The present worth benefit of increase in land values from the project implementation is shown in TABLE 10.3.1.

10.3.2 Beneficial Value of Water

It is assumed that all residents of the served area would be willing to obtain water in sufficient quantities at a given price. In general, water prices charged by the Water District are lower than the real value of water. Taking the benefits for "consumer's satisfaction" into consideration, it is assumed that the economic value of water is 20% higher than the de-escalated average rate per cu.m of water used in the Financial Analysis.

The economic value of water is shown in TABLE 10.3.2.

10.4 ECONOMIC COSTS OF THE PROJECT

The direct costs of the project should be transformed into economic costs. For this purpose, the project cost and operating and maintenance costs are considered in the study. These costs will be converted into the economic costs using factors for shadow pricing. The factors for shadow pricing applicable to the study are as follows:

- Foreign exchange component:	1.3
- Unskilled labor premium :	0.5
- Others :	1.0

All taxes should be excluded in the economic study. It is assumed that the cost for the balance of domestic component includes hidden taxes for 5% of the amount.

TABLE 10.3.1 PORTION OF LAND VALUES ATTRIBUTABLE TO PROJECT

				Commowsial/Taduet-		20 Percent
Residential	Comme rial/	rcial/Indust- Institutional	Residential (#50/sq.m)		Total Cost of Land	Due to Project
720 720	80 80 80		36,000 36,000	9,600 9,600	45,600 45,600	9,120 9,120
810 720	06	· · ·	40,500	10,800	51,300	10,260
720	80		36,000	9,600	45,600	9,120
-			· · ·			
•					•	
	·	· · · ·				
						·
		· · · · · · · · · · · · · · · · · · ·		· · ·		
	•			· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	н - м
				•		
•						
		.4	•		•	

2/ Portion of land values specifically attributable to water supply project is 20%.

TABLE 10.3.2 INCREASE IN CONSUMER STATISFACTION

	Incremental Accounted-For Water ^{3/}	Price Per	Economic Value Per	Economic Water Revenues
Year	(1,000 cu.m/year)	4/	<u>5/</u>	(₽1,000)
1988	-83	2.64	3.17	-263
1989	441	2.77	3.32	1,466
1990	972	2.44	2.93	2,846
1991	1,829	3.28	3.94	7,199
1992	2,688	4.40	5.28	14,193
1993	3,544	4.23	5.08	17,989
1994	4,403	4.04	4.85	21,346
1995	5,261	3.61	4.33	22,791
1996	5,261	3.43	4.12	21,654
1997	5,261	3.06	3.67	19,318
1998	5,261	3.23	3.88	20,392
1999	5,261	3.33	4.00	21,023
2000	5,261	3.30	3.96	20,834
2001	5,261	2.95	3.54	18,624
2002	5,261	3.42	4.10	21,591
2003	5,261	3.05	3.66	19,255
2004	5,261	3.15	3.78	19,887
2005	5,261	2.81	3.37	17,740
2006	5,261	2.84	3.41	17,929
2007	5,261	2.99	3.59	18,876

- 3/ The volume of accounted-for water of 1.13 million cu.m in 1986 is deducted from the projected water consumptions throughout the study period to obtain incremental volume.
- 4/ The price per cu.m was based on the de-escalated average rate/cu.m in the financial analysis.
- 5/ The economic value was assumed to be 1.2 times the price per cu.m of water.

10.4.1 Project Cost

By using the shadow pricing factors, the economic project cost is #114.17 million which was obtained based on the project cost used in the Financial Feasibility Analysis as shown in TABLE 10.4.1.

10.4.2 Salvage Value

TABLE 10.4.2 presents the salvage value of all the capital equipment in the project in 2007. The percentage of the salvage value was based on the remaining service life of the facilities in 2007.

10.4.3 Operating and Maintenance Costs

In the economic analysis, operating and maintenance costs of personnel, power, chemicals, and maintenance are considered. Likewise, this cost category is converted to economic costs by the shadow pricing factors. TABLE 10.4.3 shows the economic operating and maintenance cost.

10.5 ECONOMIC INTERNAL RATE OF RETURN (EIRR)

EIRR is determined based on economic costs and benefits of the projects. TABLE 10.5.1 shows the computation of the EIRR. The EIRR is 17.6%. Since this rate exceeds the opportunity cost of capital of 12%, and a number of unquantifiable benefits will be also conceived from the implementation of the project, the project is considered economically feasible and an undertaking of the project is suggested itself to proceed positively.

COST	
PROJECT	
ECONOMIC	:
10.4.1	
TABLE	

.

		:							SHADUW FRICING		•
	Financial	Foreign	:		Balance of			Foreign Exchange	Unskilled		Total
	Project Cost	Exchange Component	Domestic Component	Unskilled Labor	Domestic Component	Taxes (5%)	Others (95%)	Component x 1.3	Labor x 0.5	Others x 1.0	Economic Cost
Civil Works	:										
Deep Well Facilities	6,398	1,579	4,819	585	4,234	212	4,022	2,053	292	4,022	6 367
Transmission Facilities	789	309	480	69	411	20	391	402	5	391	828
Reservoir	3,824	1,093	2,731	294	2,437	122	2,315	1,421	147	2,315	3,883
Disinfection Facilities	306	36	267	24	243	12	231	21	12	231	294
Electric Sub-station	1,033	427	606	. 55 .	551	27	524	555	28	524	1,107
Distribution Facilities	14,214	4,457	9,757.	1,587	8,170	409	7,762	5,794	793	7,762	14,349
Service Connection	1,896	172	1,724	505	1,219	61.	1,158	223	252	بار مرتبر	1,633
	588	0	588	0 (288	6.0	644 4	0		2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	א ע ה ע ה ה
Admin. Bldg. & Ope. Ctr.	795	143	652	6/ ·	5/3	5	040 040	186	40	0 0 0 0	69/
Vehicle & Stored Material Sub-Total of Civil Works	0	8.219	0 21 624	<u>3, 198</u>	18.426	921	17.505	10.685	1.599	17.505	<u>29,789</u>
				•	.		•				•
Equipment				•							
Deep Well Facilities	7,707	5,872	1,835	O	1,835	92	1,743	7,634	0	1,743	9,377
Transmission Facilities	926	532	394	0	394	20	374	692	0	374	L, 066
Reservoir	37.8	210	168	0	168	00	160	273	0	160	433
Disinfection Facilities	479	361	118	0	118	9	112	469	0	112	581
Electric Sub-station	1,024				017	9 F 9 F	707 7	10 036	o c	107 707 7	4,144 on 2,00
Ulserlouelon raciliteles	47°471	14 040	0110			46	644°	19,127	o 'c	646	10 773
Service Connection Land Arnuisition					0	<u>,</u> 0		0	00		0
Admin. Bldg. & Ope. Ctr.	795	572	223	0	223	11	212	744		212	926
Vehicle & Stroed Material	2,757	1,704	1,053	0	1,053	52	1,001	2,215	0	1,001	3,216
Sub-Total of Equipment	51,580	40,055	11,525	0	11,525	576	10,949	52,072		10,949	63,021
Total of C.W. & Equipment	81,423	48,274	33,149	3,198	29,951	1,497	28,454	62,757	1,599	28,454	92,810
Physical Contingencies <u>6</u> /	6,514	3,862	2,652	256	2,396	120	2,276	5,021	128	2,276	7,425
Engineering Services ⁷	11,399	6,758	4,641	877	4,193	210	3,983	8,786	224	3,983	12,993
Leakage Detection	066	0	066	o	066	50	940	0	0	940	940
Project Cost	100,326	58,894	41,432	3,902	37,530	1,877	35,653	76,564	1,951	35,653	114,168

(Unit : F1,000)

· · ·

Salvage Value	Remaining Life in 2007 ² /	Economic Value	Year
6,810	50.0 %	13,620	1988
18,372 ⁻	52.5	34,995	1989
7,955	55.0	14,464	1990
8,360	57.5	14,539	1991
5,599	60.0	9,331	1992
5,832	62.5	9,331	1993
6,047	65.0	9,303	1994
5,795	67.5	8,585	1995
	· · ·	4.	1996
		· · ·	1997
			1998
			1999
			2000
			2001
			2002
			2003
			2004
	and a second second Second second		2005
			2006
64,770			2007

TABLE 10.4.2 SALVAGE VALUE IN YEAR 2007

 $\frac{8}{2}$ The average economic life of all items is assumed to be 40 years.

SHADOW PRICING	Foreign ExchangeEconomicNet EconomicOthersComponentOthersO & M(95%)x 1.3x 1.0CostCost	826 1,278 2,104	1,521 389 1,521 1,910 -194	1,894 612 1,894 2,506 402	2,241 819 2,241 3,060 956	2,775 1,110 2,775 3,885 1,781	3,289 1,426 3,289 4,715 2,611	3,844 1,690 3,844 5,534 3,430	4,377 1,980 4,377 6,357 4,253	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078	4,911 2,271 4,911 7,182 5,078
	Taxes (5%)	87	80	100	118	146	173	202	230	258	258	258	258	258	258	258	258	258	258	258	258
	Domestic Component	1,345	1,601	1,994	2,359	2,921	3,462	4,046	4,607	5,169	5,169	5,169	5,169	5,169	5,169	5,169	5,169	5,169	5,169	5,169	5,169
	Foreign Exchange Component	635	299	171	630	854	1,097	1,300	1,523	1,747	1,747	1,747	1,747	1,747	1,747	1,747	1,747	1,747	1,747	1,747	1,747
	Financial O & M Cost	1,980	1,900	2,465	2,989	3,775	4,559	5,346	6,130	6,916	6,916	6,916	6,916	6,916	6,916	6,916	6,916	6,916	6,916	6,916	6,916
	Year	1987	1988	1989	1990	1991	1992	1993	1994	1995	1996	1997	1998	1999	2000	2001	2002	2003	2004	2005	2006

	·		(Unit:	1,000 Pesos
Year	Total Economic Benefits	Total Economic Costs	Net Benefits	Present Value
1988	-263	13,426	-13,689	-13,689.0
1989	1,466	35,397	-33,931	-28,848.0
1990	2,846	15,420	-12,574	-9,088.9
1991	16,319	16,320	: ~ 	-0.(
1992	23,313	11,942	11,371	5.941.2
1993	28,249	12,761	15,488	6,880.1
1994	30,486	13,556	16,910	6,386.5
1995	31,911	13,663	18,248	5,859.4
1996	21,654	5,078	16,576	4.525.
1997	19,318	5,078	14,240	3,305.
1998	20,392	5,078	15,314	3,021.
1999	21,023	5,078	15,945	2.675.
2000	20,834	5,078	15,756	2,247.4
2001	18,624	5,078	13,546	1,642.
2002	21,591	5,078	16,513	1.702.
2003	19,255	5,078	14,177	1,242.
2004	19,887	5,078	14,809	1,103.
2005	17,740	5,078	12,662	802.
2006	17,929	5,078	12,851	692.
2007	18,876	-59,692	78,568	3,598.

TABLE 10.5.1 ECONOMIC INTERNAL RATE OF RETURN

EIRR (%)= 17.62

CHAPTER 11 ORGANIZATION AND MANAGEMENT

CHAPTER 11 ORGANIZATION AND MANAGEMENT

11.1 PRESENT ORGANIZATION STRUCTURE

The Angeles City Waterworks System (ACWS) is owned, operated and maintained by the city government of Angeles.

A total of 37 personnel is assigned to the ACWS; 31 for operation and maintenance services under the City Engineer's Office and six for the collection services under the City Treasurer's Office. (See FIGURE 11.1.1 for the present organization structure of the system).

The city mayor is the chief appointing officer. All management and operational policies emanate from the city mayor and the Sangguniang Panlungsod.

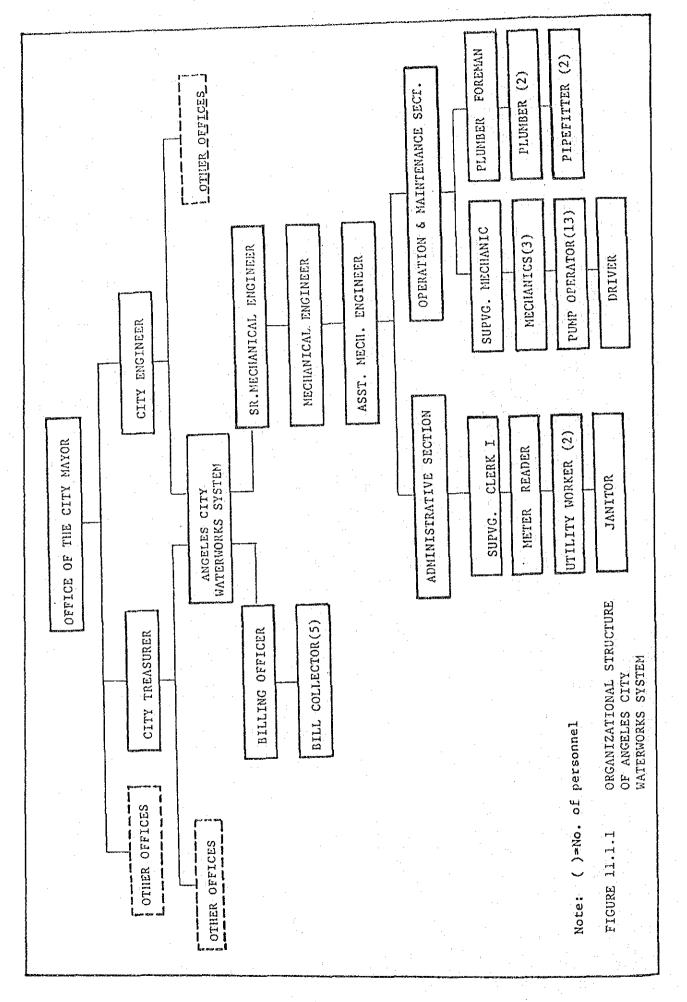
Tax Ordinance No. 8, series 1978 defines the schedule of water rates for the ACWS. It is actually the Sangguniang Panlungsod that decides on the approval of such rates and its date of effectivity.

11.2 PROPOSED ORGANIZATION STRUCTURE

11.2.1 Introduction

The proposed organization structure for the ACWS is the water district structure. The JICA Study Team believes that with the water district organization structure, the water system can look forward to dedicated policy makers providing continuity of policy and its consistent enforcement; to financial independence and business-like management; to capable, qualified and regularly trained personnel; and to freedom from political interference in its operations, particularly on hiring and water rates setting.

The JICA Study Team, however, proposes some changes on both the staffing guidelines as well as the organization chart. The basic water district structure, now existing remains the same.



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11,2.2 Examination of the LWUA Methodology Manual

The number of water district employees depends primarily on the total number of service connections. Based on this concept, LWUA's Methodology Manual proposes the following staff requirement for a water district.

	· · · · · · · · · · · · · · · · · · ·	No. c	of Connect	tions	
Personnel	2,000	5,000	10,000	15,000	20,000
General Manager	. 1	1	· 1	1	1
Administrative Staff	7	14	20	22	23
Technical Staff	16	35	59	75	92
Commercial Staff	12	25	40	52	.64
- meter readers,	(6)	(14)	(25)	(32)	(42)
bill collectors, inspectors					I
- other employees	(6)	(11)	(15)	(20)	(22)
Total Staff	36	75	120	150	180
Staff/1,000 Connections	18	15	12	10	9

Note: The above data do not include personnel for construction of new connections, etc. which are considered to be part of development cost. These development expenses entail additional staff which is assumed to be 15 personnel per 1,000 additional service connection.

