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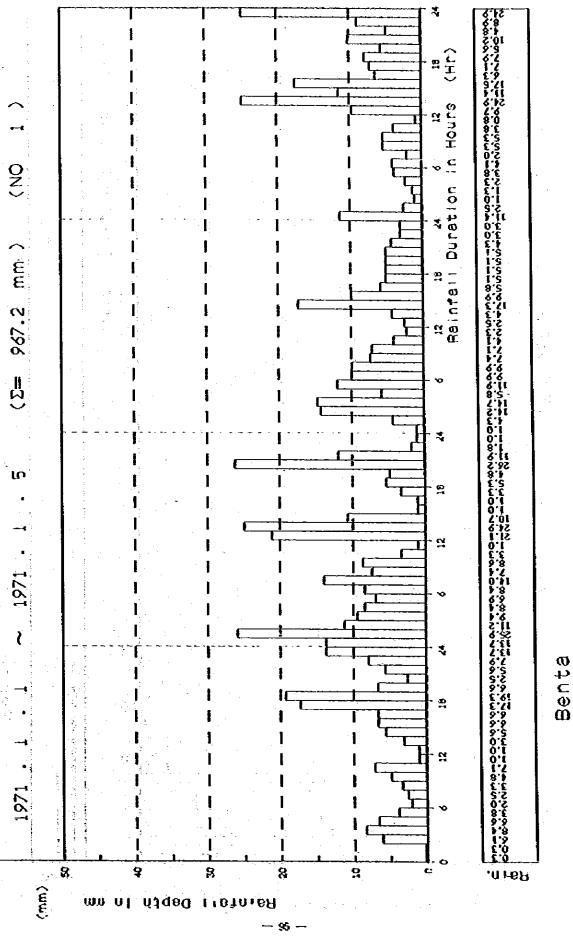
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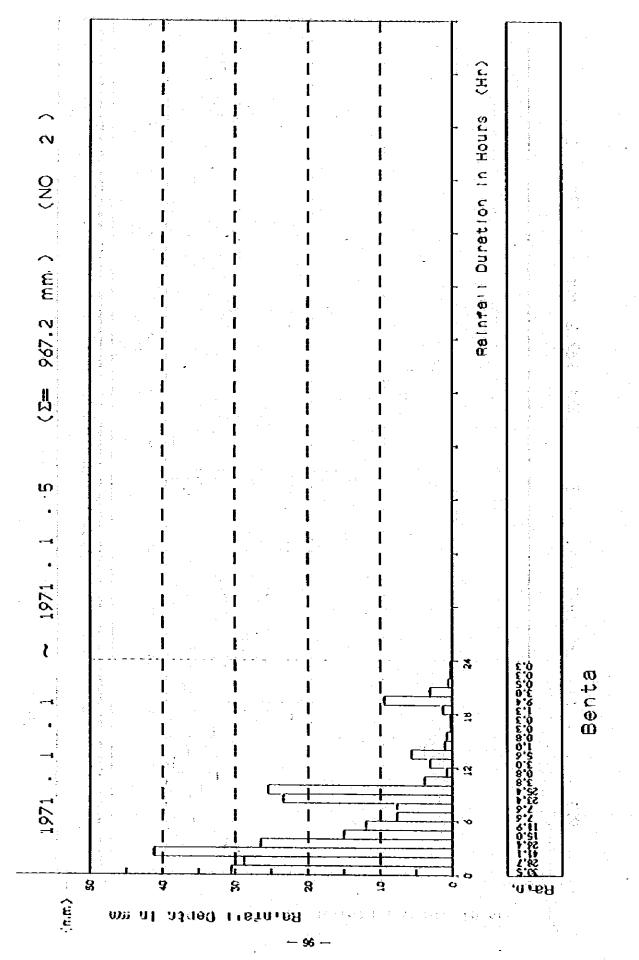
Appendix E Rainfall Patterns at Stations

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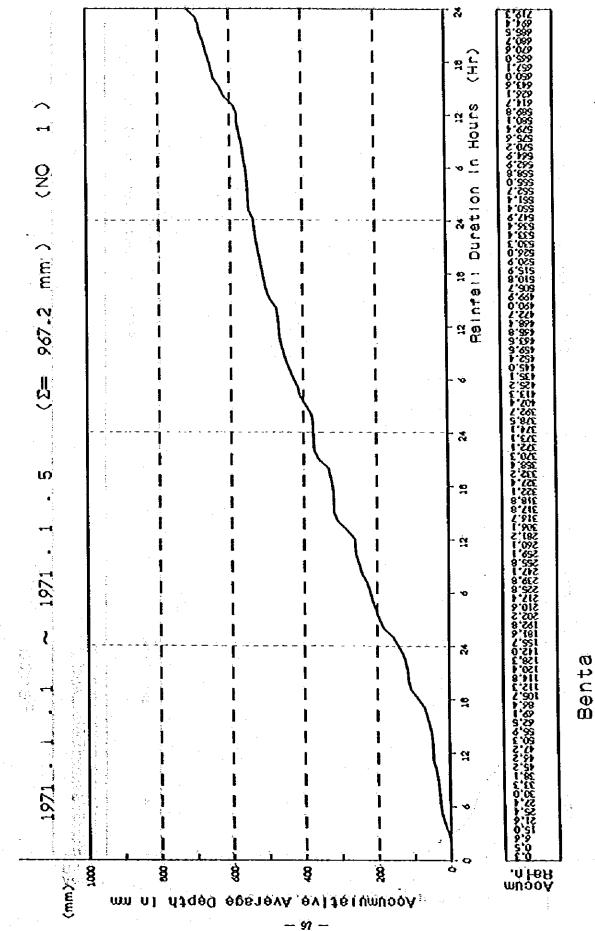
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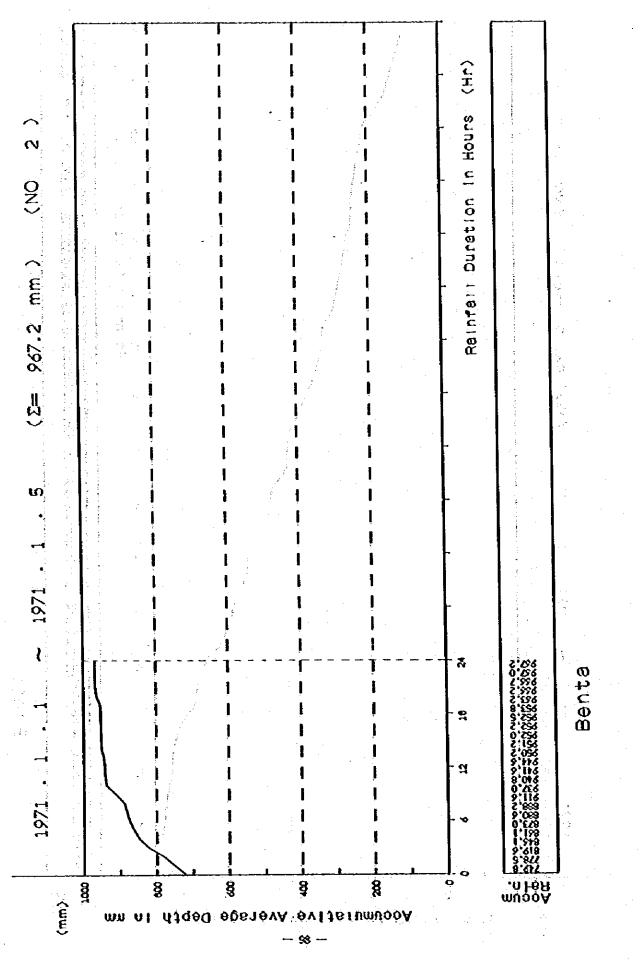
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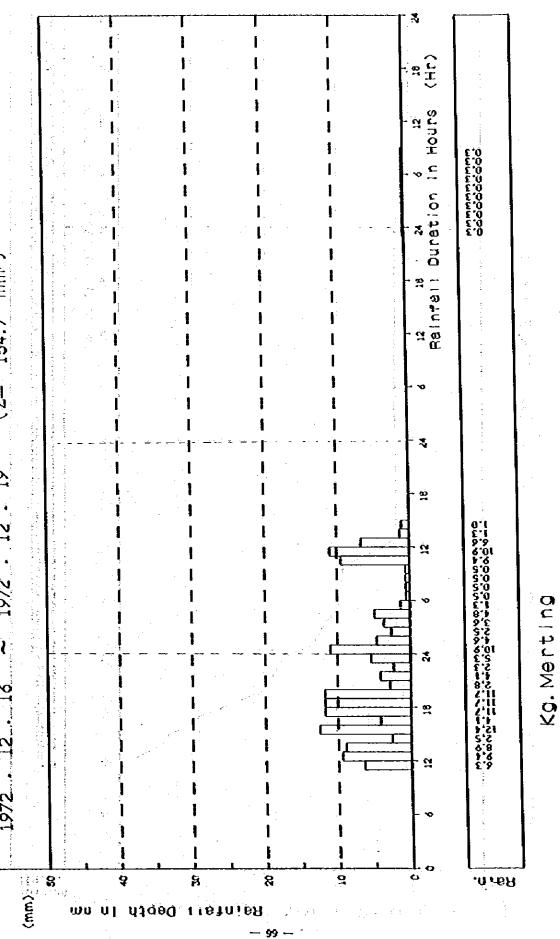


