- Alternative study for phasing the distribution network development is then conducted based on the selected network configuration.

7.2 FACTORS TO BE CONSIDERED FOR ALTERNATIVE STUDY

In evaluating alternatives, considered were; water demand for each design year, recommended water sources, design criteria for distribution system, storage requirements and locations, and other parameters relevant to the existing and proposed systems.

7.2.1 Planning Stages

In studying of alternatives, planning stages are considered both for the long term and short term development of water supply system as shown below:

Construc	ctio	n Phase	Design Period
Phase	I		1986 - 1995
Phase	II		1996 - 2010

The long term development shall be completed during the Phase II period, while the short term development by the year 1995.

The Phase I project is further divided into two stages; Stage 1 for 1990 and Stage 2 for 1995 taking into account of practical construction period for the required facilities, especially for internal network and service connections.

7.2.2 Water Demand in Each Design Year

The future water demand as projected in Chapter 5 covering barangays to be served in the respective target years is presented in TABLE 7.2.1.

TABLE 7.2.1 DEMAND PROJECTION BY BARANGAY (DAILY AVE. AND MAX.)

Unit: cu.m/day Design Year Municipality Barangay 1990 1995 2010 Cabuyao Barangay I, II & III 1,000 1,100 1,800 Bigaa 300 900 1,800 Sala 200 700 1,600 Baclaran 600 Banay-Banay 1,500 Banlic 1,600 Butong 600 Gulod 1,700 Mamatid 1,700 Marinig 2,200 Niugan 1,800 Pulo 1,700 San Isidro 600 Sub-Total 1,500 2,700 19,200 Sta. Rosa Barangay I, II & III 800 1,600 2,300 Aplaya 500 1,400 2,600 Balibago 800 1,500 2,900 Dila 1,100 200 600 Dita 500 1,100 2,200 Ibaba 100 300 500 Labas 200 1,300 600 Macabling 300 800 1,600 Tagapo 500 1,700 3,700 Caiñgin 2,100 Malitlit 1,400 Pook 2,500 Sinalhan 2,500 Sub-Total 3,900 9,600 26,700 Biñan De La Paz 1,000 3,300 6,600 1,100 7,200 Malaban 3,500 Platero 300 900 1,800 Poblacion 300 700 1,800 900 2,700 4,800 San Antonio 300 900 1,600 San Jose 1,600 3,000 San Vicente 500 200 500 900 Sto. Domingo 3,400 Canlalay 200 Casile San Anton 500 1,100 Tubigan 200 Zapote 4,600 14,100 32,500 Sub-Total 10,000 26,400 78,400 Daily Average Demand 94,000 12,500 33,000 Daily Maximum Demand

Note: Fluctuation factor (daily max./daily ave.): 1990 and 1995; 1.25 2010; 1.20

7.2.3 Water Sources Considered

Based on the study of existing and potential water sources performed in Chapter 6, the following source alternatives are considered.

(1) Realignment of Existing Deep Wells

Two existing deep wells in Biñan shall be abandoned due to low water production.

(2) Surface Water

The utilization of Laguna de Bay is a principal alternative of surface water, though it will require a set of water treatment facilities.

(3) Groundwater

The existing groundwater source, excluding the afore-mentioned two wells to be abandoned, is therefore a spring with a water production of about 9,300 cu.m/day.

For future development, the construction of deep wells, the utilization of additional springs and stand-by NIA wells are considered.

In the case of new deep well construction, Sta. Rosa area is identified to be dependable for exploiting about 5,500 cu.m/day from one well based on the hydrogeological studies. The proposed design parameter of deep well is as follows:

Proposed Deep Well	Design Parameter
Well depth	200 m
Casing diameter	300 mm
Estimated discharge	5,500 cu.m/day
Influence area	500 m (radius)

With regard to the standby NIA wells, some of them are no longer operating since the supposed irrigable area have already been converted into residential communities. In most cases, the wells were abandoned due

to stolen electric cables and switch boxes. Because of this, a list of NIA wells for possible future water supply sources was submitted to the NIA to seek for permission to utilize the said wells. During the discussions with a NIA official, it was confirmed that some wells, excluding the free-flowing ones may be converted for water supply purpose. However, observation wells constructed by the NIA cannot be utilized for this purpose.

The available amount of spring water is confirmed to be approximately 12,600 cu.m/day comprising 9,300 cu.m/day from the existing spring source (Matang-Tubig Spring) and 3,300 cu.m/day from other additional springs. Since these springs are located in the area owned by the Canlubang Sugar Estate Group, the negotiation with this owner as well as the approval from the National Water Resources Council is a prerequisite for exploitation of these additional springs.

The water right for the Canlubang Sugar Estate Group in the spring area was identified as shown in APPENDIX 7.2.1.

7.2.4 Distribution System

Alternative study of the distribution system includes the study of various alternative pipeline configurations and routing to arrive at the cheapest possible distribution system that can provide adequate water pressure in the proposed service area.

(1) Pressure Zone

The ground elevation of the proposed service area for the year 2010 varies from 1.5 m to 3.7 m above the mean sea level. In this regard, only one pressure zone is considered for Cabuyao-Sta. Rosa-Biñan area through the year 2010.

(2) System Pressure

In accordance with the LWUA Methodology Manual, the design water pressure in the main pipes with a diameter of more than 150 mm as indicated below will be achieved in the year 2010 (Phase II) for the hourly maximum flow:

o Max. static water pressure : 7.0 kg/sq.cm

o Min. dynamic water pressure : 0.7 kg/sq.cm for the residential area; 1.4 kg/sq.cm for the commer-

cial/industrial area.

During the Phase I period, a minor modification to the above-mentioned requirements may be adopted considering of the cost constraints as the intermediate measure of system improvement.

"C" value (Hazen Williams' Formula) being adopted in the said manual is also given below:

 ϕ 100 mm to ϕ 200 mm : C = 110 ϕ 250 mm to ϕ 500 mm : C = 120 ϕ 600 mm and over : C = 130

(3) Fire Protection

Full fire protection with adequate water pressure will be insured during the Phase II period. During the Phase I period, the existing fire hydrants will be utilized. The criteria on the location of fire hydrants as set forth in the LWUA Methodology Manual is summarized as follows:

	Residential Area	Commercial/ Industrial Area
Diameter of Fire Hydrants	100 mm	150 mm
Spacing	180 m	135 m
Hydrant Density	0.3 pc/ha	0.6 pc/ha

The number of fire hydrants can be determined either by spacing or by hydrant density criteria.

To verify the appropriateness of distribution system, a 22 1/sec of fire fighting water from each of two adjacent fire hydrants in the residential area will be checked by the computer-aided network analysis.

(4) Storage Facilities

In coming up with the most cost-effective storage facilities, water demand and storage volume will have to be ascertained. In compliance with the LWUA Methodology Manual, the optimum configuration of the storage facilities will be selected from among the following three alternative schemes:

- Daily maximum supply and maximum storage,
- Intermediate supply and storage (150% of the daily maximum supply), and
- Peak hour supply and minimum storage.

The location of storage facilities will also be studied in relation to the diameter of distribution pipes.

(5) Distribution Network

The most optimum distribution network will be determined from the view point of the least construction cost by means of the computer-aided network analysis by taking into account the afore-mentioned criteria of distribution system.

(6) Equipment/Materials and Unit Costs

The unit costs of water supply equipment and materials were inquired both in Manila and respective study area during the field survey.

The survey results revealed that almost all the equipment and materials are imported or partially manufactured in Manila area and distributed to local areas. These market prices which were mainly pipes and fittings, were compared with the LWUA Methodology Manual at the 1986 price level and it is concluded that the said Manual be referred to estimate the project cost.

With regard to pipe materials, PVC pipe for diameters up to 150 mm and steel pipe for diameters of 200 mm and above are utilized in this study based on the cost comparison. (See APPENDIX 7.2.2 for details)

- 7.3 ALTERNATIVE STUDY OF WATER SUPPLY SYSTEM
- 7.3.1 Water Source and Transmission
- (1) Water Source Alternatives
 - 1) Development of Alternatives

The alternative water sources in the study area are springs, groundwater and surface water of the Laguna de Bay.

The total water demand in the year 2010 was projected to be around 94,000 cu.m/day which is ten times the level of the present water production. Accordingly, a considerable amount of additional water will be required.

There are two alternatives for the development of additional water sources to meet the said water demand aside from the existing water sources, as enumerated below.

Alternative S-1

o Utilization of spring water and groundwater.

Alternative S-2

o Combination of existing springs, wells and surface water of the Laguna de Bay.

For water conveyance/transmission pipelines of the above water source alternatives, no alternative routing is available for comparison since the proposed pipeline routes are determined considering the road conditions for pipe installation with the shortest length possible. However, the configuration of pipe alignment will be subject to the cost comparison to establish the optimum plan when the staged construction program is introduced.

A schematic system configuration of the water source alternatives including transmission and treatment facilities is shown in FIGURE 7.3.1 and summarized in TABLE 7.3.1.

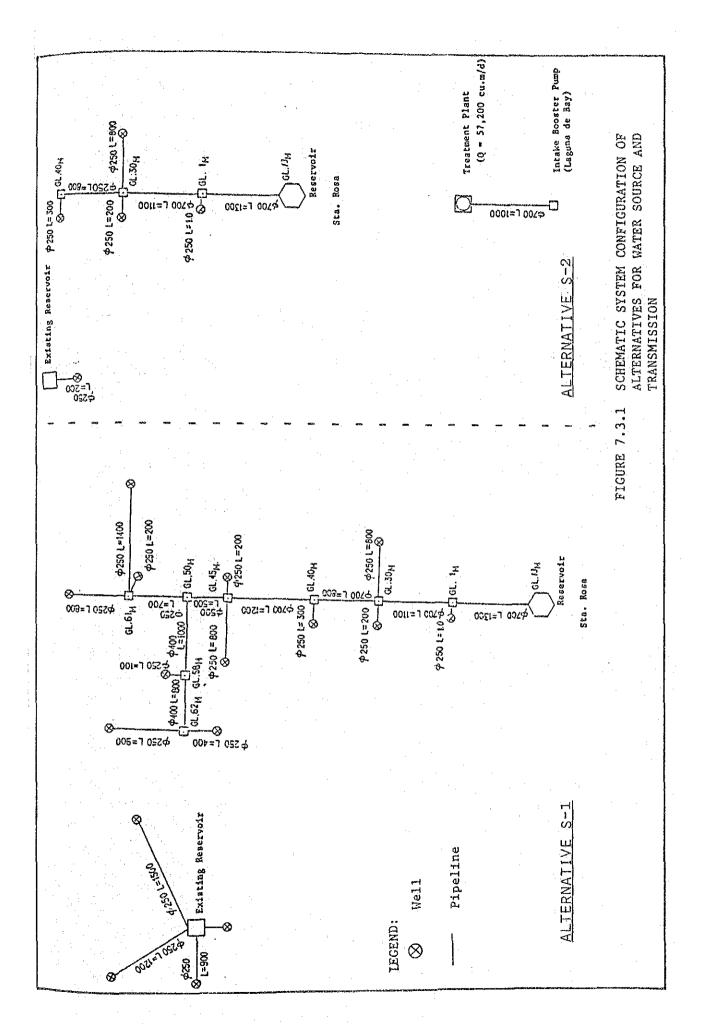


TABLE 7.3.1 MAJOR FACILITIES OF ALTERNATIVES FOR WATER SOURCE AND TRANSMISSION

Facilities	Alternative S-1	Alternative S-2
Water Source	Deep Well, 16 units (\$300mm, 200 m ^D)	Deep Well, 4 units (\$\phi 300mm, 200 m^D)
•		Water Intake, 1 unit (670 1/sec, Laguna de Bay)
Transmission Line	\$\phi700 \text{ mm} - 4,400 \text{ m}\$ \$\phi500 \text{ mm} - 500 \text{ m}\$ \$\phi400 \text{ mm} - 1,800 \text{ m}\$ \$\phi350 \text{ mm} - 700 \text{ m}\$ \$\phi250 \text{ mm} - 10,000 \text{ m}\$	\$\psi 700 \text{ mm} - 1,000 \text{ m} \\ \$\psi 450 \text{ mm} - 1,300 \text{ m} \\ \$\psi 400 \text{ mm} - 1,100 \text{ m} \\ \$\psi 250 \text{ mm} - 2,400 \text{ m}
Water Treatment	<u> </u>	Rapid Sand Filter (57,200 cu.m/day)

2) Cost Comparison

A cost comparison of water source alternatives including relevant facilities is summarized in TABLE 7.3.2 and details are given in AP-PENDIX 7.3.1.

TABLE 7.3.2 COST COMPARISON OF WATER SOURCE AND TRANSMISSION ALTERNATIVES

Facilities	Alternative S-1	Alternative S-2
Water Source	₽ 31,200,000	₽ 16,166,000
Transmission Line	17,745,000	5,997,000
Water Treatment	- -	61,204,000
Energy Consumption	23,337,000	14,508,000
(15 years)		ing disease of the second seco
Maintenance Cost	8,351,000	9,417,000
TOTAL	₽ 80,633,000	₽ 107,292,000

The cost comparison of two alternatives favors Alternative S-1 than Alternative S-2. Beside a lower cost in construction and operation/maintenance, Alternative S-1 has a potential advantage of flexibility on project implementation when it comes to the increase of water demand.

Thus, Alternative S-1 is recommended as the optimum plan for water source development toward the target year of 2010.

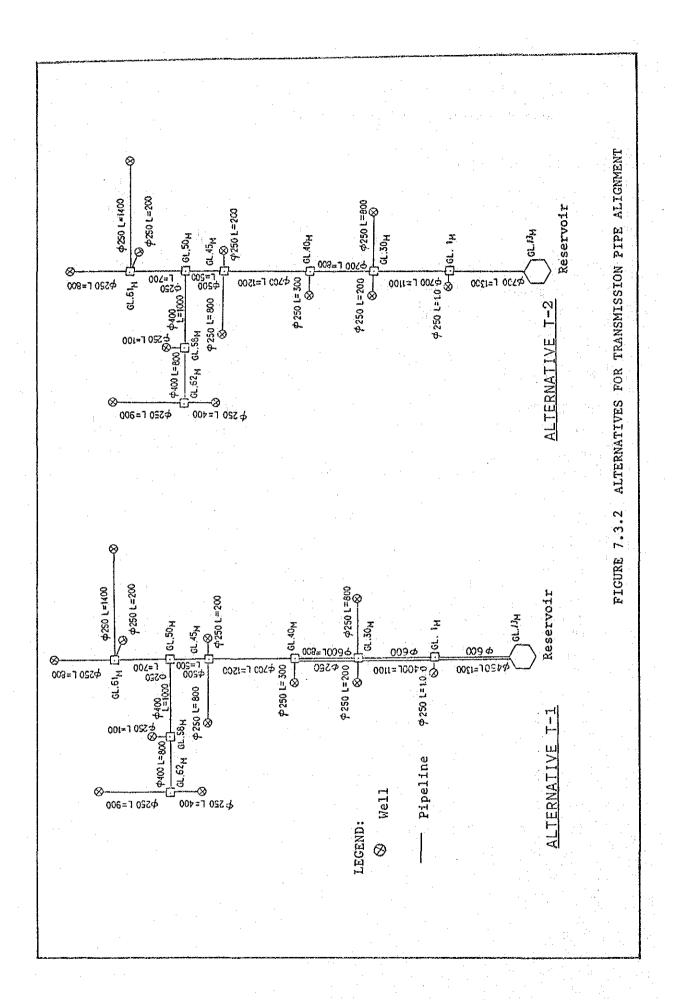
(2) Transmission Alternatives

1) Development of Alternatives

In accordance with the selected water source development plan, including transmission pipeline route, alternative pipe alignment plans, the staged construction program is developed as shown in TABLE 7.3.3 and FIGURE 7.3.2.

TABLE 7.3.3 ALTERNATIVES FOR TRANSMISSION PIPE ALIGNMENT

Trat	nsmission Line	Alt	ernative T	-1	A1t	ernative I	-2
Pipe	Alignment	Sing	gle Line		Par	allel Line	
Cons	struction Phase	Phase I	Phase II	Total	Phase I	Phase II	Total
	₫700 mm	3,200 m	1,200 m	4,400 m	<u>.</u>	1,200 m	1,200 m
	∮600 mm	-	-	-	÷ .	3,200	3,200
	∮500 mm	•• . :	500	500	-	500	500
:	ø450 mm	· -	, -	-	1,300	<u></u>	1,300
	\$400 mm	-	1,800	1,800	1,100	1,800	2,900
	∮350 mm	<u>-</u>	700	700	-	700	700
	₫250 mm	1,400	4,800	6,200	2,200	4,800	7,000
	Total	4,600 m	9,000 m	12,600 m	4,600 m	12,200 m	16,800 m



2) Cost Comparison

The cost comparison of two alternatives were made by applying 12% per annum of discount rate. The year of construction for cost comparison purpose is set as 1990 for Phase I and 1998 for Phase II pipelines, respectively.

