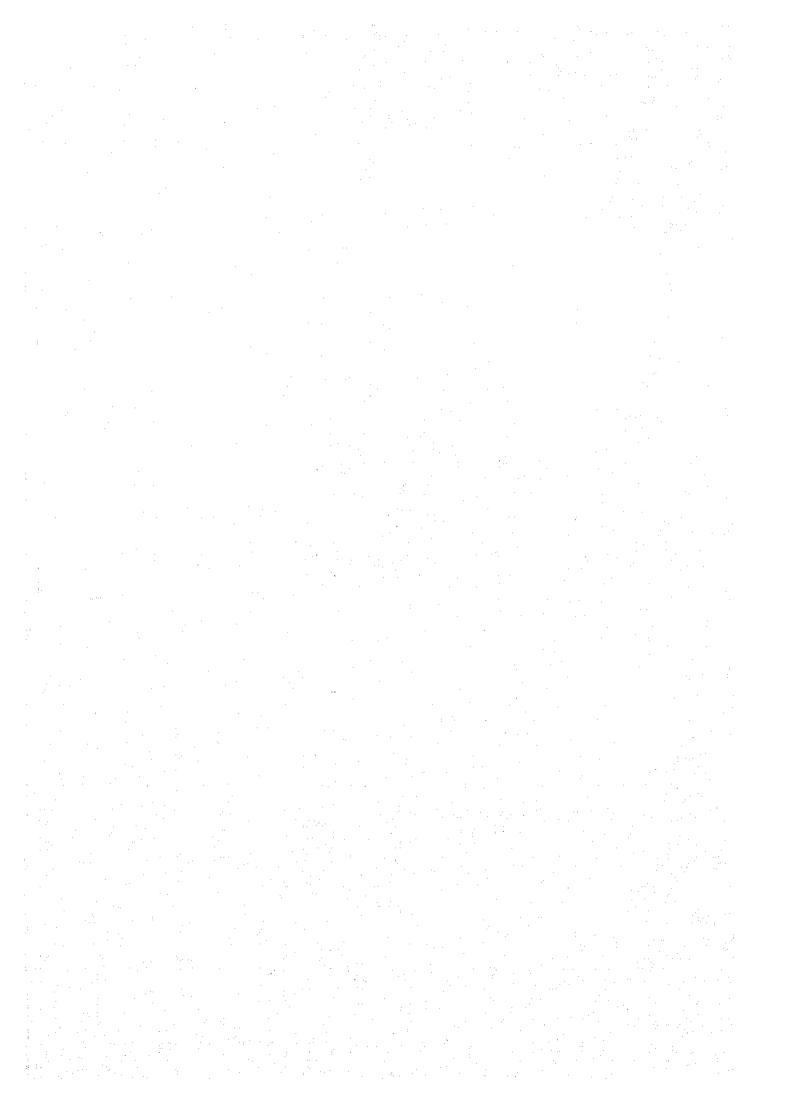
APPENDIX F

PLANNING OF FACILITIES FOR THE MASTER PLAN

(Ref.: Section 5.3., Vol. VI)



	0	s S S S S S S S S S S S S S S S S S S S								e e e e e e e e e e e e e e e e e e e			OM PORTO				and the second												
ndition	ain	Capacity	(m/s)			3.02				3.46					.:			2.94	929				4.40	06.				1.71	
Existing Condition	Existing Drain		E)			7.7×6.0 F				91x1.6V								111.48 W6.8x1.5	111.48 W16.6x1.7		 :		3.5 V,7×1.0	3.6×0.9				v5.6×1.3	
		Capacity	(s/ш)		29.65	97.62		4.84	4.84	97.62		28.61	37.50		11.48	40.40	40,40	11.48	11.48				12.25	13.07		9/.9	8.14	18.99	
	xed Drain	Velocity	(S/LL)		4 7.38	4 1.38		707 6	101 6	4 7.38		9 2/5	2.19	<u> </u>	27.7 0	5 2.07		4 /.44	4 1.44				0/./ 9	6 1.64		4/:/	0 1.22	0 1.48	
	Proposed		(m) (‰)		95 35×30 0.4	0	2.	V 20 X 1.5 1.0	1 5.0 × 1.5 1.0	V 23.5 x 3.0 0.4		8.8 × 2.4 2.0	U 45 x 24 2.0		1 25 × 1.8 2.0	W 6.0 x 2.6 1.5	11.2 x 2.6 1.5	30.4 × 3.2 0.4	V 240x 3.2 0.4				0.2 × 21 0.6	U4.1 x 2.2 0.6		5.4 1.7 1.0	25×1.8 1.0	U45x22 1.0	
	Runoff	L	(S)		128.88 V 235×30	125.73 WZ		6.56 22	6.56 14 2	27.59 42		35.32 11.4	41.00 14		15.12 12	53.10 14 6	5/56 56	147.18 2	142.05 12				15.57 54	17.51 14		7.84 42	9.99 14 2	25.20 44	
2000			(S/III)		96.68	87.30	:	4.67		88.79		25.48	29.36		10.84	38.01	36.64	102 55	98.06		 		//./5	12.48		5:70	7.23	14.61	
Year	Design	oilteo Per Ja) (111/S)		2 0.052	0000	- :	3 0.118	3 0.112	9 0.049		5 0.139	3 0.116		5 0./32	3 0112	2 0.107	3 0.047	3 0.044	·			4 0./30	3 0.1/8		0 0.176	9 0.760	2 0.108	
ië	əf	oeffici Storag			55 0.69	55 0.69		55 0.73	55 0.73	55 0.69		55 0.75	55 0.73	:	.55 0.75	55 0.73	55 0 72	55 0.68	55 0.68				55 0 74	55 0.73		55 0.80	55 0.78	55 0.72	-
<i>i</i> .=	nolitar	neonoo Tonus	2		1/2.9 0.	0		0	4	0 6,		0	5		7 0	0	0	7	0		 , ricanton and		8	0		19.7 0.	2	0	\dashv
	-	_ emiT				107.9 117.		36	2.4 40.	1.3/12		92 6	3.51		, 7 3/.	310 41	8 42	7 /25.	9 134.				1.8 31	7.0 37.		9.7 19	\$ 2 23.	2.1.42	
	Time of Flovin to the Drain	ch Total	(min) (min)		6.201			0.	3 5 30.	3.4 11		81 5.81	9.6 28		. 7 21.	Ŋ	8 32	4 1/5	2 /24	<u>.</u>			12 8 7	5.2 27.		0.7	3.5 /3	5. / 32.	
	Ë		* E		/0	400		52		670	<u>·</u>	7/	3600		27.	3900 2	4/20	4490 4	5250 9	River	 		27	14/0			006	098	-
	Length	ch To	(m)		(2000)	400	(T)	(00)	220	270 6	(O)	2360	240 36)	(2200)	300 39	220 41	370 44	760 52	Kelang	 · 		0/6	500 14	(9)	650	250 9	9	(J)
			(DU)		8	17450 4	-	<u> </u>	4/6	18/20 2) oZ	23	253 / 12	0	(2)	339.4 3	342.4		1	70 Ke	 :		40	105.8 5	70	9	45.2	8	07
	Area	Each Total	(na)		17300	159 17	7	39.6	20	245/18	7	183.3	69.8	7	82.1	4.2	3.0 34	27.4 2181.8	46.82228.6	7		-	85.8	20.0 10	7	32.4	12.8 4	14.0 165	7
-	oN v	wolin.								\odot						(O)		<u>ල</u>	}		 		- 1					(2)	
-	ON	əuil	+	<u> </u>	3		,	3	(0)	<u>)</u> છ		4)	9		3) (3)	_	(E)	(e)			-	<u> </u>	(3)		(e)	4)	<u>্</u>	
	inen ne	Nan O Oatch		///								7										N-2							

																		. Marianta	CHAPTER TO		-	-	******			~~~т		
	Renarks	7. C. S.																								111111111111111111111111111111111111111		
dition	gin	Capacity (m ³ /s)												.]		2.28	7.48	7.42						·				
Existing Condition	Existing Drain	Size Ca								-						V0.9×0.6	5.6×1.3	12 3.7×1.4									·	
	Drain	yCapacity (m ³ /s)	4.64	24.96				2.67		2.34	3 4.90				·	9.29	11.40	16.71		9/.9	1 21.41				-			
	Proposed Dro	Slope/Velocity/Capacity (%) (m/s) (m ⁷ /s)	1.2 1.13	0.9 1.53				0.3 0.60		0.3 0.60	0.3 0.68					1.2 1.34	1.0 1.32	0.8 1.33		0.8 1.05	0.8 /4							
		Size (m)	4.8 × 1.4	√ 50x25				¥ 20×1.5		4.8x1.4	v 2.5× 1.9					U3.0×1.7	U 3.0 x 2.0	E 40×2.3			1 45×25			-				
Q Q		off Storm Storm S) (m3s)	4.25 5.84	31.18				2.35 3.26	-	2.18 3.02	11 5.71					64 11.89	10.74 14.95	15.05 21.07		5.60 7.81	19.93 27.95	-						
r 2000		Perha Total Runoff (πγ̈́s) (πγ̈̀s)	0.174 4.	0.106 22.27				0.164 2.		0.195 2.	0.161 4.					0.183 8.		0.125 15		0.145 5	0.123 19			-				
in Year	ient ge tnei:	Coeffic Stora offico	55 0:80	0.72	:			0.65 0.75		0.65 0.79	0.65 0.75		,			0.60 0.79	0.60 0.75	0.60 0.73		0.60 0.75	0.73		7	:				
	jo noitori	emiT <u>E</u>	10 3 20 3 0	9 439				189 289 6		1 22.1	19.9 29.9 0					5 2/5	3 29.3	2 39 2		3 31.3	7.2 402060					i.		
	Time of Flo in the Droin	Each Tota (min) (mir	10 3 10	8 /	ver			18.9 18		12.1	1.0 19	River				// 8 //	7.8 19.	9.9		21.3 21.		River						
	ength	Each Total Each Total Each Total E Section (Min) (Min) (Min) (Min) (Min) (Min) (Min)	989	160 2020		1		(650)	3	400	40 440	Ke/ang				400	0/0/ 0/9			(7300)	90 1860	Ke/ang						
	Area	to T		210.1	70	1		G	70	Q ₁	25.5					47.2	9.72	120.4	70		162.0							
	٠	wollnI 면접 는	24.4	9/				14.		\(\text{\tinit}\\ \text{\ti}}\\ \ti}\\\ \tittt{\text{\tin}\text{\text{\text{\text{\text{\text{\texitt{\text{\text{\text{\texi}\tint{\text{\texi}\tintt{\text{\texi}\tittt{\texi}\tittt{\text{\texi}\text{\texi}\til\tex	0 1					47	24	4.5		3.8	(S)			1				
	1.745	Mam of Catchr Line	N-2 6			2 .		9		<u>(v)</u>	<u></u>				V-4	<u> </u>	<u>@</u>	(i)	:	4	(2)	7.4						
									*************************************				F	-2	•							- Care Table						

		negrajustiko, ink				46	(81=102)	^	S	60																1	
	-	Remarks			(976)	16) R2=046 (S=080)	, (3.10) , (C=5.44)	580=2)	(C=7.50 (0.32)	(0:20) S=080	(0.29)																
		മ്			R1=1.13 (8.46) 0=0.94	R1=7.43 (3.16) 0=0.22	22 = 7.09	R2 = 0.35 (C=0.85) 0 = 0:74	R2=1.61 (C=7.53) 0 =0.77 (0.32)	R2=2.31 C= 0 = 0.22 (0.	20				:												
ocition	ri Ci		(m ³ /s)		8.14	1.93	3.57			17.22																	
Existing Condition	Existing Ordin	0 0 0		1	3.0 X 1.1	V22×1.0	v 3.8×1.4			V6.0 x 1.9			<u> </u>														
L					7.67 ₩	5.65 V	7.79 V	8.72	81.01		13.16	1															
	Drain	1 3	(m/s)		1.52	1.57	1.64	1.68	1.68	1.72	987		-												1.		
	Proposed		(%)		1.5	0.7	6.0	0.0	0.8	0.8	6.0																
	P. C.	0	(E)		1.4×1.4	12.0 × 2.0	E 2.3 x 2.3	U 2.4 x 2.4	U26×26	U 2.7 × 2.7	U 2.8 x 2.8																
	Runoff	Major	E (%E)		3.24		9.85	10.05	12.57 #	13.43	15.72																
000	Rinoff	Total	Runoff (m/s)		2.35	4.52	7.08	7.22	9.02	9.62	11.26														1		
		0 1 1 1 1 1	Sto Sto Set (T/s)		0.223	0.200	0.180	0.175	0.75	0.766	0.162																
>	- tui	rage rage	otS teoD	·	0.80	0.77	0.75	0.75	3 0.75	0.74	0.74	:		3.					2.4								
j :	Ľ	Hor	in A		0.77	0.72	0.72	0.77	7 0.73	4 0.72	0.71		 _	 ::		7.75 2.75		1 - -		-	 						
	1	o eu	iT E mΩ(Γ		20.5	25.0	292	30.0	3/7	33	34.0	3.7	 						: -								
	Time of Flow	Total	(min.) (min.)		(8.7)	15.0	19.2	20.0	21.7	23.4	24.0						: .		:					i.			
	Time	E 17	(min)		8/	4.5	4.2	0.0	17	17	0.6						2										
	ļ.	1040	E		(565)	1120	1520	1595	1765	1930	0661		 . A.			20 10 20											
	Length	<u>u</u>	(E)		755	400	400	75	170	/65	90	1, 1 1, 1 1, 2 1, 2 1, 2							,								
	ed	1040	(hd)		(846) /0.53	(3.96)	39.34	(1.20)	5/51	(2.25)	(8.37) 69.50																
	Area	Ę Li	(ha)		2.07	8.11	218	0.74	0.77	4.17	3.20				14.1 1.1 1.		1,241 1,241 241 1,441										
	οM	МО	lìnI	1. 1.														1 V T				3. 3.			٠.		
)			9	(<u>0</u>)	ල ((1)	9	9	0								7.			Y.		- X	4		
	e inər	chr or name	ν. Cα	N-5																			4				

		Remarks			(RI=4.17 0=0.54)	ı,	R2 = 0.29 (X/=0.30) C = 4.84 (0.90)	C= 3.72	P.=1.02 (2.54)	C = 1.00 (0.08)(0=0.20)	S = 0.95	(R) = 225 RZ=2815=073)	S=6.06 (0=3.94)		C = 0.07 (3.82) O = 0.38 (0.09) (S=027)		C = 0.33 (1.12) 0 = 0.56 (0.17)	C=2:86 (/5.01) (O=0.56)										
Toyletion Condition	Drain	5	Capacity (m³s)		1.42	1.75	1.07	0.70								1.47	1.47	1.47					-	-				
Tyletino	Existing Drain		Size (m)		V2.9×0.9	V20×1.1	1.6x1.0	V1.3×1.2								2.7 \$	91.2	8		-								
	Drain		Slope(Velocity(Capacity (%) (m/s) (m ³ /s)		2.62	2.88	3.67			1.79	1.79	2.79	3.84	1 4.48		<u> </u>	9.33		_	-							_	
	-	ŀ	Slope/Velocity (%) (m/s)		1.0 1.30	12 1.42	1.1 1.42	1.1 1.46		1.0 1.18	1.0 1.18	0.8 1.21	1.1 148	1.1.154	ļ	<u> </u>	1.9	ļ	-			:	 					
	Proposed	-	Size Sk (m) (%	_	1.5×7.5	15×15	1 7×7.4			41.3×1.3	2.00 H1.3×1.3	1.6×1.6	11.7×1.7	1.8 × 7.8		1	\vdash	<u> </u>	1									
	Rinoff	Major	Storm (m/k)		2.92	302	Г	1 1		1.93	2.00	3.44	479		<u> </u>	 	<u> </u>		1_									
	2002	~ ⊢	Runoff (m/s)		2.11	ļ	62	(r)		1.40	7-1.45		3.45		<u> </u>			`	<u> </u>						·		_	
	Year		Setti Setha (戒s)		8 0221		 	1		0.80 0.244	0.79 0.217	8610 61	76 0.144		1	<u> </u>			0/:0									
		ieu Ide	onuR oitteoC orot& orot&	l	0.75 0.78	7	0	2 01		0.76 0.2	0.72 0.7	0.64	0.55	050 005	, ,	070	040	5 6	#/.0									i ·
	- (. 1	E Concer E Time		4 22.4	24				46/ 46	0 22.0		-	ì		27.6 37.6	J		4.04		-							
	me of Flow		Each Total (min)		1.6		, ,	+-		4.1	2.6 120	 - 		-	7 6	7 00	-	-	9.7				_					
			Total (m)	1	(755)	7035	0447	-		590	765	<u> </u>	1 0	1	7757	67/2		—	2540			-						
		Length	Each	" i .	00/ (0		1		البالم	270	9010	' '	1	1	7		1	٧	00						 		_	
		Area	Each Total		(9.30)		(1.20)			2.92 5.74		0.88 (8.36)			-	1	(62%)		286 723		-	-						
	01		wolłnI B 등		6	3 6	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \) <u>,</u>	(9)	2	0	0	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	`	00	2	,	2	7			<u> </u>		-		-		
			əuil		E	(%)	XC)(4)	 	ণ্ড	(0)	(E)	(%)	0	1		2	(3)		1			1		1.		
	ţŪ	e I Ou	Caich Caich Caich		0														1									

						nerge were		-	-		-	خصصت	po meson	quenus.	-	-	-	~~~		ويستحصون	_	*****	****	Trans.	7		-	, ,	~~~~
	() () ()	SA DILIAN		R1 = 0.32(0.16) C = 1.38(1.71) (0 = 0.65)	$R_1 = 0.09 (0.72)$ (C = 2.66)	R1 = 0.05 (2.26) (0 = 2.35)	1 = 2.73 (2.35)	I = 2.67 (S = 2.72) (S = 2.77)		C= 1.25	D= 6.44	I = 3.75 (C = 5.70)	I=6/0								and the same of th		- Targiti-L- i	THE PROPERTY OF THE PROPERTY O					
Existing Condition	Drain	Capacity (m ³ s)				0.56		2.08										!	1								·.		
Existing	Existing Drain	Size (m)				1.5 x 0.8		¥2.2×0.9					·											 					
	Ë	Slope/Velocity/Capacity (%) (m/s)		1.45	2.39	3.27	4.68	5.76		0.62	2.84	4.99	11.79	ļ						; {									
	sed Drain	be Velocity		1,72	6,	1 1.42	1	3 1.45	 	90%	601 9	97./ 9	6 7.56	ļ									: :		:				
	Proposed	Size Slope		B/2×/2 //0	1	17 97 x97 E	60 61x61A	W2.1×2.1 0.8		10.8 × 0.8	U17X1.7 0.6	WZ.1x2.1 0.6	29 x 29 0.6																
	Runoff	Major Storm (m3)		/.5/ B/	,				: .	0.60 116	3.01 11		14.08 U 2.9×29						\(\frac{1}{2}\)				,						
2000	Runoff	Total Runoff (m/k)		0//	2.09	2.87	4.07	5.04		0.44	2.18	404	6101																
Year	Design	Stored Settic Setta (m/s)		0.260		0.232				0.350	0.283	0.244	0.211								·			-					
in Y	fne	ioitteoO		19 0.81	83	71 0.81	69 0.79	70 0.77		90 0.87	90 0.30	85 0.78	75 0.77																
	uoitai	emit E mesmoo E tonusi		(8.8)		19.3 0.	21.5 0	24.1 0		/4.5 0	20.4 0	23.8 0.	24.3 0			2				7.7	4,4						* : .	-	
	Time of Flow in the Drain			(6.8)	9.1	9.3	11.5	141		4.5	10.4	13.8	14.3																
	Time in the			5 20	ļ	550 0.2	730 2.2	950 2.6	-	4.5	625 5.9	965 3.4	70 02	-															
	Length	Each Total (m)	1	(380)	- 1	20 5	180 7	220 9		265	360 62	240 06	20 970							1.						:			
	Area	Each Total E (ha) (ha) ((2.52)	(3.38)	(4.61)	`	(3.49)					(6.70)								, , , , , , , , , , , , , , , , , , ,								
	:			7.70	0.00	0.03	2.73	2.67	F	7.25	6.44	3./5	(8)	37															
L		Line wolinI			(2)	ල ල	4	(2)	6	(a)	(Z)	8	9						:										-
ţ	nem nem	Nar Catch	N-7)		9	· ·)		9)			;	:											S. 134	

			Acceptance (All Carlos	Service Marie		***************************************		-		_	T	.سر		T	T 1		1			T				1	
Remarks																					60	3663' = 342.1 +37.4 25.50			
Existing (Existing	Size Stope/Velocity Capacity Size Capacity (m) (%) (m/s) (m ³ /s) (m)	7.9 2.2 0.4 0.90 9.77 WD8XO.9 1.05		030×19 0.6 1.00 8.05 5 5/x09 1.14	70.7	2.8 × 1.7 0.6 1.33 5.57		2.9 × 1.8 0.4 1.12 5.17	80x25 0.5 1.23 28.37 V.9X10 1.0					0.5 0.95		V 30x 2. 0.8 1.21 1.18	0.5 /.	188x2.4 04 1.07 23.48 5x1.1 1.47		1.0 1.14 6.16	5 0.4 1.12 28.35	V 40×25 0.4 1.12 28.35 V 1.1×1.7 3.77			
in Year in the Drain of Elow of the Drain of Flow of the Drain of the	Each Total 音 音 記憶 さば Perha Total (mus) (mis) (mis) (mis) (mis) (mis) (mis) (mis)	0000	2	31.1 41.1 0.60 0.73 0.121 7.15 10.07	(500 1.0 33.0 43.0 0.60 0.72 0.116 (6.33 23.03 0	15.4 15.4 25.4 0.60 0.77 0.165 5.31 7.37 3		6 94 □	1800 4.1 37.7 47.7 0.60 0.72 0.110 28.05 39.32 1	River				20.8 0.50 0.79 0.154 8.87 12.26	1520 18 5 29 3 39 3 0.50 0.73 0.104 9.89 13.85 W	02 07 121 0 000 000 000 0	14.0 14.0 44.0 64.0 0711 05.05 18.25 18.51	49.0 59.0 050 0.71 0.078 21.89 31.15		17.9 17.9 27.9 0.50 0.76 0.130 5.77 8.01	2960 3.1 52.1 62.1 0.50 0.70 0.075 25.34 35.74 1	3460 77 598 698 050 070 0.069 25.27 35.82	ng River		
No	Margine Cach Total Each Total (ha) (ha) (m) (m)		78.4 (20)	59 /	(3) (4, 3, 14, 8, 100, 150, 150, 150, 150, 150, 150, 150	2	70	32.2 /200	(6) 48.8 255.0 300 78	To Kelang			6-1/	380	ə -		74.6 500	(2) 68.3 280.7 / 240	() To ()	(6) 44.4 (1200)	7 337.8 200	5 366.3 500	To Ke1		