Moreover, the above figures are related to a water supply system without treatment plant. If such plant were necessary, additional 10 to 15 employees for each treatment plant with a capacity ranging from 10,000 to 50,000 cu.m/day may be considered.

Above-indicated table shows the staff requirement corresponding up to 20,000 connections. Therefore, we cannot use this table to compute the number of personnel both for Angeles Water District and Dagupan Water District in 1995 and 2010, since they will apparently be beyond the extent of the table.

It is possible, not to mention, that it can be estimate the number of personnel exceeding 20,000, on the basis of some assumption of personnel number per 1,000 connections. but it seems to be slightly forcible.

Therefore, JICA Study Team checked the formula derived from the result of a statistical analysis of the number of personnel in relation to the number of connections for 38 existing water districts for the years 1979, 1980 and 1982.

The guideline proposes two staffing levels. The first level has been proposed for the period 1986-1995; the second for the period 1996-2010.

The upper staffing level for the period 1996-2010 was computed from the formula:

log (no. of employees) = log (no. of connections) x 0.8311 - 1.2113

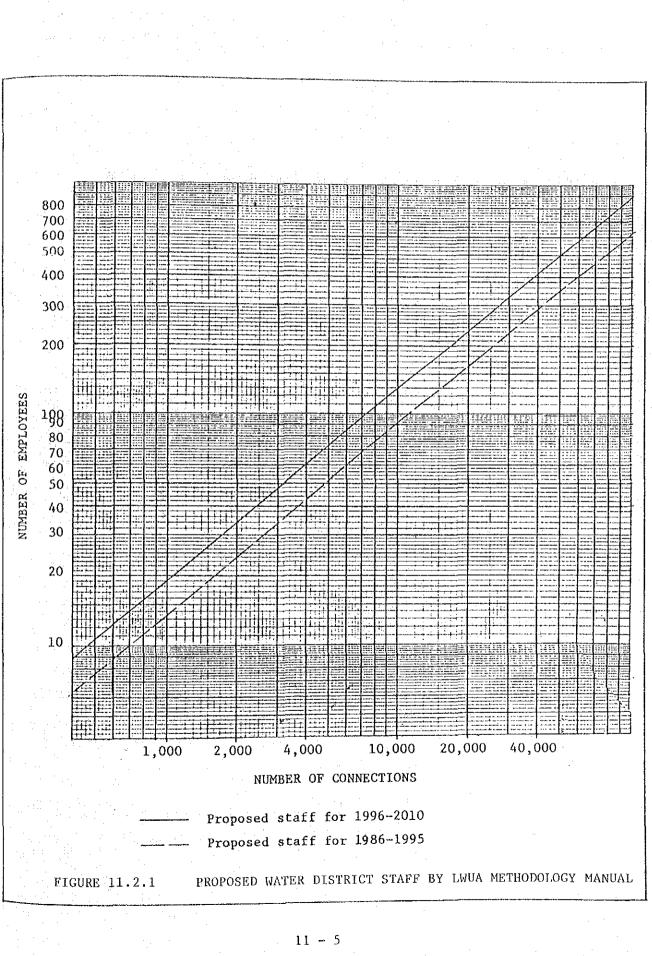
See FIGURE 11.2.1 for the number of personnel needed for the Angeles City Water District, once formed, using the LWUA Methodology Manual.

The lower level, which was assumed for the earlier period from 1986-1995 allows a 30% reduction in staff below the calculated value.

Using the above mentioned formula, the number of personnel for ACWS, if formed into a water district, could be computed as follows:

Design Year	No. of Employees
1995	179
2010	411

It is true that the formula presented by LWUA as a guideline in this study is based on the result of a statistical analysis of the existing water district. But no evaluation has been undertaken whether the figures which were derived from the formula are appropriate or not.



Considering the actual scope and content of operation and management work, the number of personnel computed from the LWUA Methodology Manual, seems to be rather high, particularly for the year 2010.

In this study, therefore, a new proposal regarding the number of personnel based on the appropriate performance of individual work may be recommended.

11.2.3 Proposed Guideline of the JICA Study Team

The task of management is to carry policy into effect with the fullest efficiency withing the limits assigned; that is, to attain maximum performance at minimum cost. It is the duty of management to create conditions which will bring about the optimum use of all resources available to the water district.

Based on this concept, the JICA Study Team examined the present structure of the existing waterworks, not only with regard to the number of personnel; but also with regard as a reference were the statistical data regarding the number of personnel and organization structures of waterworks systems in developed countries.

The procedure which has been adopted in this study is, at first, to divide the organization of a water district into two main branches.

- a) The administrative and commercial division, comprising what may be termed the business management (including matters concerning water charges) of the water district.
- b) The engineering and technical division, embracing the design and construction of minor extension or improvement works utilizing internal reserve fund; and the operation renewal and maintenance of existing works.

Secondly, the number of personnel has been computed considering the present number of personnel of the ACWS and appropriate performance of individual assignment.

TABLE 11.2.1 shows the JICA Study Team's guideline on the personnel needed to man the proposed water district by scope and content of work.

It was decided by JICA Study Team, to assign no bill collector based on the assumption that the consumers will pay water charges directly to the water district or through their affiliated banks.

TABLE 11.2.2 shows the summary of the present (1986) and the proposed staffing pattern (1995 and 2010) for the ACWS.

11.2.4 Organization Structure

The organization structure proposed for the ACWS is basically the water district structure.

The water system/district will be headed by a five-man Board of Directors, as per PD 198, who will come from the various sectors of the community and will be appointed by the mayor or governor, as the case may be. It is this Board that will set all the policies of the water system/ district.

The two broad divisions of the system/district will be:

a) Administrative and Commercial Division

b) Technical Division

Later on, it would be necessary for the ACWS (Water District) to create the position of Assistant General Manager. And as the commercial operations expand, it also might be necessary and advisable to split the Administrative and Commercial Division into two divisions.

TABLE 11.2.1 STAFFING GUIDELINE

CTON OF STAFF NUMBER NUMBER	1 Sec.23 of Provincial Water Utilities Act of 1973	MMERCIAL	3¢ rt	ion	• •	2 (< 20,000 °Establishment	connections) "Resister of Land, fixed assets	0	connections) Ceneral Info	° Board Work, Contracts	°Cash Receipts and Payments	°Revenue Expenditure	L Capital Expenditure	S.	and	CHARBAS of Doningeot and Matarial	7 (<50.000 Miscellaneous Costs	 CONNECLIONS/				
DIVISION AND POSITION OF STAFF	General Manager	ADMINISTRATIVE AND COMMERCIAL	Asst. General Manager	General Affairs Division	Manazer	۲. ۲. ۲. ۲. ۲.					Account Division		Manager	Staff					General Service Division	Manaver	Clerk	Mechanics

* Extentions, Renewals, Cleaning and Repairs, Street Repairs ° One (1) staff per every five (5) kilometer for the first 60 km, Preservation of Water Sources, Prevention of Pollution "Meter Reading (Consumption, Complaints, Reports) "Collection of Water Charges ° One (1) meter reader per 1,500 connections REMARKS then add one per every ten (10) kilometer "Transmission Mains/ Distribution Mains* "Afforestation (in case of spring source) ° Two (2) attendants per one reservoir CONTENTS OF WORKS. * Hydrants and Valves ° Painters, Blacksmith & c. ^oOperation and Maintenance Waste Inspection "Resister of Supplies Service Reservoirs Revision of Charges "Pumping Stations Meter Charges Water Survey ° Assessments Statistics Guarantees Pipeline length Proportional to the no.of conn. connections) Prop'l to the connections) connections) 2 for every 5 5 (<50,000 3 (< 30,000 2 (<20,000 stations NUMBER STAFFING GUIDELINE (continued) DIVISION AND POSITION OF STAFF Asst. General Manager (Eng'r) Reservoir Attendants Water Charges Division General Maintenance Distribution Division Pipeline Patrol Manager (Eng'r) Pump Operators Meter Reader Electrician TABLE 11.2. 1 Mechanics Manager Clerk TECHNICAL

Service Works Division Manager	r{	°Meter Fixing and Repairs
Manager	r~{	
· · ·		<pre>^Meter Fixing ^Exchanging and Testing ^Repairing</pre>
		°Plumbing
		<pre>°Laying and Maintenance of Service Pipes and Fittings °Plumbing Repairs °Testing of Fittings</pre>
Fitters	Proportional to the no.of conn.	^o Two(2) fitters for the first 20,000 connections, then add one (1) fitter for every additional 10,000 connections
Meter Repairman	Proportional to the no.of conn.	Two(2) repairers for the first 10,000 connections, then add one (1) repairer for every additional 10,000 connections
Plumbers	Proportional to the no.of conn.	* Two(2) plumbers for the first 20,000 connections, then add one (1) plumber for every additional 10,000 connections
Laborers	Proportional to the no.of conn.	° Two (2) laborers for the first 20,000 connections, then add one laborer forevery additional 10,000 connections

		· · · · · · · · · · · · · · · · · · ·	
	1986	1995	2010
Population Served No. of Connections Supply Capacity (cu.m/day) Length of T/D Main (m) No. of Pump Stations	27,600 4,128 12,000 2,000 11	101,900 22,500 31,300 17,000 14	168,200 40,000 59,800 36,000 24
General Manager		1	1
ADMINISTRATIVE AND COMMERCIAL		· · · ·	
Assistant General Manager General Affairs Accountancy General Services Water Charges	1 - 4	1 4 5 13	1 4 8 13
Manager Clerk Meter Reader Bill Collector	1 1 5	1 3 15 -	1 5 27
Sub-Tota1	7	19	33
Total	12	<u>39</u>	<u>59</u>
TECHNICAL			-
Assistant General Manager Distribution Manager Mechanics Electrician Pump Operator Reservoir Attendant Patrol General Maintenance	1 	1 2 1 5 2 4 3 19	1 2 1 10 6 8 3 32
Sub-Total	19	17	
Service Works Manager Fitter Meter Repairman Plumber Laborer	- - - -	1 3 3 3 3 3	1 5 4 5 5
Sub-Total	5	13	20
Total	25	<u>32</u>	52
GRAND TOTAL	<u>37</u>	<u>72</u>	<u>112</u>

SUMMARY OF PRESENT (1986) AND PROPOSED TABLE 11.2.2

For now, however, the two division structure is recommended and the proposed number of personnel for both divisions is:

Position/Division	1995	2010
General Manager	. 1	1
Administrative and Commercial	39	. 59
Technical	32	52
Total	<u>72</u>	112

FIGURE 11.2.2 shows the proposed organization chart for the ACWS Water District.

11.3 MANAGEMENT

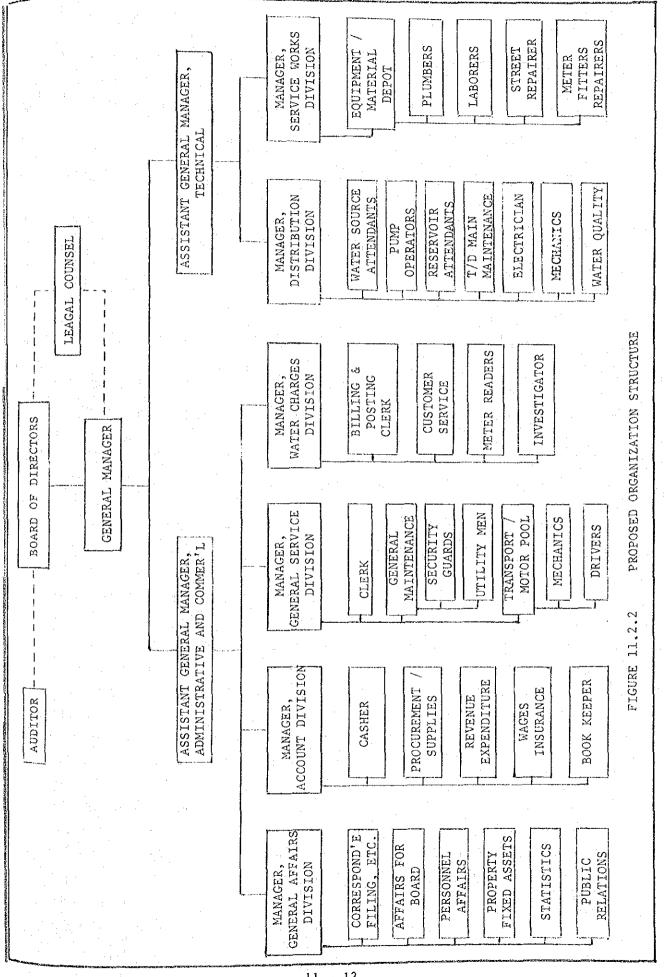
11.3.1 Importance of Management

As stated in Section 11.2.3, the task of management is to carry policy into action with the fullest efficiency within the limits assigned; that is, to attain maximum performance at minimum cost. It is the duty of management to create conditions which will bring about the optimum use of all resources available to the water district. Thus the scope of management carries with it a lot of responsibilities.

The continuous and successful operation of a water district depends largely on the patronage of its consumers. The consumers would only patronize a water district if it can provide them safe water in sufficient quantities at all times at a cost within their reach. Patronage of consumers can be achieved through good management.

11.3.2 Functions and Duties of General Manager/Staff

FIGURE 11.2.2. shows the departmental organization in a medium-sized water district whose sources of supply are deep wells. (If its sources are surface water, the treatment plant staff should be added.)



Many of the functions indicated on the chart are common to the four proposed water districts respectively, although relatively small water districts (e.g., Bayombong-Solano Water District) may find it unnecessary to departmentalize during the early stage.

A carefully planned organization is essential to ensure that the needs of consumers throughout the area can be promptly appreciated and efficiently met. Thus, it is also desirable that every member of the organization not only understand and realize the significance and importance of his or her particular function but also properly discharge that particular function.

(1) The General Manager

Provincial Water Utilities Act of 1973 prescribes the Officers and Employees as follows:

> "SEC. 23. The General Manager. --- At the first meeting of the Board, or as soon as practicable, the Board shall appoint, by a majority vote, a general manager and shall define his duties and fix his compensation. Said officer shall serve at the pleasure of the Board. (As amended by Sec. 9, PD 768)

> SEC. 24. Duties. --- The duties of the General Manager and other officers shall be determined and specified from time to time by the Board. The General Manager, who shall not be a director, shall have full supervision and control of the maintenance and operation of water district facilities, with power and authority to appoint all personnel of the district: Provided, that the appointment of personnel in the supervisory level shall be subject to approval by the Board. (As amended by Sec. 10, PD 768)"

Thus, the duties fall, to a large extent, within the sphere of administration. The General Manager should therefore, have managerial ability, and a thorough knowledge of the administrative machine which he controls. Managerial ability implies the ability to select suitably trained or qualified staff, to delegate work and responsibility wisely, and to create and maintain a spirit of co-operative enthusiasm throughout the

entire organization. The General Manager must be able to co-ordinate the efforts of the several different sections, and should keep in close touch with the more important matters being dealt with by each.

Since water industry is mainly concerned with the problems of the collection of water, its storage, treatment (at least chlorination), and distribution, it is most desirable that the General Manager is a chartered civil/sanitary engineer who also has managerial abilities on general, and especially, financial matters.

Presently, however, the four study areas are not yet managed as a water district, for these systems to start out rightly, it must be able to have a man of managerial competence and experience appointed as a General Manager.

