

APPENDIX 8.2.1.B PROJECT COST WITH FOREIGN AND LOCAL CURRENCY BREAKDOWN  
(1986 Price Level, Cabuyao-Sta. Rosa-Biñan)

SUMMARY

Phase I, Stage 1

	(Unit: thousand ₱)		
	<u>F.E.C</u>	<u>Local</u>	<u>Total</u>
Direct Construction Cost	25,702	20,737	46,439
Physical Cont. (8% of D.C.C.)	2,056	1,659	3,715
Sub Total	27,758	22,396	50,154
Leakage Detection	-	699	699
Detailed Design (10% of S.T. in Stage 1 & Stage 2)	4,836	4,835	9,671
Construction Supervision (4% of S.T.)	1,003	1,003	2,006
Total	33,597	28,933	62,530

Phase I, Stage 2

	(Unit: thousand ₱)		
	<u>F.E.C</u>	<u>Local</u>	<u>Total</u>
Direct Construction Cost	27,136	15,975	43,111
Physical Cont. (8% of D.C.C.)	2,171	1,278	3,449
Sub Total	29,307	17,253	46,560
Construction Supervision (4% of S.T.)	371	1,491	1,862
Total	29,678	18,744	48,422

Phase II

	(Unit: thousand ₱)		
	<u>F.E.C</u>	<u>Local</u>	<u>Total</u>
Direct Construction Cost	109,443	73,170	182,613
Physical Cont. (8% of D.C.C.)	8,755	5,854	14,609
Sub Total	118,198	79,024	197,222
Detailed Design (10% of S.T.)	9,861	9,861	19,722
Construction Supervision (4% of S.T.)	7,889	-	7,889
Total	135,948	88,885	224,833

The following tables show the breakdown of the project cost in each design year. The unit of all figures is thousand pesos. Project cost is further broken down into the Foreign Exchange Component and the Local Currency Component. Abbreviations in the tables are as follows:

COST	---	Construction Cost
C.FEC	---	Cost for Civil Work in the Foreign Exchange Component
C.DOM	---	Cost for Civil Work in the Local Currency Component
C.D.UNSKL	---	Cost for Unskilled Laborer of Civil Works in the Local Currency Component.
E.FEC	---	Cost for Equipments in the Foreign Exchange Component
E.DOM	---	Cost for Equipments in the Local Currency Component

$COST = C.FEC + C.DOM + E.FEC + E.DOM$

The exchange rates used in the cost estimates are as follows:

₱20 = \$1

\$1 = ¥155

No.	Cabuyao-Sta. Rosa-Binan ITEM	1988			1989			1990		
		COST	C.FEC	C.DMH	C.FEC	C.DMH	C.FEC	C.DMH	C.FEC	C.DMH
1.0	SOURCE FACILITY	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) DEEP WELL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) PUMPING FACILITY	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(3) Pumping Station	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2.0	TRANSMISSION FACILITIES	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Pipelines	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Pipe Protection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3.0	DISTRIBUTION FACILITIES	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Reservoir	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Pump Facility (Equip)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(3) Pump Facility (Civil)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Chlorination Facility	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(5) Electric Sub-station	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(6) Distribution pipes	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(7) Main Pipes	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(8) River Crossing	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(9) River Crossing Material	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(10) Valves	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(11) Internal Network	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(12) Service Connections	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(13) Water Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(14) Srvc Concn Riblin w/H	39.0	1.0	10.1	2.3	39.0	1.0	10.1	2.3	39.0
	(15) Srvc Concn Riblin w/H	355.0	3.6	35.5	10.7	301.7	14.2	301.7	14.2	355.0
	(16) Lateral Rehabilitation	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(17) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(18) Fire Protection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	500.0	4.6	45.6	13.0	522.1	17.7	522.1	17.7	500.0
4.0	Administration Bldg	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Administration Center	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5.0	Land Acquisition	180.0	4.6	45.6	13.0	522.1	17.7	522.1	17.7	180.0
	(1) Vehicle	60.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	60.0
	(2) Stored Material & Equipm't	120.0	4.6	45.6	13.0	522.1	17.7	522.1	17.7	120.0
	SUB-TOTAL	180.0	4.6	45.6	13.0	522.1	17.7	522.1	17.7	180.0
6.0	Leakage Detection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Deep Well Pump	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Chlorinator	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(3) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Water Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(5) Operation Center	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(6) Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(7) Stored Material & Equipm't	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
GRAND TOTAL		1813.0	4.6	45.6	13.0	831.3	319.5	831.3	319.5	1813.0

No.	Cabuyao-Sta. Rosa-Binan ITEM	1989			1990		
		COST	C.FEC	C.DMH	C.FEC	C.DMH	C.FEC
1.0	Deep Well Facilities	0.0	0.0	0.0	0.0	0.0	0.0
2.0	Transmission Facilities	0.0	0.0	0.0	0.0	0.0	0.0
3.0	Purification Plant	0.0	0.0	0.0	0.0	0.0	0.0
4.0	Reservoir	0.0	0.0	0.0	0.0	0.0	0.0
5.0	Disinfection Facilities	0.0	0.0	0.0	0.0	0.0	0.0
6.0	Electric Sub-station	0.0	0.0	0.0	0.0	0.0	0.0
7.0	Distribution Facilities	590.0	4.6	45.6	13.0	522.1	17.7
8.0	Service Connection	0.0	0.0	0.0	0.0	0.0	0.0
9.0	Admin. Bldg. & Equip. Ctr.	180.0	0.0	0.0	0.0	0.0	0.0
10.0	Land Acquisition	611.0	0.0	0.0	0.0	0.0	0.0
11.0	Vehicle & Stored Material	0.0	0.0	0.0	0.0	0.0	0.0
12.0	Replacement of Equipment	380.0	4.6	45.6	13.0	522.1	17.7
13.0	Leakage Detection	233.0	0.0	0.0	0.0	0.0	0.0
TOTAL		1614.0	4.6	45.6	13.0	831.3	319.5

No.	Cabuyao-Sia-Rosa-Binan ITEM	Phase I (Stage 1)				1991				1992							
		COST	C-FEC	C-BPM	C-D-UNSKI	E-FEC	E-BPM	C-FEC	C-D-UNSKI	E-FEC	E-BPM	C-FEC	C-D-UNSKI	E-FEC	E-BPM		
1.0	SOURCE FACILITY	0.0	0.0	0.0	0.0	0.0	0.0	1160.0	197.2	533.6	58.0	232.0	197.2	533.6	58.0	232.0	197.2
	(1) DEEP WELL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) PUMPING FACILITY	790.0	71.1	274.5	39.5	371.3	71.1	790.0	71.1	274.5	39.5	371.3	71.1	274.5	39.5	371.3	71.1
	(3) Pumping Station	62.0	0.0	0.0	0.0	62.0	0.0	62.0	0.0	0.0	0.0	62.0	0.0	0.0	0.0	62.0	0.0
	(4) Flow Meter	492.0	71.1	274.5	39.5	433.3	71.1	2012.0	268.3	810.1	97.5	665.3	268.3	810.1	97.5	665.3	268.3
	SUB-TOTAL	3268.0	588.2	915.0	130.7	1013.2	751.6	504.0	90.7	141.1	20.2	156.3	115.9	35.3	5.0	39.0	29.0
	(1) Pipelines	245.0	0.0	211.5	11.8	1013.2	23.5	504.0	0.0	141.1	20.2	156.3	0.0	0.0	0.0	39.0	29.0
	(2) Pipe Protection	3593.0	588.2	1126.5	142.5	0.0	775.1	0.0	90.7	0.0	0.0	0.0	115.9	35.3	5.0	0.0	0.0
	SUB-TOTAL	3417.0	888.4	2241.0	233.2	170.8	366.7	304.0	90.7	141.1	20.2	156.3	115.9	35.3	5.0	39.0	29.0
2.0	DISTRIBUTION FACILITIES	2090.0	0.0	0.0	0.0	1731.7	355.3	3084.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Reservoir	2090.0	0.0	0.0	0.0	1731.7	355.3	3084.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Pump Facility (Equip)	2090.0	0.0	0.0	0.0	1731.7	355.3	3084.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(3) Pump Facility (Civil)	2090.0	0.0	0.0	0.0	1731.7	355.3	3084.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Chlorination Facility	198.0	0.0	0.0	0.0	90.2	29.4	98.0	0.0	0.0	0.0	45.1	14.7	0.0	0.0	0.0	0.0
	(5) Electric Sub-station	3643.0	0.0	0.0	0.0	1912.5	364.3	3643.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(6) Distribution Pipes	14203.0	2556.6	3976.9	568.1	4402.8	3266.7	6241.0	1123.9	1748.3	249.8	1935.7	1436.1	0.0	0.0	0.0	0.0
	(7) Main Pipes	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(8) River Crossing	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(9) River Crossing Material	5516.0	110.3	118.4	331.0	3750.8	496.5	1578.0	31.6	331.4	94.7	1073.0	182.0	0.0	0.0	0.0	0.0
	(10) Valves	38.8	0.0	0.0	0.0	330.4	213.8	1762.0	281.9	493.4	70.5	589.1	387.6	493.4	70.5	589.1	387.6
	(11) Internal Network	4054.0	40.6	405.4	121.6	3445.8	162.2	2509.0	25.1	250.9	75.3	2132.6	100.4	250.9	75.3	2132.6	100.4
	(12) Service Connections	196.0	0.0	0.0	0.0	196.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(13) Water Meter	117.0	3.0	30.3	6.9	73.2	10.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(14) 8" Service Concnth Rhlth w/ft	1064.0	16.7	106.4	32.0	904.3	42.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(15) 8" Service Concnth Rhlth w/ft	860.0	146.2	240.8	34.4	279.5	193.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(16) Lateral Rehabilitation	226.0	0.0	0.0	0.0	226.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(17) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(18) Fire Protection	39220.0	4645.9	11785.6	1930.7	17517.0	5271.5	15275.0	1467.4	2957.3	403.2	8345.2	2665.1	744.3	145.8	213.7	388.0
	SUB-TOTAL	1583.0	0.0	0.0	0.0	563.9	221.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Administration Bldg	1583.0	0.0	0.0	0.0	563.9	221.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Operation Center	45158.0	5447.7	13837.6	2191.9	19553.4	6339.3	17791.0	1826.4	3808.5	610.9	9166.8	2989.3	1589.7	248.3	3436.0	785.3
	SUB-TOTAL	180.0	0.0	180.0	0.0	0.0	0.0	300.0	0.0	200.0	0.0	150.0	150.0	0.0	0.0	150.0	150.0
	Land Acquisition	600.0	0.0	0.0	0.0	600.0	300.0	300.0	0.0	0.0	0.0	204.1	38.9	0.0	0.0	71.4	13.6
	Vehicle	1281.0	0.0	0.0	0.0	470.8	80.2	243.0	0.0	200.0	0.0	354.1	188.9	0.0	0.0	221.4	163.6
	Stored Material & Equipment	699.0	0.0	0.0	0.0	730.8	380.2	713.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Leakage Protection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Deep Well Pump	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Chlorinator	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(3) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Water Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(5) Operation Center	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(6) Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(7) Stored Material & Equip.	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	47138.0	5447.7	14716.6	2191.9	20251.2	6719.5	18554.0	1826.4	4008.5	610.9	9520.9	3178.2	1589.7	248.3	3657.4	948.9
	GRAND TOTAL	47138.0	5447.7	14716.6	2191.9	20251.2	6719.5	18554.0	1826.4	4008.5	610.9	9520.9	3178.2	1589.7	248.3	3657.4	948.9

No.	Cabuyao-Sia-Rosa-Binan ITEM	Phase I (Stage 1)				1991				1992							
		COST	C-FEC	C-BPM	C-D-UNSKI	E-FEC	E-BPM	C-FEC	C-D-UNSKI	E-FEC	E-BPM	C-FEC	C-D-UNSKI	E-FEC	E-BPM		
1	Deep Well Facilities	852.0	71.1	216.5	39.5	433.3	71.1	2012.0	268.3	810.1	97.5	665.3	268.3	810.1	97.5	665.3	268.3
2	Transmission Facilities	3503.0	588.2	1126.5	142.5	1013.2	775.1	504.0	90.7	141.1	20.2	156.3	115.9	35.3	5.0	39.0	29.0
3	Purification Plant	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	Reservoir	3417.0	888.4	2221.1	239.2	170.8	136.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	Disinfection Facilities	196.0	9.8	66.6	5.9	90.2	29.4	98.0	4.9	33.3	2.9	45.1	14.7	0.0	0.0	0.0	0.0
6	Electric Sub-station	2643.0	564.7	801.5	72.9	1912.5	364.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	Distribution Facilities	2653.0	3128.7	8184.3	1452.2	10724.2	4525.8	12608.0	1437.4	2573.1	415.0	6167.5	2489.0	493.4	70.5	589.1	387.6
8	Service Connection	593.0	54.3	512.1	160.5	4619.3	215.3	2509.0	25.1	250.9	75.3	2132.6	100.4	250.9	75.3	2132.6	100.4
9	Adminl. Bldg. & Pge. Ctr.	1583.0	142.5	649.0	79.2	569.9	221.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10	Land Acquisition	180.0	0.0	180.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
11	Vehicle & Stored Material	1101.0	0.0	0.0	0.0	720.8	380.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12	Replacement of Equipment	4643.0	5447.7	14017.6	2191.9	20251.2	6719.5	18554.0	1826.4	4008.5	610.9	9520.9	3178.2	1589.7	248.3	3657.4	948.9
13	Leakage Protection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	TOTAL	97138.0	5447.7	14716.6	2191.9	20251.2	6719.5	18554.0	1826.4	4008.5	610.9	9520.9	3178.2	1589.7	248.3	3657.4	948.9

No.	ITEM	1993				1994				1995						
		CUST	C.FEC	C.DM	C.D. UNSK	E.FEC	F.DM	C.FEC	C.DM	C.D. UNSK	E.FEC	E.DM	C.FEC	C.DM	C.D. UNSK	E.DM
1.0	SOURCE FACILITY	1160.0	197.2	533.6	58.0	232.0	197.2	533.6	58.0	232.0	197.2	0.0	0.0	0.0	0.0	0.0
	(1) DEEP WELL															
	(2) PUMPING FACILITY	700.0	71.1	276.5	39.5	371.3	71.1	276.5	39.5	371.3	71.1	0.0	0.0	0.0	0.0	0.0
	1) Pumping Station	62.0	0.0	0.0	0.0	62.0	0.0	0.0	0.0	62.0	0.0	0.0	0.0	0.0	0.0	0.0
	2) Flow Meter	2012.0	268.3	810.1	97.5	665.3	268.3	810.1	97.5	665.3	268.3	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL															
2.0	TRANSMISSION FACILITIES	126.0	22.7	35.3	5.0	39.0	29.0	35.3	5.0	39.0	29.0	0.0	0.0	0.0	0.0	0.0
	(1) Pipelines	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Pipe Protection	126.0	22.7	35.3	5.0	39.0	29.0	35.3	5.0	39.0	29.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL															
3.0	DISTRIBUTION FACILITIES	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Reservoir	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(2) Pump Facility (Equip)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Pump Facility (Civil)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(3) Chlorination Facility	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Electric Sub-station	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(5) Distribution pipes	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	1) Main Pipes	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	2) River Crossing	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	3) River Crossing Material	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(4) Valves	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(5) Internal Network	1762.0	281.9	493.4	70.5	599.1	387.6	486.1	69.4	590.2	381.9	1715.0	274.4	480.2	68.6	583.1
	(6) Service Connections	2508.0	25.1	250.8	75.2	2131.8	100.3	250.8	75.2	2131.8	100.3	2508.0	25.1	250.8	75.2	2131.8
	(7) Water Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(8) Srvc Concntr Rbltn w/H	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(9) Srvc Concntr Rbltn w/H	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(10) Lateral Rehabilitation	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(11) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(12) Fire Protection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	4270.0	307.0	741.2	145.7	2730.9	987.9	736.9	143.6	2722.0	482.2	4223.0	299.5	731.0	143.8	2714.9
1.0	Administration Bldg	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	(1) Operation Center	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	6408.0	598.0	1589.6	248.2	3435.2	785.2	1564.6	244.6	3406.9	705.0	4223.0	299.5	731.0	143.8	2714.9
5.0	Land Acquisition	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Vehicle	300.0	0.0	0.0	0.0	150.0	0.0	0.0	0.0	150.0	0.0	0.0	0.0	0.0	0.0	0.0
	Stored Material & Equipment	85.0	0.0	0.0	0.0	71.4	13.6	81.0	0.0	70.6	13.4	63.0	0.0	0.0	53.8	10.2
	SUB-TOTAL	385.0	0.0	0.0	0.0	221.4	163.6	384.0	0.0	220.6	163.4	63.0	0.0	0.0	53.8	10.2
6.0	Leakage Detection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7.0	Replacement of Equipment	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	1) Deep Well Pump	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	2) Chlorinator	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	3) Flow Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	4) Water Meter	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	5) Operation Center	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	6) Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	7) Stored Material & Equip.	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	SUB-TOTAL	6793.0	598.0	1589.6	248.2	3435.2	785.2	1564.6	244.6	3406.9	705.0	4223.0	299.5	731.0	143.8	2714.9
	GRAND TOTAL															

No.	ITEM	1993				1994				1995						
		CUST	C.FEC	C.DM	C.D. UNSK	E.FEC	E.DM	C.FEC	C.DM	C.D. UNSK	E.FEC	E.DM	C.FEC	C.DM	C.D. UNSK	E.DM
1.0	Deep Well Facilities	126.0	22.7	35.3	5.0	39.0	29.0	35.3	5.0	39.0	29.0	0.0	0.0	0.0	0.0	0.0
	2) Transmission Facilities	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	3) Purification Plant	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	4) Reservoir	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	5) Disinfection Facilities	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	6) Electric Sub-station	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	7) Distribution Facilities	1762.0	281.9	493.4	70.5	599.1	387.6	486.1	69.4	590.2	381.9	1715.0	274.4	480.2	68.6	583.1
	8) Service Connection	2508.0	25.1	250.8	75.2	2131.8	100.3	250.8	75.2	2131.8	100.3	2508.0	25.1	250.8	75.2	2131.8
	9) Admin. Bldg. & Upe. Ctr.	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	10) Land Acquisition	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	11) Vehicle & Stored Material	385.0	0.0	0.0	0.0	221.4	163.6	384.0	0.0	220.6	163.4	63.0	0.0	0.0	53.8	10.2
	12) Replacement of Equipment	6793.0	598.0	1589.6	248.2	3435.2	785.2	1564.6	244.6	3406.9	705.0	4223.0	299.5	731.0	143.8	2714.9
	SUB-TOTAL															
	13) Leakage Detection	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	TOTAL	6793.0	598.0	1589.6	248.2	3435.2	785.2	1564.6	244.6	3406.9	705.0	4223.0	299.5	731.0	143.8	2714.9



APPENDIX 8.2.1.C OPERATION AND MAINTENANCE COST BY PHASE (Cabuyao-Sta. Rosa-Bifian)  
 (Unit; thousand pesos)

Item	Phase I			Phase II
	Stage 1	Stage 2		
	Cost	Cost	Cost	
Operation & Maintenance Cost				
Salary	1,360 ₱/M.M	539	1,322	2,301
Power	0.25 ₱/kWh	25	506	2,022
Chemical	27 ₱/kg	99	260	772
Miscellaneous		814	2,191	6,383
Maintenance		609	1,792	5,071
Total		2,086	6,071	16,549

APPENDIX 9.3.1 MARKET SURVEY

The market survey was conducted by interviews to the residents in the study area using the LWUA's interview sheet as per attached in the end of this section.

The total number of respondents and its estimated coverage ratio to the total number of households in the study area are as follows:

<u>Total Number of Respondents</u>	<u>Estimated Total Household</u>	<u>Coverage Ratio to Total Household</u>
7,291	23,725	31%

The results of the market survey are shown in TABLEs 9.3.1., 9.3.2. and 9.3.3.

From the market survey, the income distribution of the respondents is shown as follows:

<u>Income Bracket<sup>1/</sup></u>	<u>Cabuyao</u>		<u>Sta. Rosa</u>		<u>Binan</u>		<u>Total</u>	
	<u>Ave. Pesos</u>	<u>Number</u>	<u>Ave. Pesos</u>	<u>Number</u>	<u>Ave. Pesos</u>	<u>Number</u>	<u>Ave. Pesos</u>	<u>Number</u>
₱900 and below	679	160	696	703	592	1,407	630	2,270
₱901 to ₱1500	1,212	321	1,203	940	1,205	1,230	1,205	2,491
₱1,501 to ₱2,500	2,152	157	2,130	554	2,099	637	2,118	1,348
₱2,501 to ₱4,500	3,467	98	3,312	274	3,340	389	3,346	706
₱4,501 and above	8,338	13	5,357	28	5,798	78	5,972	119

<sup>1/</sup> Residential, excluding no-income and no-answer



TABLE 9.3.1 MARKET SURVEY SUMMARY  
(Cabuyao)

Total Number of Respondents: 755

1. Distribution According to Building Type

	No.	%
a. Residential	755	100.00
b. Commercial	0	0.00
c. Industrial	0	0.00

2. Distribution According to Source of Water

	No.	%
a. Connected to System	288	38.15
b. Neighbor's Connection	70	9.27
c. Public Faucet	93	12.32
d. Private System	300	39.74
e. Water Vendor	0	0.00
f. Others	4	0.53

3. Average Persons Per Household

a. Residential / Number of Sample	5.73	/	748
b. Commercial / Number of Sample	0.00	/	0
c. Industrial / Number of Sample	0.00	/	0

4. Willingness To Connect (%)

	Residential	Commercial	Industrial	Total
a. Yes	37.22	0.00	0.00	37.22
b. No	24.64	0.00	0.00	24.64
c. Undecided	0.00	0.00	0.00	0.00
d. W/ Own Conn.:	38.15	0.00	0.00	38.15

5. Average Monthly Water Needs

Type / Number of Sample	Residential	Commercial	Industrial
a. Kerosene Can / 167	13.01	0.00	0.00
b. Drum / 578	2.99	0.00	0.00
c. Gallon / 3	4.33	0.00	0.00
d. Others / 1	25.00	0.00	0.00

6. Ave. Monthly Electric Bills for Residential Users (PESO): 58.84  
Number of Effective Respondents : 740

7. Income Distribution

( Residential, Excluding No-Income and No-Answer )

	AVE. PESO	NUMBER
a. P900 and Below	679	160
b. P901 to P1500	1212	321
c. P1501 to P2500	2152	157
d. P2501 to P4500	3467	98
e. P4501 and Above	8338	13

TABLE 9.3.2 MARKET SURVEY SUMMARY  
(Sta.Rosa)

Total Number of Respondents: 2594

1. Distribution According to Building Type					
		No.		%	
a.	Residential	: 2524		97.30	
b.	Commercial	: 67		2.58	
c.	Industrial	: 3		0.12	
2. Distribution According to Source of Water					
		No.		%	
a.	Connected to System	: 619		23.86	
b.	Neighbor's Connection	: 343		13.22	
c.	Public Faucet	: 609		23.48	
d.	Private System	: 1016		39.17	
e.	Water Vendor	: 1		0.04	
f.	Others	: 6		0.23	
3. Average Persons Per Household					
a.	Residential / Number of Sample	: 5.98 /	2524		
b.	Commercial / Number of Sample	: 6.61 /	67		
c.	Industrial / Number of Sample	: 16.67 /	3		
4. Willingness To Connect (%)					
		Residential	Commercial	Industrial	Total
a.	Yes	: 44.06	32.84	100.00	43.83
b.	No	: 31.58	43.28	0.00	31.84
c.	Undecided	: 0.44	1.49	0.00	0.46
d.	W/ Own Conn.:	23.93	22.39	0.00	23.86
5. Average Monthly Water Needs					
	Type / Number of Sample	Residential	Commercial	Industrial	
a.	Kerosene Can / 550	: 11.76	22.26	0.00	
b.	Drum / 2020	: 3.24	6.03	4.33	
c.	Gallon / 16	: 10.56	0.00	0.00	
d.	Others / 5	: 2.10	0.00	0.00	
6. Ave. Monthly Electric Bills for Residential Users (PESO): 51.42 Number of Effective Respondents : 2506					
7. Income Distribution ( Residential, Excluding No-Income and No-Answer )					
		AVE.PESO		NUMBER	
a.	P900 and Below	: 696		703	
b.	P901 to P1500	: 1203		940	
c.	P1501 to P2500	: 2130		554	
d.	P2501 to P4500	: 3312		274	
e.	P4501 and Above	: 5357		28	

TABLE 9.3.3 MARKET SURVEY SUMMARY  
(Biñan)

Total Number of Respondents: 3942

1. Distribution According to Building Type

	No.	%
a. Residential	: 3838	97.36
b. Commercial	: 101	2.56
c. Industrial	: 3	0.08

2. Distribution According to Source of Water

	No.	%
a. Connected to System	: 557	14.13
b. Neighbor's Connection	: 627	15.91
c. Public Faucet	: 1231	31.23
d. Private System	: 1512	38.36
e. Water Vendor	: 0	0.00
f. Others	: 15	0.38

3. Average Persons Per Household

a. Residential / Number of Sample	: 6.07 /	3833
b. Commercial / Number of Sample	: 6.33 /	100
c. Industrial / Number of Sample	: 12.67 /	3

4. Willingness To Connect (%)

	Residential	Commercial	Industrial	Total
a. Yes	: 49.56	41.58	33.33	49.34
b. No	: 35.36	26.73	0.00	35.11
c. Undecided	: 1.43	0.99	0.00	1.42
d. W/ Own Conn.:	13.65	30.69	66.67	14.13

5. Average Monthly Water Needs

Type / Number of Sample	Residential	Commercial	Industrial
a. Kerosene Can / 794	: 10.46	15.74	0.00
b. Drum / 2898	: 3.30	5.62	6.00
c. Gallon / 141	: 22.08	9.50	0.00
d. Others / 2	: 4.50	0.00	0.00

6. Ave. Monthly Electric Bills for Residential Users (PESO): 53.25  
Number of Effective Respondents : 3753

7. Income Distribution  
( Residential, Excluding No-Income and No-Answer )

	AVE. PESO	NUMBER
a. P900 and Below	: 592	1407
b. P901 to P1500	: 1205	1230
c. P1501 to P2500	: 2099	637
d. P2501 to P4500	: 3340	389
e. P4501 and Above	: 5798	78

The existing sources of water of the respondents and their willingness to connect to each source of water are indicated below :

Sources of Water	Distribution	Willingness to Connect	
		Yes	No
	%	%	%
(1) Cabuyao			
Connected to System	38	-	-
Neighbor's Connection	9	61	39
Public Faucet	12	72	28
Private System	40	56	44
(2) Sta. Rosa			
Connected to System	24	-	-
Neighbor's Connection	13	66	33
Public Faucet	23	57	42
Private System	39	55	44
(3) Binan			
Connected to System	14	-	-
Neighbor's Connection	16	73	26
Public Faucet	31	58	41
Private System	38	51	47
(4) Total			
Connected to System	20	-	-
Neighbor's Connection	14	70	29
Public Faucet	27	58	41
Private System	39	53	46

The respondents' major sources of water are private system and public faucet, while the other respondents are connected to the system, some are dependent in their neighbours connection for their water needs. In addition, 0.4% of the respondents depend on the water vendors and others for their water sources. The above table shows that the respondents are willing to connect to the waterworks system.

The distribution of water sources and the respondents' willingness to connect according to income bracket are also obtained from the market survey as shown in TABLE 9.3.4.

TABLE 9.3.4 DISTRIBUTION OF WILLINGNESS TO CONNECT BY INCOME BRACKET

Sources of Water	Income Bracket				
	₱900 & below	₱901- ₱1,500	₱1,501- ₱2,500	₱2,501- ₱4,500	₱4,501- & above
<b>(1) Cabuyao</b>					
Connected to System	34 %	38 %	75 %	34 %	8 %
Neighbor's Connection	13	10	6	7	8
Public Faucet	26	12	4	3	8
Private System	26	40	41	56	77
<b>Willingness to Connect</b>					
Yes	34	39	31	45	62
No	32	23	22	21	31
Undecided	0	0	0	0	0
With Own Connection	34	38	47	34	7
<b>(2) Sta. Rosa</b>					
Connected to System	14	24	30	33	41
Neighbor's Connection	21	10	11	9	0
Public Faucet	35	25	16	8	6
Private System	29	40	43	49	53
<b>Willingness to Connect</b>					
Yes	45	41	46	44	32
No	40	34	24	22	26
Undecided	1	0	1	1	0
With Own Connection	14	25	29	33	42
<b>(3) Binan</b>					
Connected to System	6	15	20	26	20
Neighbor's Connection	21	16	12	9	4
Public Faucet	47	28	18	11	15
Private System	25	41	50	53	60
<b>Willingness to Connect</b>					
Yes	50	49	51	47	45
No	42	34	27	26	33
Undecided	1	1	2	1	2
With Own Connection	7	16	20	26	20

As a result of the market survey, the respondents' willingness and unwillingness to connect is summarized as follows :

<u>Answer</u>	<u>Cabuyao</u>	<u>Sta. Rosa</u>	<u>Binan</u>
Yes	37 %	44 %	49 %
No	25	32	35
With own connection	38	24	14

Note : With respect to type of users, residential users account for 100% in Cabuyao, 97% in Sta.Rosa, and 97 % in Binan, Respectively.

It is observed from the results of the survey that the majority of the respondents in the three municipalities who are not yet connected to the existing system are willing to connect to the waterworks system. It is expected therefore that more residents in the Cabuyao-Sta.Rosa-Binan will connect to the new water supply system when it is expanded.

INTERVIEW SHEET FORMAT USED IN THE MARKET SURVEY

BLOCK NO.	CITY/MUNICIPALITY				BARANGAY			ZONE		STREET	
	Building Type		User Code	PPH	Willing to Connect YES NO	Ave. Water Needs 2/	Ave. Electric Bill	Average Monthly Family Income		Bldg. Cond. Code	Respondent
	Res	Com Ind						P900 and Below	P901 to P1,500		

1/ USER CODE  
 C - Connected to System  
 HC - Neighbor's Connection  
 PF - Public faucet  
 PS - Private System (Elec./Hand Pump)  
 V - Water Vendor  
 O - Others, specify

2/ Consumption Code  
 KC - Kero-Gan or Gray Container  
 D - Drum  
 G - Gallon  
 O - Others

3/ Building Condition Code  
 A - Very Good  
 B - Good  
 C - Fair  
 D - Poor

I certify that the above information are true and correct.

Interviewer

#### APPENDIX 9.7.1 FINANANCIAL INTERNAL RATE OF RETURN (FIRR)

In the calculation of Financial Internal Rate of Return (FIRR), the following two indicators are normally used to evaluate financial profitability of a project.

(1) Internal Rate of Return on Investment (IRROI)

The term IRROI indicates the internal rate of return on total capital investment, and assesses the profitability of the Project as a whole and the ability to recover funds invested in the Project.

The IRROI is calculated based on the assumption that the total capital investment is covered by its own capital. Therefore, the financial conditions such as the loan conditions on borrowed capital, changes on the ratio of equity to total capital requirement and others have no effect on the IRROI. Accordingly, the IRROI indicates the profitability of the Project itself.

