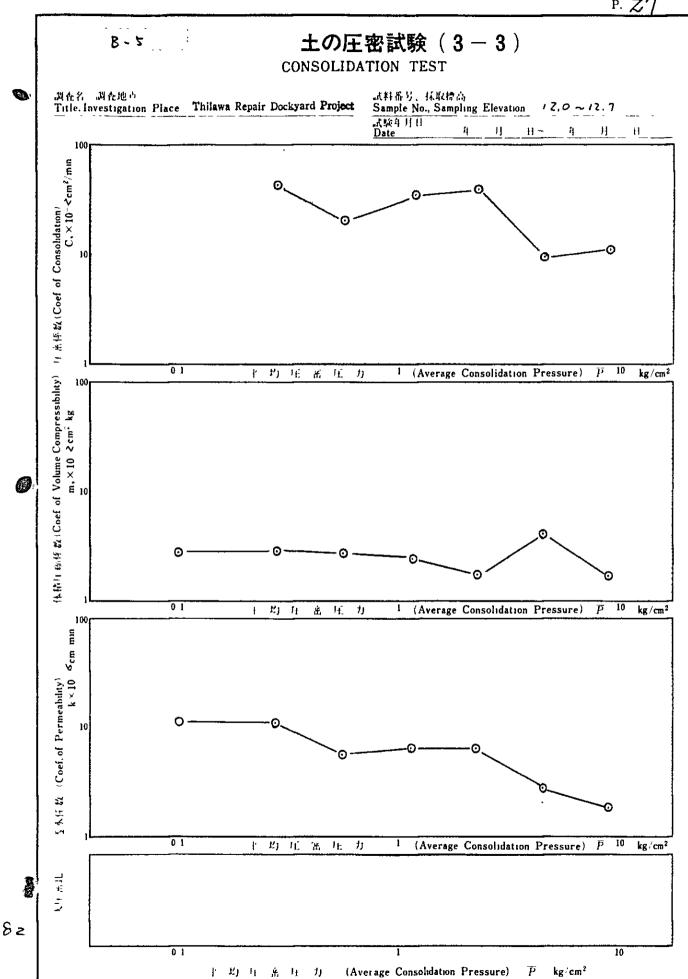
Title, Investigation Place Thilawa Repair Dockyard Project Date

Sample No Sampling Elevation BH-5 $D=12.0\sim12.7$

8

	Diameter	D	-6	.0	C#I	t H	Befor Test	Wο	45.7 %	Liguid Limit	1.1.	7,	Compression Index
THURSTONE	Height	211		٥	Ç#	Moister	After Test	Wt	9,0	Specific Gravity	Gu		c 0,48
Actinen Du	Section Area	A	28,	26	œ	e of atton	Befor Test	Sro	%	Initial Void Ratio	e _o	1,35	Consolidation Yield Stress
.S	Volume	v	56.	\$2	GP*	Safur	After Test	Sef	%	Initial Volume Ratio	fь	2,35	ء ۲۰۱۱ در





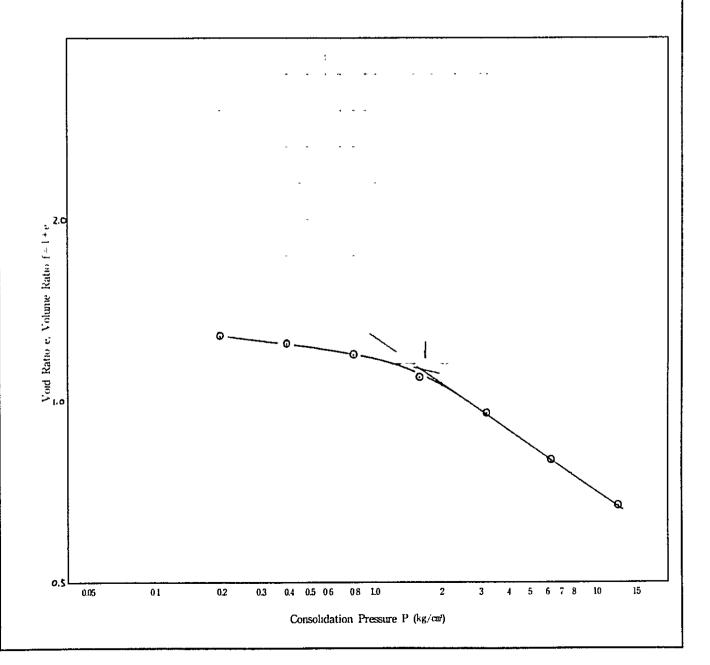


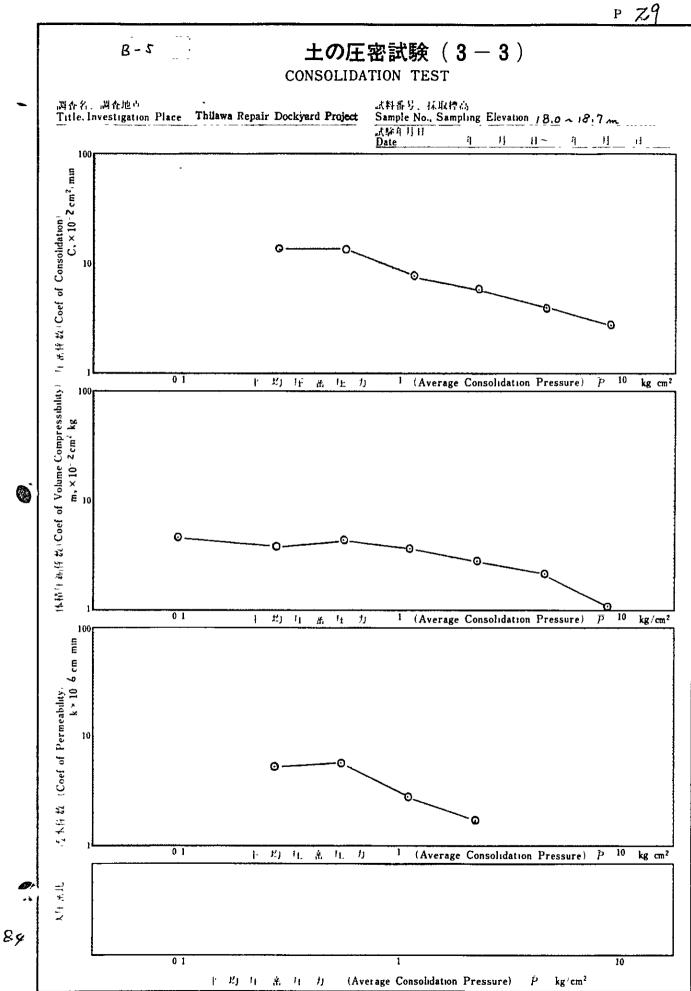
Title, Investigation Place Thilawa Repair Dockyard Project Date

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Sample No Sampling Elevation B-5 TWS-6 18.0 ~ 18.7 m

	Diameter	D	6.0 🕾	£ 4	Befor Test	Wo	478%	Liguid Limit	LL		a,	Compression Index
TYCISAIN	Height	211	2.0 🛥	Mossur	After Test	Wf	20.4 %	Specific Gravity	Gs	2.60		C 0.42
ecinien Du	Section Area	A	28,26=	atton	Befor Test	Sro	100 %	Initial Void Ratio	e _o	1.19		Consolidation Yield Stress
۶	Volume	v.	56.52 ≃	Degre	After Test	Srf	100 %	Initial Volume Ratio	fo	2.19		Py 1,-¶ kg #