Therefore, the first priority should rather be given to managerial ability rather than to be an engineer. In case any other non-technical person is appointed as the General Manager, the General Manager should be assisted by a chief engineer.

Although the General Manager is responsible for the day-to-day conduct of affairs of the water district, he should not become too immersed in details. He should exercise over-all supervision and control, thereby giving himself ample time to plan the improvement/development of the Water District carefully in anticipation of the consumer's needs, and to deal promptly with unforeseen situations of major importance as they arise.

(2) The Administrative Machine

As indicated in the FIGURE 11.2.2, the proposed organization of the water district can be divided into two main branches:--- (1) the Administrative, and (2) the Technical.

These two branches are in contact at many points, and are so interdependent that a high degree of co-operation is essential to ensure coordination of effort, economy in working, and proper balance between income and expenditure. Each of these two main branches is, in turn, divided into a number of subsections.

As efficient administration cannot be achieved by staff in watertight compartments, officers in charge of departments should not only possess a detailed knowledge of every aspect of the work of their respective departments, but should also take an interest in, and have a general knowledge of, the work of other departments.

In the organization depicted in functional form in FIGURE 11.2.2, it seems not necessary to mention the scope of the individual works one by one basis. Keynotes or recommendations are as follow:---

11.3.3 Problems Arising from the Transition of Administrations

Transition from the existing water supply organizations to the respective water districts will inevitably take time. Each water authority then should make necessary preparations for setting up respective water districts in various aspects --- legislative, budgetary, personnel and technical.

(1) Personnel

- a) Appointment of qualified personnel to the key positions is indispensable for good management of a water district. Generally, it might be somewhat difficult to get well educated or qualified personnel in local cities and municipalities, especially in areas far from Metro Manila. However, there are other aspects that may make up for apparent lack of education or qualification. These are experience, the right attitude and the potential or capacity of a person to learn.
- b) If the employees who belong to the existing water supply authorities will transfer to the proposed water districts there will be no displacement, since the number of employees of the existing water supply authorities are less than the proposed number of personnel required for water districts in the target years 1990 and 2010.

However, there may be problems on absorbing dead wood or unwanted employees. It may be best to terminate such employees services

at the start of the water district operations so as not to complicate matters further.

c) It is recommended that the experience of the personnel who have worked for a long time for the relevant water authority and are well acquainted with that water supply system, managerially or technically, be made good use of.

d) In the procedures for recruitment, if necessary, any newly formed water district may fully utilize LWUA's assistance to obtain desirable personnel, especially for managerial positions. It is expected, therefore, LWUA can use its testing system to recommend the most appropriate candidate as required water district.

Attention should be given to procedures for the recruitment, proper use and retention of technical personnel including the establishment of career structures.

f) Training of personnel on all levels and categories is a vital aspect in relation to work performance, morale and retention of staff. It was disclosed through the study, that most of the staff of all the four water supply systems have not enjoyed the benefits of any kind of training at all. Before and after the formation of these systems into water districts, LWUA should train the personnel from the board of directors and General Manager down to the plumber.

g) An information dissemination and public relations programs should be developed. The water districts serves the people and needs the support of the people, especially on payment of water bills or when the water rates are increased. It should therefore regularly inform and educate its public on the plans and programs of the water district and on the correct usage of water.

(2) Change of Charging System

e)

a) At present, in the waterworks of Angeles City, Dagupan City and Cabuyao-Sta. Rosa-Biñan, the combined use of flat rate system and metered system is adopted, while in Bayombong-Solano only a flat rate system is adopted because all the faucets are unmetered.

In principle, the proposed four water districts should be operated and managed on the basis of 100% metering, in accordance with the Letter of Instruction No. 700, June 1, 1978, to insure correct charging of water actually consumed and discourage its wasteful use. One hundred percent (100%) metering is the basis of the water tariff structure and, in effect, is the basis for financial viability.

- b) To install water meters (including replacement of the nonfunctioning/malfunctioning meters) to all the consumers takes time. Therefore, it is necessary to stipulate a provisional rate regulation of combined use of flat rate and metered system, fixing the target date by which 100% metering should be realized.
- c) Since the life of water meters is about 8 years, the water district should have a plan to replace water meters installed at every customer once in 8 years basis and to guarantee the budget and manpower necessary for its execution.

11.3.4 Problems Arising from Rapid Expansion of the Systems

(1) Coordination During Construction Period

According to the demand projection in the target year for the Short Term Development, the size of the water supply systems will more than double compared to its present size.

Considering the rapid expansion of the water supply systems, there may be much inconvenience caused by the construction work. For instance, to minimize water interruption and traffic congestion caused by pipe laying work, it is necessary to develop close coordination and cooperation between the respective water district and LWUA (Engineering Services) together with the local transportation authorities and inform the people to be affected by the expansion project. (2) Office Accommodation for Increasing Personnel

Increase of personnel requires a larger office space. In this regard, the newly established water districts may talk with the related cities/municipalities respectively of utilization of the former office space of the respective water supply authorities until the water district could afford to move to a new office.

11.4 OTHER RECOMMENDATIONS

11.4.1 LWUA Assistance

Immediately upon their formation into water districts, it is recommended that the LWUA install an appropriate commercial practice system (CPS) to back up the said water districts. Also, an effective acquisition and sustained training programs for both water district officials and personnel should be formulated and implemented as soon as possible. A rational public information and education program should also be undertaken by the new water districts.

The JICA Team also reminds that the LWUA maximizes its assistance to the proposed water district upon its formation. It is expected that partially the proposed water district will need LWUA's assistance in all aspects of its operation and financial, technical and institutional. It is believed that this package of assistance will greatly enhance the water system's capability to provide an adequate and efficient water service to its consuming public.

11.4.2 Formation of Water Districts

Basically, LWUA provides every necessary assistance once a waterworks system is formed into a water district. Yet, under the decree that created both LWUA and the water district (PD 198, as amended), the formation of a water district is at the option of the local governments concerned. There is actually a very positive trend towards the formation of water districts in the above-mentioned four study areas. Members of the Sangguniang Panlalawigan/Panlungsod have been taking an increasing interest in the importance of an improved water supply system.

The JICA Team believes LWUA can facilitate things by having a more aggressive program or campaign in water district formation itself; but should include effective information measures to promote formation through grass-roots and media based campaigns.

APPENDICES

APPENDIX 3.2.1 LIST OF SUBDIVISIONS (Narch 1983)

AFFENDIN J. S. I			•
NAME OF SUBDIVISION	OWNER/OPERATOR	: LOCATION	: AREA (In Sq. M.
		.	άð
1. Abacan Subdivision	: Nicolas Tinio	: Balibago	: 51,180.00
2. Abad Santos Subdivision	*.	••	
3. Angelina Subdivision	Rafael Lazatin	: San Jose	: 55,553.00
4. Lagong Rayan Subdivision	: Don Fepe ^L enson	: Cutcut	: ¹ +60,017.00
5. Lecong Silang Subdivision	: Don Fepe Henson	: Cutcut	: 19,882.00
6. Ealibago Subdivision	: Isabelo Concepcion	: Balibago	: 190,960,00
7. Beatriz Pangilinan	: Beatriz Fangilinan	: Balibago	: 7,433.00
8. Belen Fomesite Subdivision	: Atty. R. Morales	: Sto. Cristo	: 237,609.00
9. Carmenville Subdivision	: Renato Tayag	: Cutcut	: 126,605.00
10. Checkpoint Subdivision	: Anacleto Muñoz	: Balibago	: 138,444.00
11. Clarkriew Subdivision	: Felipe Juico	: Balibago	: 572,300,00
12. Clemente Dayrit	: Clemente Dayrit, Jr.	: Lourdes Sur East	: 473,905.70
13. Don Fepe Subdivision	: Don Pepe Henson	: Balibago	: 50,000.00
14. Don Bonifacio Subdivision	: Timoteo Cruz	: Fulung Maragul	: 720,000,00
15. Doña Afripina Subdivision	: Ernesto Lopez, Jr.	: Pandan	: 154,320.00
16. El Cano Subdivision	: Dante Timbol	: Pulung Bulu	: 193,800.00
17. El Cano Subdivision	: Dante Timbol	: Pulung Bulu	: 227,000.00
18. Essel Fark	: Jesus Lazatin	: Sto. Domingo	: 49,840.00
19. Felisa Subdivision	: Jose Galura	: Balibago	: 94,448.00
20. Lenifel Subdivision	: Enrique Baluyut	: Balibago	: 27,177.00
21. Henson Low Cost Housing	: Don Pepe Henson	: Balibago	: 76,815.00
22. Hensonville Subdivision	: Don Pepe Henson	: Malabañas	: 529,689.00
23. Holy Cross Subdivision	: Carmela Narciso	: Sapangbato	: 184,758.00
2		••	••

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(List of Subdivisions cont'd.) NAME OF SUBDIVISION	: OWNER/OPERATOR	: LOCATION	: AREA (In Sq. M.)
24. Josefa Suhdimision	: Tose Narciso	: · Balibaro	118_351_00
25. Josefaville - 1	: Jose Narciso	: Malabañas-Amsik	: 50,144.00
26. Josefaville – II	: Jose Narciso	: Malabañas	: 65,267.00
27. Kalayaan Subdivision	: Don Pcpe Henson	: Lourdes Northwest	: 14,321.00
28. Mountain View Subdivision	: Oscar Santos	: Balibago	: 195,452.00
29. Leoncia Subdivision	: Rafael Lazatin	: Sto. Domingo	: 231,053.00
30. L & Subdivision		: Sto. Domingo	¢ş
31. Marisol Subdivision	: Eusebio Lopez, Jr.	: Pandan	: 634,206.00
32. Nepomuceno I	: Francisco G. Nepomuceno	<pre>cutcut</pre>	: 116,142.50
33. hepomuceno II	: Francisco G. Neponuceno	: Cutcut	: 323,462.00
34. Meponuceno III	: Francisco G. Npeomuceno	: Cutcut	: 870,480.00
35. Nepomuceno IV	: Francisco G. Nepomuceno	: Cutcut	: 365,436.00
36. New Valley	: Bonifacio Eusebio	: Balibago	: 88,836.00
37. Cphebia	: Abelardo Tinio	: Balibago	: 17.520.00
38. Flaricel I	: Anacleto Muñoz	: Malabañas	: 479,929.00
39. Plaricel II	: Anacleto Muñoz	: Anunas-Amsik	: 352,444.00
40. Friscilla Subdivision	: Priscilla Santos	: Balibago	: 5,529.00
41. Raymond Subdivision	: Angel Reyes	: Balibago	: 22,480.00
42. Roque Henson	: Roque Henson	: Balibago	: 27,177.00
43. Rovimer Subdivision	: Vicente Henson	: Balibago	: 27,185.00
44. Riverside Subdivision	: Trinidad Lazatin	: Anunas	: 337,871.00
45. Sor Maria I.,isa	: Renato Tayag	: Balibago	: 27,177.00
46. Sta. Maria I	: Priscilla J. Tinio	: Balibago	: 36,612.00
	••		••
		ومقافلا والمراجعة والمراجع	

(List of Subdivision cont'd.) NAME OF SUBDIVISION	: OWNER/OPERATOR	: LOCATOR	: AREA (In Sq. M)
47. Sta. Maria II	: : Priscilla J. Tinio	: Balibago	: 187,360.00
46. San Angelo Eubdivision	: Anacleto Muñoz	: Sto. Domingo	: 494,300.00
49. Springside Subdivision	: Carlos Sandico	: Pandan	: 34,772.00
50. Sabina Tablante	: Sabina Gomez	: Balibago	: 37,493.00
51. san Antonio	: Renato Tayag	: Pulung Maragul	: 201,658.00
52. Sandico I	: Carlos Sandico	: Pulung Maragul	: 119,259.00
53. Sandico II	: Carlos Sandico	: Pandan	: 38,614.00
54. Sen Ignacio	: Jose P. Dizon	: Pandan	: 146,570.00
55. San Jcse I	: Jose Reynoso	: Fulung Bulu	: 62,327.00
56. Lan Jose II	: Jose Reynoso	: Pulung Bulu	50,000.00
57. San Jose III	: Jose Reynoso	: Pulung Bulu	: 108,546.00
58. Severing Diamond	: Severina Lim	: Balibago	: 578,853.00
59. Timog-Silangan Park	: Abelardo Tinio	: Cutcut	:1,090,830.00
60. T. Silangan (Resettlement)	: Abelardo Tinio	: Cutcut	: 625,572.00
61. I.a Buena-Tanhueco	: Ben Tanhueco	: Balibago	: 9,920.00
	: Vicente Henson	: Balibago	: 27,177.00
63. Villa Amanda	: <i>f</i> manda Henson	: Balibago	: 27,176.00
64. Villa Angela	: Jesus Lazatin	: Sto. Domingo	: 447,357.00
65. Villa Dolores	: Pablo Panlilio	: Sto. Domingo	: 220,382.00
66. Villa Esperanza	: Purification Flores	: Malabañas	: 31,881.00
67. Villa Gloria	: Abelardo Tínio	: San Jose	: 153,025.00
68. Villa Henson	: Don Pepe Henson	: Lourdes Northwest	: 11,859.00
69. Villa Sol	: Sabiano Sagulo	: Malabañas	: 528,000.00
	•••	••	•• .

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(13 st of Sabdivision cont'd.)			
NAME OF SUBDIVISION	: OWNER/OPERATOR	: LOCATION	: AREA (In Sq. M)
70. Villa Teresa	: Peter Nepomuceno	: Cutcut	: 650,000,00
71. Abad Eantos Compound		: Fulung Maragul	••
72. Embassy Court		: Balibago	
73. Clarkville		: Balibago	
74. East West Subdivision	•••	: Lourdes Sur East	••
75. Villa Angelina	: Rafael Lazatin	: San Jose	.
76. Pacimar Estate	: Vladimir Panlilio	: Pulung Bulu	: 26,673.00
77. Pineda Compound		••	
78. Essel Subdivision	•	••	· · · · · · · · · · · · · · · · · · ·
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APPENDIX 3.4.1 Power Rate of Angeles Electric Corporation

Residential

First	15	kwh	₽	0.40	per	kwh
Next	35	kwh		0,38	per	kwh
Next	51	kwh		0.35	per	kwh
Next	100	kwh	. ·	0.33	per	kwh
Excess		kwh		0.32	per	kwh

Minimum Charge : P4.80 for the first 12 kwh

Small General Service

Classifi-	Conn. Load			
cation	in Watts	₽0.50/kwh	₽0.40/kwh	P0.35/kwh
· · · · · ·		·		
GS-1	2,500 or less	First 50 kwh	Next 150 kwh	Next 300 kwh
GS-2	2,501 to 5,000	90	260	550
GS-3	5,001 to 10,000	160	440	1,200
GS-4	10,001 up	350	900	3,250

Monthly Excess : ₽0.33 per kwh

Minimum Charge : ₽7.20 for the first 12 kwh

Large General Service (GS-5)

Demand Charge			₽10.00 per kw
Plus Energy Charge	First	100 hrs.	₽ 0.35 per kwh
	Next	100 hrs.	0.32 per kwh
	Next	100 hrs.	0.30 per kwh
	Over	300 hrs.	0.28 per kwh

Minimum Charge : ₽300.00

APPENDIX 4.1.1.A

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Water Supply of the Subdivisions Visited by JICA Study Team

	· · · ·						
			Name	e of Subdi	ivision		
ΙΤΕ	М	Carmen- ville	Essel	Sunset	Timog Park	Villa Angela	Villa Teresa
Background Information	Year Established Number of Households	1968 550	1969 222	1985 70	1981 281		1969 500
	Land area (ha.)	47		-	32		63
	Ownership of the system	Ass'n.	Ass'n	Ass'n	Ass'n	Ass'n	Ass'n
Water Supply Status	Commencement of Op'n	1968	1976	1965	1982	-	1968
	Water source	10 well	1 well	2 well	6 well	4 well	2 well
	Dist. Tank capacity (GAL)	50,000	11,000	100000	100,000	ent a	100,000
	Number of Connections	550	222	70	174	300	350
	Served percentage	100	100	100	62	100	70

Note:

- No data provided

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APPENDIX 4.1.1.B

Water Charges for the Subdivisions Visited by JICA Study Team

Villa Teresa WW:

Water Consumption	Residential	Commercial
0 - 10 cu.m	₽14.50 cu.m/minimum	₽17.00 cu.m/minimum
11 - 20	1,50	1.80
21 - 30	1.55	1.90
31 - 40	1.60	2.00
41 - 50	1.65	2.10
51 - 60	1.70	2,20
61 - 70	1,75	2.30
71 - 100	1.90	2.50
over 100	2.00	2.70

Villa Teresa WW:

Water Consumption	Residential
0 - 10 cu.m	₽22.00 cu.m/minimum
11 - 20	2.25
21 - 30	2.30
31 - 40	2.35
41 - 50	2.40
51 - 60	2.45
61 - 70	2.50
71 - 100	2.60
over 100	2.75

Essel WW:

First 10 cu.m (min) is P28.00 plus P2.00 per cu.m in excess of 10 m³ Carmenville, Sunset and Timog Park WW - charge is flat rate at P160.00 per month.

