ANNEX II

DAM AND RESERVOIR



ANNEX III DAM AND RESERVOIR

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3.1 SELECTION OF DAM SITE

To exploit the water resources endowed in the watershed to the maximum extent, the dam site is proposed to be selected close to the debouchment of the tributary so far as the topography and the geology of foundation are allowable, so as to get the largest reservoir capacity.

Among four proposed storage dams i.e. Huai Saduang Yai, Huai Khon Kaen, Huai Yai and Khlong Chaliang Lab, the Huai Yai dam site is proposed to be shifted to about 450 m upstream from the original site in due consideration of topography, geology and reservoir planning. The original site is located at the debouchment of the Huai Yai and geologically, is unfavourable for dam construction, since stable foundation is not obtainable unless an enormous expenditure is practiced for the foundation improvement. In addition, the embankment volume at the original site is roughly estimated at about 5 times larger than the alternative site on the condition of same reservoir capacity. No drilling data is available at the alternative site at the present stage. The geological conditions in this site are roughly studied through the site inspection as mentioned in Annex I "Geology".

3.2 TOPOGRAPHY OF PROPOSED DAM SITE

(1) Huai Saduang Yai Dam

The proposed Huai Saduang Yai dam is located at about 17 km northeastward of the Lom Sak municipality, in latitude 16°53'37" north and longitude 101°20'53" east. The Huai Saduang Yai, small tributary of the Pasak river, originates in the Mt. Phykok, Pnu and Bukpaen of about 700 m or so in altitude, and drifts down from east to west to join the main reach of the Pasak river.

The total watershed of this tributary extends over about 96 km² at the proposed dam site which is located at about 1.5 km upstream from the confluence of the mainstream of the Pasak river. The river is relatively meandering and river channel totals about 35 km stretching from its origin to dam site. The mean basin width is estimated at about 2.7 km.

The gorge at the proposed dam site is relatively symmetric, having a steep slope of 25°C at its right abutment and 20°C at its left abutment. The lowest altitude of riverbed at the dam axis is about 164.0 m above MSL. At this dam site, there extend a relatively lower saddle from the left abutment with an altitude of about 195.0 m above MSL. The altitude of this saddle constitutes a decisive factor in determining the proposed dam crest. The proposed dam site has about 16 of the width-to-height ratio of dam and is blessed with much favourable topographic condition for construction of fill dam.

(2) Huai Khon Kaen Dam

The Huai Khon Kaen dam is proposed to be constructed at about 1.5 km eastward of the Wang Khon Du village in the Lom Sak district, in latitude 16°49'31" north and longitude 101°22'11" east. The Huai Khon Kaen, the largest tributary among the four selected water sources, originates in the ranges of Mt. Huai Koh, Huai Hi, Pu Mok, Pu Nam Rin, Pa Lob, etc.

The watershed of the tributary is located at due south of the Huai Saduang Yai watershed, extending to about 322 km² at the proposed dam site which is proposed at about 19.5 km upstream from the confluence of the Pasak river. The river channel totals about 53 km stretching out from its origin to the proposed dam site. The altitude in the watershed ranges from 1,110 m to 170 m above MSL.

The topography of the proposed dam site comprises one dale and two saddles by two humps. The existing river channel is meandering toward the left-hand dale. The left and right-hand saddles are likely to be created by the fluvial action of the old river channels. The altitude of the left and right saddle is 212.0 m 213.0 m above MSL, respectively. In addition, there exist a small saddle with the altitude of 214.0 m above MSL connecting with the right abutment. A small rivulet is located adjacent to the right abutment. The proposed dam site givesabout 9 of the width-to-hight ratio of dam at the main dale.

(3) Huai Yai Dam

The proposed dam site is located at about 25 km upstream of the Huai Yai from the confluence of the Pasak river, in latitude 16°28'57" north and longitude 101°19'09" east. The Huai Yai originates in the ranges of Mt. Hingumn, Ponthong, Suiroi, Saliang Tatard, etc. of about 1,200 m above MSL in altitude and drifts down about 47 km from northeast to southward joining many small tributaries, and debouches into the main reach of the Pasak river in the vicinity of the Phetchabun municipality.

The watershed of the tributary is estimated at about 78.0 km² at the dam site. The river channel totals about 22 km stretching from its origin to dam site. The lowest altitude of riverbed at the dam axis is about 185 m above MSL.

The dam site is proposed to be shifted to about 450 m upstream from the original site which is located at the debouchment of the Huai Yai. The shifted dam site has the narrowest gorge in the Huai Yai valley. The right abutment is relatively gently slanting with a slope of 15° and the left abutment is steeply slanting with a slope steeper than 25°. At the dam site, there exist a saddle of about 250 m upstream from the right abutment of dam axis. The gorge at the proposed site is about 100 m wide at the riverbed including lower terraces, and gives about 10 of the width—to—height ratio of dam which is blessed with much favourable topographic condition for construction of fill dam.

(4) Khlong Chaliang Lab Dam

The Khlong Chaliang Lab dam is proposed at about 12 km due east of the Phetchabun municipality, in latitude 16°24'35" north and longitude 101°17'24" east. The Khlong Chaliang Lab originates in the ranges of Mt. None Yang, Huai Rong, None Sra, Ta Boh, etc. of about 1,300 m in altitude.

The watershed of this tributary is located in due south of the Huai Yai watershed, extending to about 77 km² at the dam site. The river channel of about 54 km stretches out its origin to the confluence with the Pasak river; about 26 km from its origin to the proposed dam site.

The dam site is proposed close to the debouchment of the Khlong Chaliang Lab. The gorge at the proposed dam site is relatively V-shaped. Its right abutment is slanting with a slope of about 25° and the left abutment is more steeply slanting with a slope of about 30°. The maximum altitude of ridge extending from the right abutment is about 200 m above MSL. The altitude of this ridge constitutes a decisive factor for determining the height of dam. The riverbed is about 20 m wide and the lowest altitude of riverbed is about 180 m above MSL at the dam axis. The gorge at the proposed dam site gives about 10 of the width-to-height ratio of dam which is much favourable for construction of fill dam.

3.3 OPTIMUM DEVELOPMENT OF SCALE

The optimum scale of the storage dam is determined in due consideration of the following conditions at the dam site;

- (i) Topography,
- (ii) Geology,
- (iii) Storage capacity,
- (iv) Stability of dam body based on the embankment materials available in the vicinity of dam site, and
- (v) Disposition of appurtenant structures.

(1) Huai Saduang Yai Dam

As described in the previous paragraph, there exist a lower saddle with an altitude of about 195.0 m above MSL at the left abutment of the dam site. This saddle constitutes a decisive factor in determining the proposed dam crest in due consideration of the stability of dam embankment and the storage efficiency of

the reservoir. The relation between the dam height, and embankment volume and reservoir capacity is roughly estimated based on the topographic maps of the scale 1:4,000 and 1:50,000, and the result indicates that the dam crest elevation is to be 191.0 m above MSL having the best storage efficiency as shown below.

1	2	3	4	5
Dam Height	Crest Elevation	Embankment Volume	Storage Capacity	Storage Efficiency
(m)	(m)	$(x10^3m^3)$	$(x10^3 m^3)$	4/3
30.50	191.00	668	15,000	22.5
34.50	195.00	943	21,000	22.3
39.50	200.00	1,503	32,000	21.3
44.50	205.00	2,286	46,500	20.3
49.50	210.00	3,243	62,000	19.1

From the viewpoint of dam stability, it is recommended that the toe of upstream embankment is put in the convex saddle extending over about 60 m upstream from the left abutment at the dam site.

Based on the above reasons, the dam crest elevation is proposed to be 191.0 m above MSL. The storage capacity of the reservoir is calculated as shown in Table 3.1 and the elevation-storage and elevation-area curves are developed as given in Fig. 3.1. The total storage capacity at the full water level is estimated at about 15 MCM and the useful storage capacity is calculated to be about 14.04 MCM by deducting the dead storage capacity from the total storage capacity. The dead storage capacity is estimated on the assumption that the sedimentation volume of the watershed is 200 m³/km²/year and the project life is 50 years.

(2) Huai Khon Kaen Dam

The topography of the proposed dam site comprises one dale and two saddles by two humps. For the stability of dam, it would be better that the embankment is separated by two humps in due consideration of the unequal settlement of the dam body, but in this case the storage capacity becomes to be decreased. The proposed dam crest is determined to be 216.0 m above MSL based on the following considerations.

- The dam height is proposed to be high as far as possible so as to have large storage capacity in the reservoir,
- (ii) No rock materials for construction of high dam can be obtained in the vicinity of the dam site,
- (iii) The appurtenant structures must be disposed safely and economically, and
 - (iv) The relation between dam height and storage efficiency is roughly estimated as follows.

1	2	3	4	5
Dam Height (m)	Crest Elevation (m)	Embankment Volume (x10 ³ m ³)	Storage Capacity (x10 ³ m ³)	Storage Efficiency 4 / 3
52.00	216.00	2,729	28,000	10.3
56.00	220.00	4,473	35,000	7.8
61.00	225.00	7,171	44,000	6.1
66.00	230.00	10,491	52,000	5.0
71.00	235.00	14,494	63,000	4.3

The calculation sheet of the storage capacity of the reservoir is shown in Table 3.2, and Fig. 3.2 presents the elevation-storage and elevation-area curves. The total storage capacity is estimated at about 28 MCM at the full water level and the useful storage capacity is calculated to be 24.78 MCM by deducting the dead storage capacity of 3.22 MCM.

(3) Huai Yai Dam

The proposed dam site is shifted to about 450 m upstream from the original site located at the debouchment of the Huai Yai in due consideration of topographic and geological conditions. No topographic and geological data are available at this proposed site at present. According to the site inspection, a saddle can be found at about 250 m upstream from the right abutment of dam axis and this saddle constitutes a decisive factor for determining the height of dam. An emergency spillway can be easily constructed on this saddle.

The dam crest elevation is determined to be 212.50 m above MSL based on the above reasons. The total storage capacity of the reservoir is estimated at about 7.9 MCM at the full water level and the useful storage capacity is calculated to be about 7.12 MCM by deducting the dead storage capacity of 0.78 MCM. Table 3.3 shows the storage capacity of the reservoir and Fig. 3.3 presents the elevation-storage and elevation-area curves.

(4) Khlong Chaliang Lab Dam

At the dam site, there exist a ridge extending from the right abutment. The altitude of this ridge is about 200.0 m above MSL. In view of the dam stability and storage efficiency, the dam crest elevation is proposed to be 200.0 m above MSL. The relation between dam height and storage efficiency is roughly estimated as follows.

1	2	3	4	5
Dam Height(m)	Crest Elevation (m)	Embankment Volume (x10 ³ m ³)	Storage Capacity (x10 ³ m ³)	Storage Efficiency 4 / 3
25.30	200.00	151	2,300	15.2
30.30	205.00	252	3,800	15.1
35.30	210.00	385	5,700	14.8
40.30	215.00	553	7,800	14.1
45.30	220.00	1,364	10,000	7.3

The total storage capacity at the full water level is estimated at about 2.3 MCM and the useful storage capacity be at about 1.53 MCM by deducting the dead storage capacity of 0.77 MCM as shown in Table 3.4 and Fig. 3.4.

3.4 PRELIMINARY DESIGN

3.4.1 Dam Type

Various dams might be technically eligible at four proposed dam sites. But concrete dam would be left out of consideration in view of project economy. The rockfill dam type which offer relatively less embankment compared with earthfill dam, seems to be suitable for each dam site. Actually however, it is unfavourable since the informations obtained from field investigations clarify that no quarry for the rockfill dam can be found within the economic hauling distance.

Afterall, earthfill dam would be proposed for each site.

According to the outcomes of material survey, top soils in and around sites comprise various fine soils, such as silty sand, clayey sand, sandy clay, silty clay, etc. These materials are much suitable for the impervious core. Coarse materials, essential for semi-pervious or pervious zone, are insufficient in the vicinity of each site from the result of field investigations at present but these materials might be obtainable in hillside and/or layer under the topsoils.

In the view of the embankment materials available in and around dam sites mentioned above, zoned type of earthfill dam which is mechanically more stable than homogeneous type, would be recommended in this study. The proposed zone type would be composed of four zones i.e. impervious zone, semi-pervious or pervious zone, fillter zone provided behind impervious zone and rip-rap for protection of upstream slope. The rip-rap materials are obtainable from the lime stone quarry site being located at Tham Kao Phra about 15 km southwest of the Lom Sak Municipality. However, this quarry site is far from each dam site. Further investigations should be made at the next stage to quarry out rip-rap materials in the vicinity of each dam site.

3.4.2 Preliminary Design of Dam

The dam crest elevation is determined by adding overflow depth of the spillway plus freeboard to the full water level in the reservoir as shown below.