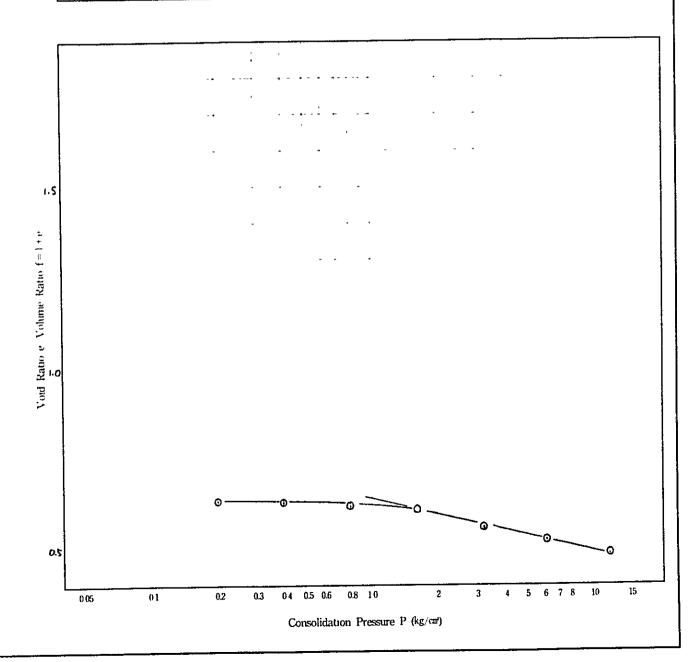


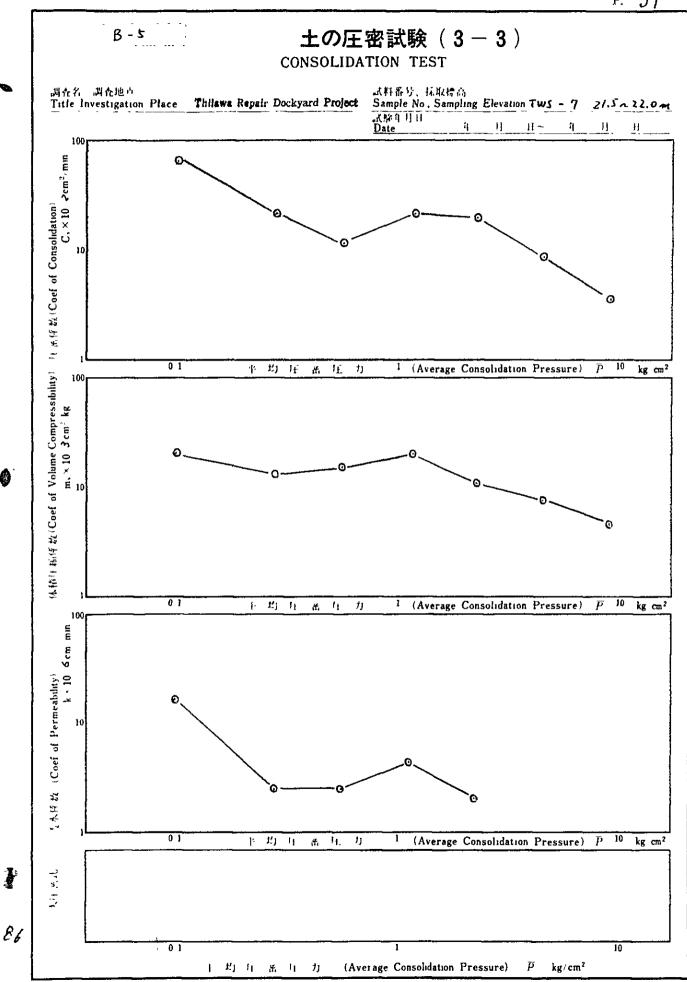


Title, Investigation Place Thilawa Repair Dockyard Project Date

Sample No. Sampling Elevation BH-5, 21.5~ 22.0 m

	Diameter	D	6.0	сıя	i z	Befor Test	Wo	23.7	%	Liguid Limit	IT		ο,	Compression Index
KAT.	Height	211	2,0	res.	Monst	After Test	Wí	18 6	9,	Specific Gravity	Gs	2.60		c 0.14
сижен Вип	Section Area	^	28.26		atton	Befor Test	Sro	96.7	%	Initial Void Ratio	e,	0.64		Consolidation Yield Stress
<i>)</i> ;	Volume	v	56.5	ζ જ	Degree	After Test	Srf	100	%	Initial Volume Ratio	f.	1.64		Pr KK 1