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APPENDIX 4.1.2

Private and point source POPULATION AND NUMBER OF HOUSEHOLDS SERVED BY TYPE OF WATER SOURCE (1980)

÷													•				1	•							•						
	Toto.	TOLAL		3, 740	651	.I.1.1.	195.	6,149	I,079	10,157	1,782	2,536	445	5,643	066	12,106	2,124	6,772	1,188	1,748	307	3,897	704	6,651	1,167	\mathbf{m}	240	2,429	426	8,578	- * *
1 point	FOILS SOUTCE WITH	pitcherpump	(}	2,456	431	1,111	195	3,841	674	6,042	1,060	2,536	445	4,389	770	10,505	1,843	6,772	1,188	1,287	226	2,746	482	4,816	845	3	1	1,916	336		Ś
	FILVALE PIPES	-	-	I,254	220			2,308	405	4,115	722			1,254	220	1,601	281	•		461	81	1,151	202	1,835	322			513	06	4,229	742
		Total	(798	140			3,328	584	•	3, 763			2,052	360	1,898		2,929	479	2,012	353	1,043	183	7,039	1,235			450	- 62	3,939	691
-	waterworks	Others		616	108			3, 328	584	21,449	3,763			519	91	1,704	299							•	r-i		-		:		
		City		182	32		· · · · · · · · · · · · · · · · · · ·	· · · ·						1,533	269	194	34	2,929	479	2,012	353	1,043	183	382	67			450:	29	3,939	691
	NO. OI HH	Population	C L	Ć	162	1,111	195	9,477	1,663	31,606	5,545	2,536	445	7,695	1,350	14,004	2,457	9,501	1,667	3,760	660	4,940	867	13,690	2,402	1,368	240	2,879	505	12,517	2,196
	Barangay			I A. del Kosario		2 Amsík		3 Anunas		4 Balibago	······································	5 Capaya		6 Claro M. Recto		7 Cutcut		8 Lourdes North-	west	9 Lourdes Sur		10 Lourdes Sur	East	11 Malabañas		12 Margot		13 Pampang		14 Pandan	
	Area										· · · · ·					Urban															

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APPENDIX 4.1.2 (cont'd)

								Private and point source	rce
Area		Barangay	No. of NH		Waterworks	S	Private pipes	Point source with	1 1 1 1
			Population	City	Others	Total	with pump	pitcherpump	70107
	29	29 Cuavan	433					433	433
	} 		10		:			76	. 92
	30	30 Cutud	672					672	672
			118					118	118
Rural	31	Mining	570					570	570
	_		100					100	100
	32	32 Tabun	684					684	684
	1		120		•		-	120	120
		Sub-Total	2,359 414					2,359 414	2,359
		TOTAL	188,912 33,148	23,681 4,120	42,178	65,859 11.520	27,872 4,891	95,181 16,737	123,053 21,628

Note: Above: Population Below: No. of HN Source: Planning and Development Sec. of Angeles City City Engineer's Office

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APPENDIX 4.1.3

LEVEL I WATER SUPPLY SYSTEMS (AS OF DEC. 1984)

	Barangay	No. of Wells	Number of HH	Estimated Pop. Served	Popu- lation (1986)	Served Percentage	Remarks
1.	A. del Rosario	3	31	186	5,069	3.7	
2.	Anunas	1	12	72	575	12.5	
3	Capaya	3	32	192	3,763	5.1	
4.	Cutcut	11	110	660	16,227	4.1	
5.	Cutud	1	10	60	943	6.4	
	Lourdes N.W.	9	90	540	11,201	4.8	
7.	Lourdes S.E.	4	40	240	7,504	3.2	
8.	Pampang	1	10	60	2,347	2.6	
9.	Pulungbulu	7	63	378	7,837	4.8	
10.	Pulung Cacutud	3	30	180	1,151	15.6	
11.	Pulung Maragul	2	20	120	4,760	2.5	
12.	Salapungan	. 2	22	132	7,615	1.7	
13.	San Jose	7	64	384	7,394	5.2	
14.	San Nicolas	1	10	60	4,184	1.4	
15.	Sapalibutad	2	18	108	2,214	4.9	
16.	Sta. Teresita	3	30	1 80	11,866	15	
17.	Sto. Cristo	2	20	120	2,811	4.3	: : :
18.	Sto. Domingo	1	9	54	14,566	0.4	
19.	Tabun	3	30	180	747	24.1	
20.	Virgen delos Remedios	Ŀ	10	60	1,940	3.1	
	Total	67	661	3,966	114,714	3.5%	

Note: Estimated population: 6 persons/HH

APPENDIX 4.1.4 LEVEL II WATER SUPPLY SYSTEMS (AS OF 1985)

Barangay	No. of HH Served	Estimated Population Served	Population (1986)	Served Percentage	Remarks
1. Anunas	60	360	575	62.6	Ave, persons
2. Cuayan	.60	360	342	100	per HH is 6
3. Cutud	40	240	943	25.5	
4. Capaya	120	720	3,763	19.1	
5. Pulung Cacu- tud	125	750	1,151	65.2	
6. Pandan	198	1,188	15,075	7.9	
Total	603	3,618	21,849	16.6%	

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LEVEL I WATER SUPPLY SYSTEM

APPENDIX 4.2.1

As of Dec. 31, 1985

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			- E					· · · · · · · · · · · · · · · · · · ·
Location	Well No.	No. OF Household Per System	Dia.	Depth (feet)	Constructed By	Date Constructed	Condition of System	Remarks
Bgy. Cutud		10	2"	140	MPW	1982	Satisfactory	
Bgy. Pampang	5	10	1-1/2"	80	MJM	1982	Satisfactory	
z	en	10	1-1/2"	1001	MPW	1982	Satisfactory	
Bgy. San Jose	4	10.	1-1/2"	1401	MPW	1982	Satisfactory	:
Bgy. San Nicolas	Ś	10	1-1/2"	1001	MPW	1982	Satisfactory	
Bgy. Cutcut	9	10	1-1/2"	1201	MDM	1982	Satisfactory	
	~	10	1-1/2"	1201	MPW	1982	Satisfactory.	
Bgy. San Jose	æ	10	1-1/2	1201	MPW	1982	Satisfactory	
Bgy. Pulung				• •				
Maragal	б	10	$1-1/2^{n}$	120'	MPW	1982	Satisfactory	
Bgy. Pulung							. :	
Caeutud	10	10	24	1401	MPW	1982	Satisfactory	
Bgy. Pulung								-
Cacutud	11	10	I-1/2"	120'	MdM	1982	Satisfactory	
Bgy. Cutcut	12	10	1-1/2"	1201	MPW	1982	Satisfactory	
Bgy. Tabun	13 5	10	1-1/2''	120	MPW	1982	Satisfactory	
Bgy. Sapalibutad	14	10	$1-1/2^{n}$	120;	MPW	1982	Satisfactory	
Bgy. Pulung								
Maragul	5	10	1-1/2"	120	MPW	1982	Satisfactory	
Bgy. Tabun	16	10	<u>م</u>	1101	MPW	1982	Satisfactory	
Bgy. Pulung		-						
Cacutud	117	10	ι. Γ	140	MPW	1982	Unsatisfactory	Musky taste
Bgy. Capaya I	18	10	τ. Γ	1201	MPW	1982	Unsatisfactory	High iron
						-		concentration
Bgy. Cutcut	61	10	ະ ້ຳ	130'	MPW	1982	Unsatisfactory	High iron
						 -		concentration

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As of Dec. 31, 1985

APPENDIX 4.2.1 (Cont'd)

	Remarks							~~~~											-												
Condition	of System	Satisfactory	Satisfactory		Satisfactory	Satisfactory	Satisfactory	Satisfactory		Satisfactory	······································	Satisfactory		Satisfactory		Satisfactory		Satisfactory		Satisfactory	Satisfactory	Satisfactory		Satisfactory		Satisfactory	Satisfactory	Satisfactory	Satisfactory		Satisfactory
Date	Constructed	1982	1982		1982	1982	1982	1982		1982		1982		1983		1983	· · ·	1983	•	1983	1983	1983		1983	-	1983	1983	1983	1983		1983
Constructed	By	RUDC	RWDC	•	RWDC	RWDC	RVDC	RWDC		RWDC		RWDC		RWDC		RUDC	•	RWDC	:	RINDC	RWDC	RVIDC		RWDC	•	RUDC	RWDC	RVIDC	RWDC		RWDC
Casing	Depth (feet)	1001	1001		601	1001	100	1001		1001	-	801		1 09		· 60		601		601	60	60	•	60'		801	404	80'	40	•	60 1
.1	Dia. (inch)	1-1/2"	1-1/2"		1-1/2"	1-1/2"	1-1/2"	1-1/2"	- :	1 - 1/2		I-1/2"		1-1/2"		I-1/2"	· · ·	1-1/2"		1-1/2''	1-1/2"	1-1/2"		1-1/2"		1-1/2"	1-1/2"	1-1/2"	1-1/2"	•	1-1/2"
NO. OF	Household Per System	10	10	• .	10	10	10	10		10		10		10	•	10		10		10	10	10		10		10	10	10	10		10
 [[a]]	No.	, ⁷ 44	45		46	47	48	65		50		51	:	52		53	· · ·	54		55	56	57		60		60	19	62	63		64
	Location	Sítio Malícava	Sitio Maligaya	Bgy. Lourdes	North West	Bgy. Cutcut	Sitio Maligaya	Sitio Maligaya	Bgy. Lourdes	North West	Bgy. Lourdes	North West	Bgy. Lourdes	Sur East	Bgy. Lourdes	Sur East	Bgy. Lourdes	Sur East	Bgy. Agapito del	ρ_{1}	Bgy. Pulungbulu	Bgy. Cutcut	Bgy Virgen de	los Remedios	Bgy. Lourdes	North West	Bgy Sto. Cristo	Bgy. Cutcut	Bgy. Sto. Cristo	Bgy. Lourdes	Sur East

LEVEL II WATER SUPPLY SYSTEM 4.2.2

APPENDIX

Remarks As of Dec. 31, 1985 structed Date Con-1981 1981 1984 1981 1984 BWP/MLG 1985 Conduc-ted by MPWH IIMAIN MPUMI MPWH HENTH Size Length Type (mm) (m) PVC PVC 1,440 PVC ы Д ы Ы មា ក 872 498 541 P.I. D. C. 207 Ļ 63-75 Steel 325-63 50 20 38-100 1 Steel 3 V=6.4m Steel Steel Stee1 Pumped | Centrifugal | Steel Tank Centrifugal Centrifugal (Centrifugal Centrifugal Centrifugal Pump Pumped [Pumped Pumped Pumped Pumped System Deepwell | Deepwell | Deepwell | Deepwell Deepwell Deepwell Source No. of Connection ,--1 ,-1 I 21 3 ഗ m No. of Household Served 125 198 120 60 60 40 tenure). Bgy. Pulung Cacutud Bgy. Pandan (land Bgy. Cauayan Bgy. Capaya Bgy. Anuas Bgy. Cutud Location

Source: City Engineer's Office

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APPENDIX 4.2.3 Pump Efficiency Test at No. 1 Pumping Station

The pump discharge rate, water pressure, and voltage and electric current were measured using different opening ratios of the gate valve. The following is a rough percentage of the opening ratio of the gate valve for the four examination steps. Approximately 50 to 60% of the ratio seemed to be allowable during the examination as a response of the pump and motor to the valve operation.

Valve Operation:

Case	Handle Ope.	Estimated Opening ratio
1	17.5	50 ~ 60%
2	11.5	40 - 50
3	6.5	30
4	4.5	20

Note: Number of turning for opening the valve completely; 23.5

The following Table shows the results of measurement.

	Discharge Rate (1/s)	TDH (m)		Va (Volts)	Op (KW)	IPM (KW)	λο (%)	OPM p (KW) (%)
1	43.3	27.9	93.3	240.0	11.8	33.0	35.8	28.1 42.0
2	43.2	29.9	92.3	246.6	12.7	33.5	37.9	28.5 44 6
3	43.1	32.9	93.0	250.0	13.9	34.2	40.6	29.1 47.8
4	42.2	34.9	94.7	250.0	14.4	34.9	41.3	29.7 48.4

TABLE 4.2.3.1 DATA ON PUMP TEST

Note: TDH = (measured water pressure) + (distance between pump operation water level and level of pressure gauge: 18.3)

Abbreviations and adopted formulas in the Table are given below.

- Q : Pump Discharge Rate (1/s)
- TDH : Total Dinamic Head (m)
- I : Current (Amp)
- V : Voltage (Volt)
- O : Pump Output (kw)
 - $^{O}p = \frac{Q \times TDH}{102}$

- 14 -

IPM	:	Input Power to Motor (Kw)
· · · · · ·		IPM <u>IaxVaxPFx 3</u> 1000
PF	:	Power Factor (0.85)
λο	•	Overall Efficiency of Pump and Motor (%)
· .		$\lambda o = \frac{OP}{IPM} \times 100$
16.4	i i ser e i	
OPM	:	Motor Output (Kw)
1		$OPM = IPM \times m$
λm	:	Motor Efficiency (0.85)
λp	:	Pump Efficiency (%)

 $\lambda p = OP \times 100$

FIGURE 4.2.3.1 shows the pump performance curb.