Name of Dam	Full Water Level	Overflow Depth of Spillway	Freeboard	Dam Crest
Huai Saduang Yai	187.50 ^m	2.15 ^m	1.35 ^m	191.00 ^m
Huai Khon Kaen	211.50	3.00	1.50	216.00
Huai Yai	209.00	2.00	1.50	212.50
Khlong Chaliang Lab	196.50	2.00	1.50	200.00

The width of the dam crest is proposed to be 7.0 to 10.0 m to provide a function of inspection road according to the dam height.

Name of Dam	Dam Height	Crest Width
Huai Saduang Yai	30.5 ^m	8.0 ^m
Huai Khon Kaen	52.0	10.0
Huai Yai	32.5	8.0
Khlong Chaliang Lab	25.3	7.0

The slope of the upstream face is designed to be 1.0 vertical to 3.0 horizontal, the downstream face be 1.0 vertical to 2.5 horizontal. The central impervious core is placed with the upstream and downstream slopes of 1.0 vertical to 0.25 horizontal, having the top width of 6.0 m. The filter for releasing the seepage water through the central impervious core is proposed to be placed along the downstream slope of the impervious zone with 2.0 m width and be connected with the downstream toe drain. Sand and river gravel would be used for the filter materials. Rip-rap is provided on the surface of upstream slope with 1.0 m width to prevent the upstream face from eroding due to variation of the reservoir water level. Behind the rip-rap, the filter is proposed to be set above the dead water level with 0.5 m in width as the transition zone between rip-rap and semi-pervious or pervious zone.

The key trench is provided at the center of the dam with the cut slope of 1.0 to 1.0 on both sides down to the bed rock, and it must be replaced with the impervious core. The foundation treatment by grouting would be made as required according to rock quality, permeability and height of dam. Based on the result of field investigations at this stage, the proposed depth of curtain grouting from the bottom of key trench is shown as follows.

Name of Dam	Depth of Curtain Grouting
Huai Saduang Yai	11.0 ^m - 18.0 ^m
Huai Khon Kaen	7.0 - 20.0
Huai Yai	10.0 - 15.0
Khlong Chaliang Lab	6.0 - 11.0

3.4.3 Preliminary Design of Appurtenant Structures

(1) Service Spillway

The service spillway is designed accounting for the flood discharge with the return period of 100 years. The side channel spillway would be proposed without any control gate in due consideration of the topographic condition at each dam site.

(a) Huai Saduang Yai Dam

The service spillway is proposed to be aligned at the right abutment. The crest length of the spillway is determined considering the flood control, even though its effect is a little, for reducing inflow peak discharge. Based on the graphic solution shown in Fig. 3.5, the outflow peak discharge is estimated as follow.

Inflow Peak Discharge	Crest Length of Spillway	Outflow Peak Discharge	Overflow Depth
268.4 m ³ /sec	70.0 m	251.0 m ³ /sec	1.56 m
	60.0	245.0	1.70
	50.0	241.0	1.90

The proposed crest length of spillway is determined to be 60.0 m in due consideration of the above result and the topographic condition at the dam site.

(b) Huai Khon Kaen Dam

The alignment of service spillway is proposed at the left abutment. In Huai Khon Kaen reservoir, the ratio of reservoir area to drainage basin is small, so the effect of flood control can not be expected. The crest length of spillway is estimated at 96.0 m as shown below.

$$B = \frac{Q}{C \cdot h^{3/2}}$$

where, B: Crest length (m)

Q : Design discharge = $697.4 \text{ m}^3/\text{sec}$

C: Coefficient of overflow = 1.84

h : Overflow depth = 2.5 m

$$\therefore B = \frac{697.4}{1.84 \times 2.5^{3/2}} = 96.0 \text{ m}$$

(c) Huai Yai Dam

The service spillway is proposed to be aligned at the left abutment. The crest length of spillway is determined to be 40.0 m based on the graphic solution of flood control shown in Fig. 3.6 and the topographic condition at the dam site.

Inflow Peak Discharge	Crest Length of Spillway	Outflow Peak Discharge	Overflow Depth
168.4 m ³ /sec	60.0 m	154.3 m ³ /sec	1.25 m
11	50.0	152.4	1.40
11	40.0	149.0	1.60

(d) Khlong Chaliang Lab Dam

The alignment of service spillway is proposed at the right abutment from the topographic condition at the dam site. The effect of flood control can not be expected as the ratio of reservoir area to drainage basin is small. The crest length of spillway is estimated at 40.0 m as shown below.

$$B = \frac{Q}{C \cdot h^{3/2}} = \frac{150.0}{1.84 \times 1.60^{3/2}} = 40.0 \text{ m}$$

(2) Emergency Spillway

The emergency spillway is designed with flood discharge of the return period of 500 years in order to protect the dam from extraordinary floods. Two types of the emergency spillway are considered structurally according to the topographic conditions. One is the side channel spillway which can be used together with the service spillway, and the other is the overflow type to be provided independently at the saddle extending from abutments.

(a) Huai Saduang Yai Dam

The saddle at the left abutment is suitable for the emergency spillway but the valley adjacent to the saddle is close to the toe of downstream embankment. The construction of emergency spillway on this saddle seems to be in danger of eroding due to overflow from the saddle. Therefore, the side channel spillway is proposed using together with the service spillway. The crest length is estimated at 30.0 m with the crest elevation of 189.2 m above MSL as shown below.

$$Q = CB_1h_1^{3/2} + CB_2h_2^{3/2}$$

where, Q: Design discharge = 354.0 m^3/sec

C : Coefficient of overflow = 1.84

 B_1 : Crest length of service spillway = 60.0 m

 h_1 : Overflow depth at service spillway = 2.15 m

 B_2 : Crest length of emergency spillway = 30.0 m

h₂: Overflow depth at emergency spillway = 0.45 m

$$\therefore$$
 Q = 1.84 x 60.0(0.45 + 1.70)^{3/2} + 1.84 x 30.0 x 0.45^{3/2}
= 364.7 m³/sec \geq 354.0 m³/sec

(b) Huai Khon Kaen Dam

The location of emergency spillway is determined at the left abutment apart from the service spillway. The overflow type is proposed according to the topographic condition. The design discharge of 940.0 m³/sec must be released by use of both service and emergency spillways. The crest length of emergency spillway is calculated to be 34.0 m as follows.

$$Q = CB_1h_1^{3/2} + CB_2h_2^{3/2}$$

where, Q: Design discharge = $940.0 \text{ m}^3/\text{sec}$

C : Coefficient of overflow = 1.84

 B_1 : Crest length of service spillway = 96.0 m

 h_1 : Overflow depth at service spillway = 3.0 m

 B_2 : Crest length of emergency spillway = 34.0 m

h₂: Overflow depth at emergency spillway = 0.50 m

 $\therefore Q = 1.84 \times 96.0 \times (0.50+2.50)^{3/2} + 1.84 \times 34.0 \times 0.50^{3/2}$ = 940.0 m³/sec

(c) Huai Yai Dam

The overflow type of emergency spillway is proposed at the saddle extending over about 250 m upstream from the right abutment. The crest length of emergency spillway is estimated at 25.0 m in due consideration of use together with the service spillway. The crest elevation is proposed to be 210.60 m above MSL.

$$Q = CB_1h_1^{3/2} + CB_2h_2^{3/2}$$

where, Q: Design discharge = 218.0 m^3/sec

C: Coefficient of overflow = 1.84

 B_1 : Crest length of service spillway = 40.0 m

h,: Overflow depth at service spillway = 2.0 m

 B_2 : Crest length of emergency spillway = 25.0 m

 h_2 : Overflow depth of emergency spillway = 0.4 m

$$\therefore Q = 1.84 \times 40.0 \times (0.4+1.6)^{3/2} + 1.84 \times 25.0 \times 0.4^{3/2}$$

= 219.8 m³/sec > 218.0 m³/sec

(d) Khlong Chaliang Lab Dam

As favourable site for emergency spillway can not be found at the dam site, the service spillway would be utilized also as the emergency one. The hydraulic calculation is shown as follows.

$$Q = CBh^{3/2}$$

where, Q : Design discharge = $193.0 \text{ m}^3/\text{sec}$

C : Coefficient of overflow = 1.84

B: Crest length of spillway = 40.0 m

h : Overflow depth at spillway = 2.0 m

$$\therefore Q = 1.84 \times 40.0 \times 2.0^{3/2} = 208.2 \text{ m}^3/\text{sec} > 193.0 \text{ m}^3/\text{sec}$$

(3) Outlet Structure

The outlet structure comprises intake tower, water way and access bridge. The intake tower of reinforced concrete would be proposed, installed with the control gates. Water taken from the regulating gate is conveyed to the downstream through the water way. The section of water way is proposed to be circular one which is strong structurally, with the diameter of 2.0 m for operation and maintenance. The access bridge is provided to the intake tower in order to operate regulating gates. This bridge would be made by steel. The pier of bridge must be constructed on the natural ground but not on the embankment.

Based on the above descriptions, main features of four proposed dams and reservoirs are summarized in Table 3.5. General layout of each dam is shown in Dwg. No. 1-5.

3.5 WORK QUANTITY

The work quantity of each dam is summarized as shown below based on the preliminary design mentioned in the previous paragraph.

				Name o	f Dam	
	Description	Hua Sad	ai Ruang Yai	Huai Khon Kaen	Huai Yai	Khlong Chaliang Lab
			idang tur	Idion Raen		Charrang bab
	Excavation (m ³)		81,000	288,000	45,000	18,000
(2)	2	e	67,700	2,728,600	327,700	151,200
(2)	Impervious zone	_	118,000	428,000	56,000	26,000
	Pervious or semi		110,000	420,000	30,000	20,000
	pervious zone (m	3) 5	503,000	2,137,000	244,000	111,000
	Rip-rap (Rock) (m	3)	20,000	76,000	11,000	5,000
	Filter (m	3)	24,000	79,000	14,000	7,000
	Sand and gravel river bed (m	_	800	3,300	1,200	1,000
	Rock at end of to of downstream (m	_	200	700	300	400
	Laterite on cres	t(m ³)	1,700	4,600	1,200	800
(3)	Sodding (m	2)	24,000	69,000	13,000	7,000
(4)	Curtain grouting	(m)	450	1,000	300	250
II.	Spillway					
(1)	Excavation (m	³)	130,000	159,000	72,000	41,000
(2)	Backfill (m	3)	18,000	39,000	17,000	15,000
(3)	Concrete (m	³)	10,000	20,000	6,000	6,000
III.	Outlet Structure		*			
(1)	Height of intake		28	43	28.	5 24.5
(2)	Length of water way (m	ı)	155	300	220	135
(3)	Length of access bridge (m		7 5	105	105	87

3.6 CONSTRUCTION TIME SCHEDULE

The construction time schedule for each dam is determined according to the following assumptions.

(1) Workable Days

- (a) Embankment works 170 days per year
 - (i) Saturday, Sunday and national holidays are excluded from the workable days.
 - (ii) Rate of workable days except item (i) is as follows.

Wet season - 50% Dry season - 80%

- (b) Excavation and concrete works 210 days per year
 - (i) Saturday, Sunday and national holidays are excluded from the workable days.
 - (ii) Rate of workable days except item (i) is assumed to be 80% through a year.

(2) Workable Capacity

In due consideration of dam scale and site condition, the workable capacity is roughly decided as shown below.

Embankment works - $1,500 - 2,000 \text{ m}^3/\text{day}$ Excavation works - $800 - 1,000 \text{ m}^3/\text{day}$ Concrete works - $80 - 100 \text{ m}^3/\text{day}$

Based on the above assumptions, the construction time schedules for respective dams are proposed as follows.

