

APPENDICES

APPENDIX 3.4.1 ELECTRIC CHARGES ADOPTED BY THE MERALCO

Residential

First 14 kwh	₱2.00 (Minimum Charge)
Next 36 kwh	0.125 per kwh
Next 50 kwh	0.15 per kwh
Next 100 kwh	0.20 per kwh
Excess kwh	0.365 per kwh

General Service

Classification

X-1 (Conn. Load : 1-500c Natis)

First 14 kwh	₱3.00 (Minimum Charge)
Next 76 kwh	0.21 per kwh
Excess kwh	0.365 per kwh

X-MD (Conn. Load : over 5,000 Natis)

Demand Charge	₱12.60 per kw
Plus Energy Charge	
First 100 hrs.	₱ 0.33 per kwh
Next 100 hrs.	0.28 per kwh
Next 100 hrs.	0.25 per kwh
Excess kwh	0.23 per kwh

APPENDIX 4.1.1 POPULATION AND NUMBER OF HOUSEHOLDS BY WATER SERVICE
MUNICIPALITY OF CABUYAO (1980)

Barangay	Population No. of HH	Level III System	Level I System (Point Source)			Total
			Well	Spring	Others	
	2298	1638	660	-	-	660
1. Bgy. I	383	273	110	-	-	110
	1590	1308	282	-	-	282
2. Bgy. II	265	218	47	-	-	47
	1698	1014	684	-	-	684
3. Bgy. III	283	169	114	-	-	114
	1320		1320			1320
4. Baclaran	220	-	220	-	-	220
5. Banay- banay	2946		2946			2946
	491	-	491	-	-	491
	3246		3222	36	-	3246
6. Banlic	541	-	537	6	-	541
	3954	972	2982			2982
7. Bigaa	659	162	497	-	-	497
	1632	-	1626	6	-	1632
8. Butong	272	-	271	1	-	272
	648		252	144	252	648
9. Casile	108	-	42	24	42	108
	588	-	576	12		588
10. Diezmo	98	-	96	2	-	98
	5112		5112			5112
11. Gulod	852	-	852	-	-	852
	4794		4794			4794
12. Mamatid	799	-	799	-	-	799
	5376		5376			5376
13. Marinig	896	-	896	-	-	896
	3588		3588			3588
14. Niugan	598	-	598	-	-	598
	450		390		60	450
15. Pittland	75	-	65	-	10	75
	2892		2892			2892
16. Pulo	482	-	482	-	-	482
	3042	582	2424	36		2460
17. Sala	507	97	404	6	-	410
	1998		1998			1998
18. San Isidro	333	-	333	-	-	333
Total	47172	5514	41112	234	312	41658
	7862	919	6852	39	52	6943

Note: Above : Population
Below : No. of Households

Source: Comprehensive Development Plan

APPENDIX 4.1.2 POPULATION AND NUMBER OF HOUSEHOLDS SERVED BY WATER
SOURCE MUNICIPALITY OF STA. ROSA (1980)

Barangay	Population No. of HH	Level III System	Level I System(Point Source)		
			Well/Spring	Others	Total
1. Kan- luran	4794 799 3198	1038 173 942	1722 287 606	2034 339 1650	3756 626 2256
2. Malusok	533 2142	157 228	101 1404	275 510	376 1914
3. Market - Area	357 7134	38 246	234 6444	85 444	319 6888
4. Aplaya	1189 5736	41 1350	1074 1788	74 2598	1148 4386
5. Balibago	956 4716	225	298 4716	433	731 4716
6. Caingin	786 2376	- 132	786 1146	- 1098	786 2244
7. Dila	396 5172	22 546	191 3558	183 1068	374 4626
8. Dita	862 1416	91	593 1416	178	771 1416
9. Don Jose	236 1626	- 366	236 492	- 768	236 1260
10. Ibaba	271 3120	61 288	82 2310	128 522	210 2832
11. Labas	520	48	385	87	472
12. Maka- biling	3312 552	-	3108 518	204 34	3312 552
13. Malit- lit	2964 494 4836	-	2898 483 4824	66 11 16	2964 494 4836
14. Pook	806	-	804	2	806
15. Pulong Sta.Cruz	3156 526	-	2634 439	522 87	3156 526
16. Sto, Domingo	714 119 6588	-	714 119 6372	- 216	714 119 6588
17.Sinalahan	1098 6900	- 840	1062 4386	36 1674	1098 6060
18. Tagapo	1150	140	731	279	1010
TOTAL	69900	5976	50502	13422	63924
	11650	996	8417	2237	10654

Note: Above : Population
Below : No. of Households

Source: Comprehensive Development Plan

APPENDIX 4.1.3 POPULATION AND NUMBER OF HOUSEHOLDS SERVED BY WATER
SOURCE MUNICIPALITY OF BINAN (1980)

Barangay	Population No. of HH	Level III	Point Source with Pitcher Pump			Point Source
			Public Well	Private Well	Total	
	8238	150	156	3216	3372	4716
1. Canlalay	1373	25	26	536	562	786
	600		24	264	288	312
2. Casile	100	-	4	44	48	52
	14766	276	282	5736	6018	8472
3. dela Paz	2461	46	47	956	1003	1412
	15174	264	264	5388	5652	9258
4. Malaban	2529	44	44	898	942	1543
	4044	858	66	1350	1416	1770
5. Poblacion	674	143	11	225	236	295
6. San	14100	2088	282	5682	5964	6048
Antonio	2350	348	47	947	994	1008
	4062	366	78	1602	1680	2016
7. San Jose	677	61	13	267	280	336
8. San	7650	222	144	2970	3114	4314
Vicente	1275	37	24	495	519	719
9. Sto.	2856	138	60	1230	1290	1428
Domingo	476	23	10	205	215	238
	228	60	12		72	156
10. Biñan	38	10	2		12	26
	222		12	48	60	162
11. Bungahan	37		2	8	10	27
	1320		36	408	456	864
12. Calabuso	220	-	6	68	76	144
	366		12	36	48	318
13. Ganado	61	-	2	6	8	53
	1608		36	456	492	1116
14. Halang	268	-	6	76	82	186
	828		24		144	660
15. Langkiwa	138	-	4	24	28	110
	762		24	180	204	558
16. Loma	127	-	4	30	34	93
	90		6	12	18	72
17. Malamig	15	-	1	2	3	12
18. Maml-	1056		30	306	336	720
lasan	176	-	5	51	56	120
	4026	216	72	1476	1548	2262
19. Platero	671	36	12	246	258	377
	1086		36	138	174	912
20. San Anton	181	-	6	23	29	152
21. Soro-	996	-	36	168	204	792
Soro	166	-	6	28	34	132
	624		18	126	144	480
22. Timbao	104	-	3	21	24	80
	2292		60	168	228	2064
23. Tubigan	382	-	10	28	38	344
	276		12	60	72	204
24. Zapote	46	-	2	10	12	34
	87270	4578	1782	31236	33018	49674
TOTAL	14545	763	297	5206	5503	8279

Note: Above : Population
Below : No. of Households
Source: Municipal Development Plan

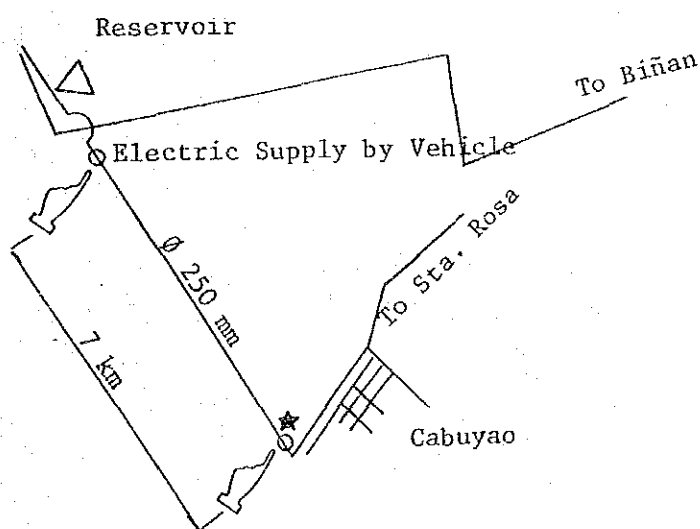
APPENDIX 4.2.1 SURVEY FOR ESTIMATION OF C-VALUE

A section of the transmission pipeline was selected for the purpose of this survey. Measurements of water pressure and flow rate were conducted at the two points throughout the day. The selection criteria for the section of the pipeline are:

- 1) There is no distribution of water in the section.
- 2) There is easy access to the points. Also electricity is available.
- 3) Water pressure can be measured using existing devices.

The section between the reservoir area to Cabuyao (diameter of pipes, 250 mm; length, approximately 7 km) was selected in the light of the above conditions.

The test at the selected points was implemented during the unaccounted-for-water/not utilized water survey. Although the two points were measured on different dates, both points were measured for about 7 hours. The flow rates in the selected section fluctuated at a certain range daily, but it was expected that the range would be negligible during week days. However, the supplementary measurements mentioned above were still taken into account. FIGURE 4.2.1.1 shows the location of measuring points.



Legend:

○ Measuring Point

★ Excavation to expose Pipes

FIGURE 4.2.1.1 LOCATION OF MEASURING POINTS

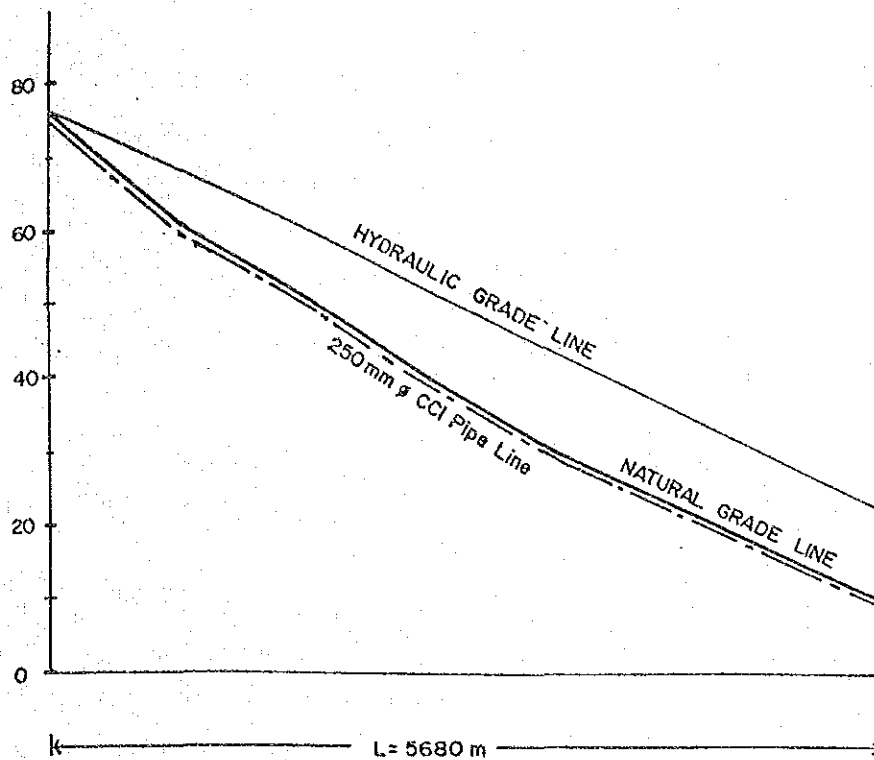
The flow rate in the Cabuyao-Sta. Rosa line is directly affected by the operation of the valve at the reservoir. The time period for analysis was determined to be for four hours, from 1:00 P.M. to 5:00 P.M. thus avoiding the valve operation time.

The average flow velocity is about 1.4 m/s, thus the time log is estimated at one hour between the two points. TABLE 4.2.1.2 shows the measurement result on the requirements.

TABLE 4.2.1.2 MEASUREMENT RESULTS

Time	Item	Reservoir			Cabuyao Inlet		
		Flow Rate	Velocity	Pressure	Flow Rate	Velocity	Pressure
		cu.m/hr.	m/s	(m)	cu.m/hr.	m/s	(m)
13:00		270	1.33	0.4	-	-	-
14:00		274	1.35	0.4	274	1.35	13.3
15:00		289	1.42	0.4	281	1.38	16.9
16:00		293	1.44	0.4	289	1.42	19.0
17:00		-	-	-	295	1.45	19.8
Average		282	1.39	0.4	285	1.40	17.3

FIGURE 4.2.1.2 - presents the profiles of hydraulic grade-line and computation result of "C" value.



UPSTREAM (Reservoir area)

$$Q = 282 \text{ m}^3/\text{hr} = 0.0783 \text{ m}^3/\text{sec}$$

$$\text{ELEVATION} = 76.2 \text{ m}$$

$$\text{PRESSURE HEAD} = 0.4 \text{ m}$$

$$\text{PIPE DIAMETER} = 250 \text{ mm}$$

COMPUTATION ;

$$C = 3.59028 \cdot Q_{ave} \cdot D^{-2.63} \cdot H^{-0.54} \cdot L^{0.54}$$

Thus;

$$C = 3.5902 \times 0.0787 \times (0.25)^{-2.63}$$

$$\times (49.2)^{-0.54} \times (5680)^{0.54} = 141$$

DOWNSTREAM (Entrance of Cabuyao)

$$Q = 285 \text{ m}^3/\text{hr} = 0.0792 \text{ m}^3/\text{sec}$$

$$\text{ELEVATION} = 10.1 \text{ m}$$

$$\text{PRESSURE HEAD} = 17.3$$

$$\text{PIPE DIAMETER} = 250 \text{ mm}$$

Where ;

$$Q_{ave.} = 1/2 (0.078 + 0.0792) = 0.0787$$

$$D = 0.250 \text{ m}$$

$$L = 5.680 \text{ m}$$

$$H = (76.2 + 0.4) - (10.1 + 17.3) = 49.2$$

FIGURE 4.2.1.2 PROFILE OF HYDRAULIC GRADE LINE

APPENDIX 4.2.2 WATER PRESSURE IN THE SERVICE AREA

The preliminary survey revealed that many of the houses possess additional faucets. The water pressure measurements were done outside the houses covering 36 points; 7 in Cabuyao, 17 in Sta. Rosa and 12 in Biñan. (10 points: Automatic Pressure Recorder). The survey was conducted for three days to cover the three areas, one day for each area. Two shifts had been adopted due to the measuring requirement throughout the day.

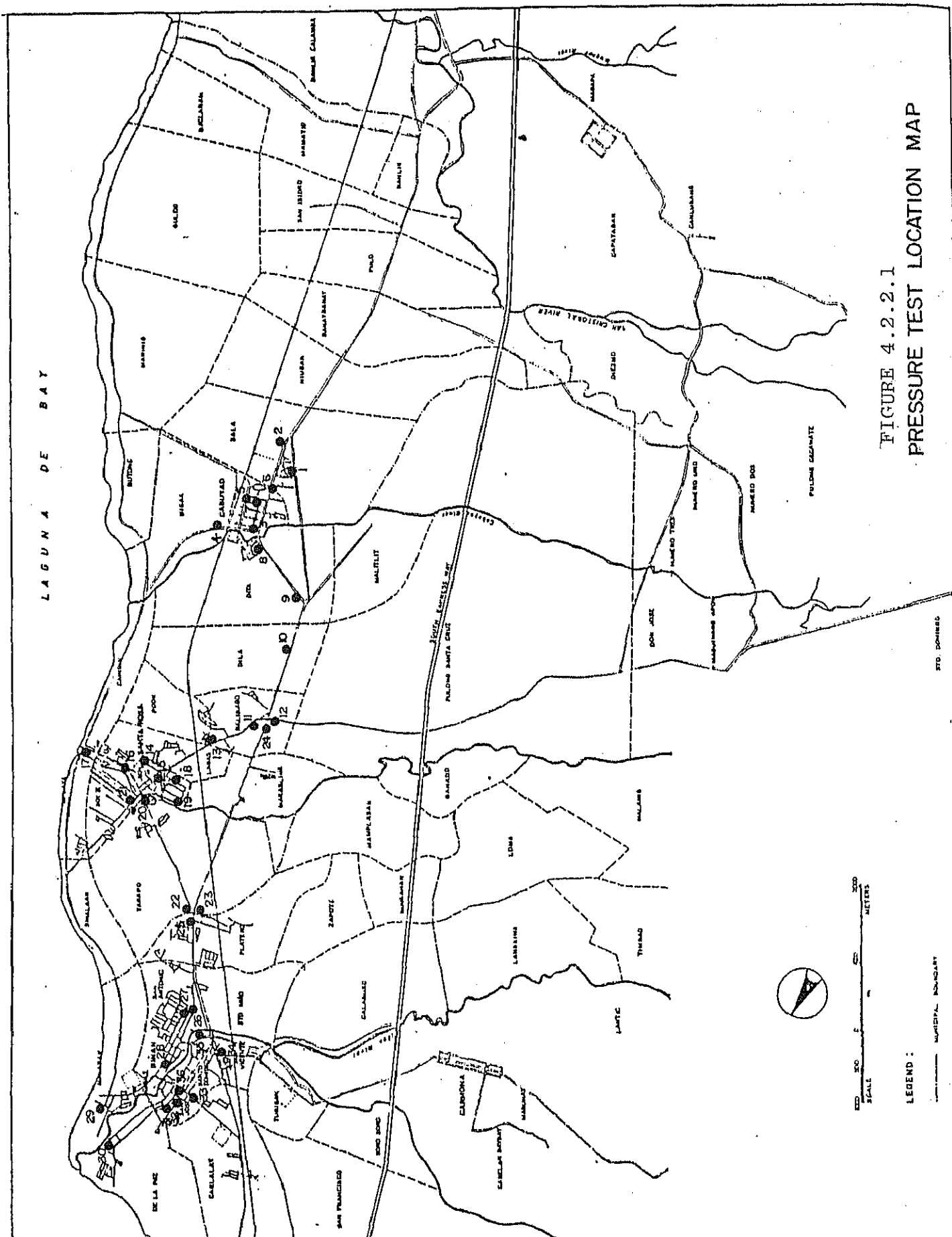
TABLE 4.2.2.1 and FIGURE 4.2.2.1. show the selected points for this test. FIGURE 4.2.2.2 shows contour line covering the 3 municipalities.

The measurement records for every hour at the 36 points throughout the day are shown in TABLE 4.2.2.2. The contour lines are prepared for the total water head and water pressure based on the records at 7:00 A.M. and 1:00 P.M. which may represent the hours in maximum and minimum water consumption throughout the day, respectively. FIGURE 4.2.2.3.A to FIGURE 4.2.2.3.L present the contour lines by municipality. The total water head at the measurement point is calculated using the topographical contour prepared by each municipality.

TABLE 4.2.2.1 LOCATION OF SELECTED POINTS FOR
PRESSURE TEST

<u>No.</u>	<u>Elevation (GL m)</u>	<u>Address of the Selected Point</u>	<u>R.M.</u>
1	13	Sala, Cabuyao	APR
2	11	144, Sala, Cabuyao	
3	8	8, P. Burgos St. Cabuyao	
4	7	156, Bigaa, Cabuyao	
5	9	M.H. Del Pilar St., Cabuyao	
6	10	59, AM. Roxas St., Cabuyao	
7	9	176, Malvar St., Cabuyao	
8	9	Max. Pueblo, Sta. Rosa	APR
9	16	1816, Dita, Sta. Rosa	
10	13	1586, Dita, Sta. Rosa	
11	12	1211, Balibago, Sta. Rosa	
12	13	13th, F. Reyes St. Balibago, Sta. Rosa	
13	9	890, Rizal St. Labas, Sta. Rosa	
14	7	1176, Lucero St. Malusok, Sta. Rosa	
15	8	J.P. Riza, Sta. Rosa	APR
16	6	180, Ibaba, Sta. Rosa	
17	4	Aplaya, Sta. Rosa	APR
18	9	1072, Lucero St., Sta. Rosa	
19	9	686, P. Gomez St., Sta. Rosa	
20	7	Tatlong Hari, Sta. Rosa	APR
21	7	Tatlong Hari, Sta. Rosa	APR
22	9	7, Tagapo, Sta. Rosa	
23	9	Hi-way, Tagapo, Sta. Rosa	
24	13	Balibago, Hi-way, Sta. Rosa	
25	9	Platero, Biñan	
26	8	Monalat, Biñan	
27	8	780, Balintawak St., Biñan	
28	7	Manabat, Biñan	APR
29	4	497, Dalampasigan St., Malabon, Biñan	
30	6	828, Dela Paz, Biñan	
31	8	Capinpin St., Biñan	APR
32	8	55, Dr. A. Gonzales St., San Jose, Biñan	
33	9	67, A. Bonifacio St. Canlacay, Biñan	
34	9	San Vicente, Biñan	
35	9	San Vicente, Biñan	APR
36	8	Bonifacio St., Biñan	APR

Note: APR ---- Automatic Pressure Recorder



(Unit: kg/cm²)

PRESSURE TEST

TABLE - 4.2.2.2

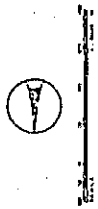
Hour

No.

Hour No.	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
1	1.3	1.3	1.7	1.7	1.7	0.8	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.8	0.9	0.9	0.9	0.9	0.9	0.9	1.2	1.3	1.3	1.3
2	1.7	1.7	1.7	1.5	1.5	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.3	0.7	0.7	0.8	0.8	0.8	0.7	1.0	1.3	1.6	1.7
3	1.9	1.9	1.8	1.8	0.9	0.4	0.3	0.3	0.5	0.4	0.1	0.5	0.5	0.7	0.4	0.4	0.4	0.4	0.4	0.4	1.4	1.7	1.9	1.9
4	2.0	2.0	2.0	2.0	1.5	0.9	0.5	0.5	0.5	0.5	0.5	0.5	0.7	0.8	0.9	1.0	1.0	1.0	1.0	1.0	1.4	1.8	2.0	2.0
5	1.7	1.7	1.7	1.2	1.0	0.3	0.1	0.3	0.2	0.2	0.2	0.1	0.3	0.5	0.7	0.7	0.7	0.7	0.7	0.6	1.1	1.2	1.5	1.6
6	1.9	1.9	1.9	1.9	1.6	0.7	0.5	0.5	0.6	0.5	0.5	0.6	0.6	0.7	0.8	0.9	0.9	0.9	0.8	0.9	1.2	1.5	1.9	1.9
7	2.0	2.2	2.5	2.5	2.4	0.5	0.5	0.6	0.6	0.5	0.6	0.6	0.8	0.8	0.8	0.8	0.9	0.9	0.8	0.8	1.2	1.5	1.6	1.8
8	1.7	1.7	1.7	1.7	1.3	0.6	0.4	0.5	0.6	0.5	0.5	0.6	0.6	0.6	0.7	0.7	0.7	0.7	0.7	0.7	1.1	1.3	1.5	1.6
9	1.0	1.0	1.0	1.0	0.4	0	0	0	0	0	0	0	0	0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.5	0.9	1.0
10	1.0	1.0	1.1	1.0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.3	0.7	0.9	1.0
11	0.8	0.8	0.8	0.7	0.4	0	0	0	0	0	0	0	0	0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.4	0.7	0.8
12	0.6	0.6	0.7	0.6	0.5	0.3	0.2	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.2	0.2	0.2	0.3	0.3	0.5	0.5	0.5	0.5
13	0.9	1.0	1.0	1.0	0.8	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.3	0.4	0.7	0.8	0.9
14	0.6	0.6	0.6	0.5	0.3	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.2	0.5	0.5	0.5	0.5
15	1.2	1.2	1.2	1.1	0.9	0.5	0.4	0.4	0.3	0.3	0.3	0.5	0.4	0.5	0.6	0.6	0.6	0.7	0.7	0.7	0.7	1.0	1.0	1.1
16	1.1	1.1	1.1	1.1	1.0	0.1	0.1	0.1	0.2	0.2	0.2	0.1	0.1	0.2	0.3	0.3	0.3	0.3	0.2	0.3	0.3	0.4	1.0	1.0
17	1.1	1.1	1.1	1.1	1.1	0.5	0.3	0.3	0.2	0.2	0.2	0.3	0.4	0.6	0.6	0.5	0.5	0.4	0.5	0.5	0.7	1.0	1.0	1.1
18	1.1	1.1	1.1	1.1	1.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.7	0.9	1.0	1.1
19	1.0	1.1	1.1	1.0	0.9	0.3	0.1	0.1	0.2	0.1	0.1	0.1	0.1	0.3	0.4	0.4	0.4	0.5	0.5	0.5	0.6	0.9	1.0	1.0
20	1.3	1.3	1.3	1.2	1.1	0.6	0.5	0.5	0.5	0.5	0.5	0.6	0.6	0.6	0.7	0.7	0.7	0.8	0.8	0.6	0.8	1.1	1.2	1.2
21	0.8	0.8	0.8	0.8	0.7	0.4	0.3	0.3	0.2	0.3	0.4	0.5	0.4	0.4	0.4	0.4	0.5	0.5	0.5	0.4	0.6	0.7	0.8	0.8
22	0.8	0.8	0.8	0.8	0.8	0.5	0.3	0.3	0.1	0.1	0.1	0.1	0.1	0.1	0.3	0.3	0.3	0.3	0.3	0.5	0.6	0.7	0.7	0.6
23	0.9	1.1	0.9	0.8	0.7	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.3	0.3	0.1	0.2	0.2	0.3	1.1	1.2	0.8	1.2
24	1.3	1.4	1.3	1.3	1.1	0.4	0.2	0.2	0.5	0.3	0.3	0.4	0.5	0.5	0.6	0.7	0.7	0.5	0.9	0.9	0.9	1.0	1.3	1.3
25	1.7	1.7	1.7	1.9	1.5	0.8	0.5	0.3	0.3	0.2	0.2	0.2	0.3	0.4	0.4	0.4	0.5	0.5	0.6	0.9	1.0	1.3	1.5	1.5
26	0.9	0.8	0.8	0.8	0.7	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.3	0.5	0.7	0.8	0.8
27	0.4	0.4	0.4	0.3	0.1	0.1	0.1	0.1	0	0	0	0	0	0	0	0	0	0.1	0.1	0.1	0.2	0.2	0.2	0.3
28	0.4	0.4	0.4	0.4	0.3	0.1	0.5	0.5	0.5	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.3	0.3	0.4	0.4
29	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
30	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
31	0	0	0	0	0	0.3	0.4	0.4	0.3	0.3	0.2	0.4	0.2	0.3	0.4	0.2	0.1	0.3	0.3	0.3	0.2	0.1	0	0
32	0.1	0.1	0.1	0.1	0	0	0.1	0.1	0.1	0	0.1	0	0	0	0.1	0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
33	0.5	0.5	0.5	0.3	0.2	0.1	0.7	0.7	0.5	0.1	0.7	0.1	0	0	0	0	0	0.1	0.1	0.1	0.3	0.4	0.5	0.5
34	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0	0	0	0	0	0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
35	0.5	0.5	0.5	0.5	0.4	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.3	0.4	0.4	0.5
36	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0	0	0	0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1

