6. Engineering Geological Assessment

6-1. Introduction

The subject of this chapter is an engineeirng geological assessment as a dam foundation to the general geological conditions of each site described in previous chapter and a guide for a foundation treatment.

6-2. Left Saddle Dam

The Left Saddle damsite has the most troublesome foundation geology among three damsites in the project site because an extension of the great tectonic line which controls a geography of the region is passing through the damsite.

The extension is not only one big fault but a fault complex composed of many faults and sheared zones. A trending of the fault complex is nearly NW-SE direction and dip is almost vertical as a total (each fault consisting the fault complex has varying dips and strikes). The tectonic line is presumed to turn almost vertically to southwest at downstream of the site and pass through a downstream area of the Main dam.

Nevertheless the site was crossed by the fault complex the bedrock (each block of the bedrock in exactly) is rather hard because the elastic wave velocity of the one is ranging from 3.2 to 3.5 km/sec except a slate having a heavy schistosity. These hard bedrock has been cut into blocks by numerous faults and altered into fragile phase or clay by hydrothermal alteration. Thus the bedrock of the site degenerates into inhomogeneous and unstable condition, geologically and physically. And such a condition is never suitable for a dam foundation.

Farthermore, these heavily altered or sheared zones occupying considerable parts of the foundation have a low permeability and also low critical pressure simultaneously. The critical pressures of these zones are mainly ranging from 2 to 5 kg/cm² and the full water head of proposed reservoir is about 5 kg/cm². This means a dangerous situation for a dam foundation that the critical pressure of considerable part of the foundation will be less than the water head of the reservoir if it will have been filled up.

In usually a critical pressure got by a permeability test has a closed relation with a critical point pressure or a strength of the rock. Although the strength of the bedrock got by the jack test and result of the reconnaissance indicate the bedrock generally has an almost enough bearing capacity for an earth fill dam, but they shows simultaneously a considerable part of the foundation has nearly same or less critical point pressure to the load by embanking. From these consideration a base of the dam should have a wide core trench to avoid a stress-concentration by embankment and be treated enough to improve and consolidate a strength and a permeability of it using a consolidation grout (or any other method capable).

The permeability of the bedrock is ranging widely from less than 1 to more than 100 lugeons as mentioned above, and a zone having a high permeability coefficient shows extreme volume of grout-intake more than one ton per meter of grout hole. From the situation a curtain grouting is not only indispensable but also required to conduct by grout rows as many as possible. These many curtain grout rows should be grouted orderly from outer rows to inner rows to form a tight impervious curtain and these grouting should contribute also to consolidate uniformly the bedrock.

The core trench should be excavated to contact a core zone with a base-rock, at least up to the base-velocity-layer (V>2.7 km/sec) drived from the seismic exploration. And concerning to the width of the core trench, the wider it is the better from a view of the foundation treatment. On the other hand, the bedrock has a tendency to be weathered rapidly by exposing to the air because the most of bedrock has been altered heavily or slightly. It means an embanking work should be

required to conduct rapidly after an excavation work has completed.

An excavation of a top-soil throughout a base of the dam should be need but the required depth of excavation is only one or two meters because the overburden at the site is generally well compacted.

The most sirious geological risk at the site is that heavily altered or sheared zones have a low permeability and a low critical pressure simultaneously. As a countermeasure for the risk a consolidation grout in the core trench is proposed from a view of availability and economization, but the availability (or efficiency) must be confirmed actually soon.

At the site, a faint sense of uneasiness for a stability of the dam foundation shall be remained even if it has been treated by excavation of wide core trench, consolidation grout and thick curtain grout (it is mainly derived from 'a behavior of the tectonic line at when an eathquake has happened). Then, to set a relief wells at a downstream toe of the dam and to observe continuously a leakage water through the dam foundation should be proposed.

Farthermore, to set a grout gallery should be considered in the near future from a view points of a conduction term for huge volume of a grouting works, an additional grouting work after completion of dam embanking and a laying or a control of various observation appliances for dam.

On the basis of these assessment and consideration a scheme for foundation treatment enumerated below has been led.

- Excavation of a foundation i. wide and deep core trench

ii. excavation of topsoil throughout the dam foundation

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- Consolidation grout ----- throughout the core trench
- Curtain grout ----- as many lows as possible

-	Relief wells	 continuous observation of water level and quality
	Others	 consideration of grout gallery

6-3. Main Damsite

The bedrock of the site consists of sandstone (and or sandy quartzite) and a few thin shale beds intercalated. The elastic wave velocity of the bedrock is high in the riverbed and rather low (V \neq 2.6 km/sec) in the both abutments. This situation seems to be derived from large scaled faults passing through a beneath of both abutments and they subjected mainly to a mountain side. It means the foundation at riverbed has rather good geological conditions than the one at both abutments in contrast with general damsites. The situation is favorable for a construction at riverbed area where occupies the most of total embanking volume and will be subjected by the maximum load by embankment, but on the other hand, the fact that both ends of riverbed which means a turning point of a bottom of the core zone have a weak zone and the rocks at both abutments where the core clings inclinedly to it are considerably cracked or loosened requires an utmost cares for construction. Especially the faults passing through a beneath of both abutments should be researched more exactly on a scale, a grade of shearing, a permeability, bearing capacity and so on, and be executed a scrupulous fault treatment based on the result.

A part of the bedrock (sound and massive sandstone) shows a low permeability of few lugeons but the other most part (cracked sandstone) shows a permeability ranging from 10 to 40 lugeons and more than 100 lugeons locally. The situation suggests a leakage of water through the dam foundation is rather heavy and an impervious treatment (usually by curtain grout) shall be necessary. In the case of using a curtain grout method, the grout holes should be drilled up to the depths because the bedrock has no tendency to decrease a permeability according to the depth. And, the limb grout at the right abutment should be connected with the grout curtain of the spillway.

At the Main damsite, a river deposit distrubutes extensively and deeply, and at some part the depth of the deposit exceeds 20 meters. The river deposit consists of mainly sand and they are generally loose, especially an upper part of it is very loose (2 - 5 of N-value). Farther, the groundwater table at the site is extremely high. These loose sediments should not be allowed to remain under the embankment, not only beneath a main part but even under the toes of dam. It means the upper loose part of the river deposit should be excavated out throughout the dam foundation. Farthermore, all of the river deposit under the core zone should be cut off completely and the core should be holded directly by the bedrock from a view points of a permeability and a bearing capacity. These consideration leads inevitably to a necessity of huge amount of excavation volume, therefore a diverting use of the deposit for embanking material should be studied in the near future.

Beside, a shutting off of the main flow and any kinds of a groundwater reduction method should be conducted throughout the dam foundation.

In the case that any structure like as a river outlet should be set under the dam, the spot at a base of the right abutment (ST.No: 10 + 30 - No: 11) should be adopted from a view of the depth of overburden and the rock quality.

At any rate, the site has the most favorable geological condition for a dam foundation among three damsites even though it has a deep river deposit and two large faults at both ends of the riverbed. The foundation treatments required at the site are follows from the consideration or assessment described above.

- Excavation of a foundation i. cut off trench up to a bedrock at core zone
 - ii. excavation of upper loose layer throughout dam foundation

- Curtain grout
- Fault treatments ----- two zones

6-4. Right Saddle Damsite

The site has the most big Height-Span Ratio because of the gentle slopes of both abutments and plain riverbed lying in high elevaiton. Almost all of the bedrock is composed of sandstone and rarely intercalating thin shale beds.

The most severe problem for a foundation of the site from a view point of engineering geology is existing of a fault complex composed by numerous faults in between from a slightly right bank side of the riverbed to a foot of the right abutment. Though the complex is rather small scaled in comparison with the one passing through the Left Saddle damsite but the width of it is about 200 meters and through the spread the bedrock has been cut into blocks by numerous faults and altered into friable phase by hydrothermal alteration. And, at the spread bedrock had been eroded deeply compared with around area, and it means the depth of overburden is large (about 15 m in average).

At the part where the fault complex passes through a core trench should be cut off up to the base-velocity-layer (by seismic exploration) even if it needs a large excavation volume. Farthermore the core trench at the part should be cut widely off and treated by a consolidation grout throughout its spread or by curtain and blanket grout rows covering its width because the foundation of the part have anxieties concerning to a permeability and a bearing capacity.

At other part of the site the core trench should be excavated up to the III velocity layer which means a weathered rock zone. The topsoil excavation should be conducted throughout the dam foundation in about one meter depth.

The characteristically thick weathered zone distributes in the damsite except a part passed through by the fault complex. These thick weathered zone is presumed to be derived from the complex but it indicate an elastic wave velocity ranging from 1.8 to 2.2 km/sec and has no problem for a bearing capacity. Then it should be treated only from a view point of permeability.

In addition to that, two small scaled low-velocity-zones were discovered besides of the fault complex by seismic exploration. These have also need to be confirmed their properties but at the present time they are supposed not to be so serious problem because they are mainly recognized only at the base-velocity-layer and not obvious at the III velocity layer.

From these assessment described above, the following items for a foundation treatment are proposed.

- Excavation of a foundation	' i. core trench (* up to the base-velocity lay- er at the fault complex.zone
	* up to the II velocity layer throughout the dam axis
	ii. excavation of topsoil throughout the dam foundation

- Curtain grout

- Consolidation grout or wide grout curtain at the fault complex zone

- Fault treatment ——— There's a possibility to be canceled according to the following investigaiton.

6-5. Spillway

The ridge of an isolated small mountain at right bank of the Main dam, where is proposed spillway site, consists of mainly sandstone (or sandy quartzite). The depth of overburden is generally thin but at both slopes of the ridge a talus deposit distributes with the depth of 7, 8 meters. The bedrock is devided into two phases, weathered one and sound one (in precisely speaking there is one more phase but it distributes in far depths). The weathered sandstone is heavily cracked phase having an elastic wave velocity ranging from 1.5 to 1.8 km/sec and distributes thickly (15 - 20 meters). The sound sandstone underlying the weathered zone is considerably hard phase and indicates an elastic wave velocity about 3.3 km/sec. The situation of thick weathered phase is presumed to be affected by several faults running parallel to a trending of the ridge.

A permeability of the bedrock is presumed to be about 10 lugeons or more actually from the consideration to their elastic wave velocity and other data, nevertheless the previous investigation result showed the permeability ranging from 1 to 10 lugeons. Therefore, also the spillway site should have a grout curtain connected with the one of the Main Dam to prevent a leakage of water through the foundation and not to get an uplift pressure to the structures of spillway.

The overburden at the downstream side slope of the ridge where means a base of the chute has a depth only ranging from 6 to 8 meters, so the structure should be based on the rock (weathered rock). Farther the chute should have a lining and certainly any foundation treatment to combine tightly the structure with the bedrock like as a rock bolt should be need because the chute is presumed to cross four or five fault zones and the rock phase is weathered and cracked.

At the flood plane where the stilling basin has been set very thick river deposit, new and old, is distributed. The depth is about 10 meters beneath the slope and farther becomes more deep according to the distance from the slope. The deposit consists of mainly sand and sandgravels, and although its lower part is rather compacted but its upper part and a part nearby the groundwater table are very loose. It means that the important structure like as a stilling basin should be based on the rock even though an excavation volume becomes so much.

The excavated materials owing to a construction of the spillway consist of mainly weathered sandstone because an overburden is not so thick generally. The rock quality of the weathered zone is presumed to be hard in each block nevertheless the zone is heavily cracked and altered locally by hydrothermal alteration. Then, these materials are certainly able to divert as an embanking materials (rock materials) and

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as a riprap materials partly.

From the assessment and consideration, the following scheme for a foundation treatment is proposed.

- Curtain grout ——— connected from the Main damsite

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- Rock bolt ----- at the base of chute structure

7. Recommendation

7-1. Subjects to be Solved

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a) Left Saddle Damsite

i. Total four lines of seismic exploration surveys should be conducted to grasp a distribution of main sheared zones or altered zone in the fault complex and to draw up a contour map of the rock surface throughout the dam foundation. (two lines parallel to the damaxis and two lines crossing the axis vertically)

ii. About two drilling holes should be drilled at both abutments to research a strength and a permeability of the II velocity layer to conclude to be cut off or not.

iii. A large scaled grouting test according to a pattern of the design should be conducted to confirm the effectiveness of a grout method.

iv. Several rock shearing tests should be conducted at adits excavated in the bottom of present core trench to grasp a cohesion and an internal friction angle actually.

b) Main Damsite

i. Total four lines of seismic exploration surveys should be conducted to draw up a contour line map of a rock surface to judge an excavation line throughout the dam foundation. (two lines parallel to the damaxis two lines crossing the axis vertically)

ii. Two or three drilling holes at both abutments should be drilled to conclude to be cut off or not concerning to the II velocity layer and to grasp a quality of the III-1 layer (V = 2.6 km/sec).

iii. Researching works for two large faults passing through the both ends of the riverbed should be done to draw up a treating plan.

iv. Grouting test should be conducted to confirm an effectivity and to conclude an optimum grout pattern effectively and economically.

c) Right Saddle Damsite

i. Four lines of seismic exploration surveys should be conducted to grasp an extend of the fault complex and to draw up the contour line map of the rock surface throughout the dam foundation. (two lines parallel to the damaxis, two lines crossing the axis)

ii. Two or three holes of drilling should be conducted at right side of the riverbed on the damaxis to grasp a property of the fault complex and to conclude to be cut off or not for the II-2 velocity layer.

iii. Fault investigation drilling should be carried out at left abutment and left side of the riverbed to grasp a property of them and to study for a necessicy and treatment methods.

d) Spillway

i. Two lines of seismic exploration surveys should be conducted to confirm a distribution of the rock surface at spillway site.(a line along the center line and a line crossing the center line)

ii. Two drilling holes should be drilled at the point of the stilling basin proposed to grasp a foundation condition exactly.

iii. Some drillings should be conducted at the ridge to search a rock quality and foundation condition for a grouting plan and investigation for an embanking materials.

e) Others

i. Throughout the all damsites, more detailed field survey should be conducted to draw up a topographic maps enough exact to produce a working plan (including the present trenches).

7-2. Procedure, Equipments and Notes

a) Investigation Drilling

i. An overburden should be drilled by all coreing method.



ii. Weak or soft layers intercalated like as heavily weathered rock, sheared zone and fault-clay (gouge) should be recovered using a so-called "non-fluid method" immediate after it occured.

iii. The operators of drilling should study and be trained for several technicalities concerning to a protection of drilling hole and core recovery (like as casing, cementation, water-supply, drilling-pressure and speed, non-fluid drilling and so on) by experts.

 $_{1v}$. The drilling cores should be stored in a core-box immediately after recovered from a hole and be kept on during the construction term. The core box should provide a capacity of 5 meters in each (five rows with one meter length) and a cover.

b) Permeability Test

i. A water-pump capable for more than 100 litters per minute of delivery, an accurate flow-meter and pressure gage should be provided for the pressure permeability test.

ii. At least three kinds of packer or packing method should be provided (like as an air packer, an expansion packer, a cement packer etc.) physically and technically.

iii. The test should be conducted using a double (up and down) stepped pressure injection method and the result should be arranged as a "pressure-intake curve" on a logarithms section graph. The testing time (keeping on time in a certain pressure step) of each step should be more than 15 minutes at least.

c) Jack Test

i. A jack with separated oil-pump should be provided for easily measurement in a restricted testing space.

ii. A dialgage with 1/100 milimeter graduations and a leg more than 50 milimeters should be provided.

iii. A loading plate should be made by steel and be enough thickness (at least more than 25 milimeters).

d) Seismic. Exploration

i. Upmost care should be taken for a control and management of the explosives. The consumption diary must be taken severely.

ii. For the safety of the work, every engineer and worker should put on a helmet and tough shoes.

iii. To keep the safe explosion, 2 - 3 watchmen should be arranged with enough distance from a blusting point and keep villagers out from a dangerous area.

iv. All shooters must be qualified and having enough experience.

v. All engineers should take cares for the safety blusting not to harm villagers, houses, buildings or constructions, cattles or domestic animals etc.

e) Equipments

The equipments which should be provided for investigation work or tests for a foundation of dams or canals in the near future are listed up below.

i.	Lateral loading tester abl	le to test a strength of rock
	or	sheared zone easily using a
	dri	illing hole.

ii. Probe set for a velocity logging — to velocity log in a drilling hole and for a measurement of interholes elastic wave velocity using a "PS-10" already provided. (Available for judging a grout effect)

iii. Coreing tools for 65 m/m ---- the equipments mentioned above are for 65 m/m diameters. (double tube core barrel, single tube core barrel, bitts etc.)

iv. Water quality tester —— able to measure a temperature and electric conductivity of water in the drilling hole or in groundwater.

Large oil jack (about 100 tons) — for jack test, especially for a rock shearing test (2 sets). vi. Others —— equipments or tools mentioned in this or previous chapters.

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8. Data and Records (sub-contents)

- Figure 8-1 Geological Loggs of Drilling Holes
- Fiugre 8-2 Analysing Charts of Permeability Test
- Figure 8-3 Records of Lateral Loading Test
- Table 8-4 References

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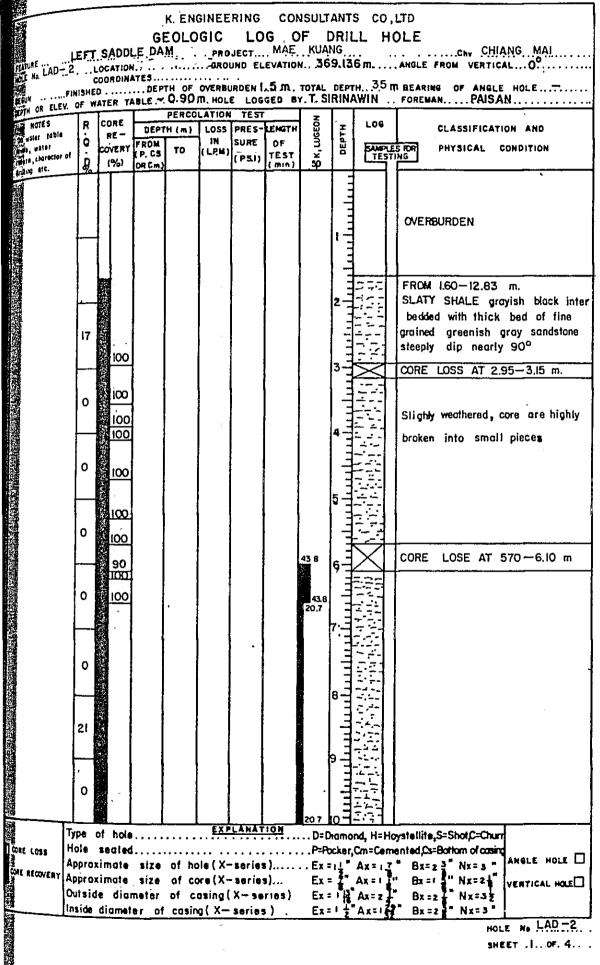
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8-1 Geological Loggs of Drilling Hole

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SHEET .1. OF. 4. .