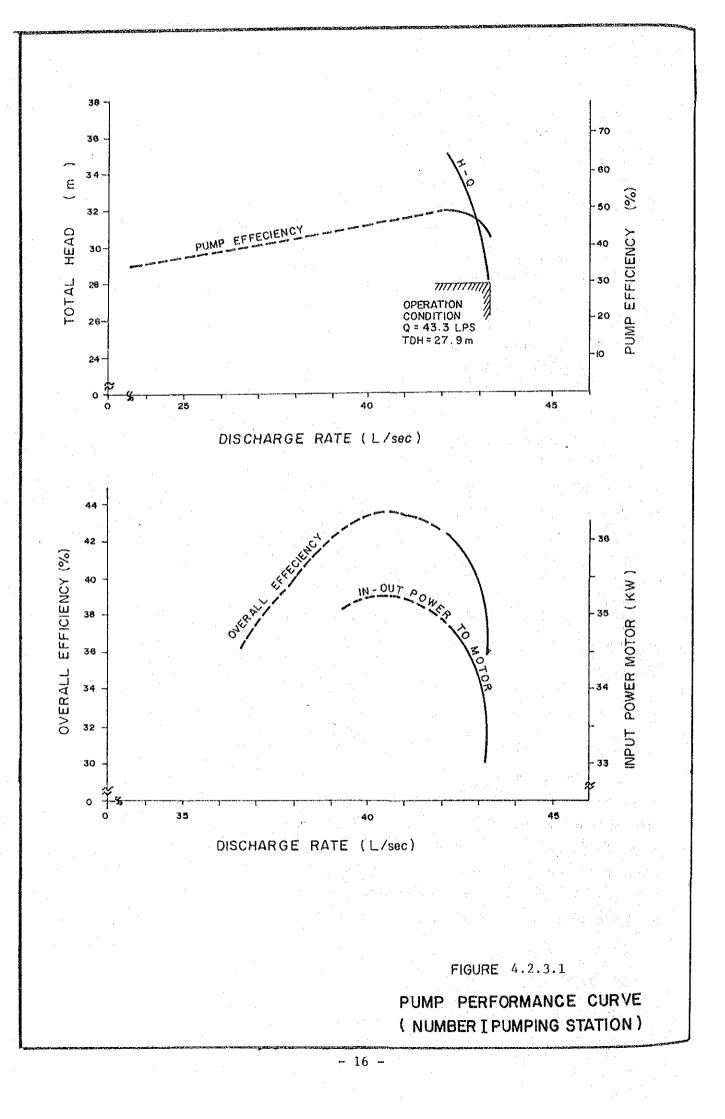
There is no data available on the conditions in selecting an appropriate pump and pump performance curb for the test pumping station. As such, a comparative study of the pump efficiency between those initially planned and operated at present cannot be made except from a general view point.

The coefficient assumed in estimating the efficiency of the pump, the pump and motor, are general figures based on field experience as follows:

> PF = 0.85 $\lambda m = 0.85 - 0.90$ under the conditions of 60 Hz, 2-6p and 30-37 Kw

The distance between water level during pump operation and the elevation of the water pressure gauge in order to estimate dynamic head is also assumed to be 18.3 m using the data on the pumping test conducted about 16 years ago. There is a possibility that the water level during pump operation at present might be about three meters below the assumed water level. This calculation is based on the information on the declining of water level at No. 9 pumping station (0.2m/year).

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The result of measurements revealed that the pump discharge rate is around 42 to 43 1/s with a dynamic water head between 28 to 35 m. The input Power to motor and motor output, gradually increased in accordance with reduction of valve open ratio. The figures of OPM varied from 28 to 30 KW.

The motor output (60 Hz) for the vertical type multi-stage turbine pump is as follows:

OPM = 30 KW Pump discharge rate : 42 1/s Dynamic water head : 35 m

The above figures are almost the same as those measured at the pumping station. Because of the lowering of the water table, the present pump operation conditions may be concluded as:

a)	Pump discharge rate	:	42 - 43 1/s
b)	Total dinamic water head	:	31 - 38 M
c)	Input power to motor	:	33 - 35 KW
d)	Motor output	:	28 - 30 KW

APPENDIX 4.2.4 Water pressure in the Service Area

· . .

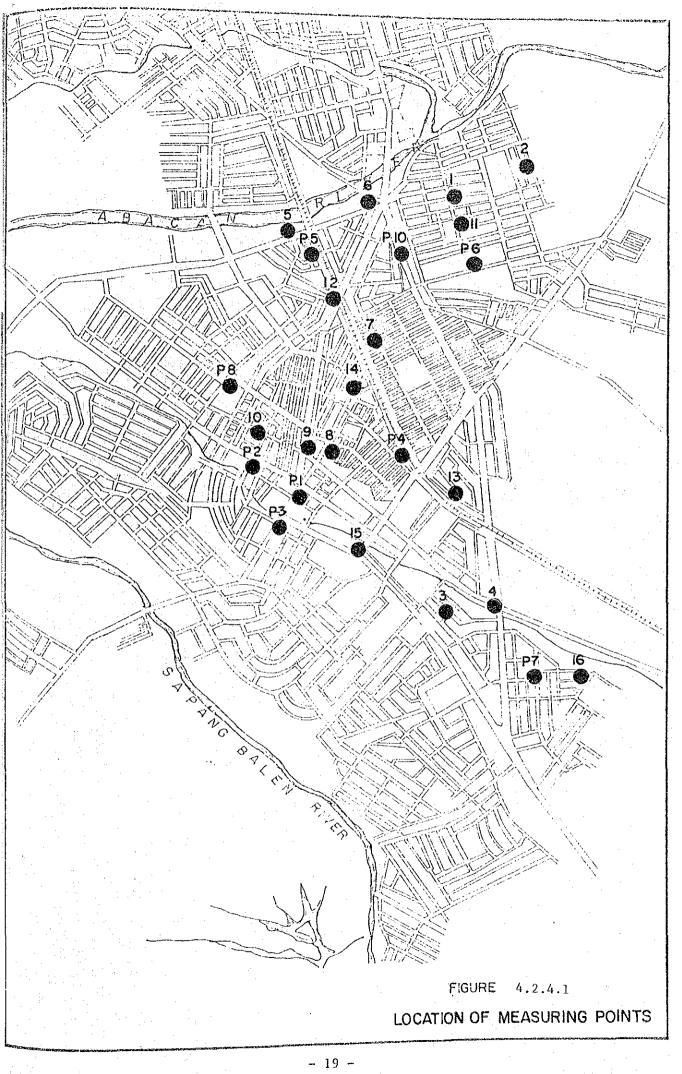
Twenty-five points were pinpointed to measure water pressure as shown in TABLE 4.2.4.1 and FIGURE 4.2.4.1. Sixteen points were service connections, while nine were pumping stations. A topographic survey was likewise conducted at 60 points.

ITEM	No.	Location/address of the Points	Consumer type of	Dia. Connect.
Pump			- · · ·	1/2"
Station	- 1	A. Mabini St.	Domestic	1/2
	. 2	San Nicolas St.		11
1997 - A. A.	3	Rizal Extension	17	11
	4 .	Kuliat St	and the second	11
· · · ·	5	Sta. Teresita	11	
	. 6	Bo, Pandan Marison	11	11 12 12 12 12 12 12 12 12 12 12 12 12 1
	÷ 7	San Angelo	11	
	8	Pampang Rd	TP	
· · ·	. 9	Division Rd. Mc Arthur Highway	1 6	11
	-			
	<u> </u>			
Service				
Area	1	224, Astoria cor. Vgutls	Domestic	1/2"
	2	308, 8th St. Marison	n n n an An	Ħ
	3	San Jose St.	15	ŧt
	4	1524, Jeus St.	11	
·	5	427, Aran Malavak	10	11
	6	23, Magkalinis St.	11	Ħ
	7	628, M.L. Quezon St.	tt	u
	-8	593, Rizal St.	Commercial	11
	9	819, Henson St.	Domestic	\$T.
	10	3 Ar 17 Rizal St.	11	11
	11	235, Harvard cor. Astoria		11
	-	1042, Henson St.	Commercial	11
	1.7			
	12		Domestic	11
	13	J. Surla St.	Domestic	99 99
	13 14	J. Surla St. 1948, Jesus Ext.		
	13	J. Surla St.		

TABLE 4.2.4.1 MEASURING POINTS

The results of the measurements are given in TABLE 4.2.4.3. The contour line of the total water head and water pressure at 12:00 and 24:00 are depicted in FIGURE 4.2.4.2 A & B and 4.2.4.3 A & B. These may represent distribution of water pressure for day time and night time, respectively. The ground elevation at each point is given in TABLE 4.2.4.2, which is used in estimating the total water head.

- 18 -

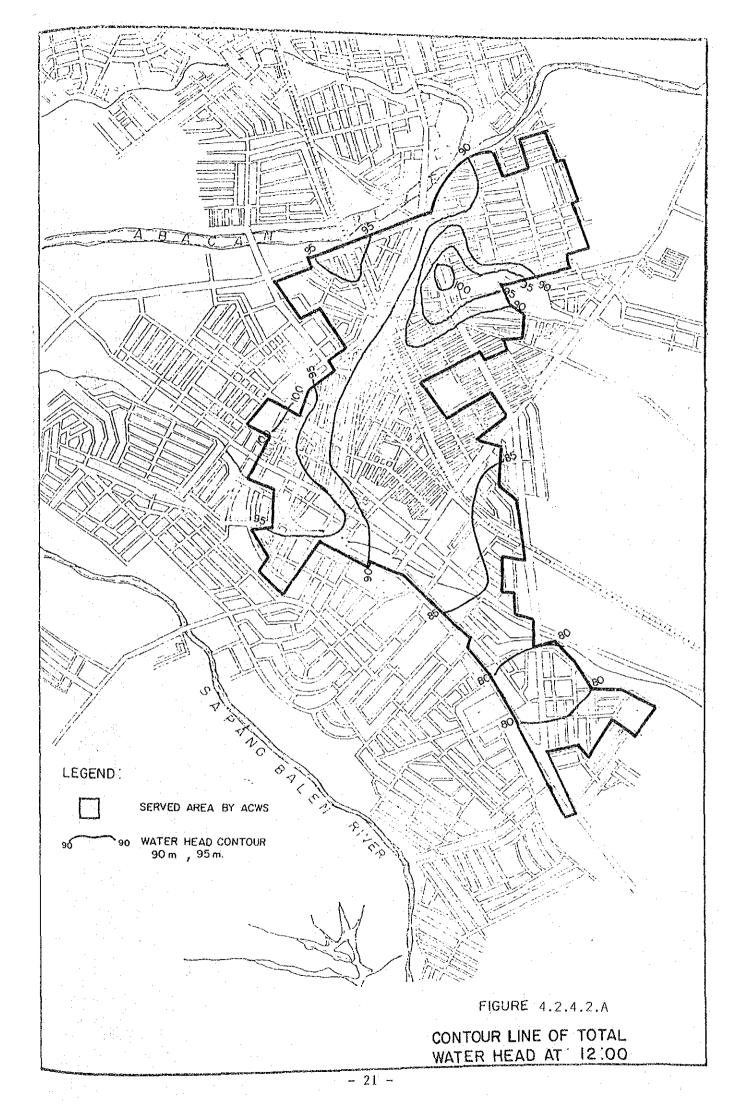


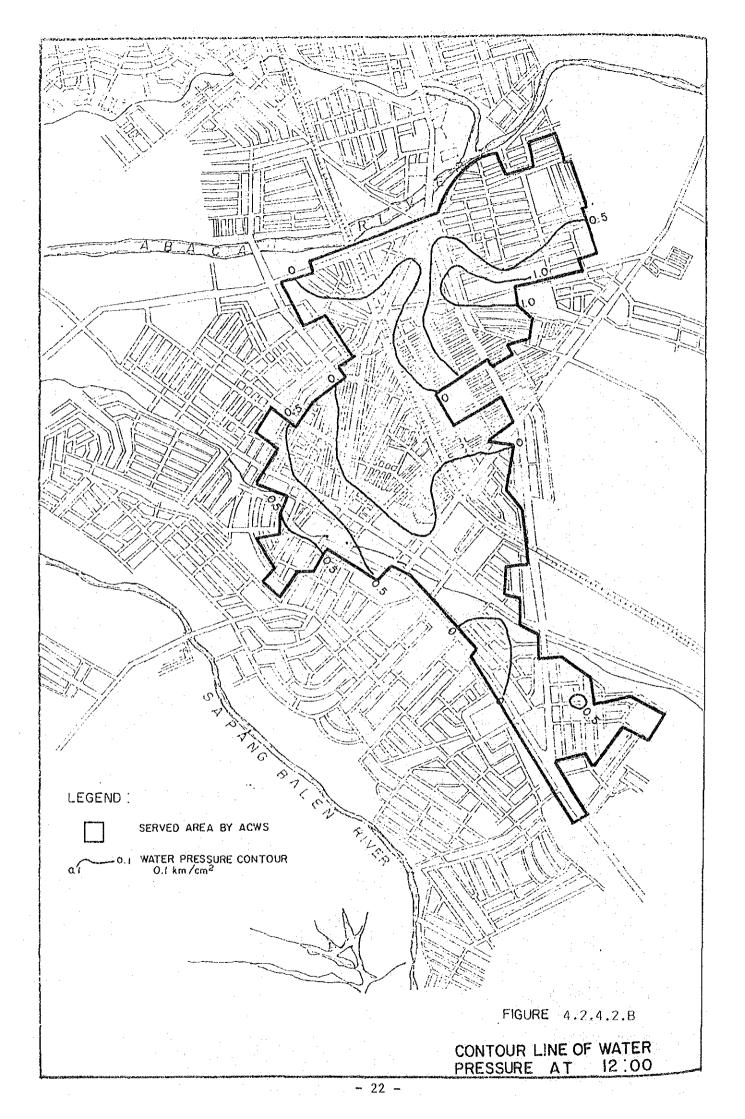
Ī	24	0H000000000000000000000000000000000000	0.8
ĺ	23	0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	0.8 0.75 1.25 1.7
	22	00000000000000000000000000000000000000	0.8 0.75 0.75 1.0 1.0
kg/cm ²	21	00.2 00.1 00.1 00.1 00.1 00.1 00.1 00.1	0.7 0.25 0.25 0.25 0.25 0.25 1.00 1.00 1.00 1.00
: Kg	20	00000000000000000000000000000000000000	0.7 0.35 0.3 1.0 0.8 1.5 1.5
Unit	19	чч заната 200000000000000000000000000000000000	0.7 0.75 0.35 0.3 1.0 0.75 1.4
	18	00000000000000000000000000000000000000	0.7 0.7 0.2 1.0 1.0 1.4
	17	00.1 00.1 00.1 00.1 00.1 0 0 0 0 0 0 0 0	0.7 0.7 0.75 0.75 1.3 1.3
	16	00.1	1.4.75
- - - -	15	000.5 000.5 000.1 000.5 000.1 000.5 000.1 000.5 000.1 0 000.5 000.5 000.5 000.5 00000000	0.00000.7
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	ω	<u> VAAA AA AAAAA</u>	сп. С. 27002ннн7 80
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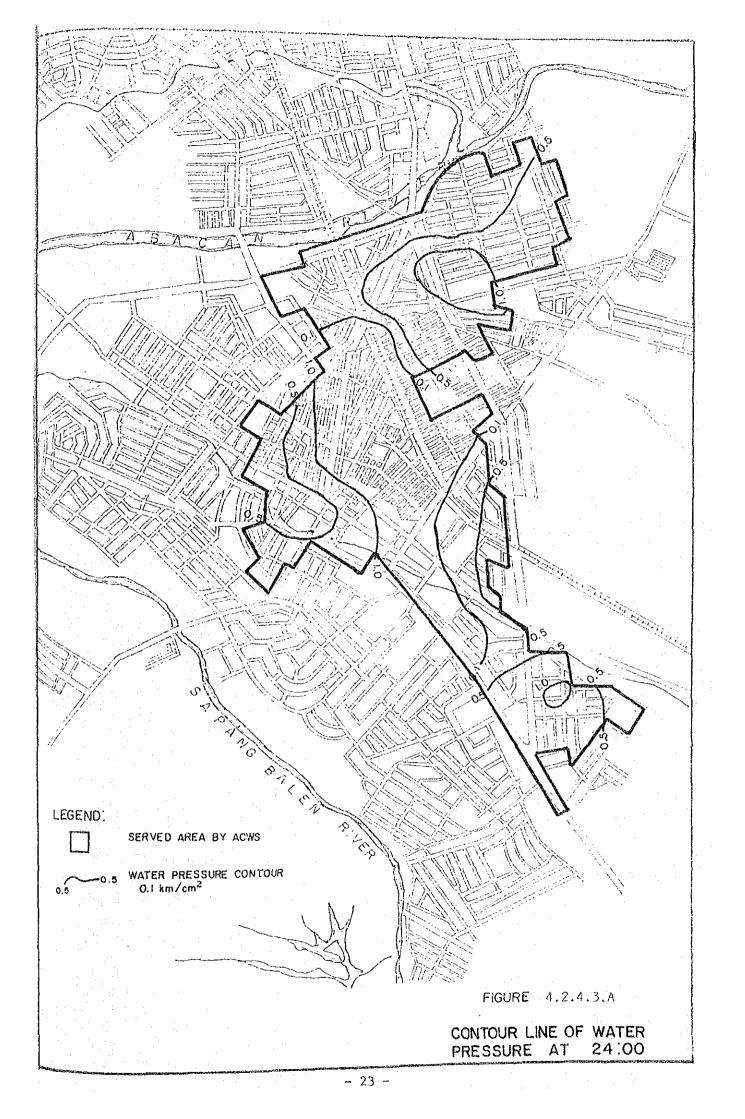
Operation of No. 2 P.S; 6:00 AM - 10: PM