Note: 0.1 : less than 0.1 kg/cm²

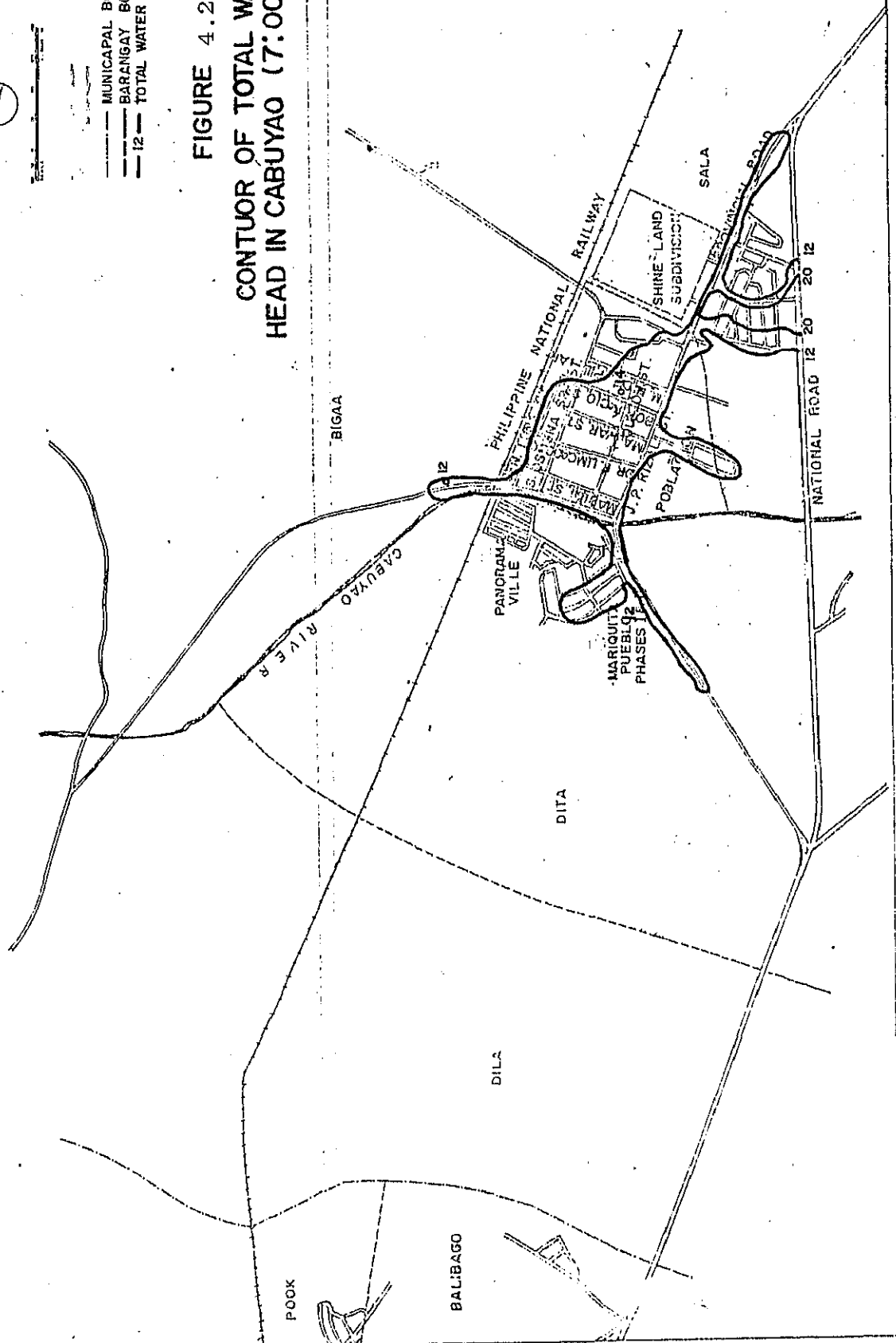
MUNICIPALITY OF CABUYAO

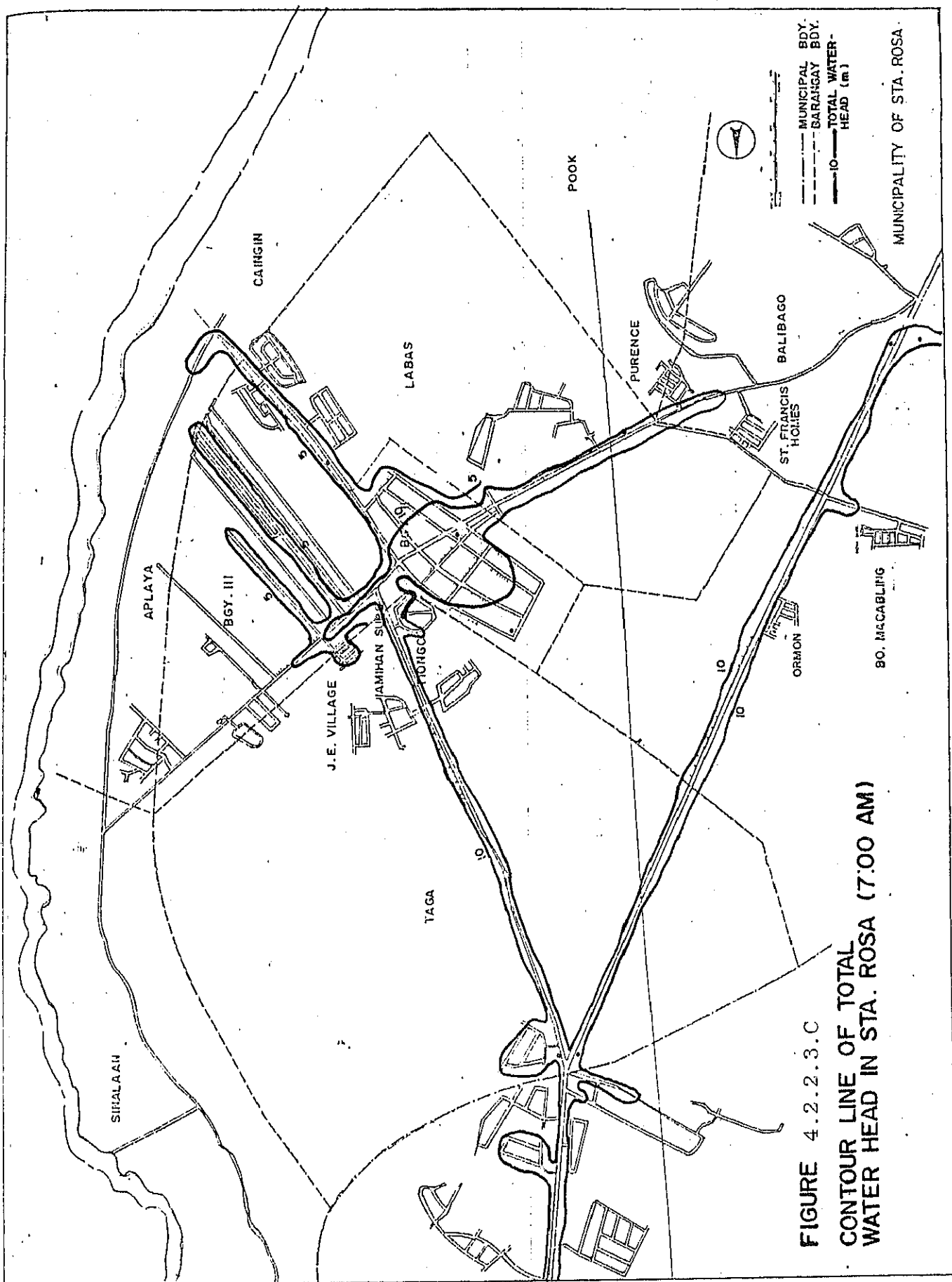


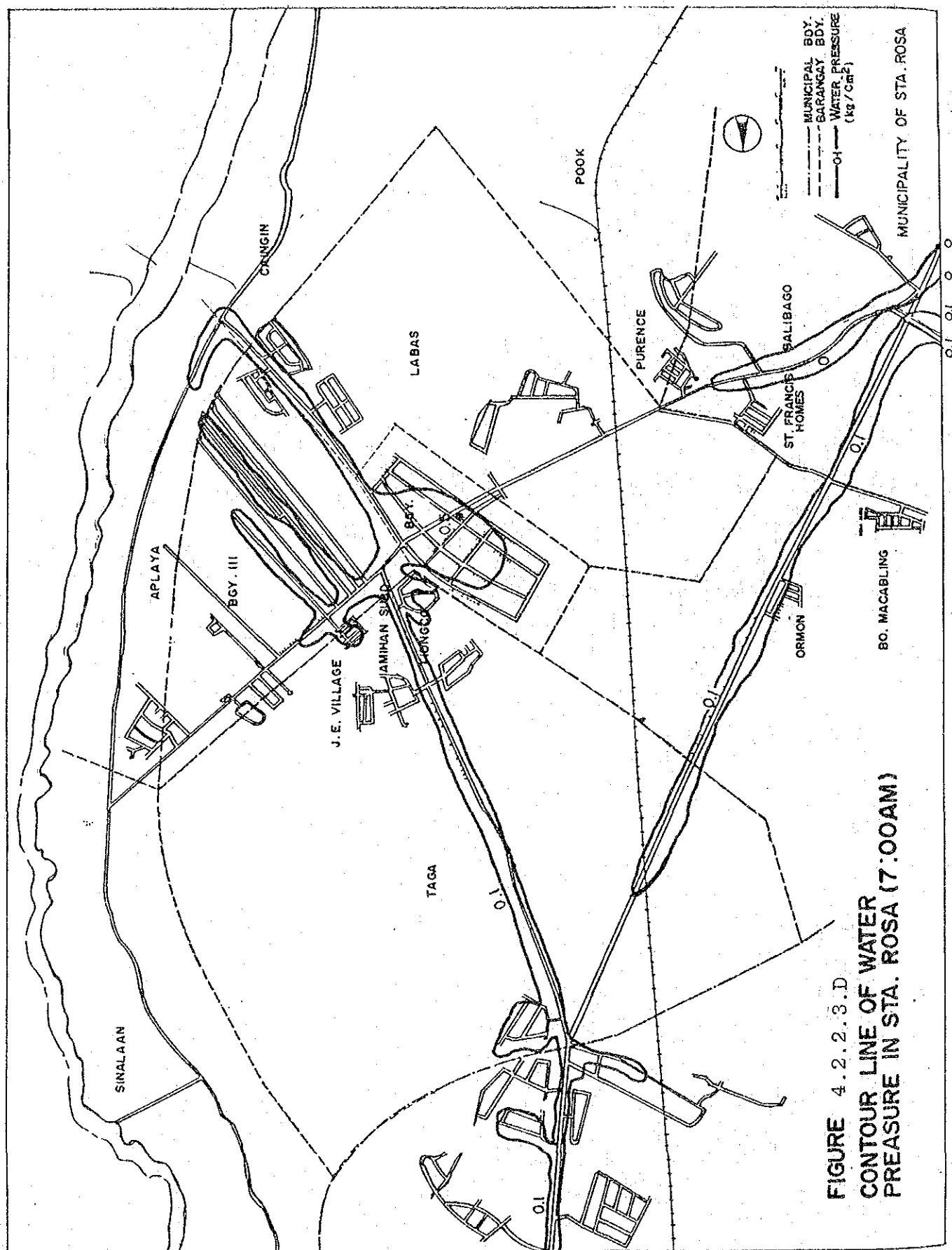
- MUNICIPAL BOUNDARY
- BARANGAY BOUNDARY
- 12 — TOTAL WATER HEAD (m)

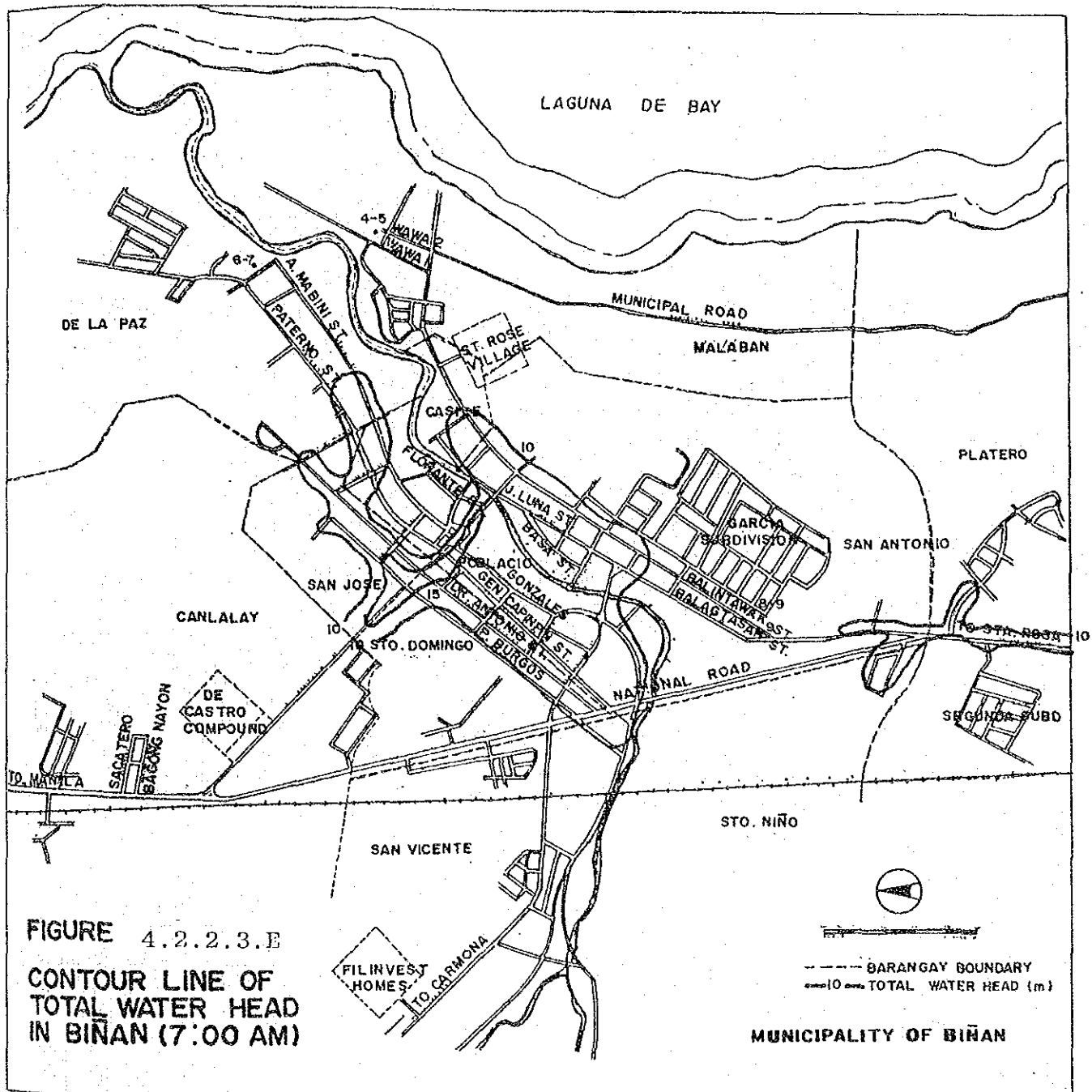
FIGURE 4.2.2.3.A

CONTOUR OF TOTAL WATER
HEAD IN CABUYAO (7:00 AM)









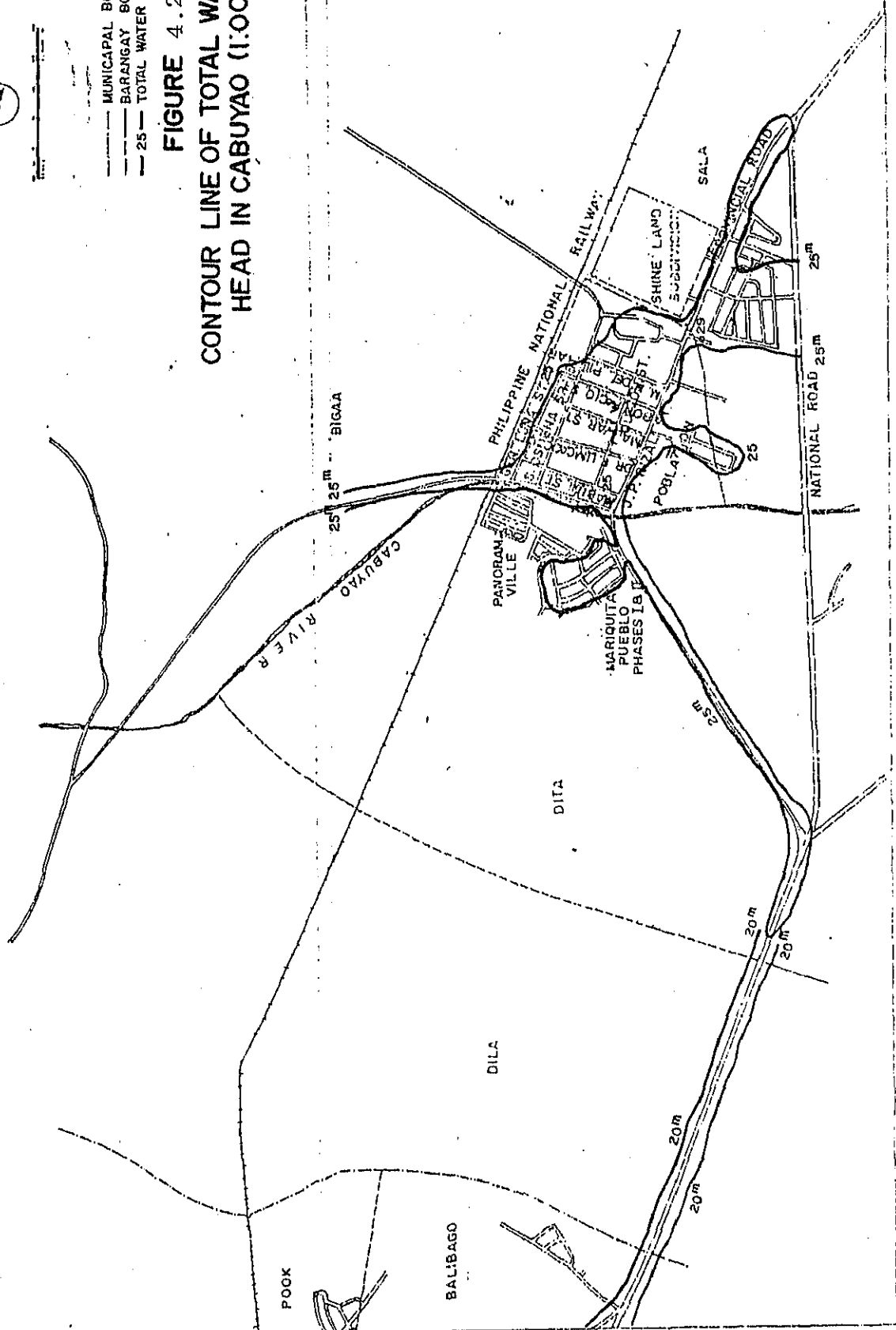
MUNICIPALITY OF CABUYAO



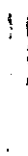
- MUNICIPAL BOUNDARY
- BARANGAY BOUNDARY
- 25 — TOTAL WATER HEAD (m)

FIGURE 4.2.2.3.C

CONTOUR LINE OF TOTAL WATER
HEAD IN CABUYAO (1:00 AM)



MUNICIPALITY OF CABUYAO



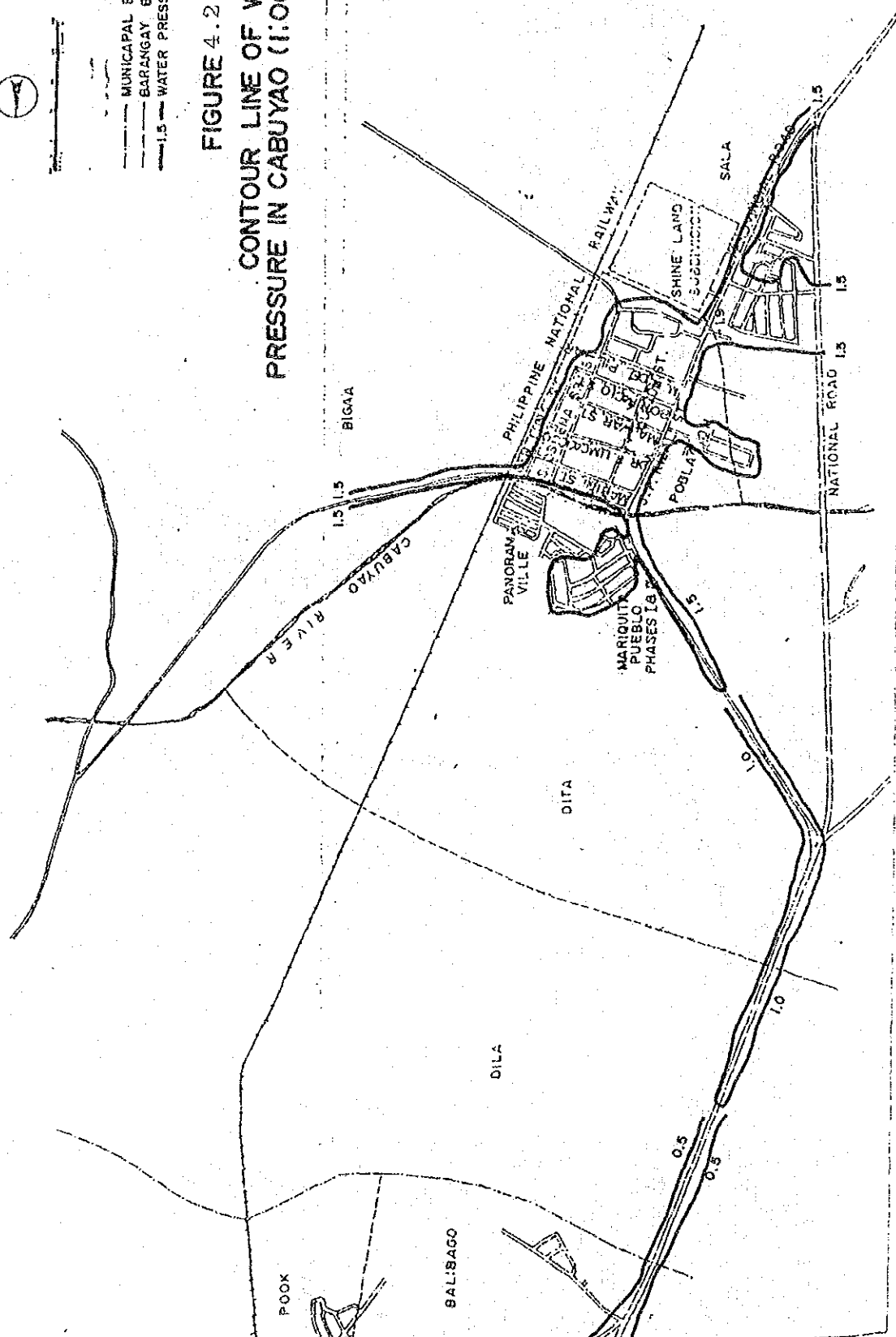
MUNICIPAL BOUNDARY

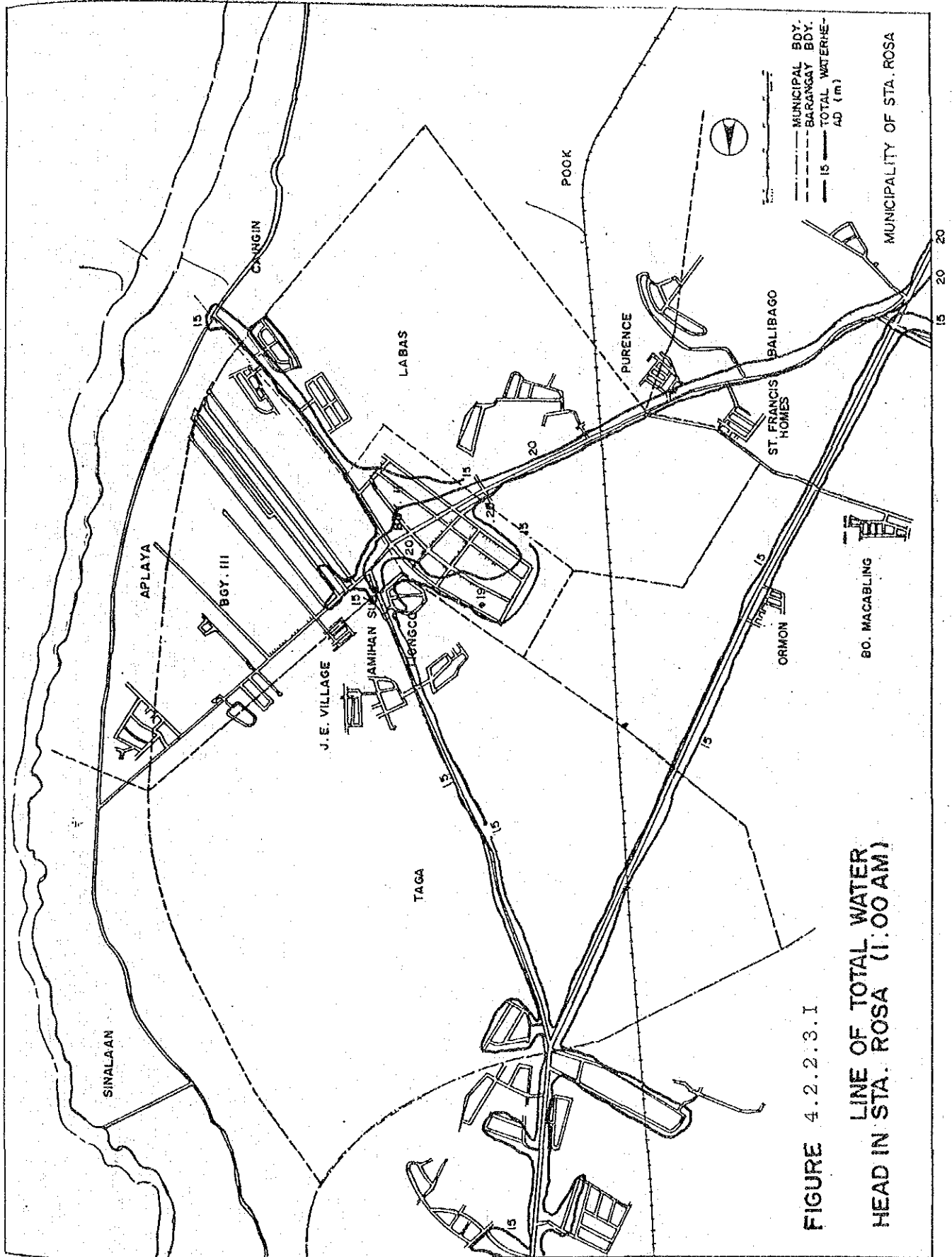
BARANGAY BOUNDARY

1.5 WATER PRESSURE (kg/cm²)

FIGURE 4.2.2.3.H

CONTOUR LINE OF WATER PRESSURE IN CABUYAO (1:00 AM)