FEATURE LEF HOLE No LAD. 2 BEGUN	2L C	OCATION	L	гн оғ 0,90	OVERBL	UND EL JRDEN . E LOG	.EVATIO 1.5m GED_B	4	369.	136 m าม 35 m	REAR	гл: ING	OF ANGLE	NOLE
NOTES On water table levels, water return, charector of	R Q	CORE RE-	DEPT	H (m)		PRES-	LENGTH OF TEST	8k,Lugeon	DEPTH	LOG SAMPL TES	ES IOR	**	ASSIFICAT	
drilling etc	18	{%}	OR Cm.}		 		<u>(min)</u>	207	╂╼╡				-	
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	0	100 100		-				20.7						
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	\vdash	100							16-	- > -	\dip			-
	24	<u>00</u>						291	11111	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	SANI grain	DST(ed	tresh, fra	om. fine to: cture dip h colcite v
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-		of hole				LANA						Å	=Shot/C=Chu iottom of canii	
	Appro	ximate	, size	⇒ot`h	ole (X	-serie	s)	Ex			∑* 8x	= z ;	Nx=a "	ANGLE H
CORE RECOVERY										Ax = 1	I" Bx	= ; }	" Nx=z-j"	VERTICAL
		de dia diame		-	_					Ax=z Ax=1			Nx=32	
		÷ ~				-		•			· ·		но	LE No LA
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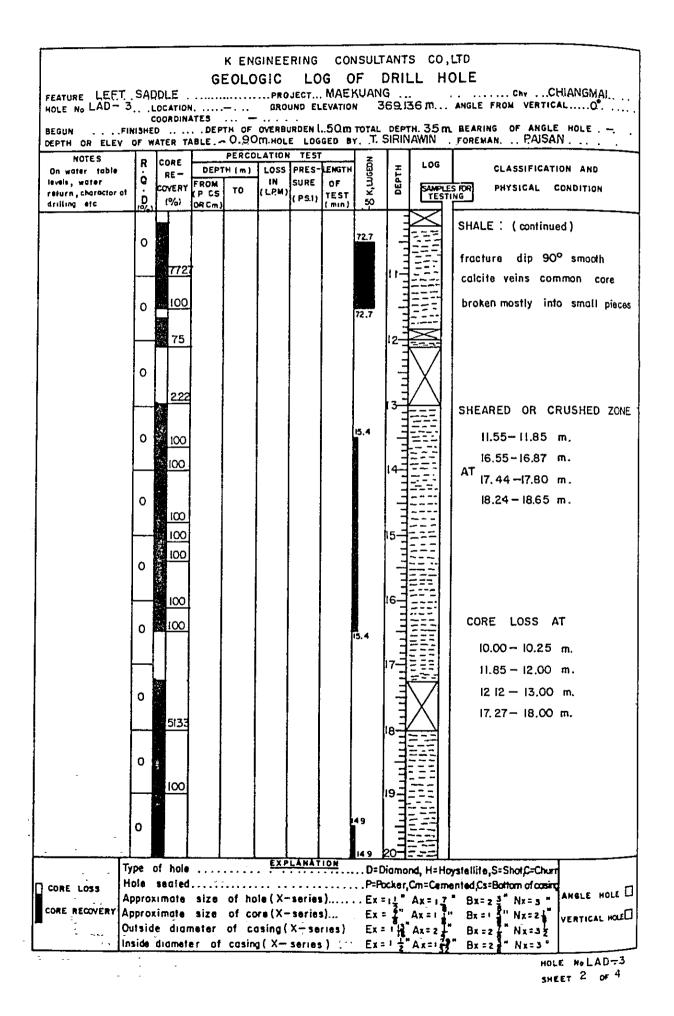
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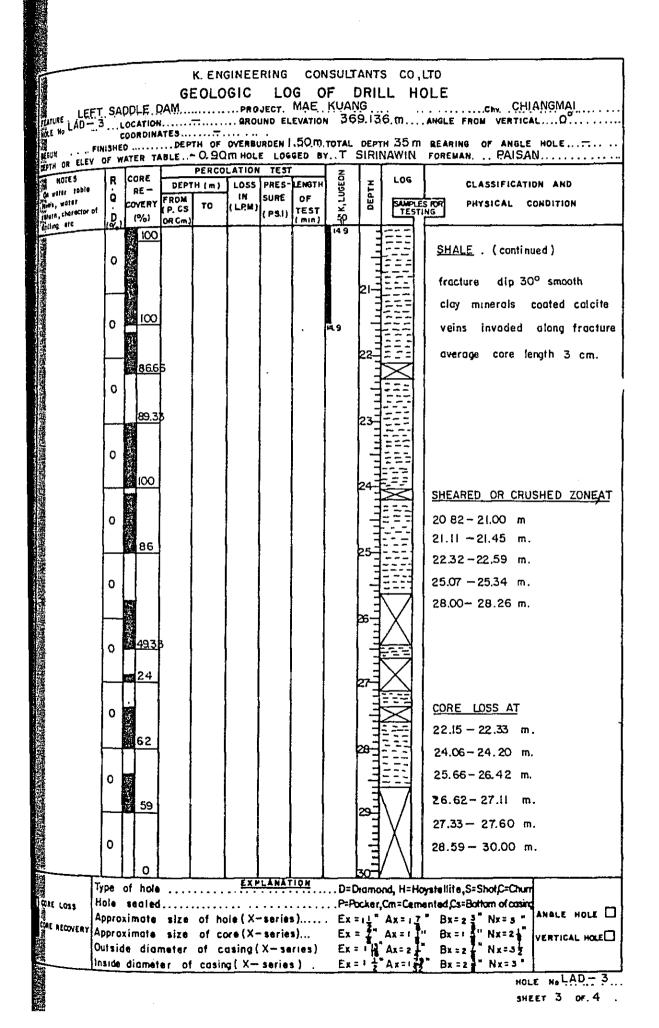
NOTES soler table	R	CORE RE - COVERY	DEPT	PERCO	LOSS IN	PRES- Sure	LENGTH OF	K, LUGEON	DEPTH	LOS	CLASSIFICATION AND
ing atc	ġ.	(%) (%)	(PCS DRCm)		(LRM)	(251)	TEST (min)	50 50		TEST	
	21	- <u>100</u>							<u>8</u>		crushed zone at 980-20.05 m. 18.55-18.65 m. 21.11 - 21.30 m.
	45	<u>100</u>				-			8		FROM 21.30.—2435 m. SANDSTONE greenish gray find grained interbedded with thick
	0	20 20 20							2		bed, grayish black slaty shale sandstone are highly filled wit calcite vein
	0	00 00							111111		crushed zone at 21.50—21.57 m. 22.00—22.20 m. 23.60—23.76 m.
	0	00 2 2									23.86–23.93 m. <u>CORE LOSE AT 24.22–24.35 m.</u> FROM 24.35–24.70 m. SLATY SHALE grayish black
	0	00 100							1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	\mathbb{V}	moderate weathered, core are highly broken into small pier crushed zone at 2460–24.70 r
	0	<u>8</u> 8			-				8 111111	\square	CORE LOSE AT 24.70-26.65
	12	100 100							זוווע	, , , / , ,	FROM 26.65–29.95 m. SANDSTONE gray fine to mediu grained fresh to moderate o o
	23								81111	/	weathered, fracture dip 40,60, smooth, calcite coated and veir filling along fracture, crushed
		100 100							21111		zone al 26.74 — 26.82
1		of hole			<u>Exp</u>	LANAT	<u>104</u>				oystellite,S=ShotC=Churr ented.Cs=Bottom of caving
A RECOVERY	ppro ppro	ximate ximate le diai	size size	of ci	ore(X-	- serie:	s)	: Ex : Ex	- - -	Ax=1.7	Bx=2 3" Nx=3 " ANGLE HOLE L " Bx=1 3" Nx=2 " VERTICAL HOLE

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FEATURE LEF	T SAD	LE DA	M		PRG	JECT.	MAE	KU/	ANG		Chy CHIANGMAI
	c	OORDIN	ATES								
BEGUN				0.90		E LOO	GED B				M BEARING OF ANGLE HOLE FOREMANPAISAN
NOTES On water table		CORE RE-	DEPT	H(m)	-	7	LENGTH	LUGEON	Ŧ	LOG	CLASSIFICATION AND
ievels, water resurn, characto	rot [COVERY	FROM P. CS	то	IN (LPM)	SURE	OF	Ĕ	DEPTH	SAMP	LES FOR PHYSICAL CONDITION
drilling etc	₽,	(%)	OR Cm)		· - · · ·	(PSI)	TEST (min.)	-*¥0			
		100									FROM 29.95 - 32.15 m.
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		58) 58)			1						grained interbedded with thick
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	27				ĺ				ĘĮ		slightly weathered
		100									
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		100			ļ					1	FROM 32.15 32.73 m.
	26								F	<u>.</u>	SANDSTONE pale gray fine
									Ь÷Э	-,-	grained fresh to slightly weather
										1	fracture dip 50, smooth, with calc
	76]]	1-	
						Í			34-		FROM 32.733500 m SANDSTONE pale gray fine
									• =		grained interbedded with thick
	33	100									
		001								<u>i</u> ::]	bed, grayish black slaty shole
									30-	<u> </u>	fresh to moderate weathered tra
											dip 60°, smooth calcite coated
						Ĩ					and vein filling along fracture
				ł					Ξ		Drecciated zone at 33.60-33.68
					ļ	ſ			ㅋ		filledwith calcite, core are
					Ì				T		highly broken into small pier
									7		at 34.70 m35.00 m.
			Í						Ξ		01 54.10 m.= 55.00 m.
						1			E		BOTTOM OF HOLE
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	Type o							D=D	iamon	d, H=He	oystellite,S=Shot,C=Churr
ORE LOSS	Approx	imate.	size	of hol	 • { X-	 Series	· · · · · · · · · · · · · · · · · · ·	.P=Po	cker,C . i "	lm=Cem ∧v.~=	ented,Cs=Bottom of cosing = Bx= 2 3 Nx= 5 ANALE HOLE [
HE RECOVERY	Approxi	imate	size	of cor	re (X	sories)	Ex =	'ŀ	AX = 1.7 Ax = 1	
	Outside	e diam	eter -	of ca	sing ()	K— ser	183)	Ex =	18.	Ax=z	Bx=z Z Nx=3 z
- * ***	Inside c	arumete	r QT	casing	1 2-1	series.	1	Ex=	15	Ax=1 A	Bx = 2 Nx = 3

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NEGUN	OF WAT	ER T	BLE ~		HOL			7. T. S T	IRIN/	AWIN	FOREMA	NPAISAN	
NOTES Da valer toble	RC	ORE RE		H (m)			LENGTH	SEON		LOG		CLASSIFICATI	ON AND
Huls, water Huls, water Hurn, charactor of	° ∝	WERY	FROM (P CS	τō	IN { L.Р.М.}	SURE (P.S.I)	O₽ TEST	K,LUGEON	DEPTH	SAMPLE	S FOR	PHYSICAL C	ONDITION
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t 1.	ype of Iole s			• • • • •		LANAT	· · · · · · · · · · · · · · · · · · ·					S=Shot/C=Chur Bottom of casin	
A	pproxi	mate	size					Ex 3	14	Ax=1.2	Bx=z	. 5 [#] Nx = 3 *	ANGLE HOLE 🗆
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FEATURE LEFT. HOLE No LAD - 3 BEGUNFIN DEPTH OR ELEV.	L C 113HE	OCATION CORDINA	AM	- тн_ог О_90	OVERBL	JECT UND EL JRDEN . E LOG	MAĘ EVATIO 1.50m GED' BI	KUAN	IG 69.1	ເວີຣິກ. ເອີຣິກ. TH .355 m	OLE
NOTES On water table levels, water return, charactor of drilling etc	R.Q.D	CORE RE – COVERY (%)		Ή (m) το	LATION LOSS IN (LPM)	PRES-	LENGTH OF	-& K, LUGEON	DEPTH	LOG SAMPLE TEST	CLASSIFICATION AND
	0	0							1		<u>SHALE</u> : (continued) fracture dip 30°smooth
	0	0							2 1 1 1 1 1		
	0	0									<u>CORE CUTTING AT</u> 30.00
	o	0							33		31. 32 - 32.77 m. 33.12 - 33.40 m.
	0	0									33.56 → 33.85 m. 34.50 → 34.73 m. Bottom of hole
			ž		7				3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		
CORE LOSS HO CORE RECOVERY Ap OU	ile prox prox tsidi	f hole sealed imate imate diarr diarrete	size size	of co of ca	le (X- re (X- ising ()	sarias sarias sarias X— ser)	.P=Po Ex = Ex = Ex =	cker, 113. 13.	Cm≖Ceme Ax=i7' Ax=i7'	

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REUN. OF ELEV OF	ED	ATES DEPTH ABLE ~ 2.0	OF OVERBU	RDEN 14 E LOGG	LOM 1 ED BY	T, SIRI	57,01, тн 50 гл. NAWIN Т	DLE 	
NOTES R de vater rable Mein, charactar at Mein, charactar at Mein, charactar at	RE -	DEPTH (n) LOSS	PRES-U	ENGTH	-S K,LUGEON	LOG SAMPLE TEST	CLASSIFICATION AND	
	of hoic		EXP		OM			CLAY brown, moderate to sli plasticity, wet N=4 SANDY CLAY greenish black slightly plasticity, wet N=2 SANDY GRAVEL of quartz of subangular, max. size 4 cm. SANDY GRAVEL of sandsto and quartz grain subangular max. size 2 cm., N=23	grain N= 14
RE LOSS Hold	souled	I				.P=Pocker		pystellite,S=Shot,C=Churt ented,Cs=Bottom of cosino "Bx=2 3" Nx=3 " ANGLE HOL "Bx=1 1" Nx=2 " VENTICAL HO	

				K. ENG						5 CO				
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HOLE No MAD - 1	L	OCATION	1	.	GROL	JND EL	EVATIO	N	JO 2.	A.C.1044	ANG	LE FR	OM VERTICA	vcQ.°
	C	OORDIN	ATES				14.0 m	-	DEPT	ы 50,0	0m _{ae}	ARING	OF ANGLE	HOLE
EPTH OR ELEV.		ATER T	ABLE 🕻	2.00 1	n, Holi	E LOG	GED B	Y: T	1. SIR T - T	INAW)	N FOI	REMAN		419
NOTES On water table	R	CORE RE-	DEPT	H (m)	LOSS	PRES-	LENGTH	K,LUGEON	H	LOG		C	LASSIFICATI	ON AND
levels , water return , charactor al		COVERY	FROM	то	IN [LPM]	SURE (P.S.I)	OF TEST		DEPT	TES	LES TOP	P	HYSICAL C	ONDITION
drilling etC.	(Pa)	(%)	ORCm			(((((((((((((((((((((min)	<u>.</u> 50	╞╌┨		F			
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			1						13-	$\langle \rangle$	1 1 1		medium	pole gray grained
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		0					ſ			$/ \setminus$		ore l	use at ISU	0-14.00 m
	<u> </u>								41			AETAS	ANDSTONE	pale gray
	0								E				-	rained high
	<u>اً</u>	6 100	4		1	1	1	1		k-7				
		40							15-	X	'	ore IC	ose at 14,72	10.20 M.
	0	+0	1		l	l	l	ļ						pale gray
	ľ		1					.			片,	o mei core	-	d highly bri
	┣	763							16-				ose at 1562	.— 15,75 m.
								ł			N	AETAS	SANDSTONE	. pale gra
	°	100												rained with
	┣				l		l		73					h and inter
			1				ł							and 9.26-9.
	52	00	-					1	11				to slightly	weathered 60 ⁹ smooth
			1			ł	ļ	17.9	18					ed, hightly -
									1 1	·	i			1 16.35-16.60
	60	100	4							$ \mathbf{N} $,
		100	1		1	1	1	179 5.7		1.				
								5.7		1				
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			<u> </u>	-				57	20-					<u> </u>
	•••				EX	PLANA	<u>T10N</u>						S=Shol/C=Chur	
		. seale		of h	ole (Y		•••			,Cm=C4 ' Ax=1			Bottom of cosin ∃"Nx=s"	ANGLE HOL
				ofc						Ax=1 Ax=1			" Nx=3 "	VERTICAL HO
(Jutsi	de dia	ameter	of	asing	(X – s	eries)	Ex	- 1 H	Ax= z	E.	Bx = z	4" Nx=32	
<u> </u>	nside	diam	ter o	1 COSI	ngtX-	- Jorie	5 }	<u> </u>	= 1 + 2	Ax=	H.	Bx = 2		LE No. MAD.
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ه ۲۰۰۰		-											2.0	-
	,			. '										

FUR FIL BYTH OR ELEV NOTES Watter table	R	CORE	DEPT	PERCO H (m)	LATION	TEST	LENGTH	Z	DEPTH	LOS	FOREMAN, PAISAN.
n, velat nis, charactor of ning eff		COVERY (%)	FROM (P.C3) ORCm)	то	(LRM)		OF TEST (min)	- - - - - - - - - - - - - - - - - - -	B	TEST	STOR PHYSICAL CONDITION
	63 89 22 12 22 57 26 76 10 33						77.9 79.8	*77.9			SLATYSHALE black with greenish gray sandstone patch slightly weathered METASANDSTONE gray fine to medium grained fresh to slightly weathered fracture dip 50° smooth calcite and iron oxide filled SLATYSHALE black moderate to slightly weathered METASANDSTONE yellowish gra to greenish gray fine to mediu grained with black shale path at the top and local quartz vein highly quartz veinlet at 3000 - 34.00 m. fresh to slightly weathered frac ure dip 30°,55°,60°,70°,90° smooth with iron oxide coated highly fracture, crushed zone 23.80 - 23.92 m. at 24.30 - 24.60 m. 35.70 - 35.80 m.
E LOSS H	ole pprox pprox utsid	sealed	size size neter	of co of co	ile (X- ire (X- ssing (series series X— se	s) s) ries)	P=R Ex : Ex : Ex :	cker : 1		

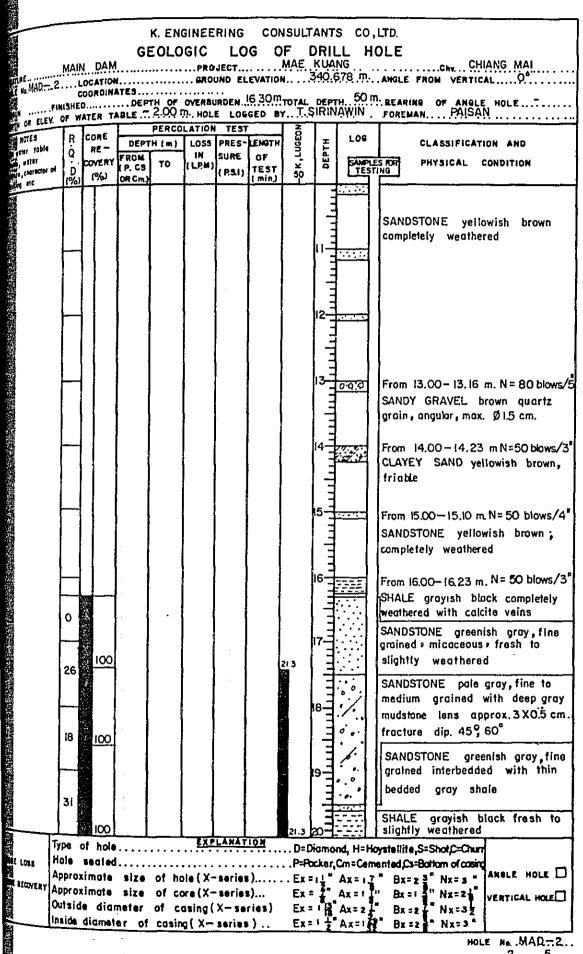
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M	AIN	DAM		EOLO			G O	EK	UAN	3	Chy CHIANG MAI
FEATURE		OCATION		.		UND EL	EVATION	i	538. İ	57.m.	ANGLE FROM VERTICAL
BEGUN FX	C NIŠHE	OORDINA	TES . DEP	TH OF	OVERBL	RDEN	14.0 m	TOTAL	DEPT	н <u>5</u> 0.0	MEARING OF ANGLE HOLE
DEPTH OR ELEV	OF W	ATER T	BLE?2	<u> </u>	P-HOLI	E LOG	GED BY	γ <u>,</u> Τ,	SIRI	VAWIN	FOREMAN PAISAN
NOTES	R	CORE				TEST	LENGTH	NO	Ŧ	LOG	CLASSIFICATION AND
On water table levels, water	là.	RE -	FROM	'H (m)	LN .	SURE	OF	K,LUGEON	DEPTI		
return, charactor of drilling atc.	<u>ل</u> ع	COVERY	(P. CS	ro	(%PM)	(PSI)	TEST	-'* 50	ō	SAMPL TEST	ES FOR PHYSICAL CONDITION
drining are.	1920		OR Cm }				1			.7	
		E.							3	$\cdot >$	
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	42							· · .			
		100						· ·	L		
		100							F	÷	
	27								1		core lose at 36.21-36.34 m.
										· · ·	METASANDSTONE pale gray
									37-		fine to medium grained fresh
		314 M						:			to slightly weathered fracture
	38										dip 50° 60° smooth calcute and
									L		clay mineral filled
	T	93.16							<u></u>	· · · ·	
	28								<u>[</u>]	≥ 1	core lose at 38.23-38.47 m.
	1								1		METASANDSTONE pale gray fine
	<u> </u>	41					84.2 96.3		39-	• / •	to medium grained fresh to
									E		slightly weathered fracture dip 60°,70° smooth with small pyrite
	57								न	\sum	crystals filled and iron oxide
							963		Ъ	;-::	coated
[1	уре	of hole			EXI	LANAT	10.0	D=[Diama	nd, H=H	loystellite,S=ShotC=Churt
- II	Hole	secier	4					P-P	hcker	Cm=Car	nented CasBottom of conint
	Appro	ximate	size	of h	ole (X	-serie	s)	Ex :	=ı <u>ı</u> " 2_	Ax=13	Bx=2 3 Nx=3 ANGLE HOLE
CORE RECOVERT	spore	oximate de dia	- SIZO	OT C	ore (X-	- serie	\$]	Ex	- I	Ax=1 Ax=2	" BX = 1 2" NX = 2 VERTICAL HOLEL
		oe oio I diamel						Ex:	- 1 - 1 -	Ax=z Ax=i	Bx = 2 N x = 3 2 Bx = 2 N x = 3
									2		HOLE No. MAD.
• _ '	- 31	. دنگه به ۱۰٫٫٫٫۰۰	•	`							SHEET 4 . OF. 5
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											<u>Ap</u>	pendix C-8 Page 16
						RING	COM	ISULT			.LTO	
MADT	AIN	DAM	GE	OLO	GIC	LO	G C	F E MAE		L F	IOLE	ANG MAI
TH OR ELEV			DEPI	н оғ 2	OVERBU	RDEN .	14 m. GED B	TOTAL Y., T	DEPT	н. 50	M-BEARING OF ANGL	
NOTES retar toble n, valtz reta, charecter of	R	CORE RE	DEPT FROM (P. CS	H (m)		PRES- SURE	LENGTH OF TEST	K, LUGEON	DEPTH	LOG	CLASSIFICAT	
and at		(%) (%)					<u>(min)</u>	90 1949-1941	1111	<i>.</i>		
		100							1111 1111	•	iocal quartz vei	ns,cores,are high
	40	100 ⁻							11111	 		40.60 m.
	38	100								·/·		41.32 m. 42.80 m.
		4 100					t I		2		STATY SHALE	black fresh to
	45							96.3	1111	, , ,	slightly weather lominated and	ed with thin convolute patch
	33	5 <u>100</u>							11111		of greenish gr fracture dip 40	°, 50°, 60° smooth
		<u>100</u>							45-T		with clay miner crashed zone at 43.	
									16 1			30-45.40 m. .68-45.76 m.
	0	<u>3 100</u>							11111		METASANDSTON fine to medium	grained fresh
	0	001									to slightly wea dip 50° 60° 70 small pyrite c	^o smooth with
		100							8		mineral filled local quartz ve	•
	0								8		at 47.26-47.4	
	19								11111	/./		
CHI 1033	Type Hole	of hold sealed	[] 	• • • • •	EXI		<u></u>	I D=(P=P	ockar	,Cm=Ci	BOTTOM O Hoystellite,S=Shot,C=Chu emented,Cs=Bottom of cos	xr irg
CHE LOIS ME ALCOVERY	Appro Appro Outsid Inside	ximate ximate de dia	size size meter	of h of c of c	ole (X· ore (X· asing w/ X·	-serie -serie (X-se	5) (5)	Ex Ex Ex	= 2" = 1 }	Ax=1 Ax=1 Ax=2 Ax=2	Bx = 1 Nx = 2 Bx = 2 Nx = 3	VENTICAL HOLE
		ulame	<u>(er 01</u>	COSI	<u>u(x-</u>	38(18	31,	EX :	<u> </u>	<u> </u>	H	ale No MAD-1. HEET 5.04.5

