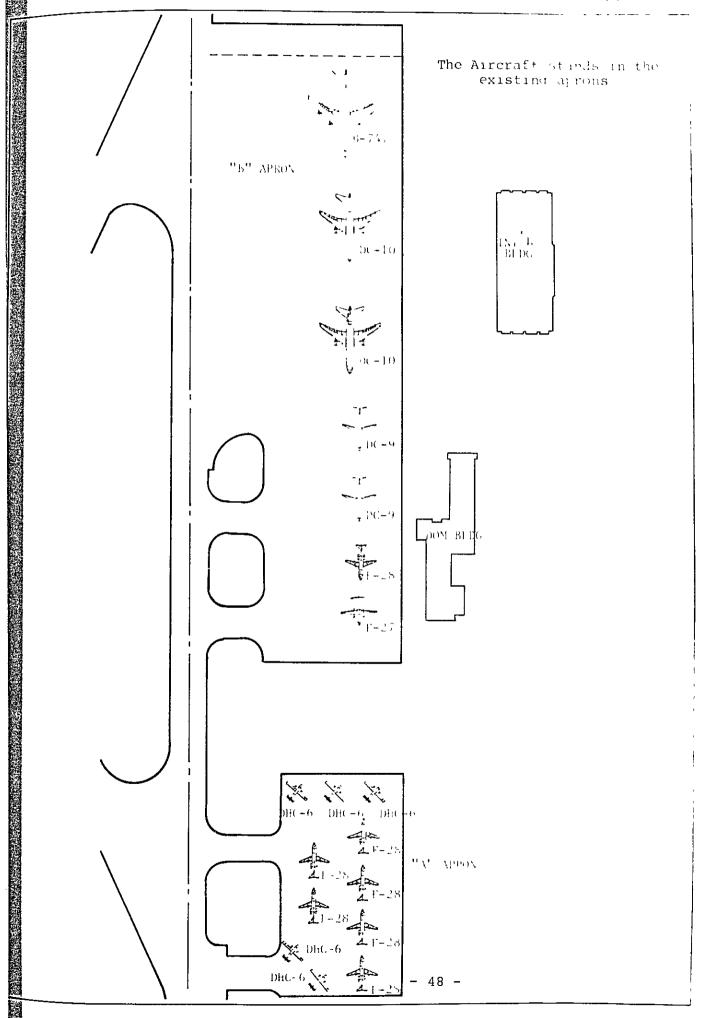
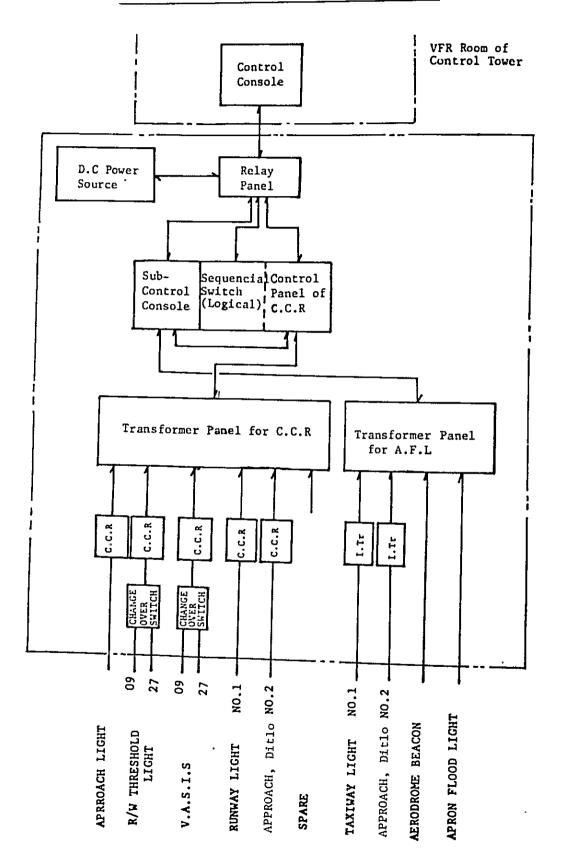
APPENDIX TO CHAPTER 6





CONTROL DIAGRAM OF AIRFIELD LIGHTING



EXISTING EQUIPMENT LIST OF AIR NAVIGATIONAL AIDS

EQUIPNENT	TYPE / MAKER	SET	FREQUENCY	IDENTIFI	P O CONSUMP	WER OUT PUT	ANTENNA	COVERING RANGE	REMARKS
							-		
NDF TRANSMITTER	G 91	7	230 KHZ	0 R	9.5 KVA	2.8 KW	VERTICAL	300 NML	
VOR TRANSMITTER	SEL KL 200 RD	2	115.5 MHZ	DPS	7 KVA	200 Watt	Combination	200 NML	
DMF TRANSPONDER	RYC 7004 7506 RAYTHEON	7	102 X	DPS	1.4 KVA	800 Watt	Discone	100 NML	
ILS TRANSMITTER a. Localizer	LS 371 LS 371 TCSF	2	110.3 NHZ	IDPS	2.4 KW	20 Watt	Par. Refl	S.18 NML	
b. Glide Path	LS 371 TCSF	2	335 MHZ	I	2.4 KW	D10W/CL.1W	Dipol Ref	10 NML	
G c. Marker Beacon	N318/52 NARDEUX	7	75 MHZ	ı	90 VA	2 Watt	Colinear Dipole		
DME TRANSPONDER	RYC 7004 7515 RAYTHION	2	X 04	DPS	1.4 KVA	800 Watt	Discone	40 NML	***
RADAR SET a. PSR	THOSON TVT	2	2750/2850- MHZ		NOM 7 KW	500 KW	Scurse - System	90 NML	A
b. SSR	THOMSON TVT	2	1030/1060- MHZ			2.5 KW	Rotating Dipole		PPEND:
D. VOR TRANSMITTER	RAYTHION RYC - 7004	2	116.2 MHZ	BLI	1.4 KVA	100 KW	Combination	200 NML	
									.3

EXISTING EQUIPMENT LIST OF AIR TRAFFIC INFORMATION SERVICE

REMARKS				
USE FOR	ATIS	1 = 1	; = I	Monitor ATIS
LOCATION	Beacon room A T I S	Radar	Tower	E Q R
ANTENNA INSTALLATION LOCATION TYPE TIME	1981	1981	1981	ı
	Dipole	ī	ı	
POWER OUT PUT	70 watt	ı	ţ	,
SET		H	н	-1
FREQUENCY	186.2 Mz	ı	1	ı
MAKER/TYPE	SU. 007	ASSMAN FAG . 100	FI A	SY - 200
EQUIPNENT	Tran mitter	Becorder/Repro- duser	Remote Control	Scanning Monitor Receiver

APPENDIZ 6.3.4

EXISTING EQUIPMENT LIST OF AIR TELECOMMUNICATION SYSTEM

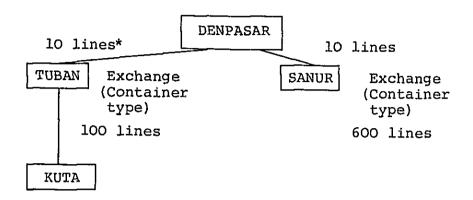
REMARKS							APPEN	DIX	ь.	3.4
USED FOR	HF Ground to Air _ " _	! = !	HF - " -	VHF	VHF - "	VHF - " -	1 1 1 = = = = 1 1 1	 	VHF - " -	P.T.T N.T.B. PTT, Semarang Sby. Solo
INSTALLATION	1969 1978	1978	1969 1978 1969	1980 1980 1980	1980 1969 1980 1980	1969	1969 1969 1969	1969	1969	1975 1978
ANTENNA TYPE	Gate Antenna	Omni	Gate Antenna Omni	Dipole - " -	Dipole	! = !	1 1 1 2 2 2 2 1 1 1	 = 	1 = 1	; ; = = 1 ;
LOCATION	Tuban _ " _	Kuta	Tuban Kuta	Surabaya Kintamani -	Kintamani Tuban Kintamani Tuban	: :	: = = = :	; = 1	! E !	Tuban/Com " /ACC
POWER OUT PUT	5 / 10 Kw 1 Kw	1	3 / 5 Kw	100 Watt 100 "	100 Watt 70 "	70 Watt	70 Watt 	70 Watt	70 Watt	100 Watt 125 "
SET	r-1 r-1	2	2 2 2	202	2222	2	1 2 2	н		н н
FREQUENCY	5673 3868 13228	l I	8819	119.3 120.7	128.3	119.7	118.1	128.5	119.3	7425 7400
TYPE / MAKER	Tx. SIEMENS Philip	Rx. RFC TELEFUNKEN	Tx. SIEMENS Rx :R F C TELEFUNKEN	Tx. THOMSON Rx. THOMSON	Tx. RHOMSON R / S Rx.THOMSON R / S	Tx. R / S	Tx. R / S Rx. R / S Becket	R / S	R / S	CODAN INTI TR.125
ЕQUІРМЕNТ	M. WARA		R. DARA	ACC WAST	G ACC EAST	A.P.P	A.D.C	Emergency	Beack Up 119.3 Sby	S.S.B

EXISTING EQUIPMENT LIST OF AIR TELECOMMUNICATION SYSTEM

Point to pont Lombok	Com.inter tower- Mobil PKPPK	Link frequency Kintamani	Link Tuban Kintamani) = 1	Com.Tuban-Kin- tamani	Apron Service	Sequrity
1975	1980	1978	1980	1980		1980	1980
Dipole	z Dipole	Directive Yagi	=	Yagi	Yagi	Dipole	1
Tuban/ACC	Tuban/PPKP	Kintamani/ Tuban	Kintamani/ Tuban	Kintamani/ Tuban	Kintamani/	Tuban	SATPAN 2 TOWER 1 EQ.ROOM 2
50 Watt	10 Watt	5 Watt -	5 Watt	5 Watt	5 Watt	5 Watt	ŧ
	4	77	7 7	2	нн	-	5
1354 143.0	UHF	385.5	416.6 418.4	432.75 435.7	420.5 425.5	410 MHz	152.630
Tx. R.F.C Rx. R.F.C	YAESU	PYE	Tx.THOMSON	Rx. THOMSON	Tx. PYE THOMSON	Tx/Rx PYE	MOTOROOLA
Direct Speech Lombok	Telecar	Link			Service link	AMC(Apron Movement commnication)	walky Talky

Telephone System

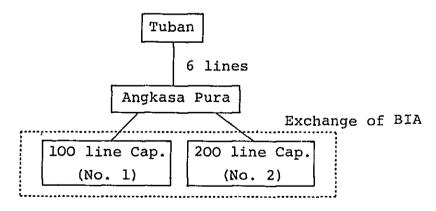
The current telephone network around Denpasar City is outlined below.



*The 300-line cable is used for the Tuban-Denpasar section. At present, 10 lines are used for communication between Denpasar and Tuban, and balance for the internal lines of Tuban.

At present, the number of telephone lines of Denpasar City total 5000 and these are fully used. There is a plan to establish 200 lines for internal city use and 600 lines for the Kuta Area by mid 1983.

The current telephone network of the airport is outlined below.



The line capacity between Tuban and Angkasa Pura is expected to increase to a total of 12 circuits in 1983.

In the airport, two types of Telephone Exchanges are used at present. One of them has 100 line capacity and the other has 200 line capacity. For the No.1 exchange, 47 out

of the 100 lines are occupied and for the No. 2 exchange, 91 out of the 200 lines are occupied although loaded capacity is 100 lines.