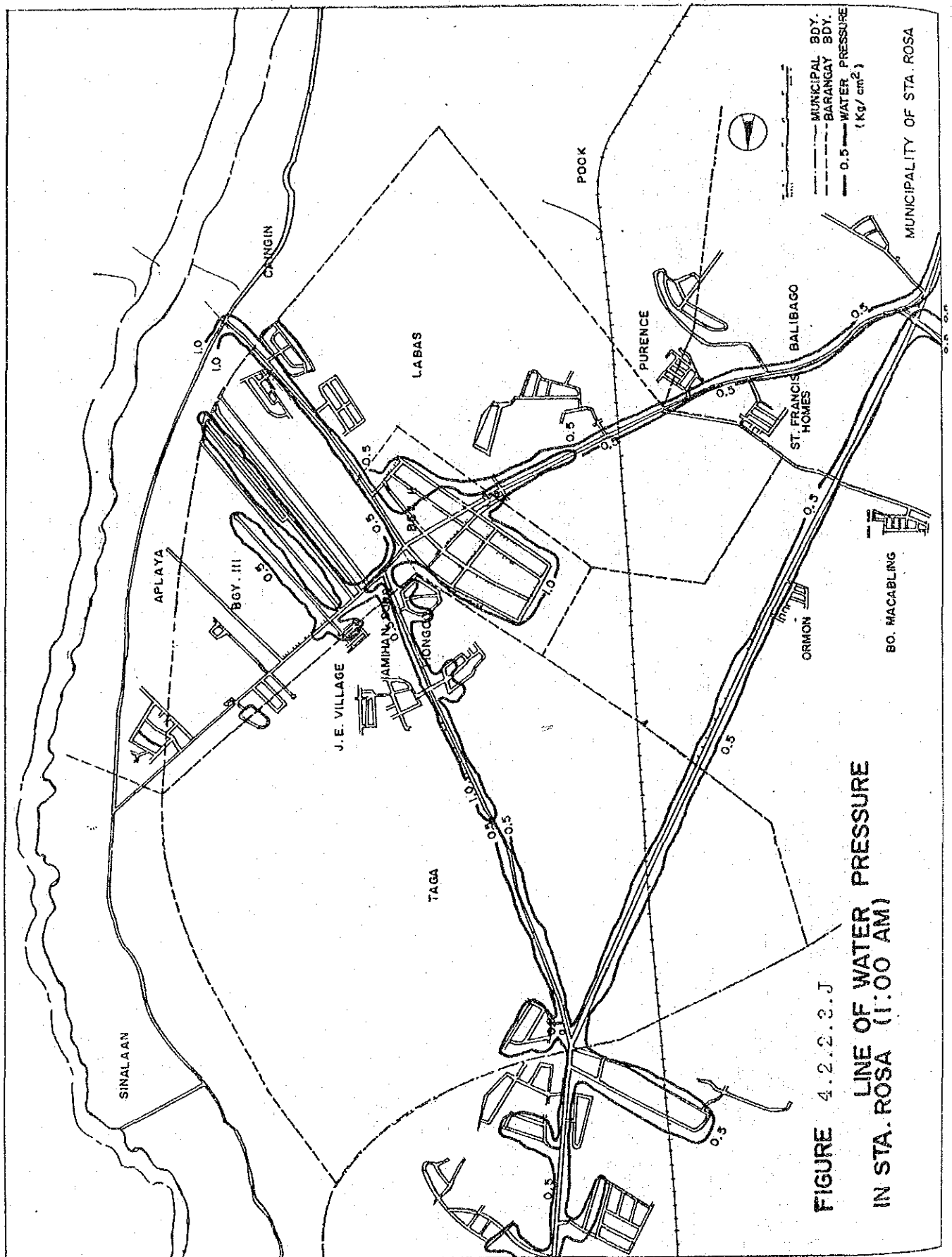
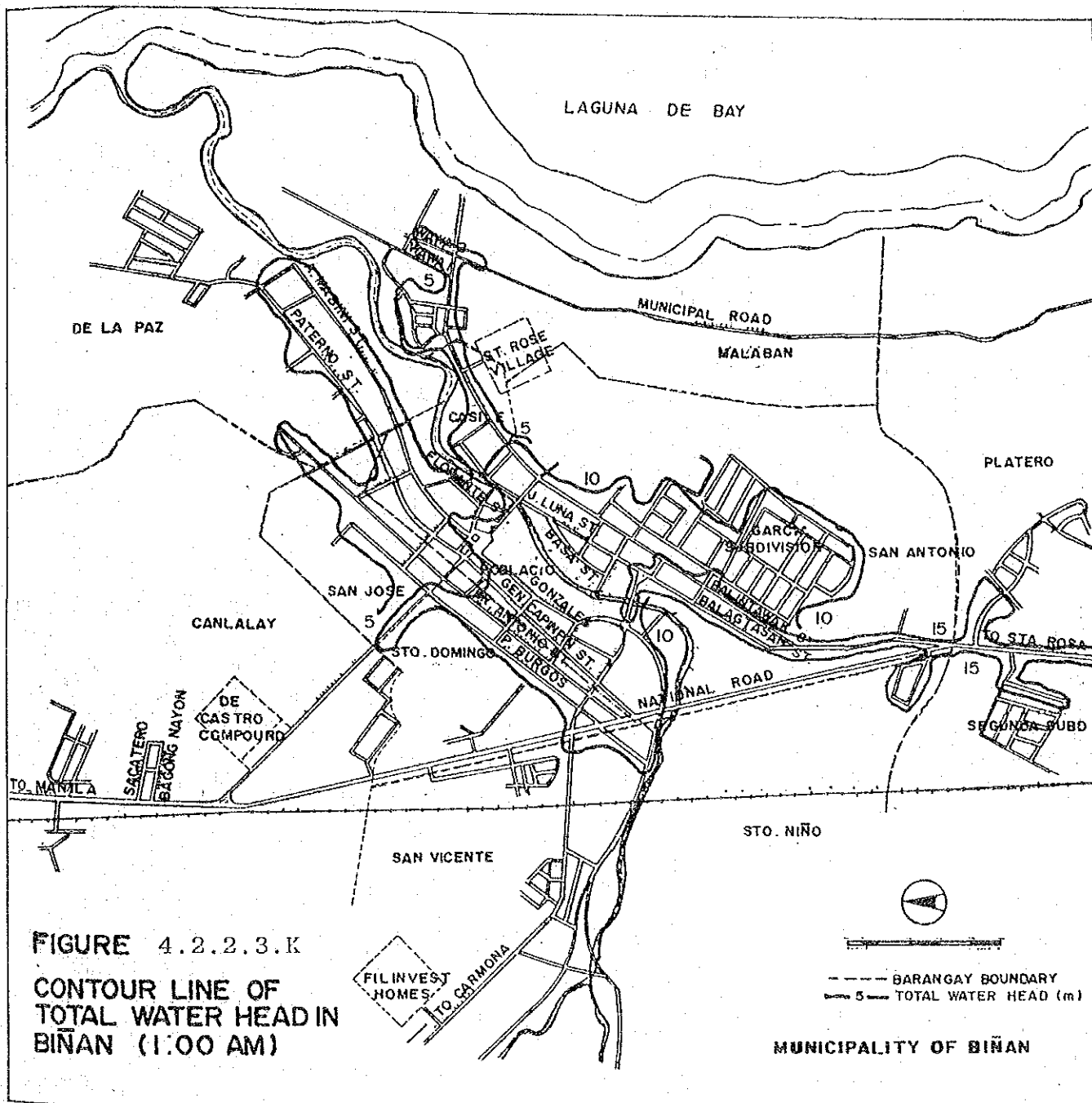
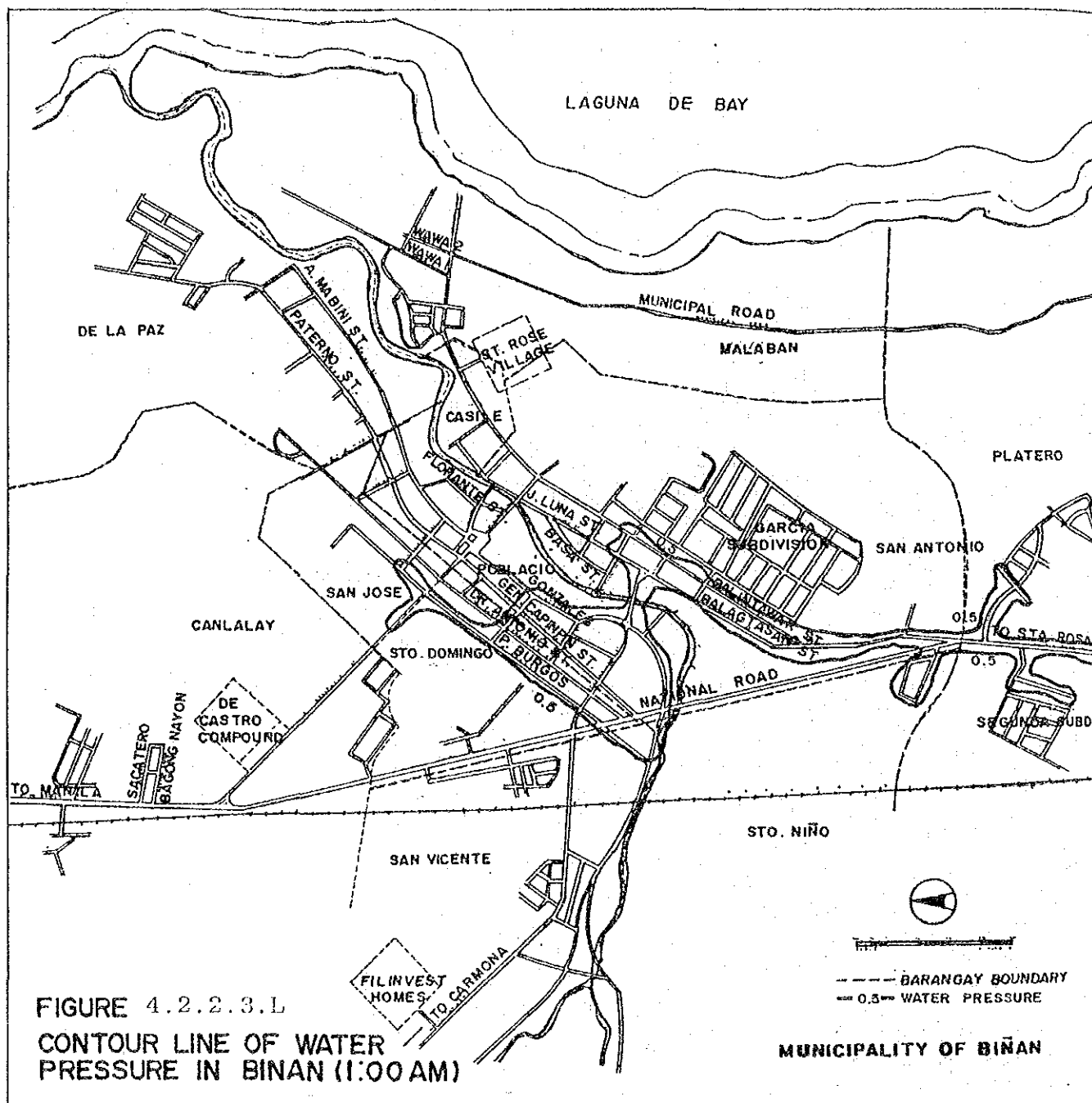


FIGURE 4.2.2.3.J
LINE OF WATER PRESSURE
IN STA. ROSA (1:00 AM)





APPENDIX 4.2.3 COLLECTED CHARGES FOR THE MONTH OF JUNE, 1986 BY
CONSUMER TYPE

Unit: Peso

Municipality	Barangay	Metered					Flat Rate							Total	
		Domestic	Commercial	Institutional	Industrial	Sub-Total	Nonfunctioning Meter		Without Meter						
							Domestic	Commercial	Insti.	Sub-Total	Domestic	Commer.	Insti.		Sub-Total
Cabuyao	1 Barangay I	6295.75	-	-	-	6295.75	750.00	10.00	30.00	790.00	44.00	14.00	30.00	88.00	7173.75
	2 "	5565.00	68.50	36.75	-	5702.25	402.00	-	-	402.00	-	-	-	-	6072.25
	3 "	3900.25	-	-	-	3960.25	287.75	-	-	287.75	-	-	-	-	4248.00
	4 Bigaa	2937.75	-	-	-	2937.75	312.75	-	-	312.75	-	-	-	-	3250.50
	5 Sala	3095.25	10.00	50.00	-	3155.25	137.75	-	10.00	147.75	14.00	-	-	14.00	3317.00
	Sub-Total	21854.00	78.50	86.75	-	22019.25	1890.25	10.00	40.00	1940.25	58.00	14.00	30.00	102.00	24061.50
Sta. Rosa	1 Aplaya	887.75	-	-	-	887.75	127.75	-	-	127.75	-	-	-	-	1015.50
	2 Balibago	10155.00	14.50	-	4219.-	14388.50	897.25	66.25	-	963.50	14.00	-	-	14.00	15366.00
	3 Barangay I	3523.00	10.00	-	-	3533.00	354.50	-	-	354.50	57.20	-	-	57.20	3944.70
	4 "	5874.25	10.00	-	-	5884.25	359.00	-	-	359.00	43.80	-	-	43.80	6287.05
	5 "	1009.00	-	-	-	1009.00	116.50	-	-	116.50	-	15.20	-	15.20	1140.70
	6 Dila	416.75	-	-	1109.00	1525.75	30.00	-	-	30.00	-	-	-	-	1555.75
	7 Dita	5990.50	2203.00	-	260.50	8454.00	260.25	-	13.00	273.25	14.00	-	-	14.00	8741.25
	8 Ibaba	1812.75	-	-	-	1812.75	10.00	-	-	10.00	14.00	-	-	14.00	1836.75
	9 Labas	1900.00	-	-	-	1900.00	139.50	-	-	139.50	-	-	-	-	2039.50
	10 Macablang	347.50	-	-	147.00	494.50	42.00	-	-	42.00	-	-	-	-	536.50
	11 Tagapo	4955.00	-	160.00	-	5115.00	69.25	-	-	69.25	28.60	-	-	28.60	5212.85
Sub-Total	36871.50	2237.50	160.00	5735.50	45004.50	2406.00	66.25	13.00	2485.75	171.60	15.20	-	186.80	47676.55	
Biñan	1 Dela Paz	313.25	-	-	-	313.25	20.00	-	-	20.00	143.60	-	-	143.60	476.85
	2 Malaban	176.50	-	-	-	176.50	34.00	-	-	34.00	340.80	-	15.20	356.00	566.50
	3 Platero	1691.00	-	-	-	1691.00	219.75	-	-	219.75	-	-	-	1910.75	
	4 Poblacion	1472.25	189.50	-	-	1661.75	133.00	-	45.00	178.00	925.40	58.40	14.60	998.40	2838.15
	5 San Antonio	7342.50	34.25	-	-	7376.75	716.25	-	-	716.25	1008.20	-	-	1008.20	9101.20
	6 San Jose	607.50	-	-	-	607.50	22.00	-	-	22.00	403.40	-	-	403.40	1032.90
	7 San Vicente	803.50	-	-	-	803.50	-	-	-	-	57.20	-	-	57.20	860.70
	8 Sto. Domingo	175.25	-	-	-	175.25	-	-	-	-	199.60	-	-	199.60	374.85
Sub-Total	12581.75	223.75	-	-	12805.50	1145.00	-	45.00	1190.00	3078.20	58.40	29.80	3166.70	17161.90	
Total		71307.25	2539.75	246.75	5735.50	79829.25	5441.25	76.25	98.00	5675.50	3307.80	87.60	59.80	3455.20	88899.95

APPENDIX 4.3.1 SPRING DISCHARGE RATE

The discharge rate from the existing spring is 115.8 l/s or 10,008 cu.m/day. The total discharge volume from the four springs in the vicinity of the existing spring box, which may be tapped as additional sources of water, is approximately 40 l/s.

TABLE 4.3.1.1 DISCHARGE RATE FROM SPRINGS

Existing/ Potential	Point	l/s	Measurement Records	
Existing spring box	ø 300	82.5	AM. 298 cu.m/hr. PM. 295	Ave. 297 cu.m/hr.
	ø 200	33.3	AM. 120 PM. 120	Ave. 120
	Sub-Total	115.8	AM. 418 PM. 415	AVE. 417
Potential Source	No. 1	5.6	Required time to fill a drum-can 245 l : 43.8 sec	
	No. 2	21.5	245 l : 11.4 sec	
	No. 3	9.6	245 l : 25.4 sec	
	No. 4	1.6	18 l : 11.0 sec	
	Sub-Total	38.3		
TOTAL		154.1		

The discharge amount from the existing spring box is almost constant through the year (approximately 8 percent increase in rainy season compared to the amount during dry season). The amount from the four potential water sources (springs) is also constant through the year. TABLE 4.3.1.2 shows the figures measured in dry and rainy season.

TABLE 4.3.1.2 SPRING WATER MEASUREMENT RECORDS

Kind	Detail	<u>Daily Discharge Rate</u>		R. M.
		Dry	Rainy	
Existing spring box	ø 300	6,826 cu.m/d.	7,128	Difference between two seasons is less than 8%
	ø 200	2,462	2,880	
Sub-Total			9,288	10,008
Potential sources	4 springs	3,283	3,309	

APPENDIX 4.4.1 WATER CONSUMPTION (METERED CONNECTIONS) BY CONSUMER TYPE

Unit: cu.m/month or cu.m/day

Municipality	Barangay	Domestic		Commercial		Institutional		Industrial		Total	
		Monthly	Daily	Monthly	Daily	Monthly	Daily	Monthly	Daily	Monthly	Daily
Cabuyao	1 Barangay I	7,573	252.4	-	-	-	-	-	-	7,573	252.4
	2 Barangay II	6,643	221.4	99	3.3	45	1.5	-	-	6,787	226.2
	3 Barangay III	4,960	165.3	-	-	-	-	-	-	4,960	165.3
	4 Bigaa	3,742	124.7	-	-	-	-	-	-	3,742	124.7
	5 Sala	4,049	135.0	1	0.0	65	2.2	-	-	4,115	137.2
	Sub-Total	26,967	898.8	100	3.3	110	3.7	-	-	27,177	905.8
Sta. Rosa	1 Aplaya	1,155	38.5	-	-	-	-	-	-	1,155	38.5
	2 Balibago	11,760	392.0	19	0.6	-	-	2,851	95.0	14,630	487.6
	3 Barangay I	4,474	149.1	10	0.3	-	-	-	-	4,484	149.4
	4 Barangay II	7,290	243.0	-	-	4	0.1	-	-	7,294	243.1
	5 "	1,212	40.4	-	-	-	-	-	-	1,212	40.4
	6 Dila	536	17.9	-	-	-	-	816	27.2	1,352	45.1
	7 Dita	7,611	253.7	1,507	50.2	-	-	212	7.1	9,330	311.0
	8 Ibaba	2,283	76.1	-	-	-	-	-	-	2,283	76.1
	9 Labas	2,249	75.0	-	-	-	-	-	-	2,249	75.0
	10 Macabling	401	13.4	-	-	-	-	147	4.9	548	18.3
	11 Tagapo	6,437	214.6	-	-	171	5.7	-	-	6,608	220.3
	Sub-Total	45,408	1,513.7	1,536	51.1	175	5.8	4,026	134.2	51,145	1,704.8
Mianan	1 Dela Paz	397	13.2	-	-	-	-	-	-	397	13.2
	2 Malaban	236	7.9	-	-	-	-	-	-	236	7.9
	3 Platero	2,100	70.0	-	-	-	-	-	-	2,100	70.0
	4 Poblacion	1,870	62.3	260	8.7	-	-	-	-	2,130	71.0
	5 San Antonio	9,283	309.4	49	1.6	-	-	-	-	9,332	311.0
	6 San Jose	761	25.4	-	-	-	-	-	-	761	25.4
	7 San Vicente	997	33.2	-	-	-	-	-	-	997	33.2
	8 Sto. Domingo	234	7.8	-	-	-	-	-	-	234	7.8
	Sub-Total	15,878	529.2	309	10.3	-	-	-	-	16,187	539.5
Total		88,253	2,941.7	1,945	64.7	285	9.5	4,026	134.2	94,509	3,150.1

APPENDIX 4.5.1 UNACCOUNTED-FOR WATER/NOT-UTILIZED WATER

This survey comprises two major elements; one for the transmission lines and another for distribution networks in the selected model areas. The two study section of the transmission lines and the four areas are shown in FIGURE 4.5.1.1 and given below.

- Transmission Line : Cabuyao - Sta. Rosa Line ; one from the spring to reservoir area and another from the reservoir area to the entrance of Cabuyao
- Distribution network :
 - a) The core area of Cabuyao.
 - b) The Sta. Rosa area including the core area of the municipality and the area along Cabuyao - Sta. Rosa line in Sta. Rosa.
 - c) The area along Biñan line in Sta. Rosa.
 - d) The Biñan area.

A flow chart for this investigation and analysis is given in FIGURE 4.5.1.2.

(1) Background Information on the Study Sections of the Transmission Pipeline and the Study Areas

There are no residents along the transmission line from the spring to the entrance of Cabuyao. However, there are large houses approximately 300 m from the transmission pipeline upstream of the reservoir as shown in FIGURE 4.5.1.1. The aforementioned four areas are high population density areas. The following are some information on the study areas based on the field survey.

1) The core area of Cabuyao

The area consists of barangays Bigaa, Sala and Poblacion. The area is predominantly a residential area except for the area along J.P. Rizal St. near the public market which is used both for commercial and institutional purposes in barangay Sala.

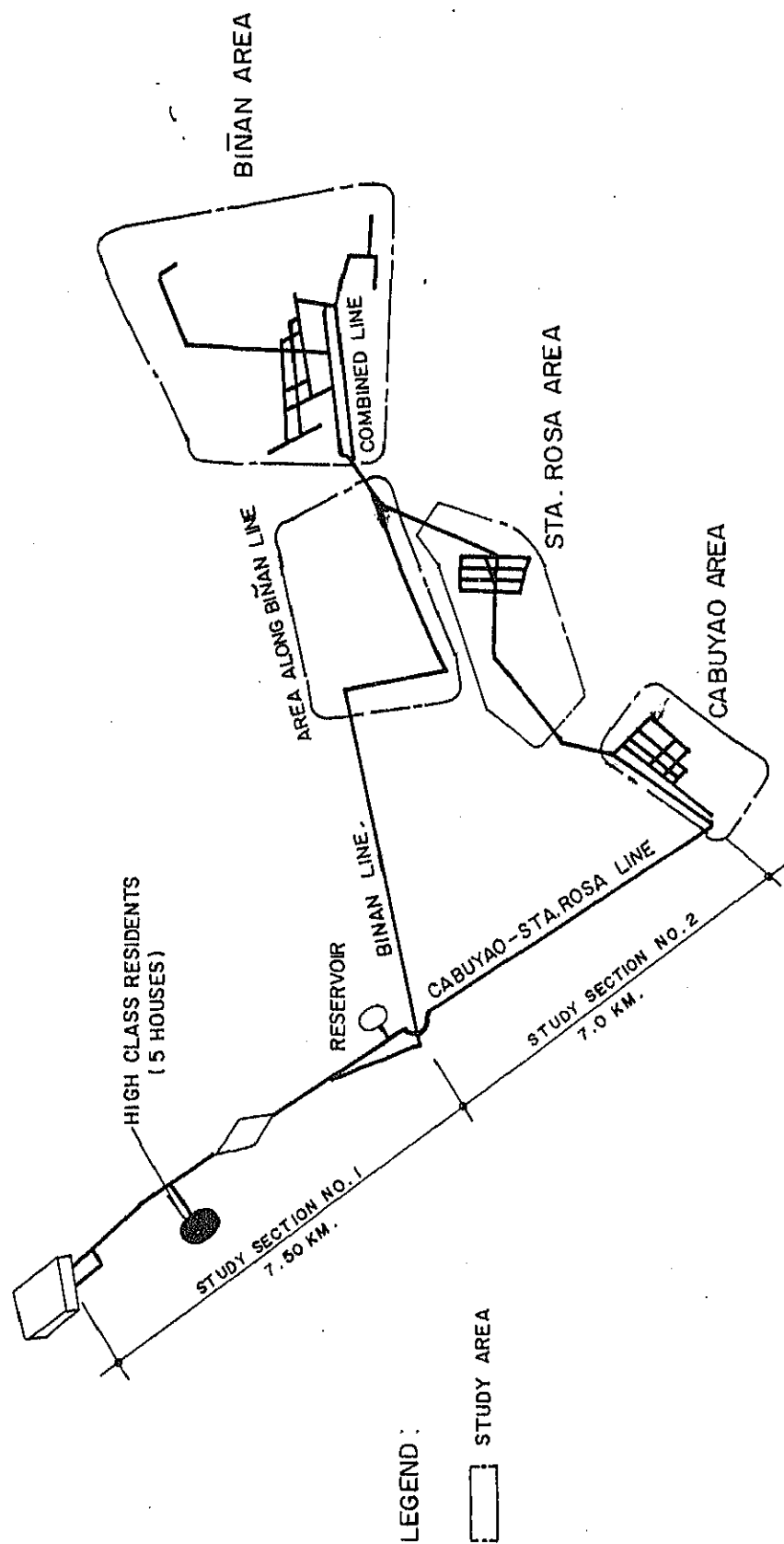
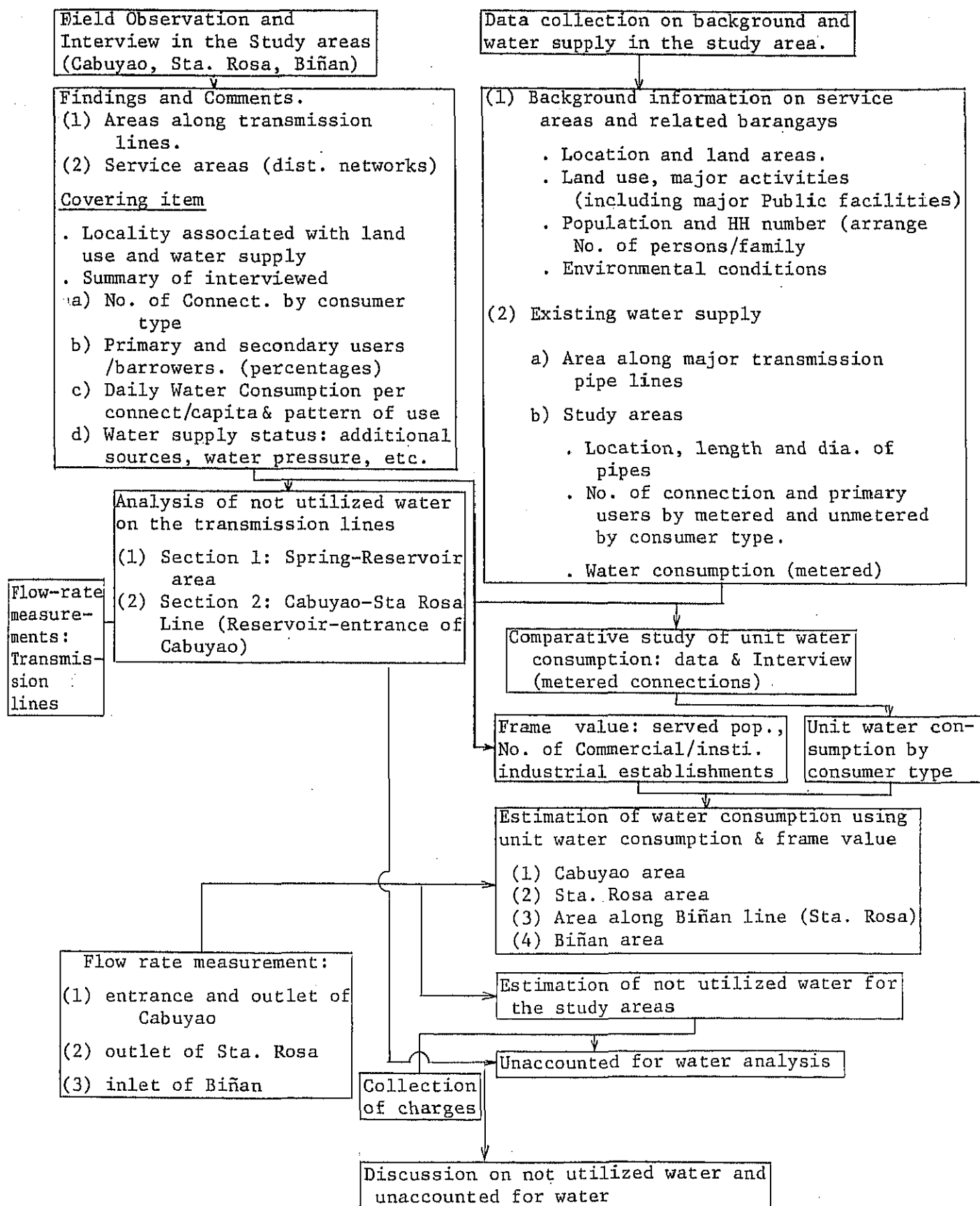


FIGURE 4.5.1.1
STUDY AREA AND PIPELINE
SECTION

FIGURE 4.5.1.2 Flow Chart for estimation of unaccounted for water/
not utilized water



Commercial establishments are mainly eateries, grocery stores and a wet market that caters to the local constituents of the municipality.

- 2) The Sta. Rosa area; the core area of Sta. Rosa and the area along Cabuyao - Sta. Rosa line.

The subject area comprises barangays Aplaya, Balibago, Barangay I, II & III, Pila, Pita, Ibaba, Labas and Tagapo. Although there are many industrial establishments in Sta. Rosa, the area served by the water system is predominantly a residential area except for the municipal building, market and the school areas at the poblacion. Commercial establishments in the poblacion mainly caters to the local constituents.

