Table 7.2 Flow Regime at Penut

				· · · · · · · · · · · · · · · · · · ·		e <del>s</del> a in the fi	(c.A	i 1380 km²
YEAR	QHAX	95 DAY	185 DAY	275 DAY	355 DAY	ОМІН	AVÉRG.	TOTAL
1961	360.38	61.60	45.20	26.69	13.37	10.86	48.99	17882.48
1962	187.07	52.35	28.94	21.37	12.56	11.22	37.52	13694.87
1963	130.12	44.60	18.10	11.45	7.67	7.14	29.92	10921.49
1964	200.39	49.21	34.45	25.05	13.43	11.41	39.59	14490.51
1965	239.48	57.91	28.26	13.29	9.30	8.99	40.18	14665.69
1966	171.71	53.89	40.62	29.39	15.48	11.68	42.80	15623.72
1967	420.73	79.14	48.16	26.17	12.64	12.02	59.77	21816.42
1968	231.43	37.26	26.26	17.99	12.40	11.00	29.69	10866.06
1969	146.32	56.77	36.78	17.57	9.56	8.88	39.99	14594.98
1970	294.91	55.62	33.59	23.87	11.12	10.27	43.40	15841.59
1971	681.31	55.34	35 . 39	23.11	13.49	12.32	58.39	21312.20
1972	1118.10	36.82	23.29	12.76	7.75	6.68	42.50	15556.09
1973	971.35	39.73	21.60	14.74	8.96	7.79	43.53	15889.25
1974	203.22	46.30	32.19	24.30	15.60	12.03	39.67	
1975	1401.46	61.25	34.33	24.34	15.36	13.33	58.68	14481.14 21418.4
1976	220.34	32.00	19.98	12.87	8.14	7.44	27.48	10057.08
1977	155.92	25.86	16.28	8.87	5.81	5.12	22.56	8233.68
i 1978	315.39	28.38	19.54	14.07	10.24	8.50		
1979	632.95	37.34	22.66	14.35	9.98		28.68	10469.25
1980	206.49	37.04	22.05	16.08	12.43	7.62	37.83	13806.96
AVERG.	414.48	47.47	29.39	19.19	11.26	9.47	30.22 40.07	11059.51
					1	7.03		14634.07
A ARREST							<b>(</b> U	nit! m³/s

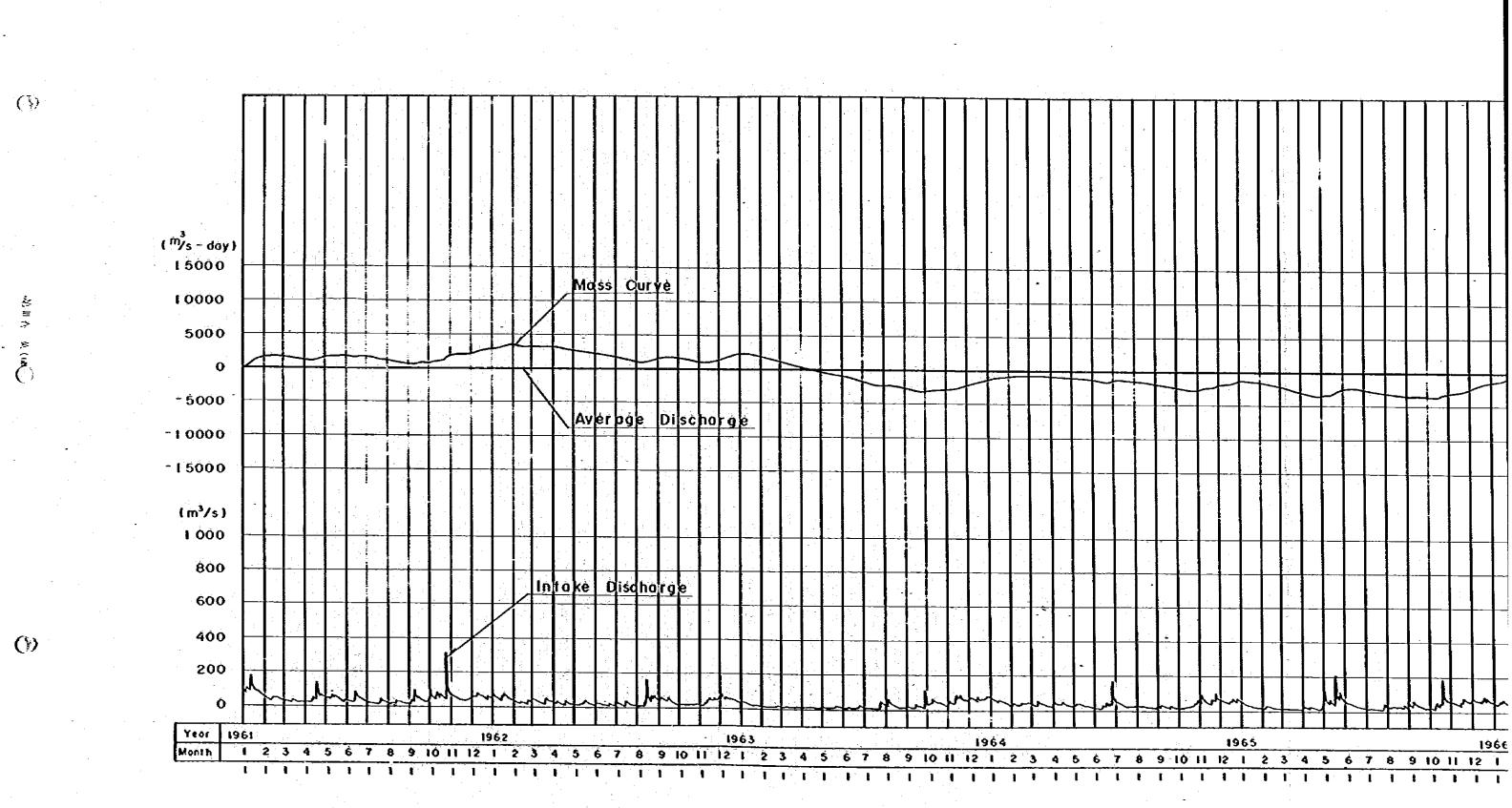
Table 7.3 (1) Flow Regime at the Lover Damsite

 $(C.A = 1380 \text{ km}^2)$ 

YEAR	∈ QXAX	95 DAY	185 DAY	275 DAY	355 DAY	иткр	AVERG.	TOTAL
1961	360.98	61.60	45.26	26.69	13:37	10.86	48.99	17882.48
1962	187.07	52.36	28.94	21.37	12.56	11.22	37.52	13694.87
1963	130.12	44,60	18.10	11.45	7.67	157.14	29.92	10921.49
1964	200.39	49.21	34.45	25.05	13,43	11.41	39.59	14490.51
1965	239.48	57.91	28.26	18.29	9,30	8.99	40.18	14665.69
1966	171.71	53.89	40.62	29.39	15.48	11168	₹42.80	15623.72
1967	420.73	79.14	48.16	26.17	12.64	12.02	59.77	21816.42
1968	231.43	37.26	26.26	17.99	12.40	11.00	29,69	10866.06
1969	146.32	56.77	36.78	17.57	9.56	128.88	39.99	14594.96
1970	294.91	55.62	33.59	23.87	11.12	10.27	43.40	15841.55
1971	681.31	55.34	35.39	23.11	13.49	12.32	58.39	21312.28
1972	1118.10	36.82	23.29	12,76	7.75	6.68	42,50	15556.09
1973	971.35	39.73	21.60	14.74	8.96	7.78	43.53	15889.25
1974	203.22	46.30	32.19	24.80	15.60	12.03	39.67	14481.14
1975	1401.46	61.25	34.33	24.34	15,36	13.33	58.48	21418.43
1976	220.34	32.00	19.98	12.37	8.14	7.44	27.38	10057.08
1977	155.92	26.86	16.28	8.87	5.81	5.12	22.56	8233.68
1978	315.39	28.38	19.54	14.07	10.24	9.50	28.68	10469.25
1979	632.95	37.34	22.66	14.35	9.98	7.62	37,83	13806.90
1980	206.49	37.04	22.05	16.03	12.43	9.47	30.22	11059.51
AVERG.	414.48	47.47	29.39	19.19	11.26		40.07	14534.07

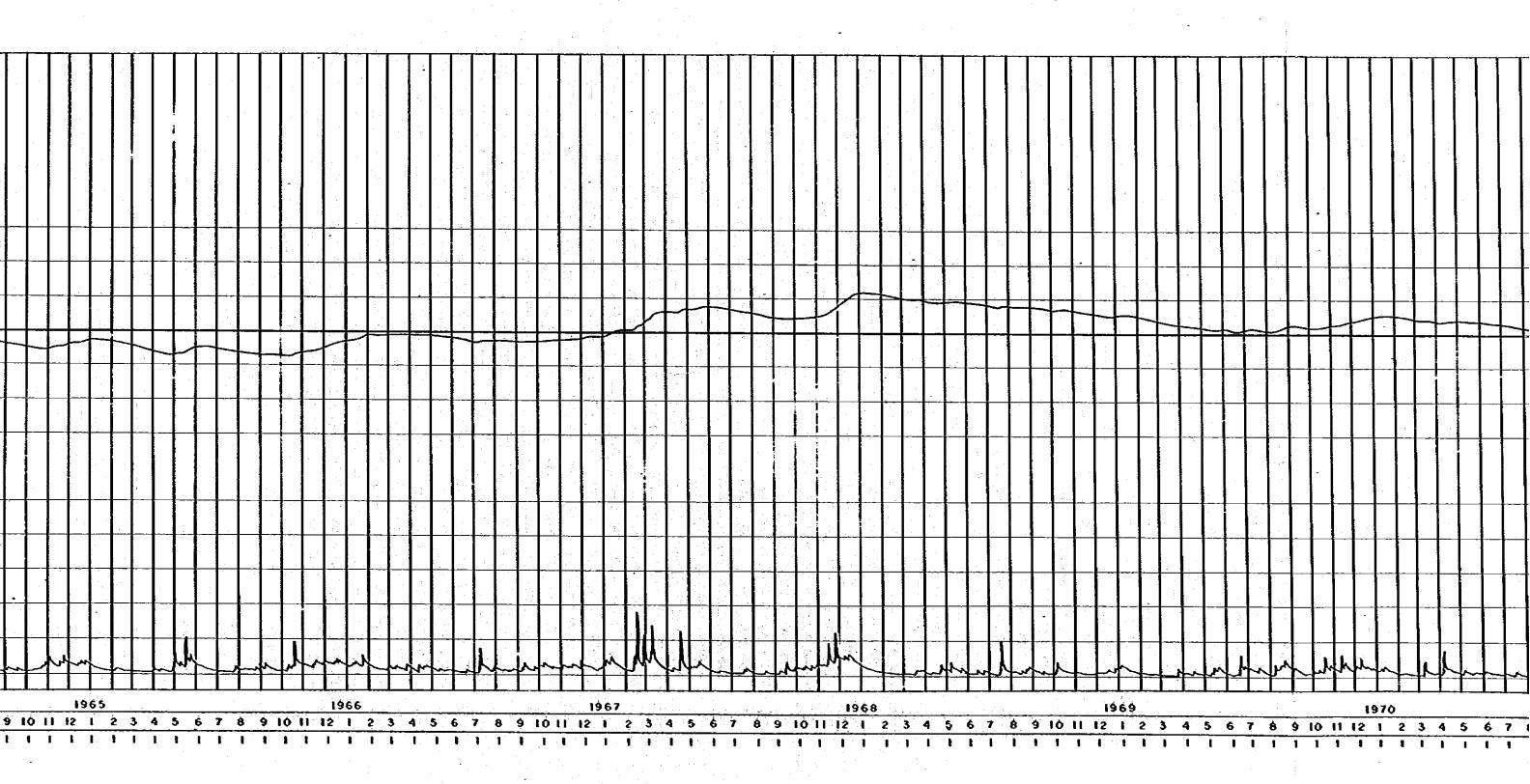
Table 7.3 (2) Plow Regime at the Upper Dansite
(C.A = 1200 km<sup>2</sup>)

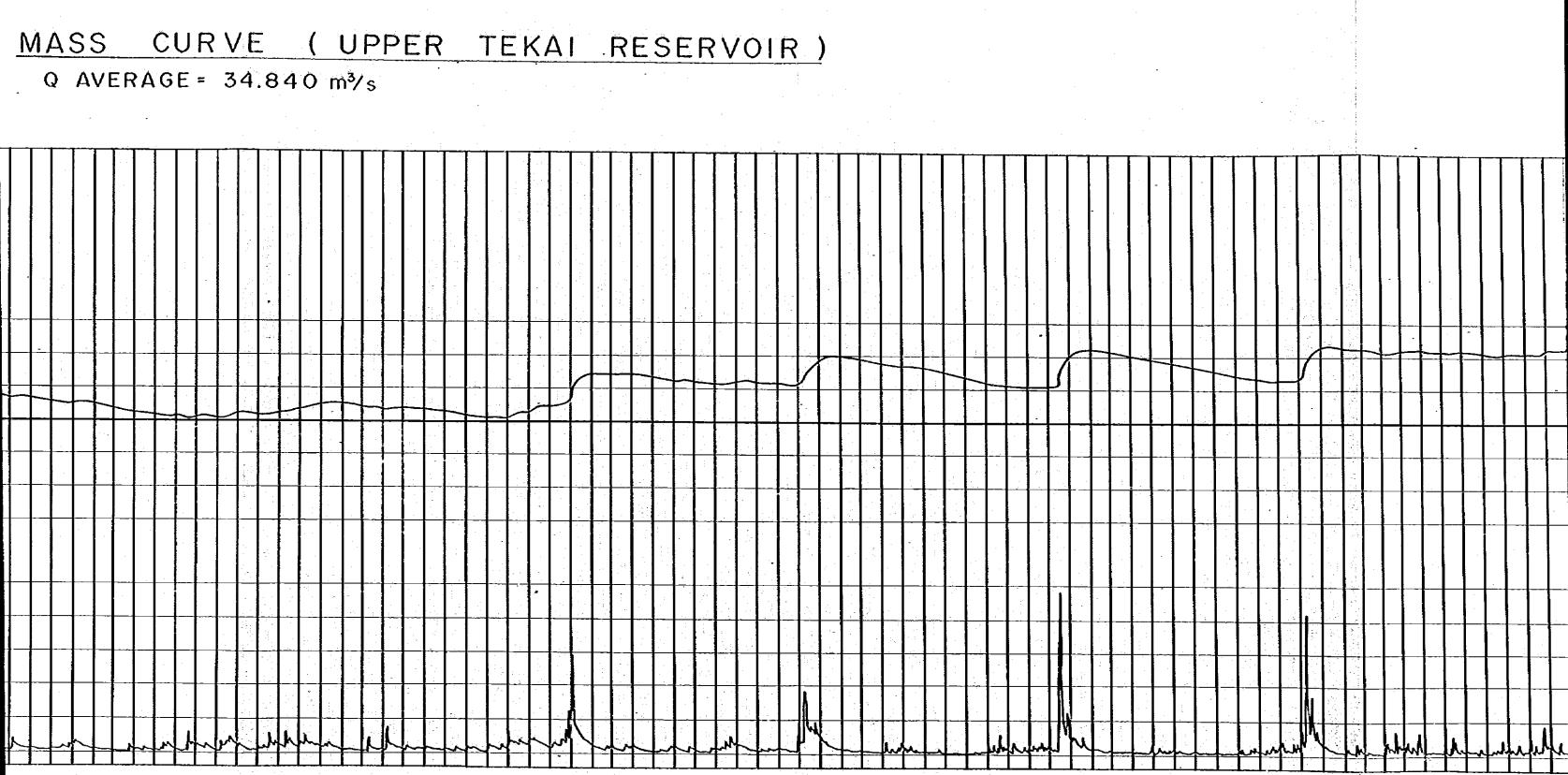
YEAR	QKAX	95 DAY	185 DAY	275 DAY	355 DAY	онти,	AVERG.	TOTAL
1961:	313.90	53.57	39.96	23.21	11,63	9.44	42,60	15549.38
1962	162.67	45.53	25.17	18.58	10.92	9.76	32.63	11908.58
1963	113.15	38.78	15.74	9.96	6.67	6.21	26.02	9496.95
1964	174.25	42.79	29.96	21.78	11.68	9.92	34.43	12600.44
1965	203,24	50.36	24.57	15.90	8.09	7.82	34.94	12752.77
1966	149.31	46.86	35.32	25.56	13.46	10.16	37.22	13585.84
1967	365.85	68.82	41.88	22.76	10.99	10.45	51.97	18970.30
1968	201.24	32.40	22.83	15.64	10.78	9.57	25.82	9448.75
1969	127.23	49.37	31.98	15.28	8.31	7.72	34.77	12691.27
1970	256.44	48.37	29.21	20.76	9.67	8.93	37.74	13775.26
1971	592.44	48.12	30.77	20.10	11.73	10.71	50.77	18532.42
1972	972,26	32.02	20.25	11.10	6.74	5.81	36.96	13527.03
1973	944.65	34.55	18.79	12.82	7.79	6.77	37.85	13816.74
1974	176.71	40.26	27.99	21.13	13.57	10.46	34.50	12592.30
1975	1218.66	53.26	29.85	21.60	13.36	11.59	51.03	19624.72
1976	191.60	27.83	17.37	11.19	7.08	6.47	23.89	8745.29
1977	135.54	23.36	14.16	7.71	5.05	4.45	19.62	7159.72
1978	274.25	24.68	16.99	12.23	8.90	7.39	24.94	9103.70
1979	550.39	32.47	19.70	12.43	8.68	6.63	32.89	12006.00
1980	179.56	32.21	19.17	13.94	10.81	8.23	26.28	9616.97
AVERG.	360.42	41.28	25.55	16.69	9.80	8.42	34.84	12725.28