	**************************************			e-midros			OUT HALL			NA FEECO							90001A36	Comment lays										
		Refiliar Ks																:										
	Ω	v Ľ	٠.	;																		-						
odi+ion	ain	Capacity	(S/m)		1.29	2.48	2.36			:		0.79	1.72			13.23				1.78				69.1				
Existing Continue	Existing Drain	l	(m)		V 2.8×1.1	5.0 X 1.5	5.8 /x/3					2.9 × 0.6	21×6			12.5 U3/x/.6				1.5 x 1.2				1.6×3.0		•		
<u>1</u>		L			7.26 ∨ 2	8.20 4 5	3 × 91					13.87 42	387 V29×1.2		4.39	1848 13				A 69		 	<u> </u>	: 52 40.				
	Drain	City Cap	(s/m) (s/		0.84 7.	8 98						ļ	1.42 /3	<u> </u>	1.25 4	1.51		<u>.</u>		1.60 3.		 		2.44 //				
	Proposed	SlopeVelocityCapacity	(%) (m/s)	· · .	0.4 0.6	0.4 0.4						1.1 1.42	1.1		/ 9 /		-			14 1.				1.9 2.				
	Prop	l	(E)		U30×20	V 35x 2.0						7.8 X V	9.8 × 0.9		V 4.2 × 1.6					97×97 n				4.66 UZ 7×20				
	Runoff	Major Storm	(W%)	.	8.81	10.53 14						7.48 W	17.63 W		5.67 V	24.12		•	. :	4.85 L				14.66 E			•	
5	-		(%) (%)		6.28	7.50	8.23		:			1263	12.62		4.12	17.40				3.55				10.67				
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Design	Storot2 Silteo Paris Paris	(m/s)		0.081	0.067						0.776	0.000		0.735	0.098				0.307				0.198				
>	JUƏ	ioilli90	O		40 0 73	40 0.71	40 0.71					40 0.78	40 0.75		.40 0.82	40 0.75				75 0.88				0.55 0.84				
-=	- }	mit Concent Tonus		.,	41.10.	53.9 0.4	57.4 0.					23.3 0.4	29.7 0.4		18.2 0.4	30.8 0.				13.7 01				16.3 0.				
			_ [37.7	43.0						13.3	19.7		8.2	20.8		<u>.</u>		3.7				6.3				
	Time of Flow	Each	(min) (min)		3/./	12.8	3.5	River				13.3	4.9		8.2	1, 1,	River			3.7	River			6.3	River			
	Length	Total	E)	: 4.	O -	1440	1620	Kelang	•							1410	2 * 5				26				Kelang			
	Lei	al Each	E	-	(75æ) 800	0 840	6 180					(00//)	5 530		380	15/100	Kelaxg			350	Ke/a 19			(906)				
	Area	Each Total	(ha) (ha)		77.5	34.5 //2.	16.6 128.	70			-	6.801	8.6 127.	70	30.5	10.5 177.	70			8.	70			53.9	7.0		-	
-	oN /	wolin Щ	-		7	Ö	7/					0/			(3	(S)								ۍ	:		-4-	
-		əuiJ			9	0	<u></u>			<u> </u>		9	(<u>o</u>)		(i)	4								0		•	- 1	\dashv
	an Inam	Man O Aoto Mar		5-1						- Colo	5-2	:							5-3				5-4					

process	ingens estat		~						-	T		******			T	-carox	en cualum f				T			~~~~~					٦
	Remarks												ro Retention Pond																
Existing Condition	Drain	Capacity (m³/s)		0.51	2.60	///		0.83				0.51					0.15	0.63				1.48	2.21	61-1			0.40		
Existing	Existing Drain	Size (m)		1.0 1.0×5×0.8	U 1.8x1.0	2.7x1.2		4.5×12				₩ 2.9×0.8					V 2.7x0.4	¥ 29 ×0.9				V52x12	3.1 √.0x2.1	V 5.1×1.2			V 4. 1×0.8		
		Capacity (m?s)		3.25	7.33	12.74	: _	6.33	17.46			8.47	25/6	25.16		<u>.</u>	432	12.45	13.53			32.81	32.81	32.81		8.87	34.89		7.13
	ed Drain	Slope/Velocity/Capacity (%) (m/s) (m ³ /s)		1.04	1.22	1.24		1.18	1.25	-	-	5 0.93	1.27		-		5 0.80	5 101	5 1.03			3 1.04	3 1.04	3 1.04		0.5 0.96	3 1.06		0.3 1.09
	Proposed	Size Slope (m) (%)		4.1x1.3 1.2	11 8.8 x 1.8 0.4	4 5.8×2.0 0.3		H3.4×1.8 0.4	V 8.5x2.1 0.7			7.6 4.0 × 1.8 0.	12.1 7.5 x 2.3 0.6			:	V 2.0 x / 7 0.5	93×1.9 05	45×20 0.5			V.5.6 0.3	15.6 10.0x 2.8 0.3	V5.6 0.3	*		15.8 0.3		
	Runoff	Storm Si (m/k) (r		4/1 4.5	9.65 HB.8	1680 145.8		8.33 H 3.4	22.23 VS.			7.5	33.27 47.	3			543 W 2.	552 4 5	7.41 25			43.68 NO.	42.06 210.	40.17 NO		1.35 43	45.87 200		9.20 137×20
2000		Total Ma Runoff St (m/k) (m		299 4	693 9	1.98 16		5.96	5.74 22			7.32 10.	23.48 33				3.87		1240 1			30.93 4	29.53 4	28.32 4		8.11	32.15 4		6.64
	Design Ru	Perha Tr (mys) ((0.187	0.154	0 /23 /		0.120	1010			0.123	0.093	-		: .	0.145		0.112			0.121	0.093	980.0		0.750	0.083		0.182
Year	əg tuəi	Coeffici Storag Coeffic		0.82	0.77	5 0.74		5 0.73	0.72			5 0.77	5 0.71	 			55 0.76	5	55 0.73			55 0.72	65 0.70	65 0.70		65 0.74	65 0.70		65 0.77
<u>.</u>	noiton †	emiT E Emonox FonuS TonuS		179 0.55		`		36.1 0.5	47.1 0.55			25.8 0.4	4				27.1 0.5	4	40.7 0.5			45.9 0.6		74.1 0.6		33.2 0.6	77.3 0.6		24.5 0.
	Flow	otal min)		2.0	74.2	25.1		1.92	37.1		:	8.5/	404	43.2			1.4.		-	er		935.9		4 64.1		2 23.2	2 67.3		5 145
	Time of in the D		1	6.1	٠,	`		26.1	077 00			15.8			Q		1.0.1	1400 10				35.9	2400 19.8			23.	3/00 3	-	1/4.
	Length	Each Total (m)	1	480	455 93		-	800	800 2600	D	-	850	400 3000	50 3050	1 · · · · ·		800	·		Kelang		(220) 7200				(38) (00)		®	(930)
		7			45.0	47.6	70	/	1.951	70			25.28	252.8				94.3	8011	70			317.5	329.3	70		387.4	To	
	Area			08/	29.0	52.4		49.7	0.6			59.5	-	╁╌			270	67.3	7.6.5			2556	619	811		54.	0.4		36.5
		wolini							(O)			_	(1)				1								, 		(<u>()</u>		_
		ריוטפ		9	(0)	(O		4	9			(S	0	0	_	_	G	<u>@</u>	0			9	<u>@</u>	<u></u>		a	9		9
	ueuı ie	Nan O Catchr		5-5								5-8					1-5					5-8		: ~_					

	<u>.</u>	S E S S S S S S S S S S S S S S S S S S																						La constant			
ondition	rain	Capacity (m3k)		0.7 /	887			-	98.0		3.02	1.83		1.51					1.54		0.24	2.47					
Existing Condition	Existing Drain	Size	+-	V.5.8×0.9					V 22 0.8		1.7×9.7F			5.5 V 7.5×0.8				-	5.8 4.7×8.4		2.0 x 8.0 U	6.9 x 1.6					
	Drain	Capacity (m ³ /s)	100		1	<u> </u>			482		5.90			7.26	7				9.07		5.47	18.23		19.8	27.03		
	1	Slope Velocity Capacity (%2) (m/s)		ļ	ļ`	ļ			0.8		0.6 7.33			0.7 7.02	0.6 1.42				0.4 7.29		0.6 1.31	0.4 1.53	·•	0.6 1.48	0.9 1.55		
	Proposed	Size Sk		↓		<u> </u>			0 001×83F	1	U2.5 x 2.0 0			4.7 × 6 0					0 02 X 0 X E		0 6:1 × 5:2 Fl	US4x250		H3.3 x 2.0 0	10.5 × 2.5 0		
***************************************	Runoff	Major Storm (m/k)	1000		58.25				6.04		7.86	20.95 V	7.12	8.62	28./7				17.95		6.77	23.84		11.26	32.33		
2000		Total Runoff (m)%)		ļ	ļ				4.36		569			6.25	20.24				8.62		4.88	19.12		9.76	23.74		
Year	Ē Design	Serric (m/s)	1110	L	L		·		8810 8.	L	8 0.191	6 0.172		161.0	691.0			-	5 0.78		16 0.184	24 0.163		8 0.206	0./60		
Ē	insi	TonnA Soeffici Storagon SorofS	0.45 0.41	0.65	0.65				0.65 0.78		0.65 0.78	0.65 0.76		0.65 0.78	0.65 0.76	-			0.70 0.75		0.70 0.76	0.70 0.74		0.70 0.78	0.70 0.74		
	10	amiT E	٦	0	_				4 23.4	ļ	7 22.7	9 26.9		7 22 7	. / 28. /			 	5 28.5		. 2 27.2	4 32.9		6 22 6	9 33 9	- 1	
	Time of Flow in the Drain	Each Total (min) (min)	27 6 42	0	2	River			13.4 /3.		12 7 12	3.5 /6		12.7 12.	1.2 18	7			18.5 /8.		17.2 17	4. 4 22.		12.6 12.	1.0 23	River	
		Total (m)	2500	3400	_							1290			1390	19 River						0/9/			0067	at	
	Length	tal Each a) (m)	1 ~	~	7	O Kelang			00//		000/	7.8 290	i	(740) 540	2.5 100	Kelang			(1400)	(O	(2007)	0	7.1	(080	1.6 90	> Kelar	
	Area	Each Total (ha)		0	0 53	70			23.2	70	29.8	34.8 87.	70	32.7	0 120.	70	:		48.4	170	26.5	30.11.05	70	39.6	0 /44	7.0	
		Puil wolini		(6)				- 1				9			9							9			<u>ම</u>		
		Non O Noteh	5-8	(<u>@</u>)	6	:		8-9	()		(Z)	ව)		(4)	(G)			2-10	\bigcirc		(9)	<u>ව</u>		4)	(5)		

A-1, A-2

generation of the second	Red A S X S																						Service of the servic			Andrew Andrews		
ndition	Drain	Capacity (m ³ /s)			0.58			1.95			2.20	1.14		4.40	0.38	-	2.97							2.45		1.69		
Existing Condition	Existing D	Size C			3.3 V0.8×0.7			7 12/8×1.3			4.7×1.2	U 7×1.0		43.5×1.2	34.88 V 0.9×0.7		V 70x18							9.92 V 5.7x12		V 5.7×14		
		Capacity (m½s)		20.23	20.23	*	27.43	28.37		9.26	37.64	36.43		8.05	34.88		4.32		7.74	34.88						5.75		2.95
	ed Drain	velocity (m/s)		9///	9// 2	-	7 7.28	5 1.23		8 1.15	5 1.25	5 1.27	_	8,/9	1 0.64		5 0.80		3 0.76	1 0.64			 -	0.6 7.53	_	0.4 1.15	_	91.19
	Proposed	Size Slope (m) (%)		V5.5×2.5 0.5	105 x 2.5 0.5		W75×20 0.7	V/3.0 × 2.5 0.5		V3.0×1.9 0.8	V 90×25 0.5	W/55×25 0.5		U30x19 0.6	25.4×2.7 0.		J 20×1.7 0.		V35×2.1 0.3	¥25.0×2.7 0.		-		43.5×210		U 3.0×1.9 0.		U1.9×1.5 0
	Runoff	Major Storm (m³s)		25.85 🗵	25.24. ₩	**	26.58	36.94 ₩	-	70.28 V	40.66	48.20 🗤		10.23	45.5/ U		5.69		10.52	43.71				13.18		7.60		3.87
2000	1 1	Total Runoff (m½)		18./3	/7.63		19.00	25.75		7.34	28.23	33.78		7.21	, 3/.8/	:	3 4.02		7.41	30.84				1 951		5.44		4 2.80
Year	Design	Serha (㎡s)		2 0.042	0.040		3 0.082	ľ !		3 0.087	9 0.037	8 0.033		1 0.065			2 0.073		8 0.019	8 0.019				77 0.15,		15 0.132		30 0.74
in Y	tne ent	fonuA Softioi Storog		0.40 0.69	040 069		040 0.73	0.40 0.69		0.40 0.73	0.40 0.69	040 0.68		0.40 0.71	040 0.48		0.40 0.72		040 0.68	0.40 0.68				0.55 0.77		0.55 0.75		0.55 0.80
	lo noitui	етіТ <u>Е</u> толоо <u>(Е</u>		99.1	,50/		404			2 36.2	3 //53	3 /323		6 55 6	7		2 472		8 254 8	245.3 255.3				6 25.6		6 31.6		1 20 1
	Time of Flow in the Drain	Each Total (min.) (min.)		1,68	60 95	<u> </u>	30.4	L `Y		26.8 26.	3 105	170 122	٠.,	45.6 45.6	LY		372 37		244.8 244.	0.5 245.	_			156 15.	1	21.6 21.6	:	01 1.01
		Total Ea (m) (rr		3				0661		?	2230	3500		. 1.	4840				2	4860	Rive							
	Length	Each (m)		(8000)	7 400	(9)	(2250)	062 9	<u>૭</u>	(30)	9 240			(2650)	2 1340		(1950)		(1)000)		Aur			(/400)	4	(/450)	4)	(000)
	Area	Each Total (ha) (ha)		43/.7	0 440	70	231.7	5.2 677.6	70	84.4	762.	260,7,023.6	70	6.011	43.711782	70	55.0	170	389.8	0 1623.0	70			0.88	70	41.2	70	1,6/
	.oN	wolinI 页 完		4			"	9		- 3	(E)	~			7				35	99	-	:						
		Marr of Catchr Line	A-1-	0	9		(<u>()</u>	(O)	,	4	9	9		6	<u>®</u>		(G)		<u>(0)</u>	(C)			A-2			0		(6)

Particular	and tracks			<u>later</u>		cycloser size I					*********					1	ALCO ACC			e proposition de la constantina della constantin				ctly.		- Former	
	0 0 7 1	\$:										Port Area (2856) is directly drained to the sea.	:		
	Ω																							Area (28 ined to			
-		>		<u></u> .																				_	:		_
Conditio	Drain	Capacity (m³/s)	0.58						0.85			0.58			76.0	0.83	. ,							10.98			
Existing Condition	Existing Drain	Size (m)	V5.0×1.4						1./x8.E.v	6.09 V28x0.8		2.33 V 3.1x /.0			4.0×0.8	5.9×0.6								7.0 X			
		Slope/Velocity/Capacity (%) (m/s) (m/s)	16.46				2.46		4.62	6.09		2.33		5.58		/4.55		:			4.74	8.07	/3.45	15.81			
	d Drain	Slope Velocity (%) (m/s)	1.62				0.64		0.75	1.17		0.00		1.24	1.34	1.07			<u> </u>		1.27	1.25	7.32	7.42			
	Proposed	Slope (%)	0.5				40		9.4	. 0		0.5		0.5							9.0 1		601	011			
	Pro	Size (m)	± 5.0 × 2.3				4.5×7.5		5.9 ×1.7	₩3.3×/.8		11/8x/5 0.5		U3.2×1.6	₩4.3×2.1	41x23					□ 2.5×1.7	山 3.5×2.1	V 3.5×2.1	082×21			
	Runoff	Major Storm (m/s)	27.83				3.17		5.84	8.19		3.77		7.37		19.20					6.24	10.64	15.21	45			
2000		Total Runoff (m/s)	/5.63				2.30		4.20			2.23		5.30		1					4.52		10.88		·.		
Year	Design	Perha (m³s)	0.117	-			0.237		0.766			0.783		0.221						,	0.269			0.192			
×	96 Iuei	Storac Siffeo	0.73				0.82		0.74	0.72		0.76		08.0	0.74	70 0.72	· ·		11:		0.80	5 0.75	5 0.74	0.74			
Ë	ļ	Tonnol	9 0.55				0 0.70		8 0.70	8 0.70		\$ 0.70		3 0.70	5 0.70						1 0.85	0	0 0.85				
		emiT E	37.		-		0 18.0		31.8	43		4 274		3 20.	34	47.					, 20.	30.	0 34.				
	of Flow Drain	Each Toral (min)	27.9			 	69		27.8			17.		.0/	245						10.	20.	24.	24.			
	Time of in the D		6.3	ه			8.0		2/8	0 27		17.4		10.3		4.0					10,		6,		<u>ک</u> نو		
	gth	Total (m)	2087	RIV						1520			11.		1560	1810	Rivel					7550		1	RIVE	14 1	
	Length	Each Tota (m) (m)	88	Aur		 	300	(D)	700	920	\bigcirc	(00)	9	(750) 570	560		Aur				750	800	230	50	Aur		
	Area	Total (ha)	/33.6	70				70		43.3	70		70		63.6	6.901	70					36.9	56.	72.4	70		
	Ar	Each (ha)	13.3				9.7		25.3	6. E.		12.2		24.0	27.4	0					8.9/	20.1	19.2	16.3			
	DN 1	wolini	<u> </u>						:						4	(O)											
	٠.	əuil	4)				9		<u>@</u>	(O)		4		৩	9	(1				9	0	<u>ි</u>	4)		·	
	men ne	Vatch O Vatch Vatch	A-2			A-3														A-5							

																							فجمتند.	este este			A-	4
	Remorks	WALLO SEEDING WALL		(C=7.50) C=7.26	(= :,43)	R/=0.08	K1 = Z30 (173) S = 1.14 (186)	R1=0.75 (5.00)	3000	C = 2.41 (3.03) (R2 = /24)	(2.61)	K = 0.62 C = 2.06	بخ	(I=5.50 C=1.03) (0=250 R/=311)						-								
Existing Condition	Drain	Capacity (m ³ /s)				0.00	4/.7						5 0.58															
xisting	Existing Drain	Size (m)				0.6 x 0.5	U.2.9×0.7						V 7.2×0.5													-		
111		Slope Velocity Capacity (%) (m/s) (m/s)		1.09	2.35	4.38	5.65	7.72		1.97	2.59	2.84	8.90	10.77					: :									
	d Drain	Velocity (m/s)		7.00	1.76	1.22	1.30	1.36		0.86	1.00	60%	1.36					_	_									
	Proposed		 	0.0	5 0.8	3.0 0.6	2.2 0.6	2.4 0.6		6 0.4	7 0.5	0.6	27 0.5	Ö	 -												- :	
		Size		/ / x / / n	U/5×1.5	H 2.0 × 2.0	U 2.2 × 2.2	12.4×2.4		9/x9/n	3.06 W.7×1.7	11.7×1.7	U2.7×2.7								.							
	Runoff	Storm (m/s)		7.13	2.65	5.35	663	8.23		217	3.06	3.56	11.13	/3.62							·							
2000	Runoff	Total Runoff (m/s)		082	1.92	3.87	4.78	5.93		1.57	2.21	2.56		\Box			:						:					
Year	. — .	Settio (R) (R)	~	0297	0.266	0.262	0.219	0.215		0.235	0.234	1120																
	96	Coeffic Storae Coeffic	1	0 0.81	0 0.78	0.7	2 0.76			4 0.77																		
Ë	rioitori	mennoo <u>C</u> TonnA	-	(18.3)	22.3 0.90	23.0 0.90		26.8 0.8	-	24.5 0.84		30.3 0.86	 	+														
	f Flow Drain of	Total (min)		(5.3) (1	-		L			(90) (
	Time of in the D	Each Total (min) (min)		4.8	0	1 7 7 1	3.4	9.0		7.5		ď	,	0.2														
	Length	Totai (m)	-	(200)		<u> </u>	ļ			(285)		1 .		—					:-									
	Len	Each (m)	3 '	06/ 9	5) 235	3	N			29) 365		,	1.0	1													1	
	Area	Each Total		200		ന		0.75 27.59	.:	241 6.70	30	- -		(/2./4)								-		1				
-	ON	wolini 명 도	1	*	"	0	بې دې	0	<u></u>	2	0	-	(S)	1	 		_	-					-		-			
		Puil		0	(3)	0	4	9	- 4	9	(E)	(0)	(0)						-			-			-			
	e tuent	Man o Calchi	A-4																					:				