- 3) The area along Biñan line in Sta. Rosa

The study area comprises barangays Balibago and Macabling in Sta. Rosa, and Platero in Biñan. The area is also residential.

- 4) Biñan area

The area includes barangays Dela Paz, Malaban, Platero, Poblacion, San Antonio, San Jose, San Vicente and Sto. Domingo. The area is predominantly a residential area. Commercial establishments include supermarkets, groceries, theaters and the supermarket where food products from other municipalities are regularly delivered.

Existing Water Supply

There are 726 metered domestic connections in Cabuyao, 719 of which are domestic, 4 commercial and 3 institutional connections.

A total of 49 domestic concessionaires or about 7% of the total number of concessionaires was interviewed.

Based on the 49 concessionaires interviewed, except for those at the fringes at barangays Bigaa and Sala, water supply including water pressure was said to be relatively adequate in the service area.

About 316 persons are served by the 49 interviewed domestic connections; 272 persons are primary users while 44 or about 16% of the primary users are secondary users or borrowers. The 49 connections registered a combined consumption of 61.257 cu.m for one day, or an average per capita consumption for the 316 served population of about 194 liters/day.

As of June 1986, there are 1,188 domestic connections, 3 commercial, 4 institutional and 5 industrial connections or a total of 1,200 service connections in the Sta. Rosa service area. One of these connections is the Nissin Monde Biscuits factory which draws about 2,850 cu.m of water monthly. Based on the 37 interviewed domestic concessionaires in the core area of Sta. Rosa, water supply and pressure are relatively adequate in the distribution system except for the area near the Balibago market which draws water from the Cabuyao - Sta. Rosa line. According to the 66 domestic concessionaires interviewed along the transmission lines, the same situation as in the market area is also experienced during the day, especially in the morning, along the Biñan transmission line in barangay Pulong St. Cruz, and the Cabuyao - Sta. Rosa transmission line along barangays Dita, Dila and Balibago. These areas get only enough water and pressure during the nighttime. Concessionaires often resort to storing water during the night for use the next day.

Along the distribution line, about 276 persons are served by the 37 interviewed domestic concessionaires; 197 persons are primary users while 79 or about 40% of the primary users are secondary users or borrowers. The 37 connections registered a combined consumption of 48.265 cu.m for one day, or an average per capita consumption for the 276 served population of about 175 liters/day.

Along the transmission line, about 572 persons are served by the 66 interviewed domestic concessionaires; 376 persons are primary users while 196 or about 52% of the primary users are secondary users or borrowers. The 37 connections registered a combined consumption of 80.100 cu.m for one day, or an average per capita consumption for the 572 served population of about 140 liters/day.

As of June 1986, there are 483 domestic and 7 commercial connections, or a total of 490 service connections in the Biñan service area.

According to the interviewed 37 domestic connections, the Biñan area has a scheduled supply of water; corresponding to the operation of the pump at the public market and at the subdivision in barangay Platero. The only area wherein water supply is available anytime is at barangays San Antonio and Platero which draw water from the Cabuyao-Sta. Rosa line. Water supply and pressure are so inadequate in the area that affluent concessionaires often install individual booster pumps. Especially in the poblacion area, interviewed concessionaires contend that they could not draw water anytime without a booster pump.

About 298 persons are served by the 35 domestic connections: 197 are primary users while 101 or 51% of the primary users are secondary users. The 35 domestic connections registered a combined consumption of 55.248 or an average per capita consumption of about 185 liters/day.

TABLE 4.5.1.1 summarizes the results of the interviews on the served population and daily consumption.

TABLE 4.5.1.1 SUMMARY OF INTERVIEWED DOMESTIC CONCESSIONAIRES

Area	Primary User		Secondary User		Secondary Primary	Total User		Daily Consumpt.	Per Capita Consumption
	H.H.	Pop.	H.H.	Pop.	%	H.H.	Pop.		liters/day
Cabuyao	49	272	7	44	16%	56	316	61.257	194
Sta. Rosa	37	197	20	79	40%	57	276	48.265	175
Biñan	35	197	23	101	51%	58	298	55.248	185
Area along Biñan line (Sta. Rosa)	66	376	49	196	52%	115	572	80.100	140

(2) Unit Water Consumption and Total Water Consumption

Unit water consumption by consumer type was estimated using reported water consumption (metered) for the month of June, 1986 including the secondary users/borrowers as based on the interview results. A summary of average unit water consumption by municipality both from recorded data for the month of June and results of meter reading during the interview is given in TABLE 4.5.1.2.

TABLE 4.5.1.2 UNIT WATER CONSUMPTION BY WATER CONSUMER TYPE (METERED)

Municipality	Domestic		Inter-view	Commer-cial	Institu-tional	Indus-trial
	Data in June l/cap.d	cu.m/con.d				
Cabuyao	173.6	1.250	194	1.100	1.223	-
Sta. Rosa	151.7	1.274	*175 140	17.033	1.450	26.840
Biñan	121.7	1.096	185	1.471	-	-
Total	150.8	1.231	174	4.621	1.357	26.840

Note: * Average in the core area. Average in the area along Biñan line.

With regard to the unit domestic consumption, the figures from meter reading are larger than those from the data for the month of June. The figures from the meter readings seem to correspond to the daily maximum as was gathered from the concessionaires during the interview. The average consumption figure by municipality corresponds to the water supply status in relation to the location of the spring water source with service

area. However the figures are comparatively larger than those of similar municipalities. It was concluded through interview with concessionaires and officials concerned that the figures may include the water caused by wastage and leakage in the section of service connection between the water meter and faucets.

The number of metered connections for commercial, institutional and industrial use is limited in the subject area. Therefore estimated average unit water consumption for these uses is not a reliable bases for future demand projection. The average figure by consumer type may however be used to estimate present water consumption by unmetered connections.

The total water consumption by study area as estimated is shown in TABLE 4.5.1.3. The following are the bases of calculation:

Metered connections:	Reported figures for the month of June by consumer type
Unmetered connections:	Estimated figures using municipal average per connection consumption and number of connections (Data on June 1986)

TABLE 4.5.1.4 summarizes the total water consumption in round figures by study area.

TABLE 4.5.1.3

NO. OF CONNECTIONS AND CONSUMPTION BY MAIN PIPE LINE

MAIN PIPELINE	MUNICIPALITY/ BARANGAY	DOMESTIC					COMMERCIAL				
		FUNCTIONAL METERED		NON-FUNCTIONAL METERED		TOTAL	FUNCTIONAL METERED		NON-FUNCTIONAL METERED		TOTAL
		CONNECTIONS	M3/d	CONNECTIONS	M3/d		CONNECTIONS	M3/d	CONNECTIONS	M3/d	
CABUYAO STA. ROSA LINE	CABUYAO										
	1. BARANGAY I	203	252	31	39	234	291	-	2	2	2
	2. BARANGAY II	178	221	28	35	206	256	3	3	-	3
	3. BARANGAY III	130	165	14	18	144	183	-	-	-	-
	4. BIGAA	99	125	8	10	107	135	-	-	-	-
	5. SALA	109	135	8	10	117	145	1	0	-	1
	SUB-TOTAL	719	898	89	112	808	1010	4	3	2	6
	STA. ROSA										
	1. APLAYA	32	39	5	6	37	45	-	-	-	-
	2. BALIBAGO	59	83	3	4	62	87	-	-	-	-
	3. BARANGAY I	126	149	19	24	145	173	1	1	-	1
	4. BARANGAY II	191	243	16	20	207	263	-	-	-	-
	5. BARANGAY III	32	40	4	5	36	45	-	1	1	1
	6. DILA	19	18	3	4	22	22	-	-	-	-
	7. DITA	202	254	12	15	214	269	1	50	-	-
	8. ISABA	64	76	2	3	66	79	-	-	-	-
	9. LABAS	52	75	5	6	57	81	-	-	-	-
	10. TIAGAPO	179	215	6	8	185	223	-	-	-	-
BIRAN LINE	SUB-TOTAL	956	1192	75	95	1031	1287	2	51	1	2
	TOTAL	1675	2092	164	207	1839	2297	6	54	3	7
	STA. ROSA										
	1. BALIBAGO	219	309	22	28	241	327	1	1	1	2
	2. MACABELING	13	13	1	1	14	14	-	-	-	-
	SUB-TOTAL	232	322	23	29	255	351	1	1	1	2
	BIRAN										
	1. PLATERO	28	32	2	2	30	34	-	-	-	-
	TOTAL	260	354	25	31	285	385	1	1	1	2

TABLE 4.5.1.3 (cont'd)

MAIN PIPELINE	INSTITUTIONAL				INDUSTRIAL			
	MUNICIPALITY/ BARANGAY	FUNCTIONAL METERED CONNECTIONS M3/d	NON-FUNCTIONAL METERED w/out METER CONNECTION M3/d	TOTAL CONNECTIONS M3/d	TOTAL FUNCTIONAL METERED CONNECTIONS M3/d	NON-FUNCTIONAL METERED w/out METER CONNECTIONS M3/d	TOTAL	
<u>CABUYAO</u>								
	1. BARANGAY I	-	3	4	3	4	-	
	2. BARANGAY II	2	-	-	2	-	-	
	3. BARANGAY III	-	-	-	-	-	-	
	4. BIGAA	-	-	-	-	-	-	
	5. SALA	1	1	41*	2	4	-	
	SUB-TOTAL	3	4	45	7	4	-	
<u>CABUYAO STA. ROSA LINE</u>								
	1. APLAYA	-	-	-	-	-	-	
	2. BALIBAGO	-	-	-	-	-	-	
	3. BARANGAY I	-	-	-	-	-	-	
	4. BARANGAY II	1	0	-	1	0	-	
	5. BARANGAY III	-	-	-	-	-	-	
	6. DILA	-	-	-	-	2#	2	
	7. DITA	-	1	2	1	7	1	
	8. IBABA	-	-	-	-	-	-	
	9. LABAS	-	-	-	-	-	-	
	10. TAGAPO	3	6	-	3	6	-	
	SUB-TOTAL	7	6	1	2	5	8	
	TOTAL	7	10	5	47	12	57	
<u>BIRAN</u>								
	STA. ROSA	-	-	-	-	-	-	
	1. BALIBAGO	-	-	-	1	95	1	
	2. MACABLING	-	-	-	1	5	1	
	SUB-TOTAL	-	-	-	2	100	2	
<u>BIRAN</u>								
	1. PLATERO	-	-	-	-	-	-	
	TOTAL	-	-	-	2	100	2	

*Municipal building consumption = 40 m³/day

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TABLE 4.5.1.3 (cont'd)

DOMESTIC

MAIN PIPELINE	BARANGAY	COMMERCIAL				DOMESTIC			
		FUNCTIONAL METERED	NON-FUNCTIONAL METERED w/out METER	TOTAL	FUNCTIONAL METERED	NON-FUNCTIONAL METERED w/out METER	TOTAL	FUNCTIONAL METERED	NON-FUNCTIONAL METERED w/out METER
		CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4
BIRAN									
COMBINED	1. DE LA PAZ	14	13	27	12	13	25	26	-
CABUYAO	2. MALABAN	8	8	16	27	30	57	38	-
AND	3. PLATERO	29	38	67	5	6	11	44	-
BIRAN	4. POBLACION	64	62	126	72	79	151	141	16
LINE	5. SAN ANTONIO	263	309	572	120	132	252	441	2
	6. SAN JOSE	31	25	56	30	33	63	58	-
	7. SAN VICENTE	38	33	71	4	5	9	38	-
	8. STO. DOMINGO	8	8	16	14	15	29	23	-
SUB-TOTAL		455	496	951	284	313	597	899	18

INDUSTRIAL

WATER SOURCE	BARANGAY	COMMERCIAL				INDUSTRIAL			
		FUNCTIONAL METERED	NON-FUNCTIONAL METERED w/out METER	TOTAL	FUNCTIONAL METERED	NON-FUNCTIONAL METERED w/out METER	TOTAL	FUNCTIONAL METERED	NON-FUNCTIONAL METERED w/out METER
		CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4	CONNECTIONS M 3/4
BIRAN									
COMBINED	1. DE LA PAZ	-	-	-	-	-	-	-	-
CABUYAO	2. MALABAN	-	-	-	-	-	-	-	-
AND	3. PLATERO	-	-	-	-	-	-	-	-
BIRAN	4. POBLACION	-	-	-	-	-	-	-	-
LINE	5. SAN ANTONIO	-	-	-	-	-	-	-	-
	6. SAN JOSE	-	-	-	-	-	-	-	-
	7. SAN VICENTE	-	-	-	-	-	-	-	-
	8. STO. DOMINGO	-	-	-	-	-	-	-	-
SUB-TOTAL		-	-	-	-	-	-	-	-

TOTAL

WATER SOURCE	BARANGAY	FUNCTIONAL METERED		NON-FUNCTIONAL METERED w/out METER		TOTAL		REMARKS
		CONNECTIONS M 3/4		CONNECTIONS M 3/4		CONNECTIONS M3/4		
BIRAN								
COMBINED CABUYAO AND BIRAN LINE	1. DE LA PAZ	14	13	12	13	26	26	
	2. HALABAN	8	8	28	32	36	40	
	3. PLATERO	29	38	5	6	34	44	
	4. FORLACION	70	141	80	90	150	231	
	5. SAN ANTONIO	264	311	120	132	384	443	
	6. SAN JOSE	31	25	30	33	61	58	
	7. SAN VICENTE	38	33	4	5	42	38	
	8. STO. DOMINGO	8	8	14	15	22	23	
SUB-TOTAL		462	577	293	326	755	903	

TABLE 4.5.1.4 SUMMARY OF WATER CONSUMPTION BY STUDY AREA

Unit: cu.m/day		
Study Area	Water Consumption	Percentage
a. Core area of Cabuyao	1,050	27.6
b. Core area and area along Cabuyao-Sta. Rosa line in Sta. Rosa	1,350	48.7
c. Area along Biñan line in Sta. Rosa	500	
d. Biñan area	900	23.7
T o t a l	3,800	100

(3) Flow Rate Measurement Results

The measurement results are summarized (rounded off) by section of pipeline in TABLE 4.5.1.5. Continuous measurement for one day was conducted at 6 points and a short time measurement at the spring and another point. Detailed records are given in TABLE 4.5.1.6.

TABLE 4.5.1.5 MEASUREMENT RESULTS

Section of Pipe Line	Measuring Point	Daily flow	R. M.
Transmission line:	Total of 2 outlets from existing spring	10,000	Details are given in "Discharge rate of Spring"
Spring - Reservoir area	Approx. 3 km from spring	11,000	reference point
Cabuyao-Sta. Rosa Line:	Manhole in the premise of Reservoir	5,600	24 hours
Reservoir - exit of Sta. Rosa	Entrance of core area of Cabuyao	5,600	"
	Outlet of Cabuyao	3,400	"
	Outlet of Sta. Rosa	50	" 53 cu.m/day = flow into Biñan line
Biñan Line:	Reservoir premise	4,050	24 hours
Reservoir area - Biñan area	Entrance of Biñan	1,550	"

TABLE 4.5.1.6 FLOW RATE MESUREMENT (24 HOURS)

CABUYAO		BIÑAN	RESERVOIR 3		CABUYAO	C. S. ROSA	SUPPLIED	OUTLET OF S. R.		SUPPLIED	ENTRANCE OF BIÑAN
TIME STA. ROSA	LINE	LINE	WATER	STORAGE	STA. ROSA	LINE	AMOUNT TO	Inflow / Outflow	AMOUNT TO		
LINE	1	2	LEVEL	VOLUME	LINE	4	CABUYAO	6	STA. ROSA	7	
0 - 1	135	167	1.42	994	191	133	58	0	5	128	88
1 - 2	100	166	1.57	1,112	197	137	60	0	6	131	88
2 - 3	122	166	1.76	1,266	201	141	60	0	6	135	88
3 - 4	100	166	1.92	1,398	206	143	63	0	5	138	88
4 - 5	244	167	2.10	1,552	211	140	71	0	2	138	85
5 - 6	234	169	2.06	1,500	219	129	90	8	0	137	78
6 - 7	272	173	1.94	1,415	223	123	110	31	0	144	56
7 - 8	282	174	1.63	1,323	219	108	111	49	0	157	37
8 - 9	293	174	1.77	1,274	219	111	108	53	0	166	32
9 - 10	299	170	1.61	1,144	222	111	111	49	0	160	41
10 - 11	302	166	1.49	1,049	218	108	110	43	0	151	45
11 - 12	293	169	1.40	978	236	128	108	48	0	176	38
12 - 13	273	173	1.31	909	267	155	112	42	0	197	46
13 - 14	262	165	1.10	750	274	168	106	28	0	196	64
14 - 15	263	(168)	1.08	736	281	175	106	25	0	200	67
15 - 16	273	(168)	1.05	714	289	182	107	20	0	202	71
16 - 17	290	(168)	0.98	662	295	186	109	32	0	218	57
17 - 18	296	(168)	0.90	604	298	183	115	39	0	222	48
18 - 19	316	(168)	0.85	569	293	182	111	39	0	221	47
19 - 20	300	(168)	0.85	569	297	182	115	20	0	202	36
20 - 21	187	161	0.85	569	224	141	83	17	0	158	74
21 - 22	135	169	0.95	641	169	112	57	6	0	118	62
22 - 23	140	168	1.11	758	176	120	56	1	0	121	88
23 - 24	131	167	1.26	870	185	129	56	0	5	124	89
TOTAL	5,264	4,033	-	983	5,610	3,417	2,192	552	29	3,940	1,533

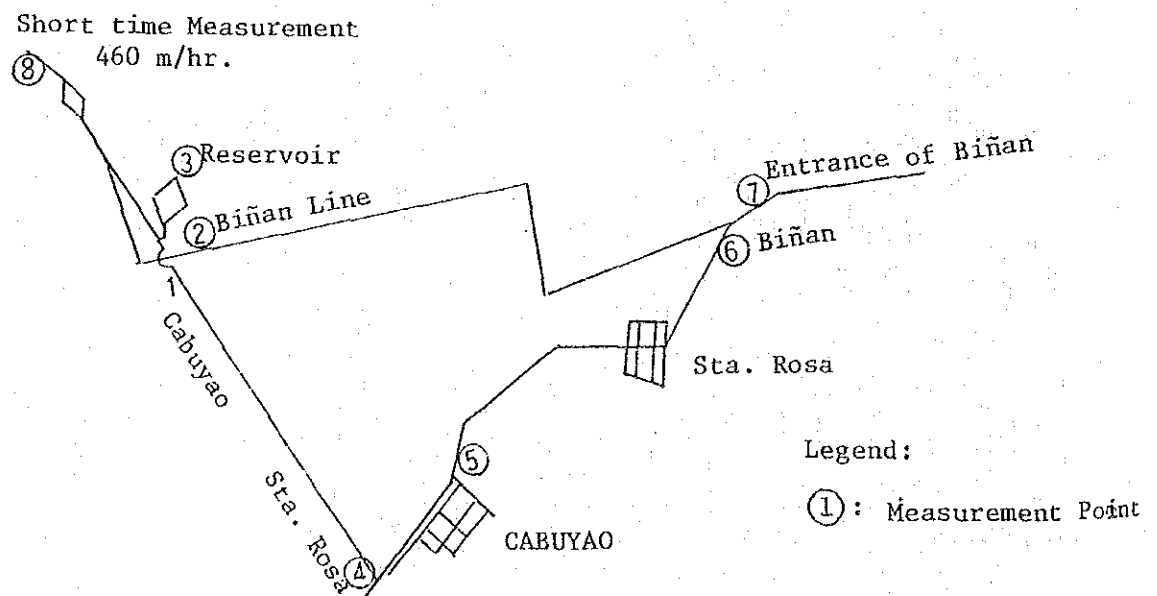


FIGURE 4.5.1.3 LOCATION OF MEASUREMENT POINT

FIGURE 4.5.1.4 shows the flow rates along the main pipeline from the spring to the service areas together with the amount of water calculated to be to major service areas.

Because of no flow/quite low pressure at the junction area of Biñan and Cabuyao-Sta. Rosa lines, approximately 550 cu.m/day is transmitted by the Biñan line to supply part of Sta. Rosa through the Cabuyao-Sta. Rosa line. Approximately 1,000 cu.m/day is discharged from the reservoir into Cabuyao - Sta. Rosa line between 4:00 A.M. and 6:00 P.M.

(4) Discussions and Conclusions on not utilized water/unaccounted-for water

1) Transmission lines

The two sections of the transmission line from the spring to the entrance of Cabuyao area were studied using flow rate measurement results.

a) Transmission line from the spring to the reservoir area

Discharge rate from the spring box : 10,000 cu.m/day

Transmitted amount at the reservoir area :

9,650 cu.m/day

Supply to the high class residents, leakage and other losses : 350 cu.m/day

Although approximately 3,000 cu.m/day were suspected to be leaked in the study section during the Phase I survey, the measurement result revealed that the amount of water transmitted from the intake of the spring box was substantial. The "C" value may be more than the expected figure taking into consideration the existing pipe alignment and the year of its construction. It is concluded that the pipeline installed is in straight alignment with the field and that the water does not contain unfavorable materials associated with "C" value and the joint portions are not damaged.

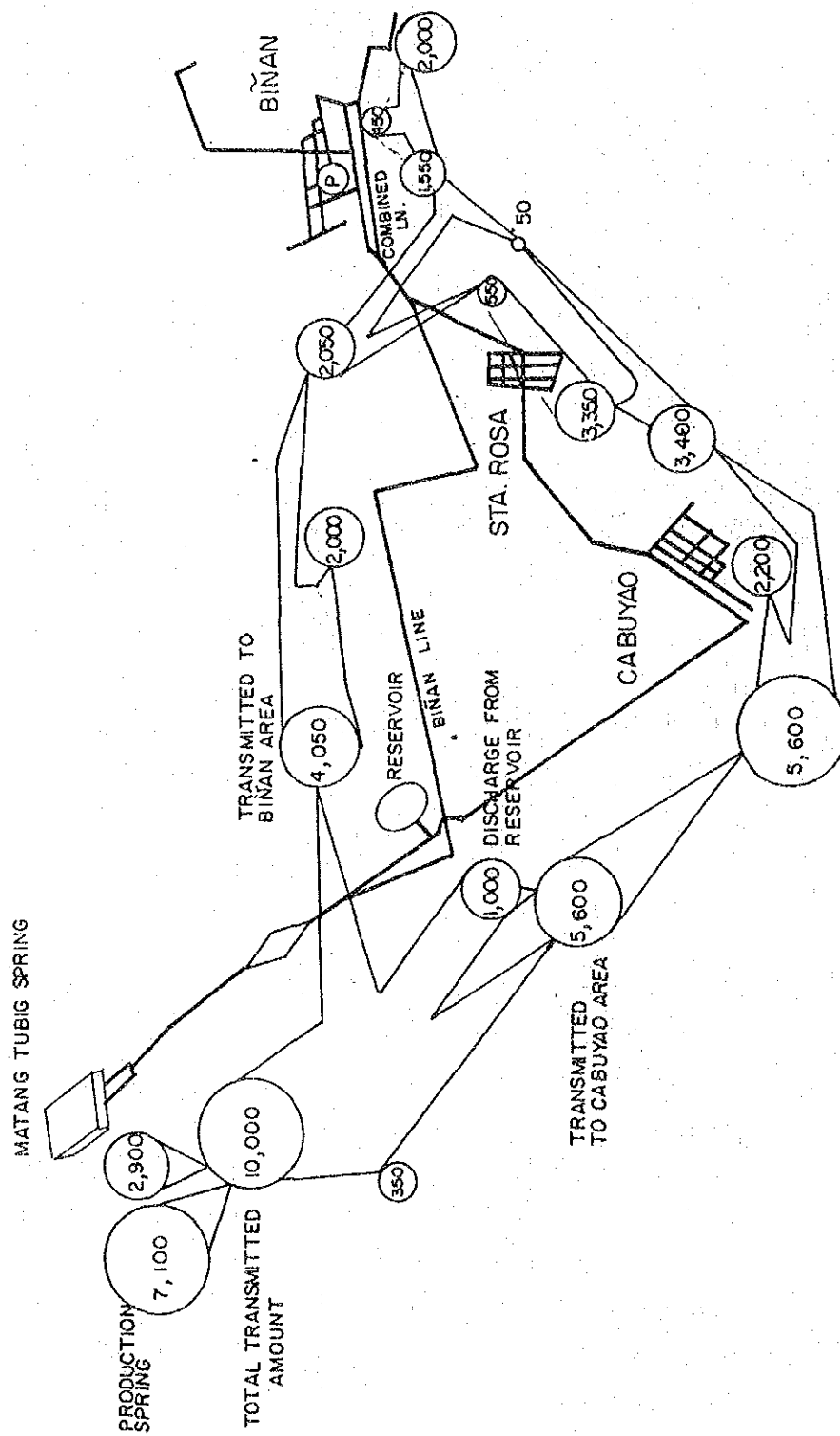


FIGURE 4.5.1.4

FLOW RATE ALONG MAIN PIPE LINES

- b) Transmission line from the reservoir area to the entrance of Cabuyao.

Transmitted rate at the reservoir area : 5,600 cu.m/day
(Cabuyao - Sta. Rosa line)

Transmitted rate at the entrance of
Cabuyao : 5,600 cu.m/day

There is no difference between the above measurement results in rounded off figures. Additional flow rate measurement between the two points were conducted thereby confirming that the figure is almost the same as the one mentioned.

2) Distribution networks

The relationship between distributed water amount and water consumption is summarized in TABLE 4.5.1.7 and FIGURE 4.5.1.5 by study area.