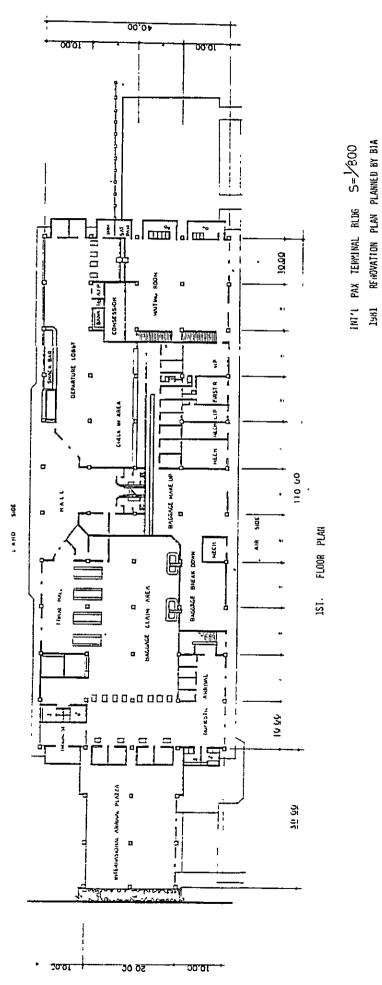
In addition to the above line capacity, 18 lines are reserved for direct telephone use, 6 lines between Angkasa Pura and Denpasar, and 10 lines for direct calls to Kuta.

AIRFIELD LIGHTING SYSTËM

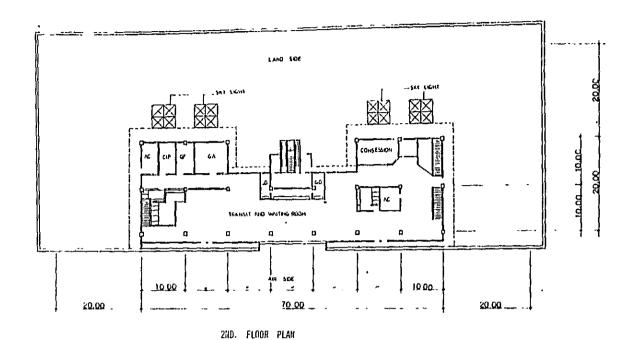
CAPACITY MAKER / TYPE INSTALLATION LOCATION REMARKS TING	it 18.865 Watt Siemen 1969 Airport Bi Direction 2700 M	ht 5.880 Watt Merk Siemen 1969 Airport .	19.220 Watt Merk AEG 1975 Airport 3 bar system	ght 5.840 Watt Merk Slemen 1969 Airport	ght Flush 7.500 Watt Merk Slemen 1978 Airport	240 Watt Merk Siemen 1978 Airport	ht 19.725 Watt Merk Siemen 1978 Airport Run Way 27 cat-1	sser 2.520 Watt Merk Stemen 1978 Airport .	con 2.000 Watt Merk Siemen 1969 Airport type DDS-101	light 800 watt Merk Siemen 1976 Airport type HF-10N	880 Watt Type LW/a 1969 Airport	240 Watt Merk Siemens 1969 Airport type LM/m	8.400 Watt Merk Slemens 1969 Airport type S5KB10	11ght 28.000 Watt Merk USA type AL-41	ic Light 400 Watt
EQUIPMENT	Run Way Light	Taxi Way Light	VASI System	Threshold Light	Threshold light Flush mounted	RELLS	n Approach 11ght I	Sequence Flasser	Rotating Beacon	Obstruction light	Landing Tee	Wind Sock	Sirene	Appron Flood light	Ground Traffic Light

EXINTING EQUIPMENT LIST OF POWER SUPPLY & GENERATOR SYSTEM

REMARKS	Supply from PLN (Goverment Ele- ctric firm)									
LOCATION	Airport	Airport	Airport	Reciever Station	V.O.R. Station	Repeater Startion	Radar Head	Airport	Airport	Airport
INSTALLATION	1978	1978	1965	1965	1965	1965	1976	1965	1965	1965
MAKER / TYPE	1	Merk DEUTZ West Germany. type BA8M (Auto - 10 second)	Merk DEUTZ West Germany. type W8V 17,5/22	Merk MAN West Germany. type D.0022 M (Auto - 10 second)	Merk MAN West Germany. type D.0022 M (Auto - 10 second)	Merk MAN West Germany. type D.0022 M	Merk MAN West Germany. type D.0846 HME	Mork AEG. West Germany.	type OF - 12.54 Merk AEG. West Germany.	
CAPACITY	1250 KVA 20KV/6KV.	2 X 850 KVA 220/380V.	3 X 360 KVA 220/380V.	2 X 30 KVA 220/380V.	1 X 16 KVA 220/380V.	3 X 16 KVA 220/380V.	1 X 80 KVA 220/380V.	2 X 1250 KVA	6000/400V. 3 X 500 KVA	6000/400V. 2 X 250 KVA 6000/400V.
EQUIPMENT	Main Power Supply	Automatic stand by genarating set	Manual stand by generrating set	Automatic stand by generating set	Automatic stand by generating set	Power generating set	Auromaric stand by genarating set	Power Transformer	Power Transformer	Power Transformer

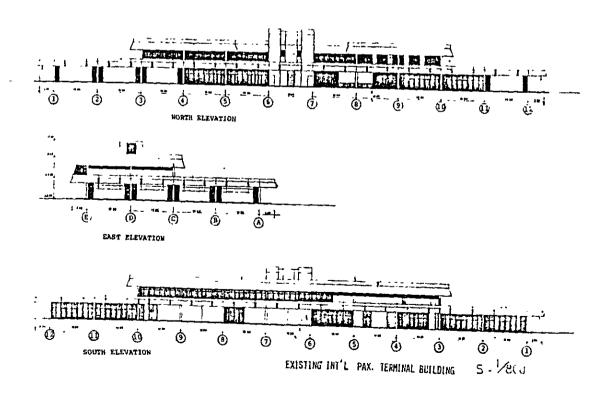


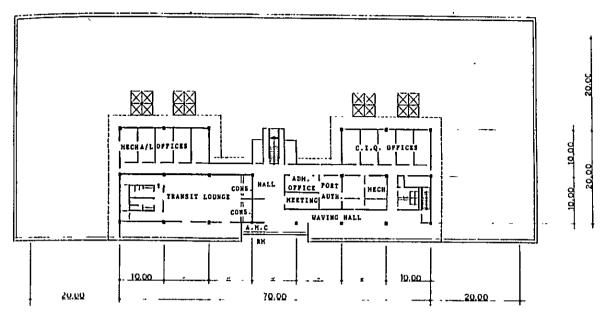
- 58 -



INT L PAX TERMINAL BLDG 3= 1/20.

1981 RENOVATION PLAN PLANNED BY BIA



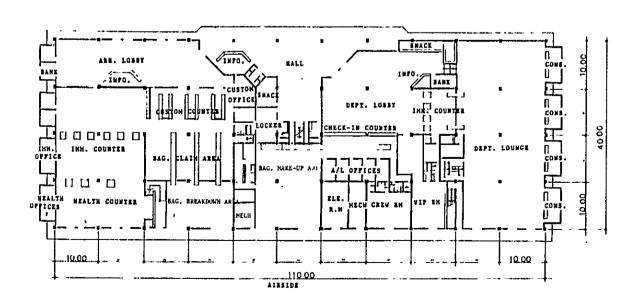


2NI). FLOOR PLAN

INT'L PAX TERMINAL BLDG $\mathbb{S} = \sqrt{2} \mathfrak{N}$.

1981 EXISTING PLAN

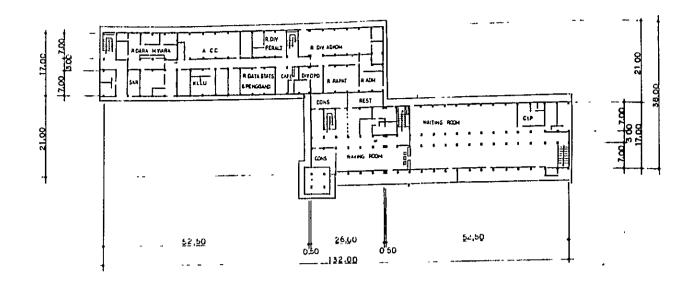
LANDSIDE



IST. FLOOR PLAN

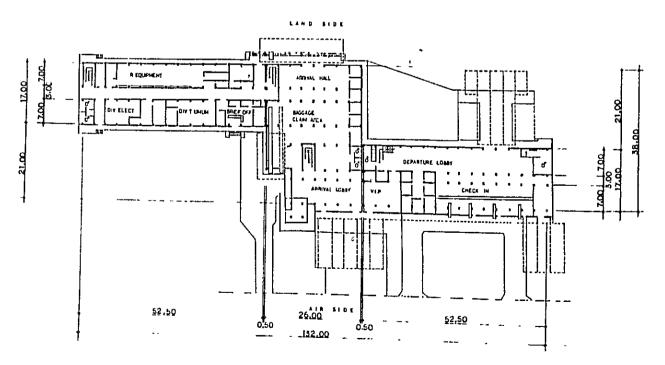
INI'L PAX TERMINAL BLDG S-120's
1981 EXISTING PLAN