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	υ 2 2 3 0 0	64 171100												9591 = 366.9+870.1× 0.30									· .							
Existing Condition	Existing Drain	Capacity	(m³/s)	÷																										
Existing	Existing	Size	(m)							·				·																
	'n	Capacity	(m³/s)		9.05		13.74	96.61		11.25	·	9.05	16.48	26.01					8.42	٠	6.84	16.89		427	18.15	-				
	d Drain	Slope Velocity Capacity	(m/s)		0.68		0.74	0.78		1.05		0.68	1.16	0.87		÷			0.67		0.74	0.0		0.82						
	Proposed	Slope	(%)		02		0.2	0.2		02		0.2	0.2	0.2					0.2		0.3	0.3	-:	0.2						
	Pro	Size	(m)		8.8 V 40×24		1.0 × 2.5	12.7 v 7.5 x 2.6		U4.9 x 2.5		-88 V 4.0 × 2.4	₩ 5.5 x 3.0	139×32				,	8.6 × 2.3		8-83 V 30x21	1.0 × 2.5		130×20	11.2 x26					
	Runoff				12.22		19.30			14.40		11.42	21.11	34./4					10.74		8.83	20.35		5.46	22.38 ₪					
2000	Runoff	Total Runoff	(m/s)		8.38		65:11	16.07		10.22	:	8.01	/4.82	23.98		-			7.54		6.25	14.36		3.93			-			
Year	Design	orogen Original Original Original	(m³/s)		0.030		0.040			0.084		0.053	0.047						0.069		0.081	0.063		0.126		L				
Ϋ́e	et ient	orot oit te	o) S	i	0.68		0.69	0.68		0.72		0.69	0.69	0.68					0.70		0.77	0.70		0.75	0.69					
Ë	tnə	ionu ioiite)OO		0.45	-	0.45	0.45		0.45		0.45	0.45	0.45				:	0.50	V .	0.50	0.50		0.50	0.50					
		emiT meono		: ,	6.991		0 /2/	210.9		45.9		84.9	6 66	2/2.5	:				69.3		56.2	79.5		29.0	82.3					
	Time of Flow in the Drain	Total	(min) (min) (-	6.951		0 ///	200.9		35.9		74.9	89.9	202.5				10.0	59.3		46.2	69.5	-	0.61	72.3				e e	
	ime of	Each	min)		156.9		0 7//	44.0		35.9		74.9	15.0	9.7					59.3		46.2	10.2		19.0	20.00					
		Total	(m)					5340					3540	5420	RIVE							2630			2780	River				
	Length	Each	(m)		3340	(S)	(4750)		©	2000	(0)	(2950) 2540	(30)	80	PU104	1.			2100)	(O)	(2000)	530	(2)	(006)	150					
	0		(ha)		٧	70) 'Y	. [70		70) · '	3/5 3	959.1	TO A			:	×	70	-	229.9	70	¥	260.7	TO A	1	\ <u>-</u>		
	Area	Each Total	(ha) (2.662		299.7	63 8 642	-	4.721	7	15/2	42.43	6 / 1	7				8 601	<u>'</u>	77.1	41.52		31.2	9		- 1			
	οN	wolt	υI					0					4	(O)								9			(3)					. !
	οN	əui	1		9		(2)	(e)		4		9	9	(1.						(<u>0</u>)	<u></u>		4	(3)	4	77	_		
	ueu Je	noM o otch	Ċ	1-0														0-2	; ' .	:							: 1).* : :			

					aio orașa	*****	****	manaém		nace of			******			Y				CV-MC-CPG	-	T				445-300		30.178.00	7
	Remarks												10729 = 2085+1008.3×0.35										589,8 = 77.8 +597.4× 0.35						
Existing Condition	Drain	Capacity (m ³ /s)										-																	
xisting	Existing Drain	Size (m)																			1.								:
		Capacity (m ³ /s)		15.77	16.96	20.36	21.77	21.77		121		3.10	21.77					26.34		6.30		2.38	3//3						
	ed Drain	Siope Velocity Capacity (%) (m/s) (m/s)		0.77	0.79	0.83	0.84	0.84		0.74		0.86	0.84					1.03		1.05		3 0.83	3 1.08						
	Proposed	Size Slopa (m.) (%)		11.4 2.7 0.2	W 60x 28 0.2	50×3.1 0.2	V 65×31 02	12.7 3.1 0.2		8.0 91×91A		U2.0 x 2.0 0.3	V 6.5 x 3.1 0.2					V 6.0 x 3.2 0.3		H 2.8 x 2.5 0.3	:	12.2 × 1.5 0.3	V 6.0 × 3.5 0.3						
	Runoff	Storm (m/s)		20.01 46	21.94 46	24.64 V 6.0×3.1	26.93 ₩ "	27.12 ₩ 6		2.24 147		4.03 42	28.23					33.19 14		8.10 113		3.04 112	39.71 √						
2000	Runoff	Total Runoff (m/s)		/3.96	15.30	19.39	18.68	1844		7.60		16.2	16.31					23.48		5.74		3 2.20							
Year	Design	Oeffic (π/s)		9 0.037	8 0.026	8 0.023	8 0.020	8 0.018		0.085		77 0.099				-		0.70 0.049	t l	0.72 0.066		0.76 0.093	0.70 0.047		· ·				
ř	ient de	Puno Soeffic Prord	1	0.35 0.69	0.35 0.68	0.35 0.68	0.35 268	0.35 0.68	·	0.35 0.75		0.35 0.77	0.35 0.68					0.35 0.		035 0		0.35 0.	0.35 0	•					
	10 noiton	emiT E		5 97.5	5 /49.5	8 175.8	8 202.8	7 223 7		8 30.8		9 23.9	3 225.3					6 88.6		0 45.0	·	9 26.9	5 71.5						
	Time of Flow in the Drain	Each Total (min) (min)		5 87.	0 /39.	3/65.	0 192	9 2/3.		8 20.		9 13.	6 2/5.					6 58.		0 35.		16.9 16.	6						-
		Total Each (min)	T	87.	6280 52	7530 26	8830 27	9830 20.		20.		.67	0/66	River				58.		35.		1/	3680				-		
	Length	Each (m)		3900	2380	1250	/300	000/	(b)	(006)	(A)	200	80	20/0h				3500	(O	2/00		800	180	Besar					, i
	Area	Each Total (ha) (ha)		377.3	211.0 5883	168.0 756.3	6	90.2 1024.4		8.8	70	20.4	0.3 1072.9					0,	70	0.48	70	23.7	0 589.8					-	
-	ON A	wolinI □ 교 은		37	2/	9/	67	6	<u> </u>	1		N	(9)		1.5		-	479.		' 0		~	9		-	-	-		
-		əuil	-					(1)					(4) (4)			: 		(i)				(C)	<u>ි</u> ල	7	-		-	-	
	1.	Man Catch	0-3	3	D	(2)	(S)	(d)		(5)	1.	9	O O				0-4			\mathcal{D}		(<u>2</u>)	9						

								winomunist	- maintone	,carcaro	and the same					-		processor.	وحبيضام	·	.,	-	_		 1	 ********	-
	v 2 2 0 0)					-						1354,0 12 = 303,3+1751,1× 0.30														
Existing Cardition	Drain	Capacity (m ³ /s)								·																	
Existing	Existing Drain	Size (m)																									
=		Capacity (m ³ /s)		16.89	18.15		10.56		3.53	32.12	33.76	33.76	33.76														
	ed Drain	Slopel Velocity Capacity (%) (m7/s)		0.01	0.93		, 03		0.89	040		0.0		-													
	Proposed	Size Slope		0x25 0.3	0x2.6 0.3		6x25 02		UZ.1x2.1. 0.3	V.00×3.1 0.2	54 3.2 0.2	V/0.0 x 3.2 0.2	5.4 3.2 0.2													-	
	Runoff	Major Storm (m/s)		23.33 V 60x 25	25.61 J. 1.0x2.6		14.85 H 4.6x 2.5		4.67 82	42.93	46.19 W 10.0x 3.2	42.38 W	42.30 V 16.4 x 3.2												-		
2000	Runoff	Total Runoff (m's)		16.20	_		10.54		3.35	29.79		28.86		ļ	- 1							·					
Year	Design	Stords Settle (T/s)		0.052			1010		0.737	0.037	0000	3 0.022										····					
n Y	fn9	Diffic Coeffic		0.50 0.69	50		50 073		50 0.76	50 068	50	50 0.68	50								-		-				
	10 noitori	emiT E mexmoo(E NonuR	-	99.1	3		4/0		27.2 0.	149.4 0.		267.3 0.	273.0														
	of Flow Drain	Each Total is the Comin (min) (min)		1.88.1		1	31.0		2 172	139.4	187.1	2 257.3	0			- 1											
	Time in the	_		89.			310		17.	```		70.	'n	L													
	Length	ach Total m) (m)		(4800)	620 5420		1900)	(4)	(00)	2060 7480	560 100	660 13700	300 /400				•							:		d	
	Area	Each Total Each Tota (ha) (ha) (m) (m)			371.3	70		70		805.2	1094.3	1311. 6 3660	42. 4 /354.0				17 17		10000								
		wolini B 타		311.7	59.6		104.4		25.6	2)303.9	1.682	2/7.3	42.4														
		Line	_			12				\bigcirc		(- 3	<u> </u>					100	-5	100					
		Nan O Mote Agi I	0-5	<u></u>	\bigcirc		3		$[\mathfrak{S}]$	P	9	9	D)		4												

	Remorks SK								Adding and																		-			
Existing Condition	Existing Drain		(m³/s)								:																·			
Existir	Existi	Size	(E)					:																					~	
	ri.	Slope Velocity Capacity	(m/⁄s)		44.44	44.44		19.26	19.26	53.70	53.10	53.70	53.10	53.70		88.11	11.88	81:101		29.87	31.83	31.83	123.86		419	7.24	9.05	12.54	14.83	
	d Drain	Velocity	(m/s)		0.93	0.93		0.94	0.94	0.95	0.95	0.95	0.95	0.95		1.10	0//	1.13		0.85	0.87	0.87	177		0.66	0.65	2 0.68	2 0.73	2 0.76	
	Proposed	Slope	(%)		0 0.2	0 0.2		7 0.3	7 0.3	0 0.2	0.02	0.2	20 02	0.02		3.8 0.2	38 02	3.9 0.2		20 63	2.8 0.2	2.8 0.2	4.1 0.2	~	1.9 0.3	2.4 0.2	2.4 0.2	2.6 0.2	2.6 0.2	
	Œ.	Size	(E)		21.0 x 3.0	21.0 x 3.0		V.4 x 2.7	11.4 × 2.7	24.0 18.0 × 3.0	12.00 × 3.0	V 18.0 x 3.0	24.0 V 18.0 × 3.0	240 x 3.0		27.6 x 3.8	U 20.0 x 3.8	298 39		17.4 ×2.7	17.6×2.8	17.6 × 2.8	33.2 V 25.0 X		V 5.8 × 1.9	7.8 × 2.4	88 740×	16.00 \ \\ 50.0 \	20.38 W 6.0x	<u> </u>
	Runoff	Major	(സ്)		59.90			24.79	25.46	46.69	88.00	87.99	63.91	57.91		120.01	110.78	136.95		39.59	42.58	4245	161.17		5.66	6.07	11.74			Ш
2000	Runoff	Total	(%E)		41.95	40.81		17.39	14.61	48.96	47.57	44.83	43.45	38.96		83.52	77.15	95.17	<u>.</u>	27.75	29.82	28.89	1/3.73		401	6.42		11.25		
ear	5	Perha	(m/ˈs)		0.030	0.029		0.059	0.055	0.027	0.024		0.00	9/00		0.024	0.022	0.076		0.030	0.027	0.025	0.076		0.077	0.062	1		0.018	
\ \>	el tri9	porc ioiii	1S 900		0.68		i	0.70	0.70	0.68		0.68	0.68			0.68	0.68	790 0		0.68	0.68		790 0	<u>L</u>	0 0.72	2 0.71	L		0 0.68	L
ع. ا		iton			6 0.40			7 040	9	040	5	6 0.40	8 0.40	5	-	0.40	9 0.40	0	_	9 0:40	0	6 0.40	.2 0.40	_	43.4 0.40	0.	60	0	0	
	I	9W)	T (Fig.	_	6 /48.6	0		7 62	0	.69/ 9:	5 193	6 217.	8 244	3		7/90.	0			44/ 6	3.7 168.	86/98	1. 2 304.		4	0	60	3	0	
	of Flow	Total	(min) (min)	-	6 /38.	4	1	7 52	01	9	0	1	2 234.	-		7 /80.	2 202	0	Г	9 134.	8	0.	_		4 33.	8	-	~	45	11
	Time of				8.67	_	<u> </u>	52		0 /2.	\ \ \	l			L	/80				134	l	L		100	33	<u> </u>			L	1 1
	Length	Fach Total	(E)		8	me a		8	3300					50 10370		8	1400 13100	700 13800		8	20 7920		<u> </u>		0087	630 1930		l		
	19	1 L	(E)		7600	14	(0)	T			٠, ١	1 7	1	7 . '		0011		1		6700	1104.4 1220		<u> </u>	- 25		-10	4	489.0 3000	9.6 2660	1 1
	Area	Each Total	(ha) (ha)		13083	0 / 1200	1	2948	30.9 325.7	80.3 1813.4	1,88.5 1981.9	153 0 2/34 9	51.7 2286.6	148.2 2434.8	7.0	3480.2	26.4.3506.6	4 8 504B 2		9051	-	5/3 //55.	0		52.1	51.4 10.3.		236.6.48	290.6 779.	70
-	oV				-	-	ļ. <u>.</u>			\bigcirc			,	-		3		6)	5			6	-					- 2	
-	οN	θl	ijŢ	1	(3)	KE)	O	(0)	(i)	4	(9)	(9)	(E)	-	(3)	(%)	(6)		3	(9)	(3)	(2)		(3)	C)(<u>4</u>	(9)	3	
	9 tnər	on or ndo	Cal	0-6																	:									

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		Remarks)				: * :*			-		·												:					
		Ω 0 20						٠																			:		
	 i	 																			<u></u>								
	Existing Condition	Drain	Capacity (m ³ s)																										
	Existing	Existing Drain	Size (m)																										
			SiopelVelocity Capacity (%) (m/s) (m ³ s)	2.98	15.77	129.28	-				·			23°												-			
		Drain	(rn/s)	0.74	0.77	1.18												:											
		Proposed	Slope\ (%)	0.2	0.2	0.2																							
		Pro	Size (m)	4.01 H 2.3×20	1.4 2.7 0.2	1 342 x4 1				11																			
		Runoff	Storm (m/s)	4.01	2/.6/	178.5/ 5	-																						
	2000	Runoff	Total Runoff (m/s)	2.86	15.07		.,																						
	Year	Design F	Settle Settle Settle (म्रेंड)	0.085					-											·				-					
	اگر	eb jeut	onota oittead	0.73	0.67													-											
	=	f fu9i	ionuA Soeffic	9 0.40	70.40	6040																							
		10 Tottru	amit E	8 37.8		6 309 6 0.	River														; ;								
		Time of Flow in the Orain	Tota (min	8 27.9	7 270.7	4 299.6					: :																		
		Time th	Eact (min	27.8		کا	g,					-				: :													
		Length	Total Each Total 喜 (min) (min)	20	760 11270	0 14520									- 121 - 121 - 121					- 1	2.4								
		Δī	Eact (m)	(7200)			Rapar						. A.													:.			
		Area	Each Total Each Total Each Total (ha) (ha) (m) (m) (min)	7	3 886.6	8 7999.3	70																				1		
				33.7	11.			:	1										11	1									
	-		eni_l wollnI	(②	(2)						1	* 1		<u>.</u>										1	1.5			
-			5 4 6	(C) 9	@	6)	e Critic Plant			, y 1												- :						<u>.</u>	
L		и́в	Nar Catch	0			ale a minel																أحبا						

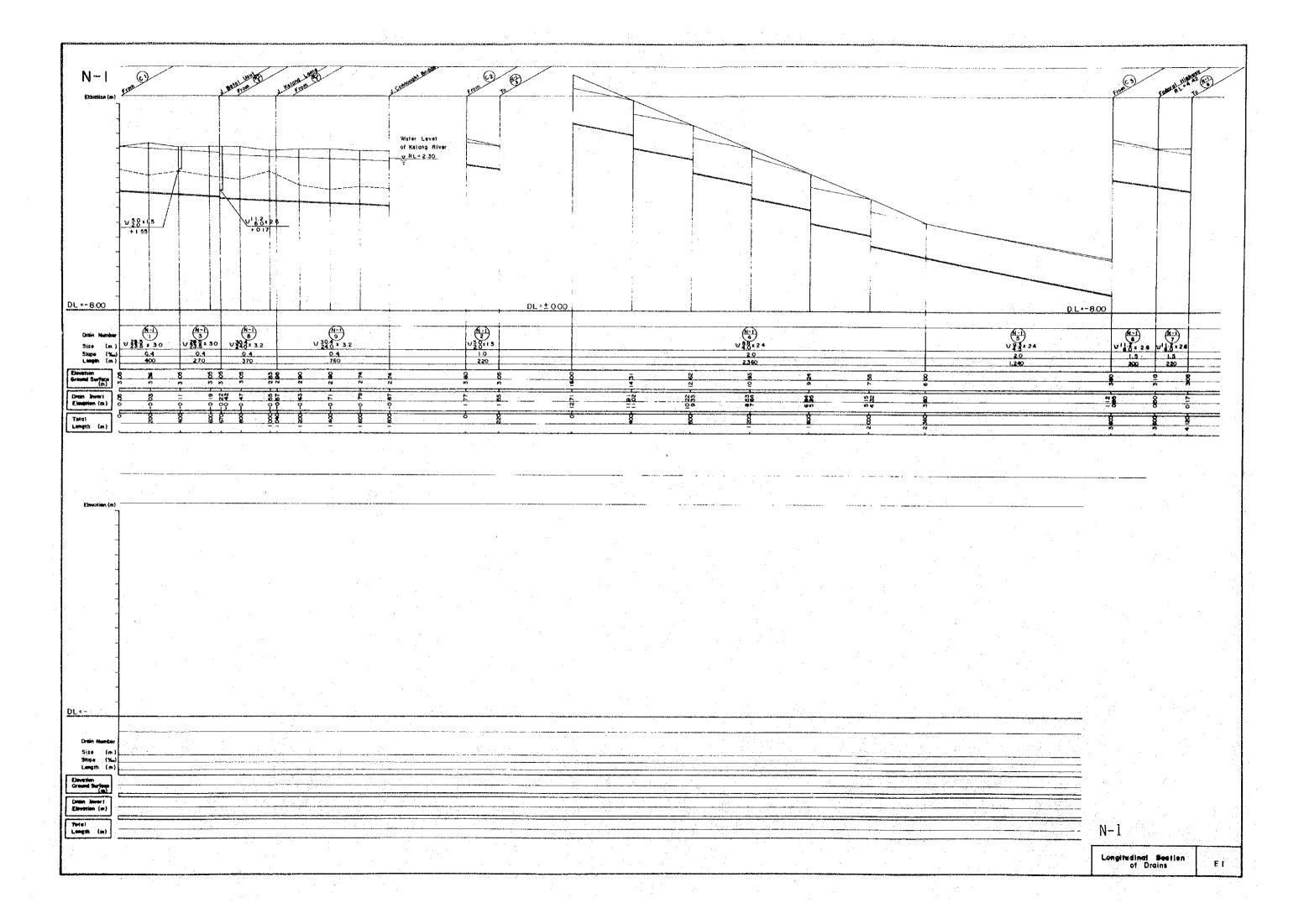
						and the second	outstand; r	a	manual.			-			-3464343								· T	***************************************		T	-			-
	22,773,232						-	1							•									-						
	Remarks																											-		
	U.	2																												
ğ		- Cit	75						_					-																
) Condit	g Drain	Capacity	(m ³ /s)												_					-	_	_						-		
Existing Condition	Existing Drain	Size	(E)	.																										
		pocity	π ³ /s)	11.18	13.78	16.71	21.81			367	427		3.84	4.48	8.33	8.33	8:33	11.92		1.45	2.39	3.27	4.68	5.76	66.11					
	Drain	Slopel Velocity Capacity	(%) (m/s) (%)	121	1.27	133	1.41			1.42	1.46		1.48	1.54	1.89	1.89	681	2.0.7		1/2	136	1.42	1.44	1.45	7.56					
	Proposed	Siopel	(%)	0.8	0.8	0.8	0.8			0 /	1.0		1/	/ /	1.9	6.7	6 /	6.7		0 /	7.2	/ ;	0.0	0.8	9.0					
	Pro	Size		2 x 2. /	7.9 3.5 × 2.2	8.6 x 2.3	9.8 × 2.4 5.0 × 2.4			71×71	U/8×1.8		W17×1.7	W/8×1.8	02.1×2.1	02:1×2.1	021×21	2.4 × 2.4	٠.	1.2 × 1.2	U / 4× 1.4	47.6 X 1.6	7.9×1.9	21x21	29 x 2.9					
	off			72 730 x	Ĭ. E.	2 40	20.00	-		7 10	7 77		7	E /.	20	0 2	<u>.</u>	<u>.</u>		23	7	23	1	Ð	ā					
0		Major Storm		4	2	7	9	:		2	رى		3	5	6	.0	.5	35		1.09	60	38	8	24	9					
2000	Runoff	Total		10.64	12.42	15.27				3.7	3.63		3.45	3.55		7 7.60	2 7.65	7 10.85			2 2.09	2 2.88	0 4.08	7	1 10.1	•				
Year	Design	Perha	(m³/s)	0150	0.736	0.124	0.123	: :		0.188	0179		0.144	6/3	0.148	0.147	0.142	0.750		0.260	0.272	0.232	_	0.19	0.21					
>	ə	fficik orag iolitie	IS	0.75	0.74	0.73	0 0:73			0 0.74	2 0.73		5 0.76	7 0.75	59 0.73	9 0.73	9 0.73	74 0.73		19.0 61	83 0.81	1 0.81	69 0.79	70 07	75 0.77					
r:	اِ ا	Jour	ਮੁ	0.60		9.0	0.6			1 0.80	1 082		6 0.55	3 0.57	2 0.6	5 0.6	2 0.69	ó		8 07	0.8	3 0.71	5 0.6	1 0	3					
9-N)	5	ime incenti		2) (26.2) 3 29.3			5 40.5			7 32.	/ 36		6 27.	3 37.3	272 37	27.5 37.	30.2 40.	30.9 40.9		(6.8) (16.	9.1.19	9.3 19	17.5 21	1 24	74.3 24.					
A ey	Time of Flow	n Total		(16.2)	7		0 30.5			(191)	26,	,	7 17.	7 2/3			1 30	7 30		9 7 0	3	2	71 2						-	
Alternative	F		(min)	85	L	<u> </u>	<u></u>			3.0	00 4.0	<u> </u>	7	3	5 1.7	60 03	2	0.		2.	530 0.	550 0.	Vi	950 2.6	l	<u> </u>			-	
Alte	Length	th Total	Ē.	(1285)		<u> </u>		-	 	(/4/0)	0 2000	 	(046)	0 1520	25 2/25	5	295 2455	85 2540		(380)	5	20 5	:	220 9.				-		
	<u> </u>	ai Each		70)		 -	 			27) 55 250	340		97) 23	72 320	_	89 3			<u> </u>	52) 72	38) 2		/		- (-	
	Area	ch Total		3.90 77.60		37.50 723.70	38.90 162.00			2.28 16.55	372 20.29		6.06 23.93	1.79 25.9	045 (4.18)	1.07 51.69	0.89 53.8	2.86 72.30		70 (2:	009 (3.38)	0.05 12.	2.73 (4.32)	267 25	(22.64)				-	
-	ON	Nol Each		<u>ب</u>	20.	3/.	38		_	2	. v		6	,			0	.01	(6 2)	<i>'</i>	0	0	""			(63)		-		-
	.oN				(0)	(e)	(4)	(F)		(S)	9	(b)		(0)	9	(9)	(3)	(3)	(A)	(S)	(4)	(3)	(9)	(9)	(8)	9			-	-
-	tnen	mol To nhot	റ്റ	$\frac{(\cdot)}{(\cdot)}$			2			9	<u> </u>	-		<u> </u>		<u>)</u>			-	<u>)</u>	<u>ک</u>	9				1	17	-		
	Ü	տոն	V		<u></u>					Ļ		ــــــــــــــــــــــــــــــــــــــ		,		<u></u>		ــــــا		<u> </u>		لبا	ــــــــــــــــــــــــــــــــــــــ	<u> </u>			<u> </u>		ــــــــــــــــــــــــــــــــــــــ	