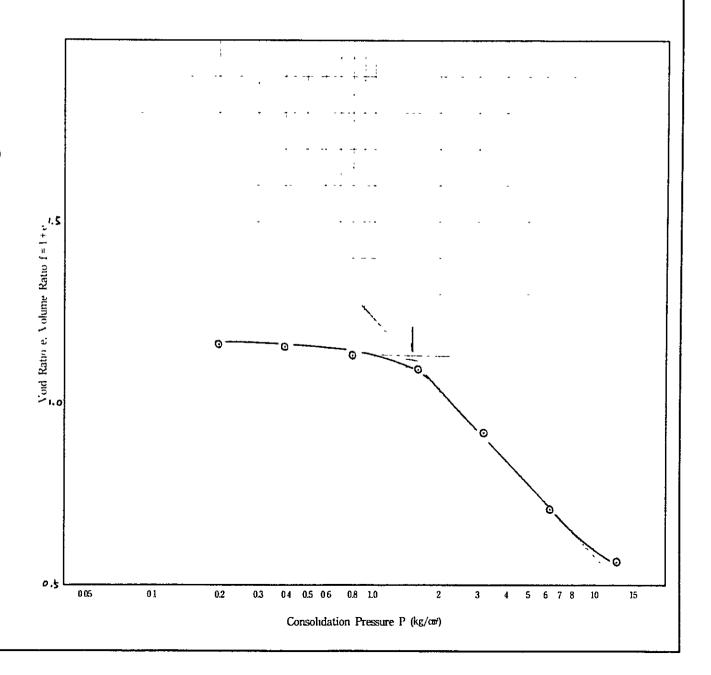


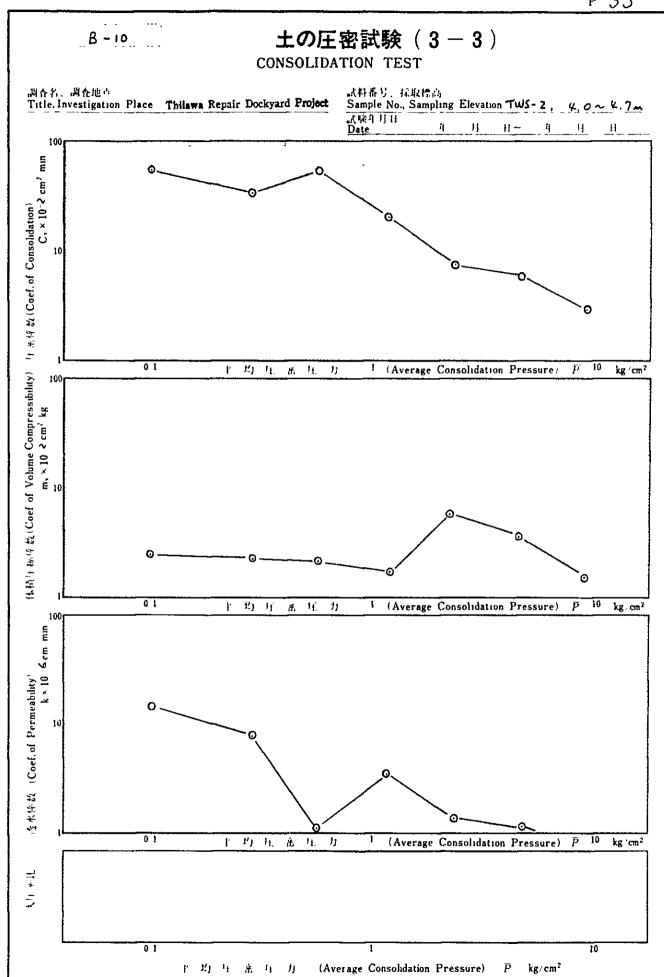


Title, Investigation Place Thilawa Repair Dockyard Project Date

Sample No Sampling Elevation B-10, TWS-2, 4.0~4.7~

	Diameter	D	6.0 **	# _	Befor Test	Wo	47.8 %	Lignid Limit	11.	a ₅	Compression Index
ITTENSION	Height	211	ζ,ο α	Moustur	After Test	Wf	21.5 %	Specific Gravity	Gs	2.60	a 0,65
рестен Б	Section Area	A	28.26 =	re of	Befor Test	Sro	/00 %	Initial Void Ratio	e _o	1.17	Consolidation Yield Stress
ds	Volume	v	\$6,52~	Degre	After Test	Srf	100 %	Initial Volume Ratio	f o	2.17	Py 1.55 kg car



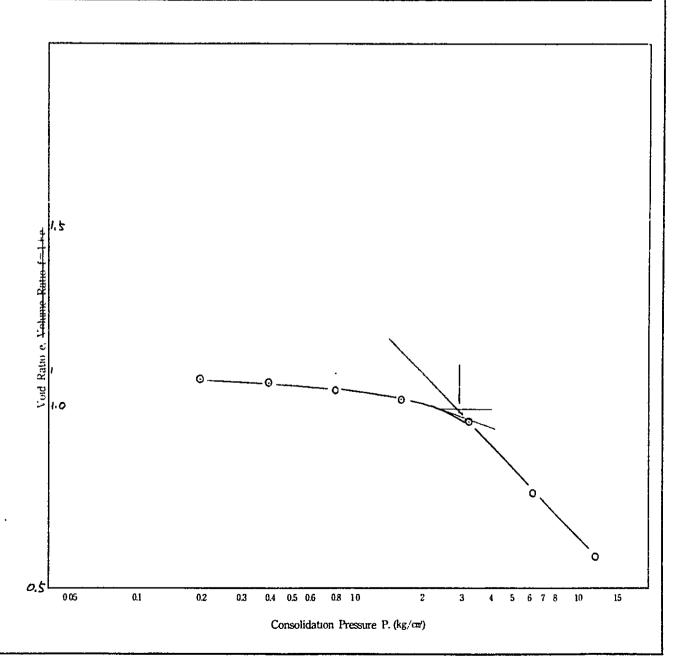


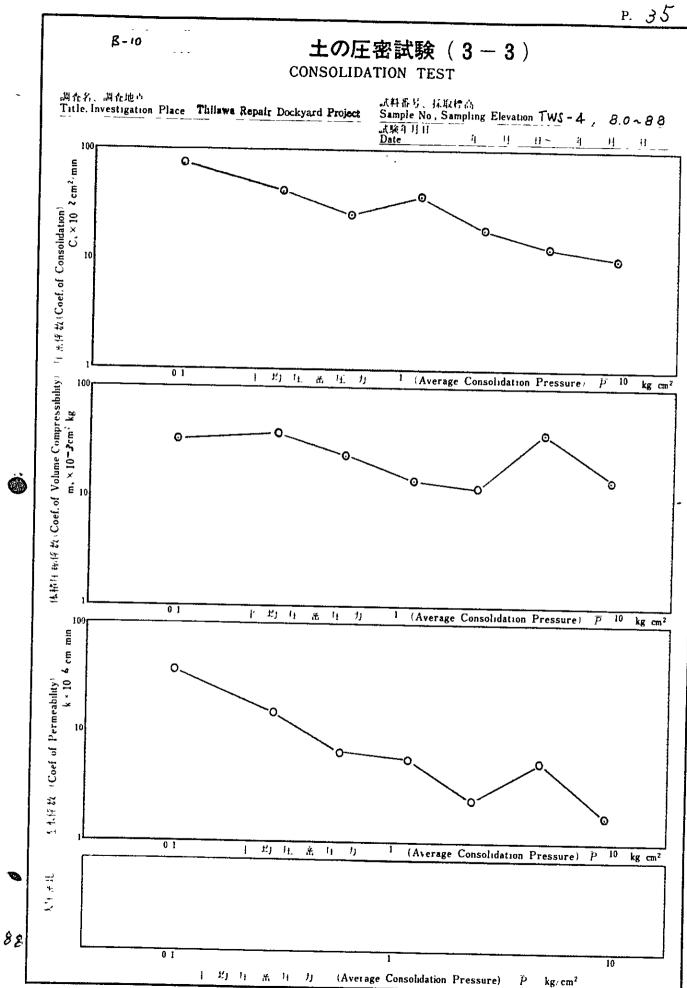
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Title, Investigation Place Thilawa Repair Dockyard Project Date

Sample No. Sampling Elevation Tws · 4 8.0 ~ 8.8

	Diameter	D	6.0 ∞	# F	Befor Test	Wo	4.0. E	۶.	Liguid Limit	ц		ئ.	Compression Index
mension	Height	211	2.0 ==	Muss	After Test	Wſ	21.3	%	Specific Gravity	Gs	2.60		c 0,69
Secured ()	Section Area	A	28.26 -	ation	Refor Test	Sno	100	%	Initial Void Ratio	e _n	0 99		Consolidation Yield Stress
,	Volume	v	56.52~	Degree	After Test	Srf	100	۹.	Initial Volume Ratio	f o	1.99		Ps 2,4 kear