A summary of cost comparison is shown in TABLE 7.3.4 and details are given in APPENDIX 7.3.1.

TABLE 7.3.4 COST COMPARISON OF ALTERNATIVE TRANSMISSION PIPE ALIGNMENT

	Alte	rnative T-	1	Alteri	Unit: ₹ x 1,000 Alternative T-2			
Costs	Phase I	Phase II	Total	Phase I	Phase I	[Total		
Construction Cost (C)	6,994	8,357	15,351	3,961	13,477	17,438		
Present Worth of Construction Cost (Cc)	4,445	2,145	6,590	2,517	3,459	5,976		
Present Work of Salvage Value (Cs)	277	418	695	157	675	832		
Net Present Worth Comparable Cost (Cn = Cc - Cs)	4,168	1,727	5,895	2,360	2,784	5,144		

Based on the result of cost comparison, Alternative T-2 is selected as the optimum plan for constructing transmission lines.

7.3.2 Location and Required Storage Capacity of Reservoirs

(1) Effective Utilization of the Existing Reservoir

With regard to the cost-effective utilization of the existing reservoir, alternative study on water distribution method from the reservoir were at first carried out. The alternatives were developed considering the potential advantages on the ground elevation of the reservoir site in respect to distribute water to Cabuyao area, as follows:

Case 1: Construction of transmission line from the existing reservoir to the vicinity of the poblacion along the existing pipeline route and distribution line, and provision of a reservoir and a pumping station before distribution.

Case 2: Expansion of the existing reservoir and construction of transmission and distribution lines using new routes.

Based on the cost comparison of two alternatives, Case 2 was selected as the optimum plan for the use of the existing reservoir. Details on this study is given in APPENDIX 7.3.2.

(2) Development of Alternatives

The pertinent selection of reservoir location and storage capacity largely contributes to economize the costs of both construction and operation/maintenance.

Following alternatives of reservoir locations as shown in FIGURE 7.3.3 were developed considering the existing water supply facilities and the projected water demand for the year 2010 in respective service area municipalities.

Alternative D-1

Cabuyao : Utilization of the existing reservoir with an

expansion to the required storage capacity.

Sta. Rosa & Biñan : Construction of a new reservoir with the required

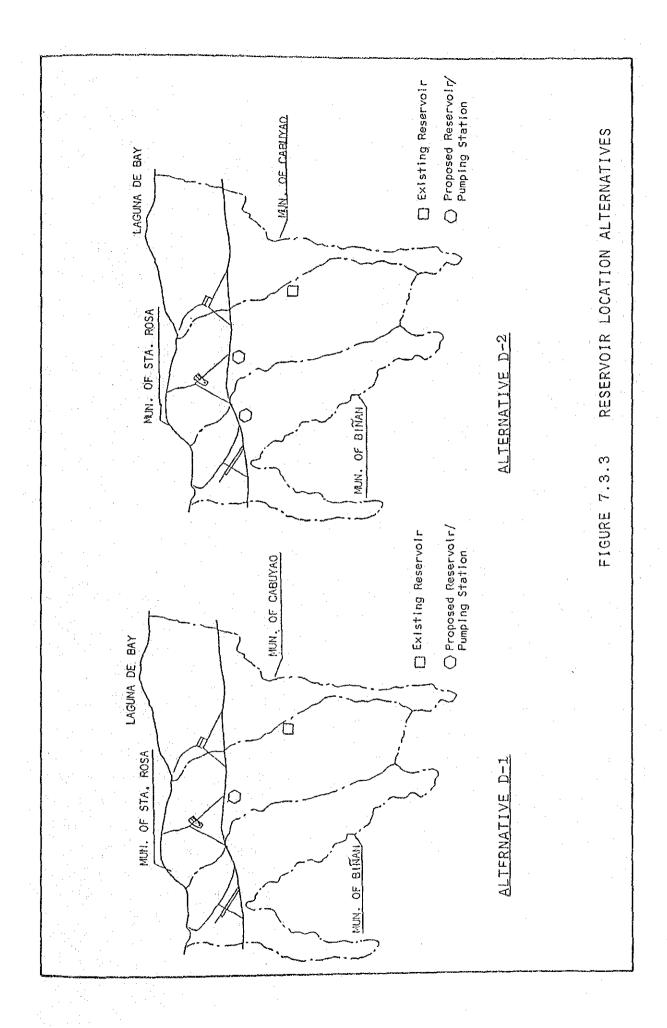
capacity in Sta. Rosa.

Alternative D-2

Cabuyao : Same as Alternative D-1

Sta. Rosa & Biñan: Construction of new reservoirs with the required

capacities in Sta. Rosa and Biñan, respectively.



To determine the most optimum storage capacity of respective reservoirs for the above alternatives, three cases for the storage requirement were considered as described in the section 7.3.1. TABLE 7.3.5 shows the required storage capacity estimated for each alternative.

TABLE 7.3.5 STORAGE REQUIREMENTS OF ALTERNATIVES

		and the second		Unit : c			
	Alterna	ative D-l	A	Alternative D-2			
Storage Volume	Cabuyao	Sta. Rosa	Cabuyao	Sta. Rosa	Biñan		
Emergency	1,920	5,920	1,920	2,670	3,250		
Maximum Operational Total	2,830 4,750	8,730 14,650	2,830 4,750	3,930 6,600	4,830 8,080		
Existing Capacity Required Capacity	1,350 3,400	- 14,650	1,350 3,400	6,600	8,080		
Intermediate Operational Total	- 1,920	- 5,920	- 1,920	- 2,670	3,250		
Existing Capacity Required Capacity	1,350 570	- 5,920	1,350 570	2,670	3,250		
Minimum Operational Total	- 1,920	- 5,920	1,920	_ 2,670	3,250		
Existing Capacity Required Capacity	1,350 570	5,920	1,350 570	2,670	3,250		

(3) Cost Comparison

According to the estimated storage requirements of each alternative, it is obvious that the case of the minimum storage requires larger construction cost for water source facilities to cope with the peak hour demand rather than that of the intermediate storage. Thus, a cost comparison of alternatives was performed for the cases of the maximum and intermediate storage capacities.

TABLE 7.3.6 shows a summary of comparative cost. In estimating construction cost of reservoir, the cost function adopted in the LWUA Methodology Manual is referred to.

TABLE 7.3.6 COST COMPARISON OF RESERVOIR ALTERNATIVES

		Alternative		Unit : № x 1,000 Alternative D-2	
Storage Volume	Unit Cost (P)	Q'ty	Cost	Q'ty	Cost
Maximum Water Source					
Deep Well Deep Well Pump	1,160,000	16 units	18,560	16 units	18,560
Station	790,000	16 units	12,640	16 units	12,640
Sub-Total	·		31,200		31,200
Reservoir Cabuyao Sta. Rosa Biñan		3,400 cu.m 14,650 cu.m		3,400 cu.m 6,600 cu.m 8,080 cu.m	3,621 5,532 6,295
Sub-Total			12,829		15,448
TOTAL			44,029		46,648
Intermediate				. :	
Water Source Deep Well	1,160,000	24 units	27,840	24 units	27,840
Deep Well Pump Station	790,000	24 units	18,960	24 units	18,960
Sub-Total			46,800	*	46,800
Reservoir Cabuyao Sta. Rosa Biñan		570 cu.m 5,920 cu.m	1,157 5,160	570 cu.m 2,670 cu.m 3,250 cu.m	1,157 3,103 3,518
Sub-Total			6,317		7,778
TOTAL		ř	53,117		54,578

Based on the above cost comparison, the case of the maximum storage capacity is cheaper than that of the intermediate storage capacity. When the cost for transmission line from water source to reservoir is taken into account, it is apparent that the case of the maximum storage capacity is more advantageous than the other one.

Thus, the daily maximum supply with the maximum storage capacity is recommended.

7.3.3 Distribution System

(1) Development of Alternatives

Two alternative studies on the distribution system is carried out for the reservoir locations as determined in the fore-going section.

A choice of alternative routes for major distribution pipeline is limited owing to road network as well as size and location of major demand centers in the proposed service area.

The most optimum network configuration of each alternative to allow the peak hour water flow in the year 2010 is determined through the computer-aided hydraulic simulation. TABLE 7.3.7 shows the required distribution facilities excluding commonly required facilities.

TABLE 7.3.7 ALTERNATIVE SYSTEM CONFIGURATIONS OF RESERVOIR AND DISTRIBUTION MAIN

Required Facilities	Alternat	ive D-1	Alterna	tive D-2	
Transmission Line	_		ø600 mm, 3	,800 m	
(Sta. Rosa to Biñan)			1 unit (38	0 1/sec, H=15m)	
Booster Pump Station	-			to Biñan	
Distribution Pump	l unit (1 H=45m) Sta. Ros	,210 1/sec, sa area	<pre>I unit (470 1/sec, H=30m Sta. Rosa area I unit (720 1/sec, H=40m Biñan area</pre>		
			binan arc		
Distribution Main/Valve	Pipe (m)	Valve (pcs.)	Pipe (m)	Valve (pcs.)	
∮ 700 mm	250	. 1	250	1	
ø 600 mm	8,310	28	5,260	18	
ø 500 mm	1,750	6	***		
ø 450 mm	1,450	5	800	-3	
ø 400 mm	3,000	10	5,800	19	
ø 350 mm	5,350	18	3,200	11	
ø 300 mm	11,400	38	14,200	47	
ø 250 mm	14,650	49	14,400	48	
ø 200 mm	7,800	26	10,300	34	
ø 150 mm	6,950	23	2,650	9	
ø 100 mm		-	1,000	.3	
Total	60,910	204	57,860	193	

(2) Cost Comparison

In accordance with the facility requirements as shown in TABLE 7.3.7, the construction costs of the alternatives were estimated including the operation and maintenance costs at the 1986 price level. Construction costs of distribution reservoirs are not considered in the cost comparison since the total storage capacity required for each alternative results in same construction cost.

TABLE 7.3.8 shows the estimated costs for construction and 15 years operation and maintenance.

TABLE 7.3.8 COST COMPARISON OF ALTERNATIVE DISTRIBUTION NETWORK

Cost	Alternative D-1	Alternative D-2
Construction Cost		
Transmission Line	e e	
		6,080
Booster Pump Station	_	5,188
Distribution Pump		
for Sta. Rosa	15,011	6,638
for Biñan	~	9,709
Distribution Main		
Pipe	50,309	44,885
Valve	13,266	10,165
		•
Total	<u>78,586</u>	82,665
Operation & Maintenance Cost		
Operators Salary	216	648
Energy	4,902	17,550
Maintenance	7,859	7,678
		· ·
Total	12,977	25,876
		 ,
GRAND TOTAL	<u>91,563</u>	108,541

Based on the above cost comparison, Alternative D-1 is selected as the most economical configuration of the distribution system.

7.3.4 Phasing of Distribution Network Development

(1) Development of Alternative

An in depth alternative study on the distribution system being selected in the foregoing section is carried out to establish the most optimum phasing of the overall system for the short term and long term development.

The computer-aided hydraulic simulation of the distribution network is a key subject in this study (See APPENDIX 7.3.3). Two alternative approaches of the study are then considered; Alternative D-1-A consists of a single pipeline alignment, and Alternative D-1-B forms a parallel pipeline alignment. Particular approaches of these alternatives are as follows:

Alternative D-1-A

- o The most optimum network configuration to allow the peak hour water flow in the year 2010 is first determined as studied in the previous section.
- o Major distribution pipes to be required in Phase I period are identified from the above-mentioned network configuration.
- o Phase II distribution network will be completed by installing additional pipes in the area to be served in this period.

Alternative D-1-B

- o Based on the optimum route of major distribution pipelines as determined in the previous section, the least cost network configuration, which is mainly the pipe sizes, is determined to satisfy a minimum of 0.7 kg/sq.cm of water pressure under the peak hour water flow during the Phase I period.
- o For the Phase II water demand, additional pipes wherever necessary for augmenting the distribution capacity to cope with all the criteria set

forth in the LWUA Methodology Manual are determined. Likewise, several pipeline routes will form a parallel pipe alignment.

The configuration of alternative distribution networks by construction phases is summarized in TABLE 7.3.9.

TABLE 7.3.9 CONFIGURATION OF ALTERNATIVE DISTRIBUTION NETWORKS

	Alt	ernative I)-1-A		Alternative	: D-1-B
Materials	Phase I	Phase II	Total	Phase I	Phase II	Tota1
Pipe (m)				:		
ø 150 mm	1,550	5,400	6,950	3,450	6,800	10,250
ø 200 mm	900	6,900	7,800	3,800	6,550	10,350
ø 250 mm	2,050	8,000	10,050	2,650	8,350	11,000
ø 300 mm	2,000	9,400	11,400	1,850	10,500	12,350
ø 350 mm	2,000	3,350	5,350	1,750	3,850	5,600
ø 400 mm	600	2,400	3,000	· _	5,200	5,200
ø 450 mm	1,450	. 🕶	1,450	900	400	1,300
ø 500 mm	1,350	400	1,750	2,900	3,800	6,700
ø 600 mm	3,800	4,500	8,300	250	4,500	4,750
ø 700 mm	250		250	_	. -	
Total	15,950	40,350	56,300	17,550	49,950	67,500
Valves (pcs.)						
ø 150 mm	5	18	23	10	23	33
ø 200 mm	3	23	26	13	22	35
ø 250 mm	7	27	- 34	9	28	37
ø 300 mm	7	32	39	6	35	41
ø 350 mm	7	11	18	6	13	19
ø 400 mm	2	8	10		17	1.7
ø 450 mm	5	-	5	3	1	4
ø 500 mm	5	1	6	10	13	23
ø 600 mm	13	15	28	1	15	. 16
ø 700 mm	1		1		***	_
Total	55	135	190	58	167	225

(2) Cost Comparison

Construction costs of each alternative are estimated. These include valves to be installed at every 300 meter intervals.

For this purpose, the year of construction is set in 1990 for Phase I and 1998 for Phase II pipelines. Discount rate applied to each phase is 12% per annum. For reference, 10% and 15% of discount rate are also considered (See APPENDIX 7.3.4).

A summary of cost comparison is presented in TABLE 7.3.10.

TABLE 7.3.10 COST COMPARISON OF ALTERNATIVE DISTRIBUTION NETWORK (NET PRESENT WORTH)

		Unit : ₽ x	1,000
	1	Discount Rate	
Distribution Network	10%	12%	15%
Alternative D-1-A			
Phase I: $(\phi 100 - \phi 700 \text{ mm})$ Pipeline: 9,770 m Valve: 33 pcs.	10,161 3,770	9,735 3,612	8,997 3,338
Sub Total	13,931	13,347	12,335
Phase II: (\$100 - \$250 mm) Pipeline: 12,140 m Valve : 41 pcs.	6,390 1,708 8,098	5,467 1,461 6,928	4,244 1,134 5,378
Sub Total TOTAL	22,029		17,713
Alternative D-1-B			ing Salah Salah Salah Salah Sa
Phase I: (\$\delta 100 - \$\delta 450 mm) \\ Pipeline: 9,770 m \\ Valve : 32 pcs.	7,716 2,027	7,392 1,942	6,831 1,794
Sub Total	9,743	9,334	8,625
Phase II: $($00 - $500 \text{ mm})$ Pipeline: 21,620 m Valve : 72 pcs.	9,819 2,518	8,401 2,154	6,520 1,672
Sub Total	12,337	10,555	8,192
TOTAL	22,080	19,889	16,817

According to the above cost comparison, the difference of the net present worth of comparable costs is less than 6%. Therefore, further comparison is made by adopting the cost escalation rate; 12% per annum from

1986 to 1989 and 10% per annum for the later project period. The inflated net present worth of comparable cost is shown in TABLE 7.3.11.