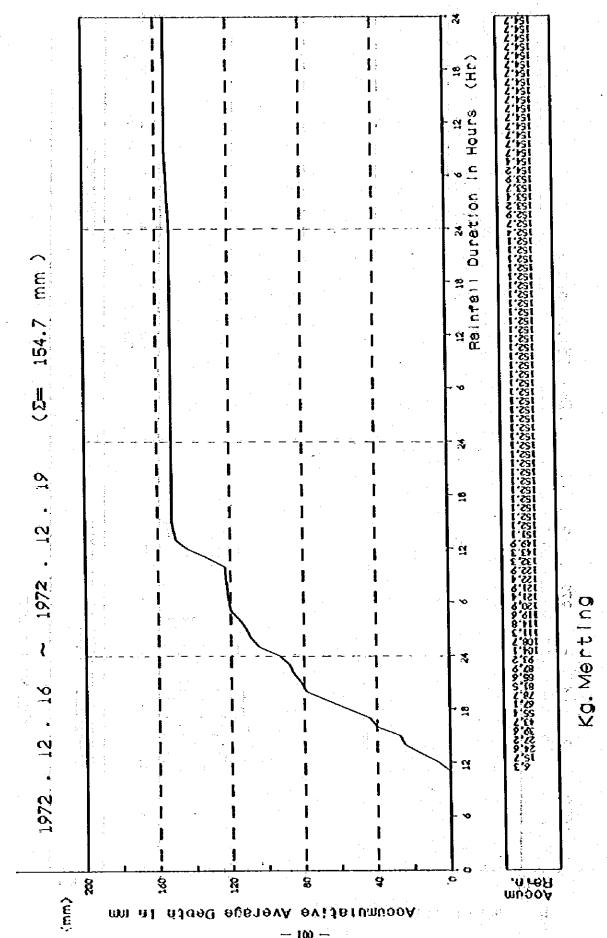
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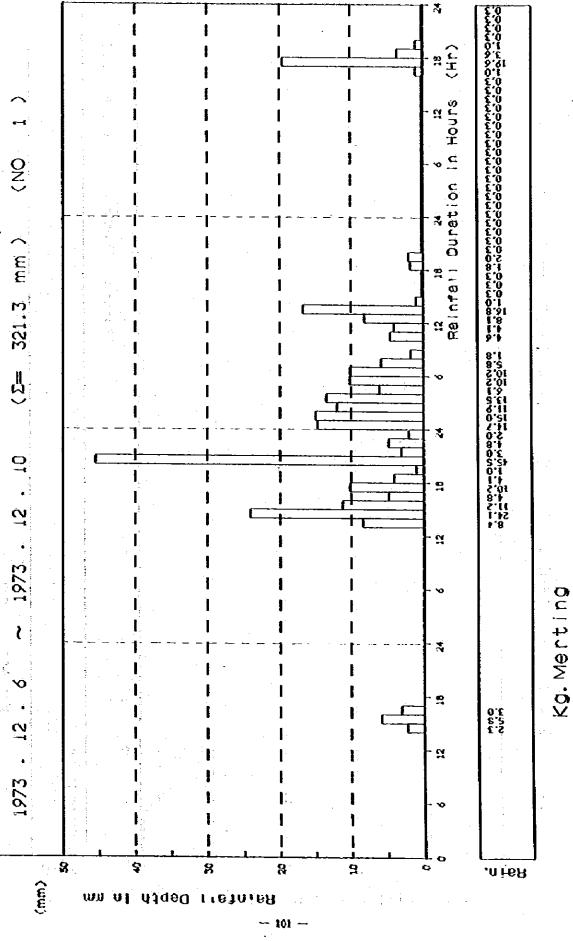
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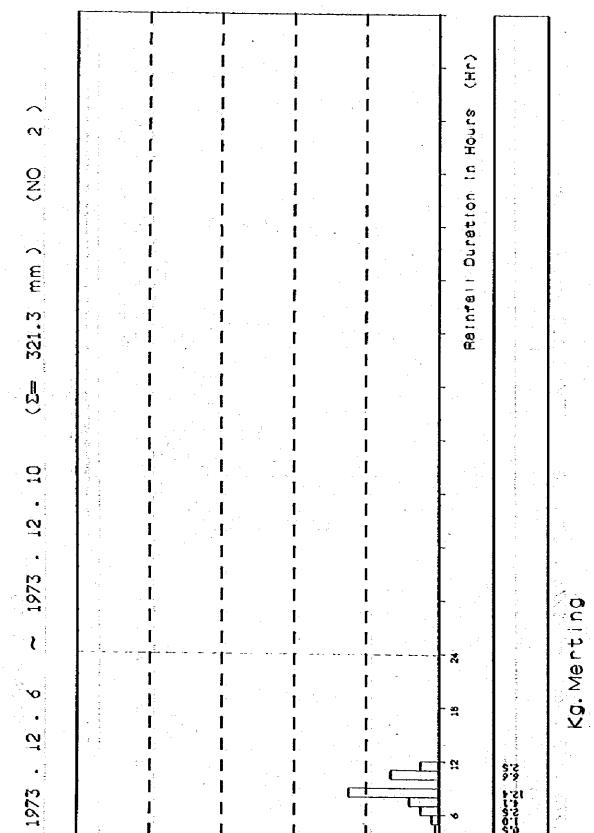