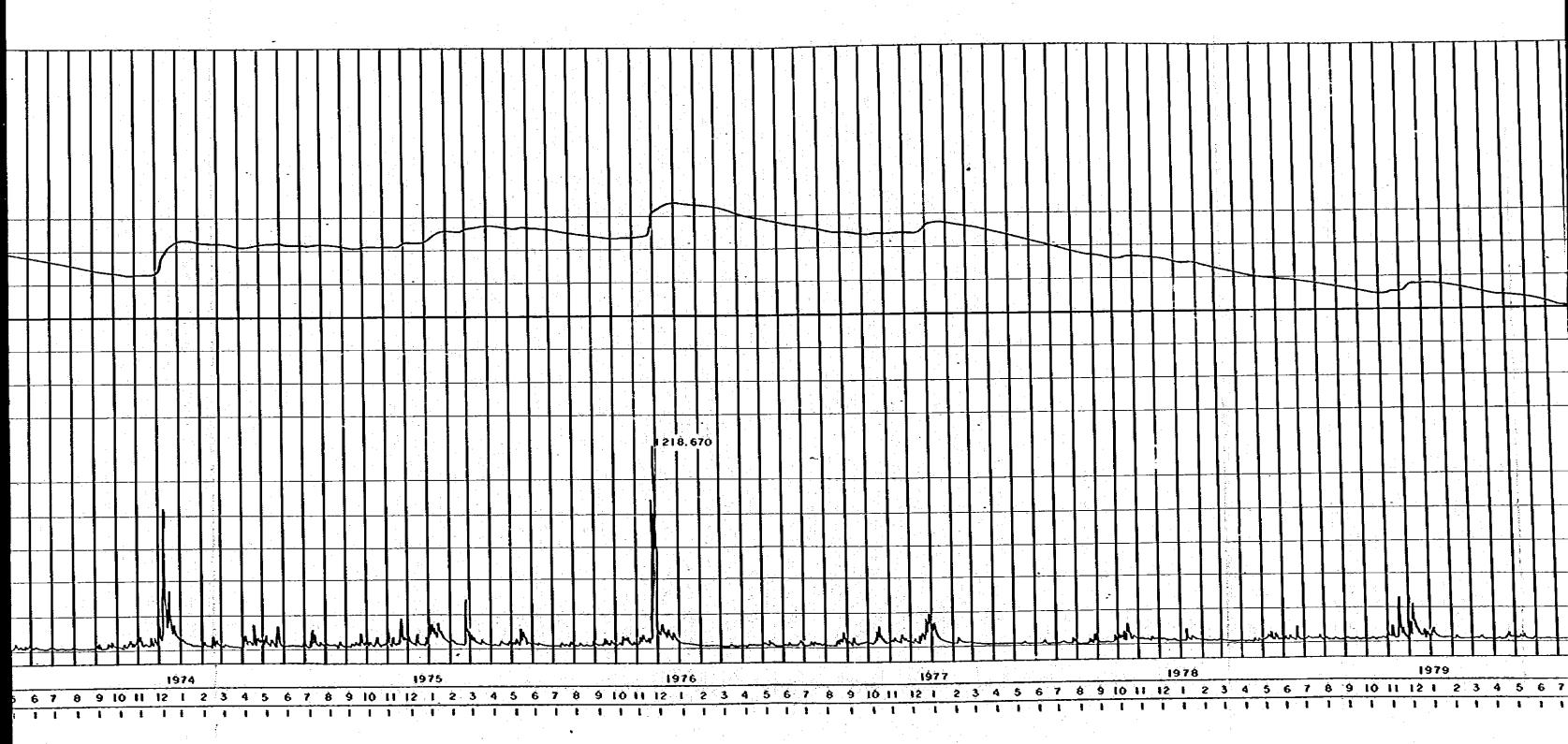
MASS CURVE ( UPPER

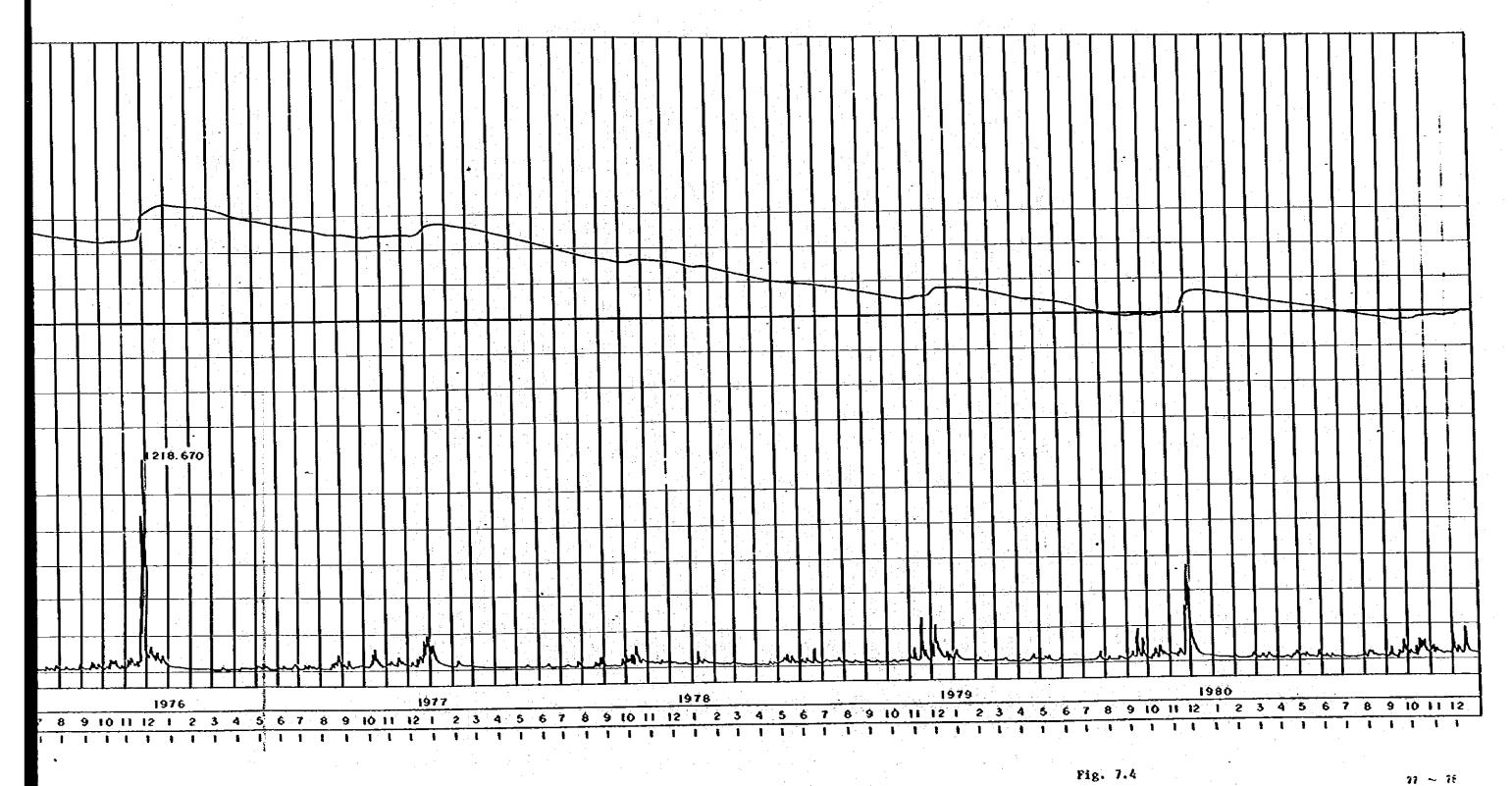
Q AVERAGE = 34.840 m<sup>3</sup>/s

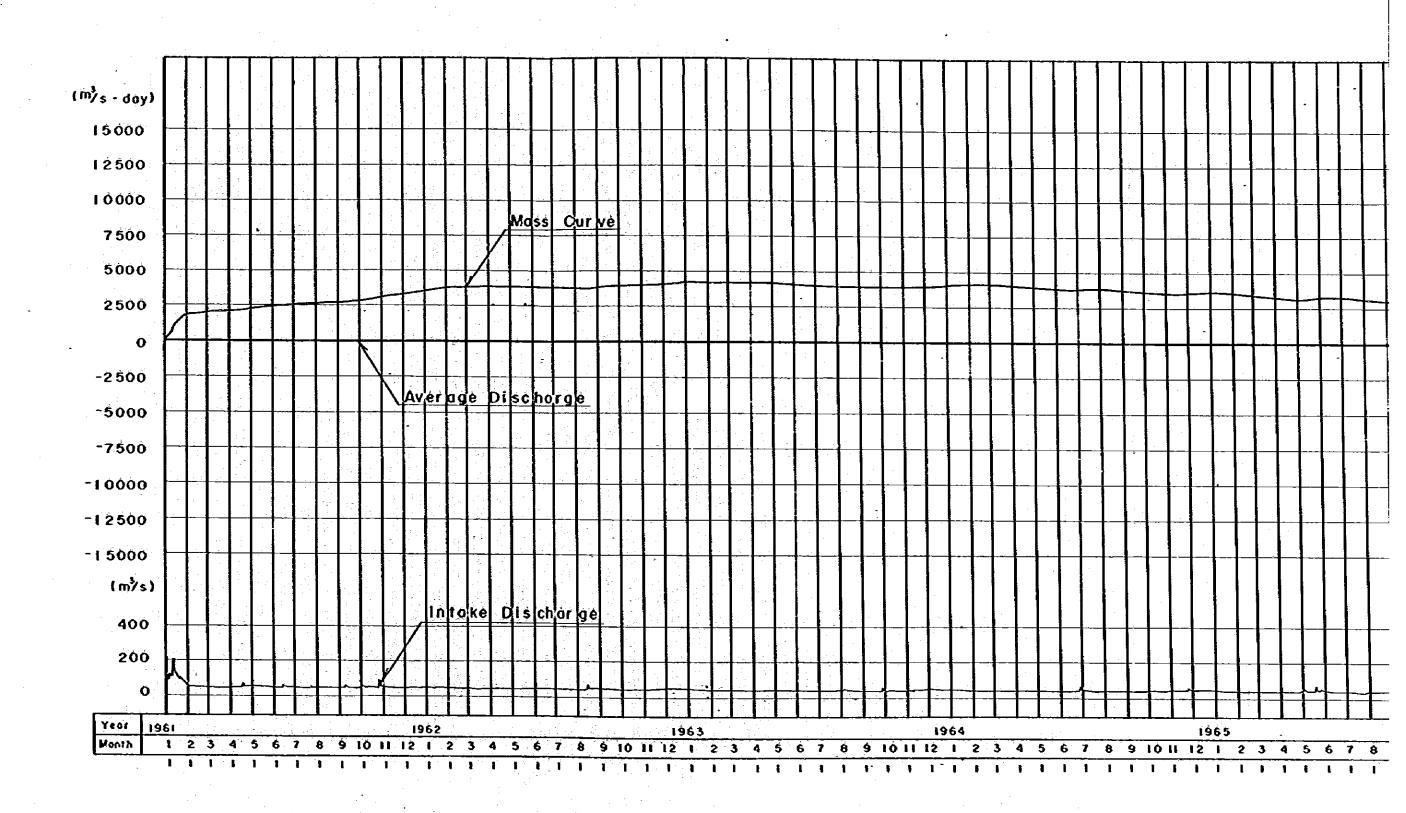




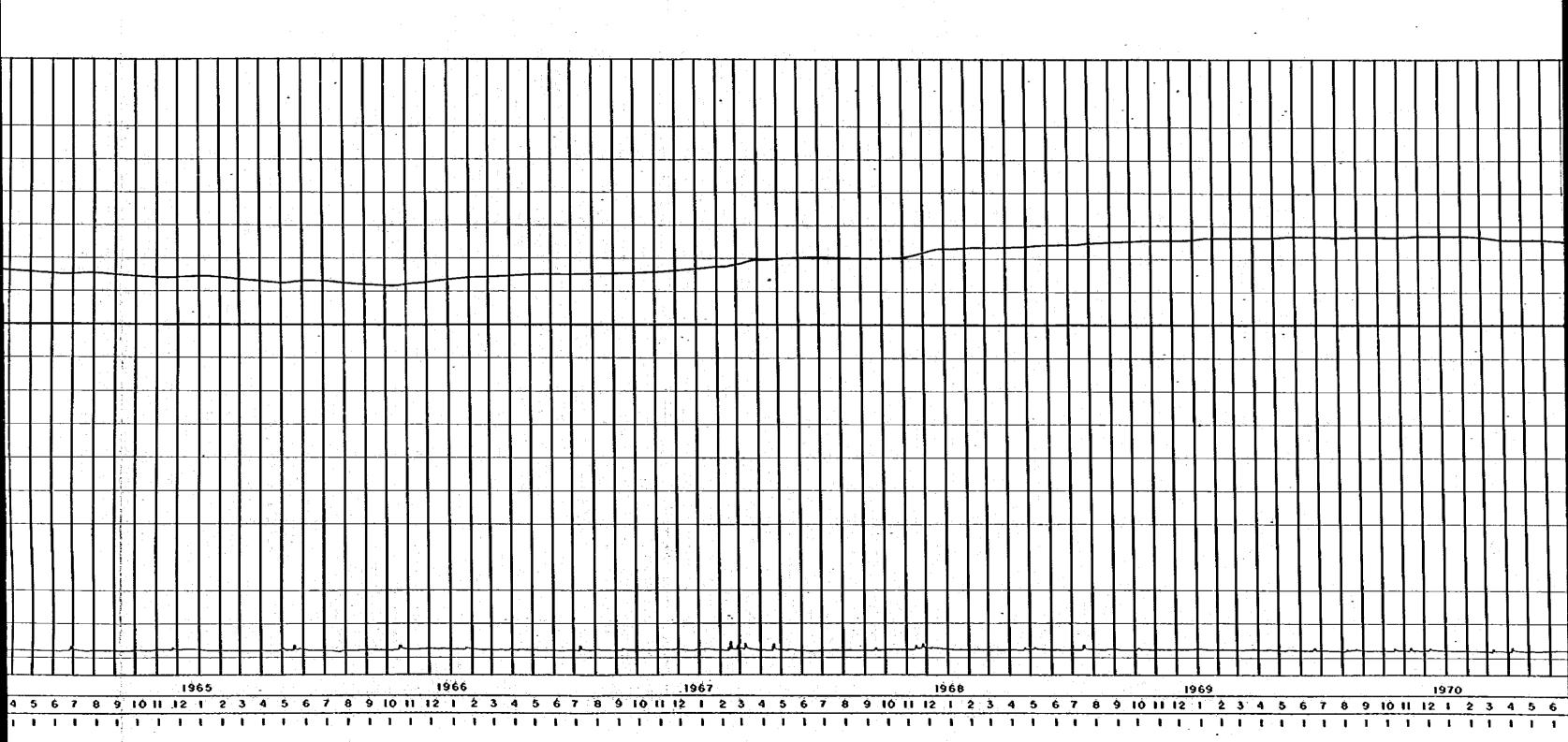
| 1969 | 1970 | 1971 | 1972 | 1973 | 1974 | 1974 | 1975 | 1974 | 1975 | 1974 | 1975 | 1975 | 1976 | 1977 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 | 1978 |





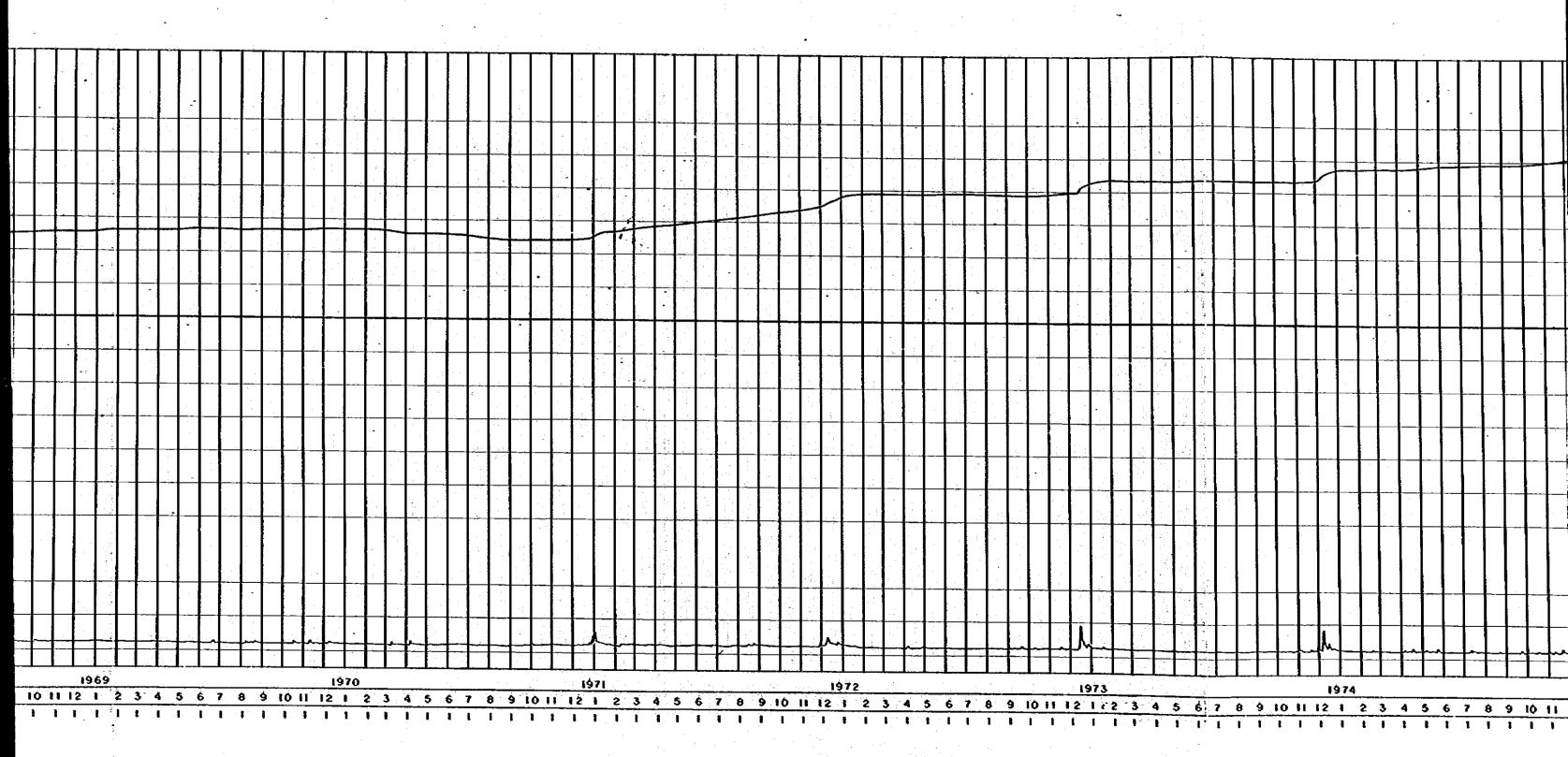


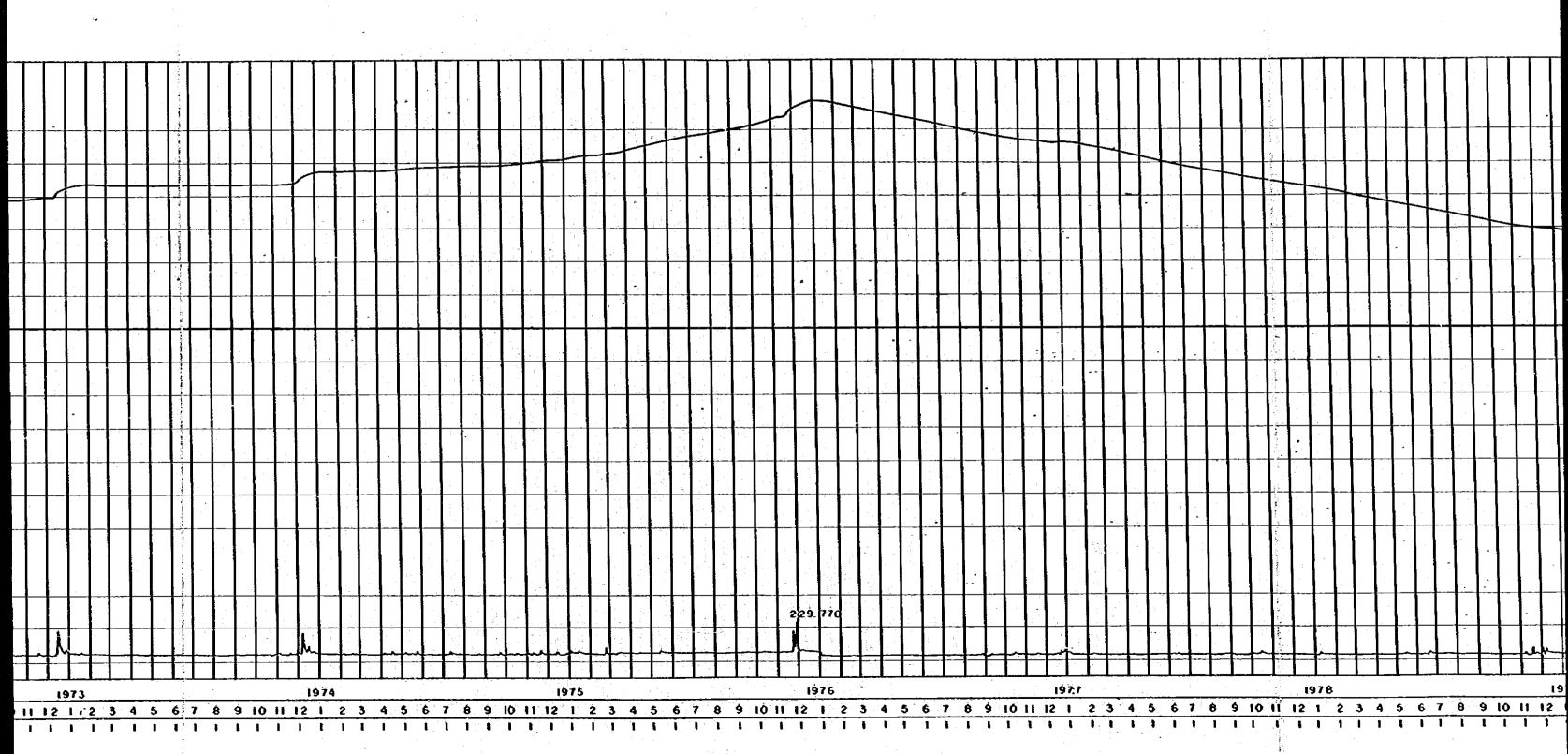
Q AVERAGE = 40.070 m3/s

