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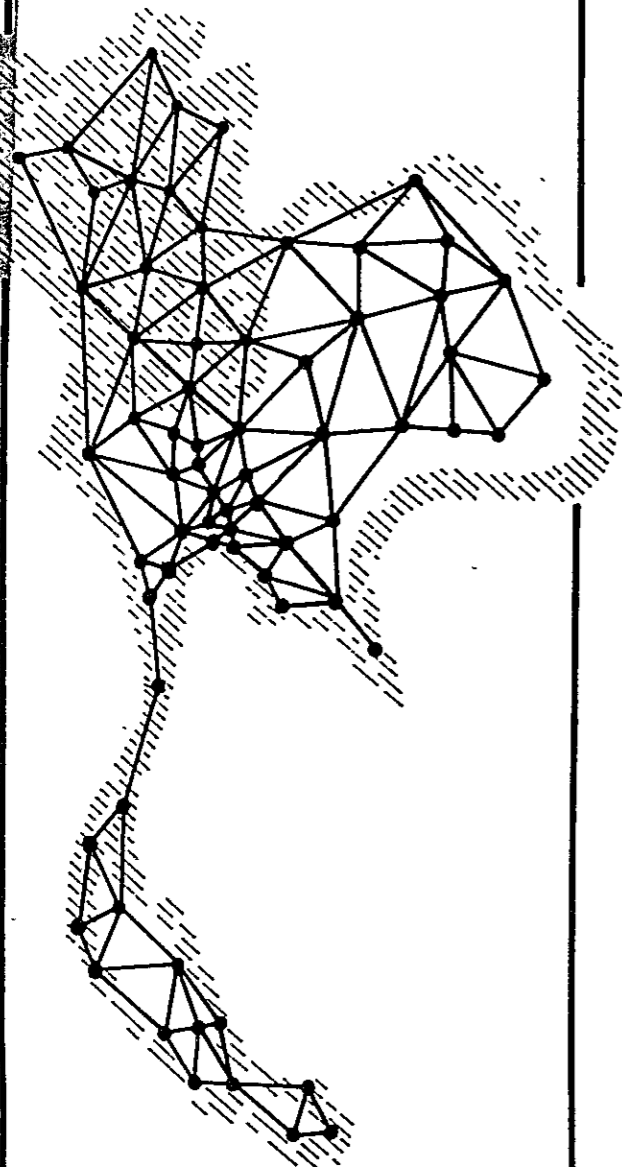


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THE KINGDOM OF THAILAND
MINISTRY OF COMMUNICATIONS
DEPARTMENT OF HIGHWAYS

ROAD DEVELOPMENT STUDY IN THE NORTHERN REGION

PHASE 2
FEASIBILITY STUDY



FINAL REPORT
VOLUME 2 ROUTE REPORT
MARCH 1982

JAPAN INTERNATIONAL
COOPERATION AGENCY

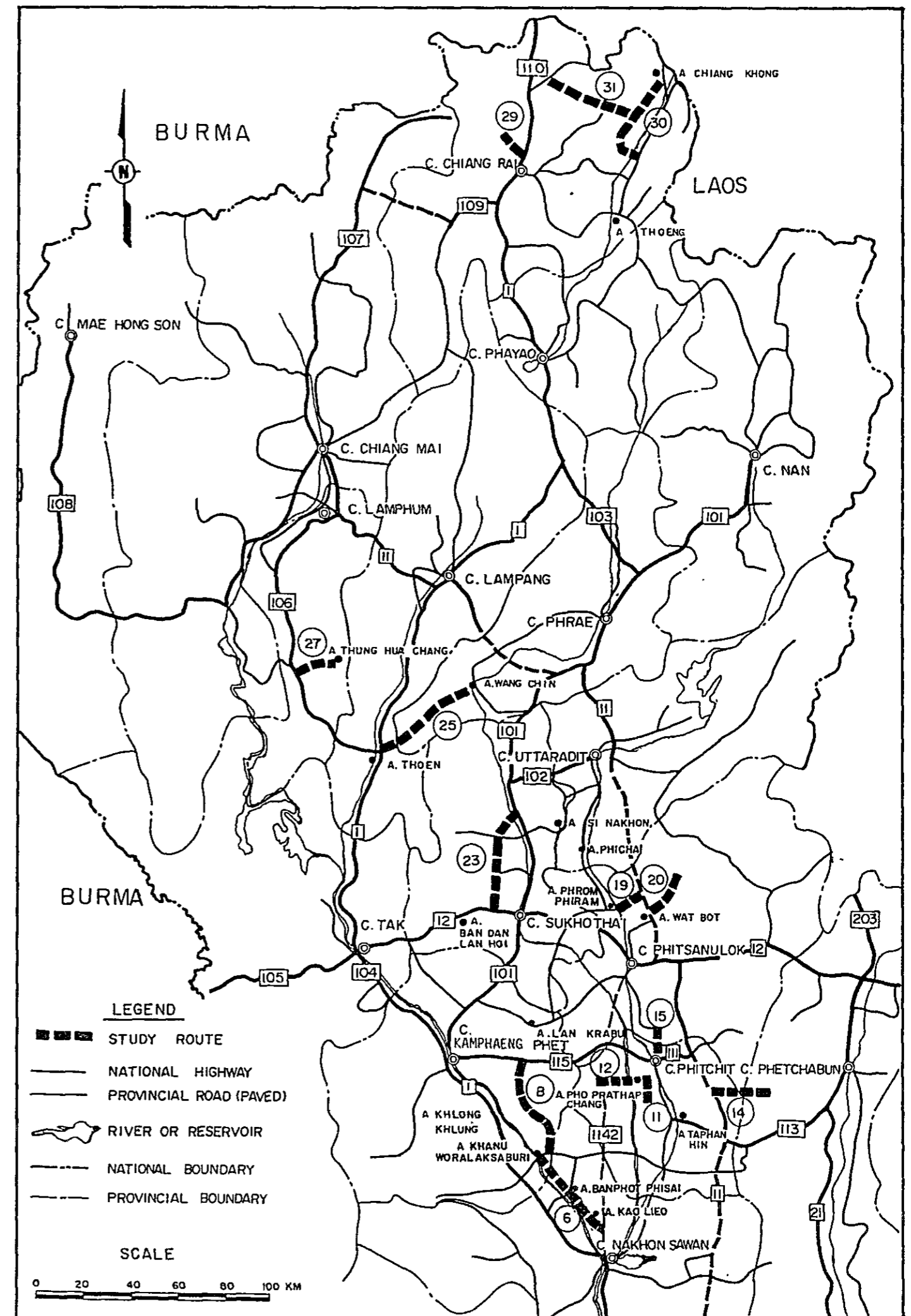
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STUDY ROADS

Study Road No.	Changwat	Origin - Destination	Route Pages	Description (Drawings)
6	Kamphaeng Phet/ Nakhon Sawan	Khanu Woralaksa Buri - Kao Liew - Rt. 117	6-1 - 6-39	(DWG. 6-1/6-6)
8	Kamphaeng Phet	Rt. 115 (B. Thung Mahachai) - B. Nong Takhian	8-1 - 8-24	(DWG. 8-1/8-6)
11	Phichit	Rt. 1068 - Pho Prathap Chang	11-1 - 11-16	(DWG. 11-1/)
12	Phichit	B. Wang Chik - Rt. 117 (B. Pa Daeng)	12-1 - 12-17	(DWG. 12-1/12-2)
14	Phichit/ Phetchabun	Rt. 11 (B. Nong Khanak) - B. Wang Pong	14-1 - 14-18	(DWG. 14-1/14-3)
15	Phichit/ Phitsanulok	B. Wang Tham - Rt. 1114 (B. Tha Makham)	15-1 - 15-16	(DWG. 15-1/)
19	Phitsanulok	Phrom Phiram - Rt. 11 (B. Nong Makhang)	19-1 - 19-17	(DWG. 19-1/19-2)
20	Phitsanulok	Wat Bot - B. Nakham	20-1 - 20-18	(DWG. 20-1/20-2)
23	Sukhothai	Rt. 12 (Muang Kao Sukhothai) - Si Satchanalai	23-1 - 23-32	(DWG. 23-1/23-6)
25	Lampang/Phrae	Toen - Wang Chin	25-1 - 25-24	(DWG. 25-1/25-6)
27	Lamphun	Rt. 106 (B. Mae Thoei) - Thung Hua Chang	27-1 - 27-18	(DWG. 27-1/27-2)
29	Chiang Rai	Rt. 110 (B. Rong Sua Ten - B. Huai Khom	29-1 - 29-17	(DWG. 29-1/29-2)
30	Chiang Rai	Rt. 1020 (B. Thung Ngiu) - Rt. 1020 (B. Chumphu)	30-1 - 30-24	(DWG. 30-1/30-6)
31	Chiang Rai	Rt. 1016 (B. Kiu Phrao) - Rt. 1174 (B. Kaen Tai)	31-1 - 31-26	(DWG. 31-1/31-7)



STUDY ROUTE NO. 6

Khanu Woraksa Buri – Kao Liao

-Rt.117 (B. Don Doo)

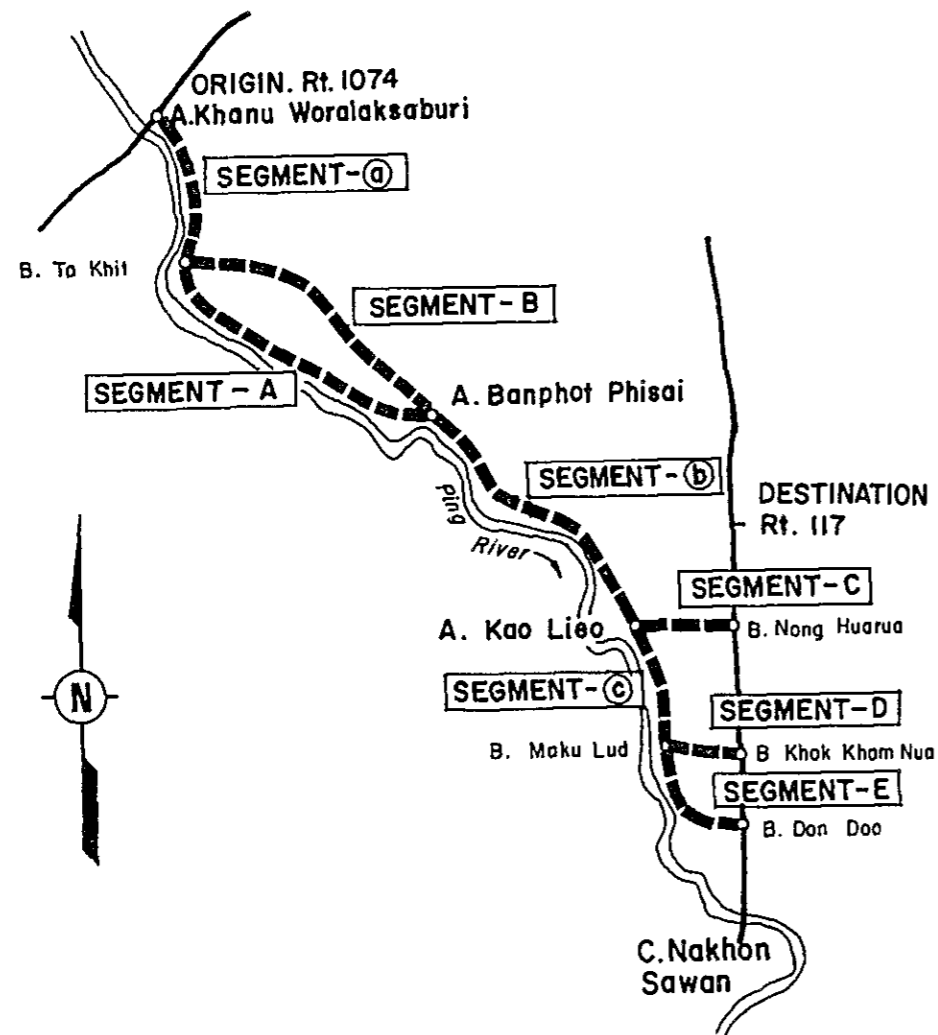
L = 46.0 Km (6-4)

Changwat : Kamphaeng Phet / Nakhon Sawan

1-2 Alternative Routes

In view of the availabilities of the existing roads in the area several alternative routes have come into the studies for the selection of the best route connecting A.Khanu W.Buri with Rt.117. All the alternatives were considered in the east side of the Ping with a view to raise agricultural and transportation benefits to the densely populated area. The routes considered for comparison are Segment A and B in the upper section between B.Ta Khit and A. Banphot Phisai and three Segments C,D and E in the lower section between A.Kao Liao and Rt.117 as shown in the following illustration.

In the middle section between A.Banphot Phisai and A.Kao Liao, no alternative routes were deemed necessary as the existing road connects those Amphoe with good alignments of short length.



Combining the Segments set out above, four alternative routes were planned for the comparative studies. The following are brief descriptions of the alternative routes.

Alternative Routes

Alternative Route	Combination of Segment	Route Characteristics	
		Segment A or B	Segment C,D or E
6 - 1	Ⓐ - A - Ⓑ - C	for the benefit of many residents living along bad road in Segment - A	the shortest access to the main road Rt. 117
6 - 2	Ⓐ - B - Ⓑ - C	for the promotion of agricultural development around Segment-B	Same as Route 6-1
6 - 3	Ⓐ - A - Ⓑ - Ⓒ - D	Same as Route 6 - 1	for the benefits of people along Segments Ⓒ and of sugar cane factory located near B.Makulua
6 - 4	Ⓐ - A - Ⓑ - Ⓒ - E	Same as Route 6 - 1	for the benefits of residents along Segment Ⓒ and E and of sugar cane factory located near B. Makulua

1-3 Conditions of Existing Road

In table 6-1-1, main features of the existing roads are summarized from the results of the road inventory survey.

Segment - (a)

The first 7.8 Km of road between the beginning point and B.Ta Khit runs in parallel with the Ping. First half is a 4-6 m wide laterite surfaced road in bad conditions and remaining section is a bad earth road of low embankment with the width of 3.5-4.0 meters. Horizontal and vertical alignments are fair in whole section.

Table 6-1-1 SUMMARY OF ROAD INVENTORY

Segment	Changwat	Route Name	Route Section		Length (km)	Terrain	Roadway Condition										Land Use	Overflow Height X Length (m)
			Origin	Destination			Surface		Alignment		Road Cross Section			Bridge				
							Earth Lat. : S.T.	Length : (km)	Condition	Hori-zontal	Verti-cal	Width (m)	Emb. H. (m)	Cut D. (m)	Nos.	Width (m)		
Seg. (a)	Kamphaeng Nakhon Sawan	Rural road	A. Khanu-Woralaksa Buri	B. Ta Khit	7.8	Flat	E : 4.1 L : 3.6 ST. : 0.1	Bad	Fair	Fair	3.5 6.0	0 0.9	-	-	-	-	Paddy	0.3x1450
Seg. A	Nakhon Sawan	Rural road	B. Ta Kit	A. Banphot Phisai	14.5	Flat	E : 9.5 L : 4.2 ST. : 0.8	Bad	Fair	Fair	3.0 8.6	0.1 0.8	0 0.2	-	-	-	Paddy	-
Seg. B	Nakhon Sawan	Rural road	B. Ta Kit	A. Banphot Phisai	15.9	Flat	E : 4.4 L : 11.5	Bad Good	Fair	Good	3.0 5.5	0 1.0	-	Timber	4.0	7.5	Paddy Sugar-cane	0.3x500 1.2x3000
Seg. (b)	Nakhon Sawan	Rural road	A. Banphot Phisai	A. Kao Lio	14.8	Flat	E : 4.6 S : 9.9 ST. : 0.3	Bad	Fair	Good	4.5 8.7	0 1.5	-	-	-	-	Sugar-cane Plan-tation	0.3x3440 0.2x40
Seg. C	Nakhon Sawan	Rural road	A. Kao Lio	Rt. 117	4.8	Flat	L : 4.7 ST. : 0.1	Fair	Fair	Good	4.5 6.0	0.3 1.0	-	-	-	-	Paddy Sugar-Cane	-
Seg. D	Nakhon Sawan	Rural road	B. Makulua	Rt. 117	4.7	Flat	L : 4.7	Bad	Bad	Fair	3.0 10.0	0.2 0.8	-	-	-	-	Sugar-cane	-
Seg. (c)	Nakhon Sawan	Rural road	A. Kao Lio	B. Makulua	5.1	Flat	L : 5.1	Fair	Fair	Fair	4.0 7.0	0 1.2	-	Con-crete	7.0	15.0	Sugar-cane	0.3x40
Seg. E	Nakhon Sawan	Rural road	B. Makulua	Rt. 117 B. Don Doo	4.5	Flat	L : 4.5	Bad	Bad Fair	Fair	4.0 5.0	0 0.9	-	-	-	-	Sugar-cane	1.2x2000

Segment - A

The route runs parallel to the Ping, about 150-200 m away from it. As the most villages are built-up to the riverside through this section, private houses and offices are not seen beside the road. About 70% of whole length of 14.5 Km is an earth road of very bad condition. The width is 3 - 6 m. with low embankment height.

Laterite surfaced section is about 4 Km. in both ends of this segment. Surface condition of it is fairly good.

Segment - B

Starting at B.Ta Khit, the route proceeds easterly for 4 Kms., then gradually directs to Banphot Phisai through flat sugarcane or paddy field. About 70% of the Segment is a fair to bad laterite surfaced road of 3.0 - 5.5 m. wide.

Vertical alignment is almost flat, but curved horizontal alignment in succession.

For 3 kms. close to Banphot Phisai, existing road has suffered overflows which sometimes exceed one meter above the road surface.

Segment - (b)

The route runs in the same situation as in the Segment - A. Laterite surfaced section which shares 70% of this Segment is fair to bad condition. Both vertical and horizontal alignments are fair.

For 3.5 Km in the middle part of the Segment is an earth road and suffered overflows 30 cm above the road surface.