(1) Huai Saduang Yai Dam

Work Item		Construc	tion Year	
work Item	lst	2nd	3rd	4th
Temporary Diversion Work				
Dam				
Excavation				
Curtain grouting	,			<u> </u>
Embankment	_			
Spillway				
Excavation				
Backfill				
Concrete works				
Outlet Structure			1	

(2) Huai Khon Kaen Dam

Work Item		Cons	truction \	ear!	
	lst	2nd	3rd	4th	5th
Temporary Diversion Work				:	
Dam	: !				
Excavation					
Curtain grouting					
Embankment					
Spillway					
Excavation	•				
Backfill	<u> </u> 				
Concrete works			Samuel and C		
Outlet Structure					

. (3) Huai Yai Dam

ttl- Them	Con	struction	Year ,,
Work Item	. lst `	2nd	3rd
Temporary Diversion Work	5		_
Dam Excavation Curtain grouting Embankment			
Spillway Excavation Backfill Concrete works			
Outlet Structure		•	

(4) Khlong Chaliang Lab Dam

771- 74	Cons	truction	Year
Work Item	lst	2nd	3rd
Temporary Diversion Work			
Dam			
Excavation	-		
Curtain grouting	-	_	
Embankment	-		
Spillway			
Excavation	-	•	
Backfill		_	_
Concrete works			<u> </u>
Outlet Structure		-	

Table 3.1 Storage Capacity of Huai Saduang Yai Dam

Mean Area Capacity Total Capacity Remark $(m2)$ $(m3)$ $(m3)$		32,250 193,500 193,500]	130,350 391,050 584,550 by topographic map of scale	299,700 599,400 1,183,950 (1:4,000	547,150 1,641,450 2,825,400	787,050 1,574,100 4,399,500]	
			130,350	299,	547,	787,	1.362.750
Area (m ²)	0	64,500	196,200	403,200	001,169	883,000	1,842,500
Elevation	EL 164.0	170.0	173.0	175.0	178.0	180.0	190.0

Table 3.2 Storage Capacity of Huai Khon Kaen Dam

Elevation	Area (m ²)	Mean Area (m ²)	Capacity (m ³)	Total Capacity (m ³)	Remark
EL 171.5	0				
175.0	41,600	20,800	72,800	72,800	· ·
180.0	288,000	164,800	824,000	896,800	and the same and
185.0	487,100	387,550	1,937,750	2,834,550	•
190.0	671,800	579,450	2,897,250	5,731,880	
195.0	830,900	751,350	3,756,750	9,488,550	by topographic map of scale
200.0	1,006,500	918,700	4,593,500	14,082,050	π:4,000
205.0	1,158,900	1,082,700	5,413,500	19,495,550	
210.0	1,385,600	1,272,250	6,361,250	25,856,800	
215.0	1,555,200	1,470,400	7,352,000	33,208,800	
220.0	1,744,000	1,649,600	8,248,000	41,456,800	

Table 3.3 Storage Capacity of Huai Yai Dam

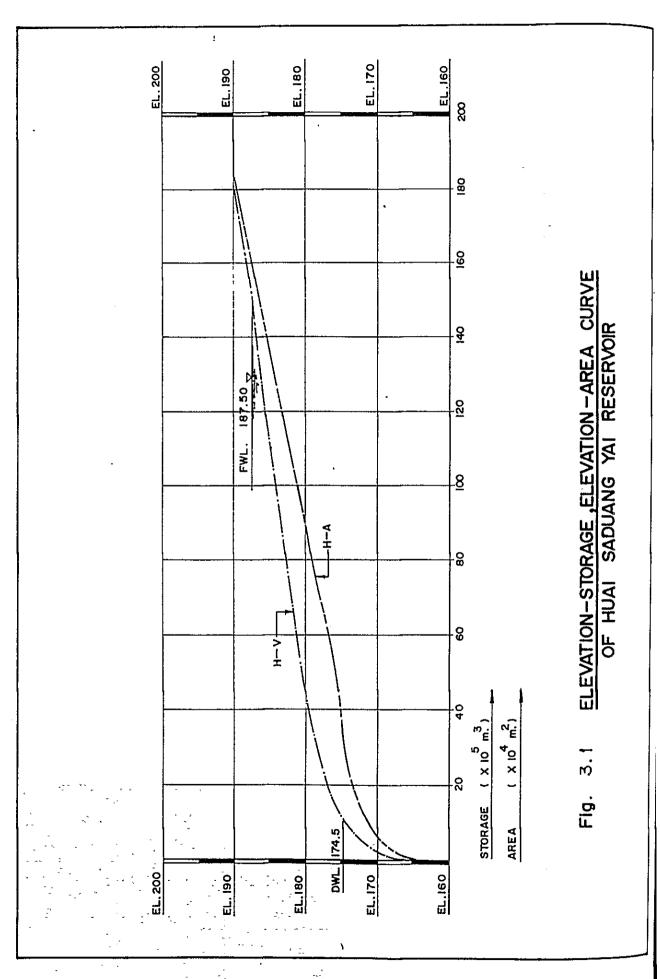
Elevation	Area (m2)	Mean Area (m ²)	Capacity (m ³)	Total Capacity (m3)	Remark
EL 185.5	0				
188.0	24,200	12,100	30,250	30,250	
190.0	41,600	32,900	65,800	96,050	
193.0	115,200	78,400	235,200	331,250	
195.0	190,700	152,950	305,900	637,150	by topographic map of scale
198.0	321,900	256,300	768,900	1,406,050	1:4,000
200.0	418,900	370,400	740,800	2,146,850	
203.0	558,400	488,650	1,465,950	3,612,800	
205.0	688,000	623,200	1,246,400	4,859,200	
203.0	882,400	785,200	2,335,600	7,214,800	_
210.0	1,012,000	947,200	1,894,400	9,109,200	
213.0	1,206,400	1,109,200	3,327,600	12,436,800	calculated by proportion
215.0	1,336,000	1,271,200	2,542,400	14,979,200	
218.0	1,530,400	1,433,200	4,299,600	19,278,800	

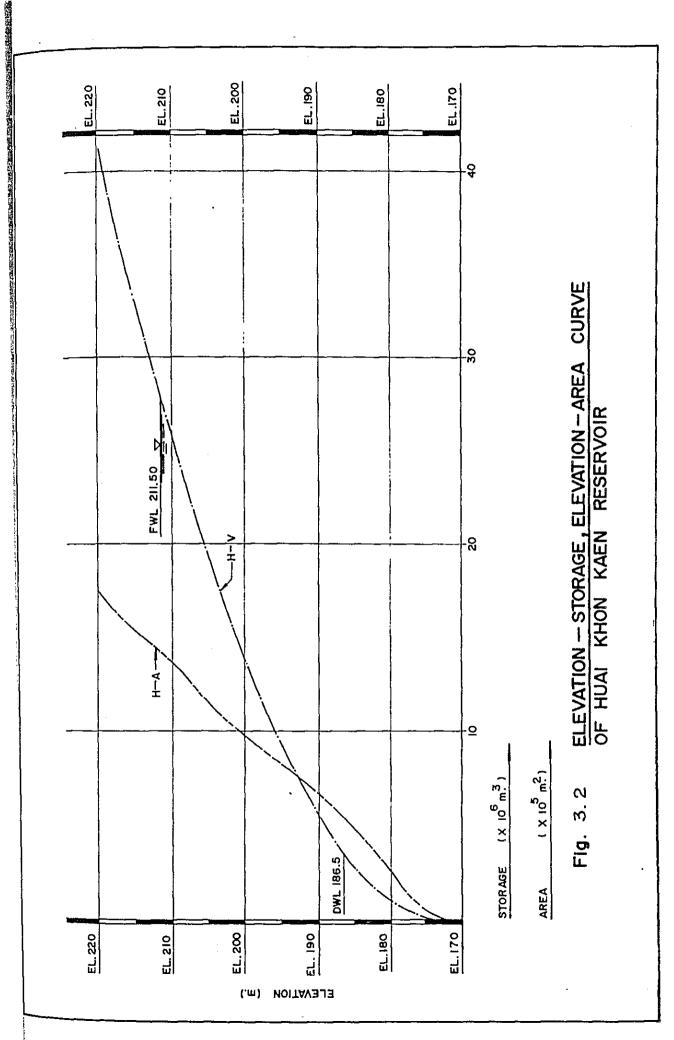
Table 3.4 Storage Capacity of Khlong Chaliang Lab Dam

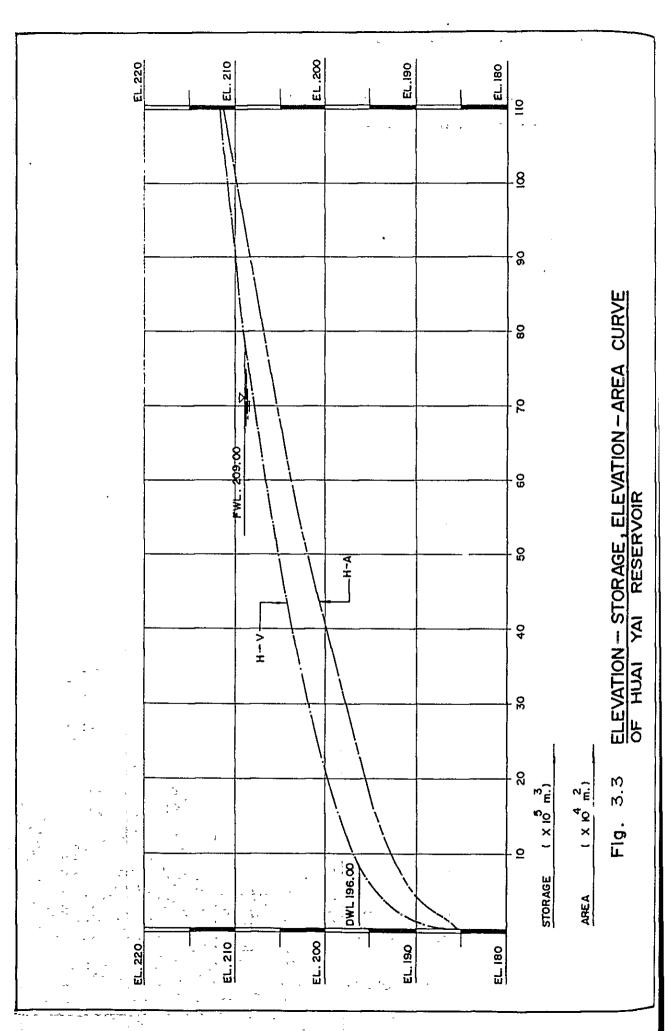
Capacity Total Capacity Remark (m^3)		37,950 37,950	100,900 138,850	331,800 470,650	307,200 777,850 \downarrow by topographic map of scale	581,100 1,358,950 1:4,000	505,800 1,864,750	1,606,500 3,471,250)
Mean Area (m ²)		12,650	50,450	110,600	153,600	193,700	252,900	321,300
Area (m ²)	0	25,300	75,600	145,600	161,600	225,800	280,000	362,600
Elevation	EL 180.0	183.0	185.0	188.0	190.0	193.0	195.0	200.0

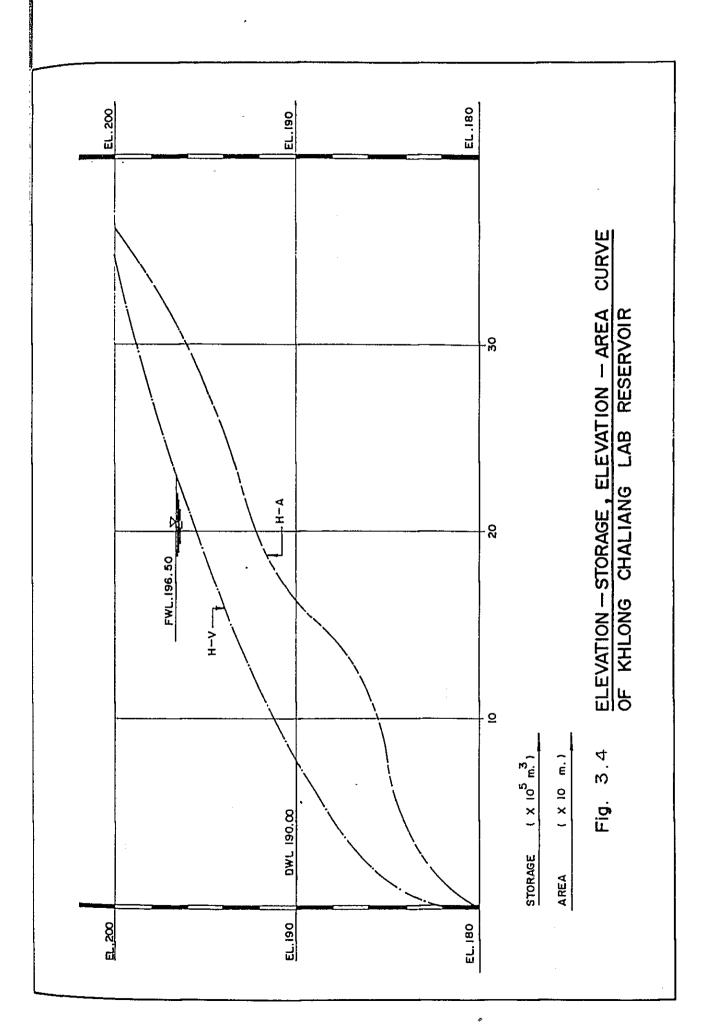
Table 3.5 Main Features of Four Storage Dams