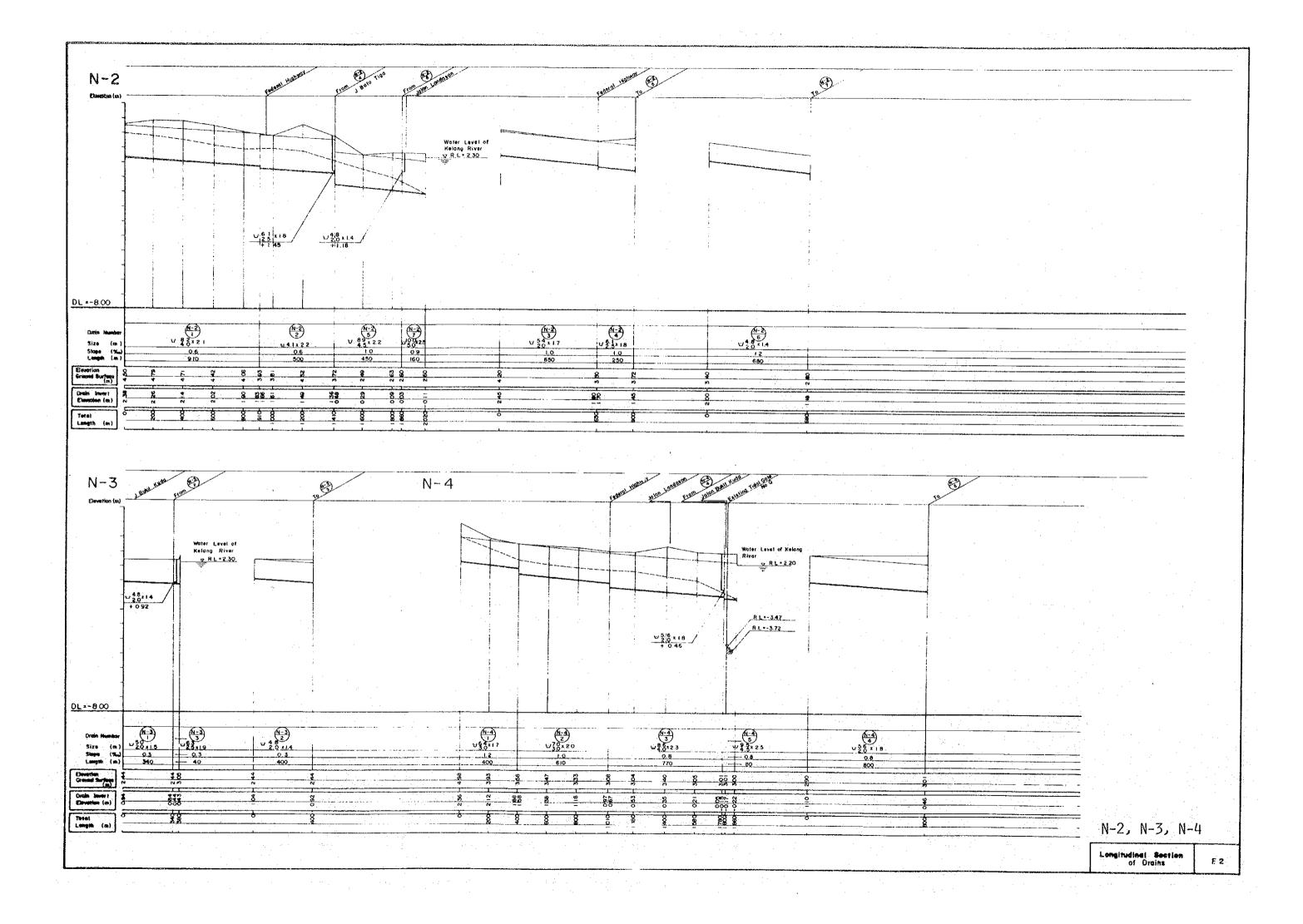
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Condition	Drain	Capacity	(m ² s)								<u>.</u>																	
Existing Condition	Existing Drain	Size	(m)																									
	ıin	Capacity	(m ³ /s)	8.22	8.22	16.26	17.60	19.55	24.75		3.62	3.62	888		1.45	2.39												
	sed Drain	Slope/Velocity/Capacity	(5/m) (m/s) (m/s)	8 1.59			8: 1.36	0.8 7.39			661 61	1.9 1.79	1.9 2.24	· 	1.0 1.12	2 1.36	1 1.42	0.9 1.44	0.8 1.45	0.6 7.56								
	Proposed	Size Slo	%) (m)	2.4 × 2.4 0.8	24×2.4 0.8	83 3.5 × 2.4	3.5 x 2.5 0.8	9.0 × 2.5	9.9 4.5 x 2.7		1 5/x5/B		21×2.1		1/2×/2/	U14x14	7 97 491	0 61x61A	2.1 × 2.1	U 29×29 0								
	Runoff		(നും	A	<u> </u>	a	Д	Α	7		6	0	0			П	_1		7	FI								
2000				7.34	6.44	14.72	16.10		22.72		301]	7.59		1.09		2.88			10.16		 ·				:		
Year	E Design	Perha) (m/s)	0.181	0.159	3 0.132	2 0.122	1 0.113	1 0.112		1 0.269	\rightarrow	0 239		1 0.260	1 0.272	1 0232	9 0210	10107	7 0211								
i Y	1n9i 90	of fic	S 900	75 075	175 0.73	55	165 072	165 071	165 071	-1	75 084	75	0.00 52.0		180 620	183 081	180 120	2.69 0.79	2.70 0.77	275 077		 					- 	
(9-7	to noitori	amiT neono ionoi	(min)	(28.4)	37.8 0.	40.7 0	45.7 0	50.2 0	51.10		(15.0)		19.9		(8.8)	1010	193 0	2/5	24.1	243 0								
9 B (N	of Flow Drain	Total	(min) (min)	(184)	5 27.8	30.7	35.7	5 40.2	1.14 6		(5.0)		6.0		(6.8)	3 91	9.3	2 //.5	6 14.1	2 /43				1 2				:
Alternative		Total Each	(m) (mi	(1400)	2250 6.5		2875 50	3245 4.5	3325 0.9		565 1.5		940 0.6		(380) 505 2:0	530 03	550 0.2	730 22	950 26	970 02								
Alt	Length	Each Tr	(m)	270 14	580	225	400	370 3	80 3		55/		90	:	/25	25	20	180	220	20					_			
	Area	Total) (ha)	(32.14)	40.48	(37.70)	0 /32.08	0 153.58	38.90 202.48		(4/8)	8 13.59	3 31.82				15 (4.61)	(4.32)	(349)	(22.64) - 48.20			: : :					
		Molitical Right	nI (br)	8.34	- I	3.90	20.00	31.50	38.9	(f.)	7.03	2.18	1843	(62)	170	0.00	0.05	2.73	2.67		વિ	4.						
1	· .	əui.		0	<u> </u>	୍ର (ଅ	4)	<u>্</u>	<u> </u>	9	((O)	6)	9	(3)	(2)	<u>ා</u>	(3)	(S)	۳	- 1 1						
+	աeև Մ	Nar O Apt	5	9-N	(8)																							

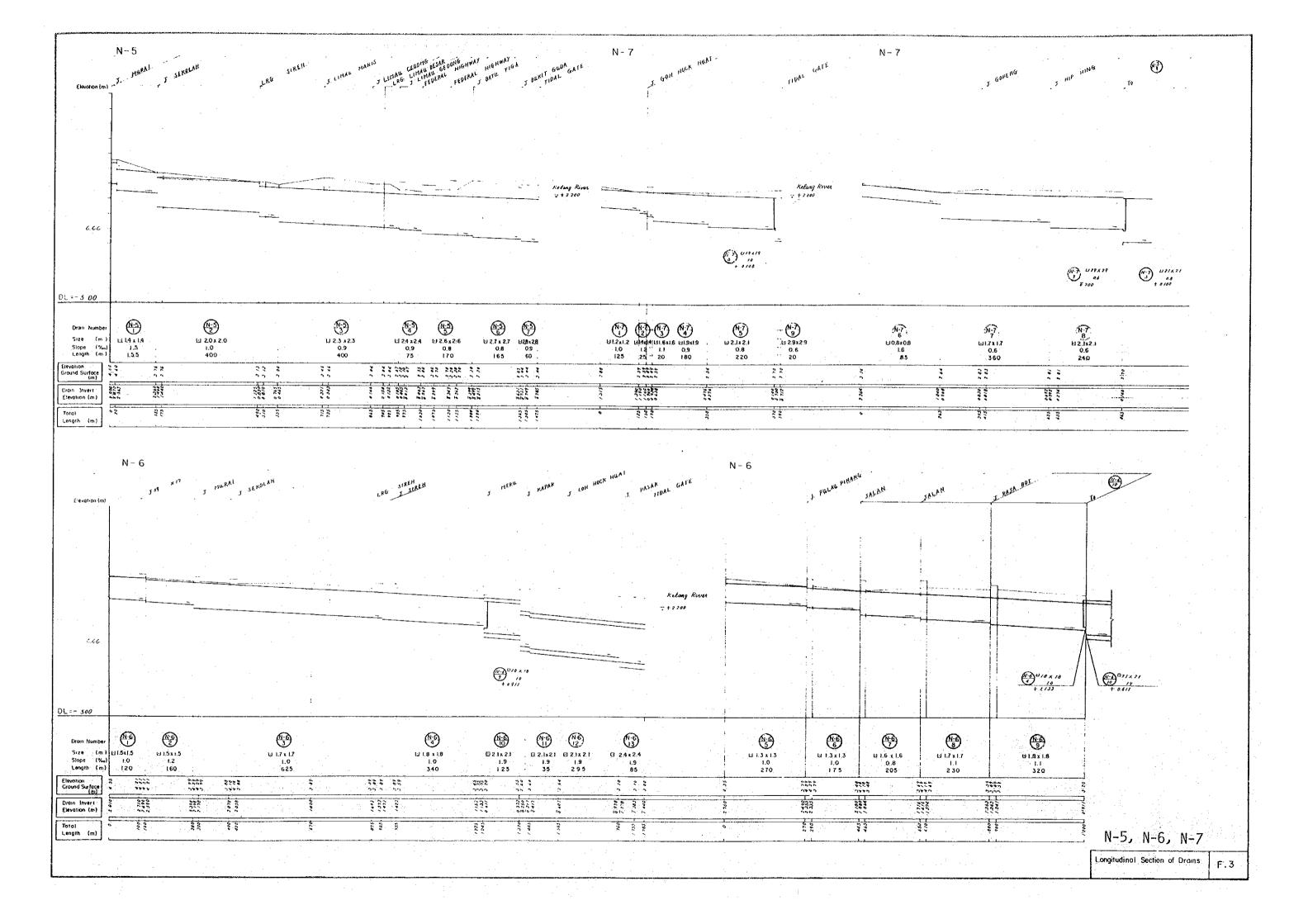
Alternative C (N-6) in Year 2000 Existing Drain Famorike Existing Condition Existin			en an annual de principal de pr		714					T		- T					·		T										200.00	2,25402
Alternative C (N-6) in Ye ar 2000 Existing Condition Existing Co													ļ																	Ì
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Carry Carr	giting	gin	apacit m³s)						İ																					
Alternative C (N-6) In Year 2000 Alternative C (N-6) Alternativ	S	ng D		-						\dashv		-		-			-													
Alternative C (N-6) In Year 2000 Alternative C (N-6) Alternativ	Xistir	Xisti	Size (m)																	.					,					:
Alternative C (N-6) in Year 2000 Alternative C (N-6) in Year		1 1111	S) S)	8/	78	11	8/8		29	29	88		22	22	117		249	3.82	4.09									-		
A		nin	g E	1	/3	9/	5/2					-														.				
A		1	elociti (m/s)	121	1.27	1.33	1.41		1.79	1.79	2.24		159	1.59	1.63		1.23	1.31	7.63								L.			
A		paso	Sope (%)	8.0	0.0	0.8	0.0		6.1	6.1	6.1			0.8	0.8		60	0.8	9.0											
Alternative C (N-6) in Year 2000 Alternative C (N-6) A		Prop			 				5,5	50/	2.1		82.7	2.4	2.5		× /.5	× /. 0	x 3 /	-										
Alternative C (N-6) in Year 2000			Siz m)	7.2 7.0 7.0 7.0 7.0	7.9 ×	8.4 8.0	9.8 5.0 x		3 / 5 /	17.5	× /.2		(0.26 U.2.4 x	12.4	U 2.5)		W/5	E 7.8	E 3.1											
Alternative C (N-6) in Year 2000 And Hornative C (N-6) in Year 2000 And Hornative C (N-6) in Year 2000 Each Total Each Each Total Each Total Each Each Each Each Each Each Each Each		off	호 E %																							•			.	
Alternative C (N-6) in Year 22 Area Length inthe Origin to Figure 1 (mg) in the Origin to Figure 1 (mg) in the Origin to Figure 2 (mg) in the Origin to Figure 2 (mg) in the Origin in the Origin to Figure 2 (mg) in the Origin i			B B B B B B B B B B B B B B B B B B B		0,	<i>L</i>	- 9	-	_	8	0		70	0	9		5	60	œ			-					-			
Alternative C (N-6) in Year Area Length Time or in A feet Design Into Cook Total Each Each Each Each Each Each Each Each	3	unof	Runo (m/s	10.64	12.42	15.2	19.8		3.0	3.2,	7.5	;	6.9	7.3	8.0		2.0	3.2	12.6											
Alternative C (N-6) in Year Area Length in time or Films of Films		. Igi			36	24			692	245	239	, .	172	191	143		222	208	143											
Columbia	9		P E	0		+	3 0.7						ļ l												-		_	-	-	
Alternative C (N-6) in the Digit of Filow of Gargo of Gar		Э	Storag	6	0.74		0.7.						5 0.7	5 0.7							: -		_	-			-	ļ		
Alternative C (N Artea Length Time of Flow Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total (Artea State Stat	. <u>c</u>	Ē .	ilonu되	1 %	0.60	0	0		Ö.	0	0		1	Ö	0		0	0	a				_	_				ļ	 	ļ
Alternative C (N Artea Length Time of Flow Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total Each Total (Artea State Stat	1	10 DOID	emiT <u>E</u> mesoxX <u>C</u>	262)	34.7	39.5	40.5	. :	(/5.0, /6.5	19.3	199	77 -	33.9	37.1	44		(18.8, 22.3	25.0	44.4											
Alternative C Area Length In the Dol (area) (m) (m) (min) (nother Dol (area) (a	1			(2.0)	7 7	50.5	30.5			93	66		18.4)	11	34.2		88)	15.0	34.4											
Alterno Area Length Cach Total Each Total (m)			4. c		+	 				8	9		1					7										-	-	
ON wolfn	1	- E	E E	-		<u> </u>					ļ	<u> </u>											ļ	-	-	-		-	-	-
ON wolfn	4	1	Total	128	1/0/	2280	236	:	(4/0	L				222		.			292			l.							<u> </u>	L
ON wolfn	\ \ \	Leng	ach	300		370	80		155	295	80		505	300	675		245	205	20											
ON wolfn			rai E	(0/2			-		121	39	1.82		1	— —		<u> </u>	37	5.53	6.54)											
ON wolfn		Area	유 구 구	\$ <mark>€</mark> \$	3 6	20 /2	70 /62			8/	43 3.		06 (3	77 4:	36 54	-	39 (2	1,6/			· ·			 			-	-		
				-	5 8	3,5	38		1	8	18	 	+	4	//	1	9	- 40	-		_		-	<u> </u>			-	-	+-	
T IN THE ROLL OF T			· · · · · ·					(P)				(6,2))			(3)				(F)					_		_		_	
				(S	<u>@</u>		a		(4)	9	0		®	(e)	9		(3)	(2)	(3)						_			1		<u> </u>
O Cotchment		in <u>ər</u>	ndri To ndət <u>o</u>	717	()		-		:						:															

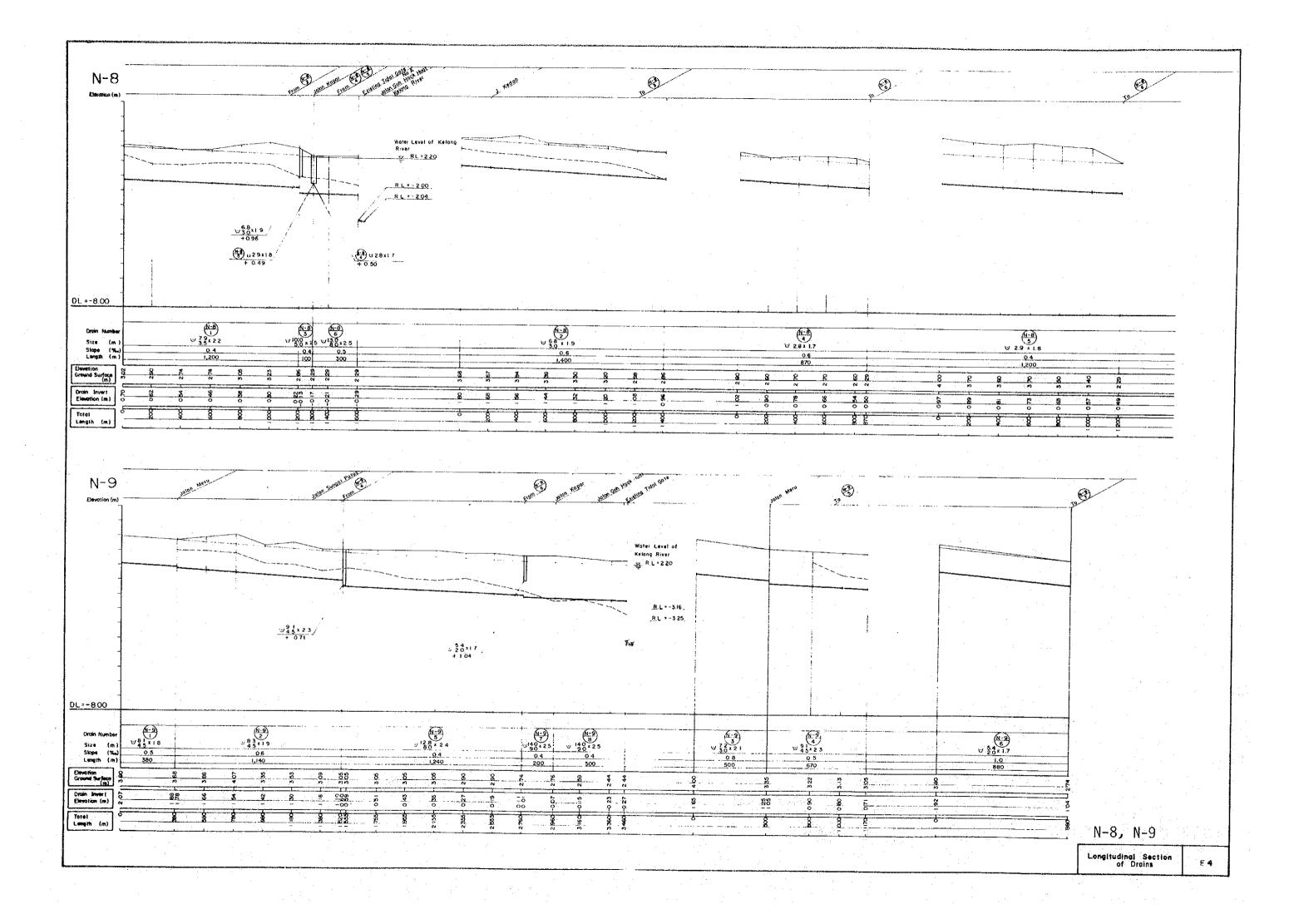
	0 2 1 0 0																										
andition	Orain	Capacity (m3/c)	, S/,																								
Existing Candition	Existing Drain		È																								
	ii	Capacity (m ^{3/c})		11.98		7.32	18.37		3.76		13.97	27.36		8.47	35.62	35.82									:		
	sed Drain	Slope Velocity Capacity	- -	3 1.24		3 7.10	4 1.53		2 1.08		3 1.27			5 093	6 1.42											:	
	Proposed		2/3	44.4×2.5 0.3		W38x2.0 0.3	152x26 04		V 63x1.4 / 2	# 37x2.0 0.4	U 56×22 03	_		V4.0×1.8 0.5	V 70x 2.9 0.6	V 7.0 x 2.9 0.5				•							
	Runoff	Storm	è	77		20	, FI		, a	А	73	`^		V.	<u>,</u>	<u>``</u> 2				*.	-						
2000	Runoff	Funoff		17.25		6.97	17.49		3.28	7.65	13.25	24.72		7.32	32.45	32.26					-:						
Year	= Design	offic Perha		7 0.164		161.0	2 0.773		2 0.205		4 0.136	-		7 0.123	0.093	1 0.092		•			:	:					
in Y	HUS	Runofic Stora		0.60 0.77		0.60 0.80	0.60 0.72		0.60 0.82	0.60 0.77	0.60 0.74	0.60 0.71		0.45 0.77	0.60 0.71	0.60 0.71						:					
(5 L	io noimi	emil (neora)	Tunna T	25.7		8.61	45.6		19.7	23.9	34.5	55.2		25 8	60.0	6 60.6											Ta .
Catchmen	Time of Flow in the Drain	th Total		15.7	. •	8 9.8	9 35.6		7 7.7	2 13.9	6 24.5	6 45.2		8 /5.8	8	0.6 50.0	,										
S7.				\$		9.	2540 19		7	935 6	1735 10			75	3740 4	7	2 River		• • • •	Assault 1							
B (Ss~	Length	Each	(1111)	280	ල	630	08/	(O)	480	455	800	800	6	850	400	50	Kelang										
Case	Area	ch Toral	(10)	68.0	70	5	7 154.8	170	o.	29.0 45.0	52.4 97.4	252.2	70	. 5	37.2 348.9		70										 1 3
		wolini B		79	•	36	(J) 49.		/6.	29	52	0		59.	(g) 31	0		1									
		eui-J	\mathcal{L}	-+		0	<u>ි</u>		⊕	(b)	9	<u></u>		@	<u>©</u>	9											
	an T	Nar Calch		<u>ک</u>	27	(8)									اب.												