Segment - C

The existing route directs north-east from Kao Lieo to Rt-117, taking the length of 4.8 Km. The laterite road is 4.5-6.0 m wide with good alignment.

Segment - D

The existing route starting at B.Makulua to Rt.117 is as narrow as 3.0 - 4.0 m, with the low embankment and curved alignment. It makes 2 Km detour between these two points.

Segment - (c)

This existing route between Kao Lieo and B.Makulua passes through the populated villages in full length. Private houses stand close to the shoulders, resulting in the shortage of right-of-way width.

The horizontal alignment with right angle curves is not good in general.

Segment - E

First half of this Segment follows the bank of the Ping, then proceeds to the east direction until Rt. 117. The riverside section passes through the villages and has the problem of the lack of right-of-way width. The sugarcane refinery factory by the road will be one of the matters in widening of the road.

2. TRAFFIC

2-1 Traffic Zone and Road Links

For four alternative routes, the traffic zoning was made as shown in Figure 6-2-1 to Figure 6-2-4.

The area of influence was divided into 4 traffic zones in cases of 6-1 and 6-2, while it was divided into 5 traffic zones in cases of 6-3 and 6-4.

The total population in the said area in 1981 amounts approximately to 39700, 36300, 48800 and 52200 for 6-1, 6-2, 6-3 and 6-4 alternatives, respectively. The densities in terms of population per unit Km of the proposed road length are 1010, 890, 1100 and 1130 for respective alternatives. Annual rate of population increase in the area is 1.0% in the past 3 years, which is lower than the averages of 2.2% in the Northern Region.

As the major destinations of transport demands originated in the area, four Amphoe of Muang Nakhon Sawan, Khanu Woralaksaburi, Banphot Phisai and Kao Lio were chosen based on the O/D survey results. Characteristics of the traffic zones by route alternative are shown in Table 6-2-1, Table 6-2-3, Table 6-2-5 and Table 6-2-7, respectively.

The existing and proposed roads in the area together with surrounding roads concerned were divided into totaling 24 road links, 6 links in the proposed roads and 18 links in the surrounding roads for alternatives 6-1 and 6-2, while 7 links in the proposed route and 17 links in the surrounding roads for alternative 6-3 and 6-4.

The details by alternative are shown in Table 6-2-2, Table 6-2-4, Table 6-2-6 and Table 6-2-8, respectively.

2-2 Transportation Demands

a) Passenger

Passenger transportation demands by O/D pair in the opening year of the project were estimated in both cases of with and without project for each alternative route. The estimated demands in terms of number of trips per day are shown in the following tables:

Passenger O/D (Alternative 6-1) -1987

without project (Trip/Day)									with project (Trip/Day)								
	1	2	3	4	11	12	21	22		1	2	3	4	11	12	21	22
1	0	166	268	132	131	0	0	0	1	0	466	643	282	178	0	0	0
2	0	0	395	146	0	82	197	213	2	0	0	762	471	0	194	471	503
3	0	0	0	687	384	312	467	233	3	0	0	0	950	385	479	818	451
4	0	0	0	0	102	0	1021	171	4	0	0	0	0	157	0	1564	286
11	0	0	0	0	0	54	122	150	11	0	0	0	0	0	74	187	186
12	0	0	0	0	0	0	0	145	12	0	0	0	0	0	0	0	168
21	0	0	0	0	0	0	0	0	21	0	0	0	0	0	0	0	0
22	0	0	0	0	0	0	0	0	22	0	0	0	0	0	0	0	0

Passenger O/D (Alternative 6-2) -1987

without project (Trip/Day)									with project (Trip/Day)								
	1	2	3	4	11	12	21	22		1	2	3	4	11	12	21	22
1	0	157	268	132	146	0	0	0	1	0	284	599	269	180	0	0	0
2	0	0	463	123	0	65	147	181	2	0	0	463	278	0	117	282	294
3	0	0	0	687	351	312	467	233	3	0	0	0	950	377	479	818	430
4	0	0	0	0	129	0	1021	171	4	0	0	0	0	178	0	1564	276
11	0	0	0	0	0	73	173	186	11	0	0	0	0	0	94	247	220
12	0	0	0	0	0	0	0	145	12	0	0	0	0	0	0	0	164
21	0	0	0	0	0	0	0	0	21	0	0	0	0	0	0	0	0
22	0	0	0	0	0	0	0	0	22	0	0	0	0	0	0	0	0

Table 6-2-1 ZONE CHARACTERISTICS (6-1)

Traf. Zone	Relative Administrat. Div.	Tambon Code	% of Popul. in Traf. Zone	Popul. in 1981 (10 ³)	Past Trend of Popul. Increase	Annual Rate of Increase 1981-1987	Projected Population in 1987	Generation Attraction
1	K.P. Khanu	150404(1)	60	5.5	2.0	1.7	11.7	11.7
	Woralaksaburi	150404(2)	40	3.7				
	N.S. Banphot Phisai	010307	15	1.4				
Total			-	10.6				
2	N.S. Banphot Phisai	010307	55	5.2	1.5	1.4	9.7	9.7
		010308	65	3.8				
Total			-	9.0				
3	N.S. Banphot Phisai	010302	88	12.1	0.3	0.7	12.5	91.0
4	N.S. Kad Leio	010401	10	0.5	0.6	0.7	12.1	32.2
		010402	80	4.9				
		010405	76	6.3				
Total			-	11.7				
11	N.S. Banphot Phisai	010307	30	2.8	0.7	0.7	4.9	4.9
		010308	35	2.0				
Total			-	4.8				
12	N.S. M.Nakhon Sawan Kao Lteo	010109	41	2.0	0.6	0.7	9.6	9.6
		010110	50	4.2				
		010401	60	2.9				
Total			-	9.1				
21	N.S.	010000	100	981.4	1.2	1.1	-	1048.0
22	K.P. Khanu Woralaksaburi	150400	100	111.0	2.0	1.7	-	122.4

Table 6-2-2 ROAD LINK CHARACTERISTICS (6-1)

NO	SN	EN	LO	GDD	GDR	LW	GWD	GWR	TD	TW	REMARKS
1	1	41	7.7	8	11	7.7	4	4	11.6	6.6	Rural
2	2	41	7.2	13	15	7.2	4	4	21.5	6.2	Rural
3	2	42	7.3	13	15	6.1	4	4	21.8	5.2	Rural
4	3	42	2.0	8	11	2.0	4	4	3.0	1.7	Rural
5	3	4	12.8	8	11	12.8	4	4	19.2	11.0	Rural
6	4	43	4.8	8	11	3.7	4	4	7.2	3.2	Rural
11	21	53	31.0	1	1	31.0	1	1	23.8	23.8	R.1
12	22	53	16.9	1	1	16.9	1	1	13.0	13.0	R.1
13	43	52	25.0	1	1	27.8	1	1	19.2	21.4	R.117
14	43	56	10.5	1	1	7.7	1	1	8.1	5.9	R.117
15	21	56	9.0	1	1	9.0	1	1	6.9	6.9	R.117
16	1	22	10.5	4	4	10.5	4	4	9.0	9.0	R.1074
17	1	51	23.0	4	4	23.0	4	4	19.7	19.7	R.1074 (OECP)
18	51	52	10.0	4	4	10.0	4	4	8.6	8.6	R.1074
19	53	54	11.9	4	4	11.9	4	4	10.2	10.2	R.1073
20	42	51	24.0	4	4	24.0	4	4	20.6	20.6	R.1073
21	54	55	15.5	4	4	15.5	4	4	13.3	13.3	R.1182 (OECP)
22	21	55	20.0	4	4	20.0	4	4	17.1	17.1	P.1182 (OECP)
23	42	54	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
24	4	55	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
25	11	41	8.0	8	11	8.0	8	11	12.0	12.0	Rural
26	11	42	8.0	8	11	8.0	8	11	12.0	12.0	Rural
27	4	12	5.1	9	12	5.1	9	12	10.3	10.3	Rural
28	12	56	6.1	9	12	6.1	9	12	12.3	12.3	Rural

Note: S_i Start Node, EN End Node, LO Link Length (M), GDD Road Grade in Dry Season (%), GDR Road Grade in Rainy Season (%), LW Link Length (M), GWD Road Grade in Dry Season (%), GWR Road Grade in Rainy Season (%), TD Time (H), TW Time (H)

Figure 6-2-1 ZONING AND ROAD NETWORK (6-1)

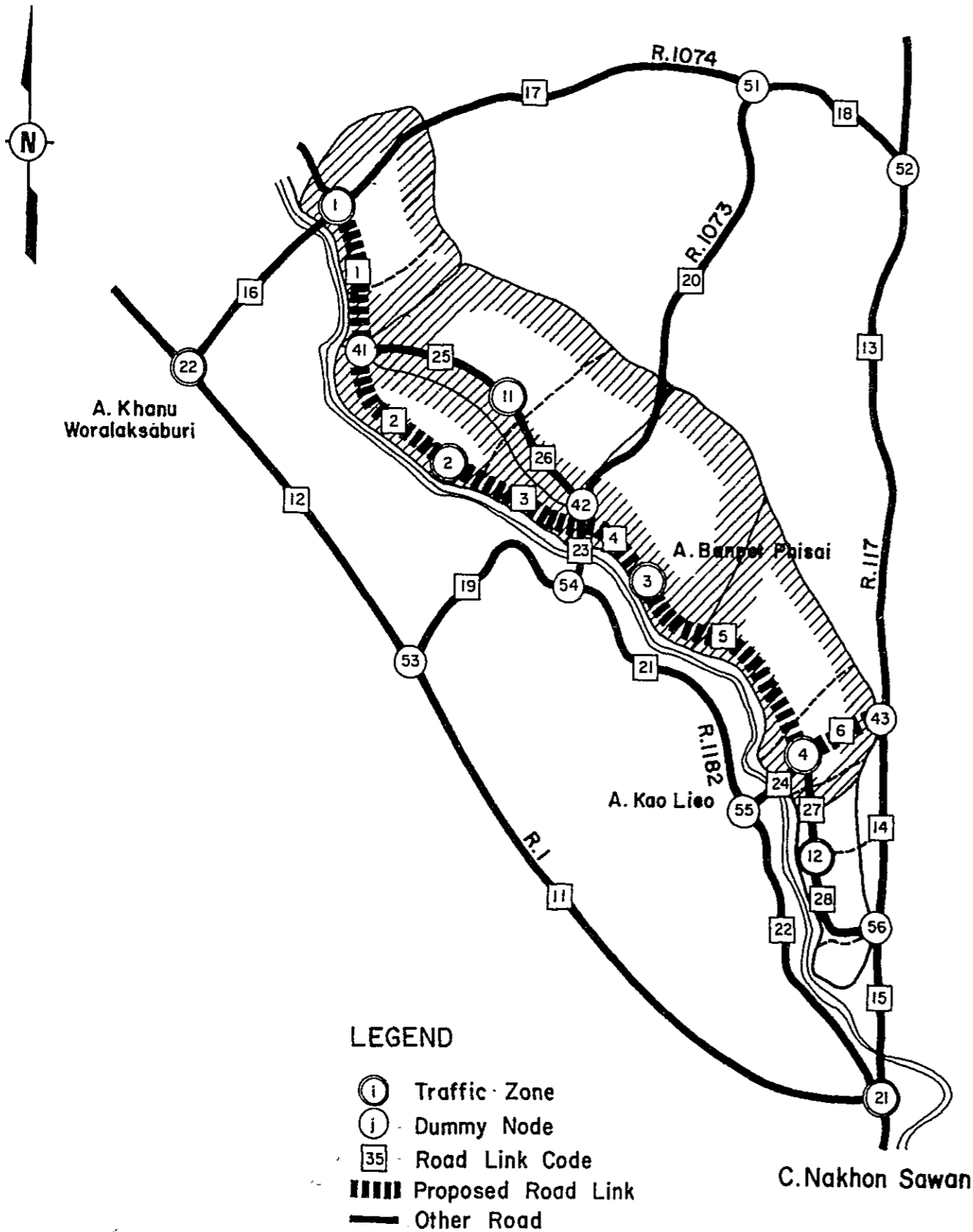


Figure 6-2-2 ZONING AND ROAD NETWORK (6-2)

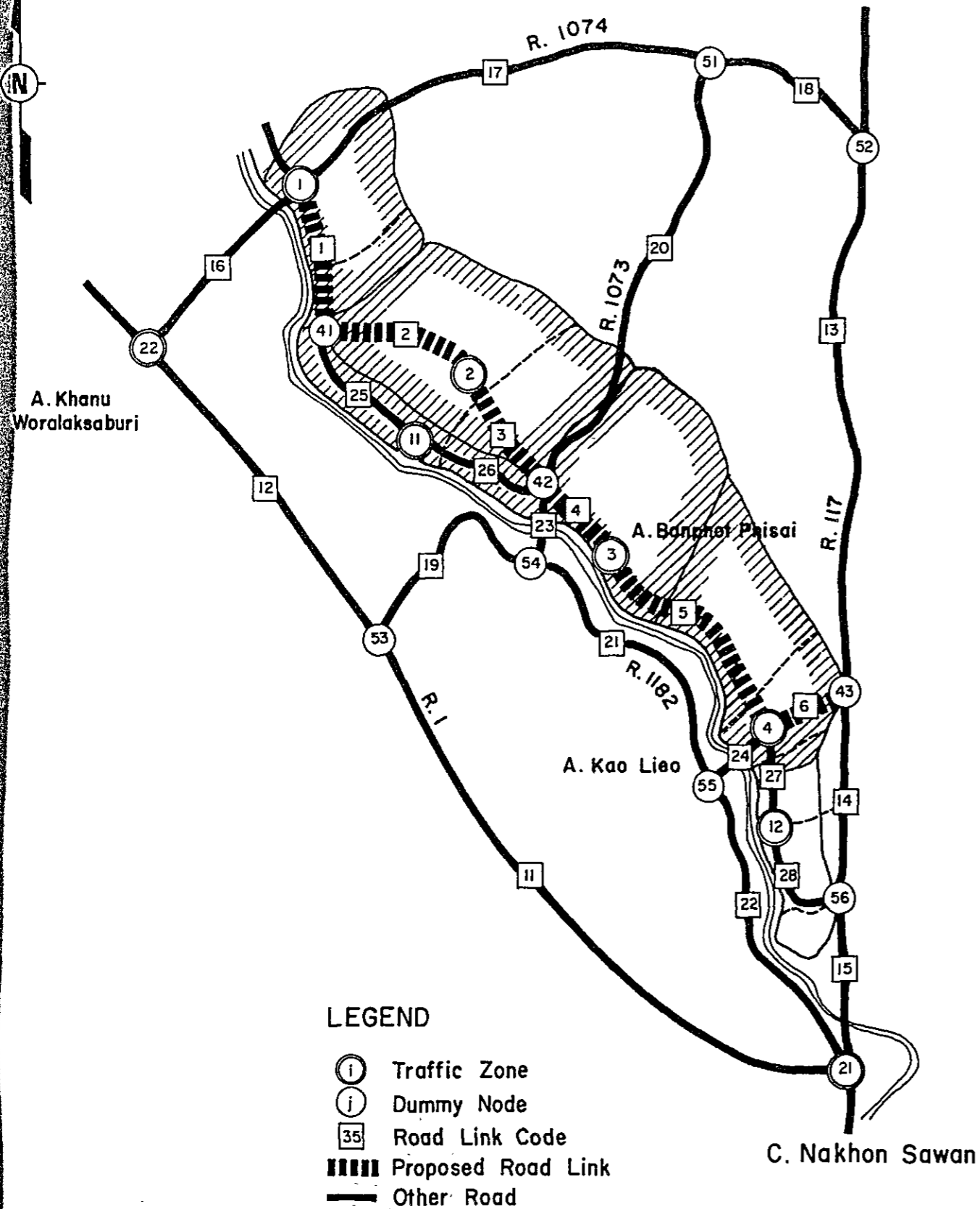


Table 6-2-3 ZONE CHARACTERISTICS (6-2)

Traf. Zone	Relative Administrat. Div.			% of Popul. in Traf. Zone	Popul. in 1981 (10 ³)	Past Trend of Popul. Increase	Annual Rate of Increase 1981-1987	Projected Population in 1987	Generation Attraction
	Changwat	Amphoe	Tambon Code						
1	K.P	Khanu Worala	150404(1)	60	5.5	2.0	1.7	11.7	11.7
			150404(2)	40	3.7				
	N.S	Bonphot Phisai	010307	15	1.4				
Total					10.6				
2		Bonphot Phisai	010307	35	3.3	1.2	1.1	5.9	5.9
	N.S	Phisai	010308	40	2.3				
Total					5.6				
3	N.S	Bonphot Phisai	010302	88	12.1	0.3	0.7	12.5	91.0
4			010401	10	0.5	0.6	0.7	12.1	32.2
	N.S	Kao Liao	010402	80	4.9				
			010405	76	6.3				
Total					11.7				
11		Bonphot Phisai	010307	50	4.7	0.9	0.9	8.5	8.5
	N.S	Phisai	010308	60	3.5				
Total					8.2				
12		M. Nakhon Sawan	010109	41	2.0	0.6	0.7	9.6	9.6
	N.S	Sawan	010110	50	4.2				
		Kao Liao	010401	60	2.9				
Total					9.1				
21	N.S	-	01000	100	981.4	1.2	1.1	-	1048.0
22	N.S	Khanu Worala	15040	100	111.0	2.0	1.7	-	122.4

Table 6-2-4 ROAD LINK CHARACTERISTICS (6-2)