					K. EN	GINEE	RING	CO	NSULT	ANT	s co.,	LTD.			
					EOLO)G C			LL H	OLE	ה נו		
FEATURE DAM	ż	.L0	CATIO		•••••••		UND EL	LEVATIO	мЗ	40. E	578. m	, ANGLE	Chy CHI.	CAL 00	••••
		co	ORDIN	ATES				6.30	-	0.5 m	m 50 -			E 1141 -	••••
BEGUN	V UP	WAT	ER T	ABLE .	2,00,1	л.ног	E LOG	GED 0	Y	SIRIN	NAWIN .	. FOREM	AN. PAISAN		•••
NOTES	-TR	4	ORE	DEP	PERCO			LENGTH	LUGEON	Ŧ	LOG		CLASSIFICA		
On water table levels, water	14	þ	RE OVERY	FROM	1	IN	SHPE	0.5	ι Υ	рертн	SUPL	IS IOR	PHYSICAL		
return, charactor drilling etc.	" [A	61	ന്ത്ര	(P.CS ORCm)		((PSI)	TEST (min)	50		TEST				
	-]						
]		İ.		,	
										ľŦ		1	1.00 - 1.45		
							Ì]		SILTY	SAND bro	own	
	1									E					
					ł					2-		From	2.00 - 2.4	5 m. N = 6	
	ĺ									E		SILTY	CLAY brow	nish gray sl	ight[:
												plastic	:		•
										3-		From :	3.00 - 3.45	m. N=21	
											000	SILTY	GRAVEL b	rown subroun	idəd
										-		of qua	rtz approx.	Ø 2 cm	
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										Ē	<u>°08</u>		5.80-6.00 m		
										٦				STONE white, and white gu	
										1		max. §	ørdined t Ø4cm.	nia minici da	urtZ
						ĺ		[ļ	_ ‡,	000	From F	5.85-7.00 m)_	
	·			ĺ	[•		Έ		QUART	Z BOULDE	R	
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										8-1				n. N = 30 bio	ws/:
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										°‡		From S	9.00-9.45	m. N = 54	
						•		1		1		1	0.00-10.15 1	n, N = 70 bion	
			7					ĺ		Ξ				n. N= 70 blow	
· · · · · · · · · · · · · · · · · · ·	سل	Ľ		[]	EXP	LANAT	101		<u>0-1</u>				m.N = 80 blow	5/4
· ·	Typ∎ Hol€				 				D=Di P=Po	amon ckar.C	kd, H=Ho; Cm=Cerne	ystellite, Inted Ca	S=Shot,C=Chur Bottom of canin		
CORE LOSS	Appro	nix	nate	size	of ho	le (X-	series)	. Ex =	14" -	Ax=1_7	Bx=1	t ∛ " Nx= a "	ANBLE HOLE	
CORE RECOVERY					of co of ca				Ex =	;	Ax = 1, ["	Ba=I	1 Nx=2	VERTICAL HOL	Ш
					casing				Ex=	· 14 	Ax= 2] Ax=1 2		2 Nx=3 2		
			-		- <u></u>						32		но	E N. MART	
	,		- 1										SH	еетIOF\$	j
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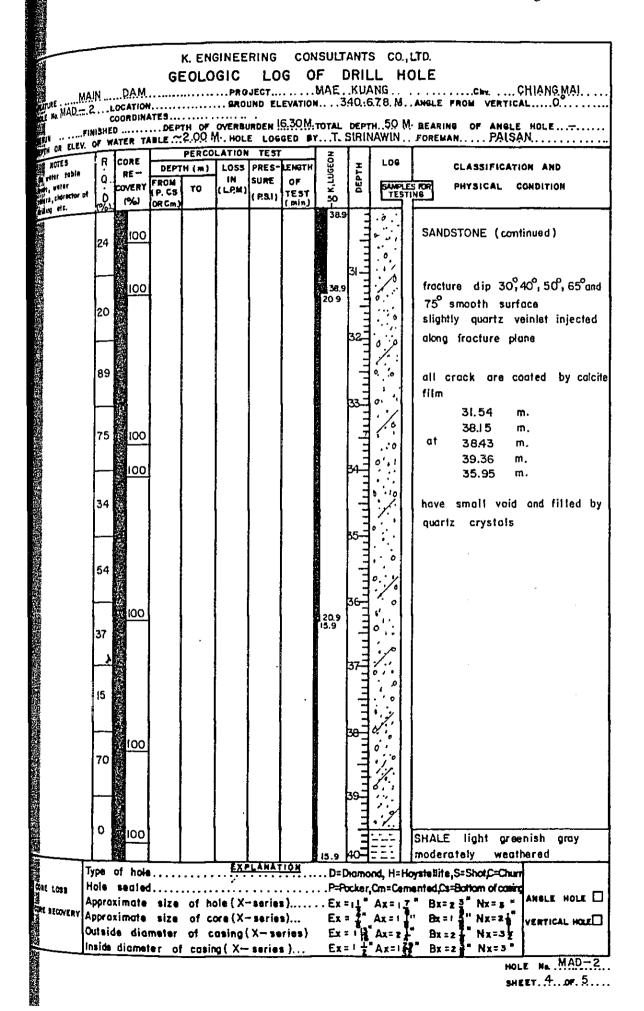
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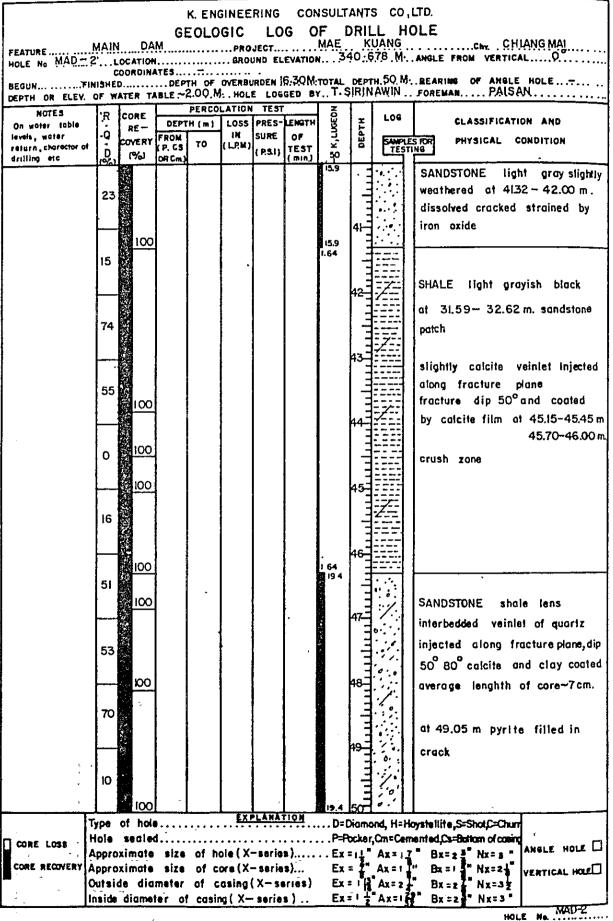


SHEET. 2. OF. 5

				K. ENG	INEE	RING	CON	ISULT	ANT	s co,	LTD.
			GE		G1C	10	6 0	f D	วิิิณ	L HO	DLE
HOLE No. MAD 2	L	OCATION				IND EL	CVALIU	1			
REGUN	C (13HE)	00RDINA D	TES . DEP	гн оғ.	OVERBL	RDEN)	6.30M	TOTAL	DEPT	г <mark>н, 5</mark> 0 М.	BEARING OF ANGLE HOLE
	1	ATER TA	BLE .~	2,00 M	ATION	E LOG	GED BI		кім Г	AW IN [FOREMANTRUNDIN
NOTES On water table	Ŕ	CORE RE-		H(m)	1055	PREST	LENGTH	LUGEON	рерти	LOG	CLASSIFICATION AND
tevels, water return, charactor of	D	COVERY %J		то	IN {L.P.M}	SURE (PSI)	OF Test	50 K		TEST	STOR PHYSICAL CONDITION
drilling etc.	%	<i>7</i> 07	OR Cm)				(min.)	213			SHALE crushed zone at 2000-2017m
	10								1 -		SANDSTONE greenish gray line
										1.	grained interbedded with black mudstone slightly weathered
		100 100							21-		
	14							213	=		
									LE	<i>Q</i>	SANDSTONE gray fine to medium
	\vdash								ľ	0.00	grained, massive, with quartz
	53								=		veins mostly filled fracture clay
	1							Ì	5		minerals partly coated along fracture
		100									fraceture dip $30,50,55,60^{\circ}$
	50]				and 70° smooth iron oxide coated
		100				ļ			e4=	, and	fresh to slightly weathered
									ΓΞ		
	61					1		}			
		100				ł			25	i viel	
		100 100									
	23										SHALE pale groy with small
		4 -0							þ6-		patch of greenish gray, fine
	20	100	ł				!	38.9			grained sandstone slightly weath-
	20			-				. ,			ered
	┝		ļ	Į	}]	1		<u>27-</u>	1	CANDGTONE fine to podium
	35			Í			1		[]		SANDSTONE gray, fine to medium grained massive with quartz vein
											at 29.72-29.88 m. are highly
	┢		ļ		ł	Į –			28-	eri.	quartz veinlets fracture dip
	67	100		l						0.3	30°, 50° and 60° smooth fresh to slightly weathered
		100]	[X	Trean to sugnity weathered
			1		Í		1		ľ.	<i>.</i>	
	52	-]]		ł			5/0	
								38.9	0	[<i>`;:</i>]	<u> </u>
	•••	of hol			EX	PLANA	<u>TION</u>			•	ioystellite,S=Shot,C=Churr
		seale etemixa		of h	 ole (X	 serie		Ex	티노	Âx≠i,	
		atimate								Ax = 1	
4		de dia diame								Ax=2 "Ax=1	Bx = 2 " Nx=3"
- 1 (*	,										HOLE No. MAD-2 SHEET. 3. OF 5
	e *	, -	- `?								SHEET

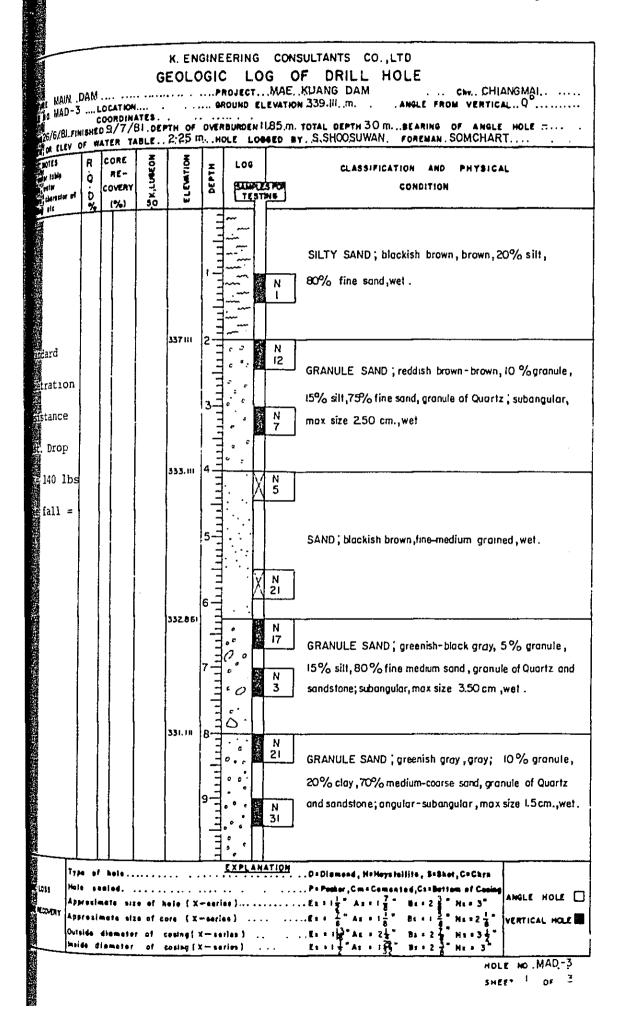
HOLE No. MAD-2.

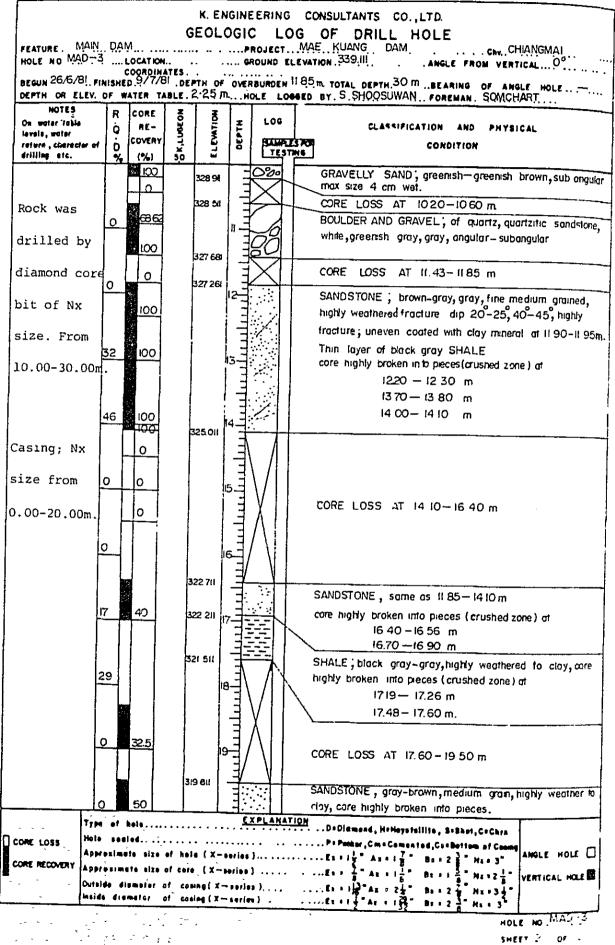




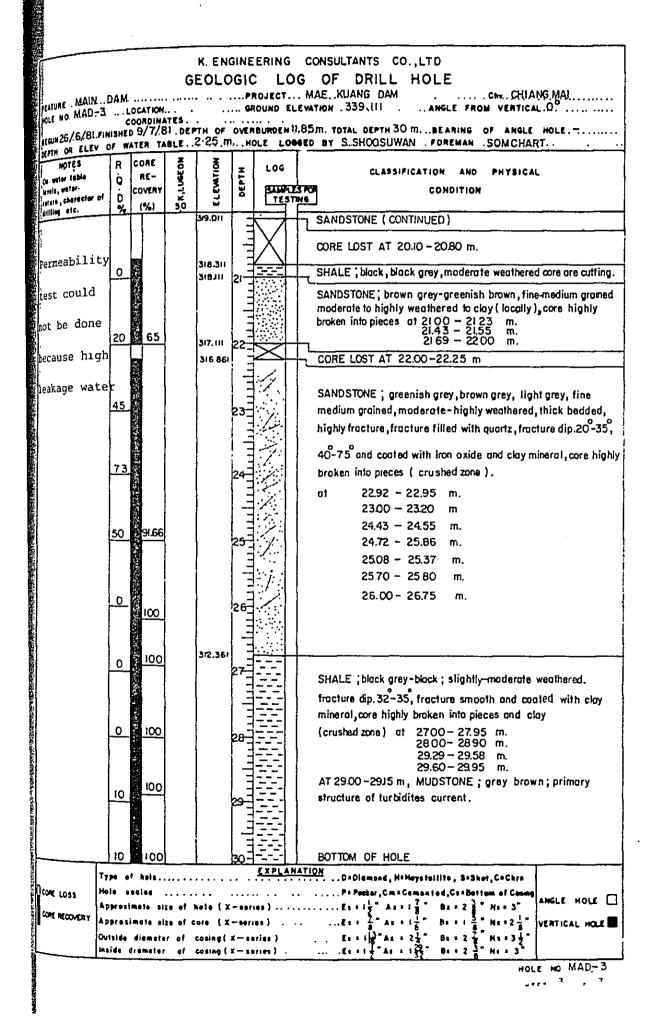
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SHEET. .5. . OF. .5....





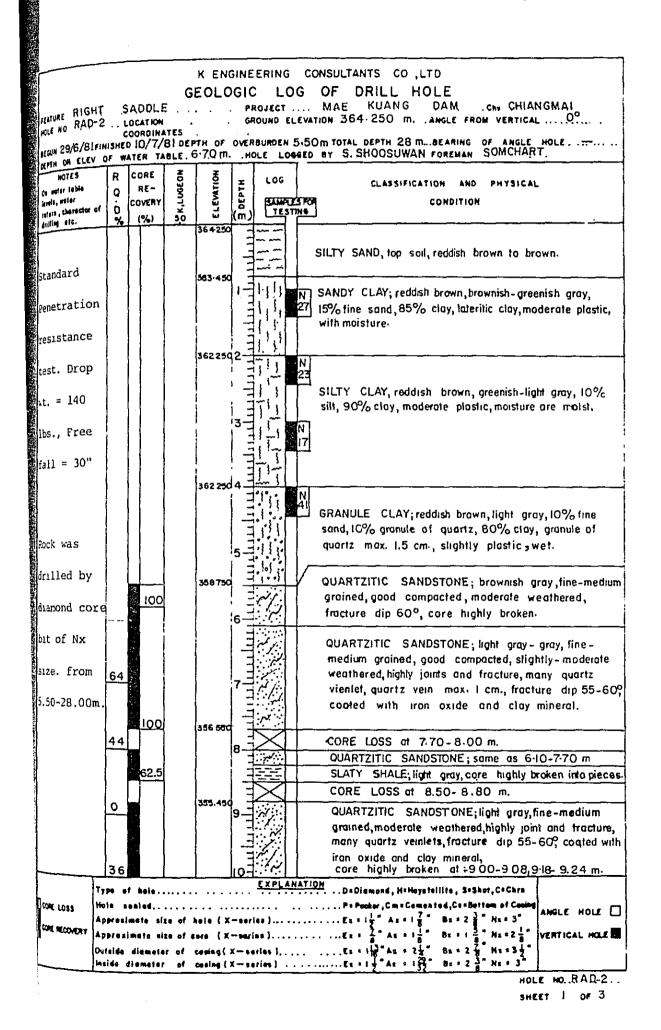
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K ENGINEERING CONSULTANTS CO., LTD. GEOLOGIC LOG OF DRILL HOLE CHIANGMAL COORDINATES BEGUN 29/6/81 FINISHED. U/7/BIDEPTH OF OVERBURDEN .7.O.M. TOTAL DEPTH. 28 M ... BEARING OF ANGLE HOLE ... DEPTH OR ELEV. OF WATER TABLE .. 10:65 m. HOLE LOGGED BY .S. SHOOSUWAN FOREMAN. SOMCHART NOTES CORE R SK,LUBEON ELEMTION LOG DEPTH On water table CLASSIFICATION AND PHYSICAL REģ iavels, water COVERY SAMPLES FOR return , chareet Ď TESTING . . deliling atc. (%) % SILTY SAND, reddish brown, yellowish brown, Standard 111 tine - madium grained. 3661201 Penetration N SANDY SILT; reddish brown, greenish red, 14 Lateritic soil, very fine sond, medium plasticity, resistance wet. 365.1202test. N [j]] SANDY CLAY; reddish brown, light - greenish brown, 19 Drop wt = 30% sond, 70% clay, low—moderate plasticity, Ξ ١į wet. 11 364 120 3 -140 lbs., Ν 111 SILTY CLAY; reddish brown, greenish groy, 15 Free fall = 1)₁₋ 10% time sond, 90% clay, highly plasticity. <u>;</u>-}| 30" 363 120 4 Ν CLAYEY SILT; reddish brown, greenish groy, 36 _} 40 % clay, 60 % silt, moderate plasticity, wet. 362.120 5-GRANULE SAND; 10% granule of guartz, sub N Rock was • • • 67 angular, 90% medium grained sand. a., drilled by SANDY GRAVEL; 15% medium - course 00 grained sand, 85% gravel of Quartz and 6 00 diamond core Quartzite, angular—subangular, max size 3.5 cm. 0000 00 bit of Nx 3601207-~~~ QUARTZITE; White weathered . size. From آيار جيريا QUARTZITIC SANDSTONE; light gray fine grained. 100 SHALE; light greenish gray, core highly broken, shale at 7.20-7.30 m, 7.60-7.85 m. 7.00-28.00 m. 359270 100 10 : 1. QUARTZITIC SANDSTONE, light gray, fine grained, good compacted fracture dip 50-60°. 8-N. 358 620 QUARTZITE; White, light gray, slightly weathered. NN good compacted, highly fractures, dip 35,60,80,90, NN 100 34 ۰. 358 220 9 CORE LOSS AT 8.90-9.10m. 358.020 QUARTZITIC SANDSTONE; metamorphosed QUARTZITE, light gray, slightly weathered highly fractures and joints, fracture dip 25,60°. 42 Hr, Gm+Gom. eber, Cm.+ Compated, Cs+Bottom af Ca CORE LOSS ANGLE HOLE _ ∎# # Z <u>≩</u> Nc + 3" $\begin{array}{c}
\mathbf{P}_{1} & \mathbf{i} & \mathbf{i} \\
\mathbf{P}_{2} & \mathbf{i} & \mathbf{i} \\
\mathbf{P}_{4} & \mathbf{i} & \mathbf{i} \\
\mathbf{P}_{4} & \mathbf{i} & \mathbf{i} \\
\mathbf{P}_{5} & \mathbf{i} & \mathbf{i} \\
\mathbf{P}_{5} & \mathbf{i} & \mathbf{i} \\
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\mathbf{P}_{5} & \mathbf{i} & \mathbf{i} \\
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\mathbf{$ CORE RECOVERY VERTICAL HOLE Bz # 2 -No a S HOLE NO RAD-1 ...

