

#### 4.2.4. Drainage Plan

##### (1) Drainage from the Irrigated Paddy Field

The paddy fields in the Project Area drain excess water by natural gravity flow depending upon the topographic slope. The drained water flows into the low-lying fields, thus inundation will occur and crops suffer from damage.

Considering the above matters, drainage modulus in the irrigated paddy fields is computed by applying the following method with five-year return period and two consecutive rainfall observed at the Mae Tha station.

$$D = A (R(n, \max) t - n (D_c + C_u))$$

- where,
- d: Water depth on paddy field at n-day after the first rainfall
  - R(n, max)t: Maximum rainfall during n-day with t-year return period (mm/day)
  - D<sub>c</sub>: Drainage capacity (mm/day)
  - C<sub>u</sub>: Consumptive use of paddy (mm/day)
  - A: Low-lying area in the Project Area; 1/A of the Area

Probable rainfall with 5- and 10-year return period which was studied at "Hydrology" in this report is given as follows:

Return Period	Consecutive Rainfall (mm)		
	1-day	2-day	3-day
5	95	117	128
10	109	128	139

Provided that little damage would not be given to paddy plants when the flood lasts less than three days and remains less than 20 cm in depth on an average, the said equation is expressed as:

$$D_1 = A \{ R(1, \text{max}) T - D_c - C_u \}$$

$$D_2 = A \{ R(2, \text{max}) T - 2D_c - 2C_u \}$$

$$\frac{D_1 + D_2}{2} = \frac{A}{2} \{ R(1, \text{max})T + R(2, \text{Max})T - 3D_c - 3C_u \} < 200$$

$$\therefore D_c > \frac{1}{3} R(1, \text{max})T + \frac{1}{3} R(2, \text{max})T - \frac{400}{3A} - C_u$$

Where,  $R(1, \text{max})5 = 95$ ,  $R(2, \text{max})5 = 117$

$$A = 5 \quad C_u = 4.9 \text{ mm/day}$$

$$D_c > \frac{1}{3} \times 95 + \frac{1}{3} \times 117 - \frac{400}{3 \times 5} - 4.9$$

$$= 70.7 - 28.5$$

$$= 42.2 \text{ mm/day}$$

$$= 4.88 \text{ } \ell/\text{s}/\text{ha}$$

On the basis of the above method, drainage modulus is obtained at 4.88  $\ell/\text{sec}/\text{ha}$ .

To apply this modulus for each drainage system, the following reduction rate should be considered under the Area effected by specific rainfall distribution i.e., spot rainfall in the tropical zone.

<u>Runoff Reduction Rate</u>	
<u>Drainage Area (km<sup>2</sup>)</u>	<u>Ratio</u>
0 - 3	1.00
3 - 8	0.90
8 - 16	0.85
16 - 30	0.80
30 - 80	0.75
80 - 160	0.70
Over 160	0.65

## (2) Drainage from Hilly Area

The peak discharge from the hilly area is estimated by Rational method.

$$Q = 1/3.6 \times f \times r_t \times A$$

Where, Q: Drainage discharge ( $m^3/s$ )

f: Runoff ratio = 0.45

$r_t$ : Average rainfall intensity within a flood concentration hour (mm/hr)

$$r_t: r_{24}/24 (24/T)^n$$

$r_{24}$ : Probable daily rainfall

5-year return period 95 mm/day

n: 0.5

T: Flood concentration hour

T is determined by Kerbay's and Rziha's Formula.

T = 1.0 m

Flood concentration hour (T) is obtained by the following procedure:

$$T = T_1 + T_2$$

Where,  $T_1 = (2/3 \times 3.28 \times l \times \frac{n}{\sqrt{s}})^{0.467}$  Kerbay's Formula

l: Runoff distance at mountain sides (m) 300

s: Average slope 1/15

n: Delaying coefficient 0.60

$$T_2 = L/W \quad W = 20 (H/L)^{0.6} \quad \text{Rziha's Formula}$$

L: Horizontal river length for flowing (m)

H: Elevation difference between upstream and downstream point (m)

In the above equation,  $T_1$  is given at 21 minutes. On the other hand,  $T_2$  and  $T$  is obtained as:

<u>L</u>	<u>H</u>	<u>W</u>	<u>T<sub>2</sub></u>		<u>T = T<sub>1</sub> + T<sub>2</sub></u>
(m)	(m)	(m/sec)	(sec)	(min)	(min)
1,000	20	1.9	526	9	30
2,000	25	1.4	1,429	24	45
3,000	30	1.3	2,308	39	60

$T$  is obtained at less than one hour, therefore,  $T$  is given at one hour in the said equation.

Drainage modulus on the hilly Area is obtained at 24.3  $\ell$ /sec/ha. this value is also modified depending upon the drainage area by using the reduction rate as shown in previous paragraph.

On-farm Development Plan

Implementation of on-farm development should be carried out by farmers themselves under assistance of the Project Office and Regional Office of RID after completion of construction of the Project.

The said on-farm facilities is composed of structures to be delivered irrigation water from a turn-out of main/lateral canal up to farm lot, such as Main Farm Ditch (MFD), Supplementary Farm Ditch (SFD), Field Ditch (FD), Division Box (DB) and End Check (EC).

A Cost of the on-farm development is estimated based on a layout of typical irrigation block as shown in Fig.

(1) Construction Cost of a Chak of On-farm Facilities

<u>Description</u>	<u>Quantity</u> (m)	<u>Unit Cost</u> (₹)	<u>Amount</u> (₹)
<b>Construction Cost</b>			
Main Farm Ditch	300	38	11,400
Supplementary Farm Ditch	4,100	25	102,500
Field Ditch	4,000	7	28,000
Division Box	4	1,500	6,000
End Check	5	750	3,750
<u>Sub-total</u>			<u>151,650</u>
Engineering, Administration and others (30%)			48,350
<b>Total</b>			<b>200,000</b>
Cost per rai			<u>₹ 800</u>
Foreign Currency			200
Local Currency			600

<u>Total Construction Cost for Case-5 (50,600 rai)</u>	¥ 40,480,000
Foreign Currency	10,120,000
Local Currency	30,360,000
<u>Total Construction Cost for Case-6 (45,900 rai)</u>	¥ 36,720,000
Foreign Currency	9,180,000
Local Currency	27,540,000

(2) Implementation Schedule and Disbursement Schedule

Implementation of the on-farm development will be carried out for a period of five years started from 1988 to 1992 and disbursement will be followed.

Case-5

<u>Year</u>	<u>Total</u> (¥'000)	<u>Foreign Currency</u> (¥'000)	<u>Local Currency</u> (¥'000)
1988	8,096	2,024	6,072
1989	8,096	2,024	6,072
1990	8,096	2,024	6,072
1991	8,096	2,024	6,072
1992	8,096	2,024	6,072
<u>Total</u>	<u>40,480</u>	<u>10,120</u>	<u>30,360</u>

Case-6

<u>Year</u>	<u>Total</u> (¥'000)	<u>Foreign Currency</u> (¥'000)	<u>Local Currency</u> (¥'000)
1988	7,344	1,836	5,508
1989	7,344	1,836	5,508
1990	7,344	1,836	5,508
1991	7,344	1,836	5,508
1992	7,344	1,836	5,508
<u>Total</u>	<u>36,720</u>	<u>9,180</u>	<u>27,540</u>

(3) Unit Construction Cost

(i) Main Farm Ditch (MFD)

$$Q = 0.116 \text{ m}^3/\text{s}, \quad I = 1/700, \quad n = 0.030, \quad V = 0.435$$

$$\text{Excavation} \quad 0.043 \text{ m}^3 \times \text{P} 70 \times 0.3 = \text{P} 0.90$$

$$\text{Embankment} \quad 0.866 \text{ m}^3 \times \text{P} 70 \times 0.5 = \text{P} 30.31$$

$$\text{Miscellaneous} \quad \text{P} 6.24$$

$$\text{Total Cost per linear meter} \quad \text{P} 37.45$$

$$\text{Say} \quad \text{P} 38.00$$

(ii) Supplementary Farm Ditch (SFD)

$$Q = 0.116 \text{ m}^3/\text{s}, \quad I = 1/700, \quad n = 0.030, \quad V = 0.425$$

$$\text{Excavation} \quad 0.040 \text{ m}^3 \times \text{P} 70 \times 0.3 = \text{P} 0.84$$

$$\text{Embankment} \quad 0.560 \text{ m}^3 \times \text{P} 70 \times 0.5 = \text{P} 19.60$$

$$\text{Miscellaneous} \quad \text{P} 4.09$$

$$\text{Total Cost per linear meter} \quad \text{P} 24.53$$

$$\text{Say} \quad \text{P} 25.00$$

(iii) Field Ditch (FD)

$$Q = 0.012 \text{ m}^3/\text{s}, \quad I = 1/500, \quad n = 0.040, \quad V = 0.297$$

$$\text{Excavation} \quad 0.068 \text{ m}^3 \times \text{P} 70 \times 0.3 = \text{P} 1.43$$

$$\text{Embankment} \quad 0.105 \text{ m}^3 \times \text{P} 70 \times 0.5 = \text{P} 3.68$$

$$\text{Miscellaneous} \quad \text{P} 1.02$$

$$\text{Total Cost per linear meter} \quad \text{P} 6.13$$

$$\text{Say} \quad \text{P} 7.00$$

(iv) Division Box and End Check

Cost per sheet

$$\text{Concrete} \quad 0.077 \text{ m}^3 \times \text{P} 6,200 = \text{P} 477.40$$