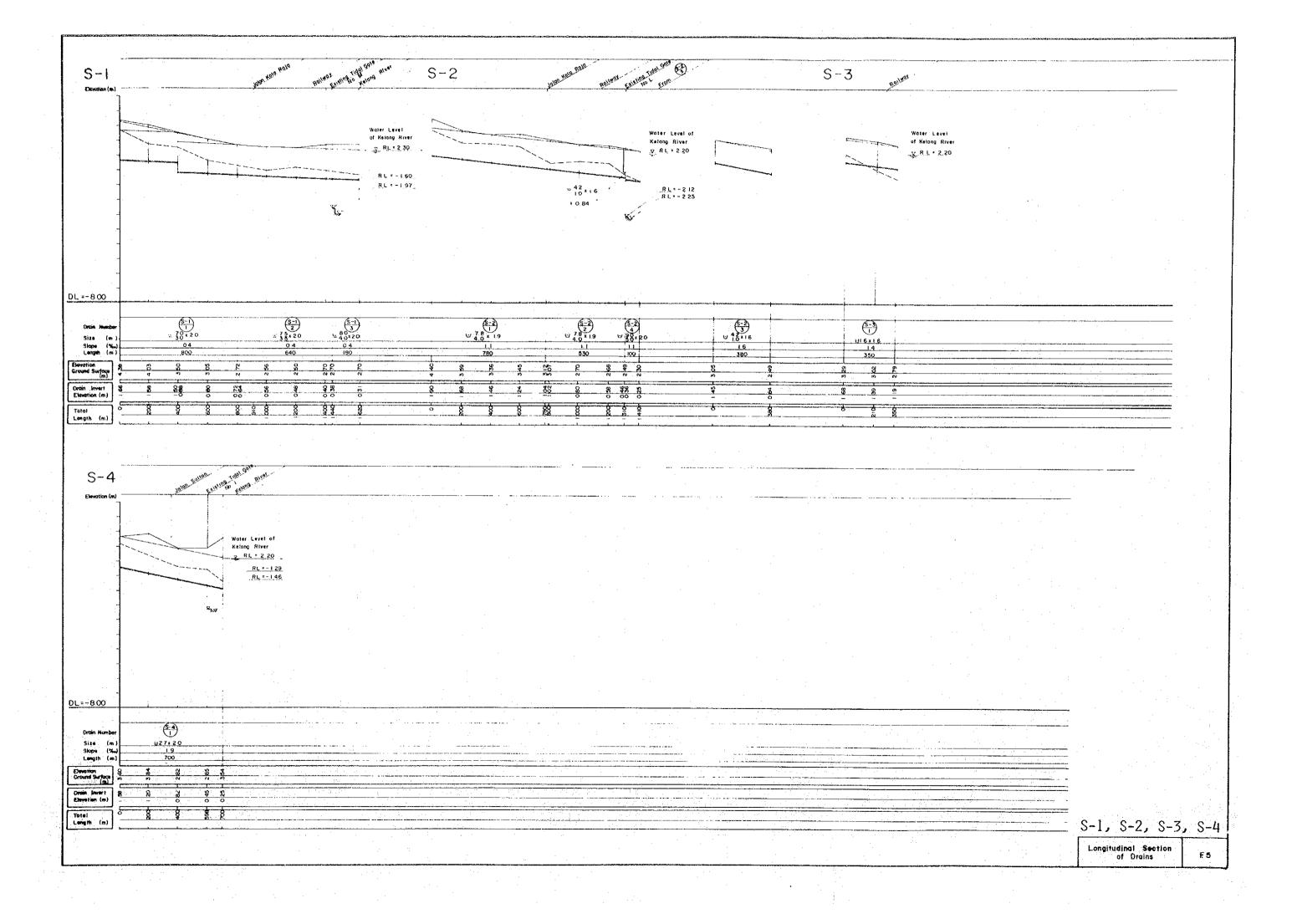
, (1122 — ()	Remarks																					 				
io									_						-								-			
Existing Condition	Existing Drain	Capacity (m ³ /s)								_											 					
Existin	Existi	ty Size	70		-		6	2		9	4	4		4											_	
	Drain	ny Capaat (m ³ /s)	10.2	.			7 489	7 642		9	8	3 11.54		3 4.64	8 15.9	1.										
	1 1	Slope Velocity Capacity (%) (m/s) (m ³ /s)	0.6 1.050				0.6 0.89	0.6 1.37		1.0 1.14	10 1.22	1.0 7.33		1.2 1.13	09 138											
	Proposed	Size S (m) (c	7.8 1.9 C				5.3 x 1.9	9.1×8.2		4 2.0 × 1.7	U 2.5×1.8	v 2.7×2.1		1 20×14	8.1 V35×23											
	Runoff	Najor Storm (㎡法)	, ,		7		ā	Я		ס	נ	⋾		Þ	>											
2000	Runoff	Total Runoff (m/s)	887		A 29 ha		454	909		5.70	723	10.96		4.25	/4.50					·						
Year	Design	Siora Coeffic (m/s)	0127		: 32:3		0.103			0.176	0.160	0.088	·	0.174	0086											
i	. tuə	Coeffici	0.55 0.74		Pond (= tc	 -	0.55 0.72	055 0.71		0.55 0.80	0.55 0.78	055 071	,-	0.55 0.80	0.55 071											
Pond)		emiT E	33.2 0	- 7-1	Retention P		45.8	520		19.7	23.2	57.7		20.3												
Datention	e of Flow	(min) (min)	23.2 23.2	tion Pond			2.6 35.8			49.	5.			10.3 10.3	49.										 	
- 1	F. F.	Total Eac (m) (m	23	To Retention	Dutflow F		2/ 0/6	2.3		6	900			9/	7	5						-		-		
# 100 + 00 T		Each 7	(750)	m3/20C	SC		099	500	9	(500)	250	3 450	0	989		Ke/ang		,								
4+!/W/ C-!W	Area	Each Total Each Total (ha) (m) (m)		10.22	3.7 7		(44.8)	20.0 (64.8	70	32.4	2.8 45.	14.8 (124.8)	70	24.4	199 (169.1)	70										
	<u> </u>	wolini 명 은	(2)				/	2		,	\(\frac{1}{2} \)	(S)			(9)							-		11.2	1. 1.	
		Carchr Line	Œ					0		(9)	4	9		9	0				1							
	əi	Mam 10 Catchr	N-2	(R.P.)				<u> </u>			<u></u>			F-	-22		<u> </u>								L	