APPENDIX 6.4.4



ZND. FLOOR PLAN

FOR PAX TERMINAL BLDG $S=\frac{1}{10}$ 1981 REHOVATION PLAN PLANNED BY BIA

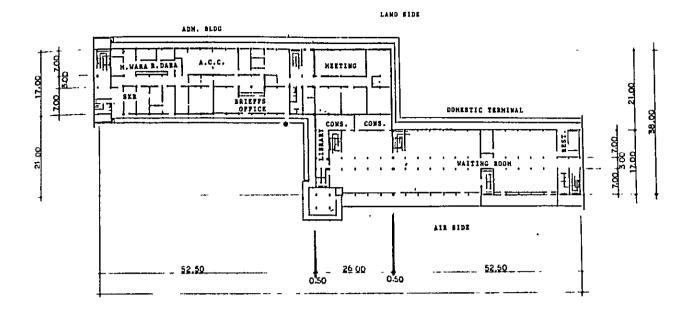


Ist. FLOOP PEAR

DITH PAX TERMINAL BLING -= 1000C

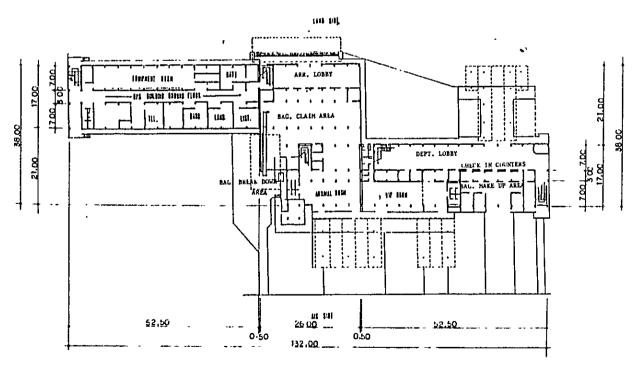
1981 REHOVATION PLAN PLANNED BY BIA

APPENDIX 6.4.5



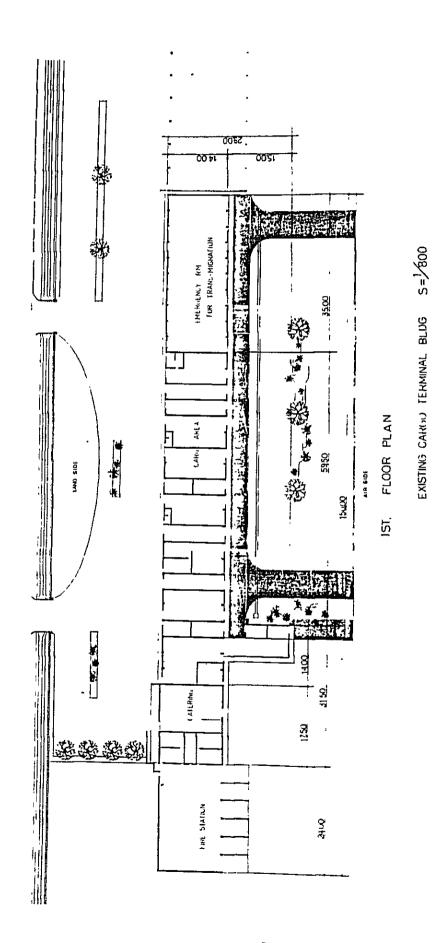
2ND. FLOOR PLAN

DOM PAX TERMINAL BLUG == 1/16 CC
1981 EXISTING PLAN

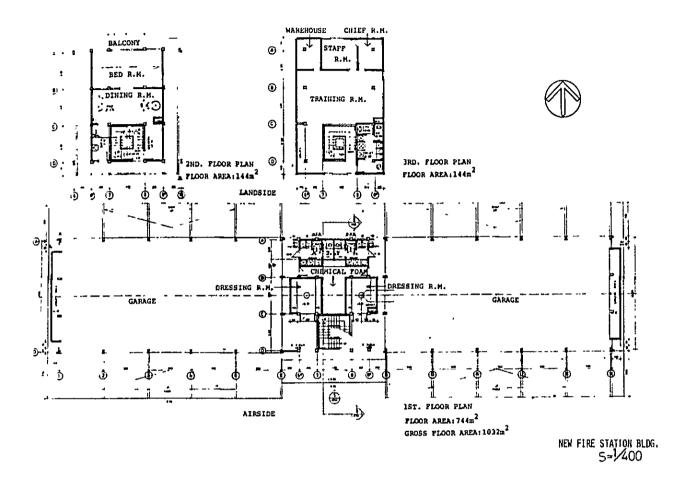


1ST. FLOOR PLAN

DOM PAX TERMINAL BLDG $5=\frac{1}{100}$ () 1981 EXISTING PLAN



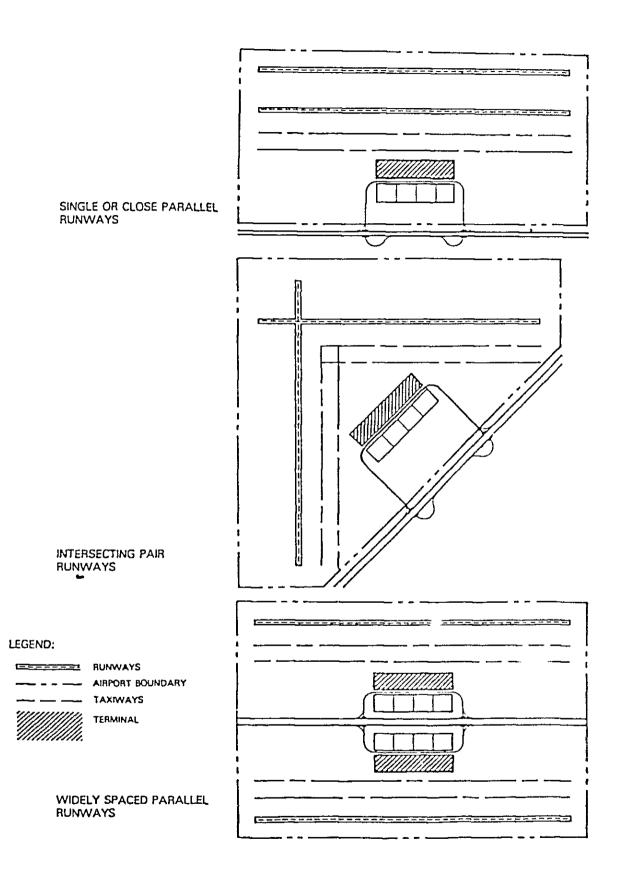
- 63 -





APPENDEX TO CHAPTER 7





FUNDAMENTAL LAYOUT OF TERMINAL AREA

IATA: Airport Terminals Reference Manual 6th. Edition ATRM 1.2.2



& *APPENDEX TO CHAPTER 99.



Evaluation of Existing Pavement Structures

No significant defects of the pavement have been the rue into this moment although B747 and DC-10 class heavy aircraft loads have been applied since 1977. The heavy aircraft load repetitions expected in the future, however, he cause damage to the pavement structure.

Based on this point of view, this section evaluates the existing runway and apron pavements and whether it would be appropriate to overlay them to accommodate the aircraft load repetitions expected during the design life of the pavement.

(1) Characteristics of the existing pavements.

Characteristics of the existing pavement structures of BIA are summarized as shown in Table 1 below.

TABLE 1 CHARACTERISTICS OF EXISTING PAVEMENT (Typical)

Location	Runway intermediate	Runway ends	Apron
Subgrade bearing capacity	CBR 9%	CBR 9% (K=5 kg/cu.cm)	CBR 9% (k=5 kg/cu.an)
Flexural strength of PCC slab		50 ku/sq.an	50 kg/sq.cm
Pavement structure	59cm 59cm 75 75 25 8MB	FCC VCI	To the control of the

Note: ACS: Asphalt Concrete Surfacing

BMB: Bitumen Macadam Base

LB: Limestone Base SSB: Sand Subbase

PCC: Pontland Cement Concrete Slab ACL: Asphalt Concrete Leveling Course

- (2) Evaluation of Existing Rigid Runway Pavement by LCN Method
 - i) Characteristics of existing pavement:
 - * LCN of existing pavement = 60
 - * Thickness of PCC slab = 30 cm
 - * Modulus of subgrade reaction

 No data about the subgrade reaction is available. However, as a approximate value, subgrade reaction is read off from Fig. 1 as 8kg/cm³.
 - * Flexural stress of PCC slab = 25 kg/cm^2
 - * Young's modulus for concrete = 350,000 kg/cm²
 - * Poison's ratio for concrete = 0.15

*
$$\ell = \left(\frac{\text{Eh}^3}{12(1-u^2) \text{ k}}\right)^{1/4} = \left(\frac{350,000 \times 30^2}{12(1-0.15^2) \times 8}\right)^{1/4}$$

= 100.2 cm

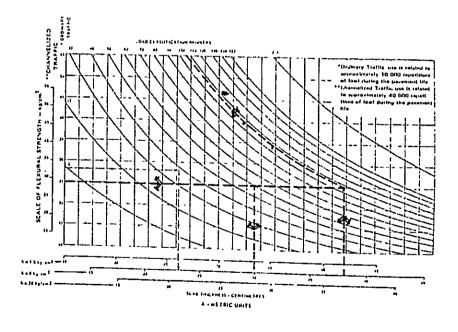
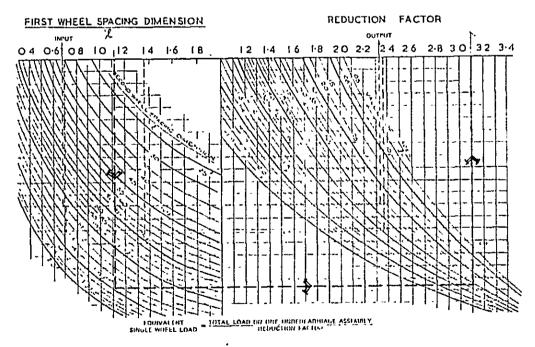


Fig. 1 Design Chart for Rigid Pavement

- ii) Characteristics of aircraft:
 - * Type of aircraft: B-747-200B
 - * Take-off load on each main leg(W) = 82,500kg
 - * Wheel arrangement on each main leg:

Dual tandem

- * Wheel track (S) = 111.8 cm
- * Wheel base (St) = 147.3 cm
- * Tire pressure (p) = 14.4 kg/cm^2
- * Contact area of each main leg (A) = W/p= 82,500/14.4 = 5,729 cm²
- iii) Obtain the inputs for Fig. 2 S/ = 111.8/100.2 = 1.12 St/ = 147.3/100.2 = 1.47 and A/ $^2 = 5,729/100.2^2 = 0.57$ Using above inputs, Reduction factor (RF) is read off as 3.1 from Fig. 2
 - iv) ESWL = W/RF = 82,500/3.1 = 26,600 kg
 - v) Input ESWL and tire pressure for Fig. 3 LCN of the aircraft can be read off as 95.



ANEA M TOTAL CONTACT AREA OF ALL WHEFES OF THE UNITERFAMILIAGE ASSEMBLY

Fig. 2 Equivalent single which look assessment entries - rigid parements - dual tandem undercarrings

vi) Required overlay thickness

From Fig. 3 , 9 cm of bituminous surfacing is seen to increase the LCN of the rigid pavement to just 35.

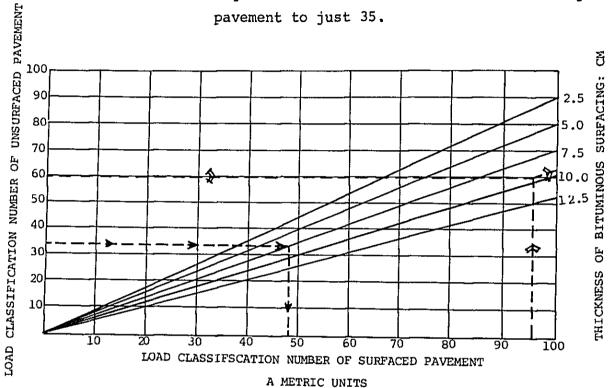


FIG. 3 Chart to show the effect upon the LCN of surfacing a rigid pavement

- (3) Evaluation of existing rigid apron pavement by LCN method characteristics of existing pavement of PCC slab with 38 cm thick and subgrade reaction factor k = 8 kg/cm³ secure LCN 105 as determined from Fig. 1, and is sufficient for aircraft specified as LCN 95.
- (4) Evaluation of existing flexible runway pavement by LCN method
 - i) Characteristics of existing pavement:
 - * LCN of existing pavement = 60
 - * Overall thickness of pavement = 83 cm
 - * Subgrade CBR = 9%

- ii) Characteristics of the aircraft: Same as ii) of (2) above.
- iii) Obtain the reduction factor (RF)
 Enter S 111.8 cm
 St 148.3 cm
 A 5729 cm² for Fig. 4
 RF is read off as 3.15.
- iv) ESWL = W/RF = 82,500/3.15 26190 kg
 - v) Input ESWL and tire pressure for Fig. 5 LCN of the aircraft is 95. Accordingly, the existing flexible pavement is evaluated as to be required to increase its LCN value 60 to 95 by overlay work.
- vi) Overlay thickness

 Concerning the overlay thickness, there is

 no clear description in the LCN method from
 which a definite overlay thickness can
 be determined.
- (5) Evaluation of existing pavement by JCAB method
 - i) Required pavement structures

 The required pavement structures for critical area determined based on the design method of the Corps of Engineers for flexible pavement and the method of the PCA (Portland Cement Association, USA) for rigid pavement are summarized in Table 2 , under the design conditions listed below:

Design conditions:

Design aircraft: B-747
Repetition of design load:10,000 times
Subgrade conditions: CBR 9% or
K=5 kg/cu.cm.