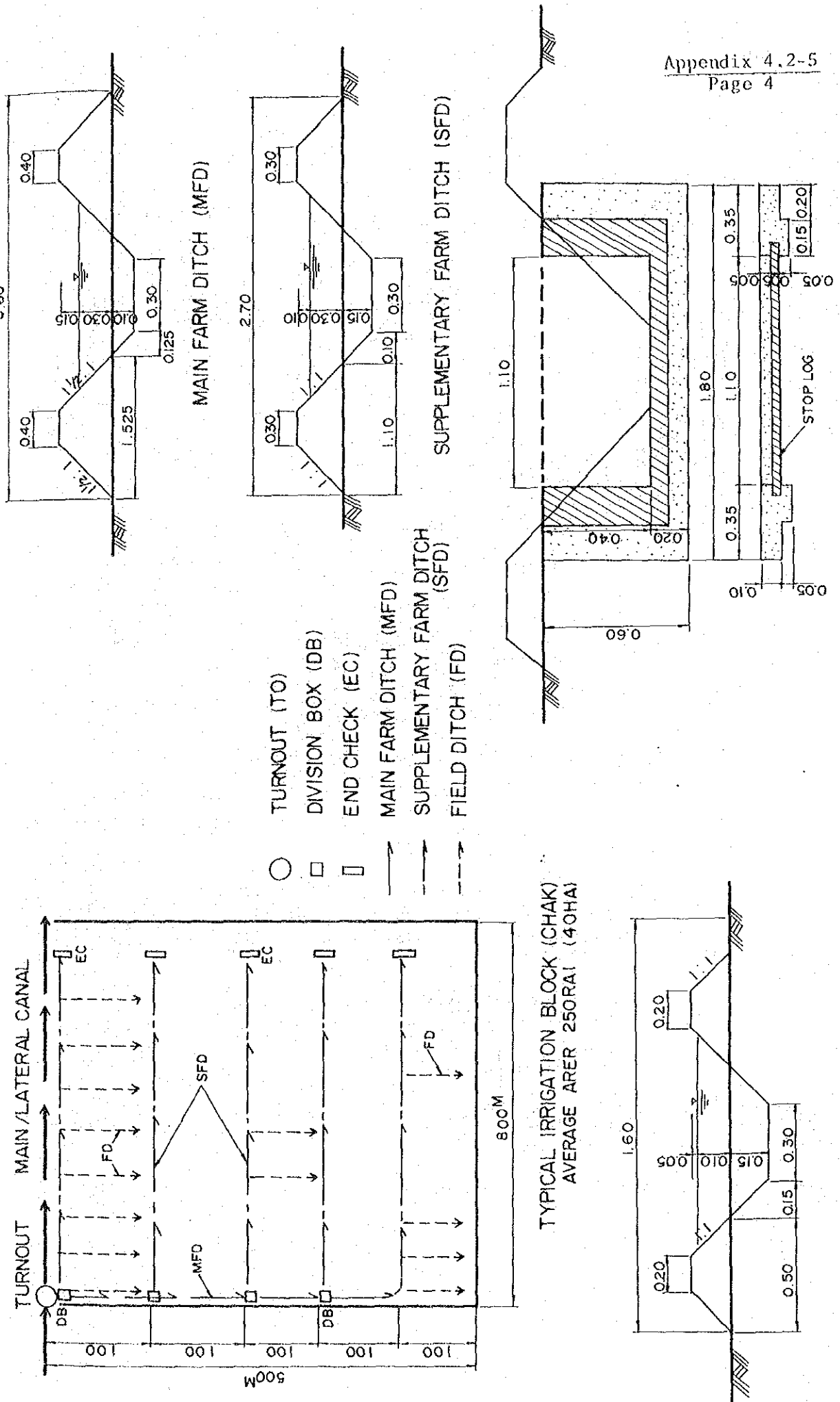
$$\text{Stop log} \quad 0.024 \text{ m}^3 \times \text{P} 6,000 = \text{P} 144.00$$

$$\text{Others including installation cost} \quad \text{P} 128.60$$

$$\text{Total} \quad \text{P} 750.00$$

ON-FARM DEVELOPMENT FACILITIES

FIG 4.2-6





4.2-6 Hydropower Development Plan

1. List of Hydropower Generating Facilities

- (1) 2 sets of 82 kW Horizontal Shaft Francis Turbine
- (2) 2 sets of Butterfly Valve
- (3) 2 sets of Governor
- (4) 1 set of Oil Pressure Supply System
- (5) 1 set of 278 kVA Horizontal Shaft Synchronous Generator
- (6) 1 set of Generator Panel
- (7) 1 set of Control Panel
- (8) 1 set of Station Service
- (9) 1 set of Diesel Generator Unit
- (10) 1 set of Distribution Line for 1 km
- (11) Transportation and Installation

2. Cost Estimate

(Unit: ¥ '000)

2-1. Civil Works

Earth Works	1,400 m <sup>3</sup>	100
Reinforced Concrete	150 m <sup>3</sup>	600
Riprap	250 m <sup>3</sup>	500
Steel Pipe (φ600 mm)	20 m <sup>3</sup>	400
Building	35 m <sup>3</sup>	175
Miscellaneous	L.S.	225
<u>Sub-total</u>		<u>2,000</u>

2-2. Hydropower Facilities

Horizontal Shaft Francis Turbine (82 kW 2 sets)	5,000
Butterfly Valve, Governor and Oil System	7,500
Horizontal Shaft Synchronous Generator (278 kVA)	3,500
Diesel Generator Unit, Panels and Station Services	8,000
Distribution Line for One Kilometer	1,000
Transportation and Installation	10,000
<u>Sub-total</u>	<u>35,000</u>

2-3. Communication and Other Facilities

5,000

2-4. Engineering and Administration

13,000

Total

55,000

### 3. DESIGN REQUIREMENT

#### 3-1. HYDRAULIC TURBINE

- |                          |   |  |
|--------------------------|---|--|
| (1) No. of Unit          | : | 2 sets   |
| (2) Type                 | : | Horizontal Shaft Francis                       |
| (3) Rated Output         | : | 82 kW each                                     |
| (4) Rated Speed at 50 Hz | : | 1000 rpm                                       |
| (5) Rated Net Head       | : | 13.0 m   |
| (6) Rated Flow           | : | 0.75 m <sup>3</sup> /s each                    |
| (7) Net Head Range       | : | 9.0 m Min. ~ 17.0 m Max.                       |
| (8) Flow Range           | : | 0.5 m <sup>3</sup> /s ~ 1.65 m <sup>3</sup> /s |

#### 3-2. SYNCHRONOUS GENERATOR

- |                          |   |   |
|--------------------------|---|---|
| (1) No. of Units         | : | 1 set   |
| (2) Type                 | : | Horizontal shaft synchronous,<br>hydraulic turbines driven at<br>both shaft ends. |
| (3) Output               | : | 278 kVA   |
| (4) Rated Voltage        | : | 416 V   |
| (5) Power Factor         | : | 80 %  |
| (6) Frequency            | : | 50 Hz   |
| (7) No. of Poles         | : | 6 P   |
| (8) No. of Phases        | : | 3 Phase   |
| (9) Rated Speed          | : | 1000 rpm  |
| (10) Rating              | : | Continuous  |
| (11) Class of Insulation | : | Class B   |

FIG. 4.2-7 AVERAGE FLOW (1952 ~ 1981)

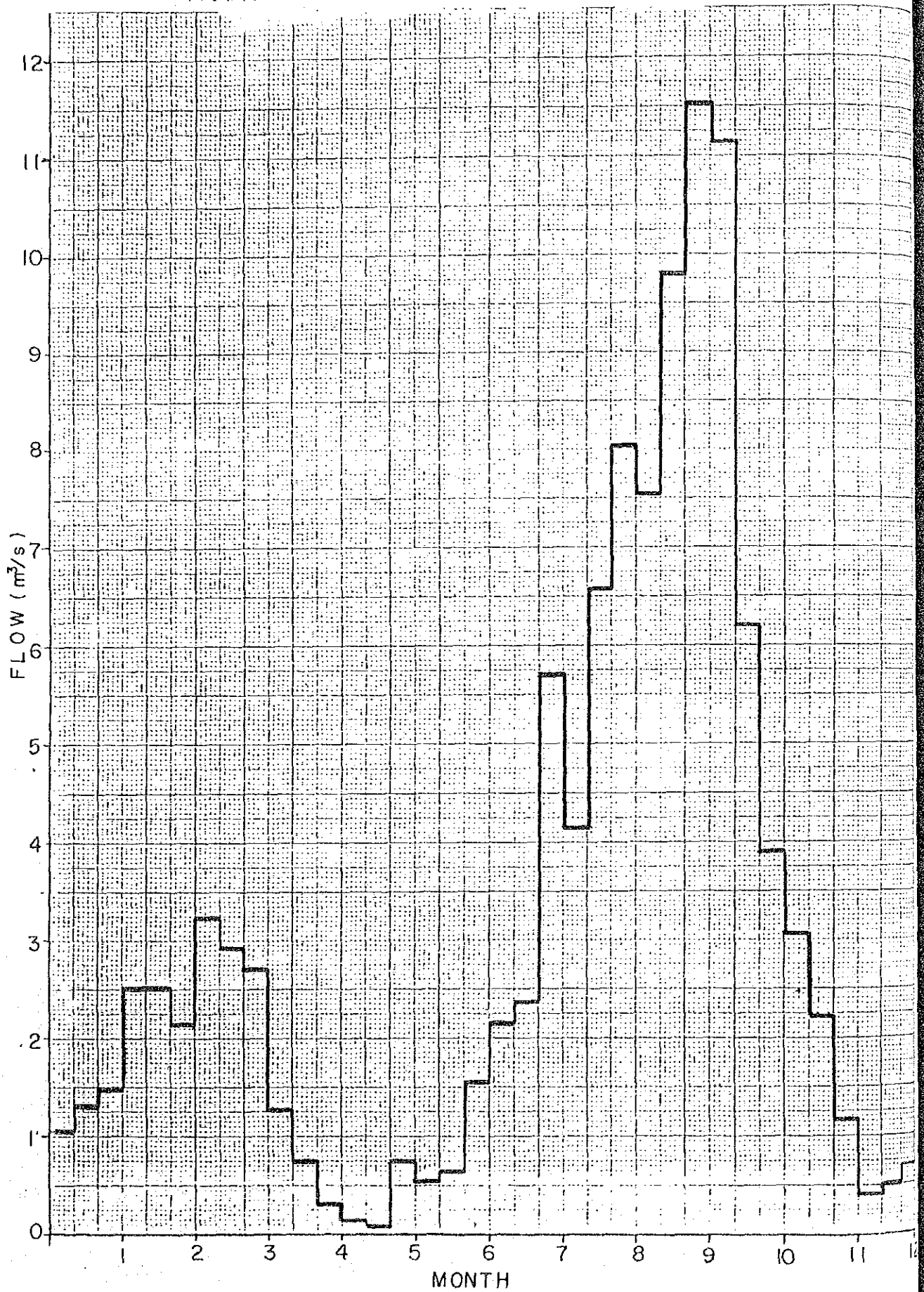


FIG. 4.2-8 FLOW DURATION CURVE (1952~1981)