TABLE 4.5.1.7 RELATIONSHIP BETWEEN DISTRIBUTED AND CONSUMED WATER

Study Area	Distributed Water (cu.m/d.)	Water Consumption (cu.m/d.)	Percentage of utilized water	R.M.
a. Core area of Cabuyao	2,200	1,050	48%	
b. Core area and area along Cabuyao - Sta. Rosa line	3,900	1,350	35	
c. Area along Bifian line in Sta. Rosa	2,000	500	25	
d. Bifian area	*2,000	900	45	
T o t a l	10,100 (8,100)	3,800 (3,300)	38 (41)	() means excluding C. area

Note : * Distributed amount includes that from the existing pumping station (reported supply amount)

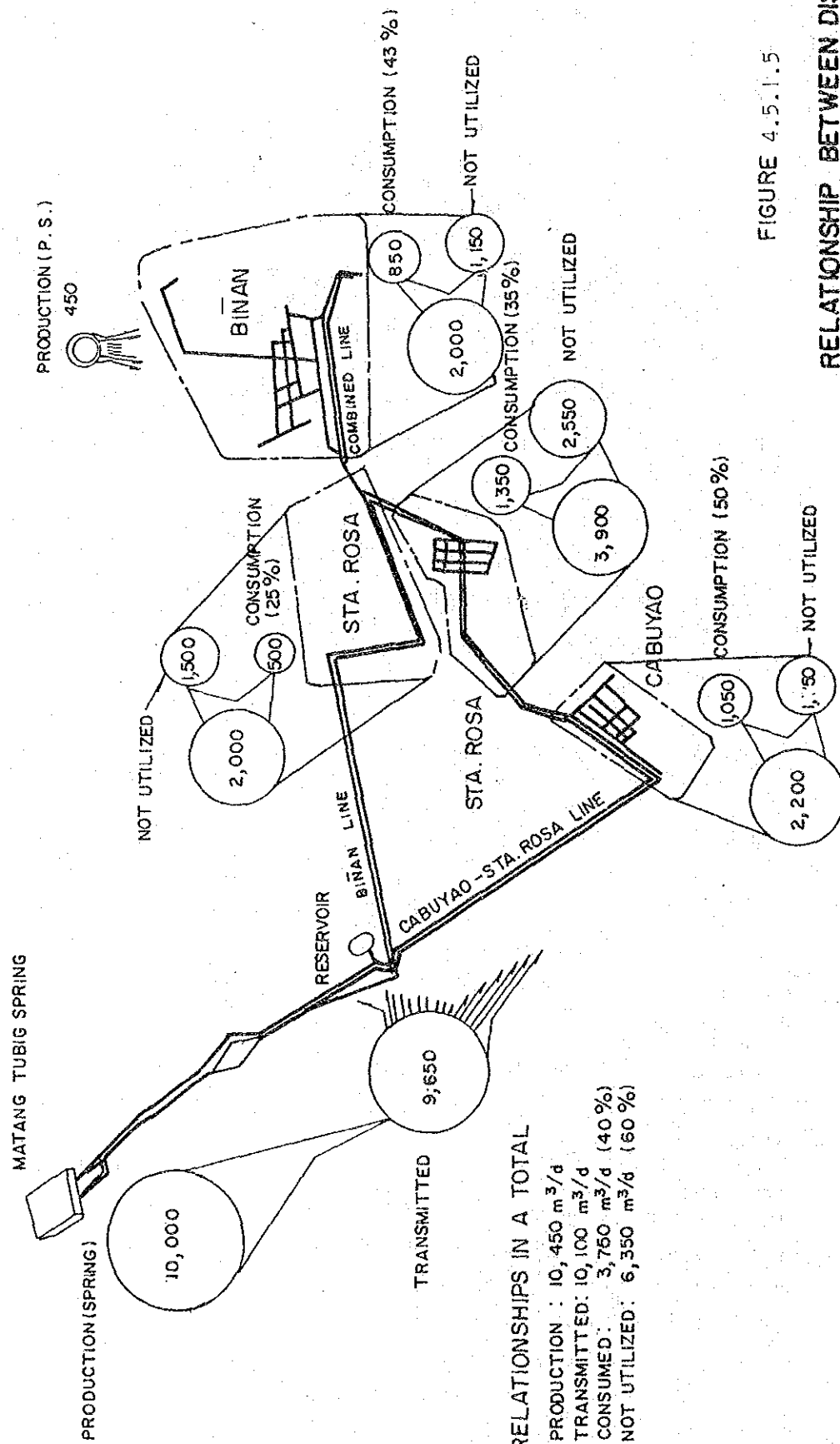


FIGURE 4.5.1.5

RELATIONSHIP BETWEEN DISTRIBUTED
CONSUMED AND NOT UTILIZED WATER
IN THE FOUR (4) AREAS

The percentage of utilized water for the four service areas shows low levels with a maximum of less than 50 percent. The water consumption is estimated using the monthly average figure (June, 1986), however, the result of the meter reading for the domestic consumption as revealed during interview was higher than the average (10% to 50%). Assuming that about 20% is added to the average water consumption, the average percentage of utilized water in the system excluding the area along Biñan line is calculated at approximately 50 percent. Accordingly almost half of distributed water is counted as unutilized water.

As to the causes of not utilized water, further detailed investigations will be required with a special emphasis on service connections with leakage, and non-metered and illegal connections as the culprits.

Unaccounted-for water

An accounted-for water was estimated by municipality based on the following:

- a) Metered connections : water consumption reported for the month of June
- b) Non-metered connections : 18 cu.m/connection-month for the concessionaires with one faucet. (P14/connect.) and 1 cu.m/additional faucet-month (P0.6/connect.)
- c) Metered but not functioning : Average charges per connection are calculated and water consumption per connection is estimated according to the metered rate.

TABLE 4.5.1.8 shows the accounted-for water by municipality.

TABLE 4.5.1.8 ACCOUNTED-FOR WATER

MUNICIPALITY	M E T E R E D			N O N - M E T E R E D			N O T F U N C T I O N I N G M E T E R E D			T O T A L C O N S U M P T I O N		
	NO. OF CONNECT	CHARGE P	CONSUMPTION m ³ /m	NO. OF CONNECT	CHARGE P	CONSUMPTION m ³ /m	NO. OF CONNECT	CHARGE P	CONSUMPTION m ³ /m	MONTHLY	DAILY	
CABUYAO STA. ROSA BIRAN	726	P 22,019.25	27,177	5	53	P 102.00	90	P 1,940.25	2,880	30,200	1,007	
	1,200	45,004.50	51,145	13	7	186.80	88	2,485.75	3,608	54,994	1,833	
	490	12,805.50	16,187	221	154	3,166.40	74	1,190.00	1,628	21,947	732	
TOTAL	2,416	P 79,829.25	94,509	239	214	P 3,455.20	252	P 5,615.50	8,116	107,141	3,572	

Approximately 3,600 cu.m/day corresponds to the accounted-for-water, while 6,500 cu.m/day or about 65 percent of production amount is unaccounted-for water.

It may be worthwhile to notice that there are a number of additional faucets in the category of non-metered connections. The water consumption in the whole system was estimated at 3,800 cu.m/day without considering the consumption at the additional faucets because there was no data available on the per faucet consumption. Additional faucets are for the primary consumer or for other families (secondary users/borrowers). Depending on who uses the faucets, per faucet consumption could differ.

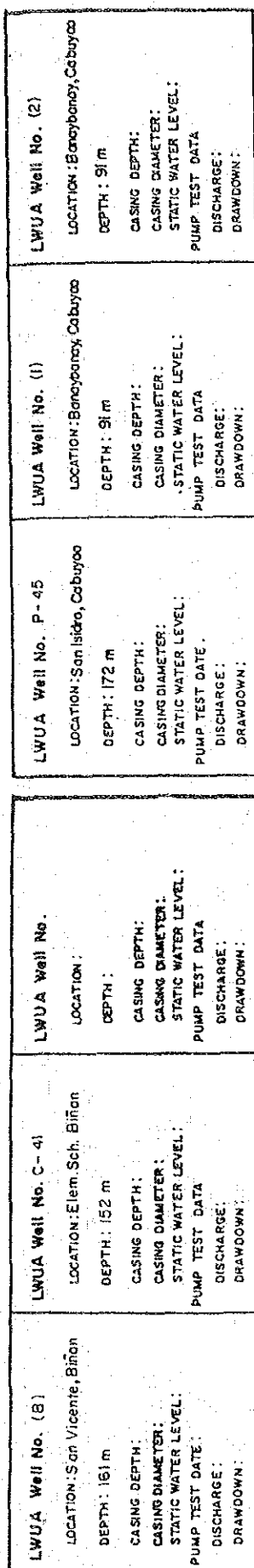
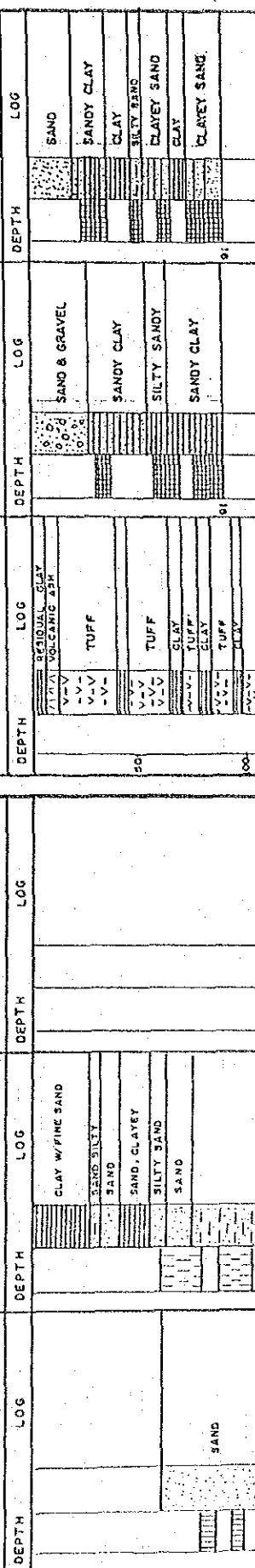
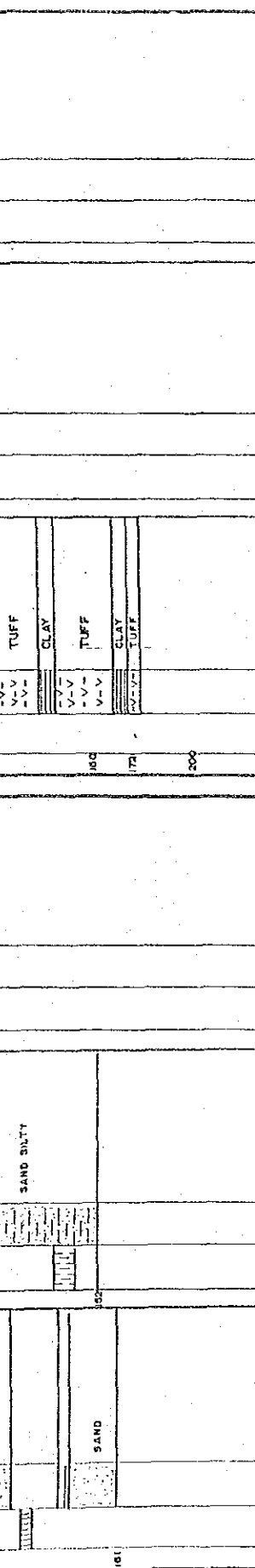
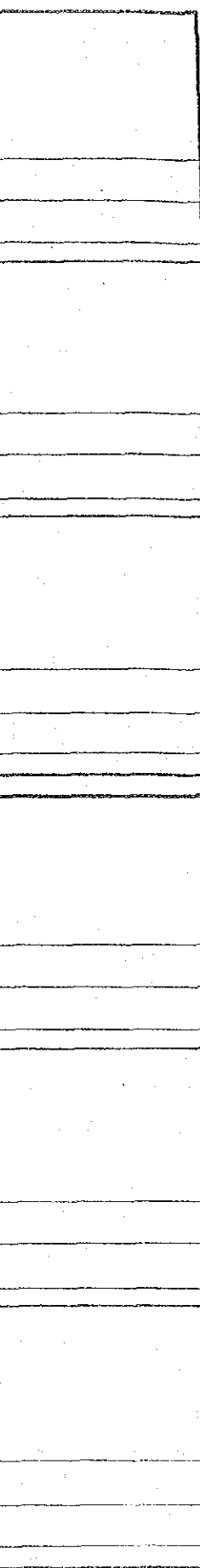
If the average per faucet consumption is assumed to be 1.231 cu.m/day which is the overall average of the domestic metered connection, about 260 cu.m/day would be the additional consumption. The following is the percentage of utilized water in the total system, except the area along Bifan line, using the above assumption.

Distributed water	:	8,100 cu.m/day
Water consumption (1)	:	3,600 (= 3,300 + 300)
Water consumption (2)		
(20% additional)	:	4,300

Percentage of utilized water	:	55%
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APPENDIX 6.7.1 WELL LITHOLOGIC LOGS

<p>LWUA Well No. P-10</p> <p>LOCATION: P. Sta. Cruz, Sta. Rosa</p> <p>DEPTH: 250 m</p> <p>CASING DEPTH:</p> <p>CASING DIAMETER:</p> <p>STATIC WATER LEVEL:</p> <p>PUMP TEST DATE:</p> <p>DISCHARGE:</p> <p>DRAWDOWN:</p>	<p>LWUA Well No.</p> <p>LOCATION:</p> <p>DEPTH:</p> <p>CASING DEPTH:</p> <p>CASING DIAMETER:</p> <p>STATIC WATER LEVEL:</p> <p>PUMP TEST DATE:</p> <p>DISCHARGE:</p> <p>DRAWDOWN:</p>	<p>LWUA Well No. P-13</p> <p>LOCATION: Mangara, Sta. Rosa</p> <p>DEPTH: 200 m</p> <p>CASING DEPTH:</p> <p>CASING DIAMETER:</p> <p>STATIC WATER LEVEL:</p> <p>PUMP TEST DATE:</p> <p>DISCHARGE:</p> <p>DRAWDOWN:</p>	<p>LWUA Well No. P-14</p> <p>LOCATION: P. Sta. Cruz, Sta. Rosa</p> <p>DEPTH: 200 m</p> <p>CASING DEPTH:</p> <p>CASING DIAMETER:</p> <p>STATIC WATER LEVEL:</p> <p>PUMP TEST DATE:</p> <p>DISCHARGE:</p> <p>DRAWDOWN:</p>	<p>DEPTH LOG</p> <p>CLAY</p> <p>CLAY, TUFF</p> <p>CLAY, TUFF</p> <p>CLAY</p> <p>TUFF, SOME CLAY</p> <p>CLAY</p> <p>TUFF, CLAY</p> <p>TUFF, SOME CLAY</p>	<p>DEPTH LOG</p> <p>RESIDUAL CLAY</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>TUFF</p> <p>VOL. ASH, TUFFACEOUS</p> <p>TUFF, PUMICEOUS</p>	<p>DEPTH LOG</p> <p>RESIDUAL CLAY</p> <p>TUFF, WEATHERED, CLAY</p> <p>VOL. ASH, CLAY</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH, SILTY</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH</p> <p>PUMICE</p> <p>TUFF, CLAY</p>	<p>DEPTH LOG</p> <p>RESIDUAL CLAY</p> <p>VOL. ASH</p> <p>TUFF, PUMICEOUS</p> <p>VOL. ASH, TUFFACEOUS</p> <p>PUMICES</p> <p>TUFF, SANDY</p> <p>VOL. ASH, TUFFACEOUS</p> <p>VOL. ASH, TUFFACEOUS</p> <p>TUFF</p> <p>VOL. ASH, TUFFACEOUS</p> <p>TUFF</p> <p>TUFF, WELOED</p> <p>VOL. ASH, CALCAREOUS</p>	<p>LWUA Well No. P-16</p> <p>LOCATION: P. Sta. Cruz, Sta. Rosa</p> <p>DEPTH: 200 m</p> <p>CASING DEPTH:</p> <p>CASING DIAMETER:</p> <p>STATIC WATER LEVEL:</p> <p>PUMP TEST DATE:</p> <p>DISCHARGE:</p> <p>DRAWDOWN:</p>	<p>LWUA Well No. P-17</p> <p>LOCATION: 8o Dto, Sta. Rosa</p> <p>DEPTH: 235 m</p> <p>CASING DEPTH:</p> <p>CASING DIAMETER:</p> <p>STATIC WATER LEVEL:</p> <p>PUMP TEST DATE:</p> <p>DISCHARGE:</p> <p>DRAWDOWN:</p>
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<div>LWUA Well No. (B)</div> <div>LOCATION: San Vicente, Binan</div> <div>DEPTH: 161 m</div> <div>CASING DEPTH:</div> <div>CASING DIAMETER:</div> <div>STATIC WATER LEVEL:</div> <div>PUMP TEST DATE:</div> <div>DISCHARGE:</div> <div>DRAWDOWN:</div>	<div>LWUA Well No. C-41</div> <div>LOCATION: Elem. Sch. Binan</div> <div>DEPTH: 152 m</div> <div>CASING DEPTH:</div> <div>CASING DIAMETER:</div> <div>STATIC WATER LEVEL:</div> <div>PUMP TEST DATE:</div> <div>DISCHARGE:</div> <div>DRAWDOWN:</div>	<div>LWUA Well No.</div> <div>LOCATION:</div> <div>DEPTH:</div> <div>CASING DEPTH:</div> <div>CASING DIAMETER:</div> <div>STATIC WATER LEVEL:</div> <div>PUMP TEST DATE:</div> <div>DISCHARGE:</div> <div>DRAWDOWN:</div>	<div>LWUA Well No. P-45</div> <div>LOCATION: San Isidro, Cabuyao</div> <div>DEPTH: 172 m</div> <div>CASING DEPTH:</div> <div>CASING DIAMETER:</div> <div>STATIC WATER LEVEL:</div> <div>PUMP TEST DATE:</div> <div>DISCHARGE:</div> <div>DRAWDOWN:</div>	<div>LWUA Well No. (I)</div> <div>LOCATION: Bantayan, Cabuyao</div> <div>DEPTH: 91 m</div> <div>CASING DEPTH:</div> <div>CASING DIAMETER:</div> <div>STATIC WATER LEVEL:</div> <div>PUMP TEST DATE:</div> <div>DISCHARGE:</div> <div>DRAWDOWN:</div>	<div>LWUA Well No. (2)</div> <div>LOCATION: Bantayan, Cabuyao</div> <div>DEPTH: 91 m</div> <div>CASING DEPTH:</div> <div>CASING DIAMETER:</div> <div>STATIC WATER LEVEL:</div> <div>PUMP TEST DATE:</div> <div>DISCHARGE:</div> <div>DRAWDOWN:</div>
<div>DEPTH</div> <div>LOG</div> <div></div>	<div>DEPTH</div> <div>LOG</div> <div></div>	<div>DEPTH</div> <div>LOG</div> <div></div>	<div>DEPTH</div> <div>LOG</div> <div></div>	<div>DEPTH</div> <div>LOG</div> <div></div>	

LWUA Well No. P-58			LWUA Well No. P-43			LWUA Well No. (A)		
LOCATION: Calabuso, Binon			LOCATION: SogSoro, Binon			LOCATION: Binon, Laguna		
DEPTH: 170 m			DEPTH: 169 m			DEPTH: 183 m		
CASING DEPTH:			CASING DEPTH:			CASING DEPTH:		
CASING DIAMETER:			CASING DIAMETER:			CASING DIAMETER:		
STATIC WATER LEVEL:			STATIC WATER LEVEL:			STATIC WATER LEVEL:		
PUMP TEST DATE:			PUMP TEST DATE:			PUMP TEST DATE:		
DISCHARGE:			DISCHARGE:			DISCHARGE:		
DRAWDOWN:			DRAWDOWN:			DRAWDOWN:		
DEPTH	LOG	DEPTH	LOG	DEPTH	LOG	DEPTH	LOG	DEPTH
170	VOL. TUFF	169	RESIDUAL CLAY					
165	VOL. TUFF	168	CLAY, TUFFACEOUS					
160	CLAY TUFFACEOUS	167	VOL. TUFF					
155	VOL. TUFF	166	CLAY					
150	VOL. TUFF	165	VOL. TUFF					
145	VOL. TUFF	164	VOL. TUFF					
140	VOL. TUFF	163	VOL. TUFF					
135	VOL. TUFF	162	VOL. TUFF					
130	VOL. TUFF	161	VOL. TUFF					
125	VOL. TUFF	160	VOL. TUFF					
120	VOL. TUFF	159	VOL. TUFF					
115	VOL. TUFF	158	VOL. TUFF					
110	VOL. TUFF	157	VOL. TUFF					
105	VOL. TUFF	156	VOL. TUFF					
100	VOL. TUFF	155	VOL. TUFF					
95	VOL. TUFF	154	VOL. TUFF					
90	VOL. TUFF	153	VOL. TUFF					
85	VOL. TUFF	152	VOL. TUFF					
80	VOL. TUFF	151	VOL. TUFF					
75	VOL. TUFF	150	VOL. TUFF					
70	VOL. TUFF	149	VOL. TUFF					
65	VOL. TUFF	148	VOL. TUFF					
60	VOL. TUFF	147	VOL. TUFF					
55	VOL. TUFF	146	VOL. TUFF					
50	VOL. TUFF	145	VOL. TUFF					
45	VOL. TUFF	144	VOL. TUFF					
40	VOL. TUFF	143	VOL. TUFF					
35	VOL. TUFF	142	VOL. TUFF					
30	VOL. TUFF	141	VOL. TUFF					
25	VOL. TUFF	140	VOL. TUFF					
20	VOL. TUFF	139	VOL. TUFF					
15	VOL. TUFF	138	VOL. TUFF					
10	VOL. TUFF	137	VOL. TUFF					
5	VOL. TUFF	136	VOL. TUFF					
0	VOL. TUFF	135	VOL. TUFF					
	VOL. TUFF	134	VOL. TUFF					
	VOL. TUFF	133	VOL. TUFF					
	VOL. TUFF	132	VOL. TUFF					
	VOL. TUFF	131	VOL. TUFF					
	VOL. TUFF	130	VOL. TUFF					
	VOL. TUFF	129	VOL. TUFF					
	VOL. TUFF	128	VOL. TUFF					
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	VOL. TUFF	126	VOL. TUFF					
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	VOL. TUFF	114	VOL. TUFF					
	VOL. TUFF	113	VOL. TUFF					
	VOL. TUFF	112	VOL. TUFF					
	VOL. TUFF	111	VOL. TUFF					
	VOL. TUFF	110	VOL. TUFF					
	VOL. TUFF	109	VOL. TUFF					
	VOL. TUFF	108	VOL. TUFF					
	VOL. TUFF	107	VOL. TUFF					
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	VOL. TUFF	105	VOL. TUFF					
	VOL. TUFF	104	VOL. TUFF					
	VOL. TUFF	103	VOL. TUFF					
	VOL. TUFF	102	VOL. TUFF					
	VOL. TUFF	101	VOL. TUFF					
	VOL. TUFF	100	VOL. TUFF					
	VOL. TUFF	99	VOL. TUFF					
	VOL. TUFF	98	VOL. TUFF					
	VOL. TUFF	97	VOL. TUFF					
	VOL. TUFF	96	VOL. TUFF					
	VOL. TUFF	95	VOL. TUFF					
	VOL. TUFF	94	VOL. TUFF					
	VOL. TUFF	93	VOL. TUFF					
	VOL. TUFF	92	VOL. TUFF					
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	VOL. TUFF	90	VOL. TUFF					
	VOL. TUFF	89	VOL. TUFF					
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	VOL. TUFF	84	VOL. TUFF					
	VOL. TUFF	83	VOL. TUFF					
	VOL. TUFF	82	VOL. TUFF					
	VOL. TUFF	81	VOL. TUFF					
	VOL. TUFF	80	VOL. TUFF					
	VOL. TUFF	79	VOL. TUFF					
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	VOL. TUFF	70	VOL. TUFF					
	VOL. TUFF	69	VOL. TUFF					
	VOL. TUFF	68	VOL. TUFF					
	VOL. TUFF	67	VOL. TUFF					
	VOL. TUFF	66	VOL. TUFF					
	VOL. TUFF	65	VOL. TUFF					
	VOL. TUFF	64	VOL. TUFF					
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	VOL. TUFF	55	VOL. TUFF					
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	VOL. TUFF	53	VOL. TUFF					
	VOL. TUFF	52	VOL. TUFF					
	VOL. TUFF	51	VOL. TUFF					
	VOL. TUFF	50	VOL. TUFF					
	VOL. TUFF	49	VOL. TUFF					
	VOL. TUFF	48	VOL. TUFF					
	VOL. TUFF	47	VOL. TUFF					
	VOL. TUFF	46	VOL. TUFF					
	VOL. TUFF	45	VOL. TUFF					
	VOL. TUFF	44	VOL. TUFF					
	VOL. TUFF	43	VOL. TUFF					
	VOL. TUFF	42	VOL. TUFF					
	VOL. TUFF	41	VOL. TUFF					
	VOL. TUFF	40	VOL. TUFF					
	VOL. TUFF	39	VOL. TUFF					
	VOL. TUFF	38	VOL. TUFF					
	VOL. TUFF	37	VOL. TUFF					
	VOL. TUFF	36	VOL. TUFF					
	VOL. TUFF	35	VOL. TUFF					
	VOL. TUFF	34	VOL. TUFF					
	VOL. TUFF	33	VOL. TUFF					
	VOL. TUFF	32	VOL. TUFF					
	VOL. TUFF	31	VOL. TUFF					
	VOL. TUFF	30	VOL. TUFF					
	VOL. TUFF	29	VOL. TUFF					
	VOL. TUFF	28	VOL. TUFF					
	VOL. TUFF	27	VOL. TUFF					
	VOL. TUFF	26	VOL. TUFF					
	VOL. TUFF	25	VOL. TUFF					
	VOL. TUFF	24	VOL. TUFF					
	VOL. TUFF	23	VOL. TUFF					
	VOL. TUFF	22	VOL. TUFF					
	VOL. TUFF	21	VOL. TUFF					
	VOL. TUFF	20	VOL. TUFF					
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	VOL. TUFF	17	VOL. TUFF					
	VOL. TUFF	16	VOL. TUFF					
	VOL. TUFF	15	VOL. TUFF					
	VOL. TUFF	14	VOL. TUFF					
	VOL. TUFF	13	VOL. TUFF					
	VOL. TUFF	12	VOL. TUFF					
	VOL. TUFF	11	VOL. TUFF					
	VOL. TUFF	10	VOL. TUFF					
	VOL. TUFF	9	VOL. TUFF					
	VOL. TUFF	8	VOL. TUFF					
	VOL. TUFF	7	VOL. TUFF					
	VOL. TUFF	6	VOL. TUFF					
	VOL. TUFF	5	VOL. TUFF					
	VOL. TUFF	4	VOL. TUFF					
	VOL. TUFF	3	VOL. TUFF					
	VOL. TUFF	2	VOL. TUFF					
	VOL. TUFF	1	VOL. TUFF					
	VOL. TUFF	0	VOL. TUFF					

APPENDIX 6.8.1 SELECTION OF SAMPLING POINT

Sampling points in the project area took into account the following:

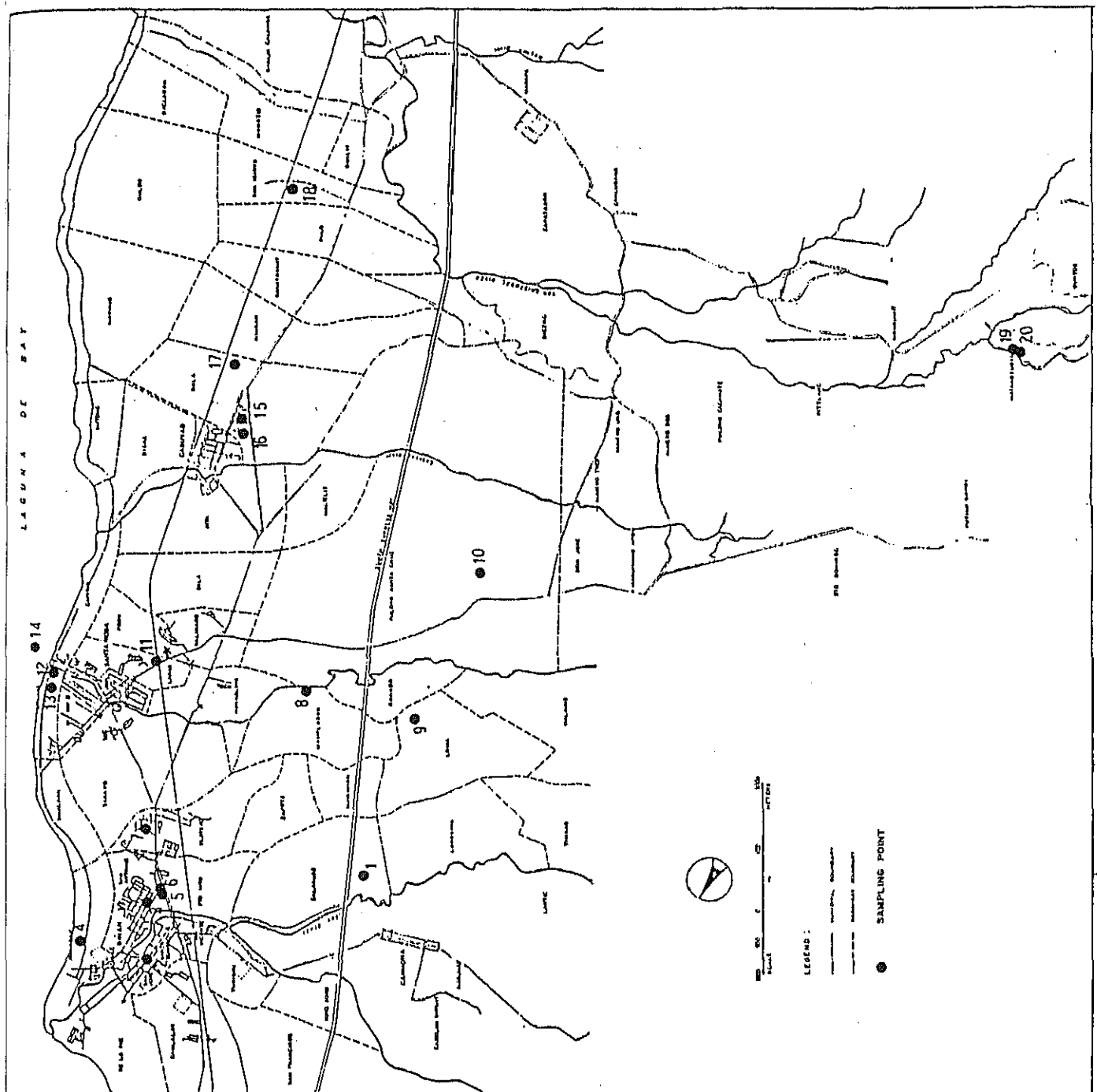
- Existing sources, i.e., spring and deep well, to evaluate the qualitative characteristics of the present water system;
- The other deep well sources at representative locations, thus, a general impression on the areas overall water quality could be established;
- Important well sources, e.g. the free-flowing wells in Sta. Rosa. By comparing test results of each, relative analysis on the continuity of the aquifer i.e. similar values would probably describe some water source, could be done; and
- Large well source (NIA wells) and the Laguna de Bay as they pose to be alternative sources for the system.