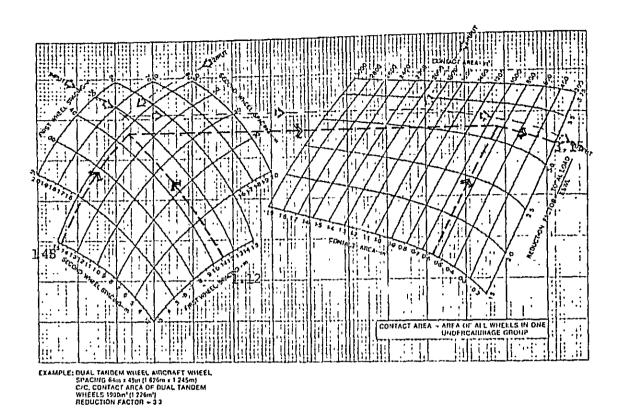


Figure \underline{A} fquivalent single wheel load assensment cutves - Rigid pavements: dust tandem which undersarriage

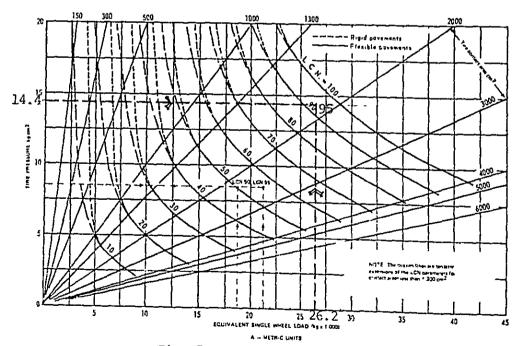


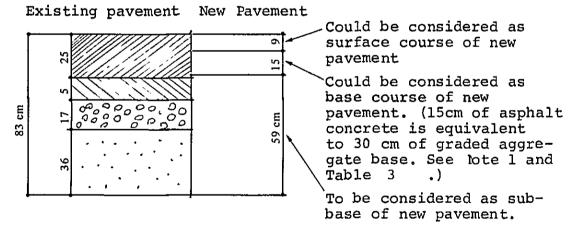
Fig. 5 LOAD CLASSIFICATION NUMBERS

TABLE 2 REQUI	D PAVEMENT	STRUCTURE
---------------	------------	-----------

Type of pavement	Flexible pavement	Rigid pavement
Pavement Structure	Asphalt concrete surface course Graded aggregate base course Sand subbase course	8 ////

ii) Evaluation of existing runway flexible pavement

Each component of the existing pavement structures of flexible runway pavement is evaluated as follows:



Based on the above analysis, it can be said that the existing flexible runway pavement structure is not sufficient for the 10,000 design load repetitions and at least 6 cm (15 - 9 cm) of flexible overlay will be required.

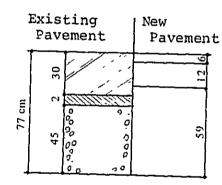
Note 1. According to the structural design standard of pavement by JCAB, the equivalency conversion factor of

asphalt concrete materials to be graded for aggregate base is defined as 2.0 as shown in Table 3

TABLE 3 CONVERSION FACTOR FOR OVERLAY DESIGN

Materials of existing pavement structure	Conversion factor to base material of new pavement
Asphalt concrete surface course	2.0
PCC concrete slab	2.5
Graded aggregate base course	1.0

iii) Evaluation of existing rigid runway pavement
Each component of existing rigid runway pavement structure is also evaluated as follows:



Could be considered as surface course of new pavement.

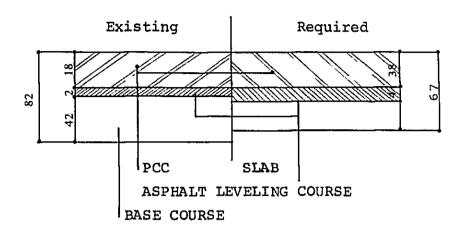
Could be considered as base course of new pavement (12 cm of PCC xlab is equivalent to 30 cm of graded aggregate base, See Table 3 .)

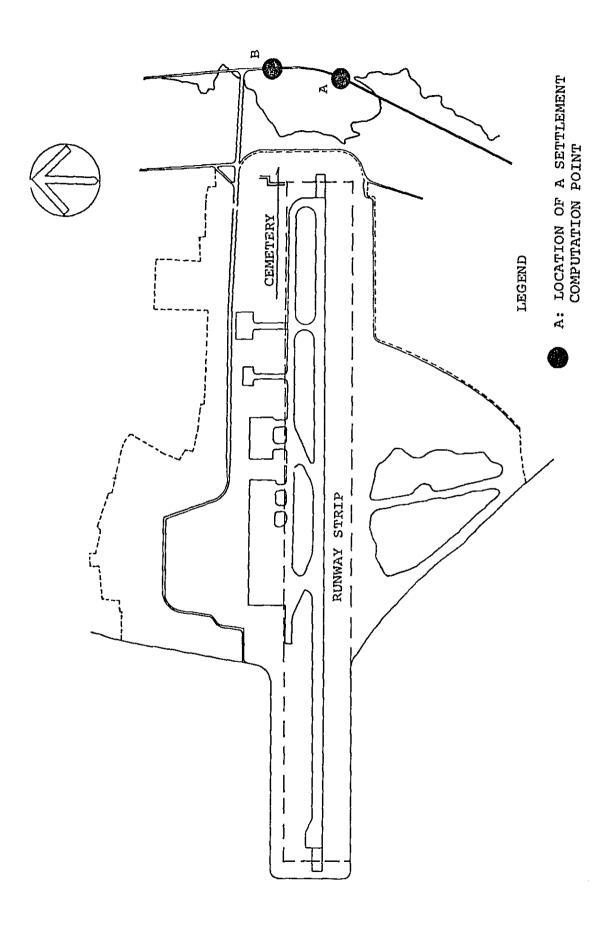
To be considered as subbase of new pavement.

According to the above analysis, it can be said that the existing rigid pavement structure of runway is not sufficient for the 10,000 design load repetitions, and at least 9.0 cm (15 - 6.0 cm) of flexible overlay will be required.

iv) Evaluation of existing rigid apron pavement Based on the comparison between the existing and required rigid pavement structures in Fig. 6 below, it can be said that the existing rigid pavement structure for apron is sufficient for the 10,000 design load repetitions.

Fig. 6 Comparison of Rigid Pavement





Data for Consolidation Settlement due to Embankment

APPENDIX 9.4.1

minute in the first of the base of the		APPENDIX 9.4.1
SEP		
	A: VALUE OF SET	
	A. VALUE OF SET	TLEMENT (cm)
281 XIII		
10.0 MO		
MEV 110		
M 106 009 1		
A A A		
30 CD		
8 W W W W W W W W W W W W W W W W W W W		
一眼的过去式和过去分词 医二氏腺肿瘤 化复工厂	그녀 회사 회사 회사에 가는 용지 있다.	3 1 10 1 1 1 1 1 1 1
EMBANKMENT HIGHNESS (m)	SETTLEMENT	
	11-21111 - 76 - 11-11-11	

Evaluation of Existing Storm Water Drainage System

Fig. A shows the existing storm water drainage system at BIA. The open ditch drainage system forapron, autoparking and north-east side area of the new fire fighting station are functioning efficiently; however, the drainage system for the area of the landing strip is not functional due to the lack of proper surface water collecting devices although the underground piping networks are installed. It is recommended, therefore, to prepare an adequate surface water collecting system for the said area.

The capacity of drainage pipe installed on the north side edge of the apron is not sufficient for the expansion of apron and passenger terminal building, and therefore, a new pipe line parallel with the existing line will be required.

The drainage pipeline installed along the west side edge of the apron shall be relocated or demolished since a new parallel taxiway will be constructed on the existing drainage pipeline. The open channel installed parallel with the north side of existing parallel taxiway shall also be relocated for the same reason.

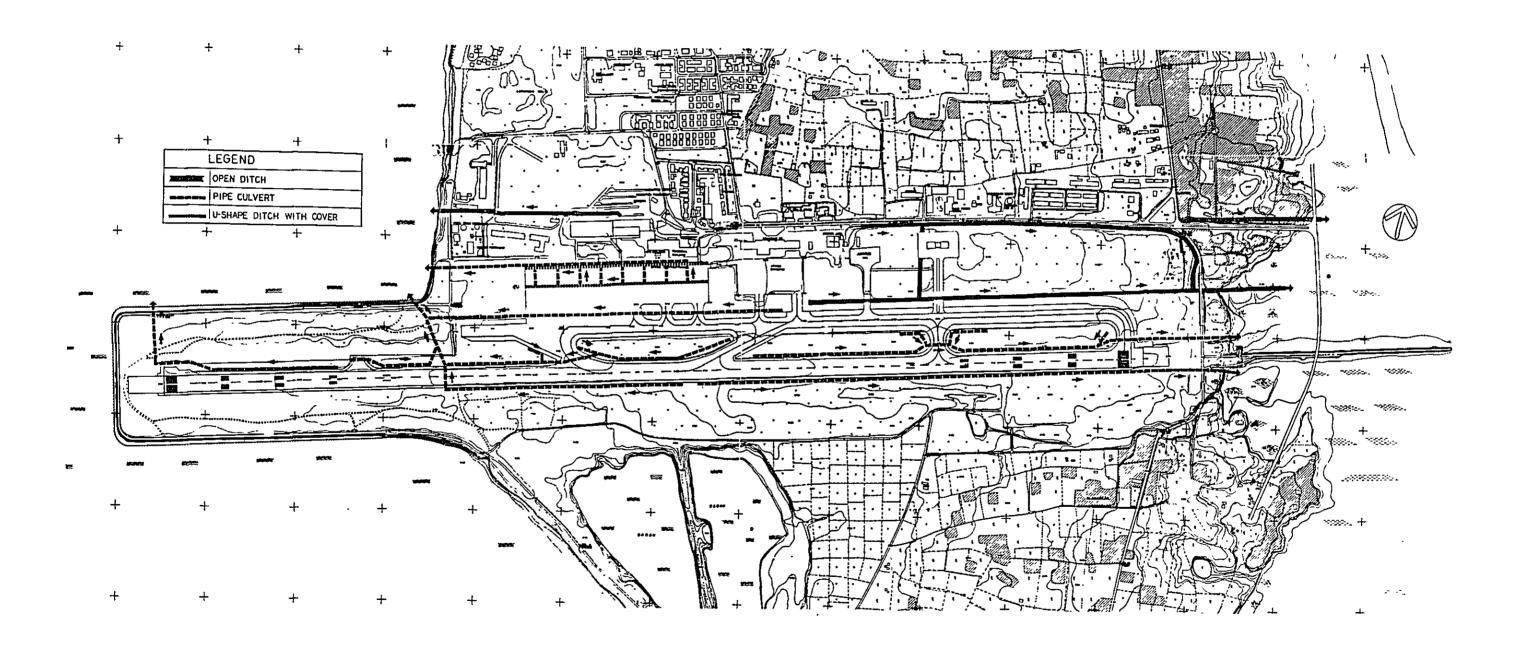


Fig. A EXISTING STORM WATER DRAINAGE SYSTEM
S=1:10000

57.00V.1

Design criteria for storm water drainage system The planning criteria employed for the stormwater drainage facilities requirement are summarized as as follows:

(a) Runoff estimation

The design peak rate of stormwater runoff is estimated by using the following Rational formula.

 $O = 1/360 \times C \times I \times A$

Where, Q : Peak rate of runoff in cu.m/sec

C : Runoff coefficient

I: Rainall intensity in mm/hr during the assumed design storm with a rainfall duration of t minutes.

A: Tributary area in ha.

(b) Runoff coefficients : C

Pavement area: 0.95

Building roof: 0.90

Sodding area : 0.40

(c) Rainfall intensity

The rainfall conditions of BIA is as follows: The mean annual precipitation amounts to 1832mm. A distinct rainy season continues from November to March with daily precipitation ranging 75 mm to 330 mm as a maximum.