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4.2.4.3 RESULT OF WATER PRESSURE TEST TABLE







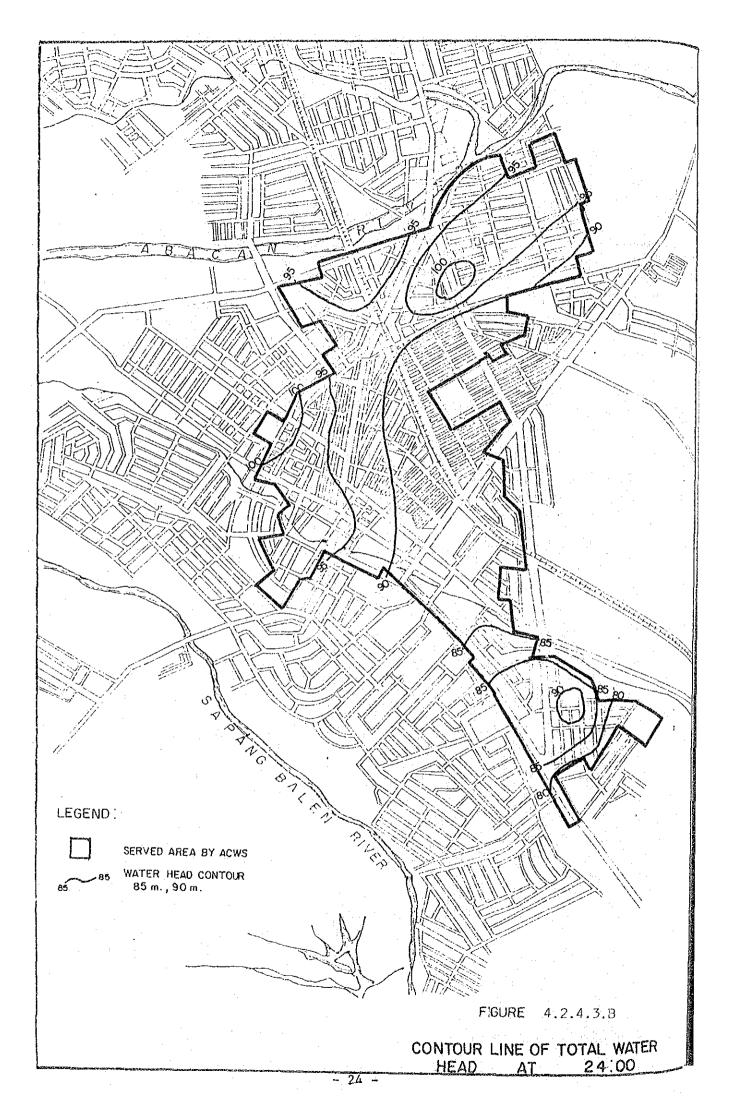


TABLE 4.2.4.2

GROUND ELEVATION AT SELECTED MEASURING POINTS

Point	1	2	3	4	5	6	7	. 8	9	10	11	12	13	14	15	16
Service Area G.L. (m)	86	84	82	81	95			<u> </u>	90					90	89	72
Point	1	2	3	4	5	6	7	8	9	10				<u> </u>	<u> </u>	
Pumping Station G.L. (m)	91	93	91	86	94	84	79	97		88						

Source: Topographic survey (JICA) and data from City Engineers Office

The service area has a gentle slope of one to two percent form northeast to southeast. The maximum difference of ground level in the area is approximately 20 meters.

The location of the existing 11 pumping stations in ground level terms is delineated below.

a) Northern portion of the area with a higher elevation; 8 P.S.

b) Western portion of the area with a higher elevation ; 2 P.S.

c) Southern portion of the area with a lower elevation : 1 P.S.

The ground level of the pumping stations affects the distribution status of total water head and the area covered by each pumping station, as shown in FIGURE 4.2.4.2.A and 4.2.4.3.A. In addition to the influence made by topographic conditions, water pressure in the area is directly affected by the pumping stations. The figures at the pumping stations vary from 0.2 - 0.4 kg/cm² at No. 5 P.S. to 1.2 - 1.7 kg/cm² at No. 10 P.S. The results of measurements made at the pumping stations revealed a dominant tendency in the daytime and nighttime; the highest water pressure during the hours from 8:00 A.M. to 5:00 P.M.; and the lowest from 6;00 A.M. to 7:00 P.M. This corresponds to general water use patterns during the day. With regard to the distribution of water pressure in the area and the hourly variation, Figures 4.2.4.2.B and 4.2.4.3.B show the following:

- a) The water pressure changes through the day
- b) The figure during daytime (6:00 A.M. to 7:00 P.M.) is quite low (less than 0.1 kg/cm²) in most of the area except for the limited area in the vicinity of the pumping stations. Furthermore, there is no water supply to the central portion of the service are in the daytime (about 1/3 of service area).
- c) Water pressure throughout the service are in the nighttime (8:00 P.M. to 5:00 A.M.)shows a little better figure than that in daytime. However the pressure in the central portion (1/3 of service area) is still quite low with a figure of less than 0.1 kg/cm^2 .

APPENDIX 4.2.5 NUMBER OF CONNECTION BY METERED AND UNMETERED

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Tocal	1275	976	33690		10			263	7026	15	1057			, end	515	2553	15		293	S	Ś	.~*
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50	158	16	552	1	1	•	- 1	1	1	1		1	ł	1	•	45	1	1	ñ	ı	н	
61	41 7 7 7	165	7072	1	1	. 1	. 1	7	275	1		ł	1	1		223	. 1	1	3	ι	1	
18	475	94	1900	•	1	1.	1	1	1	1	1	ı	1	I,	ł	218	I	. 1			ı	l
1	115	27	792	t	1	t,		1	15	1	4	ı	J	I	,	148	1	1	1	 	•	'
91	531	61	433	1	I	 I	•		1			. I	1	ı	· ,	210	3	,	ŝ.		1	1
15	527	27	1011	.1 :	t	L				•	1	E.	1	1	- 1	761	, i	-	10	3	1	 F
-1	630	33	1356	1	1	i.:	- 1		50	1	-	.,		1	1	113	1	1.	5	1		1
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101	690	ີ 	667 3				: 	11	513	÷	1152	 	1		1	5 186		1	32			1
6	<u> </u>	 	7 332					-	5.430		62	1			'	155	1		=	· ·	t	
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	351	89	2754					-	00		7	<u>}</u>	1	1	1	92	1	1	~	1	'	2
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Zone No.	Type.	No. of Connec	m ³ /month	No. of Connect.	J_month	No of	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	No. of			3Ге	tio. of Connect		No. of Connect.	Jdav	No. of Connedt	4	No. of Connect	1		lio. of Connect	
	1		1/2		7/1	ł	1	1	4 	1		-	، 	<u> </u>		1/2	3/4		1/2	3/4	-	1/2
	Consumer			Dones-	E ÅC		1				Commer- cial			Institu-	tional		Domest Lic			Coamer-	cial	Institu- tional
		 	<u> </u>					Metered										:	חח- הפנינים			

Note: Data in May 1926

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NUMBER OF CURNECTIONS, CONSUMPTION AND WATER CHARGES 4.4.1 APPENDIX

DOMESTIC:

Connections (?)
181
66 25
2,858.25
136 45
92.00 24.00
トレ
2-516
· . · · · · · · · · · · · · · · · · · ·
89 173 90

- 28 -

4.4.1 (cont'd)

APPENDIX

COMMERCIAL & INSTITUTIONAL

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егед	Charges Number (Ÿ) Connect	00 607	795.00	00	00	30	30.00	00	6,471.35 2	78.90	00	999.60		00	50.00	86.00	00	1		270.00	0	0	5
U n m e t (Number of Connections	5		4	2	2	37	10	68	11	34		ł	F - 4	ъ	10	Ś	I	I	ເ	4	цŲ	87
	Charges (P)	126.40	641.60			219.60	•	152.	•	·0	~	5	81.60	1	104.00	1] (48.00		608.00	1	291	3,494.60
ered	Consumption (m ³ /month)	34	350	•	- 1	n,	597		3,948	/24	C 0 0	20			20	F	1 -	C1	1 1	C/7	1 5 7	10/	1,114
Ļ.	Number of Connections	2	m		1 (57 2	-		Q .	14	-4 ¢	۲] =-		۱ ۲	~	14	ר נ נ	
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- 29 -

(cont'd)

4.4.1

APPENDIX

INSTITUTIONAL:

Zone Nun	Metered	тед		Unmetered	red	Tota1	
	Number of Connections	Consumption (m ³ /month)	Charges (\$)	Number of Connections	Charges (F)	Number of Connections	Charges (F)
, tu, tu,	· I met	- 414	- 210.00	≈ 1	181.20	C2 1ml	181.20 710.00
Sub-Total	F-1	414	710.00	2	181.20	5	891.20
TOTAL	1,250	42,197	55,682.28	2,878	85,428.10	4,128	141,110.38

APPENDIX 4.4.2 COMPOSITION OF EACH BARANGAY IN TERMS OF WATER ZONE

Barangay	Composition (Zone Number)	Percentage of the Zone	Remarks
A. del Rosario	11	75	
	13	100	
Claro M. Recto	1.5	20	
· · · · · · · · · · · · · · · · · · ·	16	90	
Cutcut	21	50	
Lourdes Northwest	12	100	
· · · · · · · · · · · · · · · · · · ·	22	50	
Lourdes Sur	7	75	
	9	100	
Lourdes Sur East	16	10	
	17	90	
Malabanas	15	50	
	18	5	
Pampang	22	45	
Pandan	18	95	
	19	100	
Pulung Bulu	3	25	
······································	4	100	
	5	60	
Salapungan	20	100	
San Jose	1	5	
	2	50	
	3	30	
San Nicolas	8	70	
Mail (1COTAS	10		
	10	60 25	
Sta. Teresita	14	100	
ha, ierestra	22		
Sta. Trinidad	10	5	
Sto. Cristo	and the second	40	
JUD, CLISEO	3	15	
and the second	5	40	
	6	50	•
	1. 1.	25	
	17	10	·
Sto. Domingo	1	95	
	2	50	
Sta. Rosario	3	30	
	6.	50	
	8	30	
4	21	50	
lirgen de los Remedios	15	30	

an Alama Arang taona

Note: The percentage is calculated using household number distributed in related barangays.

APPENDIX 4.4.3 WATER CONSUMPTION

4.4.3.A

.A UNIT COMMERCIAL WATER CONSUMPTION (METERED)

6	Water Consu Monthly	mption(m3) Daily	No. of Connection	Per Connection Per Day(m3)	Remarks
Zone	Montniy	Dally	GUIIIEE EION		<u>ay na </u>
6	630	21	63	0.333	
8	4173	139	159	0.874	
9	1228	41	35	1.171	
10	1113	37	39	0.949	
11	82	3	1	3.000	
12	1297	43	18	2.389	
13	0	0	0	-	
21	432	14	14	1.000	
22	570	19	57	0.333	
Total	9525	317	386	0.821	

4.4.3.B

ESTIMATION OF TOTAL WATER CONSUMPTION

<u>No. o</u>	f Connection	ıs	Unit Consumption	Daily Consumption	Remarks		
Domestic	Sub-Total	3,567	$1.567 \text{ m}^3/\text{d}$	5,589 m ³ /d			
Commer- cial	Metered Unmetered	298 255	0.821 2.18*	245 556			
	Sub-Tocal	553		801			
	Total	4,120		6,390			

Note: * Average Consumption in Balibago Waterworks System

APPENDIX 4.5.1 Unaccounted-for Water/Not Utilized Water

The Zone No. 1, southern tip of the existing service area was selected as a model area in accordance with the following criteria.

- a) Easy measurements of flow rate into or out of the area ensuring served population of about 10% of the city total population served.
- b) The total maximum number of flow meter required should be less than three.
- c) The area should be predominantly residential.
- d) Easy data collection

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0

FIGURE 4.5.1.1 shows the flow chart to analyze unaccounted for water/not utilized water.

(1) Background information and existing water supply in Zone No. 1

1) Description of the model study area

The model study area, Zone No. 1 (see FIGURE 4.5.1.2) is located in the southern tip of the existing service area with an approximate area of 60 ha and population of 2,112 (352 households).

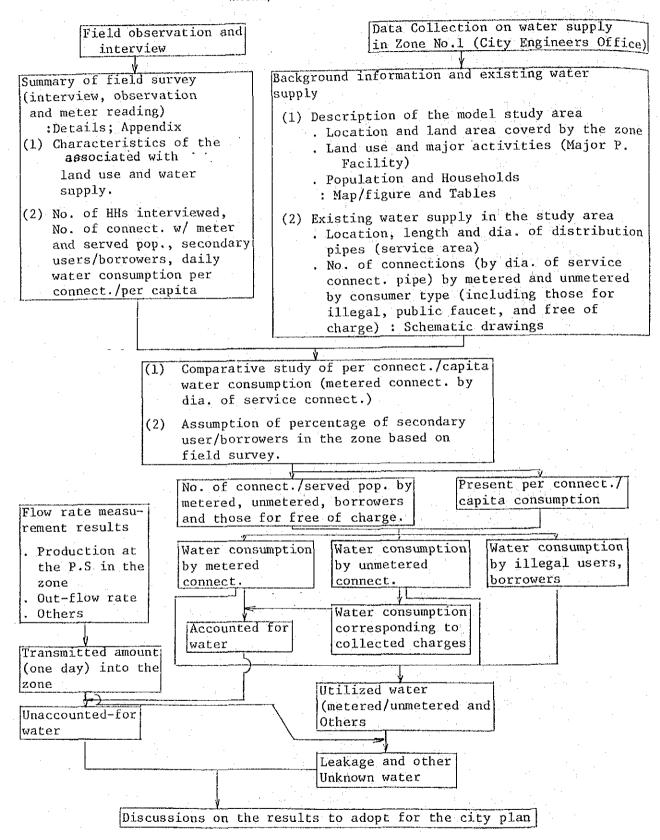
The area is predominantly residential with small commercial establishments along the Mc Arthur Highway. The major public facilities within the zone are St. Domingo Elementary School and Camp Tomas Pepito. The following is information obtained through the field survey.