(2) Internal Rate of Return on Equity (IRROE)

The term IRROE indicates the internal rate of return on equity, and assesses the profitability only with respect to equity and the ability to recover funds invested in the Project as equity. Here, the IRROE is calculated on the basis of such financial conditions proper to the Project as the loan conditions on borrowed capital and amount of capital owned.

In this study, the FIRR was calculated using the same method applied in the study report of the BACOLOD CITY WATER DISTRICT PHASE II WATER SUPPLY FEASIBILITY STUDY, DRAFT REPORT VOLUME 3 by LWUA.



APPENDIX 9.8.1 FINANCIAL RECOMMENDATION

The proposed water rates for 1/2 inch connections of commercial users, and 3/4 inch connections of domestic and commercial users to achieve financial self-sufficiency are as follows :

(1) Water rate for 1/2 inch connections of commercial users

Period	Rate/ Unit	First 10cu.m	11-20cu.m	21-35cu.m	Above 35cu.m
1988	P0.9	P 45.0	P 5.6	P 7.2	P 9.4
1989	1.3	65.0	8.2	10.4	13.6
1990	1.3	65.0	8.2	10.4	13.6
1991	1.9	95.0	11.8	15.2	20.0
1992	1.9	95.0	11.8	15.2	20.0
1993	2.4	120.0	15.0	19.2	25.2
1994	2.4	120.0	15.0	19.2	25.2
1995	2.7	135.0	16.8	21.6	28.4
1996	2.7	135.0	16.8	21.6	28.4
1997	3.5	175.0	21.8	28.0	36.8

(2) Water rate for 3/4 inch connection of domestic users

Period	Rate/ Unit	First 10cu.m	11-20cu.m	21-35cu.m	Above 35cu.m
1988	P0.9	P 36.0	P 4.5	P 5.8	P 7.5
1989	1.3	52.0	6.6	8.3	10.9
1990	1.3	52.0	6.6	8.3	10.9
1991	1.9	76.0	9.4	12.2	16.0
1992	1.9	76.0	9.4	12.2	16.0
1993	2.4	96.0	12.0	15.4	20.2
1994	2.4	96.0	12.0	15.4	20.2
1995	2.7	108.0	13.4	17.3	22.7
1996	2.7	108.0	13.4	17.3	22.7
1997	3.5	140.0	17.4	22.4	29.4

(3) Water rate for 3/4 inch connection of commercial users

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<u>Period</u>	<u>Rate/ Unit</u>	<u>First 10cu.m</u>	<u>11-20cu.m</u>	<u>21-35cu.m</u>	<u>Above 35cu.m</u>
1988	P0.9	P 72.0	P 9.0	P11.6	P15.0
1989	1.3	104.0	13.2	16.6	21.8
1990	1.3	104.0	13.2	16.6	21.8
1991	1.9	152.0	18.8	24.4	32.0
1992	1.9	152.0	18.8	24.4	32.0
1993	2.4	192.0	24.0	30.8	40.3
1994	2.4	192.0	24.0	30.8	40.3
1995	2.7	216.0	26.8	34.6	45.4
1996	2.7	216.0	26.8	34.6	45.4
1997	3.5	280.0	34.8	44.4	58.8

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バヨンボン-ソラノ, ヌエバ・ビスカヤ県



1. Residential Consumers

First 15 kwh	₱ 31.80 per month (Minimum Charges)
Over 15 kwh	₱ 2.24 per kwh

2. Commercial Consumers

First 15 kwh	₱ 43.20 per month (Minimum Charges)
Over 15 kwh	₱ 2.28 per kwh

3. Industrial Consumers

Demand Charges	₱ 15.00 per kwh
Plus Energy Charge	₱ 2.11 per kwh

As one of the factors for evaluation of the present status of transmission/distribution pipelines, the estimated "C" value provides an important information to determine the capacity of such pipelines to deliver water. Furthermore, it can contribute to the development of a plan of water supply system associated with the determination of pipe diameter.

There are three (3) major transmission/distribution lines in the study area.

Location:

Pipe Material & Length:

a. Borrobbob Spring to reservoir	Ø200 mm, CCI, 4,300 m
b. Reservoir to Bayombong	Ø250 mm, CCI, 1,200 m
c. Bayombong to Solano	Ø250 mm, CCI, 5,000 m

Since all of these pipelines were installed at the time of inauguration of the waterworks, their general backgrounds are more or less the same. Thus the "C" value survey was carried out in a certain representative span of the pipeline, which can follow the selected criteria below:

- a. Leakage shall be minimal and easily measured.
- b. No service pipe is connected within the survey span.
- c. Water pressure can be measured easily.
- d. Ground elevation in the survey span is known or can be surveyed.

Through the reconnaissance survey, all the said pipeline routes were evaluated with the above criteria. Only the span from the Borrobbob Spring up to Barangay Masoc with approximate length of 1,400 m was identified. The survey section is shown in FIGURE 4.2.1.1.

Due to the lack of electric supply in the survey area, two (2) units of 12 V car battery was utilized for the power supply.

This "C" valve survey was carried out simultaneously with the topographic survey of the subject pipeline from the spring to the reservoir to measure elevations of the two points and the distance between them.

Considering the time lag between the two (2) survey points, the Ultrasonic Flow Meter was initially installed at the downstream side (Masoc) until the measurement at the upstream side (spring) was completed.

The results of the field measurement and the "C" valve estimation are presented in FIGURE 4.2.1.2.

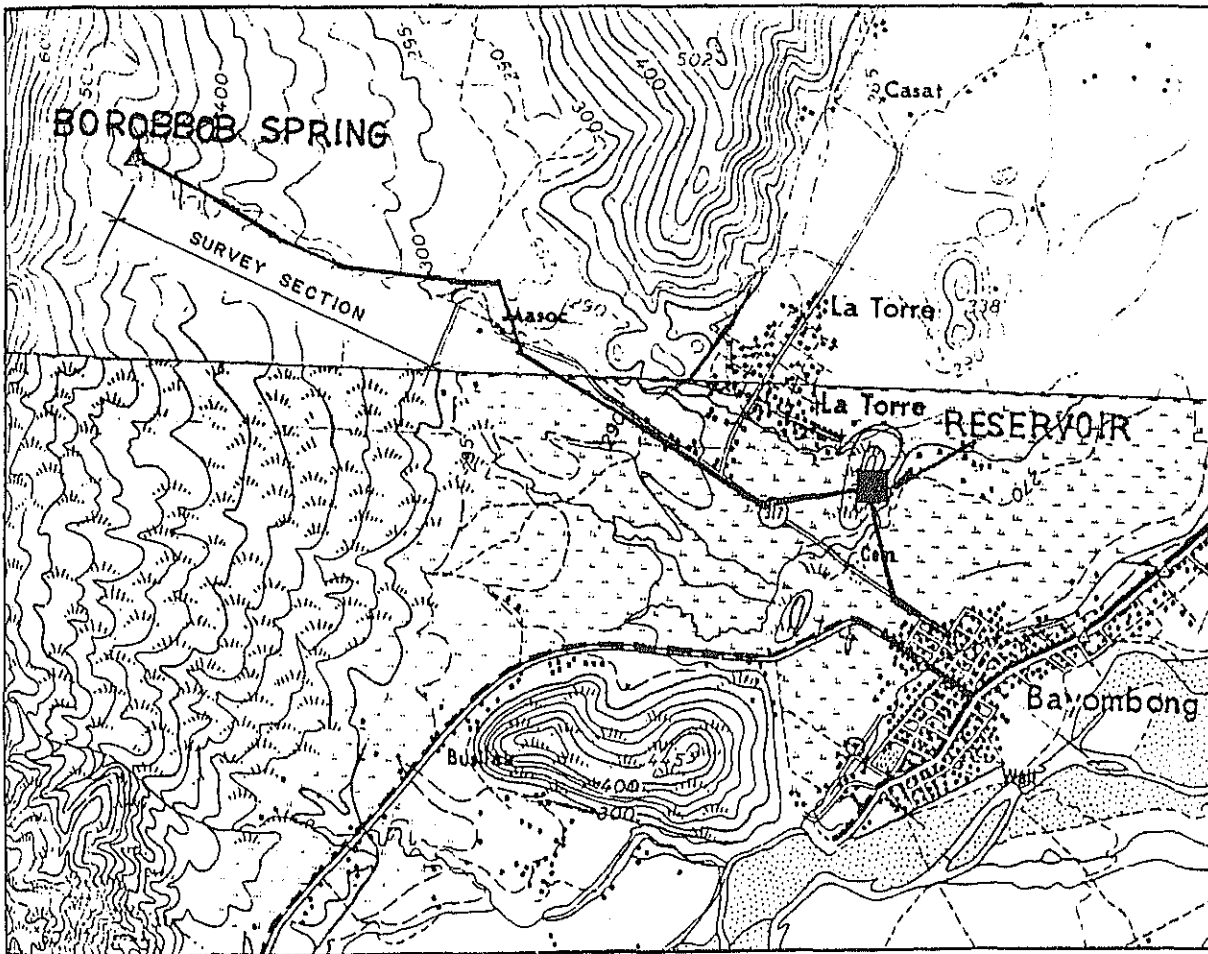
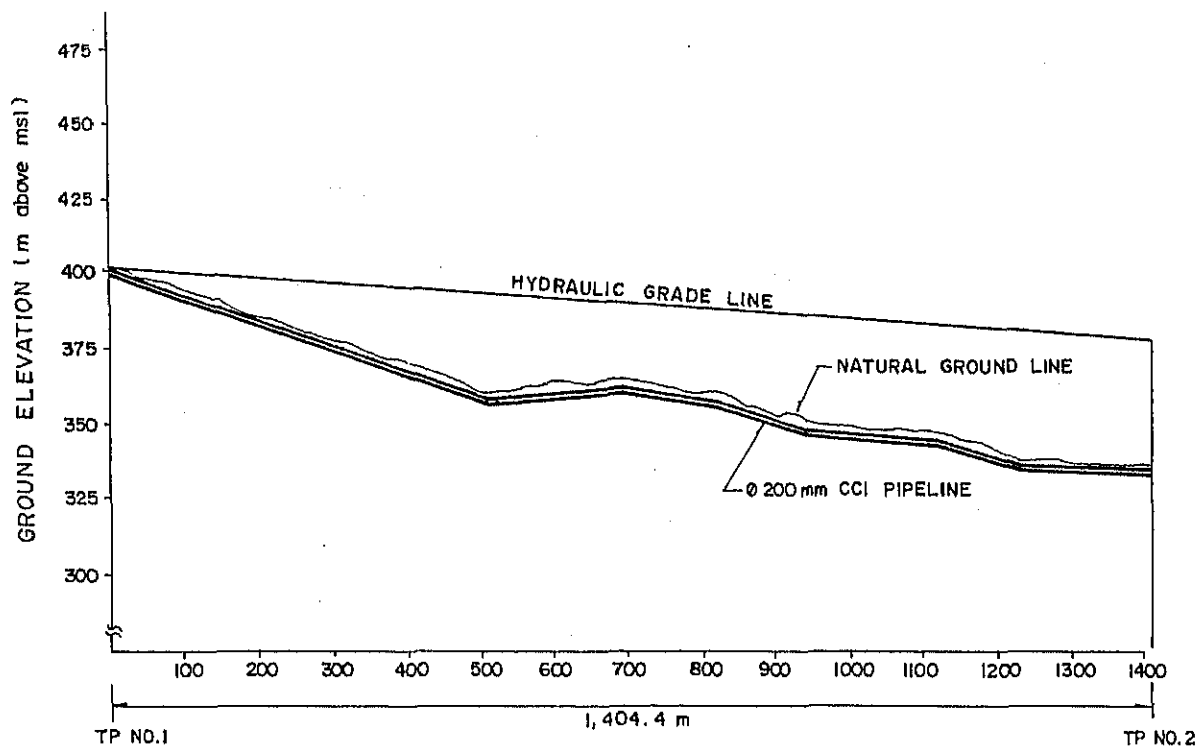


FIGURE 4.2.1.1 LOCATION OF SURVEY SECTION



TP NO.1

$Q = 200 \text{ m}^3/\text{hr} = 0.0556 \text{ m}^3/\text{sec}$

ELEVATION = 402.6 m above MSL  
(Center of pipe)

PRESSURE HEAD = 0 m

PIPE DIAMETER = 200 mm

DATE OF TEST = 16 JULY 1986

TP NO.2

$Q = 197 \text{ m}^3/\text{hr} = 0.0547 \text{ m}^3/\text{sec}$

ELEVATION = 336.0 m above MSL  
(Center of pipe)

PRESSURE HEAD =  $4.3 \text{ kg/cm}^2 = 43 \text{ m}$

PIPE DIAMETER = 200 mm

DATE OF TEST = 16 JULY 1986

COMPUTATION:

$$C = 3.59028 \times Q \times D^{-2.63} \times H_{\text{LOSS}}^{-0.54} \times L^{0.54}$$

THUS;

$$C = 3.59028 \times (5.52 \times 10^{-2}) \times 0.2^{-2.63} \times 23.6^{-0.54} \times 1404.4^{0.54}$$

$C = 125$

WHERE:

$Q_{\text{AVE}} = 5.52 \times 10^{-2} \text{ m}^3/\text{sec}$

$D = 0.2 \text{ m}$

$H_{\text{LOSS}} = 402.6 - 336.0 - 43$   
 $= 23.6 \text{ m}$

$L = 1,404.4 \text{ m}$

FIGURE 4.2.1.2  
ESTIMATION OF "C" VALUE



To determine the measuring points, the following conditions were taken into account:

- a. Measuring points were selected so as to develop a contour line of water pressure in the survey area.
- b. Several points along the major distribution lines were so selected to monitor the variation of water pressure through the day in relation to the water use.

Considering the above mentioned conditions, a total of 27 measuring points; 12 points for Bayombong and 15 points for Solano, were identified through the preliminary survey.

Since the survey area consists of two towns, the field measurements was carried out one after another.

The highest and lowest water pressures through the day obtained from the field measurement are presented in TABLES 4.2.2.1 and 4.2.2.2.

From the field measurement result, the following facts are obtained:

- a. The highest water pressure was observed at 0.49 kg/sq. cm during the peak demand hours in Bayombong and 0.35 kg/sq.cm during the less demand hours in Solano, respectively.
- b. The lowest data was 0 kg/sq. cm both in Bayombong and Solano.

TABLE 4.2.2.1 WATER PRESSURE IN BAYOMBONG:

Measuring Point	LOCATION	GL (m)	HIGHEST PRESSURE		LOWEST PRESSURE	
			kg/sq.cm	Time Range	kg/sq.cm	Time Range
1	Corner Ponce St./ Nat'l Road	+ 269.29	0.21	7:00-12:00	0	22:00
2	Corner Squing/ Nat'l Road	+ 270.56	0.35	6:00-12:00	0	21:00- 1:00
3	Along Luna St.	+ 269.76	0.35	8:00-13:00	0.11	22:00-16:00
4	Corner Nat'l Rd/ Mkt.	+ 271.00	0.49	9:00	0.07	18:00- 5:00
5	San Nicolas St.	+ 273.00	0.14	9:00-13:00	0.107	18:00-20:00
6	Corner Gonong St./ Rizal St.	+ 274.10	0.07	constant	0.07	constant
7	Corner Mabini St./ Rizal St.	+ 273.07	0.14	0:00-14:00	0.07	19:00- 6:00
8	Corner Sgt. B. Peres St./ Rizal St.	+ 272.02	0.21	8:30-12:00	0	19:00- 6:00
9	Corner Burgos St./ Gadingan St.	+ 271.34	0.32	8:00-12:00	0.04	21:00- 5:00
10	Corner Burgos St./ Zulueta St.	+ 270.66	0.21	7:00-12:00	0.11	18:00- 4:00
11	Corner Rizal St./ Ponce St.	+ 269.35	0.11	9:30-11:30	0.04	17:00- 8:00
12	Corner Gomez -St./ Burgos St.	+ 272.51	0.35	6:00- 7:00	0	13:00-19:00

TABLE 4.2.2.2 WATER PRESSURE IN SOLANO:

Measuring Point	LOCATION	GL (m)	HIGHEST PRESSURE		LOWEST PRESSURE	
			kg/sq.cm	Time Range	kg/sq.cm	Time Range
1	Boundary-Bayombong/ Solano	+ 256.60	0.15	10:00-13:00	0.04	12:00- 6:00
2	Corner Bintacan/ Layebana	+ 250.05	0.28	12:00-22:00	0.12	12:00- 5:00
3	Corner Homapa/ Gaddang	+ 250.24	0.21	14:00-23:00	0.13	12:00-10:00
4	Corner Layebana/ Cementerio	+ 250.98	0.35	16:00-22:00	0.14	5:00- 6:00
5	Corner Aratal/ Layebana	+ 251.50	0.21	14:00-21:00	0.11	23:00- 9:00
6	Corner Layebana/ Yogad	+ 252.88	0.29	13:00-23:00	0.04	9:00-12:00
7	Near to corner Burgos /Lumabang	+ 254.41	0.26	10:00-14:00	0	12:00
8	Corner Burgos/ Nat'l Rd.	+ 254.93	0.21	16:00-21:00	0.11	23:00- 6:00
9	Corner Burgos/ Gaddang	+ 253.58	0.21	10:00-14:00 15:00-20:00	0.07	22:00
10	Corner Gaddang/ Mabini	+ 253.23	0.14	11:00-24:00	0	0:00- 6:00
11	Corner Washington/ Bontal	+ 253.00	0.11	16:00-21:00	0	12:00-15:00
12	Corner Bonifacio/ Bacarmo	+ 252.05	0.21	15:00-22:00	0.07	0:00- 5:00
13	Corner Lumabang/ Washington	+ 253.17	0.11	20:00-21:00	0	23:00- 5:00
14	Corner Mabini/ Cemetery	+254.24	0.17	13:00-16:00	0.07	16:00- 6:00

#### APPENDIX 4.3.1 DISCHARGE RATE OF SPRING

The collected water at the spring box flows into the transmission line. However, it was confirmed during Phase I survey that the water which overflowed and leaked from the spring box flowed into nearby streams.

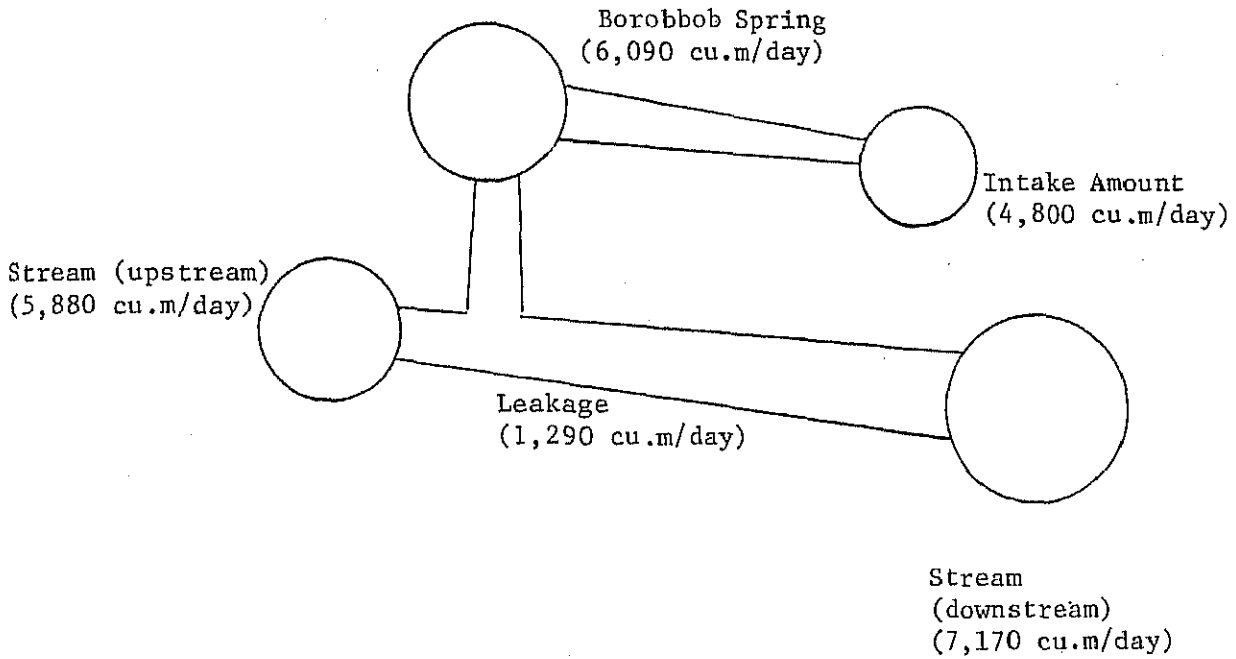
The measurement of flow rate was conducted to cover the total water collected at the spring. The following are the manner of measurement done in consideration of the present conditions at the spring site.

- 1) When there was an overflow from the spring box, the flow rate was estimated by measuring the overflow depth at the top of the spring box.
- 2) The total water volume which flowed into the nearby stream was estimated using the flow rates of the nearby stream at two points before and after joining the water from the spring box. The flow rate of the stream at two points was measured by means of a current meter.
- 3) The intake amount for water supply was measured at the outlet of the spring box (8-inch diameter, CCI) using the Ultrasonic Flow Meter.

The interval of flow rate measurement at the outlet pipe of the spring had been scheduled at every one (1) minute for an approximately one (1) hour duration so as to obtain an average intake amount.

The intake amount measured is approximately 4,800 cu.m/day (July, 1986) which corresponds to 4,740 cu.m/day measured in the Phase I survey. On the other hand, there was no overflow from the spring box, except the leakage at an amount of 1,290 cu.m/day. Thus, the total discharge of the Borrobbob Spring is considered to be approximately 6,090 cu.m/day. The spring discharge and the intake amount are illustrated in FIGURE 4.3.1.1.

FIGURE 4.3.1.1 WATER BALANCE AT BOROBBOB SPRING (As of July, 1986)



The total discharge of 6,090 cu.m/day is being considered as the minimum potential discharge at the end of the dry season because the actual start of the rainy season in the study area was delayed at the time of the field survey.

The same measurement conducted in April 1986 resulted that about 6,260 cu.m/day overflowed and leaked from the spring box.

APPENDIX 4.5.1 SURVEY FOR ESTIMATION OF UNACCOUNTED-FOR  
WATER/NOT UTILIZED WATER

The following conditions were considered for the selection of a sample study area:

- a) The area shall be a representative of the whole study area so as to reflect the survey output in the entire service area.
- b) The volume of transmitted/distributed water and water consumption/ accounted-for-water in the selected area should be measured/ estimated at a reasonable level of reliability.

On the other hand, there are several constraints as identified during the Phase I Study for implementation of this survey as follows:

- a) No quantitative data/record on the water transmission/distribution and the water consumption is available from the Provincial Water-Works Office.
- b) Two (2) different groups of water supply services have been practiced in the existing system;
  - Barangays Masoc and La Torre, which are rural barangays of Bayombong municipality, are served on a continuous 24-hour basis with high water pressure (3.8 kg/sq.cm to 4.3 kg/sq. cm). Service pipes are directly connected to the transmission line between the section of the Borobbob Spring and the reservoir.
  - The town proper of Bayombong and Solano are supplied from the reservoir via their respective transmission line for 14 hours a day (4:00 AM to 9:00 PM).

Under the above mentioned conditions, the scope of the survey for this subject was determined to cover the entire service area. The study of un-accounted-for water/not utilized water accordingly included some sections of transmissison/distribution lines and four service areas as enumerated

below.

- a) Masoc/La Torre area along the transmission line
- b) Town proper of Bayombong
- c) Bonfal area along the transmission line
- d) Town proper of Solano

The section of transmission line between the spring and the reservoir was specially taken into account based on the result of analysis in the Phase I survey. It was identified that some 660 cu. m/day was distributed to the area in barangay Masoc and La Torre along the transmission line. The distributed amount corresponds to the water consumption for more than 400 households based on a common consumption per household. However there are only 118 connections in the said area. In this connection, the study of not utilized water in this section was a highlight among others.

A total of eight flow rate measuring points were selected along the transmission/distribution line from the spring to the proper of Solano to cover the above mentioned study areas. In other words, those points that are inlet and outlet of the clustered areas/town proper served by the Waterworks in addition to the outlet of the spring and the reservoir; were covered as shown in FIGURE 4.5.1.1. TABLE 4.5.1.1 presents detailed information on the measuring points.

TABLE 4.5.1.1 MEASURING POINTS OF FLOW RATE

<u>Survey Point No.</u>	<u>Purpose</u>	<u>Location</u>
1	Intake Volume	Ø 200 mm CCI pipe at 10 m downstream from springbox (exposed pipe)
2	Transmitted volume from spring to entrance of Brgy. Masoc	Ø 200 mm CCI pipe at Masoc Ranch Guard House (exposed pipe)
3	Distributed volume to Brgy. Masoc	Ø 200 mm CCI pipe near boundary of Brgys. Masoc and La Torre (excavated pipe)
4	Transmitted volume to the reservoir	Ø 200 mm CCI pipe at the entrance of reservoir (exposed pipe)
5	Transmitted volume from reservoir	Ø 300 mm CCI pipe near outlet of reservoir

6	Transmitted volume to Solano area and part of Bayombong	ø259 mm CCI pipe at river crossing in Bayombong town proper (exposed pipe)
7	Transmitted volume to Solano area and Brgy. Bonfal of Bayombong	ø250 mm CCI pipe at highway junction (exposed pipe)
8	Transmitted volume to Solano area	ø250 mm CCI pipe near municipal boundary of Bayombong and Solano (exposed pipe)

A supplementary investigation on the unit water consumption in the absence of water meter through the area was planned to get information on the present water consumption. The actual water consumption per connection was field measured using two sets of water meter. The survey was conducted in the three major service areas; Masoc/La Torre, Bayombong and Solano areas, having installed the water meter at about random- selected 10 connections in each area for one day measurement of water consumption. An interview with the concessionaires was also supplemented to collect additional information on the actual population served per connection.

A flow chart for estimation of unaccounted-for-water/not utilized water using the survey result is shown in FIGURE 4.5.1.2.



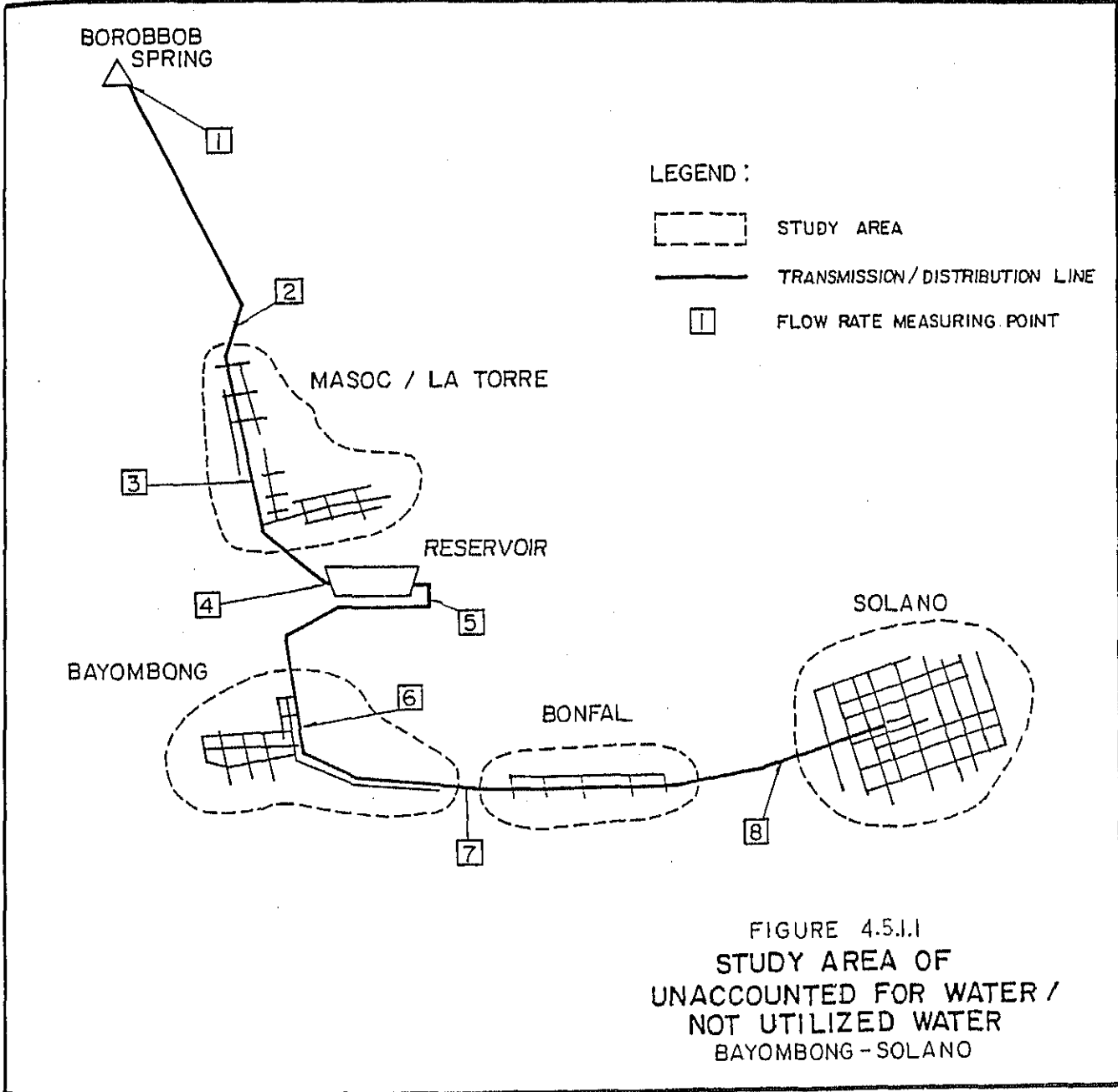
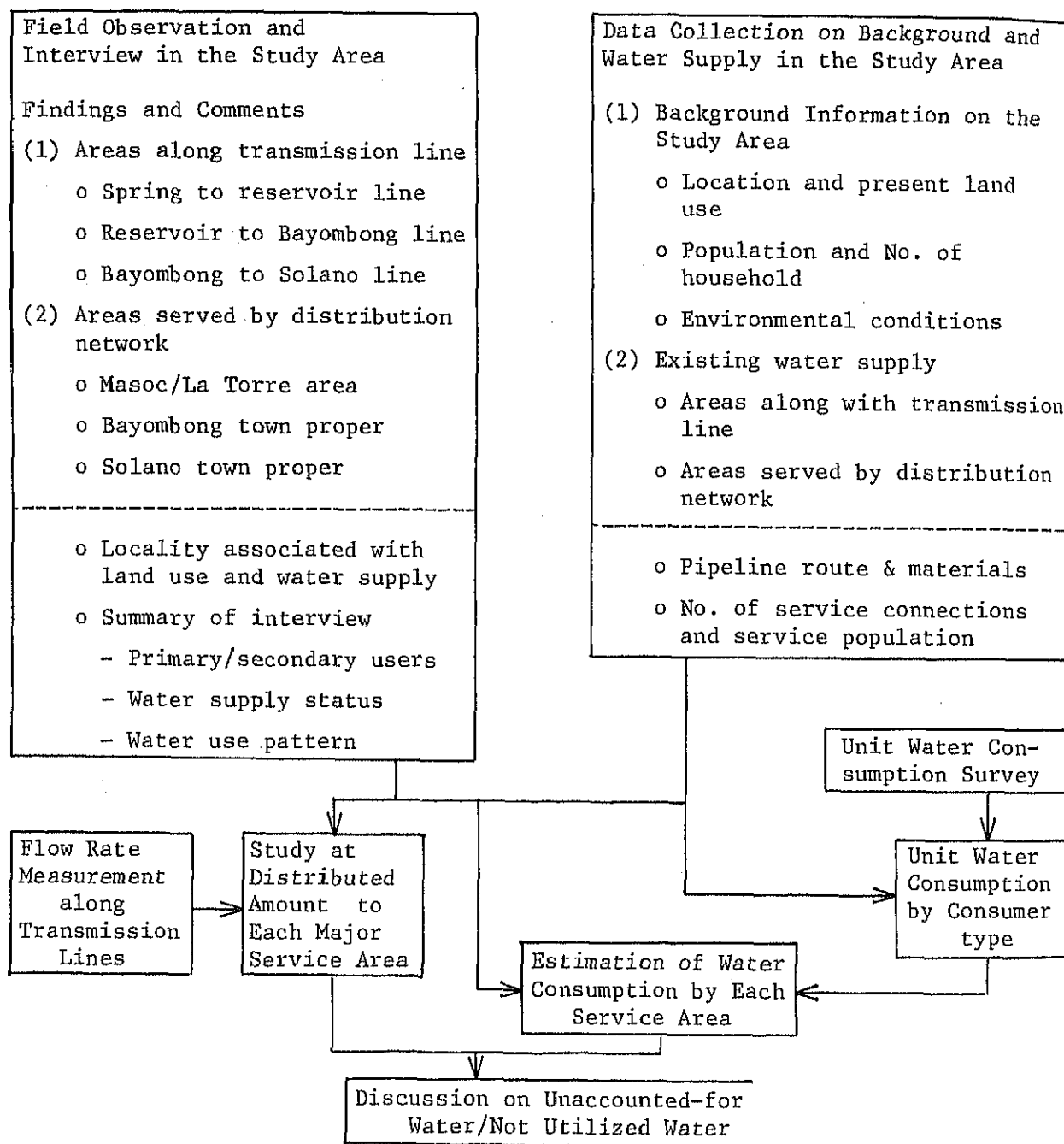


FIGURE 4.5.1.2 FLOW CHART FOR ESTIMATION OF UNACCOUNTED-FOR WATER/  
NOT UTILIZED WATER



## Background information and existing water supply

### a) Masoc/La Torre Area

Barangay Masoc is located on the hilly area near the mountain flank where the Borobob Spring is situated. Houses are distributed along the provincial road connecting to the town proper of Bayombong via Barangay La Torre .