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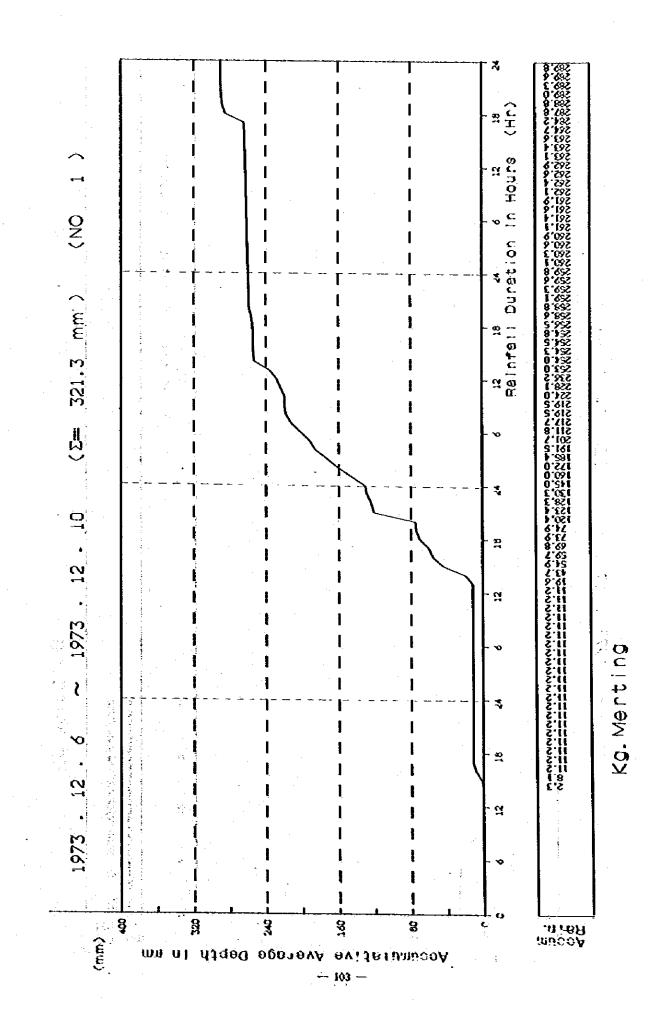
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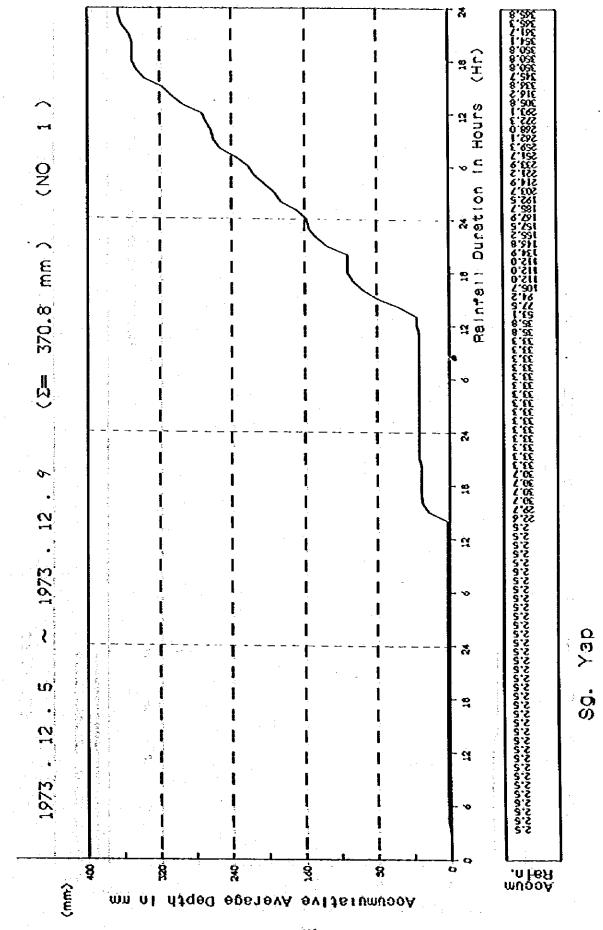
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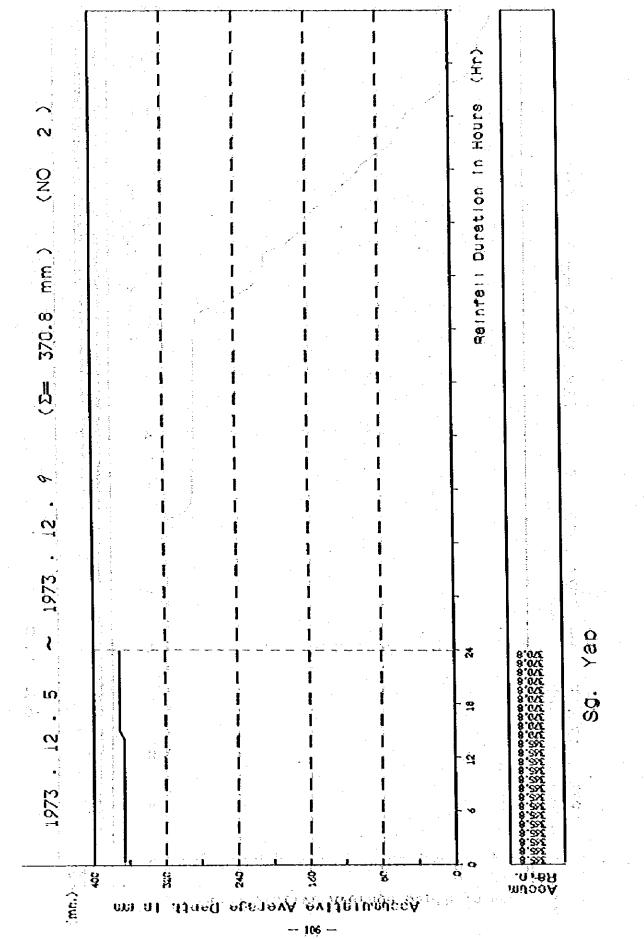
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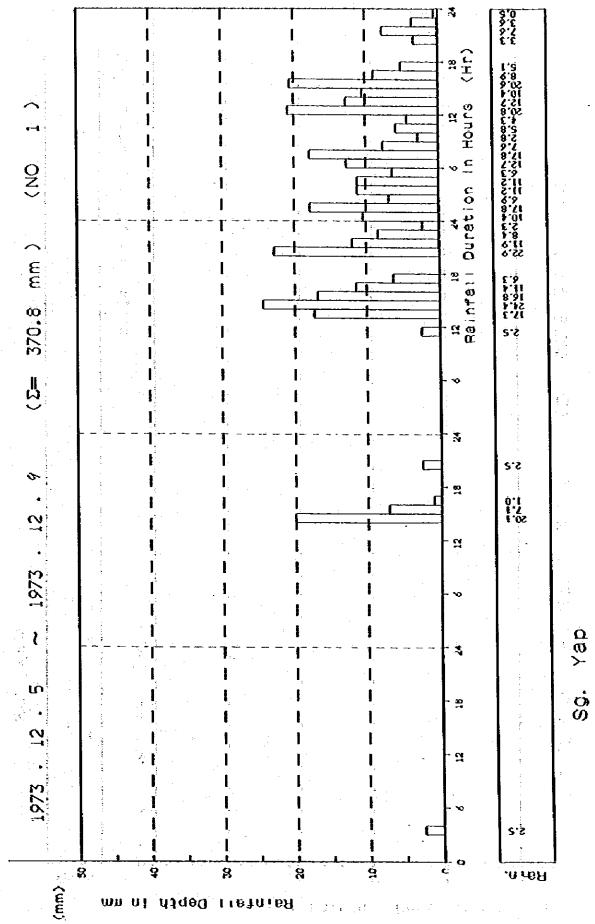
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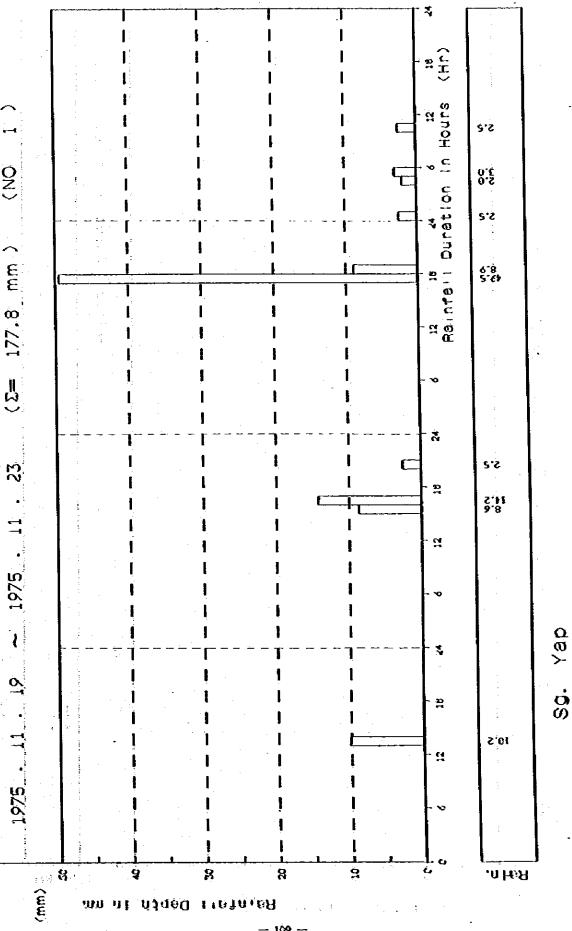