A maximum rainfall expected once 5 years is generally recommended for estimating runoff for airport stormwater drainage system design. From the data of maximum daily rainfall precipitation in annum (1974 - 1981) obtained from BIA meteorological observatory, the maximum daily rainfall intensity is estimated as 200 mm/day for 5 yeras frequency. The rainfall intensity for the period of concentration required for the surface runoff to flow is estimated by the following formula and as shown in Fig. 9.3.2.

a and a special graduation of the contract of

 $It = \begin{array}{ccc} R24 & 24 & 2/3 \\ 24 & t & \end{array}$

Where, It: Intensity of rainfall for t time period (mm/hr)

R24: Maximum daily rainfall intensity

(mm/day)

: Duration of rainfall (hr.)

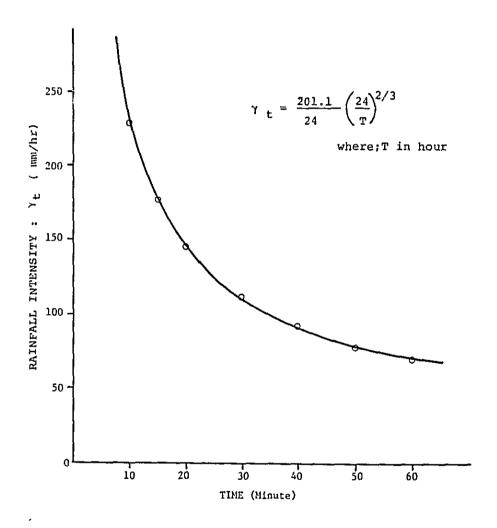
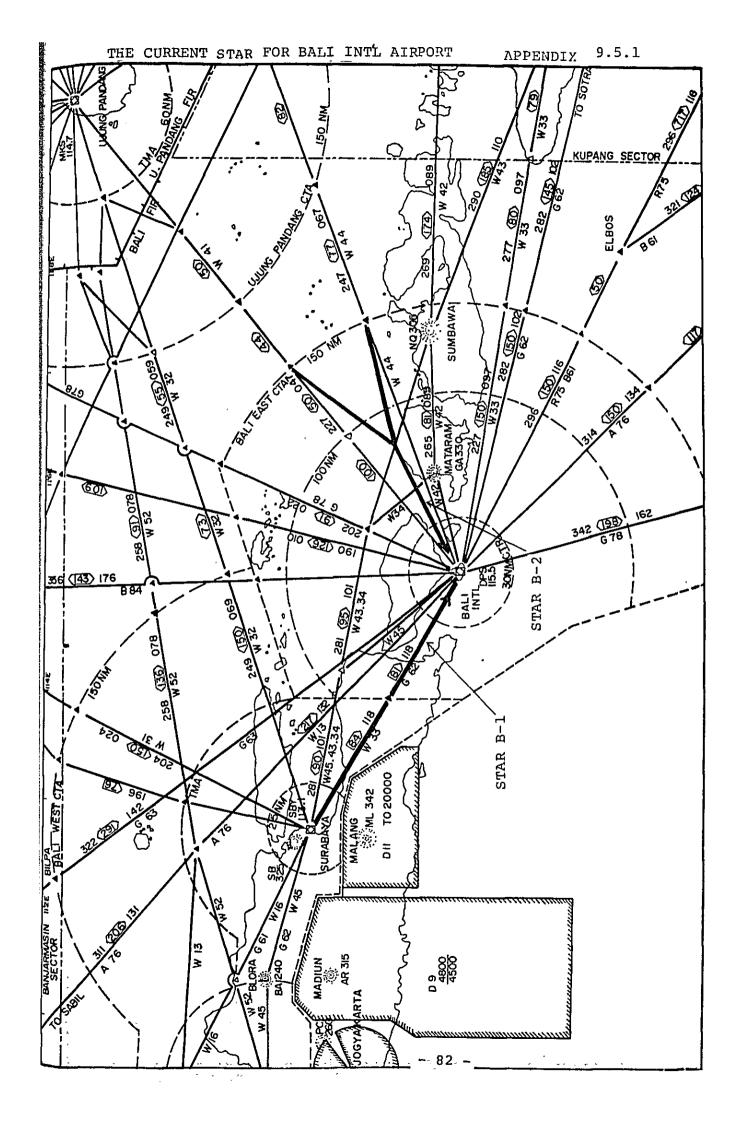
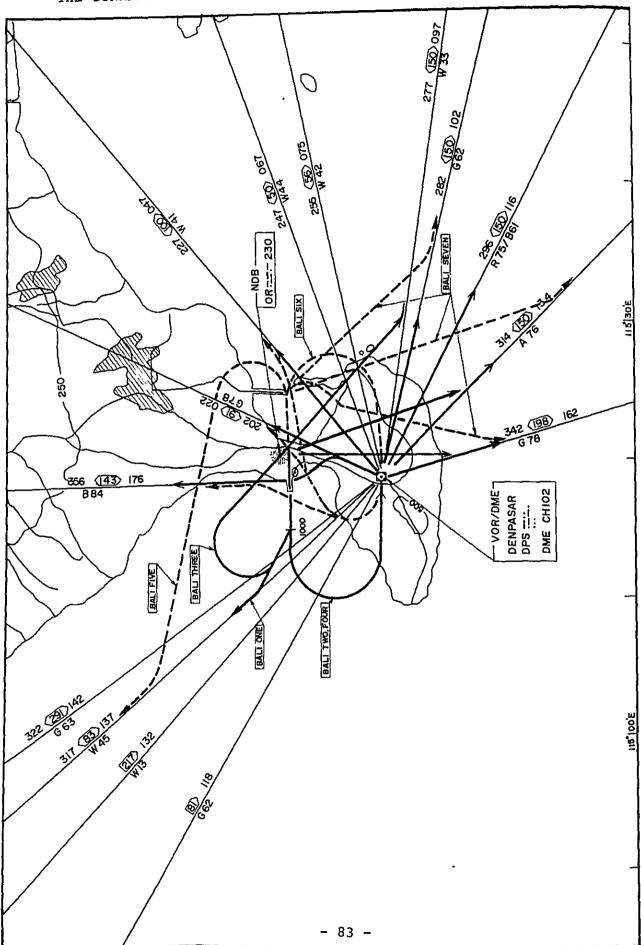


Fig.9.3.5 Rainfall intensity curve



THE CURRENT SID AT BALI INTL AIRPORT

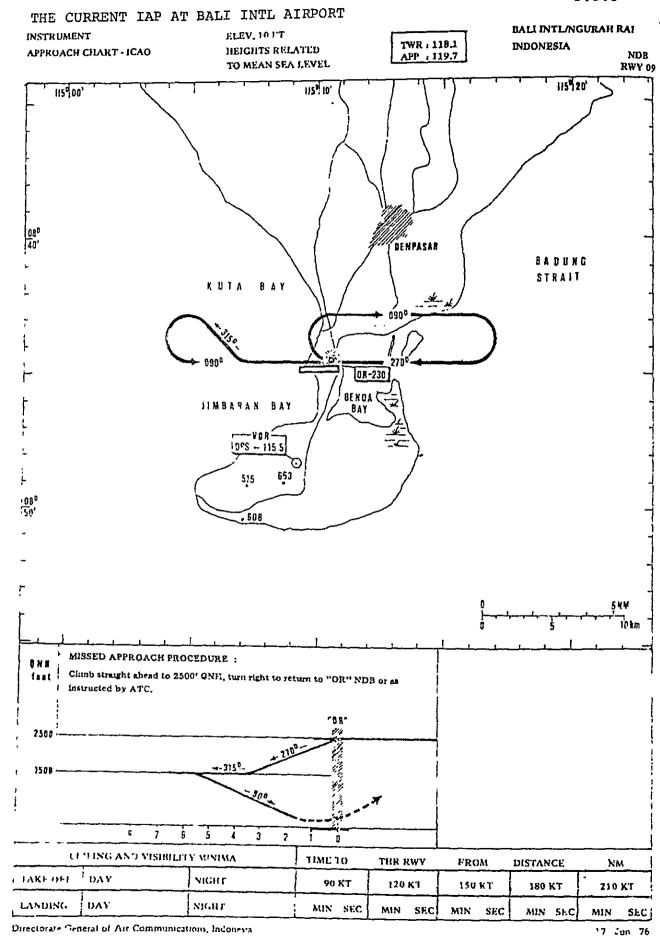


THE CURRENT IAP AT BALI INTL AIEPORT BALLINTLINGURAH RAL -TWR |181 APP | 119.7 ELEV. 10FT INSTRUMENT INDONESIA ILS/VOR HEIGHTS RELATED TO APPROACH CHART-ICAD RWY- 27 M.S L 115 10 mam reet alt 000 - 090 4000-25NM 40 MAM SECT ALT 270 - 360 9000-25 NM BADUNG Bay NDB-OR 230 -VORIDME-DPS - 115.5 ID DME 50 - 607 Mam Sect alt mam sect alt 180-270 090-180 2000-25NM 3000-25 NM Transition Level F1330
Transition Albitude 11000 SCALE 1:250.000 113 20 115 00 115 10 115100 L 1 115110 L 1 115110 L turn left to proceed to DPS VOR or as instructed bt ATC VOR/DME 2500FT 10 NAUTICAL MILES, FROM GLIDE PATH RWY 27 TIME TO THE RWY -DISTANCE FROM CEILING AND VISIBILITY MINIMA 90 KT 120 KT 150 KT 180 210 K1 180 KT NIGHT : 100-3/4 DAY 1,300 - 3/4 STR-IN MIN SEC