Barangay La Torre is located rather down hill of Masoc along the said provincial road. The households in this barangay are also located along the provincial road.

The land in the Masoc/La Torre service area is mainly used for agricultural purposes (rice field). At the backside of houses, especially in Masoc, there are many small scale fish ponds owned by farmers. Also covered in rural households are piggery and "Carabao" (Filipino; water buffalo). Commercial land use is limited in the Masoc/La Torre service area. Only small scale business establishments, i.e., "Sari-Sari Store" are existing. Likewise, most of the economic activities in this area are agriculture and fish-breeding, and consumable goods are purchased in the Bayombong town proper.

### b) Bayombong Town Proper Area

As a capital town of Nueva Vizcaya, there are many government offices and schools in the town proper of Bayombong. Most of government offices are located within the capital compound of the province at the northern part of the town proper. Schools, colleges and universities are scattered in the town proper. A public market is located in the western part of the town proper.

Likewise, the town proper is used mainly for residential and institutional purposes and commercial area is limited along the national road Route 5 which is running across the town proper.

The east end of the town proper is facing the flood plain of Magat River, west end is up to a bypass of the said national road, south end is up to hilly area, and north side is opened to Solano area.

c) Solano Town Proper Area

Solano town proper is located five (5) km north of Bayombong. This area is a center of commercial activities of the province. The national road Route 5 coming from Bayombong runs across the town proper going to Santiago, Isabela Province.

Likewise, commercial establishments are concentrated along the national roads and the residential area is surrounding the commercial zone. A public market is located at the northeastern part of the town proper.

The water supply status in the entire service area is summarized as follows:

- a) The area directly connected to the transmission line (Borobbob Spring reservoir) on a continuous 24-hour supply basis with comparatively high pressure; Masoc/La Torre area
- b) The area served from the reservoir for 17-hours a day; Town proper of Bayombong and Solano and Bonfal area.

Masoc/La Torre area is being served with a water pressure ranging from 3.8 kg/sq.cm at Masoc Elementary School to 4.3 kg/sq.cm in La Torre area, while the rest of the existing service is seriously suffering from low water pressure throughout the day and there is obviously no water supply in the afternoon.

1) Number of Connections in the service area.

The number of connections by consumers type is listed in TABLE 4.5.1.2. Majority of the connections (more than 95 percent) is for domestic use and there are some institutional and commercial connections and public faucets.

TABLE 4.5.1.2 NUMBER OF CONNECTIONS BY CONSUMER TYPE

Area	Domestic	Institutional	Commercial	Public Market	Total
Masoc/La Torre	130	2	-	2	134
Bayombong	570	18	-	4	592
Bayombong to Solano	71	-	-	-	71
Solano	528	9	10	1	548
Total	1,299	29	10	7	1,345

2) Served Population in the Study Area

The actual served population consists of primary users and secondary users/borrowers as confirmed through the interview with inhabitants.

The interview for the domestic connections was conducted in the service areas of Masoc/La Torre, Bayombong, and Solano. Approximately 20 to 40 households in each area were covered considering the total number of registered concessionaires. Average percentages of the number of borrowers to that of primary users were estimated at 60% in Masoc/La Torre and Solano areas, and 45% in Bayombong and Bonfal areas.

The actual population served was estimated using the interview result as shown in TABLE 4.5.1.3. The served population for the public faucet is based on the interview covering all faucets in the area.

TABLE 4.5.1.3 SERVED POPULATION BY THE STUDY AREA:

Area	Domestic Connection			Public Faucet	Total	R.M.
	No. of Conn.	Primary	Borrower			
Masoc/La Torre	130	658	395	51	1,104	
Bayombong	570	2,874	1,293	45	4,212	
Bayombong to Solano	71	359	162	-	521	
Solano	528	2,847	1,708	22	4,577	
Total	1,331	6,738	3,558	118	10,414	

### Result of Flow Rate Measurement

The result of flow rate measurement at the selected points is summarized in TABLE 4.5.1.4.

TABLE 4.5.1.4 RESULT OF FLOW RATE MEASUREMENT

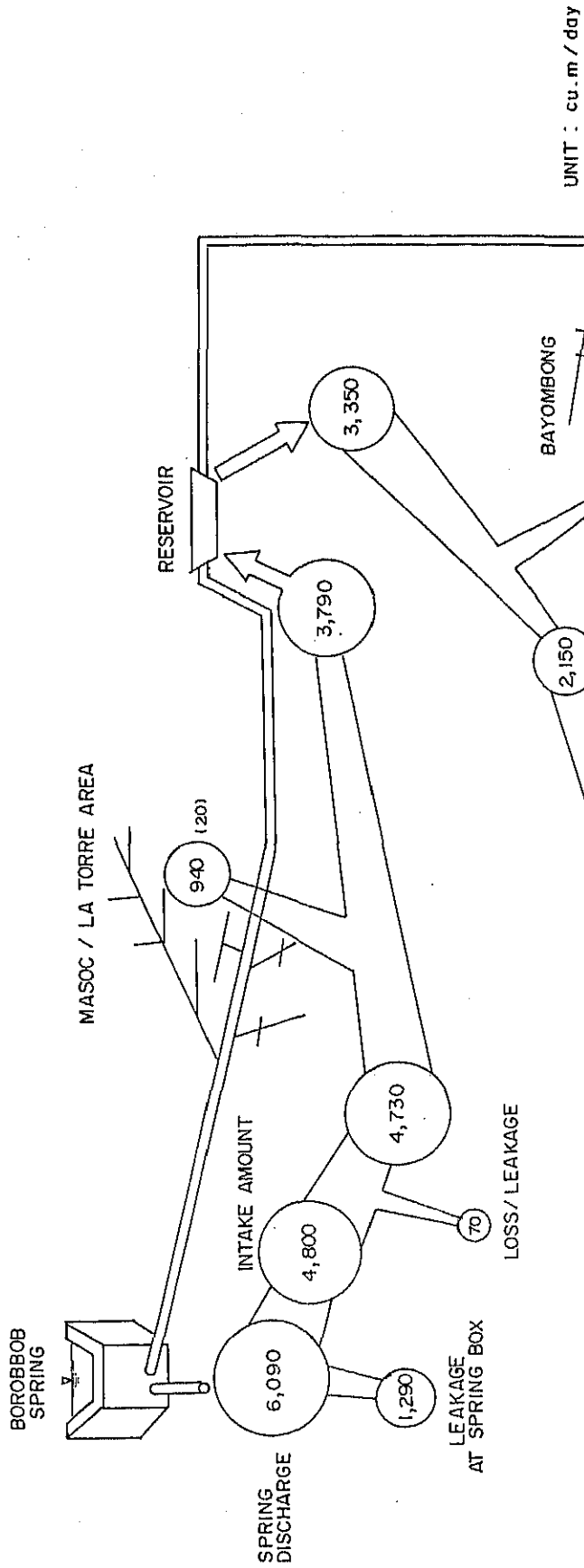
Measuring Point	Flow Rate (cu.m/day)
1	4,800
2	4,730
3	4,060
4	3,790
5	3,350
6	2,190
7	2,230
8	1,790

A schematic diagram of transmitted/distributed water is shown in FIGURE 4.5.1.3. Of the total 4,800 cu.m/day intake amount, 20% or 940 cu.m/day is distributed to Masoc/La Torre area and 78.5% or 3,790 cu.m/day is transmitted to the reservoir, while 1.5% or 70 cu.m/day is /leaked along the transmission line. From the reservoir, a total of 3,350 cu.m/day is distributed to the respective service area; 36% or 1,200 cu.m/day for Bayombong, 11% or 360 cu.m/day for Bonfal area; and the rest, 53% or 1,790 cu.m/day for Solano.

Based on the estimation of distributed water and accounted-for water/utilized water, the unaccounted-for water/not utilized water was assessed as shown in FIGURE 4.5.1.4.

As a whole, approximately 40% of the total distributed amount is estimated to be utilized in the present service area or about 60 % is assumed to be unaccounted-for water/not utilized water.

Major causes of high percentage of the unaccounted-for water/not utilized water may be the presence of unknown consumption and leakage and wastage.



UNIT : cu.m / day

FIGURE 4.5.1.3  
 TRANSMITTED / DISTRIBUTED AMOUNT  
 BAYOMBONG - SOLANO, NUEVA VIZCAYA

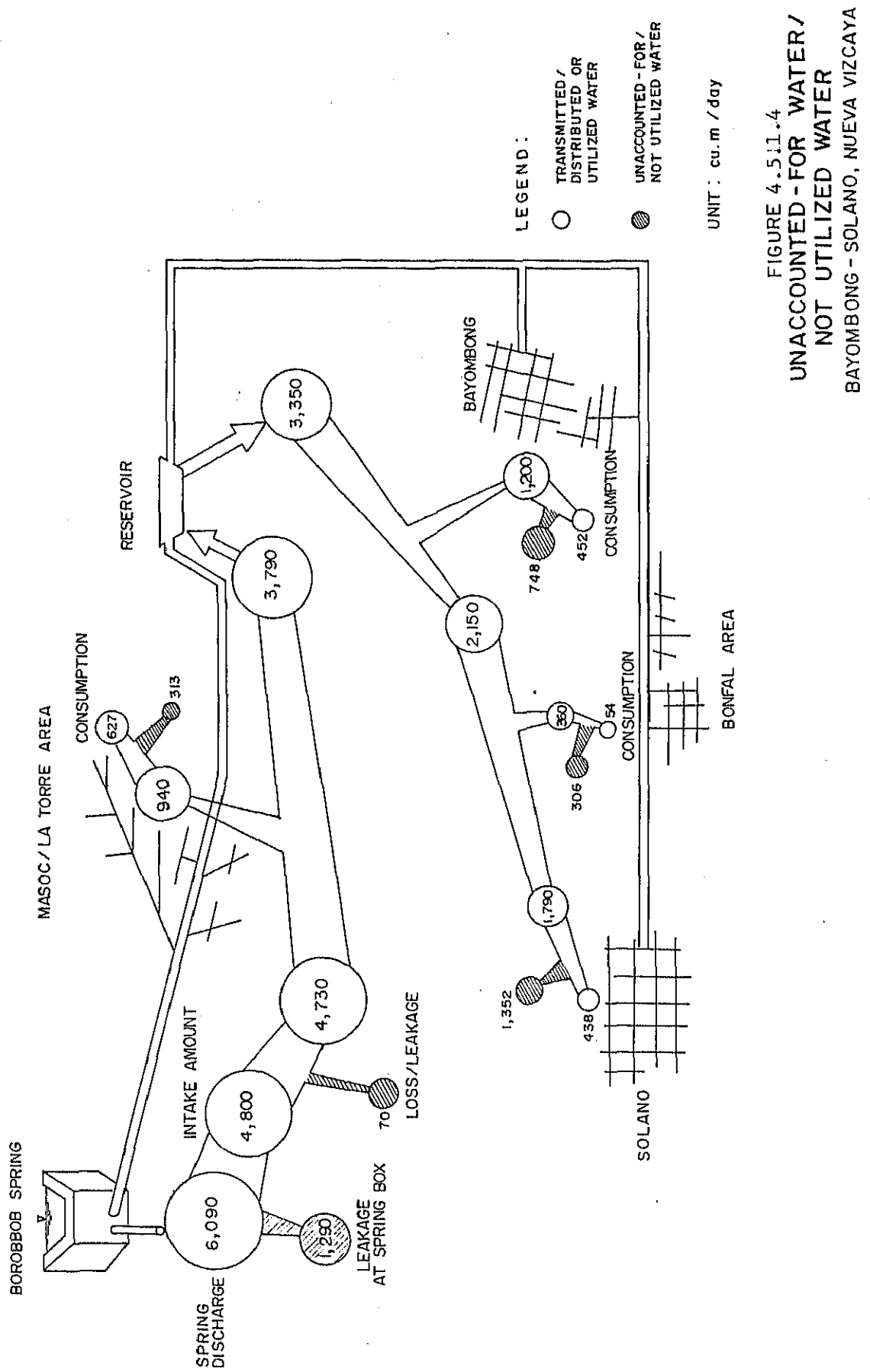


FIGURE 4.5.11.4  
 UNACCOUNTED - FOR WATER/  
 NOT UTILIZED WATER  
 BAYOMBONG - SOLANO, NUEVA VIZCAYA



LWUA/JICA WELL NO. 5 OWNER'S NO. 9048 LOCATION: AGGUB PLAZA, SOLANO		LWUA/JICA WELL NO. 9 OWNER'S NO. 52-84-07 LOCATION: CONCEPCION BLISS, SOLANO	
GROUND ELEVATION: 245 m WELL DEPTH: 5.2 m CASING DEPTH: 4.9 m CASING DIAMETER: 150 mm STATIC WATER LEVEL: 1.6 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:		GROUND ELEVATION: 320 m WELL DEPTH: 31.2 m CASING DEPTH: 25.2 m CASING DIAMETER: 125 mm STATIC WATER LEVEL: 12.0 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:	
DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN
5	150mm Ø CASING	5	125mm Ø PIPE
10		10	
15		15	
20		20	
25		25	SLOTTED CASING
30		30	
35		35	
40		40	
45		45	
50		50	
55		55	
60		60	
			STRATA DESCRIPTION
			BROWN CLAY SANDSTONE
			SILTY CLAY
			TUFF
			BLUE CLAY
			LIMESTONE
			TUFF
			BASALT

LWUA/JICA WELL NO. 1 OWNER'S NO. 52-85-03 LOCATION: UDDAYAN, SOLANO		LWUA/JICA WELL NO. 4 OWNER'S NO. 6766 LOCATION: LATTAUAN, SOLANO	
GROUND ELEVATION: 280 m WELL DEPTH: 12 m CASING DEPTH: 12 m CASING DIAMETER: 32 mm STATIC WATER LEVEL: 7.2 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:		GROUND ELEVATION: 278 m WELL DEPTH: 40 m CASING DEPTH: 112 mm CASING DIAMETER: 4.6 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:	
DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN
5	32 mm Ø GIP	5	
10	32 mm Ø SLOTTED PIPE	10	
15		15	
20		20	
25		25	
30		30	
35		35	
40		40	
45		45	
50		50	
55		55	
60		60	
			STRATA DESCRIPTION
			STICKY CLAY w/ BOULDERS
			COARSE SAND
			BROWN STICKY CLAY
			SOLID ROCK

APPENDIX 6.6.2  
WELL LITHOLOGIC LOGS  
BAYOMBONG - SOLANO

LWUA / JICA WELL NO. 15 OWNER'S NO. 6761 LOCATION: PUBLIC MARKET, SOLANO		LWUA / JICA WELL NO. 17 OWNER'S NO. 52-82-10 LOCATION: CURIFANG, SOLANO	
GROUND ELEVATION: 250 m WELL DEPTH: 23.2 m CASING DEPTH: 10.4 m CASING DIAMETER: 125 mm STATIC WATER LEVEL: 0.9 m bgl DISCHARGE: SPECIFIC CAPACITY: 2.07 TRANSMISSIVITY:		GROUND ELEVATION: 245 m WELL DEPTH: 15.2 m CASING DEPTH: 10.4 m CASING DIAMETER: 125 mm STATIC WATER LEVEL: 4.2 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:	
DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN
STRATA DESCRIPTION	STRATA DESCRIPTION	STRATA DESCRIPTION	STRATA DESCRIPTION
5		5	125mm Ø CASING
10	CLAY WITH SAND	10	125mm Ø SLOTTED PIPE
15	SANDSTONE	15	CLAY
20		20	TUFF
25		25	BOULDER
30		30	FINE SAND
35		35	BASALT
40		40	
45		45	
50		50	
55		55	
60		60	

LWUA / JICA WELL NO. 11 OWNER'S NO. 20973 LOCATION: BINGAR, SOLANO		LWUA / JICA WELL NO. 14 OWNER'S NO. 40601 LOCATION: SOLANO NORTH E/S, SOLANO	
GROUND ELEVATION: 225 m WELL DEPTH: 9.2 m CASING DEPTH: 1.8 m BGL CASING DIAMETER: STATIC WATER LEVEL: DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:		GROUND ELEVATION: 250 m WELL DEPTH: 13.7 m CASING DEPTH: 13.7 m CASING DIAMETER: STATIC WATER LEVEL: 0.9 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:	
DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN
STRATA DESCRIPTION	STRATA DESCRIPTION	STRATA DESCRIPTION	STRATA DESCRIPTION
5		5	CLAY W/ SAND
10	SANDY SOIL	10	COARSE SAND W/ CLAY
15	SAND AND GRAVEL	15	COARSE SAND W/ BOULDERS
20		20	BOULDERS W/ SAND
25		25	BOULDERS W/ SAND AND GRAVEL
30		30	SAND AND GRAVEL
35		35	
40		40	
45		45	
50		50	
55		55	
60		60	

LWUA/JICA WELL NO. 18  
 OWNER'S NO. 52-85-03  
 LOCATION: BUGAY, QUIRINO, SOLANO

GROUND ELEVATION: 240 m  
 WELL DEPTH: 18.5 m  
 CASING DEPTH: 15.5 m  
 CASING DIAMETER: 125 mm  
 STATIC WATER LEVEL: 4.1 m bgl  
 DISCHARGE: \_\_\_\_\_  
 SPECIFIC CAPACITY: \_\_\_\_\_  
 TRANSMISSIVITY: \_\_\_\_\_

LWUA/JICA WELL NO. 19  
 OWNER'S NO. CLIRIFANG, SOLANO

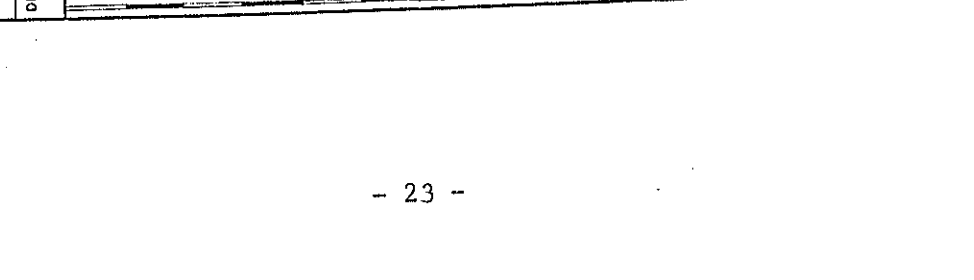
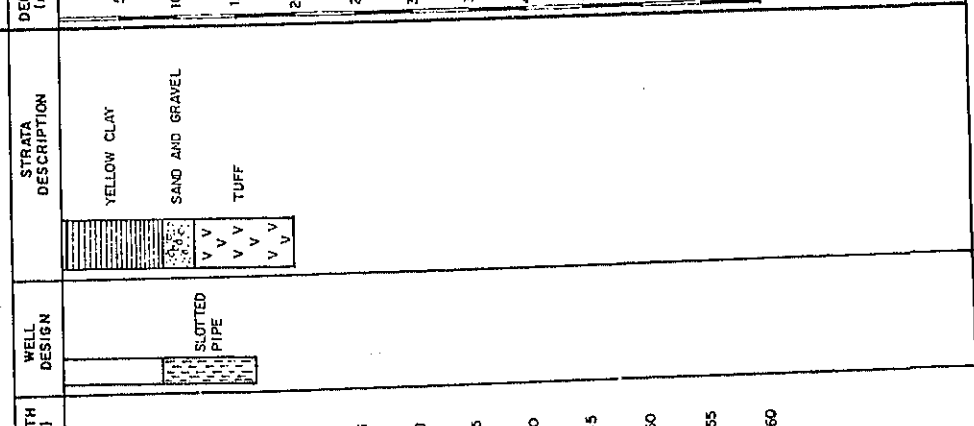
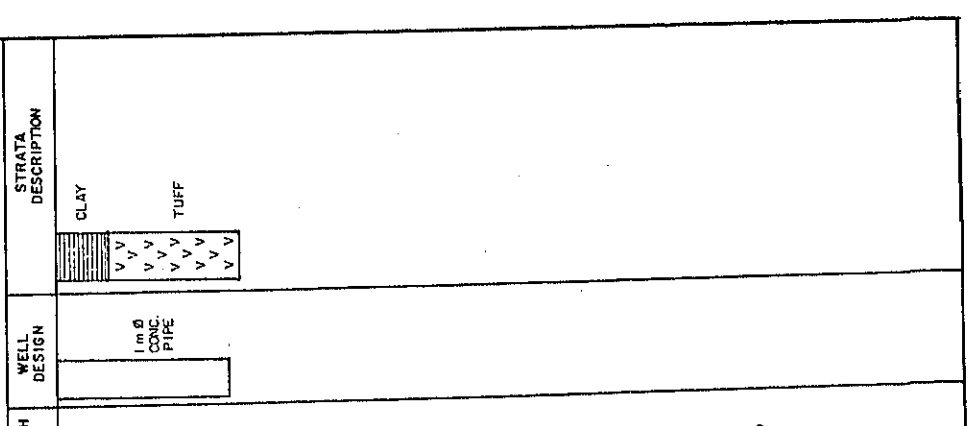
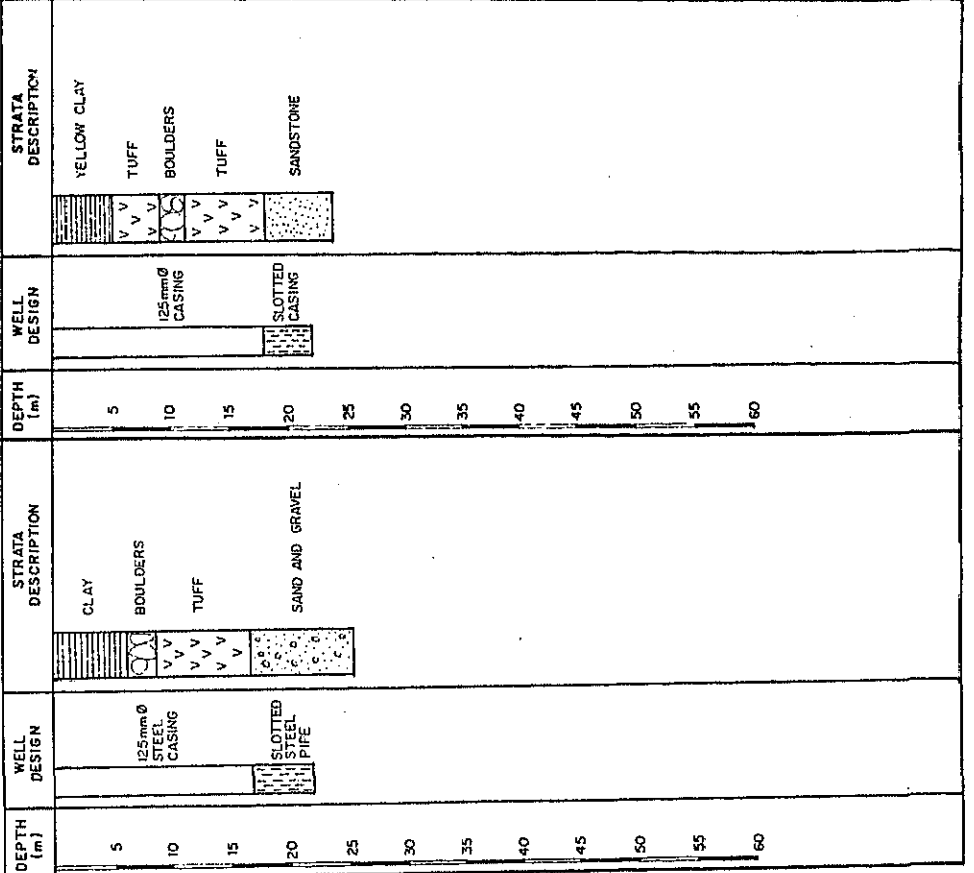
GROUND ELEVATION: \_\_\_\_\_  
 WELL DEPTH: 15 m  
 CASING DEPTH: 14 m  
 CASING DIAMETER: 1 m  
 STATIC WATER LEVEL: 11.2 m bgl  
 DISCHARGE: \_\_\_\_\_  
 SPECIFIC CAPACITY: \_\_\_\_\_  
 TRANSMISSIVITY: \_\_\_\_\_

LWUA/JICA WELL NO. 21  
 OWNER'S NO. 52-82-07  
 LOCATION: LA TORRE E/S, BAYOMBONG

GROUND ELEVATION: 280 m  
 WELL DEPTH: 24.5 m  
 CASING DEPTH: 21.4 m  
 CASING DIAMETER: 125 mm  
 STATIC WATER LEVEL: 1.8 m bgl  
 DISCHARGE: \_\_\_\_\_  
 SPECIFIC CAPACITY: \_\_\_\_\_  
 TRANSMISSIVITY: \_\_\_\_\_

LWUA/JICA WELL NO. 22  
 OWNER'S NO. 52-82-08  
 LOCATION: LUYANG, BAYOMBONG

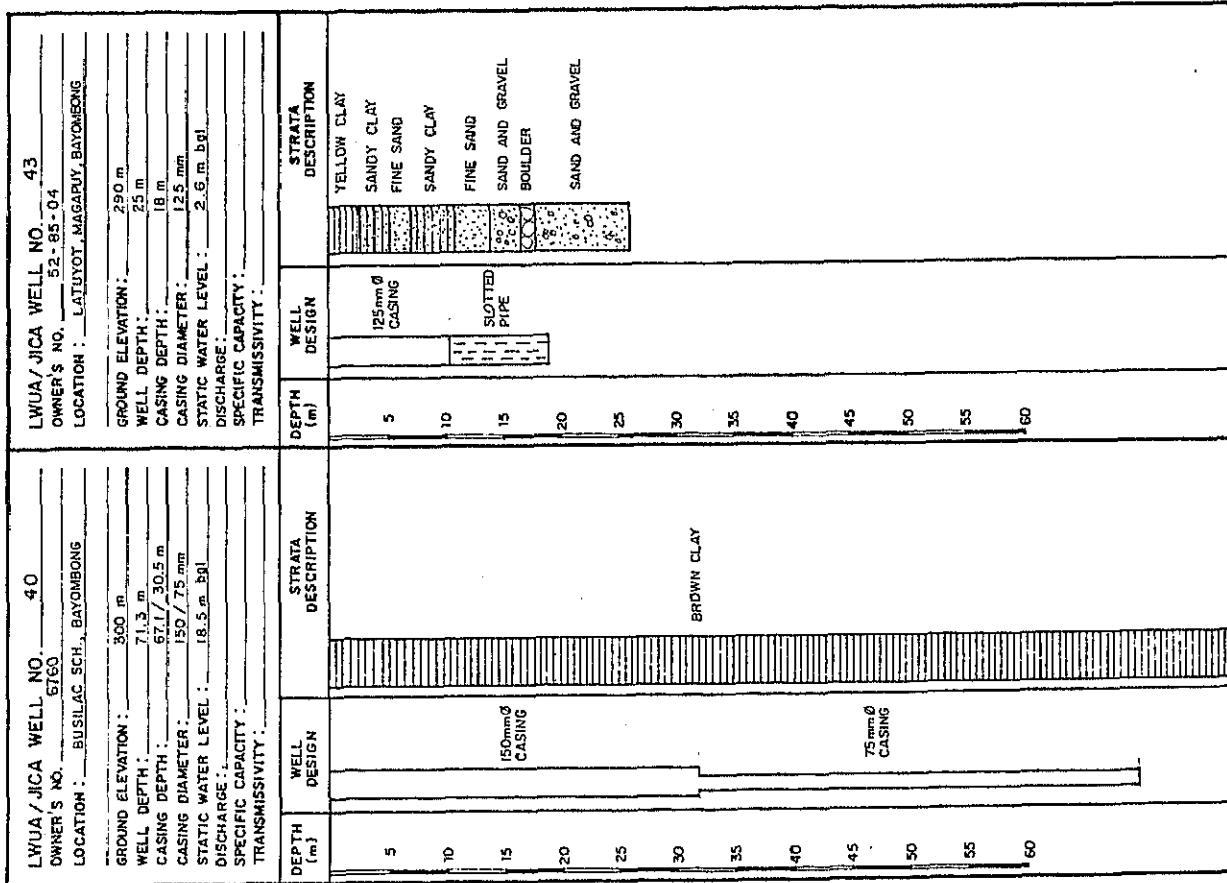
GROUND ELEVATION: 280 m  
 WELL DEPTH: 23 m  
 CASING DEPTH: 21.9 m  
 CASING DIAMETER: 125 mm  
 STATIC WATER LEVEL: 2.9 m bgl  
 DISCHARGE: \_\_\_\_\_  
 SPECIFIC CAPACITY: \_\_\_\_\_  
 TRANSMISSIVITY: \_\_\_\_\_