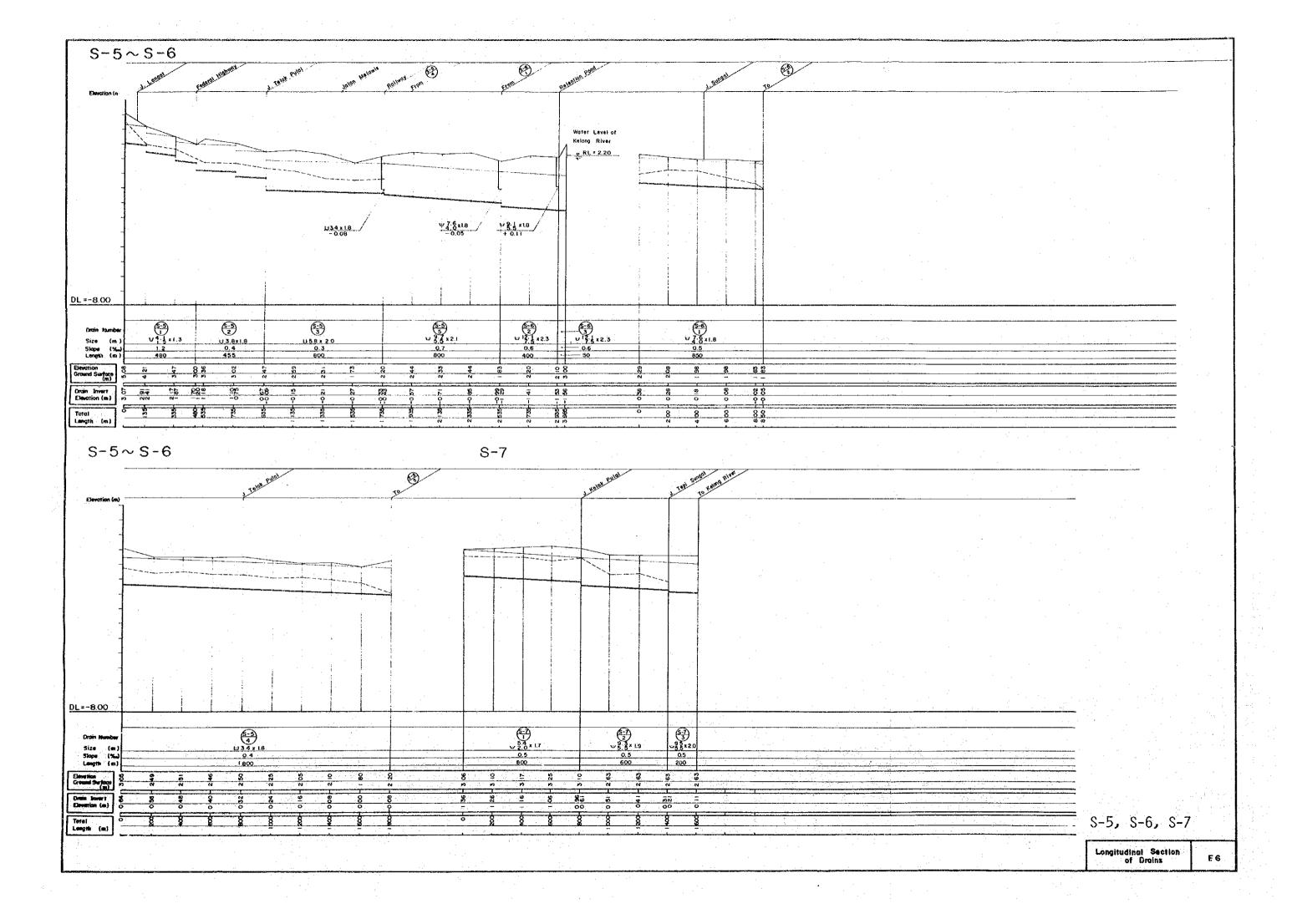


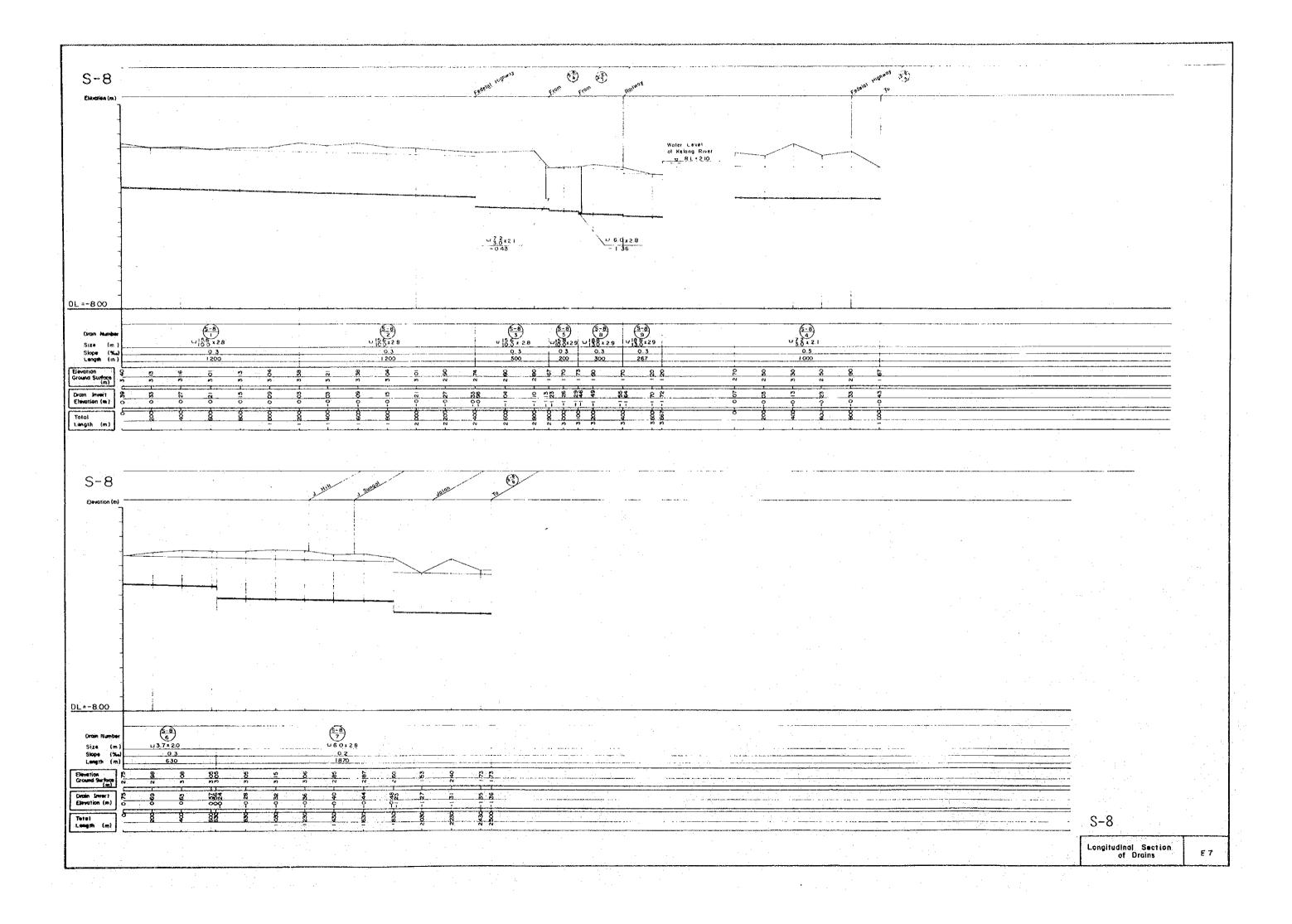


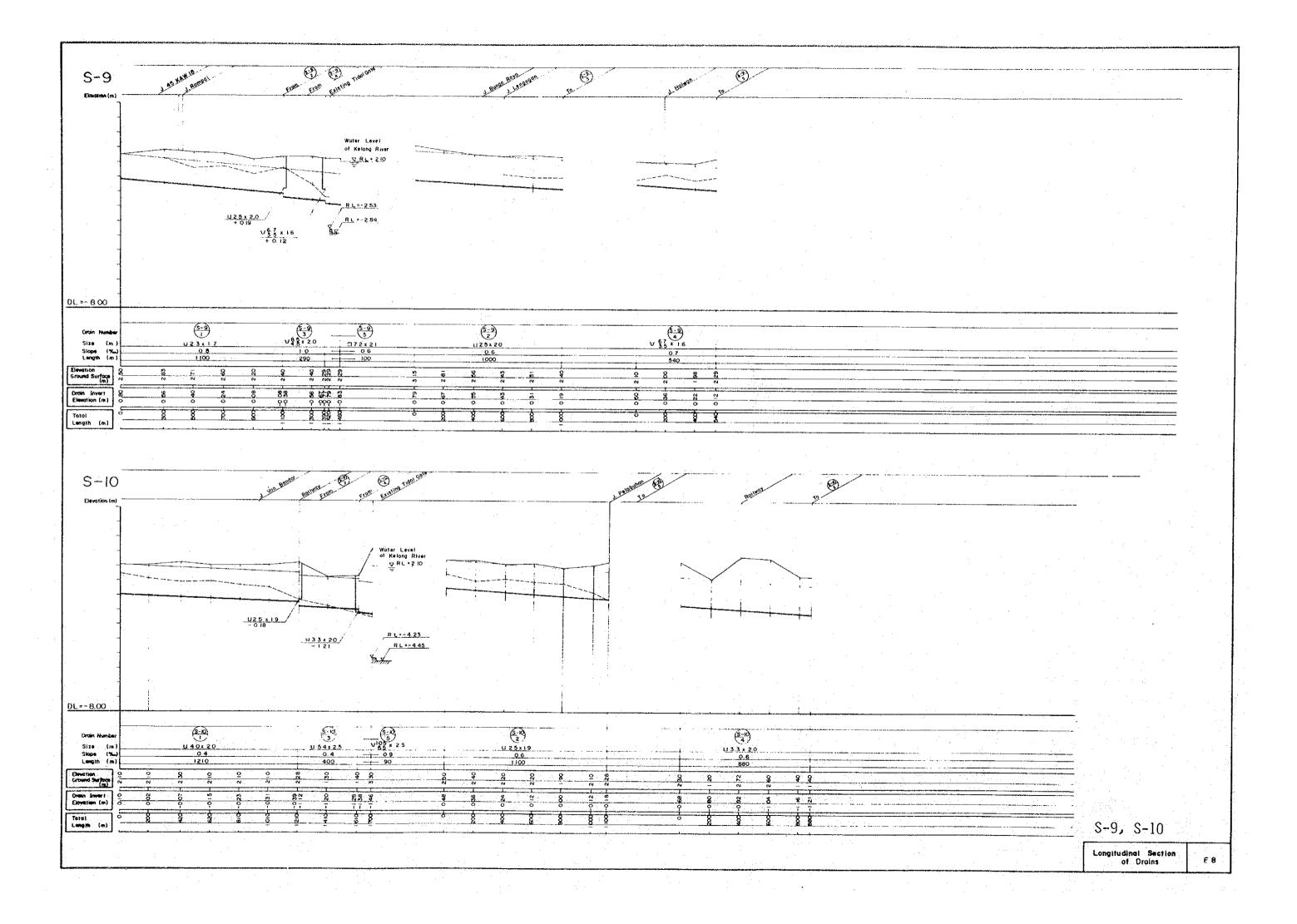


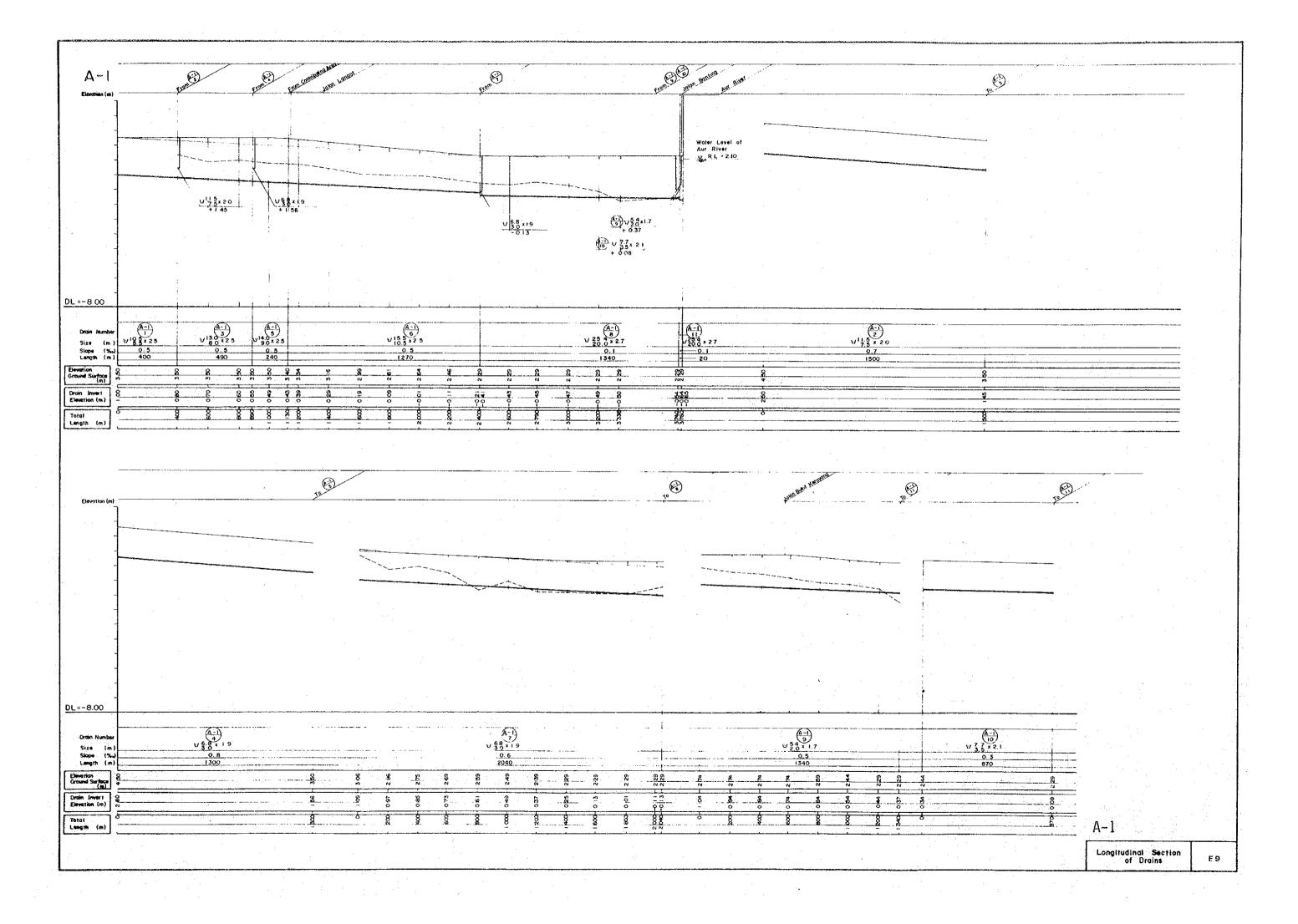


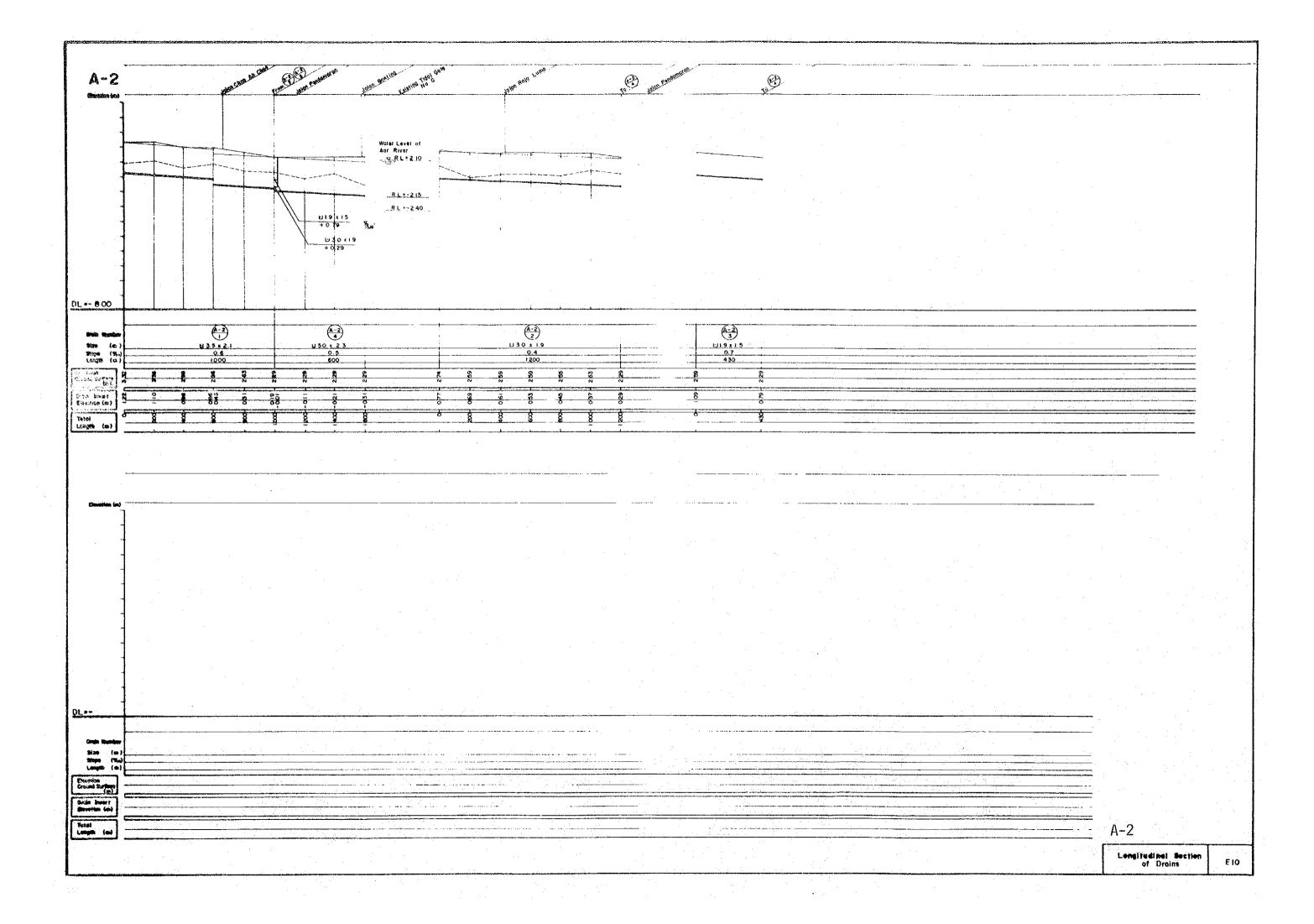


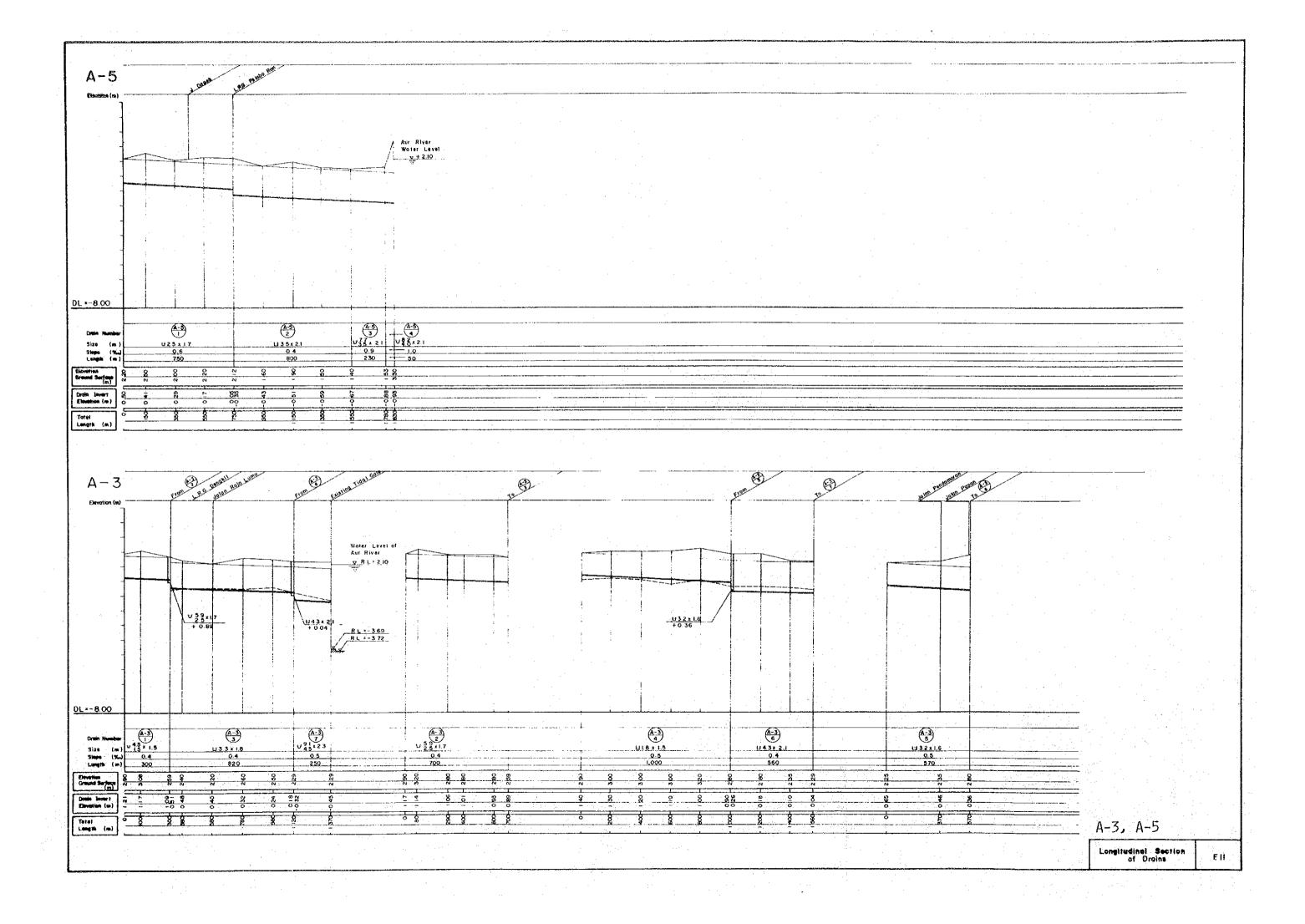


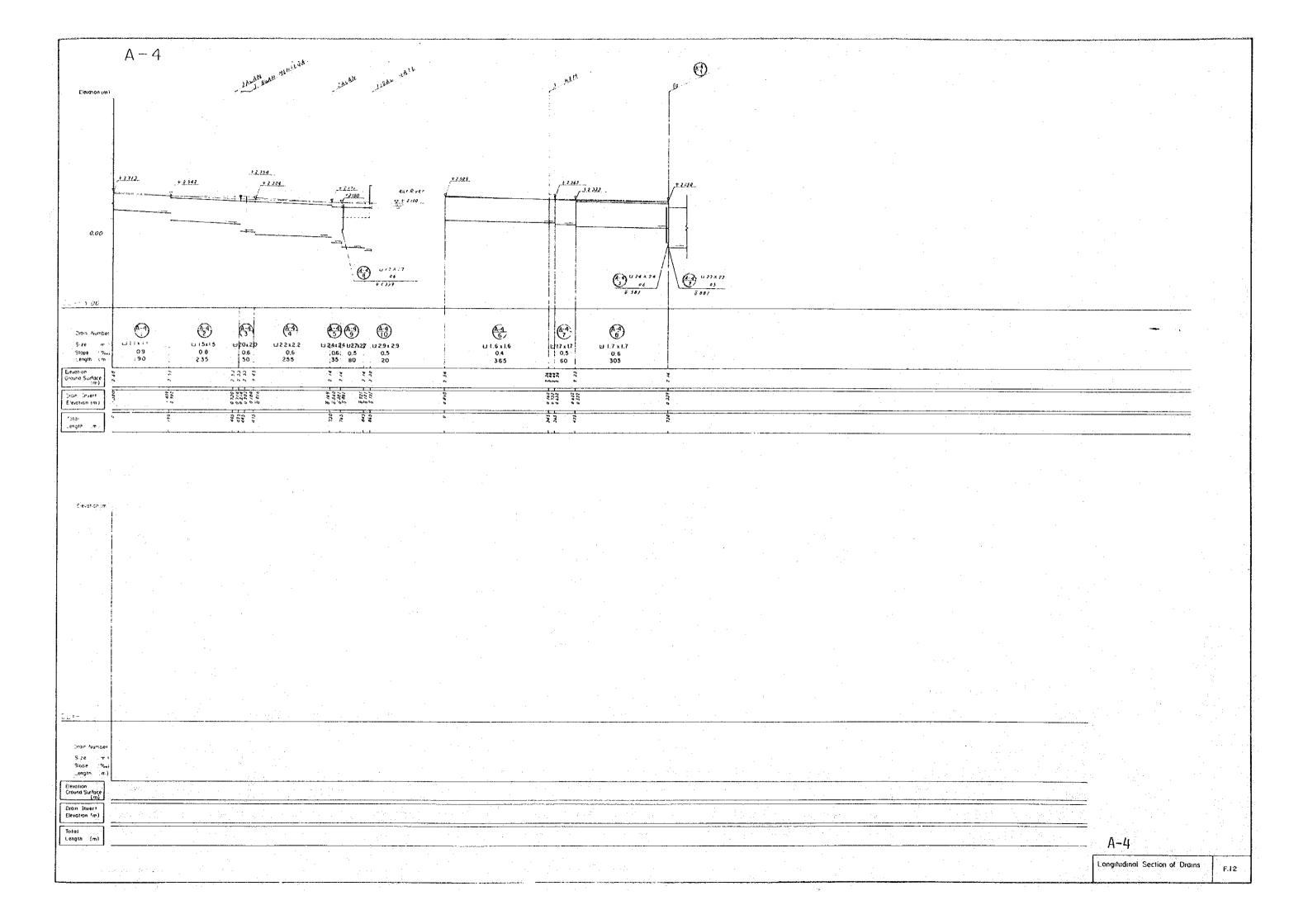


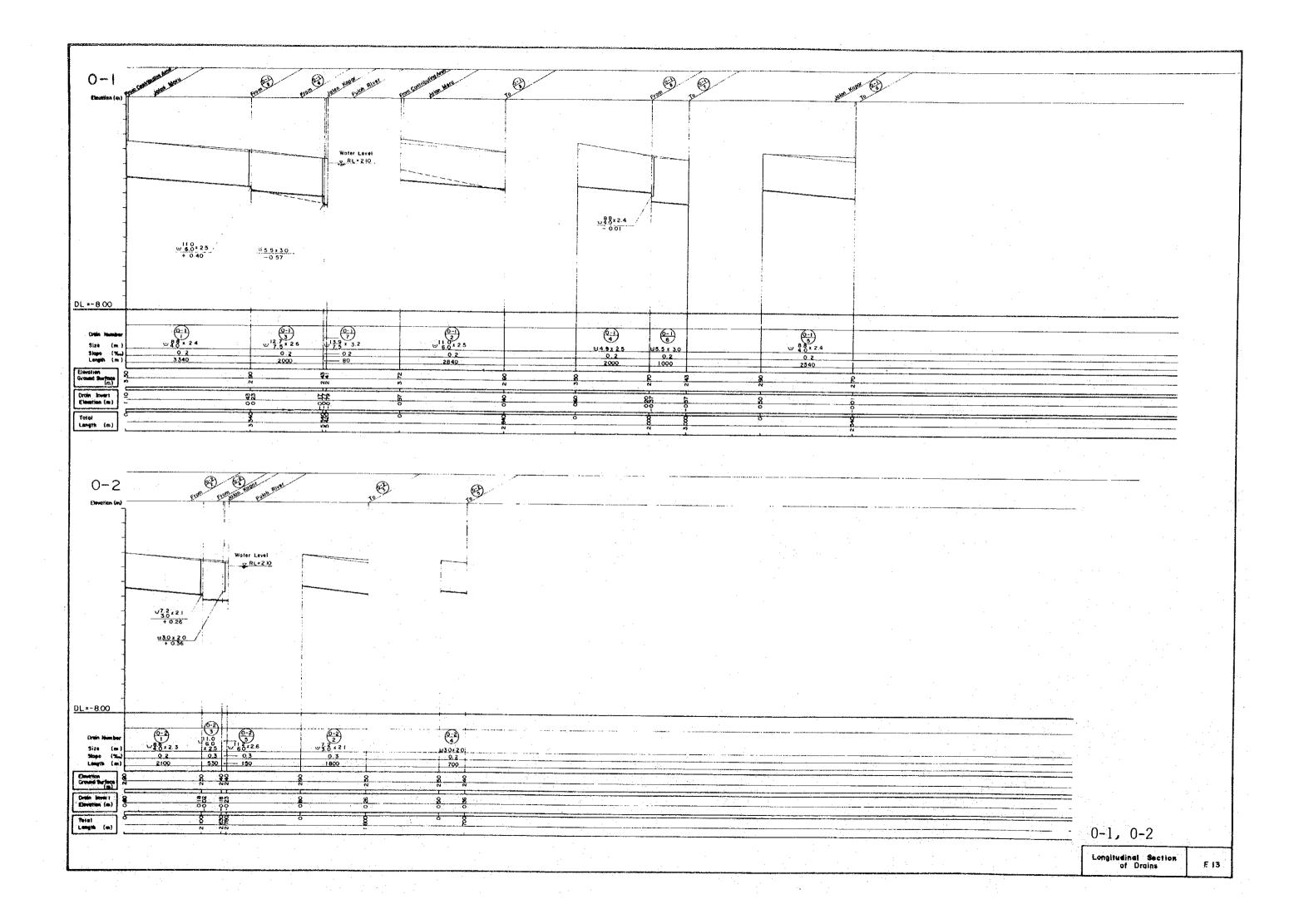


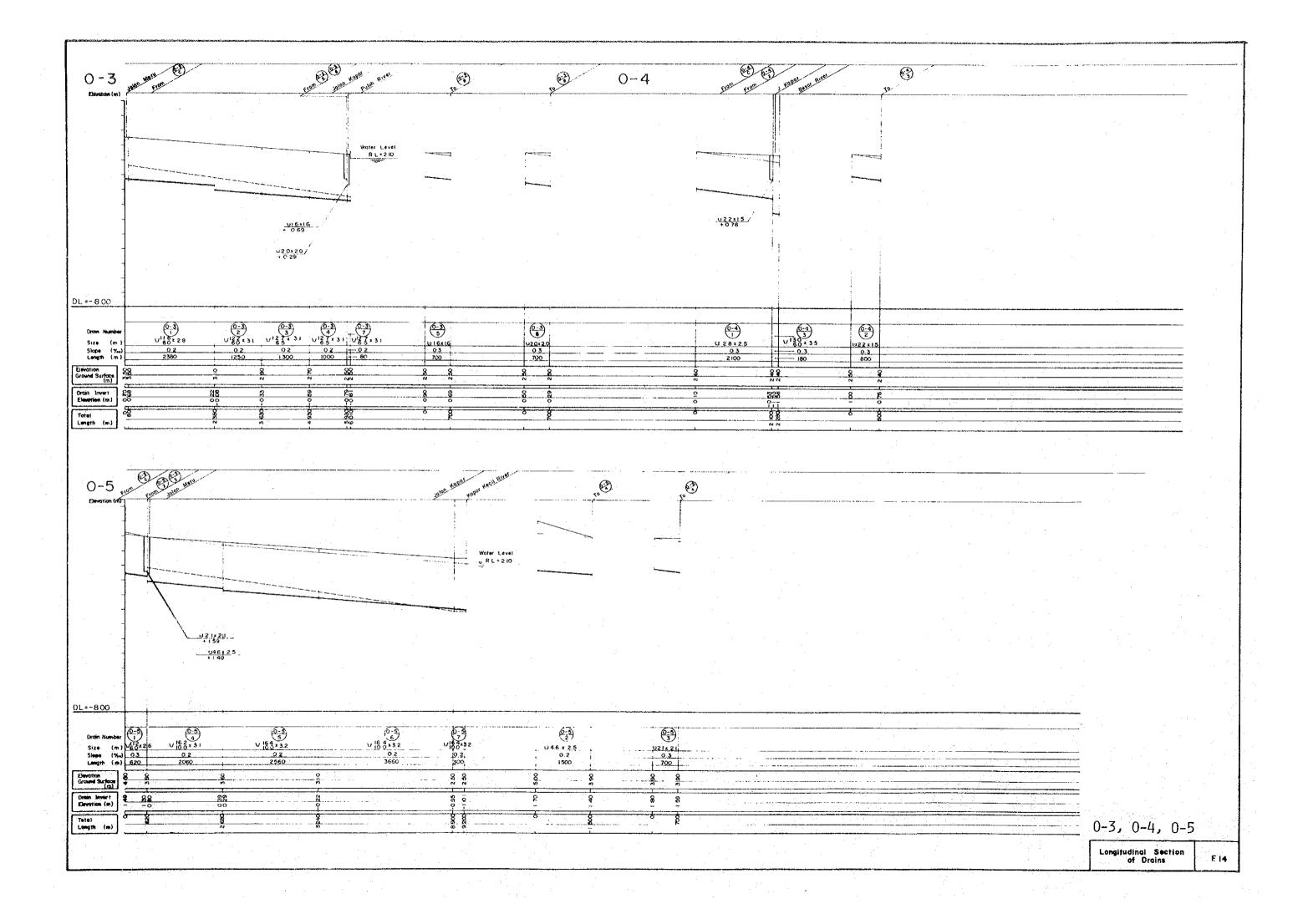


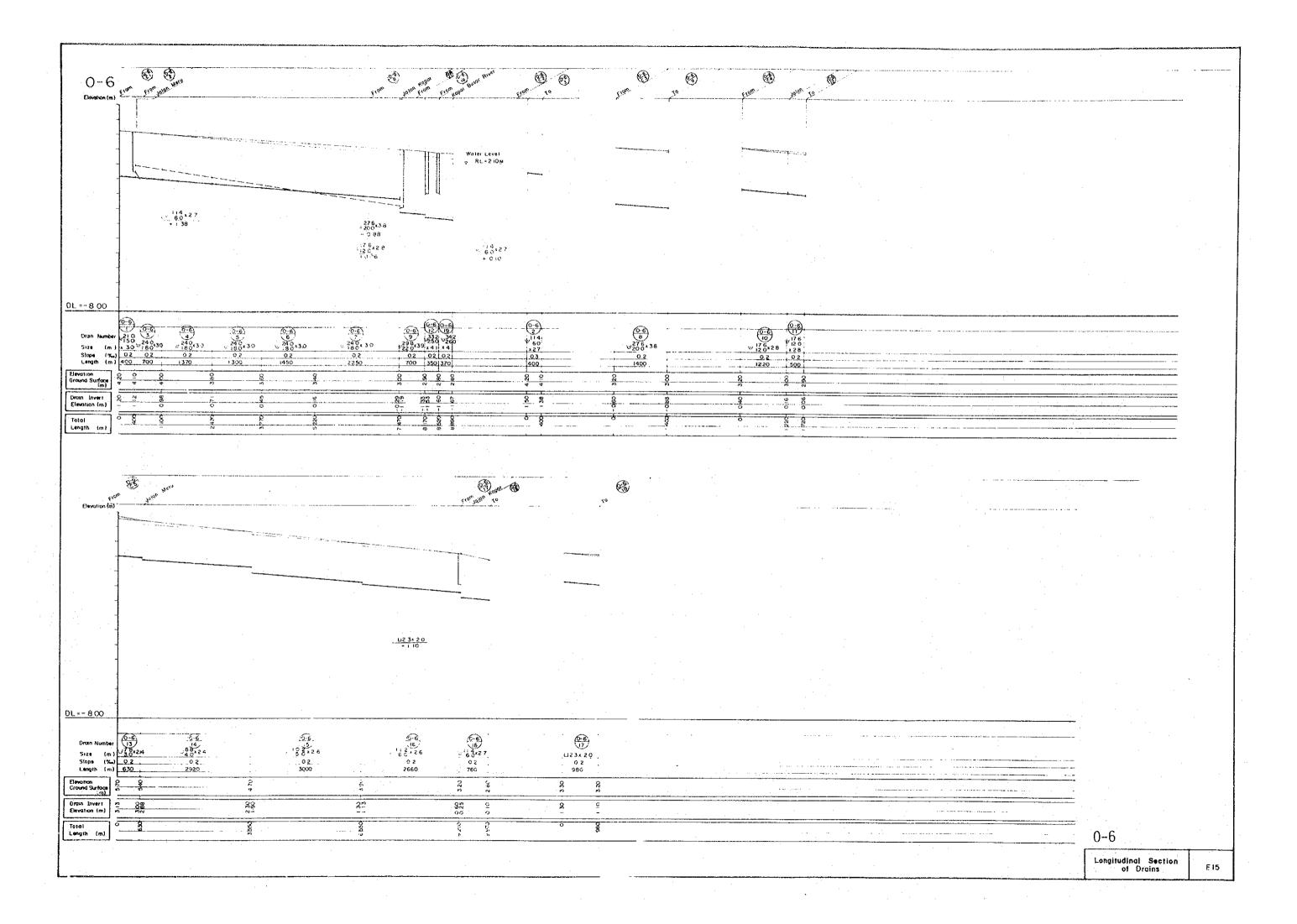












APPENDIX G

IMPLEMENTATION PRIORITIES

(Ref.: Table 6.1., Vol. VI)

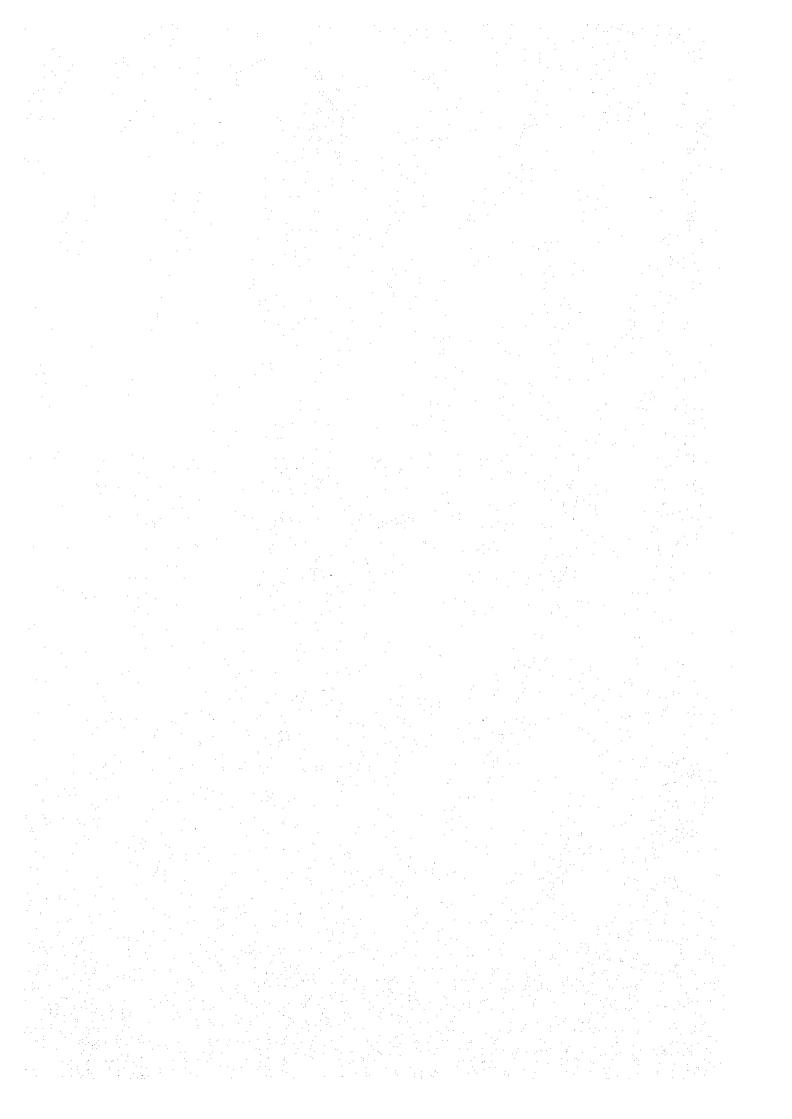


Table G.1. Population Density of Catchments

	Aı	Area	Served Pop	Population	Population Density	Density	Rai	Rating
Catchment Code No.	Served Area (ha)	Contributing Area	1980 (Person)	2000 (Person)	1980 (Person/ha)	2000 (Person/ha)	1980	2000
		(114)					c	061
N-1	372.7	2,378.3	5,166	22,578	13.9	9.06	>	077
	210.1	1	006,6	20,270	47.1	96.5	8	160
1 (1	25.5	ţ	798	3,045	31.3	119.4	70	200
` <	162.0	ı	11,890	16,047	73.4	1.66	120	160
t v	5 69	1	6,959	7,904	100.1	113.7	200	200
۷ (72.3	1	7,286	7,024	9.96	97.2	160	160
) [2.2.7	ı	2,604	3,252	60.2	67.5	08	120
- α	255.0	ı	13,559	21,380	53.2	83.8	8	160
) _,	342.7	39.4	1	14,550	i I	42.5	0	8
, <u>[</u>	- 199	63.5	1,724	2,060	26.5	31.6	70	70
) i %	169.8	7.7	3,691	3,360	21.7	19.8	700	0
ו ניי	11.8	ŧ	863	1,056	73.1	89.5	120	160
) 4	5 5 3 6	1	2,898	2,016	53.8	37.4	8	040
- ເ r	156.1	1	6,035	13,441	38.7	98.9	70	160
, vc	96.7	1	1,620	5,140	16.8	63.5	0	120
2	110.8	1	4,941	10,212	44.6	92.2	80	160
- ∞	539.2	* 1	22,574	56,732	41.9	66.7	8	160
		ł						i
-								

(to be Cont'd)

Table G.1. (Cont.)