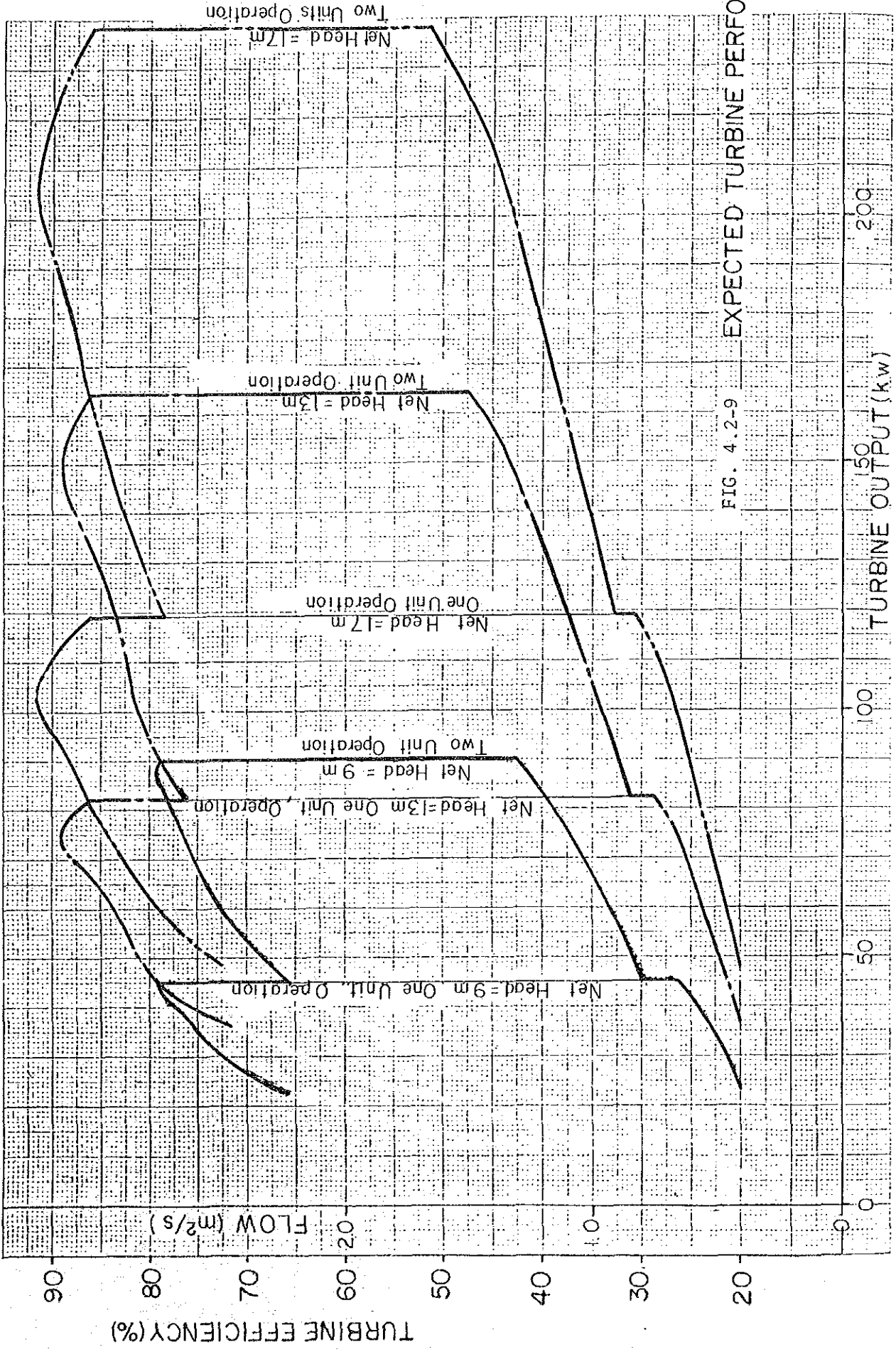


FIG. 4.2-9 EXPECTED TURBINE PERFORMANCE

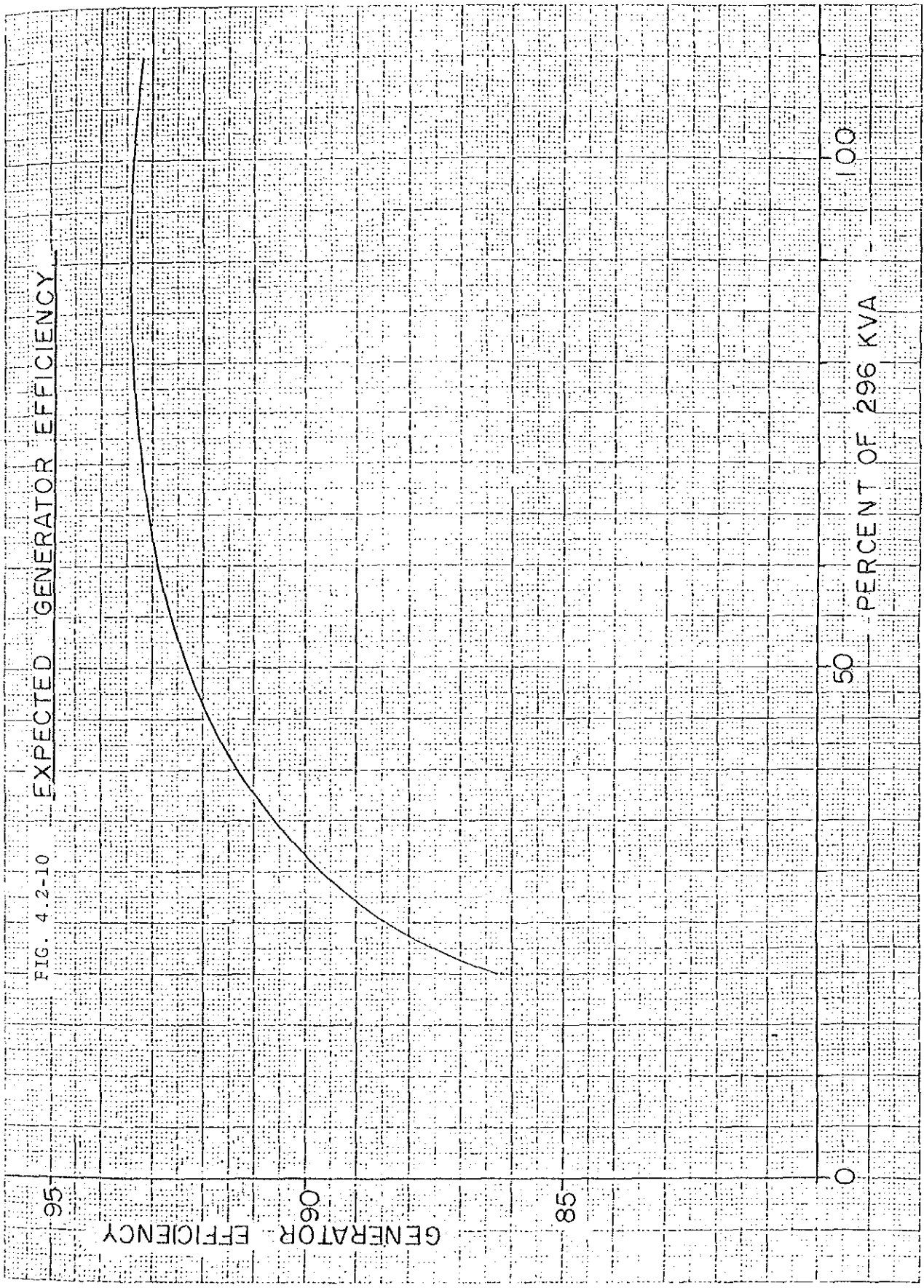
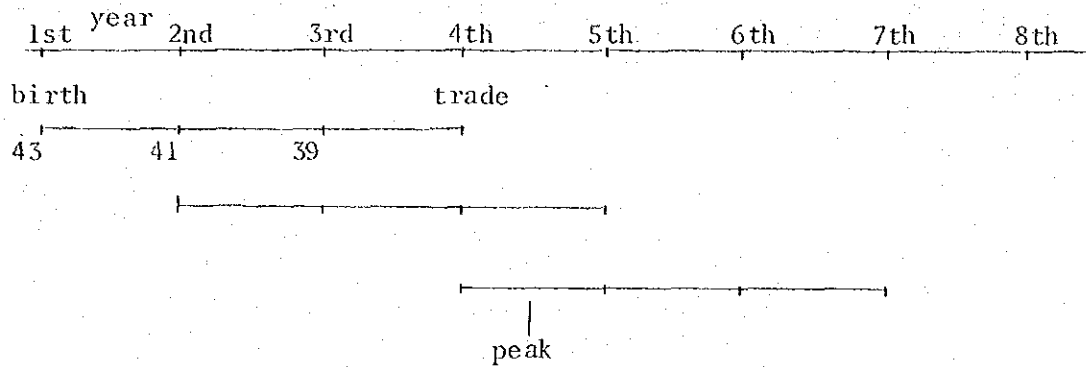


FIG. 4.2-10 EXPECTED GENERATOR EFFICIENCY

Table 4.3-1 Raising Pattern



3) Number of Feeding Cattle

Number of Raising Unit	30	$5,208^t \div 171.7^t$
Number of Feeding Cattle	6,480 <sup>head</sup>	$216^{\text{head}} \times 30$
Number of Selling Cattle	1,170 <sup>head</sup>	$39^{\text{head}} \times 30$

For other cases, computation has been made in the same methods.

Table 4.3-2 Program for Increasing Beef Cattle

Case 5	130%		
Paddy	W.	40,500 <sup>rai</sup>	(NG 24,300 <sup>rai</sup> , G 16,200 <sup>rai</sup> )
Soybean	W.	5,000	
	D.	4,400	

## 1. Paddy Rice Straw (Same Case 1)

Available straw for feeding.

## 2. Soybean Shell

W.	125 <sup>kg/rai</sup>	x 5,000	x 0.9 = 563 <sup>t</sup>
D.	150	x 4,400	x 0.9 = 594
<u>Total</u>			<u>1,157</u>

## 3. T.D.N.

Rice Straw	12,533 <sup>t</sup>	x 0.371 = 4,650 <sup>t</sup>
Soybean Shell	1,157	x 0.483 = 559
<u>Total</u>		<u>5,209</u>

## 4. Feeding Plan for Cattle (Same Case 1)

Number of Raising Unit	30.3	5,209 ÷ 171.7
Number of Feeding Cattle	6,545 <sup>head</sup>	216 x 30.3
Number of Selling Cattle	1,182 <sup>head</sup>	39 x 30.3



Case 6 135%

Paddy	W.	36,400 <sup>rai</sup>	(NG 21,800 <sup>rai</sup> , G 14,600 <sup>rai</sup> )
Soybean	W.	3,100	
	D.	4,800	

1. Paddy Rice Straw (Same Case 1)

Available straw for feeding.