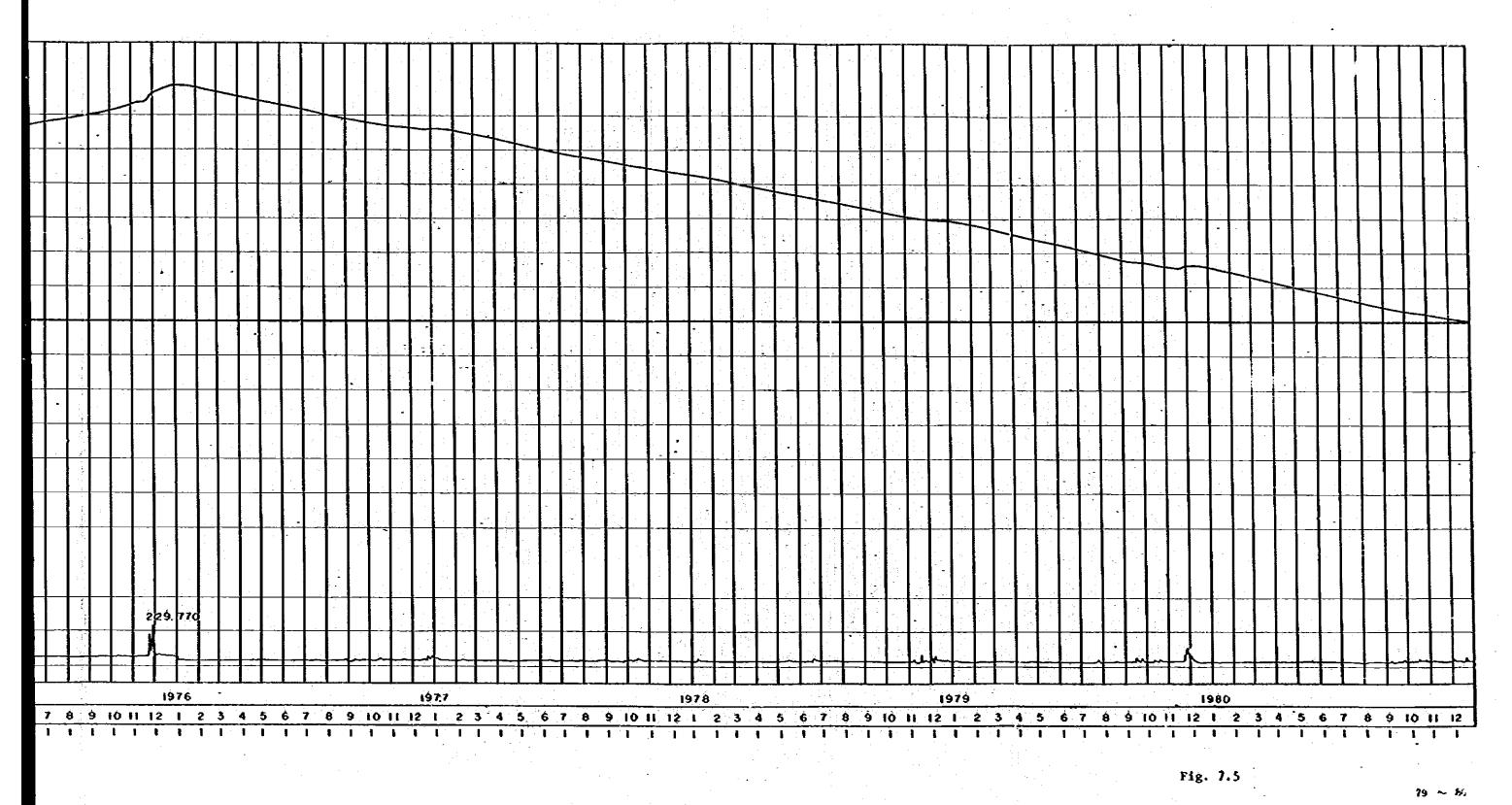


# MASS CURVE ( LOWER TEKAI RESERVOIR )

Q AVERAGE =  $40.070 \text{ m}^3/\text{s}$ 







# 8. ESTIMATION OF FLOOD HYDROGRAPH

## 8. ESTIMATION OF FLOOD HYDROGRAPH

Plood hydrograph estimations are required for design floods at the proposed dansite, to determine the flood mitigation effect of the storage. This section describes the assessment of stora rainfall, an estimation method to convert rainfall to runoff, its application to reproduce recorded floods.