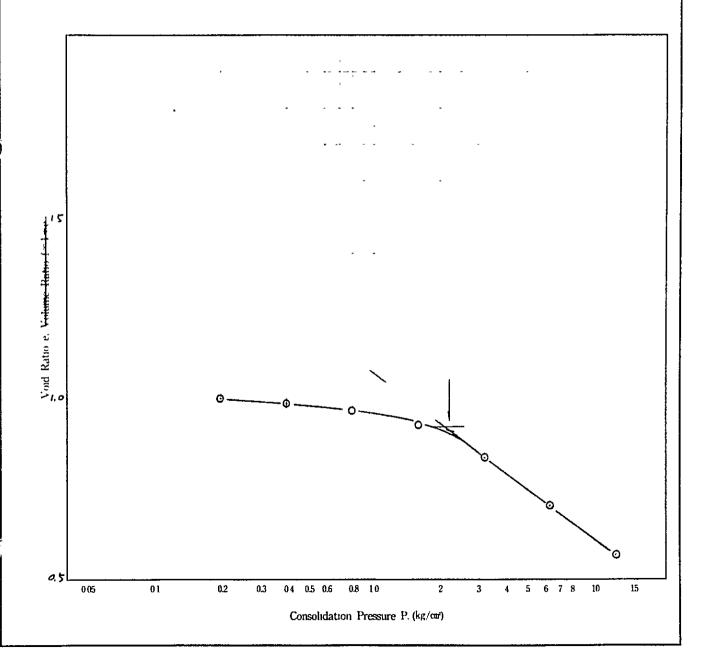


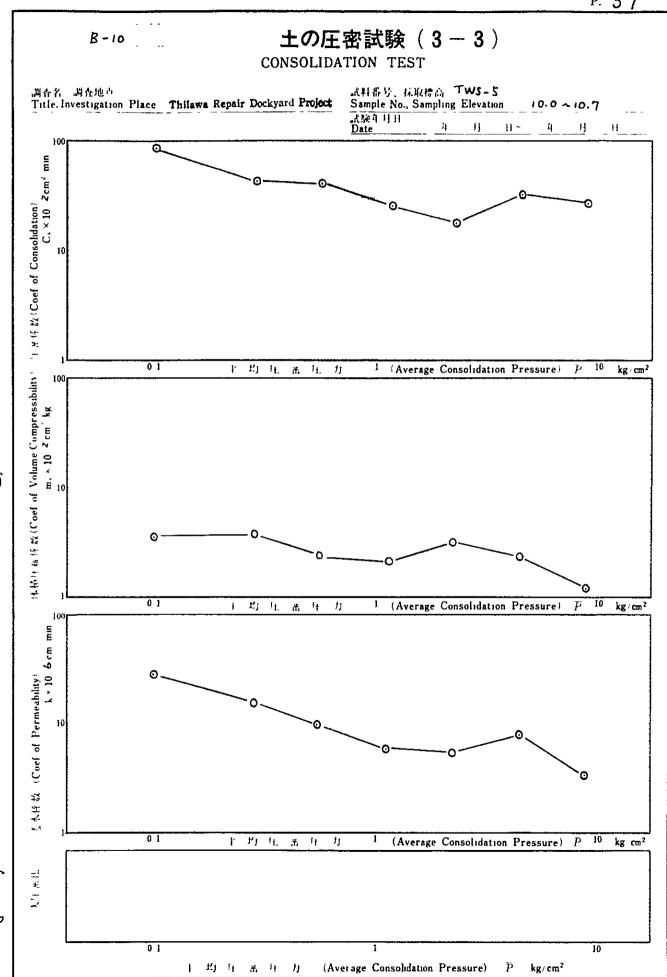
Title, Investigation Place

Thilawa Repair Dockyard Project Date

Sample No. Sampling Elevation 8-10, Tws-5, 10,0 210,7 m

	Diameter	D	6.0 =	gu.	Befor Test	Wo	39,5	%	Ligard	LL		o,o	Compression Index
Sperimen Dime	Height	214 -	۲, 0 ∞	Moistur	After Test	Wí		9,0	Specific Gravity	Ga	2.60		Cc 0,46
	Section Area	A	28.26 =	Degree of	Befor Test	Sro	100	%	Initial Void Ratio	e _o	1,01		Consolidation Yield Stress
	Volume	v	\$6.52=		After Test	Srf	100	%	Iratial Volume Ratio	f.	201		Љ 2.7 каж



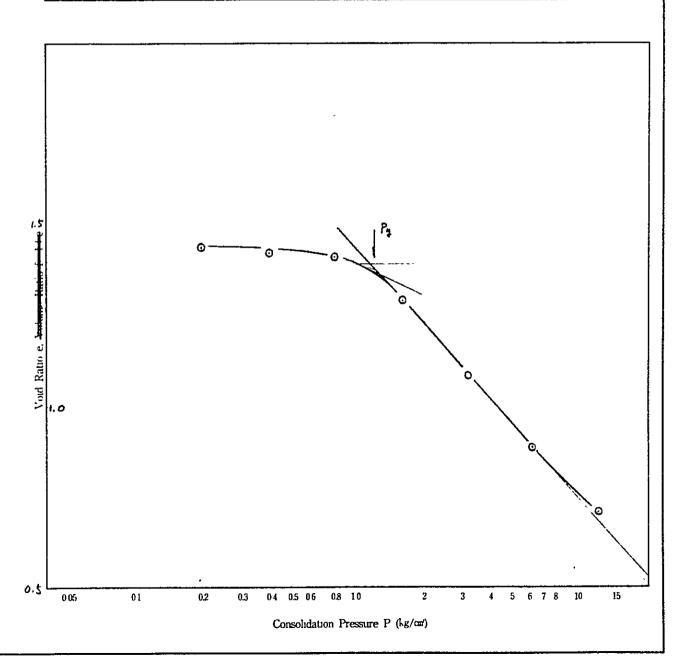


CONSOLIDATION TEST (1)

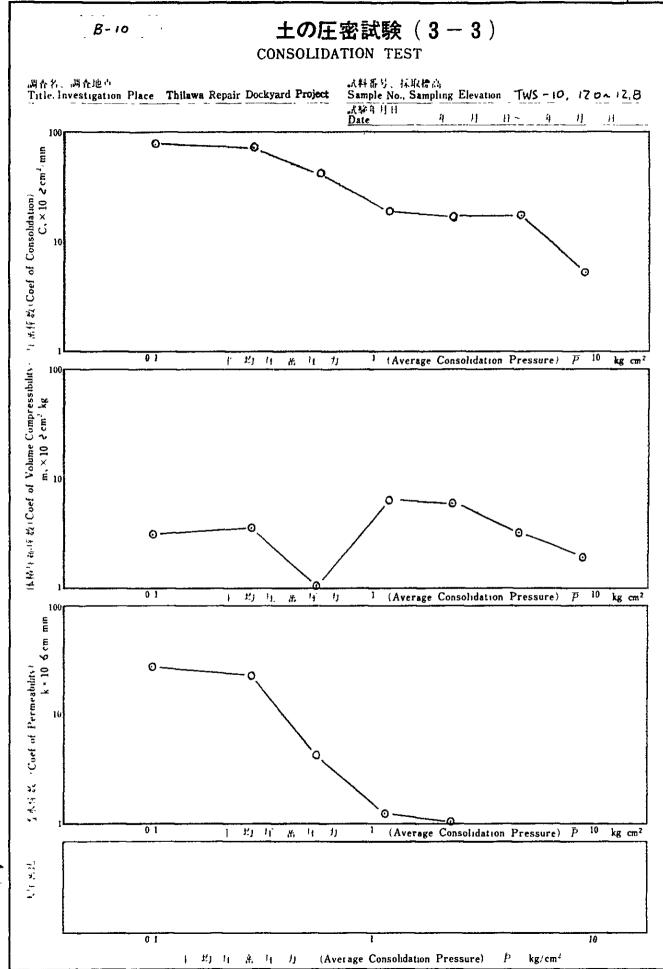
Thilawa Repair Dockyard Project Title, Investigation Place Date

Sample No Sampling Elevation Tws - 6 12.0~ 12.8

	Diameter	D	6.0	Can	ę.	Befor Test	Wo	53.7	%	Ligned	ш		%	Compression Index
_		ļ		_	ushr rtent		<u> </u>			Limit				
Imension	Height	231	2,0	сш	S. S.	After Test	Wi	21.8	%	Specific Gravity	Gs	2.60		α 0,7
ovcimen D	Section Area	٨	28. 2 6	7	ee of	Befor Test	Sro	96,3	%	Instal Void Ratio	e _o	1.45		Consolidation Yield Stress
<i>y</i>	Volume	v	56.52	a	Degre Satur	After Tost	Srf	100	%	Initial Volume Ratio	fo	2. 4.5		P5 1,2 kg/am







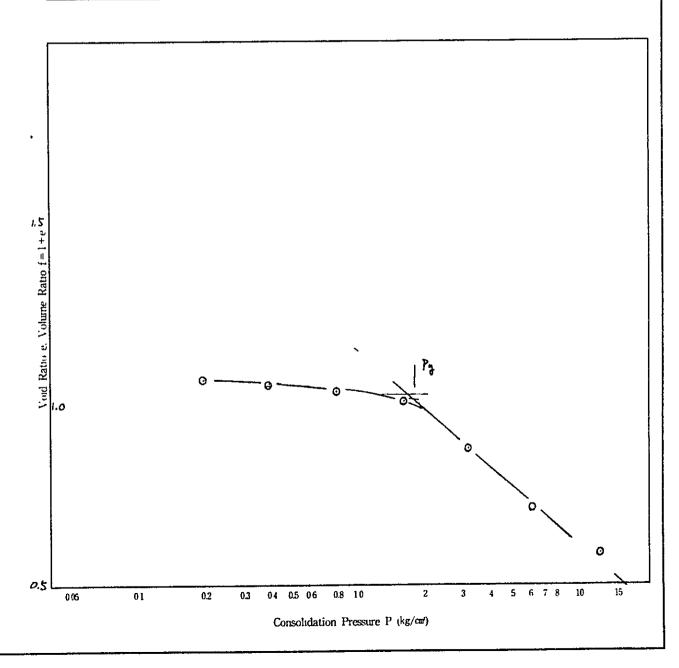
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CONSOLIDATION TEST (1)