To summarize:

- Two (2) existing deep wells in the city water supply system
- One (1) existing spring in the city water supply system
- Seven (7) deep wells
- Four (4) shallow wells
- One (1) potential spring
- One (1) surface water source (Laguna de Bay)
- Three (3) faucets and one (1) shallow well for bacteriological analysis

FIGURE 6.8.1.1 shows location of the selected points.

FIGURE 6.8.1.1 LOCATION OF SUMPLING POINTS



APPENDIX 6.8.2 WATER QUALITY ANALYSIS - CABUYAO-STA. ROSA-BINAN

Sample No.	Well No.	Location	Group	Turb. (FTU)	TDS (mg/l)	pH (-)	EC (μ S/cm)	Alk. (mg/l)	Hard. (mg/l)	Acid. (mg/l)	Na (mg/l)	K (mg/l)	Ca (mg/l)	Mg (mg/l)	CO ₃ (mg/l)	HCO ₃ (mg/l)	Cl (mg/l)	SO ₄ (mg/l)	Fe (mg/l)	Mn (mg/l)	E.Coli. (MPN)	NO ₃ -N (mg/l)	NH ₄ -N (mg/l)	
1	NIA P-58	Calaboso, Biñan	C	0.67	320	7.43	470	233	146	22	49	9.7	31.6	16.3	0	294.3	13.9	5	0.15	0.2	-	-	1.24	0.01
2	W-1	Biñan Market, Biñan	A	0.61	334	7.21	530	235	199	12	59	9.4	42.0	22.8	0	344.7	18.6	7.5	0.13	nil	-	-	0.78	0.02
3	Faucet	San Antonio, Biñan	G	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	(+)	-	-	-
4	B-2	Malaban, Biñan	D	1.25	840	7.63	800	328	336	9	80	15	76.5	40.1	0	400.2	83.6	90	0.10	nil	-	-	9.67	0.06
5	B-1	San Antonio, Biñan	D	1.31	352	7.56	560	256	172	28	50	8.7	32.8	21.9	0	312.3	18.6	5	0.15	0.10	-	-	0.87	0.04
6	B-1	San Antonio, Biñan	G	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	(+)	-	-	-
7	W-2	Tulay Beto, Biñan	A	-	-	6.95	400	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8	NIA M-3	Macabiling, Sta. Rosa	C	1.45	314	7.41	455	237	176	32	39.5	7.4	32.8	22.8	0	293.1	13.9	3.5	0.13	nil	-	-	1.09	0.01
9	NIA P-56	Mamplasan, Biñan	C	0.45	288	6.92	415	218	176	36	33	6.4	32.8	22.8	0	236	11.6	3	0.06	nil	-	-	1.46	nil
10	NIA P-18	Paguio, Sta. Rosa	C	0.58	314	7.26	470	228	191	14	30	7.2	40.4	21.9	0	278.2	11.6	3.5	0.06	0.10	-	-	1.58	0.01
11	SR-1	Labas, Sta. Rosa	D	2.45	374	7.73	520	275	205	9	47	6.7	36	28.2	0	335.5	18.6	2	0.10	0.05	-	(+)	0.87	0.01
12	Faucet	Aplayo, Sta. Rosa	G	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13	NIA P-57	Aplayo, Sta. Rosa	C	0.43	355	7.84	535	256	139	6	72.5	8.5	26.8	17.5	0	312.3	18.6	10.0	0.20	nil	-	-	0.44	0.01
14	-	Laguna de Bay, Sta. Rosa	F	25.8	576	7.47	910	142	184	12	120	7.1	44.8	17.5	0	173.2	190.4	34.5	0.20	0.20	-	-	5.04	0.03
15	Faucet	Waterworks Office, Cabuyao	G	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	(-)	-	-	-
16	C-1	Waterworks Office, Cabuyao	D	2.75	526	7.73	455	247	191	14	36	7.5	38.8	22.8	0	301.3	13.9	3	0.06	0.05	-	-	0.50	0.02
17	AFP-MEAL	Cabuyao	C	0.75	352	7.73	515	255	221	16	27.5	6.8	43.6	27.2	0	324.5	11.6	3	0.10	nil	-	-	0.39	0.01
18	NIA P-45	San Isidoro, Cabuyao	C	-	-	6.87	400	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19	Spring 1	Matan Tubig	B	0.41	250	7.01	330	176	157	33	17.5	5	38.8	14.6	0	214.7	11.6	5	0.13	nil	-	-	4.50	0.01
20	Spring 2	Matan Tubig	E	0.43	237	6.96	340	171	165	28	15	4.8	36.0	18.2	0	208.6	9.3	4	0.13	nil	-	-	4.48	nil

*: Category: A - Deep wells in the city water supply system

B - Spring in the city water supply system

C - Deep well

D - Shallow well

E - Potential spring for the city water supply system

F - Surface water source

G - For bacteriological analysis

Philippine National Standard for Drinking Water

Water Quality: Physical, Chemical and Radiological Requirements

Bacteriological Quality Standards

Parameter		Maximum Permissible level*
Turbidity		5 units
Color		5 units (s) **
Odor		Unobjectionable
Threshold odor number		Note more than 3
Taste		Unobjectionable
Total Solids		500 (s)
pH		6.5 - 8.5
Phenolic substances		0.001
Radioactive Subs.	Gross Alpha	3 pCi/l
	Gross Beta	30pCi/l
Trace Elements	Arsenic	0.05
	Barium	1.0
	Cadmium	0.01
	Chromium	0.05
	Copper	1.0
	Cyanide	0.05
	Fluoride	0.6
	Iron	1.0
	Lead	0.05
	Manganese	0.5 (s)
	Mercury	0.002
	Selenium	0.01
	Zinc	5.0 (s)
Organic Chemicals	: Synthetic Detergents (MBAS)	0.5
	Oil & Grease	Nil
Persistent Pesticides	: Aldrin	0.001
	DDT	0.05
	Dieldrin	0.001
	Chlordane	0.003
	Endrin	0.0002
	Heptachlor	0.0001
	Lindane	0.004
	Toxaphane	0.005
	Methoxychlor	0.1
	2,4 --E	0.1
	2, 4, 5 --- T	0.01
PCB		Nil
Other Chemicals	: Calcium	75
	Chloride	200 (s)
	Magnesium	50 (s)
	Nitrate (NO ₃)	30
	Sulfate	200 (s)
	Hydrogen sulfide	0.05 (s)

Minimum Requirements on Bacteriological Quality

a) Chlorinated or Otherwise Disinfected Supplies

Efficient treatment culminating in chlorination or some other form of disinfection should yield a water free of any coliform organism however polluted the original raw water may have been. In practice it should not be possible to demonstrate the presence of coliform organisms in any sample of 100ml. The efficacy of the purification process and method of sampling should be looked into when a sample of the water entering the distribution system does not conform to this standard. In testing chlorinated water, presumptive positive tubes should always be subjected to appropriate confirmatory tests.

b) Non-disinfected Supplies

Where supplies of this sort exist, no water entering the distribution system should be considered satisfactory if it yields E. coli in 100ml. If E. coli is absent, the presence of not more than 3 coliform organisms per 100ml may be tolerated in occasional samples from established non-disinfected pipes supplies, provided that they have been regularly and frequently tested and that the catchment area and storage conditions are found to be satisfactory. If repeated samples show the presence of coliform organisms, steps should then be taken to discover and, if possible, remove the source of pollution. If the number of coliform organisms increases to more than 3 per 100ml, the supply should be considered unsuitable for use without disinfection.

c) Individual or Small Community Supplies

Where supply of waters are individual wells, bores and springs everything possible should be done to prevent pollution of the water. It should be possible to reduce the coliform count of water from even a shallow well to less than 10 per 100ml. Persistent failure to achieve this, particularly if E. coli is repeatedly found, should, as a general rule lead to chlorination or boiling of the water for domestic consumption.

* All units are in mg/l unless, otherwise stated.

** (s) - Secondary standards; compliance with the standard and analysis are not obligatory.

APPENDIX 7.2.1 WATER RIGHT IN THE SPRING AREA

User Name	Water Volume		Purpose	Remarks
	l/sec	cu.m/day		
1) Canlubang Sugar Estate	461	39,830	industry	the water after power plant
2) Yulo	96	8,294	irrigation	
3) Canlubang Pulp Manufacturing	457	39,485	electric power	
Total	1,014	87,609		

Source; NWRG

The total of 48,124 cu.m/day comprising item 1) and 2) in the above table may be water amount available including river water and spring water. The amount was also field confirmed as follows:

- a) Existing spring for the CSBWS waterworks and potential springs
: 12,600 cu.m/day
- b) Transmitted amount to Canlubang Sugar estate
: 15,000 cu.m/day
- c) Flow rate at the upstream of nearby river
: 15,500 cu.m/day
- Total : 43,100 cu.m/day

The water utilized for the Canlubang Sugar Estate at present might be used for the CSBWS waterworks in the future when its business activities would be discontinued as related to the survey term. It is advantageous to consider spring water sources for the water supply, however, a total of 12,600 cu.m/day may be the maximum available volume at this stage. Negotiations with the private sector to acquire additional spring water should be done in the future.

APPENDIX 7.2.2 DATA ON THE UNIT COST FOR ESTIMATION OF PROJECT COST

(1) Deep Well Construction : Peso

Depth (m)	Casing size (m/m)	Cost
200	250	940,000
200	300	1,160,000
250	150	640,000

BREAKDOWN OF COSTS IN %

	Material	Local Component		F E C		Total
		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	17	-	-	-	20	37
Civil Works	33	8	5	-	17	63
Total	50	8	5	-	37	100

(2) Deep Well Pump Station (Electric Motor Drive) : Thousand Peso

KW	Cost
7	450
15	560
22	640
29	720
37	790
44	840
51	890
59	960
66	1,020
74	1,080

BREAKDOWN OF COSTS IN %

	Material	Local Component		F E C		Total
		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	9	-	-	42	5	56
Civil Works	21	9	5	-	9	44
Total	30	9	5	42	14	100

(3) Booster Pump Station

$$C = (72.16 - 13.68 \log Q) \times Q^{(0.42 + 0.1 \log Q)} \times H^{0.305(\log Q - 0.7)} \times (6/H - 0.25)$$

where,

C = cost for electric motor drive (thousand peso)

Q = design capacity (l/sec)

H = total dynamic head (m)

BREAKDOWN OF COSTS IN %

	Material	Local Component		F E C		Total
		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	11	-	-	53	2	66
Civil Works	17	9	6	-	2	34
Total	28	9	6	53	4	100

(4) Pipeline Cost

Following pipe materials are presently available in the Philippines:

- GI (galvanized iron),
- PE (poly-ethylene),
- PB (poly-butylene),
- PVC (poly-vinyl-chloride),
- SP (steep pipe),
- CI (cost iron), and
- AC (asbestos cement).

Among these materials, the use of CI pipe is limited due to its high cost and AC pipe is also rare by safety reason.

Followings are comparison of unit cost at the 1985 price level.

Diameter	(Unit: ₱/m)				
	GI	PE	PB	PVC	SP
13	20.8	13.8	9.1	-	-
19	24.7	19.9	13.6	-	-
25	32.3	25.3	22.0	-	-
38	59.2	41.5	44.7	-	-
50	87.5	61.4	76.4	33.9	-
63	117.7	-	-	48.0	-
75	180.3	-	-	81.3	-
100	230.8	-	-	122.4	235.0
150	-	-	-	256.9	250.0
200	-	-	-	506.5	290.0
250	-	-	-	-	315.0
300	-	-	-	-	425.0
400	-	-	-	-	520.0
500	-	-	-	-	700.0
600	-	-	-	-	890.0

Based on the above comparison, SP is advantageous for the diameter of 200 mm and above than PVC. Thus, for the cost estimates of major transmission and distribution pipes, SP is considered for diameter of 200 mm and above, while PVC for diameter of less than 150 mm taking into account the transportation cost and easy installation.

Diameter (mm)	Unit Cost (₱/m)
150 (PVC)	410
200 (SP)	520
250 (")	630
300 (")	760
350 (")	900
400 (")	970
450 (")	1,160
500 (")	1,330
600 (")	1,600
700 (")	1,910

Source : LWUA Design Depart

BREAKDOWN OF COSTS IN %

	Material	Local Component		F E C		Total
		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	23	-	-	4	27	54
Civil Works	17	7	4	-	18	46
Total	40	7	4	4	45	100

(5) Valve In-place Cost

Diameter (mm)	Gate Valve (₹)	Butterfly Valve (₹)
50	1,700	-
75	2,900	-
100	3,900	-
150	5,300	-
200	6,700	-
250	11,200	-
300	-	34,800
350	-	74,400
400	-	95,200
450	-	125,900
500	-	174,000
600	-	243,600
700	-	313,200

Source : LWUA Design Depart

BREAKDOWN OF COSTS IN %

	Material	Local Component		F E C		Total
		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	9	-	-	63	5	77
Civil Works	12	3	6	-	2	23
Total	21	3	6	63	7	100

(6) Internal Network

Population Density (Person/ha)	Total Length of Pipeline (m/ha)	Unit Cost (P/ha)	
		Diameter (100/150)	Diameter (75/100)
50	64	18,300	14,900
60	67	19,300	15,700
75	72	20,900	16,800
100	80	23,100	18,700
150	90	25,700	21,000
200	100	28,300	-
250	108	30,400	-
300	116	32,500	-

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	22	-	-	7	27	56
Civil Works	17	7	4	-	16	44
Total	39	7	4	7	43	100

(7) In-place of Service Connections

Diameter (inch)	Without Meter P/unit	With Meter P/unit	Meters P/unit
1/2	450	810	400
5/8 - 3/4	520	1,280	880

SERVICE CONNECTION WITHOUT METER

BREAKDOWN OF COSTS IN %

	Material	Local Component		F E C		Total
		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	9	-	-	60	2.5	71.5
Civil Works	17	3	6	-	2.5	28.5
Total	26	3	6	60	5	100

SERVICE CONNECTION WITHOUT METER
BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	4	-	-	83	2	89
Civil Works	6	1	3	-	1	11
Total	10	1	3	83	3	100

(8) Fire Hydrant In-place Cost

Type	Size (mm)	Unit Cost (P)
Commercial	150	16,800
Residential	100	9,400

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	8	-	-	57	5	70
Civil Works	10	8	10	-	2	30
Total	18	8	10	57	7	100

(9) Elevated Tank/Ground Reservoir

Elevated Tank: $C = 0.615 H^{1.144} V^{0.749}$

Ground Reservoir: $C = 20.05 V^{0.639}$

where, C = cost (thousand peso)

H = overflow elevation above ground level

V = storage volume (cu.m)

BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	4	-	-	3	2	9
Civil Works	53	5	7	-	26	91
Total	57	5	7	3	28	100

(10) Gas Chlorinator In-place Cost

Type	Water Flow Condition	Maximum Chlorine Feed (kg/day)	Unit cost ^{1/} (₹)
I-A	constant	22	98,100
I-B	constant	45	119,100
II-A	Variable	22	147,700
II-B	Variable	45	169,300

^{1/} Empty gas cylinders and automatic switchover include

TYPE I-A, I-B BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	15	-	-	41	5	61
Civil Works	25	6	3	-	5	39
Total	40	6	3	41	10	100

TYPE II-A, II-B BREAKDOWN OF COSTS IN %

	Local Component			F E C		Total
	Material	Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	21	-	-	53	2	76
Civil Works	12	6	2	-	4	24
Total	33	6	2	53	6	100

(11) Administration & Operation Building

Future Service Population	Administration Bldg. (Thousand Peso)	Operation Center (Thousand Peso)
30,000	1,000	810
40,000	1,110	890
50,000	1,220	990
60,000	1,320	1,090
70,000	1,410	1,180
80,000	1,500	1,280
100,000	1,610	1,380
110,000	1,820	1,590

ADMINISTRATION BUILDING
BREAKDOWN OF COSTS IN %

		Local Component		F E C		Total
Material		Labor		Direct	Indirect	
		Skilled	Unskilled			
Equipment	20	-	-	-	16	36
Civil Works	42	7	5	-	10	64
Total	62	7	5	-	26	100

OPERATION CENTER
BREAKDOWN OF COSTS IN %

	Local Component			F E C		
	Material	Labor		Direct	Indirect	Total
		Skilled	Unskilled			
Equipment	14	-	-	30	6	50
Civil Works	26	10	5	-	9	50
Total	40	10	5	30	15	100

(12) Energy Cost

$$C = N_p (h) (P_u) (E_m)^{-1}$$

where,

C = cost (thousand peso)

N_p = pump power demand (kw)

h = hours of operation

P_u = unit power cost (₱/kwh)

E_m = motor efficiency (0.85)

(13) Chemical Cost

$$C = (\text{Annual Water Demand}) \cdot D \cdot U_{CL} \times 10^{-3}$$

where,

C = annual cost for chlorine (₱)

D = chlorine dosage (mg/l)

U_{CL} = unit cost of chlorine gas (₱/kg)

(14) Minimum Cost Diameter

Following cost function is applied to determine the most economical diameter of pipelines that are not simulated by the network analysis.

$$D_{min.} = 187.7 Q^{0.486} C^{-0.315} (E_c/O_e)^{0.17}$$

where,

$D_{min.}$ = minimum cost diameter

Q = water flow (l/sec)

C = "C" value (Hazen William Formula)

E_c = energy cost (₱/kwh)

O_e = overall efficiency

APPENDIX 7.3.1 ALTERNATIVE STUDY OF WATER SOURCE AND TRANSMISSION

(1) Cost Estimates of Water Source and Transmission Alternatives

Required Facilities	Unit Cost (P)	Alternative S-1 Q'ty	Cost (P1,000 peso)	Alternative S-2 Q'ty	Cost (P1,000 peso)
<u>Construction Cost</u>					
1. Water Sources					
Deep Well	1,160,000	16 units	18,560	4 units	4,640
Deep Well Pump	790,000	16 units	12,640	4 units	3,160
Intake Pump Station	8,366,000	-	-	1 unit	8,366
<u>Sub Total</u>			<u>31,200</u>		<u>16,166</u>
2. Transmission Line					
φ 250 mm	630 /m	10,000 m	6,300	2,400 m	1,512
φ 350 mm	900 /m	700 m	630	-	-
φ 400 mm	970 /m	1,800 m	1,746	1,100 m	1,067
φ 450 mm	1,160 /m	-	-	1,300 m	1,508
φ 500 mm	1,330 /m	500 m	665	-	-
φ 700 mm	1,910 /m	4,400 m	8,404	1,000 m	1,910
<u>Sub Total</u>			<u>17,745</u>		<u>5,997</u>
3. Water Treatment					
Rapid Sand Filter	61,204,000	-	-	1 unit	61,204
<u>TOTAL</u>			<u>48,945</u>		<u>83,367</u>
<u>Operation & Maintenance Cost (15 years)</u>					
1. Energy	P 0.3 /KWH	77,790	23,337	48,360 MWH	14,508
2. Laborer	P 1,200 /MM	16 persons	3,456	5 persons	1,080
3. Maintenance (10% of Const- ruction Cost)		-	4,895	-	8,337
<u>TOTAL</u>			<u>31,688</u>		<u>23,925</u>
<u>GRAND TOTAL</u>			<u>80,633</u>		<u>107,292</u>

(2) Cost Estimates of Transmission Alternatives

Pipe Size	Pipe Length (m)			Unit Cost (₱/m)	Cost (₱ x 1,000)		
	Phase I	Phase II	Total		Phase I	Phase II	Total
<u>Alternative T-1</u>							
ϕ 250 mm	1,400	4,800	6,200	630	882	3,024	3,906
ϕ 350 mm	-	700	700	900	-	630	630
ϕ 400 mm	-	1,800	1,800	970	-	1,746	1,746
ϕ 500 mm	-	500	500	1,330	-	665	665
ϕ 700 mm	3,200	1,200	4,400	1,910	6,112	2,292	8,404
<u>TOTAL</u>	4,600	9,000	13,600		6,994	8,357	15,351
<hr/>							
<u>Alternative T-2</u>							
ϕ 250 mm	2,200	4,800	7,000	630	1,386	3,024	4,410
ϕ 350 mm	-	700	700	900	-	630	630
ϕ 400 mm	1,100	1,800	2,900	970	1,067	1,746	2,813
ϕ 450 mm	1,300	-	1,300	1,160	1,508	-	1,508
ϕ 500 mm	-	500	500	1,330	-	665	665
ϕ 600 mm	-	3,200	3,200	1,600	-	5,120	5,120
ϕ 700 mm	-	1,200	1,200	1,910	-	2,292	2,292
<u>TOTAL</u>	4,600	12,200	16,800		3,961	13,477	17,438

APPENDIX 7.3.2 COST COMPARISON OF THE TWO CASES

Case 1 Construction of transmission line from the spring to the existing reservoir area, enabling use of the additional springs.

Case 2 Development of an additional well in Biñan with a production of approx. 5,200 cu.m/day, as an alternative, to utilize new spring water sources.

A new well site for Case 2 is tentatively scheduled for development in Sta. Rosa, with a minimum transmission pipeline length of 1 km. The cost requirement for Case 1 is bigger than that of Case 2.

Case 1

Item	Unit	Quantity	Unit Cost	Cost	R.M.
Pipeline ø350	m	4,100	970	3,977,000	
Total				3,977,000	3,300cu.m/day

Case 2

Item	Unit	Quantity	Unit Cost	Cost	R.M.
Well Construction ø250 x 200m	unit	1	940,000	940,000	Q=5,200 cu.m/d
Pipe line ø200	m	1,000	410	410,000	
Pump Station	unit	1		790,000	3.7 cu.m/min x 40m x 37kw
Sub-Total				2,140,000	
Operation & Maintenance Cost					15 years
Electricity	kwh	5,208,000	0.3P/kwh	1,562,400	
Labor	unit	1		216,000	
Maintenance				214,000	Const. Cost x 0.1
Sub-Total				1,992,400	
Total				4,132,400	for 5,200 cmd
				(2,622,500	for 3,300 cmd)

FIGURE 7.3.2.1 SCHEMATIC LAYOUT OF CABUYAO

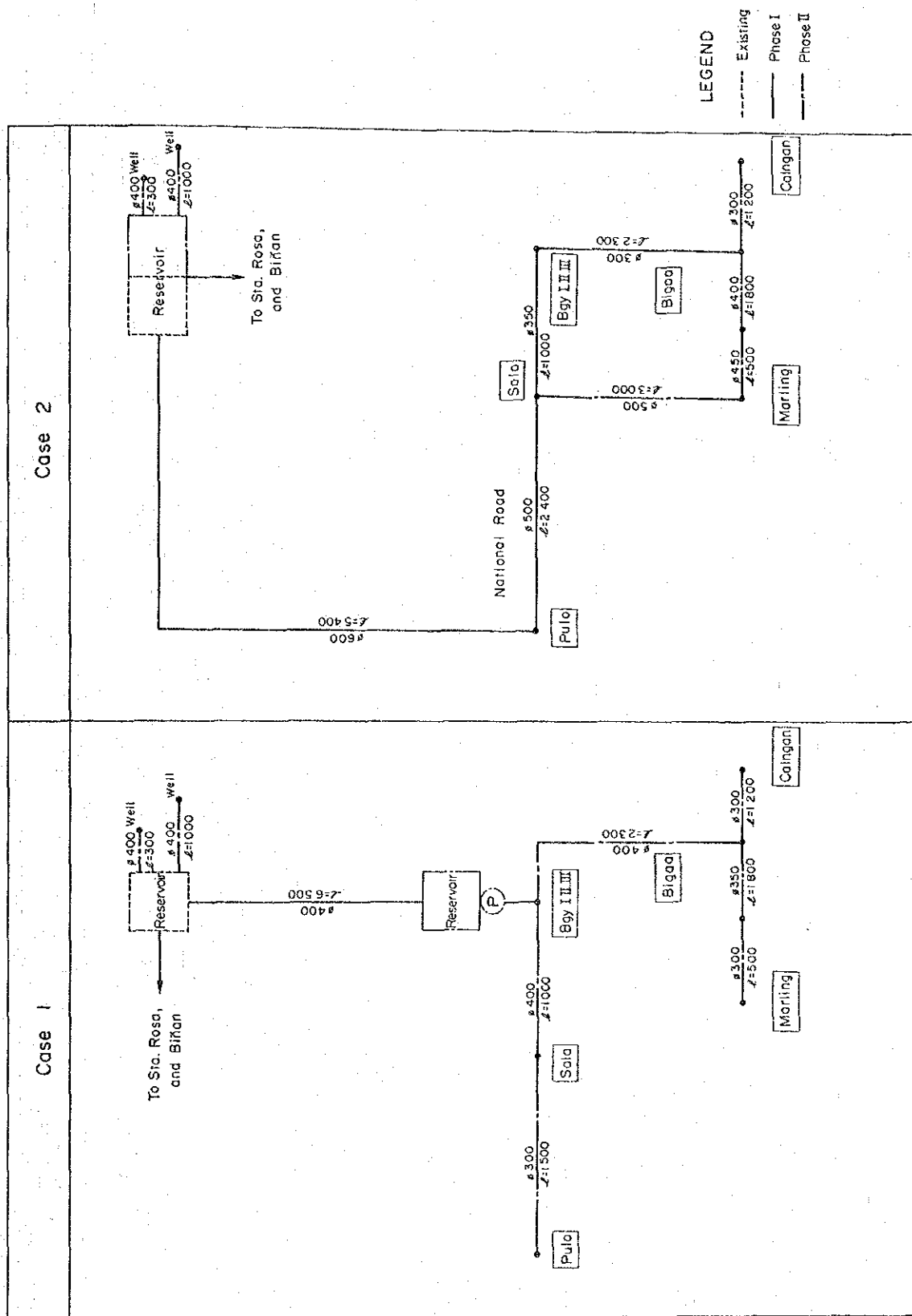


TABLE 7.3.2.1 COST COMPARISON ON CABUYAO SYSTEM

Case I

Phase	Item	Unit	Unit Cost	Cost	Remarks
I	Transmission Lain ø400	6,500 m	970 ₱/m	6,305,000	
	Sub Total			6,305,000	
	Distribution Lain				
	ø400	3,300	970 ₱/m	3,201,000	
	ø350	1,800	900 ₱/m	1,620,000	
	ø300	3,200	760 ₱/m	2,432,000	
	Reservoir V = 1,800 cu.m	1 unit		2,411,000	
II	Pump Station	1 unit		20,449,000	ø250x7.0 cu.m/min x 35m x 59kw x 4sets
	Sub Total			30,113,000	
I & II	Operation & Maintenance Electricity	69,532,000 kwh	0.3 ₱/kwh	20,859,600	
	Labor	1 unit		1,944,000	Mechanical Engineer 1 Pump Operator 4 Labor 4
	Maintenance			3,641,800	Construction Cost x 0.1
	Sub Total			26,445,400	
	Land Acquisition	900 sq.m	120 ₱/m	108,000	
				Total	62,971,400

TABLE 7.3.2.1 COST COMPARISON ON CABUYAO SYSTEM (CONT'D)

Case 2

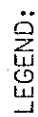
Phase	Item	Unit	Unit Cost	Cost	Remarks
I	Pipe Line				
	ø600	5,400m	1,600 P/m	8,640,000	
	ø500	2,400m	1,330 P/m	3,192,000	
	ø350	1,000m	900 P/m	900,000	
	ø300	2,300m	760 P/m	1,748,000	
	Sub Total			14,480,000	
II	Distribution Pipe Line				
	ø500	3,000m	1,330 P/m	3,990,000	
	ø450	500m	1,160 P/m	580,000	
	ø400	1,800m	970 P/m	1,746,000	
	ø300	1,200m	760 P/m	912,000	
	Reservoir V = 1,800 cu.m	1 unit		2,411,000	
	Sub Total			9,639,000	
I & II	Operation & Maintenance				
	Maintenance			2,412,000	Construction Cost x 0.1
II	Land Acquisition	900 sq.m	120 P/m	108,000	
	Ground Total			26,639,000	

o List of Computed Cases

Alternative	D-1-A	(1995, 2010)
	D-1-B	(1995, 2010)
	D-2	(2010)

o Note

This appendix show the results of Hydraulic Analysis aided by the computer. The distribution network is shown in the figure of following page. The nodes, however, with no flow and 20.00 m in Dynamic Head was treated as a dummy node. Those nodes can be ignored and have no relation to the computation results.