TABLE 7.3.11 ESCALATED PROJECT COST OF ALTERNATIVE DISTRIBUTION NETWORK

	Unit : ₱ x 1,000						
	Discount Rate						
Distribution Network	10%	12%	15%				
Alternative D-1-A							
Phase I Phase II	21,920 27,314	21,002 23,368	19,409 18,140				
TOTAL	49,234	44,370	37,549				
Alternative D-1-B							
Phase I Phase II	15,331 41,612	14,687 35,602	13,571 27,631				
TOTAL	56,943	50,289	41,202				

Based on the above cost comparison, it is concluded that Alternative D-1-A (single pipeline alignment) is the most optimum plan for phasing the development of the distribution network.

CHAPTER 8 RECOMMENDED PLAN

CHAPTER 8 RECOMMENDED PLAN

8.1 GENERAL

This Chapter presents the recommended plan for the short term and long term development of the CSBWS based on the results of foregoing alternative studies.

The recommended plan consists of two construction phases; Phase I (1986-1995) and Phase II (1996-2010). Phase I period is further divided into two stages; Stage 1 (1986-1990) and Stage 2 (1991-1995) considering the scope of immediate improvements and capital cost requirements.

The proposed implementation program includes the development of water sources and the construction of conveyance, transmission, treatment and disinfection, storage and distribution facilities. The repair and rehabilitation of existing water supply facilities are also considered in the Phase I activities.

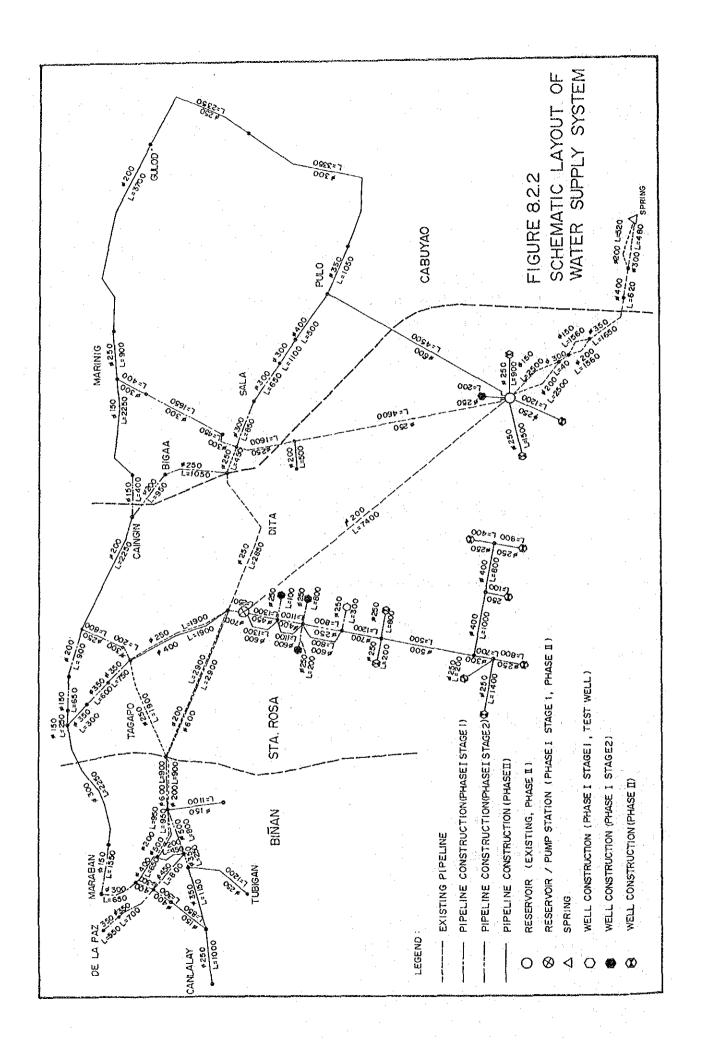
This Chapter also presents capital and annual project costs of the recommended implementation schemes.

Some deviations from the technical standards being adopted in the LWUA Methodology Manual have been applied in the Phase I period due to cost considerations. These deviations are: staging of some waterworks facilities to provide lower initial construction costs; provision of lower system pressure, and the postponement of the construction of the administration building to Phase II. All costs presented in this chapter are based on 1986 price level.

8.2 RECOMMENDED WATER SUPPLY SYSTEM

8.2.1 Layout of the Water Supply System

The general layout of the recommended water supply system for Cabuyao-Sta. Rosa-Biñan is shown in FIGURE 8.2.1 and its schematic layout is shown in FIGURE 8.2.2.



The description of the required facilities by project phase is given in TABLE 8.2.1 and a summary of the major facilities is also given in TABLE 8.2.2.

TABLE 8.2.1 DESCRIPTION OF REQUIRED MAJOR FACILITIES BY PHASE

Phase	Facility	Major Facilities Required
Phase I Stage 1	Source	The existing Matang-Tubig Spring (9,300 cu.m/day) and a test well (5,500 cu.m/day, constructed during this Feasibility Study at Barangay Pulong Santa Cruz in Sta. Rosa) will be used.
•		Total water production will be 14,800 cu.m/day.
	Treatment	Each one unit of constant flow chlorinator will be installed at the existing reservoir and a new reservoir (Sta. Rosa).
	Transmission	Pipe protection work will be carried out for the parallel pipe alignment section (each 400 m for ϕ 200 mm and ϕ 300 mm) along the river bed from the spring to the existing reservoir.
		A new transmission line (ϕ 250 mm- ϕ 450 mm, 3,500 m) will be constructed from the test well to the new reservoir (Sta. Rosa).
· .	Distribution	A new reservoir (3,105 cu.m) equipped with booster pump (Q=147 1/sec and H=50 m) will be constructed at Balibago in Sta. Rosa.
		New distribution main (\$250 mm-\$700 mm, 12,550 m) will be constructed.
		Internal network will be constructed to cover 340 ha of the existing service area.
	Electricity	Substation will be constructed at the new reservoir (Sta. Rosa). (300 KVA)
· .	Leakage Detection & Repair	All the existing service connections will be subject to the leakage detection survey.
	**	All the unmetered service connections will be installed with water meters and a half of the metered service connections will be subject to replacement of water meters.
		About 20% of lateral pipes will be subject to replacement.

TABLE 8.2.1 DESCRIPTION OF REQUIRED MAJOR FACILITIES BY PHASE (CONTINUED)

Phase	Facility	Major Facilities Required
Phase I Stage 2	Source	Additional 4 new wells (5,500 cu.m/day/well) will be constructed in Sta. Rosa area.
		Two existing wells in Biñan area will be abandoned.
	Transmission	Additional transmission line ($\phi 250$ mm, 1,300 m) will be constructed from new wells to the transmission main.
	Distribution	One additional booster pump (Q=373 1/sec, H=50 m) will be installed at the new reservoir (Sta. Rosa).
		Additional distribution main (\$150-\$500 mm, 9,300 m) will be constructed to cover the expanded service area.
	·	Additional internal network will be constructed to cover 400 ha of the expanded service area and to increase the network density in Stage 1 service area.
Phase II	Source	A total of 11 additional deep wells (5,500 cu.m/day/well) will be constructed in Sta. Rosa area.
	Treatment	Each one unit of additional constant flow chlorinator will be installed at the existing and new reservoirs.
	Transmission	Additional transmission line (ϕ 250- ϕ 700 mm, 12,200 m) will be constructed from new wells to the new reservoir (Sta. Rosa).
	Distribution	Existing reservoir will be augmented for additional 2,534 cu.m.
		New reservoir (Sta. Rosa) will also be expanded for additional 12,431 cu.m together with one additional booster pump (Q=690 1/sec, H=50 m).
		Additional distribution main (ϕ 150- ϕ 600 mm, 34,450 m) and internal network will be constructed to cover the expanded service area (500 ha).
	Electricity	Substation in Sta. Rosa will be renewed/augmented. (750 KVA)
	Others	Equipment (pump, water meter, etc.) will be renewed corresponding to the respective service-life.

TABLE 8.2.2 SUMMARY OF MAJOR FACILITIES REQUIRED BY PHASE

Phase	Phase	Ĭ	Phase II	Total	Remarks	
Item	Stage 1 Stage 2		inase II	iotai	Remarks	
1. Source Facility		:				
(1) Deep well (\$250 x200m)	-	4 units	ll units	15 units		
(2) Pumping facility 1) Pumping station	l unit	4 units	11 units	16 units		
2) Flow meter (\$150)	(Test well) 1 set	4 sets	11 sets	16 sets	+4.	
2. Transmission Facility						
(1) Pipe protection (\$200, \$300)	800 m	·. -	-	800 m	Spring -	
(2) Main pipes (\$250 - \$700)	3,500 m	1,300 m	12,200 m	17,000 m	Existing Reservoir	
3. Distribution Facility			•			
(1) Reservoir	1 unit (3,105 cu.m)	(2,534 & 12,4	2 units 431 cu.m)	3 units		
(2) Pump facility	1 unit	1 unit	1 unit	3 units		
(3) Pump house	1 unit	-	l unit	2 units		
(4) Chlorination facility (constant flow, 22kg/day) (constant flow, 45kg/day)	2 sets	1 set	1 set 1 set	4 sets 1 set		
(5) Power Substation	1 unit	-	1 unit	2 units		
(6) Distribution pipes						
1) Main pipes						
(ø150 - ø700 mm) 2) Valve (ø150 - ø700 mm)	12,550 m 43 sets	9,300 m 40 sets	34,450 m 112 sets	56,300 m 195 sets		
3) Internal network $(6100 - 6150 \text{ mm})$ $(675 - 6100 \text{ mm})$	- 4,160 m	1,620 m 35,460 m	6,700 m 60,590 m	8,320 m 100,210 m		
4) Service Connection 5) Water meter	4,967	15,476	42,953	63,396		
$(\phi 1/2^n - \phi 3/4^n)$	1,945		n anito	1,945 3 units		
6) Flow meter (\$\phi 350 mm) 7) Fire protection	1 unit	-	2 units 410 sets	410 sets	:	
4. Administration & Operation Building	1 unit	• ·	1 unit	2 units		
5. Leakage Detection Service connections	2,907 sets		 .	2,907 sets		

Note: Excluding replacement/renewal of equipment.

(1) Source Facility

A potential water production at the existing Matang-Tubig Spring will be fully utilized and additional new deep wells ($\phi 300$ mm x 200 m-depth, 5,500 cu.m/day/well) will be constructed in Sta. Rosa area. The existing deep well will be abandoned during the Stage 2 of Phase I due to small discharge (See FIGURE 8.2.3).

(2) Treatment Facility

A constant flow chlorinator will be installed at the reservoir.

(3) Transmission Facility

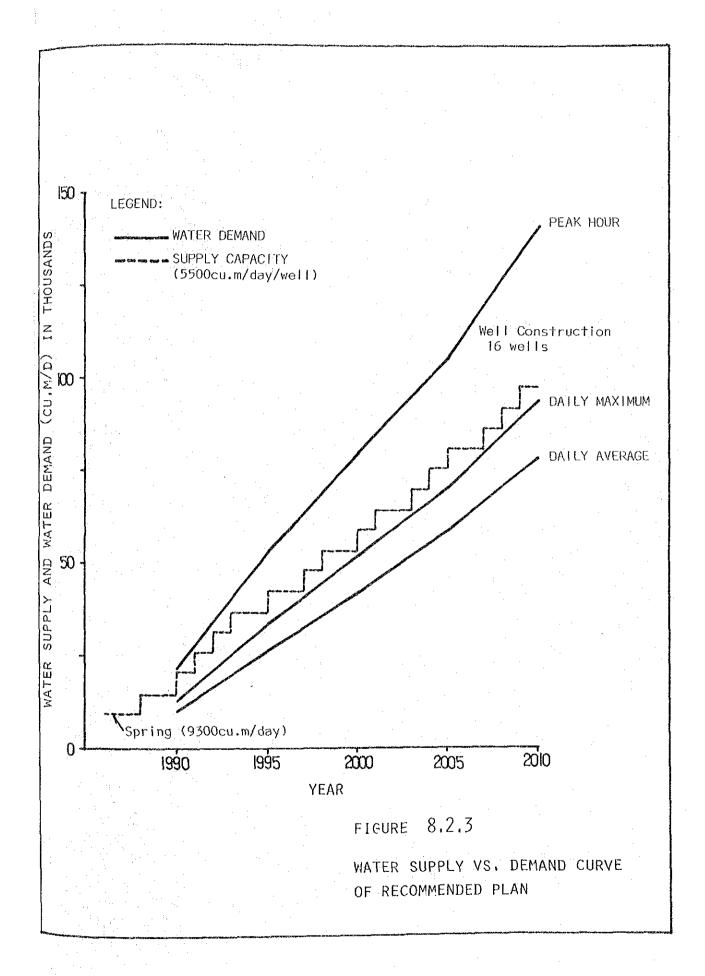
Improvement/protection of the transmission line installed along the bottom of the river between the existing spring and the reservoir shall be given priority. A pipeline from the new wells to the reservoirs will also be constructed.

(4) Distribution Facility

The proposed service area is determined to include the existing service area, the planned urban development area and the area along the major roads (50 m from each side of road shoulder).

The existing reservoir will be fully utilized after necessary expansion of its storage capacity. An additional reservoir will be constructed in Sta. Rosa area.

The storage capacity of the reservoir is determined to be the maximum storage volume (100% of distribution in 24 hour period) together with an emergency storage equivalent to 2 hour supply of the daily maximum water demand. The size of the reservoir is determined in compliance with water demand in each project phase and the emergency storage will be satisfied in the Phase II period.



The existing distribution lines will be utilized as a part of the proposed internal network, but subject to the leakage detection survey and repair/replacement, if necessary.

(5) Service Connections

As it has been observed during the field survey of existing water supply facilities, there are considerable number of service connections with not-functioning water meters and unmetered service connections. Leakages from service connections were also confirmed.

Resultant from these situation, the implementation of leakage detection survey and repair/replacement of damaged water meters and service connections are to be carried out in the early stage of the Project in order to increase the accounted-for water. The installation of water meters at new service connections and the provision of preventive maintenance program are another prerequisites for the sound operation of future water supply system.

8.2.2 Implementation Program

In accordance with the facility requirements as described in the previous section, the project implementation program is developed as shown in FIGURE 8.2.4.

8.2.3 Project Cost

The project cost is estimated based on the implementation program using the current market price of equipment and materials and the LWUA Methodology Manual at 1986 price level.

A summary of estimated project cost is shown in TABLE 8.2.3 and the detailed break down is given in APPENDIX 8.2.1.

A standby generator for stable supply and the chlorine neutralization equipment for safety measure are not considered in this planning stage, though these are to be studied prior to the expansion of the water supply service.