2. Soybean Shell

W.	125 <sup>kg/rai</sup>	x 3,100	x 0.9 = 349 <sup>t</sup>
D.	150	x 4,800	x 0.9 = 648
<u>Total</u>			<u>997</u>

3. T.D.N.

Rice Straw	11,208 <sup>t</sup>	x 0.371 = 4,158 <sup>t</sup>
Soybean Shell	997	x 0.483 = 482
<u>Total</u>		<u>4,640</u>

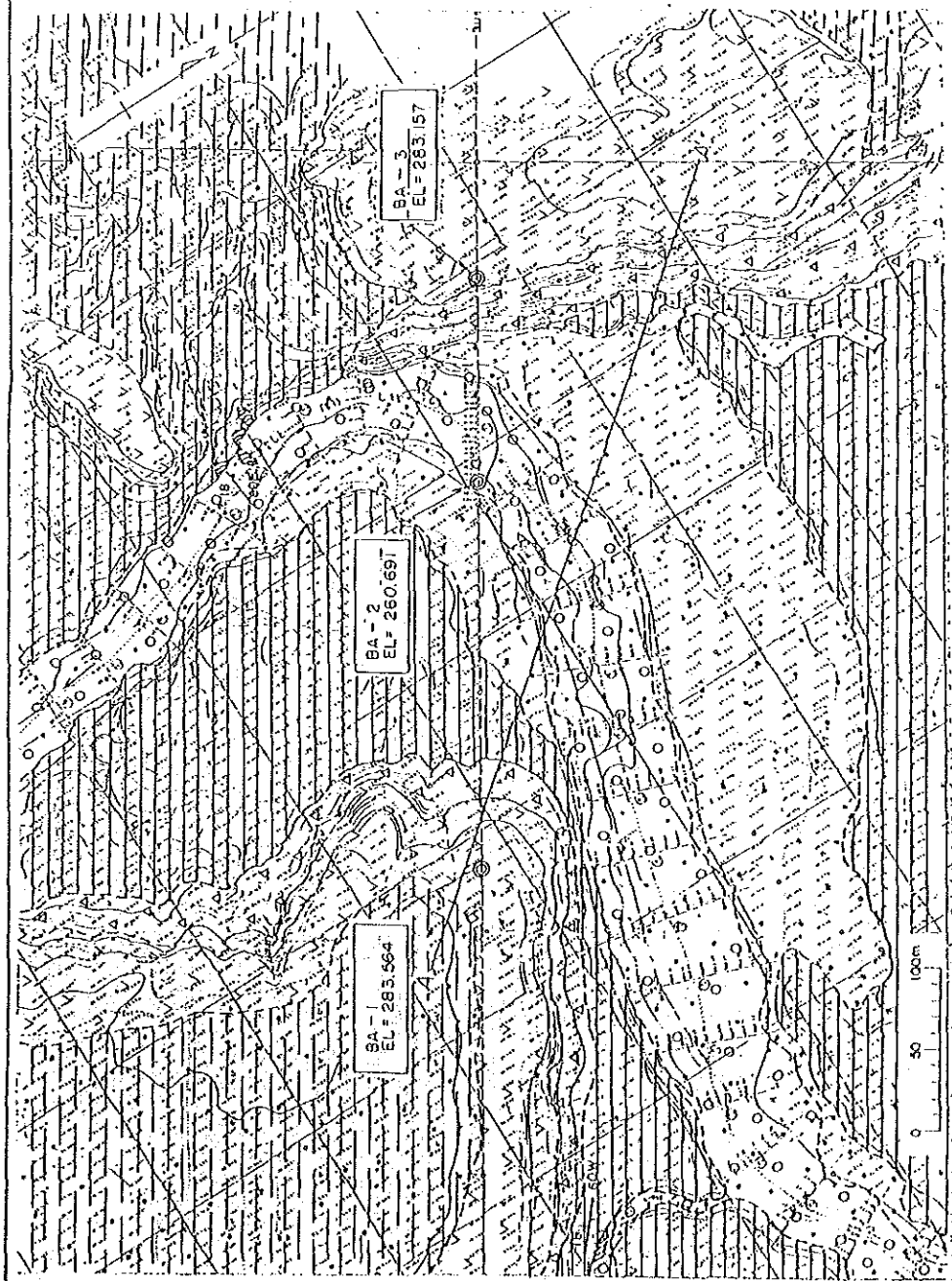
4. Feeding Plan for Cattle (Same Case 1)

Number of Raising Unit	27	4,640 <sup>t</sup> ÷ 171.7 <sup>t</sup>
Number of Feeding Cattle	5,832 <sup>head</sup>	216 x 27
Number of Selling Cattle	1,053 <sup>head</sup>	39 x 27

Table 4.4-1 Rock Classification

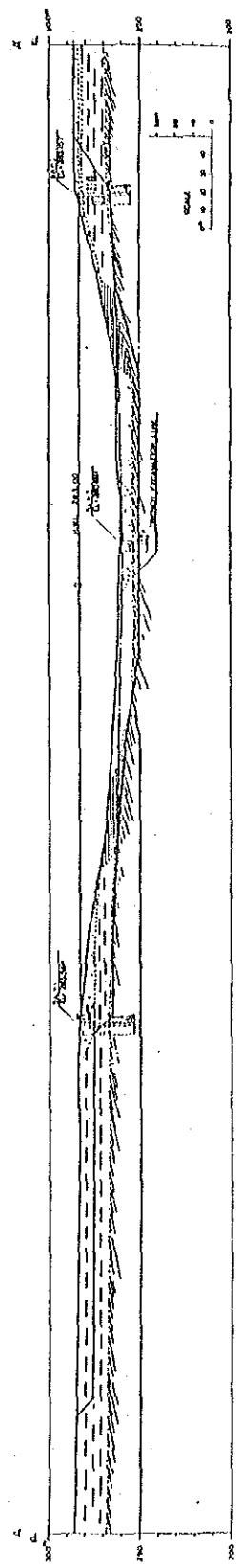
Class	Characteristics
A	<p>Very fresh and hard rock. Minerals particle is not weathering and change in quality. Crack and joint is hard, connecting. By hammering sound is clear. Boring core recovery is usually 90% over.</p>
B	<p>Fresh rock, but sometimes partly minerals and particle is little weathering and change in quality. Crack and joint is adhere. Hard rock by hammering sound is clear. Boring core recovery is almost 70% over and core pieces length over 5 cm. sometimes broken pieces</p>
CH	<p>Minerals and particle usually change in quality except quartz but rock is usually hard. By strong hammering, crack and joint separate, sometimes thin clay adhere to separated plane. By hammering sound is little dirty. Boring core recovery is 60-70% core length 3-5 cm, core form is almost columnar.</p>
CM	<p>Minerals and particle usually change in quality little weathering Soft to medium hard rock. By strong hammering crack and joint easily separate, thin clay adhere to separated plane. By hammering sound is not clear. Boring core recovery is 50-60%, core length 1-5 cm, sometimes columnar or sometimes broken pieces</p>
CL	<p>Minerals and particle is almost weathering and changing quality, so becomes soft rock. By soft hammering easily separate along the joint and breaks. Almost separated plane has clay, in a soft rock sometimes changing clay. Boring core recovery by using water is 30-50%, core form almost broken pieces.</p>
D	<p>Minerals and particle is heavily weathered. Almost soft rock change in quality like clay. In a hard rock crack and joint is almost open, by soft hammering easily broken. Boring core by using water almost loss.</p>

Recent		Age	Sym	Formation	Description
Recent	o	Recent	o	River Deposit	sand, silt, clay, gravel Muechang river deposit 6-8m thickness
Recent	Δ	Recent	Δ	Talus Deposit	sand, silt and basalt boulder 1-3m thickness
Alluvium	o	Alluvium	o	Lower Terrace Deposit	mostly silty sand partly sand and gravel 4-6m thickness N 10-15
Alluvium	o	Alluvium	o	Middle Terrace Deposit	mostly sandy silt partly sand and gravel 4-6m thickness N 15-30
Diluvium	v	Diluvium	v	Basalt	hard rock, distributed along the river 2-8m thickness
Diluvium	o	Diluvium	o	Higher Terrace Deposit	silty sand and silt partly sand and gravel semiconsolidated layer N 30-50 over 15-20m thickness
Mesozoic	o	Mesozoic	o	Lampung Group Mudstone	bedrock, partly tuff- aceous, sandy, and conglom- erate, low permeability soft rock, compression 80-100kg/cm (presumption)



(o) Boring Point  
 --- Geological Boundary  
 --- Dip and Strike  
 --- Fault  
 A --- Geological Section  
 --- Dam axis  
 N = Penetration Test  
 Lu = Lugon Test  
 --- Geological Boundary  
 --- Ground Water Table  
 Boring Point

FIG. 4.4-1  
Geological Map of  
Storage Dam A



Age	Sym. Formation	Description
Recent	••••• ••••• •••••	sand, silt, clay, gravel River Maechang river deposit 3-5m thickness
Recent	Δ	hill area limestone boulder, river side sand and silt. loose 1-3m thickness
Alluvium		mostly sandy silt dis- tributed right and left bank. 3-5m thickness N=15-35
Meso-zoic		hard rock, massive but partly heavy cracky and high permeability Massive compression 500kg/cm <sup>2</sup> limestone cover

◎	Boring Point	○	Sink hole
-----	Geological Boundary		Adit
20	Dip and Strike		
C-----C	Geological Section (Dam axis)		

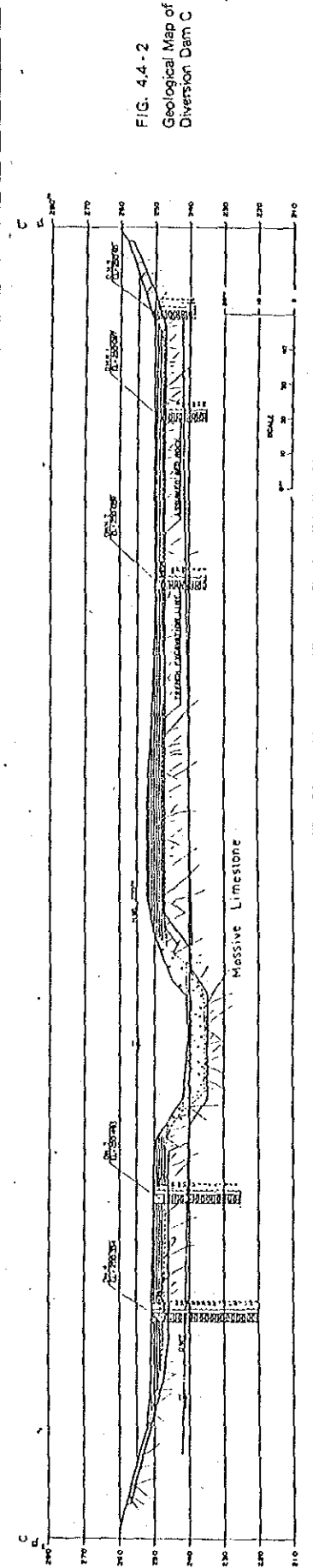
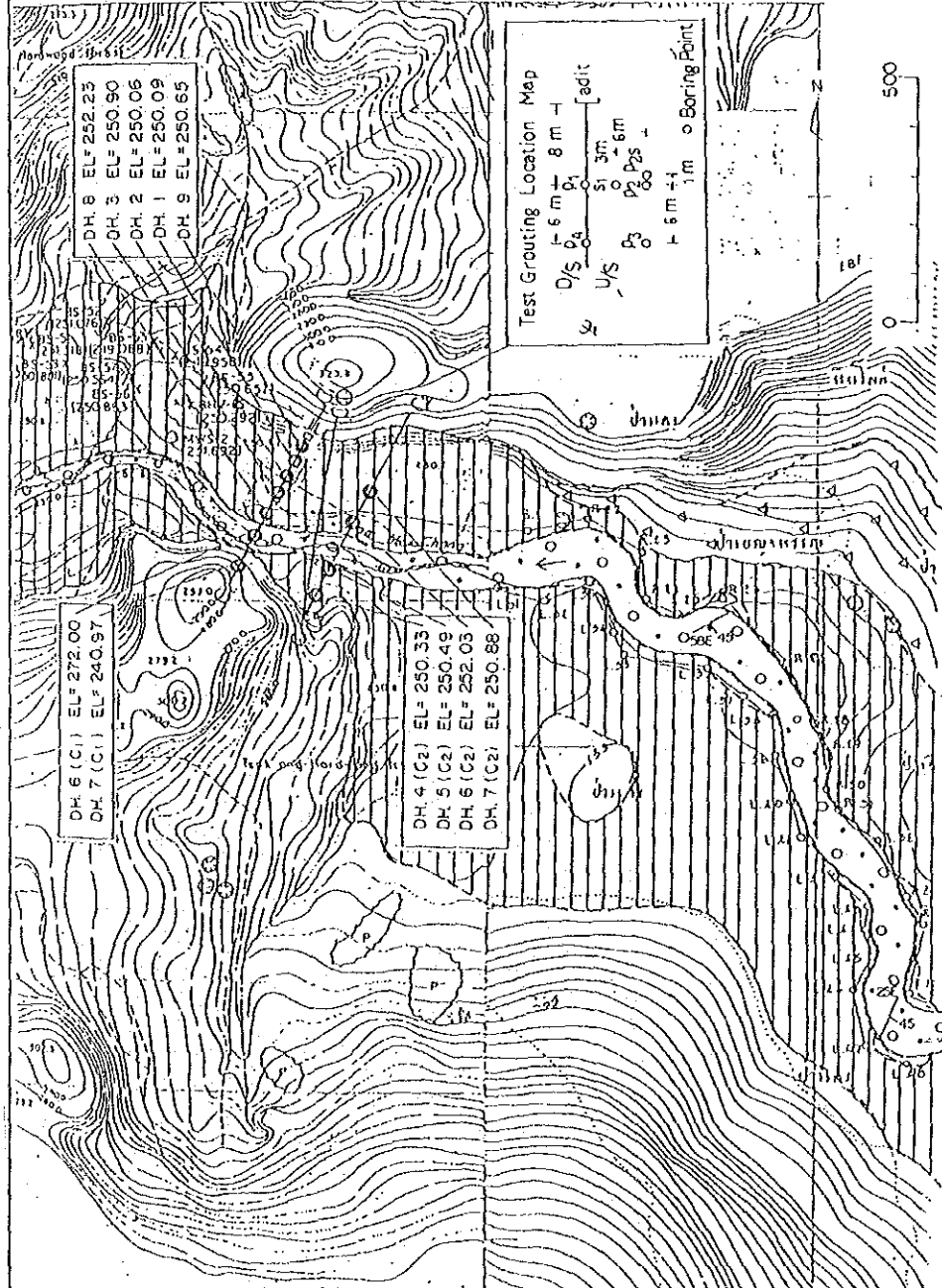
  