NO	SN	EN	LD	GDD	GDR	LW	GWD	GWR	TD	TW	REMARKS
1	1	41	7.7	8	11	7.7	4	4	11.6	6.6	Rural
2	2	41	8.0	8	11	8.0	4	4	12.0	6.9	Rural
3	2	42	8.0	8	11	8.6	4	4	12.0	5.7	Rural
4	3	42	2.0	8	11	2.0	4	4	3.0	1.7	Rural
5	3	4	12.8	8	11	12.8	4	4	19.2	11.0	Rural
6	4	43	4.8	8	11	3.7	4	4	7.2	3.2	Rural
11	21	53	31.0	1	1	31.0	1	1	23.8	23.8	R.1
12	22	53	16.9	1	1	16.9	1	1	13.0	13.0	R.1
13	43	52	25.0	1	1	27.8	1	1	19.2	21.4	R.117
14	43	56	10.5	1	1	7.7	1	1	9.1	5.9	R.117
15	21	56	9.0	1	1	9.0	1	1	6.9	6.9	R.117
16	1	22	10.5	4	4	10.5	4	4	9.0	9.0	R.1074
17	1	51	23.0	4	4	23.0	4	4	19.7	19.7	R.1074 (OECF)
18	51	52	10.0	4	4	10.0	4	4	8.6	8.6	R.1074
19	53	54	11.9	4	4	11.9	4	4	10.2	10.2	R.1073
20	42	51	24.0	4	4	24.0	4	4	20.6	20.6	R.1073
21	54	55	15.5	4	4	15.5	4	4	13.3	13.3	R.1182 (OECF)
22	21	55	20.0	4	4	20.0	4	4	17.1	17.1	R.1182 (OECF)
23	42	54	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
24	4	55	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
25	11	41	7.2	13	15	7.2	13	15	21.5	21.5	Rural
26	11	42	7.3	13	15	7.3	13	15	21.8	21.8	Rural
27	4	12	5.1	9	12	5.1	9	12	10.3	10.3	Rural
28	12	56	6.1	9	12	6.1	9	12	12.3	12.3	Rural

Note: S1 Start Node, EN End Node, LD Link Length (M), GDD Road Grade in Dry Season (%), GDR Road Grade in Rainy Season (%), LW Link Length (M), GWD Road Grade in Dry Season (%), GWR Road Grade in Rainy Season (%), TD Time (M), TW Time (M)

Table 6-2-5 ZONE CHARACTERISTICS (6-3)

Traf. Zone	Relative Administrat. Div.	Tambon Code	% of Popul. in 1981	Popul. in 1981 (10 ³)	Past Trend of Popul. Increase	Annual Rate of Increase 1981-1987	Projected Population in 1987
	Changwat	Amphoe	Traf. Zone				Generation Attraction
1	K.P. Khanu Worakasaburi	150404(1)	60	5.5	-	1.7	11.7
		150404(2)	40	3.7			
		010307	15	1.4			
		Total	-	10.6	2.0	1.7	11.7
2	N.S. Banphot Phisai	010307	55	5.2	1.5	1.4	9.7
		010308	65	3.8			
		Total	-	9.0	1.5	1.4	9.7
3	N.S. Banphot Phisai	010302	88	12.1	0.3	0.7	12.5
4	N.S.	010401	10	0.5	0.6	0.7	12.1
		010402	80	4.9			
		010405	76	6.3			
		Total	-	11.7	0.6	0.7	32.2
5	N.S.	M.Nakhon Sawan 010109	41	2.0	0.6	0.7	9.6
		010110	50	4.2			
		Kao Liew 010401	60	2.9			
		Total	-	9.1	0.6	0.7	9.6
11	N.S. Banggit Phisai	010307	30	2.8	0.7	0.7	4.9
		010308	35	2.0			
		Total	-	4.8	0.7	0.7	4.9
21	N.S.	010000	100	981.4	1.2	1.1	1048.0
22	K.P. Khanu Worak	150400	100	111.0	2.0	1.7	122.4

Table 6-2-6 ROAD LINK CHARACTERISTICS (6-3)

NO	SN	EN	LO	GDD	GDR	LW	GWD	GWR	TD	TW	REMARKS
1	1	41	7.7	8	11	7.7	4	4	11.6	6.6	Rural
2	2	41	7.2	13	15	7.2	4	4	21.5	6.2	Rural
3	2	42	7.3	13	15	6.1	4	4	21.8	5.2	Rural
4	3	42	2.0	8	11	2.0	4	4	3.0	1.7	Rural
5	3	4	12.8	8	11	12.8	4	4	19.2	11.0	Rural
6	4	5	5.1	9	12	5.7	4	4	10.3	4.9	Rural
7	5	43	4.7	9	12	2.9	4	4	9.5	2.5	Rural
11	21	53	31.0	1	1	31.0	1	1	23.8	23.8	R.1
12	22	53	16.9	1	1	16.9	1	1	13.0	13.0	R.1
13	52	56	25.0	1	1	25.0	1	1	19.2	19.2	R.117
14	43	56	7.3	1	1	8.3	1	1	5.6	6.4	R.117
15	21	43	12.2	1	1	11.2	1	1	9.4	8.6	R.117
16	1	22	10.5	4	4	10.5	4	4	9.0	9.0	R.1074
17	1	51	23.0	4	4	23.0	4	4	19.7	19.7	R.1074 (OECl)
18	51	52	10.0	4	4	10.0	4	4	8.6	8.6	R.1074
19	53	54	11.9	4	4	11.9	4	4	10.2	10.2	R.1073
20	42	51	24.0	4	4	24.0	4	4	20.6	20.6	R.1073
21	54	55	15.5	4	4	15.5	4	4	13.3	13.3	R.1184
22	21	55	20.0	4	4	20.0	4	4	17.1	17.1	R.1184
23	42	54	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
24	4	55	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
25	11	41	8.0	8	11	8.0	8	11	12.0	12.0	Rural
26	11	42	8.0	8	11	8.0	8	11	12.0	12.0	Rural
27	4	56	4.8	8	11	4.8	8	11	7.2	7.2	Rural

Note: SN: Start Node, EN: End Node, LO: Link Length (M), GDD: Road Grade in Dry Season (%), GDR: Road Grade in Rainy Season (%), LW: Link Length (M), GWD: Road Grade in Dry Season (%), GWR: Road Grade in Rainy Season (%), TD: Time (M), TW: Time (M)

Figure 6-2-3 ZONING AND ROAD NETWORK (6-3)

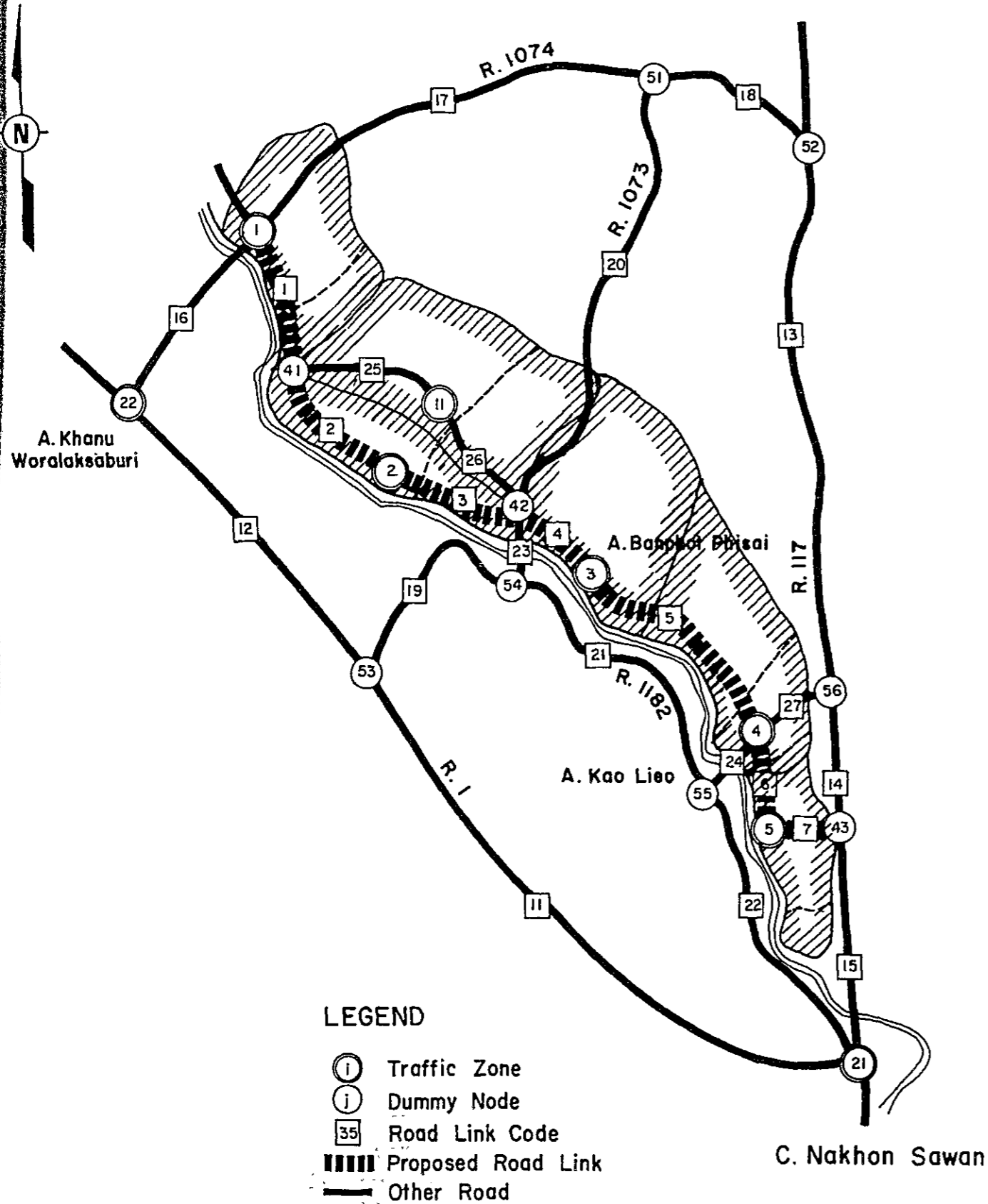


Table 6-2-7 ZONE CHARACTERISTICS (6-4)

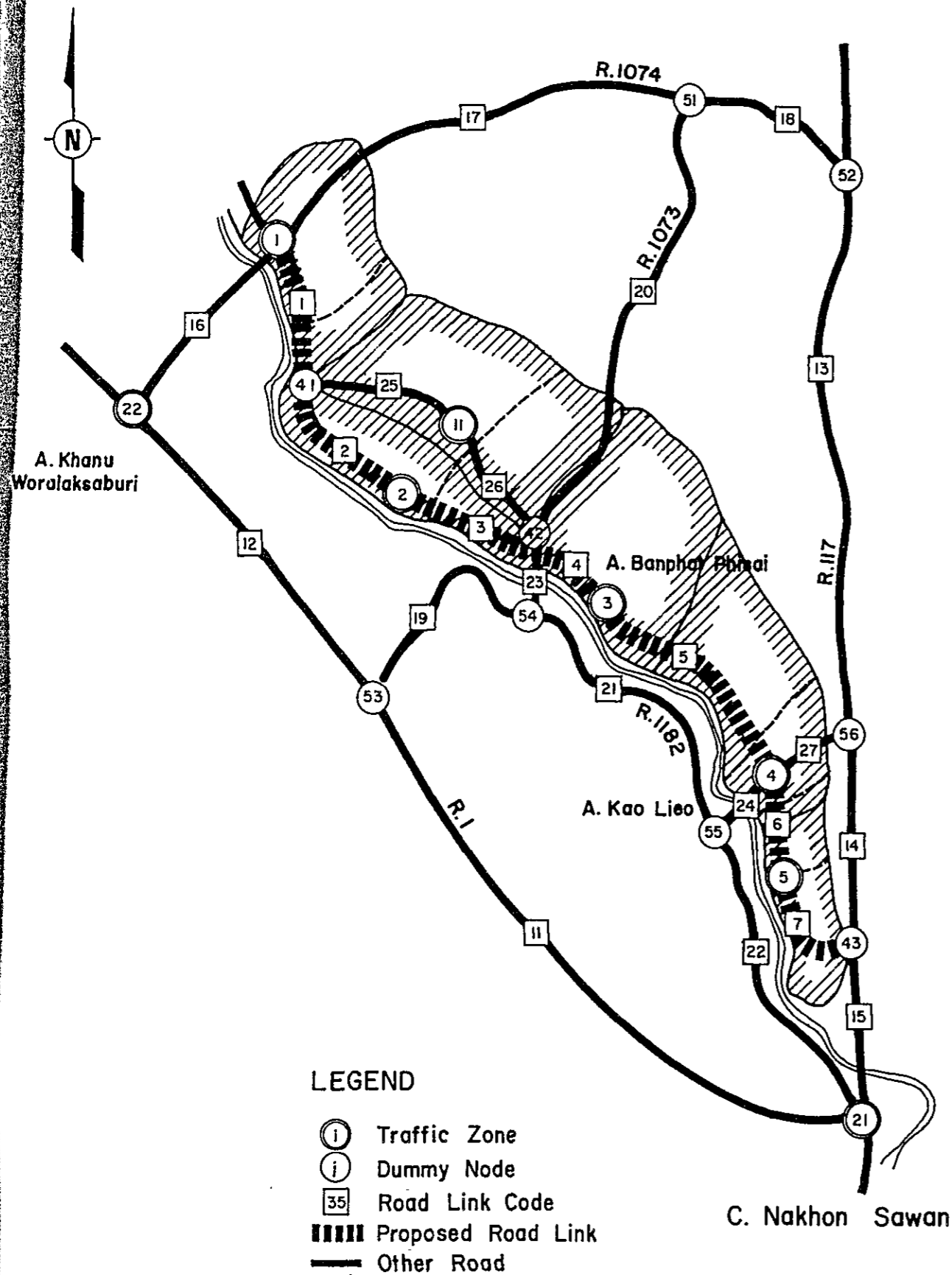
Traf. Zone	Relative Administrat. Div.	Tambon Code	% of Popul. in Traf. Zone	Popul. in 1981 (10 ³)	Past Trend of Popul. Increase	Annual Rate of Increase 1981-1987	Projected Population in 1987	Projected Population in 1987 Generation Attraction
1	K.P. Khanu	150404(1)	60	5.5	-	-	-	-
	Woraklaksaburi	150404(2)	40	3.7				
	N.S. Banphot Phisai	010307	15	1.4				
		Total	-	10.6	2.0	1.7	11.7	11.7
2	N.S. Banphot Phisai	010307	55	5.2	-	-	-	-
		010308	65	3.8				
		Total	-	9.0	1.5	1.4	9.7	9.7
3	N.S. Banphot Phisai	010302	83	12.1	0.3	0.7	12.5	91.0
4	N.S. Kao Liew	010401	10	0.5	-	-	-	-
		010402	60	4.9				
		010405	76	6.3				
		Total	-	11.7	0.6	0.7	12.1	32.2
5	N.S. Kao Liew	H.Nakhon Sawan 010109	60	2.9	-	-	-	-
		010110	80	6.7				
		010401	60	2.9				
		Total	-	12.5	0.6	0.7	13.1	13.1
11	N.S. Banphot Phisai	010307	30	2.8	-	-	-	-
		010308	35	2.0				
		Total	-	4.8	0.7	0.7	4.9	4.9
21	N.S. Nakhon Sawan	010000	100	981.4	1.2	1.1	-	1048.0
22	K.P. Khanu Woraklaksaburi	150400	100	111.0	2.0	1.7	-	122.4

Table 6-2-8 ROAD LINK CHARACTERISTICS (6-4)

NO	SN	EN	LO	GDC	GDR	LW	GWD	GWR	TO	TW	REMARKS
1	1	41	7.7	8	11	7.7	4	4	11.6	6.6	Rural
2	2	41	7.2	13	15	7.2	4	4	21.5	6.2	Rural
3	2	42	7.3	13	15	6.1	4	4	21.8	5.2	Rural
4	3	42	2.0	8	11	2.0	4	4	3.0	1.7	Rural
5	3	4	12.8	8	11	12.8	4	4	19.2	11.0	Rural
6	4	5	5.1	9	12	5.7	4	4	10.3	4.9	Rural
7	5	43	6.1	9	12	4.5	4	4	12.3	3.9	Rural
11	21	53	31.0	1	1	31.0	1	1	23.8	23.8	R.1
12	22	53	16.9	1	1	16.9	1	1	13.0	13.0	R.1
13	52	56	25.0	1	1	25.0	1	1	19.2	19.2	R.117
14	43	58	10.5	1	1	11.5	1	1	8.1	8.8	R.117
15	21	43	9.0	1	1	8.0	1	1	6.9	6.2	R.117
16	1	22	10.5	4	4	10.5	4	4	9.0	9.0	R.1074
17	1	51	23.0	4	4	23.0	4	4	19.7	19.7	R.1074 (OEFC)
18	51	52	10.0	4	4	10.0	4	4	8.6	8.6	R.1074
19	53	54	11.9	4	4	11.9	4	4	10.2	10.2	R.1073
20	42	51	24.0	4	4	24.0	4	4	20.6	20.6	R.1073
21	54	55	15.5	4	4	15.5	4	4	13.3	13.3	R.1182 (OEFC)
22	21	55	20.0	4	4	20.0	4	4	17.1	17.1	R.1182 (OEFC)
23	42	54	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
24	4	55	5.5	16	16	5.5	16	16	30.0	30.0	Ferry
25	11	41	8.0	8	11	8.0	8	11	12.0	12.0	Rural
26	11	42	8.0	8	11	8.0	8	11	12.0	12.0	Rural
27	4	56	4.8	8	11	4.8	8	11	7.2	7.2	Rural

Note S1 Start Node, EN End Node, LO Link Length (M), GDC Road Grade in Dry Season (M), GDR Road Grade in Rainy Season (M), LW Link Length (M), GWD Road Grade in Dry Season (M), GWR Road Grade in Rainy Season (M), TO Time (M), TW Time (M)

Figure 6-2-4 ZONING AND ROAD NETWORK (6-4)