Q=1980 Outflow at the Node in 2010
(1200) Outflow at the Node in 1995
GL+14.0 Ground Elevation at the Node

(Cabuyao-Sta. Rosa-Binan)

ALTERNATIVE D-1-A (Recommended Plan, Single Pipeline Alignment)
2 Reservoir System, Year 1995

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	24.00	0.00	44.00	20.00	51.00
2	22.00	0.00	42.00	20.00	53.00
3	25.00	0.00	45.00	20.00	50.00
4	26.00	0.00	46.00	20.00	47.00
5	10.00	0.00	30.00	20.00	65.00
6	5.00	0.00	25.00	20.00	70.00
7	5.00	0.00	25.00	20.00	70.00
8	5.00	0.00	25.00	20.00	70.00
9	21.00	0.00	41.00	20.00	54.00
10	18.00	0.00	38.00	20.00	57.00
11	14.00	1400.00	34.45	10.46	61.00
12	10.00	730.00	31.65	14.65	65.00
13	10.00	730.00	31.65	14.65	65.00
14	10.00	730.00	31.65	14.65	65.00
15	10.00	730.00	31.65	14.65	65.00
16	5.00	0.00	25.00	20.00	70.00
17	9.00	900.00	23.43	14.43	66.00
18	10.00	1100.00	23.64	13.64	65.00
19	5.00	900.00	25.00	20.00	70.00
20	25.00	0.00	45.00	20.00	50.00
21	16.00	1100.00	32.75	35.75	50.00
22	12.00	1200.00	33.05	39.05	51.00
23	12.00	1000.00	34.46	42.46	53.00
24	5.00	0.00	25.00	20.00	70.00
25	10.00	2000.00	36.40	36.40	65.00
26	10.00	1600.00	32.50	42.50	65.00
27	9.00	1200.00	33.39	33.39	66.00
28	9.00	1600.00	33.77	30.77	66.00
29	10.00	800.00	33.28	29.28	65.00
30	10.00	600.00	33.77	27.77	65.00
31	5.00	700.00	31.65	32.05	55.00
32	5.00	2100.00	32.91	27.91	70.00
33	10.00	300.00	39.74	29.74	65.00
34	5.00	1700.00	41.52	31.52	65.00
35	5.00	0.00	25.00	20.00	70.00
36	5.00	0.00	25.00	20.00	70.00
37	5.00	1700.00	47.38	38.38	66.00
38	5.00	1800.00	46.74	37.74	66.00
39	5.00	1800.00	46.03	37.03	66.00
40	6.00	0.00	46.03	38.03	67.00
41	8.00	2100.00	38.21	30.21	67.00
42	8.00	1800.00	42.91	34.91	67.00
43	8.00	1000.00	38.24	30.24	67.00
44	8.00	1600.00	38.00	30.00	67.00
45	5.00	0.00	25.00	20.00	65.00
46	5.00	1400.00	30.95	20.95	65.00
47	8.00	0.00	36.91	28.91	67.00
48	8.00	1800.00	39.37	31.37	67.00
49	8.00	1800.00	38.29	30.29	67.00
50	8.00	900.00	38.29	30.29	67.00
51	8.00	3300.00	36.19	28.19	67.00
52	5.00	3300.00	36.47	31.47	67.00
53	10.00	0.00	36.00	20.00	65.00
54	5.00	0.00	25.00	20.00	65.00
55	25.00	0.00	45.00	20.00	50.00

ALTERNATIVE D-1-A (Recommended Plan, Single Pipeline Alignment)
2 Reservoir System, Year 1995

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
55	5.00	5600.00	34.54	29.54	70.00
57	15.00	0.00	35.00	20.00	60.00
58	70.00	0.00	75.00	5.00	5.00
59	75.00	0.00	75.00	0.00	0.00
60	55.00	0.00	55.00	0.00	20.00

ALTERNATIVE D-1-A (Recommended Plan, Single Pipeline Alignment)
2 Reservoir System, Year 1995

<< PIPELINE >>

PIPE No.	NODE from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0.70)
1	12-13	300	850	120	-1400	-0.23	-0.22
2	13-14	300	450	120	-730	-0.12	-0.09
3	14-15	250	450	120	-2730	0.64	0.97
4	15-16	250	1500	120	-5500	0.97	8.12
5	16-17	250	1050	120	900	0.21	0.29
6	17-18	250	200	120	1100	0.26	0.27
7	18-19	250	750	120	-1100	-0.26	-0.30
8	19-20	250	900	120	-2300	-0.54	-1.41
9	20-21	250	230	120	-44700	-1.34	-0.54
10	21-22	250	650	120	7039	1.66	8.06
11	22-23	250	750	130	32817	1.34	12.40
12	23-24	250	500	110	1544	0.57	2.60
13	24-25	250	500	120	5035	1.19	2.50
14	25-26	250	2150	130	31255	1.28	5.97
15	26-27	250	2150	110	1472	0.54	2.38
16	27-28	250	250	120	3839	0.91	2.62
17	28-29	300	250	120	4200	0.69	4.03
18	29-30	250	800	120	-2761	-0.65	-1.75
19	30-31	350	750	120	800	0.10	0.03
20	31-32	250	450	120	3400	0.80	0.45
21	32-33	250	350	120	2800	0.66	0.79
22	33-34	200	900	110	2100	0.77	4.14
23	34-35	250	1100	120	-4451	-1.05	-5.86
24	35-36	600	400	130	25405	1.04	0.65
25	36-37	200	400	110	1195	0.44	1.62
26	37-38	600	500	130	23685	0.97	0.71
27	38-39	200	500	110	1115	0.41	0.71
28	39-40	500	400	120	-8	0.00	0.00
29	40-41	400	350	120	21250	1.25	5.12
30	41-42	200	950	110	1750	0.67	3.28
31	42-43	450	500	120	-2100	-0.15	-0.04
32	43-44	450	500	120	21200	1.54	3.55
33	44-45	450	300	120	-3100	-0.23	-0.05
34	45-46	250	1550	110	-1400	-0.92	-13.65
35	46-47	250	200	120	-7000	-1.65	-2.45
36	47-48	300	450	120	7000	1.15	2.27
37	48-49	400	300	130	12400	1.14	1.08
38	49-50	400	300	120	900	0.08	0.01
39	50-51	350	700	120	6500	0.79	1.50
40	51-52	350	550	120	3300	0.40	0.33
41	52-53	250	450	130	-5600	-1.22	-3.33
42	53-54	600	10	130	-5600	-0.23	-0.00

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	24.00	1430.00	64.60	40.60	51.00
2	22.00	2300.00	61.47	39.47	53.00
3	25.00	1080.00	57.88	32.88	50.00
4	28.00	2880.00	48.79	18.79	47.00
5	10.00	3060.00	35.52	25.52	65.00
6	5.00	1080.00	25.33	20.33	70.00
7	5.00	3960.00	13.12	8.12	70.00
8	5.00	2970.00	15.97	10.97	70.00
9	5.00	990.00	19.92	14.92	70.00
10	21.00	2030.00	61.73	40.73	54.00
11	16.00	3240.00	45.42	27.42	57.00
12	14.00	1820.00	35.98	21.98	61.00
13	13.00	900.00	27.31	17.31	65.00
14	10.00	1260.00	29.43	19.43	65.00
15	10.00	1080.00	27.34	17.34	65.00
16	5.00	1080.00	16.58	11.58	70.00
17	9.00	3240.00	20.39	11.39	66.00
18	10.00	1980.00	27.58	17.58	65.00
19	15.00	130.00	17.18	12.18	70.00
20	25.00	2320.00	33.48	8.48	50.00
21	16.00	1980.00	31.81	15.81	59.00
22	14.00	1980.00	38.56	24.56	61.00
23	12.00	1310.00	52.38	40.38	63.00
24	5.00	2770.00	17.18	12.18	70.00
25	10.00	6160.00	42.95	32.95	65.00
26	10.00	2880.00	45.71	35.71	65.00
27	8.00	4530.00	36.93	27.93	66.00
28	8.00	1980.00	32.26	23.26	66.00
29	10.00	1980.00	30.40	20.40	66.00
30	10.00	900.00	25.39	15.39	65.00
31	5.00	1170.00	22.41	17.41	70.00
32	5.00	3510.00	17.41	12.41	70.00
33	10.00	1080.00	27.74	17.74	65.00
34	10.00	3300.00	31.26	21.26	65.00
35	5.00	1130.00	23.22	15.22	70.00
36	5.00	3370.00	17.19	12.19	70.00
37	5.00	3300.00	31.83	22.83	66.00
38	8.00	3240.00	29.45	20.45	66.00
39	8.00	2880.00	26.80	17.80	66.00
40	8.00	900.00	24.98	16.98	67.00
41	8.00	2700.00	23.27	15.27	67.00
42	8.00	2880.00	23.96	15.96	67.00
43	8.00	1620.00	22.05	14.05	67.00
44	8.00	2700.00	22.06	14.06	67.00
45	10.00	1980.00	20.69	10.69	65.00
46	5.00	5180.00	13.62	8.62	70.00
47	8.00	360.00	18.15	10.15	67.00
48	8.00	2880.00	21.69	13.69	67.00
49	8.00	2160.00	21.46	13.46	67.00
50	8.00	2880.00	21.37	13.37	67.00
51	8.00	5940.00	17.02	9.02	67.00
52	5.00	5940.00	16.06	11.06	70.00
53	10.00	3060.00	20.23	10.23	65.00
54	5.00	3060.00	17.58	8.58	65.00
55	25.00	0.00	34.41	9.41	50.00

Iteration Times : 42

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
56	5.00	7780.00	15.12	10.12	70.00
57	15.00	360.00	26.09	11.09	60.00
58	70.00	0.00	74.97	4.97	5.00
59	75.00	0.00	75.00	0.00	0.00
60	55.00	0.00	55.00	0.00	20.00

ALTERNATIVE D-1-A (Recommended Plan, Single Pipeline Alignment)
2 Reservoir System, Year 2010

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	1	58	450	130	-30730	-1.26	-10.37
2	2	350	400	120	13312	1.60	3.13
3	3	400	500	120	15987	1.47	7.84
4	4	350	500	120	11012	1.32	5.74
5	5	300	1150	120	9932	1.63	5.51
6	6	300	2250	120	7052	1.15	11.25
7	7	250	2350	120	3992	0.94	10.19
8	8	200	2350	110	2912	1.07	12.21
9	9	200	2250	110	-1048	-0.39	-2.85
10	10	250	900	120	-4018	-0.95	-3.95
11	11	300	2050	120	-5837	-0.95	-7.39
12	12	150	1000	110	830	0.54	3.34
13	13	300	900	120	13951	2.29	16.31
14	14	300	850	120	10717	1.75	9.44
15	15	300	850	120	8797	1.44	6.55
16	16	300	450	120	-6737	-1.10	-2.12
17	17	250	1600	120	-3337	-0.79	-4.98
18	18	250	450	120	4137	0.98	2.08
19	19	250	1050	120	5019	1.18	6.95
20	20	250	200	120	-1952	-0.46	-0.23
21	21	150	1650	110	-250	-0.16	-0.06
22	22	200	950	120	1779	0.66	3.21
23	23	200	1000	120	-3942	-0.93	-4.24
24	24	250	1250	110	11	0.01	0.00
25	25	250	500	120	-2520	-0.59	-0.92
26	26	250	750	120	-5922	-1.40	-6.75
27	27	250	900	120	-7902	-1.86	-13.82
28	28	250	250	120	-10413	-3.14	-10.47
29	29	250	550	120	26382	2.43	9.43
30	30	250	550	120	7664	1.81	14.51
31	31	250	600	130	58407	2.39	5.67
32	32	250	750	110	3748	1.01	5.67
33	33	200	1000	110	-2252	-0.83	-5.23
34	34	200	600	120	2606	1.99	6.02
35	35	250	600	120	6278	1.48	6.02
36	36	250	2150	130	56657	2.28	14.88
37	37	250	2150	110	2619	0.96	14.88
38	38	250	650	120	5244	1.24	4.67
39	39	250	650	120	18052	1.56	4.67
40	40	250	250	120	8627	1.41	1.85
41	41	250	800	120	2037	0.48	1.00
42	42	250	350	120	11551	1.39	4.52
43	43	250	450	120	6647	1.57	5.02
44	44	250	350	120	5747	1.36	2.98
45	45	250	900	110	2326	0.86	5.00
46	46	250	900	110	-1184	-0.88	-5.81
47	47	350	900	120	-10471	-1.26	-4.52
48	48	350	1100	120	-1263	-0.30	-0.57
49	49	350	900	120	8157	1.34	6.03
50	50	350	1350	120	4787	0.78	3.37
51	51	350	400	130	51299	2.10	2.38
52	52	350	400	110	2414	0.89	2.38
53	53	350	500	130	48205	1.97	2.63
54	54	350	500	110	2268	0.84	2.63
55	55	500	400	120	25377	1.50	1.8

ALTERNATIVE D-1-A (Recommended Plan, Single Pipeline Alignment)
2 Reservoir System, Year 2010

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
55	39	42	500	120	20194	1.19	2.83
56	39	42	200	110	1653	0.61	2.83
57	39	57	350	110	360	0.24	0.71
58	40	41	500	120	24777	1.44	1.70
59	41	42	250	120	-2280	-0.54	-0.69
60	41	43	450	120	13762	1.00	1.22
61	41	44	350	120	10295	1.24	1.22
62	41	44	350	120	16996	1.22	2.28
63	42	48	450	120	12142	0.88	0.58
64	43	49	450	120	1980	0.47	1.57
65	44	45	250	110	5615	0.68	1.82
66	44	53	350	120	-393	-0.26	-0.84
67	46	56	180	110	-853	-2.01	-3.54
68	47	48	250	120	8173	1.34	3.03
69	47	56	300	120	5283	0.49	0.22
70	48	49	400	120	3385	0.31	0.10
71	49	50	400	120	11880	1.43	4.44
72	49	51	350	120	555	0.33	1.73
73	50	53	180	110	5940	0.71	0.91
74	51	52	350	120	3060	0.72	2.35
75	53	54	250	1000	-5857	-1.38	-4.56
76	55	58	250	4600	-36587	-1.50	-0.03
77	58	59	600	130			-3.18

ALTERNATIVE D-1-B (Parallel Pipeline Alignment of Recommended Plan)
2 Reservoir System, Year 1995

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	24.00	0.00	44.00	20.00	51.00
2	22.00	0.00	42.00	20.00	53.00
3	25.00	0.00	45.00	20.00	50.00
4	28.00	0.00	48.00	20.00	47.00
5	10.00	0.00	30.00	20.00	65.00
6	5.00	0.00	25.00	20.00	70.00
7	5.00	0.00	25.00	20.00	70.00
8	5.00	0.00	25.00	20.00	70.00
9	5.00	0.00	25.00	20.00	70.00
10	21.00	0.00	41.00	20.00	54.00
11	12.00	1400.00	38.00	20.43	57.00
12	10.00	730.00	43.77	33.77	59.00
13	10.00	730.00	44.96	34.96	55.00
14	10.00	730.00	44.69	34.69	55.00
15	10.00	730.00	44.69	34.69	55.00
16	5.00	0.00	25.00	20.00	70.00
17	9.00	900.00	29.95	20.95	65.00
18	10.00	1100.00	44.73	34.73	55.00
19	5.00	900.00	25.21	21.27	70.00
20	25.00	0.00	45.00	20.00	50.00
21	16.00	0.00	46.29	30.29	59.00
22	14.00	1100.00	48.66	34.66	61.00
23	12.00	1000.00	53.55	41.55	63.00
24	5.00	0.00	25.00	20.00	70.00
25	10.00	2000.00	42.35	32.35	65.00
26	10.00	1500.00	48.43	38.43	65.00
27	9.00	1200.00	38.03	27.03	65.00
28	9.00	1500.00	31.35	22.35	65.00
29	10.00	600.00	27.20	17.20	65.00
30	10.00	600.00	22.15	12.15	65.00
31	5.00	700.00	19.41	14.41	70.00
32	5.00	2100.00	15.27	10.27	70.00
33	10.00	800.00	29.01	19.01	65.00
34	10.00	1700.00	31.81	21.81	65.00
35	5.00	0.00	25.00	20.00	70.00
36	5.00	0.00	25.00	20.00	70.00
37	5.00	1700.00	34.71	25.71	66.00
38	5.00	1800.00	32.80	23.80	66.00
39	5.00	1800.00	30.68	21.68	66.00
40	5.00	0.00	27.85	19.85	67.00
41	8.00	2100.00	25.01	17.01	67.00
42	8.00	1800.00	26.39	18.39	67.00
43	8.00	1800.00	21.06	13.06	67.00
44	10.00	1800.00	22.19	14.19	67.00
45	5.00	0.00	30.00	20.00	65.00
46	5.00	1400.00	13.16	8.16	70.00
47	5.00	0.00	18.79	10.79	70.00
48	5.00	1800.00	19.80	11.80	67.00
49	5.00	1800.00	19.16	11.16	67.00
50	5.00	900.00	19.08	11.08	67.00
51	5.00	3300.00	14.32	6.32	70.00
52	10.00	0.00	20.00	20.00	65.00
53	5.00	0.00	20.00	20.00	65.00
54	5.00	0.00	20.00	20.00	65.00
55	25.00	0.00	33.71	27.71	60.00

ALTERNATIVE D-1-B (Parallel Pipeline Alignment of Recommended Plan)
2 Reservoir System, Year 1995

<< PIPELINE >>

PIPE No.	NODE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m)
1	12-14	250	850	120	-1400	-0.33	-0.53
2	13-14	150	450	110	-730	-0.48	-1.19
3	14-55	250	1600	120	-4238	-1.00	-7.75
4	15-250	250	450	120	1358	0.32	0.60
5	17-150	1050	1050	110	1800	1.16	14.73
6	17-150	200	200	120	-1162	-0.27	-0.09
7	17-150	150	950	110	900	0.59	3.89
8	18-21	250	1000	120	-2262	-0.53	-1.51
9	21-22	250	750	120	-3352	-0.79	-2.37
10	22-23	250	900	120	-4562	-1.08	-3.55
11	23-60	600	250	130	-48562	-1.34	-5.34
12	23-25	250	650	120	8450	1.99	11.30
13	23-25	250	750	110	2629	0.97	5.22
14	23-25	500	750	120	31922	1.88	5.22
15	25-27	250	600	120	6450	1.52	6.33
16	25-37	200	2150	110	2507	0.92	13.72
17	26-37	500	2150	120	30443	1.79	13.72
18	27-28	250	650	120	5250	1.24	4.68
19	28-29	250	250	110	4200	1.55	4.15
20	28-34	250	800	120	-1350	-0.32	-0.47
21	28-33	150	750	110	800	0.52	2.34
22	29-30	200	450	110	3400	1.25	5.05
23	30-31	200	350	110	2800	1.03	2.74
24	31-32	200	900	120	2100	0.77	4.14
25	34-37	250	1100	120	-3950	-0.72	-2.90
26	37-38	200	400	110	2145	0.79	1.91
27	37-38	500	400	120	26556	1.54	1.91
28	38-39	200	500	110	2008	0.74	2.12
29	38-39	500	500	120	24392	1.44	2.12
30	39-40	350	400	120	12612	1.52	2.84
31	39-42	250	950	110	2084	0.77	4.30
32	39-40	200	950	110	2084	0.77	4.30
33	40-41	350	400	120	12612	1.52	2.84
34	41-43	300	500	120	6912	1.46	2.84
35	41-44	150	250	110	1500	1.05	2.84
36	42-48	300	650	120	10188	1.67	6.57
37	45-49	300	300	120	7912	1.30	6.37
38	45-56	200	1550	110	-1400	-0.52	-3.36
39	47-48	300	200	120	-7000	-1.01	-2.27
40	47-55	300	450	120	-7000	-1.01	-2.27
41	48-49	300	300	110	1388	0.51	0.64
42	49-50	350	300	120	900	0.21	0.08
43	49-51	300	700	120	6500	3.17	4.63
44	51-52	350	550	120	3300	0.78	3.05
45	55-55	350	4500	120	-4238	-1.00	-22.28
46	55-59	350	10	120	-4237	-0.51	-0.01

ALTERNATIVE D-1-B (Parallel Pipeline Alignment of Recommended Plan)
2 Reservoir System, Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	24.00	1430.00	64.33	40.33	51.00
2	22.00	2300.00	61.19	39.19	53.00
3	25.00	1080.00	57.60	32.60	50.00
4	28.00	2880.00	46.50	18.50	47.00
5	10.00	3060.00	35.22	25.22	65.00
6	5.00	1080.00	25.01	20.01	70.00
7	5.00	3360.00	12.76	7.76	70.00
8	5.00	2970.00	13.59	10.59	70.00
9	5.00	990.00	19.53	14.53	70.00
10	21.00	2030.00	61.30	40.30	54.00
11	18.00	3240.00	53.14	35.14	57.00
12	14.00	1920.00	42.95	28.95	61.00
13	10.00	900.00	27.51	17.51	65.00
14	10.00	1260.00	31.21	21.21	65.00
15	10.00	1080.00	29.04	19.04	65.00
16	5.00	1080.00	14.09	9.09	70.00
17	9.00	3240.00	18.73	9.73	66.00
18	10.00	1980.00	29.18	19.18	65.00
19	5.00	1510.00	16.38	11.38	70.00
20	25.00	2520.00	34.95	9.95	50.00
21	16.00	1980.00	32.55	15.55	59.00
22	14.00	1980.00	35.37	24.37	61.00
23	12.00	1310.00	50.74	36.74	63.00
24	5.00	2270.00	16.46	11.46	70.00
25	10.00	6160.00	42.13	32.13	65.00
26	10.00	2880.00	46.02	36.02	65.00
27	8.00	4590.00	36.74	27.74	65.00
28	9.00	1980.00	32.66	23.66	66.00
29	10.00	1980.00	30.45	20.45	66.00
30	10.00	900.00	25.30	15.30	65.00
31	5.00	1170.00	22.21	17.21	70.00
32	5.00	3510.00	16.78	11.78	70.00
33	10.00	1080.00	29.50	19.50	65.00
34	10.00	3300.00	32.25	22.25	65.00
35	5.00	1130.00	21.57	16.57	70.00
36	5.00	3370.00	16.90	11.90	70.00
37	5.00	3300.00	33.62	24.62	66.00
38	9.00	3240.00	31.18	22.18	66.00
39	9.00	2880.00	28.46	19.46	66.00
40	8.00	900.00	25.92	18.92	67.00
41	8.00	2700.00	24.86	16.86	67.00
42	8.00	2880.00	25.41	17.41	67.00
43	8.00	1620.00	23.64	15.64	67.00
44	8.00	2700.00	22.91	14.91	67.00
45	10.00	1980.00	21.49	11.49	65.00
46	5.00	5180.00	14.69	8.69	70.00
47	8.00	350.00	21.64	13.64	67.00
48	8.00	2880.00	23.42	15.42	67.00
49	8.00	1600.00	23.04	15.04	67.00
50	8.00	2880.00	22.87	14.87	67.00
51	8.00	5940.00	19.19	11.19	67.00
52	5.00	5940.00	17.81	12.81	70.00
53	10.00	3060.00	21.16	11.16	65.00
54	9.00	3060.00	18.51	9.51	65.00
55	25.00	0.00	35.87	10.87	50.00

Iteration Times : 32

ALTERNATIVE D-1-B (Parallel Pipeline Alignment of Recommended Plan)
2 Reservoir System, Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu. m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
56	5.00	7780.00	17.91	12.91	70.00
57	15.00	360.00	27.74	12.74	60.00
58	70.00	0.00	74.98	4.98	5.00
59	75.00	0.00	75.00	0.00	0.00
60	55.00	0.00	55.00	0.00	20.00

ALTERNATIVE D-1-B (Parallel Pipeline Alignment of Recommended Plan)
2 Reservoir System, Year 2010
<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	32 35	150	900	110	-1078	-0.71	-4.88
57	33 35	300	900	120	9390	1.54	7.83
58	34 37	250	1100	120	-2037	-0.46	-1.25
59	35 36	300	900	120	7182	1.18	4.77
60	36 45	300	1350	120	3812	0.62	2.21
61	37 38	500	400	120	29715	1.75	2.44
62	37 38	200	400	110	22447	0.90	6.10
63	37 38	450	400	120	22524	1.64	2.44
64	38 39	500	500	120	27956	1.65	2.72
65	38 39	200	500	110	2302	0.85	5.45
66	38 39	450	500	120	21150	1.54	2.72
67	38 39	150	1000	110	17534	0.24	1.51
68	39 40	450	400	120	11514	1.08	3.05
69	39 40	350	400	120	9054	1.09	3.21
70	39 40	350	950	120	8217	0.99	1.53
71	39 42	350	950	110	1729	0.64	3.05
72	39 42	200	950	120	15076	1.38	2.06
73	40 41	400	400	120	10812	1.28	5.15
74	41 42	250	450	120	-2011	-0.47	-0.55
75	41 43	400	500	120	10093	0.93	1.22
76	41 44	300	250	120	8858	1.45	1.95
77	41 44	300	500	120	4735	0.78	1.22
78	41 44	150	250	110	1312	0.89	1.95
79	41 44	400	650	120	11385	1.05	3.05
80	42 48	300	300	120	5343	0.87	1.99
81	42 48	400	300	120	891	0.69	0.59
82	43 49	300	300	120	4219	0.69	0.59
83	43 49	300	1200	120	1880	0.47	1.42
84	45 53	350	1150	120	5490	0.66	1.52
85	45 56	200	1550	110	-1368	-0.50	-3.22
86	47 48	300	200	120	-9508	-1.56	-8.90
87	47 56	300	450	120	9148	1.50	3.73
88	48 49	300	300	120	3299	0.54	0.38
89	48 49	200	300	110	1041	0.38	0.38
90	49 50	300	300	120	2168	0.38	0.38
91	49 51	250	300	120	4543	1.07	3.86
92	49 51	250	700	120	1342	0.32	0.17
93	49 51	300	700	120	1337	1.20	3.86
94	50 53	150	850	110	630	0.41	1.71
95	50 53	250	550	120	2970	0.70	1.38
96	51 52	250	550	120	2970	0.70	1.38
97	51 52	250	1000	120	3056	0.72	2.55
98	52 54	350	4600	120	-5742	-1.35	-38.10
99	55 58	300	10	120	-30174	-1.24	-0.02
100	58 59	350	10	120	-6749	-0.81	-0.02
101	58 59	350	10	120			