LWUA / JICA WELL NO. 23 OWNER'S NO. 9042 LOCATION: BONFAL SCH., BAYOMBONG		LWUA / JICA WELL NO. 29 OWNER'S NO. 92-76-31 LOCATION: BAY WEST SCH., BAYOMBONG		LWUA / JICA WELL NO. 31 OWNER'S NO. 53-83-02 LOCATION: MFEWVRH, BAYOMBONG		LWUA / JICA WELL NO. 32 OWNER'S NO. MFEWVRH, BAYOMBONG	
GROUND ELEVATION: 265 m WELL DEPTH: 7.6 m CASING DEPTH: 7.0 m CASING DIAMETER: 150 mm STATIC WATER LEVEL: 2.8 m bgl DISCHARGE: SPECIFIC CAPACITY: 0.31 TRANSMISSIVITY:		GROUND ELEVATION: 265 m WELL DEPTH: 12.8 m CASING DEPTH: 12.8 m CASING DIAMETER: 112 mm STATIC WATER LEVEL: 1.2 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:		GROUND ELEVATION: 290 m WELL DEPTH: 30 m CASING DEPTH: 150 mm CASING DIAMETER: 150 mm STATIC WATER LEVEL: 2.9 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:		GROUND ELEVATION: 290 m WELL DEPTH: 18.6 m CASING DEPTH: 16.8 m CASING DIAMETER: 125 mm STATIC WATER LEVEL: 2.7 m bgl DISCHARGE: SPECIFIC CAPACITY: TRANSMISSIVITY:	
DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN	DEPTH (m)	WELL DESIGN
5		5	113 mm Ø CASING	5		5	
10	BROWN CLAY CLAY WITH SANDSTONE SAND AND GRAVEL	10	113 mm Ø SCREEN	10	CLAY SAND AND GRAVEL	10	RED CLAY SAND AND GRAVEL YELLOW CLAY GRAY TUFF YELLOW SILTY CLAY SAND AND GRAVEL GRAY TUFF GRAY BASALT LIMESTONE ANDESITE BASALT TUFF BASALT
15		15		15	YELLOW CLAY	15	
20		20		20	GRAY TUFF	20	
25		25		25	YELLOW SILTY CLAY	25	
30		30		30	SAND AND GRAVEL GRAY TUFF	30	
35		35		35		35	
40		40		40		40	
45		45		45		45	
50		50		50		50	
55		55		55		55	
60		60		60		60	

LWUA / JICA WELL NO. 33 OWNER'S NO. 52-76-25 LOCATION : BAY CENTRAL SCH., BAYOMBONG		LWUA / JICA WELL NO. 34 OWNER'S NO. 52-83-03 LOCATION : NVSIT., BAYOMBONG		LWUA / JICA WELL NO. 35 OWNER'S NO. 52-76-06 LOCATION : DIV. OFF. PUBLICATION, BAYOMBONG	
GROUND ELEVATION : 270 m	GROUND ELEVATION : 285 m	GROUND ELEVATION : 270 m	GROUND ELEVATION : 270 m	GROUND ELEVATION : 270 m	GROUND ELEVATION : 270 m
WELL DEPTH : 15.2 m	WELL DEPTH : 30.5 m	WELL DEPTH : 10.7 m	WELL DEPTH : 10.7 m	WELL DEPTH : 15.5 m	WELL DEPTH : 15.5 m
CASING DEPTH : 12.2 m	CASING DEPTH : 16.5 m	CASING DEPTH : 10.7 m	CASING DEPTH : 10.7 m	CASING DEPTH : 14.4 m	CASING DEPTH : 14.4 m
CASING DIAMETER : 112 mm	CASING DIAMETER : 125 mm	CASING DIAMETER : 38 mm	CASING DIAMETER : 38 mm	CASING DIAMETER : 125 mm	CASING DIAMETER : 125 mm
STATIC WATER LEVEL : 1.7 m bgl	STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 6.8 m bgl	STATIC WATER LEVEL : 6.8 m bgl
DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____
SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____
TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____
DEPTH (m)	WELL DESIGN	STRATA DESCRIPTION	DEPTH (m)	WELL DESIGN	STRATA DESCRIPTION
5	112 mm Ø CASING	SANDY CLAY GRAVEL	5	125 mm Ø SLOTTED CASING	GRAVEL
10		QUICKSAND	10		SAND AND GRAVEL
15		SAND AND GRAVEL	15		
20			20		CLAY AND BOULDERS
25			25		TUFF
30			30		AGGLOMERATE
35			35		TUFF
40			40		
45			45		BASALT
50			50		
55			55		
60			60		

LWUA / JICA WELL NO. 33 OWNER'S NO. 52-76-06 LOCATION : DIV. OFF. PUBLICATION, BAYOMBONG		LWUA / JICA WELL NO. 34 OWNER'S NO. 52-83-03 LOCATION : NVSIT., BAYOMBONG		LWUA / JICA WELL NO. 35 OWNER'S NO. 52-76-25 LOCATION : BAY CENTRAL SCH., BAYOMBONG	
GROUND ELEVATION : 270 m	GROUND ELEVATION : 285 m	GROUND ELEVATION : 270 m	GROUND ELEVATION : 270 m	GROUND ELEVATION : 270 m	GROUND ELEVATION : 270 m
WELL DEPTH : 10.7 m	WELL DEPTH : 30.5 m	WELL DEPTH : 10.7 m	WELL DEPTH : 10.7 m	WELL DEPTH : 15.5 m	WELL DEPTH : 15.5 m
CASING DEPTH : 10.7 m	CASING DEPTH : 16.5 m	CASING DEPTH : 10.7 m	CASING DEPTH : 10.7 m	CASING DEPTH : 14.4 m	CASING DEPTH : 14.4 m
CASING DIAMETER : 38 mm	CASING DIAMETER : 125 mm	CASING DIAMETER : 38 mm	CASING DIAMETER : 38 mm	CASING DIAMETER : 125 mm	CASING DIAMETER : 125 mm
STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 1.6 m bgl	STATIC WATER LEVEL : 6.8 m bgl	STATIC WATER LEVEL : 6.8 m bgl
DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____	DISCHARGE : _____
SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____	SPECIFIC CAPACITY : _____
TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____	TRANSMISSIVITY : _____
DEPTH (m)	WELL DESIGN	STRATA DESCRIPTION	DEPTH (m)	WELL DESIGN	STRATA DESCRIPTION
5	38 mm Ø GIP	GRAVEL	5	125 mm Ø SLOTTED CASING	CLAY
10		SAND AND GRAVEL	10		TUFF
15			15		BOULDERS
20			20		BASALT
25			25		
30			30		
35			35		
40			40		
45			45		
50			50		
55			55		
60			60		



APPENDIX 6.7.1 WATER QUALITY EXAMINATION

(1) Sampling Points

Water samples were collected from the total of 12 sampling points for physical, chemical and bacteriological examination as shown in TABLE 6.7.1.1 and FIGURE 6.7.1.1.

TABLE 6.7.1.1 SAMPLING POINTS

	<u>Physical &amp; Chemical</u>	<u>Bacteriological</u>
1. Reservoir inlet from spring	0	X
2. Service connection in Bayombong	0	0
3. Service connection in Solano	0	X
4. Shallow well, Magat River flood plain	0	X
5. Surface water, Magat River	0	X
6. Shallow well, Uddiawan Solano	0	X
7. Deep well, Lattauan, Solano	0	X
8. Shallow well, Curipang, Solano	0	X
9. Shallow well, Bonfal, Bayombong	0	X
10. Shallow well, Bayombong Town Proper	0	X
11. Shallow well, Puy, Bayombong	0	X
12. Shallow well, Vista Hill, Bayombong	0	X

(2) Sampling and pretreatment

Water samples were collected and pretreated in such a manner as to correspond to their respective chemical constituents as analyzed in the laboratory.

<u>Sample Container and Pretreatment</u>	<u>Items to be analyzed</u>
o 1,000 ml polyethelene bottle	Water Temperature, EC, pH TDS, Ca <sup>2+</sup> , Mg <sup>2+</sup> , Na <sup>+</sup> , K <sup>+</sup> , SO <sub>4</sub> <sup>2-</sup> , CO <sub>3</sub> <sup>2-</sup> , HCO <sub>3</sub> <sup>-</sup> , Cl <sup>-</sup> ,
	Fe <sup>2+</sup> , Mn
o 500 ml polyethylene bottle with 1 ml of conc. H <sub>2</sub> SO <sub>4</sub> to maintain pH below 1.0	NH <sub>3</sub> -N, NO <sub>3</sub> -N, Fe <sup>+</sup>

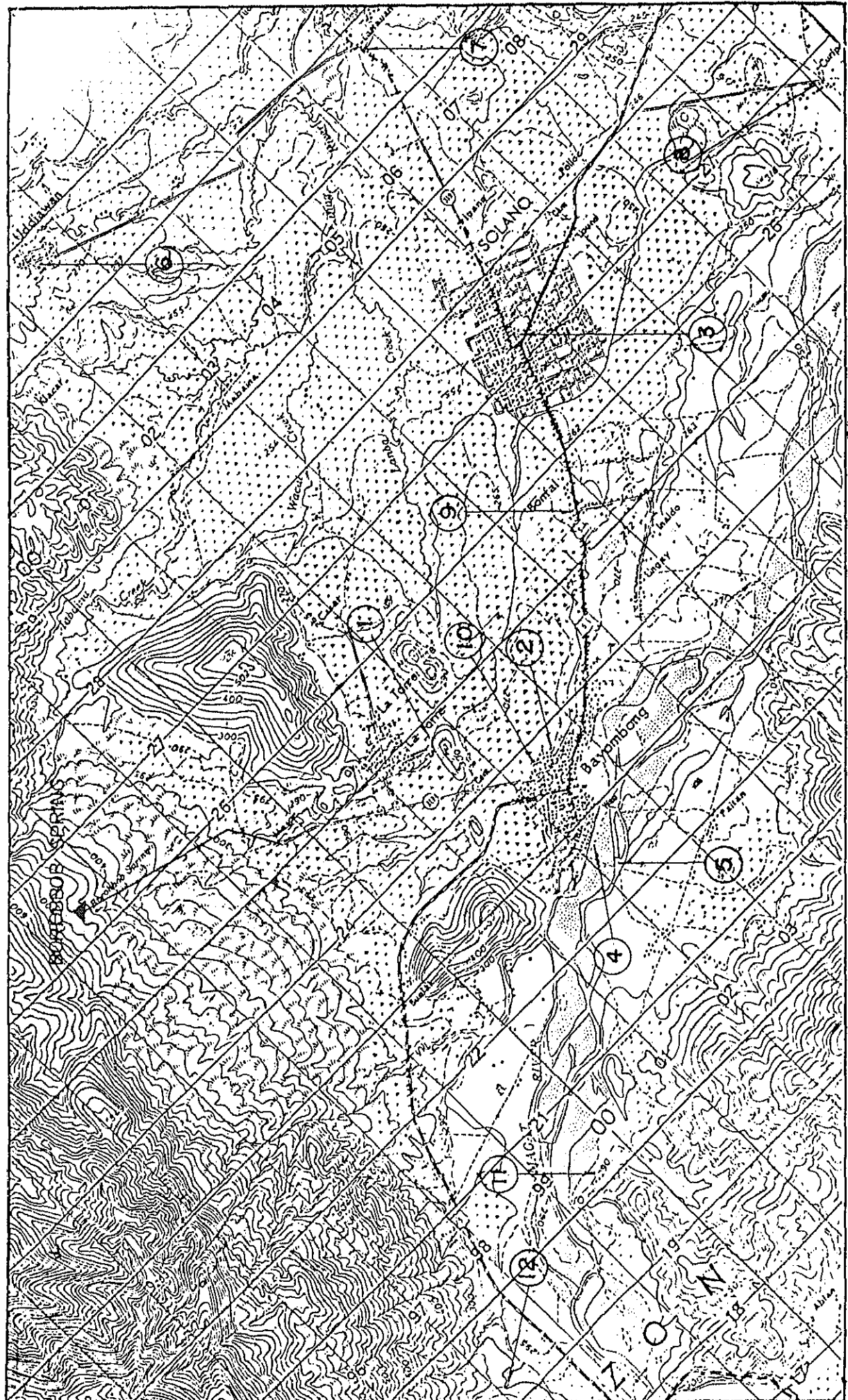


FIGURE 6.7.1.1 LOCATION OF SAMPLING POINTS



2) Laboratory examination

The procedure of water quality analysis was based on the Philippine Standard Methods for the Analysis of Air and Water (Vol. 2). The result of laboratory examination is presented in TABLE 6.7.3.

o 100 ml pre-disinfected polyethylene  
bottle

Coliform Group Bacteria

In addition to the chemical pretreatment, all the collected water samples in polyethylene bottles were kept in black polyethylene bags with crushed ice to maintain water temperature below 4C during its transportation to the laboratory.

(3) Field examination

Water temperature, pH and EC were measured in the field water sampling.

Implementation Procedure

Since the collected water samples were to be delivered to the laboratory within 8-hours from the time of sampling, the water sampling and field examination were carried out in the early morning from 6:30 AM to 8:30 AM on 14 August 1986.

(4) Result of water quality examination

1) Field examination

The results of field examination is presented in TABLE 6.7.2.

TABLE 6.7.2 RESULT OF FIELD EXAMINATION

<u>Sample No.</u>	<u>WT(°C)</u>	<u>pH</u>	<u>EC (micro-S/cm)</u>
1	25.8	7.04	491
2	27.1	7.22	487
3	25.8	7.34	478
4	26.2	7.06	521
5	25.8	7.83	272
6	27.1	6.94	550
7	27.1	6.63	580
8	26.2	7.17	484
9	26.0	6.84	485
10	26.2	6.55	603
11	25.8	6.08	910
12	25.8	6.04	680

TABLE 6.7.3 WATER QUALITY ANALYSIS AT LABORATORY

Sample No. & Location	Turb. (FTU)	TDS (mg/l)	pH	EC (mg/l)	Alk. (mg/l)	Hard. (mg/l)	Acid. (mg/l)	Na (mg/l)	K (mg/l)	Ca (mg/l)	Mg (mg/l)	CO <sub>3</sub> (mg/l)	HCO <sub>3</sub> (mg/l)	Cl (mg/l)	SO <sub>4</sub> (mg/l)	Fe (mg/l)	MN (mg/l)	Colitrus (MPN)	NO <sub>3</sub> -N (mg/l)	NH <sub>4</sub> -N (mg/l)
1. Reservoir inlet	0.79	352	7.11	550	247	240	12	23.0	0.6	26.8	42.0	0	301.3	18.6	14.0	0.075	Nil	-	3.32	Nil
2. Faucet in Bayombong	0.76	333	7.44	520	209	325	12	20.0	0.5	42.0	29.2	0	255.0	37.1	14.0	0.17	0.05	-	2.99	Nil
3. Faucet in Solano	0.54	326	7.56	510	190	217	9.0	19.0	0.5	48.0	23.6	0	231.8	27.9	14.0	0.10	Nil	-	3.04	Nil
4. S.W. Flood Plain	0.91	374	7.48	585	190	255	18.0	19.0	0.5	38.8	38.4	0	231.8	32.5	72.0	0.03	Nil	-	0.52	Nil
5. Magat River	74.5	205	7.94	320	102	142	0	13.0	0.4	24.0	19.9	45.6	31.7	18.6	37.0	0.19	0.08	-	2.69	Nil
6. S.W. Uddiawan	19.5	397	7.12	620	275	277	20.0	24.0	0.2	62.8	29.2	0	335.5	23.2	12.0	0.12	Nil	-	3.33	Nil
7. S.W. Lattanuan	3.64	394	7.25	615	266	285	35.0	19.0	0.5	72.0	25.5	0	324.5	23.2	22.0	0.12	Nil	-	0.99	Nil
8. S.W. Curipang	1.26	326	7.53	510	218	232	9.0	19.0	0.4	68.8	14.6	0	266.0	18.0	20.0	0.12	Nil	-	2.42	Nil
9. S.W. Bonfal	0.72	352	7.29	550	209	247	12.0	19.0	0.4	66.0	19.9	0	255.0	23.2	23.0	0.12	Nil	-	4.70	Nil
10. S.W. Bayombong	22.3	442	7.06	690	275	300	18.0	23.0	0.4	90.0	18.2	0	335.5	37.1	29.0	0.10	Nil	-	2.00	Nil
11. S.W. Vistra Hill	0.93	634	6.86	990	114	457	32.0	31.0	1.3	110.8	43.7	0	139.1	37.1	345.0	0.11	Nil	-	9.42	Nil
12. S.W. Magapay	0.84	480	7.04	750	304	360	22.0	11.0	1.0	102.0	25.5	0	370.9	32.5	38.0	0.075	Nil	-	-	Nil

Philippine National Standard for Drinking Water

Water Quality: Physical, Chemical and Radiological  
Requirements

Bacteriological Quality Standards

Parameter		Maximum Permissible level*
Turbidity		5 units
Color		5 units (s) **
Odor		Unobjectionable
Threshold odor number		Note more than 3
Taste		Unobjectionable
Total Solids		500 (s)
pH		6.5 - 8.5
Phenolic substances		0.001
Radioactive Subs.	Gross Alpha	3 pCi/1
	Gross Beta	30pCi/1
Trace Elements	Arsenic	0.05
	Barium	1.0
	Cadmium	0.01
	Chromium	0.05
	Copper	1.0
	Cyanide	0.05
	Fluoride	0.6
	Iron	1.0
	Lead	0.05
	Manganese	0.5 (s)
	Mercury	0.002
	Selenium	0.01
	Zinc	5.0 (s)
Organic Chemicals	: Synthetic Detergents (MBAS)	0.5
	: Oil & Grease	Nil
Persistent Pesticides	: Aldrin	0.001
	: DDT	0.05
	: Dieldrin	0.001
	: Chlordane	0.003
	: Endrin	0.0002
	: Heptachlor	0.0001
	: Lindane	0.004
	: Toxaphene	0.005
	: Methoxychlor	0.1
	: 2,4 -- E	0.1
: 2, 4, 5 -- T	0.01	
PCB		Nil
Other Chemicals	: Calcium	75
	: Chloride	200 (s)
	: Magnesium	50 (s)
	: Nitrate (NO <sub>3</sub> )	30
	: Sulfate	200 (s)
	: Hydrogen sulfide	0.05 (s)

**Minimum Requirements on Bacteriological Quality**

a) Chlorinated or Otherwise Disinfected Supplies  
Efficient treatment culminating in chlorination or some other form of disinfection should yield a water free or any coliform organism however polluted the original raw water may have been. In practice it should not be possible to demonstrate the presence of coliform organisms in any sample of 100ml. The efficacy of the purification process and method of sampling should be looked into when a sample of the water entering the distribution system does not conform to this standard. In testing chlorinated water, presumptive positive tubes should always be subjected to appropriate confirmatory tests.

b) Non-disinfected Supplies  
Where supplies of this sort exist, no water entering the distribution system should be considered satisfactory if it yields E coli in 100ml. If E. coli is absent, the presence of not more than 3 coliform organisms per 100ml may be tolerated in occasional samples from established non-disinfected pipes supplies, provided that they have been regularly and frequently tested and that the catchment area and storage conditions are found to be satisfactory. If repeated samples show the presence of coliform organisms, steps should then be taken to discover and, if possible, remove the source of pollution. If the number of coliform organisms increases to more than 3 per 100ml, the supply should be considered unsuitable for use without disinfection.

c) Individual or Small Community Supplies  
Where supply of waters are individual wells, bores and springs everything possible should be done to prevent pollution of the water. It should be possible to reduce the coliform count of water from even a shallow well to less than 10 per 100ml. Persistent failure to achieve this, particularly if E. coli is repeatedly found, should, as a general rule lead to chlorination or boiling of the water for domestic consumption.

\* All units are in mg/l unless, otherwise stated.  
 \*\* (s) - Secondary standards; compliance with the standard and analysis are not obligatory.

## F. COST COMPARISON

General

Analysis and evaluation of alternative are based largely on present-worth cost studies, taking into consideration the salvage value after the design period. Cost comparison is based on present worth of net disbursement during the period of 1980-2010 without any escalation factor applied to the 1980 unit prices.

If the differences between net PW cost of an alternative and that of the least-cost alternative is within the limit of cost estimating accuracy (10-15%) further cost comparison shall be made applying escalation factor to 1980 unit prices. For escalation rates, refer to Chapter VII-C: Escalation Rates. Moreover, non-economic parameters may also be influence the selection of the recommended plan.

Construction Cost

Construction cost estimates of the proposed improvements are based on the projected July 1980 unit prices. All estimates on imported materials are based on an exchange rate of ₱7.40 per 1 US dollar. Further, it is assumed that no custom duty will be charged on items imported for the public water supply project. The cost of any facility to be replaced during the design period (1980-2010) is included under the capital cost for the particular year.

Annual Cost

Annual costs are all costs associated with the maintenance, operation, and management of the project. These include labor, power, chemical and maintenance costs. These estimates are carried out for the period 1980-2010. The present-worth cost of annual expenditure is based on uniform and gradient series at a given interest.

Personnel and maintenance costs may abruptly increase as additional facilities are put into operation - e.g., the power cost at a pump station increases in relation to the daily pumpage of water.

Salvage Value

The salvage values of facilities at the end of the design period 2010 are important in calculating net present worth of the total expenditures. It is assumed that the value of a facility depreciates linearly throughout its service life therefore, a facility with longer service life depreciates less than a facility with shorter service life (Refer to Table VI-1 for service life of different facilities). Moreover, a facility constructed at a later stage has higher salvage value than one constructed at an earlier stage.

TABLE VI-1

SERVICE LIFE CATEGORIES OF FACILITIES

Civil Works	Economic Life	Equipment	Economic Life
Wells	30 years	Wells (pumping engine or motors)	15 years
Springs	50	Springs (vales, pipes)	50
Transmission Mains	50	Transmission (pipes, valves)	50
Storage Facilities	50	Storage (valves, pipes, level gauge, etc.)	50
Disinfection Facilities	50	Disinfection facilities (chlorinators, mech-	
Distribution Mains	50	anical equipment and filter equipment,	
Internal Network	50	pipes, valves)	15
Service Connections	50	Distribution mains (pipes, valves)	50
Fire Hydrants	50	Internal networks (pipes, valves)	50
Operational Buildings	50	Service connections (meters, pipes)	50
		Operational buildings (workshop, etc.)	15
		Fire hydrants	30
		Vehicles	7

### Net Present Worth

The net present worth cost of an alternative scheme is the difference between the total present worth of capital cost and annual cost minus the present worth of salvage values.

For Construction Cost:

$$C_n = C_c - C_s$$

$$C_c = C \times \frac{1}{(1+i)^n}$$

$$C_c = C \times \frac{1}{(1+i)^{nx}} \times \left(1 - \frac{nx - n}{SL}\right)$$

For Annual Cost:

$$C_c = A_c \times \frac{1}{(1+i)^n}$$

where,

$C_n$  = net present worth comparable cost

$C_c$  = present worth of construction cost

$C_s$  = present worth of salvage value (design year)

$C$  = construction cost

$SL$  = service life

$i$  = discount rate

$nx$  = number of years between design year and base year

$n$  = number of years between year of construction and base year

$A_c$  = annual cost

## 7.2.1.1. Hydraulic Simulation of Technical Alternatives

The infiltration gallery and the radial well are considered as the alternatives for water intake facility. To determine the design parameter of each facility, the following hydraulic simulation was carried out.

Radial Well

$$Q = \pi k (H^2 - h^2) / \ln(R/r_o)$$

where,

Q = planned yield; 12,880 cu.m/day

k = permeability coefficient;  $2 \times 10^{-3}$  m/sec

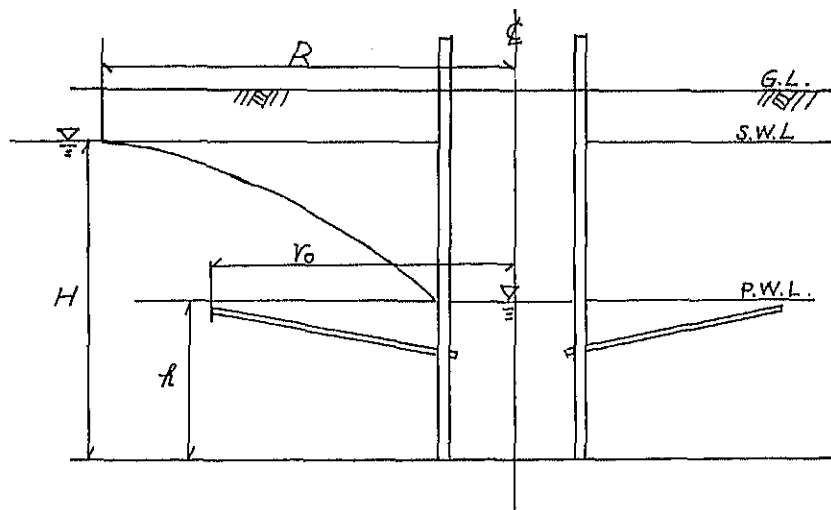
H = aquifer thickness; 9 m

h = effective aquifer thickness during pumping (m)

R = radius of influence area; 250 m

$r_o$  = radius of radial collector (m)

H - h = drawdown (m)



Through the trial simulation of the above formula, the design parameter of the radial well was determined to be 2 m for the effective aquifer thickness and 10 m for the radius of radial collector. By these parameters, the planned yield is computed to be approximately 13,000 cu.m/day. Thus, two units of radial well can meet the required water demand.

## Infiltration Gallery

$$Q = k \times L \times (H^2 - h_o^2) \times R^{-1} \times (t + 0.5 \times r_o) / h_o \times \sqrt{2 \times h_o - t} / h_o$$

where,

Q = planned yield; 12,880 cu.m/day

k = permeability coefficient;  $2 \times 10^{-3}$  m/sec

L = length of gallery (m)

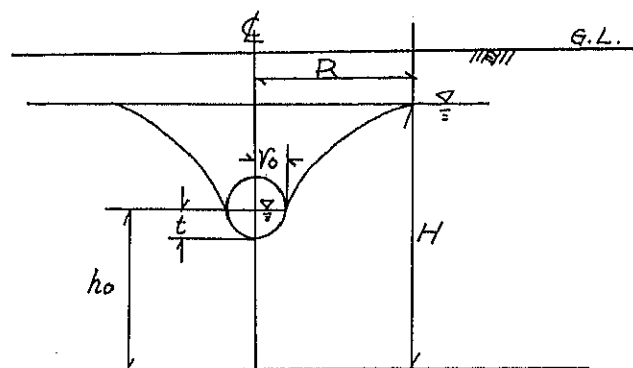
H = aquifer thickness; 14 m

$h_o$  = height of gallery from the bottom of aquifer; 10 m

R = radius of influence area; 250 m

t = depth of water in gallery (m)

$r_o$  = radius of gallery (m)



As well as the radial well, the design parameter of infiltration gallery was determined to be 1.5 m for both the depth of water in gallery and the radius of gallery, and 355 m for the gallery length. By these parameters, the planned yield is computed to be approximately 13,000 cu.m/day.

### 7.2.1.2. Cost Estimates

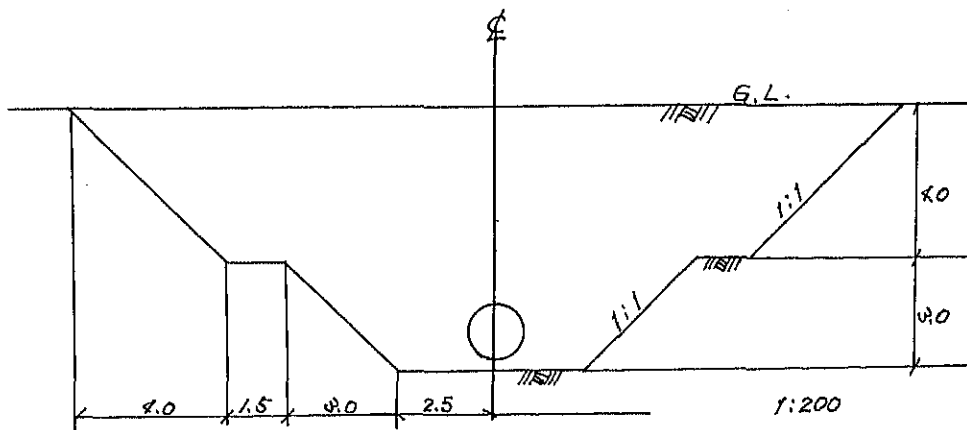
#### Radial Well

The construction cost of the radial well is estimated to be approximately ₦1,240,000 per one unit, as shown in TABLE 7.2.1.1.



## Infiltration Gallery

In order to install the infiltration gallery in the river bed, the volume of excavation and backfilling with relevant civil works is required. The following is a rough sketch of the cross-section of river bed excavation.



The given cross-section has an area of 96 sq.m. When 355 m of the required length of infiltration gallery is taken into account, the total excavation volume will be about 34,000 cu.m. At ₱103/cu.m of the unit cost for excavation, this work item will require at about ₱3.5 million.

### 7.2.1.3 Conclusion

In accordance with the above cost estimates, it is clear that the radial well is quite economical than the infiltration gallery.