Passenger O/D (Alternative 6-3)-1987

without project (Trip/Day)									with project (Trip/Day)								
	1	2	3	4	5	11	21	22		1	2	3	4	5	11	21	22
1	0	166	268	132	0	131	0	0	1	0	466	643	282	0	178	0	0
2	0	0	395	146	82	0	197	213	2	0	0	762	471	256	0	471	503
3	0	0	0	687	312	384	467	233	3	0	0	0	950	701	385	819	451
4	0	0	0	0	588	102	1021	171	4	0	0	0	0	588	157	1568	286
5	0	0	0	0	0	54	1004	146	5	0	0	0	0	0	93	1752	195
11	0	0	0	0	0	0	122	150	11	0	0	0	0	0	0	188	188
21	0	0	0	0	0	0	0	0	21	0	0	0	0	0	0	0	0
22	0	0	0	0	0	0	0	0	22	0	0	0	0	0	0	0	0

Passenger O/D (Alternative 6-4)-1987

without project (Trip/Day)									with project (Trip/Day)								
	1	2	3	4	5	11	21	22		1	2	3	4	5	11	21	22
1	0	166	268	132	0	131	383	0	1	0	466	643	282	0	178	386	0
2	0	0	395	146	89	0	197	213	2	0	0	762	471	277	0	492	503
3	0	0	0	687	428	384	467	233	3	0	0	0	950	957	385	864	451
4	0	0	0	0	802	102	1021	171	4	0	0	0	0	802	157	1704	286
5	0	0	0	0	0	59	1337	198	5	0	0	0	0	0	100	1845	267
11	0	0	0	0	0	0	122	150	11	0	0	0	0	0	0	194	188
21	0	0	0	0	0	0	0	0	21	0	0	0	0	0	0	0	0
22	0	0	0	0	0	0	0	0	22	0	0	0	0	0	0	0	0

b) Agricultural Freight

According to the agro-economic survey, the major products of sugarcane in the area are mostly carried to the refinery factory located at 7 km south of Amphoe Kao Liao, in traffic zone 12 for alternative 6-1 and 6-2 and 5 for alternative 6-3 and 6-4. The another major assembly market is Nakhon Sawan. The estimated agricultural freight O/D volumes, in a unit of 1,000 tons per year, for each alternatives are shown in the following tables:

Agri. Freight O/D (Alternative 6-1)-1987

without project (1000 ton/year)									with project (1000 ton/year)								
	1	2	3	4	11	21	22			1	2	3	4	11	21	22	
1	0.0	0.0	2.0	0.0	0.0	0.0	2.9	0.0	1	0.0	0.0	2.0	0.0	0.0	0.0	2.9	0.0
2	0.0	0.0	3.6	0.0	0.0	2.0	2.4	0.0	2	0.0	0.0	3.6	0.0	0.0	2.0	2.4	0.0
3	0.0	0.0	0.0	0.0	0.0	74.7	22.9	0.0	3	0.0	0.0	0.0	0.0	0.0	74.7	22.9	0.0
4	0.0	0.0	0.0	0.0	0.0	29.9	3.1	0.0	4	0.0	0.0	0.0	0.0	0.0	29.9	3.1	0.0
11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	12	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Agri. Freight O/D (Alternative 6-2)-1987

without project (1000 ton/year)									with project (1000 ton/year)								
	1	2	3	4	11	21	22			1	2	3	4	11	21	22	
1	0.0	0.0	2.2	0.0	0.0	3.9	3.3	0.0	1	0.0	0.0	2.2	0.0	0.0	3.9	3.3	0.0
2	0.0	0.0	11.1	0.0	0.0	10.5	9.9	0.0	2	0.0	0.0	11.1	0.0	0.0	10.5	9.9	0.0
3	0.0	0.0	0.0	0.0	0.0	74.7	22.9	0.0	3	0.0	0.0	0.0	0.0	0.0	74.7	22.9	0.0
4	0.0	0.0	0.0	0.0	0.0	29.9	3.1	0.0	4	0.0	0.0	0.0	0.0	0.0	29.9	3.1	0.0
11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	12	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Agri. Freight O/D (Alternative 6-3)-1987

without project (1000 ton/year)									with project (1000 ton/year)								
	1	2	3	4	5	11	21	22		1	2	3	4	5	11	21	22
1	0.0	0.0	2.0	0.0	0.0	0.0	2.9	0.0	1	0.0	0.0	2.0	0.0	0.0	0.0	2.9	0.0
2	0.0	0.0	3.6	0.0	2.0	0.0	2.4	0.0	2	0.0	0.0	3.6	0.0	2.0	0.0	2.4	0.0
3	0.0	0.0	0.0	0.0	59.7	0.0	21.8	0.0	3	0.0	0.0	0.0	0.0	59.7	0.0	21.8	0.0
4	0.0	0.0	0.0	0.0	23.5	0.0	2.1	0.0	4	0.0	0.0	0.0	0.0	23.5	0.0	2.1	0.0
5	0.0	0.0	0.0	0.0	0.0	0.0	20.8	0.0	5	0.0	0.0	0.0	0.0	0.0	0.0	20.8	0.0
11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Agri. Freight O/D (Alternative 6-4)-1987

without project (1000 ton/year)									without project (1000 ton/year)								
	1	2	3	4	5	11	21	22		1	2	3	4	5	11	21	22
1	0.0	0.0	2.0	0.0	0.0	0.0	2.9	0.0	1	0.0	0.0	2.0	0.0	0.0	0.0	2.9	0.0
2	0.0	0.0	3.6	0.0	2.0	0.0	2.4	0.0	2	0.0	0.0	3.6	0.0	2.0	0.0	2.4	0.0
3	0.0	0.0	0.0	0.0	59.7	0.0	21.8	0.0	3	0.0	0.0	0.0	0.0	59.7	0.0	21.8	0.0
4	0.0	0.0	0.0	0.0	23.5	0.0	2.1	0.0	4	0.0	0.0	0.0	0.0	23.5	0.0	2.1	0.0
5	0.0	0.0	0.0	0.0	0.0	0.0	25.7	0.0	5	0.0	0.0	0.0	0.0	0.0	0.0	25.7	0.0
11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

c) Non-agricultural Freight

The non-agricultural freight transportation demands are estimated based on the model described in 3-3-3 in the Summary Report. Their movements on each road link were obtained relating with the passenger movements which were derived from the assignment of the passenger O/D volumes shown in the above a).

2-3 Traffic Composition, Occupancy and Loading Ratio

a) Traffic Composition

In accordance with the examination of the classified traffic counts in the Phase I and II studies and DOH's traffic records, the traffic composition on the existing roads of the project area was estimated as follows:

Existing Traffic Composition

Survey Points and Source	Passenger Traffic						Freight Traffic				
	P/C	P/P	L/B	M/B	H/B	Total	P/T	4/T	6/T	10/T	Total
OD-2(PhaseI)	.17	.00	.45	.38	.00	1.00	.35	.12	.13	.40	1.00
M-12(PhaseI)	.21	.34	.42	.03		1.00	.54	.21	.25		1.00
NO.1(PhaseII)	.19	.37	.13	.07	.24	1.00	.54	.03	.25	.19	1.00
R. 1182(DOH)	.30	.17	.33	.20		1.00	.40	.33	.27		1.00
R. 1073(DOH)	.50	.05	.36	.09		1.00	.41	.55	.04		1.00
R. 1074(DOH)	.36	.14	.41	.09		1.00	.61	.27	.12		1.00
Estimated	.21	.24	.31	.15	.09	1.00	.37	.11	.28	.24	1.00

Changes in traffic composition due to income growth and road surface condition were determined for the both cases of with and without projects as shown in the following tables:

Freight Traffic Composition

Year	Without Project					With Project				
	P/C	P/P	L/B	M/B	H/B	P/C	P/P	L/B	M/B	H/B
1981	.21	.24	.31	.15	.09	.21	.24	.31	.15	.09
1987	.23	.26	.29	.14	.08	.26	.24	.27	.12	.11
1993	.25	.29	.26	.13	.07	.30	.24	.24	.09	.13
2001	.28	.32	.23	.11	.06	.36	.24	.19	.05	.16

Passenger Traffic Composition

Year	Without Project				With Project			
	P/T	4/T	6/T	10/T	P/T	4/T	6/T	10/T
1981-2001	.37	.11	.28	.24	.33	.02	.35	.25

b) Occupancy

Occupancy by vehicle type and the average were determined as follows:

<u>Occupancy</u>	
Vehicle Type	Person per Vehicle
P/C	3.1
P/P	4.4
L/B	10.9
M/B	16.2
H/B	38.3
Ave. (1993, \bar{w})	9.7
(1993, w)	11.0

c) Loading Ratio

Loading ratio by vehicle type and the average were determined as follows:

<u>Loading Ratio</u>			
Vehicle Type	Ave. Load of Loaded Truck	Rate of Loaded Trucks	Loading Ratio (ton)
P/T	0.65	.45	0.3
4/T	2.0	.50	1.0
6/T	4.1	.55	2.3
10/T	12.6	.60	7.6
Ave. (\bar{w})	-	-	2.7
(w)	-	-	2.8

2-4 Growth Rates of Transportation Demands

The growth rates of passenger, agricultural freight and non-agricultural freight transport demands for the periods of 1987-1993 and 1993-2001 were projected. The basis for the estimation of growth rate for passenger, and the projected rates are shown in the following tables:

The Basis for Estimation of Passenger Demands Growth

Indicator	<u>Annual Growth Rate (%)</u>		Elasticity
	1987 - 1993	1993 - 2001	
Per capita Income	5.8	5.6	1.08
Transportation price	3.6	3.6	-0.24
Population	1.0	1.1	1.00

Growth Rate of Transportation Demands

Type of Demand	<u>Annual Growth Rate (%)</u>		Index 1987=100	
	1987 - 1993	1993 - 2001	1993	2001
Passenger	6.4	6.3	145	237
Agri. Freight	0.1	0.1	100.6	101.4
Non-Agri. Freight	7.6	7.5	155	277

2-5 Forecasted Traffic

a) Forecasted Traffic by Vehicle Type

The forecasted traffic by route alternative is summarized in the following table:

Forecasted Traffic

Alter- tive	Year	P/C	L/B	M/B	H/B	P/P P/T	4/T	6/T	10/T	ADT	M/C
6-1	1987	87	91	40	37	132	3	48	34	471	552
	1993	146	117	44	63	189	4	67	48	677	686
	2001	281	148	39	125	306	6	109	78	1092	831
6-2	1987	75	78	35	32	120	3	47	33	423	492
	1993	126	101	38	55	169	4	63	45	599	611
	2001	242	128	34	108	269	6	98	70	955	746
6-3	1987	96	100	44	41	147	3	54	38	523	598
	1993	161	129	48	70	210	4	75	54	751	740
	2001	309	163	43	138	340	7	123	88	1211	889
6-4	1987	113	118	52	48	173	4	63	45	617	682
	1993	189	151	57	82	249	5	90	64	887	840
	2001	365	193	51	162	404	8	148	105	1435	995

b) Forecasted Traffic by Road Link

Details of the forecasted traffic by road link by traffic type for route alternatives are shown in Table 6-2-9, taking a sample of the case of 1993.

Table 6-2-9 FORECASTED TRAFFIC BY ROAD LINK

TRAFFIC VOLUME ON ROUTE 6-1 (1993)

LINK	1	2	3	4	5	6	AVR.
N+D	64	53	70	96	104	71	78
P/C	I	62	58	78	93	76	49
DV	0	0	0	0	0	0	0
TOTAL	125	111	147	179	179	120	146
N+D	51	42	56	77	83	57	62
L/B	I	49	47	62	66	61	39
DV	0	0	0	0	0	0	0
TOTAL	100	89	118	143	144	96	117
N+D	19	16	21	29	31	21	23
M/B	I	18	17	23	23	15	20
DV	0	0	0	0	0	0	0
TOTAL	38	33	44	54	54	36	44
N+D	28	23	30	42	45	31	34
H/B	I	27	25	34	36	21	29
DV	0	0	0	0	0	0	0
TOTAL	54	48	64	78	78	52	63
N+D	73	60	84	116	159	93	104
P/P&T	I	76	71	97	105	60	85
DV	0	0	0	0	0	0	0
TOTAL	149	131	180	221	256	153	189
N+D	1	1	1	2	4	2	2
A/T	I	1	1	2	2	1	2
DV	0	0	0	0	0	0	0
TOTAL	3	2	3	4	6	3	4
N+D	21	16	26	36	70	33	39
S/T	I	25	23	32	36	20	28
DV	0	0	0	0	0	0	0
TOTAL	45	39	57	72	103	52	67
N+D	15	12	18	26	50	23	28
IO/T	I	18	16	23	25	14	20
DV	0	0	0	0	0	0	0
TOTAL	32	28	41	51	74	37	48
N+D	271	222	306	427	546	331	370
ADT	I	275	259	349	379	218	307
DV	0	0	0	0	0	0	0
TOTAL	546	481	656	802	897	549	677
N+D	348	296	379	492	537	390	416
M/C	I	259	254	315	318	277	201
DV	0	0	0	0	0	0	0
TOTAL	607	550	694	809	815	592	686
N+D	519	518	685	915	1084	722	786
TOTAL	I	535	512	664	696	624	420
DV	0	0	0	0	0	0	0
TOTAL	1153	1031	1349	1611	1709	1141	1263

TRAFFIC VOLUME ON ROUTE 6-2 (1993)

LINK	1	2	3	4	5	6	AVR.
N+D	54	51	69	98	104	72	77
P/C	I	43	41	45	51	60	44
DV	0	0	0	0	0	0	0
TOTAL	107	92	114	149	164	115	126
N+D	51	41	55	78	83	57	62
L/B	I	35	33	36	41	48	35
DV	0	0	0	0	0	0	0
TOTAL	86	73	91	119	131	92	101
N+D	19	15	21	29	21	21	23
M/B	I	13	12	13	15	13	15
DV	0	0	0	0	0	0	0
TOTAL	32	27	34	45	49	35	38
N+D	28	22	30	42	45	31	33
H/B	I	19	18	19	22	19	21
DV	0	0	0	0	0	0	0
TOTAL	47	40	49	65	71	50	55
N+D	75	60	93	128	167	95	108
P/P&T	I	53	49	55	65	76	54
DV	0	0	0	0	0	0	0
TOTAL	128	109	148	193	243	149	169
N+D	1	1	2	3	4	2	2
A/T	I	1	1	1	1	1	1
DV	0	0	0	0	0	0	0
TOTAL	2	2	3	4	6	3	4
N+D	22	17	35	46	77	35	43
S/T	I	17	15	18	22	17	20
DV	0	0	0	0	0	0	0
TOTAL	39	33	53	68	103	53	63
N+D	16	12	25	33	55	25	31
IO/T	I	12	11	13	15	12	14
DV	0	0	0	0	0	0	0
TOTAL	28	23	38	48	74	38	45
N+D	277	219	330	458	567	339	380
ADT	I	193	179	199	233	275	219
DV	0	0	0	0	0	0	0
TOTAL	469	398	529	691	842	534	599
N+D	351	290	382	504	541	393	415
M/C	I	186	182	185	199	221	180
DV	0	0	0	0	0	0	0
TOTAL	537	471	568	703	763	573	611
N+D	628	509	712	962	1109	731	795
TOTAL	I	379	361	385	432	496	375
DV	0	0	0	0	0	0	0
TOTAL	1006	870	1097	1393	1605	1107	1210

TRAFFIC VOLUME ON ROUTE 6-3 (1993)

LINK	1	2	3	4	5	6	7	AVR.
N+D	64	53	70	96	104	118	111	86
P/C	I	63	59	81	87	89	75	63
DV	0	0	0	0	0	0	0	0
TOTAL	126	112	151	183	193	193	174	161
N+D	51	42	56	77	83	95	99	69
L/B	I	50	47	65	70	71	60	50
DV	0	0	0	0	0	0	0	0
TOTAL	101	89	121	147	154	154	139	129
N+D	19	16	21	29	31	35	33	26
M/B	I	19	18	24	26	27	22	19
DV	0	0	0	0	0	0	0	0
TOTAL	38	34	45	55	58	58	52	48
N+D	28	23	30	42	45	51	48	37
H/B	I	27	26	35	38	39	32	27
DV	0	0	0	0	0	0	0	0
TOTAL	55	48	65	80	83	94	75	70
N+D	73	60	84	116	153	181	148	116
P/P&T	I	77	72	101	111	113	95	80
DV	0	0	0	0	0	0	0	0
TOTAL	150	132	185	227	266	277	228	210
N+D	1	1	1	2	4	5	3	3
A/T	I	1	1	2	2	2	2	2
DV	0	0	0	0	0	0	0	0
TOTAL	3	2	3	4	6	6	5	4
N+D	21	17	26	36	65	80	55	44
S/T	I	25	23	33	38	33	27	31
DV	0	0	0	0	0	0	0	0
TOTAL	46	40	59	73	104	113	82	75
N+D	15	12	18	26	46	57	39	31
IO/T	I	18	16	24	27	28	23	19
DV	0	0	0	0	0	0	0	0
TOTAL	33	28	42	52	74	80	58	54
N+D	271	222	307	423	531	622	527	411
ADT	I	280	263	355	398	407	343	287
DV	0	0	0	0	0	0	0	0
TOTAL	551	486	672	821	939	965	813	751
N+D	348	295	379	492	535	595	559	450
M/C	I	263	258	328	332	324	266	234
DV	0	0	0	0	0	0	0	0
TOTAL	611	554	707	824	859	861	793	740
N+D	619	519	686	915	1066	1217	1036	861
TOTAL	I	543	521	693	731	731	608	520
DV	0	0	0	0	0	0	0	0
TOTAL	1162	1040	1378	1646	1797	1825	1606	1491

TRAFFIC VOLUME ON ROUTE 6-4 (1993)

LINK	1	2	3	4	5	6	7	AVR.
N+D	91	70	97	114	126	149	140	109
P/C	I	63	60	83	90	99	90	77
DV	0	0	0	0	0	0	0	0
TOTAL	144	130	171	204	225	239	217	189
N+D	55	56	70	91	101	119	112	86
L/B	I	51	48	67	72	79	72	62
DV	0	0	0	0	0	0	0	0
TOTAL	115	104	137	163	180	191	174	151
N+D	24	21	26	34	38	45	42	32
M/B	I	19	18	25	27	30	27	24
DV	0	0	0	0	0	0	0	0
TOTAL	43	39	51	61	67	72	65	57
N+D	35	30	38	49	54	65	60	47
H/B	I	27	26	36	39	43	39	34
DV	0	0	0	0	0	0	0	0
TOTAL	63	56	74	88	97	104	94	82
N+D	95	82	106	139	182	221	187	145
P/P&T	I	79	74	105	115	128	117	100
DV	0	0	0	0	0	0	0	0
TOTAL	174	156	211	254	310	339	287	249
N+D	2	1	2	3	4	5	4	3
A/T	I	1	1	2	2	2	2	2
DV	0	0	0	0	0	0	0	0
TOTAL	3	3	4	5	7	8	6	5
N+D	28	24	33	44	75	94	69	54
S/T	I	26	24	35	40	44	41	35
DV	0	0	0	0	0	0	0	0
TOTAL	54	48	69	84	120	135	104	90
N+D	20	17	24	31				

Figure 6-3-1 TYPICAL CROPPING CALENDAR - Route 6

3. AGRICULTURAL DEVELOPMENT

3-1 Crop Production

Sugar cane is the most predominant product in the area of influence. A big sugar refinery plant 3,000 ton per day, exists 7 km south of Amphoe Kao Liao and collects about 350,000 tons of sugar cane annually from the surrounding sugar cane fields in Changwat Nakhon Sawan, Kamphaeng Phet and Uthai Thani. The factory still has enough capacity to absorb future increase of cane production in the area.