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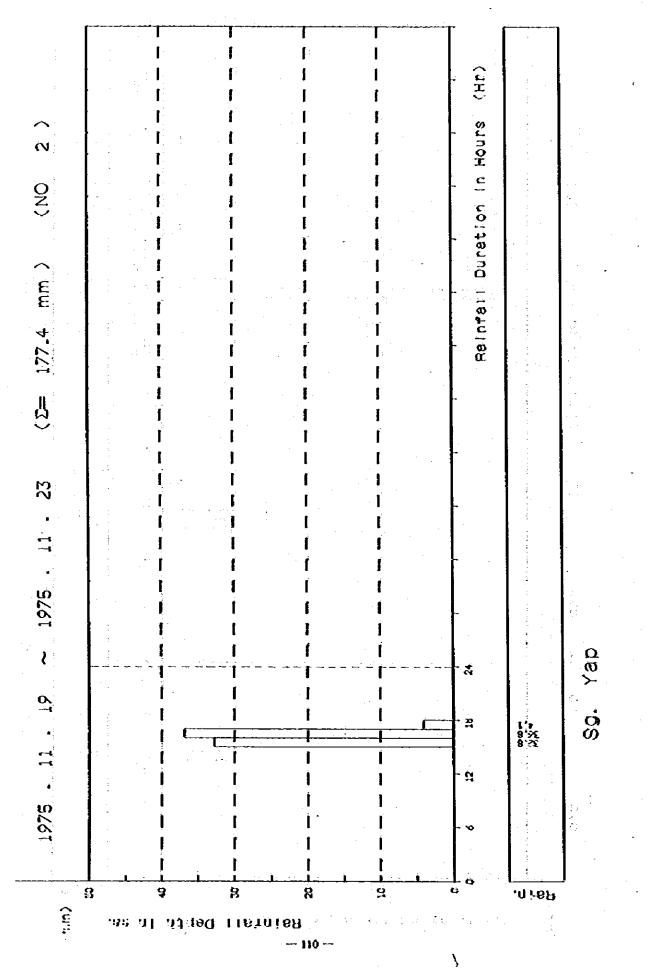
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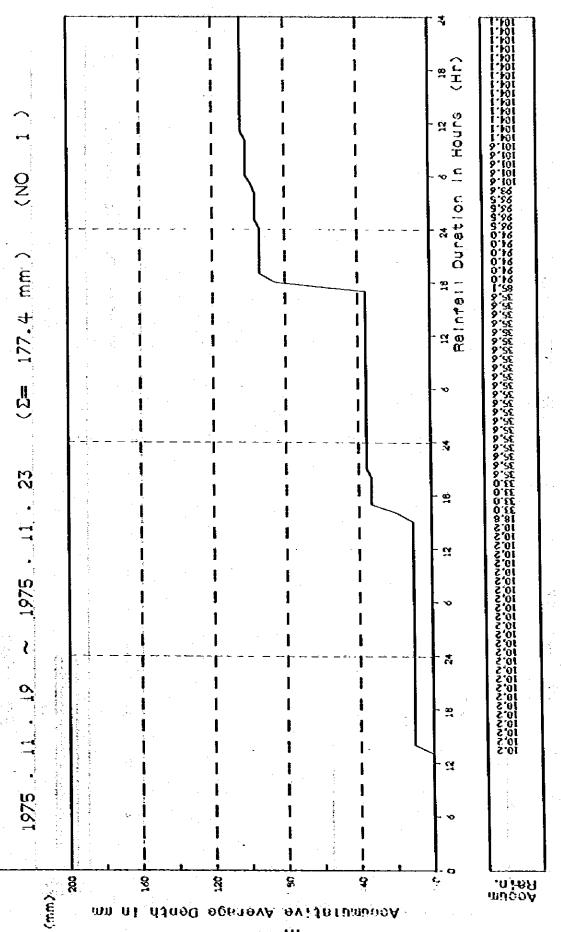
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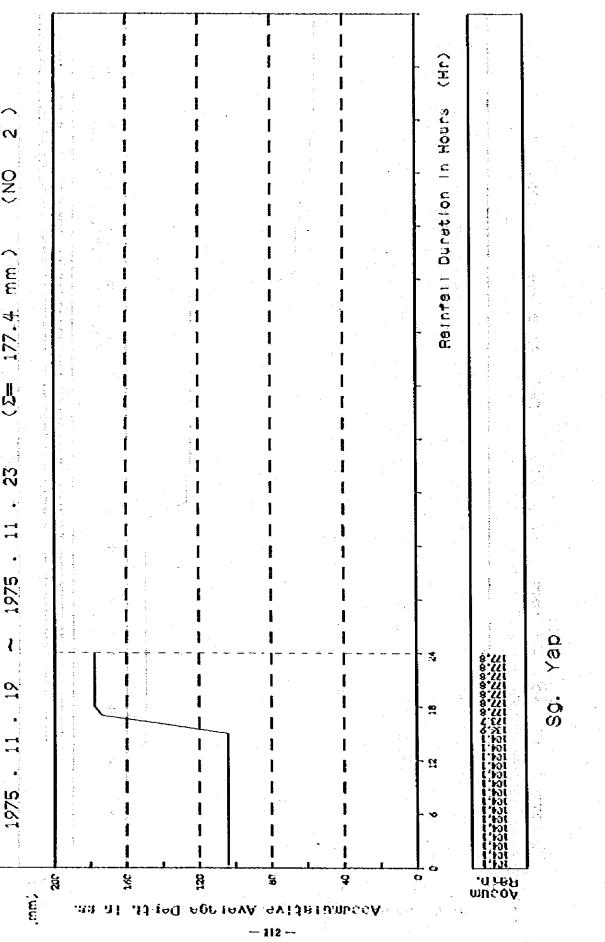