				Name	of Dam	
	Description	•	Huai	Huai		Khlong
	pescription		Saduang	Khon	Huai Yai	-
			Yai	Kaen		Lab
I. <u>R</u>	teservoir				- · · · · · · · · · · · · · · · · · · ·	
(1)	Drainage Area (kr	ղ2)	96	322	78	7 7
(2)	Total Storage			· · · · ·	70	,,
	Capacity $(x10^3 m^3)$	١	15,000	28,000	7,900	2,300
(3)	Dead Storage					
743	Capacity (x10 ³ m ³)	ı	960	3,220	780	770
(4)	Useful Storage		7.4.040			
151	Capacity (x10 ³ m ³)		14,040	24,780	7,120	1,530
	High Water Level		189.65	214.50	211.00	198.50
	Full Water Level	•	187.50	211.50	209.00	196.50
	Dead Water Level		174.50	186.50	196.00	190.00
(8)	Reservoir Area at Full Water Level		1.00			
	rull water rever	(Km²)	1.60	1.44	0.95	0.31
II.	Dam					
(1)	Dam Type	zo	one earth	- zone earth-	zone earth-	zone earth-
		fj	ill dam	fill dam	fill dam	fill dam
(2)	Dam Crest Elevati	on (m)	191.00	216.00	212.50	200.00
(3)	Freeboard	(m)	1.35	1.50	1.50	1.50
	Dam Height	(m)	30.0	52.0	32.5	25.3
(5)	Dam Crest Length	(m)	413.0	912.0	280.0	207.0
(6)	Dam Crest Width	(m)	8.0	10.0	8.0	7.0
(7)	Embankment Slope					
	Upstream		1:3.0	1:3.0	1:3.0	1:3.0
	Downstream		1:2.5	1:2.5	1:2.5	1:2.5
(8)	Embankment Volume	_				
	$(x10^{3})$	m³)	667.7	2,728.6	327.7	151.2
III.	Spillway					
III-3	l. Service Spillwa	y				
	Design Discharge	_	245.0	698.0	149.0	150.0
	Type				side charrel	130.0
(3)	Crest Length	(m)	60.0	96.0		
	The state of the s	(m)	1.70	2.50	1.60	40.0 1.60
	2. Emergency Spill		1170	2.50	1.60	1.60
			•			
	Design Discharge	-		940.0	218.0	193.0
	Туре		channel		overflow	side channel
(3)	Crest Length	(m)	30.0	34.0	25.0	40.0
(4)	Overflow Depth	(m)	0.45	0.50	0.40	0.40
IV. I	ntake Structure					
(1)	Design Discharge	(m ³ /s)	2.0	6.0	1.3	0.6
(2)	Туре		einforced			
			concrete	concrete	concrete	concrete
			tower	tower	tower	tower









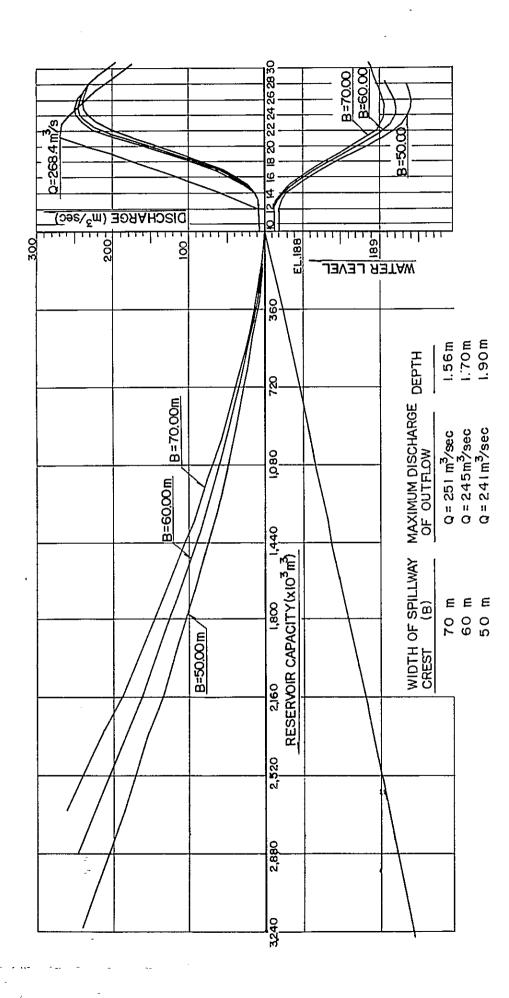
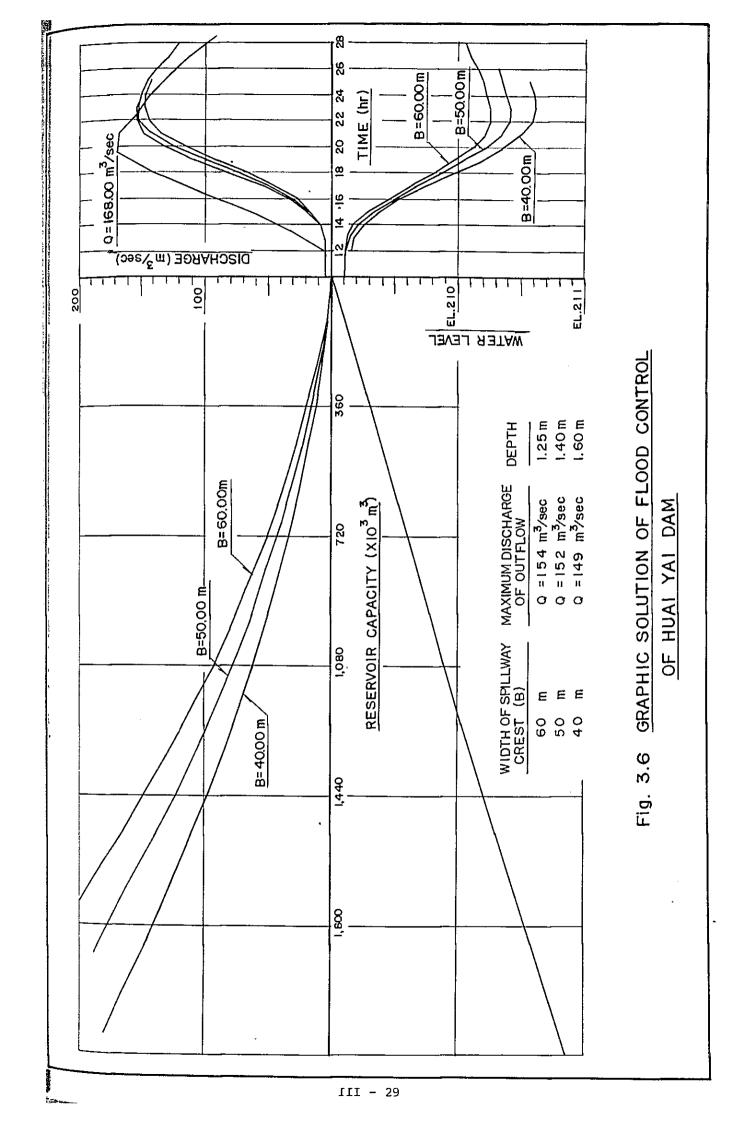
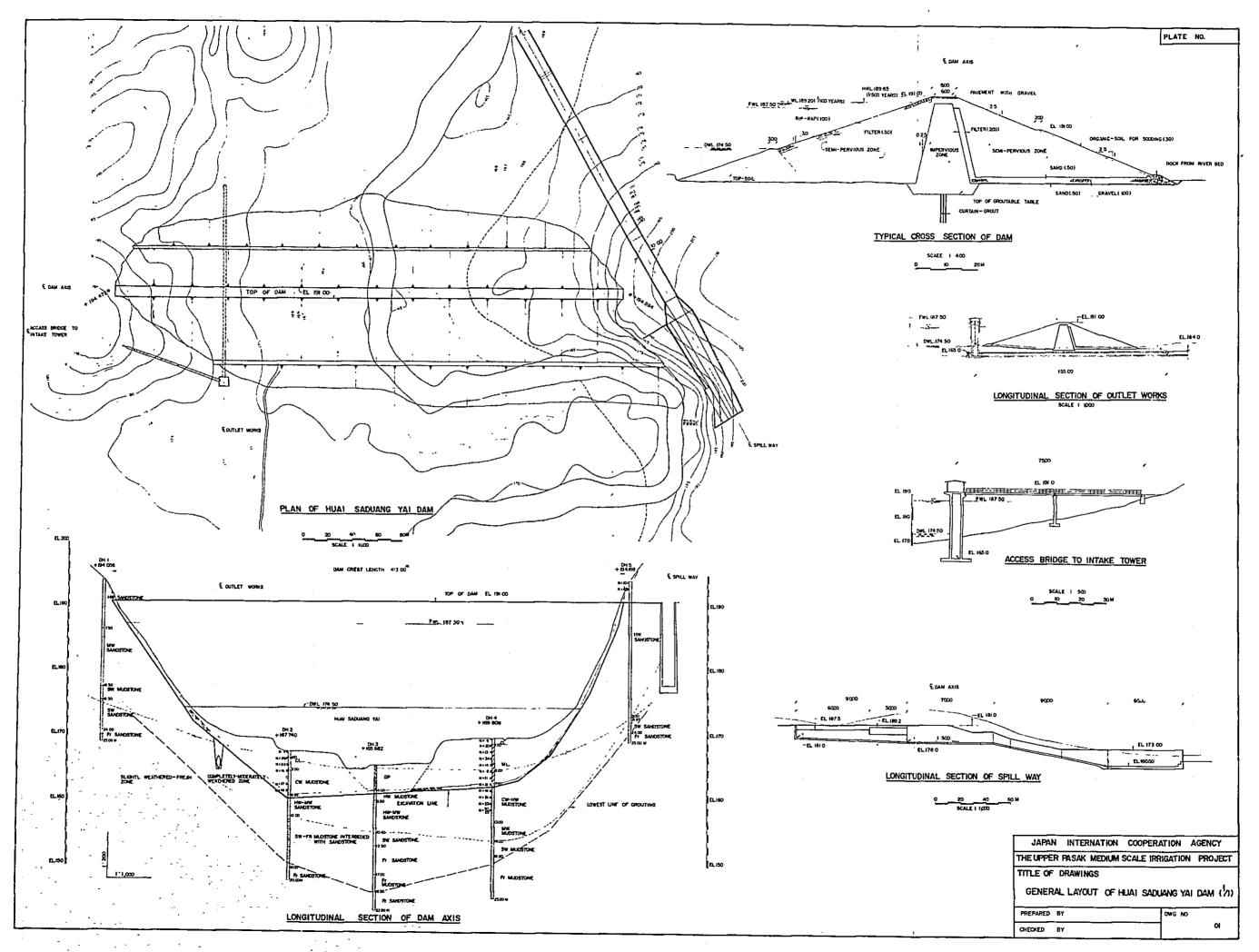
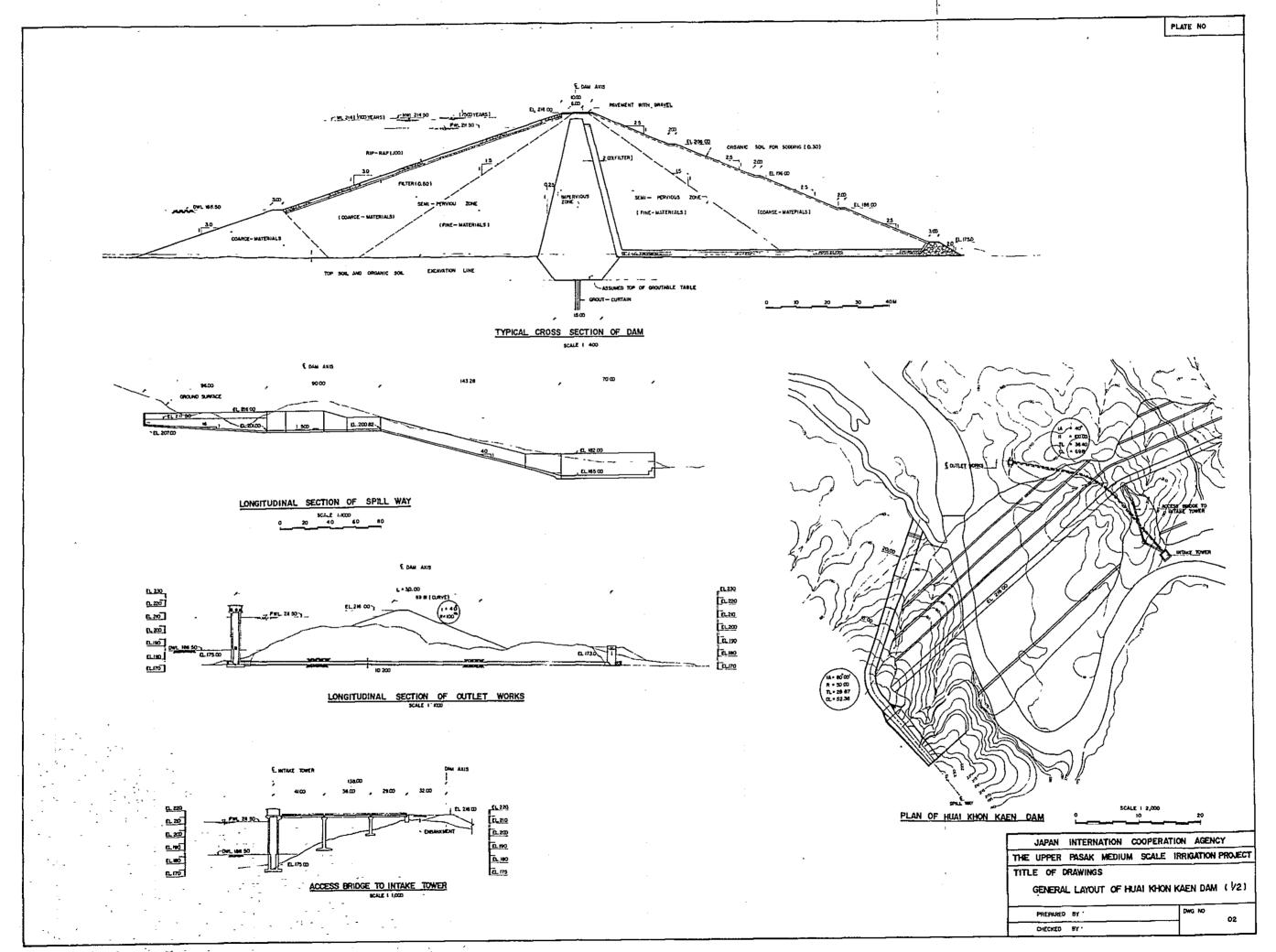
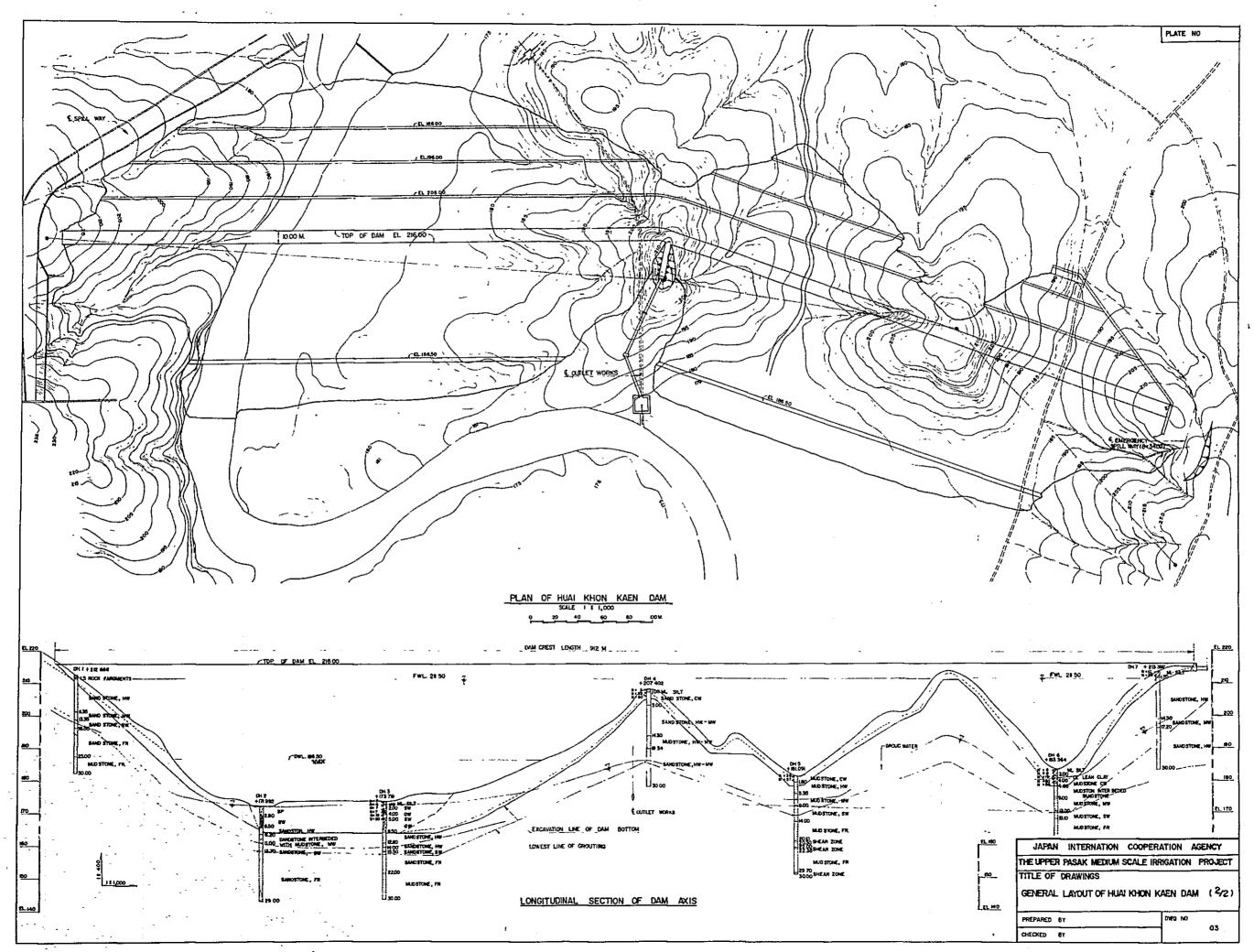