NIGHT - 800-11/2

CIRCLING DAY . 800 - 17/2

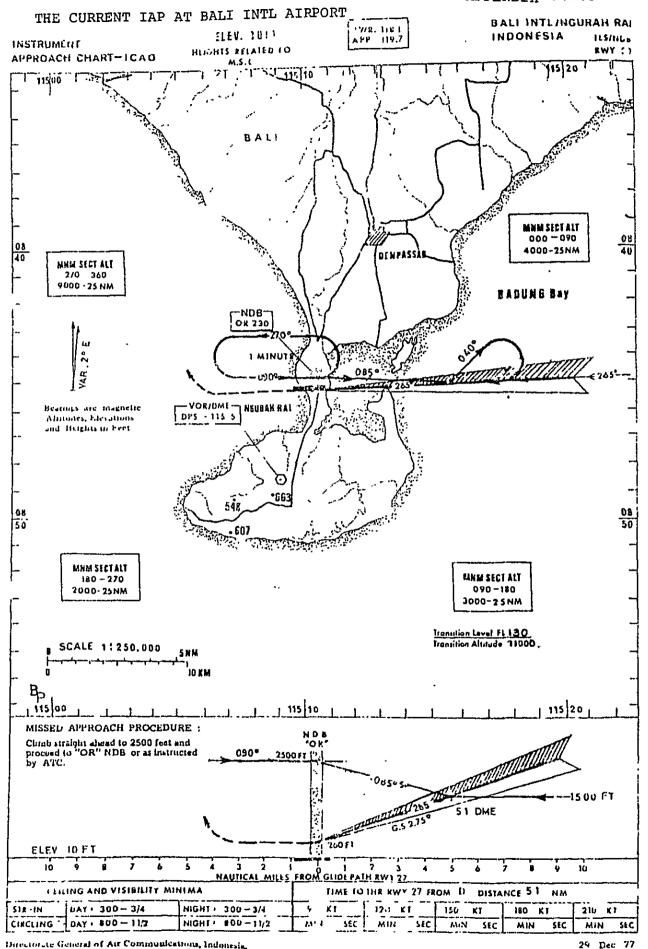
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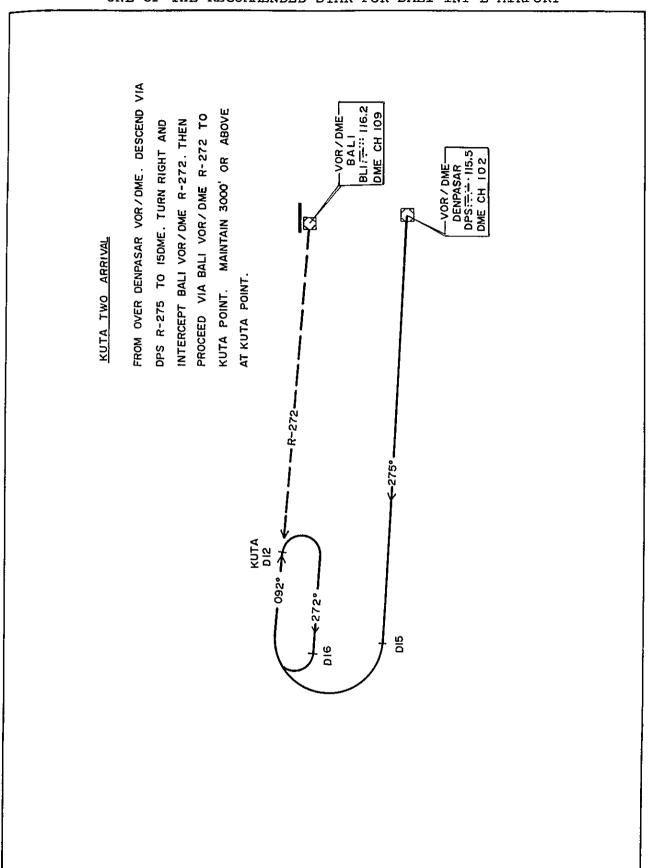
THE CURRENT IAP AT BALI INTL AIRPORT INSTRUMENT ELEV. 10 FT BALI INTL/NGURAH RAI APPROACH CHART - ICAO HEIGHTS RELATED TWR: 118.1 APP: 119.7 INDONESIA TO AERODROME ELEV. NDB **RWY 27** 115° 00′ 1150 10' 1150 20' MNM SECT ALT MNM SECT ALT 272°-002° 0029-092° 9000-25 NM 4000-25 NM BEARINGS ARE MAGNETIC ALTITUDES, ELEVATIONS AND HEIGHTS IN FEET 1080 084 40, KUTA BAY BADUNG NDB OR-230 BAY2 MIN 090 BENOA JIMBARAN BAY VOR-DPS-115.5 663 :08 080 50 50· **e** 608 MNM SECT ALT 092°-182° MNM SECT ALT 3000-25 NM 182°-272 2000-25 NM 5 NM 0 O SCALE 1: 250.000 10 Km 115° 00 · 115° 10. MISSED APPROACH PROCEDURE . Climb straight ahead to 2500 QNH and return to NDB' OR' or as instructed by ATC. NDB OR 2500 TRANSITION LEVEL FLI30 TRANSITION ALTITUDE 11000 1500 NM CEILING AND VISIBILITY MINIMA TIME TO THE RWY FROM DISTANCE NM TAKE OFF DAY NIGHT 90 KT 120 KT 150 KT 180 KT 210 KT LANDING DAY NIGHT MIN MIN SEC MIN MIN SEC SEC

Directorate General of An Communications, Indonesia

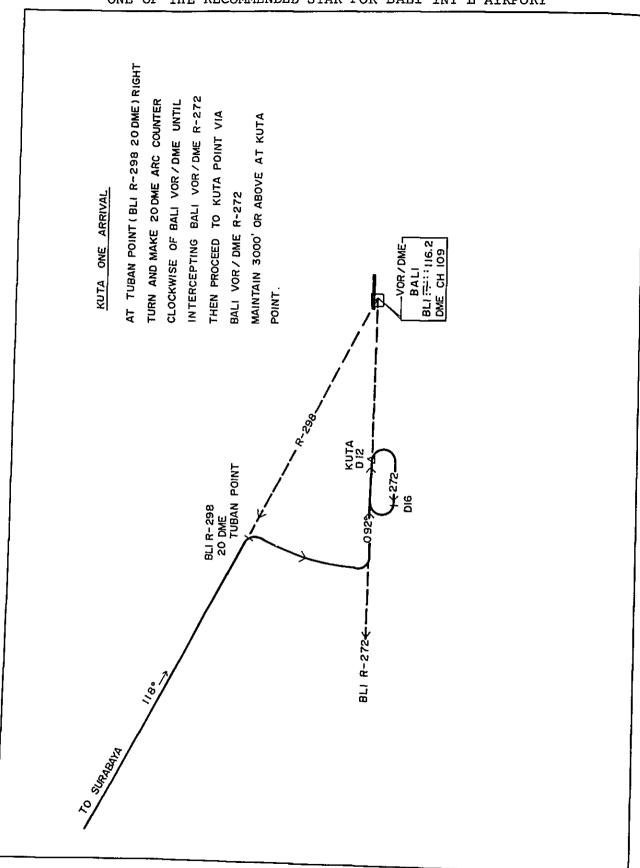
APPENDIX 9.5.3



ONE OF THE RECOMMENDED STAR FOR BALL INT'L AIRPORT

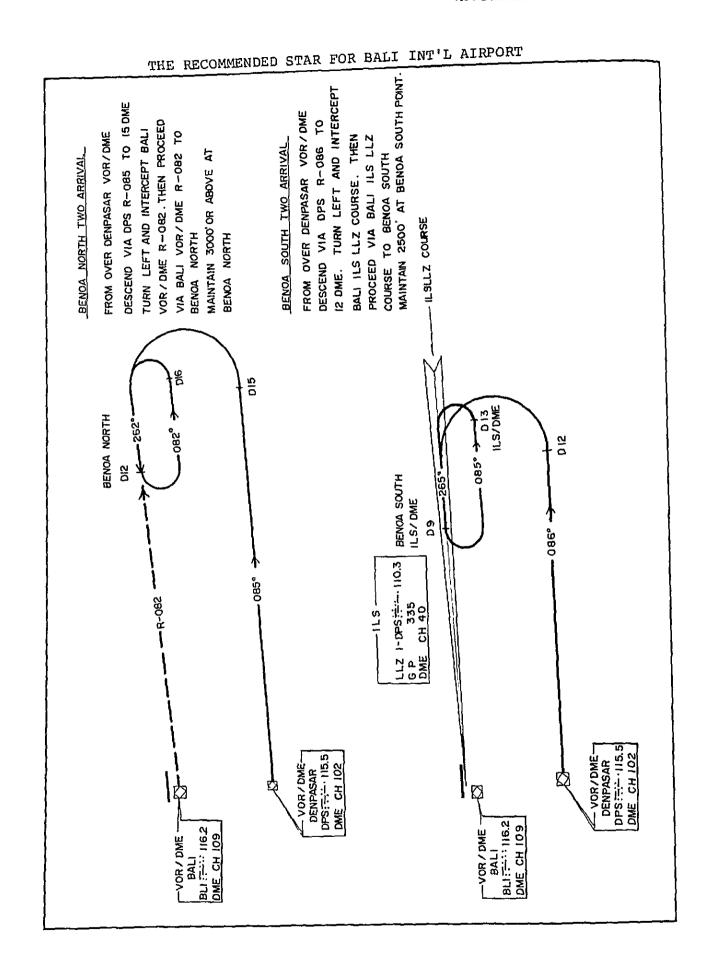


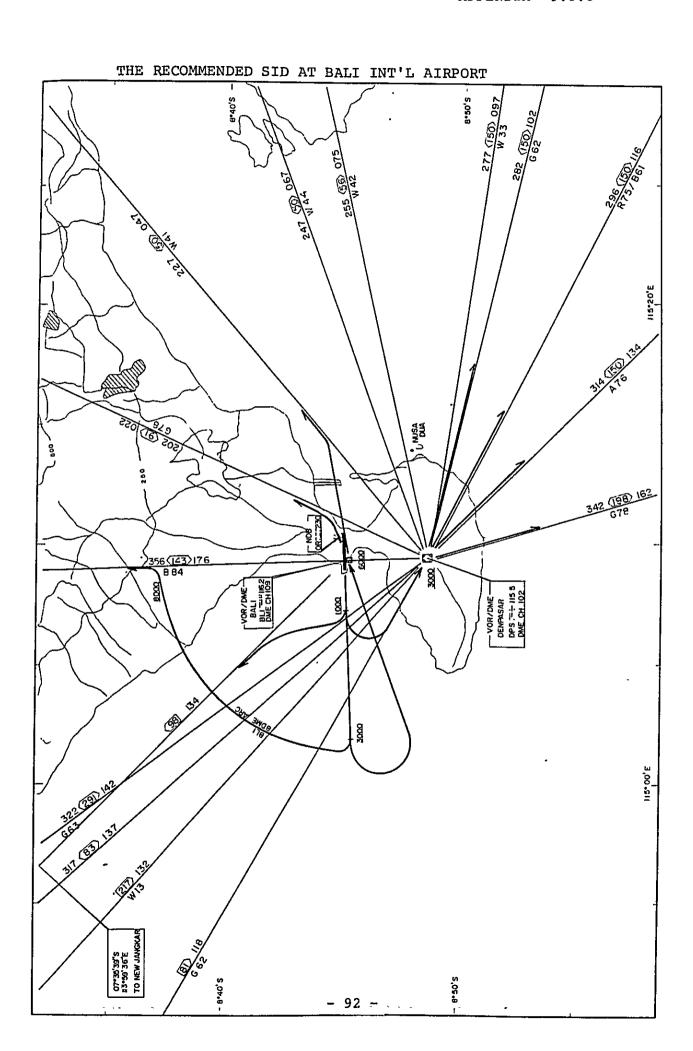
ONE OF THE RECOMMENDED STAR FOR BALI INT'L AIRPORT