			-				1					
			Phas	se I		 .	Phase	· 1		· .	Phase II	
Description	:		Sta	ge 1			Stage	2	-	٠.	tuas: ti	
	: '87	188		90	,91	192			195	2000:	2005:	2016
Appraisal & Losn Procedure												
Engineering Service												:
- Detailed Design				***								
- Construction Supervision]		.)]		
- Leakage Detection		969	969	969				.				
	·			·							5.3	
Source Facility												
- Deep Well (\$300x200m)	:				-1	_1_	1	1		 	¹¹	·
- Deep Well Pump Station (37 kw)			_1_	ļ ·	1	<u> </u>		1			<u>1</u> 1	
			ļ				:	ļ				
Transmission Facility			,									
- Pipe Protection (\$300, \$200)			800			200	200					
- Main Pipe (\$250-\$450)			3,500		800	- 200	700	100			12,200	
- Main Pipe (\$250-\$700)			:					ļ			. 1-110.	
Discribution Fscility												
- Reservoir (V= 3,050cu.m)	,]		 						
- Reservoir (V=12,430cu.m)										L I	:	
- Reservoir (V= 2,530cu.m)			•									
- Booster Pump Station		·										
- Main Pipe (#150-#700)			8,450	4,100	9,300							
- Main Pipe (#150-#600)											34,450	
- Internal Network		ĺ					j !				·	i
o Residential Area (675-6100)	:		2,080	2,080	7,100	7,090	7,090	7,090	7,090		60,590	
o Commercial Area (\$100-\$150)					330	320	320	320	320	<u> </u>	€,700	1
- Fire Protection (#150)						l					_ 15	
- Fire Protection (\$100)											3 <u>3</u> 5	
- Service Connection (#1/2")			2,450	2,450	3,093	3,093	3,093	3,093	1	<u> </u>	42,879	
(\$3/4")			33	33	3	1 3	2	2 	<u> 3 </u>	 -	74	
- Rehabilitation												1
o Water Meter			{								ļ	
o Service Connection					1						·	•
o Laterals		}		<u> </u>	1							\
- Disinfection (Chlorinator)			-		*****				ľ			į
- Electric Sab-Station										<u> </u>		
									l			
Others - Operation Center					,							
	1 '			1		1	1					
	l .	1 .										
- Administration Building							.					
		2			1	1	1	ı		11111	11111	1 1

Legend: --- : Continuous Work; - - - : Intermittent Work; * : Procurement

TABLE 8.2.3 SUMMARY OF PROJECT COST

			Unit	: ₽x1,000
Facility	Phas Stage 1	se I Stage 2	Phase II	Total
	Stage I	Stage 2		
1) Source	852	8,048	22,132	31,032
2) Transmission	3,503	819	13,477	17,799
3) Distribution	35,577	32,283	112,327	180,187
4) Elec. Substation	3,643	0	4,858	8,501
5) Operation Center/	34043		1,030	0,501
Adm. Bldg.	1,583	0	1,820	3,403
6) Material & Equip.	1,101	1,761	5,741	8,603
7) Land Acquisition	180	200	1,410	1,790
8) Replacement of Equipmen		200	20,848	20,848
of Repracement of Equipmen	· ·	· · · · · · · · · · · · · · · · · · ·	20 , 0,0	20,010
C.1 m1	46,439	ka 111	182,613	272,163
Sub Total		43,111	14,609	272,103
Physical Contingency (8%)	3,715	3,449	14,009	21,173
TOTAL	50,154	46,560	197,222	293,936
Leakage Detection	699			699
Engineering Charge	. 099			0,7,7
Detailed Design	1 "			
(10% of TOTAL)	9,671		19,722	29,393
Construction Supervision	· · · · · · · · · · · · · · · · · · ·		17,722	20,000
(4% of TOTAL)	2,006	1,862	7,889	11,757
(4% OI TOTAL)	2,000	1,002	,,00	129757
GRAND TOTAL	62,530	48,422	224,833	335,785
GRIND IOIRD		40,424	224,033	
Operation & Maintenance	2 007	6 071	16. 840	
Cost (per annum)	2,086	6,071	16,549	

CHAPTER 9 FINANCIAL FEASIBILITY ANALYSIS

CHAPTER 9 FINANCIAL FEASIBILITY ANALYSIS

9.1 GENERAL

The selected technical alternative for the realization of water supply for the Short Term Development Plan has been shown in Chapter 8. Such a plan must however be verified from a financial point of view. A financial feasibility of the project was analyzed and a financial plan for the allocation of available funds, based on LWUA's guideline, was developed in this chapter.

The analysis of the financial viability of the project covers the revenues which mainly come from water sales, the development costs, the operating and maintenance costs, debt service on the loans, etc. during the first 10 years from the start of the construction of the water system. The Financial Internal Rate of Return (FIRR) is also calculated.

The proposed water rates should be fair, reasonable, and realistic based on the ability of the consumers to pay. In order to achieve this, a socialized rate structure will be adopted.

9.2 EXISTING SYSTEM

9.2.1 Rate Structure

As of July 1986, CSBWS has 2,907 metered and unmetered connections. The following rate structure has been effective since 1984 in the waterworks system.

(1) Water Rates

There are two (2) types of water rates, i.e., flat rates and metered rates, as follows:

a) Flat Rates

With one faucet P14.00
Additional faucet P 0.60 every addition

These rates are applicable to unmetered connections.

b) Metered Rates

	Water Rates	Consumption Ranges
(min.)	P10.00/month	0-10 cu.m
	0.50/cu.m	11-30 cu.m
	0.75/cu.m	31-50 cu.m
	1.00/cu.m	51-70 cu.m
	1.25/cu.m	71-100 cu.m
	1.50/cu.m	Over 100 cu.m

These rates are applicable to metered connections.

(2) Connection Service Fees:

Guaranty deposit P50.00
Registration and Tapping Fee P15.00

The cost of water meter shall be paid by each applicant.

9.2.2 Revenue and Expenditure

The annual revenue and expenditure of the waterworks system in the last four years are shown below:

Year	Revenue	Expenditure	Net Revenue
1982	P 681,807	P545,868	₱135,949
1983	682,175	596,598	85,577
1984	942,559	646,442	296,117
1985	1,055,330	879,566	175,764

The rapid increase of revenue during the last two years was due to the increased number of metered connections and increased volume of water sales.

In 1985, the waterworks' revenue accounted for 2.2% of the Province's total revenue while its expenditure amounted to 2.2% of the Province's total expenditure.

The breakdown of the revenue of the waterworks is shown in TABLE 9.2.1 below.

TABLE 9.2.1 BREAKDOWN OF REVENUE OF CSBWS

	1984	1985
Water Sales	P935,125.00	P1,044,828.00
Connection Service Fees	3,000.00	2,835.00
Others	4,434.00	7,667.00
Total	₱942,559	P1,055,330

The current water rate system is largely beneficial to the customers who consume 10 cu.m to 70 cu.m of water per month. Most residential users fall on this category. In this case, a socialized water rate scheme has to be adopted in the future wherein huge consumers of water will have to pay more.

A breakdown of CSBWS expenditures is shown in TABLE 9.2.2, with the corresponding breakdown in percentages being shown below:

	1983	1984	1985
Personal Services	53.1%	54.1%	50.3%
Maintenance & Operation Expenses	46.3	45.9	49.7
Capital Outlay	0.6	0	0
Total Expenditure	100.0	100.0	100.0

Salaries and wages are the biggest expenditure item followed by supplies & material and electricity cost.

TABLE 9.2.2 EXPENDITURE OF CSBWS

	1983	1984	1985
l. Personal Service			
- Salaries and Wages	P268,289.22	n.a.	₱347,396.81
- Life	6,184.60		7,854.38
- Retirement	13,393.57		17,078.93
- Medicare	2,366.70		2,892.15
- 1% State Insurance	2,533.67	:	3,420.87
- Pagibig Premium	7,960.50		6,797.08
- COLA	15,630.70		57,144.55
Sub-Total	P316,559	P349,442	₱442,085
2. Maintenance & Operating	- · · · · · · · · · · · · · · · · · · ·		
- Traveling Expenses	P 34,664.45	n.a.	P 28,000.00
- Supplies & Material	113,691.67		189,848.4
- Repair of Equipment	27,424.30		61,151.59
- Illumination	98,588.99	•	143,000.00
- Retirement Gratuity			15,481.61
- Other Services	1,669.64		
Sub-Total	₱276,039	P297,000	₱437,482
			•
3. Capital Outlay	P4,000	0	0
Total Expenditure	₱596 , 598	₱646,442	₱879,567

9.2.3 Bill Rendering and Collection

Meter readers undertake monthly reading on all the metered connections. Bills are issued for each class of service and are delivered to the customers by bill collectors of the waterworks. Normally, one month elapse between bill issuance and payment. Collections are being done on the day the bills are delivered. About 70% of the bills delivered are paid at the first call of collectors. The average rate of bill collection is placed at 80%.

As a matter of policy, delinquent consumers are given five days from the date the notice of disconnection is received to pay their bills, otherwise service disconnection will be effected. However, in case of non-compliance, another notice demanding payment for the said delinquent bill within 24 hours is given, after which disconnection is finally made if the consumer still fails to pay his bill.

9.3 MARKET SURVEY

The market survey for the Cabuyao-Sta.Rosa-Binan study area was conducted from June 2 to June 15, 1986 with LWUA's cooperation using the simplified market survey method. The number of respondents totaled 7,291 and the estimated coverage ratio in the study area is 31%.

The details of the market survey results is shown in Appendix 9.3.1.

From the market survey, the income distribution of the respondents is determined as follows:

	Cal	ouyao	Sta	. Rosa	Biı	nan	То	tal
Income Bracket1/	Ave.	Number	Ave. Pesos	Number	Ave. Pesos	Number	Ave. Pesos	Number
P900 and below	679	160	696	703	592	1,407	630	2,270
₱901 to ₱1500	1,212	321	1,203	940	1,205	1,230	1,205	2,491
P1,501 to P2,500	2,152	157	2,130	554	2,099	637	2,118	1,348
P2,501 to P4,500	3,467	98	3,312	274	3,340	389	3,346	706
₱4,501 and above	8,338	13	5,357	28	5,798	78	5,972	119

As a result of the market survey, the respondents' willingness and unwillingness to connect is summarized as follows:

^{1/} Residential, excluding no-income and no-answer

A	nswer	Cabuyao	Sta. Rosa	Binan
Yes		37 %	44 %	49 %
No		25	32	35
With own	connection	38	24	14

Note: With respect to type of users, residential users account for 100% in Cabuyao, 97% in Sta.Rosa, and 97% in Binan, respectively.

It is observed from the results of the survey that the majority of the respondents in the three municipalities who are not yet connected to the existing system are willing to connect to the waterworks system. It is expected therefore that more residents in the Cabuyao-Sta.Rosa-Binan will connect to the new water supply system when it is expanded.

9.4 PROJECT COST AND FINANCING

9.4.1 Project Implementation Schedule and Project Period

As described in Chapter 8, in the project schedule of this study, it is assumed that :

- Design will be completed in 1988
- Construction will be started in 1989 and completed by 1995.

The financial analysis covers a 10-year period which includes the construction period. However, a 20 year period is adopted for the calculation of FIRR.

9.4.2 Financial Conditions

The major potential sources of funds for the Water District are the operating sources and the non-operating sources.

The operating sources are the excess of revenue over expenses.

The LWUA's guideline suggests that the District should make a

contribution of 10% to the project cost as equity. However, should the waterworks cannot provide the said equity due to its current tight financial situation, it is suggested that the government will provide a grant to the District to make up for the equity. In this study, 5% of the project cost is set as the equity.

The non-operating sources include loans. In this study, to realize a reasonable water rate structure, the introduction of the LWUA's soft loan into the project in addition to the regular loan is recommended. The loan funds from LWUA will be utilized to finance the balance of the project cost plus capitalized interest. Presently, LWUA's terms include:

(1) Regular loan

Interest : First P2 million : 10% per annum

Next P5 million: 12% "

Above \$7 million: 14%

Duration : Thirty-year loan; disbursements are assumed

to be made at mid-year, and the maximum

disbursement period is four years.

Principal: Principal repayment is thirty years with one

year grace period.

(2) Soft loan

Interest: 10% per annum

Interest is not charged for the first 5 years, with the district to start paying interest on the 6th year. In cases where the project is not completed within 5 years, interest will be capitalized from the 5th year up to the time of completion.

Principal: Principal Repayment is 20 years starting on the 11th year after the start of disbursement.

9.4.3 Project Cost

On the basis of the cost estimate developed in Chapter 8, the project cost for the District in the Short Term Development Plan is estimated at #214.64 million and its breakdown is shown in TABLE 9.4.1.

The proposed financing scheme for the project is as follows:

Project Cost	P214.64 million
Equity 5%	10.73
LWUA Regular Loan 50%	107.32 "
LWUA Soft Loan 45%	96.59
Capitalized Interest	P28.08
Total Project Cost	P242.72 million

The computation of capitalized interest and the subsequent debt service obligations of the District is presented in TABLE 9.4.2. The long term borrowing from 1988-95 amounts to \$231.99 million.

9.4.4 Operating and Maintenance Cost

The operating and maintenance costs shown in TABLE 9.4.3, described in detail in Chapter 8, include all annual expenses necessary in operating the system and maintaining its revenue producing capacity. In the financial projections, the operating and maintenance costs are assumed to increase according to the increase in the capacity of the system with expected inflationary effect as shown in TABLE 9.4.4.

TABLE 9.4.1 BREAKDOWN OF PROJECT COST

1988 1989 1990 1991 1992 1,381 37,357 7,701 18,534 6,794 6 110 2,989 616 1,483 544 1,491 40,346 8,317 20,017 7,338 7 481 21,015 6,230 20,244 9,635 12 12,790 0 0 0 0 0 75 121 14,547 40,261 16,972 19 75 121 175 0 0 0 75 121 175 0 0 0 15,149 64,169 15,536 41,871 17,651 20 157 2,454 582 1,610 679								(D)	(Unit : 1.000 Pesos	Pesos)
ncies 1,381 37,357 7,701 18,534 6,794 ncies 110 2.989 616 1,483 544 e,794 es 481 21,015 6,230 20,244 9,635 truction Cost - 1,972 61,361 14,547 40,261 16,972 e 12,790 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Year	1988	1989	1990	1881	1992	1993	1994	1995	Total
110 2,989 616 1,483 544 1,491 40,346 8,317 20,017 7,338 481 21,015 6,230 20,244 9,635 12,790 0 0 0 0 0 233 233 233 0 0 0 75 121 175 0 0 0 75 121 175 0 0 15,149 64,169 15,536 41,871 17,651 t : Based on 1986 Price	Construction Cost	1,381	37,357	7,701	18,534	6,794	6,793	6,703	4.287	89,550
1.491 40,346 8,317 20,017 7,338 481 21,015 6,230 20,244 9,635 12,790 0 0 0 0 0 75 121 175 0 0 15,149 64,169 15,536 41,871 17,651	Physical Contingencies	110	2,989	616	1,483		543	536	343	7,164
tetion Cost - 1.972 61,361 14,547 40,261 16,872 12,790 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	- Sub Total -	1.491	40,346	8,317	20,017	7,338	7,336	7,239	4,630	96,714
on Cost - 1.972 61,361 14,547 40,261 16,972 12,790 0 0 0 0 233 233 233 0 0 0 75 121 175 0 0 0 75 2,454 582 1,610 679 15,149 64,169 15,536 41,871 17,651	Price Contingencies	481	21,015	6,230	20,244	9,635	12,179	14,906	11.658	96,346
12,790 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		- 1.972	61,361	14,547	40,261	16,972	19,515	22,145	16.288	193,060
233 233 0 0 0 75 121 175 0 0 0 75 121 175 0 0 0 1 15.149 64.169 15,536 41,871 17,651 157.53 157.53	Engineering Charge	12,790	0 !	0	0	0	0	0	0	12,790
75 121 175 0 0 0 77 2,454 582 1,610 679 679 15,149 64.169 15,536 41,87! 17,651 177.51	Leakage Detection(L/D)	233	233	233	O	0	0	0	0	669
79 2,454 582 1,610 679 15,149 64,169 15,536 41,87! 17,651 : Based on 1986 Price	Price Escaration for L/0	75	121	175		0	0	0	O	371
15,149 64,169 15,536 41,871 17,651 : Based on 1986 Price	Construction Supervision	62 4	2,454	582	1,610	629	181	886	652	7,722
	Total Project Cost	15,149	64,169	15,536	41,871	17,651	20,296	23,031	16,940	214,643
Physical Contingencies : 8% of Construction Cost Engineering Charge : (10% of Construction Cost & Physical Contingencies) + (Price Escalation) Construction Supervision : 4% of Estimated Construction Cost	Note) Construction Cost Inflation Rate Physical Contingencies Engineering Charge Construction Supervision	Based on 1: 15% p.a. 8% of Con (10% of Con 4% of Est	986 Price struction nstruction imated Con	Cost & Ph Cost & Ph struction	ysical Co Cost	ntimgencies	s) + (Price	Escalatio	(u.	:

TABLE 9.4.2 PROJECTED DEBT SERVICE SCHEDULE

	Disburser Regula Soft]	Disbursement Amount Regular Loan Soft Loan	nount		#203.91 107.32 96.59	03.91 million 07.32 " 96.59 "	ŭ	:	:	
	Capital Regu Soft	Capitalized Interest Regular Loan Soft Loan	cerest		28 0	28.08 " 0 "				
	Total Loan	Loan	*		P 231	P231.99 million	ŭ		: 	
	Financed Regul	ed Interest ular Loan	est 1	Ĭ±,	First	P2 million	10%	per annum		
	4 0			N A	Next Above	P5 million P7 million	12%	= =		
	100	r Loan		٠		том р.а.			(Unit : P	million
	1988	1989	1990	1991	1992	1993	1994	1995	1996	1997
1. Regular Loan		7	i u	r .		1 - 1 - 1 - 1 - 1				:
Dispursement Capitalized Interest	0.10	1.94	11.19	14.94	1 1	.	i, I,		.	1 1
	•	1	1	ŧ	18.78	18.78	18.72	18.66	18.59	18.51
Principal Repayment Debt Services	1 0	10	1 0	1 0	0 18.78	0.39 19.17	0.45	0.51 19.17	0.58	0.66
2. Soft Loan			- 1 - 1 - 1 - 1 - 1 - 1	07-66	15.15	17.80	20.53	13.71		1
Capitalized Interest	ı		ı	. l		1) i j f	1	1	J
		1	t	ı	1	•	1	1	99.6	99.6
Principal Repayment	1		1		1	1	1.		0	0
Debt Services	0	0	0	 0	O .	0	0	0	99.6	9,00
Total Debt Services	0	0	0	0	18.78	19.17	19.17	19.17	28.83	28.83
Debt at End of Year	15.15	81.26	107.99	164.80	179.95	197.36	217.44	230.64	230.06	229.40

TABLE 9.4.3 PROJECTED OPERATION AND MAINTENANCE COST (Unescalated) (Unescalated)

Year	Total O&M Cost	Administrative Expenses	Energy	Chemicals	Maintenance	Miscellaneous
1988	1,064	490	1	84	225	301
1989	1,576	522	16	79	417	557
1990	2,086	539	25	66	609	814
1661	2,889	702	121	131	846	1,089
1992	3,679	849	217	163	1,085	1,365
1993	4,481	1,012	314	196	1,319	1,640
1994	5,268	1,159	410	228	1,555	1,916
1995	6,071	1,322	206	260	1,792	2,191
1996	6,071	1,322	909	260	1,792	2,191
1997	6,071	1,322	206	260	1,792	2,191
1998	6,071	1,322	905	260	1,792	2,191

TABLE 9.4.4 PROJECTED OPERATION AND MAINTENANCE COST (Escalated) TABLE >.