## 8.1 Basic Data between the decimal with the

Ploods in S. Tekai occured in December 1972, November 1975 and November 1979.

The Calibration of model was carried out by comparing the calculated hydrographs with the recorded hydrographs based on the abovementioned floods.

The basic data for reproduction of the hydrographs and computation for the calibration are described in this section.

#### 8.1.1 Flood hydrographs

The December 1972 flood hydrograph was recorded from December 14 to December 20. The rise at Penut was 9.6m reaching its peak at midnight of the 17th. Peak discharge was estimated to be 1,260 m<sup>3</sup>/s at Penut.

The November 1975 flood hydrograph was recorded from November 24 to November 30. Two big rises were recorded, the first rise was 7.1 m reaching its peak on the morning of the 25th, the second rise was of 7.0m in the evening of the 28th. Peak discharge was estimated to be 1,620 m<sup>3</sup>/s at Penut from the rating curve.

The November 1979 flood hydrograph was recorded from November 24 to November 30. The rise at Penut was 11.3m on the morning of the 27th. Peak discharge was estimated to be 1,330 m<sup>3</sup>/s at Penut from the rating curve.

Runoff depth for the period of recorded floods are shown in Table 8.1.

## 8.1.2 Rainfalls to see the training word a biretain by

Records of the heavy rainfall causing the abovementioned floods were obtained at Kg. Kerting, Kuala Tahan, Kamgsar and Ulu. Tekai.

The only record of heavy rainfall during the December 1972 flood was obtained at Kg. Kerting, near to the catchment area of S.Tekai. The hourly rainfall records were obtained at Kuala Tahan and Kamgsar in during November 1975 flood, and at Ulu. Tekai, Kuala Tahan and Kamgsar in during November 1979 flood.

Rainfalls at each station for the period of recorded floods are shown in Table 8.1.

Table 8.1 Rainfalls and Runoff Depth During Floods

	December 1972 Flood	November 1975 Plood	November 1979 Flood
Catchment Runoff (mm)	243	252	276
Peak Discharge (m³/s)	1,260	1,620	1,330
Rainfall át	9 (1 <sup>5</sup> ) 3 (1 년 <b>1</b> 년 <b>1</b>	ier zir diet zeleten.	
Kg. Herting	284*	<u>ាំគ្រប់</u> ប្រធាំទូទី	erit Tible
Kuala Tahan	) ကန်းချုံသည် အသုံး အသ	<u> 279</u>	192
Kamgsar Ja II - kay gatiga		· ·	
Ulu. Teksi (zz)	toptopic	ញ ភាសាស្ម្រាស់នាំ	224

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<sup>\*</sup> This value included the daily rainfall values.

## 8.2 Estimation Method

Three methods of flood hydrograph estimation are available for possible use in this study. These are the storage function method, the synthetic unit hydrograph method and the rational formula method. If there is no need to consider storage in rivers, a rational formula may be applied. However, if water resources development plans include a flood control dam, power-generation dam and retarding basin within the basin, the storage function method and the synthetic unit hydrograph method are adequate to calculate the hydrograph at many points on rivers. The storage function method was adopted to calculate the discharge at damsites considered in this report.

#### 8.2.1 Constants of the Hodel

Constants and division of the catchment area for the Storage Function Hethod are described as follows.

## (a) Division and layout of river basins and channels

The river basin was divided into four parts and the channel was divided into two. These were determined using a topographical map of 1/63360 and in view of dansite and accuracy of basin mean rainfall (the mean rainfall at the basin was assumed as a point rainfall of representative station). Fig. 8.1 shows the basin and channel model.

## (b) Coefficient of the function

The values of K and T<sub>1</sub> were obtained from empirical equations as rough estimates. It is preferable to obtain them from river discharge data by trial calculation.

(i) K

For the basin 
$$K = 43.4 \text{ C·I}_1^{-1/3}.L^{1/3}$$

For the channel
$$K = 0.166 \text{ L} \cdot 12^{-1/2}$$

C | Izzard constant (=0.12)

I<sub>1</sub> : Average basin slope

L : Length of river channel

12 : Average channel slope

(ii) T<sub>L</sub>

## For the basin

$$T_{\rm L} = 0.647 \cdot L - 0.56 \, (hr) \, (L > 11.9 \, km)$$

(L <u>≥</u> 11.9 km)

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The state of the s

L : Length of channel reach (km)

I : Average channel slope

#### For the channel

$$\tilde{T}_L = (7.36 \times 10^{-4}) \cdot L \cdot L^{-0.5}$$

(iii) P

### For the basin

$$\mathbf{p} = 0.33$$

## Por the channel with the best to the control of the channel

 $\mathbb{P}_{\hat{p}} \triangleq 0.6$ 

Constants

r , je ka	No.	Basin Area	Length of			Constants		
e mari in the	NO.	(km²)	River Channel	Channel Slope	К	P	TL(hr)	
	1	180.0	22.2	0.0239	50.7	0.33	0.48	
<b>n</b>	2	293.0	50.0	0.0098	89.4	0.33	1.79	
Basin	3	244.0	37.4	0.0131	73.7	0.33	1.20	
	4	663.0	52.0	0.0181	73.8	0.33	1.88	
on and a	I	-	18.1	0.0017	73.0	0.6	0.32	
Channe 1	11		24.2	0.0019	92.0	0.6	0.41	

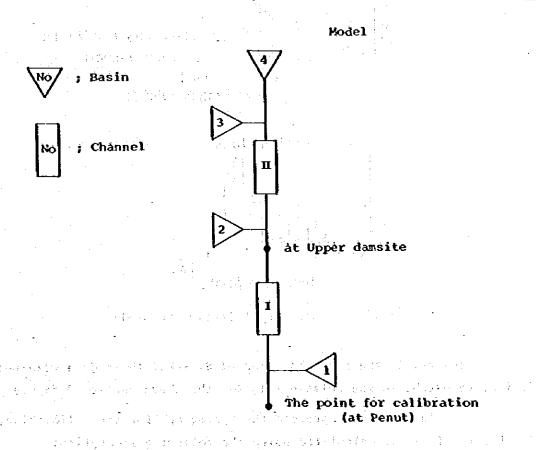


Fig. 8.1 Hodel of River System in S. Tekai

#### 8.2.2 Effective rainfall

Effective rainfall is the most difficult to calculate when making flood runoff calculations. Since the loss phenomenon of rainfall (percolation, storage in low-lying areas, etc.) is not clear, effective rainfall must be estimated on the basis of many assumptions. Where substantial runoff data for the past are available, total rainfall on the total direct runoff depth are obtained for each flood as shown in Figure 8.2

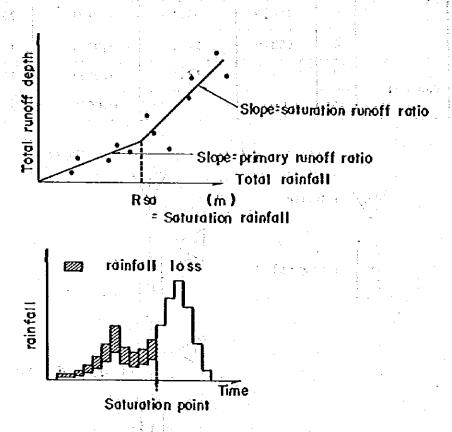


Fig. 8.2 Consept of Effective Rainfall

However, in the catchment area of S. Tekai these data were unsufficient to establish the relationship for the above-mentioned figure.

In order to reproduce the hydrograph for the calibration, effective rainfall was calculated using the following assumption:

- (i) The saturation rainfall (Rsa) ..... 100 am to 150 am
- (ii) The primary runoff ratio (f1) ..... 0.3 to 0.8

(iii) The saturation runoff ratio (f2) ... 1.0

Thee coefficients were estimated by trial and error.

## 8.3 Verification of the Plood Runoff Hodel

The verification calculation was made using a large IBM computer, model IBM 4341. The constant of the reservoir function was corrected from time to time by comparing the actual hydrograph and the calculated hydrograph for the flood described in 8.1.

The comparative verification finally produced the calculated hydrograph shown in Fig. 8.3. Constants used in this calculation are as follows:

- o Primary runoff ratio ..... 0.5
- o Saturation rainfall ..... 100 mm
- o Saturation runoff ratio ..... 1.0

## Constants of the Model

s g Ag	No.	Basin Aréa	Constants		
		(km²)	K -	P	TL(hr)
vykyki raježitove ježito. V		180.0	70.7	0.33	0.48
Basin	2	293.0	89.4	0.33	1.79
and the sign	3	244.0	73.7	0.33	1.20
	4	663.0	73.8	0.33	1.88
Channel (1884);	1		73.0	0.6	0.32
viidinie 1 × 1 1 · · · · · · · · ·	11		92.0	0.6	0.41

Representative stations for catchment rainfall during the floods were selected as follows:

- o The December 1972 flood ..... Kg. Merting
- o The November 1975 flood ..... Kuala Tahan
- o The November 1979 flood ...... Ulu. Tekai, Kuala Tahan

The above constants were judged as reasonable for the flood runoff model for the Tekai River Basin for the following reasons!

#### a. Flood in 1972

The discharge hydrograph observed at Penut has two conspicuous peaks as shown in Fig. 8.3.

The timely rainfall of Kg. Kerting used in this reproduction calculation, however, has no conspicuous first peak.

This calculation hydrograph does not have the first peak and thus does not agree with the actual hydrograph.

ngaragan na ngagangan ngalinggan ang ang ang kalan na kabipatèh an an ngalawak aka nga

## b. Plood in 1975 the letter at the letter letter at the letter at the

As in the case of the flood in 1972, there is a substantial difference between calculated and measured discharges at the first peak. These values agree well at the second peak.

This may be attributed to the fact that, as in the case of the flood in 1972, the first-peak timely rainfall at Kuala Tahan does not represent the entire basin. The effect f backwater from the Tembeling River may also be considered responsible because the difference between the measured and calculated discharges is large at around the peak.

However, due to lack of discharge data of this river, it is impossible to grasp the above effect qualitatively and quantitatively.

### c. Flood in 1979

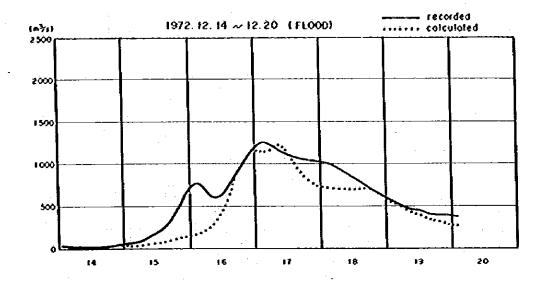
The calculated discharge as a whole is relatively small as compared with the mesured discharge.

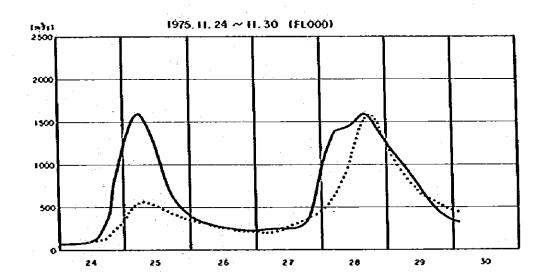
This is attributed to the fact that the total rainfall (224 mm, 192 mm) at Ulu. Tekai, Kuala Tahan chosen as a mean rainfall of the basin is smaller than the discharge (276 mm).

Fig. 4.2 shows the rainfall in the upstream area rather larger than that at both stations. In this flood, it is estimated that the actual rain fall is far above the rainfall at both stations.

· 自己的自己的人,这是一个人的人,但是一个人的人的人,但是一个人的人的人。

- 83 -





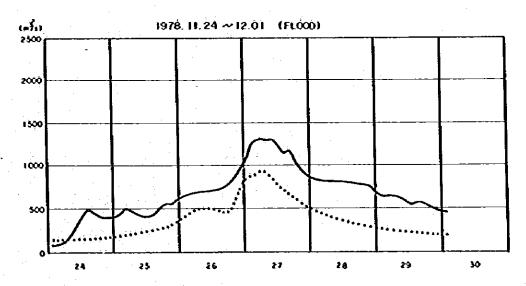


Fig 6.3 Comparison between the calculated and recorded hydrogramphs

# 9. FLOOD CONTROL

#### 9. PLOOD CONTROL

#### 9.1 Introduction

To determine the spillway capacity required to provide safety for the most critical flood runoff from a particular basin, it will be necessary to estimate the spillway design flood, the method of operation and the reservoir flood-routing computations.

The procedure for making these estimations is described in this section.

## 9.2 Basic Plood Discharge

Basic flood discharge at the damsite would be obtained by converting from the spillway design storm through the previously calibrated model. Basic flood discharge' is a discharge hydrograph without considering the flood control by dam, etc. In this study the design scale is established as follows, by taking into consideration the type of dam.

- Upper Dam: The design scale is established by assuming a 10,000 years return period flood because the Upper Dam is a rockfill type dam and the overflow exerts a very bad influence on the dam.
- Lower Dam: In this case the influence of the overflow is minor compared with a fill dam because the Lower Dam is a gravitytype concrete dam. Therefore, the design scale is established by assuming period of 1,000 years.

### 9.2.1 The design storm

## (a) The design rainfall depth

The 3 cases of flood mentioned in the Chapter 8 are typical examples of flood occurred in the past in the Tekai River.

The rainfall duration was approximately 5 days in the 3 cases of flood mentioned above. Therefore, the design rainfall depth is determined by assuming a rainfall duration of 5 days.

The rainfall depths for 10,000 years return period and 1,000 years return period in Kg. Merting and Jerantut, located nearby the Tekai River basin, are shown in the Table 9.1.

The design scale of the Upper Dam assumes a period of 10,000 years and the rainfall depth for 10,000 years return period is 750mm to 840mm according to the Table 9.1.

On the other hand, according to the distribution of rainfall for 10,000 years return period shown in the Fig 4.2 of the Chapter 4, the rainfall depth in the basin of the Tekai River is 600 mm to 1,100 mm, with an average value of 850mm. Therefore, the rainfall for 10,000 years return period in the basin of the Tekai River is assumed to be 840mm, which is the value observed at the Kg. Merting Station.

Kg. Merting was close to the rainfall of a period of 1,000 years at Jerantut and Kg. Merting, the design rainfall depth for the catchment area of S. Tekai was estimated to be about 600 mm.

Table 9.1 Five Day Rainfalls for a Period of 10,000 Years and 1,000 Years

-	er in a	75311 11
Jerantut 3922069	1 12.50 mg	(567) 
<u> </u>		(583)
Kg. Herting	1 - Marin 123 - 12	577 (501)
4223115	<b>G</b> 353334 (334)	842 (670)

( )1 1,000 years

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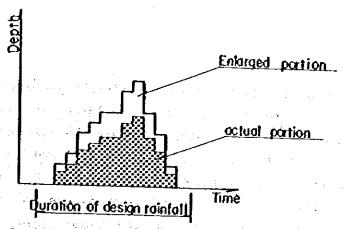
## (b) Adjustment between the actual storm and the design storm

The adjustment means that the actual storm depth is enlarged so that it becomes equal to the design storm depth as shown in Figure 9.1

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Pig 9.1 Adjustment between the Actual Storm and Design Storm

The adjustment factors used to enlarge from the actual storm during experienced floods to the design storm are listed in Table 9.2.