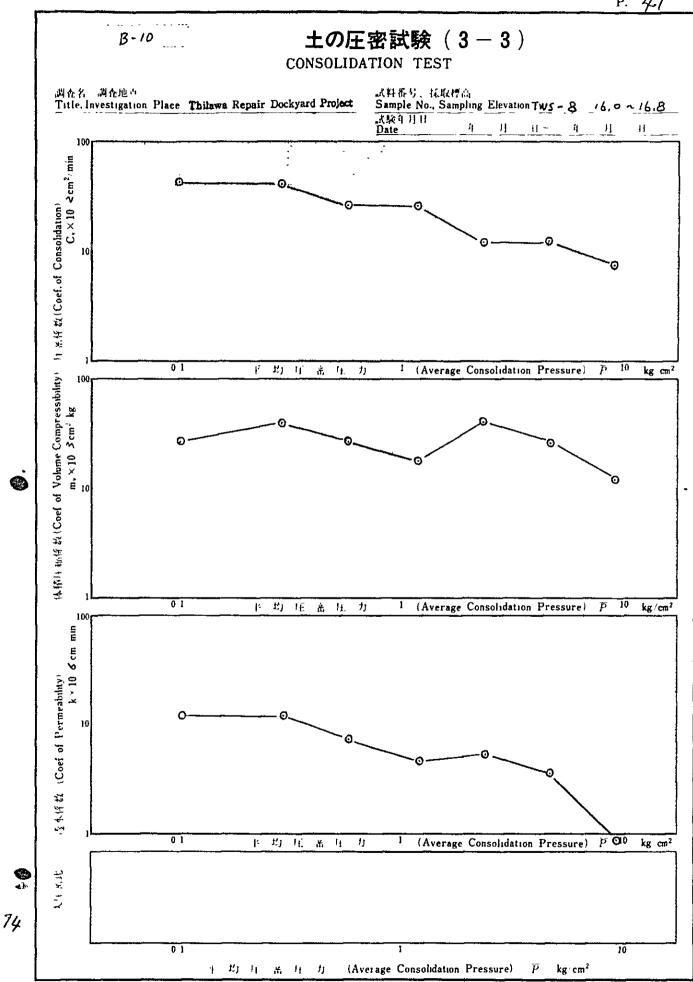
Title, Investigation Place Thilawa Repair Dockyard Project Date

Sample No. Sampling Elevation B-10, $16.0 \sim 16.8$

	Diameter	D	6.0 =		Befor Test	wo	43,7	%	Liguid	LL		%	Compression Index
rtsion	Height	211	Z, Ø 🚥	Montum	After Test	wı		•,,	Limit Specific Gravity	Gs.	2,60		Cr 0,72
n men Dim	Section Area	Α	28,26 =	thon the	Befor Test	Sro	100	%	Initial Void Ratio	eυ	1.08		Consolidation Yield Stress
<i>}</i>	Volume	v	56.52=	Satura	After Test	Srf	100	3∕6	Initial Volume Ratio	f o	z.08		Py 1,83 4x100





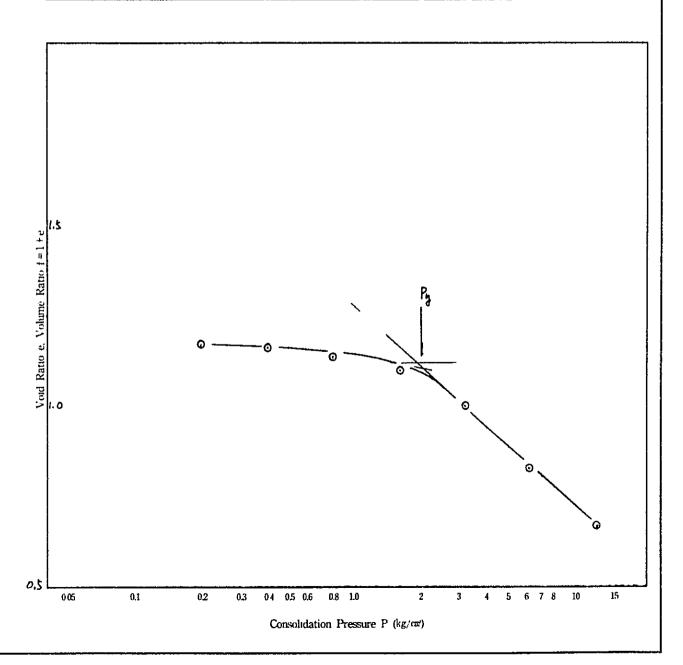


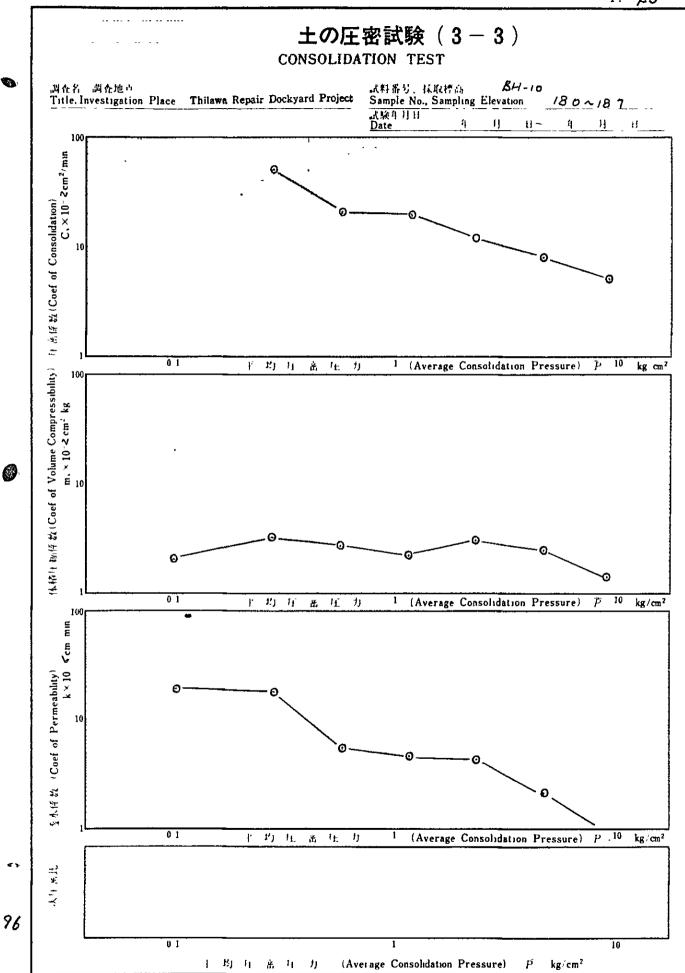
CONSOLIDATION TEST (1)