						(Unit : ₱1,000)
Year	Total O&M Cost	Administrative Expenses	Energy	Chemicals	Maintenance	Miscellaneous
1988	1,407	979	. 0	63	298	398
1989	2,396	794	24	97	634	847
1990	3,649	943	77	173	1,065	1,424
1661	5,810	1,412	243	263	1,702	2,190
1992	8,510	1,964	502	377	2,510	3,157
1993	11,919	2,692	835	521	3,509	4,362
1994	16,114	3,545	1,254	269	4,757	5,861
1995	21,358	4,651	1,780	915	6,304	7,708
1996	24,562	5,349	2,047	1,052	7,250	8,864
1997	28,246	6,151	2,354	1,210	8,337	10,194
1998	32,483	7,074	2,707	1,392	9,588	11,723

9.4.5 Escalation of Costs

To account for the effects of inflation, the investment costs and the annual operating and maintenance costs are escalated by 15% compounded per annum.

9.4.6 Reserve Requirements

The reserve requirements are tied directly to the acquisition of development loans from LWUA. These are considered as funds necessary to support capital development. LWUA guidelines suggest that 10% of direct water sales be set aside as reserve funds. In this analysis, the allocation for reserves is to be 5% from 1990 to 1991 and 10% from 1992 onward.

9.5 REVENUE ANALYSIS AND WATER RATES

9.5.1 Derivation of Revenue Units

LWUA recommends the adoption of the optional method which is a combination of the revenue unit and the quantity block methods.

The number of revenue units has been computed using the optional method. Under this method the minimum charge varies depending on the size of the connection. 2/ In addition, factors are applied to successive quantity blocks of water consumed per month.

These factors increase as the level of consumption increases since they are set to attain a socialized tariff structure where users with low consumption pay a low average rate while users with high level of consumption pay higher average rate. This scheme will also encourage the poor population to connect to the system.

$$3/8" = 1.0$$
 $3/4" = 4.0$ $1/2" = 2.5$ $1" = 8.0$

^{2/} The use factors for the first 10 cu.m which depend on the size of connections are as follows:

The factors used for the minimum charge (Service Charge Revenue Units or SCRUs), and commodity charges for different sizes and types of service connections and total equivalent volume are shown in TABLE 9.5.1 and TABLE 9.5.2, respectively.

9.5.2 Water Rate Structure

It is assumed that the following socialized rate structure for domestic/institutional users with 1/2 inch connections is employed in this analysis:

		
Quar	ntity Block	Factor
First 1	0 cu.m/month	1.00
11-20	cu.m/month	1.25
21-35	cu.m/month	1.60
Above 3	5 cu.m/month	2.10

For commercial/industrial users, the use factors are double.

9.5.3 Feasibility of Charges

The feasibility of charges can be determined by subjecting the required water rates for 1/2 inch connections to the following requirements:

- 1. Minimum charge (for the first 10 cu.m) must not exceed 5% of the average family income of the low income group.
- 2. Increase must be limited to 60% of the existing rates in the previous year.

The proposed water rates necessary to achieve financial viability are within the ability-to-pay of the customers. From the projected revenue forecast shown in TABLE 9.5.3, the following water charges for the initial 10 cu.m do not exceed 5% of the average income of the low income class, and anticipated increases are within the 60% limit throughout the study period.

TABLE 9.5.1 SERVICE CONNECTIONS AND SERVICE CHARGE REVENUE UNITS

STRATIFICATION OF SERVICE CONNECTIONS (CSRBUS)

												X . 12	13	240	463	683	914	1,141	1,366	1.592
	Sub-TTL	28	400	772	1.148	1.524	1.901	2.277	2,653		rial	Sub- Total	140	2,000	3,860	5.740	7.620	9,505	11.385	13,265
	3/4 1										Commercial/Industria	x 16.0	. 0	0	0	0	0	0	0	Ø
	3/4	0	0	0	0	0	0	0	0	(CSRBWS)	Commerc	0 8 X	Đ	0	0	0	0	O	0	0
ze (inch)	1/2	28	400	772	1,148	1,524	1,901	2,277	2.653	JE UNITS		X 5.0	140	2,000	3,860	5,740	7,620	9,505	11,385	13,265
Connection Size (inch	Sub-TTL	2,879	4.991	7.102	9.821	12,540	15,259	17.978	20,697	COMPUTATION OF SERVICE CHARGE REVENUE UNITS	-	X .12	867	1,506	2.145	2.961	3,778	4,594	5.410	6,226
S	overnmen t						. •			SERVICE CI	nmont	Sub- Totai	7.222	12,551	17,878	24,679	31,481	38,281	45.083	51,884
0/ - 1-1-0	3/4 I	. 81	49	82	84	87	83	92	94	PUTATION OF	Domestic/Government	X 8.0	0	0	0	0	Q		0	0
	1/2	2,863	4,942	7,020	9,737	12,453	15.170	17.886	20,603	55	Dom	X 4.0	64	196	328	336	348	356	368	376
	ي در			٠		:						X 2.5	7,158	12,355	17,550	24,343	31,133	37,925	311.50	51.508
Total	Connections	2,907	5,391	7,874	10,969	14,064	17,160	20,255	23,350		Total	(1,000)	883	1,746	2,609	3,850	4,692	5,734	6,776	7,818
	Vear	1988	1989	1990	1881	1992	1993	1994	1995			Year	1988	1989	1590	1991	1992	1993	1994	1995

TABLE 9.5.2 EQUIVALENT VOLUME OF WATER SOLD

(1,000)	Total Equivalent Volume	2.054	250	2.304	 Total Equivalent Volume	2,580	916	3,496	Total Equivalent Volume	3.877	1,609	5.486
(Unit: X 1,000)	Above 35 m3 (2.10)	353 2.10 741	4.20 1.76		Above 35 m3 (2.10)	319 2.10 670	121 4.20 510		Above 35 m3 (2.10)	514 2.10 1,080	206 4.20 865	
	21-35 m3 (1.60)	203 1.60 324	3.20		21-35 m3 (1.60)	183 1.60 293	3.20 135		21-35 m3 (1.60)	295 1.80 473	3.20 229 229	
	11-20 m3	98 1.25 123	2.50 11		 11-20 m3 (1.25)	89 1.25 111	2.50 31		11-20 m3 (1.25)	143 1.25 179	2.50 5.50 5.20	
	First 10 m3	345 0	, m c		First 10 m3	0 0	48		First 10 m3	852 0	93	
	Consumption (m3)	. 666 6	8		Consumption (m3)	1.190	224		Consumption (m3)	1,805	391	
	SCRUS	867			SCRUS	1,506	1. 240		SCRUS	2,145	463	
	1988	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.	1989	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.	1990	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.
								٠			. 4.	

TABLE 9.5.2 (Cont'd)

X 1,000 >	Total Equivalent Volume	5,385	2,441	7,826		Totai Equivalent Volume	6,891	3,273	10,165	Total Equivalent Volume	8,399	4,105	12,505
(Unit : X 1,000	Above 35 m3 (2.10)	720 2:10 1,512	315 4.20 1,322			Above 35 m3 ()	925 2.10 1,943	424 4.20 1,780		Above 35 m3 ()	1,131 2.10 2,375	533 4.20 2,237	
	21-35 m3 (1.60)	413 1.60 661	3.20 3.50		÷.	21-35 m3 (1.60)	531 1.60 850	3.20 472		21-35 m3 (1.60	649 1.60 1,039	3.20 593	÷
	11-20 m3 (1.25	200 1.25 250	32 2.50 80	-		11-20 m3	257 1,25 321	43 2.50 107		11-20 m3 (1.25)	314 1.25 393	2.50 135	
	First 10 m3	1,179	138			First 10 m3	1,505	183		First 10 m3	1,831	228	
	Consumption (#3)	2,512	594		:	Consumption (m3)	3,218	787		Consumption (m3)	3,925	1,000	
	SCRUS	2,961	689			SCRUS	3,778	914		SCRUS	4,594	1,141	•
	1991	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.		1992	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.	1993	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.

TABLE 9.5.2 (Cont'd)

(Unit: X 1,000)

Total Equivalent Volume	906'6	4,937	14,843	Total Equivalent Volume	11,414	5.773	17,187
Above 35 m3 (1,336 2.10 2,805	642 4.20 2,694		Above 35 m3	1,541 2.10 3,237	4.20 3,155	
21-35 m3 (1.60) (767 1.60 1,227	223 3.20 714		21-35 m3	885 1.60 1,416	261 3.20 836	
11-20 m3 (1.25) (371 1.25 464	65 2.50 163		11-20 m3	428 1.25 535	2.50 191	
First 10 m3	2,157 0	273		First 10 m3	2,484	318	
Consumption (m3)	4.631	1.203	i	Consumption (m3)	5,338	1,407	
SCRUS	5,410	1,366		SCRUS	6,226	1,592	
1994	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.	1995	Domestic/Gov. Factor E.v.	Commercial/Ind. Factor E.v.	Total E.V.

2.304 2,074 2.304 2,074 3.496 4,545 5.486 7,132 7.826 14,870 10,165 19,313 12.505 30,012 14.843 35,624 17.187 46,405	İ			Total	£	Bad	-		
0.9 2.304 2,074 62 2,011 604 1.3 3.496 4,545 136 4,409 2,013 1.3 5.486 7,132 214 6,918 3,269 1.9 7,826 14,870 446 14,424 8,614 1 1.9 10,165 19,313 579 18,733 -8,553 2.4 12,505 30,012 900 29,111 -1,976 2.4 14,843 35,624 1,069 34,555 -727 2.7 17,187 46,405 1,392 45,013 4,487		Expenses	Kare Unit	tquivalent Volume	Sales	vents (3%)	Sales	Surplus	Surplus
1.3 3.496 4,545 136 4,409 2,013 1.3 5.486 7,132 214 6,918 3,269 1.9 7,826 14,870 446 14,424 8,614 1 1.9 10,165 19,313 579 18,733 -8,553 2.4 12,505 30,012 900 29,111 -1,976 2.4 14,843 35,624 1,069 34,555 -727 2.7 17,187 46,405 1,392 45,013 4,487		1,407	6.0	2,304	2,074	62	2,011	604	604
3,649 1.3 5.486 7,132 214 6,918 3,269 5,810 1.9 7,826 14,870 446 14,424 8,614 1 27,286 1.9 10,165 19,313 579 18,733 -8,553 31,087 2.4 12,505 30,012 900 29,111 -1,976 35,282 2.4 14,843 35,624 1,069 34,555 -727 40,526 2.7 17,187 46,405 1,392 45,013 4,487		2,396	.3	3,496	4,545	136	4,409	2,013	2,617
5,810 1.9 7,826 14,870 446 14,424 8,614 1 27,286 1.9 10,165 19,313 579 18,733 -8,553 31,087 2.4 12,505 30,012 900 29,111 -1,976 35,282 2.4 14,843 35,624 1,069 34,555 -727 40,526 2.7 17,187 46,405 1,392 45,013 4,487		3,640	es	5.486	7,132	214	6,918	3,269	5,887
27,286 1.9 10,165 19,313 579 18,733 -8,553 31,087 2.4 12.505 30,012 900 29,111 -1,976 35,282 2.4 14.843 35,624 1,069 34,555 -727 40,526 2.7 17.187 46,405 1,392 45,013 4,487		5,810	9.1	7.826	14,870	446	14,424	8,614	14,500
31,087 2.4 12.505 30,012 900 29,111 -1,976 35,282 2.4 14.843 35,624 1,069 34,555 -727 40,526 2.7 17.187 46,405 1,392 45,013 4,487		27,286	9.1	10.165	19,313		18,733	-8,553	5,948
35,282 2.4 14.843 35,624 1,069 34,555 -727 40,526 2.7 17.187 46,405 1,392 45,013 4,487	1993	31,087	2.4	12.505	30,012		29,111	-1,976	3,972
2.7 17.187 46,405 1,392 45,013 4,487	994	35,282	2.4	14,843	35,624	1,069.	34,555	-727	3,245
		40,526	2.7	17,187	46,405	1,392	45,013	4,487	7,732

However, when another LWUA's regular loan is applied to the project instead of the LWUA's soft loan which is employed here, the minimum charge should be increased in a few years before starting the debt services payment for this LWUA regular loan. It is therefore, advisable to apply the LWUA's soft loan into the project.

Period	Mimimum Charge	Monthly Family Income	Percentage of Income Allocated to Water	Percentage Increase
1988	₱22.5	₽ 826	2.7	· ·
1989	32.5	946	3.4	44
1990	32.5	1,083	3.0	0
1991	47.5	1,234	3.9	46
1992	47.5	1,407	3.4	0
1993	60.0	1,604	3.7	26
1994	60.0	1,829	3.3	0
1995	67.5	2,085	3.2	13

9.6 FINANCIAL SUMMARY

The financial analysis was based on the forecasts of the following financial statements:

- 1. Income Statement (TABLE 9.6.1)
 - 2. Cash Flow Statement (TABLE 9.6.2)
 - 3. Balance Sheet (TABLE 9.6.3).

The following assumptions were used for the financial projections:

- 1. Revenue-Tariff levels were based on the following objectives:
 - a) Revenue generation should be adequate to meet the minimum financial internal rate of return of 12-14%.