RAD L LOCATION	GEOLOGIC LO	Appendix C-8 Page 26 CONSULTANTS CO., LTD IG OF DRILL HOLE MAE. KUANG DAM LEVATION 367.120 m ANGLE FROM VERTICAL
INTES R CORE		CLASSIFICATION AND PHYSICAL
ng; Nx 48 from 100		QUARTZITIC SANDSTONE; locally metamorphosed to QUARTZITE, white, ligth gray, fine grained, good compacted, highly fracture, fracture dip 30-35,50-60, 80-90, coated with iron axide. AT 980-984, 10.08 -10.23, 11.61-11.67m. Thin layer of greenish gray-black gray SLATY SHALE.
32 100 ability 59	354.15013	QUARTZTIC SANDSTONE; light-brownish gray, Slightly metamorphosed; fine-medium grained, slightly-moderate waethered, highly joints and fractures, fracture dip 30-35, iron oxide coated. AT 12.97-13.CO,13.10-13.12,13.15-13.19m Thin layer of reddish brown SHALE, layer dip 40°
; USE 1001	353.520	CORE LOSS AT 1360-1400 m
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		QUARTZITIC SANDSTONE; light gray, fine-medium grained, good compacted, highly fracture, core highly broken into pieces, highly quartz veinlets Thin layersof greenish gray-light gray SHALE at 1455-14.57m, 15.11-15.25m. CORE LOSS AT 14.70-15.00m. QUARTZITIC SANDSTONE; light gray, gray, fine-medium grained, locally metamorphoshed to QUARTZITE, good compacted, slightly weathered, highly joints and fractures, dip 20-30,50-60 and coated with iron oxide, highly quartz veinlets. AT 17.63-17.64 very Thin layer of black gray SHALE. CORE highly broken at 17.00-17.26 m
10 10 100 44 100 70		CORE highly broken at 17.00-17.20 m 17.46-17.64 m 18.21-18.32 m 2066-20.73 m.
Type of belo Hele seeled Approximate size of Approximate size of Outside diameter of	<u>EXPLAN</u> bale (X-sories) sore (X-sories) cosing(X-sories) cosing(X-sories)	ATION D=Diemond, H=Hoystolilits, S=Shot,C=Chrs P=Pouber,Cm=Comented,Cs=Bottom of Cooleg Ex = t = "Ax = 17" Bx = 23" Hx = 3" Ex = 1 = "Ax = 17" Bx = 23" Hx = 3" Ex = t = "Ax = 12" Bx = 22" Hx = 3 = " Ex = t = "Ax = 27" Bx = 27" Hx = 37"
		HOLE NO.RADH

The number of the state of b construct and decay of the state of	FEATURE, RIG HOLE NO RAD BEGUN 29 /6/81 DEPTH OR ELET NOTES On white table	1 L C FINISHER / OF W R	OCATIO OORDIN D 11/7. ATER 1 CORE	N NATES . /81. DE TABLE .	ртн ог 10:65	ен оvен пн	ROUND I	. MAE KUANG DAM
100 346.020 20 100 346.020 346.020 20 100 100 16.95 20 100 100 16.95 20 100 100 16.95 20 100 100 16.95 20 100 100 16.95 20 100 100 16.95 20 100 100 16.95 20 100 100 16.95 100 16.95 100 16.95 100 16.95 100 16.95 100 16.95 100 16.95 100 100 341.20 23 11 24 100 341.20 341.20 24 11 24 100 341.20 11 24 11 341.20 11 341.20	Jevela, water	. I T		X,LUBE	LEWI	05971	SALA	
AT 27.68-27.70 thin layer of black gray SHALE. 48 100 3.90 339.120 9 BOTTOM OF HOLE 29 30 Type of bole		54 20 59 34 74		18-95	346.020 344.720 342.470 341.320	N N N N N N N N N N N N N N N N N N N		QUARTZTIC SANDSTONE; light gray, gray, fine-medium grained, slightly metamorphosed, good compacted, Slightly weathered; highly fractures, fracture dip 40-45 coated with iron oxide. AT 21 36-21 38,22.16-22.19 m. thin layer of greenish gray SHALE. Core highly broken at 21.22-2158 m. 21 86-21.92 m. QUARTZTIC SANDSTONE, light gray, fine grained, good compacted, slightly metamorphosed, Slightly-moderate weathered, highly fractures and quartz veinlets, fracture dip 30-45, coated with jron oxide. SHALE; black gray, gray, slightly-moderate weathered, slightly metamorphosed, core highly broken into pieces at 25.60-25.75 m. CORE LOSS AT 24.70-2500 m. AT 25.35 - 25.38 QUARTZTIC SANDSTONE, fine-medium grained. QUARTZTIC SANDSTONE; black gray, medium grained QUARTZTIC SANDSTONE; black gray, gray, fine grained, Slightly metamorphoshed, good compacted, slightly
Type of bole		48	юо	3.90	339.120.	ليبين	27. N	AT 27.68—27.70 thin layer of black gray SHALE.
CORE LOSS	, 1		beła .		Q 11		XPLAN	
ORE RECOVERY	, A		ste "siz	e of h	ele (X-	serie:	• • • • • • • • • • • • • • • • • • •	······Et = 1 = Au = 1 = Bu = 2 = Na = 3 = Na = 3



HOLE NO RAD-2 BEGUN 29/6/81.FIN DEPTH OR ELEV O	Û ISWFI	OCATION OORDINA 0 IQ/7/1 ATER TA	TES Bldef	•тноғ 5•7,0 m.	OVER	BURDEN S	EVATION 364-250 M. ANGLE FROM VERTICAL 0°. -SOM TOTAL DEPTH 28 M. BEARING OF ANGLE HOLE BED BY S. SHOOSUWAN FOREMAN SOMCHART.
NOTES On water table levels, water return, charactor of drilling etc.	R Q	CORE RE- COVERY (%)	GK,LUGEON	ELEVATION	DEPTH	LOG SAMPLE TESTI	CLASSIFICATION AND PHYSICAL
Casing; Nx size from 0.00-9.44m.	<u>52</u>	<u>ष्ठा स</u> ा <u>100</u>	16 54	363-400			SANDSTONE; black gray, fine-medium grained -moderate weathered, moderate compacted, joints and fractures, fracture dip 30-55°, with iron-oxide and clay mineral. SHALE; greenish-black gray, slightly weathered highly broken into piece. SANDSTONE; greenish gray, pale gray, slightly- weathered, highly quartz veinlets, fracture dip 25 and coated with iron oxide.
Permeability test; Use single-packe		100		351 750 350 630	-13-		Thin layers of black gray SHALE at 11-42 a SANDSTONE; light gray, fine-medium grained moderate weathered, slightly metamorphosed, r quartz veinlets, fracture dip 30°, 60-70°, c with clay mineral, highly broken at 13-09-1
System. Depth tested 10.50-15.50m	48		16.24	348750	4		SANDSTONE; light-greenish gray, medium gra moderate weathered, highly guartz veinlets, filled with quartz dip 40°, 65-70°, caated clay mineral. Care highly broken at 14.15-14.21 m.
17.70-22.70π	<u>30</u> 15 100	<u>74-1</u>	23-00		16		CORE LOSS at 15.50-15.85 m. SANDSTONE; greenish gray, light-pale gray madium grained, slightly - moderate weathe many quartz veinlets, highly joints and frac fractures coated with clay mineral and with quartz, fracture dip 30-35°, 50-60 Locally, thin layers of black gray SHAL Core highly broken at 15.85-16.11 m. 16.28-16.33m. 16.45-16.60m. 16.67-17.00m. 18.00-18.25 m
· · · · · · · · · · · · · · · · · · ·	82	100		349.060	19	EXPLAN	SLATY SHALE; block gray, slightly metam SANDSTONE; tight gray - gray, fine-medium gra good compacted, fracture dip 55°.
CORE RECOVERY AP	prani prozi side	mate 'siz diamate	e of l e of c r of c	hale (X ore (X coning(- Mri X - si	eu) eu) rins). :	$\begin{array}{cccccccccccccccccccccccccccccccccccc$

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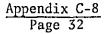
NOTES	R	CORE				LOG		TOTAL DEPTH 28 M. BEARING OF ANGLE HOLE
On water table lards, water return, cherector of drilling atc	O D %	RE- COVERY (%)	NO SOL NO SON	EL EVATION	DEPTH	SAMPLE TEST		CLASSIFICATION AND PHYSICAL CONDITION
		100		343 87			[SANDSTONE; continued from 19.43 m.
	<u>62</u> 88	100			22 22 22 22 22 22 22 22 22 22 22 22 22			SANDSTONE; light gray-greenish gray, fine-mediu grained, slightly-moderate weathered, highly joints and fractures, many quartz veinlet, fracture dip 40~50°, 68°-75° and coatin with clay mineral.
	20	19 C		341 65 340 62	231111			SANDSTONE, light-greenish gray, fine grained good compacted, fracture dip 50°60°, unever, coated with clay mineral and steep dip quartz vein filled, SLATY SHALE, cutting at 23:11-23:23 r
	24 73	100			24111111			SANDSTONE: geenish—light gray, fine—medium grained,good compacted, slightly—moderate weathered, highly fracture and quartz veinlets fracture dip 20-35,58-65,80, cooted with clay mineral.
		<u>100</u>	:		יוודוו			core highly broken at 2480—2485 m 25.25—25.43m
	32	<u>100</u> 100		338 35	للبالبيا			SHALE; light gray, gray, good compacted, slightly weathered, fracture uneven, dip.60°,85°, SANDSTONE; greenish gray, very fine grained, patch at 26.40~26.50 m.
		100		33725 33645				SANDSTONE; light gray, fine grained, good comp highly fractures and dip 50-55°, core high broken at 27.13-27.30 m., 27.37-27.53 m.
	<u>32</u>	100		336.25	8			<u>SHALE; light gray, moderate highly weathered</u> BOTTOM OF HOLE at 28.00 m.
DHE LOSS Hol App DHE HECOVERY App	e en Merti Diozi:	ialed nate siz wate siz	e of h e of c	els (X- pro (X-	-serie -serie		• • • • •	D=Diemond, H=Heysteillte, S=Shot, C=Chrn P=Pocker, Cm=Cemented, Cs=Shot, C=Chrn ANGLE HOLE Ex = $\frac{1}{2}$ " Ax = $\frac{1}{3}$ " Bx = 2 $\frac{3}{3}$ " Nx = 3" Ex = $\frac{1}{2}$ " Ax = $1\frac{1}{8}$ " Bx = $1\frac{2}{3}$ " Nx = $2\frac{1}{8}$ " VERTICAL HOLE Ex = $\frac{1}{13}$ " Ax = $2\frac{1}{4}$ " Bx = $2\frac{7}{4}$ " Nx = $3\frac{1}{2}$ "

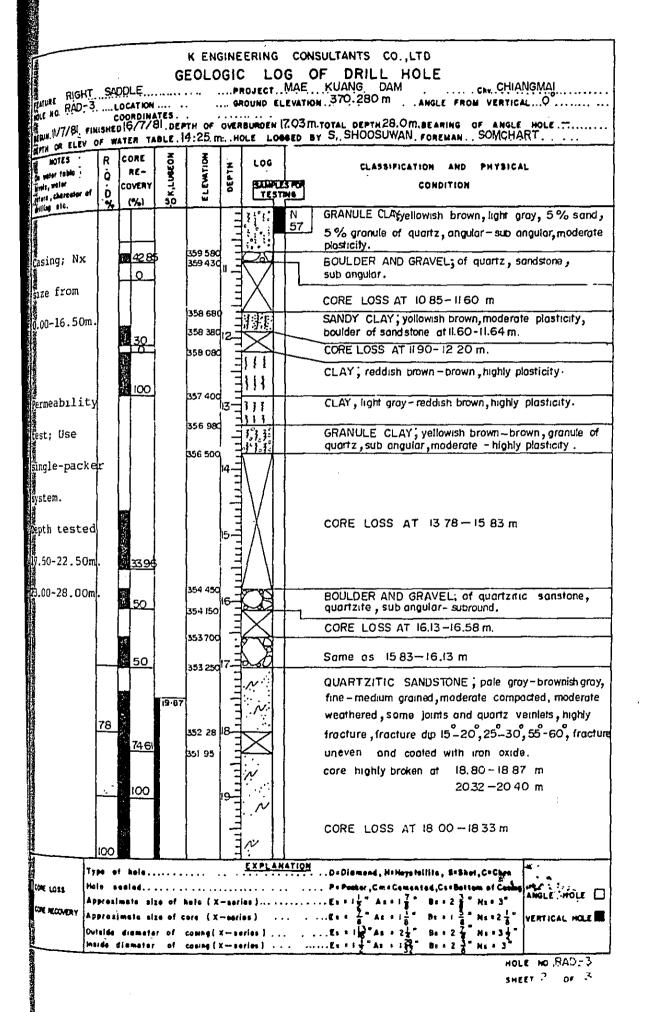
14-13 (well a

DEPTH OR ELEV. C	DF W	ATER TA	BLE	4:25 n	С но Т П	E LOGOED	M. TOTAL DEPTH 28 M BEARING OF ANGLE HOLE
MULES On water table levels, water return, charactor of drilling stc.	R Q D %	CORE RE- COVERY (%)	NO 301' NOEON	ELEWATION	DEPTH	LOG SUMPLESTON TESTING	CLASSIFICATION AND PHYSICAL CONDITION
Standard					IIIIII		SANDY CLAY; reddish brown; 15% fine sand, 75% clay, moderate plasticity; wet.
Penetration				369 28		₩ ₩ ₩ 13	SILTY CLAY; reddish brown-brown; 10% fine sa 15% silt, 75% clay, moderate plasticity, wet.
resistance				368.28	2		
test. Drop wt = 140 lbs				390,20		, : . N 18	CLAY; Light gray, reddish brown, highlyplasticity, Some gravel of quartz, max. size 0.30 cm, sub angu
Free fall =				367.29	3 =	E N	
30 "					a dunlanda	13 13 0 17	CLAY; Light gray, yellowish — reddish brown, moderate — highly plasticity, some gravel of quartz max.size 0.8 cm., angular — subangular.
Rock was					5	2 .	
liamond core				564 28		• 1	
oit of Nx				04 20	IIII	N 27	SANDY CLAY; yellowish brown-brown, 10 % 50
ize. from			3	63 28	7	1	90%clay, moderate plasticity wet.
0.50-28.00m						N 37 ; N ; N ; N 48	SILTY CLAY, yellowish brown, light gray, 5% fine sand, 15% silt, 80% clay, moderate plasticity, wet some gravel of quartz max.size 0.20 cm, angular- sub angular.
			3	61.28 5		۲ : ۲ : ۲ : ۲ : ۲ : ۲ : ۲ : ۲ : ۲ : ۲ :	SANDY CLAY; greenish gray, yellowish brown, 10% sand,90% clay, moderate plasticsity, some granules of quartz, wet.
CORE LOSS Hold App CORE RECOVERY App Ovta	o oo raxin raxin ida i	Glod Nata siza Nata siza	e ef-ha e ef-ha e ef ce	ite (X- le (X- leing(X	- serie - serie - serie	<u>x PLANATION</u>]	D=Diamond, H=Heystellits, S=Shet, C=Chrn P=Peeber, Cm = Cemented, Cs=Bettem of Cooling Ex = 1 2 " Az = 1 2 " Ss = 2 3 " Hs = 3" Ex = 1 4 Az = 1 3 " Bx = 1 2 " Ns = 2 4 " Ex = 1 3 " Ax = 1 4 " Bx = 2 4 " Ns = 2 4 " Ex = 1 3 " Ax = 2 4 " Bx = 2 4 " Ns = 5 4 "

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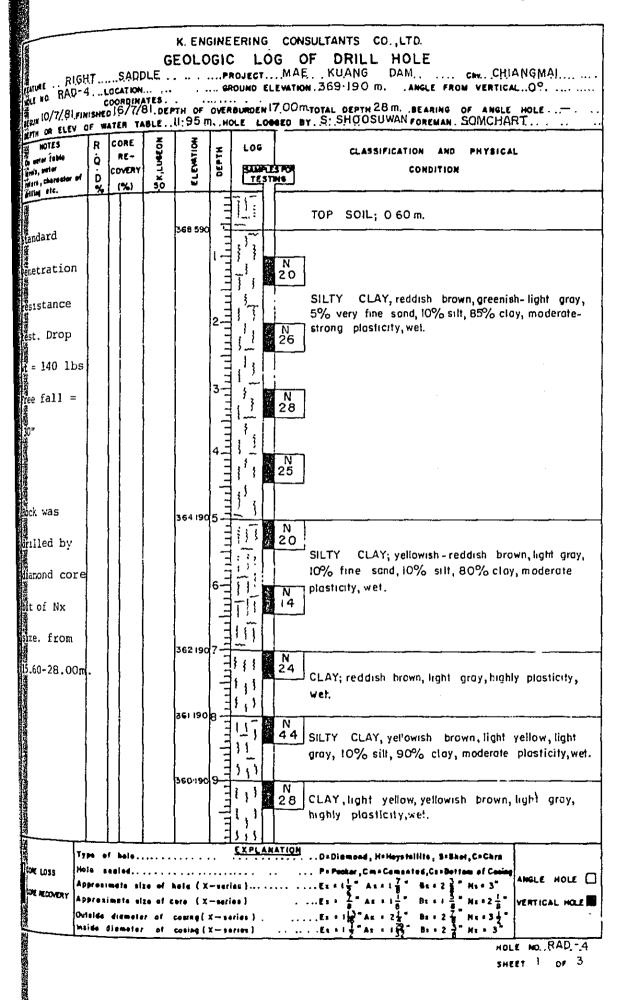


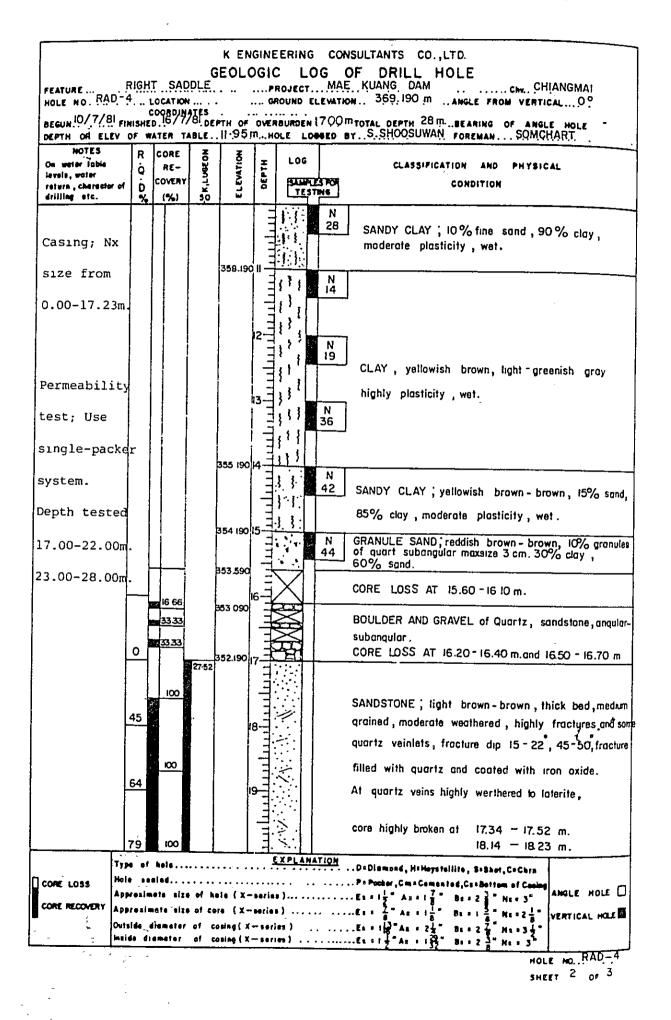


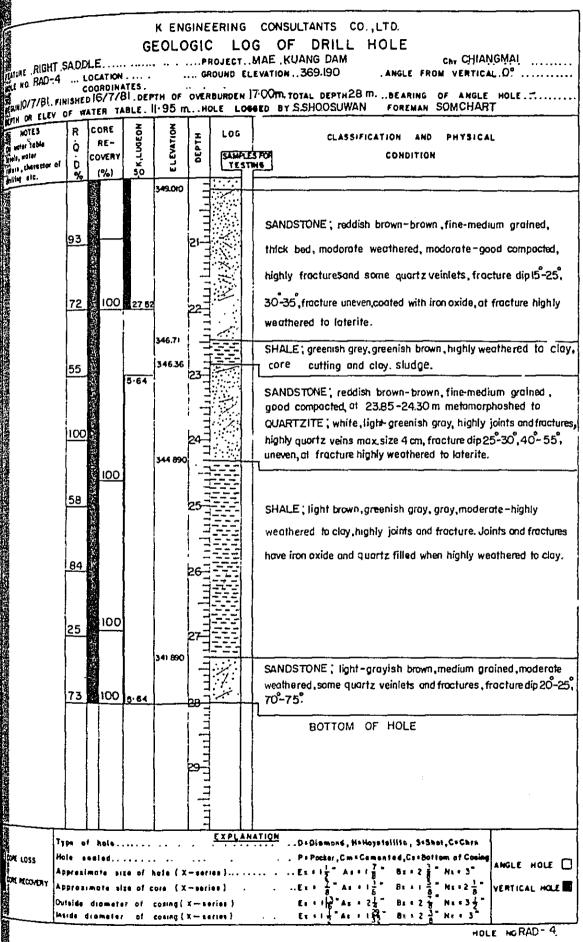
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FEATURE RIC HOLE NO. RAP BEGUN II/7/81			N	BEOLO	OGI(ROJECT.	MAE KUANG DAM CHIANGMAI LEWATION
DEPTH OR EL NOTES On water tablé levels, water return, cherocic drilling etc.	R	CORE RE- COVERY	NEON	ELEWION	DEPTH	LOG LOG SAMPL TEST	CLASSIFICATION AND PHYSICAL CONDITION
	77	<u>100</u>	1	348 73	2	N N	QUARTZITIC SANDSTONE (continued)
	43	100			22	N. N	QUARTZITIC SANDSTONE; brownish gray, pale gra fine-medium grained, good compacted, slighty-mod
	60		2·65	-	11111	NN	weathered, highly fractures and quartz veinlets, fracture dip 25°-30°, 50°-60°, uneven coated with iron oxide.
	81	< 100		347.000	1111		SANDSTONE; black gray-gray, fine-medium grain moderate compacted; slightly-moderate weathered Some fracture and quartz veinlet, fracture dip 20 ⁴
	23	8108		346 210 345 910 345 410			CORE LOSS AT 2407-2437 m. SLATY SHALE, black - black gray, slightly metamorp and slightly weathered, with turbidite structure, core high broken.
	10			344910 344 680	- IIIIII		CORE LOSS AT 24 87-25.37 m SLALY SHALE, same as 2437-2487 m SANDSTONE, greenish gray, fine grain Thin layer of
		<u>6428</u>		344 380 343 88	26	~~ 	Quartzitic sandstone, light gray, fine medium grained good compacted, at 26 22-26 25 patchs of greenish shale (cutting).
	94			342 98		/	SANDSTONE; Same as 23.28-24.07 m QUARTZITIC SANDSTONE; light argy fine-medium
	81	- 100	2.65		28- 1	<i></i>	grained, good compacted, skightly weathered some frac and quartz venlets, fracture dip 70–75. BOTTOM OF HOLE
					1111 North		
	1	hels				EXPLANA	NTION
ONE LOSS		nota siz diometer	a at so of c	Ne [X-	- serie (— ser)u) 10)	