Title, Investigation Place Thilawa Repair Dockyard Project Date

Sample No. Sampling Elevation BH-10. $18.0 \sim 18.7$ m

	Diameter	D	6.0	C.B.	gu .	Befor Test	Wo.	48.g %	Lignid	ш		e,	Compression Index
e de la constante de la consta	Height	2H	7.0	C#	Moistur	After Test	Wí	24.2 %	Specialic Gravity	Gr	2.60		c 0,7
ecunen Dur	Section Area	٨	z8.z6	-	e of	Befor Test	Sro	100 %	Initial Void Ratio	e _o	1.18		Consolication Yield Stress
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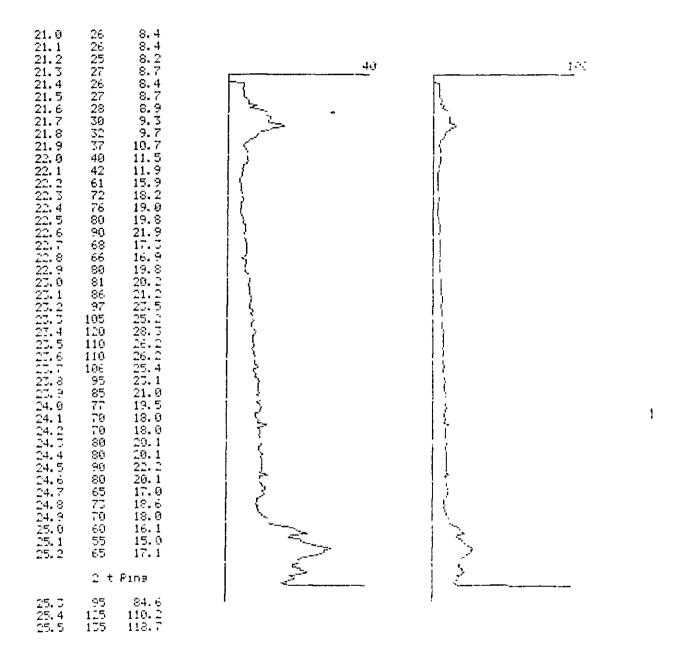




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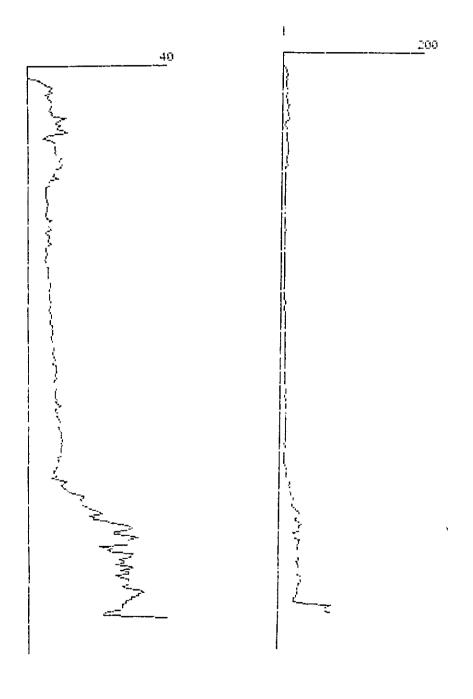
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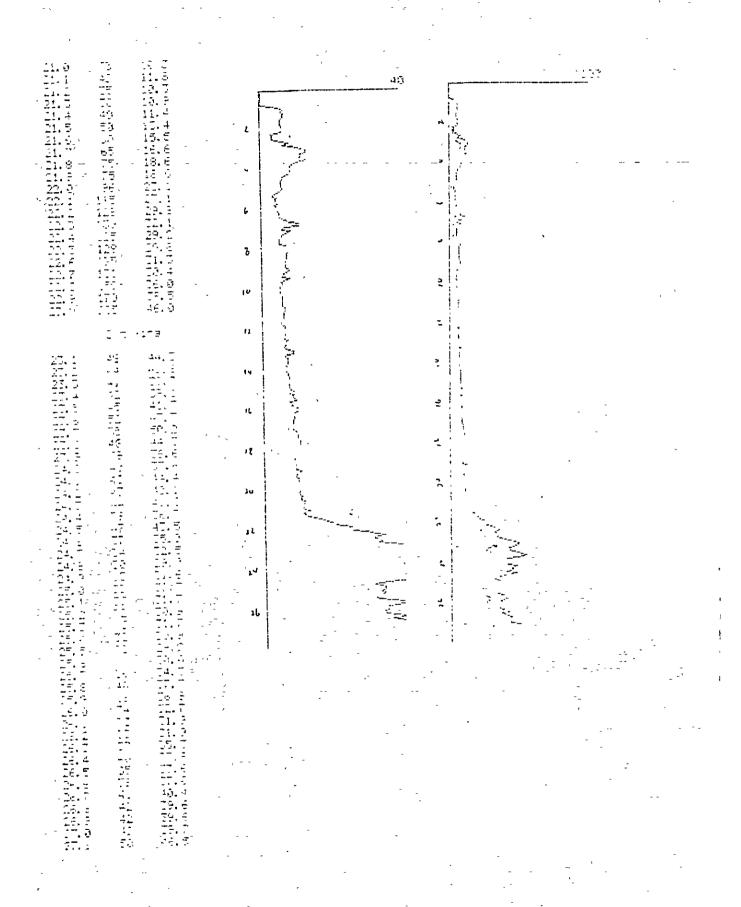
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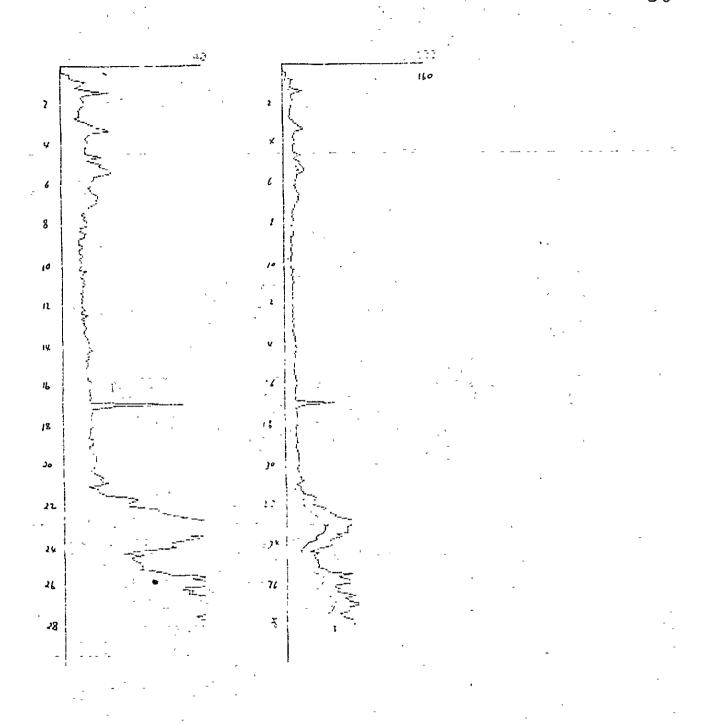
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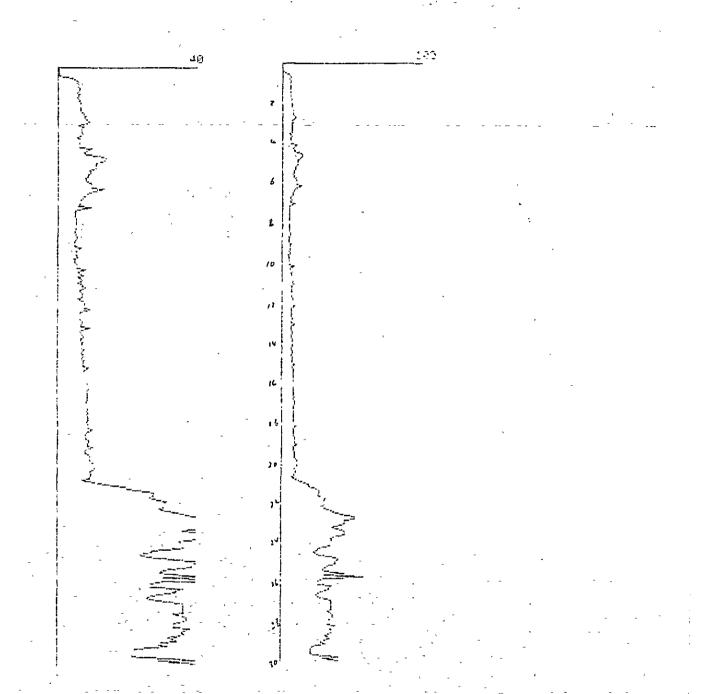
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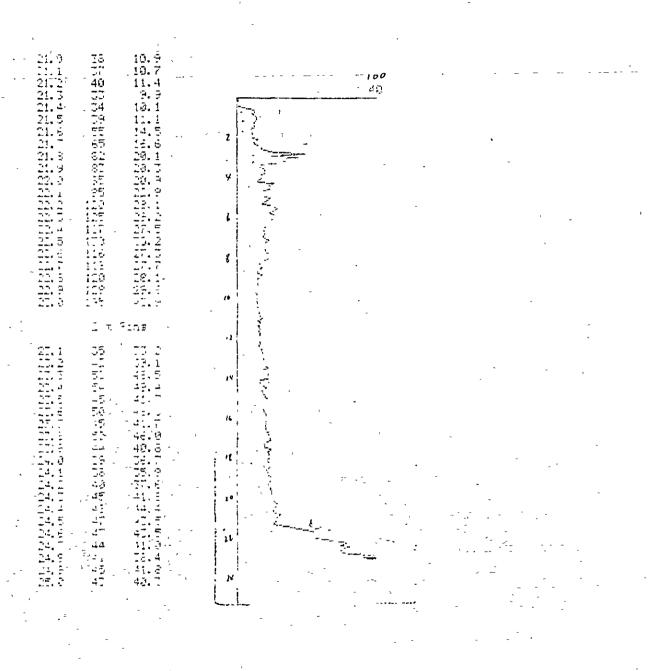