TABLE 9.6.1 PROJECTED INCOME STATEMENT

			* * * *						. :		
Pesos)	1998	23.350 9,636 30 6.745	3.5	1,805	32,483	1.392	1,805	34,287	27,672 8,068	21.604 28.078	-6,474
1,000	1997	23,350 9,636 30 6,745	3.5	61,959	28,246	1,210 8,337	1,805	30,051	31,909	25.841 28,169	-2,328
Cunit	1996	23,350 9,636 8,745	17,187	1,392	24,562	7,250 7,250 8,864	1,392	25,954	21,843	15.775	-12,474
	1995	23.350 9,636 6,745	17,187 2.7 46,405	1,392	21,358	915	1,392	22,750	25,047 6,068	18,979 18,660	318
	1394	20,255 8,579 32 5,834	14,843 2.4 35,624	1,069	3.545	697 697 4 757 8 861	1,069	17,183	19,510 5,644	13,865	4,856
	1993	17,160 7,462 34 4,925	12.505 2.4 30.012	900	11,919	835 3,509 4,809	•	12,819	18,093 5,069	13.024	-5.751
	1992	14,064 6,273 36 4,015	10,165	579	8,510	377	579	9,089	10,803	6,242 18.776	-12,534
	1991	10,969 5,010 3,106	7,826	446	5,810	243 263 1.702 2 190	446	6,256	9,060	4,940	4,940
	1930	7,874 3,660 40 2,196	5,486	214	3,649	173 173 1.065	214	3.863	3,483	783	783
	1989	5,391 2,571 45	3,496	136	2,396	634 834 834	136	2,532	2,149 2,031	118	118
	1988	2,907 2,126 50 1,063	2,304	62	1,407 648	258 258 348	62	1.469	667 379	288	288,
	Year	No. of Service Connections Production (m3 x 1,000) Unaccounted for Water (%) Consumption (m3 x 1,000)	Equivalent Volume (x 1,000) Rate Unit Water Sales	Other Revenues - Total Revenues -	Direct Cost Administrative Expenses	Fower & Fuel Chemicals Maintenance & Repair Mischalfanous	Bad Debts	- Total Costs -	income Before Depreciation Depreciation	Income Before Interest Interest	Net Income

TABLE 9.6.2 PROJECTED CASH FLOW STATEMENT

									(Unit	1,000	Pesos >
Year	1988	1989	1990	1991	1992	1993	1994	1995	9661	1397	1998
Sources of Funds					600	000		700 1000	0	5	
	15,149	66.110	26,732	56,810	15,151	17,796	20,531	13,708	21,043	51,808 0	210,12
Equity				٠.	2,500	2,500	ંજે	3,232	0	0	0
Grant -Total Sources of Funds-	0 15,816	68, 259	30,215	65.870	28,454	38,389	42,541	41,987	21.843	31.909	27.672
Applications of Funds Investment in Project	15, 149	9	ري ري	1.87	17,651	20,296	23,031	16.940	0	0	
<pre>(apitalized Interest [Total Investment]</pre>	15,149	1,841 66,110	11,196	14,939 56,810	17,651	20,296	23,031	16.940	90	50	0
Interest (Regular Loan) Interest (Soft Loan) < Total Operational Interest >	000	000	000	000	18,776	18,775	18,721 0 18,721	18,660	18,590 9,659 28,249	18.510 9.659 28,169	18.419 9.659 28.078
n) aymen	500	000	000	60 6		നന				658 058	749 0 749
[Fota! Debt Services]	0	0	. • •	0	18.776	19,168	19,168	19,168	28,827	28.827	28,827
Working Capital Increase Cash & Other Current Assets	30 54	21 66 66	89.0		295				1,867	2.108	589
Accounts receivable Inventories	340 11	<u>8</u> 9	29	44.0	- CO	87.00	3,33	153		•	
Reserves Accounts Payable Customers' Deposit	235 145	333 333 333 333	0 808 569	968	1,418 1,485	1.987 2,288	2,686	3.560	4,641 4.094 3,503	4,708 4,540	5 414
-Total Applications of funds-	15, 179	66, 131	26.800	57,204	36,456	39.960	43.904	36.897	30,694	30,935	28.228
Cash Surplus	638	2.128	3.415	8,665	-8,002	-1,572	-1,363	5,090	-8,851	974	-556
Cumulative Cash Surplus	929	2,765	6.180	14,845	6.843	5,272	3,908	8.999	148	1,122	566
Cach Flow	838	2.128	4	99		1		1,858		974	-556
Cash Flow	838	2,128	3.415	8,665	-10,502	-4.072	-3,863	1.858	-8,851		974

TABLE 9.6.3 PROJECTED BALANCE SHEET

Year	1088	0001	0000	:							
	0.00	1909	1303		1991 1992	1993	1994	1995	1996	1981	1998
Fixed Asset Depreciation	15,149	81.259	107,991	164,801	182,452	202,748	225,779	30,572	242,719 36,640	15,149 81,259 107,991 164,801 182,452 202,748 225,779 242,719 242,719 242,719 242,719 242,719 242,719 242,719 242,719 242,719 243,504 24,504 2	242.719 48.776
	1,047	3,605	7,476	17,485	10,289	10,585	12,038	19,593	3,605 7,476 17.485 10,289 10.585 12,038 19,593 13,144	17,877	17,877 17,428
-Total Assets-	15,817	82,453	110,357	173,056	178,949	194,473	213,313	231,740	219,222		211.371
Capital Equity Government Grant Operational Surplus	288	0 0 406	1.189	6,129	2,500	5.000	7,500	10,732	10,732	2,500 5,000 7,500 10,732 10,732 10,732 10,732 0,	10,732
Total Equity	288	406	1,189		-3,908	-7,157	-9.512	-5,961	-18.435	6.129 -3,908 -7,157 -9.512 -5,981 -18,435 -20,763 -27,237	-27,237
Long Term, Debt	15,149	81,259	107,391	164,801	179,952	197,355	217,439	230,639	230,061	15,149 81,259 107,991 164,801 179,952 197,355 217,439 230,639 230,061 229,403 228,654	228,654
Current Liabilities	380	789	1,177	1,177 2,126		2,903 4,275		7,062	5,386 7,062 7,596	9,248	9,954
·Total Equity and Liabilities-	15,817	82,453	110,357	173,056	178,949	194,473	213,313	231.740	219,222	82,453 110,357 173,056 178,949 194,473 213,313 231.740 219,222 217,888 211,371	211,371

- b) The District should be able to meet its cash requirements and after project completion, to undertake a reasonable amount of investment to continue expansion and improvement of the system.
- 2. Reserve Fund; 10% of direct water sales.
- 3. Cash; 1 month of administrative expenses.
- 4. Accounts Receivable; 2 months of direct water sales.
- 5. Bad Debts; 3% of direct water sales.
- 6. Inventories; 2 months of expenses of chemicals.
- 7. Depreciation; 2.5% of average gross value of fixed assets.
- 8. Accounts Payable; 2 months of direct operating and maintenance costs.
- 9. Customer's Deposits; \$50.00 per service connection.
- 10. Other Operating Revenues; 3% of direct water sales.

9.7 FINANCIAL INTERNAL RATE OF RETURN (FIRR)

The Financial Internal Rate of Return is a major consideration affecting capital investment decisions. It measures the effective utilization of the total investment and also the equity employed in the project and shows the compounded growth of investments within the project period.

The rate of return was computed based on the present value of cash inflows and outflows. As shown in TABLE 9.7.1, the rate of return, with the assumptions made, is estimated to be 13.4%.

TABLE 9.7.1 FINANCIAL INTERNAL RATE OF RETURN

	i		*						٠.													
	Present Value	מדמע ^	636	1,876	2,654	5,936	-6,341	-2,167	-1,813	492	-3,227	313	-158	1,097	263	835	-59	783	-152	199	-799	779-
	Net Cach Taflow	Casil Filt IOW	636	2,128	3,415	8,665	-10,502	-4,072	-3,863	1,858	-8,851	974	-556	4,392	1,192	4,301	-343	5,193	-1,141	1,698	-7,738	-7,077
Working	Capital	דוורי בסמ	30	21	89	394	29	967	1,705	789	1,867	2,108	-665-	1,611	-792	1,718	-1,048	2,081	-1,386	1,693	-1,832	1,507
Total	Debt	מבד י דר עפ	i 1	ì	1	1	18,776	19,168	19,168	19,168	28,827	28,827	28,827	28,827	28,827	30,514	30,514	30,514	30,514	30,514	30,514	30,514
	Total	דוואפארוועוור	15,149	66,110	26,732	56,810	17,651	20,296	23,031	16,940												
	TITTA TOS	השחק החשקו	15,149	66,110	26,732	56,810	15,151	17,796	20,531	13,708												
Income	Defore	חפהדפרדחוו	299	2,149	3,483	090'6	10,803	18,093	19,510	25,047	21,843	31,909	27,672	34,830	29,227	36,533	29,123	37,788	27,988	33,905	20,944	24,945
	Rate/Unit	\r\\.\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	0.9	1.3	E. I	1.9	1.9	2.4	2.4	2.7	2.7	3.5	ю. М	4.2	4.2	5.0	5.0	0.09	6.0	7.0.	7.0	.⊒• •
	Year		1988	1989	1990	1991	1992	1993	1994	1995	1996	1997	1998	1999	2000	2001	2002	2003	2004	2005	2006	2007

9.8 FINANCIAL RECOMMENDATION

The recommended project for the Short Term Development Plan is financially feasible. The proposed water rates for 1/2 inch connections of domestic users to achieve financial self-sufficiency are:

Period	Rate/ Unit	First 10cu.m	11-20cu.m	21-35cu.m	Above 35cu.m
1988	P0.9	P22.5	P2.8	P3.6	P 4.7
1989	1.3	32.5	4.1	5.2	6.8
1990	1.3	32.5	4.1	5.2	6.8
1991	1.9	47.5	5.9	7.6	10.0
1992	1.9	47.5	5.9	7.6	10.0
1993	2.4	60.0	7.5	9.6	12.6
1994	2.4	60.0	7.5	9.6	12.6
1995	2.7	67.5	8.4	10.8	14.2
1996	2.7	67.5	8.4	10.8	14.2
1997	3.5	87.5	10.9	14.0	18.4

As shown above, rapid increases of Rate/Unit are inevitable in this project during the first three years, since the existing water rate structure is comparatively lower in level. However, after 1991 a moderate increase in the percentage of the water rate is expected annually.

CHAPTER 10 ECONOMIC FEASIBILITY ANALYSIS

CHAPTER 10 ECONOMIC FEASIBILITY ANALYSIS

10.1 GENERAL

The objective of the project is to uplift the social welfare of the community. Therefore, an evaluation of the effectiveness of the project, in terms of socio-economic factors not considered in the financial analysis, is made in this economic analysis.

It may not be possible to evaluate all the costs and the benefits of a project because some of them are not quantifiable or the technical methods to evaluate them quantitatively are not available. It is for this reason that only quantifiable benefits and costs will be included in the analysis.

The comparison between the costs and benefits shall allow an assessment of the economic feasibility of the project.

10.2 METHOD OF ANALYSIS

To evaluate economic feasibility of the project, the Economic Internal Rate of Return (EIRR) should be assessed through the calculations on the economic benefits and costs anticipated from the implementation of the project.

The project is considered economically feasible if the EIRR is higher than the opportunity cost of capital, or the rate of return that can be obtained from the best alternative use of the available capital. For public investment programs such as water supply projects, the opportunity cost of capital is from 12% to 15%.

10.3 ECONOMIC BENEFITS OF THE PROJECT

The implementation of the project will provide the following direct and indirect benefits:

Direct Benefits:

- Increase in the area and population to be served
- Continuous supply of safe water

Indirect Benefit:

- Increase of employment opportunity
- Improvement of health condition
- Increase in consumer satisfaction
- Increase in land values
- Reduction in fire damage
- Increase in income in some productive sectors

The quantifiable benefits considered in the economic feasibility analysis are: increased land value, improved health conditions, and the beneficial value of water (consumer's satisfaction).

Except for the beneficial value of water, all other benefits are held constant after completion of the project.

10.3.1 Increase in Land Values

The water supply improvement project will contribute to an increase in land value of the service area. However, this increased value could be the result of a general increase of productivity due to improved infrastructure which includes a water supply project.

The portion of land values attributable to the water supply system in the service area was determined by comparing the market values of land served and not served by the water system. The average market value of residential land served by the water system is \$\mathbb{P}60-120/sq.m, and commercial land is \$\mathbb{P}120-200/sq.m, while the average market value of residential land unserved by the water system is \$\mathbb{P}30-50/sq.m, and commercial land is \$\mathbb{P}120-200/sq.m, respectively.

Generally, it is assumed that 20% of the incremental value of land could be attributed to the water supply project.

The present worth benefit of increase in land values from the project implementation is shown in TABLE 10.3.1.

10.3.2 Improved Health Conditions

The provision of safe, potable water to the population is a prerequisite to attain minimum health standards. However, quantification of health benefits is usually difficult although the direct relationship between safe water supply and improved health is obvious.

In quantifying health benefits, two factors were considered: the cost of time lost due to illness, and the cost of medical expenses.

In computing the cost of the time lost due to illness, not all persons afflicted with water-borne diseases are income-earners. It is assumed in this study that 30% of the population is actually economically active. The average morbidity rate from 1981 to 1985 in Cabuyao was 1,464 out of 10,000. The final figure for the cost of time lost due to illness was derived by taking the economically active portion of those afflicted by water-borne diseases multiplied by \$\frac{p}{2}57.00 \text{ and } 15 \text{ days based on the assumption that workers earning \$\frac{p}{2}57.00/\text{day}^3/\text{ are unable to work for an average of 15 days per year.}

The cost of medical expenses was derived by multiplying the morbidity rate by the served population and the average annual expenditure for medical expenses of \$200.00.

The sum of the two economic costs related to health benefits was adjusted by 20% to account for the fact that not all water-borne diseases are caused by a poor water system but may also be due to poor personal hygiene or lack of sewerage facilities.

. The total present values derived from health benefits is shown in TABLE 10.3.2.

^{3/ ₱57.00} is assumed as the minimum wage for industrial and agricultural workers.

TABLE 10.3.1 PORTION OF LAND VALUES ATTRIBUTABLE TO PROJECT

	Land Us	Land Use (1,000 sq.m) ^{1/}		Cost of Land (P1,000)	*1,000)	
Year	Residential	Commercial/Indust- rial/Institutional	Residential (P30/sq.m)	Commercial/Indust- rial/Institutional (₹120/sq.m)	Total Cost of Land	20% Due to Project
1988 1989						
1990	720	80	21,600	9,600	31,200	6,240
1992	720	08	21,600	0,000	31,200	6,240
1994	720 720	80 80	21,600 21,600	9,600	31,200 31,200	6,240
1996						
1997 1998						
1999						
2001						
2002 2003						
2004						
2006						
2007						

1/ The service area of 340 ha from 1988 to 1990 is projected to increase annually by 80 ha from 1991 up to 1995. Land use is assumed to be 90% residential and 10% commercial.

2/ Portion of land values specifically attributable to water supply project is 20%.

TABLE 10.3.2 HEALTH BENEFITS

(Unit : ₱1,000)

Year	Served Population	Cost of Time Lost to4/ Illness4/	Cost of Medical Expenses 5/	Total Economic Losses	20% Reduction Due to Project (Benefit)
1988	23,560	885	690	1,575	315
1989	27,070	1,017	793	1,810	362
1990	38,390	1,442	1,124	2,566	513
1991	52,800	1,983	1,546	3,529	706
1992	67,200	2,523	1,968	4,491	898
1993	81,600	3,064	2,389	5,453	1,091
1994	96,000	3,605	2,811	6,416	1,283
1995	110,350	4,144	3,231	7,375	1,475
1996	110,350	4,144	3,231	7,375	1,475
1997	110,350	4,144	3,231	7,375	1,475
1998	110,350	4,144	3,231	7,375	1,475
1999	110,350	4,144	3,231	7,375	1,475
2000	110,350	4,144	3,231	7,375	1,475
2001	110,350	4,144	3,231	7,375	1,475
2002	110,350	4,144	3,231	7,375	1,475
2003	110,350	4,144	3,231	7,375	1,475
2004	110,350	4,144	3,231	7,375	1,475
2005	110,350	4,144	3,231	7,375	1,475
2006	110,350	4,144	3,231	7,375	1,475
2007	110,350	4,144	3,231	7,375	1,475

^{4/} 30.0% x $\frac{1,464}{10,000}$ x SP x p57 x 15 days

 $[\]frac{5}{10,000} \times SP \times P200$

10.3.3 Beneficial Value of Water

It is assumed that all residents of the served area would be willing to obtain water in sufficient quantities at a given price. In general, water prices charged by the Water District are lower than the real value of water. Taking the benefits for "consumer's satisfaction" into consideration, it is assumed that the economic value of water is 20% higher than the de-escalated average rate per cu.m of water used in the Financial Analysis.

The economic value of water is shown in TABLE 10.3.3.

10.4 ECONOMIC COSTS OF THE PROJECT

The direct costs of the project should be transformed into economic costs. For this purpose, the project cost and operating and maintenance costs are considered in the study. These costs will be converted into the economic costs using factors for shadow pricing. The factors for shadow pricing applicable to the study are as follows:

- Foreign exchange component: 1.3
- Unskilled labor premium : 0.5
- Others : 1.0

All taxes should be excluded in the economic study. It is assumed that the cost for the balance of domestic component includes hidden taxes for 5% of the amount.

10.4.1 Project Cost

By using the shadow pricing factors, the economic project cost is P124.99 million which was obtained based on the project cost used in the Financial Feasibility Analysis as shown in TABLE 10.4.1.

TABLE 10.3.3 INCREASE IN CONSUMER STATISFACTION

				· · · · · · · · · · · · · · · · · · ·
<u>Year</u>	Incremental Accounted-For Water 6/ (1,000 cu.m/Year)	Price Per	Economic Value Per cu.m8/	Economic Water Revenues (P1,000)
1988	-306	2.01	2.41	-737
1989	45	2.96	3.55	160
1990	827	2.67	3.20	2,646
1991	1,737	3.51	4.21	7,313
1992	2,646	3.15	3.78	10,002
1993	3,556	3.56	4.27	15,184
1994	4,465	3.19	3.83	17,101
1995	5,376	3.21	3.85	20,698
1996	5,376	2.86	3.43	18,440
1997	5,376	3.31	3.97	21,343
1998	5,376	2.96	3.55	19,085
1999	5,376	3.17	3.80	20,429
2000	5,376	2.83	3.40	18,278
2001	5,376	3.01	3.61	19,407
2002	5,376	2.69	3.23	17,364
2003	5,376	2.88	3.46	18,601
2004	5,376	2.57	3.08	16,558
2005	5,376	2.68	3.22	17,311
2006	5,376	2.39	2.87	15,429
2007	5,376	2.47	2.96	15,913

^{6/} The volume of accounted-for water 1.37 of million cu.m in 1986 is deducted from the projected water consumptions throughout the study period to obtain incremental volume.