SHEET 3 OF 3





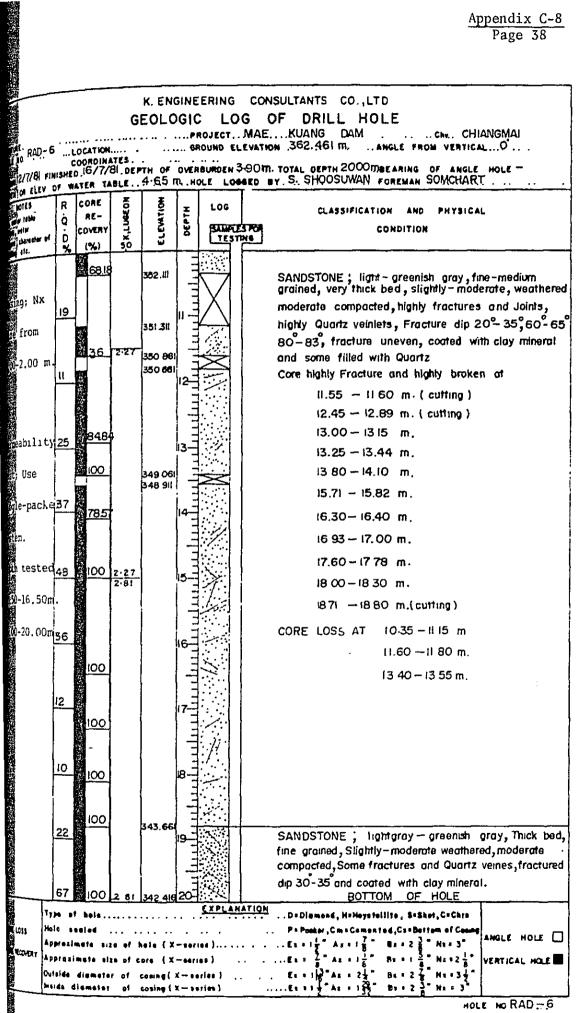


SHEET & CF 3

	С (OCATION	I ATES		e		LEVATION	E. KUANG DAM
DEPTH OR ELEV	OF W	ATER T	ABLE .4	1·65 m	н.	OLE LO	NED BI	Y.S. SHOQSUWAN FOREMAN SOMCHART
NOTES On water table levels, water return, chargeter of drilling etc.	R Q D	CORE RE- COVERY (%)	CK,LUBEON	ELEMTION	DEPTH	LOG SILUF() TEST		CLASSIFICATION AND PHYSICAL CONDITION
				1		191		GRANULE CLAY; yellowish brown-reddish
Standard			}		=			brown. LATERITIC SOIL. Granule of Quartz
Gunduru		[] ·						and Sandstone; Subangular max Size 3 cm
Penetration						9;9	N 20	Moderate plasticity.
resistance				360 861	Ξ		<u> </u>	CORE LOSS AT 1.60-185 m.
test, Drop	12	<u> </u>		360 611	2_			LATERITE; reddish brown-brown, LATERITIC SOIL,
Lese, stop				360 161		T.	ļ	BOULDER AND GRAVEL of SANSTONE, Subangular
wt = 140 lbs		75		359.861			ļ	CLAY, brownish-greenish gray, moderate plasticit
Free fall =	33			359.311	31	Х		CORE LOSS AT 2.60 - 3 15 m
		8, J		502.511		1,11		
30"		52 17		358711		73.11		CLAY; light gray, lightbrown, moderate plasticity
	0			358.56	4-	\sim		CORE LOSS AT 3.75-390 m SANDSTONE; Light gray-light brown,fine-medium
		80				<i>.</i> []		grained, moderate - highly weathered to clay, highly
						· . ,		fructure, fracture dip 20 - 25,40 - 60, uneven,
lock was	62		:		- 5	1.		coated with Iron axide and clay mineral.
rilled by					3	· · ·		
-		001		357011		ĊH		
iamond core	11					ХII		CORE LOSS AT 5.45-592 m
it of Nx				356 461	6	<u>,</u>		SANDSTONE; Same as 390-5.45 m
					Ш	<u> </u>		SANDSTONE; Light gray; fine-medium grained,
ize. From		<u>63 8</u>	4		Ţ	4		moderate weathered, highly joints and fracture, dip 30–35°AT 6.70–690m Patch with thin laye
	281	100		35536	7-			black SLATY SHALE, core highly broken into piec
.60-20.00m.					- T	\times		at 640-6.50m, 673-687m.
			1	355.011	Ŧ			CORE LOSS AT 7.10-7.45 m
	0	52.2	2	354561	8-1			SANDSTONE; Same as 600-7.10m
	ĺ	100			Ī	23		SHALE, greenish—light gray; moderate weathered, highly joints, with small patch of greenish gray,
		100			-			fine grained SANDSTONE at 8.54 8.63 m,
	12			ĺ	_ =			core highly broken into picces at 800-837 m
1		100		353.211	거			8.70-925m
	ſ		ŀ		E.	$\times \Pi$	(CORE LOSS AT 9.25-960 m.
	0		L	352 861 352.611	Ŧ	<u>==</u>		SHALE; Same as 790~925m.
 	· · · · ·		I	1	[EXPLANA	TION	
CORE LOSS Hel			•••••• ••••	· · · · · ·	••••	· · · · · · · · · · · · · · · · · · ·	· · · · · · ·	. D+Olemond, H=Heyatallito, S+Shet,C+C+Chra . P= Peeber, Cm+Cemented,C+Bettem of Center
A p								Ex = 1 - Ax = 1 - Bx = 2 - Hx = 3 ANGLE HOLE
			-	•		9 8)		
		diemotor Amotor				ries)		

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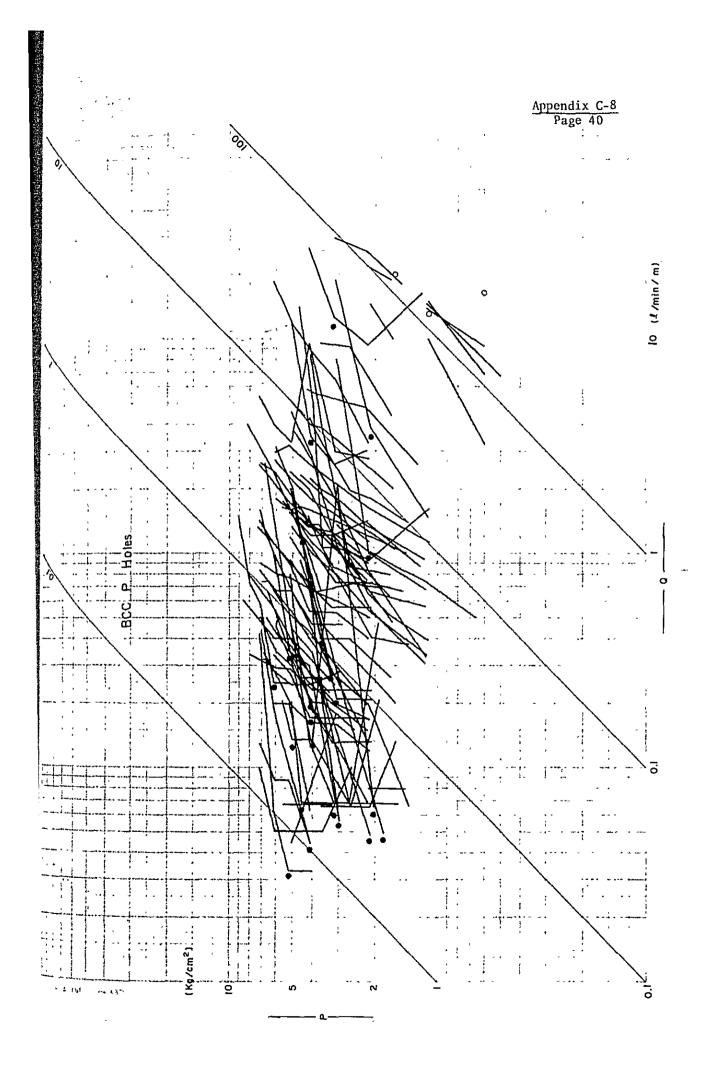
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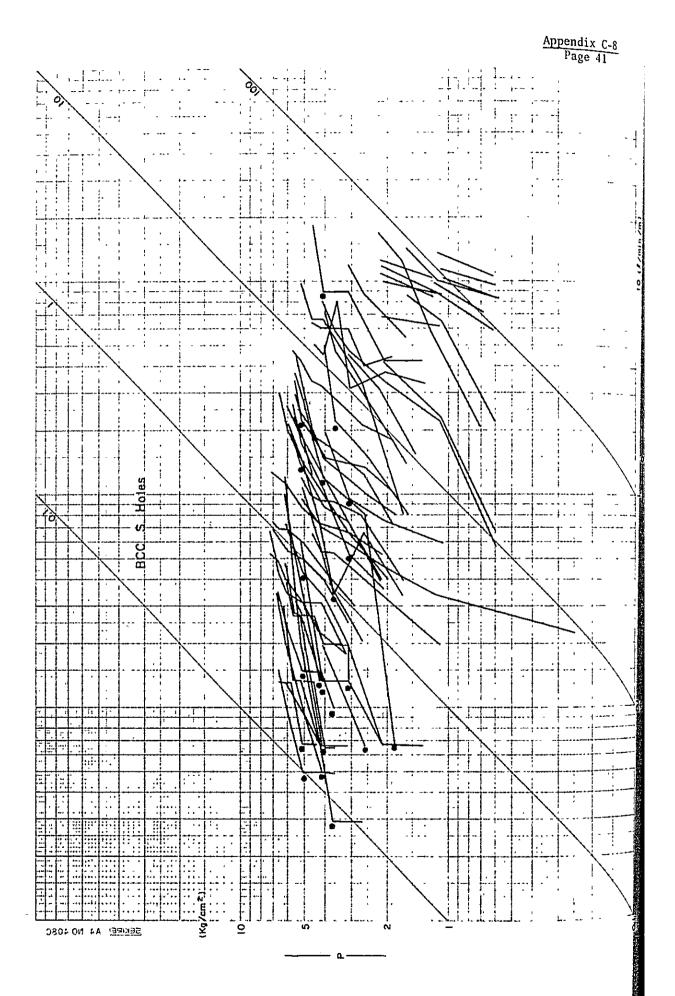


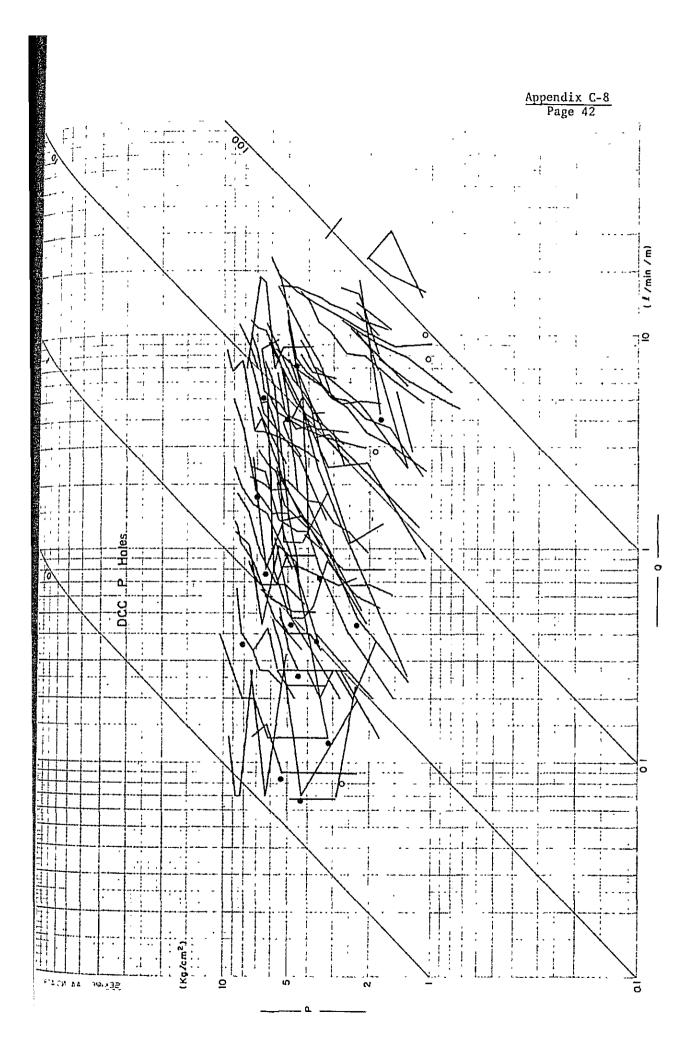
8-2 Analysing Charts of Permeability Test

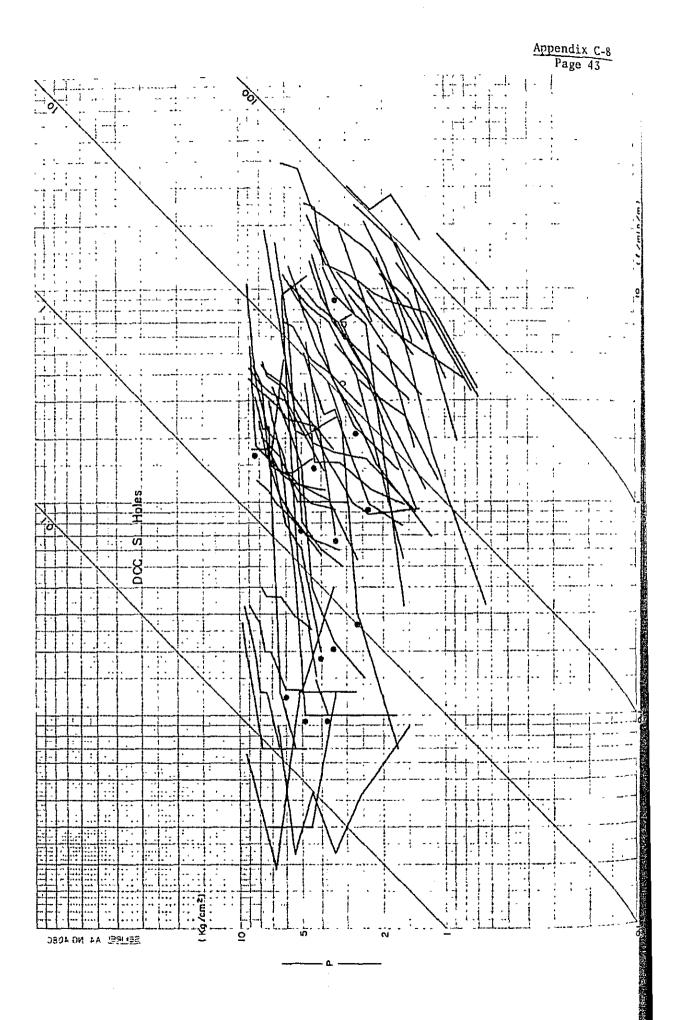
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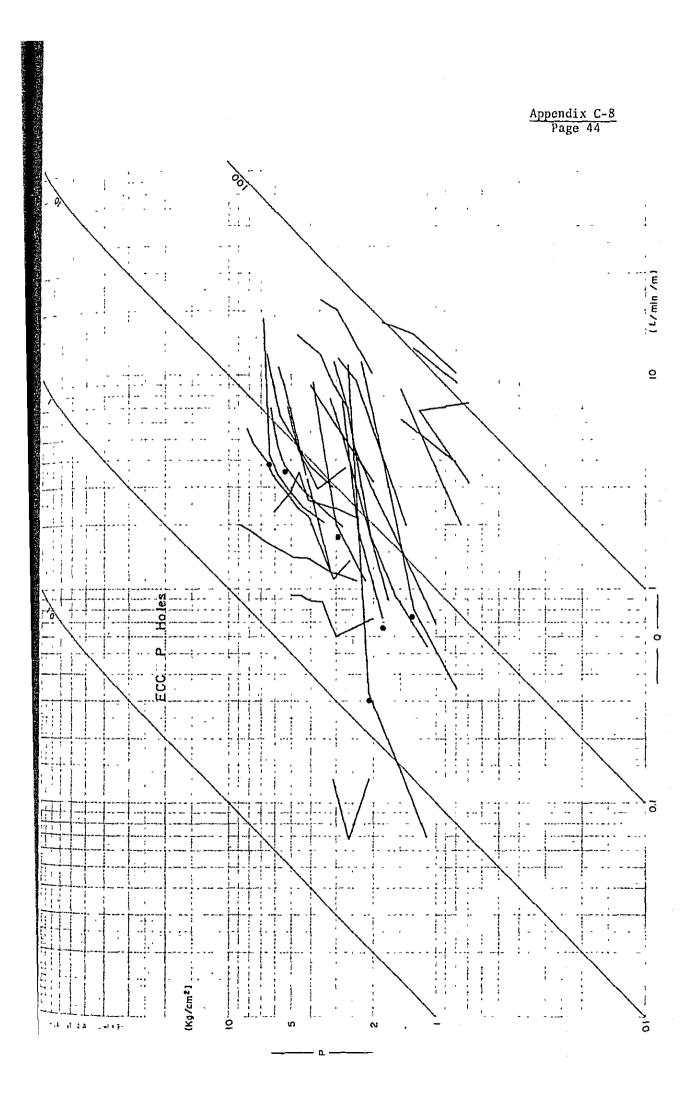


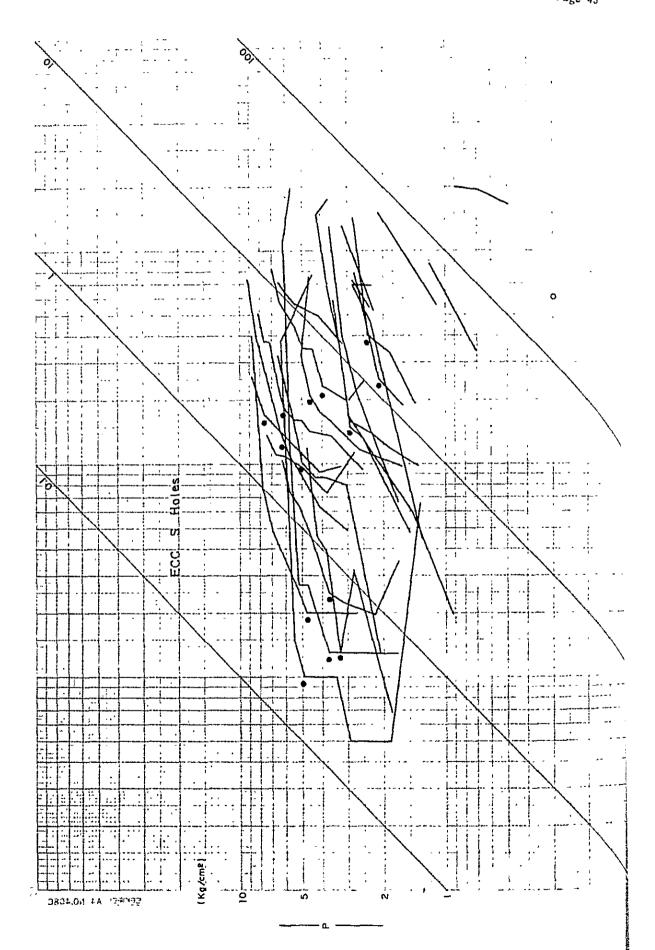




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8-3 Records of Lateral Loading Test