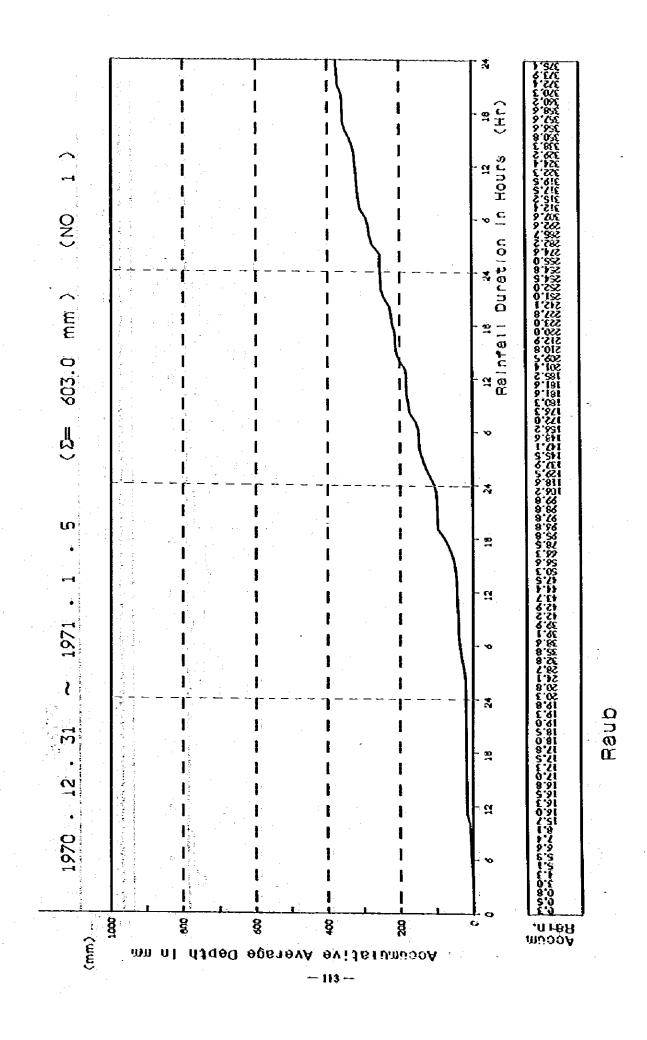
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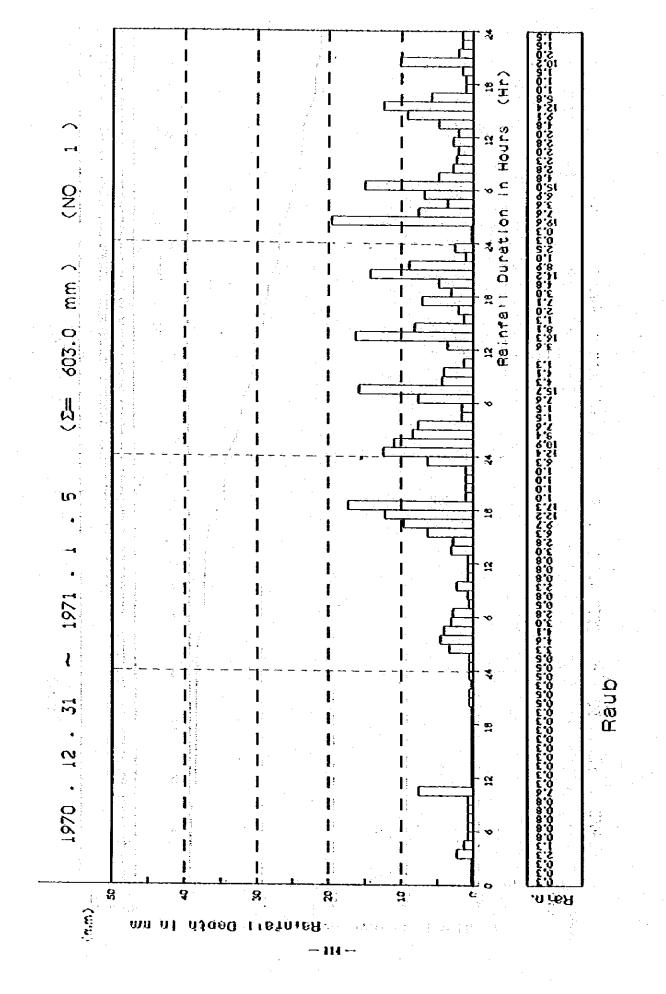
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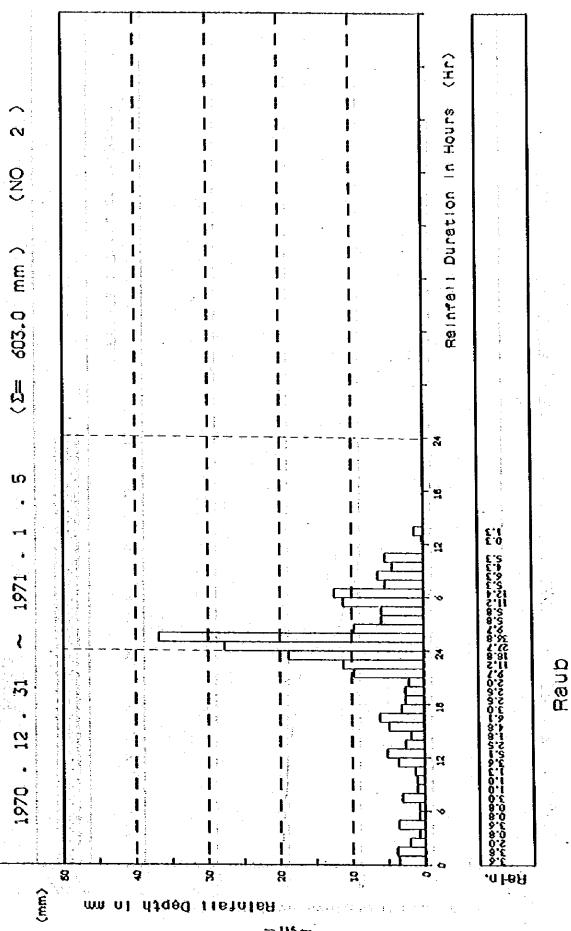
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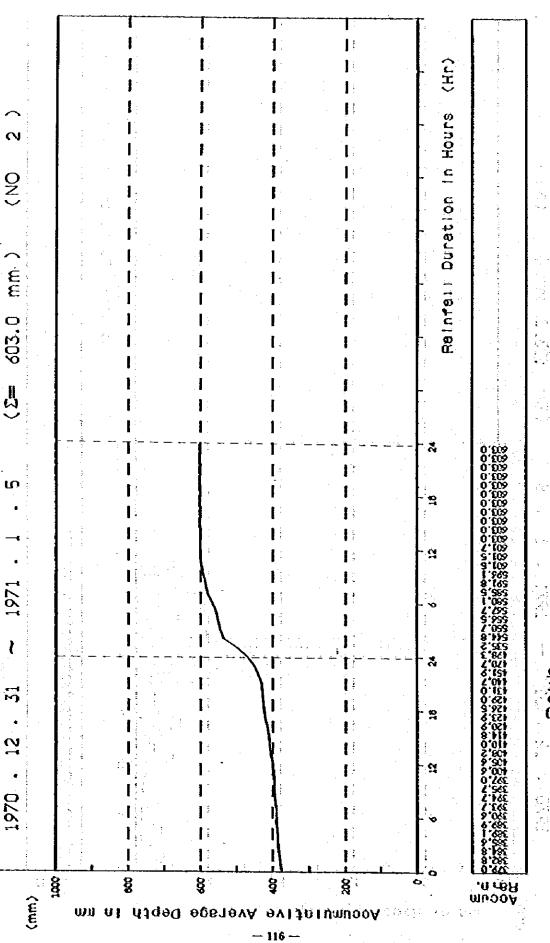






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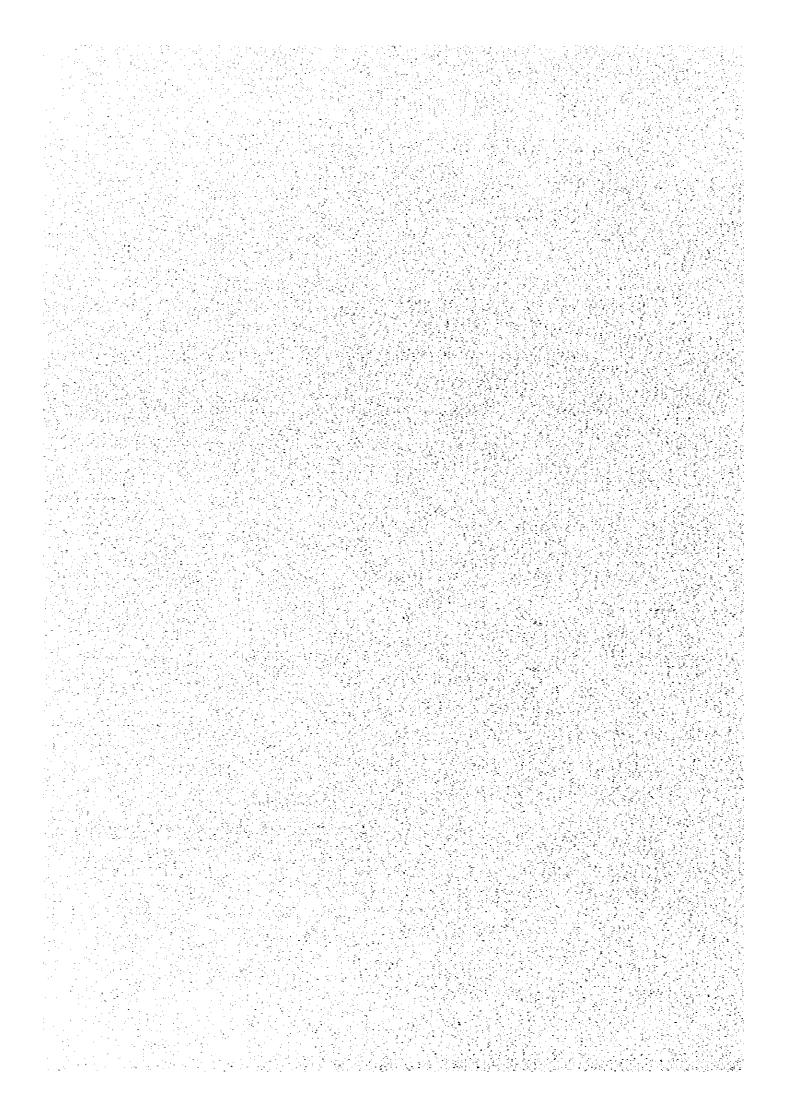
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Appendix F

Estimation of Plotting - position, Iwai Method and Curve Fitting.



Estimation of return period by Hazen plotting

Plotting of data and return period

When a probability paper is chosen for use, the plotting of data on the paper requires the knowledge of plotting positions. Numerous methods have been proposed for the determination of the plotting position, for example, Mazen method, Weibull (or Thomas) method, Gringorton method and Chegodayev method.

Many kinds of probability graph papers are used in order to plot the data, such as normal curve paper, log normal curve paper, extream curve paper.

If observed data are plotted by these method in log normal paper, the probability can be estimated from the plotting. When variable N hydrological data are obtained such as annual maximum rainfall or flood, they are arranged in order of magnitude.

The maximum data is named as x_1 , the second is x_2 , and the i-th is x_1 in general.

If there are same value data among them, each order must be given to each data, as the total number sums up N. Taking x_1 , x_2 ... x_N on the axis of abscissa, the rectangles which area is 1/N respectively can be drawn with the centers of x_1 , x_2 , ... x_N on the axis of abscissa, as shown fig.-1. If the area of all rectangles is sumed

up, it becomes as unit area 1.0.