TABLE 7.2.1.1.1 COST ESTIMATES OF RADIAL WELL

Item	Description	Unit	Qty	Unit Cost (Peso)	Cost (Peso)
1. Concrete Caisson					
1.1 Steel Shoe	φ5.0 m(ID) x φ6.0 m(OD) x 1.0 m(H) (t=6 mm)	kg	2,400	26	62,400
1.2 Escavation	φ6.0 m x 11.5 m(D)	cu.m	325	103	33,475
1.3 Concrete (3,000 psi)	Caisson: φ5.0 m(ID) x φ6.0 m(OD) x 16.5 m(H) for wall φ5.0 m(ID) x 1.5 m(H) for anchor Cover with slab: φ6.0 m x 0.2 m(H) + (0.3x0.3x4.8x4 - 0.3x0.3x0.3x2)	cu.m	172	1,170	201,240
1.4 Reinforcement Steel Bar	20 kg for every 1.0 cu.m of concrete	kg	3,586	15	53,790
1.5 Formwork	Caisson: (φ5.0 m x φ6.0 m) x 16.5 m(H) Cover/slab: φ6.0 m for horizontal 0.3x(0.75x4x1.4x4)x4 for vertical	sq.m	570	110	62,700
		sq.m	28.3	140	3,960
		sq.m	10.3	110	1,130
	<u>Total for Item 1</u>				<u>427,235</u>
2. Radial Collector					
2.1 Collector Pipe	GI, φ75 mm x 4 m (φ12 mm, 400 holes) x 48 collectors	m	192	270	51,840
	GI, φ75 mm x 2 m (φ12 mm, 300 holes) x 48 collectors	m	96	270	25,920
	GI, φ75 mm x 2.5 m (blank) x 48 collectors	m	120	180	21,600
	GI, Cap, φ75 mm	pcs.	48	30	1,440
2.2 Gate Valve	φ75 mm	pcs.	48	2,690	129,120
2.3 Water Stopper	φ75 mm	pcs.	48	1,340	64,320
2.4 Horizontal Boring	Caisson Wall, t=0.5 m Radial Well, 7 m x 48 collectors	holes	48	1,500	72,000
		m	336	1,160	389,760
	<u>Total for Item 2</u>				<u>756,000</u>
	<u>TOTAL OF ITEM 1 &amp; 2</u>				<u>1,183,235</u>
3. Temporary Facility	5% of Total of Item 1 & 2				59,162
	<u>TOTAL COST</u>				<u>1,242,397</u>

APPENDIX 7.2.2 DATA ON THE UNIT COST FOR ESTIMATION OF PROJECT COST

(1) Deep Well Construction : Peso

<u>Depth (m)</u>	<u>Casing size (m/m)</u>	<u>Cost</u>
200	250	940,000
200	300	1,160,000
250	150	640,000

BREAKDOWN OF COSTS IN %

	<u>Local Component</u>			<u>F E C</u>		<u>Total</u>
	<u>Material</u>	<u>Labor</u>		<u>Direct</u>	<u>Indirect</u>	
		<u>Skilled</u>	<u>Unskilled</u>			
Equipment	17	-	-	-	20	37
Civil Works	33	8	5	-	17	63
Total	50	8	5	-	37	100

(2) Deep Well Pump Station (Electric Motor Drive) : Thousand Peso

<u>KW</u>	<u>Cost</u>
7	450
15	560
22	640
29	720
37	790
44	840
51	890
59	960
66	1,020
74	1,080

BREAKDOWN OF COSTS IN %

	<u>Local Component</u>			<u>F E C</u>		<u>Total</u>
	<u>Material</u>	<u>Labor</u>		<u>Direct</u>	<u>Indirect</u>	
		<u>Skilled</u>	<u>Unskilled</u>			
Equipment	9	-	-	42	5	56
Civil Works	21	9	5	-	9	44
Total	30	9	5	42	14	100

(3) Booster Pump Station

$$C = (72.16 - 13.68 \log Q) \times Q^{(0.42 + 0.1 \log Q)} \times H^{(6/H - 0.25) \times 0.305(\log Q - 0.7)}$$

where,

C = cost for electric motor drive (thousand peso)

Q = design capacity (l/sec)

H = total dynamic head (m)

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	11	-	-	53	2	66
Civil Works	17	9	6	-	2	34
Total	28	9	6	53	4	100

(4) Radial Well

Inner Diameter (m)	Inner Depth (m)	Collection Pipe Length (m/hole)	Unit Cost (₱)
5	10	7.5	1,240,000
6	16	12.5	1,760,000

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	1	-	-	18	1	20
Civil Works	25	2	2	-	51	80
Total	26	2	2	18	52	100

(5) Pipeline Cost

Following pipe materials are presently available in the Philippines:

- GI (galvanized iron),
- PE (poly-ethylene),
- PB (poly-butylene),
- PVC (poly-vinyl-chloride),
- SP (steep pipe),
- CI (cast iron), and
- AC (asbestos cement).

Among these materials, the use of CI pipe is limited due to its high cost and AC pipe is also rare by safety reason.

Followings are comparison of unit cost at the 1985 price level.

Diameter(mm)	(Unit: ₱/m)				
	GI	PE	PB	PVC	SP
13	20.8	13.8	9.1	-	-
19	24.7	19.9	13.6	-	-
25	32.3	25.3	22.0	-	-
38	59.2	41.5	44.7	-	-
50	87.5	61.4	76.4	33.9	-
63	117.7	-	-	48.0	-
75	180.3	-	-	81.3	-
100	230.8	-	-	122.4	235.0
150	-	-	-	256.9	250.0
200	-	-	-	506.5	290.0
250	-	-	-	-	315.0
300	-	-	-	-	425.0
400	-	-	-	-	520.0
500	-	-	-	-	700.0
600	-	-	-	-	890.0

Based on the above comparison, SP is advantageous for the diameter of 200 mm and above than PVC. Thus, for the cost estimates of major transmission and distribution pipes, SP is considered for diameter of 200 mm and above, while PVC for diameter of less than 150 mm taking into account the transportation cost and easy installation.

<u>Diameter (mm)</u>	<u>Unit Cost (₱/m)</u>
150(PVC)	410
200(SP)	520
250( ")	630
300( ")	760
350( ")	900
400( ")	970
450( ")	1,160
500( ")	1,330
600( ")	1,600
700( ")	1,910

Source : LWUA Design Dept.

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	23	-	-	4	27	54
Civil Works	17	7	4	-	18	46
Total	40	7	4	4	45	100

(6) Valve In-place Cost

<u>Diameter (mm)</u>	<u>Gate Valve (₱)</u>	<u>Butterfly Valve (₱)</u>
50	1,700	-
75	2,900	-
100	3,900	-
150	5,300	-
200	6,700	-
250	11,200	-
300	-	34,800
350	-	74,400
400	-	95,200
450	-	125,900
500	-	174,000
600	-	243,600
700	-	313,200

Source : LWUA Design Dept.

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	9	-	-	63	5	77
Civil Works	12	3	6	-	2	23
Total	21	3	6	63	7	100

(7) Internal Network

Population Density (Person/ha)	Total Length of Pipeline (m/ha)	Unit Cost (₹/ha)	
		Diameter (100/150)	Diameter (75/100)
50	64	18,300	14,900
60	67	19,300	15,700
75	72	20,900	16,800
100	80	23,100	18,700
150	90	25,700	21,000
200	100	28,300	-
250	108	30,400	-
300	116	32,500	-

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	22	-	-	7	27	56
Civil Works	17	7	4	-	16	44
Total	39	7	4	7	43	100

(8) In-place of Service Connections

Diameter (inch)	Without Meter ₱/unit	With Meter ₱/unit	Meters ₱/unit
1/2	450	810	400
5/8 - 3/4	520	1,280	880

SERVICE CONNECTION WITHOUT METER  
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	9	-	-	60	2.5	71.5
Civil Works	17	3	6	-	2.5	28.5
Total	26	3	6	60	5	100

SERVICE CONNECTION WITHOUT METER  
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	4	-	-	83	2	89
Civil Works	6	1	3	-	1	11
Total	10	1	3	83	3	100

(9) Fire Hydrant In-place Cost

Type	Size (mm)	Unit Cost (₱)
Commercial	150	16,800
Residential	100	9,400

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	8	-	-	57	5	70
Civil Works	10	8	10	-	2	30
Total	18	8	10	57	7	100



(10) Elevated Tank/Ground Reservoir

Elevated Tank:  $C = 0.615 H^{1.144} V^{0.749}$

Ground Reservoir:  $C = 20.05 V^{0.639}$

where, C = cost (thousand peso)

H = overflow elevation above ground level

V = storage volume (cu.m)

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	4	-	-	3	2	9
Civil Works	53	5	7	-	26	91
Total	57	5	7	3	28	100

(11) Gas Chlorinator In-place Cost

Type	Water Flow Condition	Maximum Chlorine Feed (kg/day)	Unit cost <sup>1/</sup> (₱)
I-A	constant	22	98,100
I-B	constant	45	119,100
II-A	Variable	22	147,700
II-B	Variable	45	169,300

<sup>1/</sup> Empty gas cylinders and automatic switchover include

TYPE I-A, I-B  
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	15	-	-	41	5	61
Civil Works	25	6	3	-	5	39
Total	40	6	3	41	10	100

TYPE II-A, II-B  
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	21	-	-	53	2	76
Civil Works	12	6	2	-	4	24
Total	33	6	2	53	6	100

(12) Administration & Operation Building

Future Service Population	Administration Bldg. (Thousand Peso)	Operation Center (Thousand Peso)
30,000	1,000	810
40,000	1,110	890
50,000	1,220	990
60,000	1,320	1,090
70,000	1,410	1,180
80,000	1,500	1,280
100,000	1,610	1,380
110,000	1,820	1,590

ADMINISTRATION BUILDING  
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	20	-	-	-	16	36
Civil Works	42	7	5	-	10	64
Total	62	7	5	-	26	100

OPERATION CENTER  
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	14	-	-	30	6	50
Civil Works	26	10	5	-	9	50
Total	40	10	5	30	15	100

(13) Energy Cost

$$C = N_p \times h \times P_u \times (E_m)^{-1}$$

$$N_p = Q \times g \times H \times (\text{Eff.} \times 1,000)^{-1}$$

where,

- C = cost (thousand peso)
- $N_p$  = pump power demand (kw)
- h = hours of operation
- P = unit power cost (₱/kWh)
- $E^u$  = motor efficiency (0.85)
- $Q^m$  = water pumped (kg/sec)
- g = gravity constant (9.81m/sq.sec)
- H = manometric head (m)
- Eff. = pump efficiency (average = 0.70)

(14) Chemical Cost

$$C = (\text{Annual Water Demand}) \cdot D \cdot U_{CL} \times 10^{-3}$$

where,

- C = annual cost for chlorine (₱)
- D = chlorine dosage (mg/l)
- $U_{CL}$  = unit cost of chlorine gas (₱/kg)

(15) Minimum Cost Diameter

Following cost function is applied to determine the most economical diameter of pipelines that are not simulated by the network analysis.

$$D_{min.} = 187.7 Q^{0.486} C^{-0.315} (E_c/O_e)^{0.17}$$

where,

- $D_{min.}$  = minimum cost diameter
- Q = water flow (l/sec)
- C = "C" value (Hazen William Formula)
- $E_c$  = energy cost (₱/kwh)
- $O_e$  = overall efficiency

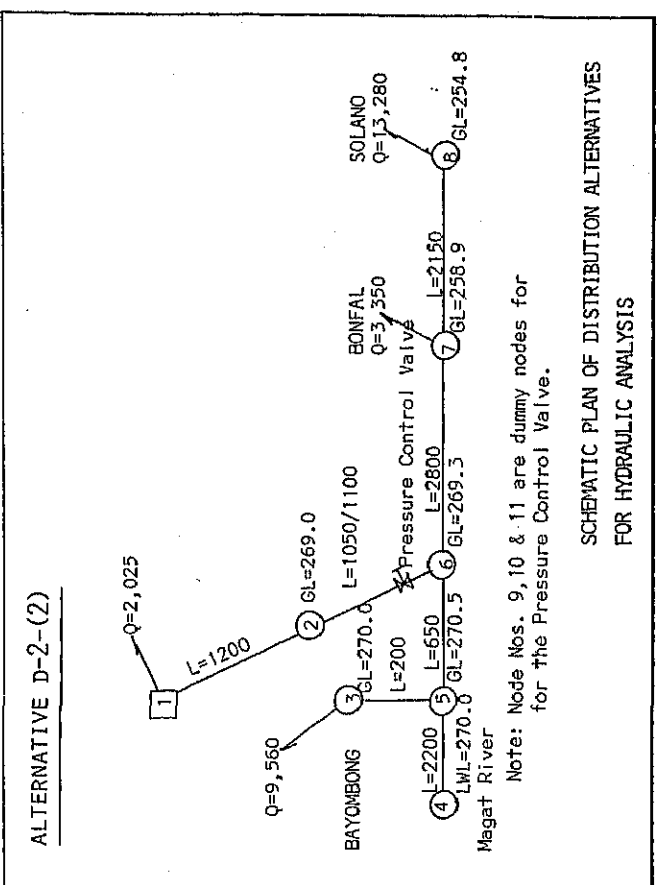
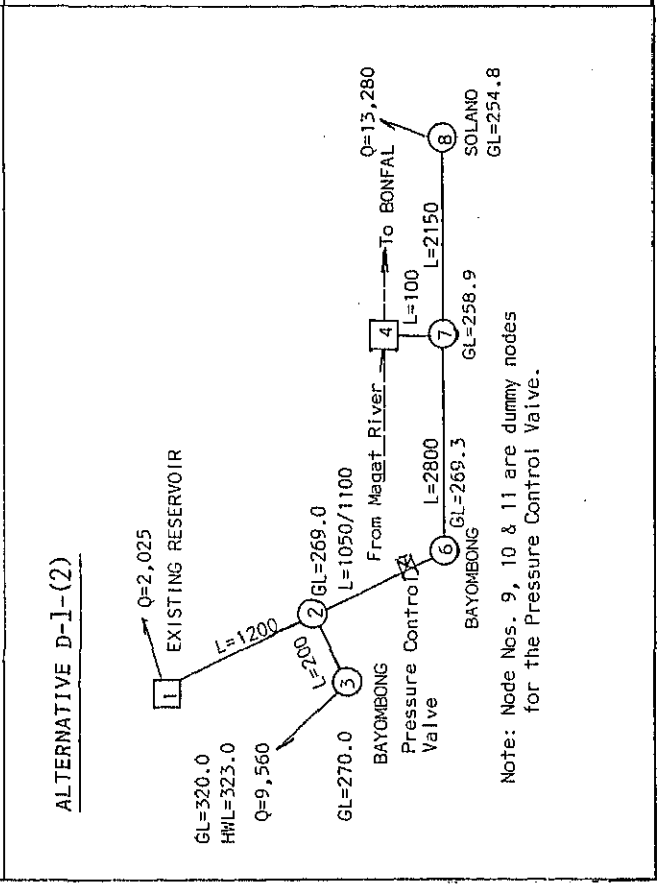
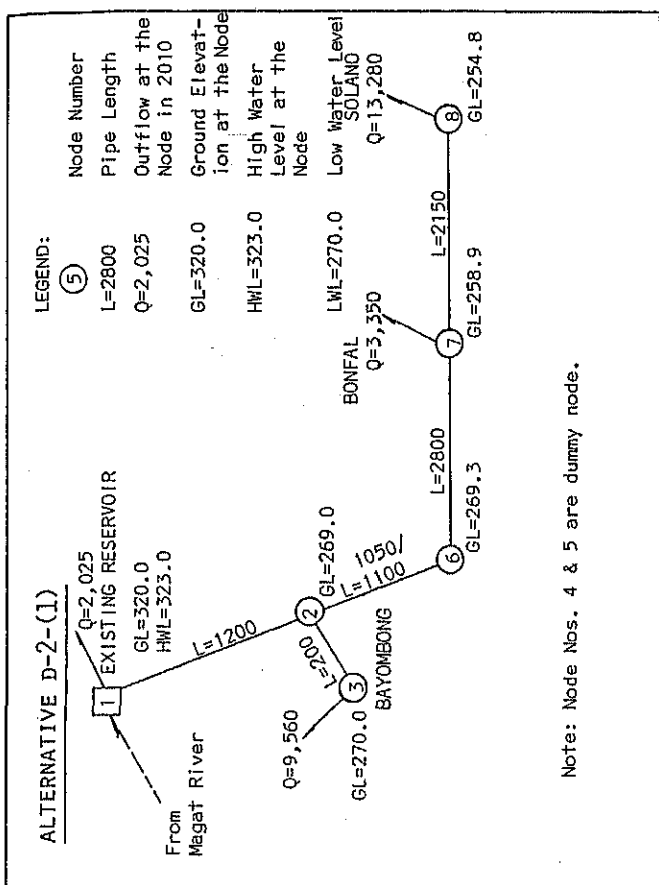
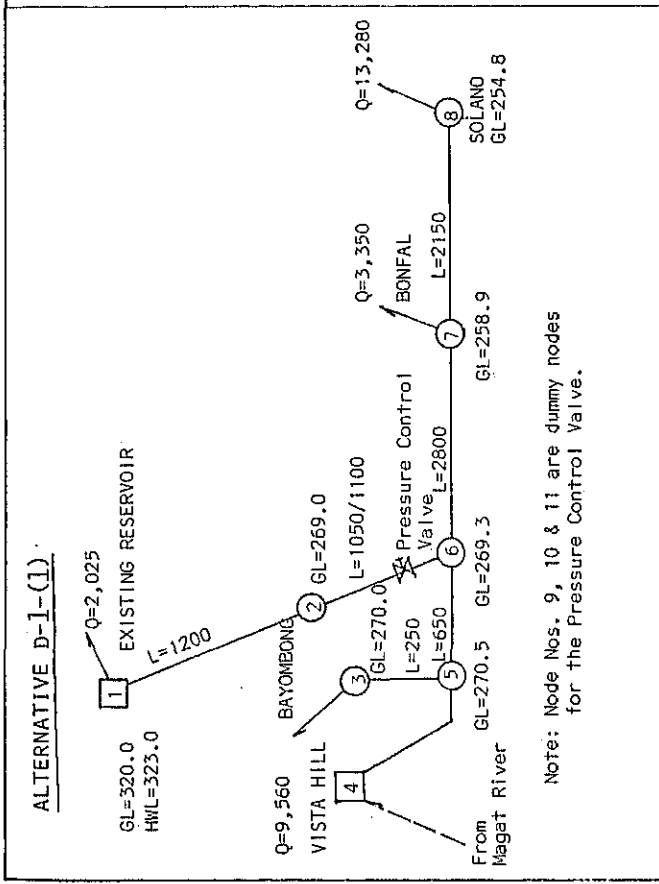
APPENDIX 7.3.1      COMPUTER-AIDED HYDRAULIC ANALYSIS OF DISTRIBUTION  
SYSTEM (Bayombong-Solano)

o List of Computed Cases

Alternative	D-1-(1)	(2010/Dry & Rainy Season)
	D-1-(2)	(2010/Dry & Rainy Season)
	D-2-(1)	(2010)
	D-2-(2)	(2010/Dry & Rainy Season)
	D-2-(2)-A	(1995, 2010/Dry & Rainy Season)
	D-2-(2)-A	(Fire at Solano)
	D-2-(2)-B	(1995, 2010/Dry & Rainy Season)
	ML-1	(2010)
	ML-1	(Fire at La Torre)
	ML-2	(2010/Masoc)
	ML-2	(2010/La Torre)
	ML-2	(Fire at Masoc, Fire at La Torre)
	ML-3	(2010)
	ML-3	(Fire at La Torre)

o Note

This appendix shows the results of Hydraulic Analysis aided by the computer. The distribution network is shown in the figure of following page. The nodes, however, with no flow and 20.00 m in Dynamic Head was treated as a dummy node. Those nodes can be ignored and have no relation to the computation results.



SCHEMATIC PLAN OF DISTRIBUTION ALTERNATIVES FOR HYDRAULIC ANALYSIS

ALTERNATIVE D-1-(1) Dry Season  
 2 Reservoir System (Existing & Vista Hill), Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	316.74	47.04	53.30
3	270.00	9560.00	277.01	7.01	53.00
4	270.00	-18094.00	300.29	30.29	53.00
5	270.50	0.00	295.03	25.53	52.50
6	269.30	0.00	293.80	24.50	53.70
7	258.90	3350.00	293.02	24.12	64.10
8	254.80	13280.00	274.17	19.37	68.20
9	269.30	0.00	313.81	44.51	53.70
10	0.00	0.00	293.81	293.81	323.00
11	0.00	0.00	313.80	313.80	323.00

Iteration Times : 40

ALTERNATIVE D-1-(1) Dry Season  
 2 Reservoir System (Existing & Vista Hill), Year 2010  
 << PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL (m/sec)	HEADLOSS (m) (0/00)
1	2 1	300.	1200.	120.	-5006.	-0.82	-3.26
2	2 1	250.	1200.	120.	-3099.	-0.73	-2.71
3	2 9	250.	1050.	120.	3147.	0.74	2.93
4	2 9	300.	1100.	120.	4958.	0.81	2.66
5	3 5	200.	250.	110.	-9560.	-3.52	-19.03
6	4 5	500.	1750.	120.	18094.	1.07	4.25
7	5 6	350.	650.	120.	8534.	1.03	2.24
8	6 10	350.	5.	120.	-8105.	-0.97	-3.13
9	6 7	250.	2800.	120.	3744.	0.88	10.78
10	6 7	400.	2800.	120.	12886.	1.19	10.78
11	7 8	250.	2150.	120.	3980.	0.91	8.84
12	7 8	350.	2150.	120.	9400.	1.13	8.84
13	9 11	350.	5.	120.	8105.	0.97	0.52

ALTERNATIVE D-1-(1) Rainy Season  
 2 Reservoir System (Existing & Vista Hill), Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	308.23	38.53	53.30
3	270.00	9560.00	278.06	8.06	53.00
4	270.00	-9960.00	298.50	28.50	53.00
5	270.50	0.00	297.09	25.59	52.50
6	269.30	0.00	297.08	27.78	53.70
7	258.90	3350.00	286.30	27.40	64.10
8	254.80	13280.00	277.46	22.66	68.20
9	269.30	0.00	297.64	28.34	53.70
10	0.00	0.00	297.14	297.14	323.00
11	0.00	0.00	297.58	297.58	323.00

Iteration Times : 32

ALTERNATIVE D-1-(1) Rainy Season  
 2 Reservoir System (Existing & Vista Hill), Year 2010  
 << PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL (m/sec)	HEADLOSS (m) (0/00)
1	2 1	300.	1200.	120.	-10018.	-1.64	-11.77
2	2 1	250.	1200.	120.	-6202.	-1.46	-11.77
3	2 9	250.	1050.	120.	6299.	1.49	10.59
4	2 9	300.	1100.	120.	9922.	1.62	10.59
5	3 5	200.	250.	110.	-9560.	-3.52	-19.03
6	4 5	500.	1750.	120.	9960.	0.59	1.41
7	5 6	350.	650.	120.	402.	0.05	0.01
8	6 10	350.	5.	120.	-16221.	-1.95	-0.06
9	6 7	250.	2800.	120.	3744.	0.88	10.78
10	6 7	400.	2800.	120.	12886.	1.19	10.78
11	7 8	250.	2150.	120.	3880.	0.91	8.84
12	7 8	350.	2150.	120.	9400.	1.13	8.84
13	9 11	350.	5.	120.	16221.	1.95	0.06

ALTERNATIVE D-1-(2) Dry Season  
 2 Reservoir System (Existing & Bonfal), Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	305.14	35.44	53.30
3	270.00	9560.00	300.92	30.92	53.00
4	258.90	-6605.00	282.48	23.58	64.10
5	270.50	0.00	290.50	20.00	52.50
6	269.30	0.00	297.12	27.82	53.70
7	258.90	0.00	282.42	23.52	64.10
8	254.80	13280.00	273.57	18.77	58.20
9	269.70	0.00	308.19	38.49	53.30
10	0.00	0.00	305.19	305.19	323.00
11	0.00	0.00	308.14	308.14	323.00

Iteration Times : 40

ALTERNATIVE D-1-(2) Rainy Season  
 2 Reservoir System (Existing & Bonfal), Year 2010  
 << PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 10	350.	5.	120.	-16249.	-1.95	-0.06
2	2 6	250.	1050.	120.	5419.	1.29	8.02
3	2 6	150.	1100.	110.	1264.	0.83	8.02
4	2 3	200.	200.	110.	4780.	1.76	4.22
5	2 3	200.	200.	110.	4780.	1.76	4.22
6	4 7	450.	100.	120.	6597.	0.48	0.06
7	4 7	250.	2800.	110.	4427.	1.04	14.70
8	6 7	200.	2800.	110.	2256.	0.83	14.70
9	7 8	250.	2150.	120.	3880.	0.91	8.84
10	7 8	350.	2150.	120.	9400.	1.13	8.84
11	9 1	300.	1200.	120.	-10036.	-1.64	-11.81
12	9 1	250.	1200.	120.	-6213.	-1.49	-11.81
13	9 11	350.	5.	120.	16249.	1.95	0.06

ALTERNATIVE D-1-(2) Dry Season  
 2 Reservoir System (Existing & Bonfal), Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	261.72	12.92	53.30
3	270.00	9560.00	277.51	7.51	53.00
4	258.90	-14739.00	283.36	24.46	64.10
5	270.50	0.00	290.50	20.00	52.50
6	269.30	0.00	282.20	12.90	53.70
7	258.90	0.00	283.08	24.18	64.10
8	254.80	13280.00	274.24	19.44	68.20
9	269.70	0.00	316.74	47.04	53.30
10	0.00	0.00	281.74	281.74	323.00
11	0.00	0.00	316.72	316.72	323.00

Iteration Times : 39

ALTERNATIVE D-1-(2) Dry Season  
 2 Reservoir System (Existing & Bonfal), Year 2010  
 << PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 10	350.	5.	120.	-8107.	-0.98	-3.13
2	2 6	250.	1050.	120.	-1182.	-0.28	-0.48
3	2 6	150.	1100.	110.	-276.	-0.18	-0.48
4	2 3	200.	200.	110.	4780.	1.76	4.22
5	2 3	200.	200.	110.	4780.	1.76	4.22
6	4 7	450.	100.	120.	14738.	1.07	0.28
7	4 7	250.	2800.	120.	-966.	-0.23	-0.31
8	6 7	200.	2800.	110.	-492.	-0.18	-0.31
9	7 8	250.	2150.	120.	3980.	0.91	8.84
10	7 8	350.	2150.	120.	9400.	1.13	8.84
11	9 1	300.	1200.	120.	-5009.	-0.82	-3.26
12	9 1	250.	1200.	120.	-3101.	-0.73	-3.26
13	9 11	350.	5.	120.	8107.	0.98	0.02

ALTERNATIVE D-2-(2) Rainy Season  
 1 Reservoir System, Transmission to the Town Proper, Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	309.04	39.34	53.30
3	270.00	9560.00	282.29	12.25	53.00
4	270.00	-16047.00	298.99	28.99	53.00
5	270.50	0.00	297.47	26.97	52.50
6	269.30	0.00	296.77	27.47	53.70
7	268.90	3350.00	285.99	27.09	64.10
8	254.80	13280.00	277.14	22.34	58.20
9	269.30	0.00	301.79	32.49	53.70
10	0.00	0.00	296.79	296.79	323.00
11	0.00	0.00	301.77	301.77	323.00

Iteration Times : 50

ALTERNATIVE D-2-(2) Rainy Season  
 1 Reservoir System, Transmission to the Town Proper, Year 2010  
 << PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 1	300.	1200.	120.	-9643.	-1.58	-10.96
2	2 1	100.	1200.	110.	-492.	-0.72	-10.96
3	2 9	250.	1050.	120.	5131.	1.21	7.25
4	2 9	250.	1100.	120.	5004.	1.18	7.25
5	3 5	200.	200.	110.	-9560.	-3.52	-15.22
6	4 5	600.	2200.	130.	16047.	0.66	1.52
7	5 6	400.	650.	120.	6487.	0.60	0.70
8	6 10	350.	5.	120.	-10135.	-1.22	-0.02
9	6 7	250.	2800.	120.	3744.	0.66	10.78
10	6 7	400.	2800.	120.	12886.	1.19	10.78
11	7 8	250.	2150.	120.	3880.	0.91	8.84
12	7 8	350.	2150.	120.	9400.	1.13	8.84
13	9 11	350.	5.	120.	10135.	1.22	0.02

ALTERNATIVE D-2-(2) Dry Season  
 1 Reservoir System, Transmission to the Town Proper, Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	317.33	47.63	53.30
3	270.00	9560.00	280.50	10.50	53.00
4	270.00	-21459.00	288.33	28.33	53.00
5	270.50	0.00	289.72	25.22	52.90
6	269.30	0.00	293.55	24.25	53.70
7	268.90	3350.00	282.77	23.87	64.10
8	254.80	13280.00	273.93	19.13	58.20
9	269.30	0.00	315.56	46.26	53.70
10	0.00	0.00	293.56	293.56	323.00
11	0.00	0.00	315.55	315.55	323.00

Iteration Times : 54

ALTERNATIVE D-2-(2) Dry Season  
 1 Reservoir System, Transmission to the Town Proper, Year 2010  
 << PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 1	300.	1200.	120.	-4500.	-0.74	-2.67
2	2 1	100.	1200.	110.	-229.	-0.34	-2.67
3	2 9	250.	1050.	120.	2394.	0.56	1.77
4	2 9	250.	1100.	120.	2355.	0.55	1.77
5	3 5	200.	200.	110.	-9560.	-3.52	-15.22
6	4 5	600.	2200.	130.	21469.	0.89	2.61
7	5 6	400.	650.	120.	11909.	1.10	2.16
8	6 10	350.	5.	120.	-4729.	-0.57	-0.01
9	6 7	250.	2800.	120.	3744.	0.68	10.78
10	6 7	400.	2800.	120.	12886.	1.19	10.78
11	7 8	250.	2150.	120.	3880.	0.91	8.84
12	7 8	350.	2150.	120.	9400.	1.13	8.84
13	9 11	350.	5.	120.	4729.	0.57	0.01



ALTERNATIVE D-2-(1) Both Season  
 1 Reservoir System, Transmission to the Existing Reservoir, Year2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	2025.00	320.00	0.00	3.00
2	269.70	0.00	311.58	41.88	53.30
3	270.00	8560.00	295.35	26.35	53.00
4	258.90	0.00	278.90	20.00	54.10
5	270.50	0.00	290.50	20.00	52.50
6	269.30	0.00	300.49	31.19	53.70
7	258.90	3350.00	283.03	24.13	54.10
8	254.80	13280.00	274.19	19.39	58.20

Iteration Times : 4

ALTERNATIVE D-2-(1) Both Season  
 1 Reservoir System, Transmission to the Existing Reservoir, Year2010

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 1	300.	1200.	120.	-8362.	-1.37	-8.42
2	2 1	400.	1200.	120.	-17820.	-1.64	-8.42
3	2 6	250.	1050.	120.	6455.	1.52	11.09
4	2 6	300.	1100.	120.	10169.	1.67	11.09
5	2 3	200.	200.	110.	9580.	3.52	15.22
6	6 7	250.	2800.	120.	4858.	1.15	17.45
7	6 7	350.	2800.	120.	11769.	1.42	17.45
8	7 8	250.	2150.	120.	3880.	0.91	8.84
9	7 8	350.	2150.	120.	9400.	1.13	8.84