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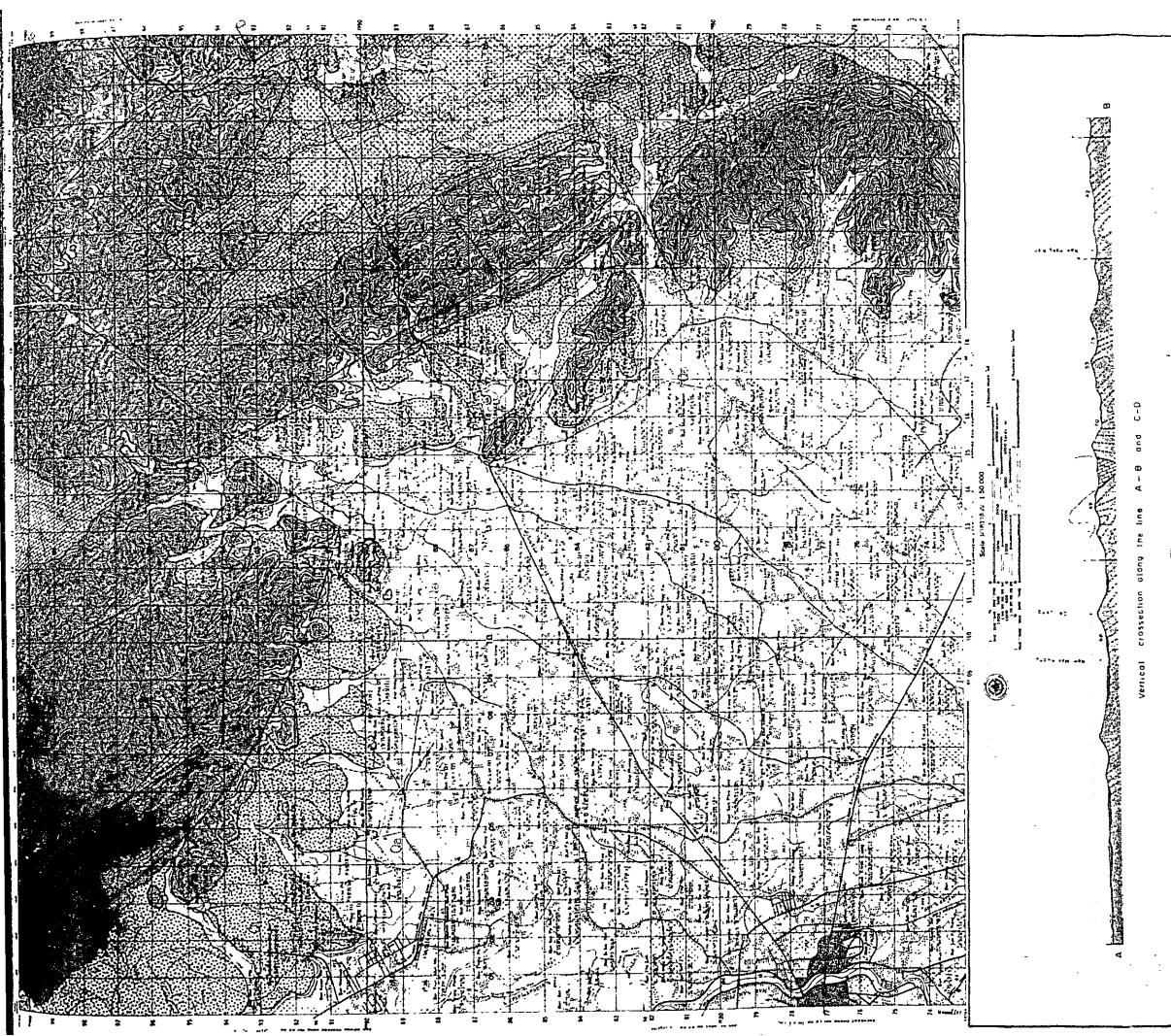
8-4 References

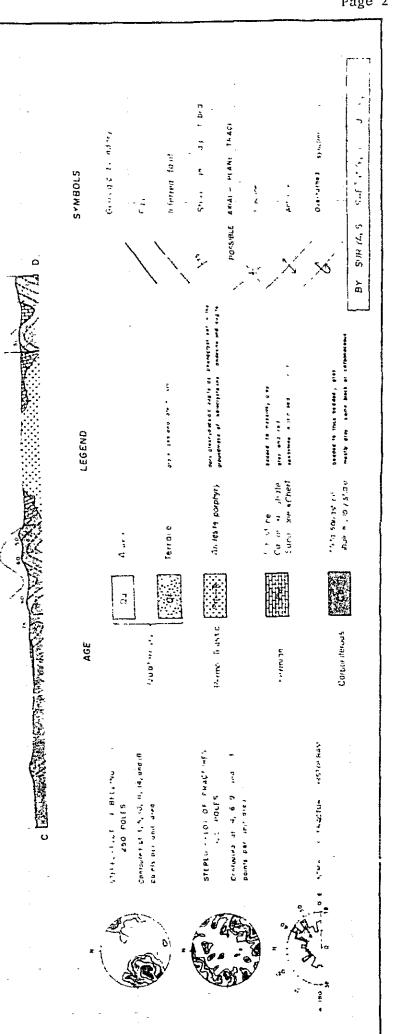
G-1	Report on Geological Investigation of the Mae Kuang Project 1974, R.I.D.
G-2	Report on Geological Investigation of the Mae Kuang Project Additional Drillings 1979, R.I.D.
G-3	Drilling Loggs and Profiles, Investigation for Overburden 1978, R.I.D.
G-4	Grouting Records of Zone B, C, D and E 1981, R.I.D.
G-5	Permeability Tests Records of Zone B, C, D and E 1981, R.I.D.
G-6	Geological Map "Changwat Chiang Rai" (1/250,000) D.M.R.
G-7	Geological Map of Thailand (1/1,000,000) D.M.R.
G-8	Report of Master Plan Study of the Greater Mae Klong River Basin Development Project 1980, JICA
G-9	Grouting Plan of the Left Saddle Dam R.I.D.
G-10	Contact Grouting Plan at Outlet in the Left Saddle Dam R.I.D.
G-11	Brieflet of Mae Ngat Project 1977, R.I.D.
G-12	Design of Small Dams Bureau of Reclamation,USA
G-13	Design Criteria for Fill-Dam M.A.F. Japan
G-14	Geology and Palacontology of Southeast Asia 1964, T. KOBAYASHI, Japan

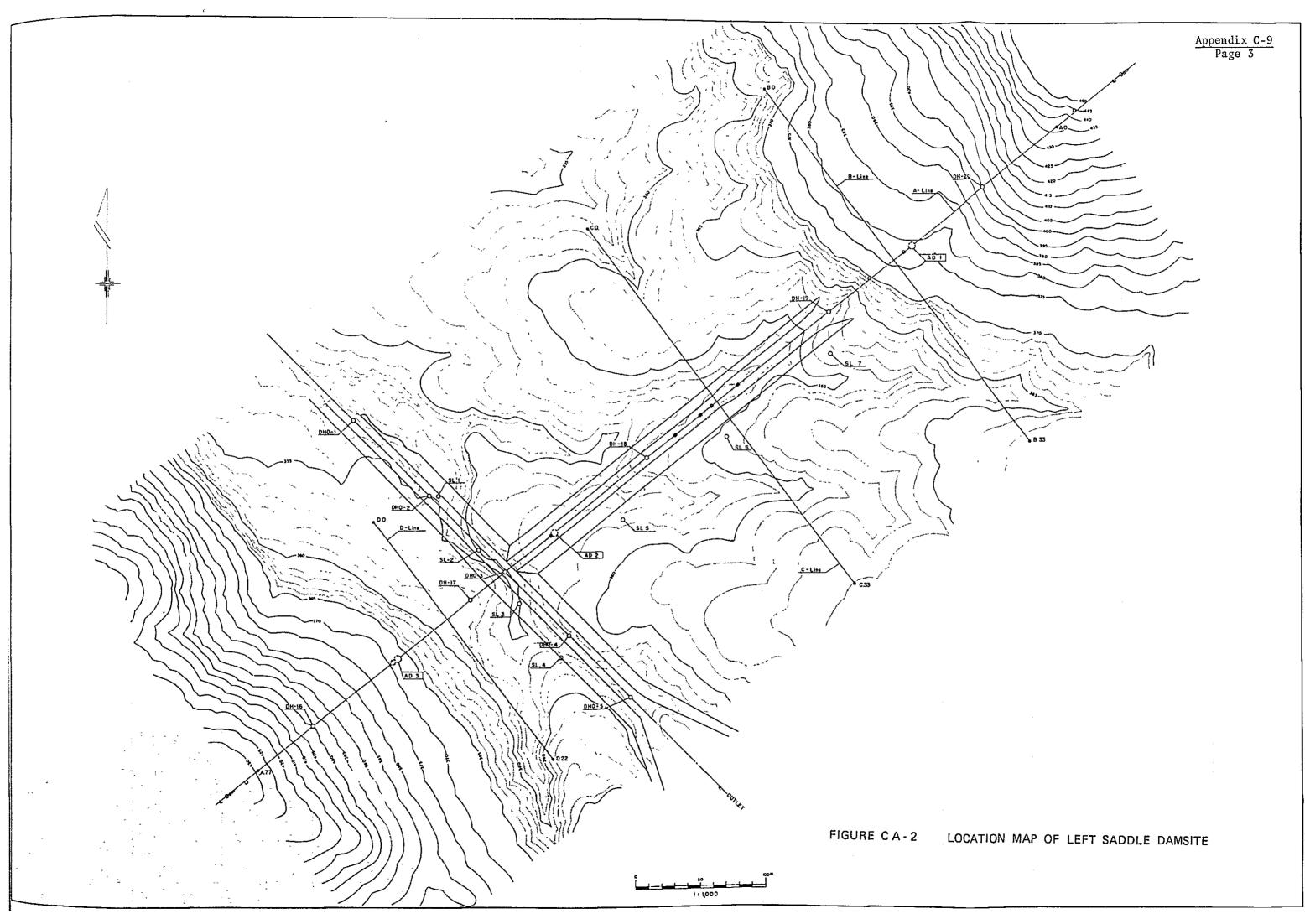
9. Attached Figures

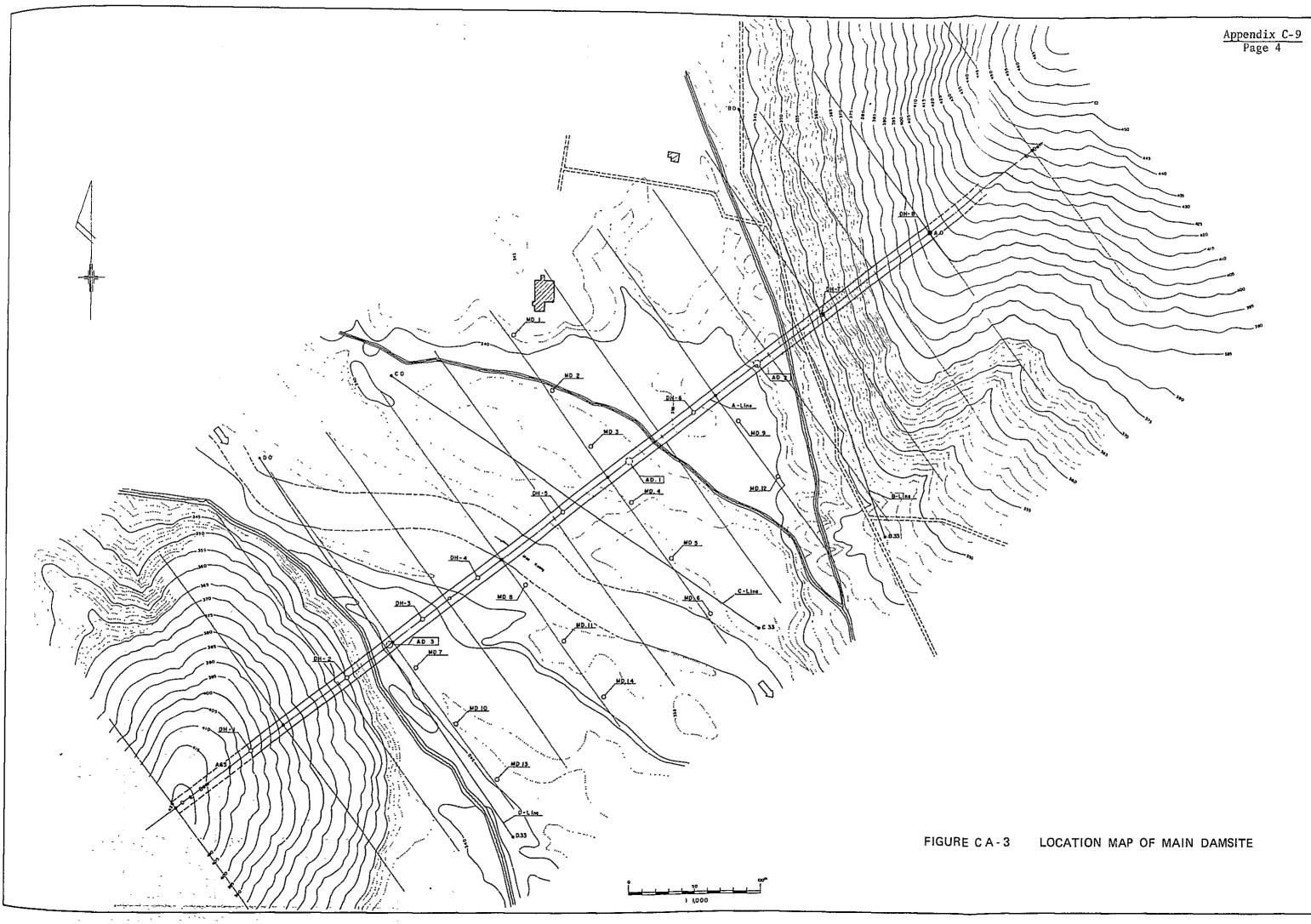
Figure	CA-1	Geological Map
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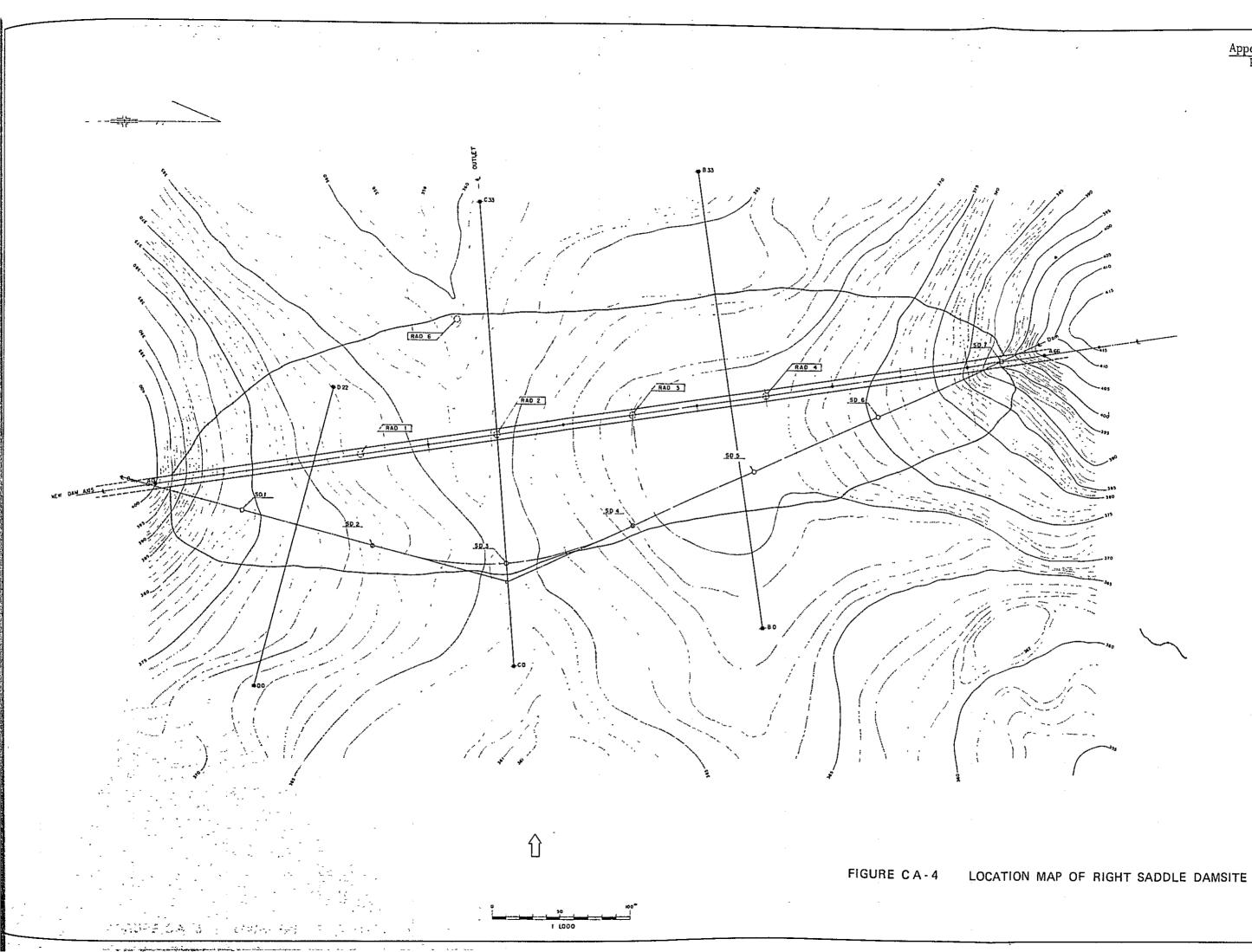
- Figure CA-2 Location Map of Left Saddle Damsite
- Figure CA-3 Location Map of Main Damsite
- Figure CA-4 Location Map of Right Saddle Damsite
- Figure CA-5 Location Map of Spillway
- Figure CA-6 Geological Profile of Left Saddle Damsite
- Figure CA-7 Geological Profile of Main Damsite
- Figure CA-8 Geological Profile of Right Saddle Damsite
- Figure CA-9 Geological Profile of Spillway
- Figure CA-10 Analysing Chart of Seismic Exploration (I), (Left Saddle A-Line)
- Figure CA-11 Analysing Chart of Seismic Exploration (II), (Left Saddle B, C, D-Lines)
- Figure CA-12 Analysing Chart of Seismic Exploration (III), (Main Dam A-Line)
- Figure CA-13 Analysing Chart of Seismic Exploration (IV), (Main Dam B, C, D-Lines)
- Figure CA-14 Analysing Chart of Seismic Exploration (V), (Right Saddle A-Line)
- Figure CA-15 Analysing Chart of Seismic Exploration (VI), (Right Saddle B, C, D-Lines)
- Figure CA-16 Analysing Chart of Seismic Exploration (VII), (Spillway M, S, E-Lines)