^{7/} The price per cu.m was based on the de-escalated average rate/cu.m in the financial analysis.

^{8/} The economic value was assumed to be 1.2 times the price per cu.m of water.

8% of Total Cost of Civil Work & Equipment as Engineering Charge and Construction Supervision જો જે

10.4.2 Salvage Value

TABLE 10.4.2 presents the salvage value of all the capital equipment in the project in 2007. The percentage of the salvage value was based on the remaining service life of the facilities in 2007.

TABLE 10.4.2 SALVAGE VALUE IN YEAR 2007

(Unit : ₱1,000)

	Year	Economic Value	Remaining Life in 2007	Salvage Value	
	1988	13,114	50.0 %	6,557	
	1989	47,401	52.5	24,886	
	1990	9,772	55	5,375	
	1991	23,517	57.5	13,522	
	1992	8,621	60	5,173	
	1993	8,619	62.5	5,387	
	1994 .	8,505	65	5,228	
÷	1995	5,440	67.5	3,672	
	1996	i i			
	1997				
, · ·	1998				
	1999	•			
	2000				
	2001				
	2002				
	2003				
٠	2004				
	2005				
	2006				
	2007			69,800	

^{11/} The average economic life of all items is assumed to be 40 years.

10.4.3 Operating and Maintenance Costs

In the economic analysis, operating and maintenance costs of personnel, power, chemicals, and maintenance are considered. Likewise, this cost category is converted to economic costs by the shadow pricing factors. TABLE 10.4.3 shows the economic operating and maintenance cost.

10.5 ECONOMIC INTERNAL RATE OF RETURN (EIRR)

EIRR is determined based on economic costs and benefits of the projects. TABLE 10.5.1 shows the computation of the EIRR. The EIRR is 12.3%. Since this rate exceeds barely the opportunity cost of capital of 12%, and a number of unquantifiable benefits will be also conceived from the implementation of the project, the project is considered economically feasible and an undertaking of the project is suggested itself to proceed positively.

(00		r c	1																			
(Unit : P1,000)		Net Economic	Cost	# 	175	717	1,254	2,040	2,916	3,751	4,573	5,410	5,410	5,410	5,410	5,410	5,410	5,410	5,410	5,410	5,410	5,410
		Economic	Cost	892	1,067	1,609	2,146	2,932	3,808	4,643	5,465	6,302	6,302	6,302	6,302	6,302	6,302	6,302	6,302	6,302	6,302	6,302
	CING	7 7 1	x 1.0	789	860	1,194	1,535	2,234	2,647	3,208	3,755	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318
	SHADOW PRICING	Foreign Exchange	x 1.3	208	207	415	611	869	1,161	1,435	1,710	1,984	1,984	1,984	1,984	1,984	1,984	1,984	1,984	1,984	1,984	1,984
		, , ,	(95%)	789	860	1,194	1,535	2,234	2,647	3,208	3,755	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318	4,318
		(; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	(5%)	36	45	63	81	118	139	169	198	227	227	227	227	227	227	227	227	227	227	227
		4000	Component	720	905	1,257	1,616	2,352	2,786	3,377	3,953	4,545	4,545	4,545	4,545	4,545	4,545	4,545	4,545	4,545	4,545	4,545
		Foreign	Component	160	159	319	470	537	893	1,104	1,315	1,526	1,526	1,526	1,526	1,526	1,526	1,526	1,526	1,526	1,526	1,526
	4	Financial	Cost	880	1,064	1,576	2,086	2,889	3,679	4,481	5,268	6,071	6,071	6,071	6,071	6,071	6,071	6,071	6,071	6,071	6,071	6,071
			Year	1987	1988	1989	1990	1661	1992	1993	1994	1995	1996	1999	2000	2001	2002	2003	2004	2005	2006	2007

TABLE 10.5.1 ECONOMIC INTERNAL RATE OF RETURN

			(Unit:	1,000 Pesos)
<u>Year</u>	Total Economic Benefits	Total Economic Costs	Net Benefits	Present Value
1988	-422	13,289	-13,711	-13,711.0
1989	522	48,118	-47,596	-42,375.3
1990	3,159	11,026	-7,867	-6,235.8
1991	14.259	25,557	11,298	-7,973.1
1992	17,140	11,537	5,603	3,520.4
1993	22,515	12,370	10,145	5,674.9
1994	24.624	13,078	11,546	5,750.2
1995	28,413	10.850	17,563	7,787.4
1996	19,915	5,410	14,505	5,726.0
1997	22.718	5,410	17,308	6,083.1
1998	20,560	5,410	15,150	4,740.6
1999	21,904	5,410	16,494	4,595.0
2000	19,753	5,410	14,343	3,557.5
2001	20,882	5.410	15,472	3,416.6
2002	18.839	5,410	13,429	2,640.2
2003	20.076	5,410	14,666	2,567.1
2004	18,033	5,410	12,623	1,967.1
2005	18,786	5,410	13,376	1,855.8
2006	16,904	5,410	11,494	1,419.8
2007	17,388	-64,390	81,778	8,993.6

EIRR (%)= 12.32

CHAPTER 11 ORGANIZATION AND MANAGEMENT

CHAPTER 11 ORGANIZATION AND MANAGEMENT

11.1 PRESENT ORGANIZATION STRUCTURE

The CSBWS is owned by the provincial government of Laguna and is managed and operated by the Cabuyao-Sta. Rosa-Biñan Provincial Waterworks.

A total of thirty-one (31) personnel compose the present staff of the system. It is headed by a waterworks superintendent who has direct supervision over all operations. See FIGURE 11.1.1 for the present organizational structure of the system.

The waterworks system basically follows the rules and regulations promulgated by the defunct NAWASA. Since the system's transfer from NAWASA to the province in 1979, only one resolution was passed, that is; the increase in water rates in january 1984.

11.2 PROPOSED ORGANIZATION STRUCTURE

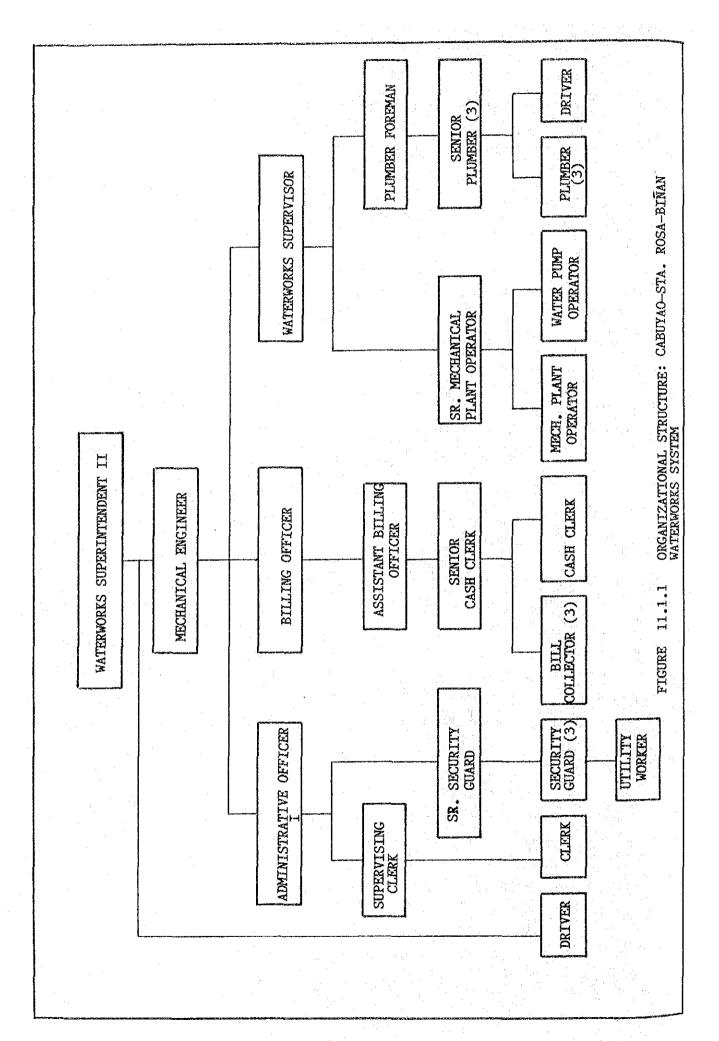
11.2.1 Introduction

The proposed organization structure for the CSBWS is the water district structure. The JICA Study Team believes that with the water district organization structure, the water system can look forward to dedicated policy makers providing continuity of policy and its consistent enforcement; to financial independence and business-like management; to capable, qualified and regularly trained personnel; and to freedom from political interference in its operations, particularly on hiring and water rates setting.

The JICA Study Team, however, proposes some changes on both the staffing guidelines, as well as the organization chart, without shaking the basic water district structure.

11.2.2 Examination of the LWUA Methodology Manual

The number of water district employees depends primarily on the total number of service connections. Based on this concept, LWUA's Methodology Manual proposes the following staff requirement for a water district.



		No.	of Connect	ions	
Personnel	2,000	5,000	10,000	15,000	20,000
					·
General Manager	1	. 1	1	: 1	1
Administrative Staff	7	14	20	22	23
Technical Staff	16	35	59	75	92
Commercial Staff	12	25	40	52	64
- meter readers,	(6)	(14)	(25)	(32)	(42)
bill collectors,				•	•
inspectors					÷
- other employees	.(6)	(11)	(15)	(20)	(22)
					·
Total Staff	36	75	120	150	180
Staff/1,000			····		
Connections	18	15	12	10	9
			- 		

Note: The above data do not include personnel for construction of new connections, etc. which are considered to be part of development cost. These development expenses entail additional staff which is assumed to be 15 personnel per 1,000 additional service connection.

Moreover, the above figures are related to a water supply system without treatment plant. If such plant were necessary, additional 10 to 15 employees for each treatment plant with a capacity ranging from 10,000 to 50,000 cu.m/day may be considered.

Above-indicated table shows the staff requirement corresponding up to 20,000 connections. Therefore, we cannot use this table to compute the number of personnel both for Angeles Water District and Dagupan Water District in 1995 and 2010, since they will apparently be beyond the extent of the table.

It is possible, not to mention, that it can be estimate the number of personnel exceeding 20,000, on the basis of some assumption of personnel number per 1,000 connections. But it seems to be slightly forcible.

Therefore, JICA Study Team checked the formula derived from the result of a statistical analysis of the number of personnel in relation to the number of connections for 38 existing water districts for the years 1979, 1980 and 1982.

The guideline proposes two staffing levels. The first level has been proposed for the period 1986 - 1995; the second for the period 1996 - 2010.

The upper staffing level for the period 1996 - 2010 was computed from the formula:

log (no. of employees) = log (no. of connection) x 0.8311 - 1.2113

See FIGURE 11.2.1 for the number of personnel needed for the Cabuyao-Sta. Rosa-Biñan Water District, once formed, using the LWUA Methodology Manual.

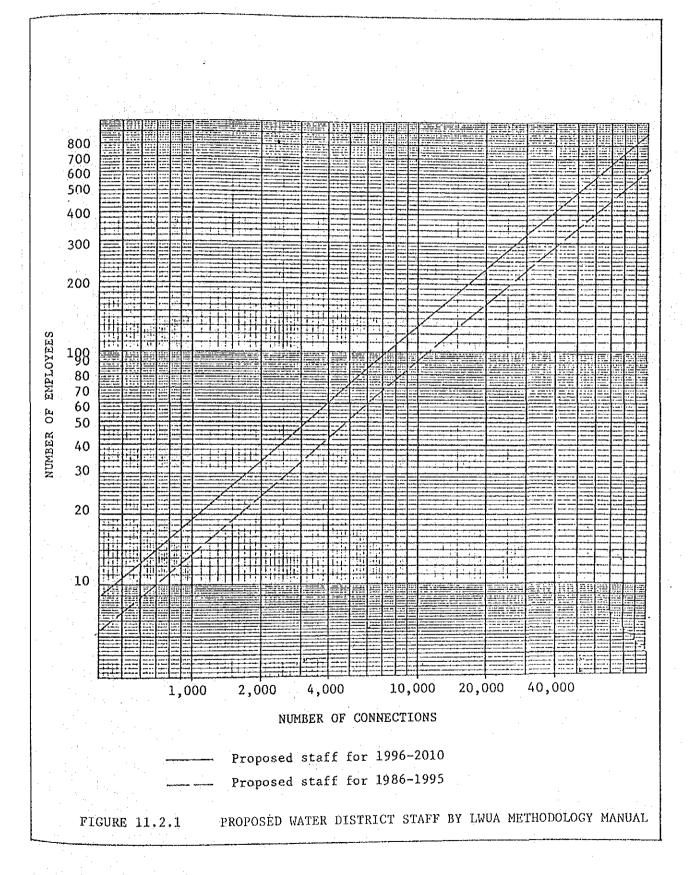
The lower level, which was assumed for the earlier period from 1986 - 1995, allows a 30 percent reduction in Staff below the calculated value.

Using the above mentioned formula, the number of personnel for CSBWS, if formed into a water district, could be computed as follows:

Design Year	No. of Employees
1995	184
2010	626

It is true that the formula presented by LWUA as a guideline in this study is based on the results of a Statistical analysis of the existing water districts. But no evaluation has been indicated whether the figures which were derived from the formula are appropriate or not.

Considering the actual scope and content of operation and management work, the number of personnel computed from the LWUA Methodology Manual, seems to be rather high, particularly for the year 2010.



In this study, therefore, a new proposal regarding the number of personnel based on the appropriate performance of individual work may be recommended.

11.2.3 Proposed Guideline of JICA Study Team

The task of management is to carry policy into effect with the fullest efficiency within the limits assigned; that is, to attain the maximum performance at the minimum cost. It is the duty of management to create conditions which will bring about the optimum uses of all resources available to the water district.

Based on this concept, the JICA Study Team closely examined the present structure of the existing CSBWS, not only regarding to the number of personnel but also the quality of the services to be provided to the consumers. Also considered as a reference were the statistical data regarding to the number of personnel and the organizational structures of CSBWS in a developed country.

The procedure which has been adopted in this study is, at first, to divide the organization of a water district into two main branches:--

- a) The administrative and commercial division, comprising what may be termed the business management (including matters concerning water charges) of the water district.
- b) The engineering and technical division, embracing the design and construction of minor extension or improvement works utilizing internal reserve fund; and the operation, renewal, and maintenance of existing works.

Secondly, the number of personnel has been computed considering the present number of personnel of said waterworks and appropriate performance of individual assignment.

TABLE 11.2.1 shows the JICA Study Team's on the personnel needed to man the proposed water district by scope and content of work.

TABLE 11.2.1 STAFFING GUIDELINE

DIVISION AND POSITION OF STAFF	NUMBER	CONTENTS OF WORKS, REMARKS
General Manager	1	Sec.23 of Provincial Water Utilities Act of 1973
ADMINISTRATIVE AND COMMERCIAL		
Asst. General Manager	-: :\	
General Affairs Division		°Correspondence
Manager	H	Friing Acondas
Staff	2 (< 20,000	"Establishment
	connections)	Resister of Land, fixed assets
	4 (< 50,000	Statistics
	connections)	General Information and Returns
Account Division		°Cash Receipts and Payments
, , , , , , , , , , , , , , , , , , ,	·	
וופוופלבד	1	×
Staff	3 (<10,000	ving
	connections)	Rates and Rating "Wages and Insurance
	5 (< 30,000	Recoverable Charges
	connections)	ent
	7 (<50,000	Supplies Miscellaneous Costs
	connections)	
General Service Division		Store-keeping
Manager	· •••••	Transportation "Utilities
Clerk	- -1	
Mechanics	I	
Staff	4 - 10	4 for less than 10,000 conn., 6 for less than 20,000 conn.,

TABLE 11.2. 1 STAFFING GUIDELINE (continued)

DIVISION AND POSITION OF STAFF	NUMBER	CONTENTS OF WORKS, REMARKS
Water Charges Division Manager Clerk Meter Reader	1 2 (<20,000 connections) 3 (<30,000 connections) 5 (<50,000 connections) Proportional to the no.of conn.	"Assessments "Resister of Supplies "Guarantees "Mater Charges "Revision of Charges "Statistics "Water Survey "Meter Reading (Consumption, Complaints, Reports) "Collection of Water Charges "Collection of Water Charges "One (1) meter reader per 1,500 connections
TECHNICAL		
Asst. General Manager (Eng'r)	1	
Distribution Division Manager (Eng'r) Mechanics Electrician Pump Operators Reservoir Attendants Pipeline Patrol General Maintenance	2 2 for every 5 stations 2 Prop'l to the Pipeline length 3	**Preservation of Water Sources, Prevention of Pollution **Afforestation (in case of spring source) **Operation and Maintenance **Service Reservoirs **Transmission Mains/ Distribution Mains* **Pumping Stations ** Waste Inspection ** Extentions, Renewals, Cleaning and Repairs, Street Repairs ** Waste Inspection ** Extentions, Renewals, Cleaning and Repairs, Street Repairs ** Two (2) attendants per one reservoir ** One (1) staff per every five (5) kilometer for the first 60 km, then add one per every ten (10) kilometer ** Painters, Blacksmith & c.