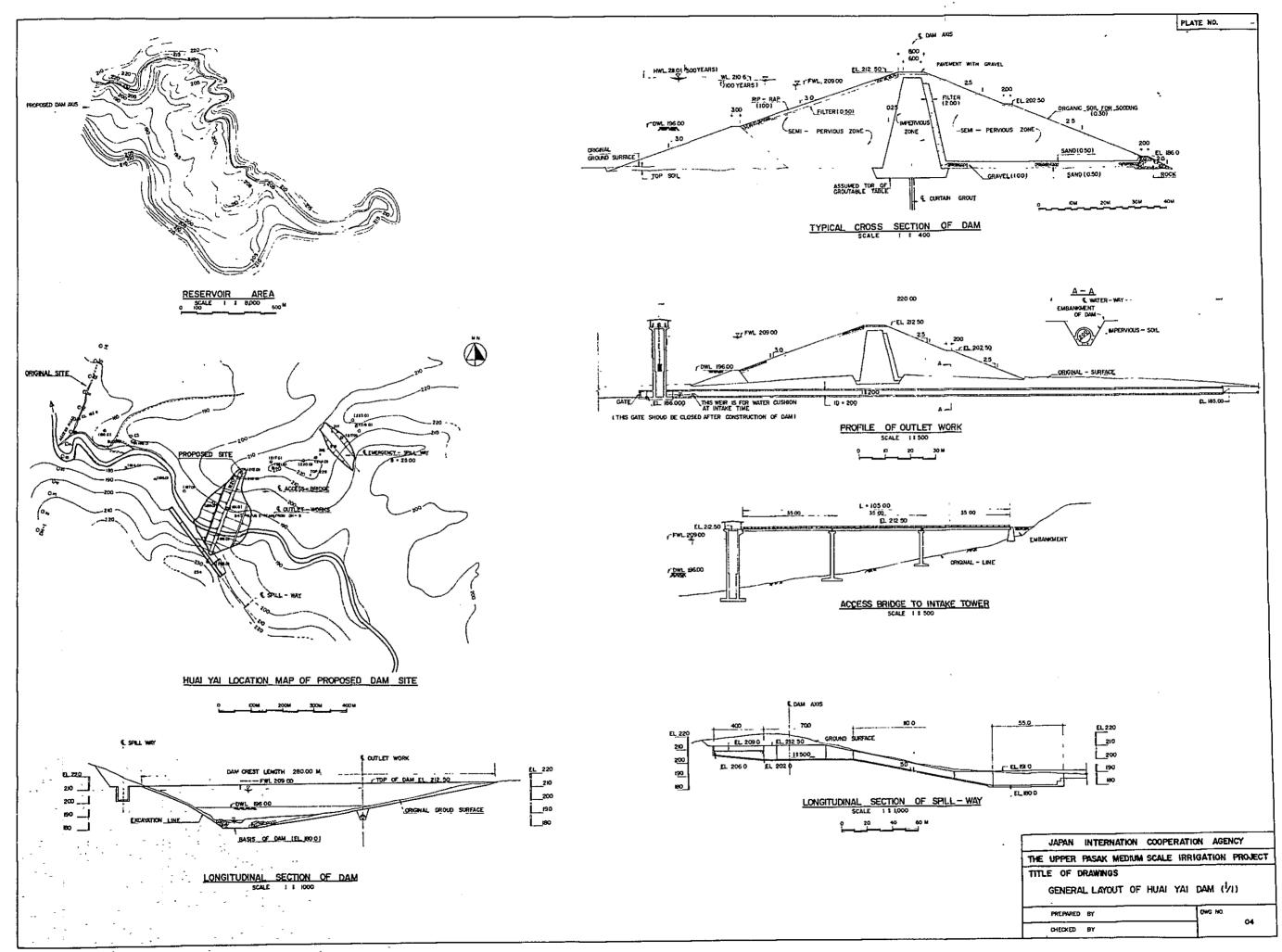
Fig. 3.5 GRAPHIC SOLUTION OF FLOOD CONTROL OF HUA! SADUANG YA! DAM