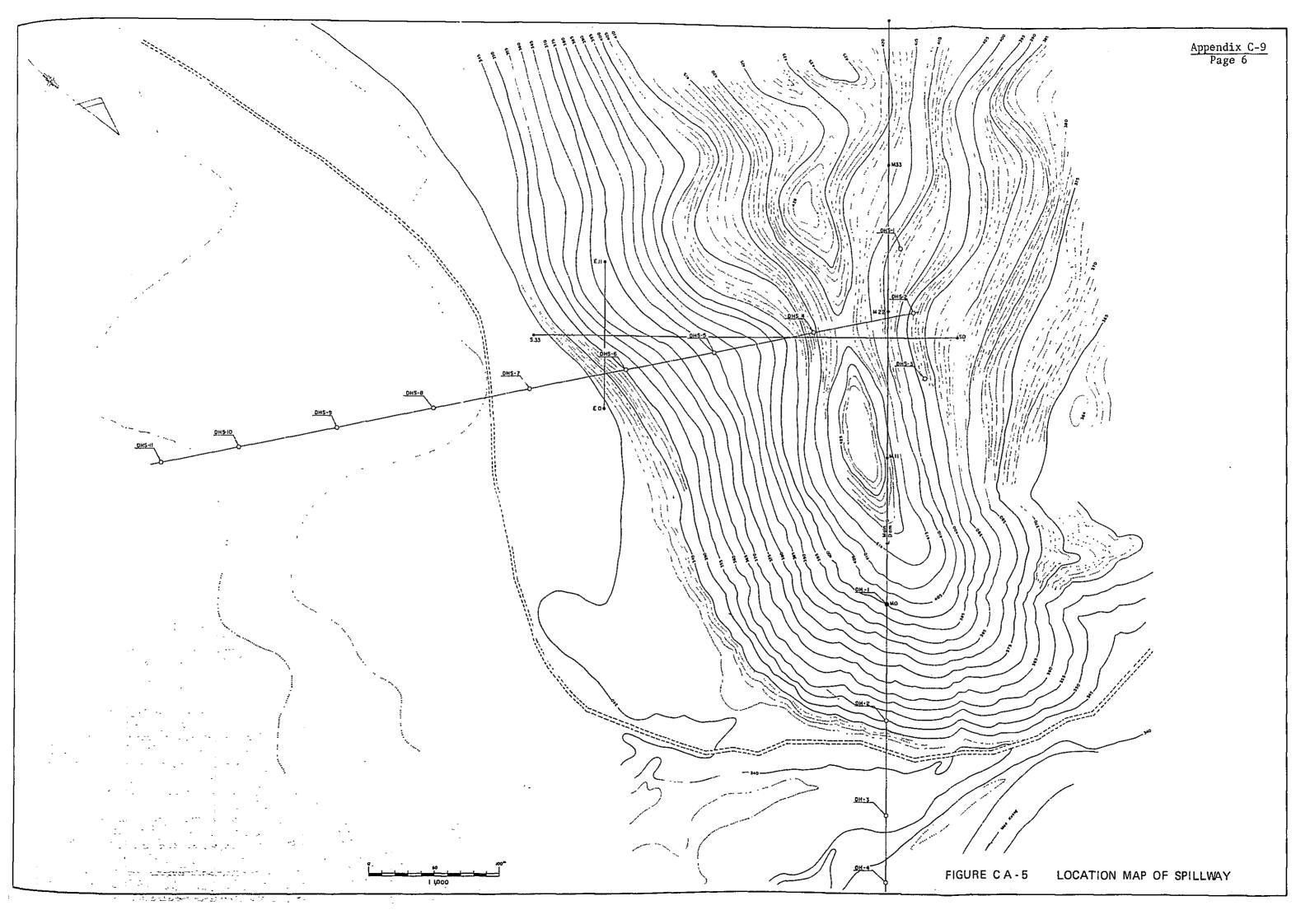


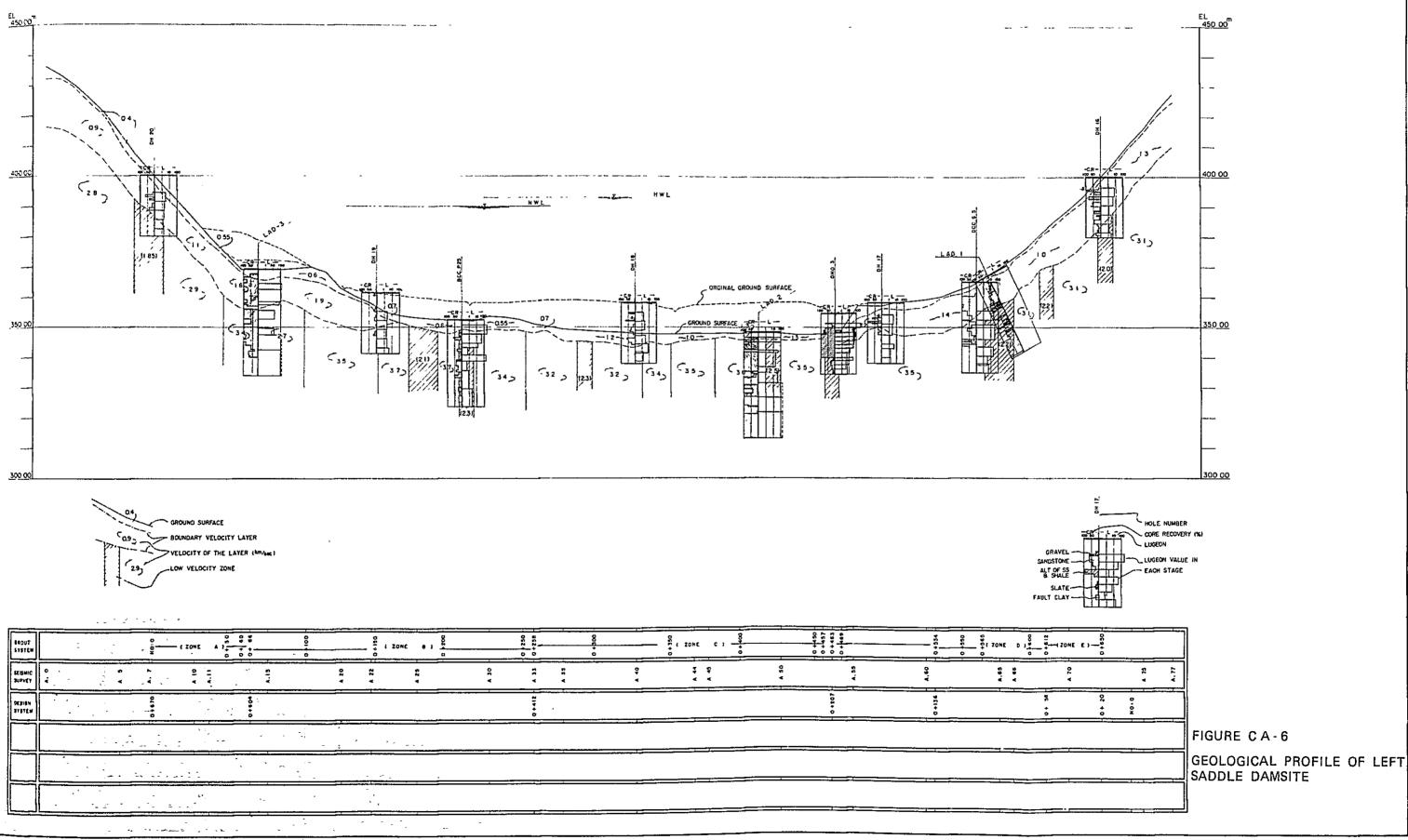






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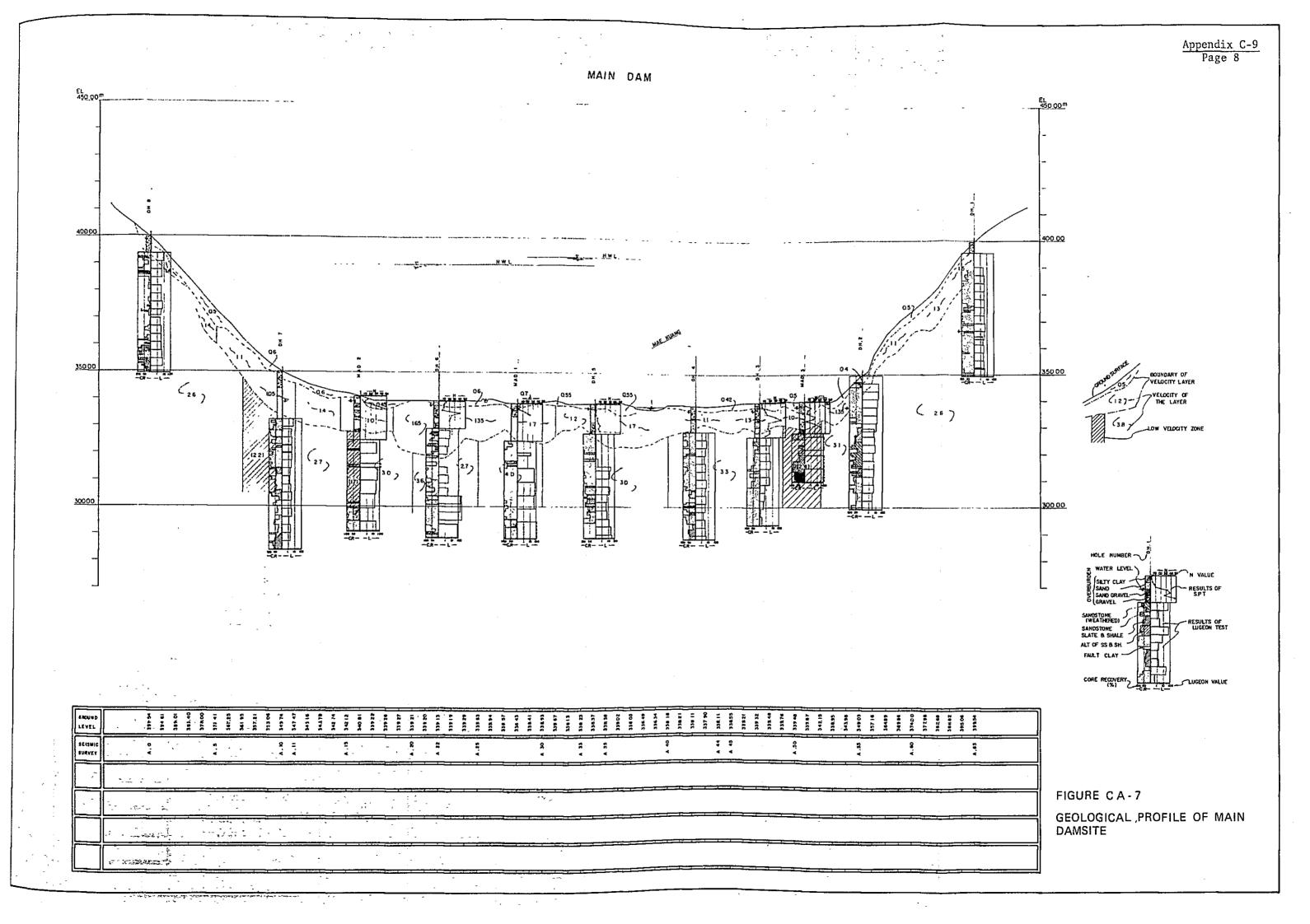


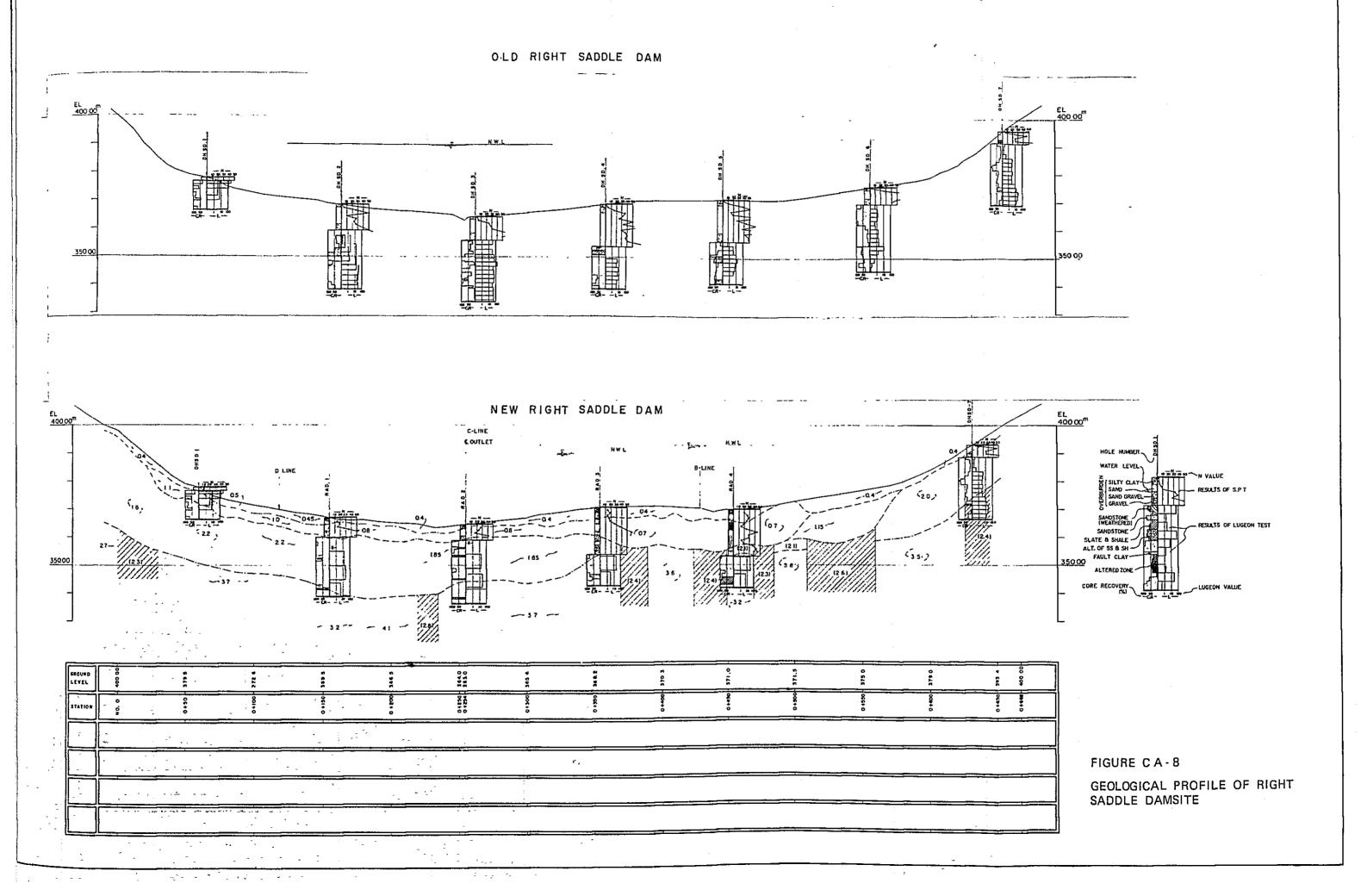
LEFT SADDLE DAM

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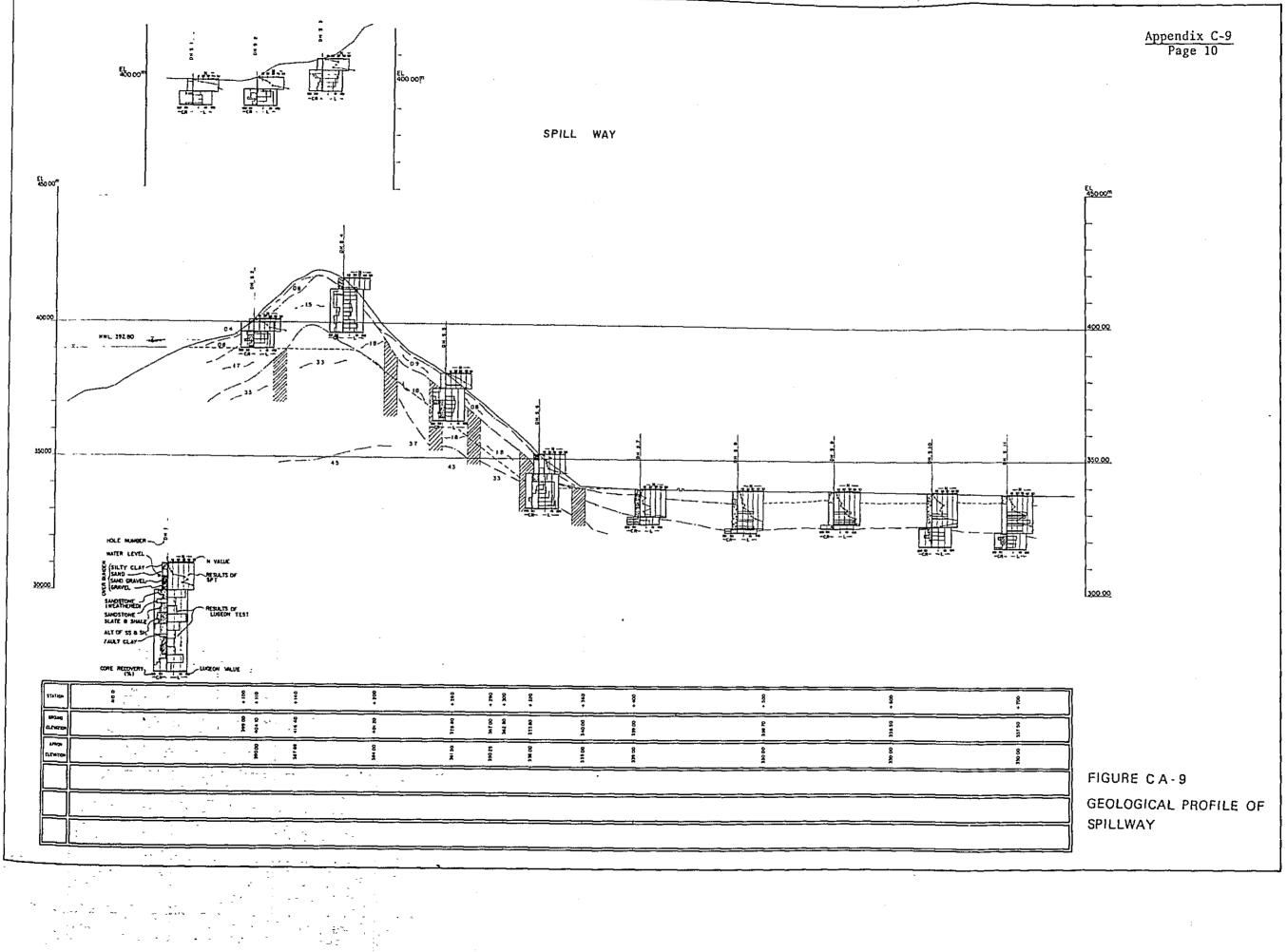
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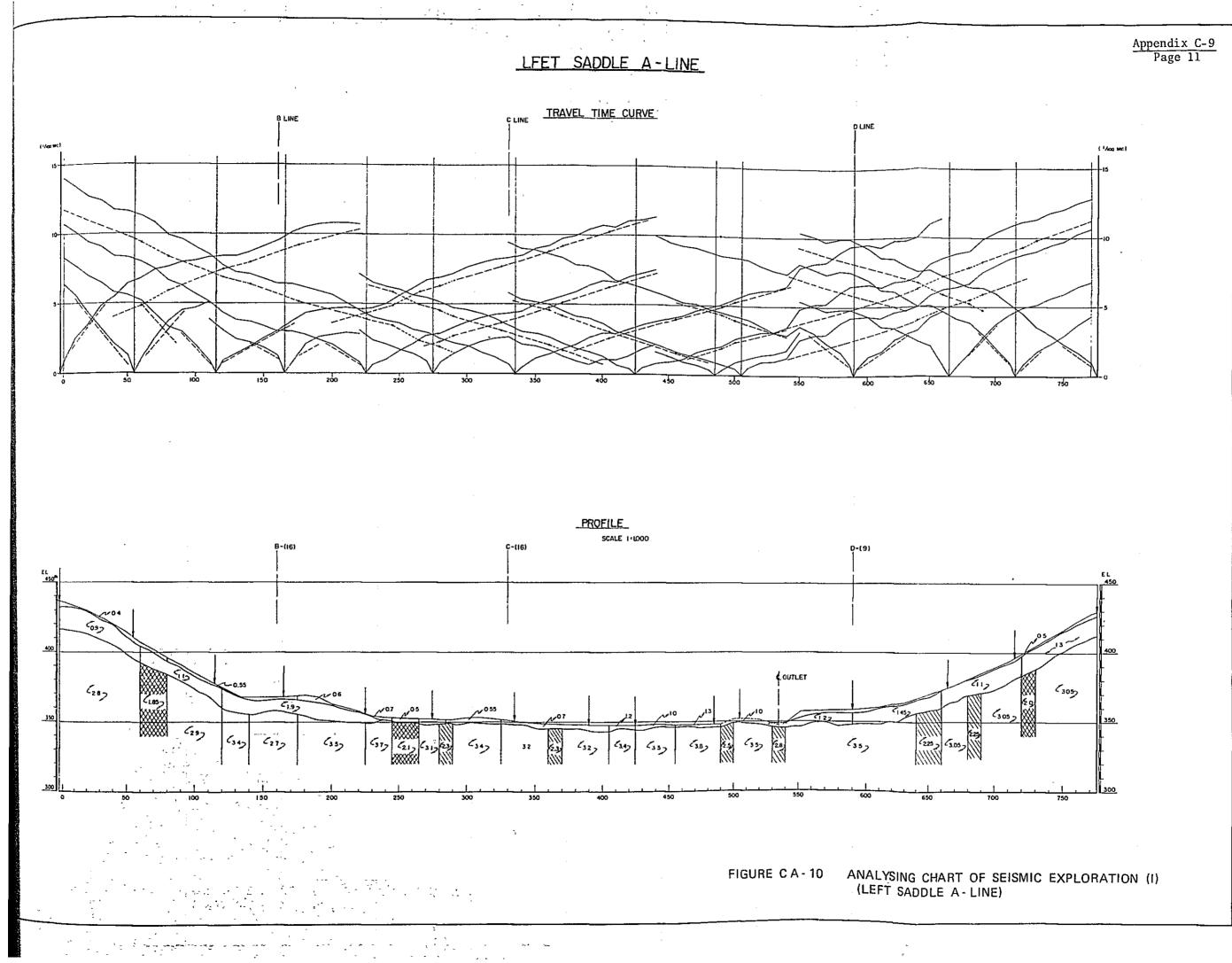
Appendix C-9 Page 9

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LEFT SADDLE B-LINE

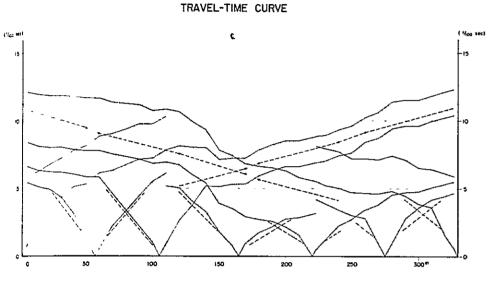
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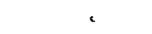
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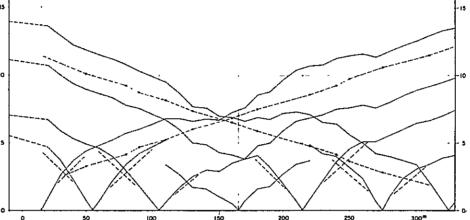
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LEFT SADDLE C-LINE

TRAVEL-TIME CURVE

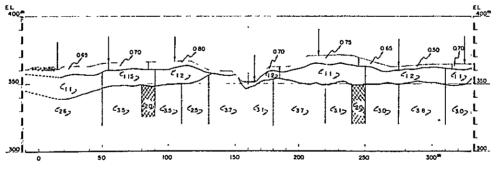








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LEFT SADDLE D-LINE

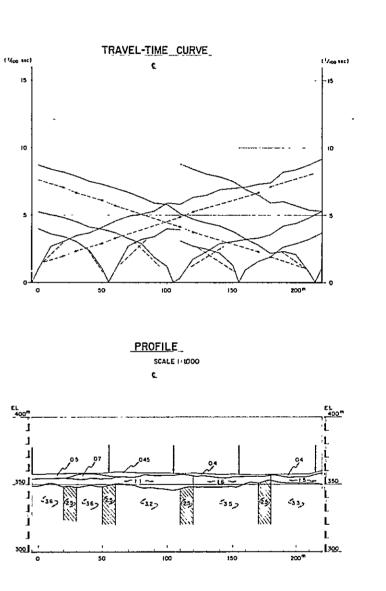
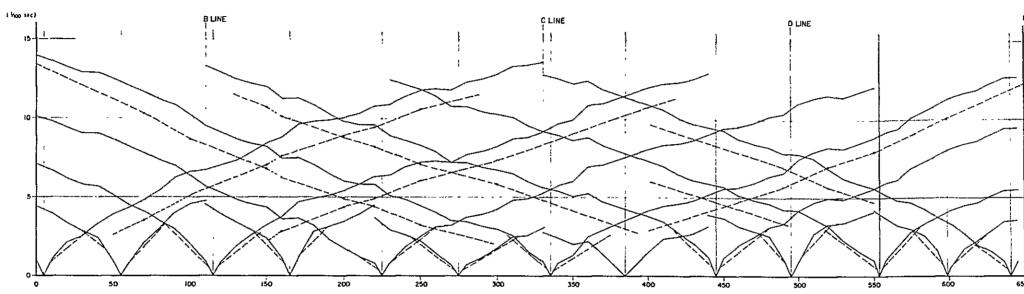


FIGURE CA-11 ANALYSING CHART OF SEISMIC EXPLORATION (II) (LEFT SADDLE B, C, D-LINES)