St. Domingo Elementary School : 1,300 pupils and 33 teaching staff members

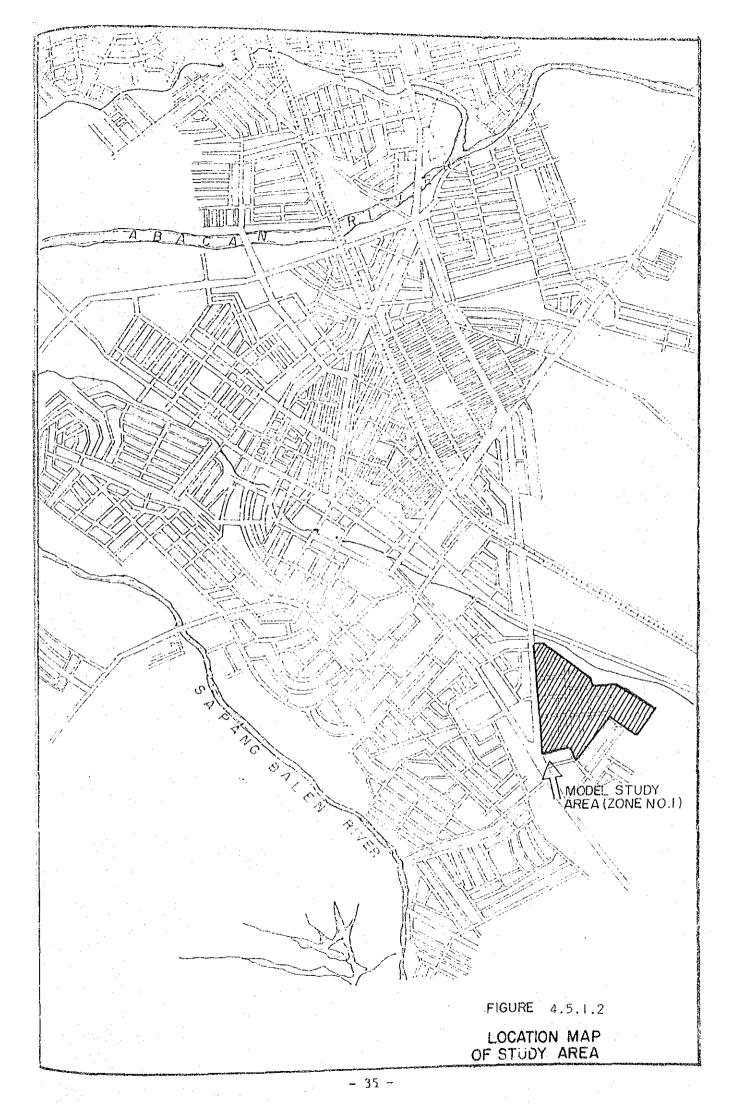
Camp Tomas Pepito : 155 personnel and 150 detainees

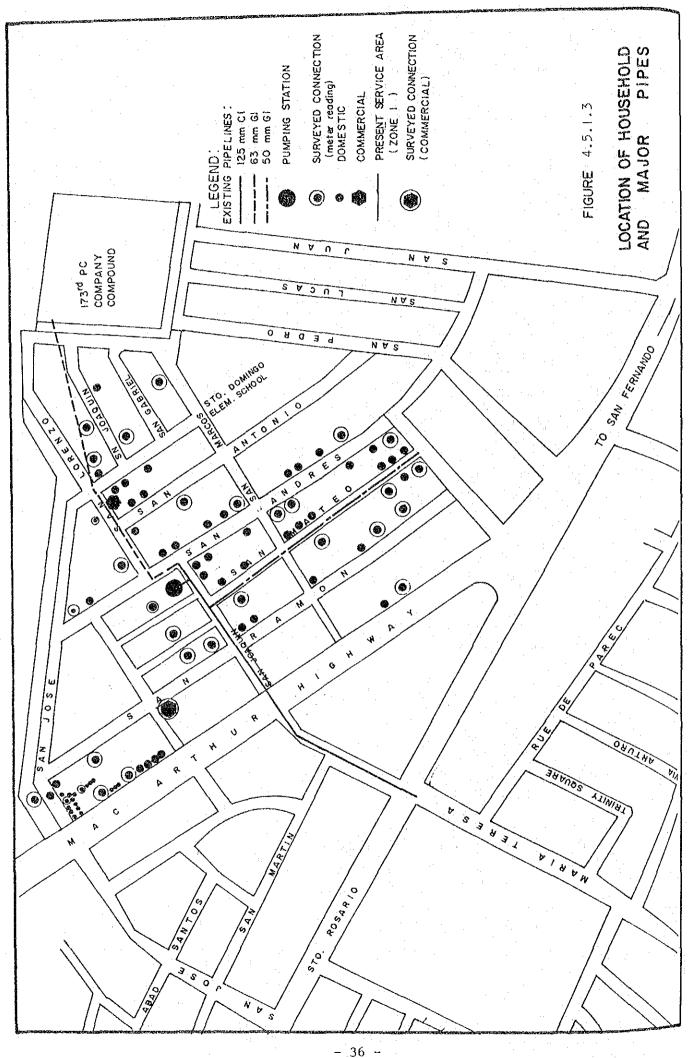
The location of households, commercial establishments and public facilities as well as road networks in the zone is given in FIGURE 4.5.1.3.

FIGURE 4.5.1.1 FLOW CHART FOR ESTIMATION OF UNACCOUNTED-FOR-WATER/ AND SOME FUNDAMENTALS



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2) Existing water supply in the zone

The water sources for the zone is a deep well with a pumping station (No. 7). The water produced at the pumping station is used not only for the zone including the Camp area but also for Zone No. 2. However the water pressure during daytime in the area far from the pump station in Zone No. 1 is quite low. There is no water supply to the elementary school during daytime. The diameter of distribution pipes ranges from 50 to 125 mm with a total length of 620 m. (Refer to FIGURE 4.5.1.3 on the location of distribution pipes).

a) Served population in the Zone

The served population comprises primary users and secondary users/borrowers, which were identified through the field interview. The following is the outline of the findings.

Field interview results

Investigations on the 36 domestic connections, which account for 40% of total metered connections, revealed the following composition of users.

Туре	No. of HHs	Served Pop.		
Primary users Secondary users/	36	228		
borrowers	21	85		
Total	57	313		

The percentage of secondary users/borrowers to the primary users is calculated at 37%. The figure seems to reflect the fact that the zone is predominantly composed of apartment houses.

Estimating the population in the zone and the number of concessionaires

The study zone consists of 15 sub-areas. Population by sub-area was estimated knowing the number of households and

using six persons as the average number of persons per HH. Number of primary users was also estimated in the same manner.

The population served is the total of primary users and secondary users/borrowers. The percentage of secondary users/borrowers to the primary users was assumed to be 35, based on the field investigation, although this figure should be further studied before an average figure for the city is made (See TABLE 4.5.1.1).

TABLE 4.5.1.1 POPULATION IN THE ZONE AND CONCESSIONAIRES

Sub-	Zone		Served Popul			
Area	Popula- tion	Primary	Secondary/ borrowers	Total	Served Percent	- R. М.
1	180	24	8	32	17.8	
2	54	12	4	16	29.6	
3&4	186	84	29	113	60.8	
5	138	42	15	57	41.3	
6	102	66	23	89	87.3	
7	60	60		60	100.0	All primary user
8	114	108	6	114	100.0	Most of population
						is primary user
9	312	210	74	284	91.0	
10					-	No connection
11	108	78	27	105	97.2	
12	96	66	23	89	92.7	
14	300	84	29	113	37.7	
15	378	222	78	300	79.4	
	: 					· · · · · · · · · · · · · · · · · · ·
otal	2,112	1,098	331	1,429	67.7	

Note : a) Secondary users/borrowers is 35% of primary users; average number of HH : 6 persons

b) Population/pupils at the camp and elementary school is not included in the Table.

The estimated total population served is 1,098 out of the Zone population of 2,112. The percentage of primary users to the zone population is approximately 52%, while total population served is 68%.

Water supply status in the Zone. **b**)

Number of connections by type of consumer as of May 1986 is summarized in TABLE 4.5.1.2. Public faucets installed in the premise of elementary school and those for the Camp were excluded. The total number of connections is 190 of which 91 (48%) is metered.

	1		<u> </u>					Ineti		1		Total	
Sub-	Domestic No. of Meter- Unme-				Commercial Neter- Unme-				Institutiona Meter-Unme-		Meter-Unme-		
Area	NO. OI HHS	ed	tered	Total		tered			tered		ed	tered	Tota
1	30	2	2	4		1	1	-		_	2	3	5
2	9		1	1	-				1	1	-	2	2
3&4	31	11	3	14	1	-	1	-		-	12	3	15
5	23	1	6	7	1	3	4			-	2	9	11
6	17	8	3	11	-	-	-	-		-	8	3	11
7	10	4	6	10	-	-	-	-		-	4	6	10
8	19	15	3	18				-		-	15	3	18
9	52	4	31	35	-	-	-	-	-		4.	31	35
10				-	-		10	-	-	-	-		-
11	18	10	3	13	-		-	-	-		10	3	13
12	16	7.	4	11	-	-	-	-			7	4	11
13	14	4	3	. 7	-	-	-	-	-	-	4	3	1
14	50	9	5	14	-	-				-	9	5	. 14
15	63	14	22	36		1	1	-	1	1	14	24	38
Total	352	89	92	181	2	5	7	-	2	2	91	99	190

TABLE 4.5.1.2 NUMBER OF CONNECTIONS BY CONSUMER TYPE

Diameter of service connections by consumer type Note: Domestic: metered and unmetered; 1/2 inches Commercial: metered, one 1/2 inch and another 3/4 inch; unmetered, 1/2 inch institutional: unmetered, 1/2 inch

TABLE 4.5.1.3 SUMMARY OF WATER CONSUMPTION AND CHARGES

Consumer	Met	ered		Unmeter	ed	Total		
2.	No. of Connect.	m ³ / month.	Charge (ア)	No. of Connect.	Charge (분)	No. of Connect.	Charge (ダ)	
Domestic	89	2,754	2,802	92	2,077.04	181	4,879.04	
Commercial	2	34	126.4	5	409	7	535.40	
Institu-				· · · · · · · ·				
tional	-		-	2	181	2	181	
Total	91	2,788	2,928.4	99	2,618.04	190	5,546.44	

Data Source : City Engineers Office

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The following is a summary of connections.

Type of Consumer	Metered	Unmetered	<u>Total</u>	Unmetered Additional Faucet
Domestic	89 (1/2")	92 (1/2")	181	182
Commercial	2 (1/2" & 3/4")	5 (1/2")	7	
Institutional		2 (1/2")	2	
Total	91	99	190	182

Water consumption and collected charges by consumer

type

c)

Water consumption and collected charges from metered/unmetered connections by consumer type for the month of May, 1986 are summarized in TABLE 4.5.1.3. (Details are given in TABLE 4.5.1.4.A to 4.5.1.4.C)

Per capita water consumption and domestic daily water consumption.

Per capita water consumption was studied by metered and unmetered connection due to the following:

- 1) Insufficiency of water supply because of limited water sources and pump capacity, especially in the remote areas where water pressure is less than 0.1 kg/cm^2 through the day.
- Some areas are provided with water supply only during daytime or nighttime by means of valve operation of the distribution pipes.
- iii) Most of the metered connections are installed along the main distribution pipeline, where comparatively good service is provided. On the other

TABLE 4.5.1.4.A	WATER	CONSUMPTION AND CHARGES	(DOMESTIC CONNECTION)

	· ·	Metered	·	Uni	metered			Total	
Sub- area	No. of Connect.	m ³ /month	Charge (F)	No. of Connect			No. of Connect	Charge(₱)	R.M.
1	2	28	35.20	2	42		4	77.2	
2		-÷		1	20	· · ·	1	20.	
3 & 4	11	31.8	337.20	3	68		14	405.2	
5	1	35	34.	6	120		7	154	
6	8	266	260.80	3	74		11	334.8	
7	4	99	103.20	6	156		10	259.2	÷.,
8	15	328	370.	3	69.45		18	439.45	· .
9	4	111	114.4	31	668.06		35	782.46	
11	10	269	276.8	3	62.		13	338.80	•
12	7	232	251.6	. 4	90		11	341.60	
13	4	162	153.6	3 -	66.		7	219.60	
14	9	405	378.	5	114		14	492.	· .
15	14	501	487.2	22	527.53	}	37	1,014.73	
Total	89	2,754	2,802.	92	2,077.04		182	4,879.24	

TABLE 4.5.1.4.B WATER CONSUMPTION AND CHARGES (COMMERCIAL CONNECTION)

Metered				Unmetered Total					
Sub- area	No. of Connect.	m ³ /month	Charge(₽)	No. of Connect.	Charge(P)	No. of Connect.	Charge(₱) R.M.		
1	- -		-	1	90.	1.	90.0		
3 & 4	1	14	70.40		· · · · ·	1	70.40		
5	1	20	56.	3	270	4	326.		
15				1	49	1	49 w/meter but not working		
Total	2	34	126.40	5	409	7	535.40		

TABLE 4.5.1.4.C WATER CONSUMPTION AND CHARGES (INSTITUTIONAL CONNECTION)

		• •						
	Me	etered		Unmeter	ed	То	tal	· · · · · · · · · · · · · · · · · · ·
Sub- area	No. of Connect.	m ³ /month	Charge(₽)	No. of Connect.	Charge(₱)	No. of Connect.	Charge(₱)	R.M.
2	 . 	t. - : . .		1	125.	1	125.	· · ·
15	-			1	56.	1	56.	
Total	:	······································		2	181.	2	181.	

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hand, the remaining areas experience shortage of water.

Water consumption and per capita daily consumption (metered)

Daily water consumption (metered)was reported for the month of May at 91.8 cu.m/day for the 89 connections. Total population served for the metered connections is about 700 calculated by estimating the zone total population served (1,429 persons) in Table 2.3.1 and the percentage of metered connections to the total number of connections(89/181 = 49%).

Daily per capita water consumption was calculated to be 131 l/cap.day.

The calculated figure using the meter reading results from concessionaires selected at random during field survey is 183 l/cap.day.

The summary of findings is shown in TABLE 4.5.1.5 in addition to the water consumption for the month of May. The average per capita consumption for the month of May for the 36 concessionaires is calculated at 157 1/cap.day. The figures calculated using data collected through field survey are 20 to 40% bigger than the estimated zone average for the month of May. This result may be attributed to the difference of service level in the zone due to insufficiency in water supply and the daily fluctuation in consumption. The average figure of 131 1/cap.day, can be the daily zone average considering the constraints of the present water supply.