Table 9.2 Adjustment Factors A period of 1,000 years

	1972	1975	1979
*Actual rainfall	284	279	221
Design rainfall	600	600	600
Adjustment factor	2.112	2.151	2.715

## A period of 10,000 years

	1972	1975	1979
*Actual rainfall	284	279	221
Design rainfall	840	840	840
Adjustment factor	2.958	3.011	3.801

<sup>\*</sup> The value in five day's duration.

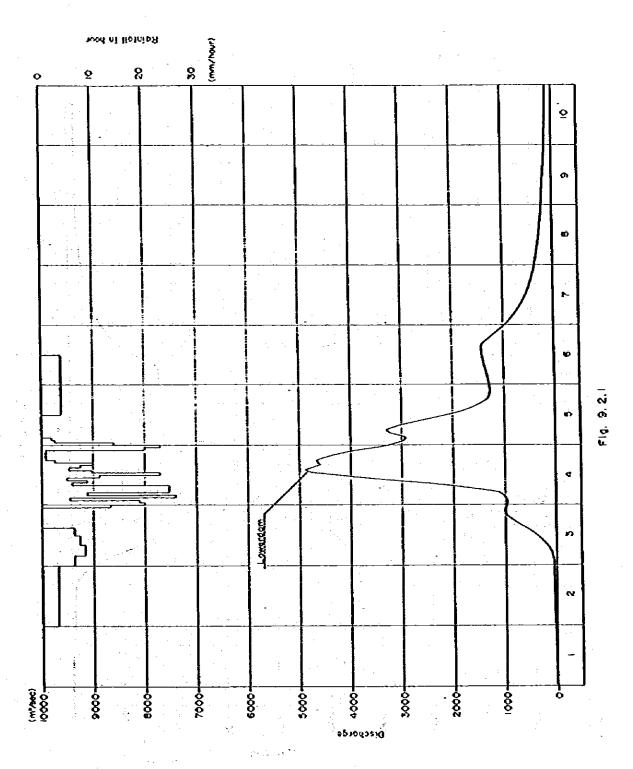
In the case of selecting the design rainfall pattern it is necessary to take care not to make an excessively large adjustment between the actual storm and the design storm, because the rain pattern pronounced differences in the cases of large rainfall and small rainfall.

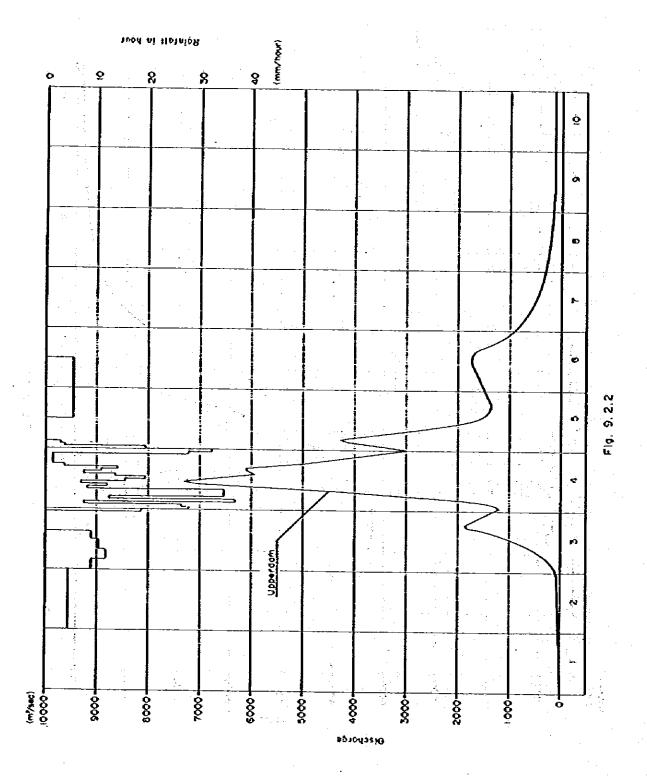
Such being the case, the rainfall pattern of 1972 is adopted as design rainfall pattern of this study because it has the smallest adjustment between the actual storm and the design storm.

### 9.2.2 Basic Plood discharge

The basic flood discharges of the Upper Dam and Lower Dam are calculated from the design rainfall, by means of the calibrated model based on the storage function method.

The results of calculation are shown in the Fig 9.2 and the peak values of the basic flood discharge are shown in the next table.





#### 9.3 Calculation of Control

Plood control by upper and lower damsites is made for the case where the above basic flood discharge occurs. Then the maximum discharge at dam flood is determined. The scales of flood to be considered are Upper Dam = 10,000 years return period flood and Lower Dam = 1,000 years return period flood. In the Lower Dam the object of consideration is the flood hydrograph of the 1,000 years return period flood (peak discharge at the Upper Dam = 4,600 m<sup>3</sup>/s) submitted to flood control at the Upper Dam.

#### 9.3.1 Flood control calculation

The process of computing the reservoir stage, storage volumes and outflow rates corresponding to the hydrograph of inflow is commonly referred to as "flood-routing".

The maximum reservoir level obtained by routing the flood through the reservoir is determined by the following:

- o Initial reservoir stage
  - o Rate and volume of inflow into the reservoir
  - o Rate of outflow: Discharge over spillway
  - o Storage capacity above initial reservoir level

The increase or decrease in storage and rate of outflow resulting from the volumes of inflow during successive short increments of time are computed using a step-by-step computation.

## 9.3.2 Plood control calculation of the upper dassite

In the case of the upper damsite, there is no dam which can control the flood on its upstream. Therefore, the inflow hydrograph (dam design flood discharge) becomes the following basic flood discharge. Sample flood routings at the Upper Tekai Reservoir were carried out under the following conditions:

- (a) Initial reservoir stage .... EL 157.00

  The initial reservoir stage was at the same water level as H.W.L.
- (b) Inflow into the reservoir .... The design flood (7,300 m3/s)
- (c) Discharge over the spillway

  The crest length of the spillway is 40m and the rating-curve of the spillway is shown in Figure 9.3

### (d) Storage capacity

The storage capacity curve above the crest of the spillway is shown in Figure 9.3

After step-by-step computation, the relation between the reservoir stages and rate of outflow during the spillway design flood was obtained as shown in Figure 9.4

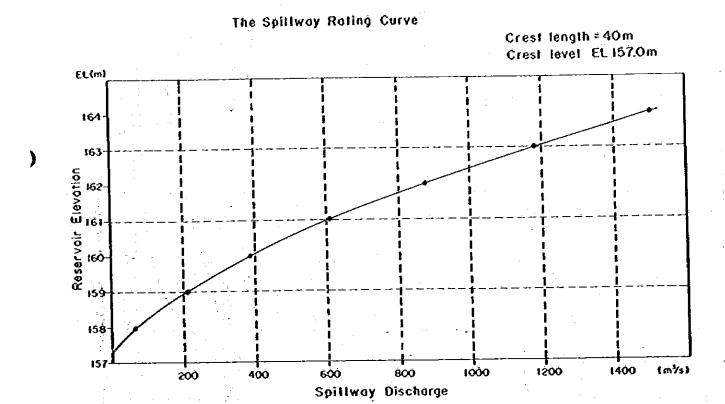
The maximum water level, the surcharge volumes and the maximum outflow during the flood are listed in Table 9.3

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Table 9.3 Design Flood at the Upper Damsite

	<u>. 1945 y a svanaří sakredě komitin se nes</u> t
Peak discharge of inflow	7,300 m <sup>3</sup> /s
Maximum water level	77. 45.7 EL 164.0 1 1111
Surcharge volumes	580.0 x 106 m3
Haximum outflow	1,504 m <sup>3</sup> /s



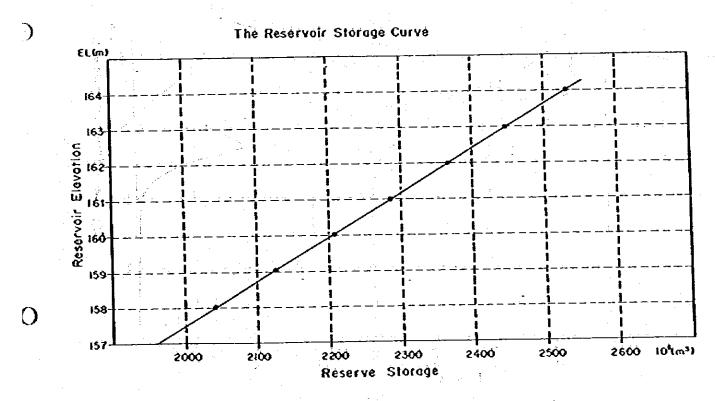
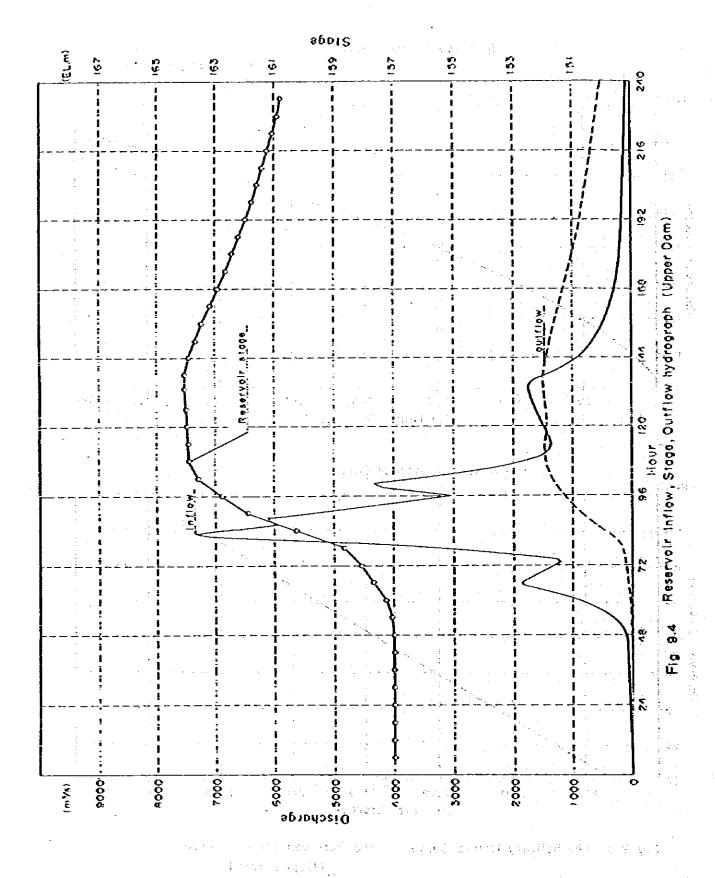


Fig. 9.3 The Spillway Rating Curve and the Reservoir Storage Curve (Uppen Dam)



-100 -

## 9.3.3 Lower Damsite Flood Control Calculation

The peak value of the flood with 1,000 years return period is  $4,600 \text{ m}^3/\text{s}$ . Of that total approximately 3,750 m $^3/\text{s}$  are cut by the flood control at the Upper Dam and as consequence the maximum discharge of the Upper Dam becomes 850 m $^3/\text{s}$ . Therefore, the design flood discharge of the Lower Dam becomes approximately 1,100 m $^3/\text{s}$ , i.e., 850 m $^3/\text{s}$  plus runoff of residual area.

Therefore, the dam design flow discharge of the lower damsite is about  $1,100~\text{m}^3/\text{s}$ .

The flood control calculation for the lower damsite is made under the following conditions:

- a) Initial water level of reservoir: BL 75,000m (H.W.L.)
- b) Plood amount: Dam design flood discharge
- c) Plood discharge rate

The crest length of the flood discharge is 84m. The crest elevation of the central 42m section is at EL 75.00 and that of other sections is at EL 76.00 m.

Fig. 9.5 shows the water level - discharge curve.

### d) Reservoir capacity

Fig. 9.5 shows the reservoir capacity curve above the flood discharge crest top end.

The relationship between the inflow rate, reservoir water level and discharge rate is shown in Fig. 9.6. Table 9.4 shows the maximum reservoir level, control capacity and maximum discharge rate.

Table 9.4 Design Plood at the Lover Dansite

Peak discharge of inflow	1,100 m <sup>3</sup> /s
Maximum water level	EL 79.00
Surcharge volumes	30.5 x 106 m3
Maximum outflow	1,100 m <sup>3</sup> /s

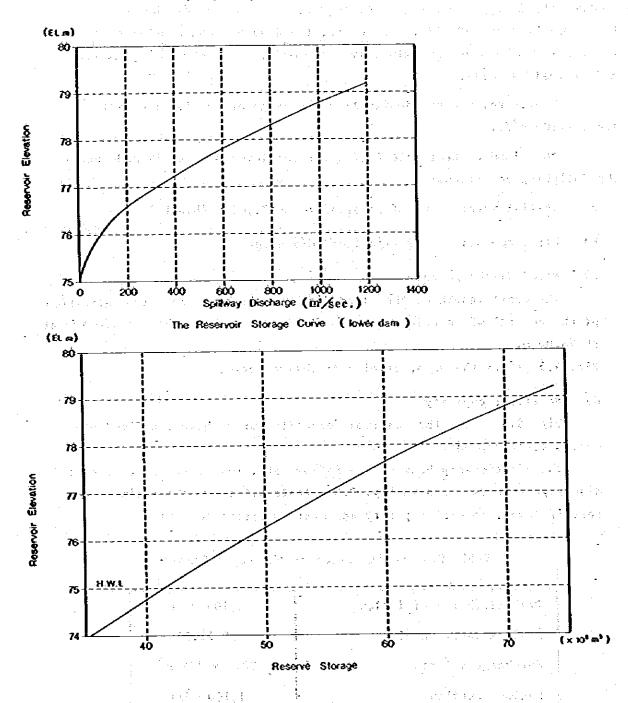
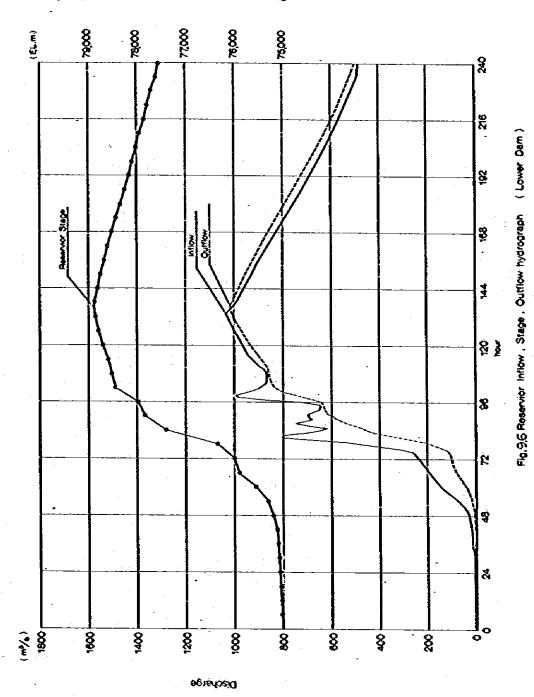