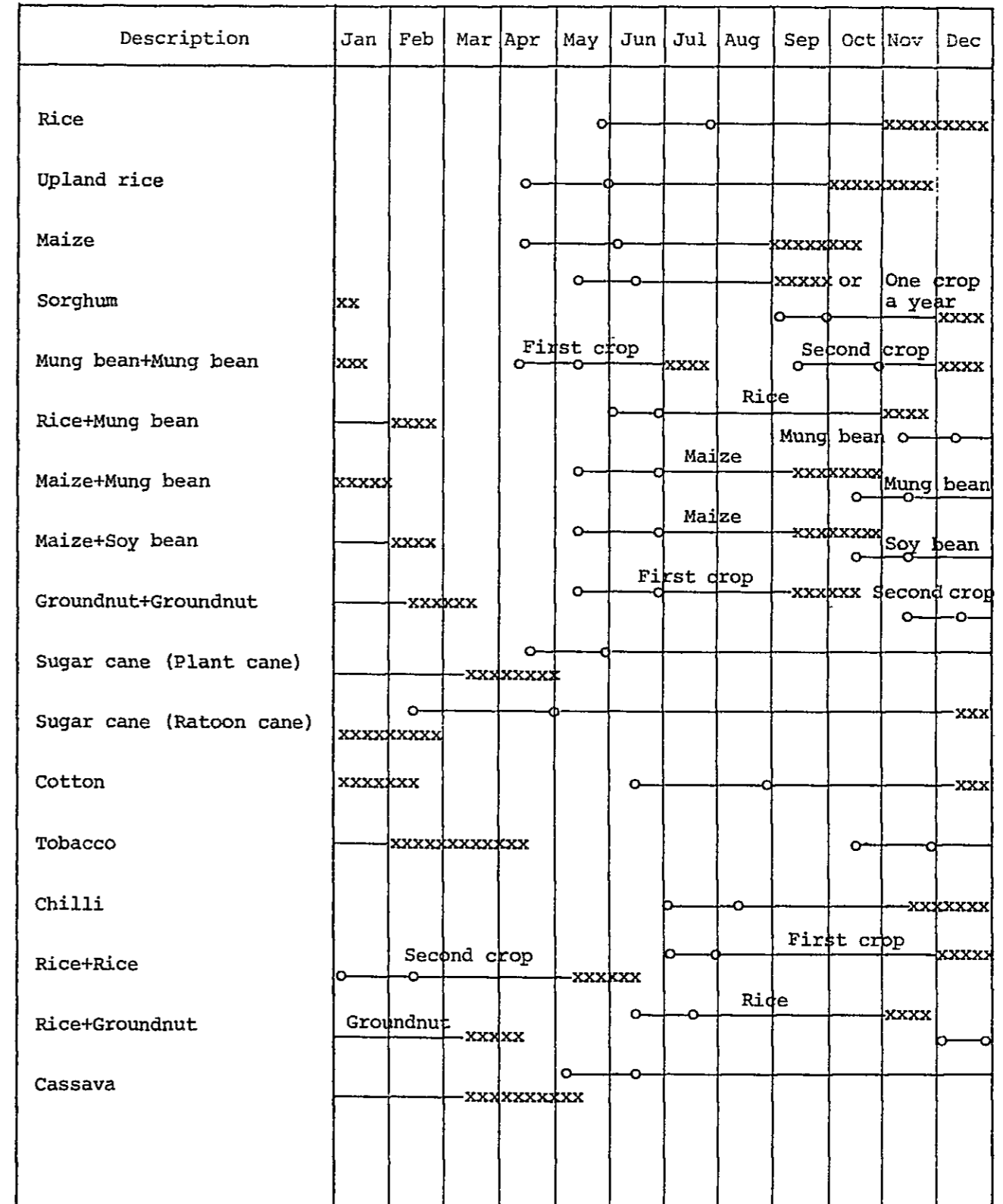
Second major products is paddy. Other crops share only 10% of the total production in the area.

Land use and capability in the area of influence is illustrated in Figure 6-3-2. Typical cropping calendar in Nakhon Sawan area is also shown in Figure 6-3-1. Based on the estimated planted area and yields, the future crop production in the area of influence after opening of the proposed road is given in the following Table 6-3-1 to 6-3-4.

3-2 Net Value Added

In accordance with the concept discussed in Chapter 4 of Summary Report, net value added was calculated for both cases, with project and without project. The agricultural development benefits, indicated by the increment of net value added of crop production in the with project case, attributable to the project are estimated as follows:

Agricultural Benefits (Million Baht)			
Alternative	1987	1993	2001
6-1	7.47	8.60	10.11
6-2	10.49	12.05	14.13
6-3	7.91	9.12	10.76
6-4	8.44	9.77	11.56



NOTE: o ———— xxxxxxxxxxxxxxxx
 Sawing Season Growing Season Harvesting Season

Figure 6-3-2 LAND USE AND CAPABILITY
OF INFLUENCE AREA
STUDY ROUTE NO. 6

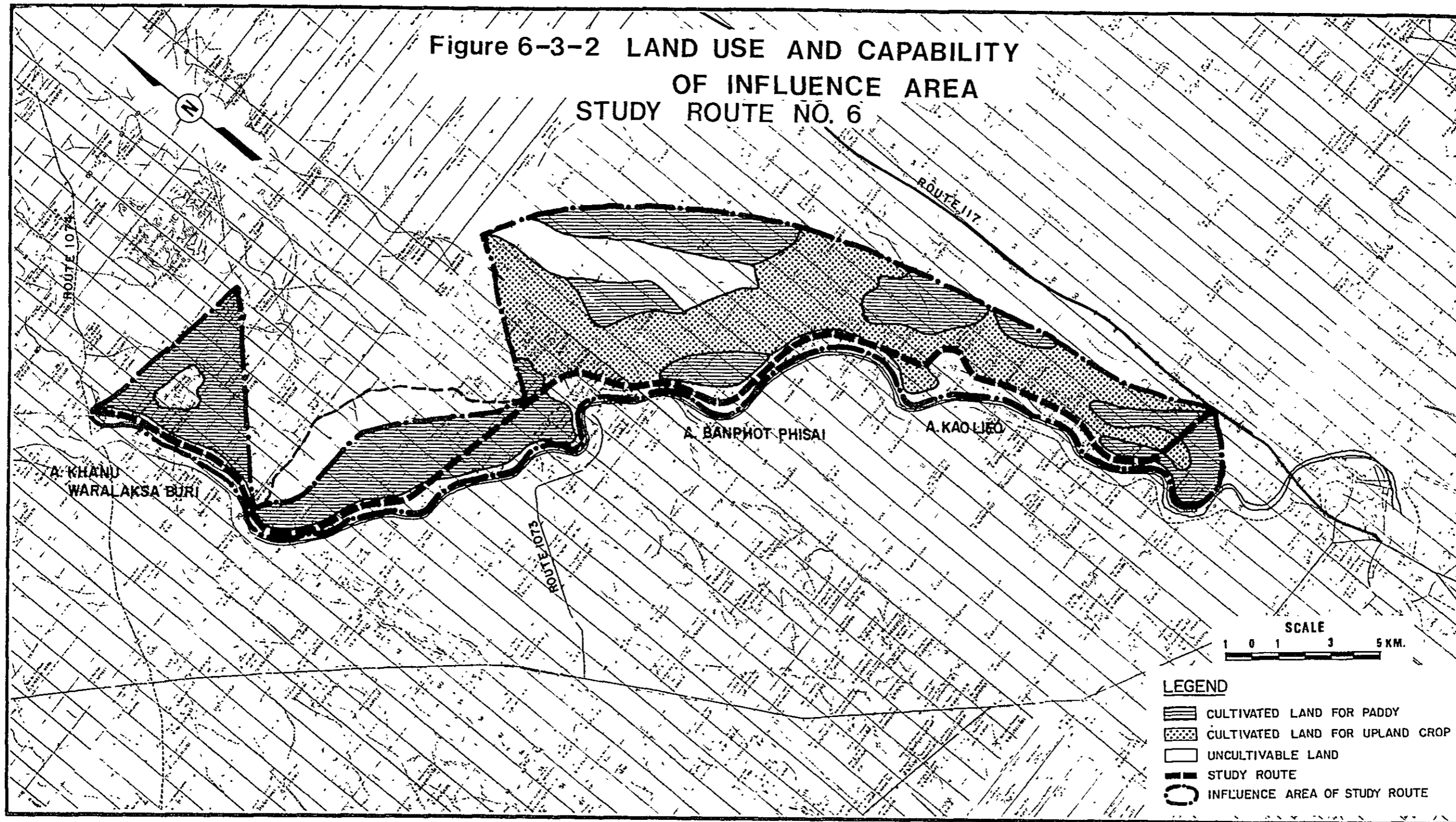


Table 6-3-1 CROP PRODUCTION - Route 6-1

CROP	(1000 TON)					
	1987		1993		2001	
	W/O	W	W/O	W	W/O	W
PADDY	29.9	29.9	30.0	30.5	30.3	31.3
MAIZE	1.2	1.2	1.2	1.2	1.2	1.2
MUNG BEAN	1.9	1.9	1.9	1.9	1.9	2.0
SOY BEAN	0.0	0.0	0.0	0.0	0.0	0.0
GROUND NUTS	0.1	0.1	0.1	0.1	0.1	0.1
SORGHUM	0.1	0.1	0.1	0.1	0.1	0.1
CASSAVA	0.1	0.1	0.1	0.1	0.1	0.1
SUGAR CANE	105.9	105.9	105.9	106.0	105.9	106.1
TOBACCO	0.0	0.0	0.0	0.0	0.0	0.0
COTTON	0.0	0.0	0.0	0.0	0.0	0.0
GARLIC	0.0	0.0	0.0	0.0	0.0	0.0
CHILLI	0.3	0.3	0.3	0.3	0.3	0.3
SESAME	0.0	0.0	0.0	0.0	0.0	0.0
VEGETABLES	2.4	2.4	2.4	2.4	2.4	2.4
FRUITS	1.6	1.6	1.6	1.6	1.6	1.6
OTHERS	0.0	0.0	0.0	0.0	0.0	0.0

Table 6-3-2 CROP PRODUCTION - Route 6-2

CROP	(1000 TON)					
	1987		1993		2001	
	W/O	W	W/O	W	W/O	W
PADDY	41.0	41.1	41.3	41.9	41.6	42.9
MAIZE	2.9	2.9	2.9	3.0	2.9	3.0
MUNG BEAN	3.4	3.4	3.4	3.4	3.4	3.5
SOY BEAN	0.1	0.1	0.1	0.1	0.1	0.1
GROUND NUTS	0.3	0.3	0.3	0.3	0.3	0.3
SORGHUM	0.1	0.1	0.1	0.1	0.1	0.1
CASSAVA	0.3	0.3	0.3	0.3	0.3	0.3
SUGAR CANE	116.1	116.1	116.1	116.3	116.1	116.5
TOBACCO	0.0	0.0	0.0	0.0	0.0	0.0
COTTON	0.0	0.0	0.0	0.0	0.0	0.0
GARLIC	0.0	0.0	0.0	0.0	0.0	0.0
CHILLI	0.6	0.6	0.6	0.6	0.6	0.6
SESAME	0.1	0.1	0.1	0.1	0.1	0.1
VEGETABLES	3.7	3.7	3.7	3.7	3.7	3.7
FRUITS	2.8	2.8	2.8	2.8	2.8	2.8
OTHERS	0.0	0.0	0.0	0.0	0.0	0.0

Table 6-3-3 CROP PRODUCTION - Route 6-3

CROP	(1000 TON)					
	1987		1993		2001	
	W/O	W	W/O	W	W/O	W
PADDY	29.9	30.0	30.1	30.6	30.3	31.4
MAIZE	1.6	1.6	1.6	1.7	1.5	1.7
MUNG BEAN	2.1	2.1	2.1	2.2	2.1	2.2
SOY BEAN	0.0	0.0	0.0	0.0	0.0	0.1
GROUND NUTS	0.1	0.1	0.1	0.1	0.1	0.1
SORGHUM	0.3	0.3	0.3	0.3	0.3	0.3
CASSAVA	0.2	0.2	0.2	0.2	0.2	0.2
SUGAR CANE	100.5	100.5	100.5	100.7	100.5	100.9
TOBACCO	0.1	0.1	0.1	0.1	0.1	0.1
COTTON	0.0	0.0	0.0	0.0	0.0	0.0
GARLIC	0.0	0.0	0.0	0.0	0.0	0.0
CHILLI	0.3	0.3	0.3	0.3	0.3	0.3
SESAME	0.0	0.0	0.0	0.0	0.0	0.0
VEGETABLES	3.3	3.3	3.3	3.3	3.3	3.3
FRUITS	2.1	2.1	2.1	2.1	2.1	2.1
OTHERS	0.0	0.0	0.0	0.0	0.0	0.0

Table 6-3-4 CROP PRODUCTION - Route 6-4

CROP	(1000 TON)					
	1987		1993		2001	
	W/O	W	W/O	W	W/O	W
PADDY	30.1	30.2	30.3	30.8	30.6	31.6
MAIZE	2.0	2.0	2.0	2.0	2.0	2.0
MUNG BEAN	2.4	2.4	2.4	2.4	2.4	2.4
SOY BEAN	0.1	0.1	0.1	0.1	0.1	0.1
GROUND NUTS	0.1	0.1	0.1	0.1	0.1	0.1
SORGHUM	0.5	0.5	0.5	0.5	0.5	0.5
CASSAVA	0.2	0.2	0.2	0.2	0.2	0.2
SUGAR CANE	103.2	103.2	103.2	103.4	103.2	103.6
TOBACCO	0.1	0.1	0.1	0.1	0.1	0.1
COTTON	0.0	0.0	0.0	0.0	0.0	0.0
GARLIC	0.0	0.0	0.0	0.0	0.0	0.0
CHILLI	0.3	0.3	0.3	0.3	0.3	0.3
SESAME	0.0	0.0	0.0	0.0	0.0	0.0
VEGETABLES	4.0	4.0	4.0	4.0	4.0	4.0
FRUITS	2.5	2.5	2.5	2.5	2.5	2.5
OTHERS	0.1	0.1	0.1	0.1	0.1	0.1

4. ROAD USERS COST SAVINGS

In accordance with the concept and basic data described in Chapter 5 of Summary Report, sums of VOC on each road link concerned were calculated in both cases of with project and without project.

Road users cost savings, defined as the difference of total link VOC in the case of with project and that in the without project case, were estimated as follows:

Road Users Cost Savings
(Million Baht/Year)

<u>Alternative</u>	<u>1987</u>	<u>1993</u>	<u>2001</u>
6-1	21.68	32.10	55.90
6-2	19.27	28.03	48.36
6-3	26.83	39.18	67.21
6-4	31.40	45.98	78.99

5. ENGINEERING

5-1 Soils and Materials^{1/}

Test results of subgrade soil, materials for subbase and shoulders and crushed rocks along the route or in the vicinity of the project area are shown in Table 6-5-1.

Location of samplings for the tests above in this study are shown in the Location Map of Figure 6-1-1.

5-1-1 Subgrade Soils

Subgrade soils along the study route consist mainly of silty clay and clayey silt of low and medium plasticity index ranging from 10.0 to 18.0%. They are classified as A-4 and A-7-6 in the AASHTO Classification. CBR values are from 2.0 to 3.5%.

5-1-2 Subbase and Shoulder Materials

Since the study route lies in a flat area of recent alluvial basin, laterite deposits are not found along the route.

Materials for subbase and shoulder were planned to carry from the laterite deposits adjacent to project area. The nearest sources are Khao Kalon located 4 km southeast of B. Pa Phutsa and Khao I-Kok, 8 km northeast of B. Pa Phutsa as shown in the location map.

Test results of the laterites in these sources indicated that the plasticity index of the portion passing No. 4 sieve was from N.P. to 6.0% and the soaked CBR was from 60.0% to 90.0%. The laterites from these sources are suitable for use for subbase and shoulder.

Note: 1/ Some testing data in this section are extracted from the following source; "MATERIALS INVESTIGATION REPORT" Kamphaeng Phet - Tha Makhua - Pa Phutsa - Khanu Woralaksa Buri - Route 1 June 1979, Louis Berger International, Inc.

5-1-3 Rock Material

Two rock quarries are available for the proposed road.