A probability density curve is obtained by smothing each rectangle. The probability on specimen beyond x_1 is $W_1=1/(2N)$, it is $W_2=3/(2N)$ beyond x_2 , and it is $W_1=(2j-1)/(2N)$ beyond x_1 , in general.

The probability W_i of being equaled or exceeded in i-th data is given as the total area of right side rectangles from x_i , as shown in fig.-1 Therefore Hazen plot is obtained.

$$W_{i} = \frac{i-1}{N} + \frac{1}{2N} = \frac{2i+1}{2N}$$

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fig.-1

Where,
Wi : probability of being equalled or exceeded in 1-th data
1 : the rank of the event in order of magnitude (The largest event
has 1=1)

N . The number of recorded years.

In stead of probability W_i , return period T_i , is frequently used to define the design rainfall or the design flood.

i i i

Return period and probability are resiprocals.

 $T_i = 1/H_i$

The probability of non-exceedance is one minus the probability of exceedance.

 $F_i = 1 - W_i$

There are other various formulas for plotting positions shown as follows.

Weibull (or Thomas) $W_i = i/(N+1)$

California $W_i = i/N$ Gringorten $W_i = (i-0, 44)/(N+0, 12)$

Chengodayev $W_{1} = (1-0.3)/(N+0.4)$

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Iwai method

This method is derived from lognormal distribution by applying the experimental distribution.

$$F(x) = \frac{1}{\sqrt{\pi}} \int_{-\infty}^{\xi} e^{-\xi^{2}} d\xi$$

$$\xi = a \log 10 \frac{x+b}{x_{0}+b} , (-b \le x \le 0)$$
• Foundamental equation;

$$\log_{10}(x+b) = \log_{10}(x_{0}+b) + \frac{1}{a}\xi$$
where, \dot{a} , b , x_{0} : constants
• Estimation of the constants;
• $b = \frac{1}{m} \frac{N}{j=1} b_{s}$, $(m = \frac{N}{10})$

$$b_{s} - \frac{xyx_{s} + xq^{2}}{2x - (xy+x_{s})}$$
, $(t + s = N + 1)$

$$\log_{10}x_{g} = \frac{1}{N} \frac{N}{i=1} \log_{10}x_{i}$$
• $\log_{10}(x_{0} + b) = \frac{1}{N} \frac{\sum_{i=1}^{N} \log_{10}(x_{i} + b)}{i=1} = \frac{1}{Y}$
+ $\frac{1}{a} = \sqrt{\frac{2}{N-1}} \frac{\sum_{i=1}^{N} (\log_{10}\frac{x_{i}+b}{x_{0}+b})^{2}} = \sqrt{\frac{2N}{N-1}} \cdot S_{x}$

$$S_{X} = \sqrt{\frac{1}{N}} \frac{\sum_{i=1}^{N} \{\log_{10}(x_{i} + b)\}^{2} - \{\log_{10}(x_{0} + b)\}^{2}} = \sqrt{\frac{1}{Y^{2}} - (\frac{1}{Y})^{2}}$$

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Relation between N and $\boldsymbol{\xi}$

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Curve Fitting

After the hydrologic data are plotted on a probability paper, a curve may be fitted to the plotted points. The curve is a straight line if linearization of the distribution is attempted. The straight line can be essentially represented by Eq. (1). Curve fitting may be done either mathematically or graphically. In general, a mathematical curve fitting can be achieved by three methods: the method of moments, the method of léast squares, and the method of likelihood. Of course, the mathematical fitting does not necessarily require data plotting on a probability paper. By graphical fitting, a straight line is simply drawn to fit the plotted data by eye-fit, and this method is the simplest but involves human error.

 $\mathbf{x} = \mathbf{x} + \sigma \mathbf{k} \quad \dots \quad (1)$

- x : variaty
- x: the mean
- σ : standard deviation
- k : frequency factor

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Method of Moments

By this method, the statistical parameters or moments are computed from the data and then substituted in the probability function of the given distribution. This method gives a theoretically exact fitting but the accuracy can be substantially affected by any errors involved in the data at the tails of the distribution where the moment arms are long and the errors are thus magnified. The method originally proposed by Gumbel to fit Type I extremal distirubtion is a method of moments. Liebelin modified this method by order statistics and developed a procedure which maintains the original time order of the extreme-value series, divides the values into subgroups, and then weighs each observation according to its ordered rank in the subgroup which in turn is a function of the sample size. Hershfield made a comparison of the two procedures and concluded that the Gumbel procedure gives a better estimate beyond the range of data for the really independent data tests, but overestimates the longer recurrence-intervals in the dependent data tests.

Method of Least Squares

By this method, a regression line is computed to fit the plotted data. The curve so obtained may not represent the exact theoretical distribution but it gives a better overall fit than the method of moments. For extremal distributions, Gumbel introduced a modified least-squares method by minimizing both vertical and horizontal deviations and taking the geometric mean of the parameters obtained from the two minimizations. Based on the general equation for hydrologic frequency analysis, Eq. (1), proposed by Chow, a leastsquares procedure for fitting a normal, lognormal, or extremal distribution was developed by Brakensiek.

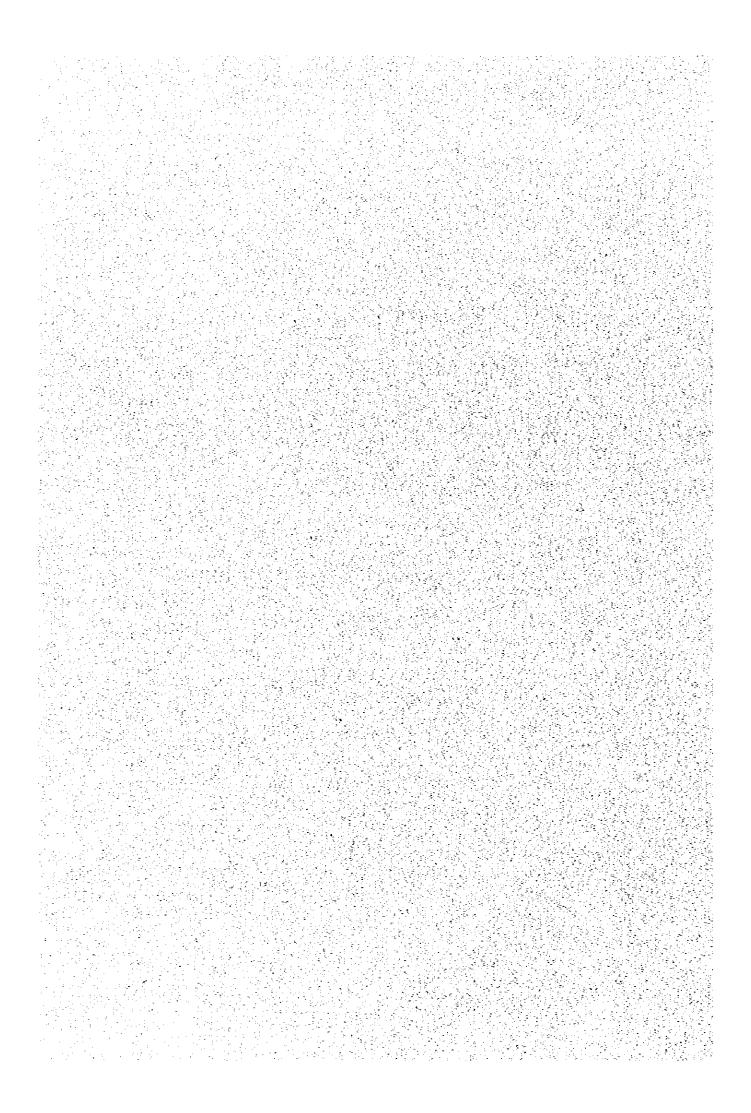
Method of Maximum Likelihood

By this method, the value of a parameter is determined to make the probability of obtaining the observed outcome as high as possible. Mathematically, $\partial \log p(x)/\partial u = 0$, where p(x) is probability density and u is a statistical parameter. This method provides the best estimate of the parameters but it is usually very complicated for practical application. Kimball has suggested this method for fitting extremal distributions, and a practical procedure was later developed by Panchang and Aggarwal.

Appendix G

Tank Model

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TANK MODEL METHOD

Explanation

The tank model is intended for calculation of a run-off with the catchment area of a river substituted by a combination of a number of storage type model vessels (or called tanks in the following). It was proposed by Dr. Masami Sugawara. For example, let's consider a model of four tanks arranged in series as illustrated in Fig. 2.1. The outlets on the right hand side of the respective tanks represent run-offs and that at the bottom represents an infiltration.