Fig. 9.5 The Spillway Rating Curve and the Reservoir Storage Curve (Lower Dam)

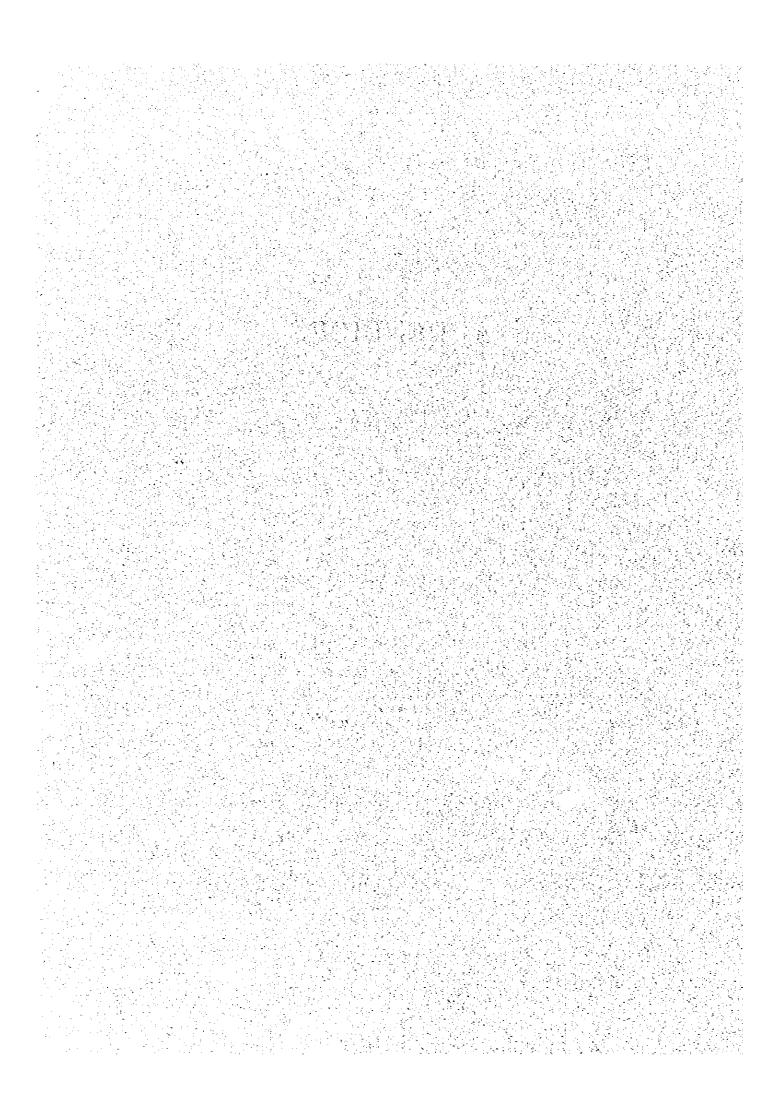




# APPENDICES

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## Appendix A

Maximum Rainfall Data Used in Analysis

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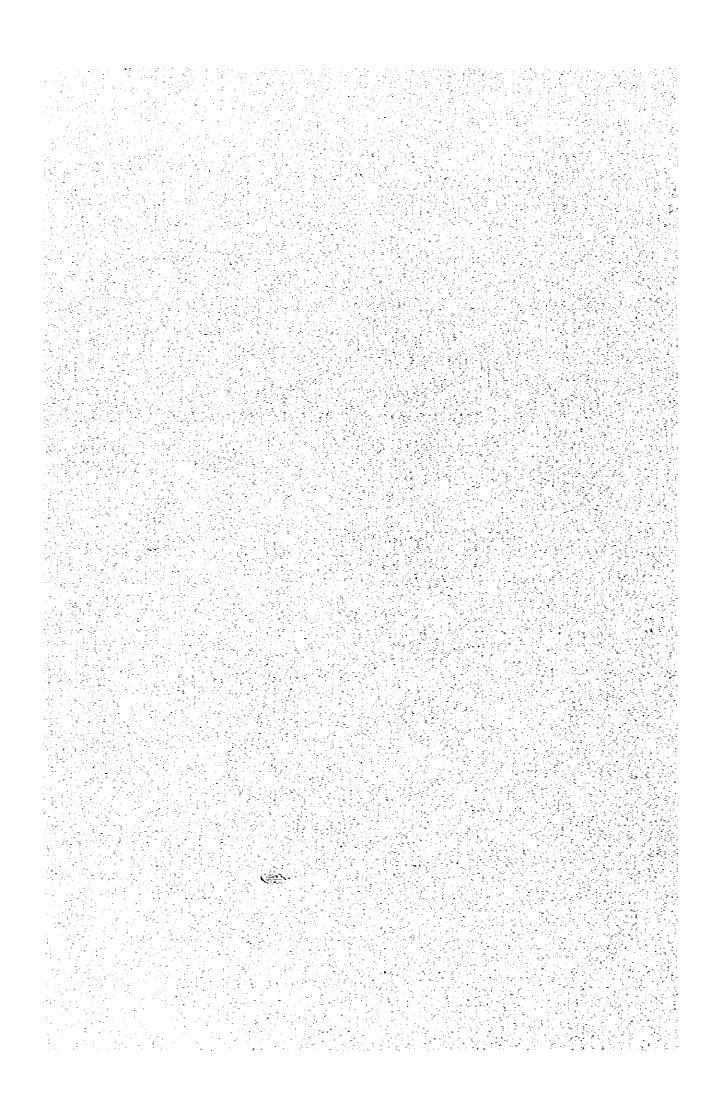
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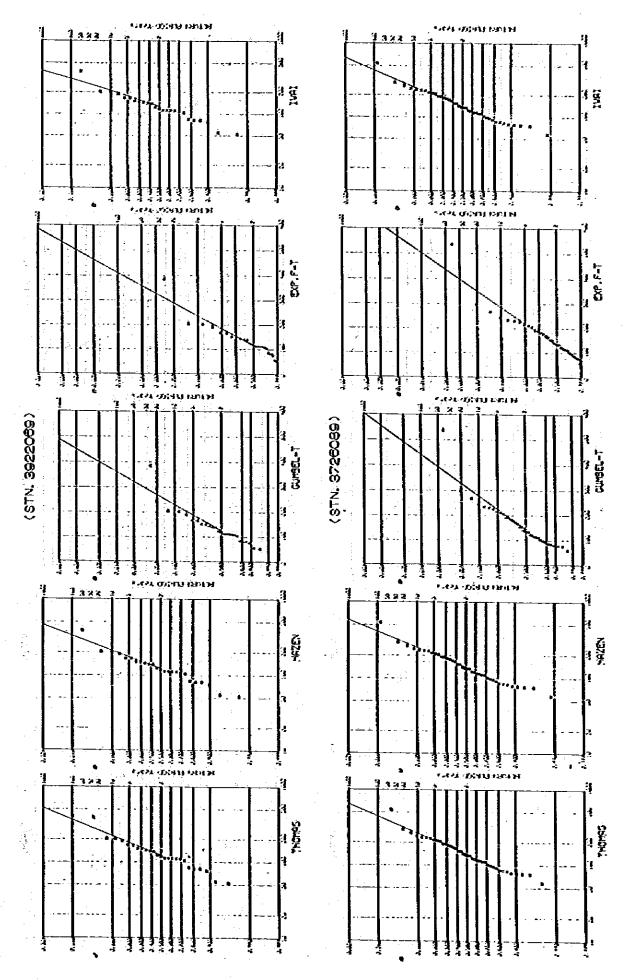
Appendix B

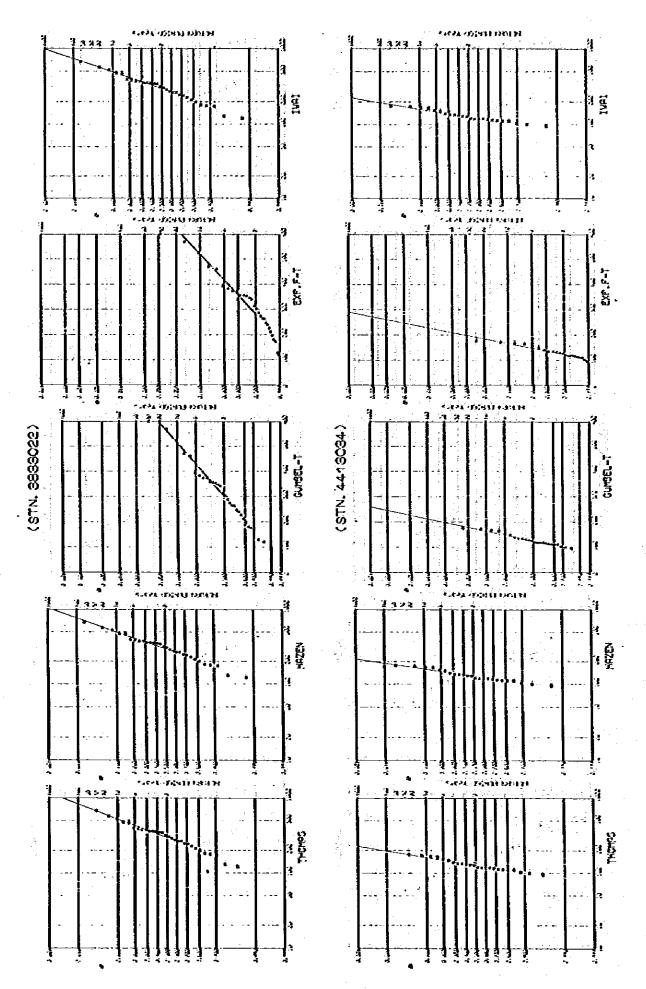
Probability Curves

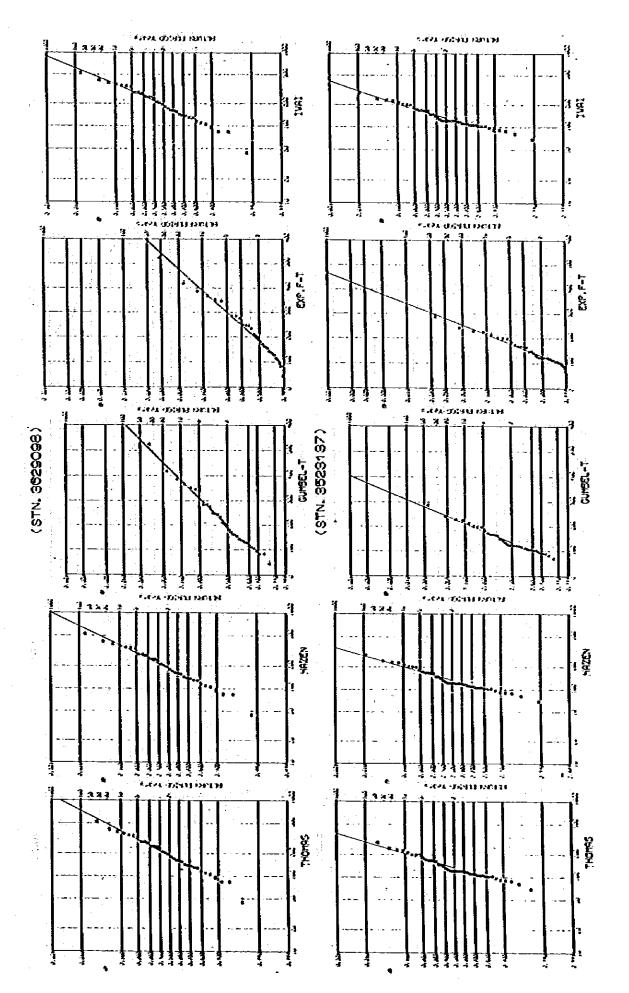
The Curves for 5 days maximum

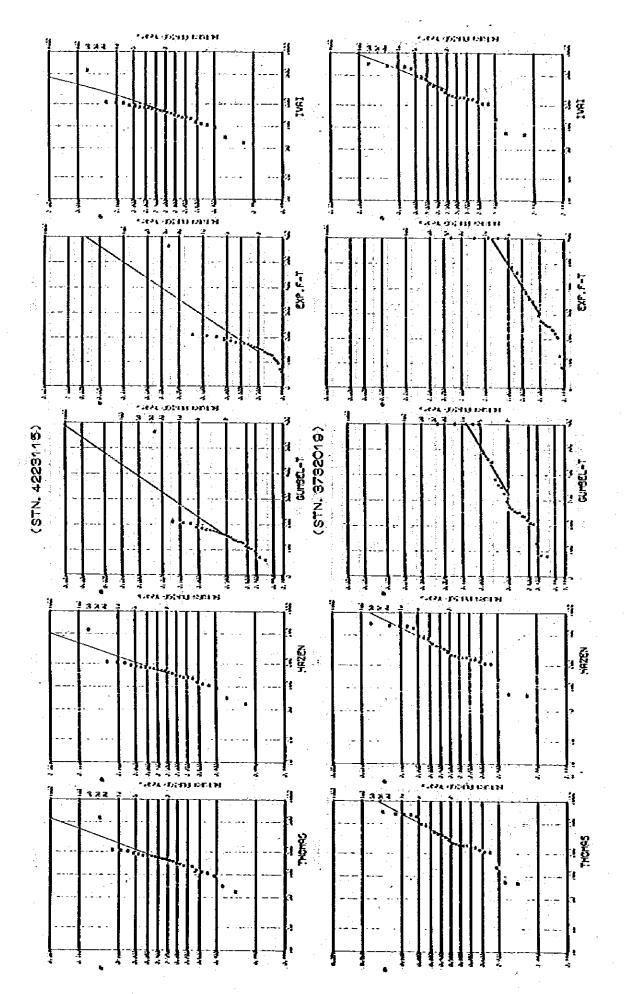
Rainfall estimated by each distribution at each station

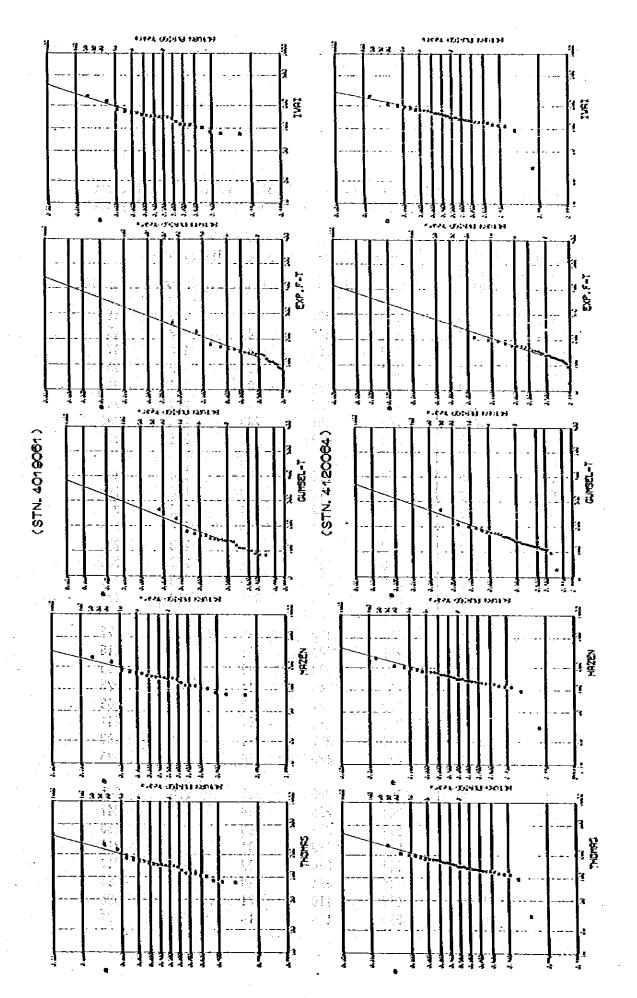












Comparison of 1 and 5 Day Rainfalls at 3523137 estimated with Various Frequency Distributions Period of Record 1930 - 1978 (43 Full years).