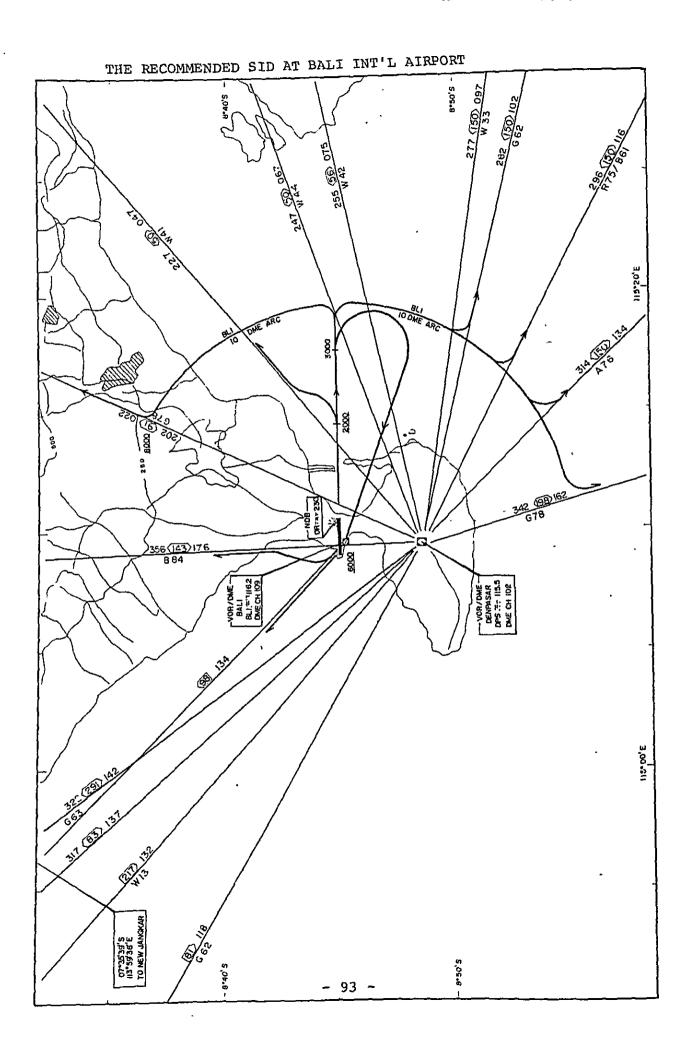


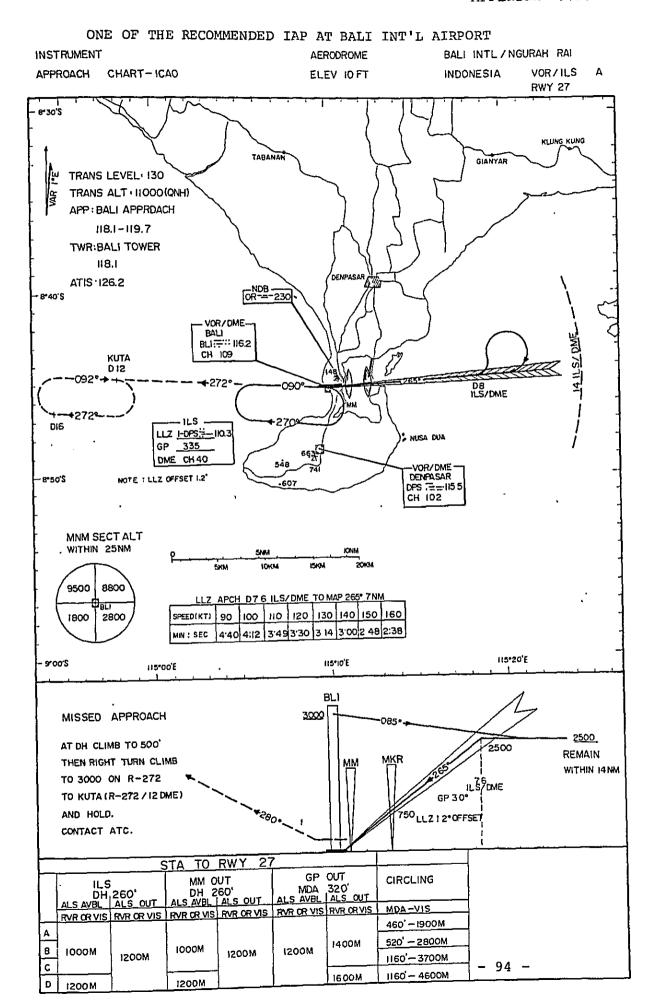
THE RECOMMENDED STAR FOR BALI INT'L AIRPORT DPS R-047/ISONM DPS R-047/150NM FROM FROM °\$/2, GA :==:330 -NDB -GA==: 330 -NOB (B) ILS LLZ COURSE MAINTAIN 2500' AT BENOA SOUTH POINT. THEN PROCEED TO BENDA SOUTH POINT AT SANUR POINT (BLI R-075 20 DME) CLOCKWISE OF BALI VOR / DME UNTIL INTERCEPTING BALI ILS LLZ COURSE LEFT TURN AND MAKE A 20 DME ARC → BLI R-082 BENOA SOUTH ONE ARRIVAL VIA ILS LLZ COURSE. BLI R-075 20 DME BLI R-075 20 DME SANUR SANUR ILS/DME BENOA N R-075 R-075 BENOAS ILS/DME90ME INTERCEPTING BALI VOR / DME R-082. THEN PROCEED TO BENOA NORTH POINT LEFT TURN AND MAKE A 20 DME ARC AT SANUR POINT (BLI R-075 20 DME) CLOKWISE OF BALI VOR / DIME UNTIL VIA BALI VOR / DME R-082 MAINTAIN 3000' OR ABOUE AT BENOA NORTH ONE. ARRIVAL 8∟1 :÷:: 116.2 VOR / DME-DME CH 109 BENOA NORTH POINT. BALI BAL! BLI := ':' 116.2 DME CH 109 -VOR / DME

- 90 -

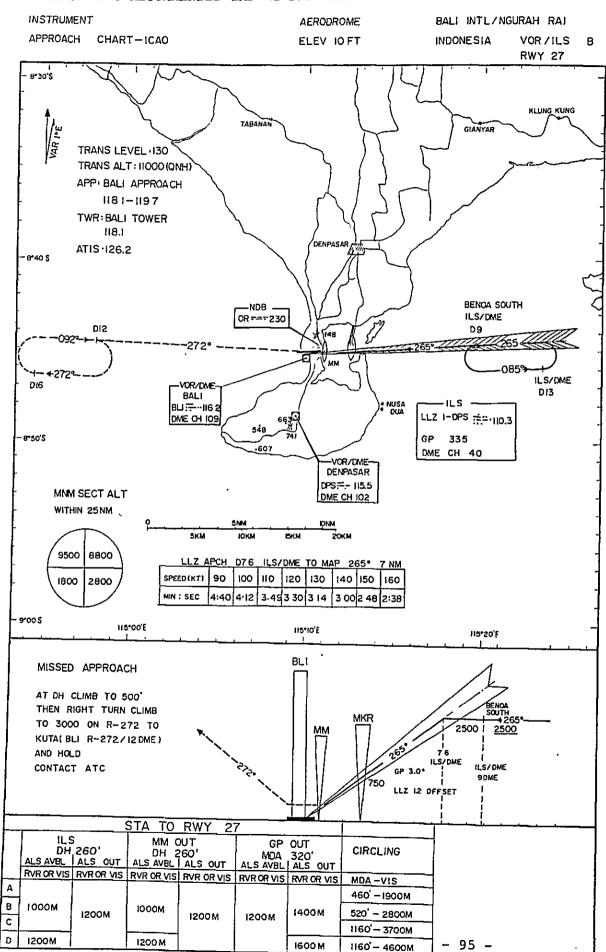








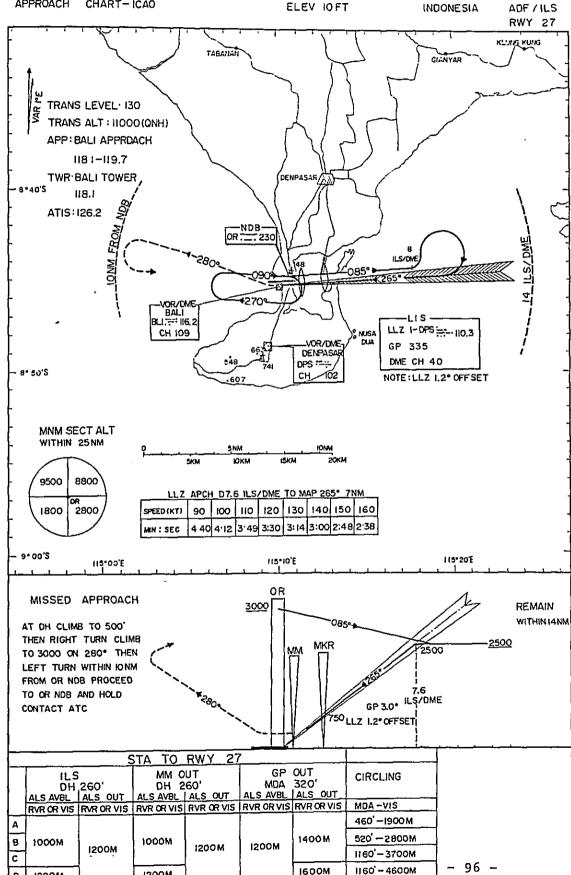
ONE OF THE RECOMMENDED IAP AT BALL INT'L AIRPORT



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APPENDIK 9.5.6

ONE OF THE RECOMMENDED IAP AT BALL INT'L AIRPORT INSTRUMENT **AERODROME** BALI INTL/NGURAH RAI APPROACH CHART-ICAO ELEV IOFT INDONESIA KENNE KRING TARAZIAN GIANYAR

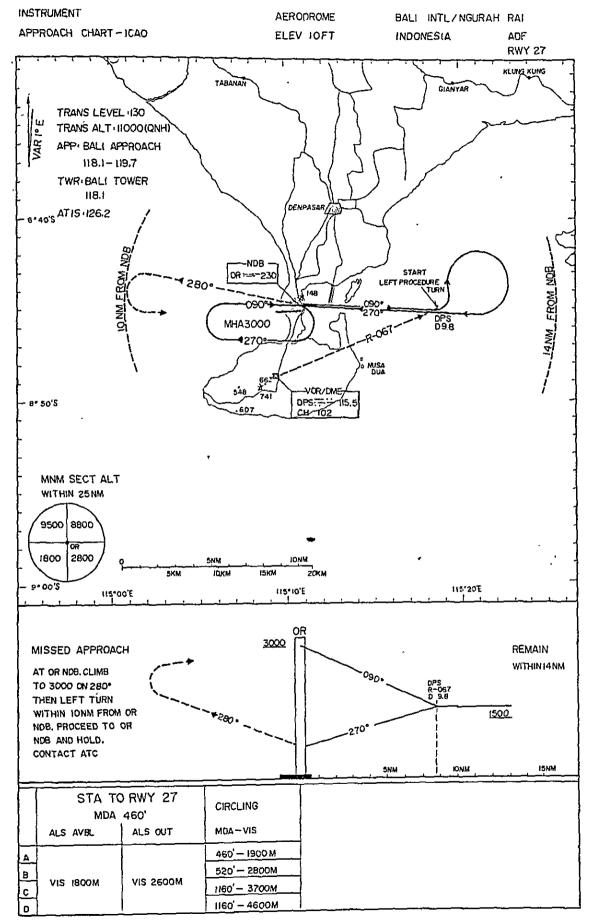


1200M

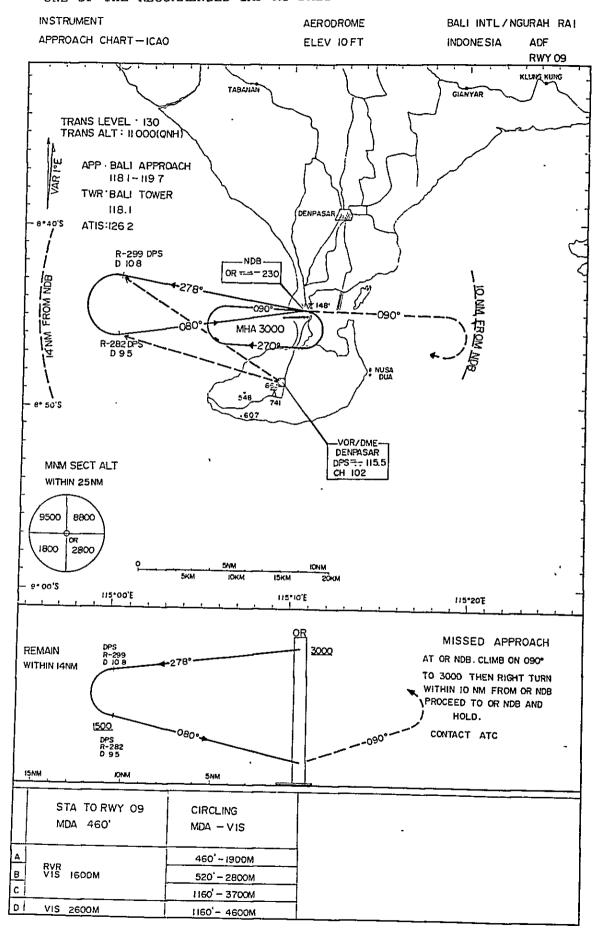
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1200M

ONE OF THE RECOMMENDED IAP AT BALL INT'L AIRPORT



ONE OF THE RECOMMENDED IAP AT BALL INT'L AIRPORT



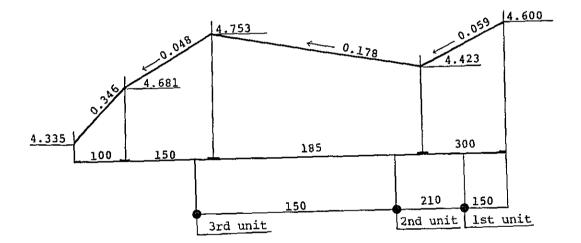
THE RELOCATION OF THE V.A.S.I'S UNIT.