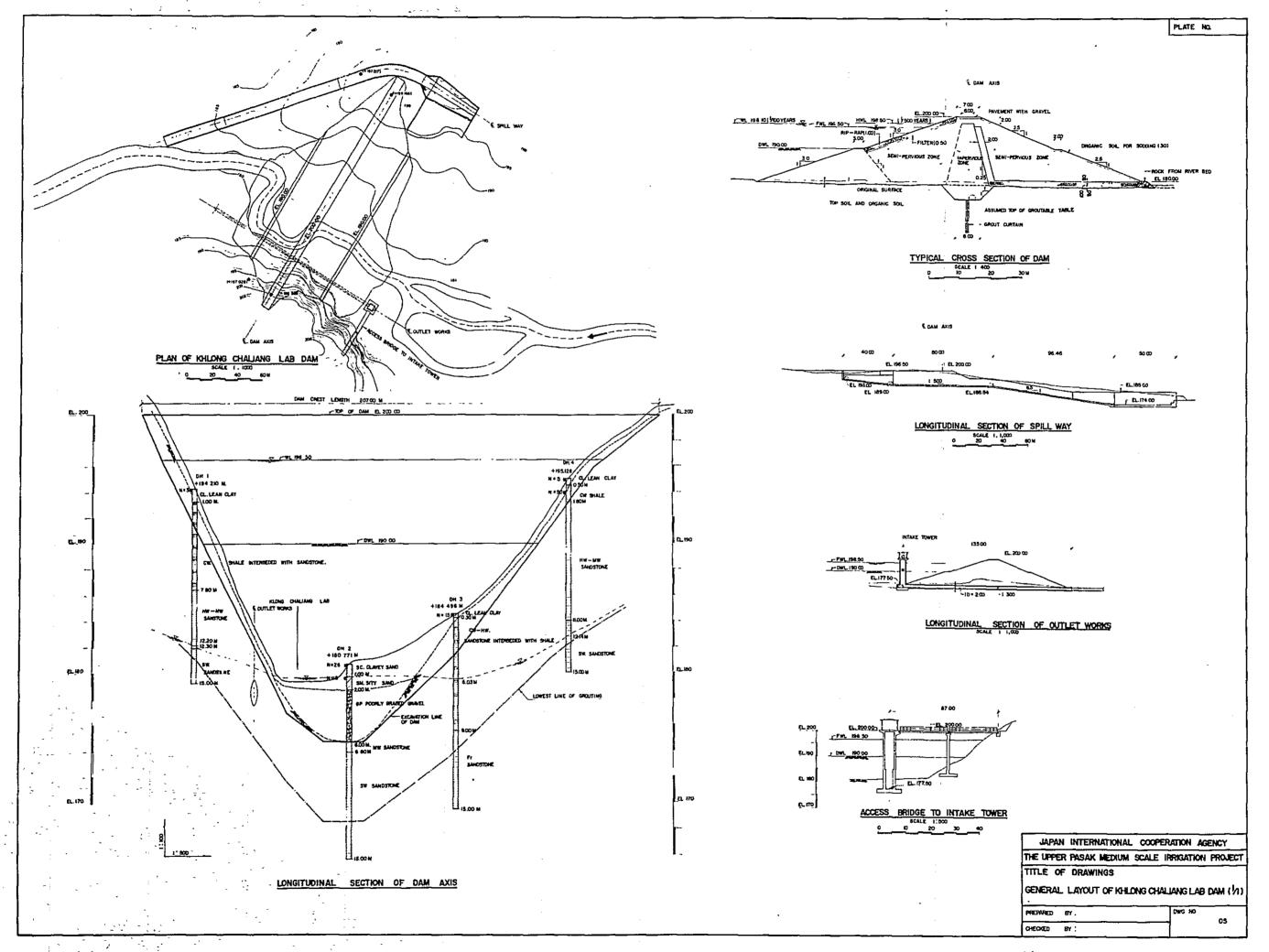






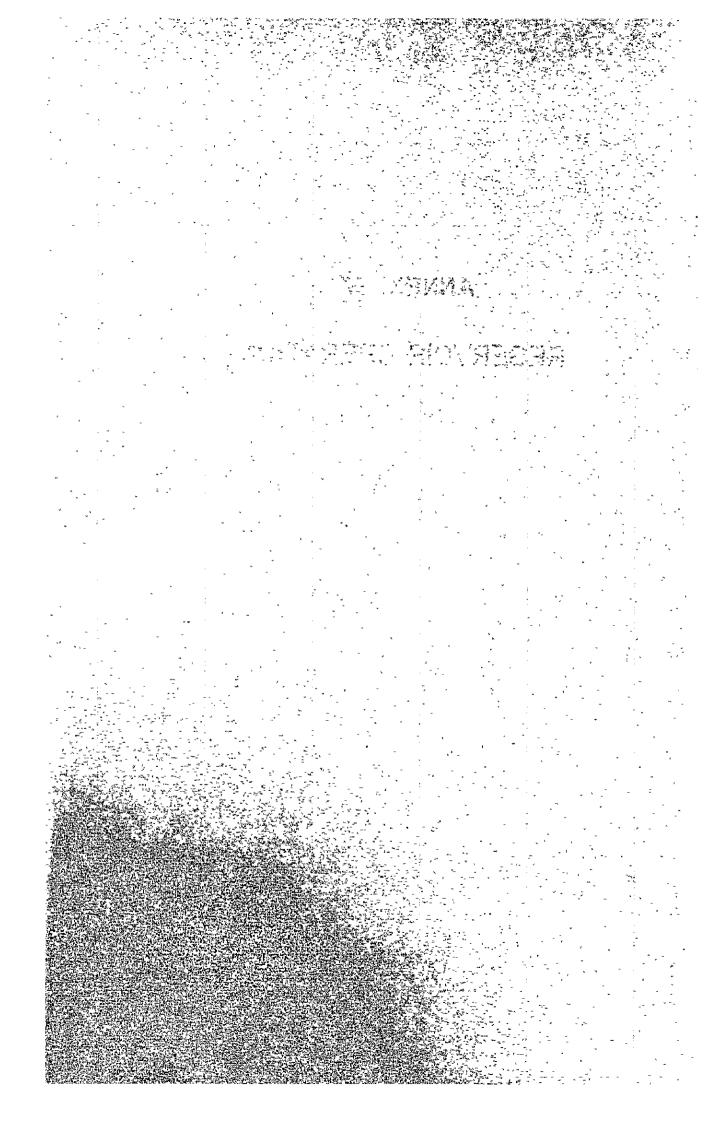








ANNEX IV RESERVOIR OPERATION



ANNEX IV RESERVOIR OPERATION

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4.1 GENERAL

To clarify the efficient use of the installed capacity of reservoir, the operation study is made for 26 years from 1952 to 1977 in according with the presumed set of operation rules as described below.

The storage capacity in the reservoir would be adjusted by evaporation and precipitation in the reservoir site. Natural inflow would be balanced with total demand to clarify surplus or deficit of water resources in the current month. The surplus water would be stored for the subsequent month. In case of the deficit of water resources, the storage water in the reservoir would be released to meet the deficit. Total amount of water exceeding the maximum storage capacity of reservoir would be unavailingly outspilled.

Since the exploited water in the Huai Saduang Yai reservoir would be used as the supplementary supply for the Sri Chan Irrigation Project area and the Pasak Left Bank Irrigation Project area, the Huai Saduang Yai reservoir would be operated accounting for non-regulated runoff in the mainstream of the Pasak river. Special operation rules are set forth for the Huai Saduang Yai reservoir as mentioned below.

The initial operation condition is presumed so same as the other reservoirs. Where the runoff in the Pasak river exceeds irrigation demand for the service areas of 37,500 rai (6,000 ha), no storage water in the Huai Saduang Yai reservoir would be released. Otherwise, in case of the deficit of irrigation water demand for the service areas of 37,500 rai (6,000 ha), the storage water in the reservoir would be supplied to meet the deficit. As regards balance of supply and demand, and spilling out of reservoir, the same rules for other three reservoirs would be also applied for the Huai Saduang Yai reservoir.

4.2 SUPPLY AND DEMAND

4.2.1 Supply

The monthly runoff of the tributary is produced by multiplying monthly rainfall by the runoff coefficient which is read on the line B in the estimated chart of runoff coefficient authorized in RID. The estimated runoffs in respective tributaries for 26 years from 1952 to 1977 are shown in Table 4.1(1) and (2). These values are used for the supply data of the operation study.

4.2.2 Demand

The demand of water resources comprises irrigation requirement, municipal water and downstream maintenance flow.

(1) Irrigation Requirement

The irrigation requirement is calculated on the monthly basis of meteorological data, recommendable cropping pattern and proposed crop intensity. As discussed in Annex VI, the crop intensity of 135% and 125% is applied for the Lom Sak area and the Phetchabun area, respectively. The estimated irrigation requirement is summarized as follows.

	Huai Saduang Yai	Huai Khon Kaen	Huai Yai	Khlong Chaliang Lab
Month	C.I = 135%	C.I = 135%	C.I = 125%	C.I = 125%
	I.A = 37,500 rai		I.A = 9,380 rai	
	(x10 ³ m ³)	(x103m3)	(x103m3)	(x103m3)
Jan.	2,934	2,152	524	80
Feb.	3,972	2,913	695	106
Mar.	4,020	2,948	780	120
Apr.	1,686	1,236	348	53
May	330	242	65	10
Jun.	2,880	2,112	908	139
Jul.	9,546	7,000	1,674	257
Aug.	1,998	1,465	756	116
Sep.	3,288	2,411	501	77
Oct.	12,018	8,813	2,907	446
Nov.	5,520	4,048	1,526	234
Dec.	0	0	0	0

(2) Municipal Water

The municipal water of 5,000 m³/day, about 270 liter per day per capita equivalence, including conveyance losses of 20% would be daily supplied for the Lom Sak municipality during dry season, Nov. to Apr., from the Huai Saduang Yai and the Huai Khon Kaen reservoir, respectively.

(3) Downstream Maintenance Flow

The river channel maintenance flow of 100 <code>f/sec/100 km²</code> would be released to downstream through a year in due consideration of riparian right of downstream population.

4.3 OPERATION STUDY

The operation study on the respective proposed reservoirs is made by use of the estimated supply and demand according to the operation rules abovementioned. The calculation sheets are shown in Table 4.2(1) to (4) and the results are summarized in Fig. 4.1(1) to (4).

The outcomes of the study show that all of the proposed reservoirs are efficiently operated even in the 20% recurrence of droughty year.

Table 4.1 Estimated Runoff

(1) <u>Huai Saduang Yai</u> and <u>Huai Khon Kaen</u> Reservoir

Year : 1952

:				:	Runoff (m3)		
Yonth	:	Rainfall (mm)	Runoff Coefficient (%)	nt Runoff (mm)	:	Huai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	;				:	70-4 is 22, 11, 12 is 1	
Apr	:	36.1	6.3	2.2	:	211200.	708400.
Hay	:	89.4	13.2	11.8	•:	1132800.	3799600.
Jun	1	147.1	20.7	30.4	:	2918400.	9788800.
Jul	:	177.4	28.6	50.7	:	4867200.	16325400.
Aug	:	183.6	29.4	53.9	:	5174400.	17355800.
Sep	:	300.2	49.5	148.5	;	14256000.	47817000.
Oct	:	124.9	26.8	33.4	:	3206400.	10754800.
Nov	;	2.0	5.8	.1	;	9800.	32200.
Dec	:	0.0	*****	0.0	;	0.	0.
Jan	ı	28,5	9.3	2.6	:	249600.	837200.
Feb	:	67.7	14.4	9.7	:	931200.	3123400.
Har	:	16.8	7.7	1.2	:	115200.	386400.
	•				:		

Total : 1173.7 29.4 344.5 :

Huai Saduang Yai and Huai Khon Kaen

33072000.

110929000.

:			D 44	:	:	Run	off (m3)
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: H	uai Saduane Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	;				:		
Apr	:	105.8	15.3	16.1	:	1545600.	5184200.
May	;	203.0	27.9	56.6	:	5433600.	18225200.
Jun	:	211.5	29.0	61.3	:	5884800.	19738600.
Jui	1	166.7	27.2	45.3	:	4348300.	14586600.
Aus	4	136,2	23.2	31.5	:	3024000.	10143000.
Sep	:	272.8	46.0	125.4	:	12038400.	40378800.
0ct	:	51.8	17.3	8.9	:	854400.	2865800.
Nov	:	45.9	11.5	5.2	;	499200.	1674400.
Dec	:	0.0		0.0	:	0.	0.
Jan	1	Ŏ . O		0.0	:	0.	Ô.
- Feb	:	0.0		0.0	:	Ü.	0.
Mar	:	20.7	8.3	1.7	:	163200.	547400.
• •	: -	•			:		
,	:				 :		
Total -	:	1214.4	29.0	352.0	:	33792000.	113344000.

Year : 1954

	:		D		ŧ	Runoff (m3)		
Month	; ;	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	:	Huai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	;				:			
Apr	:	3.8	2.1	0,0	:	0,	0.	
May	:	18.8	4.0	.7	:	67200.	225400.	
Jun	:	211.1	29.0	61.2	:	5875200.	19706400.	
Jul	:	126.8	22.0	27.8	:	2668800.	8951600.	
Aus	;	181.1	29.1	52.7	:	5059200.	16969400.	
Sep	:	450.1	69.0	310.5	ţ	29303000.	99981000.	
0ct	:	83.8	21.4	17.9	:	1718400.	5763800.	
Nov	:	3.6	6.0	.2	:	19200.	64400.	
Dec	:	11.5	7.1	.8	;	76800.	257600.	
Jan	:	0.0		0.0	:	0.	0.	
Feb	:	12.3	7.2	.8	:	76800.	257600.	
Har	:	30.4	9.5	2.8	;	268800.	901600.	
	:				;			
					:			
Total	:	1133.3	41.9	475,4	:	45638400.	153078800.	

Huai Saduang Yai and Huai Khon Kaen

	;		Runoff Coefficient (%)		:	Runoff (m3)		
Month	:	Rainfall (mm)			: : }	Nai Saduane Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	:			*****	:			
Apr	:	151.8	21.3	32.3	:	3100800.	10400600.	
Mar	;	113.2	16.3	18.4	:	1766400.	5924800.	
Jun	:	478.6	63.7	304.8	:	29260800.	98145600.	
Jul	;	89.0	17.1	15.2	:	1459200.	4894400.	
Aus	:	325,9	47,9	156.1	:	14985600.	50264200.	
Ser	:	244.1	42,2	103.0	:	9853000.	33166000.	
0ct	:	14.9	12.5	1.8	:	172800.	579600.	
Nov	:	0.0		0.0	į	ů.	0.	
Dec	:	0.0		0.0	:	Û.	ŷ.	
Jan	:	0.0		0.0	;	0.	0.	
Feb	:	65.6	14.1	9.2	:	883200.	2962400.	
Mar	:	63.5	13.8	8.7	:	835200.	2801400.	
	:				:			
	:				:		# # # # # # # # # # # # # # # # # # #	
Total	;	1546.6	42.0	649.5	:	62352000.	209139000.	

Huai Saduang Yai and Huai Khon Kaen

	:		m 20	:		Runoff (m3)		
Month	# #	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: }	Huai Saduane Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	:				;			
Apr	:	171.9	23.9	41.0	:	3936000.	13202000.	
May	:	235.0	32.1	75.4	.:	7238400.	24278300.	
Jun	:	101.0	14.7	44.8	`:	1420800.	4765600.	
Jul	:	200.3	31.6	63.2	:	6067200.	20350400.	
Aus	:	230.7	35.5	81.8	:	7852800.	26339600.	
Sep	;	310.9	50.9	158.2	;	15137200.	50940400.	
Oct	:	75.6	20.4	15.4	:	1478400.	4958800.	
Nov	:	0,0		0.0	:	Ü.	0.	
Dec	;	0.0		0.0	;	0.	0.	
Jan	:	0.0		0.0	:	0.	Û.	
Feb	:	20.3	8.2	1.6	:	153600.	515200.	
Mar	;	186.1	29.7	55,2	;	5299200.	17774400.	
	:				;			
	 :				;			
Total	:	1531,8	33.1	506.6	:	48633600.	163125200.	