Duration	Return	Rainfall	Estimated	by Each	Distribution	
(days)	Period (years)	Thomas	Kazen	lvai	Gumbel	Exp-Dis
One	5	124	122	122	128	124
Day	10	144	140	141	149	-151
	20	163	158	160	170	178
	30	174	168	170	182	194
	50	187	180	184	197	215
	100	206	196	202	218	242
	1000	267	251	263	286	333
	10000	331	307	328	355	424
îvo	5	153	150	150	153	150
Day	10	176	172	173	178	180
	20	199	192	194	202	211
	30	211	204	208	216	228
	50	227	218	221	233	251
	100	248	237	242	257	281
*	1000	318	293	309	333	381
	10000	391	363	379	410	482
Three	5	178	175	175	181	177
Day	10	207	201	203	212	215
	20	234	228	229	242	253
	30	249	240	244	259	275
	50	268	257	282	280	303
	100	294	280	287	309	341
	1000	381	357	371	404	467
	10000	471	436	460	499	593
Five	5	203	205	205	210	205
Day	10	241	235	236	244	247
	20	272	263	265	277	289
	30	289	279	281	296	314
	50	311	298	302	320	344
	100	340	324	329	352	386
	1000	436	410	421	458	525
	10000	536	498	516	584	664

Comparison of 1 and 5 Day Rainfalls at 3829098 estimated with Various Frequency Distributions Period of Record 1931 - 1980 (34 Full years).

Duration (days)	Return Period	Rainfall Estimated by Each Distribution					
(ua)s)	(years)	Thomas	Kazen	lvai	Gumbel	Exp-Dis	
One	5	171	166	166	171	166	
Day	10	216	205	201	209	212	
7 -	20	261	245	235	245	258	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	30	289	269	254	266	285	
- 14J	50 😭	324	300	279	292	319	
r di	100	374	342	312	328	364	
, - E	1000	559	496	424	444	517	
* * .	10000	779	675	544	580	669	
Two	5	254	245	245	253	246	
Day	10	323	312	303	313	317	
5.5	20	407	380	358	369	389 -	
- <u>1</u>	30	454	421	391	402	431	
i i	50 ≅ 1	516	474	432	443	484	
1 1 1	100	808	549	488	498	555	
. 1	1000	948	831	684	680	793	
  	10000	1367	1170	899	862	1031	
Three	5	307	298	298	308	299	
Day	10	398	377	370	381	388	
	20	492	459	443	451	477	
44.5	30	550	503	486	492	529	
417	50	626	574	541	542	594	
; i ·	100	735	667	618	810	683	
1, 5, 3		1152	1011	891	835	978	
	10000	1668	1426	1203	1059	1273	
Five	5	310	300	300	310	300	
Day	10: ii.	400	385	375	385	385	
214	20	495	470	445	455	480	
	301	570 :	540	490	495	530	
# 7 : 1 7 : 1	50	630	590 <u>;</u>	545	545	600	
5.11	100%	740		620	615	685	
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	10000	1670	2.4	1290	1060	1280	
		*i : = = = = = = = = = = = = = = = = = =		\$1¥ 	***		

Comparison of 1 and 5 Day Rainfalls at 3732019 estimated with Various Frequency Distributions Period of Record 1954 - 1980 (28 Full years).

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Duration (days)	Return Period	Rainfall	Estimated	by Each D	istribution	
<u> </u>	(years)	Thomas	Hazen	Irai	Gunbé I	Exp-Dis
One	5	231	273	281	235	288
Day	10	373	350	342	365	375
	20	457	422	401	433	461
*	30	509	465	434	471	512
	50	576	520	476	520	575
	100	672	599	534	585	862
: 1	1000	1033	887	727	801	349
	10000	1474	1226	933	1016	1236
îvo	5	417	399	399	415	403
Day	10	542	507	492	512	522
	20	672	617	583	808	841
4.1	30	753	684	636	860	710
	50	858	<b>?</b> ?1	703	727	798
<b>;</b>	100	1008	894	785	818	917
	1000	1587	1352	1114	1118	1311
	10000	2309	1903	1464	1417	1708
Three	5	512	489	491	508	494
Day	10	673	628	603	629	640
	20	843	771	710	744	787
<b>5</b>	30	948	858	772	811	873
• =	50	1086	972	848	894	981
	100	1285	1134 🕕	852	1007	1127
1.7	1000	2061	1748	1305	1378	1815
	10000	3045	2493	1678	1749	2102
Five	5	811	582	584	627	811
Day '	10 <sup>- 1</sup>	815	758	751	790	814
1.	20	1035	842	922	947	1016
	30 😂	1172	1055	1025	1037	1134
	<b>50</b> 🗁	1354	1204	1159	1150	1283
	100	1618	1417	1349 👍	1302	1485
	1000	2669		2058	1804 6 5	2157
	10000	4035	3263	2910	2308	2829

Comparison of 1 and 5 Bay Rainfalls at 3726089 estimated with Various Frequency Distributions Period of Record 1930 - 1980 (40 Full years).

and the state of t

Duration (days)	Return Period	Rainfal	l Estimated	by Each D	istribution	
(00)3)	(Aeats)	Thomas	Kazen	lvai	Cumbel	Exp-Dis
One	5	116	113	113	118	115
Day	10	138	133	134	141	144
-	20	159	152	154	163	172
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	30	171	164	166	176	188
	50	187	177	181	192	203
	100	208	196	202	213	238
	1000	282	261	273	284	331
1 881 4 3	10000	362	323	351	354	425
<b>T</b> wo	5	173	169	169	182	177
Day	10	214	206	207	223	229
	20	255	242	244	263	281
	30	279	264	266	286	311
	50	310	291	284	315	349
113	100	353	329	333	354	401
11.1	1000	510	464	472	482	573
• 42.3	10000	690	616	623	<b>603</b>	745
Three	<b>ට</b> ිග	213	207	208	224	213
Day	10	284	254	257	277	285
7 A . A	20	316	300	308	327	350
	30	347	327	336	358	389
	50	<b>386</b>	362	373	393	437
1	100	442	411	427	442	502
1.3	1000	643	583	621	603	720
<b>1</b>	10000	876	780	848	764	938
Five	5	287	260	261	281	274
Day	10	337	323	325	349	359
1.1	20	409	387	391	414	443
	30	452	424	430	452	492
	50	508	473	480	493	555
*	100	588	541	551	562	639
٠	1000	878	789	810	771	920
	10000	1224	1077	1114	979	1200

Comparison of 1 and 5 Day Rainfalls at 3833022 estimated with Various Frequency Distributions Period of Record 1947 - 1979 (30 Full years).

Duration	Return	Rainfal	l Estimated	by Each Di	stributión	
(days)	Period (years)	Thomas	Hazen	lwai	Gumbel	Exp-Dis
One	5	251	244	246	248	240
Day	10	300	288	282	290	230
1 21 1	20	348 -	329	313	331	341
-	30	376	353	331	355	371
	50	410	383	352	385	409
	100	458	424	379	425	459
	1000	624	563	463	558	<b>628</b>
	10000	805	711	541	691	797
Two	5	353	343	342	344	335
Day	10	427	403	336	407	409
- :	20	500	473	445	468	484
	30	543	<b>511</b>	472	502	528
	50	597	558	505	546	582
	100	672	622	548	604	657
	1000	935	844	683	798	904
7 = 1 1,5 (	10000	1228	1086	814	991	1152
ihree	5	423	417	417	428	418
Day	10	523	500	498	513	521
•	20	618	581	572	595	624
	30	670	829	815	642	684
	50	740	688	669	700	780
	100	836	770	742	780	863
	1000	1176	1055	991	1041	1205
. 1. 2	10000	1580	1369	1254	1302	1547
Five	5	512	497	498	513	501
Day	10	626	598	597	817	627
	20	739	697	694	717	754
,	30	806	754	750	775	828
* :	50	891	827	820	847	922
	100	1009	928	917	944	1048
	1000	1429	1279	1251	1265	1469
	10000	1908	1667	1616	1585	1890
<del> </del>		1.37	<u></u>		<del></del>	<del></del>

Comparison of 1 and 5 Day Rainfalls at 3922069 estimated with Various Frequency Distributions Period of Record 1955 - 1980 (25 Full years).

Duration (days)	Return Period			by Each (	distribution	
	(years)	Thomas	Hazen	Ivai	Gustel	Exp-Dis
One	5	132	128	130	147	145
Day	10	. 16 <b>5</b>	157	161	184	194
	20	198	188	194	220	242
	30	218	202	214	241	270
	50	243	224	233	286	306
· .	100	278	254	275	301	354
	1000	408	361	407	416	514
	10000	553	483	565	530	674
Two	5	161	158	159	170	167
Day	10	197	188	189	208	216
	20	233	220	217	245	284
:	30	254	238	233	286	292
	50	281	261	253	232	328
	100	319	293	279	327	378
	1000	452	404	385	444	536
	10000	604	527	453	581	696
Three	. 5	182	176	179	189	186
Day	10	223	212	212	230	238
	20	284	248	241	270	290
2.1	30	288	268	258	293	321
	50	319	285	278	322	359
	100	362	331	304	361	411
	1000	516	457	390	489	585
4 위 : <u></u>	10000	691	598	474	617	758
Fire	5	217	210	210	225	221
Day	10	266	252	254	275	284
1, 7	20`	315	294	298	323	346
1.1	- 30	343	319	324	350	383
\$ 955	50	380	350	357	384	428
	100	431	392	403	431	491
	1000	614	541	587	583	698
	10000	821	708	753	736	905

Comparison of 1 and 5 Day Rainfalls at 4019081 estimated with Various Frequency Distributions Period of Record 1947 - 1975 (23 Full years).

Duration (days)	Return Period	Rainfal	Estimated	by Each D	istribution	l
	(years)	Thomas	Hazen	Ivai	Gumbel	Exp-Dis
One	5	110	107	108	110	107
Day	10	128	122	122	127	128
- \.	20	144	137	136	144	150
	30	154	145	143	153	162
	50	165	154	152	168	177
	100	181	168	164	182	198
14 J	1000	234	211	203	238	288
	10000	288	255	240	283	338
Two	5	148	145	145	149	147
Day	10	170	164	165	173	178
	20. :	191	182	183	198	208
	30	203	192	194	209	223
****	50	217	204	207	228	245
	100	238	221	225	248	274
	1000	300	274	283	321	372
	10000	368	328	343	394	470
Three	5	175	171	172	177	175
Day	10	201	194	195	206	210
	20	226	215	218	233	245
	30	239	227	230	249	266
×*.	50	257	242	246	268	292
	100	273	261	267	285	327
	1000	355	324	336	383	444
j i	10000	432	388	407	470	567
Five	5	221	218	216	224	221
Day	10	256	247	247	282	268
	20	289	278	275	299	315
·	30	308	292	291	320	343
	50	332	312	310	346	378
	100	364	340	338	381	425
	1000	470	429	420	489	582
	10000	581	520	503	616	739

Comparison of 1 and 5 Day Rainfalls at 4120064 estimated with Various Frequency Distributions Period of Record 1946 - 1980 (32 Full years).

Duration (days)	Return Period	Rainfall	Estimated	by Each D	istribution	
(44)3)	(rears)	Thosas	Hazen	lvai	Gumbel	Exp-Dis
Ône	5	134	131	132	136	133
Day	10	161	156	158	162	165
	20	187	179	179	187	197
	30 🐪	202	192	192	201	216
	50	221	203	208	<b>21</b> 9	233
. 1	100	247	232	230	243	271
*- [	1000	338	311	303	323	378
	10000	438	397	379	403	484
Two	5	157	155	157	156	153
Day	10	183	178	181	180	182
	20	207	200	203	204	212
	30	221	212	215	217	229
	50	238	228	230	234	251
	100	261	249 <sup>= 1</sup>	251	257	281
•	1000	333	318	317	332	379
<del></del> .	10000	420	389	384	408	478
Three	5	188	183	183	181	177
Day	10	215	210	204	207	208
	20	243	235	222	232	239
2.5	30	258	249	231	247	257
٠.	50	278	267	243	265	280
÷.	100	304	290	258	289	311
:, *	1000	392	368	301	369	413
	10000	483	448	340	449	517
Five	5	225	221	235	220	215
Day	10	262	254	267	253	254
	20	297	286	293	285	294
8.	30	317	304	307	303	317
	50	341	326	323	326	346
	100	375	358	343	357	386
<u> </u>	1000	488	455	400	459	517
*	10000	608	558	448	561	649

Comparison of 1 and 5 Day Rainfalls at 4223115 estimated with Various Frequency Distributions Period of Record 1948 - 1977 (28 Full years).

Duration (days)	Return Period	Rainfall	Estimated	by Each l	Distribution	; ;	
	(years)	Thomas	Hazen	Ivai	Gv≖bel	Exp-Di	S
One	5	136	132	143	141	139	
Cay	10	160	154	163	168	173	
ŧ	20	183	174	180	194	207	
	30	186	186	183	203	227	9!
i	50	212	200	200	228	253	
:	100	235	219	213	253	287	
÷	1000	311	284	252	337	400	
	10000	393	352	286	420	514	
Two	5	184	178	188	197	194	
Day	10	221	212	221	240	243	
	20	259	245	250	281	304	
	30	280	264	267	305	336	43 <sup>13</sup>
(1) (1) (4) (4)	50	308	288	286	335	377	
	100 📲	346	320	313	376	432	
	1000	479	432	395	509	615	
	10000	627	554	474	642	797	
Three	5	215	209	222	228	228	
Day	10	253	248	280	279	289	
. •	20	302	286	294	327	352	
	30	327	308	312	354	389	2 8 %
	50	359	336	334	388	435	
	100	402	374	383	435	499	
	1000	558	504	453	588	708	
1	10000	727	645	538	740	913	
Fire	5	253	246	266	266	282	
Day	10	304	291	308	322	333	11
	20	353	335	344	378	404	- 1
	30	382	360	383	407	446	
-	50	418	391	388	446	498	
	100	468	434	418	498	588	-
- 1	1000	842	581	501	670	804	
	10000	833	740	577	842	1039	

Comparison of 1 and 5 Day Rainfalls at 4413034 estimated with Various Frequency Distributions Period of Record 1954 - 1980 (24 Full years).