Water consumption and per capita daily consumption for unmetered connections

Collections from unmetered connections for May was P2,077.04. These come from the flat rate connections as well as metered connections whose meters are under repair. For the latter, water charge is an average of previous month's consumption. It is difficult to assess the number of faucets per household using collection as a basis. Estimates were made, therefore, using the following:

Water consumption per connection for the flat rate (P18/month, 1/2" service connection) is 15 cu.m/month ((P18-P14); P0.8/cu.m + 10 cu.m = 15 cu.m). On the other hand, the consumption range for the metered connection (See TABLE 4.5.1.6) gives an idea of the average figure for majority of the concessionaires.

The Table reveals that approximately 90% of metered connections belongs to the consumption range of 0-60 cu.m/month. The average consumption within the range was calculated at 24.2 cu.m/month.

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TABLE 4.5.1.5

				Field Su	rvey			
	No.	Address	Daily Consump.	Primary User	Secondary User	Pop. Served	Data	R. M.
.	1	1-1-8	0.7	5	12	17	0.6	
	2	13-13A	0.8	3		3	0.8	
	3	1-3-13E	0.7	7		7	0.7	
1	4	1-8-12M	0.5	4		4	0.8	
	5.	1-8-12G	1.6	5		5	2.7	
	6	1-5-5	2.1	5		5	1.1	
:	7	1-3-2	1.7	12		12	1.8	
		1-3-19A	0.5	3	· ·	3	0.5	Commercial connec-
	1.1	an an Frankis Ar Maria		· · ·				tion (excluded from
		the second second					l a te	the Total)
	8	1-9-17A	1.0	6		6	.1.4	
	9	1-9-25	1.0	5		5	0.8	
•	10	1-14-8	2.3	10		10	2.1	
	11	1-15-50B	2.2	15		15	0.7	
	12	1-14-20A	6.6	5	- 26 .	31	4.3	
	13	1-15-51	1.0	10		10	1.1	
	14	1-14-29	1.0	8		8	(1.0)	
	15	1-15-50E	1.8	4	2	6	1.5	
	16	1-15-50F	2.3	8	5	13	1.3	
	17	1-15-61	1.0	3		3	0.5	
	18	1-15-17	0.9	5		_ 5	0.2	
•	19	1-15-30	2.0	5	10	15	1.7	
	20	1-15-37	2.2	6	4	10	2.5	
	21	1-11-23	0.8	7		7	0.3	
	22	1-13-9	1.7	6		6	(1.7)	
	23	1-12-8A	1.4	5		- 5	0.9	
	24	1-11-12	2.0	5	2	7	1.9	
	25	1-14-28	1.9	8		8	1.3	
	26	1-12-2	0.6	^{5.} 3		3	0.4	
	27	1-9-46	0.3	3		3	1.1	

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TABLE 4.5.1.5 (cont'd)

		Fie	ld Survey	7 - 1	·		
No,	Address	Daily Consump.	Primary User	Secondary User	Pop. Served	Data	R. M.
28	1-7-2	1.4	5		5	(1.4)	
29	1-12-3A	1.9	4	10	14	2.1	
30	1-11-7A	2.4	5	· . • ·	5	2.3	
31	1-6-8	0.6	3		3	0.7	
32	1-6-4	1.4	· 6		6	0.5	
33	1-6-1A	3.0	1.0	- 8	18	2.1	
34	1-6-19	0.9	6	5	11	1.5	
35	1-6-17	1.4	10		10	1.3	
36	1-6-14B	2.5	11	3	14	(2.5)	
Tota	<u> </u>						
:	estic)	57.6	228	87	315	49.6	

Note : Daily consumption obtained by meter reading (Field survey)

() : used same figures of measured

TABLE 4.5.1.6

RANGE OF WATER CONSUMPTION (METERED) AND MAJORITY AVERAGE CONSUMPTION

A	1			Calcu	lation	of major:	ity av	erage
Consumption. Range (cu.m/mon.)	No. of Connect.	Percen- tage	Summation of percent.	Ave. cu.m	cu.m/ mon.	· · · · · · · · · · · · · · · · · · ·	Ave. cu.m /mon.	l/cap. day
0 - 10	15	16.9	16.9	5	84.5			i
11 - 20	22	24.7	41.6	15	370.5			
21 - 30	16	18.0	59.6	25	450			
31 - 40	13	14.6	74.2	35	511			
41 - 50	6	6.7	80.9	45	301.5			· · · ·
51 ~ 60	7	7.9	88.8	55	434.5	2,152	24.2	102
61 - 70	4	4.5	93.3	65	292.5	2,152		102
71 - 80	3	3.4	96.7	75	255			
81 - 90	1	1/1	97.8	85	93.5			
91 - 100	0	0.0	97.8	95				
101 - 110	1	1.1	98.9	105	115.5	· · · .		
111 -	1	1.1	100.0	120	132	3,040.5	30.4	128
Total	89	100.0						

Note :

Overall average; 128 which corresponds to that estimated using data of May (131)

Per capita daily consumption for the unmetered connections was estimated using estimated served population and number of connections as follows:

i) Figure corresponding to the flat charge:

15 cu.m/month x 92 connect ÷ 30 day+(1429 - 700) = 0.063 cu.m/cap.day

 11) The average consumption of majority of metered connections 24.2 cu.m/month x 92 connect.* 30 * 729
 = 0.102 cu.m/cap.day

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Per average per capita consumption for the unmetered connections seems to be between 63 to 131 1/cap.day. Being within the consumption range, 102 1/cap.day may be used for the purpose of this study.

Daily water consumption for the unmetered concessionaires is accordingly estimated at 74.2 cu.m/day (24.2 cu.m/month \div 30 days x 92 connection).

The water consumption of the domestic origin is, therefore, 166 cu.m/day as a total of metered (91.8 cu.m/day) and unmetered (74.2 cu.m/d).

d) Unit water consumption and total consumption for commercial use: Water consumption by the existing metered connections for the month of May was recorded at 34 cu.m/month. Daily per connection consumption is calculated at 0.57 cu.m/day. A figure of 0.5 cu.m/day was meter-read in one of the two connections during the field survey. Inferred consumption for the unaccounted connections was estimated using charge composition as follows:

(₱90 - ₱40) ÷ ₱1.6/cu.m + 10 cu.m = 41.25 cu.m/month 41.25 ÷ 30 = 1.375 cu.m/day

The figure of 1.375 cu.m/day may be used for the unmetered connections since it corresponds to the charges paid by them on a same basis of the metered connection.

Water consumption for commercial use is 8.01 cu.m/day as shown below.

metered	:	1.13 cu.m/day (2 c	connections)
unmetered	:	1.375 cu.m/day x 5	5 = 6.88 cu.m/day
		(5 connections)	

Total : 8.01 cu.m/day

e)

Unit water consumption and total consumption for the institutional use: The existing two connections; the camp and elementary school are unmetered. Unit water consumption based on the charges is same as that for the commercial unmetered connections. However, it was confirmed in the field that there is no water supply to the elementary school during daytime. Therefore water consumption for the institutional use in the zone may be accounted to be 1.38 cu.m/day (one governmental office).

(4) Water consumption in the zone (Utilized water in Zone No. 1)

The total water consumption including domestic/commercial and institutional uses is estimated at 175.39 cu.m/day.

Estimate of the water transmitted/distributed to Zone No. 1 from No. 7 pumping station

Preliminary survey revealed that part of water produced at no. 7 pumping station is distributed to the Zone No. 2 and is also provided to the Camp without charge (unmetered).

Under these conditions, flow rate measurement was conducted through the day at the two points as shown in FIGURE 4.5.1.4 (the point connected to the Zone No. 2 and that before the Camp compound.) The measurement records are given in TABLE 4.5.1.7.

The water balance between production and distribution/consumption is given below.

Q =	$(q_1 + q_2)$) =	q ₃	+ q ₄
	where,	ģ	:	production (cu.m/day)
		q 1	:	supply to the Camp (cu.m/day)
		^q 2	:	supply to the No. 2 zone
• ¹ •	• •	q ₃	:	Consumption in the No. 1 zone
		q ₄	:	water not utilized in zone No. 1
				(leakage and unknown water)

The results of flow rate measurement are summarized as follows: Q = 812,8 cu.m/day

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 $q_1 = 114.5$ $q_2 = 495.0$ $q_3 + q_4 = Q - (q_1 + q_2)$:water distributed to Zone No. 1 = 203.3 cu.m/day

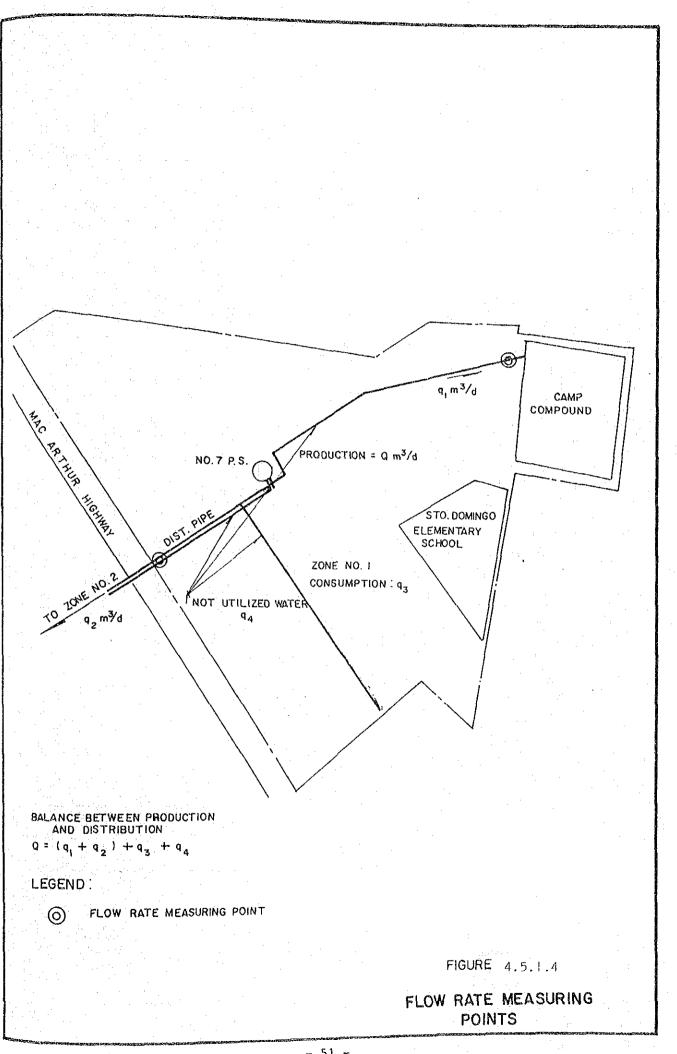


TABLE 4.5.1.7 FLOW RATE MEASUREMENT RESULTS

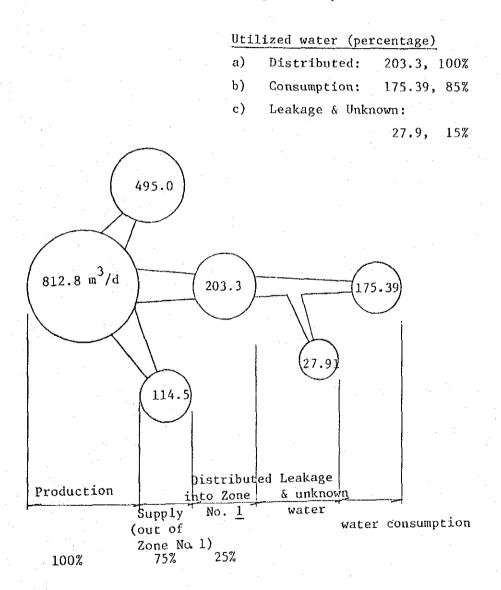
				herry	Unit: cu.m/hour
Time	Production (No. 7 P.S.)	Flow rat Connection (Zone 1 & 2)	e Near Capamp	Distributed into Zone No. 1	R. M.
0 - 1	32.9	26.5	6.4	0.0	
1 - 2	32.9	26.6	6.3	0.0	
2 - 3	32.9	26.9	6.0	0.0	
3 - 4	32.9	26.8	6.1	0.0	
4 - 5	33.0	26.4	6.4	0.2	
5 - 6	33.9	22.1	4.9	6.9	
6 - 17	34.7	17.1	3.4	14.2	
7 - 8	34.6	15.6	3.6	15.4	
8 - 9	34.5	16.0	3.5	15.0	
9 - 10	33.9	16.1	3.5	14.3	
10 - 11	34.0	16.8	3.6	13.6	
11 - 12	34.3	17.6	3.8	12.9	
12 - 13	34.4	17.1	3.8	13.5	
13 - 14	34.2	19.0	4.2	11.0	
14 - 15	34.2	20.3	4.8	9.1	
15 - 16	34.2	19.6	4.5	.10.1	
16 - 17	34.4	17.2	3.6	13.6	
17 - 18	34.5	18.3	3.6	12.6	
18 - 19	34.2	18.8	4.1	11.3	
19 - 20	34.1	17.9	4.2	12.0	
20 - 21	33.9	19.8	4.7	9.4	
21 - 22	33.8	21.9	5.7	6.2	
22 - 23	33.4	24.6	6.8	2.0	
23 - 0	33.0	26.0	7.0	0.0	
Hourly A	ve. 33.9	20.6	4.8	8.5	
Daily To	tal 812.8	495.0	114.5	203.3	

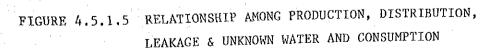
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Utilized water in the zone and unaccounted-for water

Based on the above water consumption and production/distribution from the water source, water utilized in Zone No. I is calculated at approximately 85% of distributed amount from No. 7 pumping station (See FIGURE 4.5.1.5).

This FIGURE is comparatively high attributed seemingly to the low water pressure and water use through the day.





Of the water distributed into the Zone (203.3 cu.m/day), approximately 15% (27.9/cu.m/day) is lost to leakage and other reasons. Accounted-for water was estimated according to the following procedure.

Domestic consumption

a) metered = 91.8 cu.m/day (89 connections)

b) unmetered :

87 connections (¥18 - ¥14) ; 0.8 + 10 cu.m

= 15 cu.m/connect.month(0.5 cu.m/connect.day)

 $0.5 \times 87 = 43.5 \text{ cu.m/day}$

182 additional faucet $P2 \div 0.8$

= 2.5 cu.m/faucet.month (0.083 cu.m/faucet.day)

 $0.083 \times 182 = 151 \text{ cu.m/day}$

5 metered (but not functioning)

1.03 cu.m/connect.day (metered average) x 5

= 5.2 cu.m/day

Sub-total 63.8 cu.m/day

(92 connections & 182 additional faucets)

Total <u>l</u>

155.6 cu.m/day

(181 connections & 182 additional faucets)

Commercial consumption

a) metered : <u>1.1 cu.m/day</u> (2 connections)
b) unmetered : (₱90 - ₱40) + ₱1.6 cu.m + 10 cu.m = 41.3 cu.m/connect.month (1.375 cu.m/conn.day) 1.375 x 5 = 6.9 cu.m/day
Total : 8 cu.m/day (7 connections)

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Institutional consumption

a) unmetered: <u>1.4 cu.m/day</u> (1 connection)

Total accounted-for water is accordingly 165 cu.m/day. The percentage of accounted-for water distributed form the source is approximately 80, which seems to be above the city's average.