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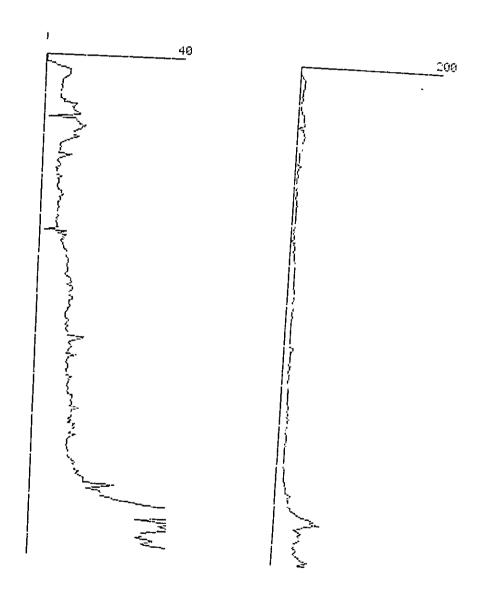
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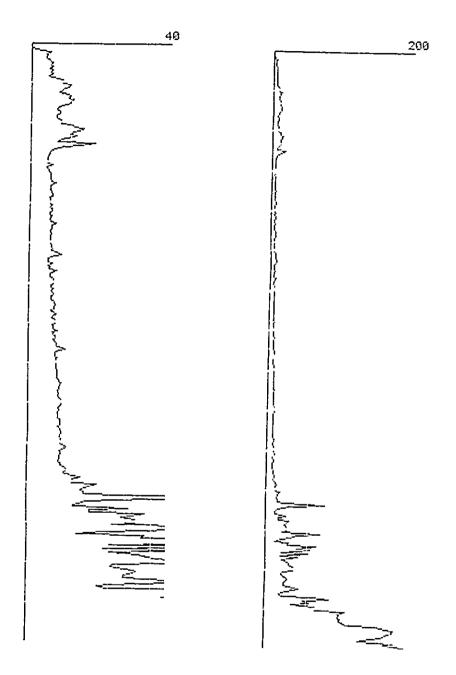
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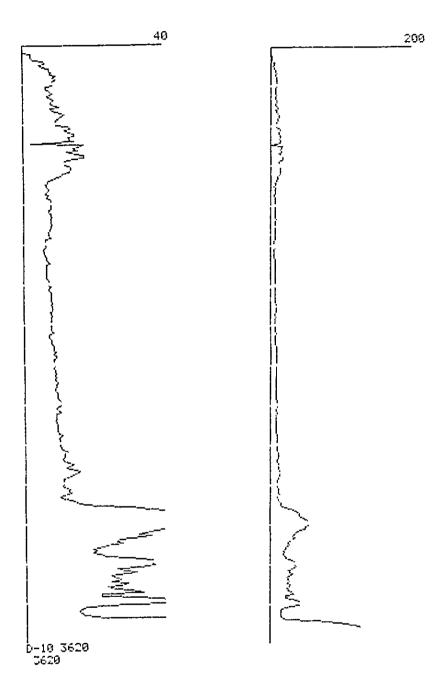
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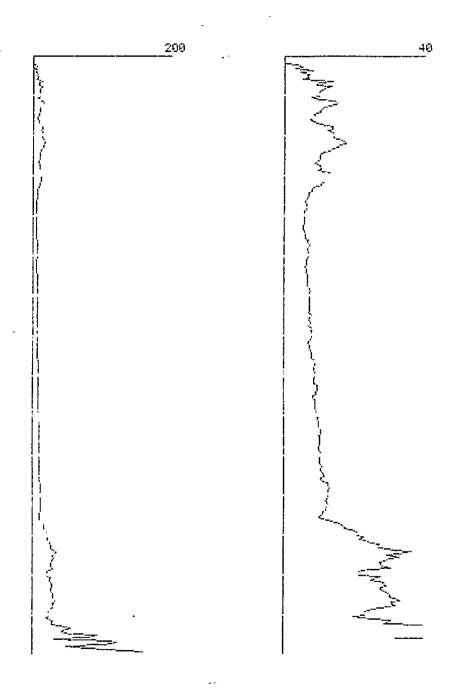