Duration (days)	Return Period	Rainfal								
	(years)	Thomas	Hazèn	Iwai	Gumbel	Exp-Dis				
One	5	96	94	94	96	95				
Day	10	106	103	104	103	111				
	20	116	111	113	121	126				
	30	121	116	118	128	135				
•	50	128	122	124	136	147				
	100	136	123	132	148	182				
	1000	163	152	158	186	214				
· .	10000	190	174	183	225	265				
Two	- 5	128	127	127	128	127				
Day	10	142	139	139	145	147				
	20	155	149	151	180	166				
	30	162	155	157	168	177				
	50	170	163	165	179	191				
	100	181	172	175	194	210				
	1000	215	202	207	242	274				
	10000	248	230	237	231	338				
Three	5	143	147	147	149	147				
Day	- 10	162	159	159	165	166				
	20	174	169	170	179	185				
	30	180	175	176	188	196				
	50	188	181	183	198	210				
	100	198	190	192	213	229				
	1000	229	217	220	260	291				
·	10000	258	241	247	308	357				
Five	5	194	191	191	193	191				
Da <b>y</b>	10	213	207	208	214	216				
	20	229	222	223	235	242				
	30	238	230	231	246	257				
	50	249	239	241	281	275				
	100	263	252	254	280	301				
	1000	308	230	294	345	385				
٠	10000	351	328	332	410	470				

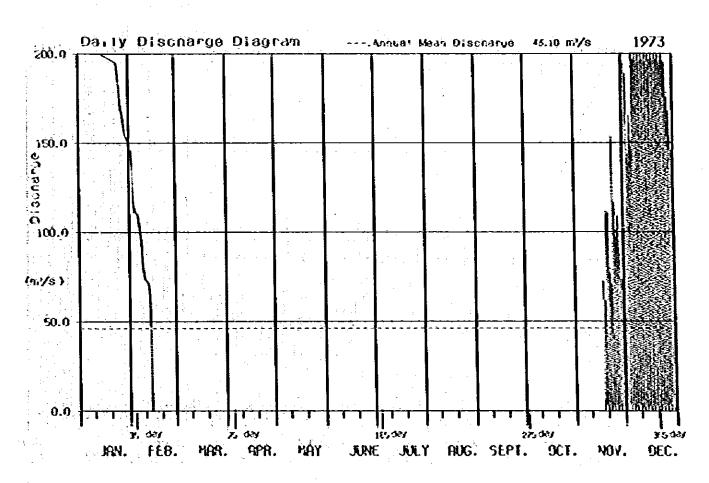
## Appendix C

Daily Discharge Tables and Duration Curves at Four stations

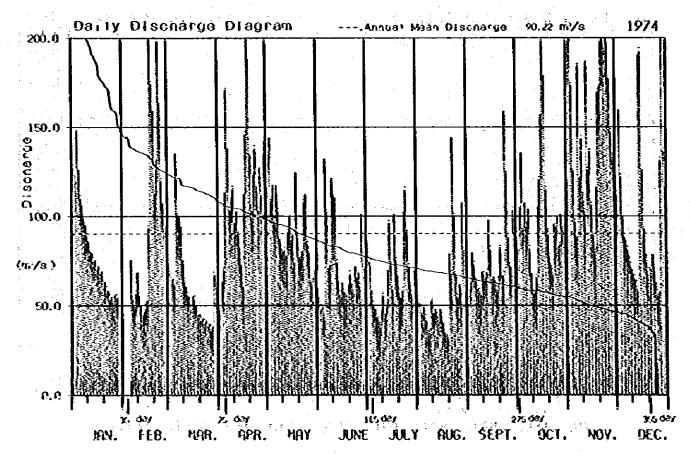
Jeram Teras Penut Kuala Tahan Kg: Pagi

23 - 33 - 34 - 34 - 34 - 34 - 34 - 34 -		

	Daily	Daily Discharge Table			<b>:</b>	(G. A. = 2700.0 km) (Unit.m/s)						1973
dâ	JAN.	FEB.	KAR.	SPR.	KAY	JUNE	JULY	GUG.	SEPT.	GCI.	NCY.	CEC
1												37£ 7 254.3
. 3								•				i £2.5
4	•	1.										145.5 145.
					:		•					(22).
7						4						237.
Ļ										·		155% 2755
ำจึ		_3		2		-						1335.3
115	٠	,			:							1020.
13		1.	÷	•		•						5;5. 374.
15									1			347.
15			1.									(535.
13				1	1.						:-	311. 271.
15	1	-	,								72.22 51.54	ં ડુંટેંંડેં.
20	55 c. 1875 a. — 18	-								•	111.42 ( 12.30)	322
-											( (2),34) 110,43	< 550.
55 51			. :	4		-					77,74	313.
いない						4					70.74 152.85	370, 271,
Ž					1.	*					(105.05)	23E
25								4		100	133 F2	225.
- 27			• .				: .				136.75	174. 176
2527.27.33				11:50						-	/3.4/	157.
31		$F_{i}$		200					*		222.43	157. 145.
<u>. i                                    </u>	. با معادد دا					<b>.</b>					4120,041	2
c la											1375.47 45.83	1545
lean Isa						6.*					222.45	47E 2755
ei -	1	<u>.</u>	1.1-1			<u> </u>	1 1					145
	1 11 1	1.5	Max.	y.581	7,521	Its des	275 581	3.5 de/	Mia.	Mean		¥o.
		• * * * * * * * * * * * * * * * * * * *	2755 Œ	111.42	-					45.13		14524



	Daily	Disc	narge	Táblè		•	C. A. = 2	700.0	kn/) (Un	lt:mys	<b>)</b>	1974
day	Jrid.	ft.B.	Kes.	FFR.	MAY	JUNE	JULY	rug.	SEPI,	CC1.	HOV.	CF.C.
123	137.13 127.41 121.53 150.26 123.41 (133.35)		11.27 71.71 55.71 54.05 571.00	# (6) # (8) # (8) # (8) # (8) # (8)	115,24 135,34 141,42 111,52 117,63 (125,30)	55.70 54.77 54.50 59.50 46.47 52.77	6.97 6.86 7.86 7.75 7.75 7.75	53.52)	4. % 67. % 57. 77. 61 68. 49	\$6.43 61.57 104.67 135.65 74.42 (101.02)	121.52 257.53 175.54 125.54 125.54 111.65	122.43 114.54 157.64 123.76 22.44 (124.13
10 10	(101.23)	75500000000000000000000000000000000000	175.34 187.07 180.21 25.57 (182.01)	172.23 186.35 186.35 110.30 117.35 1126.83	22.57 117.54 104.54 104.51 104.51 104.51 105.51	132.43 134.14 74.63 731.45 130.63	45.57 41.57 47.57 45.77	45.33 41.26 37.23 53.21 (4.27)	5,61 51,41 57,77 57,61 55,27 53,27	107.48 27.37 104.37 63.27 ( 22.33)	76 62 66 53 165 55 121 64 145 47 (126 36) 113 62	70.47 87.01 83.34 76.33 74.25 4.82.57
12 13 14 19	77.43 78.53 78.53	57.67 50.16 45.51 45.55 51.04 (50.51) 52.71	\$5.73 \$5.73 \$5.73 \$5.74 \$5.54 \$5.55 \$5.50	75.53 194.67 71.53 64.62 77.63 (79.15)	も を た ガ カ カ カ カ カ カ カ カ カ カ カ カ カ	114.02 110.E3 75.0E 55.44 56.45 ( £4.57)	57.04 45.45 50.30 70.01 77.60 73.50	45.20 50.04 45.25 45.17 47.31 45.25	9,07 50,53 70,53 70,55 E1,40 (17,43)	31.76 55.60 47.43 100.33 123.060	104.42 167.14 127.22 136.13	57.24 54.46 53.63 170.47 (20.53
167	71.72 47.45 48.55 ( 70.47)	21.25 427.50 576.27 152.45 (162.16)	(5.0) (5.7) (9.73 (7.15 (5.42) (50.43)	42.91 112.44 147.22 307.34 167.20 4144.60	ES.ES. E2.37 134.30 124.30 E2.E5. 23.37	53.60 53.57 57.37 58.33 52.13 53.33	74.84 50.72 101.27 77.21 55.52 ( 75.21)	40,47 37,03 34,14 34,21	54.62 55.11 54.54 61.30 55.72 55.06)	157,13 207,62 177,17 171,17 151,77 151,41 (151,71)	106. N 67. 67 61. 43 115. 45 171. 47 (113. 52)	125.33 24.55 77.10 71.44 (313.77
5. 5.5 5.5 5.5	( 57.03)	114.70 132.82 153.75 131.77 137.70	43.% 42.97 42.41 41.73 40.55 (42.50	134.55 105.00 123.95 137.65 117.13 (124.00)	77.25 77.77 80.75 102.46 112.46 22.30	49.37 42.16 53.73 72.10 47.10	9.55 51.47 30.47 115.55 176.55	77.09 42.57 144.30 101.75 47.61 22.57	63.75 55.75 57.43 157.13 125.44 (100.63)	77.92 72.97 72.75 75.75 (10.75	173.07 143.54 223.77 214.17 237.75 (238.77)	72.55 78.54 73.15 62.67 671.75
27 27 26 27 31	54.22 66.58 54.43 54.26	197.32 35.47 24.50	5).53 52.55 55.53 52.33 52.33 (52.33	105.52 127.55 114.37 134.62 144.47	53.50 57.71 5.55 72.52 57.47 (71.52)	35.77 101.44 75.54 71.51	22.53 73.51 55.22 55.43 52.54 73.16 571.76)	55.55 42.00 52.12 57.50 57.55 (71.50)	55.03 74.13 71.65 105.21 20.37 ( 65.22)	102.41 101.65 26.40 25.40 102.72	25, 15 177, 18 19, 52 18, 57 18, 75	55.77 57.74 133.86 188.92 115.11
Tela War Var	2411.43 76.45 150,26	2535.47 57.52 427.53	1232,54 - 52,57 135,34	3410.74 113.52 202.34	2925.00 24.32 144.42	2207.33 73.55 132.40	2115.55 55.26 115.55	1834.25 58.22 144.33	2275.52 75.52 157.13	3155.7E 131.83 227.82	4837.97 153.27 343.54	1265, 47 105, 99 38, 38
Aiv.			3£ 35 Max. 127. 50	34, 45 34, 45 46, 42	75,55 75,687 103,42	1E E7 1E5 689 75.37	275 del 52,27	34.14 FG 689 37.25	<u>ы.</u> го Мір.	55 63 Mean 20,22	£1.50	57.74 Total 32731.25



JERAM TERAS

			nargé		3	((	C. 4. = 2	700. Ó	km) (Ur	it.mys	a	1975
day	JAN,	FEB.	MAR.	APR.	KAY	JUNE	JULY	RUG.	SEPT.			DEC.
12545 57690	(222.13)	13.23   73.71   73.71   73.75   73.85	155.47 161.63 165.31 177.19 131.37 117.46 113.23 160.39 131.25 160.39 131.25 160.39	127.35 73.77 70.81 50.44	27.43 130.32 20.62 153.46 132.65	95.49 120.40 92.53 62.65 6.74.50 77.75 63.51 73.21 74.23 103.37	53.77 43.83 40.74 40.24 37.24 41.60 42.45 52.44 52.44 52.44 52.44	43.23 37.01 37.01 36.02 34.02 34.97 32.07 32.07 32.07 32.07 32.07	\$9.76 70.37 57.49 55.59 \$9.20 41.33 41.33 41.33 41.33 41.33 76.56 76.37	47.03 41.34 38.23 38.23 38.23 38.23 39.24 39.35 112.50 12.50 12.27 55.42	55.60 52.62 42.85 55.65 12.54	311,27 244,42 202,43 191,64 1625,11 1225,11 1225,11 157,31 171,21 154,37 146,55
 11 12 13 14 15	225.35 175.35 227.35 227.35 227.35	42.77 42.40 42.40 42.40 43.40	83.41 83.41 81.41 75.51	66.72 56.33 56.73 56.73	102.97 65.33 22.34 128.52	22.45 24.13 23.27 23.27 23.26	17.22 17.45 17.47	45.15 32.97 22.58 26.04 25.63 ( 32.53)	93.20 83.25 46.63 55.11 172.71 173.33	57.34 103.08 23.43 142.43 113.42 1104.04)	201.24 113.50 65.3i 72.01	135,30 136,32 136,32 136,32 136,33 136,33 136,33 137,30
15 17 18 17 20	257.22 258.25 248.77 257.13 178.20 (241.31)	55.74 55.74 55.74	72. % 123. % 123. % 102. %	81.10 81.10 81.10 81.10	244.93 \$3.27 204.47 145.71	70.55 51.27 57.07 56.21	77.54 47.48 50.77 75.42	28.33 28.33 28.33 29.46	51.05 76.74 50.00 52.17	23.10 82.32 82.47 15.23	54.17 55.91 70.00 65.77 65.37 674.140	(150, 32) 362, 46 246, 40 163, 23 154, 72 137, 14 (221, 74)
2002 2002 2002 2002 2002 2002 2002 200	170, 52 154, 42 145, 33 137, 22 137, 11	51.44 140.44 1000,13 217,12	11.53 11.53 11.53 10.43	\$.77 \$.49 \$3.57 \$0.27	135.52 117.75 107.00 25.52	54.33 56.59 51.39 50.42 50.58	91.21 41.23 40.60 86.40	11.67 27.46 27.46 57.41	47.71 43.27 73.67 75.40 50.28	55, 62 51, 14 45, 25 41, 55 42, 77	£4.07 100.76 135.45 473.09 1104.50	162,72 162,72 152,42 137,66 221,64 224,50
25 25 25 35 35 35	117.02 109.25 109.25 109.24 27.41 24.21 22.10 (100.17)	231.£1 247.35 373.47	\$5.53 \$7.82 \$1.77 \$7.43 \$6.16 \$3.53 \$2.53	92.76 95.77 95.17 97.60 104.43	197.46 111.91 104.45 71.72 20.63 63.49 ( 26.50	45.87 45.87 45.87 41.77 55.79	17.81 37.31 84.65 123.73 70.51 47.22 (73.45)	42.00 43.81 43.75 43.75 43.75 55.76 (53.27)	72.77 162.45 74.74 77.59 42.73 54.41	33.77 71.74 53.67 52.44 57.45	415.92 2363.75 1240.63 451,22	137.27 144.75 127.19 113.42 115.42
Tclar Mean Mean Mean Mean Min	6719.82 215.77 422.57	3714.00 132.44 1500.13	3i12,54 100,40 127,12 53,50	3075.21 52.20 127.35 51.75	4121.77 132.25 3£3.27 53.42	71.17 71.17 120.40 41.77	1761.85 54,70 123.73 37.22	123£.79 37,75 57,83 25,55	1763.52 46.02 105.55 41.33	2242.85 72.55 147.47 34.07		5527.34 178.30 362.46 106.21
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