The rock source 6/R-1 is from Khao Rok Kachan, adjacent to national highway Rt. 1, about 30 km North - West from Nakhon Sawan. The aggregate is found to be sound durable limestone of good quality for pavement or concrete aggregates. The fine aggregates passing #40 sieve is non-plastic and Los Angeles abrasion loss was around 26%, far less than specified 40% in DOH specification. One test result indicated that the CBR value is 78%, a little bit less than DOH specified value.

Another rock source R-2 is from Khao Kalon, about 4 km East of Ban Pa Phutsa, beginning point of the proposed road. The rock is a blue-grey limestone with the specific gravity of 2.56. One CBR test showed a high value of 120%, but the abrasion loss was between 44 and 63 per cent.

5-2 Preliminary Design

Engineering studies on the proposed road are described by alternative route (6-1, 6-2, 6-3 and 6-4) formulated in Chapter 1.

Each alternative route comprises several segments as referred to the figure and table in Chapter 1:

Alternative Route	Segments	Origin - Destination
6-1	(a)-A-(b)-C	Khanu W. Buri-Rt. 117 (B. Nong Huarua)
6-2	(a)-B-(b)-C	Khanu W. Buri-Rt. 117 (B. Nong Huarua)
6-3	(a)-A-(b)-(c)-D	Khanu W. Buri-Rt. 117 (B. Khok Kham Nua)
6-4	(a)-A-(b)-(c)-E	Khanu W. Buri-Rt. 117 (B. Don Doo)

Table 6-5-1 TEST RESULTS OF SOILS AND MATERIALS

Description	Sample No.	Location of Source (KM)	Depth (m)	Description of Sample	AASHO Classification	Sieve Analysis (% Passing)								Plasticity		Compaction DH-T STD.		Lab. CBR		Moisture Content (After Soaked) (%)	Abrasion (%)	
						50.0	25.0	19.0	9.5	#4	#10	#40	#200	LL (%)	PI (%)	Opt. Mc. (%)	γd gm/cc.	CBR (%)	Swell (%)			
Subgrade Soil	6/S-1	6 ^{KM} +700 (L.10m)	0.2-1.0	silty clay	A-4	-	-	-	-	-	100	98.6	92.8	33.0	10.2	17.8	1.684	2.8	1.24	19.9		
	6/S-2	19+500 (L.10)	0.2-1.0	silty clay	A-4	-	-	-	-	-	100	98.8	94.6	33.0	9.7	17.8	1.689	2.3	1.14	19.8		
	6/S-3	21+000 (L.10)	0.3-1.0	clayey silt	A-7-6	-	-	-	-	-	100	99.6	98.2	44.8	18.5	27.8	1.465	2.2	2.30	28.1		
	6/S-4	32+500 (L.7)	0.15-1.0	clayey silt	A-6	-	-	-	-	100	99.6	97.0	90.8	35.2	11.0	20.4	1.632	3.3	1.12	22.8		
	6/S-5	41+200 (L.7)	0.3-1.0	clayey silt	A-6	-	-	-	-	100	99.4	97.6	86.0	34.7	10.6	19.4	1.687	3.5	1.02	20.8		
Subbase/ Shoulder Material ^{1/}	6/L-1	Kao Kalom 4Km from B. Pa Phutsa			GM*	-	100	97	79	46	21	16	14	N - P	6.7	2.21	98.2	0		5.5		
	6/L-2	Kao I KoK 8Km from B. Pa Phutsa			GM-GC*	-	93	87	63	45	28	18	16	21	6	7.0	2.23	58.0	0		6.3	
	6/SA-1	15 Km upstream of B. Pa Phutsa		coarse sand	-	-	-	-	-	-	#16	#50	#100									
Crushed Rock	6/R-1	Khao Rok Kachan Rt.1, Km. 274+500 (R)		lime stone	A-1-a	100	94.7	91.5	76.0	51.3	30.9	10.5	4.7	N - P	7.3**	2,260**	78.0	-			27.4	
	6/R-2	Khao Kalom 4Km from B. Pa Phutsa		lime stone	A-1-b	100	93.1	87.0	73.7	63.2	44.6	26.5	20.5	N - P	6.3**	2.203**	120.0	0.22				63/0

Note: ^{1/} Extracted from "Materials Investigation Report", op. cit.

* Classification by Unified Method.

** Compaction by DH-T-MOD.

The design standards adopted for the preliminary designs is Class F4 of DOH Design Standard.

As the terrain is flat all along the proposed road, the design speed of 80 km/hr is taken except for villages sections where it is reduced to 40 km/hr.

5-2-1 Alignment and Earthwork

1) Alternative Route 6-1

As the horizontal and vertical alignments of this route are in fair condition, no major improvements of alignments to the existing road was required. However, the roads width is not enough for Design Standard. Widening required for this route varies from 0.3 to 6.0 m.

The raising up of the existing embankment is also necessary, the height of which ranges from 0.5 m to 1.5 m. The raising up section is indicated in the Drawings.

A. Banphot Phisai and A. Kao Liew where the existing road passes through, are densely populated with many houses close to the road. It was judged from the situation that the acquisition of right-of-way width of 20 m is difficult from the economical and social view points.

Bypasses were thus planned in these two Amphoe centers of populated areas, length of 7.0 km and 1.2 km, respectively.

The planned bypass at A. Banphot Phisai takes the route on swampy area. The embankment height proposed in the section was 2.0 m. For Kao Liew bypass, 1.5 m high embankment was proposed.

The existing road of Segment C takes the route of north-eastern direction from A. Kao Liew, thus constituting an unfavorable detoured route to Nakhon Sawan. For prompt access to/from Nakhon Sawan, a new alignment was planned on the extension of a part of the bypass proposed above. The new alignment section is 3.3 km long, with an average embankment height of 1.5 m.

2) Alternative Route 6-2

The alternative route 6-2 takes the route of Segment B instead of Segment A in the route 6-1.

The existing road in Segment B passes through flat area but rather curved horizontal alignment. But no major improvement of horizontal alignment was required.

Widening required for this route varies from 0.3 m to 6.0 m. and required raising up of the existing embankment ranges 0.5 m to 2.0 m.

For 2.9 km of the end section of this route, a new alignment was introduced to detour A. Banphot Phisai. As the new alignment takes the route on swampy area, 2.0 m high embankment was proposed for this section.

Other design features are referred to the descriptions mentioned in 1) above.

3) Alternative Route 6-3

The alternative route 6-3 takes the same route as Route 6-1 between the origin point and A. Kao Liew.

Segment © between Kao Liao and B. Makulua passes through populated villages in full length. Private houses stand close to the shoulders, resulting in the shortage of right-of-way width. Judging from the present situation, Bypass of 2.1 km long was planned in the east side of the existing road avoiding the houses. The embankment height of the bypass section is 1.5 m.

Segment D is a newly planned route in the route 6-3, connecting B. Makulua with Rt. 117 with short distance. The new alignment section is 2.9 km long, with an average embankment height of 1.5 m.

Other design features of this route are referred to the descriptions mentioned in alternative route 6-1.

4) Alternative Route 6-4

The alternative route 6-4 takes the route of Segment E instead of Segment D in the route 6-3.

The design features on the route until B. Makulua from the origin point are referred to the descriptions mentioned in 1) and 3) above.

Segment E was planned as new alignment section, The first 2.5 km is a bypass to avoid B. Makulua, B. Sra Ngam and B. Yang En along the existing road, and the remaining 2.0 km, as new access road to Rt. 117. Average embankment height in the new alignment section is 1.5 m.

5) Summary of Route Length

The length of route by alternative routes is summarized as follows:

Alter- native Route	Improve- ment Section	New Construction Section					total	Total (km)
		By-pass			Access Road			
		Ban Phot Phisai	Kao Liao	B. Ma- kua				
6-1	27.6	7.0	1.2	-	3.7	11.9	39.5	
6-2	33.0	2.9	1.2	-	3.7	7.8	40.8	
6-3	29.6	7.0	2.8	2.1	2.9	14.8	44.4	
6-4	29.7	7.0	2.8	2.5	4.0	16.3	46.0	

5-2-2 Pavement Design

Pavement structures for F4 class road were designed in accordance with DOH Method introduced in Volume 1.

There is a sugar cane refinery factory at about 7.0 km south of A. Kao Liao along the study road. For transportation of the sugar cane, traffic of over-loading heavy trucks which are about 30,000 kg in gross weight is expected. Therefore, in about 8.6 km long section near the sugar cane factory, the asphalt concrete pavement was designed taking this into considerations.

A) SBST

1) Design Traffic Number

There is no significant difference in ADT among the four alternative routes. Design traffic number was, therefore estimated based on ADT in 6-4 alternative route as follows:

Item	H/T	M/T	H/B	Total	Remark
Average number of Heavy Truck at 1981	39	54	41	134	ADT
	19.5	27	20.5	67	One direction
Traffic					
Component (%) (a)	29	40	31	100	
Gross Weight (kg) (b)	15,400	6,150	12,300		
Average Gross Weight (kg) (a) x (b)	4,466	2,460	3,813	10,739	

Note: H/T : Heavy Truck
M/T : Medium Truck
H/B : Heavy Bus

When single-axle load limit is 8,200 kg, from the traffic analysis chart, (ITN) = 1.9.

As traffic growth rate is 6.0%.

Design Traffic Numbers (DTN) are ;

DTN 7 (7 years design period) = $19 \times 0.42 \div 8$

DTN 15 (15 year design period) = $19 \times 1.16 \div 22$

2) Design CBR

Design CBR (80% percentile value) was calculated corresponding to the number of tests.

Test Value of CBR (%)					Design CBR (%)
1	2	3	4	5	
2.8	2.3	2.2	3.3	3.5	2.5

3) Thickness of Pavement

i) Thickness of Full-Depth Asphalt Concrete

Entering the thickness design chart with DTN and Design CBR;

Pavement thickness for 7 years design period.

(TA7) = 220 mm

Pavement thickness for 15 years design period.

(TA15) = 250 mm

ii) Thickness of SBST Pavement

Assuming that substitution factor is 2.0 for crushed stone base and 2.7 for laterite subbase.

Thickness of pavement structures of SBST was determined from calculated TA7 250 mm full depth asphalt concrete as follows.

SBST 12 mm

Crushed stone base CBR \geq 80 150 mm

Soil aggregate subbase CBR \geq 20 390 mm

iii) Thickness of Overlay

TA15 - TA7 = 250 mm - 220 mm = 30 mm

When overlay is planned by SBST pavement, the thickness is:

SBST 12 mm

Crushed stone base CBR \geq 80 60 mm

B) Asphalt Concrete Pavement

1) Design Traffic Number

Design of pavement was carried out based on ADT of link No. 6 in the alternative route 6-3.

According to forecasted O/D of agricultural products, 25% of heavy trucks are used for the transportation of agricultural and 95% of them is estimated to be used for the transportation of sugar cane.

The average gross weight of the heavy truck for the transportation of the sugar cane was estimated as follows:

Average Gross Weight of Heavy Truck for Sugar Cane

Description	Gross Weight (kg)	Empty Rate (%)	Average Gross Weight (kg)
Laden	30.000	60	
Empty	7.000	40	20,800

For the gross weights of other traffic, the values estimated in Volume 1 were applied.

Design traffic number was calculated by the following procedures:

Item	Heavy Truck		Medium Truck	Heavy Bus	Total	Remark
	For Sugar Cane	Others				
Number of Heavy Truck at 1989	15 7.5	46 23	86 43	49 29.5	196 98	ADT One Direction
Traffic Composition% (a)	8	23	44	25	100	
Gross Weight (kg) (b)	20,800	15,400	6,150	12,300		
Average Gross Weight (kg) (a) x (b)	1,664	3,542	2,706	3,075	10,987	

When single-axle load limit is 8,200 kg, from the traffic analysis chart, (ITN) = 33

As traffic growth rate is 6.0%,

Design Traffic Number (DTN) are;

DTN 7 (7 years design period) = $33 \times 0.42 = 14$

DTN 15 (15 years design period) = $33 \times 1.16 = 38.5$

2) Design CBR

Design CBR is the same value as used for the design of SBST.

3) Thickness of Pavement

i) Thickness of Full-Depth Asphalt Concrete

Entering the thickness design chart with DTN and Design CBR;

Pavement thickness for 7 years design period.

(TA7) = 240 mm

Pavement thickness for 15 years design period.

(TA15) = 270 mm

ii) Thickness of Asphalt Concrete Pavement

Considering workability, the thickness of the asphalt concrete surface was determined at 50 mm.

Where the thickness of the crushed stone base is 150 mm, the pavement structures are as follows:

Asphalt concrete surface		50 mm
Crushed stone base	CBR \geq 80	150 mm
Soil aggregate subbase	CBR \geq 20	330 mm

iii) Thickness of Overlay

TA15 - TA7 = 270 mm - 240 mm = 30 mm

30 m thickness of overlay was calculated theoretically. However 40 mm thickness of overlay by asphalt concrete was finally designed considering workability.

5-2-3 Drainage

Pipe culverts with the diameter of 1 m were installed at the intervals of 200 m for all alternative routes, except for segment (b) and (c), where intervals of 500 m were proposed.

Box culvert (2.4m x 2.4m) was planned at the water course having small catchment area. The number of cells of culvert was determined by comparing discharge with flow capacity.

Box culvert was proposed only in one place of Segment (c) related to the alternative route 6-3 and 6-4 as follows:

List of Box Culvert

Station	Existing Structure	Catchment Area (km ²)	Intensity (mm/h)	Discharge (m ³ /sec)	Proposed Structure	Capacity (m ³ /sec)
41+300	-	3	60	40	C-B-2 (2.4x2.4) -10.0	50

5-2-4 Bridge

Short span concrete bridges were planned where the river is relatively narrow and shallow. The length of bridges was determined by comparing discharge with flow capacity of bridge opening.

The proposed bridges are shown in the following tables by route alternative.

Study Route No. 6-1

List of Bridge

Station	Existing Structure	Catchment Area (km ²)	Intensity (mm/h)	Discharge (m ³ /sec)	Proposed Structure	Capacity (m ³ /sec)
15+800	C-B-4 (1.8x1.8) -4.0	5	57	62	BR-C (7.0x10.0)	63
22+200	-	10	45	97	BR-C (7.0x14.0)	98

Total length = 24.0 m

Study Route No. 6-2

List of Bridge

Station	Existing Structure	Catchment Area (km ³)	Intensity (mm/h)	Discharge (m ³ /sec)	Proposed Structure	Capacity (m ³ /sec)
15+700	BR-T (4.0x7.5)	4	70	62	BR-C (7.0x10.0)	63
22+200	-	10	45	97	BR-C (7.0x14.0)	98

Total length = 24.0 m

Study Route No. 6-3/6-4

List of Bridge

Station	Existing Structure	Catchment Area (km ³)	Intensity (mm/h)	Discharge (m ³ /sec)	Proposed Structure	Capacity (m ³ /sec)
15+800	BR-T (1.8x1.8) -4.0	5	57	62	BR-C (7.0x10.0)	63
22+200	-	10	45	97	BR-C (7.0x14.0)	98
36+800	-	13	37	102	BR-C (7.0x15.0)	106

Total length = 39.0 m

6. CONSTRUCTION COST

Construction costs were obtained by applying the unit rates to the respective work quantities calculated on the basis of the engineering studies.

Rock materials used for SBST, asphaltic concrete, base course and structure works were supposed to be transported from rock quarries 6/R-1 and 6/R-2 with a weighted average hauling distance of 36 km. The transportation cost for this hauling distance was reflected to each unit rate.

The construction cost together with land acquisition cost are given in Table 6-6-1.

The construction period for the proposed road was estimated to be 3 years. Yearly disbursements of construction cost together with price contingency are shown in the following table.

YEARLY COST DISBURSEMENT - Route 6-4

	(Million Baht)								
	1984		1985		1986		Total		
	L/C ^{1/}	F/C ^{2/}	L/C	F/C	L/C	F/C	L/C	F/C	Total
Construction Cost	11.5	10.5	28.7	26.4	17.2	15.9	57.4	52.8	110.2
Price Contingency ^{3/}	3.7	1.7	13.0	6.2	10.3	4.9	27.0	12.8	39.8
Total	15.2	12.2	41.7	32.6	27.5	20.8	84.4	65.6	150.0
							(3.68)	(2.85)	(6.53)

Note: 1/ Local Currency

2/ Foreign Currency

3/ At assumed annual escalation rates as follows (% p.a.):

	<u>Local C.</u>	<u>Foreign C.</u>
1981 - 1983	15	7.5
1983 - 1987	10	6.5

() Million US\$ Equivalent (1US\$ = 22.63 Baht)

CONSTRUCTION COST - ROUTE 0

Description	Unit of quantity	Financial Unit Rate (Baht)	6-1 (F-4/39.5 KM)		6-2 (F-4/40.8 KM)		6-3 (F-4/44.4 KM)		6-4 (F-4/46.0 KM)	
			Quantity	Economic Cost (10 ³ ฿)	Quantity	Economic Cost (10 ³ ฿)	Quantity	Economic Cost (10 ³ ฿)	Quantity	Economic Cost (10 ³ ฿)
Clearing & Grubbing	ha	17,000	93	1,439	92	1,423	106	1,640	110	1,702
Roadway Excavation - Classified Earth	m ³	36	0	0	0	0	0	0	0	0
Road Excavation - Classified Soft Rock	m ³	80	0	0	0	0	0	0	0	0
Embankment - Side Borrow	m ³	45	421,200	17,248	389,700	15,958	467,300	19,136	499,300	20,466
Embankment - Borrow Pit	m ³	60	0	0	0	0	0	0	0	0
Embankment - Selected Material	m ³	80	0	0	0	0	0	0	0	0
Subbase - Soil Aggregate	m ³	106	159,900	15,085	165,200	15,585	179,700	16,953	186,200	17,566
Base - Crushed Rock	m ³	309	38,500	10,945	39,800	11,314	43,300	12,309	44,900	12,764
Shoulder - Soil Aggregate	m ³	170	16,600	2,512	17,100	2,587	18,600	2,814	19,300	2,920
Asphaltic Prime Coat	m ²	10.8	256,800	2,552	265,200	2,635	288,600	2,868	299,000	2,971
Single Bituminous Surface Treatment	m ²	27.6	217,300	5,398	224,400	5,574	244,200	6,066	253,000	6,285
R.C. Pipe Culvert	m	2,400	1,880	4,151	1,880	4,151	2,090	4,514	2,210	4,880
R.C. Box Culvert	m	18,000	0	0	0	0	20	324	20	324
R.C. Bridge - Short Span	m	39,500	24	844	24	844	39	1,371	39	1,371
P.C. Bridge - Long Span	m	68,700	0	0	0	0	0	0	0	0
Sub-total				60,174		60,071		67,995		71,229
Miscellaneous Works ^{1/}				4,212		4,205		4,760		4,986
Total Direct Construction Cost				64,386		64,276		72,755		76,215
PHYSICAL CONTINGENCY ^{2/}				9,658		9,641		10,913		11,432
DESIGN AND CONSTRUCTION SUPERVISION ^{3/}				6,439		6,428		7,276		7,622
Total				80,483		80,345		90,944		95,269
Land Acquisition										
Highly Devel'd Land	ha	50,000	85	4,250	77	3,850	96	4,800	102	5,100
Less Devel'd Land	ha	15,000	1	15	1	15	1	15	1	15
Grand Total				84,748		84,210		95,759		100,384
FINANCIAL COST (10 ³ Baht)				(93,118)		(92,587)		(105,606)		(110,306)

Note: 1/ 7% of direct construction cost of major work items.
 2/ 15% of direct construction cost.
 3/ 10% of direct construction cost.