FIG. 4.4-2  
Geological Map of  
Diversion Dam C

Recent

Age	Sym	Formation	Description
Re-cent	o/o	River Deposit	sand, silt, clay, gravel Maechang river deposit 5~7m thickness
Re-cent	Δ	Talus Deposit	sand, silt and limestone boulder. 2~5m thickness
Alluvium	o/o	Lower terrace Deposit	mostly silty sand partly sand and gravel 3~5m thickness N-10-15
Alluvium	o/o	Middle terrace Deposit	mostly sandy silt partly sand and gravel 10~15m thickness
Diluvium	o/o	Higher terrace Deposit	silty sand and silt partly sand and gravel semiconsolidated. N30~50 over 15~20m thickness
	o/o	Calcareous Sandstone	hard rock, banded, distributed river and river-side, compression 200~ 300kg/cm <sup>2</sup> . Lu=10 under fresh rock.
Mesozoic		Shale Sandstone alternation	Shale, sandstone alternation partly conglomerate tufaceous, compression 150~300kg/cm <sup>2</sup> Lu=generally 20 under fresh rock
		Massive Limestone	hard rock, distributed out of damsite, cracky

© Boring Point D ——— D' Geological Section  
 20 30N Dip and Strike Boring Point Dam axis  
 Fault ——— Geological Boundary  
 Ground Water Table

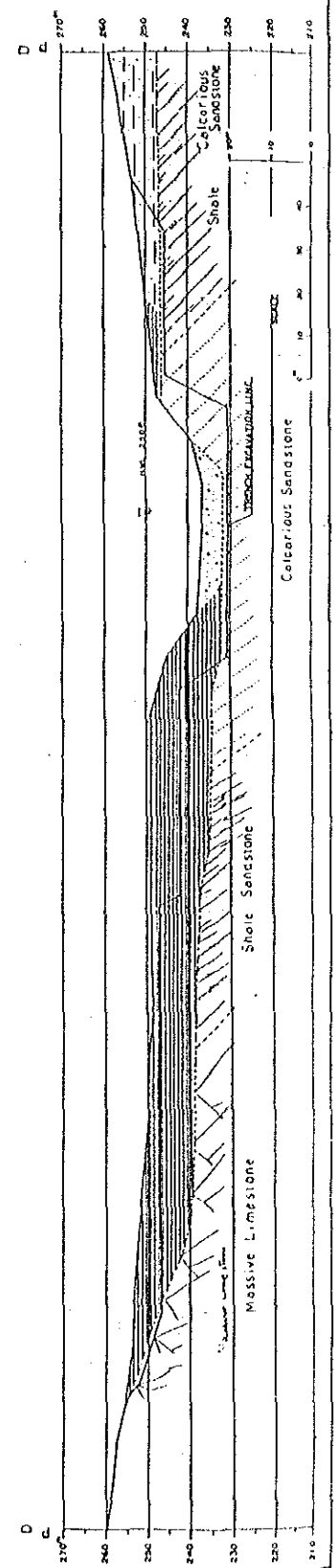
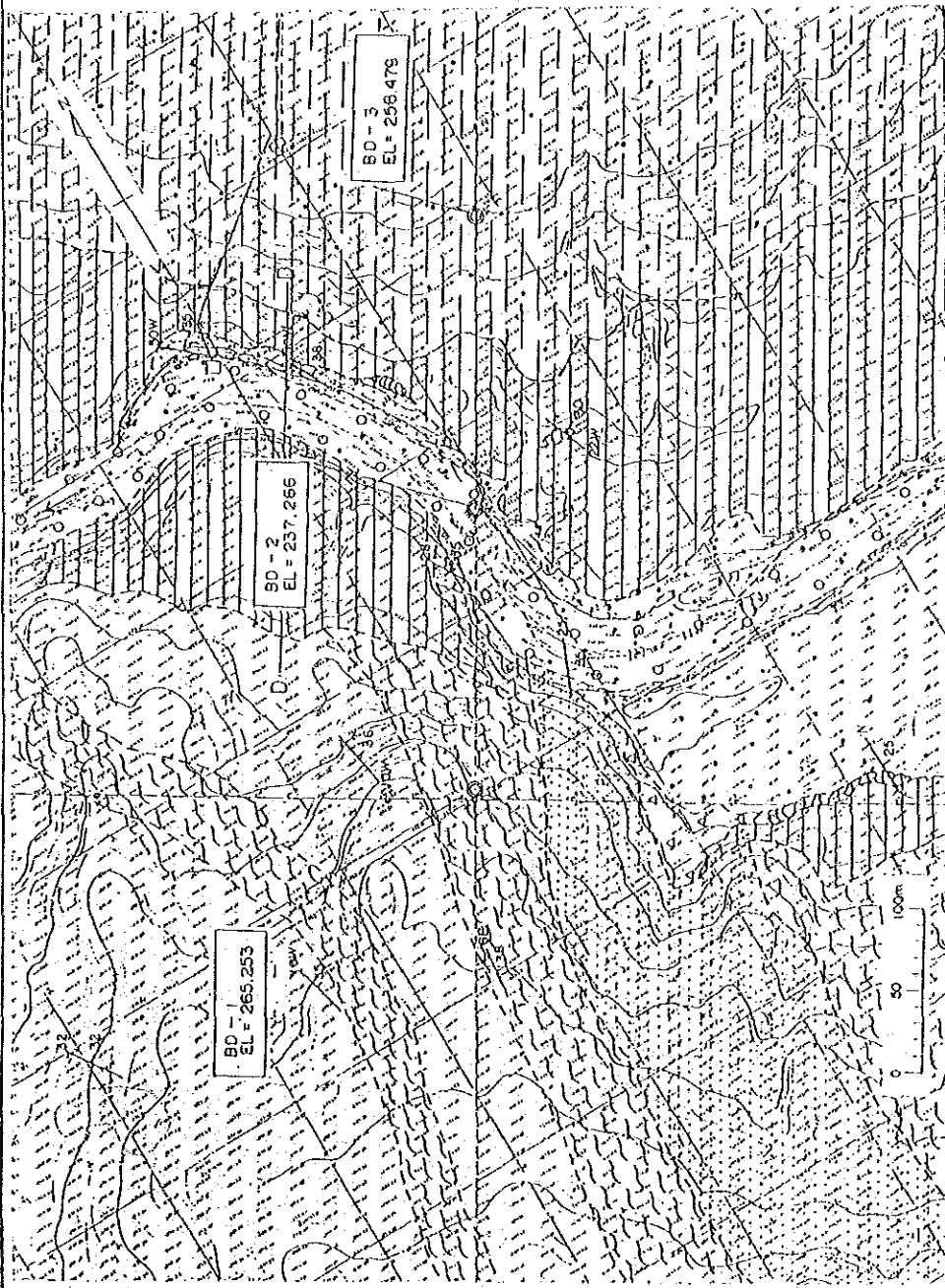


FIG. 4.4-3  
Geological Map of  
Diversion Dam D

Regent

Age	SYM-formation	Description
Recent	Recent River Deposit	sand, silt, clay, gravel Maechang river deposit 3-5m thickness
Recent	Recent Talus Deposit	hill area limestone boulder, river side sand and silt, loose 1-3m thickness
Alluvium	Alluvium Middle Terrace Deposit	mostly sandy silt dis- tributed right and left bank, 3-5m thickness M-15-35
Mesozoic	Mesozoic Lampang Group Massive Limestone	hard rock, massive but partly heavy cracky and high permeability compression 500kg/cm <sup>2</sup> over