ALTERNATIVE D-1-B (Parallel Pipeline Alignment of Recommended Plan)
2 Reservoir System, Year 2010
<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	58	600	4500	130	-3184	-1.28	-10.65
2	1 2	350	400	120	13317	1.60	3.14
3	1 10	400	500	120	16438	1.51	3.02
4	2 3	300	650	120	11017	1.33	3.99
5	4 5	300	1150	120	9937	1.63	11.11
6	5 6	300	2200	120	7057	1.16	11.27
7	6 7	350	2350	120	3997	0.94	10.21
8	7 8	200	1350	110	2917	1.07	12.25
9	8 9	250	2250	110	-1043	-0.38	-2.83
10	9 10	250	300	120	-4013	-0.95	-3.94
11	10 11	150	1000	110	1080	0.71	5.45
12	11 12	300	2050	120	-6083	-1.00	-7.98
13	12 13	300	900	120	14408	1.73	8.16
14	13 14	300	850	120	11188	1.83	10.19
15	14 15	150	850	110	1785	1.17	11.74
16	15 16	250	850	120	7453	1.76	11.74
17	16 17	250	450	120	-5635	-1.33	-3.69
18	17 18	150	450	110	-1348	-0.88	-3.69
19	18 19	250	1600	120	-3222	-0.76	-2.92
20	19 20	250	450	120	4236	1.17	2.32
21	20 21	250	1050	110	3165	1.00	10.37
22	21 22	250	1650	110	-1583	-0.95	-10.32
23	22 23	250	200	120	1583	0.35	0.14
24	23 24	150	550	110	765	0.46	2.47
25	24 25	150	850	110	765	0.46	2.47
26	25 26	250	1000	120	-3483	-0.82	-3.33
27	26 27	250	150	120	-100	-0.07	-0.07
28	27 28	250	550	120	-2520	-0.59	-0.85
29	28 29	250	750	120	-5463	-1.29	-5.81
30	29 30	250	900	120	-7443	-1.76	-12.37
31	30 31	250	250	120	-13505	-2.21	-17.05
32	31 32	300	400	120	25118	2.31	4.26
33	32 33	300	750	120	30209	2.31	4.61
34	33 34	300	250	120	-9558	-3.71	-17.05
35	34 35	300	650	120	1291	1.72	8.61
36	35 36	250	650	120	2487	0.92	4.72
37	36 37	250	750	120	30209	1.78	4.72
38	37 38	250	1000	110	-2370	-0.87	-5.75
39	38 39	250	600	120	20345	1.87	5.38
40	39 40	250	500	120	5910	1.39	5.38
41	40 41	250	210	120	2826	1.70	2.40
42	41 42	250	2150	110	2374	0.37	12.40
43	42 43	250	2150	120	26326	1.70	12.40
44	43 44	250	650	120	16786	1.55	4.08
45	44 45	250	650	120	4877	1.15	4.08
46	45 46	250	250	120	5864	1.38	2.21
47	46 47	250	250	120	9529	1.15	3.16
48	47 48	250	250	120	2989	1.10	2.21
49	48 49	250	250	120	1263	0.30	0.41
50	49 50	250	750	110	941	0.62	3.16
51	50 51	250	450	110	3436	1.27	5.15
52	51 52	250	450	110	3436	1.27	5.15
53	52 53	250	350	110	2986	1.10	3.09
54	53 54	250	350	110	2986	1.10	3.09
55	54 55	250	350	110	2432	0.90	2.43

ALTERNATIVE D-2
3 Reservoir System, Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
1	24.00	1430.00	63.45	39.45	51.00
2	22.00	2300.00	61.81	39.81	53.00
3	25.00	1880.00	56.19	33.19	50.00
4	28.00	2880.00	48.98	18.98	41.00
5	10.00	3060.00	33.57	25.57	65.00
6	5.00	1080.00	25.12	20.12	70.00
7	5.00	3960.00	12.50	7.50	70.00
8	5.00	2870.00	15.09	10.09	70.00
9	5.00	2990.00	18.94	13.94	70.00
10	21.00	2030.00	59.98	36.98	54.00
11	11.00	3240.00	50.42	32.42	57.00
12	14.00	1920.00	37.95	23.95	61.00
13	10.00	900.00	26.62	16.62	65.00
14	10.00	1260.00	28.81	18.81	65.00
15	10.00	1080.00	24.92	14.92	65.00
16	5.00	1080.00	14.20	9.20	70.00
17	5.00	3240.00	17.75	8.75	65.00
18	10.00	1980.00	24.93	14.93	65.00
19	5.00	1510.00	14.27	9.27	70.00
20	25.00	2620.00	32.97	7.97	50.00
21	15.00	1980.00	26.58	10.58	59.00
22	14.00	1980.00	30.39	16.39	61.00
23	12.00	1310.00	39.54	27.54	63.00
24	15.00	2770.00	14.14	6.14	70.00
25	10.00	6162.00	32.42	22.42	65.00
26	10.00	2880.00	29.53	19.53	65.00
27	9.00	4590.00	28.18	19.18	66.00
28	9.00	1090.00	25.13	15.13	66.00
29	10.00	1980.00	23.30	13.30	65.00
30	10.00	900.00	21.26	11.26	65.00
31	5.00	1170.00	18.33	13.33	70.00
32	5.00	3510.00	16.66	11.66	70.00
33	10.00	1080.00	22.62	12.62	65.00
34	10.00	3300.00	25.08	15.08	65.00
35	5.00	1130.00	17.68	12.68	70.00
36	5.00	3370.00	15.05	10.05	70.00
37	5.00	3300.00	27.55	18.55	66.00
38	5.00	3240.00	37.89	28.89	66.00
39	5.00	2580.00	34.94	25.94	66.00
40	5.00	3000.00	31.12	23.12	67.00
41	5.00	2700.00	27.51	19.51	67.00
42	5.00	2880.00	27.67	19.67	67.00
43	5.00	1620.00	24.41	15.41	67.00
44	5.00	2700.00	25.50	17.50	67.00
45	10.00	1980.00	20.55	10.55	65.00
46	5.00	5180.00	14.47	8.47	70.00
47	5.00	360.00	21.30	13.30	67.00
48	5.00	2880.00	22.95	14.99	67.00
49	5.00	2160.00	22.88	14.88	67.00
50	5.00	2880.00	22.70	14.70	67.00
51	5.00	5940.00	18.44	10.44	67.00
52	5.00	5940.00	16.39	11.39	70.00
53	10.00	3060.00	19.74	9.74	65.00
54	25.00	3060.00	33.59	8.59	50.00

Iteration Times : 29

ALTERNATIVE D-2
3 Reservoir System, Year 2010

<< NODES >>

NODE No.	GROUND ELEV. (m)	FLOW (cu.m/day)	H.G.L. ELEV. (m)	DYNAMIC HEAD (m)	STATIC HEAD (m)
56	5.00	7780.00	16.45	11.45	70.00
57	15.00	360.00	29.81	14.81	50.00
58	70.00	0.00	74.97	4.97	5.00
59	75.00	0.00	75.00	0.00	0.00
60	40.00	0.00	40.00	0.00	35.00
61	40.00	0.00	40.00	0.00	35.00

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
56	39	57	100.	100.	360.	0.53	5.13
57	40	41	450.	120.	27821.	2.02	5.13
58	41	42	200.	110.	-522.	-0.19	-0.35
59	41	43	400.	120.	16550.	1.53	3.09
60	41	44	300.	120.	8994.	1.47	2.01
61	42	48	400.	120.	18053.	1.66	4.57
62	43	49	400.	120.	15030.	1.58	1.54
63	44	45	200.	120.	1980.	0.73	4.35
64	44	53	250.	1150.	4314.	1.02	5.76
65	45	56	300.	1550.	-3336.	-0.55	-1.98
66	47	48	350.	200.	11476.	-1.38	-1.19
67	47	55	300.	450.	11115.	1.82	5.35
68	48	49	400.	300.	3597.	0.34	0.11
69	49	50	400.	300.	4586.	0.43	0.18
70	49	51	350.	700.	11880.	1.43	4.44
71	50	53	200.	850.	1806.	0.67	2.96
72	51	52	300.	550.	5940.	0.97	2.05
73	53	54	250.	1000.	3060.	0.72	2.65
74	55	58	250.	4600.	-5895.	-1.39	-41.07
75	58	59	500.	130.	-38414.	-1.57	-0.03

<< PIPELINE >>

PIPE No.	PIPE No. from-to	DIA. (mm)	LENGTH (m)	H-W C	FLOW (cu.m/day)	VEL. (m/sec)	HEADLOSS (m) (0/00)
1	1	58	4500.	130.	-32518.	-1.33	-11.51
2	1	10	400.	120.	13365.	1.23	1.65
3	1	10	500.	120.	17723.	1.63	3.47
4	2	3	350.	120.	11655.	1.33	3.52
5	3	4	1150.	120.	9883.	1.63	11.21
6	4	5	2200.	120.	7103.	1.16	11.42
7	5	6	2350.	120.	4045.	0.95	10.44
8	6	7	1450.	110.	2965.	1.09	12.63
9	7	8	2250.	110.	-935.	-0.37	-2.59
10	8	9	900.	120.	-3657.	-0.93	-7.85
11	9	13	2050.	120.	-5857.	-0.98	-7.88
12	10	11	1000.	110.	1002.	0.66	4.74
13	10	11	900.	120.	15933.	1.89	9.56
14	11	12	850.	120.	12453.	2.04	12.47
15	12	14	300.	850.	10533.	1.72	9.14
16	13	14	450.	120.	-6857.	-1.12	-2.19
17	14	15	1500.	120.	-3376.	-0.80	-5.09
18	14	15	450.	120.	3152.	1.37	3.89
19	15	17	250.	1050.	5099.	1.20	1.16
20	15	18	250.	120.	-387.	-0.09	-0.01
21	16	19	150.	1650.	-78.	-0.05	-0.07
22	17	19	200.	950.	1859.	0.68	3.49
23	18	21	250.	1000.	-2367.	-0.66	-1.65
24	19	24	200.	1250.	371.	0.10	0.13
25	20	25	250.	500.	-2520.	-0.59	-0.92
26	20	25	250.	750.	-4347.	-1.03	-3.81
27	21	23	250.	900.	-6327.	-1.49	-5.16
28	22	30	250.	120.	40623.	2.22	10.18
29	23	25	250.	650.	22665.	2.09	7.12
30	23	25	250.	550.	6855.	1.55	10.95
31	23	26	200.	750.	3736.	1.36	13.36
32	24	31	200.	1000.	-1989.	-0.74	-4.19
33	25	27	500.	600.	17892.	1.65	4.24
34	25	27	250.	120.	5198.	1.23	4.24
35	26	28	250.	110.	855.	0.32	1.87
36	27	28	250.	650.	4155.	0.98	3.05
37	28	29	400.	120.	14335.	1.32	3.05
38	28	30	250.	120.	8574.	1.40	1.84
39	28	34	350.	800.	437.	0.10	0.06
40	28	35	750.	120.	8408.	1.01	2.51
41	29	30	300.	450.	5894.	1.03	2.03
42	30	31	250.	350.	5854.	1.34	2.93
43	31	32	250.	900.	2326.	0.60	1.67
44	32	35	200.	900.	-984.	-0.36	-1.02
45	33	35	300.	120.	7328.	1.20	4.95
46	34	37	250.	1100.	-2863.	-0.67	-2.58
47	35	36	300.	900.	5214.	0.65	2.63
48	36	45	300.	1350.	1644.	0.30	0.58
49	37	38	200.	400.	-5307.	-1.95	-10.24
50	37	38	600.	110.	51016.	2.09	2.94
51	38	39	200.	500.	2401.	0.88	2.94
52	38	61	600.	250.	-6193.	-2.54	-2.11
53	39	40	450.	120.	28721.	2.09	3.83
54	39	42	400.	950.	18668.	1.72	7.28
55	42	200.	550.	110.	2757.	1.02	7.28

F. COST COMPARISON

General

Analysis and evaluation of alternative are based largely on present-worth cost studies, taking into consideration the salvage value after the design period. Cost comparison is based on present worth of net disbursement during the period of 1980-2010 without any escalation factor applied to the 1980 unit prices.

If the differences between net PW cost of an alternative and that of the least-cost alternative is within the limit of cost estimating accuracy (10-15%) further cost comparison shall be made applying escalation factor to 1980 unit prices. For escalation rates, refer to Chapter VII-C: Escalation Rates. Moreover, non-economic parameters may also be influence the selection of the recommended plan.

Construction Cost

Construction cost estimates of the proposed improvements are based on the projected July 1980 unit prices. All estimates on imported materials are based on an exchange rate of ₱7.40 per 1 US dollar. Further, it is assumed that no custom duty will be charged on items imported for the public water supply project. The cost of any facility to be replaced during the design period (1980-2010) is included under the capital cost for the particular year.

Annual Cost

Annual costs are all costs associated with the maintenance, operation, and management of the project. These include labor, power, chemical and maintenance costs. These estimates are carried out for the period 1980-2010. The present-worth cost of annual expenditure is based on uniform and gradient series at a given interest.

Personnel and maintenance costs may abruptly increase as additional facilities are put into operation - e.g., the power cost at a pump station increases in relation to the daily pumpage of water.

Salvage Value

The salvage values of facilities at the end of the design period 2010 are important in calculating net present worth of the total expenditures. It is assumed that the value of a facility depreciates linearly throughout its service life therefore, a facility with longer service life depreciates less than a facility with shorter service life (Refer to Table VI-1 for service life of different facilities). Moreover, a facility constructed at a later stage has higher salvage value than one constructed at an earlier stage.

TABLE VI-1

SERVICE LIFE CATEGORIES OF FACILITIES

Civil Works	Economic Life	Equipment	Economic Life
Wells	30 years	Wells (pumping engine or motors)	15 years
Springs	50	Springs (vales, pipes)	50
Transmission Mains	50	Transmission (pipes, valves)	50
Storage Facilities	50	Storage (valves, pipes, level gauge, etc.)	50
Disinfection Facilities	50	Disinfection facilities (chlorinators, mech-	
Distribution Mains	50	anical equipment and filter equipment,	
Internal Network	50	pipes, valves)	15
Service Connections	50	Distribution mains (pipes, valves)	50
Fire Hydrants	50	Internal networks (pipes, valves)	50
Operational Buildings	50	Service connections (meters, pipes)	50
		Operational buildings (workshop, etc.)	15
		Fire hydrants	30
		Vehicles	7

Net Present Worth

The net present worth cost of an alternative scheme is the difference between the total present worth of capital cost and annual cost minus the present worth of salvage values.

For Construction Cost:

$$C_n = C_c - C_s$$

$$C_c = C \times \frac{1}{(1+i)^n}$$

$$C_c = C \times \frac{1}{(1+i)^{nx}} \times \left(1 - \frac{nx - n}{SL}\right)$$

For Annual Cost:

$$C_c = A_c \times \frac{1}{(1+i)^n}$$

where,

C_n = net present worth comparable cost

C_c = present worth of construction cost

C_s = present worth of salvage value (design year)

C = construction cost

SL = service life

i = discount rate

nx = number of years between design year and base year

n = number of years between year of construction and base year

A_c = annual cost

Cabuyao-Sta. Rosa-Binan	UNIT COST	Phase 1 (Stage 1)	Phase 1 (Stage 2)	Phase 1 Total	Phase 1 Cost
ITEM		NUMBER	COST	NUMBER	COST
1 SOURCE FACILITY					
(1)DEEP WELL CONSTRUCTION	1160000	0	0	4	4640
(2)DEEP WELL PUMP w/HOUSE	790000	1	790	5	3950
Flow Meter D=150	62000	1	62	5	310
SUB-TOTAL			852		8900
2 TRANSMISSION FACILITIES					
(1)Pipe Protection					
0=200	251	400	100	400	100
0=300	337	400	135	400	135
(2)Main Pipes					
0=250 (Steel Pipe)	630	1100	693	1300	819
0=350 (Steel Pipe)	900	0	0	0	0
0=400 (Steel Pipe)	970	1100	1067	0	0
0=450 (Steel Pipe)	1160	1300	1508	0	0
0=500 (Steel Pipe)	1330	0	0	0	0
0=600 (Steel Pipe)	1600	0	0	0	0
0=700 (Steel Pipe)	1910	0	0	0	0
SUB-TOTAL			3503		819
3 DISTRIBUTION FACILITIES					
(1)Reservoir			3417	0	0
(2)Pump Facility (Equip.)			2090	3084	5174
-do- (Civil)			2666	0	2666
(3)Chlorin Facility 22kg/d	98100	2	196	1	98
- do - 45kg/d	119000	0	0	0	0
(4)Electric Sub-station			3643		
(5)Distribution pipes					
1)Main Pipes					
0=150 (PVC Pipe)	410	0	0	2950	1210
0=200 (Steel Pipe)	520	0	0	1850	962
0=250 (Steel Pipe)	630	2050	1292	0	0
0=300 (Steel Pipe)	760	2000	1520	2300	1748
0=350 (Steel Pipe)	900	1450	1305	1400	1260
0=400 (Steel Pipe)	970	600	582	0	0
0=450 (Steel Pipe)	1160	1450	1682	0	0
0=500 (Steel Pipe)	1330	950	1264	800	1064
0=600 (Steel Pipe)	1600	3800	6080	0	0
0=700 (Steel Pipe)	1910	250	478	0	0
2)Valves					
0=150 (Gate Valve)	5300	0	0	15	80
0=200 (Gate Valve)	6700	0	0	6	40
0=250 (Gate Valve)	11200	7	79	0	0
0=300 (Butterfly Valve)	34800	7	243	8	278
0=350 (Butterfly Valve)	74400	5	372	5	372
0=400 (Butterfly Valve)	95200	2	190	3	286
0=450 (Butterfly Valve)	125900	5	630	0	0
0=500 (Butterfly Valve)	174000	3	522	3	522
0=600 (Butterfly Valve)	243600	13	3167	0	0
0=700 (Butterfly Valve)	313200	1	313	0	0
3)Internal Network					
Commercial 150000/ha	25700	0	0	18	463
Commercial 250000/ha	30400	0	0	0	0
Residential 100000/ha	18700	52	972	0	0
Residential 150000/ha	21000	0	0	394	8274
Residential 250000/ha	30400	0	0	0	0
4)Service Conections					
0=1/2	810	4901	3970	15464	12525
0=3/4	1280	86	84	12	17
5)Rehabilitation					
Water Meter 1/2"	400	491	196	0	0
Water Meter 3/4"	880	0	0	0	0
Old Laterals			860	0	0
Service Connctn.w/Meter	480	246	117	0	0
Service Connctn.w/Meter	880	1208	1064	0	0
6)Flow Meter D=150	62000	1	62	0	0
-do- D=350	164000	1	164	0	0
7)Fire Protection					
0=150	16800	0	0	0	0
0=100	9400	0	0	0	0
SUB-TOTAL			39220		32283
4					
(1)Administration Bldg.					
(2)Operation Center		1	1583		1583
SUB-TOTAL			1583		1583
5 Land Acquisition					
Vehicle	300000	2	600	4	1200
Stored Material & Equip.			501		561
SUB-TOTAL			1281		1961
6 Replacement of Equipment					
TOTAL			46439		43111
7 Leak Detection					
GRAND TOTAL	240	2907	899	0	2907
			47138		90249

(Unit: thousand Pesos)

Cabuvao-Sta. Rosa-Binan		UNIT COST		1988		1989		1990		1991	
ITEM		NO	COST	NO	COST	NO	COST	NO	COST	NO	COST
1 SOURCE FACILITY											
(1) DEEP WELL CONSTRUCTION	1160000	0	0	0	0	0	0	0	0	1	1180
(2) DEEP WELL PUMP w/HOUSE	790000	0	0	1	790	0	0	0	0	1	790
Flow Meter 0=150	62000	0	0	1	62	0	0	0	0	1	62
SUB-TOTAL			0	2	852		0		0		2012
2 TRANSMISSION FACILITIES											
(1) Pipe Protection											
0=200	251		0	400	100		0		0		0
0=300	337		0	400	135		0		0		0
(2) Main Pipes											
0=250 (Steel Pipe)	830		0	1100	693		0		800		504
0=350 (Steel Pipe)	900		0	0	0		0		0		0
0=400 (Steel Pipe)	970		0	1100	1067		0		0		0
0=450 (Steel Pipe)	1160		0	1300	1508		0		0		0
0=500 (Steel Pipe)	1330		0	0	0		0		0		0
0=600 (Steel Pipe)	1600		0	0	0		0		0		0
0=700 (Steel Pipe)	1910		0	0	0		0		0		0
SUB-TOTAL			0		3503		0		0		504
3 DISTRIBUTION FACILITIES											
(1) Reservoir				1	3417						
(2) Pump Facility (Equip.)					2090						3084
-do- (Civil)					2866						
(3) Chirntrn Facility 22kx/d	98100			2	196		0		0	1	98
-do- 45kx/d	119000				0		0		0	0	0
(4) Electric Sub-station				1	3643						
(5) Distribution pipes		1988		1989		1990		1991			
1) Main Pipes											
0=150 (PVC Pipe)	410		0		0		0		2950		1210
0=200 (Steel Pipe)	520		0		0		0		1850		962
0=250 (Steel Pipe)	630		0	1050	662	1000	630		0		0
0=300 (Steel Pipe)	760		0	700	532	1300	988		2300		1748
0=350 (Steel Pipe)	900		0	750	675	700	630		1400		1260
0=400 (Steel Pipe)	970		0	300	291	300	291		0		0
0=450 (Steel Pipe)	1160		0	650	754	800	928		0		0
0=500 (Steel Pipe)	1330		0	950	1264		0		800		1064
0=600 (Steel Pipe)	1600		0	3800	6080		0		0		0
0=700 (Steel Pipe)	1910		0	250	478		0		0		0
2) Valves											
0=150 (Gate Valve)	5300		0		0		0		15		80
0=200 (Gate Valve)	6700		0		0		0		6		40
0=250 (Gate Valve)	11200		0	4	45	3	34		0		0
0=300 (Butterfly Valve)	34800		0	3	104	4	139		8		278
0=350 (Butterfly Valve)	74100		0	3	223	2	149		5		372
0=400 (Butterfly Valve)	95200		0	1	95	1	95		3		286
0=450 (Butterfly Valve)	125900		0	2	252	3	378		0		0
0=500 (Butterfly Valve)	174000		0	3	522		0		3		522
0=600 (Butterfly Valve)	243600		0	13	3167		0		0		0
0=700 (Butterfly Valve)	313200		0	1	313		0		0		0
3) Internal Network											
Commercial 15000p/ha	25700		0		0		0		4		103
Commercial 25000p/ha	30400		0		0		0		0		0
Residential 10000p/ha	18700		0	26	486	26	486				0
Residential 15000p/ha	21000		0		0		0		79		1659
Residential 25000p/ha	30400		0		0		0				0
4) Service Connections											
0=1/2	810		0	2451	1985	2450	1985		3093		2505
0=3/4	1280		0	33	42	33	42		3		4
5) Rehabilitation											
Water Meter 1/2"	400	491	196	0	0	0	0				0
Water Meter 3/4"	880	0	0	0	0	0	0				0
Old Laterals			0		430		430				0
Service Connctn.w/Meter	480	82	39	82	39	82	39				0
Service Connctn.w/Meter	880	403	355	403	355	402	354				0
6) Flow Meter 0=150	62000		0	1	62		0				0
-do- 0=350	164000		0	1	164		0				0
7) Fire Protection											
0=150	16800		0		0		0				0
0=100	9400		0		0		0				0
SUB-TOTAL			590		31032		7598				15275
4 Administration Bldg.											
(2) Operation Center				1	1583						
SUB-TOTAL			0	1	1583		0				0
5 Land Acquisition											
Vehicle	300000	2	600		0		0		2000		200
Stored Material & Equip.			11		387		103				243
SUB-TOTAL			791		387		103				743
6 Replacement of Equipment											
TOTAL			1381		37257		7701				18531
7 Leak Detection											
	210	969	233	969	233	969	233				0
GRAND TOTAL			1611		37590		7931				18531

(Unit: thousand Pesos)

Cabuyao-Sta.Rosa-Binan		UNIT COST	1992		1993		1994		1995	
ITEM			NO	COST	NO	COST	NO	COST	NO	COST
1 SOURCE FACILITY										
(1)DEEP WELL CONSTRUCTION	1160000		1	1160	1	1160	1	1160	0	0
(2)DEEP WELL PUMP w/HOUSE	790000		1	790	1	790	1	790	0	0
Flow Meter D=150	62000		1	62	1	62	1	62	0	0
SUB-TOTAL			3	2012	3	2012	3	2012	0	0
2 TRANSMISSION FACILITIES										
(1)Pipe Protection										
D=200	251		0	0	0	0	0	0	0	0
D=300	337		0	0	0	0	0	0	0	0
(2)Main Pipes										
D=250 (Steel Pipe)	630	200	126		200	126	100	63		0
D=350 (Steel Pipe)	900		0			0		0		0
D=400 (Steel Pipe)	970		0			0		0		0
D=450 (Steel Pipe)	1160		0			0		0		0
D=500 (Steel Pipe)	1330		0			0		0		0
D=600 (Steel Pipe)	1600		0			0		0		0
D=700 (Steel Pipe)	1910		0			0		0		0
SUB-TOTAL				126		126		63		0
3 DISTRIBUTION FACILITIES										
(1)Reservoir										
(2)Pump Facility (Equip.)										
-do- (Civil)										
(3)Chlorin Facility 22kg/d	98100		0	0	0	0	0	0	0	0
-do- 45kg/d	119000		0	0	0	0	0	0	0	0
(4)Electric Sub-station										
(5)Distribution pipes			1992		1993		1994		1995	
1)Main Pipes										
D=150 (PVC Pipe)	410			0		0		0		0
D=200 (Steel Pipe)	520			0		0		0		0
D=250 (Steel Pipe)	630			0		0		0		0
D=300 (Steel Pipe)	760			0		0		0		0
D=350 (Steel Pipe)	900			0		0		0		0
D=400 (Steel Pipe)	970			0		0		0		0
D=450 (Steel Pipe)	1160			0		0		0		0
D=500 (Steel Pipe)	1330			0		0		0		0
D=600 (Steel Pipe)	1600			0		0		0		0
D=700 (Steel Pipe)	1910			0		0		0		0
2)Valves										
D=150 (Gate Valve)	5300			0		0		0		0
D=200 (Gate Valve)	6700			0		0		0		0
D=250 (Gate Valve)	11200			0		0		0		0
D=300 (Butterfly Valve)	34800			0		0		0		0
D=350 (Butterfly Valve)	74400			0		0		0		0
D=400 (Butterfly Valve)	95200			0		0		0		0
D=450 (Butterfly Valve)	125900			0		0		0		0
D=500 (Butterfly Valve)	174000			0		0		0		0
D=600 (Butterfly Valve)	243600			0		0		0		0
D=700 (Butterfly Valve)	313200			0		0		0		0
3)Internal Network										
Commercial 150pop/ha	25700	4	103		4	103	3	77	3	77
Commercial 250pop/ha	30400		0			0		0		0
Residential 100pop/ha	18700		0			0		0		0
Residential 150pop/ha	21000	79	1659		79	1659	79	1659	78	1638
Residential 250pop/ha	30400		0			0		0		0
4)Service Connections										
D=1/2	810	3093	2505		3093	2505	3093	2505	3092	2505
D=3/4	1280	3	4		2	3	2	3	2	3
5)Renovation										
Water Meter 1/2"	400		0			0		0		0
Water Meter 3/4"	880		0			0		0		0
Old Laterals			0			0		0		0
Service Connctn.w/Meter	480		0			0		0		0
Service Connctn.v/Meter	880		0			0		0		0
6)Flow Meter D=150	62000		0			0		0		0
-do- D=350	164000		0			0		0		0
7)Fire Protection										
D=150	16800		0			0		0		0
D=100	9400		0			0		0		0
SUB-TOTAL				1271		1270		1211		1223
4 Administration Bldg.										
(2)Operation Center										
SUB-TOTAL				0		0		0		0
5 Land Acquisition										
Vehicle	300000	1	300		1	300	1	300		0
Stored Material & Equip.			85			85		81		64
SUB-TOTAL			385			385		381		64
6 Replacement of Equipment										
TOTAL				6791		6793		6703		4287
7 Leak Detection										
	240		0			0		0		0
GRAND TOTAL				6791		6793		6703		4287