1980	Served Population
27.00	
	ı
	1
	1
1.6	1,591.
	1
	ı
	ì
	ı
	ŧ
0.1	870.1
4.0	164.0
8	1,008.3
7.4	597.4
T.	1,751.1
1.6	8,151.6
16,623.0	

Notes: * Excluding 2,817 persons ** Excluding inhabitants of North Port area

Table G.2. Ratio of Flooded Area

		Ratio of				Ratio of	
				:		Flooded Area	•
	Flooded	to	Rating		Flooded	ţ	Rating
Catchment	Area (ha)	Catchment Area	Points	Catchment Code No.	Area (ha)	Catchment Area (%)	Foince
, code	1911					:	
N-1	0	0	0	S-11	0	0) O
2	13.9	7	0	A- 1	25.2	en	0
, (*	3.7	15	0	2	26.2	20	50
) 7	15.9	10	0	en.	17.7	17	0
≻ L∕	18.2	26	50	7	14.6	28	50
n vo	29.1	07	100	ν.	72.3	72	150
7	30.7	79	150	9	0	H	0
œ	31.1	12	0	н	0	0	0
6	24.3	7	0	2	0	0	0
S L	0	0	0	۳	0		0
2	27.2	16	0	7	0	0	0
ო	2.5	21	20	Ŋ	0	0	0
7	7.5	14	0	9	0	0	0
Ŋ	145.1	66	200				
9	7.96	100	200				:
7	89.2	81	200				
œ	127.4	24	50				
6	30.9	56	50	· .		.: -:	
10	24.9	17	0				

Table G.3. Ratio of Estimated Stormwater Runoff to Existing Drain Capacity

1			•														- 1
Rating Points	07	80	200	200	120	80	200	07	200	07	707	07	0	707	70		·
Ratio of Estimated Stormwater Runoff to Existing Drain Capacity	8. 4	7.2	i	24.7	11.3	7.9	20.3	1.3	ı	1.4	1	2.3	1.0	3.1	2.4		
Catchment Code No.	8-9	10	11	A-1	2	: m	7	· •	9	0-1	2	m	7	· ω	9		
Rating Points	200	80	200	07	07	07	07	120	160	07	80	40	80	08	120	160	200
Ratio of Estimated Stormwater Runoff to Existing Drain Capacity	25.0	6.5	1	2.4	1.0	3.8	o m	13.2	15,9	8°°°	8.2	1.9	6.3	9.9	14.5	19.5	36.7
Catchment Code No.	N-1	2	m	7	'n	9	7	œ	<u>_</u>	S-1		ო	4	٠ ٠ ٧٦	9	7	œ

Table G.4. Ratio of Commercial & Industrial Land

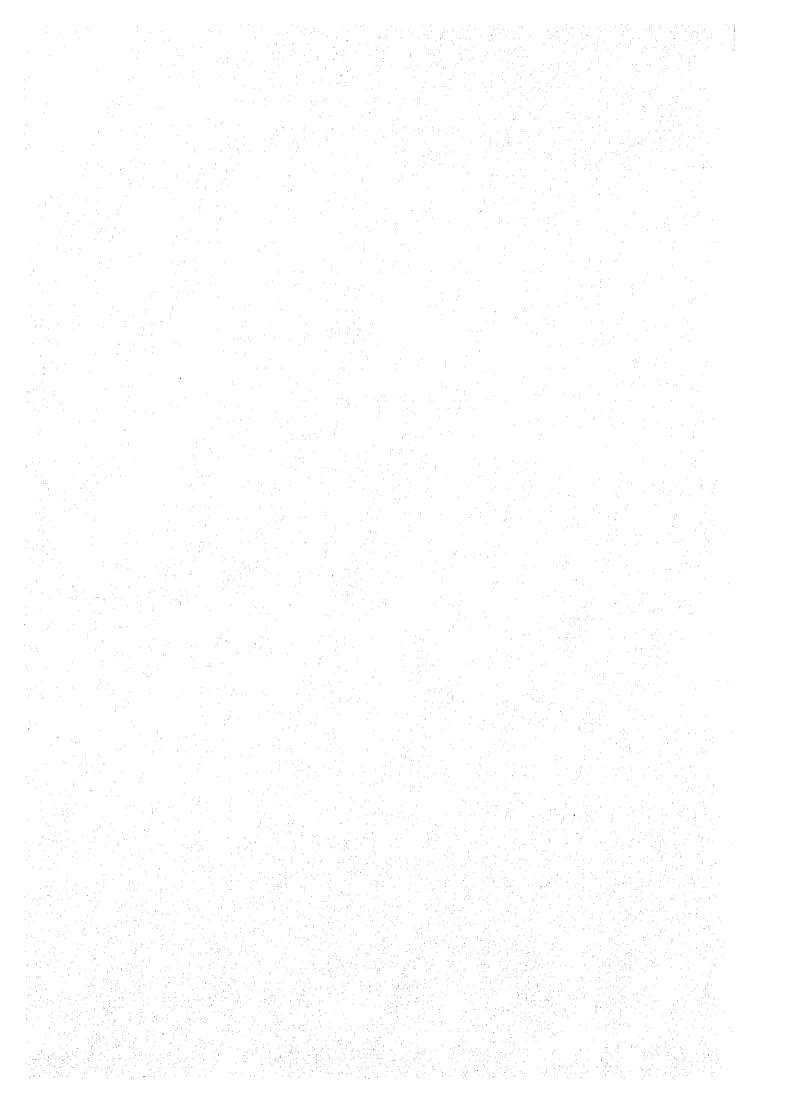
nage & Ind					
	& Industrial Use Areas to Catchment Area *	Rating Points	Drainage Catchment Code No.	& Industrial Use Areas to Catchment Area * (%)	Rating Points
	(9)				(
N-1	22.5	25	S-9	14.9	5
2	1.6	0	07	6•99	75
	2.0	0	i-1 i-1	97.8	100
7	14.4	0	A-1	1.2	0
	31.4	25	2	30.5	25
	63.8	75	ო	25.1	25
, ,	86.3	100	7	65.7	75
- 00	21.3	25	ıń	80.4	100
) o	24.9	25	9	64.7	75
, 1	1	0	0-1	31.1	25
2	10.1	0	. 2	56.4	20
<u> </u>	74.6	75	٣	13.2	0
7	65.7	75	7	E,	0
٠,	8.0	0	Ŋ	50.7	20
vo	7.3	0	9	5.3	0
7	1.4	0			
∞	10.3	0			

Note: * Excluding contributing area

APPENDIX H

COST ESTIMATION

(Ref.: Section 5.3.4., Vol. VI)



a. Trunk Drain

Line *	Length	Width x Height	Construc- tion Cost	Land Acqui-	Remarks
<u> </u>	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
i	400	$R = \frac{29.5}{23.5} \times 3.0$	664	3,131	
2	220	$R = \frac{5.0}{2.0} \times 1.5$	180	585	·
3	270	$R = \frac{29.5}{23.5} \times 3.0$	448	2,360	
4	2,360	$R = \frac{8.8}{4.0} \times 2.4$	2,572	- ·	
5	1,240	$R = \frac{9.3}{4.5} \times 2.4$	1,364	· <u>-</u>	
6	300	$R = \frac{11.2}{6.0} \times 2.6$	356	88	
7	170	$R = \frac{11.2}{6.0} \times 2.6$	201	633	
	50	B 4-2.8 x 2.6	426	. : .	
8	363	$R = \frac{30.4}{24.0} \times 3.2$	635	1,647	
	7	Br 30.4	638		
9	746	$R = \frac{30.4}{24.0} \times 3.2$	1,306	152	
	14	Br 2-30.4	1,276	-	•
					•
Sub Total	6,140 m		10,066		
b. Tidal	Gate		665	the second second	
c. Retent	ion Pond		- -		
d. Bund					* · ·
Total	· .		10,731	ing selection of the se	
B. Land Acqu	isition Cost		-	8,596	
C. Engineeri	ng Fee		1,610	in the second se	
), Contingen	cy Cost		2,468		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel B : Box Culvert Br: Bridge

Line *	Length	111.1.1.1	- Nedala	Construc- tion Cost	Land Acqui- tion Cost	Remarks
10.	(m)	(m)	x Height (m)	(M\$1,000)	(M\$1,000)	
1	910	R	8.2 4.0 × 2.1	919	1,492	
2	450	С	4.1 x 2.2	878	- -	
•	50	В 2	-2.05 x 2.2	140	•	
3	650	R	5.4 2.0 × 1.7	569	962	· .
4	200	R	$\frac{6.1}{2.5} \times 1.8$	182	226	
	50	B 2	-3.05 x 1.8	153		
5	430	R	8.9 4.5 × 2.2	449	50	
	20	В 3	-3.00 x 2.2	129	•	
6	680	R	4.8 2.0 × 1.4	541	69	
7	140	R .	0.0 5.0 x 2.5	160	36	
	20	В 4	-2.5 x 2.5	155	-	

	Sub Total 3,600 m	4,275	
	b. Tidal Gate	210	
	c. Retention Pond	i.	
	d. Bund		
	Total	4,485	
В.	Land Acquisition Cost	-	2,835
c.	Engineering Fee	<u>673</u>	
D.	Contingency Cost	1,032	
		,	
	GRAND TOTAL	9,025	

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel

B : Box Culvert

C : Railway Crossing

Catchment N-3

A. Construction Cost

a. Trunk Drain

Line* No.	Length (m)	Width x Height (m) (m)	Construction Cost (M\$1,000)	Land Acqui- tion Cost (M\$1,000)	Remarks
1	320	$R = \frac{5.0}{2.0} \times 1.5$	262	35	
	20	B 2-2.5 x 1.5	51	.	
2 .	400	$R = \frac{4.8}{2.0} \times 1.4$	318	41	
3	40	$R = \frac{6.3}{2.5} \times 1.9$	37		

·	Sub T	otal 780 m	· .	668	
	ь. Т	idal Gate		140	
٠.	c. I	Recention Pond			wa n
	d. I	Bund		112	
	Total	ı		920	
в.	Land	Acquisition Cost		en e	<u>76</u>
c.	Engi	neering Fee		138	
р.	Cont	ingency Cost		<u>212</u>	
		GRAND TOTAL		1,346	

* Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel B : Box Culvert

Line* No.	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1	400	$R = \frac{6.4}{3.0} \times 1.7$	354	370	
2	610	$R = \frac{7.0}{3.0} \times 2.0$	592	-	
3	, 700	$R = \frac{8.6}{4.0} \times 2.3$	746	25	
	70	В 3-2.9 ж 2.3	455	-	
4	800	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	720	101	
5	60	R 9.5×2.5	678	15	
	20	B 3-3.2 x 2.5	150	<u>-</u>	

	Sub	Total 2,660 m			3,695	÷		
	b .	Tidal Gate			200			
	c.	Retention Pond			- -		100	
	d.	Bund			117			
	Tota	al	* * .	•	4,012			
В.	Lan	d Acquisition Cost			-		<u>511</u>	
c.	Eng	ineering Fee			602			
D.	Con	tingency Cost	•		923			
		GRAND TOTAL			6,048			

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel B : Box Culvert

	a, Itume Di								
	Line*	Length	Wic	lth x	Helght	Construc- tion Cost	Land Acc	u1- t	Remarks
		(m)		n)	(m)	(M\$1,000)	(M\$1,00	0)	
	1	135	С		x 1.4	99	_		
		20	В	1.4	x 1.4	24	-		
	`2	360	С		x 2.0	421			
		40	В	2.0	x 2.0	74	-		. "
	3	320	С	2.3	x 2.3	 448			
		80	В	2,3	x 2.3	174		٠	
	4	'55	С	2.4	x 2.4	. 81.	-		
		20	В	2.4	x 2.4	45	, 		
	5	100	c	2.6	x 2.6	165	-		
		70	В	2.6	x 2.6	172	-		
	6	145	С	2.7	x 2.7	251			
		20	В	2.7	x 2.7	51	· <u>-</u>		
	7	40	С	2.8	x 2.8	72			
	,	20	В	2.8	3 x 2.8	53	-		
	•		•		;				
	Sub Total	1,425 m				2,130	· :		
	b. Tidal	Gate				155			
	c. Retent	Ion Pond				·	_		
	d. Bund					8			
	Total				•	2,293			
	Iotai								
В.	Land Acqui	sition Cost				-	<u>.</u>		
		. P				<u>344</u>			
c.	Engineerin	g ree				<u> </u>	•.		
D.	Contingenc	y Cost			1.	<u>527</u>		* 1	
	•					 			
							and the second second		and the second of the second o

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

GRAND TOTAL

3,164

C : Concrete Channel
B : Box Culvert

a,	Trunk	Drain

	Line* No. Length	Width x Height	Construc- tion Cost	Land Acqui- sition Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
	1 100	C 1.5 x 1.5	80	••	
	20	B 1.5 x 1.5	26	 .	
	.2 160	C 1.5 x 1,5	128		
	3 555	c 1.7 x 1.7	527	·	
	70	B 1.7 x 1.7	105	p.e	
	4 270	C 1.8 x 1.8	278	-	
	70	B 1.8 x 1.8	116		
	5 270	C 1.3 x 1.3	176	-	
	6 155	C 1.3 x 1.3	101	· ·	
	20	B 1.3 x 1.3	21	-	
	7 185	C 1.6 x 1.6	161	· _	
	20	B 1.6 x 1.6	28		
	8 210	C 1.7 x 1.7	200		
	20	B 1.7 x 1.7	30	-	
•	9 300	C 1.8 x 1.8	309		
	20	B 1.8 x 1.8	33		
	10 125	B 2.1 x 2.1	244	<u>.</u>	Depth of Box >1.5m Culvert
	11 35	B 2.1 x 2.1	82		H
	12 295	B 2.1 x 2.1	693		: n
	13 85	B 2.4 x 2.4	225	- · · · · · · · · · · · · · · · · · · ·	u
	Sub Total 2,985 m		3,563		
	b. Tidal Gate		155		
	c. Retention Pond		-		
	d. Bund		24		
	Total		3,742		
В.	Land Acquisition Cost		-	-	4 (2) 4 (4) (4) (4) (4) (4) (4) (4) (4) (4)
c.	Engineering Fee	•	<u>561</u>		
D.	Contingency Cost		861		
	GRAND TOTAL		5,164		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel
B : Box Culvert

Line *	Length	Width x Height (m) (m)	Construction Cost (M\$1,000)	Land Acqui- tion Cost (M\$1,000)	Remarks
1	125	C 1.2 x 1.2	75	-	
2	25	C 1.4 x 1.4	18		
' 3	20	B 1.6 x 1.6	28	-	
4	180	C 1.9 x 1.9	198	•••	
5	220	C 2.1 x 2.1	275	-	
6	265	С 0.8 х 0.8	80	, see	
7	340	C 1.7 x 1.7	323	••• •	•
	20	B 1.7 x 1.7	30	-	
8	210	C 2.1 x 2.1	263	115	
	30	B 2.1 x 2.1	59	-	
9	20	C 2.9 x 2.9	37	-	
			*		

Sub	Total 1,455 m	1,386		÷
ъ.	Tidal Gate	155		
c.	Retention Pond	· _		
d,	Bund	50		
Tot	al	1,591		
B. Lan	d Acquisition Cost	~ .	<u>115</u>	
C. Eng	ineering Fee	239		
D. Con	ntingency Cost	<u>366</u>		
	GRAND TOTAL	2,311		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel
B : Box Culvert

a. Trunk Drain

Line *	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1 .	1,200	$R = \frac{7.9}{3.5} \times 2.2$	1,236	130	
.2	1,380	$R = \frac{6.8}{3.0} \times 1.9$	1,297	248	
•	20	в 3-2.3 х 1.9	65	-	
3	80	$R = {10.0 \atop 5.0} \times 2.5$	91	21	
	20	B 3-3.35 x 2.5	155		
4	870	C 2.8 x 1.7	1,131	-	
5	1,200	C 2.9 x 1.8	1,656	· -	
6	293	$R = \frac{13.0}{8.0} \times 2.5$	349	40	
	7	Br 13.0	273	-	

Sub Total 5,070 m	6,253	
b. Tidal Gate	260	
c. Retention Pond	• • • • • • • • • • • • • • • • • • •	
d. Bund	71	
Total	6,584	
B. Land Acquisition Cost	- ,	439
C. Engineering Fee	988	
D. Contingency Cost	<u>1,514</u>	Average and the second
GRAND TOTAL	9,525	

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel R : Rubble Wall Channel

B : Box Culvert

Br: Bridge

a. Trunk Drain

Line *	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1	380	$R = \frac{8.1}{4.5} \times 1.8$	353	76	
2	1,100	$R = \frac{8.3}{4.5} \times 1.9$	1,056	-	
,	. 40	в 3-2.8 х 1.9	203	-	
3	500	$R = \frac{7.2}{3.0} \times 2.1$	498	87	-
4	650	$R = \frac{9.1}{4.5} \times 2.3$	696	103	
•	20	в 3-3.05 х 2.3	135	: . -	
5	1,240	$R = \frac{12.8}{8.0} \times 2.4$	1,438	283	
6	880	$R = \frac{5.4}{2.0} \times 1.7$	770	106	
7	200	$R = {14.0 \atop 9.0} \times 2.5$	241	67	·
8	486	$R = {14.0 \atop 9.0} \times 2.5$	586	322	
	14	Br 14.0	588	-	

	Sub Total 5,510 m		6,564	
•	b. Tidal Gate		275	
•	c. Retention Pond			
	d. Bund		·-	
	Total	E 1	6,839	
В.	Land Acquisition Cost		· . . -	1,044
c.	Engineering Fee		1,026	
D.	Contingency Cost		1,573	

10,482

GRAND TOTAL

R : Rubble Wall Channel B : Box Culvert Br: Bridge

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

a. Trunk Drain

Line *	Length	Wie	dth x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(1	m) (m)	(M\$1,000)	(M\$1,000)	
1	(800)	R	7.0 x 2.0	-		Outside of the Project Area
,2	(600)	R	7.5 x 2.0		-	11
	(20)	В	3-2.5 x 2.0		<u></u>	tt
	(20)	В	3-2.5 x 2.0	,-	- -	· n
3	180	R	8.0 4.0 x 2.0	176	33	

			e e
	Sub Total 180 m		176
	b. Tidal Gate		165
	c. Retention Pond		
	d. Bund	."	
	Total		<u>341</u>
В.	Land Acquisition Cost		- 33
C.	Engineering Fee		<u>51</u>
	$\mathcal{L}_{\mathcal{L}}(\mathcal{L}_{\mathcal{L}})$ (2.2)		
D.	Contingency Cost		78
	GRAND TOTAL		503

* Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel B : Box Culvert

Line * No.	Length (m)	W1di (m)	th x Height) (m)	Construction Cost (M\$1,000)	Land Acqui- tion Cost (M\$1,000)	Remarks
. 1	780	R	7.8 4.0 x 1.9	745	187	
.2	510	R	$\frac{7.8}{4.0}$ x 1.9	487	129	
	. 20	В	3-2.6 x 1.9	92	·	
3	380	R	$\frac{4.2}{1.0} \times 1.6$	319	71	
4	80	R	$_{5.0}^{9.0}$ x 2.0	80	1	•
	20	В	3-3.0 x 2.0	555		Cross the railway line

	Sub Total 1,790 m;	2,278	
	b. Tidal Gate	180	
	c. Retention Pond		
	d. Bund	112	
	Total	<u>2,570</u>	
В.	Land Acquisition Cost	<u> 388</u>	. •
C	Engineering Fee	<u>386</u>	
D,	Contingency Cost	<u>591</u>	
	GRAND TOTAL	<u>3,935</u>	

R: Rubble Wall Channel B: Box Culvert

Line *	Length (m)	Width x Height (m) (m)	Construction Cost (M\$1,000)	Land Acqui- tion Cost (M\$1,000)	Remarks
1	330	c 1.6 x 1.6	284	83	
	20	B 1.6 x 1.6	150		Cross the railway line

	Sub Total 350 m	434	e e er	
÷	b. Tidal Gate	100		
	c. Retention Pond	.		•
	d. Bund	15		
	Total	549		
В.	Land Acquisition Cost	· -	83	
c.	Engineering Pee	<u>82</u>		
D.	Contingency Cost	126		
	CRAND TOTAL	840		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel B : Box Culvert

Line*	Length (m)	Width x Height (m) (m)	Construction Cost (M\$1,000)	Land Acqui- tion Cost (M\$1,000)	Remarks
1	680	c 2.7 x 2.0	952	82	
	20	B 2.7 x 2.0	44	<u></u>	

	Sub Total 700 m		996		
	b. Tidal Gate	, 1	120		
	c. Retention Pond		-	:	
	d. Bund		20		
	Total		1,136		
В,	Land Acquisition Cost	•	-	82	
c.	Engineering Fee		<u>170</u>		
D.	Contingency Cost	tions	<u>261</u>		
	GRAND TOTAL		1,649		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI $\,\cdot\,$

C : Concrete Channel
B : Box Culvert

Line*	Tanash	1936b - 19-44b	Construc-	Land Acqui- tion Cost	Remarks
No.	Length	Width x Height	tion Cost	LION COST	Kenarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1	460	$R = \frac{4.1}{1.5} \times 1.3$	352		
	20	B 2-2.05 x 1.3	43	_	
2	435	C 3.8 x 1.8	718		
	20	B 2-1.9 x 1.8	50		
3	760	C 5.8 x 2.0	1,702	502	
	40	в 2-2.9 х 2.0	125	crea	
4	1,780	C 3.4 x 1.8	2,706	<u>.</u>	
	20	В 2-1.7 х 1.8	46	- .	
5	780	$R = \frac{9.2}{5.0} \times 2.1$	800	-	
·	20	B 3-3.3 x 2.1	630	.	Cross the railway line

٠.	Sub Total 4,335 m	7,172		
	b. Tidal Gate	180	$x^{\prime\prime}$	
	c. Retention Pond	<u>-</u>		
	d. Bund	72		
	Total	7,424		
В.	Land Acquisition Cost	-	502	
C.	Engineering Fee	1,080		
D.	Contingency Cost	1,656		: :
	GRAND TOTAL	10,662		