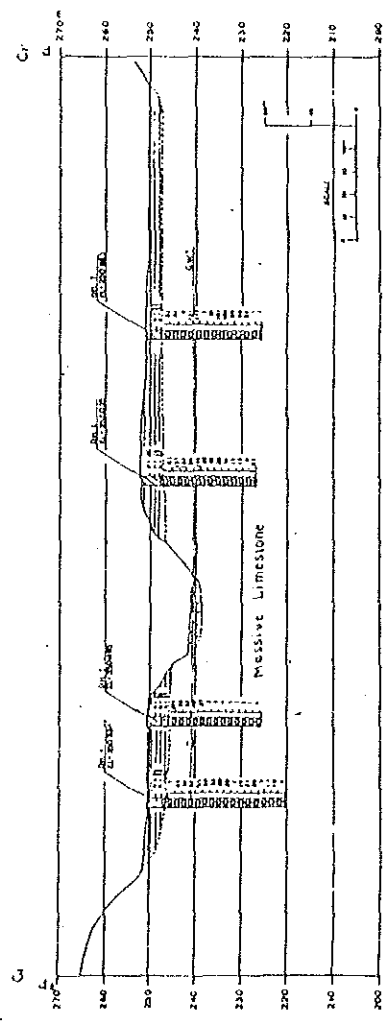
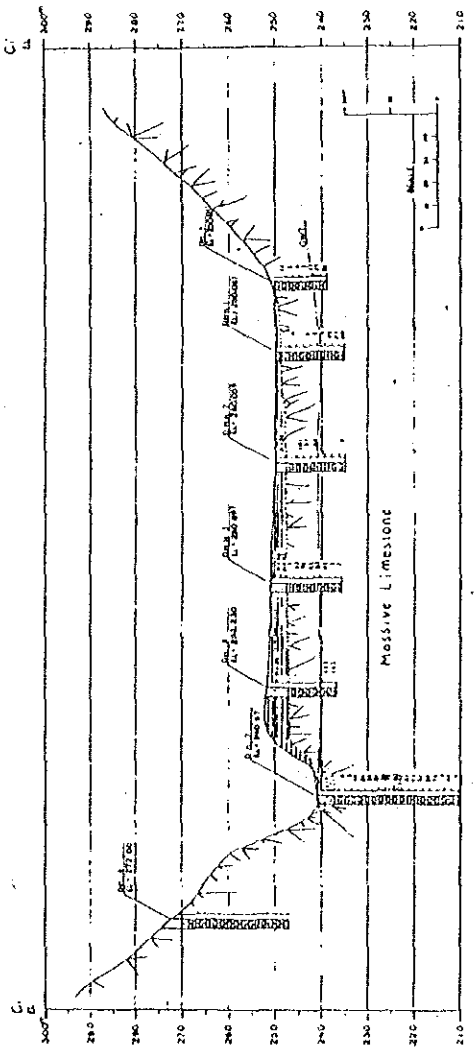
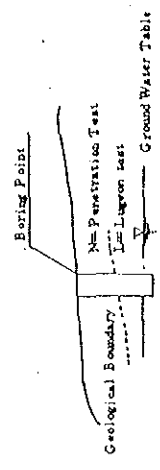
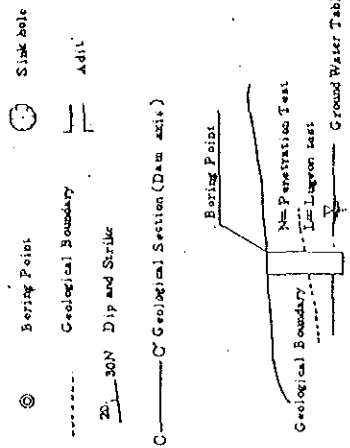
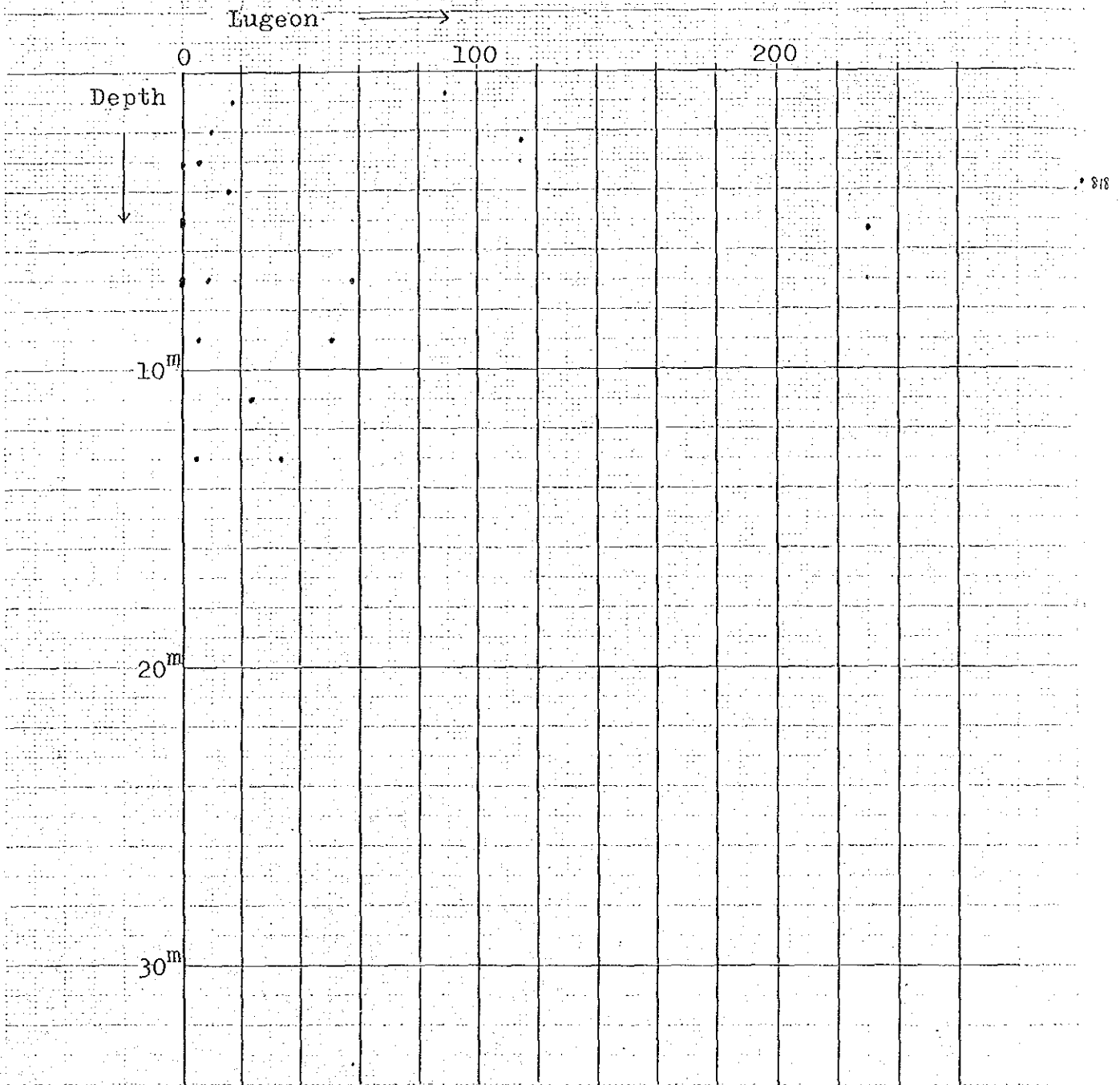


FIG. 4.4-4  
Geological Cross Section of Diversion Dam C

FIG. 4.4-5 THE RELATIONSHIP BETWEEN LUGEON AND DEPTH

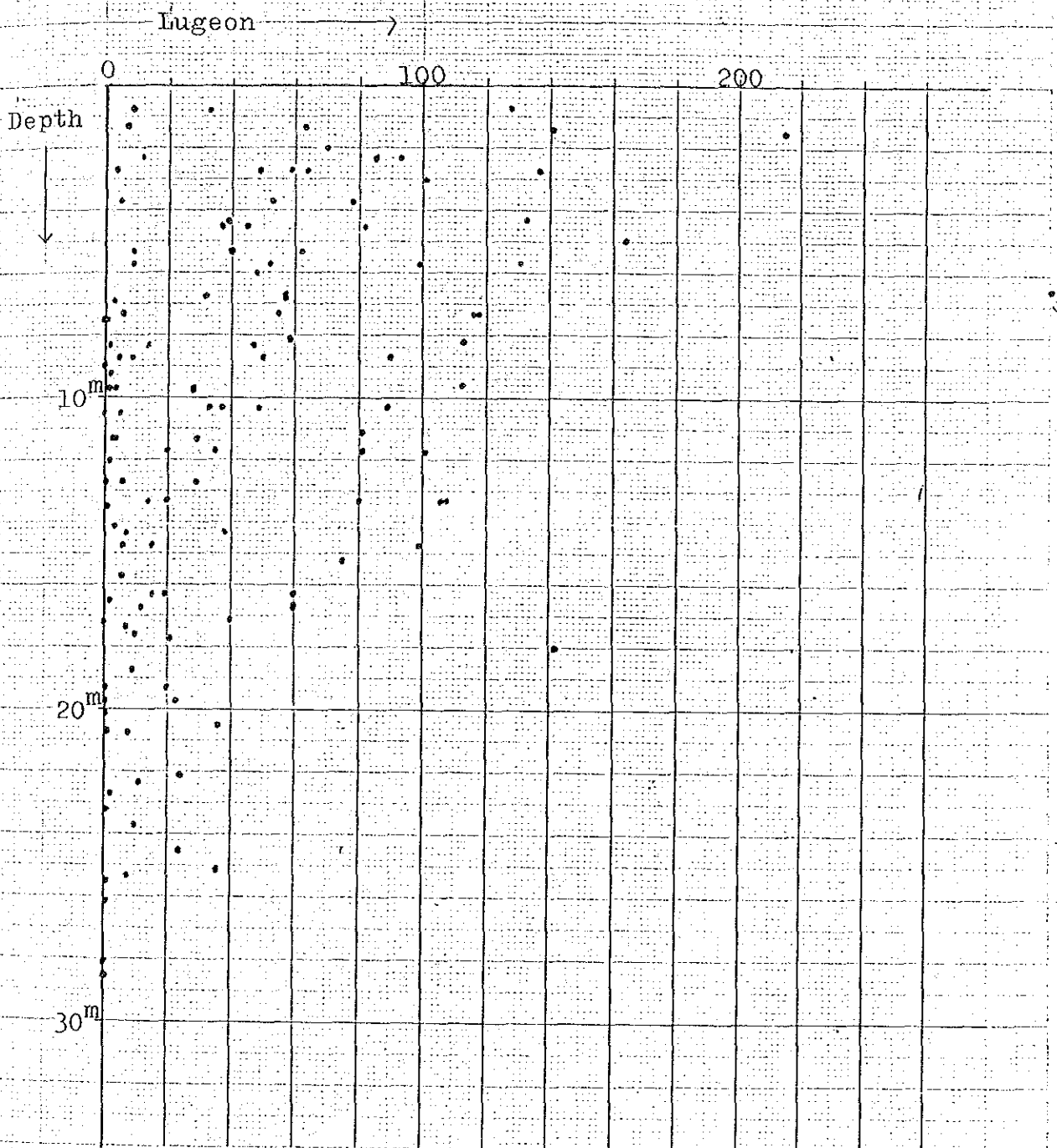
(STORAGE DAM A)



\* Depth (from rock surface to middle of Testing Length)

FIG. 4.4-6 THE RELATIONSHIP BETWEEN LUGEON AND DEPTH

(DIVERVERSION DAM C)