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 1995, Dry Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300.	300.	120.	-1057.	-0.17	-0.05
2	3	300.	500.	120.	936.	0.15	0.06
3	4	150.	700.	110.	121.	0.08	0.07
4	5	250.	500.	120.	573.	0.14	0.06
5	6	250.	800.	120.	339.	0.08	0.04
6	7	300.	800.	120.	113.	0.03	0.00
7	8	1	1200.	120.	-5713.	-0.94	-4.16
8	8	1	100.	110.	-291.	-0.43	-4.16
8	8	49	250.	120.	3040.	0.72	2.75
8	8	49	250.	120.	2964.	0.70	2.50
9	10	200.	1100.	120.	102.	-0.04	0.00
10	11	200.	200.	110.	-102.	-0.04	0.00
11	12	200.	150.	110.	-316.	-0.12	-0.02
12	13	11	12	100.	100.	100.	0.14
13	14	11	24	100.	450.	110.	0.19
14	15	12	13	200.	250.	110.	0.23
15	16	12	14	200.	250.	110.	0.27
16	17	12	14	200.	250.	110.	0.27
17	18	12	21	100.	193.	0.05	0.01
18	19	20	200.	110.	93.	0.14	0.08
19	20	200.	400.	110.	639.	0.24	0.20
20	21	22	200.	110.	1168.	0.43	0.36
21	22	200.	230.	110.	1168.	0.43	0.36
22	23	250.	5.	120.	-3934.	-0.93	-0.02
23	24	15	100.	250.	113.	0.17	0.15
24	25	16	200.	150.	110.	0.03	0.00
25	26	17	100.	250.	110.	-0.30	-0.44
26	27	18	100.	200.	113.	0.17	0.12
27	28	19	150.	150.	110.	0.28	0.15
28	29	20	100.	300.	427.	0.29	0.51
29	30	200.	5.	110.	-1465.	-0.54	-0.01
30	31	22	100.	300.	8.	-0.01	0.00
31	32	23	200.	220.	844.	0.31	0.19
32	33	24	100.	500.	-4.	-0.01	0.00
33	34	27	200.	200.	531.	0.20	0.07
34	35	53	500.	110.	8476.	0.50	0.30
35	36	49	500.	700.	-7010.	-0.41	-0.29
36	37	47	400.	650.	3075.	0.28	0.18
37	38	27	200.	700.	215.	0.08	0.05
38	39	47	250.	2800.	-7042.	-0.48	-3.51
39	40	47	400.	2800.	-7029.	-0.65	-3.51
40	41	30	200.	350.	1241.	0.46	0.61
41	42	33	250.	1550.	7253.	1.71	21.62
42	43	31	150.	400.	478.	0.31	0.48
43	44	32	150.	350.	239.	0.26	0.12
44	45	33	34	150.	395.	0.26	0.38
45	46	35	37	250.	1288.	0.47	0.55
46	47	36	150.	500.	4529.	1.07	2.74
47	48	37	38	200.	644.	0.42	0.63
48	49	38	39	200.	1968.	0.73	0.41
49	50	39	200.	300.	2200.	0.81	1.50
50	51	40	200.	800.	808.	0.30	0.27
51	52	41	42	200.	1840.	0.68	1.25
52	53	41	43	150.	848.	0.31	0.17
53	54	42	46	150.	424.	0.28	0.67
54	55	43	44	150.	404.	0.28	0.58
55	56	44	150.	350.	404.	0.26	0.31
56	57	48	53	600.	-6004.	-0.72	-0.02
57	49	51	350.	10.	6004.	0.72	0.02

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 1995, Dry Season, Modification of ALT. D-2-(2)

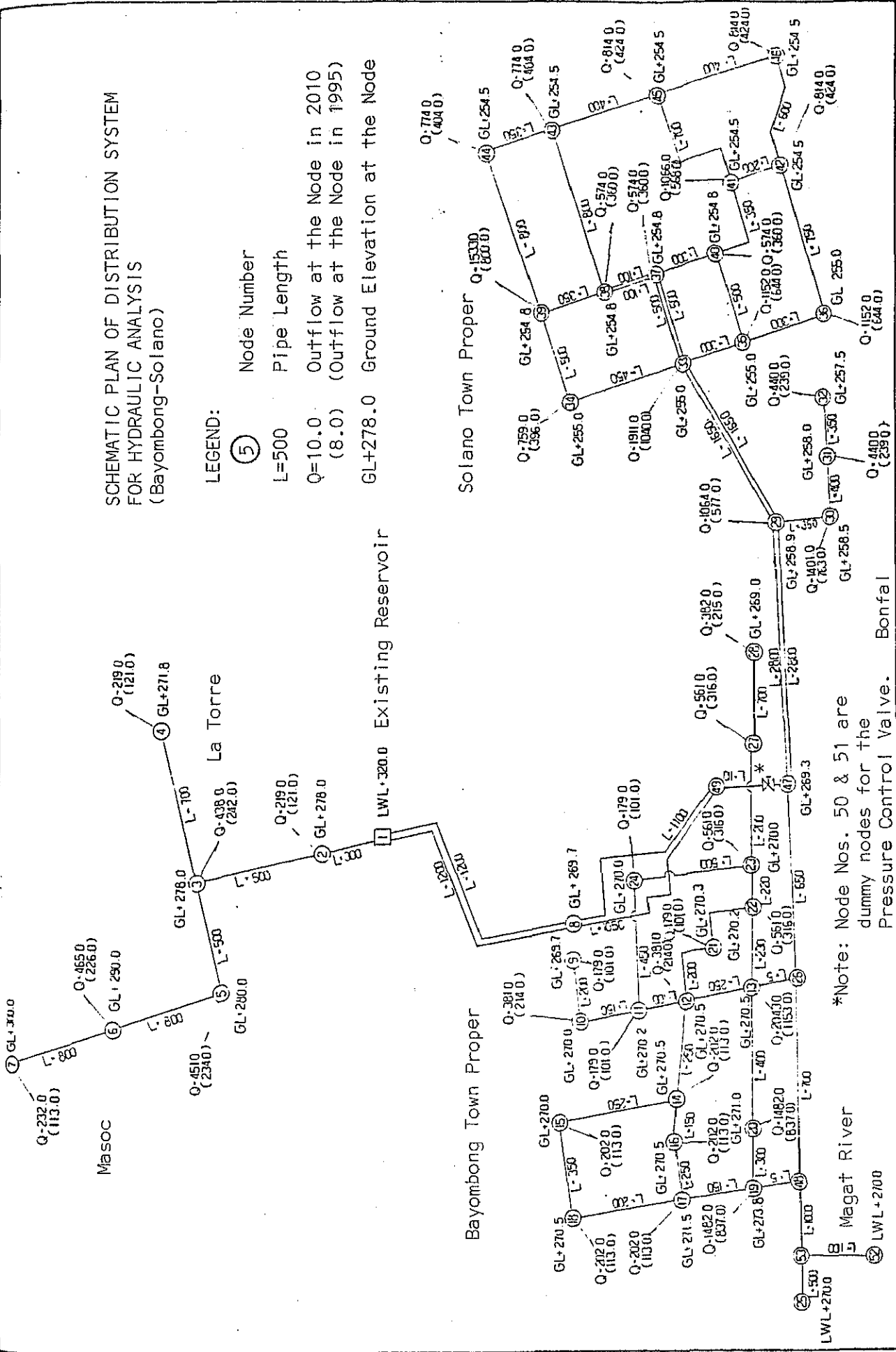
<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	48	53	600.	1000.	-8476.	-0.39	-0.21
57	49	51	350.	10.	6004.	0.72	0.02

**SCHEMATIC PLAN OF DISTRIBUTION SYSTEM  
FOR HYDRAULIC ANALYSIS  
(Bayombong-Solano)**

**LEGEND:**

- ⑤ Node Number
- L=500 Pipe Length
- Q=10.0 Outflow at the Node in 2010  
(8.0) (Outflow at the Node in 1995)
- GL+278.0 Ground Elevation at the Node



\*Note: Node Nos. 50 & 51 are dummy nodes for the Pressure Control Valve. Bonfal

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 1995, Dry Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	121.00	319.95	41.95	45.00
3	278.00	242.00	319.89	41.89	45.00
4	271.80	121.00	319.83	48.03	51.20
5	280.00	234.00	319.83	39.83	43.00
6	290.00	226.00	319.80	29.80	33.00
7	300.00	113.00	319.79	19.79	23.00
8	269.70	0.00	315.84	46.14	53.30
9	269.70	101.00	292.40	22.70	53.30
10	270.00	214.00	292.40	22.40	53.00
11	270.20	101.00	292.42	22.22	52.80
12	270.50	214.00	292.46	21.96	52.50
13	270.50	1153.00	292.73	22.23	52.50
14	270.50	113.00	292.45	21.95	52.50
15	270.00	113.00	292.30	22.30	53.00
16	270.50	113.00	292.45	21.95	52.50
17	271.50	113.00	292.89	21.39	51.50
18	270.50	113.00	292.77	22.27	52.50
19	273.80	837.00	293.03	19.23	49.20
20	271.00	837.00	292.53	21.53	52.00
21	270.30	101.00	292.37	22.07	52.70
22	270.20	316.00	292.37	22.17	52.80
23	270.00	316.00	292.19	22.19	53.00
24	270.00	101.00	292.19	22.19	53.00
25	270.00	-8477.00	293.56	23.56	53.00
26	270.50	0.00	292.75	22.25	52.50
27	269.30	316.00	292.11	22.81	53.70
28	269.00	215.00	292.07	23.07	54.00
29	258.90	577.00	289.07	30.17	64.10
30	258.50	763.00	288.46	29.96	64.50
31	258.00	239.00	287.98	29.98	65.00
32	257.50	239.00	287.86	30.36	65.50
33	255.00	1040.00	267.45	12.45	68.00
34	255.00	396.00	267.07	12.07	68.00
35	255.00	644.00	266.89	11.89	68.00
36	255.00	644.00	266.26	11.26	68.00
37	254.80	360.00	264.71	9.91	68.20
38	254.80	360.00	264.30	9.50	68.20
39	254.80	800.00	264.03	9.23	68.20
40	254.50	360.00	263.21	8.71	68.50
41	254.50	568.00	261.95	7.45	68.50
42	254.50	424.00	261.78	7.28	68.50
43	254.50	404.00	263.68	9.18	68.50
44	254.50	404.00	263.37	8.87	68.50
45	254.50	424.00	261.27	6.77	68.50
46	254.50	424.00	261.20	6.70	68.50
47	269.30	0.00	292.57	23.27	53.70
48	273.80	0.00	293.05	19.25	49.20
49	269.30	0.00	313.09	43.79	53.70
50	0.00	0.00	292.59	292.59	323.00
51	0.00	0.00	313.07	313.07	323.00
52	270.00	0.00	290.00	20.00	53.00
53	275.00	0.00	293.26	18.26	48.00

Iteration Times : 43

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 1995, Rainy Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	121.00	319.95	41.95	45.00
3	278.00	242.00	319.89	41.89	45.00
4	271.80	121.00	319.83	48.03	51.20
5	280.00	234.00	319.83	39.83	43.00
6	290.00	226.00	319.80	29.80	33.00
7	300.00	113.00	319.79	19.79	23.00
8	269.70	0.00	305.73	36.03	53.30
9	269.70	101.00	292.73	23.03	53.30
10	270.00	214.00	292.73	22.73	53.00
11	270.20	101.00	292.75	22.55	52.60
12	270.50	214.00	292.79	22.29	52.50
13	270.50	1153.00	293.09	22.59	52.50
14	270.50	113.00	292.77	22.27	52.50
15	270.00	113.00	292.62	22.62	53.00
16	270.50	113.00	292.77	22.27	52.50
17	271.50	113.00	293.00	21.50	51.50
18	270.50	113.00	292.88	22.38	52.50
19	273.80	837.00	293.12	19.32	49.20
20	271.00	837.00	292.85	21.85	52.00
21	270.30	101.00	292.72	22.42	52.70
22	270.20	316.00	292.72	22.52	52.80
23	270.00	316.00	292.53	22.53	53.00
24	270.00	101.00	292.53	22.53	53.00
25	270.00	-2825.00	293.19	23.19	53.00
26	270.50	0.00	293.11	22.61	52.50
27	269.30	316.00	292.46	23.16	53.70
28	269.00	215.00	292.41	23.41	54.00
29	258.90	577.00	289.73	30.83	64.10
30	258.50	763.00	289.12	30.62	64.50
31	258.00	239.00	288.64	30.64	65.00
32	257.50	239.00	288.52	31.02	65.50
33	255.00	1040.00	268.11	13.11	68.00
34	255.00	396.00	267.73	12.73	68.00
35	255.00	644.00	267.55	12.55	68.00
36	255.00	644.00	266.93	11.93	68.00
37	254.80	360.00	265.37	10.57	68.20
38	254.80	360.00	264.96	10.16	68.20
39	254.80	800.00	264.69	9.89	68.20
40	254.50	360.00	263.87	9.37	68.50
41	254.50	568.00	262.61	8.11	68.50
42	254.50	424.00	262.44	7.94	68.50
43	254.50	404.00	264.34	9.84	68.50
44	254.50	404.00	264.03	9.53	68.50
45	254.50	424.00	261.93	7.43	68.50
46	254.50	424.00	261.86	7.36	68.50
47	269.30	0.00	293.24	23.94	53.70
48	273.80	0.00	293.13	19.33	49.20
49	269.30	0.00	296.30	27.00	53.70
50	0.00	0.00	293.30	293.30	323.00
51	0.00	0.00	296.24	296.24	323.00
52	270.00	0.00	290.00	20.00	53.00
53	275.00	0.00	293.15	18.15	48.00

Iteration Times : 39

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 1995, Rainy Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300	300	120	-1057	-0.17	-0.05
2	3	300	500	120	936	0.15	0.12
3	4	150	700	110	121	0.08	0.07
4	5	250	500	120	573	0.14	0.12
5	6	250	800	120	339	0.08	0.06
6	7	300	800	120	113	0.03	0.01
7	8	100	1200	110	-1117	-1.82	-1.89
8	9	250	1050	120	567	0.84	0.89
9	10	250	1100	120	5915	1.39	9.43
10	11	200	200	110	769	1.36	8.57
11	12	200	150	110	-102	-0.04	-0.02
12	13	200	100	110	-317	-0.12	-0.14
13	14	200	450	110	-520	-0.19	-0.23
14	15	200	250	110	101	0.15	0.22
15	16	200	250	110	-1020	-0.38	-1.21
16	17	200	250	110	201	0.07	0.01
17	18	200	200	110	84	0.12	0.07
18	19	200	400	110	697	0.26	0.24
19	20	200	230	110	1185	0.44	0.37
20	21	250	5	120	-4053	-0.95	-0.92
21	22	100	250	110	113	0.17	0.15
22	23	200	150	110	-30	-0.01	0.00
23	24	100	250	110	-143	-0.21	-0.23
24	25	100	200	110	113	0.17	0.12
25	26	150	150	110	-369	-0.24	-0.11
26	27	100	300	110	140	0.21	0.27
27	28	200	5	110	-1352	-0.50	-0.01
28	29	100	300	110	-11	-0.02	-0.02
29	30	200	220	110	851	0.31	0.19
30	31	200	500	110	0	0.00	0.00
31	32	200	200	110	531	0.20	0.07
32	33	200	200	110	2822	0.17	0.04
33	34	500	500	120	-1455	-0.09	-0.02
34	35	400	650	120	-2604	-0.24	-0.13
35	36	200	700	110	215	0.08	0.05
36	37	250	2800	120	-2042	-0.48	-3.51
37	38	400	2800	120	-7029	-0.65	-3.51
38	39	200	350	110	1241	0.46	0.61
39	40	250	1650	120	7253	1.71	21.62
40	41	150	400	110	476	0.31	0.48
41	42	150	350	110	239	0.16	0.12
42	43	150	450	110	396	0.26	0.35
43	44	200	300	110	1288	0.47	0.56
44	45	250	500	120	4529	1.07	2.74
45	46	150	300	110	644	0.42	0.63
46	47	200	100	110	1966	0.73	0.41
47	48	200	300	110	2200	0.81	1.50
48	49	200	350	110	800	0.29	0.27
49	50	200	800	110	808	0.30	0.63
50	51	200	350	110	1840	0.68	1.26
51	52	200	200	110	848	0.31	0.17
52	53	150	700	110	424	0.28	0.85
53	54	150	500	110	424	0.28	0.58
54	55	150	350	110	404	0.25	0.31
55	56	350	10	120	-11684	-1.41	-0.06

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 1995, Rainy Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	48	53	600	130	-2822	-0.12	-0.03
57	49	51	350	120	11684	1.41	0.06

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	219.00	319.85	41.85	45.00
3	278.00	438.00	319.64	41.64	45.00
4	271.80	219.00	319.44	47.64	51.20
5	280.00	451.00	319.43	39.43	43.00
6	290.00	465.00	319.29	29.29	33.00
7	300.00	232.00	319.27	19.27	23.00
8	269.70	0.00	317.34	47.64	53.30
9	269.70	179.00	288.87	19.17	53.30
10	270.00	381.00	288.88	18.88	53.00
11	270.20	179.00	288.94	18.74	52.80
12	270.50	381.00	289.04	18.54	52.50
13	270.50	2043.00	289.67	19.17	52.50
14	270.50	202.00	289.04	18.54	52.50
15	270.00	202.00	289.04	19.04	53.00
16	270.50	202.00	289.05	18.55	52.50
17	271.50	202.00	290.75	19.25	51.50
18	270.50	202.00	289.57	19.07	52.50
19	273.80	1482.00	291.47	17.67	49.20
20	271.00	1482.00	289.18	18.18	52.00
21	270.30	179.00	288.69	18.39	52.70
22	270.20	561.00	288.69	18.49	52.80
23	270.00	561.00	288.15	18.15	53.00
24	270.00	179.00	288.17	18.17	53.00
25	270.00	-12883.00	293.35	23.35	53.00
26	270.50	0.00	289.73	19.23	52.50
27	269.30	561.00	287.95	18.65	53.70
28	269.00	382.00	287.81	18.81	54.00
29	258.90	1064.00	276.79	17.89	64.10
30	258.50	1401.00	274.91	16.41	64.50
31	258.00	440.00	273.42	15.42	65.00
32	257.50	440.00	273.06	15.56	65.50
33	255.00	1911.00	269.99	14.99	68.00
34	255.00	759.00	267.78	12.78	68.00
35	255.00	1152.00	267.57	12.57	68.00
36	255.00	1152.00	264.94	9.94	68.00
37	254.80	574.00	268.06	13.26	68.20
38	254.80	574.00	267.28	12.48	68.20
39	254.80	1533.00	266.35	11.55	68.20
40	254.50	574.00	265.69	11.19	68.50
41	254.50	1066.00	263.39	8.89	68.50
42	254.50	814.00	263.09	8.59	68.50
43	254.50	774.00	264.00	9.50	68.50
44	254.50	774.00	263.57	9.07	68.50
45	254.50	814.00	262.97	8.47	68.50
46	254.50	814.00	262.05	7.55	68.50
47	269.30	0.00	287.57	18.27	53.70
48	273.80	0.00	291.51	17.71	49.20
49	269.30	0.00	315.58	46.28	53.70
50	0.00	0.00	287.58	287.58	323.00
51	0.00	0.00	315.57	315.57	323.00
52	270.00	-8588.00	293.04	23.04	53.00
53	275.00	0.00	292.70	17.70	48.00

Iteration Times : 66

ALI. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
Year 2010, Dry Season, Modification of ALI. D-2-(2)

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300	300	120	-2024	-0.33	-0.15
2	3	300	500	120	1805	0.30	0.21
3	4	150	700	110	219	0.14	0.20
4	5	250	500	120	1148	0.27	0.22
5	6	250	800	120	697	0.16	0.14
6	7	250	800	120	232	0.05	0.02
7	8	300	1200	120	-4491	-0.74	-2.66
8	9	250	1050	120	-229	-0.34	-2.22
9	10	250	1100	120	2389	0.56	1.76
10	11	200	200	110	2330	0.55	1.76
11	12	200	200	110	-119	-0.07	-0.01
12	13	200	150	110	-560	-0.21	-0.06
13	14	200	100	110	-938	-0.35	-0.10
14	15	200	450	110	199	0.29	0.77
15	16	200	250	110	-1521	-0.56	-0.63
16	17	200	250	110	9	0.00	0.00
17	18	200	200	110	200	0.30	0.35
18	19	200	400	110	1036	0.38	0.50
19	20	200	230	110	2024	0.75	0.99
20	21	250	5	120	-6623	-1.56	-0.06
21	22	100	250	110	16	0.02	0.02
22	23	150	150	110	-217	-0.08	-0.01
23	24	150	350	110	-186	-0.27	-0.53
24	25	100	250	110	-419	-0.52	-1.70
25	26	100	200	110	388	0.51	1.18
26	27	150	150	110	-1009	-0.66	-0.72
27	28	100	369	110	446	0.66	2.29
28	29	200	5	110	-2937	-1.08	-0.04
29	30	200	300	110	21	0.03	0.01
30	31	200	200	110	1484	0.55	0.53
31	32	200	500	110	-20	-0.02	-0.01
32	33	200	200	110	943	0.35	0.21
33	34	500	500	120	12883	0.76	0.65
34	35	500	700	120	-18534	-1.09	-1.78
35	36	400	650	120	11911	1.10	2.16
36	37	200	700	110	382	0.14	0.14
37	38	250	2800	120	-3744	-0.88	-10.78
38	39	400	2800	120	-12888	-1.19	-10.78
39	40	200	350	110	2281	0.84	1.88
40	41	250	1650	120	3881	0.92	6.79
41	42	350	1550	120	9404	1.13	6.79
42	43	150	400	110	880	0.58	1.49
43	44	150	350	110	440	0.29	0.35
44	45	200	450	110	1022	0.67	2.21
45	46	200	300	110	2851	1.05	2.43
46	47	250	300	120	3751	0.88	1.93
47	48	300	500	120	263	0.39	1.43
48	49	100	300	110	1395	0.91	2.62
49	50	150	500	110	304	0.45	1.88
50	51	100	750	110	243	0.36	1.95
51	52	200	100	110	2798	1.03	0.78
52	53	150	100	110	1313	0.86	0.78
53	54	200	300	110	2816	1.04	2.37
54	55	200	350	110	1562	0.58	0.93

ALI. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
Year 2010, Dry Season, Modification of ALI. D-2-(2)

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	38	43	200	110	1975	0.73	3.28
57	39	44	100	800	292	0.43	2.78
58	40	41	200	350	2546	0.94	2.30
59	41	42	200	200	1152	0.42	0.30
60	41	45	150	700	328	0.21	0.42
61	42	45	150	500	581	0.38	1.04
62	43	44	150	350	482	0.32	0.43
63	43	45	150	400	719	0.47	1.03
64	45	46	100	400	233	0.34	0.92
65	47	50	350	10	-4720	-0.57	-0.01
66	48	53	600	1000	-21471	-0.88	-1.19
67	49	51	350	10	4720	0.57	0.01
68	52	53	350	100	8588	1.03	0.35



ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300	300	120	-2024	-0.33	-0.15
2	3	300	500	120	1805	0.30	0.21
3	4	150	700	110	218	0.14	0.28
4	5	250	500	120	1148	0.27	0.22
5	6	250	800	120	697	0.16	0.14
6	7	250	800	120	230	0.09	0.02
7	8	300	1200	120	-9630	-1.58	-10.94
8	9	250	1050	120	5124	1.21	7.23
9	10	200	1100	120	4997	1.18	7.23
10	11	200	200	110	-178	-0.07	-0.01
11	12	200	150	110	-559	-0.21	-0.06
12	13	200	100	110	-930	-0.34	-1.02
13	14	100	450	110	192	0.28	0.72
14	15	200	250	110	-1624	-0.60	-2.86
15	16	200	250	110	130	0.05	0.01
16	17	100	200	110	183	0.27	0.29
17	18	200	400	110	1119	0.41	0.57
18	19	200	230	110	2047	0.75	1.91
19	20	250	5	120	-6829	-1.61	-11.72
20	21	100	250	110	66	0.10	0.06
21	22	150	150	110	-137	-0.05	0.00
22	23	100	350	110	-136	-0.20	-0.30
23	24	100	200	110	-339	-0.50	-1.15
24	25	100	200	110	338	0.50	0.91
25	26	150	150	110	-879	-0.58	-3.72
26	27	100	300	110	363	0.53	1.56
27	28	200	5	110	-2721	-1.00	-7.43
28	29	200	300	110	5	0.01	0.00
29	30	200	220	110	1490	0.55	0.54
30	31	200	500	110	-13	-0.02	-0.01
31	32	200	200	110	943	0.35	0.21
32	33	500	500	120	12038	0.71	0.57
33	34	500	700	120	-13330	-0.79	-1.38
34	35	400	650	120	6502	0.60	0.71
35	36	200	700	110	382	0.14	0.14
36	37	250	2800	120	-3743	-0.88	-10.78
37	38	400	2800	120	-12886	-1.19	-10.78
38	39	200	350	110	2281	0.84	1.88
39	40	250	1650	120	3881	0.92	6.79
40	41	350	1650	120	9403	1.13	6.79
41	42	300	400	110	880	0.58	1.49
42	43	150	350	110	440	0.29	0.36
43	44	150	450	110	1022	0.67	2.21
44	45	200	300	110	2851	1.05	2.43
45	46	250	500	120	3751	0.88	1.93
46	47	250	500	120	3751	0.88	1.93
47	48	100	500	110	253	0.39	1.43
48	49	150	300	110	1395	0.91	2.62
49	50	100	300	110	304	0.49	1.88
50	51	100	750	110	243	0.36	1.85
51	52	200	100	110	2798	1.03	0.78
52	53	150	100	110	1313	0.86	0.78
53	54	200	300	110	2816	1.04	2.37
54	55	200	350	110	1562	0.58	0.93

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	38	43	200	110	1975	0.73	3.28
57	39	44	100	800	292	0.43	2.78
58	40	41	200	350	2546	0.94	2.30
59	41	42	200	200	1152	0.42	0.30
60	41	45	150	700	328	0.21	0.42
61	42	46	150	500	581	0.38	1.04
62	43	44	150	350	482	0.32	0.43
63	43	45	150	400	719	0.47	1.03
64	45	46	100	400	233	0.34	2.30
65	47	50	350	10	-10121	-1.22	-0.05
66	48	53	500	1000	-16051	-0.66	-0.69
67	49	51	350	10	10121	1.22	0.05
68	52	53	350	100	4013	0.48	0.85

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	219.00	319.85	41.85	45.00
3	278.00	438.00	319.64	41.64	45.00
4	271.80	219.00	319.44	47.64	51.20
5	280.00	451.00	319.43	39.43	43.00
6	290.00	465.00	319.29	29.29	33.00
7	300.00	232.00	319.27	19.27	23.00
8	269.70	0.00	309.06	39.36	53.30
9	269.70	179.00	290.55	20.85	53.30
10	270.00	381.00	290.56	20.56	53.00
11	270.20	179.00	290.62	20.42	52.80
12	270.50	381.00	290.72	20.22	52.50
13	270.50	2043.00	291.43	20.93	52.50
14	270.50	202.00	290.71	20.21	52.50
15	270.00	202.00	290.66	20.66	53.00
16	270.50	202.00	290.72	20.22	52.50
17	271.50	202.00	291.87	20.37	51.50
18	270.50	202.00	290.95	20.45	52.50
19	273.80	1482.00	292.42	18.62	49.20
20	271.00	1482.00	290.86	19.86	52.00
21	270.30	179.00	290.43	20.13	52.70
22	270.20	561.00	290.43	20.23	52.80
23	270.00	561.00	289.89	19.89	53.00
24	270.00	179.00	289.90	19.90	53.00
25	270.00	-12037.00	293.72	23.72	53.00
26	270.50	0.00	291.49	20.99	52.50
27	269.30	561.00	289.68	20.38	53.70
28	269.00	382.00	289.54	20.54	54.00
29	258.90	1064.00	280.01	21.11	64.10
30	258.50	1401.00	278.13	19.63	64.50
31	258.00	440.00	276.64	18.64	65.00
32	257.50	440.00	276.28	18.78	65.50
33	255.00	1911.00	273.22	18.22	68.00
34	255.00	759.00	271.01	15.01	68.00
35	255.00	1152.00	270.79	15.79	68.00
36	255.00	1152.00	268.16	13.16	68.00
37	254.80	574.00	271.29	16.49	68.20
38	254.80	574.00	270.50	15.70	68.20
39	254.80	1533.00	269.57	14.77	68.20
40	254.50	574.00	268.91	14.41	68.50
41	254.50	1066.00	266.61	12.11	68.50
42	254.50	814.00	266.31	11.81	68.50
43	254.50	774.00	267.22	12.72	68.50
44	254.50	774.00	266.79	12.29	68.50
45	254.50	814.00	266.19	11.69	68.50
46	254.50	814.00	265.28	10.78	68.50
47	269.30	0.00	290.79	21.49	53.70
48	273.80	0.00	292.46	18.66	49.20
49	269.30	0.00	301.83	32.53	53.70
50	0.00	0.00	290.83	290.83	323.00
51	0.00	0.00	301.79	301.79	323.00
52	270.00	-4012.00	293.24	23.24	53.00
53	275.00	0.00	293.15	18.15	48.00

Iteration Times : 30

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALT. D-2-(2), Fire at SOLANO

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	132.00	319.94	41.94	45.00
3	278.00	264.00	319.86	41.86	45.00
4	271.80	132.00	319.78	47.98	51.20
5	280.00	272.00	319.78	39.78	43.00
6	290.00	280.00	319.72	29.72	33.00
7	300.00	140.00	319.71	19.71	23.00
8	269.70	0.00	318.60	48.90	53.30
9	269.70	108.00	291.09	21.39	53.30
10	270.00	230.00	291.09	21.09	53.00
11	270.20	108.00	291.11	20.91	52.80
12	270.50	230.00	291.16	20.66	52.50
13	270.50	1231.00	291.37	20.87	52.50
14	270.50	122.00	291.16	20.66	52.50
15	270.00	122.00	291.17	21.17	53.00
16	270.50	122.00	291.17	20.67	52.50
17	271.50	122.00	292.11	20.61	51.50
18	270.50	122.00	291.51	21.01	52.50
19	273.80	893.00	292.48	18.68	49.20
20	271.00	893.00	291.20	20.20	52.00
21	270.30	108.00	291.00	20.70	52.70
22	270.20	338.00	290.99	20.79	52.80
23	270.00	338.00	290.78	20.78	53.00
24	270.00	108.00	290.79	20.79	53.00
25	293.50	0.00	293.50	0.00	29.50
26	270.50	0.00	291.39	20.89	52.50
27	269.30	338.00	290.70	21.40	53.70
28	269.00	230.00	290.65	21.65	54.00
29	258.90	641.00	282.02	23.12	64.10
30	258.50	844.00	281.29	22.79	64.50
31	258.00	265.00	280.71	22.71	65.00
32	257.50	265.00	280.56	23.06	65.50
33	255.00	1152.00	276.56	21.56	68.00
34	255.00	458.00	275.16	20.16	68.00
35	255.00	694.00	275.17	20.17	68.00
36	255.00	694.00	273.62	18.62	68.00
37	254.80	346.00	274.51	19.71	68.20
38	254.80	346.00	273.46	18.66	68.20
39	254.80	924.00	272.86	18.06	68.20
40	254.50	346.00	272.49	17.99	68.50
41	254.50	643.00	270.10	15.60	68.50
42	254.50	491.00	269.90	15.40	68.50
43	254.50	2366.00	265.23	10.73	68.50
44	254.50	2366.00	260.72	6.22	68.50
45	254.50	491.00	266.60	12.10	68.50
46	254.50	491.00	268.06	13.56	68.50
47	269.30	0.00	289.68	20.38	53.70
48	273.80	0.00	292.50	18.70	49.20
49	269.30	0.00	317.68	48.38	53.70
50	0.00	0.00	289.68	289.68	323.00
51	0.00	0.00	317.68	317.68	323.00
52	293.50	0.00	293.50	0.00	29.50
53	275.00	0.00	293.21	18.21	48.00