7. EVALUATION

In accordance with the basic conditions of economic evaluation discussed in Summary Report and economic costs and benefits as estimated in the foregoing sections, internal rate of return was calculated for each alternative as follows:

	<u>IRR</u>
Alternative 6-1	25.8 %
" 6-2	25.8 %
" 6-3	26.7 %
" 6-4	28.5 %

Details of costs and benefit streams are given in Table 6-7-1 to 6-7-5. Having the highest IRR, Alternative 6-4 is recommendable to be selected. Furthermore, Alternative 6-4 is most desirable in view of the better services to the local population, while Alternative 6-3 is best positioned to serve for sugar cane transportation during only a limited period in a year.

Considering that it is usual practice for sugar cane transportation to load more than 20 tons per truck, asphalt concrete pavement was planned for a section of 8.6 km which is used to have heavy sugar cane traffic. Due to this measure, construction cost was raised up 3.3% and IRR was reduced to 27.9% from 28.5%.

Table 6-7-1 COSTS AND BENEFITS STATEMENT - Route 6-1

(1000 BAHT)							
YEAR	COST		BENEFITS			DISCOUNTED (12%)	
	CONST. COST	AGRI. BENEFIT	VOC SAVING	RMC SAVING	TOTAL	COST	BENEFIT
1983	0	0	0	0	0	0	0
1984	16,950	0	0	0	0	23,814	0
1985	42,370	0	0	0	0	53,149	0
1986	25,428	0	0	0	0	28,479	0
1987	0	7,470	21,683	-11	29,142	0	26,020
1988	0	7,659	23,419	18	31,096	0	24,789
1989	0	7,848	25,155	47	33,049	0	23,524
1990	0	8,037	26,890	76	35,003	0	22,245
1991	0	8,225	28,626	105	36,956	0	20,970
1992	0	8,414	30,362	134	38,910	0	19,713
1993	0	8,603	32,098	162	40,863	0	18,485
1994	27,111	8,792	35,073	212	44,077	12,264	17,802
1995	0	8,981	38,048	262	47,291	0	17,054
1996	0	9,170	41,023	312	50,504	0	16,261
1997	0	9,359	43,998	361	53,718	0	15,443
1998	0	9,547	46,973	411	56,931	0	14,613
1999	0	9,736	49,948	461	60,145	0	13,784
2000	0	9,925	52,923	511	63,359	0	12,964
2001	-41,287	10,114	55,898	560	66,572	-7,543	12,163
TOTAL	70,572	131,879	552,117	3,621	687,617	110,162	275,828
DISCOUNTED ECONOMIC COSTS :					110,162		
DISCOUNTED ECONOMIC BENEFITS :					275,828		
AGRICULTURAL DEVELOPMENT BENEFIT					57,283		
VOC SAVING					217,456		
RMC SAVING					1,090		
NET PRESENT VALUE :					165,666		
BENEFIT COST RATIO :					2.50		
INTERNAL RATE OF RETURN :					25.8 %		

Table 6-7-2 COSTS AND BENEFITS STATEMENT - Route 6-2

(1000 BAHT)

YEAR	COST		BENEFITS			DISCOUNTED (12%)	
	CONST. COST	AGRI. BENEFIT	VOC SAVING	RMC SAVING	TOTAL	COST	BENEFIT
1983	0	0	0	0	0	0	0
1984	16,840	0	0	0	0	23,659	0
1985	42,100	0	0	0	0	52,810	0
1986	25,270	0	0	0	0	28,302	0
1987	0	10,490	19,265	20	29,774	0	26,584
1988	0	10,750	20,725	40	31,515	0	25,123
1989	0	11,010	22,186	59	33,255	0	23,670
1990	0	11,270	23,646	79	34,995	0	22,240
1991	0	11,530	25,106	99	36,736	0	20,845
1992	0	11,790	26,567	119	38,476	0	19,493
1993	0	12,050	28,027	139	40,216	0	18,192
1994	27,870	12,310	30,569	174	43,052	12,607	17,388
1995	0	12,570	33,110	209	45,889	0	16,548
1996	0	12,829	35,651	245	48,725	0	15,688
1997	0	13,089	38,192	280	51,562	0	14,823
1998	0	13,349	40,734	315	54,398	0	13,963
1999	0	13,609	43,275	351	57,234	0	13,117
2000	0	13,868	45,816	386	60,071	0	12,292
2001	-40,824	14,128	48,357	421	62,907	-7,458	11,493
TOTAL	71,256	184,641	481,226	2,937	668,804	109,920	271,458

DISCOUNTED ECONOMIC COSTS :	109,920
DISCOUNTED ECONOMIC BENEFITS :	271,458
AGRICULTURAL DEVELOPMENT BENEFIT	80,263
VOC SAVING	190,252
RMC SAVING	943
NET PRESENT VALUE :	161,538
BENEFIT COST RATIO :	2.47
INTERNAL RATE OF RETURN :	25.8 %

Table 6-7-3 COSTS AND BENEFITS STATEMENT - Route 6-3

(1000 BAHT)

YEAR	COST		BENEFITS			DISCOUNTED (12%)	
	CONST. COST	AGRI. BENEFIT	VOC SAVING	RMC SAVING	TOTAL	COST	BENEFIT
1983	0	0	0	0	0	0	0
1984	19,150	0	0	0	0	26,904	0
1985	47,880	0	0	0	0	60,061	0
1986	28,729	0	0	0	0	32,176	0
1987	0	7,910	26,832	-13	34,729	0	31,008
1988	0	8,114	28,890	18	37,022	0	29,514
1989	0	8,317	30,947	49	39,314	0	27,983
1990	0	8,521	33,005	81	41,607	0	26,442
1991	0	8,725	35,063	112	43,899	0	24,910
1992	0	8,928	37,120	143	46,192	0	23,402
1993	0	9,132	39,178	174	48,484	0	21,932
1994	30,470	9,336	42,682	228	52,246	13,783	21,101
1995	0	9,540	46,186	282	56,008	0	20,197
1996	0	9,744	49,690	335	59,769	0	19,244
1997	0	9,948	53,194	389	63,531	0	18,264
1998	0	10,152	56,698	443	67,293	0	17,272
1999	0	10,356	60,202	497	71,055	0	16,284
2000	0	10,560	63,706	550	74,817	0	15,309
2001	-46,649	10,764	67,210	604	78,579	-8,523	14,356
TOTAL	79,580	140,047	670,605	3,892	814,544	124,402	327,218

DISCOUNTED ECONOMIC COSTS :	124,402
DISCOUNTED ECONOMIC BENEFITS :	327,218
AGRICULTURAL DEVELOPMENT BENEFIT	60,785
VOC SAVING	265,265
RMC SAVING	1,168
NET PRESENT VALUE :	202,816
BENEFIT COST RATIO :	2.63
INTERNAL RATE OF RETURN :	26.7 %

Table 6-7-4 COSTS AND BENEFITS STATEMENT - Route 6-4

(1000 BAHT)

YEAR	COST		BENEFITS			DISCOUNTED (12%)	
	CONST. COST	AGRI. BENEFIT	VOC SAVING	RMC SAVING	TOTAL	COST	BENEFIT
1983	0	0	0	0	0	0	0
1984	20,080	0	0	0	0	28,211	0
1985	50,190	0	0	0	0	62,958	0
1986	30,114	0	0	0	0	33,728	0
1987	0	8,440	31,399	-19	39,820	0	35,553
1988	0	8,661	33,829	13	42,503	0	33,883
1989	0	8,882	36,258	46	45,186	0	32,163
1990	0	9,103	38,688	78	47,869	0	30,422
1991	0	9,324	41,118	111	50,553	0	28,685
1992	0	9,545	43,548	143	53,236	0	26,971
1993	0	9,766	45,978	175	55,919	0	25,295
1994	31,570	9,987	50,104	232	60,323	14,281	24,364
1995	0	10,209	54,231	288	64,727	0	23,341
1996	0	10,430	58,357	344	69,131	0	22,258
1997	0	10,651	62,484	401	73,535	0	21,140
1998	0	10,872	66,610	457	77,939	0	20,005
1999	0	11,094	70,737	514	82,344	0	18,871
2000	0	11,315	74,863	570	86,748	0	17,750
2001	-48,939	11,536	78,990	626	91,152	-8,941	16,653
TOTAL	83,015	149,814	787,191	3,980	940,985	130,237	377,354

DISCOUNTED ECONOMIC COSTS :	130,237
DISCOUNTED ECONOMIC BENEFITS :	377,354
AGRICULTURAL DEVELOPMENT BENEFIT	64,982
VOC SAVING	311,192
RMC SAVING	1,181
NET PRESENT VALUE :	247,118
BENEFIT COST RATIO :	2.90
INTERNAL RATE OF RETURN :	28.5 %

Table 6-7-5 COSTS AND BENEFITS STATEMENT - ROUTE 6-4 (with Asphalt Concrete)

(1000 BAHT)

YEAR	COST		BENEFITS			DISCOUNTED (12%)	
	CONST. COST	AGRI. BENEFIT	VOC SAVING	RMC SAVING	TOTAL	COST	BENEFIT
1983	0	0	0	0	0	0	0
1984	20,742	0	0	0	0	29,141	0
1985	51,855	0	0	0	0	65,047	0
1986	31,113	0	0	0	0	34,847	0
1987	0	8,440	31,399	-19	39,820	0	35,553
1988	0	8,661	33,829	13	42,503	0	33,883
1989	0	8,882	36,258	46	45,186	0	32,163
1990	0	9,103	38,688	78	47,869	0	30,422
1991	0	9,324	41,118	111	50,553	0	28,685
1992	0	9,545	43,548	143	53,236	0	26,971
1993	0	9,766	45,978	175	55,919	0	25,295
1994	30,795	9,987	50,104	232	60,323	13,930	24,364
1995	0	10,209	54,231	288	64,727	0	23,341
1996	0	10,430	58,357	344	69,131	0	22,258
1997	0	10,651	62,484	401	73,535	0	21,140
1998	0	10,872	66,610	457	77,939	0	20,005
1999	0	11,094	70,737	514	82,344	0	18,871
2000	0	11,315	74,863	570	86,748	0	17,750
2001	-50,469	11,536	78,990	626	91,152	-9,221	16,653
TOTAL	84,036	149,814	787,191	3,980	940,985	133,744	377,354

DISCOUNTED ECONOMIC COSTS :	133,744
DISCOUNTED ECONOMIC BENEFITS :	377,354
AGRICULTURAL DEVELOPMENT BENEFIT	64,982
VOC SAVING	311,192
RMC SAVING	1,181
NET PRESENT VALUE :	243,610
BENEFIT COST RATIO :	2.82
INTERNAL RATE OF RETURN :	27.9 %

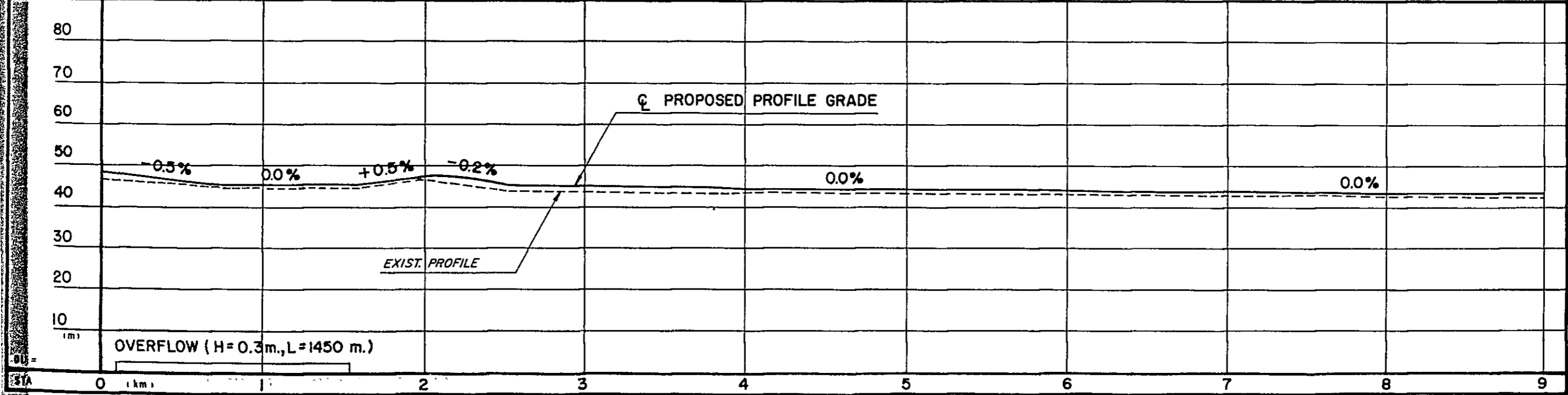
ABBREVIATIONS for PLAN and PROFILE

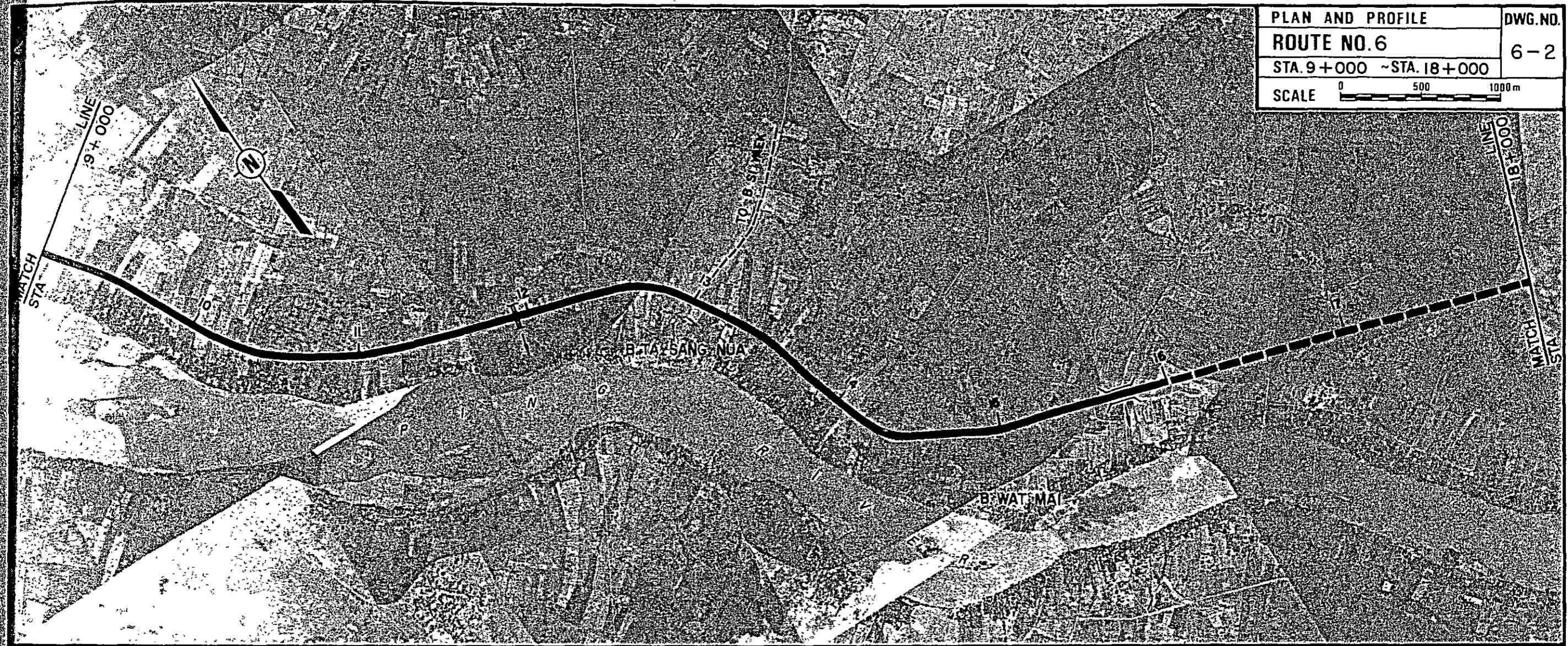
C-P-n ϕ a-l	EXISTING PIPE CULVERT, n (ROW), ϕ a (DIAMETER, M), l (LENGTH, M)
C-B-n (a x b)-l	EXISTING (below grade line) and PROPOSED (above grade line) BOX CULVERT, n (NOS. OF TUBE), a x b (LATERAL x VERTICAL, M), l (LENGTH, M)
BR-T (a x l)(n)	EXISTING (below grade line) and PROPOSED (above grade line) TIMBER BRIDGE, a x l (WIDTH x LENGTH, M), n (NOS. OF SPAN)
BR-C (a x l)(n)	EXISTING (below grade line) and PROPOSED (above grade line) CONCRETE BRIDGE, a x l (CARRIAGE WAY WIDTH x LENGTH, M), n (NOS. OF SPAN)