^{*} Line Nos are shown in Fig. 5.8, of Vol.VI

C : Concrete Channel

R : Rubble Wall Channel

B : Box Culvert

a. Trunk Drain

Line*	Length	Wic	lth x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(r	m) (m)	(M\$1,000)	(M\$1,000)	
· 1	830	R	7.6 4.0 x 1.8	768	169	
	20	В	3-2.55 x 1.8	83	 ,	
2	400	R	$\frac{12.1}{7.5}$ x 2.3	444	170	
3	50	R	$\frac{12.1}{7.5}$ x 2.3	56	-	

	Sub Total 1,300 m	1,351	•	
	b. Tidal Gate	238	•	
	c. Retention Pond	1,480	2,160	$V = 118,000 \text{ m}^3$ $A = 54,000 \text{ m}^2$
	d. Bund	122		
	Total	3,191		
В.	Land Acquisition Cost		2,499	
c.	Engineering Fee	1,107		
D.	Contingency Cost	1,698		
	GRAND TOTAL	8,495	· · · · · · · · · · · · · · · · · · ·	·

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel

B : Box Culvert

Line *	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	•
1	800	$R = \frac{5.4}{2.0} \times 1.7$	704	120	
2	580	$R = \frac{9.3}{5.5} \times 1.9$	568	81	
	20	B 3-3,1 x 1.9	114	-	
3	180	$R = \frac{9.5}{5.5} \times 2.0$	18 0	28	
	20	B 3-3.2 x 2.0	125	-	

	Sub Total 1,600 m	1,691	
	b. Tidal Gate	185	
	c. Retention Pond	<u></u>	
	d. Bund	12	
	Total	1,888	
В,	Land Acquisition Cost	· -	229
c.	Engineering Fee	<u>450</u>	
D.	Contingency Cost	<u>688</u>	$\mathcal{J}_{i} = 0$
	GRAND TOTAL	<u>3,255</u>	14 - 4

- R : Rubble Wall Channel B : Concrete Channel

a. Trunk Drain

	Line *	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
		(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
	1	1,200	$R = \frac{15.6}{10.0} \times 2.8$	1,584	2,592	
	2	1,200	$R = \frac{15.6}{10.0} \times 2.8$	1,584	2,832	
	3	493	$R = \frac{15.6}{10.0} \times 2.8$	651	211	
		7	Br 15.6	328	≟ :	e.
	4	980	$R = \frac{7.2}{3.0} \times 2.1$	975	90	
		20	B 2-3.6 x 2.1	94	- .	
	. 5	200	$R = \frac{15.8}{10.0} \times 2.9$	270	74	
	6	630	C 3.7 x 2.0	1,077	-	
	7	1,810	C 6.0 x 2.8	6,570	-	
		60	B 2-3.0 x 2.8	325		20m/No. x 3 No. = 60m
	. 8	300	R 18.8 x 2.9	423	122	
	9	257	$R = \frac{18.8}{13.0} \times 2.9$	362	67	
		10	13.0 Br 18.8	1,045	-	Cross the railway line
			•			•
	Sub Total	4,167 m		15,288		•
	b. Tidal	Gate		390		· · · · · · · · · · · · · · · · · · ·
	o Parent	ion Pond				
	c Retent	Zon rond				
	d Bund					
	Total			15,678		
_					5,988	
В.	Land Acqui	sition Cost			3,733	
C.	Engineerin	ng Fee		2,352		
D.	Contingenc	y Cost		3,606		
				· · · · · · · · · · · · · · · · · · ·		
	GRANI	TOTAL		27,624		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel

C : Concrete Channel
B : Box Culvert

Br: Bridge

Line*	Length (m)		th x Height (m) (m)	Construction Cost (M\$1,000)	Land Acqui- tion Cost (M\$1,000)	Remarks
1	1,060	c	2.3 x 1.7	1,219	-	
	40	В	2.3 x 1.7	74		2-Box Culverts
2	960	С	2.5 x 2.0	1,296	_	
	40	В	2.5 x 2.0	83	· -	
3	290	R	$\frac{8.5}{4.5} \times 2.0$	287	191	
4	520	R	$\frac{6.7}{3.5}$ x 1.6	450		
	20	В	3-2,25 x 1,6	61	-	
5	100	В	3-2.4 x 2.1	2,350	-	Cross the railway line

	Sub Total 3,030 m	5,820		
	b. Tidal Gate	185		
	c. Retention Pond	960	960	$v = 70,000 \text{ m}^3$
	d. Bund Total	6,965		
В.	Land Acquisition Cost		1,151	
С.	Engineering Fee	1,045		
D.	Contingency Cost	1,602		
	GRAND TOTAL	10,763		- And Andrews -

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel

R : Rubble Wall Channel B : Box Culvert

a. Trunk Dr	ain
-------------	-----

Line* No.	Length	Width × Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1	1,190	C 4.0 x 2.0	2,142	e- ,	
•	20	$B = 2-2.0 \times 2.0$	53	-	
2	1,080	C 2.5 x 1.9	1,404		
	20	B 2.5 x 1.9	41	e==	
3	380	C 5.4 x 2.5	1,140	**	a tho
	20	B 2-2.7 x 2.5	452		Cross the railway line
4	860	c 3,3 x 2.0	1,376	131	
	20	B 3.3 x 2.0	265		Cross the railway line
5	90	$R = \begin{array}{c} 10.5 \\ 5.5 \end{array} \times 2.5$	104	-	

	Sub Total 3,680 m		6,977		
	b. Tidal Gate		215		
	c. Retention Pond		1,160	1,230	$V = 88,000 \text{ m}^3$
	d. Bund Total		8,352		
В.	Land Acquisition Cost		-	1,361	
c.	Engineering Fee		1,253		
D.	Contingency Cost	. •	• 1,921		· · · · · · · · · · · · · · · · · · ·
	GRAND TOTAL		12,887		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel
R : Rubble Wall Channel
B : Box Culvert

a. Trunk Drain

(m) (m) (m) (k§1,000) (k§1,000) 1 400 R 5.5 x 2.5 458 562 2 1,500 R 11.5 x 2.0 1,545 2,727 3 490 R 13.0 x 2.5 583 794 4 1,300 R 6.8 x 1.9 1,222 1,240 5 240 R 19.0 x 2.5 289 428 6 1,263 R 15.5 x 2.5 1,553 165 7 Br 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 2.0 x 2.7 1,970 346 9 1,320 R 2.0 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 23,534		Line*	Length	Width x Height	Construc- tion Cost	Land Acqui- sition Cost	Remarks
2 1,500 R 11.5 x 2.0 1,545 2,727 3 490 R 13.0 x 2.5 583 794 4 1,300 R 6.8 x 1.9 1,222 1,240 5 240 R 14.0 x 2.5 289 428 6 1,263 R 15.5 x 2.5 1,553 165 7 Br 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 1.7 1,970 346 9 1,320 R 5.4 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond d. Bund (for A-6 Catchment) 83 Total 13.075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2.5 583 794			•		(M\$1,000)	(M\$1,000)	
3 490 R 13.0 x 2.5 583 794 4 1,300 R 6.8 x 1.9 1,222 1,240 5 240 R 14.0 x 2.5 289 428 6 1,263 R 15.5 x 2.5 1,553 165 7 Br 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 3.5 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond d. Bund (for A-6 Catchment) 83 Total 13.075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2.5 289 428 1,240 x 1.9 1,222 1,240 1,24		. 1	400	$R = \frac{10.5}{5.5} \times 2.5$	458	562	
4 1,300 R 6.8 x 1.9 1,222 1,240 5 240 R 14.0 x 2.5 289 428 6 1,263 R 15.5 x 2.5 1,553 165 7 Rr 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 2.0 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Cate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2.5 1,553 165 1 1,400 x 2.534 2.5 1		2	1,500	$R = \frac{11.5}{7.5} \times 2.0$	1,545	2,727	
5 240 R 14.0 x 2.5 289 428 6 1,263 R 15.5 x 2.5 1,553 165 7 Br 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Fond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2.5 1,553 165		. 3	490	$R = \frac{13.0}{8.0} \times 2.5$	583	794	
6 1,263 R 15.5 x 2.5 1,553 165 7 Br 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		4	1,300	$R = \frac{6.8}{3.0} \times 1.9$	1,222	1,240	
7 Br 15.5 326 - 7 2,040 R 6.8 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2.534		5 .	240	$R = \frac{14.0}{9.0} \times 2.5$	289	428	
7 2,040 R 3.0 x 1.9 1,918 - 8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		6	1,263	$R = \frac{15.5}{10.5} \times 2.5$	1,553	165	
8 1,340 R 25.4 x 2.7 1,970 346 9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.5 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for Λ-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534			7	Br 15.5	326		
9 1,320 R 5.4 x 1.7 1,155 - 20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		7	2,040		1,918	. -	
20 B 3-1.8 x 1.7 56 - 10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		8	1,340		1,970	346	·
10 870 R 7.7 x 2.1 870 55 11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		9	1,320	R $\begin{array}{c} 5.4 \\ 2.0 \end{array}$ x 1.7	1,155	-	
11 13 R 25.4 x 2.7 19 - 7 Br 25.4 533 - Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534			20	B 3-1.8 x 1.7	. 56	·	
7 Br 25.4 533 Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		10	870	$R = \frac{7.7}{3.5} \times 2.1$	870	55	
Sub Total 10,810 m 12,497 b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		11	13	$R = \frac{25.4}{20.0} \times 2.7$	19	_	
b. Tidal Gate 495 c. Retention Pond - d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534			7	Br 25.4	533		
c. Retention Pond d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		Sub Total	10,810 m		12,497	÷	
d. Bund (for A-6 Catchment) 83 Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		b. Tidal G	ate		495		
Total 13,075 B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		c. Retenti	on Pond		-		
B. Land Acquisition Cost - 6,317 C. Engineering Fee 1,608 D. Contingency Cost 2,534		d. Bund (f	or A-6 Cat	chment)	83		
C. Engineering Fee 1,608 D. Contingency Cost 2,534		Total			13,075		
C. Engineering Fee 1,608 D. Contingency Cost 2,534	В.	Land Acquis	ition Cost		-	6,317	
D. Contingency Cost 2,534		•			ers.		
	c.	Engineering	Fee		1,608		
7	D.	Contingency	Cost	• iA	2,534		
GRAND TOTAL $\underline{23,534}$		· · · · · · · · · · · · · · · · · · ·				<u> </u>	
		GRAND	TOTAL		23,534		

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

R : Rubble Wall Channel Br: Bridge

a. Trunk Drain

Line* No.	Length	Wid	th x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m	(m)	(M\$1,000)	(M\$1,000)	
1	980	C	3.5 x 2.1	1,676		
	20	В	3.5 x 2.1	51		
2	1,160	С	3.0 x 2.1	1,798	••	
	40	В	3.0 x 2.1	94	-	
3	430	c	1.9 x 1.5	404	· -	
4	560	С	5.0 x 2.3	1,238	-	
	40	В	2-2.5 x 2.3	124	-	

	Sub Total 3,230 m		5,385	
	b. Tidal Gate	•	155	
	c. Retention Pond		-	
	d. Bund		-	
	Total		5,540	
В.	Land Acquisition Cost		-	en Sur en d e en
c.	Engineering Fee		<u>831</u>	
D.	Contingency Cost		1,274	
	GRAND TOTAL		7,645	

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel
B : Box Culvert

a. Trunk Drain

Line* No.	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)		(M\$1,000)	(M\$1,000)	
1	300	$R = \frac{4.5}{1.5} \times 1.5$	245	~	
2	700	$R = \frac{5.9}{2.5} \times 1.7$	616		
3	780	C 3.3 x 1.8	1,162	-	
	40	B 3.3 x 1.8	92	-	
4	1,000	C 1.8 x 1.5	900	-	
5	530	C 3.2 x 1.6	721	-	
	40	B 2-1.6 x 1.6	84	-	
6	560	C 4.3 x 2.1	1,092	-	
7	250	$R = \frac{9.1}{4.5} \times 2.3$	269	-	

	Sub Total 4,200 m	5,181
-	b. Tidal Gate	190
	c. Retention Pond	<u>.</u>
	d. Bund	29
	Total	5,400
В.	Land Acquisition Cost	· · · · · · · · · · · · · · · · · · ·
c.	Engineering Fee	<u>810</u>
D.	Contingency Cost	1,242
	GRAND TOTAL	<u>7,452</u>

C : Concrete Channel
R : Rubble Wall Channel
B : Box Culvert

Line*	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1	190	c 1.1 x 1.1	95	. <u>-</u>	
2	215	C 1.5 x 1.5	172	-	
•	20	B 1.5 x 1.5	26		
3	30	C 2.0 x 2.0	35	-	
	20	B 2.0 x 2.0	37		
4	255	C 2.2 x 2.2	339	***	
5	35	B 2.4 x 2.4	79	·	
· 6	345	C 1.6 x 1.6	300	-	
	20	B 1,6 x 1.6	28	-	
7	60	C 1.7 x 1.7	57	-	
8	305	c 1.7 x 1.7	290	-	
9	80	C 2.7 x 2.7	138	-	
10	20	C 2.9 x 2.9	37	-	

Sub Total 1,59	5 m	1,633	
b. Tidal Gate		1.55	
c. Retention Pon	nd	- -	
d. Bund	·	+ + + + + + + + + + + + + + + + + + +	
Total		<u>1,788</u>	
B. Land Acquisition	Cost		
C. Engineering Fee		<u>268</u>	
D. Contingency Cost		411	
GRAND TOTAL		2,46 <u>7</u>	

^{*} Line Nos are shown in Fig. 5.8. of Vol.VI

C : Concrete Channel
B : Box Culvert

a. Trunk Drain

Line*	Length	Wid	th x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(n	n) (m)	(M\$1,000)	(M\$1,000)	
1	730	С	2. x 1.7	883	-	
	20	В	2.5 x 1.7	38	- .	
2	780	С	3.5 x 2.1	1,334	-	
	20	В	3.5 x 2.1	51	-	
3	230	В	$\frac{7.7}{3.5} \times 2.1$	230	76	
4	50	R	$^{8.2}_{4.0}$ x 2.1	51	4 .	

Sub Total 1,830 m	2,587	
b, Tidal Gate	170	
c. Retention Pond	770	1,080 $V = 53,000 \text{ m}^3$
d Bund	-	
Total	3,527	
B. Land Acquisition Cost		1,160
C. Engineering Fee	<u>529</u>	
D. Contingency Cost	<u>811</u>	
GRAND TOTAL	6,027	

^{*} Line Nos are shown in Fig. 5.8. of Vol.Vİ

C : Concrete Channel

R : Rubble Wall Channel B : Box Culvert

a.	Trunk	Drain

e	Line*		ر الم ال	th x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	No.	Length (m)	(m)		(M\$1,000)	(M\$1,000)	: ·
	1	(3,320)		8.8 4.0 × 2.4	<u></u>	- -	Outside of the Project Area
		20	B :	3-2.95 x 2.4	1.37	-	
	2	(2,833)	R	11.0 x 2.5	······································	-	Outside of the Project Area
		7	Br	11.0	231	- .	
:	3	2,000	R	$\frac{12.7}{7.5} \times 2.6$	2,420	882	٠.
	4	2,000	C	4.9 x 2.5	5,480	204	
	5	2,520	R	8.8 4.0 x 2.4	2,747	816	
		20	В	3-2.95 x 2.4	137	-	
	6	1,000	С	5.5 x 3.0	3,600	120	•
	7	73	R	$\frac{13.9}{7.5}$ x 3.2	101		·
	4 . •	7	Br	13.9	292	iæ-	
	. 5						
:	Sub Total	7,647 m			15,145		
	b. Tidal	Gate			-		
	;				· · · <u>-</u>		
	c. Retent	tion Pond					
	d, Bund				-		·
	Total			· · ·	15,145		
В.	Land Acqui	isition Cost			- :	2,022	
c.	Engineeri	ng Fee			2,272		
D.	Contingen	cy Cost			3,483		
			:	·			

^{*} Line Nos are shown in Fig. 5.9. of Vol.VI

GRAND TOTAL

22,922

C : Concrete Channel

R : Rubble Wall Channel
B : Box Culvert

Br: Bridge

a. Trunk Drain

Line*	Length	Wid	th x lleight	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1	(2,100)	R	8.6 4.0 x 2.3	· -	-	Outside of the Project Area
2	(1,800)	R	$\frac{7.2}{3.0} \times 2.1$, 		н
3	530	R	$\frac{11.0}{6.0}$ x 2.5	612	153	·
4	700	C .	3.0 x 2.0	1,050	. 32	
5	143	R	$^{11.2}_{6.0} \times 2.6$	169	. •	
	7	Br	11.2	235	-	

	Sub Total 1,380 m	2,066	
	b. Tidal Gate		
	c. Retention Pond	-	
	d. Bund	-	
	Total	2,066	
В.	Land Acquisition Cost	<u>185</u>	
c. :	Engineering Fee	310	
D.	Contingency Cost	<u>475</u>	
	GRAND TOTAL	3,036	

C : Concrete Channel
R : Rubble Wall Channel
Br: Bridge

Line*	Lèngth	Width x Height	Construc- tion Cost	Land Acqui- tion Cost	Remarks
	(m)	(m) (m)	(M\$1,000)	(M\$1,000)	
1,	(2,373)	$R = \frac{11.6}{6.0} \times 2.8$	-	, 	Outside of the Project Area
	7	Br 11.6	244	-	~ *
2	(650)	$R = \frac{12.2}{6.0} \times 3.1$	-	_	Outside of the Project Area
	600	$R = \frac{12.2}{6.0} \times 3.1$	798	194	
3	1,300	$R = \frac{12.7}{6.5} \times 3.1$	1,742	441	`
4	1,000	$R = \frac{12.7}{6.5} \times 3.1$	1,340	339	
5	700	C 1.6 x 1.6	602	~	
6	700	C 2.0 x 2.0	812	-	
7	73	$R = \frac{12.7}{6.5} \times 3.1$	98	-	
•	7	Br 12.7	267	- .	

	Sub Total 4,387 m		5,903	
	b. Tidal Gate		-	:
	c. Retention Pond		-	
	d. Bund	*	-	
	Total		5,903	
В.	Land Acquisition Cost		·	<u>974</u>
c.	Engineering Fee		885	
D.	Contingency Cost		1,358	
	GRAND TOTAL		9,120	

^{*} Line Nos are shown in Fig. 5.9. of Vol.VI

C : Concrete Channel R : Rubble Wall Channel Br: Bridge

a, Trunk Drain

Line*	Length	ength Width x Height		Construc- tion Cost	Land Acqui- tion Cost	Remarks
* .	(m)	(m	(m)	(M\$1,000)	(M\$1,000)	
1	2,100	c	2.8 x 2.5	3,528	82	
` 2	800	С	2.2 x 1.5	824	-	
3	173	R	$^{13.0}_{6.0} \times 3.5$	251	-	
	7	Br	13.0	273	-	

		,			
	Sub Total 3,080 m		4,876		
	b. Tidal Gate		-		
	c. Retention Pond		-		
	d. Bund		- .		
	Total		4,876		
В.	Land Acquisition Cost	•	-	82	
c.	Engineering Fee		<u>731</u>		
D.	Contingency Cost		1,121		
				·	
*** 	GRAND TOTAL		6,810		

* Line Nos are shown in Fig. 5.9. of Vol.VI

C : Concrete Channel
R : Rubble Wall Channel
Br: Bridge

Line *	Length	Width x Height	Construc- tion Cost	Land Acqui- tion Cost (M\$1,000)	Remarks
	(m)	(m) (m)	(M\$1,000)	(1191,000)	
1	620	$R = \frac{11.2}{6.0} \times 2.6$	735	182	
.2	1,500	C 4.6 x 2.5	3,315	140	
3	700	C 2.1 x 2.1	- 868	 .	
4	(2,053)	$R = \frac{16.2}{10.0} \times 3.1$	-		Outside of the Project Area
	7	Br 16.2	340		
5	(2,560)	$R = \frac{16.4}{10.0} \times 3.2$.	Outside of the Project Area
6	(3,660)	$R = \frac{16.4}{10.0} \times 3.2$		<u></u>	fl
7	293	$R = \frac{16.4}{10.0} \times 3.2$	425	162	
	7	Br 16.4	344		

	Sub Total 3,127 m		1 11 1	5,991	
	b. Tidal Gate		'	: <u>-</u>	
	c. Retention Pond				
	d. Bund	•		-	
	Total			<u>5,991</u>	
В.	Land Acquisition Cost			<u>.</u> -	484
С.	Engineering Fee			899	erina di Salamania. Ny INSEE dia mampiasa ny kaominina dia kaominina dia mpikambana ara-daharanjaraharanjaraharanjaraharanjarahara
D.	Contingency Cost			1,378	
	GFAND TOTAL			8,752	

^{*} Line Nos are shown in Fig. 5.10. of Vol.VI

C : Concrete Channel R : Rubble Wall Channel Br: Bridge

a. Trunk	Drain
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	d. Bund Total	ion Pond		- 24,987		
	Sub Total b. Tidal	and the second second		24,987		
	19	370	$R = \frac{34.2}{26.0} \times 4.1$	808		
		753 7	Br 11.4	239	~	
	17 18	980	C 2.3×2.0 R $\frac{11.4}{6.0} \times 2.7$	1,235 913	226	
	16	(2,660)	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	-	**	
	15	(3,000)	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	**		Outside of the Project Area
		20	$3-2.95 \times 2.4$	137	. –	
*. *		(2,600)	$R = \begin{array}{c} 8.8 \\ 4.0 \times 2.4 \end{array}$	-	-	Outside of the Project Area
	14	300	$R = \begin{array}{c} 8.8 \\ 4.0 \\ \end{array} \times 2.4$	327	67	
: .	13	630	R 7.8 x 2.4	677	185	4
	12	350	$R = \frac{33.2}{25.0} \times 4.1$	754		
٠		7	Br 17.6	370	-	
	11	493	$R = \begin{array}{c} 17.6 \\ 12.0 \end{array} \times 2.8$	670	290	
	10	220	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1,659	717	
		7 .	Br 29.8	626		
	9	693	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1,379	-	
	8 .	1,400	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2,660	1,016	
	7	2,250	$\begin{array}{ccc} R & 24.0 & \times & 3.0 \\ 18.0 & \times & 3.0 \end{array}$	3,488	1,391	·
	6	1,450	$R = \begin{array}{c} 24.0 \\ 18.0 \\ \end{array} \times 3.0$	2,248	896	
	. 5	1,300	$R = \begin{array}{c} 24.0 \\ 18.0 \\ \end{array} \times 3.0$	2,015	881	
	4	1,370	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2,124	929	
	•	7	Br 24.0	504	· 	
	3	693	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1,074	470	
	2	400	$R = \frac{11.4}{6.0} \times 2.7$	486	120	
	1	400	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	594	235	
	No.	Length (m)	Width x Height (m) (m)	tion Cost (M\$1,000)	sition Cost (M\$1,000)	Remarks

^{*} Line Nos are shown in Fig. 5.10, of Vol.VI