A precipitation at a given time R(t) is added to the uppermost tank V_1 . The water reserved in the tank V_1 runs off through the outlets on the right hand side or infiltrates through the outlet at the bottom into the tank V_2 in the second stage. The storage water in the tank V_2 supplied from the tank V_1 then runs-off through the outlets on the right hand side or infiltrates through the outlet at the bottom into the tank V_3 in the third stage. The process is repeated to the last tank.

The model may be readily understood when it is considered in reference to the mechanism of run-off in a basin shown schematically in Fig. 2.2.

Rain wets the soil layer on the surface of the ground. When the surface layer contains water more or less, the rain water flows over the ground usrface. In the model of Fig. 2.1, the outlet provided slightly above the bottom on the right hand side of the tank V_1 corresponds to such run-off.

When the rainfall continues so that the surface layer contains water sufficiently, the surface run-off increases. This corresponds to the upper coutlet on the right hand side of the tank V_1 .

The water contained in the surface layer continues to infiltrate into the lower side, and this is represented by the outlet provided at the bottom of the tank V_1 .

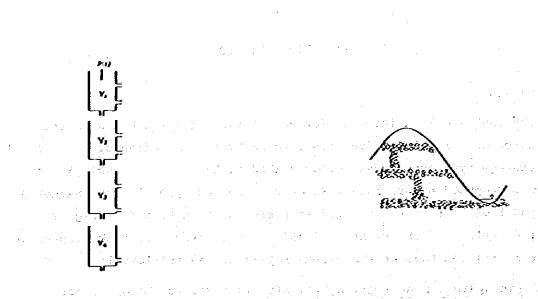


Fig. 1 Tank model in serial four stages

Schematic representation of the mechanism of run-off in a catchment area

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The water infiltrating from the surface layer stays in a first aquifer. When such water accumulates in excess of a certain limit, it begins to runs-off from the aquifer. Water seeping out of a mountain-side is an example. This corresponds to the run-off from the tank V_2 .

Fig. 2

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The water infiltrating further from the first aquifer to the lower side stays in a second aquifer and presents a similar behavior to that in the first aquifer. Water seeping out of a mountain foot is a typical example. This is represented by the run-off from the tank V3.

Water infiltrating further below is stabilized as underground water and runs off gently at the time of a low or droughty water level of river. This is represented by the run-off from the tank V_{i} .

The total of the run-offs from the outlets on the right hand side of the respective tanks is given as a value of calculation for the runoff of the catchment area.

Thus, seeing the tanks in the model against the run-off components, the tank Vi in the uppermost stage corresponds approximately to the

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surface run-off, the tank V_2 in the second stage to the intermediate run-off, and the tanks V_3 and V_4 in the third stage and after to the base flow discharge.

In the tank model, three to four tanks are arranged in series generally, as illustrated in the foregoing. However, various arrangements of tanks can be considered according to the characteristics of the basin.

Appendix H

Storage Function Method

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STORAGE FUNCTION METHOD

I. Outline of storage function method

The storage function method is a run-off calculation to obtain flood discharge from the rainfall which has fallen into the river basin or the inflow discharge to a river channel. This method was proposed by Dr. T. Kimura and it has been widely applied in Japan for flood routing.

As the flood run-off is non-linear feature it has an advantage to express a storage characteristic between rainfall and run-off like tank model method. And it can show the actual feature of flood runoff flow which changes slowly as unsteady flow, if its coefficients are determined properly by water level records.

1. Basic equation

If flood run-off is assumed by Manning's formula, the storage amount (S_1) of a river basin of a river channel is expressed as an exponential function of run-off discharge (G_L) .

 $S_1 = K \cdot Q_1^P$

where, K. P : Constants for a basin or a channel

This equation of motion is combined with the following continuous equation for a river basin or a river channel.

(1) for river basin

$$\frac{1}{3.6} \quad f \cdot R_{ave} \cdot A - Q_1 = \frac{d}{dt} S_1$$

where, f : Inflow coefficient

Rave: Average rainfall in the basin (mm/hr)

A : Catchment area at the calculated point (km²)

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T₁ : Lag time (hr)

 $Q_1(t) = Q(t+T_1)$ (m³/sec)

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Run-off discharge subtracting a certain base flow from the river basin after lag time considered S1 : Apparent storage amount in the basin

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(2) for river channel

 $\sum_{j=1}^{n} f_j I_j - Q_1 = \frac{d}{dt} S_1$

where, I_j : Inflow discharge into the channel from the basin, tributaries and/or upper boundary of the channel (m³) f_j : Inflow coefficient T_1 : Lag time (hr)

 $Q_1(t) = Q(t+T_1)$ (m³/sec)

Discharge at lower boundary of channel after lag time

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S1 : Apparent storage in channel

2. Division and layout of river basins and river channels

A river basin is divided into smaller basins when its catchment area is extremely large or when the discharge from such smaller basin as tributary, upstream of water level gauging station or upstream of dam is needed.

The application range of storage function is $10 \ 1,000 \ \text{km}^2$ in basin area, however, a basin is usually divided into smaller basin than $500 \ \text{km}^2$. For the storage function in river channel, $10 \ 100 \ \text{km}$ is preferable in the length of river channel.

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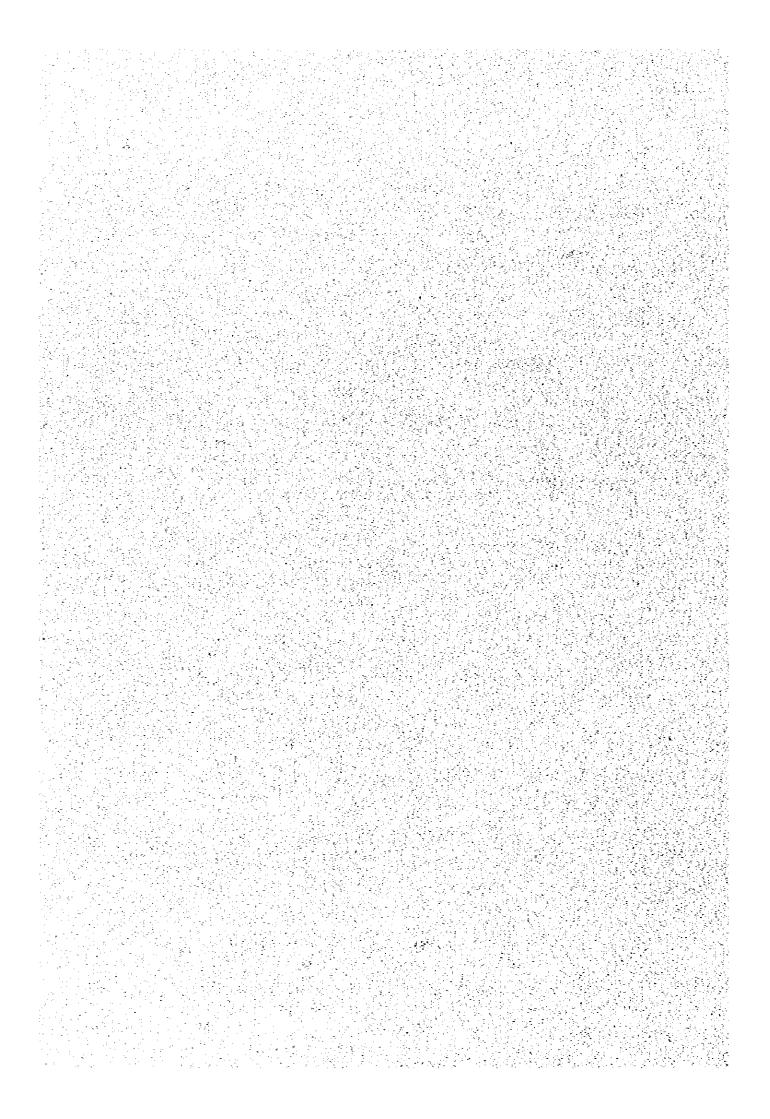
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Appendix I

Monthly rainfall data used in analysis Daily rainfall data at Kg. Merting Recorded hydrograph in 1972 Flood