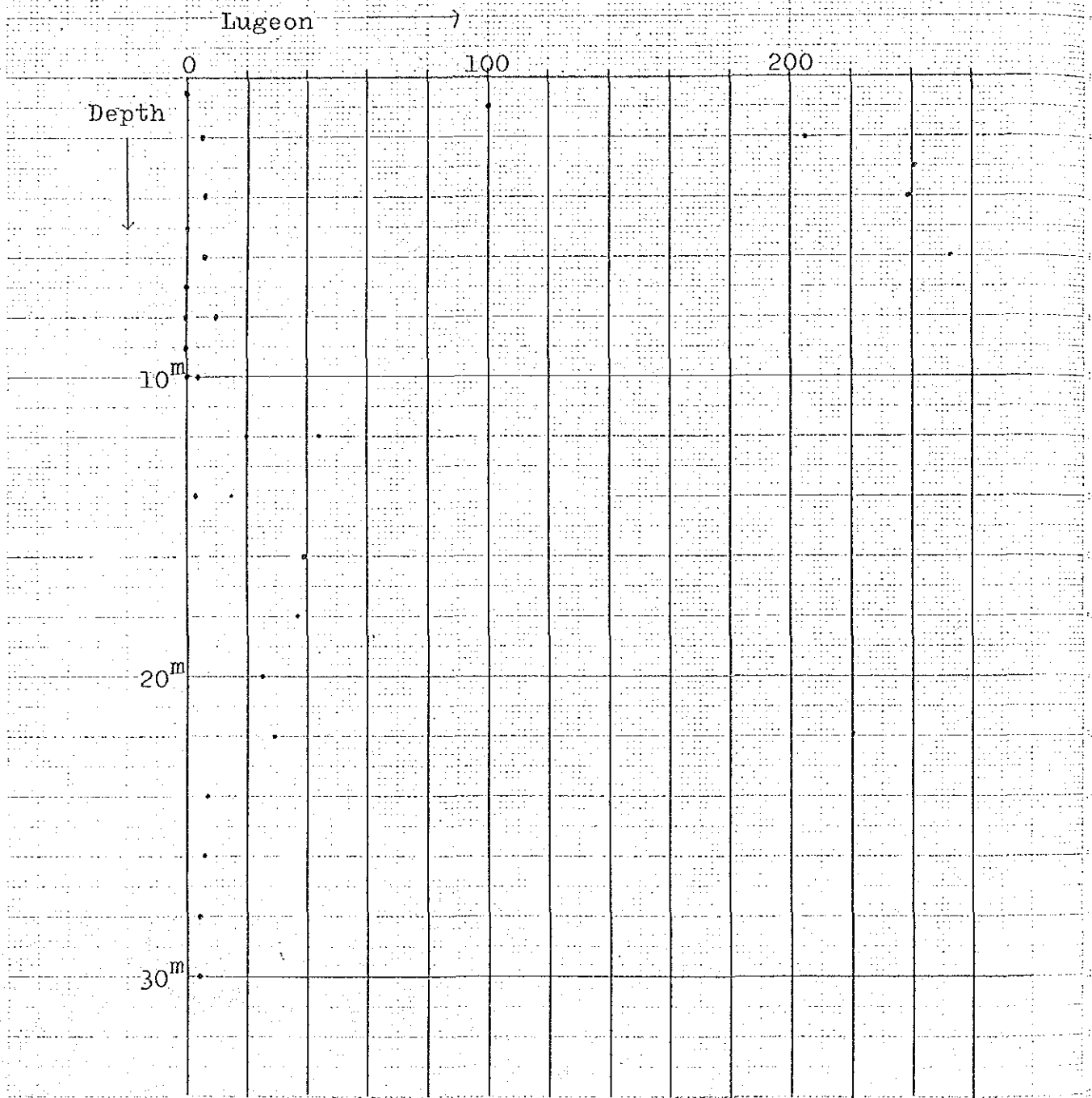


\* Depth (from rock surface to middle of Testing Length)



FIG. 4.4.7 THE RELATIONSHIP BETWEEN LUGEON AND DEPTH

(DIVERSION DAM D)



\* Depth (from rock surface to middle of Testing Length)

FIG. 4.4 - 8 Boring Log at Storage Dam A (1)

ROD = TOTAL CORE LENGTH (NO<sup>CM</sup> OVER) / DRILLING LENGTH

BITS: MC-METAL CROWN SDB-SURFACE DIAMOND BIT IDB-IMPREGNATED DIAMOND BIT

S-SINGLE  
W-DOUBLE

PROJECT Macchang Irrigation		HOLE NO. BA-1													
BEGUN by 25	FINISHED May 27 1983	HOLE DIAMETER 59.4 ~ 75.0mm	HOLE LOGGED I. Kimura												
GROUND ELEVATION 283.564	TOTAL DEPTH 30.00m		FOREMAN S. Itoh/glim												
ANGLE FROM VERTICAL 0°	WATER TABLE GL -18.20m														
DATE	DEPTH (m)	LOG	STRATA	CLASSIFICATION	COLOUR	CORE RECOVERY (%)	R. Q. B. (%)	MAX CORE LENGTH (cm)	HARDNESS	PERMEABILITY (cm/sec)	PHYSICAL CONDITION	BITS	DRILLING TIME (min)	SPRING WATER AND SECEGE WATER (l/min)	RUNNING WATER (l/min)
	0.00		Basalt	CM	dark grey						very hard rock. 3~10cm pitch crack. partly like a pebble cracking face is soiled by water.				
	6.00		Silty Sand		brown						medium sand with all hemiconsolidated. by water drilling many times cave in and no core. with a wide variety of sedimentary facies generally silty sand, interbedded with thin clay				
	19.00		Mudstone	CM	bluish grey						soft rock. by soft hammering easy broken. little weathered. some calcareous.				
	30.00		Sandy Mudstone	CH CM CM	greenish grey						19.30~23.80m good rock, but soft. by hammering easy broken. massive sandy mudstone. partly conglomeratic. 21.70m $\phi$ 0.5~1cm including pebble. 23.80~25.10m conglomerate. 25.10~30.00m hard rock but cracky. including $\phi$ 1~3cm sandstone. 17.00~30.00m increase sandy materials.				
											Bottom of Hole				

FIG. 4.4 - 9 Boring Log at Storage Dam A (2)

RQD =  $\frac{\text{TOTAL CORE LENGTH (NO. OVER)}}{\text{DRILLING LENGTH}}$

BITS: MC - METAL CROWN SDB - SURFACE DIAMOND BIT IDB - IMPREGNATED DIAMOND BIT

S = SINGLE  
W = DOUBLE

PROJECT		Macchang Irrigation		HOLE NO. BA - 2									
BEGUN	May 15	FINISHED	May 15	1983	HOLE DIAMETER	59.4 ~ 75.0mm	HOLE LOGGED	I. Kimura					
GROUND ELEVATION			260.691		TOTAL DEPTH		15.00m						
ANGLE FROM VERTICAL			0°		WATER TABLE		GL - - 1.40m						
DATE	DEPTH (m)	LOG	STRATA	CLASSIFICATION	COLOR	CORE RECOVERY (%)	R. O. D. (N)	PERMEABILITY (cm/sec)	PHYSICAL CONDITION	BITS	DRILLING TIME (min)	SPRING WATER AND FLOWAGE WATER (lit)	SPRING WATER (cm)
	1		Sand		brown				river deposits medium to coarse sand with some silt. very loose				
	5.00		Sand and gravel		gray - brown				river deposit 50.3 ~ 1cm small pebble sand and silt. loose.				
	7.90		Sandy Mudstone	CM	reddish brown				good condition, soft rock. by soft hammering easy broken. partly including thin fine sandstone.				
	9.15				bluish green				soft rock partly nocore by water drilling.				
	9.25												
	12.00												
	15.00								Bottom of Hole				

FIG. 4.4 - 10 Boring Log at Storage Dam A (3)

NO. TOTAL CORE LENGTH (100% OVER)  
DRILLING LENGTH

BITS: MC-METAL CROWN SDB-SURFACE DIAMOND BIT IDD-IMPREGNATED DIAMOND BIT

S-SINGLE  
W-DOUBLE

PROJECT		Machane Irrigation		HOLE NO. BA-3									
DEPTH	LOG	STRATA	CLASSIFICATION	COLOR	CORE RECOVERY (%)	R. O. B. (%)	MAX CORE LENGTH (cm)	HARDNESS	PETREABILITY (cm/sec)	PHYSICAL CONDITION	DRILLING TIME (min)	FLOWING WATER AND COLLAGE (cm)	FLOWING WATER
0		Basalt		dark gray						0~2.0m vesalt lava flow like a boulder weathered rock.			
2.00		Sand and Gravel with Clay		brown grey						2.00~14.00m semiconsolidated $\phi$ 1~3cm pebble 20% matrix silty sand. very dense			
3.00		Silty Sand		gray						very dense			
7.60		Silt		dark grey									
8.50		Silty Sand		pale brown						8.50~14.00m very stiff. like a soft rock. fine to medium sand with silt.			
11.00										11.00~14.00m including pebble $\phi$ 1~3cm			
14.00		Sandy Mudstone	CL	reddish bluish green						14.00~19.70m soft rock.			
16.70		Sandy Mudstone	CM	reddish brown grey						14.00~16.00m heavy weathered soft. 16.00~18.70m fresh, partly core loss.			
24.00		Sand Stone	CM	gray						13.70~24.00m silty~fine sandstone. almost no core by water drilling. soft rock.			
26.00		Congio-merate	CM	reddish brown						including pebble $\phi$ 1~3cm partly muddy			
28.00		Sandy Mudstone	CM	reddish brown						massive			
										Bottom of Hole			

FIG. 4.4 - 11 Boring Log at Diversion Dam C (1)

HOLE NO. DHN.1

HOLE NO. DHN.2

GROUND ELEVATION +250.087

GROUND ELEVATION +250.059

WATER TABLE GL -9.80

WATER TABLE GL -9.00

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	
1		clay	0.00 - 0.90 M. CL (Lean Clay) about 90% med-high plasticity fines, 10% fine sand, dark brown, moist.		1		clay	0.00 - 2.30 M. CL (Lean Clay) about 80% med-high plasticity fines, 20% fine sand, reddish brown, dry.		
2		massive limestone	0.90 - 1.26 M. slightly-mod. weathered, grey, dense, <u>Limestone</u>		2		massive limestone		N = 28	
3			- max. core recovery 10 CMS.		3			2.30 - 15.00 M. mod. weathered dark gray, dense		
4			1.26 - 1.40 M. CL (Lean Clay) about 90% med-highly plasticity fines, 10% fine sand, reddish brown, dry.	Lu = 4	4			<u>Limestone</u>		
5					5			- max. core recovery 41 CMS.		
6			1.40 - 14.90 M. mod. weathered, dark gray, dense, <u>Limestone</u>	Lu = 9	6			- fracture dips 80°, 60°, 40°.	Lu = 101	
7			- max. core recovery 68 CMS.		7			- broken into pieces at 13.20 - 13.50 M. and 14.00 - 14.45 M.	Lu = 37	
8			- fracture dips 60°, 30° vertical, with limonite along fracture.	Lu = 6	8			- Limonite along fracture and holes.	Lu = 48	
9			- calcite veinlets rarely but rather common from 13.40 - 14.90 M	Lu = 5	9			- Calcite veinlets rather common.	Lu = 0	
10					10				Lu = 0	
11				Lu = 89	11				Lu = 0	
12				Lu = 81	12				Lu = 0	
13				Lu = 80	13				Lu = 0	
14					14				Lu = 2	
				Bottom of hole		15			Bottom of hole	

FIG. 4.4 - 12 Boring Log at Diversion Dam C (2)