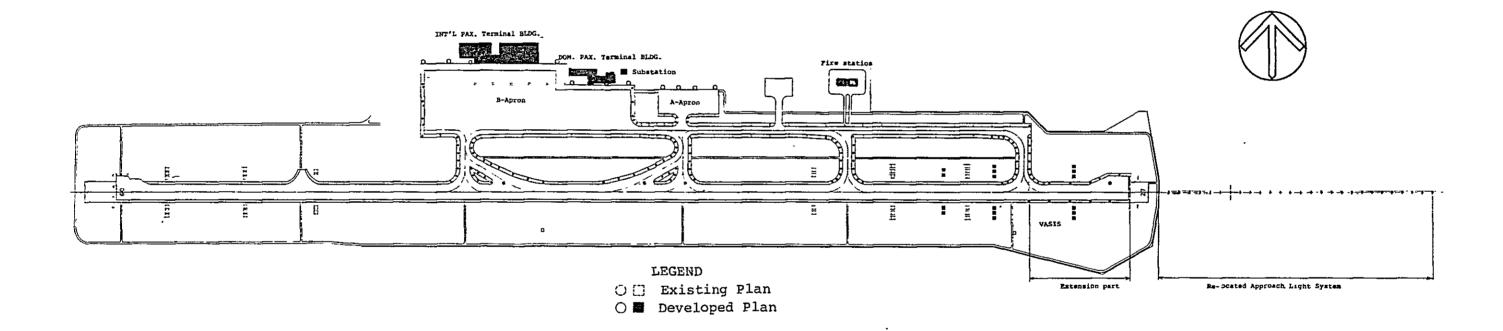
To relocate the Visual Approach Slope Indicator System, it is necessary to confirm with the profile of the runway slope for the 300m expanded condition.

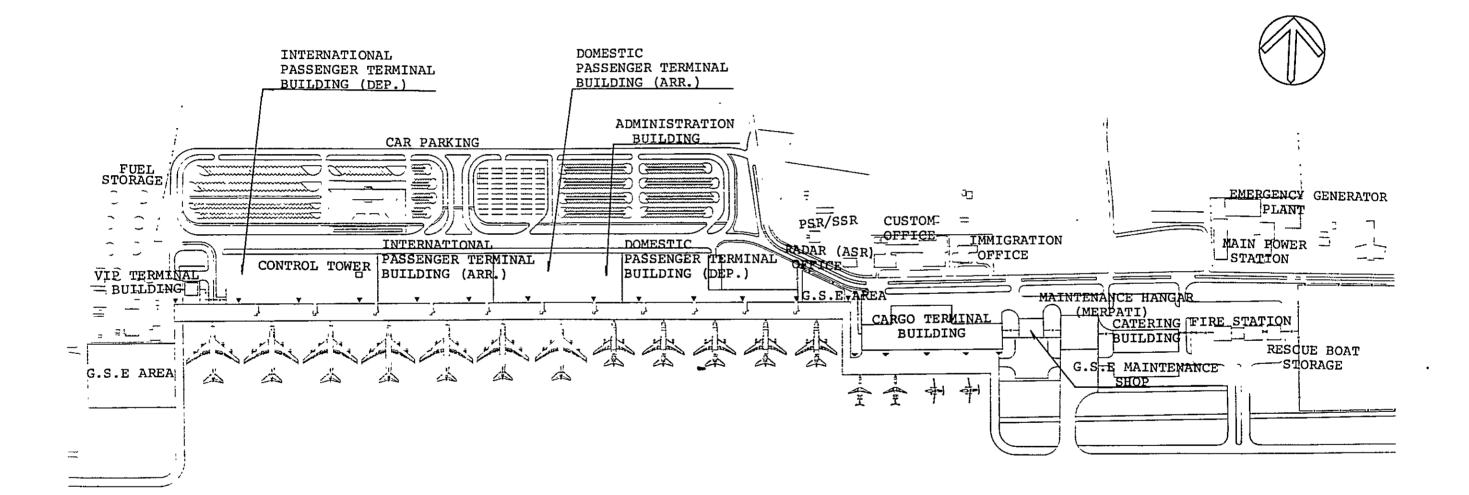
The design condition are as follows,

- System : 3- Bar - Glide Slope : 3°

- Runway Profile : Shown drawing below,







LEGEND

APRON FLOOD LIGHT

LAYOUT OF FLOOD LIGHT (2010) S= 1:4,000 - 102 -

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Charles to State

-

CATALLEY CA

.a.e area

Boarding Bridge Installation

BIA is an international airport which is the main eastern gate way for Indonesia, and it is the second largest international airport in Indonesia based on the annual passenger volume in 1981. Moreover, annual passenger volumes are forecast to be about 2.4 million by the year 1990.

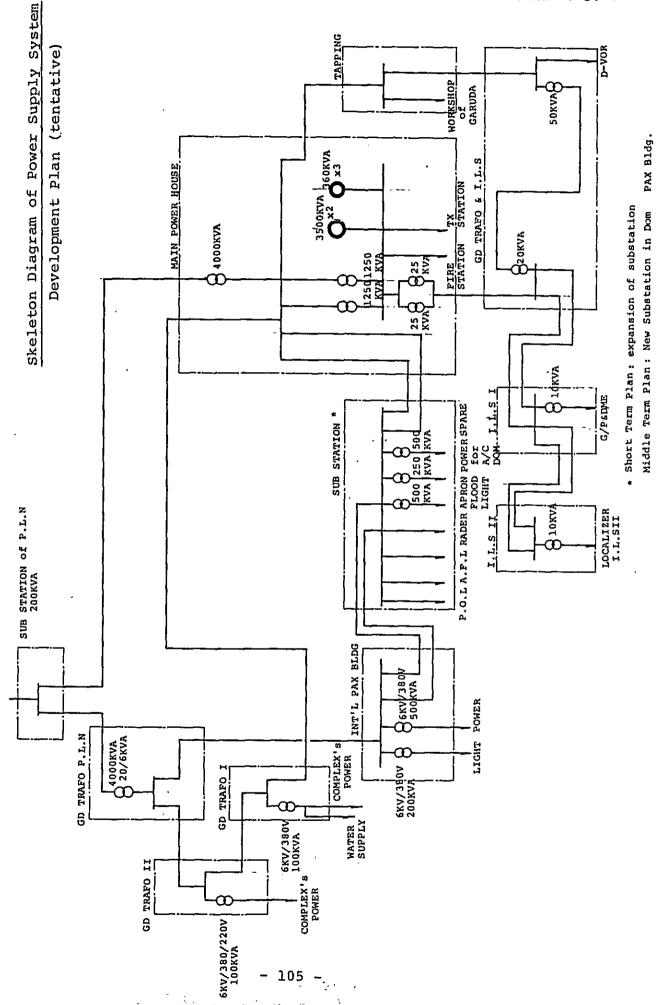
It is strongly recommended, therefore, that boarding bridges be installed in order to handle a large number of passengers safely and smoothly within a short time period.

With regard to the location of a traditional Bali ceremonial garden, BIA is able to express a warm welcome to all the tourist at the ceremonial garden in front of International passenger terminal building on the landside instead of the airside. The ceremonial garden located on the landside is more suitable than on the airside considering passenger safety control.

The results of a comparative study made for the two cases (i.e. Boarding bridge installed and not-installed) is shown in Table.

COMPARISON BETWEEN TWO CASES: (1) BOARDING BRIDGE INSTALLED AND (2) NOT INSTALLED

		*		
Cases Study Items	With Boarding Bridge	Without Boarding Bridge		
1. Terminal Concept 1 1/2 levels	Compatible	Not compatible		
2. Ground Service Equipment Maneuvering Efficiency	Advantageous	Disadvantageous		
3. Pax. handling efficiency 1) Confort 2) Safety 3) Mass handling capacity 4) Time control	Advantageous Ditto Ditto Ditto	Disadvantageous Ditto Ditto Ditto		
4. The ceremony garden	Not to be located at apron	To be located at apron		
Evaluation Summary	Recommendable	Not recommendable		



Standard of Water Quality

ITEMS	INDONESIA	WHO Over a year less than MPN 10		
· Coliform group	Negative/100ml			
No. of bacteria	Less than 100/1ml	<u>-</u> -		
Odor Taste	Clear, with no taste or odor			
Color Impurity	Not more than lmg/l	=		
Evaporated residue	Less than 1,000 ppm soluble substances	<u> </u>		
pH value	6.5 ∿ 9.0	7.0 ~ 8.5 (6.5 ~ 9.2)		
Total hardness	5 ∿ 10рн	100 ∿ 500 ppm*		
Amount of KMnO, consumed Cl	Less than 10 ppm 250 ppm	10 ppm 200 (400) ppm		
H ₂ SO ₄	250 ppm	200 (400) ppm		
Ammonia nitrogen Nitrite nitrogen	None allowed None allowed (excluding deep wells)	0.5 ppm		
Nitrate nitrogen Fe	20 ppm 0.2 ppm	40 (80) ppm** 0.3 (1.0) ppm		
Mn F	0.1 ppm 1 ∿ 1.5 ppm	0.1 (0.5) ppm 1.0 (1.5) ppm		
Pb As	0.05 ppm 0.05 ppm	0.1 ppm 0.2 ppm		
Selenium Cr	-	0.05 ppm 0.05 ppm		
Cu Zn	3.0 ppm 5.0 ppm	1.0 ppm 5.0 (15.0) ppm		
Phenol Cyanogen	-	0.001 (0.002) ppm 0.01 ppm		
Hg Ba	-			
Cd Anionic active agent	•	_		
Radioactivity	· — · · · · · · · · · · · · · · · · · ·	α-rays 10 ⁻⁹ μc/ml β-rays 10 ⁻⁹ μc/ml		
Organic phosphorus	•			
Free available chlorine	~			
Мд	125 ppm	50 (150) ppm		
Ca	~	75 (200) ppm		
Remarks	Temperature: lower than air temperature Erosive carbonic acid: none allowed H2S: none allowed Temperature: lower than () is for unavoidable cases * as CaCO3 ** as NO3			

OUTLET TO CANNEL BOD Porn CHLORINATION TANK CHLORINATOR SEDIMENTATION TANK WAYE PITFOR SLUDGE ONZ BLOWEOR TRICKING FILTER IMHOFF TANK PUMPPIT ____32qy48_____ TO SLUDGE DRYING BED BY VACAME CAR PUMP PIT AIRLIFT PUMP AERATED CRIT CHAMBER GRIT SPACE 88

SCHEMATIC DIAGRAM

SEWAGE TREATMENT SYSTEM as Reference only

RAW SEWAGE PLMP	LIFTING PUMP	CIRCULATING PUMP	CIRCULATING PUMP	FLOW METER	LEVEL INDICATOR	BLOWER	CLARIFIER	HOTOR WALVE CONTROL ED BY TIMER!		
(P) 2 R	(P) L	(P) 2.6 C	(P), g	3 (j)	1) ()	(B) ₁₂ B	<u>၁</u>	H) 0%		

- 107 -