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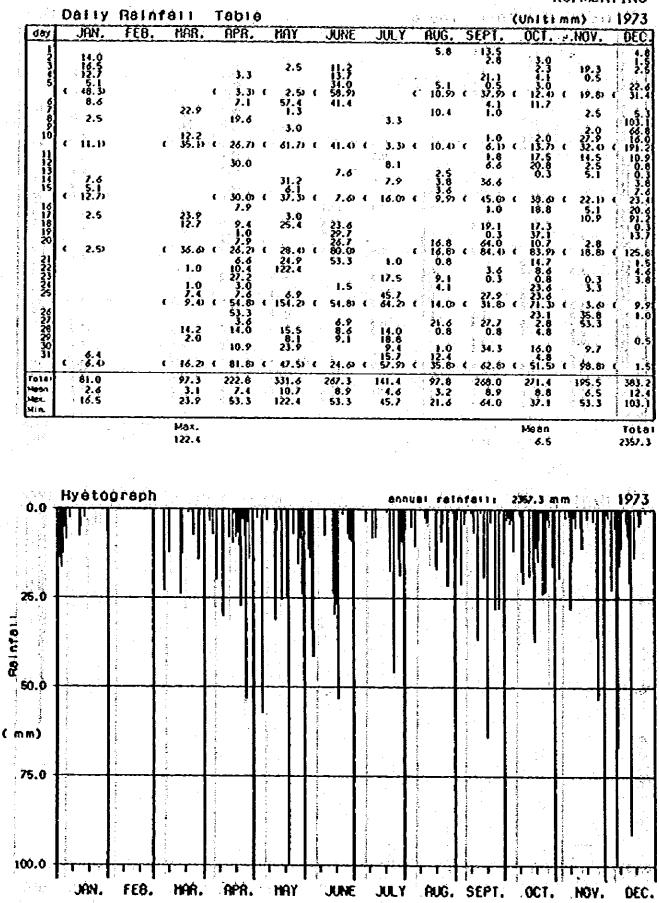
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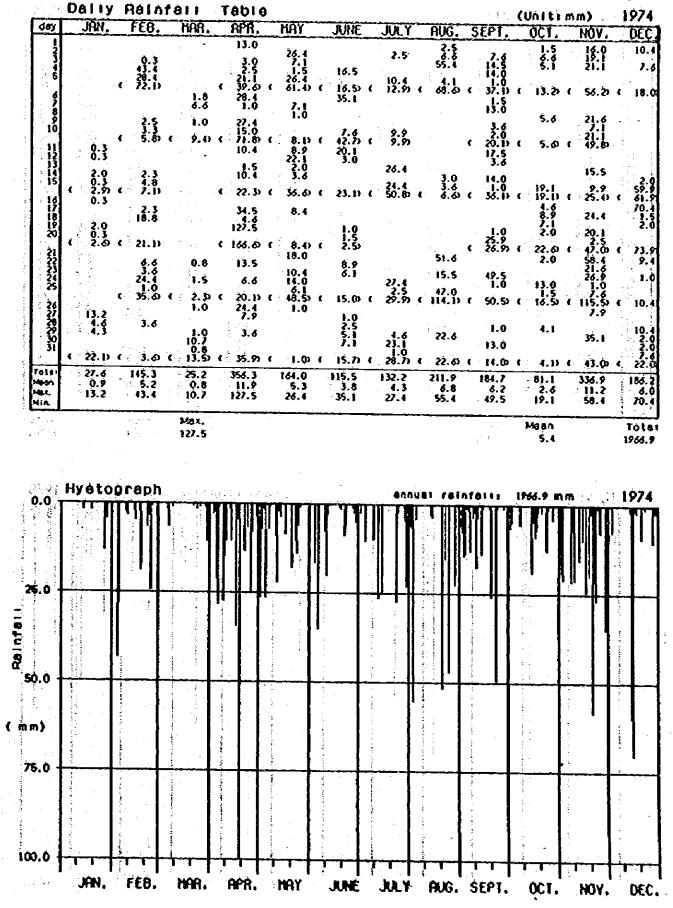
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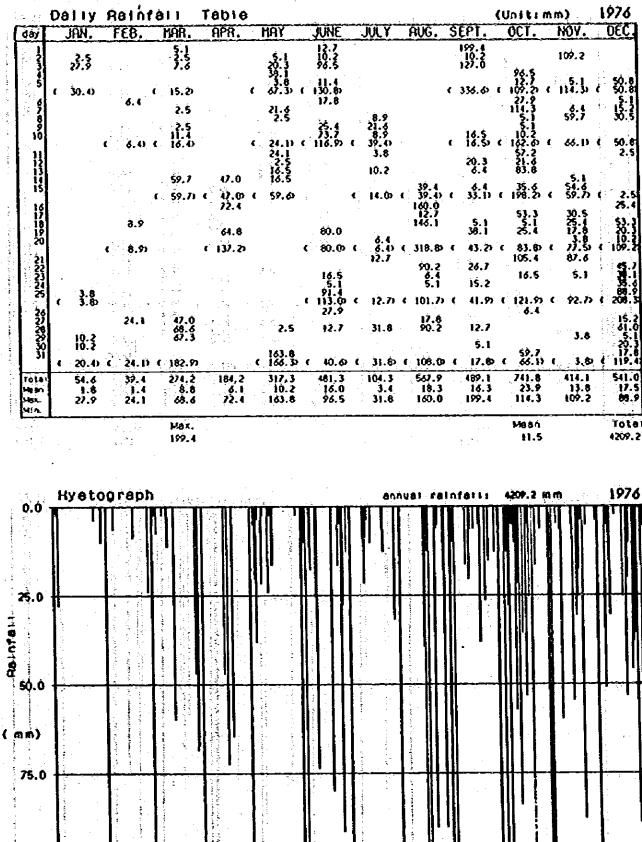
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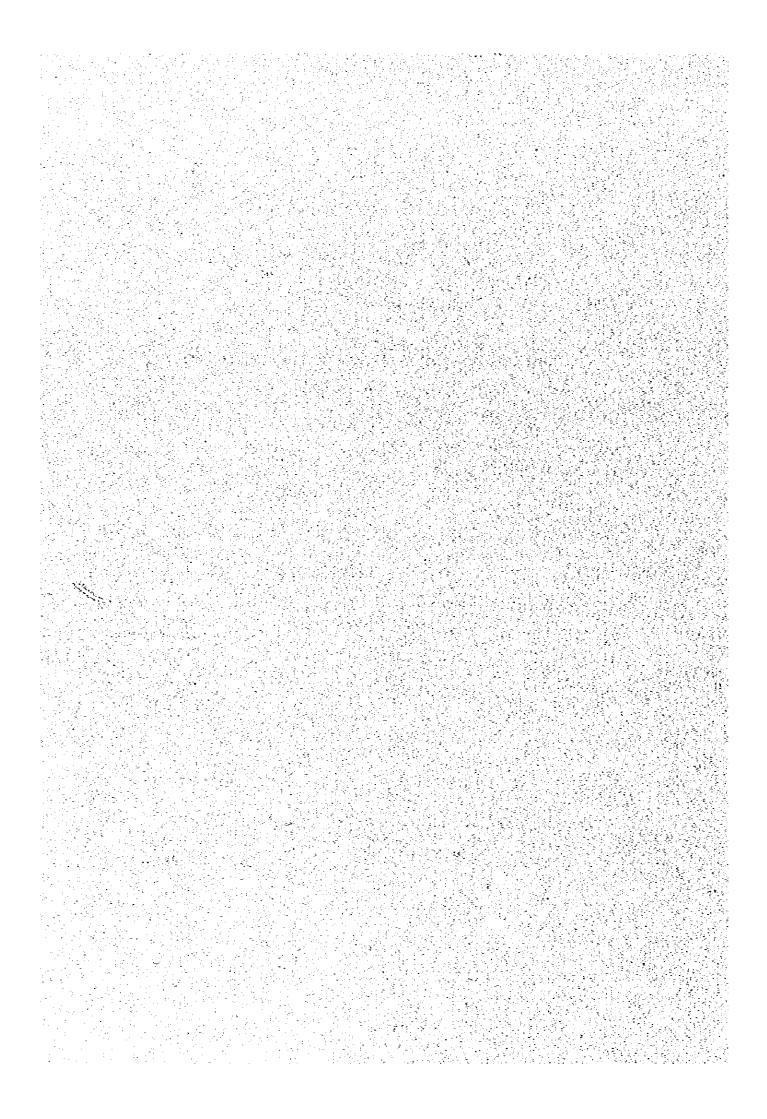
Appendix J

Adjusted Rainfall for the Daily Runoff

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