It was decided by JICA Study Team, to assign no bill collector based on the assumption that the consumers will pay water charges directly to the water district of through their affiliated banks.

TABLE 11.2.2 shows the summary of the present (1986) and the proposed staffing pattern (1995 and 2010) for Cabuyao-Sta. Rosa-Biñan.

11.2.4 Proposed Organization Structure

The organization structure proposed for the CSBWS is basically the water district structure.

The water system/district will be headed by a five-man Board of Directors, as per PD 198, who will come from the various sectors of the community and will be appointed by the mayor or governor, as the case may be. It is this Board that will set all the policies of the water system/district.

The everyday affairs of the water system/district will be managed by the General Manager who will be appointed by the Board of Directors.

The two broad divisions of the system/district will be:

- a) Administration and Commercial Division
- b) Technical Division

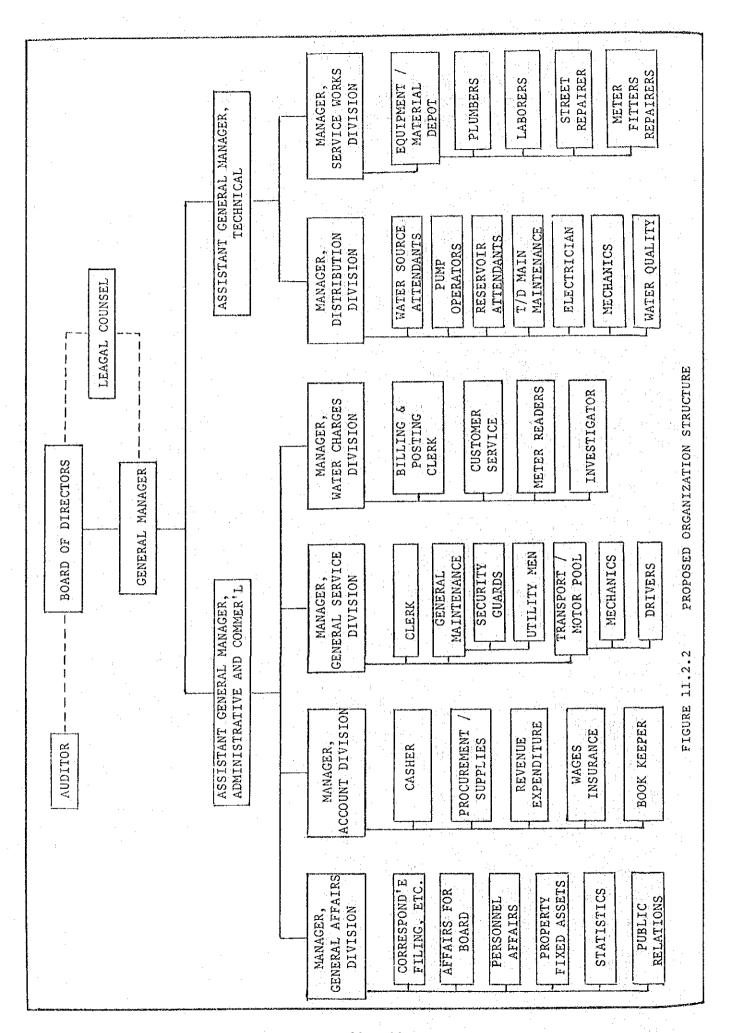
The proposed number of personnel for both divisions is:

en e	1995	2010
General Manager	1	1
Administrative and Commercial	44	79
Technical	36	61
<u>Total</u>	81	<u>141</u>

FIGURE 11.2.2 shows the proposed organization chart for the Cabu-yao-Sta. Rosa-Biñan Water District.

TABLE 11.2.2 SUMMARY OF PRESENT (1986) AND PROPOSED (1995 and 2010) STAFFING PATTERN (CABUYAO-STA. ROSA-BINAN)

4.7.7.7.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4	1986	1995	2010
Population Served	22 560	110 (00	007 000
No. of Connections	23,560	110,400	287,800
	2,907	23,400	66,400
Supply Capacity (cu.m/d)	10,000	32,900	94,000
Length of T/D Main (m)	27,000	35,000	73.000
No. of Pump Stations	2	6	17
General Manager	-	1	1
ADMINISTRATIVE AND COMMERCIAL		_	
Assistant General manager		1	1
General Affairs		1	1
	2.	4	6
Accountancy	2	6	8
General Services	. 7	13	13
Water Charges			4.3
Manager	. → ¹ .	1	1
Clerk	4	3	6
Meter Reader	-	16	. 44
Bill Collector	3	-	-
Sub-Total	7	20	51
Total	18	44	79
TECHNICAL		. —	· · · · · · · · · · · · · · · · · · ·
Assistant General Manager	1	1	1
Distribution			
Manager	- .	1	1
Mechanics	3	2	2
Electrician	_	1	. 1
Pump Operator	1	$\bar{3}$	4
Reservoir Attendant		. 4	6
Patrol		7	13
		3	3
General Maintenance	,		
Sub-Total	4	22	31
Service Works			
Manager	e e e e e e e e e e e e e e e e e e e	1	. 1
		3	7
Fitter	·	4	8
Meter Repairman	7	. 3	
Plumber	1	3	7
Laborer	<u>.</u>		
Sub-Total	8	14	30
Total	<u>13</u>	<u>36</u>	61
GRAND TOTAL	31	21	141



11.3 MANAGEMENT

11.3.1 Importance of Management

As stated in Section 11.2.3, the task of management is to carry policy into action with the fullest efficiency within the limits assigned; that is, to attain maximum performance at minimum cost. It is the duty of management to create conditions which will bring about the optimum use of all resources available to the water district. Thus the scope of management carries with it a lot of responsibilities.

The continuous and successful operation of a water district depends largely on the patronage of its consumers. The consumers would only patronize a water district if it can provide them safe water in sufficient quantities at all times at a cost within their reach. Patronage of consumers can be achieved through good management.

11.3.2 Functions and Duties of General Manager/Staff

FIGURE 11.2.2. shows the departmental organization in a mediumsized water district whose sources of supply are deep wells. (If its sources are surface water, the treatment plant staff should be added.)

Many of the functions indicated on the chart are common to the four proposed water districts respectively, although relatively small water districts (e.g., Cabuyao-Sta. Rosa-Biñan Water District) may find it unnecessary to departmentalize during the early stage.

A carefully planned organization is essential to ensure that the needs of consumers throughout the area can be promptly appreciated and efficiently met. Thus, it is also desirable that every member of the organization not only understand and realize the significance and importance of his or her particular function but also properly discharge that particular function.

(1) The General Manager

Provincial Water Utilities Act of 1973 prescribes the Officers and Employees as follows:

"SEC. 23. The General Manager. --- At the first meeting of the Board, or as soon as practicable, the Board shall appoint, by a majority vote, a general manager and shall define his duties and fix his compensation. Said officer shall serve at the pleasure of the Board. (As amended by Sec. 9, PD 768)

SEC. 24. Duties. --- The duties of the General Manager and other officers shall be determined and specified from time to time by the Board. The General Manager, who shall not be a director, shall have full supervision and control of the maintenance and operation of water district facilities, with power and authority to appoint all personnel of the district: Provided, that the appointment of personnel in the supervisory level shall be subject to approval by the Board. (As amended by Sec. 10, PD 768)"

Thus, the duties fall, to a large extent, within the sphere of administration. The General Manager should therefore, have managerial ability, and a thorough knowledge of the administrative machine which he controls. Managerial ability implies the ability to select suitably trained or qualified staff, to delegate work and responsibility wisely, and to create and maintain a spirit of co-operative enthusiasm throughout the entire organization. The General Manager must be able to co-ordinate the efforts of the several different sections, and should keep in close touch with the more important matters being dealt with by each.

Since water industry is mainly concerned with the problems of the collection of water, its storage, treatment (at least chlorination), and distribution, it is most desirable that the General Manager is a chartered civil/sanitary engineer who also has managerial abilities on general, and especially, financial matters.

Presently, however, the four study areas are not yet managed as a water district, for these systems to start out rightly, it must be able to

have a man of managerial competence and experience appointed as a General Manager.

Therefore, the first priority should rather be given to managerial ability rather than to be an engineer. In case any other non-technical person is appointed as the General Manager, the General Manager should be assisted by a chief engineer.

Although the General Manager is responsible for the day-to-day conduct of affairs of the water district, he should not become too immersed in details. He should exercise over-all supervision and control, thereby giving himself ample time to plan the improvement/development of the Water District carefully in anticipation of the consumer's needs, and to deal promptly with unforeseen situations of major importance as they arise.

(2) The Administrative machine

As indicated in the FIGURE 11.2.2, the proposed organization of the water district can be divided into two main branches:--- (1) the Administrative, and (2) the Technical.

These two branches are in contact at many points, and are so interdependent that a high degree of co-operation is essential to ensure co-ordination of effort, economy in working, and proper balance between income and expenditure. Each of these two main branches is, in turn, divided into a number of subsections.

As efficient administration cannot be achieved by staff in watertight compartments, officers in charge of departments should not only possess a detailed knowledge of every aspect of the work of their respective departments, but should also take an interest in, and have a general knowledge of, the work of other departments.

In the organization depicted in functional form in FIGURE 11.2.2, it seems not necessary to mention the scope of the individual works one by one basis. Keynotes or recommendations are as follow:---

11.3.3 Problems Arising from the Transition of Administrations

Transition from the existing water supply organizations to the respective water districts will inevitably take time. Each water authority then should make necessary preparations for setting up respective water districts in various aspects — legislative, budgetary, personnel and technical.

(1) Personnel

- a) Appointment of qualified personnel to the key positions is indispensable for good management of a water district. Generally, it might be somewhat difficult to get well educated or qualified personnel in local cities and municipalities, especially in areas far from Metro Manila. However, there are other aspects that may make up for apparent lack of education or qualification. These are experience, the right attitude and the potential or capacity of a person to learn.
- b) If the employees who belong to the existing water supply authorities will transfer to the proposed water districts there will be no displacement, since the number of employees of the existing water supply authorities are less than the proposed number of personnel required for water districts in the target years 1990 and 2010.

However, there may be problems on absorbing dead wood or unwanted employees. It may be best to terminate such employees services at the start of the water district operations so as not to complicate matters further.

- c) It is recommended that the experience of the personnel who have worked for a long time for the relevant water authority and are well acquainted with that water supply system, managerially or technically, be made good use of.
- d) In the procedures for recruitment, if necessary, any newly formed water district may fully utilize LWUA's assistance to obtain de-

sirable personnel, especially for managerial positions. It is expected, therefore, LWUA can use its testing system to recommend the most appropriate candidate as required water district.

- e) Attention should be given to procedures for the recruitment, proper use and retention of technical personnel including the establishment of career structures.
- f) Training of personnel on all levels and categories is a vital aspect in relation to work performance, morale and retention of staff. It was disclosed through the study, that most of the staff of all the four water supply systems have not enjoyed the benefits of any kind of training at all. Before and after the formation of these systems into water districts, LWUA should train the personnel from the board of directors and General Manager down to the plumber.
 - g) An information dissemination and public relations programs should be developed. The water districts serves the people and needs the support of the people, especially on payment of water bills or when the water rates are increased. It should therefore regularly inform and educate its public on the plans and programs of the water district and on the correct usage of water.

(2) Change of Charging System

a) At present, in the waterworks of Angeles City, Dagupan City and Cabuyao-Sta. Rosa-Biñan, the combined use of flat rate system and metered system is adopted, while in Bayombong-Solano only a flat rate system is adopted because all the faucets are unmetered.

In principle, the proposed four water districts should be operated and managed on the basis of 100% metering, in accordance with the Letter of Instruction No. 700, June 1, 1978, to insure correct charging of water actually consumed and discourage its wasteful use. One hundred (100) percent metering is the basis of the water tariff structure and, in effect, is the basis for financial viability.

- tioning/malfunctioning meters) to all the consumers takes time.

 Therefore, it is necessary to stipulate a provisional rate regulation of combined use of flat rate and metered system, fixing the target date by which 100% metering should be realized.
- c) Since the life of water meters is about 8 years, the water district should have a plan to replace water meters installed at every customer once in 8 years basis and to guarantee the budget and manpower necessary for its execution.

11.3.4 Problems Arising from Rapid Expansion of the Systems

(1) Coordination During Construction Period

According to the demand projection in the target year for the Short Term Development, the size of the water supply systems will more than double compared to its present size.

Considering the rapid expansion of the water supply systems, there may be much inconvenience caused by the construction work. For instance, to minimize water interruption and traffic congestion caused by pipe laying work, it is necessary to develop close coordination and cooperation between the respective water district and LWUA (Engineering Services) together with the local transportation authorities and inform the people to be affected by the expansion project.

11.4 OTHER RECOMMENDATIONS

11.4.1 LWUA Assistance

Immediately upon their formation into water districts, it is recommended that the LWUA install an appropriate commercial operation system (CPS) for back of the said water districts. Also, an effective and sustained training programs for both water district officials and personnel should be formulated and implemented as soon as possible. A rational public information and education program should also be undertaken by the new water districts.

The JICA Team also reminds that the LWUA maximize its assistance to the proposed water district upon its formation. It is expected that partially the proposed water district will need LWUA's assistance in all aspects of its operation and financial, technical and institutional. It is believed that this package of assistance will greatly enhance the water system's capability to provide an adequate and efficient water service to its consuming public.

11.4.2 Formation of Water Districts

Basically, LWUA provides every necessary assistance once a water-works system is formed into a water district. Yet, under the decree that created both LWUA and the water district (PD 198, as amended), the formation of a water district is at the option of the local governments concerned.

There is actually a very positive trend towards the formation of water districts in the above-mentioned four study areas. Members of the Sangguniang Panlalawigan/Panglunsod have been taking an increasing interest in the importance of an improved water supply system. The JICA Team believes LWUA can facilitate things by having a more aggressive program or campaign in water district formation itself; but should include effective information measures to promote formation through grass-roots and media based campaigns.

11.4.3 Mutual Interdependence

Another recommendation is to study the possibilities regarding a "mutual interdependence" between LWUA and MWSS for areas close to Metro-Manila.

This does not intend to contradict to the spirit of the legislation of Provincial Water Utilities Act and its related Letter of Instructions (No. 683, March 30, 1978).

Cabuyao-Sta. Rosa-Biñan area, one of the four study areas of this Municipal Water Supply project, will have enough water source to cope with

its future demand, even in the target year of the long term development plan, 2010.

This is because said area is blessed with an abundant groundwater resources which was listed up once as one of the nine potential sources of water supply for Metro Manila beyond the year 1990.

At present, MWSS supplies water to the Metro Manila and 29 contiguous municipalities. The service area to be covered by the on-going extension program (which is called as MWSP III) includes Muntinglupa and part of the Province of Cavite (Rosario, Imus and Bacoor) in its southern area.

Cabuyao-Sta. Rosa-Biñan area, especially Biñan area is adjacent to this southern limit of MWSS service area.

Therefore, if Cabuyao-Sta. Rosa-Biñan area was supplied by MWSS, the water charge of MWSS would be applied. This means that however the cost of expansion works for the said area may be, it will be shouldered by millions of consumers who live in MWSS service area. Considering the amount of the construction cost and the corresponding population served, the increase of water rates charged the consumers would be minimal.