Huai Saduang Yai and Huai Khon Kaen

	:				:	: Runoff (m3)		
Month	:	Kainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	•	Huai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	;				:			
Apt	:	78.1	11.7	9.1	:	873600.	2930200.	
May	:	125.9	17.9	22.5	:	2160000.	7245000.	
Jun	:	245.8	33.5	82.3	:	7900300.	26500600.	
ժա	-:	263.6	39.8	104.9	:	10070400.	33777800.	
Ana _	ت : - ً	223.8	34.6	77.4	:	7430400.	24922800.	
Ser	- ;	431.7	66.6	287.5	:	27600000.	92575000.	
0ct i	-c - _t	81.1	21.1	17.1	a 0	1641600.	5506200.	
Nov	:	0.0		0.0	:	0.	0.	
Dec	, ;	0.0		0.0	:	0.	0	
Jan	:	31.1	9.6	2.9	;	278400.	933800.	
Feb	:	3.5	- 6.0	.2	:	19200.	64400.	
Mar		0.0		ů,ů	:	0.	ů.	
	:				;	•	••	
		·	`		:			
, Total -	· :	1484.6	49.7	603,9	:	57974400 . -	194455800.	

Year : 1958

Year : 1959

	:		Du 66		:	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	; ;	Huai Saduang Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	:				:			
Apr	:	76.4	11.5	8.7	:	835200.	2801400.	
May	:	63.1	9.8	6.1	:	585600.	1964200.	
Jun	:	192.7	26.6	51.2	:	4915200.	16486400.	
Jul	:	164.1	26.9	44.1	;	4233600.	14200200.	
Aus	:	305.6	45.2	138.1	:	13257600.	44468200.	
Ser	:	235.3	41.1	96.7	:	9283200.	31137400.	
Oct	:	100.0	23,5	23.5	:	2256000.	7567000.	
Nov	:	0.0		0.0	:	0.	0.	
Dec	ŧ	0.0		0.0	:	0.	0.	
Jan	:	0.0		0.0	:	0.	0.	
Feb	:	10.3	6.9	.7	:	67200.	225400.	
Mar	1	43.4	11.2	4.8	5	460300.	1545800.	
	:				:			
	:				:			
Total	:	1190.9	31.4	373.9	:	35894400.	120395800.	

Huai Saduang Yai and Huai Khon Kaen

	:				:	Run	off (m3)
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	; ; H	uai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	:				;		
Apr	:	87.7	12.9	11.3	:	1084800.	3638600.
May	:	105.6	15.3	16.1	:	1545600.	5184200,
Jun	:	96.1	14.0	13.4	:	1286400.	4314800.
Jul	:	197.2	31.2	61.5	:	5904000.	19803000.
Aus	:	176.2	28.4 •	50.0	1	4800000.	16100000.
Sep	:	483.2	73.3	354.1	:	33993600.	114020200.
0ct	:	0.0		0.0	4	0.	0.
Nov		0.0		0.0	1	0.	0.
Dec	:	0.0		0.0	:	Ů.	0.
Jan	:	2.9	5.9	.1 .	:	9600.	32200.
Feb	:	9.8	8.8	.6	:	57600.	193200.
Mar	:	43.3	11.2	4.8	:	460800.	1545600.
	:				:		
	:				:		#*************************************
Total	:	1202.0	42.6	511.9	:	49142400.	164831800.

Year : 1960

:	:				:	ƙu	noff (m3)
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)			Huai Khon Kaen (322 Km2)
	:				:		
Apr	:	14,4	3.4	.4	:	38400.	128800.
May	;	269.4	36.5	98.3	:	9436800.	31652600.
מניט	:	156.0	21.8	34.0	`:	3264000.	10948000.
Jul	:	84.9	16.6	14.0	:	1344000.	4508000.
Aus	:	269.6	40.6	109.4	:	10502400.	35226800.
Ser	:	182.5	34.3	62.5	;	6000000.	20125000.
Oct	:	81.8	21.2	17.3	:	1660800.	5570600.
Nov	:	0.0	~~~	0.0	:	0,	Û.
Dec	:	0.0		0.0	:	0.	0.
Jan	:	0.0		0.0	:	0.	0.
Feb	:	23.8	8.7	2.0	1	192000.	644000.
Mar	:	64.4	13.9	8.9	:	854400.	2865800.
	8				:		
****	:		···		:		
Total	;	1146.8	30.2	346.8	:	33292800.	111669600.

Huai Saduang Yai and Huai Khon Kaen

	:		D 66	Runoff (mm)	:	: Runoff (m3)		
Month	:	Rainfall (gmp)	Runoff Coefficient (%)		:	Huai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	` :				:			
Apr-	:	91.2	13.4	12.2	;	1171200.	3928400.	
May	. :	177.9	24.7	43.9	•	4214400.	14135800.	
Jun	1	155.7	21.8	33.9	:	3254460.	10915800.	
Jul	:	219.0	34.0	74.4	;	7142400.	23956300.	
Aug .		, 158.2	26.1	41.2	:	3955200.	13266400.	
Sep	``.` ŧ .'	327.4	53.1	173.8	:	16684800.	55963600.	
Oct	-1	120.1	26.2	31.4	:	3014400.	10110800.	
- Nov	:	0.0		0.0	:	0.	0.	
- Dec		3.8	6.1	.2	;	19200.	64400.	
Jan	:	0.0		0.0	:	0.	0.	
Feb	:	0.0	<u></u>	0.0	:	0.	0.	
* Mar	;	1.1	5.7	0.0	:	Ů.	Ů.	
	.1.	, -	-	1	:	•		
	· - ;				:		~~~	
Total	2	1254.4	32.8	411.0	:	39455000.	132342000.	

Year : 1962

	:	•	n	Runoff (mm)	:	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)		; ; ;	luai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	•				:			
ÁРГ	: 1	87.0	12.9	11.2	;	1075200.	3606400.	
May	:	95.5	14.0	13.3	:	1276300.	4232600.	
Jun	•	122,2	17.4	21,2	:	2035200.	6826400.	
Jul	;	191.5	30.4	53.2	:	5537200.	18740400,	
Aus	:	150.2	25.1	37.7	:	3619200.	12139400.	
Sep	:	371.3	58.8	218.3	;	20956800.	70292600.	
Oct	•	64.6	19.0	12.2	:	1171200.	3928400.	
Nov	:	0.0		0.0	:	0.	0.	
Dec	:	0.0		0.0	:	0.	0.	
Jan	:	0.0		0.0	;	0.	0.	
Feb	. :	11.8	7.1	.8	:	76800.	257600.	
Mar	:	133.0	22.8	30.3	:	2908800.	9756600.	
	:				:			
	:				:			
Total	, :	1227.1	32,9	403.2	:	33707200.	129830400.	

Huai Saduang Yai and Huai Khon Kaen

Nonth	:		Runoff Coefficient (%)	Runoff (mm)	:	Runoff (m3)		
	:	Rainfall (mm)			: - : Hu	ai Saduane Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	;				:	,,		
Apr	:	57.0	9.0	5.1	:	489600.	1642200.	
May	:	59.4	9.3	5.5	;	528000.	1771000.	
Jun	;	268.7	36,4	97.8	;	9388800.	31491600.	
մսն	. :	220.6	34,2	75.4	:	7238400.	24278800.	
Aus	ŧ	202.1	31,8 .	64.2	:	6163200.	20672400.	
Sep	:	212.5	33.1	80.9	:	7766400.	26049860.	
0ct	:	198.8	36.4	72.3	;	6940800.	23280600.	
Nov	;	43.3	11.2	4.8	:	460900.	1545600.	
Dес	:	21.2	8.3	1.7	:	163200.	547400.	
Jan	;	0,0		0.0	:	Ŭ.	0.	
Feb	;	0.0	*****	0.0	•	Û,	0.	
Mar	:	0.0		0.0	:	0.	0.	
	:		1		:			
	:				:			
Total	:	1283.6	31.8	407.7	:	39139200.	131279400.	

Year : 1964

	:		B 346	Runoff : (mm) :	Runoff (#3)		
Month -	1	Rainfall (mm)	Runoff Coefficient (%)			Huai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	:				:		^ .
Apr	;	82.7	12.3	10.1	:	969600.	3252200.
May	:	313.5	42.3	132.6	٠;	12729600.	42697200.
Jun	:	98.9	14.4	14.2	:	1363200.	4572400.
Jul	•	157.8	26.0	41.0	:	3936000.	13202000,
Aus	:	184.0	29.4	54.0	:	5184000.	17388000.
Ser	:	244.1	42.2	103.0	;	9385000.	33166000.
Oct	: :	194.4	35.8	69.5	:	6672000.	22379000.
Nov	:	7.3	6.5	.4	:	38400.	128800.
Dec		0.0		0.0	:	0.	0.
Jan	1	0.0		0.0	:	Û.	0.
Feb	:	32.0	9.7	3.1	:	297600.	998200.
Mar	:	16.7	7.7	1.2	:	115200.	386400.
	:			·	:		
	:				:		
Total	1	1331.4	32.2	429.1	;	41193600.	138170200.

Huai Saduang Yai and Huai Khon Kaen

Year - : 1965

	:		D 66		:	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	:	Huai Saduans Yai (96 Km2)	Huai Khon Kaén (322 Km2)	
	:	:			:			
Арг	1	0.0		0.0	:	0.	Ú.	
May.	~ :	165.8	23.1	38.2	:	3667200.	12300400.	
Jun-	- 1	173.9	24.1	41.9	:	4022400.	13491800.	
Jul	1	66.8	14.2	9.4	:	902400.	3026800.	
Aug	` :	244.4	37.3	91.1	:	8745600.	29334200.	
- Sep		193.0	35.6	68.7	:	6595200.	22121400.	
Oct	•	· 52.7 ·	17.4	9.1	:	873600.	2930200.	
Nev	, :	0.0		-0.0	:	. 0.	0.	
Dec	- •	0.0	·	0.0	:	Ű.	0.	
Jan	:	0.0	*	0.0	:	Û.	Û.	
Feb	:	0.0		0.0	:	Ű.	0.	
Mar	1	7.6	6.6.	.5	;	48000.	161000.	
· ·	•		#= x x x x x x x x x x		:	-		
					:			
Total .	•	904.2	23.5	253.9	:	24354400.	83365900.	
		•	- · · · · · · · ·	\$ v				
-	•			IV - 10	5			
	-	t		. 1	-			
-	4		•	- .				
• ' -	,							

Year : 1966 Huai Saduang Yai and Huai Khon Kaen

	:		D		:	Runoff (m3)	
Month -	: :	Rainfall (mm) -	Runoff Coefficient (%)	Runoff (mm)	: : Ht	vai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	:				:		
Apr	:	22.1	4.4	.9	:	86400.	289800.
Hay	:	131.7	18.7	24.6	:	2361600.	7921200,
Jun	:	64.0	9.9	6.3	:	604800.	2028600.
Jul	:	49.8	12.0	5.9	:	566400.	1879800.
Aus	:	104.2	19.1	19.9	:	1910400.	6407800.
Ser	:	52.9	17.4	9.2	:	883200.	2962400.
0ct	' ;	82.2	21.2	17.4	:	1670400.	5602800.
Nov	:	15.9	7.6	1.2	:	115200.	386400.
Dec	:	26.0	8.9	2.3	;	220800.	740600.
Jan	;	.5	5.6	0.0	:	0.	0.
Feb	;	0.0		0.0	:	0.	0.
Mar	:	3.0	6.0	.1	:	9600.	32200.
	:				:		
	:		*		:	·	
Total	:	552.3	15.9	37.3	:	8428800.	28271600.

Huai Saduang Yai and Huai Khon Kaen

Runoff (m3) Runoff Month Rainfall Coefficient : Huai Saduans Yai Runoff Huai Khon Kaen (mar) (%) (mm) (96 Km2) (322 Km2) 100.4 14.6 APt-14.6 1401600. 4701200. May 63.7 10.5 7.2 691200. 2318400. Jun 86.2 12.8 11.0 1056000. 3542000. Jul 115.6 20.6 23.8 : 7663600. 2284300. Aus 114.4 2236800. 20.4 23.3 7502600. Sep ύ. 0. 0.0 0.0 : 0. 0ct 0.0 0.0 : 0.