PLAN AND PROFILE	DWG. NO.
ROUTE NO. 6	6-1
STA 0+000 ~ STA. 9+000	
SCALE 0 500 1000 m	

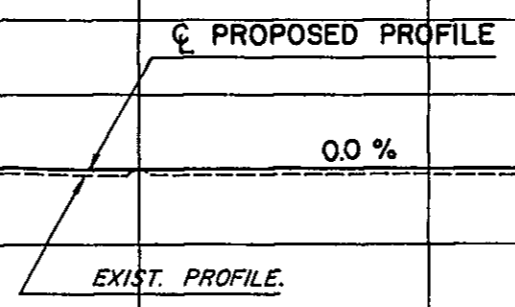


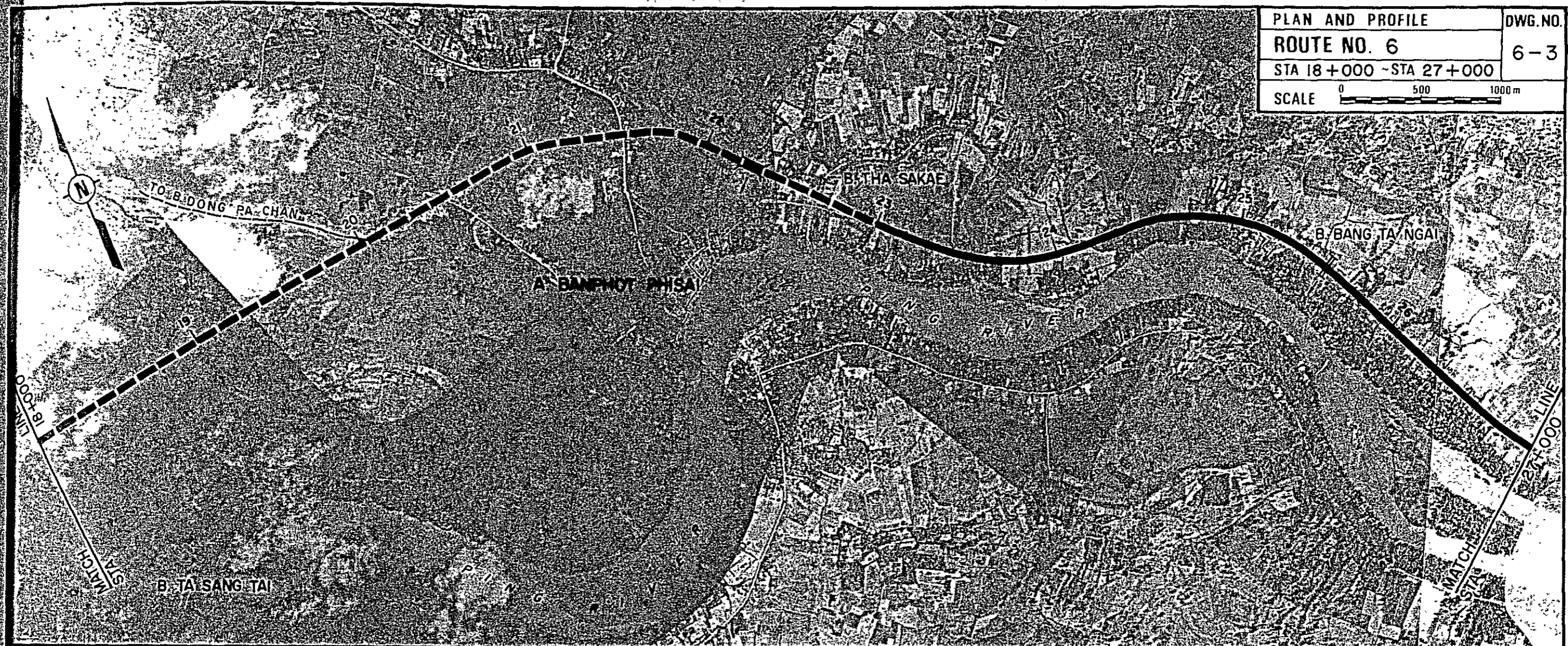
CONSTRUCTION	IMPROVEMENT (4)									
FORMATION (m)	UP	1.0	0.5	1.5	1.0	0.5	1.0	0.5	1.0	
PIPE CULVERT		NEW 36 m.	NEW 12 m.	NEW 108 m.	NEW 120 m.	NEW 24 m.	NEW 36 m.	NEW 12 m.		
80										
70										
60										
50										
40										
30										
20										
10										
0										
STA	0	1	2	3	4	5	6	7	8	9





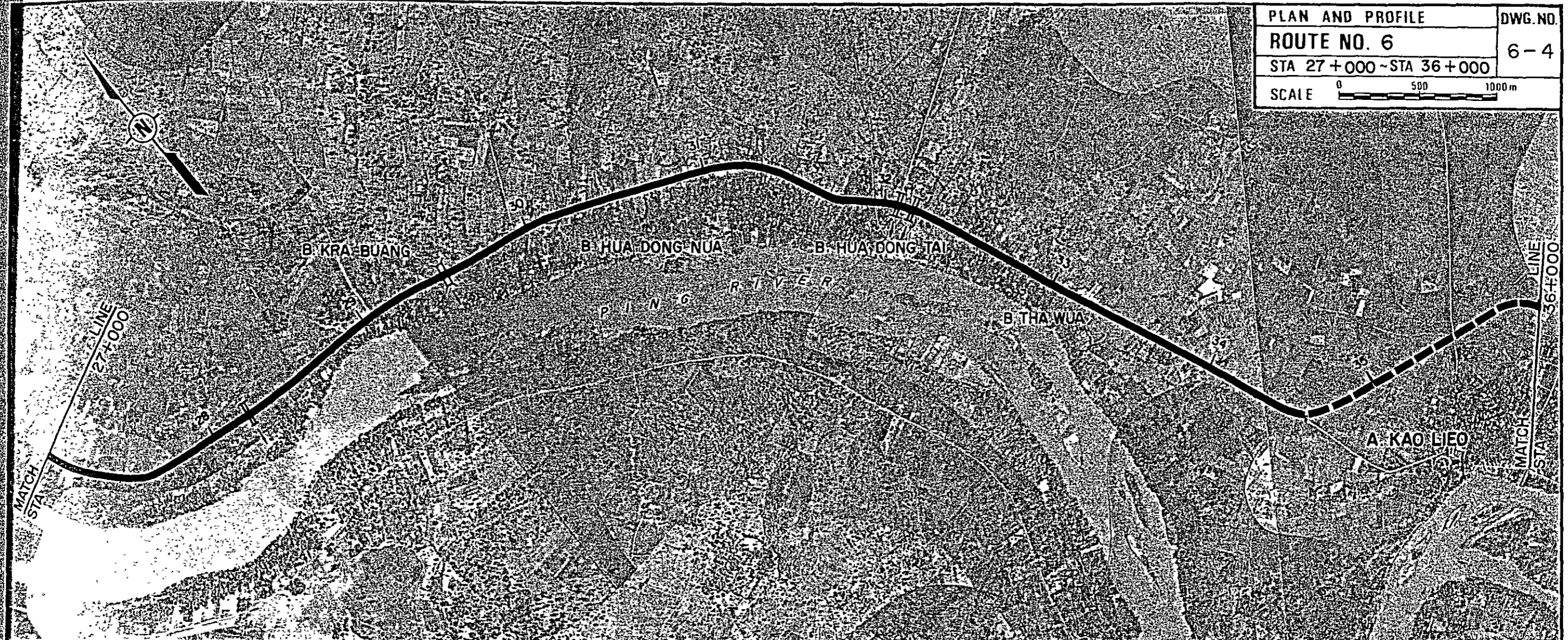
CONSTRUCTION	IMPROVEMENT (F 4)				NEW CONSTRUCTION (F 4)					
FORMATION UP	1.0		0.5		1.0		2.0			
DOWN										
PIPE CULVERT	NEW 564 m.									
70										
60										
50										
40	0.0%	0.3%			0.0%			0.0%		
30										
20										
10										
0										
BL =										
STA	9 (km)	10	11	12	13	14	15	16	17	18





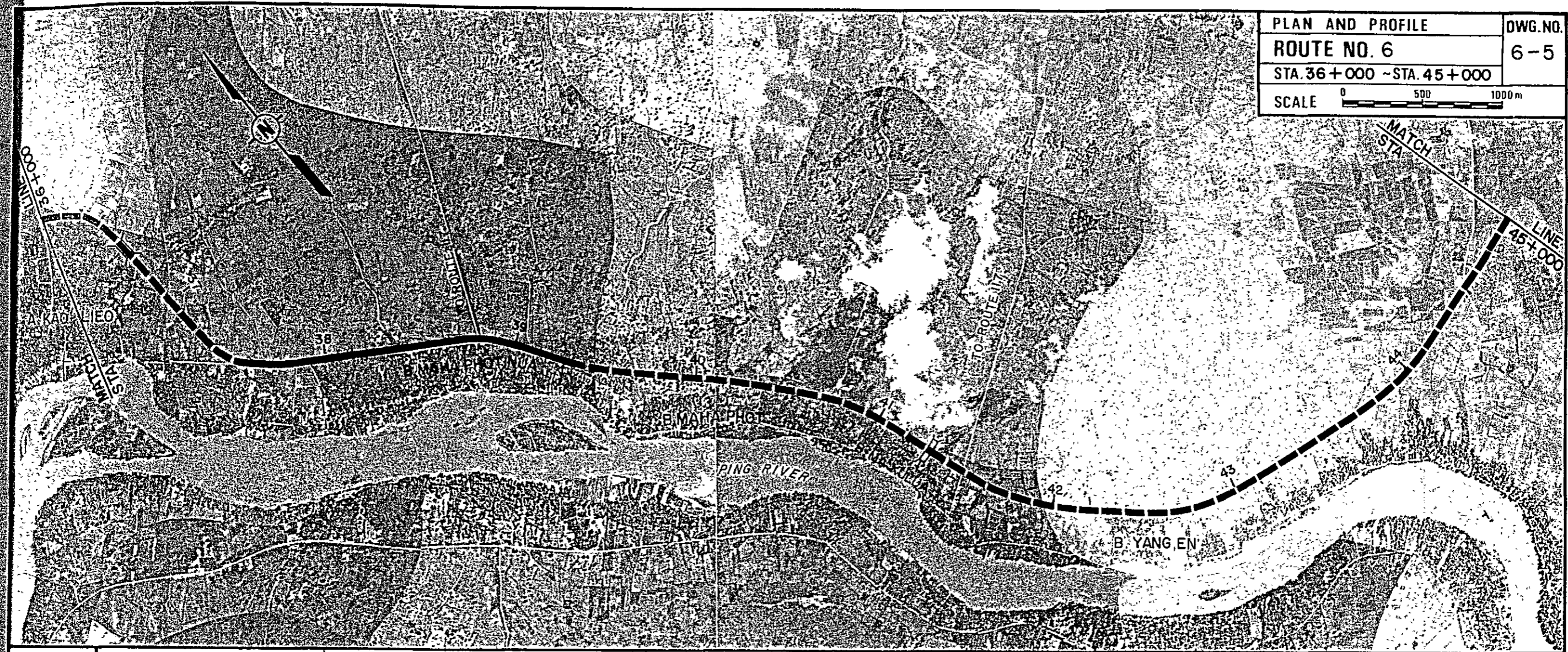
CONSTRUCTION	NEW CONSTRUCTION (F4)		IMPROVEMENT (F4)							
FORMATION (m)	UP 2.0		DOWN 1.0							
PIPE CULVERT	NEW 252 m.		NEW 180 m.							
70										
60										
50										
40										
30										
20										
10										
0										
(m)										
DL =	OVERFLOW (H=1.0 m., L=5100 m.)		OVER FLOW (H=0.1-0.3 m., L 3400 m.)							
STA	18	19	20	21	22	23	24	25	26	27

PLAN AND PROFILE	DWG. NO.
ROUTE NO. 6	6-4
STA 27+000 ~ STA 36+000	
SCALE 0 500 1000 m	



CONSTRUCTION	IMPROVEMENT (F4)		NEW CONSTRUCTION (F4)
FORMATION (m)	UP 1.0	DOWN 0.5	1.5
PIPE CULVERT	NEW 240 m.		NEW 168 m.
70			
60			
50			
40			
30			
20			
10			
0			
DL =			
STA	27 (km)	28	29
		30	31
		32	33
		34	35
			36

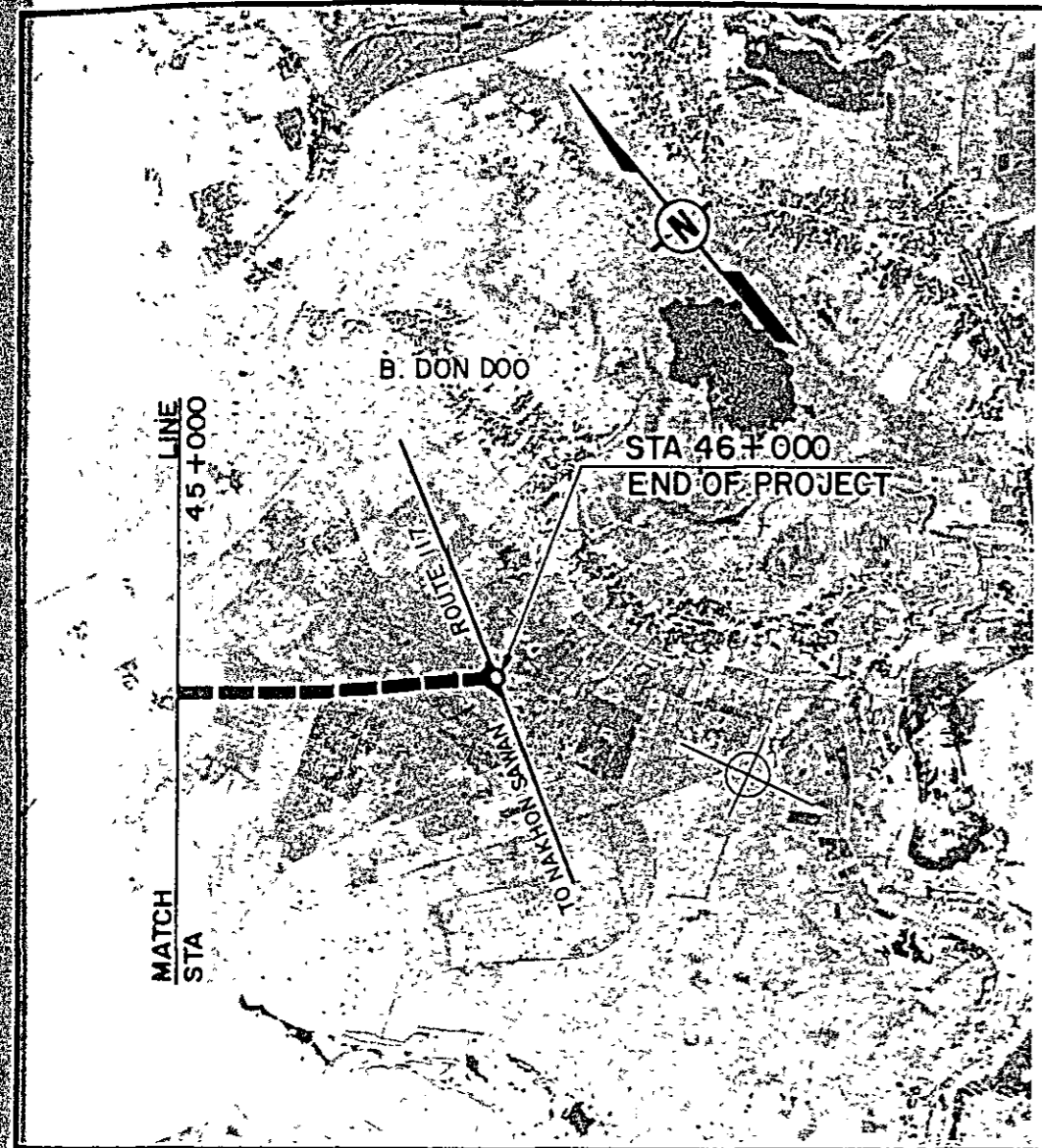
PLAN AND PROFILE
ROUTE NO. 6
 STA. 36+000 ~ STA. 45+000
 SCALE 0 500 1000 m
 DWG. NO.
6-5



CONSTRUCTION	NEW CONSTRUCTION (F4)	IMPROVEMENT (F4)	NEW CONSTRUCTION (F4)
FORMATION UP	1.5	0.5	1.5
DOWN			2.0
PIPE CULVERT		NEW 60	NEW 408

60										
50	BR-C (70x150)									
40										
30										
20			0.0%					0.0%		
10										
0										
(m)										
DL =										OVERFLOW (H = 0.3-1.2m, L = 2000m)
STA	36+000	37+000	38+000	39+000	40+000	41+000	42+000	43+000	44+000	45+000

PLAN AND PROFILE	DWG. NO.
ROUTE NO. 6	6-6
STA. 45+000 ~ STA. 46+000	
SCALE	0 500 1000 m



CONSTRUCTION	NEW CONSTRUCTION (F4)										
FORMATION (m)	UP	2.0									
	DOWN										
PIPE CULVERT											
ELEVATION (m)	60										
	50										
	40										
	30										
	20										
	10										
	0										
	DL =	OVER FLOW (H=0.3 -1.2m., L= 2000m.)									
STA	45+00 (km)	46+000									