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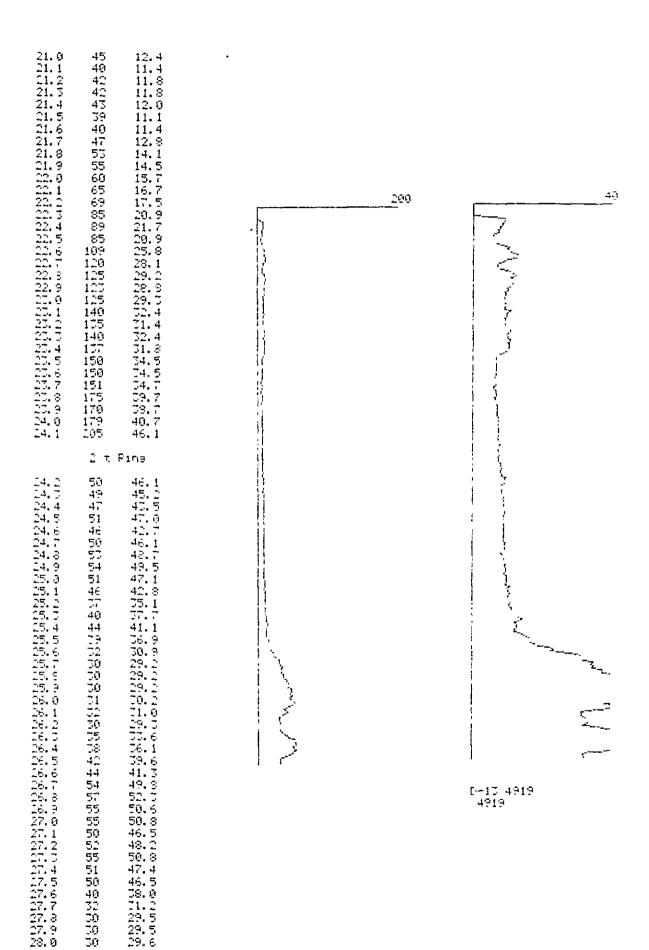
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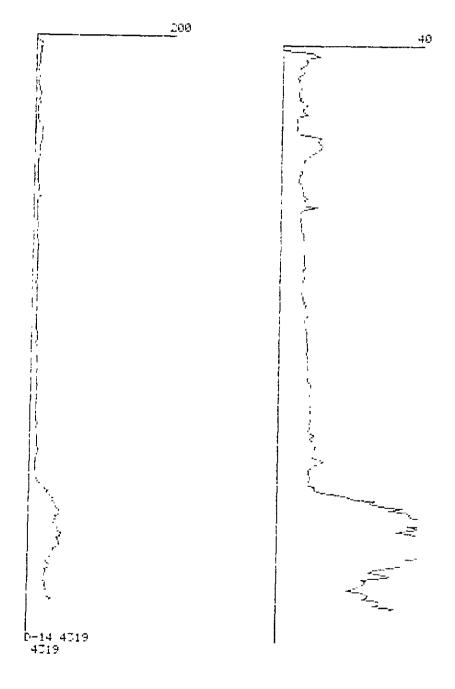
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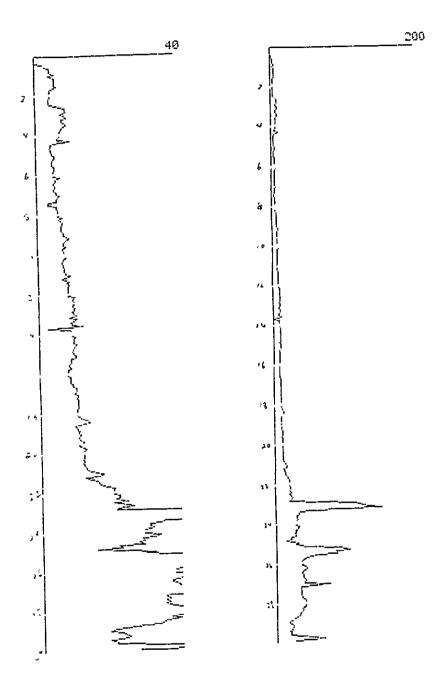
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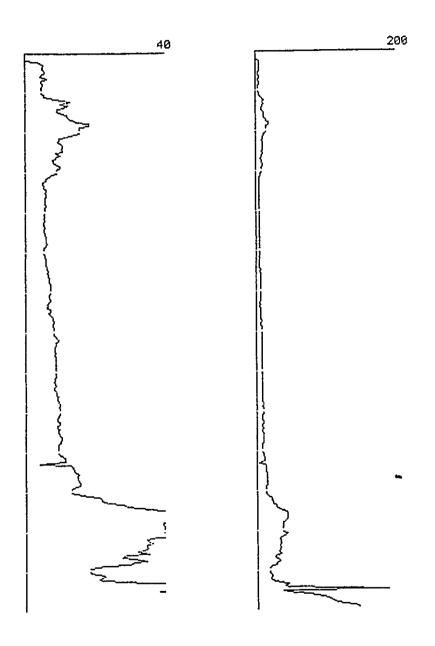
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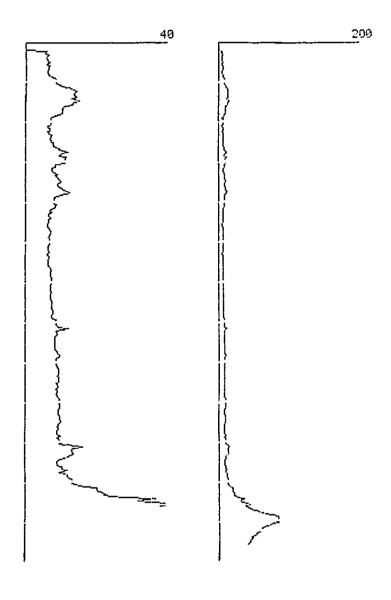
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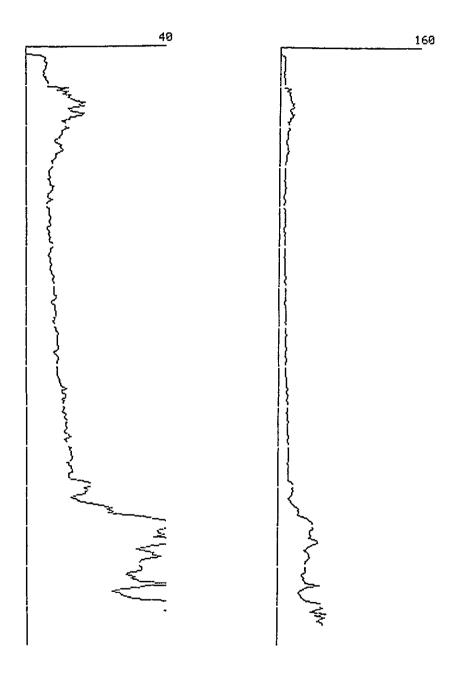


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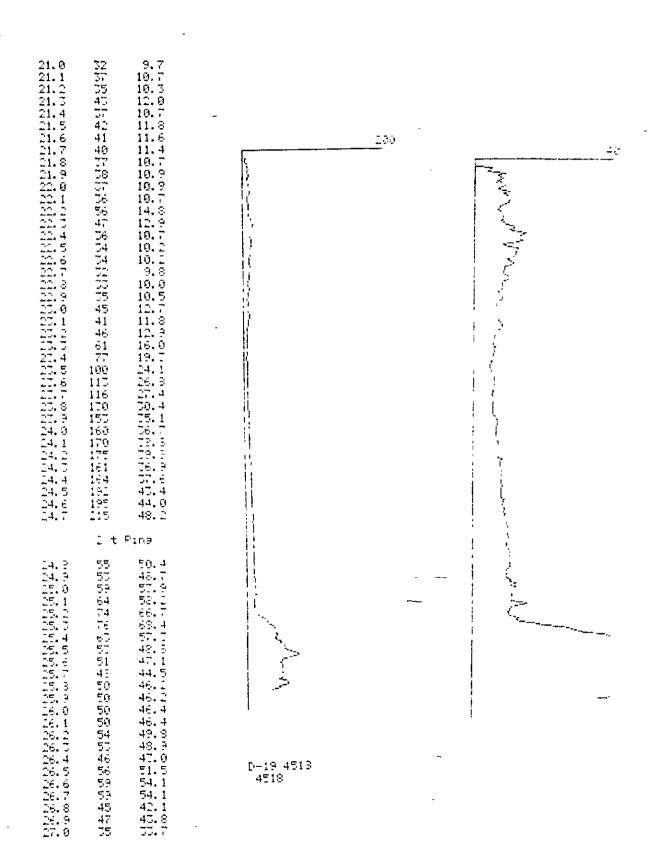
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26. 1	130	30.8	
26. 2	130	30.8	
26. 3	127	30.1	
26. 4	110	26.6	

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