Iteration Times : 41

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALI. D-2-(2), Fire at SOLANO  
 << PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/100)
1	2	300.	300.	120.	-1220.	-0.20	-0.20
2	2	300.	500.	120.	1088.	0.18	0.08
3	3	4 150.	700.	110.	132.	0.11	0.16
4	3	5 250.	500.	120.	692.	0.16	0.08
5	5	6 250.	800.	120.	430.	0.10	0.05
6	6	7 250.	800.	120.	140.	0.03	0.01
7	7	8 300.	1200.	120.	-3188.	-0.52	-1.40
8	8	9 100.	1200.	110.	-161.	-0.24	-1.40
9	8	49 250.	1050.	120.	1685.	0.40	0.92
10	9	10 200.	1100.	120.	1644.	0.39	0.84
11	10	11 200.	150.	110.	-338.	-0.12	0.00
12	11	12 200.	100.	110.	-570.	-0.21	-0.04
13	11	24 100.	450.	110.	124.	0.18	0.32
14	12	13 200.	250.	110.	-844.	-0.31	-0.21
15	12	14 200.	250.	110.	-87.	-0.03	0.00
16	12	21 100.	200.	110.	130.	0.19	0.16
17	13	20 200.	400.	110.	568.	0.21	0.16
18	13	22 200.	230.	110.	1205.	0.44	0.38
19	13	25 250.	5.	120.	-3849.	-0.91	-0.02
20	14	15 100.	250.	110.	-26.	-0.04	-0.04
21	14	16 200.	150.	110.	-184.	-0.07	-0.01
22	15	18 100.	350.	110.	-148.	-0.22	-0.34
23	16	17 100.	250.	110.	-306.	-0.45	-0.95
24	17	18 100.	200.	110.	270.	0.40	0.60
25	17	19 150.	150.	110.	-697.	-0.46	-0.36
26	19	20 100.	300.	110.	325.	0.48	1.27
27	19	48 200.	5.	110.	-1915.	-0.71	-0.02
28	20	21 100.	300.	110.	22.	0.03	0.01
29	21	22 200.	220.	110.	890.	0.33	0.21
30	22	23 200.	500.	110.	-16.	-0.02	-0.01
31	23	24 100.	200.	110.	568.	0.21	0.08
32	23	27 200.	200.	110.	-14345.	-0.85	-1.11
33	26	48 500.	700.	120.	10496.	0.97	1.71
34	26	47 400.	650.	120.	230.	0.08	0.05
35	27	28 200.	700.	110.	-3112.	-0.73	-7.65
36	29	47 250.	2800.	120.	-10711.	-0.99	-7.65
37	29	47 400.	2800.	120.	1374.	0.51	0.73
38	29	33 200.	350.	110.	3450.	0.81	5.46
39	29	33 250.	1650.	120.	8358.	1.01	5.46
40	29	33 350.	1650.	120.	530.	0.35	0.58
41	30	31 150.	400.	110.	265.	0.17	0.14
42	31	32 150.	350.	110.	798.	0.52	1.40
43	33	34 150.	450.	110.	2110.	0.76	1.39
44	33	35 200.	300.	110.	3874.	0.91	2.05
45	33	37 250.	500.	120.	3974.	0.91	2.05
46	34	39 100.	500.	120.	340.	0.50	2.30
47	34	36 150.	300.	110.	1048.	0.69	1.55
48	35	40 100.	500.	110.	368.	0.54	2.68
49	35	40 150.	750.	110.	354.	0.52	3.73
50	36	42 100.	100.	110.	3252.	1.21	1.05
51	37	38 200.	100.	110.	1540.	1.01	1.05
52	37	38 150.	100.	110.	2580.	0.95	2.02
53	40	200.	300.	110.	1231.	0.45	0.60
54	38	39 200.	350.	110.	3245.	0.95	6.73
55	38	43 200.	800.	110.		1.20	10.29

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALI. D-2-(2), Fire at SOLANO  
 << PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/100)
56	39	44	100.	110.	647.	0.95	12.14
57	40	41	200.	110.	2602.	0.96	2.39
58	41	42	200.	110.	928.	0.34	0.20
59	41	45	150.	110.	1031.	0.68	3.50
60	42	46	150.	110.	791.	0.52	1.84
61	43	44	150.	110.	1719.	1.13	4.51
62	43	45	150.	110.	-840.	-0.55	-1.37
63	45	46	100.	110.	-300.	-0.44	-1.46
64	47	50	350.	10.	-3329.	-0.40	-0.01
65	48	53	600.	1000.	-16260.	-0.67	-0.71
66	49	51	350.	10.	3328.	0.40	0.01
67	53	25	500.	120.	-8410.	-0.50	-0.59
68	53	52	350.	120.	-7850.	-0.54	-0.29

ALT. D-2-(2)-A, Recommended Plan, Single Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2), Fire at SOLANO

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	132.00	319.94	41.94	45.00
3	278.00	264.00	319.86	41.86	45.00
4	271.80	132.00	319.78	47.98	51.20
5	280.00	272.00	319.78	39.78	43.00
6	290.00	280.00	319.72	29.72	33.00
7	300.00	140.00	319.72	19.72	23.00
8	269.70	0.00	310.02	40.32	53.30
9	269.70	108.00	292.32	22.62	53.30
10	270.00	230.00	292.33	22.33	53.00
11	270.20	108.00	292.35	22.15	52.80
12	270.50	230.00	292.39	21.89	52.50
13	270.50	1231.00	292.67	22.17	52.50
14	270.50	122.00	292.39	21.89	52.50
15	270.00	122.00	292.37	22.37	53.00
16	270.50	122.00	292.39	21.89	52.50
17	271.50	122.00	292.86	21.36	51.50
18	270.50	122.00	292.49	21.99	52.50
19	273.80	893.00	293.08	19.28	49.20
20	271.00	893.00	292.45	21.45	52.00
21	270.30	108.00	292.28	21.98	52.70
22	270.20	338.00	292.28	22.08	52.80
23	270.00	338.00	292.07	22.07	53.00
24	270.00	108.00	292.07	22.07	53.00
25	293.50	0.00	293.50	0.00	29.50
26	270.50	0.00	292.69	22.19	52.50
27	269.30	338.00	291.98	22.68	53.70
28	269.00	230.00	291.93	22.93	54.00
29	258.90	641.00	284.73	25.83	64.10
30	258.50	844.00	283.99	25.49	64.50
31	258.00	265.00	283.41	25.41	65.00
32	257.50	265.00	283.27	25.77	65.50
33	255.00	1152.00	279.27	24.27	68.00
34	255.00	458.00	277.87	22.87	68.00
35	255.00	694.00	277.87	22.87	68.00
36	255.00	694.00	276.33	21.33	68.00
37	254.80	346.00	277.21	22.41	68.20
38	254.80	346.00	276.16	21.36	68.20
39	254.80	924.00	275.57	20.77	68.20
40	254.50	346.00	275.20	20.70	68.50
41	254.50	643.00	272.80	18.30	68.50
42	254.50	491.00	272.60	18.10	68.50
43	254.50	2366.00	267.93	13.43	68.50
44	254.50	2366.00	263.42	8.92	68.50
45	254.50	491.00	269.30	14.80	68.50
46	254.50	491.00	270.76	16.26	68.50
47	269.30	0.00	292.38	23.08	53.70
48	273.80	0.00	293.10	19.30	49.20
49	269.30	0.00	303.42	34.12	53.70
50	0.00	0.00	292.42	292.42	323.00
51	0.00	0.00	303.38	303.38	323.00
52	293.50	0.00	293.50	0.00	29.50
53	275.00	0.00	293.38	18.38	48.00

Iteration Times : 36

ALT. D-2-(2)-A. Recommended Plan, Single Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2). Fire at SOLANO

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300	300	120	-1220	-0.20	-0.06
2	3	300	500	120	1088	0.18	0.08
3	4	150	700	110	132	0.09	0.16
4	5	250	500	120	692	0.15	0.08
5	6	250	800	120	420	0.10	0.05
6	7	300	1200	120	9155	-0.03	-0.01
7	8	100	1200	110	-487	-0.69	-8.32
8	9	250	1050	120	4877	1.15	6.60
9	10	250	1100	120	4755	1.12	6.00
10	11	200	200	110	-108	-0.04	-0.02
11	12	200	100	110	-338	-0.12	-0.16
12	13	200	100	110	-562	-0.21	-0.40
13	14	200	450	110	116	0.17	0.28
14	15	200	250	110	-976	-0.35	-0.28
15	16	200	200	110	73	0.03	0.00
16	17	200	200	110	111	0.16	0.12
17	18	200	400	110	670	0.25	0.55
18	19	200	230	110	1233	0.45	0.39
19	20	250	5	120	-4111	-0.97	-4.58
20	21	100	250	110	38	0.06	0.02
21	22	100	150	110	-86	-0.03	-0.01
22	23	100	350	110	-84	-0.12	-0.35
23	24	100	200	110	-208	-0.31	-1.86
24	25	100	200	110	205	0.30	0.47
25	26	150	150	110	-527	-0.35	-1.83
26	27	100	300	110	223	0.33	0.22
27	28	200	5	110	-1652	-0.61	-2.95
28	29	200	300	110	3	0.00	0.00
29	30	200	220	110	898	0.33	0.21
30	31	200	500	110	-8	-0.01	0.00
31	32	200	200	110	558	0.21	0.08
32	33	200	700	120	-8301	-0.49	-0.40
33	34	400	650	120	4190	0.39	0.31
34	35	200	700	110	230	0.08	0.05
35	36	250	2800	120	-3112	-0.73	-2.73
36	37	250	2800	120	-10711	-0.99	-7.65
37	38	200	350	110	1374	0.51	0.73
38	39	200	1650	120	3450	0.81	5.46
39	40	350	1650	120	8358	1.01	5.46
40	41	150	400	110	530	0.35	0.59
41	42	150	350	110	265	0.17	0.14
42	43	150	450	110	798	0.52	1.40
43	44	200	300	110	2110	0.78	1.39
44	45	250	500	120	3874	0.91	2.05
45	46	250	500	120	3874	0.91	2.05
46	47	100	500	110	340	0.50	2.30
47	48	150	300	110	1048	0.69	1.53
48	49	100	500	110	368	0.54	2.68
49	50	100	750	110	354	0.52	3.73
50	51	100	100	110	3282	1.21	1.05
51	52	100	100	110	1540	1.01	1.05
52	53	200	300	110	2580	0.95	2.02
53	54	200	350	110	1231	0.80	1.71
54	55	200	800	110	3245	1.20	8.23
55	56	200	800	110	3245	1.20	8.23

ALT. D-2-(2)-A. Recommended Plan, Single Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2). Fire at SOLANO

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	39	44	100	110	647	0.95	12.14
57	40	41	200	110	2802	0.95	2.39
58	41	42	200	110	928	0.34	0.20
59	41	45	150	110	1031	0.68	3.50
60	42	45	150	110	791	0.52	1.84
61	43	44	150	110	1719	1.13	4.51
62	43	45	150	110	-840	-0.55	-3.42
63	45	46	100	110	-300	-0.44	-1.46
64	47	50	10	120	-9633	-1.16	-0.29
65	48	53	1000	130	-9633	-0.41	-0.29
66	49	51	10	120	9633	1.16	0.04
67	53	25	500	120	-5148	-0.30	-0.24
68	53	52	350	120	-4805	-0.58	-1.19

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 1995, Dry Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	121.00	319.61	41.61	45.00
3	278.00	242.00	319.10	41.10	45.00
4	271.80	121.00	319.03	47.23	51.20
5	280.00	234.00	319.04	39.04	43.00
6	290.00	226.00	319.00	29.00	33.00
7	300.00	113.00	319.00	19.00	23.00
8	269.70	0.00	315.86	46.16	53.30
9	269.70	101.00	290.07	20.37	53.30
10	270.00	214.00	290.07	20.07	53.00
11	270.20	101.00	290.09	19.89	52.80
12	270.50	214.00	290.13	19.63	52.50
13	270.50	1153.00	290.33	19.83	52.50
14	270.50	113.00	290.13	19.63	52.50
15	270.00	113.00	289.98	19.98	53.00
16	270.50	113.00	290.14	19.64	52.50
17	271.50	113.00	291.57	20.07	51.50
18	270.50	113.00	291.45	20.95	52.50
19	273.80	837.00	291.85	18.05	49.20
20	271.00	837.00	290.22	19.22	52.00
21	270.30	101.00	290.00	19.70	52.70
22	270.20	316.00	289.99	19.79	52.80
23	270.00	316.00	289.81	19.81	53.00
24	270.00	101.00	289.81	19.81	53.00
25	270.00	-8477.00	293.16	23.16	53.00
26	270.50	0.00	290.35	19.85	52.50
27	269.30	316.00	289.74	20.44	53.70
28	269.00	215.00	289.69	20.69	54.00
29	258.90	577.00	279.24	20.34	64.10
30	258.50	763.00	278.64	20.14	64.50
31	258.00	239.00	278.15	20.15	65.00
32	257.50	239.00	278.04	20.54	65.50
33	255.00	1040.00	269.16	14.16	68.00
34	255.00	396.00	268.78	13.78	68.00
35	255.00	644.00	268.60	13.60	68.00
36	255.00	644.00	267.98	12.98	68.00
37	254.80	360.00	266.42	11.62	68.20
38	254.80	360.00	266.02	11.22	68.20
39	254.80	800.00	265.75	10.95	68.20
40	254.50	360.00	264.92	10.42	68.50
41	254.50	568.00	263.66	9.16	68.50
42	254.50	424.00	263.49	8.99	68.50
43	254.50	404.00	265.39	10.89	68.50
44	254.50	404.00	265.08	10.58	68.50
45	254.50	424.00	262.99	8.49	68.50
46	254.50	424.00	262.91	8.41	68.50
47	269.30	0.00	288.60	19.30	53.70
48	273.80	0.00	291.87	18.07	49.20
49	269.30	0.00	313.12	43.82	53.70
50	0.00	0.00	288.62	288.62	323.00
51	0.00	0.00	313.10	313.10	323.00
52	270.00	0.00	290.00	20.00	53.00
53	275.00	0.00	292.87	17.87	48.00

Iteration Times : 38

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 1995, Dry Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	200	300	110	-1057	-0.39	-0.39
2	3	200	500	110	936	0.34	0.51
3	4	250	700	110	121	0.08	0.09
4	5	250	500	120	573	0.14	0.06
5	6	250	800	120	339	0.08	0.05
6	7	250	800	120	112	0.03	0.01
7	8	300	1200	120	-5702	-0.93	-4.14
8	1	100	1200	110	-291	-0.23	-3.45
8	49	250	1050	120	3034	0.72	2.74
8	49	250	1100	120	2958	0.70	2.49
9	10	200	200	110	-101	-0.04	-0.02
10	11	200	150	110	-316	-0.12	-0.14
11	12	200	100	110	-531	-0.20	-0.35
12	13	200	450	110	114	0.17	0.28
13	14	100	450	110	-822	-0.30	-0.81
14	15	200	250	110	-42	-0.02	-0.00
15	16	200	250	110	118	0.17	0.13
16	17	200	200	110	465	0.17	0.28
17	18	200	400	110	1133	0.42	0.34
18	19	200	230	110	-3577	-0.84	-0.02
19	20	250	5	120	113	0.17	0.15
20	21	100	250	110	-266	-0.10	-0.02
21	14	150	150	110	-381	-0.56	-1.43
22	16	200	250	110	113	0.17	0.13
23	17	100	200	110	-607	-0.40	-0.28
24	17	150	150	110	372	0.55	1.63
25	19	200	300	110	-1818	-0.67	-0.02
26	19	200	5	110	17	0.02	0.01
27	20	200	300	110	834	0.31	0.18
28	21	22	200	110	-13	-0.02	-0.01
29	22	23	200	110	531	0.20	0.07
30	23	24	200	110	8477	0.50	0.30
31	23	27	200	120	-6559	-0.80	-1.52
32	25	53	500	120	3082	0.73	1.75
33	26	48	700	120	215	0.08	0.05
34	26	47	650	120	-3468	-0.82	-9.36
35	27	25	700	110	1241	0.46	0.61
36	29	47	2800	120	2448	0.90	10.08
37	29	47	2800	120	478	0.31	0.48
38	29	30	350	110	239	0.16	0.12
39	29	33	1650	120	396	0.26	0.38
40	29	33	1530	110	1288	0.47	0.56
41	30	21	400	110	4528	1.07	2.74
42	31	32	350	110	644	0.42	0.63
43	33	34	450	110	1968	0.73	0.41
44	33	35	300	110	2200	0.81	1.50
45	35	36	500	120	800	0.29	0.27
46	35	36	300	110	800	0.29	0.27
47	37	38	100	110	1840	0.30	0.63
48	37	40	300	110	848	0.31	0.17
49	38	39	200	110	424	0.28	0.67
50	38	43	800	110	404	0.28	0.58
51	40	41	200	110	404	0.28	0.58
52	41	42	200	110	404	0.28	0.58
53	41	45	150	110	404	0.28	0.58
54	42	45	150	110	404	0.28	0.58
55	43	44	150	110	404	0.28	0.58

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 1995, Dry Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	47	50	350	120	-5992	-0.72	-0.02
57	48	53	450	120	-8477	-0.52	-1.00
58	49	51	350	120	5992	0.72	0.02



ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 1995, Rainy Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	121.00	319.61	41.61	45.00
3	278.00	242.00	319.10	41.10	45.00
4	271.80	121.00	319.03	47.23	51.20
5	280.00	234.00	319.04	39.04	43.00
6	290.00	226.00	319.00	29.00	33.00
7	300.00	113.00	319.00	19.00	23.00
8	269.70	0.00	305.76	36.06	53.30
9	269.70	101.00	292.65	22.95	53.30
10	270.00	214.00	292.66	22.66	53.00
11	270.20	101.00	292.68	22.48	52.80
12	270.50	214.00	292.71	22.21	52.50
13	270.50	1153.00	293.01	22.51	52.50
14	270.50	113.00	292.70	22.20	52.50
15	270.00	113.00	292.55	22.55	53.00
16	270.50	113.00	292.70	22.20	52.50
17	271.50	113.00	292.98	21.48	51.50
18	270.50	113.00	292.86	22.36	52.50
19	273.80	837.00	293.11	19.31	49.20
20	271.00	837.00	292.78	21.78	52.00
21	270.30	101.00	292.64	22.34	52.70
22	270.20	316.00	292.64	22.44	52.80
23	270.00	316.00	292.45	22.45	53.00
24	270.00	101.00	292.45	22.45	53.00
25	270.00	-2825.00	293.29	23.29	53.00
26	270.50	0.00	293.03	22.53	52.50
27	269.30	316.00	292.38	23.08	53.70
28	269.00	215.00	292.34	23.34	54.00
29	258.90	577.00	284.93	26.03	64.10
30	258.50	763.00	284.33	25.83	64.50
31	258.00	239.00	283.85	25.85	65.00
32	257.50	239.00	283.73	26.23	65.50
33	255.00	1040.00	274.85	19.85	68.00
34	255.00	396.00	274.47	19.47	68.00
35	255.00	644.00	274.29	19.29	68.00
36	255.00	644.00	273.67	18.67	68.00
37	254.80	360.00	272.11	17.31	68.20
38	254.80	360.00	271.71	16.91	68.20
39	254.80	800.00	271.44	16.64	68.20
40	254.50	360.00	270.61	16.11	68.50
41	254.50	568.00	269.35	14.85	68.50
42	254.50	424.00	269.18	14.68	68.50
43	254.50	404.00	271.08	16.58	68.50
44	254.50	404.00	270.77	16.27	68.50
45	254.50	424.00	268.68	14.18	68.50
46	254.50	424.00	268.60	14.10	68.50
47	269.30	0.00	294.29	24.99	53.70
48	273.80	0.00	293.12	19.32	49.20
49	269.30	0.00	296.36	27.06	53.70
50	0.00	0.00	294.36	294.36	323.00
51	0.00	0.00	296.29	296.29	323.00
52	270.00	0.00	290.00	20.00	53.00
53	275.00	0.00	293.25	18.25	48.00

Iteration Times : 54

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/100)
1	2	200	300	110	-1057.	-0.39	-0.39
2	3	200	500	110	936.	0.34	0.51
3	4	150	700	110	121.	0.08	0.07
4	5	250	500	120	573.	0.14	0.06
5	6	250	800	120	339.	0.08	0.04
6	7	250	800	120	113.	0.03	0.00
7	8	300	1200	120	-1102.	-1.82	-14.24
8	8	100	1200	110	-566.	-0.83	-14.24
8	8	49	1050	120	5908.	9.41	8.96
8	8	250	1100	120	5761.	1.36	9.41
9	9	200	200	110	-102.	-0.04	0.00
10	10	200	150	110	-317.	-0.12	-0.02
11	11	200	100	110	-521.	-0.19	-0.03
12	11	200	100	110	102.	0.15	0.22
13	11	200	450	110	-1005.	-0.37	-0.29
14	12	100	250	110	183.	0.07	0.01
15	12	13	200	110	87.	0.13	0.07
16	12	14	200	110	681.	0.25	0.23
17	12	21	200	110	1179.	0.43	0.36
18	13	20	400	110	-4025.	-0.95	-0.02
19	13	22	200	120	113.	0.17	0.15
20	13	25	5	120	-15.	-0.02	0.00
21	14	15	250	110	41.	0.02	0.00
22	14	16	150	110	-160.	-0.24	-0.28
23	15	17	200	110	113.	0.17	0.12
24	17	18	200	110	-386.	-0.25	-0.12
25	17	19	150	110	156.	0.23	0.33
26	19	20	300	110	-1385.	-0.51	-0.01
27	19	48	5	110	-15.	-0.02	0.00
28	21	22	300	110	846.	0.31	0.19
29	22	23	200	110	-1.	0.00	0.00
30	23	24	500	110	531.	0.20	0.07
31	23	27	200	110	2822.	0.17	0.04
32	25	53	500	120	-1435.	-0.17	-0.09
33	26	48	700	120	-2591.	-0.61	-1.27
34	26	47	250	120	215.	0.08	0.05
35	27	28	700	110	-3469.	-0.82	-3.36
36	29	47	250	120	-5603.	-0.92	-3.34
37	29	47	300	120	1241.	0.46	0.61
38	29	30	350	110	4804.	1.13	10.08
39	29	33	250	120	2449.	0.90	10.08
40	33	200	1650	110	478.	0.31	0.48
41	30	31	400	110	239.	0.16	0.12
42	31	32	350	110	396.	0.26	0.36
43	33	34	450	110	1288.	0.47	0.56
44	33	35	300	110	4529.	1.07	2.74
45	33	37	250	120	644.	0.42	0.63
46	35	36	300	110	1968.	0.73	0.41
47	37	38	200	100	2200.	0.81	1.50
48	37	40	300	110	500.	0.29	0.27
49	38	39	200	350	808.	0.30	0.63
50	38	43	800	110	1840.	0.65	1.26
51	40	41	200	350	848.	0.31	0.17
52	41	42	200	110	424.	0.28	0.67
53	41	45	150	700	424.	0.28	0.59
54	42	46	150	500	404.	0.25	0.31
55	44	44	350	110	404.	0.25	0.31

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/100)
56	47	50	350.	10.	-11669.	-1.40	-0.06
57	48	53	450.	1000.	-2822.	-0.21	-0.13
58	49	51	350.	10.	11669.	1.40	0.06

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	219.00	319.83	41.83	45.00
3	278.00	438.00	319.60	41.60	45.00
4	271.80	219.00	319.40	47.60	51.20
5	280.00	451.00	319.38	39.38	43.00
6	290.00	465.00	319.24	29.24	33.00
7	300.00	232.00	319.22	19.22	23.00
8	269.70	0.00	317.33	47.63	53.30
9	269.70	179.00	288.40	18.70	53.30
10	270.00	381.00	288.41	18.41	53.00
11	270.20	179.00	288.47	18.27	52.80
12	270.50	381.00	288.57	18.07	52.50
13	270.50	2043.00	289.19	18.69	52.50
14	270.50	202.00	288.57	18.07	52.50
15	270.00	202.00	288.57	18.57	53.00
16	270.50	202.00	288.58	18.08	52.50
17	271.50	202.00	290.40	18.90	51.50
18	270.50	202.00	289.16	18.66	52.50
19	273.80	1482.00	291.16	17.36	49.20
20	271.00	1482.00	288.71	17.71	52.00
21	270.30	179.00	288.22	17.92	52.70
22	270.20	561.00	288.21	18.01	52.80
23	270.00	561.00	287.67	17.67	53.00
24	270.00	179.00	287.69	17.69	53.00
25	270.00	-12883.00	293.40	23.40	53.00
26	270.50	0.00	289.24	18.74	52.50
27	269.30	561.00	287.47	18.17	53.70
28	269.00	382.00	287.33	18.33	54.00
29	258.90	1064.00	278.52	19.62	64.10
30	258.50	1401.00	276.41	17.91	64.50
31	258.00	440.00	274.92	16.92	65.00
32	257.50	440.00	274.56	17.06	65.50
33	255.00	1911.00	273.38	18.38	68.00
34	255.00	759.00	271.17	16.17	68.00
35	255.00	1152.00	270.95	15.95	68.00
36	255.00	1152.00	268.33	13.33	68.00
37	254.80	574.00	271.45	16.65	68.20
38	254.80	574.00	270.67	15.87	68.20
39	254.80	1533.00	269.74	14.94	68.20
40	254.50	574.00	269.07	14.57	68.50
41	254.50	1066.00	266.78	12.28	68.50
42	254.50	814.00	266.47	11.97	68.50
43	254.50	774.00	267.38	12.88	68.50
44	254.50	774.00	266.95	12.45	68.50
45	254.50	814.00	266.36	11.86	68.50
46	254.50	814.00	265.44	10.94	68.50
47	269.30	0.00	287.06	17.76	53.70
48	273.80	0.00	291.20	17.40	49.20
49	269.30	0.00	315.57	46.27	53.70
50	0.00	0.00	287.07	287.07	323.00
51	0.00	0.00	315.56	315.56	323.00
52	270.00	-8588.00	293.10	23.10	53.00
53	275.00	0.00	292.75	17.75	48.00

Iteration Times : 45

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2)

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H. G. L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	320.00	0.00	320.00	0.00	3.00
2	278.00	219.00	319.83	41.83	45.00
3	278.00	438.00	319.60	41.60	45.00
4	271.80	219.00	319.40	47.60	51.20
5	280.00	451.00	319.38	39.38	43.00
6	290.00	465.00	319.24	29.24	33.00
7	300.00	232.00	319.22	19.22	23.00
8	269.70	0.00	309.06	39.36	53.30
9	269.70	179.00	290.05	20.35	53.30
10	270.00	381.00	290.06	20.06	53.00
11	270.20	179.00	290.12	19.92	52.80
12	270.50	381.00	290.22	19.72	52.50
13	270.50	2043.00	290.93	20.43	52.50
14	270.50	202.00	290.22	19.72	52.50
15	270.00	202.00	290.17	20.17	53.00
16	270.50	202.00	290.22	19.72	52.50
17	271.50	202.00	291.44	19.94	51.50
18	270.50	202.00	290.49	19.99	52.50
19	273.80	1482.00	292.01	18.21	49.20
20	271.00	1482.00	290.37	19.37	52.00
21	270.30	179.00	289.92	19.62	52.70
22	270.20	561.00	289.92	19.72	52.80
23	270.00	561.00	289.39	19.39	53.00
24	270.00	179.00	289.39	19.39	53.00
25	270.00	-12037.00	293.53	23.53	53.00
26	270.50	0.00	290.99	20.49	52.50
27	269.30	561.00	289.18	19.88	53.70
28	269.00	382.00	289.04	20.04	54.00
29	258.90	1064.00	281.74	22.84	64.10
30	258.50	1401.00	279.63	21.13	64.50
31	258.00	440.00	278.14	20.14	65.00
32	257.50	440.00	277.78	20.28	65.50
33	255.00	1911.00	276.60	21.60	68.00
34	255.00	759.00	274.39	19.39	68.00
35	255.00	1152.00	274.17	19.17	68.00
36	255.00	1152.00	271.55	16.55	68.00
37	254.80	574.00	274.67	19.87	68.20
38	254.80	574.00	273.88	19.08	68.20
39	254.80	1533.00	272.95	18.15	68.20
40	254.50	574.00	272.29	17.79	68.50
41	254.50	1066.00	269.99	15.49	68.50
42	254.50	814.00	269.69	15.19	68.50
43	254.50	774.00	270.60	16.10	68.50
44	254.50	774.00	270.17	15.67	68.50
45	254.50	814.00	269.57	15.07	68.50
46	254.50	814.00	268.66	14.16	68.50
47	269.30	0.00	290.27	20.97	53.70
48	273.80	0.00	292.05	18.25	49.20
49	269.30	0.00	301.82	32.52	53.70
50	0.00	0.00	290.32	290.32	323.00
51	0.00	0.00	301.77	301.77	323.00
52	270.00	-4012.00	293.04	23.04	53.00
53	275.00	0.00	292.95	17.95	48.00

Iteration Times : 30

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/100)
1	2	200	300	110	-663	-0.25	-0.17
2	2	250	300	120	-1341	-0.32	-0.17
3	2	200	500	110	609	0.22	0.23
4	2	250	500	120	1196	0.28	0.47
5	3	150	700	110	219	0.14	0.28
6	3	250	500	120	1148	0.27	0.43
7	5	250	800	120	697	0.16	0.17
8	6	250	800	120	232	0.09	0.02
9	7	300	1200	120	-4495	-0.74	-2.67
10	8	100	1200	110	-229	-0.34	-2.22
11	8	250	1050	120	2392	0.56	1.68
12	8	250	1100	120	2332	0.55	1.76
13	9	200	200	110	-179	-0.07	-0.01
14	10	200	150	110	-550	-0.21	-0.06
15	11	200	100	110	-940	-0.35	-0.10
16	11	200	450	110	200	0.30	0.78
17	12	200	250	110	-1499	-0.55	-2.45
18	12	200	250	110	-18	-0.01	0.00
19	12	200	200	110	203	0.30	0.36
20	13	200	400	110	1019	0.39	0.48
21	13	200	230	110	2019	0.74	0.98
22	13	250	5	120	-6591	-1.55	-10.94
23	14	150	250	110	5	0.01	0.00
24	14	200	150	110	-233	-0.09	-0.08
25	15	100	350	110	-197	-0.20	-1.68
26	15	100	250	110	-435	-0.54	-1.82
27	17	100	200	110	399	0.59	1.24
28	17	150	150	110	-1036	-0.68	-5.04
29	19	200	300	110	463	0.68	2.45
30	19	200	5	110	-2981	-1.10	-8.80
31	21	200	300	110	24	0.04	0.04
32	22	200	220	110	1483	0.55	0.53
33	23	200	500	110	-21	-0.03	-0.01
34	23	200	200	110	943	0.33	0.21
35	25	300	500	120	12883	0.76	1.30
36	26	350	700	120	-7638	-0.92	-2.80
37	26	400	700	120	-10852	-1.00	-1.96
38	26	400	650	120	3479	0.82	2.18
39	26	47	350	120	8429	1.01	2.18
40	27	200	700	110	382	0.14	0.14
41	29	250	2800	120	-3301	-0.78	-8.54
42	29	47	300	120	-5332	-0.87	-8.54
43	29	47	350	120	-7997	-0.96	-8.54
44	29	30	350	110	1141	0.75	2.11
45	29	30	350	110	1141	0.75	2.11
46	29	33	1550	120	3329	0.79	5.14
47	29	33	200	120	1837	0.69	5.14
48	29	33	350	120	8090	0.97	5.14
49	30	31	400	110	880	0.58	1.49
50	31	32	350	110	440	0.29	0.36
51	33	34	450	110	1022	0.57	2.21
52	33	35	200	110	2851	1.05	4.92
53	33	37	500	120	3751	0.88	1.93
54	33	37	550	120	3751	0.88	1.93
55	34	100	500	110	263	0.39	1.43