HOLE NO. DHN.3

GROUND ELEVATION +250.897

WATER TABLE GL -10.30

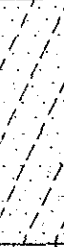

DEPTH	LOG	GEO-LOGGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
1		clayey sand	0.00 - 3.30 M. SC - CL (Clayey sand) about 55% med. high plasticity fine, 45% fine sand, brown, dry.	
2				N = 46
3				N = 42
4		massive limestone	3.30 - 15.15 M. slightly weathered, gray and pinkish gray,  <u>Limestone</u> - max. core recovery 105 CMS.  - fracture dips 70°, 30° with limonite along fractures.	Lu = 9
5				
6				Lu = 93
7				Lu = 78
8				
9				Lu = 62
10				Lu = 57
11				Lu = 47
12				
13				Lu = 3
14				
15		Bottom of hole	Lu = 3	

FIG. 4.4 - 13 Boring Log at Diversion Dam C (3)

HOLE NO. DHN.4

GROUND ELEVATION +250.334

WATER TABLE GL -9.80

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE			
1		clay	0.00 - 3.30 M. CL(Lean Clay) Approx. 90% high plasticity fines, 10% or less than, very fine sand, reddish brown, moist.		20				Lu = 19			
2				N = 26	21				Lu = 21			
3				N = 15	22				Lu = 20			
4					23				Lu = 8			
5		massive limestone	3.30 - 4.00 M. CL(Sandy Clay) Approx. 75% med-high fines, 25% fine sand reddish-brown moist.  4.00 - 30.00 M. mod. weathered, gray-dark gray, <u>Limestone</u> - max. core recovery 39 CMS, mostly broken into pieces. - fractures dip 40°, 70° with limonite and Calcareous clay along fractures. - Calcite veinlets common.	Lu = 7	24							
6				Lu = 49	25				Lu = 11			
7				Lu = 39	26				Lu = 10			
8				Lu = 52	27				Bottom of hole			
9				Lu = 55	28							
10				Lu = 50	29							
11				Lu = 33	30							
12				Lu = 20								
13				Lu = 20								
14				Lu = 6								
15												
16												
17												
18												
19												
20												

FIG. 4.4 - 14 Boring Log at Diversion Dam C (4)

HOLE NO. DHN.5

GROUND ELEVATION +250.493

WATER TABLE GL -10.0

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
1	//	clay	0.00 - 3.30 M. CL (Lean Clay) Approx. 85% Medium plasticity fines, 15% fine	
2			- very fine sand, brown-dark brown, dry, some roots.	N = 27
3				N = 29
4				N = 37
5	[Brick Pattern]	massive limestone	3.30 - 5.00 M. CL (Sandy Clay) Approx. 75% medium fines, 25% fine sand, reddish brown, dry.	
6				Lu = 128
7			5.00 - 25.00 M. slightly-mod. weathered, gray-dark gray.	Lu = 85
8			<u>Limestone</u>	
9			- max. core recovery 24 CMS., most of core are broken into pieces.	Lu = 53
10			- fractures dips 45°, 70° nearly vertical, with limonite and calcareous clay along fractures.	Lu = 40
11			- some calcite veinlets.	
12				Lu = 57
13				Lu = 2
14				Lu = 2
15				Lu = 2
16				Lu = 4
17				Lu = 6
18				Lu = 7
19				
20				

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
21	[Brick Pattern]			Lu = 6
22	[Brick Pattern]			Lu = 7
23	[Brick Pattern]			
24	[Brick Pattern]			Lu = 9
25	[Brick Pattern]		Bottom of hole	



HOLE NO. DHN.6

GROUND ELEVATION +252.025

WATER TABLE GL -11.80

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	
1	[diagonal lines]	silty sand	0.00 - 4.30 M. SM-ML (silty sand) Approx. 55% low plasticity fines, 45% poorly graded fine sand, reddish brown dry.		[brick pattern]					
2				N = 20					21	Lu = 15
3				N = 18					22	Lu = -
4				N = 22					23	Lu = -
5	[diagonal lines]	clay	4.30 - 5.00 M. CL (Sandy Clay) Approx. 70% medium plasticity fines, 30% fine sand, reddish brown, dry.		24			Bottom of hole	Lu = 23	
6	[brick pattern]	massive limestone	5.00 - 25.00 M. slightly-moderately weathered, gray-dark gray  <u>Limestone</u> - max. core recovery, 35 CMS, core are much broken from 10.00 - 12.00 M. 13.00 - 16.10 M. 17.13 - 25.00 M.  - fracture dip 70°, 45°, 20°, with limonite along fractures some calcite veinlets.	Lu = 63	25					
7										
8				Lu = 137						
9				Lu = 133						
10										
11				Lu = 99						
12				Lu = 116						
13										
14				Lu = 90						
15										
16				Lu = 49						
17				Lu = 35						
18	Lu = 14									
19										
20	Lu = 15									



FIG. 4.4 -17 Boring Log at Diversion Dam C (7)

HOLE NO. DH.8

HOLE NO. DH.9

GROUND ELEVATION +252.230

GROUND ELEVATION +250.548

WATER TABLE GL -11.90

WATER TABLE GL -8.80

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
1		silty sand	0.00 - 4.65 M. SM-ML (Silty Sand) About 65 - 75% poor graded fine sand, 30 - 35% low plasticity fines, brown, dry, some roots.	N = 29	1		massive limestone	0.00 - 0.85 M. Slightly-mod. weathered, dark gray, dense.	
2					Limestone (Block insitu)			Lu = 12	
3					0.85 - 1.00 M. SC-CL (Clayey Sand) About 55% fine sand, 45% low-med. plasticity fines, dark brown, moist.			Lu = 5	
4								Lu = 9	
5	massive limestone	4.65 - 12.00 M. <sup>s<sub>ss</sub></sup> Slightly-mod, weathered, pinkish gray, dense  Limestone - max. core recovery 44 CMS.  fracture dips 60°, 45° with limonite along fracture.  - Broken into pieces 9.45 - 10.40 M and 11.20 - 11.80 M.	Lu = 113	5	1.00 - 12.00 M. Slightly-mod. weathered, dark gray, dense			Lu = 3	
6				Limestone	Lu = 14				
7				- max. core recovery 32 CMS.	Lu = 28				
8				- fracture dips 70°, 20°, 70°, 10° with limonite along fracture.	Lu = 29				
9				- Calcite veinlets are rather common.	Lu = 29				
10				- Broken into pieces from 3.20 - 4.05 M.	Lu = 38				
11					Lu = 75				
12					Lu = 60				
13									
14									
15									
16									
17									
18					Bottom of hole			Lu = 142	



FIG. 4.4 - 19 Boring Log at Diversion Dam C (9)

HOLE NO. P.2

GROUND ELEVATION +250.65

WATER TABLE GL -9.80

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
1			0.00 - 1.50 M. No Sample		21				
2	massive limestone		1.50 - 5.17 M. Slightly weathered-fresh gray-dark gray, massive limestone.		22				Lu=1.2
3					23				
4			- max. core recovery 65 CM, core are normally broken along fracture.		24			25.10 - 31.50 M. Moderately weathered-fresh, gray-dark gray, massive limestone.	Lu=2.6
5			- fracture space 2 - 10 CM, fracture dip 5°, 30° and 85° with limonite and some secondary calcite along fracture.	Lu=82.1	25				
6			- some calcite veinlet.		26			- max. core recovery 28 CM, core are broken along fracture.	Lu=1.3
7					27				
8					28			- fracture space 5 - 10 CM, fracture dip 0°, 5°, 30°, 45° with limonite and some secondary calcite along fracture.	Lu=0.5
9			5.17 - 25.10 M. Fresh, dark gray, dense massive limestone.	Lu= 0.4	29				
10			- good core recovery, max. core 68 CM.		30				
11			- fracture space 1 - 2 CM fracture dip 5°, 30°, 45°, and 80° with some limonite along fracture.	Lu = 5.4	31			Bottom of hole	
12			- calcite veinlet rather common.						
13									
14									
15					Lu= 1.3				
16									
17									
18					Lu= 1.9				
19									
20									



FIG. 4.4 - 21 Boring Log at Diversion Dam C (11)

HOLE NO. P.4

GROUND ELEVATION +250.65

WATER TABLE GL -6.40




DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE	DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE						
1		massive limestone	0.00 - 0.59 M. No Sample		21			17.70 - 30.25 M. Moderately weathered, dark gray, dense limestone.							
2			0.59 - 4.44 M. Slightly weathered-fresh dark gray, massive limestone.	Lu=59.3	22					- max. core recovery 56 CM, core are broken into pieces at 18.00 - 18.25 M, 21.15 - 21.20 M, and 26.65 - 23.10 M.	Lu=24.4				
3			- max. core recovery 50 CM, mostly broken along fracture.	Lu=164.2	23						- fracture space 2 - 7 CM, fracture dip 15°, 45°, 60° and 80° with limonite along fracture and some calcite veinlet.	Lu=36.2			
4					- fracture space 1 - 3 CM, fracture dip 15°, 30° and vertical with limonite along fracture.							Lu=2.2	24	- some quartz vein with 3 CM max. size at 20.75 M.	Lu=0.3
5													- some calcite veinlet.		
6			- brecciated zone, max. size of breccia 5 CM at 2.00 - 2.90 M.	Lu=9.6	26										
7					4.47 - 17.70 M. Fresh, dark gray, dense massive limestone.						Lu=108.0	27			
8			- good core recovery with max. core 80 CM.	Lu=9.6								28			
9					- fracture space 1 - 7 CM, fracture dip 15°, 30°, 60° and 80° with limonite along fracture.						Lu=9.6	29			
10			- some calcite veinlet.	Lu=9.6								30			
11					- fault breccia with 5 CM max. size at 6.77 - 7.00 M.	Lu=9.6									
12															
13															
14															
15															
16															
17															
18															
19															
20															

FIG. 4.4 - 22 Boring Log at Diversion Dam C (12)

HOLE NO. P.25

HOLE NO. S.1

GROUND ELEVATION +250.65

GROUND ELEVATION +250.65

WATER TABLE

WATER TABLE

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
			0.00 - 0.80 M. No Sample	
1		massive limestone	0.86 - 6.00 M. Slightly weathered-fresh, gray-dark gray, dense massive limestone.  - max. core recovery 45 CM  - fracture space 4 - 8 CM. fracture dip 15°, 45°, 60° and 80° with limonite along fracture.  - calcite veinlet rather common.	Lu=83.3
2				
3				
4				
5				
6				
			Bottom of hole	

DEPTH	LOG	GEO-LOGY	CLASSIFICATION AND PHYSICAL CONDITION	TESTING VALUE
			0.00 - 1.60 M. No Sample	
1		massive limestone	1.60 - 4.60 M. Slightly weathered-fresh, gray-dark gray, massive limestone.  - max. core recovery 70 CM.  - fracture space 3 - 5 CM fracture dip 15°, 30°, 45° and 80° with limonite along fracture.  - some calcite veinlet.	Lu=215.3
2				
3				
4				
			Bottom of hole	