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TRAVEL-TIME_CURVE

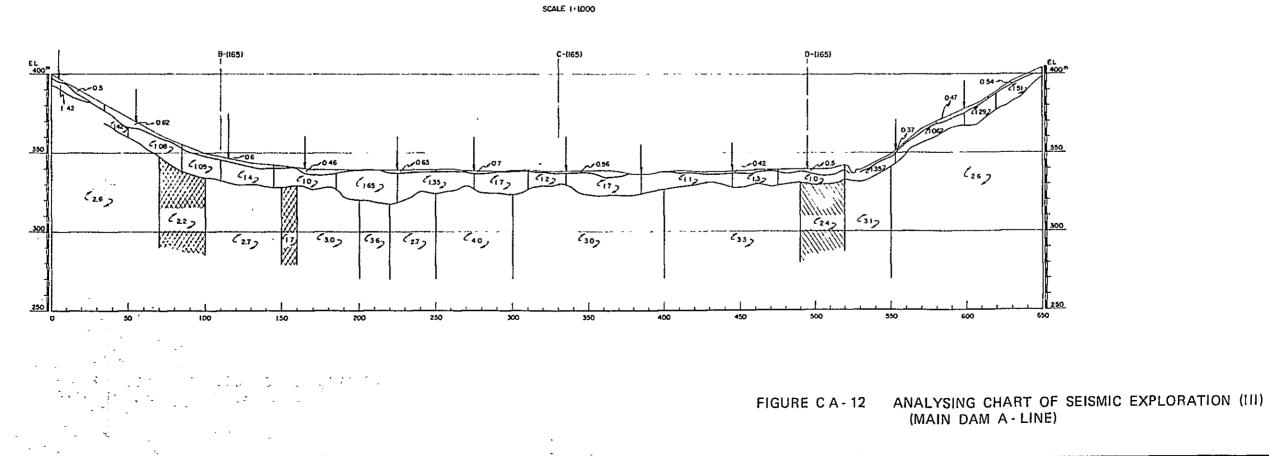
MAIN DAM A-LINE

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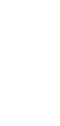


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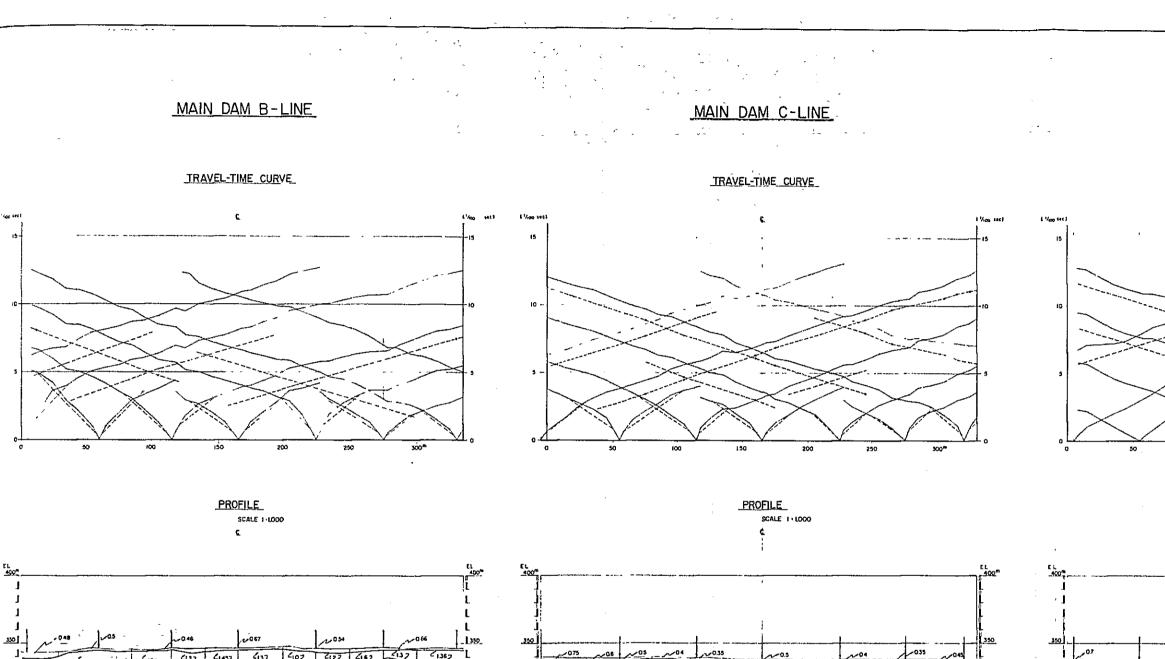
Appendix C-9 Page 13







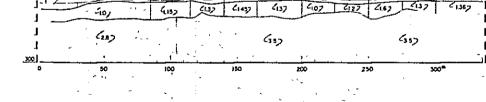




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MAIN DAM D-LINE TRAVEL-TIME CURVE C 6 1600 ANG) 300 ** 150 200 250 PROFILE SCALE 1 1000 ¢

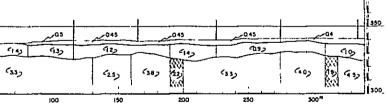
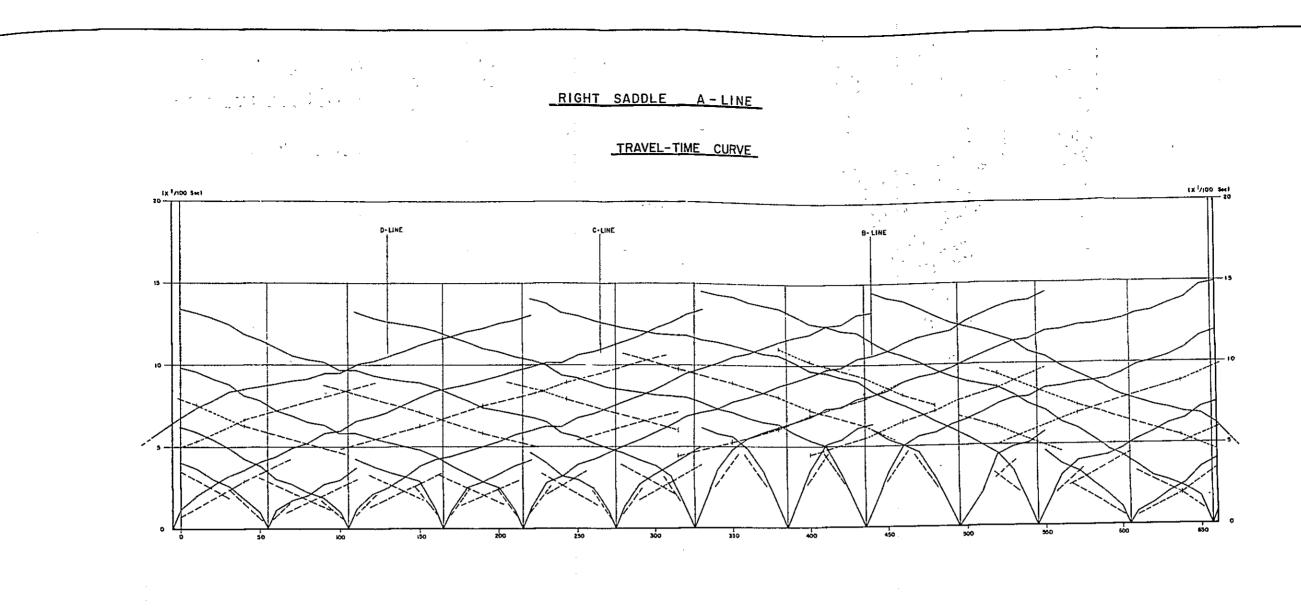
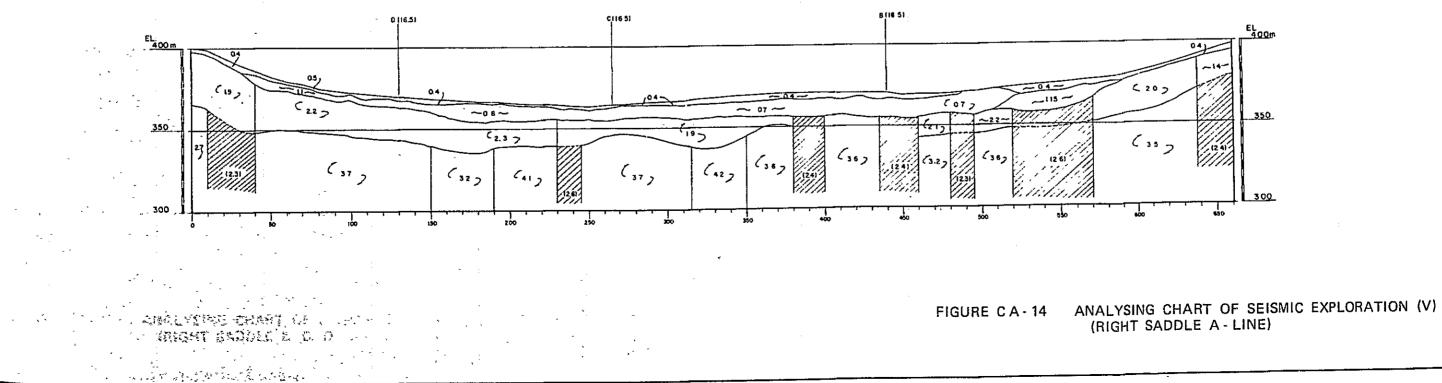


FIGURE CA-13 ANALYSING CHART OF SEISMIC EXPLORATION (IV) (MAIN DAM B, C, D-LINES)

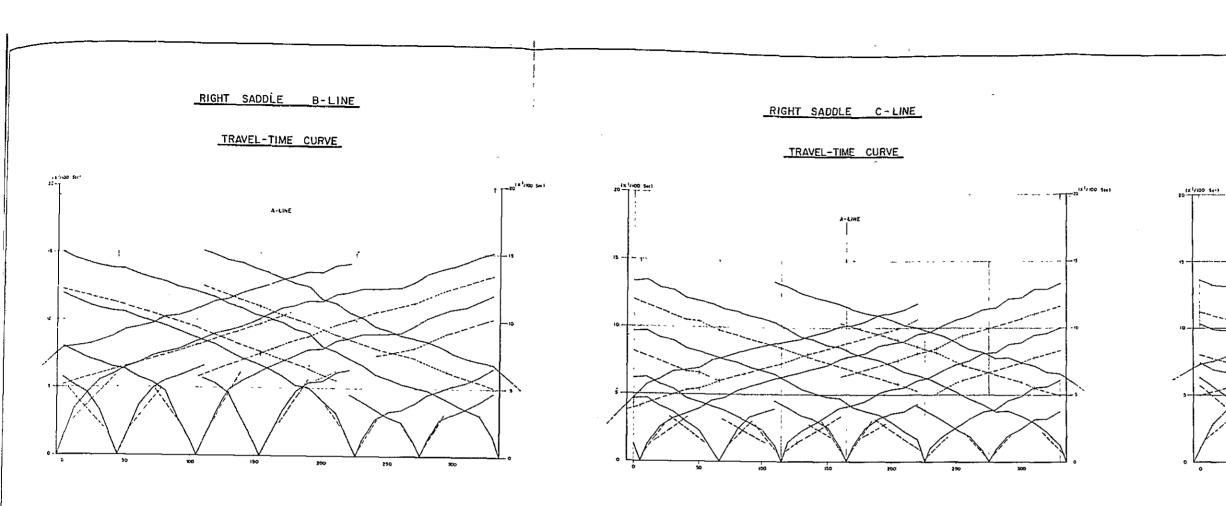


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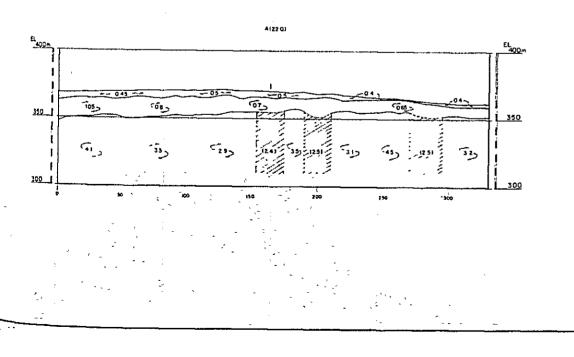


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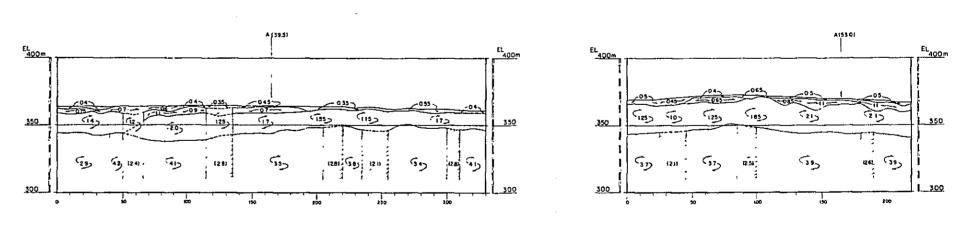
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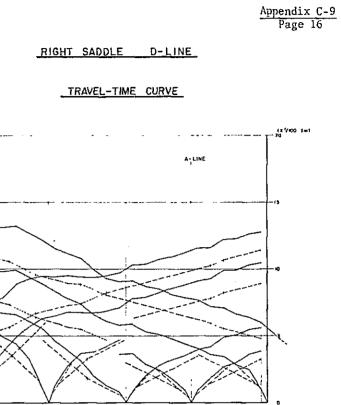






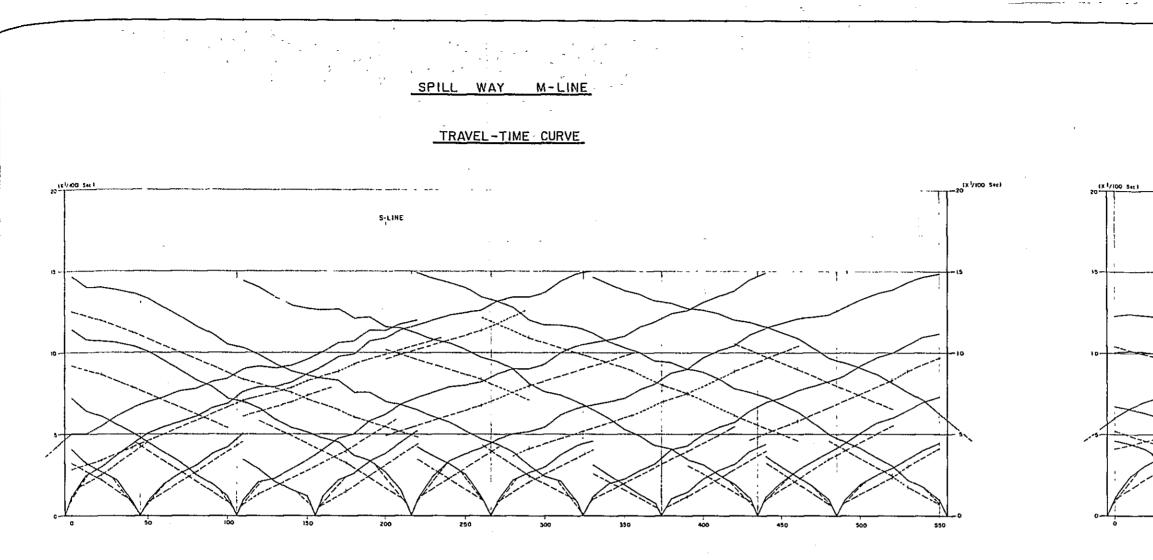




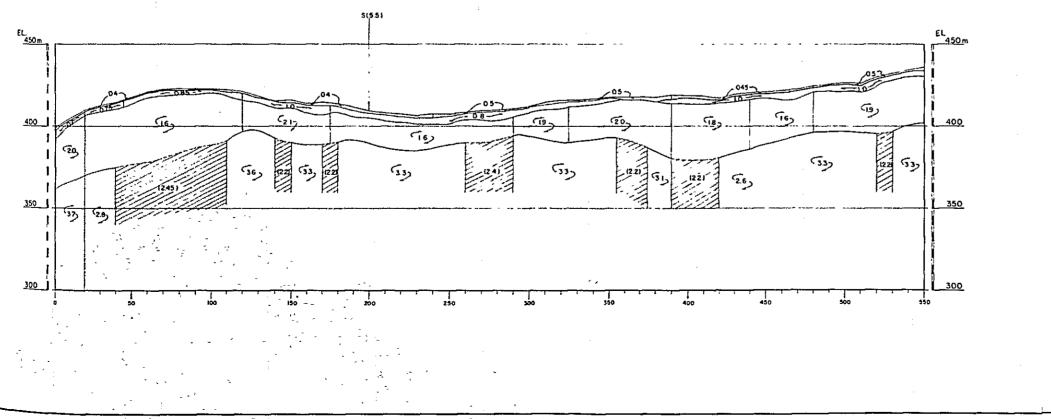


PROFILE

FIGURE CA - 15 ANALYSING CHART OF SEISMIC EXPLORATION (VI) (RIGHT SADDLE B, C, D - LINES)

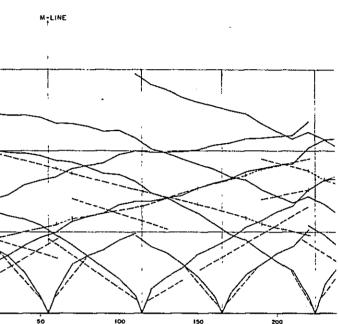


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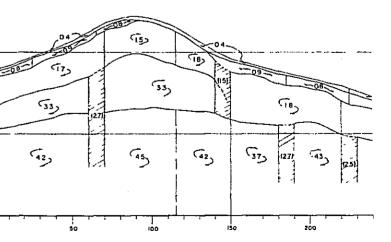


SPILL WAY S-LINE

TRAVEL-TIME CURVE



PROFILE



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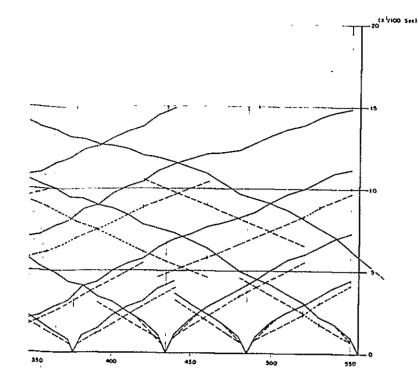
SPILL WAY S-LINE

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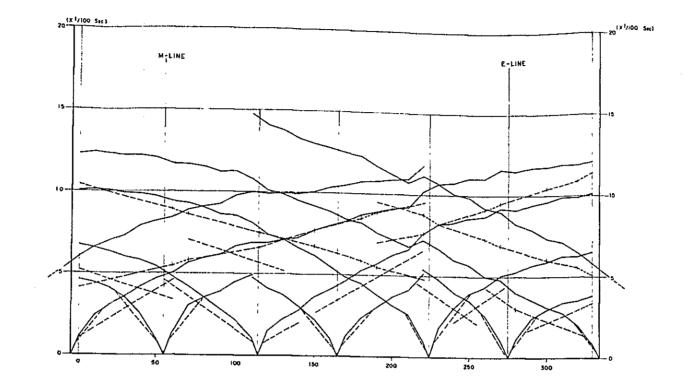
TRAVEL-TIME CURVE

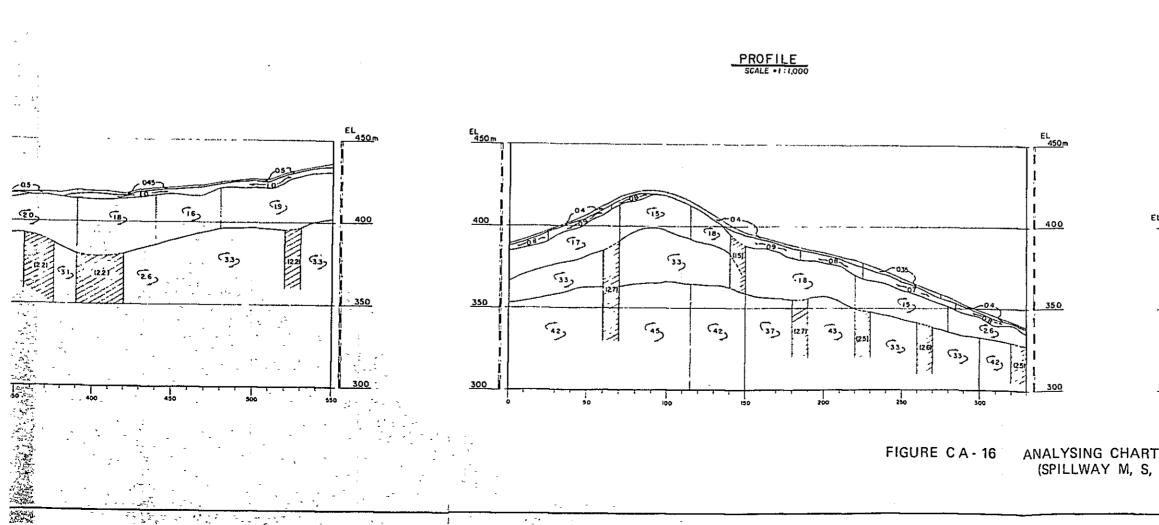


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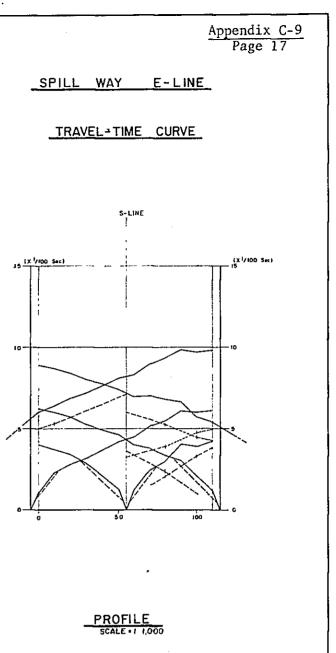
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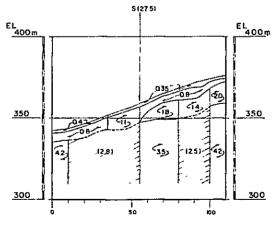
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ANALYSING CHART OF SEISMIC EXPLORATION (VII) (SPILLWAY M, S, E - LINES)