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 2010, Dry Season, Modification of ALT. D-2-(2)

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/100)
56	35	36	150	110	1395	0.91	2.52
57	35	40	100	110	304	0.45	1.85
58	36	42	100	110	243	0.36	1.85
59	37	38	200	110	2798	1.03	0.78
60	37	38	150	110	1313	0.86	0.78
61	37	40	200	110	2816	1.04	2.37
62	38	39	200	110	1562	0.58	0.93
63	38	43	200	110	1975	0.73	3.28
64	39	44	100	110	292	0.43	2.78
65	40	41	200	110	2546	0.94	2.30
66	41	42	200	110	1152	0.42	0.30
67	41	45	150	700	328	0.21	0.42
68	42	46	150	600	581	0.38	1.04
69	43	44	150	350	482	0.32	0.43
70	43	45	150	400	719	0.47	1.03
71	45	46	100	110	233	0.34	0.92
72	47	48	350	10	-4724	-0.57	-0.01
73	48	53	450	1000	-10735	-0.78	-1.55
74	48	53	450	1000	-10735	-0.78	-1.55
75	49	51	350	10	4724	0.57	0.01
76	52	53	350	100	6586	1.03	0.35

ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2)

<<< PIPELINE >>>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (D/100)
1	2	1	200	110	-683.	-0.25	-0.17
2	2	1	250	120	-1341.	-0.32	-0.17
3	2	3	200	110	609.	0.22	0.47
4	2	3	250	120	1196.	0.28	0.47
5	3	4	150	110	219.	0.14	0.28
6	3	5	250	120	1148.	0.27	0.43
7	5	6	250	120	597.	0.15	0.17
8	6	7	250	120	230.	0.05	0.02
9	8	1	300	120	-9634.	-1.58	-10.94
10	8	1	100	120	-491.	-0.72	-9.12
11	8	49	250	120	5126.	1.21	7.23
12	8	49	250	120	4999.	1.18	6.58
13	9	10	200	110	-178.	-0.07	-0.05
14	10	11	200	110	-559.	-0.21	-0.40
15	11	12	200	110	-931.	-0.34	-0.10
16	11	24	100	110	193.	0.28	1.62
17	12	13	200	110	-1611.	-0.59	-2.81
18	12	14	200	110	114.	0.04	0.02
19	12	20	200	110	186.	0.27	1.59
20	13	20	200	110	1109.	0.41	2.65
21	13	22	200	110	2044.	0.75	4.37
22	13	26	200	120	-6803.	-1.60	-11.64
23	14	15	100	110	60.	0.09	0.05
24	14	16	200	110	-147.	-0.05	-0.03
25	15	18	100	110	-142.	-0.21	-0.32
26	16	17	100	110	-349.	-0.51	-4.85
27	17	18	100	110	344.	0.51	4.72
28	17	19	150	110	-695.	-0.59	-2.65
29	19	20	100	110	374.	0.55	2.50
30	19	48	200	110	-2749.	-1.01	-17.57
31	21	22	100	110	7.	0.01	0.00
32	22	23	200	110	1490.	0.55	2.43
33	23	24	100	110	-14.	-0.02	-0.01
34	23	27	200	110	943.	0.35	1.04
35	25	53	500	120	12038.	0.71	1.14
36	26	48	350	120	-5495.	-0.66	-1.52
37	26	48	400	120	-7807.	-0.72	-1.07
38	26	47	250	120	1899.	0.45	1.10
39	26	47	350	120	4601.	0.55	1.10
40	27	28	200	110	382.	0.14	0.14
41	29	47	250	120	-3301.	-0.78	-3.05
42	29	47	300	120	-5332.	-0.87	-3.05
43	29	47	350	120	-7997.	-0.96	-3.05
44	29	30	150	110	1140.	0.75	2.11
45	29	30	150	110	1140.	0.75	2.11
46	29	33	200	120	3339.	0.79	3.11
47	29	33	200	120	1857.	0.68	2.14
48	29	33	250	120	6089.	0.97	3.11
49	30	31	150	110	860.	0.58	1.49
50	31	32	150	110	440.	0.29	1.03
51	33	34	150	110	1022.	0.67	2.21
52	33	35	200	110	2851.	1.05	2.43
53	33	37	250	120	3751.	0.88	1.93
54	33	37	250	120	3751.	0.88	1.93
55	34	39	100	110	253.	0.39	1.43

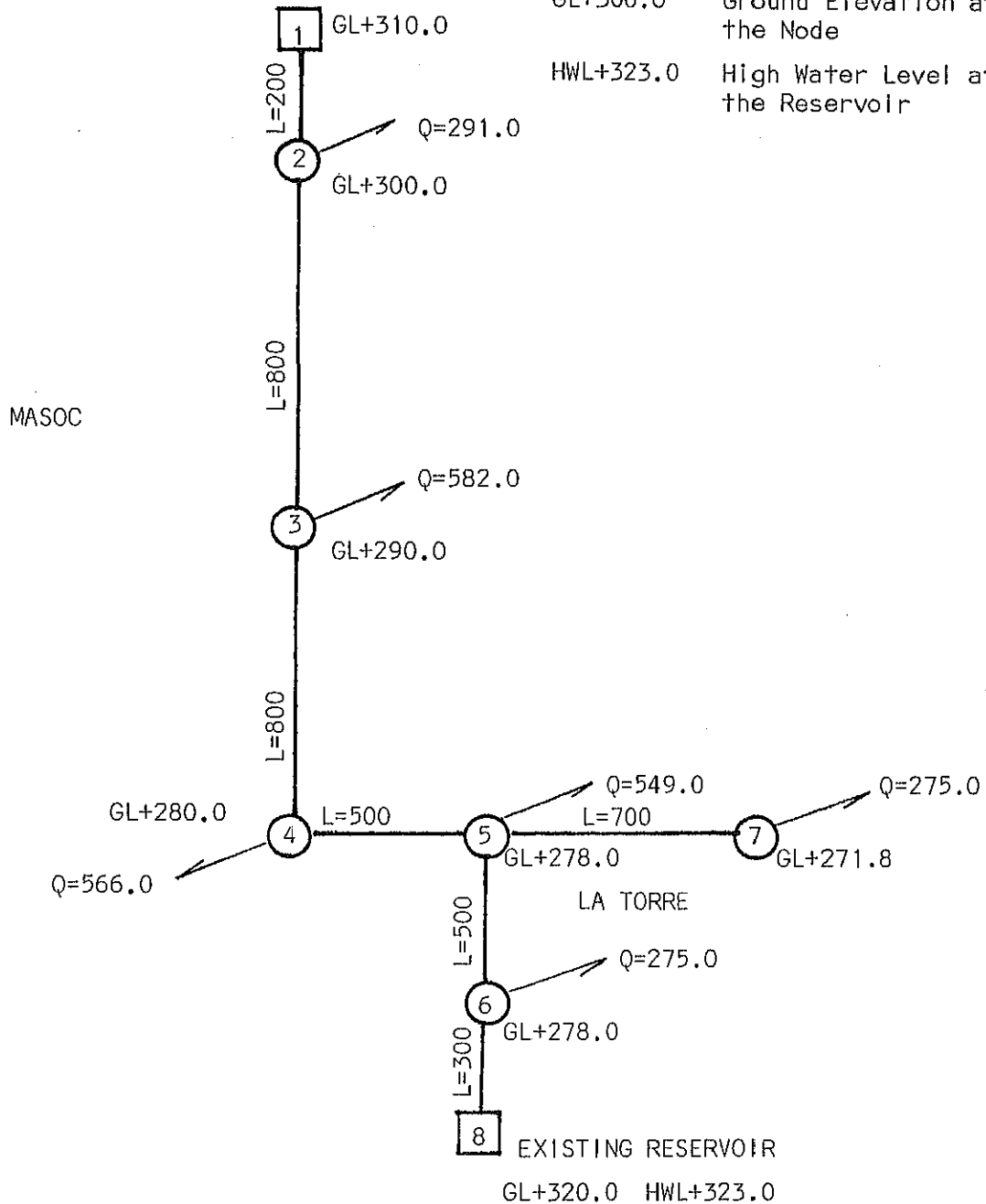
ALT. D-2-(2)-B, Recommended Plan, Parallel Pipeline Alignment  
 Year 2010, Rainy Season, Modification of ALT. D-2-(2)

<<< PIPELINE >>>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (D/100)
56	35	36	150	110	1395.	0.91	2.62
57	35	40	100	110	304.	0.45	1.85
58	36	42	100	110	243.	0.36	2.47
59	37	38	200	110	2798.	1.03	0.78
60	37	38	150	110	1313.	0.86	0.78
61	37	40	200	110	2816.	1.04	2.37
62	38	39	200	110	1562.	0.58	2.66
63	38	43	200	110	1875.	0.73	3.28
64	39	44	100	110	292.	0.43	3.48
65	40	41	200	110	2546.	0.94	6.57
66	41	42	200	110	1152.	0.42	0.30
67	41	45	150	110	328.	0.21	0.42
68	42	46	150	110	581.	0.38	1.04
69	43	44	150	110	482.	0.32	0.43
70	43	45	150	110	719.	0.47	1.03
71	45	46	100	110	233.	0.34	2.30
72	47	50	350	10	-10125.	-1.22	-4.72
73	48	53	450	120	-8025.	-0.58	-0.90
74	48	53	450	1000	-8025.	-0.58	-0.90
75	49	51	350	10	10125.	1.22	4.72
76	52	53	350	100	4013.	0.48	0.09

LEGEND:

- ⑤ Node Number
- L=500 Pipe Length
- Q=200.0 Outflow at the Node in the year 2010
- GL+300.0 Ground Elevation at the Node
- HWL+323.0 High Water Level at the Reservoir



SCHEMATIC PLAN OF DISTRIBUTION SYSTEM FOR HYDRAULIC ANALYSIS  
(Bayombong-Solano, Masoc/La Torre Area)

ALTERNATIVE ML-2, Masoc/La Torre Area (Masoc)  
 2 Reservoir System (New & Existing), Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	310.00	0.00	13.00
2	300.00	291.00	309.69	9.69	23.00
3	290.00	582.00	308.97	18.97	33.00
4	280.00	291.00	308.58	28.58	43.00

Iteration Times : 14

ALTERNATIVE ML-2, Masoc/La Torre Area (La Torre)  
 2 Reservoir System (New & Existing), Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	330.00	20.00	13.00
2	300.00	0.00	320.00	20.00	23.00
3	290.00	0.00	310.00	20.00	33.00
4	280.00	275.00	316.35	36.35	43.00
5	278.00	549.00	316.56	38.56	45.00
6	278.00	275.00	319.37	41.37	45.00
7	271.80	275.00	316.26	44.46	51.20
8	320.00	0.00	320.00	0.00	3.00

Iteration Times : 10

ALTERNATIVE ML-2, Masoc/La Torre Area (Masoc)  
 2 Reservoir System (New & Existing), Year 2010

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 1	200.	200.	110.	-1164.	-0.43	-0.31
2	3 2	200.	800.	110.	873.	0.32	0.72
3	4 3	150.	800.	110.	291.	0.19	0.36

ALTERNATIVE ML-2, Masoc/La Torre Area (La Torre)  
 2 Reservoir System (New & Existing), Year 2010

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	4 5	150.	500.	110.	-274.	-0.18	-0.22
2	5 6	150.	500.	110.	-1098.	-0.72	-2.81
3	6 7	150.	700.	110.	275.	0.18	0.30
4	8 6	200.	300.	110.	-1373.	-0.51	-0.63



ALTERNATIVE ML-1, Masoc/La Torre Area  
 1 Reservoir System (New Reservoir), Year 2010, Fire at LA TORRE  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	310.00	0.00	4.00
2	300.00	145.00	309.45	9.45	14.00
3	290.00	291.00	304.34	14.34	24.00
4	280.00	283.00	299.77	19.77	34.00
5	278.00	2174.00	290.93	12.93	36.00
6	278.00	137.00	290.87	12.87	36.00
7	271.80	2037.00	278.59	6.79	42.20

Iteration Times : 10

ALTERNATIVE ML-1, Masoc/La Torre Area  
 1 Reservoir System (New Reservoir), Year 2010, Fire at LA TORRE  
 << PIPELINE >>

PIPE No.	NODE from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300.	200.	120.	-5057.	-0.83	-0.55
2	3	250.	800.	120.	4920.	1.16	5.11
3	4	250.	800.	120.	4629.	1.09	4.56
4	5	200.	500.	110.	4347.	1.50	8.84
5	6	150.	500.	110.	137.	0.09	0.05
6	5	150.	700.	110.	2037.	1.33	12.35
							17.64

ALTERNATIVE ML-1, Masoc/La Torre Area  
 1 Reservoir System (New Reservoir), Year 2010  
 << NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	310.00	0.00	4.00
2	300.00	291.00	309.85	9.85	14.00
3	290.00	582.00	308.55	18.55	24.00
4	280.00	565.00	307.96	27.96	34.00
5	278.00	549.00	307.27	29.27	36.00
6	278.00	275.00	307.05	29.05	36.00
7	271.80	275.00	306.97	35.17	42.20

Iteration Times : 14

ALTERNATIVE ML-1, Masoc/La Torre Area  
 1 Reservoir System (New Reservoir), Year 2010  
 << PIPELINE >>

PIPE No.	NODE from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2	300.	200.	120.	-2533.	-0.41	-0.15
2	3	250.	800.	120.	2247.	0.53	1.20
3	4	250.	800.	120.	1665.	0.39	0.69
4	5	200.	500.	110.	1099.	0.40	0.69
5	6	150.	500.	110.	275.	0.18	0.22
6	5	150.	700.	110.	275.	0.18	0.30
							0.43

ALTERNATIVE ML-2, Masoc/La Torre Area  
 2 Reservoir System (New & Existing), Year 2010, Fire at MASOC

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	310.00	0.00	13.00
2	300.00	145.00	308.92	8.92	23.00
3	290.00	291.00	305.07	15.07	33.00
4	280.00	436.00	293.14	13.14	43.00
5	278.00	274.00	303.50	25.50	45.00
6	278.00	137.00	317.70	39.70	45.00
7	271.80	137.00	303.42	31.62	51.20
8	320.00	0.00	320.00	0.00	3.00

Iteration Times : 18

ALTERNATIVE ML-2, Masoc/La Torre Area  
 2 Reservoir System (New & Existing), Year 2010, Fire at LA TORRE

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	310.00	0.00	13.00
2	300.00	145.00	309.31	9.31	23.00
3	290.00	291.00	306.92	16.92	33.00
4	280.00	283.00	300.16	20.16	43.00
5	278.00	2174.00	297.41	19.41	45.00
6	278.00	137.00	316.88	38.88	45.00
7	271.80	2037.00	285.06	13.26	51.20
8	320.00	0.00	320.00	0.00	3.00

Iteration Times : 10

ALTERNATIVE ML-2, Masoc/La Torre Area  
 2 Reservoir System (New & Existing), Year 2010, Fire at MASOC

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 1	200.	200.	110.	-2296.	-0.85	-1.08
2	2 2	200.	800.	110.	2151.	0.79	3.85
3	3 4	150.	800.	110.	1660.	1.22	11.93
4	4 5	150.	500.	110.	-2223.	-1.46	-10.36
5	5 6	150.	500.	110.	-2634.	-1.72	-14.19
6	5 7	150.	700.	110.	136.	0.09	0.08
7	6 8	200.	300.	110.	-2771.	-1.02	-2.30

ALTERNATIVE ML-2, Masoc/La Torre Area  
 2 Reservoir System (New & Existing), Year 2010, Fire at LA TORRE

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 1	200.	200.	110.	-1805.	-0.67	-0.69
2	2 2	200.	800.	110.	1660.	0.61	2.38
3	3 4	150.	800.	110.	1369.	0.90	6.76
4	4 5	150.	500.	110.	1086.	0.71	2.75
5	5 6	150.	500.	110.	-3125.	-2.05	-19.48
6	5 7	150.	700.	110.	2037.	1.33	12.35
7	6 8	200.	300.	110.	-3262.	-1.20	-3.12

ALTERNATIVE ML-3, Mascoc/La Torre Area  
 1 Reservoir System (Existing Reservoir), Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	330.00	20.00	13.00
2	300.00	291.00	318.89	18.89	23.00
3	290.00	582.00	318.92	28.92	33.00
4	280.00	566.00	319.13	39.13	43.00
5	278.00	549.00	319.46	41.46	45.00
6	278.00	215.00	319.77	41.77	45.00
7	271.80	215.00	319.15	47.35	51.20
8	320.00	0.00	320.00	0.00	3.00

Iteration Times : 27

ALTERNATIVE ML-3, Mascoc/La Torre Area  
 1 Reservoir System (Existing Reservoir), Year 2010, Fire at MASOC

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	310.00	0.00	330.00	20.00	13.00
2	300.00	3945.00	307.86	7.86	23.00
3	290.00	291.00	311.25	21.25	33.00
4	280.00	263.00	315.12	35.12	43.00
5	278.00	274.00	317.85	39.85	45.00
6	278.00	137.00	319.17	41.17	45.00
7	271.80	137.00	317.77	45.97	51.20
8	320.00	0.00	320.00	0.00	3.00

Iteration Times : 18

ALTERNATIVE ML-3, Mascoc/La Torre Area  
 1 Reservoir System (Existing Reservoir), Year 2010

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 3	250.	800.	120.	-292.	-0.07	-0.03
2	3 4	250.	800.	120.	-573.	-0.21	-0.26
3	4 5	250.	500.	120.	-1439.	-0.34	-0.66
4	5 6	300.	500.	120.	-2253.	-0.37	-0.31
5	5 7	150.	700.	110.	275.	0.18	0.30
6	5 8	300.	300.	120.	-2538.	-0.42	-0.23

ALTERNATIVE ML-3, Mascoc/La Torre Area  
 1 Reservoir System (Existing Reservoir), Year 2010, Fire at MASOC

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu. m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	2 3	250.	800.	120.	-3945.	-0.93	-3.39
2	3 4	250.	800.	120.	-4236.	-1.00	-3.87
3	4 5	250.	500.	120.	-4519.	-1.07	-2.73
4	5 6	300.	500.	120.	-4930.	-0.81	-1.82
5	5 7	150.	700.	110.	136.	0.09	0.08
6	5 8	300.	300.	120.	-5067.	-0.83	-0.83

(Unit: thousand Pesos)

Bayombong-Solano ITEM	UNIT COST	Phase I (Stage 1)		Phase I (Stage 2)		Phase I Total		Phase II Cost	
		NUMBER	COST	NUMBER	COST	NUMBER	COST	NUMBER	COST
<b>1 SOURCE FACILITY</b>									
(1) Spring Box Rehabil.	30000	1	30	0	0	1	30	0	0
Drain Pipe D=250	340	10	3	0	0	10	3	0	0
Valve D=250	11200	1	11	0	0	1	11	0	0
(2) RADIAL WELL	1240000	0	0	1	1240	1	1240	1	1240
WELL PUMP 22kw	358000	0	0	3	1074	3	1074	3	1074
Pump House on Well	820000	0	0	1	820	1	820	1	820
(3) Electric Sub-station			0		0		0	150KVA	2580
SUB-TOTAL			44		3134		3178		5714
<b>2 TRANSMISSION FACILITIES</b>									
Main Pipe D=250 (Steel)	630	4300	2709	0	0	4300	2709		0
Valve D=250	11200	2	22	0	0	2	22		0
SUB-TOTAL			2731		0		2731		0
<b>3 DISTRIBUTION FACILITIES</b>									
(1) Chlorination Facility	98100	1	98	1	98	2	196	0	0
(2) Distribution pipes									
1) Main Pipes									
D=100 (PVC Pipe)	250	1200	300	0	0	1200	300	0	0
D=150 (PVC Pipe)	410	700	287	3300	1353	4000	1640	500	205
D=200 (Steel Pipe)	520	0	0	1155	601	1155	601	0	0
D=250 (Steel Pipe)	630	3200	2016	5	3	3205	2019	500	315
D=300 (Steel Pipe)	760	800	608	0	0	800	608	0	0
D=350 (Steel Pipe)	900	10	9	0	0	10	9	1650	1485
D=400 (Steel Pipe)	970	2800	2716	650	631	3450	3347	0	0
D=500 (Steel Pipe)	1330	0	0	1100	1463	1100	1463	0	0
D=600 (Steel Pipe)	1600	0	0	1000	1600	1000	1600	0	0
RIVER CROSSING CD=2.0m	2090	0	0	100	209	100	209	100	209
D=500 Materials	718	0	0	100	72	100	72	0	0
D=350 Materials	486	0	0	0	0	0	0	100	49
2) Valves									
D=100 (Gate Valve)	3900	1	4	0	0	1	4	0	0
D=150 (Gate Valve)	5300	2	11	11	58	13	69	2	11
D=200 (Gate Valve)	6700	0	0	5	34	5	34	0	0
D=250 (Gate Valve)	11200	8	89	1	11	9	100	2	22
D=300 (Butterfly Valve)	34800	3	104	0	0	3	104	0	0
D=350 (Butterfly Valve)	74400	1	74	0	0	1	74	2	149
D=400 (Butterfly Valve)	95200	1	95	1	95	2	190	0	0
D=500 (Butterfly Valve)	174000	0	0	2	348	2	348	0	0
D=600 (Butterfly Valve)	243600	0	0	1	244	1	244	0	0
Prssre Cntrl Valve 350	390000	1	390	0	0	1	390	0	0
3) Internal Network									1762
UPTO 1990	5152		5152		0		5152		
UPTO 1995	2930		0		2930		2930		
4) Service Connections									
D=1/2	810	1685	1365	4608	3733	6293	5098	6791	5501
D=3/4	1280	9	12	10	15	19	27	13	17
5) Rehabilitation									
Water Meter 1/2''	400	1309	524	0	0	1309	524		
Old Laterals			488	0	0	0	488		
Service Connect.wo/Metr	480	669	321	0	0	669	321		
6) Flow Meter									
D=250	93000	1	93	0	0	1	93		0
D=300	126000	1	126	0	0	1	126		0
D=400	215000	0	0	1	215	1	215		0
7) Fire Protection									
D=150	16800	0	0	0	0	0	0	34	571
D=100	9400	0	0	0	0	0	0	151	1419
SUB-TOTAL			14882		13713		28595		11715
<b>4</b>									
1) Administration Bldg.								1	1320
2) Operation Center		1	1090			1	1090		
SUB-TOTAL		1	1090		0	1	1090	1	1320
<b>5 Land Acquisition</b>	35.75	2225	158	0	0	2225	158		0
Vehicle	300000	2	600	1	300	3	900	1	300
Stored Material & Equip.			214		196		410		205
SUB-TOTAL			972		496		1468		505
<b>6 Replacement of Equipment</b>			0		0		0		7425
<b>T O T A L</b>			19719		17343		37062		26679
<b>7 Leakage Detection</b>	240	1338	321	0	0	1338	321		0
<b>GRAND TOTAL</b>			20040		17343		37383		26679

(Unit: thousand Pesos)

Bayombong-Solano		UNIT COST		1988		1989		1990		1991	
ITEM		NO	COST	NO	COST	NO	COST	NO	COST	NO	COST
<b>1 SOURCE FACILITY</b>											
(1) Spring Box Rehabili.	30000		0	1	30		0		0		0
Drain Pipe D=250	340		0	10	3		0		0		0
Valve D=250	11200		0	1	11		0		0		0
(2) RADIAL WELL	1240000		0		0		0	1	1240		
WELL PUMP 22kw	358000		0		0		0	2	716		
Pump House on Well	820000		0		0		0	1	820		
(3) Electric Sub-station			0		0		0		0		0
SUB-TOTAL			0		44		0		2776		
<b>2 TRANSMISSION FACILITIES</b>											
Main Pipe D=250 (Steel)	630		0	4300	2709		0		0		0
Valve D=250	11200		0	2	22		0		0		0
SUB-TOTAL			0		2731		0		0		0
<b>3 DISTRIBUTION FACILITIES</b>											
(1) Chlorination Facility	98100		0	1	98		0	1	98		
(2) Distribution pipes		1988		1989		1990		1991			
1) Main Pipes											
D=100 (PVC Pipe)	250		0		0	1200	300		0		0
D=150 (PVC Pipe)	410		0	700	287		0	3300	1353		
D=200 (Steel Pipe)	520		0		0		0	1155	601		
D=250 (Steel Pipe)	630		0	2100	1323	1100	693	5	3		
D=300 (Steel Pipe)	760		0	800	608		0		0		
D=350 (Steel Pipe)	900		0		0	10	9		0		
D=400 (Steel Pipe)	970		0		0	2800	2718	650	631		
D=500 (Steel Pipe)	1330		0		0		0	1100	1463		
D=600 (Steel Pipe)	1600		0		0		0	1000	1600		
RIVER CROSSING CD=2.0m	2090		0		0		0	100	209		
D=500 Materials	718		0		0		0	100	72		
D=350 Materials	486		0		0		0		0		
2) Valves		1988		1989		1990		1991			
D=100 (Gate Valve)	3900		0		0	1	4		0		
D=150 (Gate Valve)	5300		0	2	11		0	11	58		
D=200 (Gate Valve)	6700		0		0		0	5	34		
D=250 (Gate Valve)	11200		0	6	67	2	22	1	11		
D=300 (Butterfly Valve)	34800		0	3	104		0		0		
D=350 (Butterfly Valve)	74400		0		0	1	74		0		
D=400 (Butterfly Valve)	95200		0		0	1	95	1	95		
D=500 (Butterfly Valve)	174000		0		0		0	2	348		
D=600 (Butterfly Valve)	243600		0		0		0	1	244		
Prssre Cntrl Valve 350	390000		0		0	1	390		0		
3) Internal Network											
UPTO 1990	5152		0		2576		2576				
UPTO 1995	2930										586
4) Service Connections											
D=1/2	810	0	0	843	683	842	682	921	746		
D=3/4	1280	5	6	2	3	2	3	2	3		
5) Rehabilitation											
Water Meter 1/2"	400	1309	524	0	0	0	0				
Old Laterals			0		244		244				
Service Connect.wo/Metr	480	223	107	223	107	223	107				
6) Flow Meter											
D=250	93000		0	1	93		0		0		
D=300	126000		0	1	126		0		0		
D=400	215000		0		0		0	1	215		
7) Fire Protection											
D=150	16800		0								
D=100	9400		0								
SUB-TOTAL			637		6330		7915		8370		
<b>4 Administration Bldg.</b>											
<b>2) Operation Center</b>											
SUB-TOTAL			0	1	1090		0		0		0
<b>5 Land Acquisition</b>											
Vehicle	300000	1	300	1	300		0	1	300		
Stored Material & Equip.			12		108		94		109		
SUB-TOTAL			470		408		94		409		
<b>6 Replacement of Equipment</b>											
TOTAL			1107		10603		8009		11555		
<b>7 Leakage Detection</b>											
SUB-TOTAL	240	446	107	446	107	446	107		0		
GRAND TOTAL			1214		10710		8116		11555		

(Unit: thousand Pesos)

Bayombong-Solano		1992		1993		1994		1995	
ITEM	UNIT COST	NO	COST	NO	COST	NO	COST	NO	COST
<b>1 SOURCE FACILITY</b>									
(1) Spring Box Rehabil.	30000		0		0		0		0
Drain Pipe D=250	340		0		0		0		0
Valve D=250	11200		0		0		0		0
(2) RADIAL WELL	1240000		0		0		0		0
WELL PUMP 22kw	358000		0	1	358		0		0
Pump House on Well	820000		0		0		0		0
(3) Electric Sub-station			0		0		0		0
SUB-TOTAL			0		358		0		0
<b>2 TRANSMISSION FACILITIES</b>									
Main Pipe D=250 (Steel)	630		0		0		0		0
Valve D=250	11200		0		0		0		0
SUB-TOTAL			0		0		0		0
<b>3 DISTRIBUTION FACILITIES</b>									
(1) Chlorination Facility	98100		0		0		0		0
(2) Distribution pipes		1992		1993		1994		1995	
1) Main Pipes									
D=100 (PVC Pipe)	250		0		0		0		0
D=150 (PVC Pipe)	410		0		0		0		0
D=200 (Steel Pipe)	520		0		0		0		0
D=250 (Steel Pipe)	630		0		0		0		0
D=300 (Steel Pipe)	760		0		0		0		0
D=350 (Steel Pipe)	900		0		0		0		0
D=400 (Steel Pipe)	970		0		0		0		0
D=500 (Steel Pipe)	1330		0		0		0		0
D=600 (Steel Pipe)	1600		0		0		0		0
RIVER CROSSING CD=2.0m	2090		0		0		0		0
D=500 Materials	718		0		0		0		0
D=350 Materials	486		0		0		0		0
2) Valves		1992							
D=100 (Gate Valve)	3900		0		0		0		0
D=150 (Gate Valve)	5300		0		0		0		0
D=200 (Gate Valve)	6700		0		0		0		0
D=250 (Gate Valve)	11200		0		0		0		0
D=300 (Butterfly Valve)	34800		0		0		0		0
D=350 (Butterfly Valve)	74400		0		0		0		0
D=400 (Butterfly Valve)	95200		0		0		0		0
D=500 (Butterfly Valve)	174000		0		0		0		0
D=600 (Butterfly Valve)	243600		0		0		0		0
Prssre Cntrl Valve 350	390000		0		0		0		0
3) Internal Network									
UPTO 1990	5152								
UPTO 1995	2930		586		586		586		586
4) Service Connections									
D=1/2	810	922	747	922	747	922	747	921	746
D=3/4	1280	2	3	2	3	2	3	2	3
5) Rehabilitation									
Water Meter 1/2"	400								
Old Laterals									
Service Connect.wa/Metr	480								
6) Flow Meter									
D=250	93000		0		0		0		0
D=300	126000		0		0		0		0
D=400	215000		0		0		0		0
7) Fire Protection									
D=150	16800								
D=100	9400								
SUB-TOTAL			1356		1336		1330		1335
<b>4</b>									
1) Administration Bldg.									
2) Operation Center									
SUB-TOTAL			0		0		0		0
<b>5 Land Acquisition</b>									
Vehicle	35,75		0		0		0		0
Stored Material & Equip.	300000		0		0		0		0
SUB-TOTAL			20		27		20		20
<b>6 Replacement of Equipment</b>									
SUB-TOTAL			20		27		20		20
<b>T O T A L</b>									
			1356		1721		1356		1355
<b>7 Leakage Detection</b>									
SUB-TOTAL	240		0		0		0		0
<b>GRAND TOTAL</b>			1356		1721		1356		1355