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Year : 1967

Nov

Dec

Jan

0.0

0.0

0.0

Feb Mar -	:	0.0 34.7	10.1	0.0 3.5	:	0. 336000.	0. 1127000.
Total	:	520.0	16.0	83.4	;	3006400.	26854800.

Huai Saduang Yai and Huai Khon Kaen

Year	•	1948
rear.		1700

	:			· Runoff (mm)	;	Runoff (m3)	
Month	:	Rainfall (mm)	Runoff Coefficient (%)		: H	uai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	: :				ì		
Apr	:	115.0	16.5	18.9	8	1814400.	6085800.
May	:	300.5	40.6	122.0	į	11712000.	39284000.
Jun	:	163.4	22.8	37.2	:	3571200.	11978400.
Jul	:	177.9	23.7	51.0	:	4896000.	16422000.
Aus	:	115.5	20.6	23.7	:	2275200.	7631400.
Ser	:	73.2	20.1	14.7	:	1411200,	4733400,
Oct	:	70.0	19.7	13.7	:	1315200.	4411400.
Nov	:	.4	5.6	0.0	;	0.	0.
Dec	:	0.0		0.0	:	0.	0.
Jan	ţ	28.3	9,2	2.6	;	249600.	837200.
Feb	:	3.6	6.0	.2	;	19200.	64400.
Mar	;	100.4	18.6	13.6	:	1795600.	5989200.
	:				:		
	:			******	:		
Total	:	1143.2	26.4	302.6	4	29049600.	97437200.

	:		D 66		:	Runoff (m3)	
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	; ; H	uai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	:				;		**************************************
Apr	:	50.3	8.1	4.0	:	384000.	1288000.
May	· :	130.1	18.5	24.0	1	2304000.	7728000.
Jun	:	69,2	10.5	7.2	1	691200.	2318400.
Jul	;	91.2	17,4	15.8	:	1516300.	5037600.
Aus	. :	96.5	18.1	17.4	:	1670400.	5602800.
Ser	•	296.8	49.1	145.7	:	13937200.	46915400,
Oct	;	55.7	17.8	9.9	:	950400.	3187800.
Nov	, 't	17.7	7.9	1.3	:	124800.	418600.
î Dec	' ť	0.0		0.0	:	0.	0.
Jan	:	.3	5.6	0.0	:	Ů.	0.
Feb	- :	1.0	5.7	0.0	:	- 0.	0.
- F Mar -	:	126.0	21.9	27.5	:	2640000.	3355000 , -
	:	· · · · · · · · · · · · · · · · · ·	f==		:		
	:	•		*******	;		
. Total :	- :	934.8	27.0	252.3	:	24268900.	81401600.

Year	 1970

	:		Runoff Eoefficient (%)	Runoff (mm)	:	Runoff (m3)	
Month	:	Rainfa)} {mm)			: - : Hu :	ai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	:				:		
Aer	;	35.1	6.1	2.1	:	201600.	676200.
May	:	108.3	15.6	16.8	:	1612800.	5409600.
Jun	;	245,3	33.4	81.9	:	7862400.	26371800.
Jul	:	40.5	10.3	4.3	ţ	412800.	1384600.
Aus	:	245.4	37.4	91.7	:	8303200.	29527400.
Sep	:	184.2	34.5	63,5	:	6096000.	20447000,
Oct	:	92,9	22.6	20.9	:	2006400.	6729800.
Nov	:	2.8	5.9	.1	:	9600.	32200.
Dec	2	18,2	7.9	1.4	1	134400.	450800.
Jan	:	.4	5.6	0.0		0.	0.
Feb	:	25.7	8.9	2,2	:	211200.	708400.
Mar	:	43.0	11.1	4.7	:	451200.	1513400.
	:				;		
	 ;	,	,	.,,	;		
Total	•	1041.8	27.8	239.6	:	27801600.	93251200.

Huai Saduang Yai and Huai Khon Kaen

Year	:	1971
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•			,		:	Run	off (m3)
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (nm)	; ; ;	luai Saduans Yai (96 Km2)	
	:				•		
APr	:	6.6	2.4	.1	:	9600.	32200.
May	:	169.5	23.6	40.0	;	384000û.	12330000.
dun	:	167.0	23.2	38.7	:	3715200.	12461400.
Jul	:	103.5	19.6	21.2	;	2035200.	6326400.
Aus		217.5	33.8	73.5	;	7056000.	23667000.
Ser	:	148.2	29.8	44.1	:	4233600.	14200200.
űct	:	84.6	21,5	18.1	:	1737600.	5328200.
Nov	:	3،	5.6	0.0	:	ů.	0.
Dec	:	3.0	6.0	.1	:	9600.	32200.
Jan	;	0.0		0.0	;	0.	0.
Feh	ţ	31.6	9,7	3.0	:	288000.	966000.
Har	:	41.7	11.0	4.5	:	432000.	1449000.
	:				:		
	 ;				;		
Total	:	978.5	24.9	243.3	:	23356800.	78342600.

Huai Saduang Yai and Huai Khon Kaen

:					:	Runoff (m3)		
Month	:	Rainfall (mar)	Runoff Coefficient (%)	. Runoff (mm)	; ; ;	luai Saduans Yai (96 Km2)		
	;				;			
Apr	:	57.5	9.0	5.1	:	489600.	1642200.	
May	;	81.3	12.1	9.8	:	940800.	3155600.	
Jun	t	161.4	22.5	36.3	:	3484800.	11688600.	
Jul	:	80.5	16.0	12.8	:	1228800.	4121600.	
Aug	:	211.2	33.0	69.6	:	6681600.	22411200.	
Sep	:	170.8	32.7	55.3	:	5356800.	17967600.	
Oct	:	83.8	21.4	17.9	:	1718400.	5763800.	
Nov	:	16.5	7.7	1.2	:	115200.	386400.	
Dec	:	6.7	6.4	.4	:	38400.	128800.	
Jan	1	0.0		0.0	:	Û.	0.	
Feb	:	.9	5.7	0.0	1	Q.	0.	
Har	:	55.2	12.7	7.0	0	672000.	2254000.	
	:				:			
	1			*	:	·		
Total	:	925.3	23,3	215.9	:	20726400.	69519800.	

Huai Saduang Yai and Huai Khon Kaen

;					:	Runoff (m3)		
Month	Month : F	Rainfall (mm)	Runoff Coefficient (%)	Runoff (nm)	:	Huai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	:				:			
Aer	:	64.1	9.9	6.3	:	604800.	2028600,	
May	;	135.6	19.2	26.0	:	2496000.	8372000.	
ปนกั	t	136.4	19.3	26.3	:	2524800.	8468600.	
Jul -	:	141.8	24.0	34.0	;	3264000.	10948000.	
Aus -		96.6	18.1	17.4	:	1670400.	5602800.	
Sep 🧈	10.1	133.2	28.5	39.3	:	3772800.	12654600.	
Oct .	` :	10.1	11.9	1.2	;	115200.	386400.	
Nov-	:	8.4	6.7	.5	;	48000.	161000.	
Ũес	`:	0.0		0.0	:	0.	Û.	
, Jan '	' ;	1.0	5.7	0.0	;	Û.	Ú.	
Feb	:	2.5	5.9	.1	:	9600.	32200.	
Mar	:	57.3	- 13,0	7.4	*	710400.	2382800.	
	٠.;	·	*		:			
-	- :	, , ,		+	:			
Total	:	792.0	2ù.0	158.5	:	15216000,	51037000.	

Year : 1974

:					;	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: - : Hu	ai Saduane Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	:				:			
Apr-	1	74.8	11.3	8.4	:	806400.	2704800.	
May	:	184.4	25.5	47.0	;	4512000.	15134000.	
Jun	:	137.8	19.5	26.8	:	2572800.	8629600.	
Jul	:	132.1	22.7	29.9	:	2870400.	9627800.	
euA	:	180.8	29.0	52.4	:	5030400.	16872800.	
Ser	:	101.6	23.8	24.1	;	2313600.	7760200.	
0ct	*	87.3	21.9	19.1	:	1833600.	6150200.	
Nov	:	40.1	10.8	4.3	:	412800.	1384600.	
Dec	:	0.0		0.0	;	0.	0.	
Jan	:	51.9	12.3	6.3	:	604800.	2028600.	
Feb	:	51.7	12.3	6.3	:	604800.	2028600.	
Mar	:	39.3	10.7	4.2	:	403200.	1352400.	
	:				:			
	:				;			
Total	8 9	1081.8	21.1	228.8	:	21964800.	73673600.	

Huai Saduang Yai and Huai Khon Kaen

	:				:	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: H:	uai Saduane Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
*	:				:			
Apr	:	1.9	1.8	0.0	:	0.	0.	
May	:	101.2	14.7	14.8	:	1420300.	4765600.	
Jun	:	124.9	17.8	22.2	:	2131200.	7148400.	
Jul		159.9	26.3	42.0	:	4032000.	13524000.	
Aus	4	187.0	29.8 .	55.7	:	5347200.	17935400.	
Ser	:	178.1	33.7	60.Û	:	5760000.	19320000.	
Oct	g 4	110.8	24.9	27.5	:	2640000.	8855000.	
Nov	:	54.8	12.7	6.9	:	662400.	2221800.	
Dec	:	0.0		0.0	:	0,	0.	
Jan	t	ύ. 0		0.0.	:	ů.	Ú.	
Feb	:	77.0	15.6	12.0	:	1152000.	3864000.	
Mar	:	4.8	6.2	.2	:	19200.	644û0.	
	ŧ				:			
	:	·***		-4	:			
Total	:	1000.4	24.1	241.3	:	23164800.	77693600.	

Huai Saduang Yai and Huai Khon Kaen

:					:	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (sm)	: Ht	uai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)	
	:							
Apr	:	46.6	7.6	3.5	;	336000.	1127000.	
May	:	194.1	26.8	52.0	ŧ	4592000.	16744000.	
Jun	: -	135.3	19, 1	25.8	:	2476800.	8307600	
Jul	:	215.3	33.5	72.1	;	5921600.	23216200.	
Aup	:	229.5	35,4	81.2	:	7795200.	26146400.	
Sep	:	183.9	34.4	63.2	:	606720û.	20350400	
Oct	1	126.5	27.0	34,1	ţ	3273600.	10930200.	
Nov	:	7.0	6. 5	, 4	;	38400.	128800.	
Dec	;	0.0		0.0	:	Û.	0.	
Jan	:	* 4	5.6	0.0	:	0.	0.	
Feb	;	0,0		0.0	:	0.	٥.	
Mar	:	57.6	13.0	7.4	:	710400.	2382800.	
	;				:			
	1		****	,	:		ار سیان ۱۹۵۵ که و در ۱۹۵۰ که در	
Total	:	1196.2	28.4	339.7	:	32611200.	109383400.	

Huai Saduang Yai and Huai Khon Kaen

	:			:	Runoff (m3)		
Month	; ;	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: H:	Jai Saduans Yai (96 Km2)	Huai Khon Kaen (322 Km2)
	:				;		
19Ĥ	•	66.7	10,2	6.9	:	652800.	2189600.
May	, :	134.8	19.1	25.7	:	2467200.	8275400.
Jun .		70.0	1û.7	7.4	ł	710400.	2382800.
ડેવા 📜	. :	127.4	22.1	28.1	:	2697600.	9049200.
Ųúa	;	126.9	22.0	27.9	:	2678400.	8983800
Ser	, :	180.6	34.0	61.4	;	5394400.	19770300.
Oct	•	43.9	16.3	7.1	:	681600.	2286200.
Nov	(C)	2.6	5.9	1.1	:	9800.	32200.
Dec	•	43.9	11.3	74.9	:	470400.	1577800.
Jan	, t.	2.5	5.9	1	:	9600.	32200.
Feb	: .	28.0	9.2	2.5	:	240000.	805000
Mar _.	, 1	7.4	6.5	.4	:	3840û.	128300.
.=					•		,.,
	:			·~			
Total -	• ;	334.7 🦠	20.7,	-172.4	:	16550400.	55512800.

Table 4.1 Estimated Runoff

(2) <u>Huai Yai</u> and <u>Khlong Chaliang Lab</u> Reservoir

*			D 44	1			Run	eff (m3)
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: Ho	uai Yai (78 Kn2)	Khlone Chaliane Lab (77 Km2)	
	:	`		-	:			
Apr	:	18.2	3.9	.7	:	54600.	53900.	
Hay	•	192,3	26.5	50.9	;	3970200.	3919300.	
Jun	:	237.2	32.4	76.8	:	5990400.	5913600.	
Jul	:	77.0	15.6	12.0	:	936000.	924000.	
Aus		172.1	27.9	48.0	:	3744000.	3696000.	
Sep	:	128.4	27.2	34.9	:	2722200.	2687300.	
0ct	:	70.0	19.7	13.7	:	1068600.	1054900.	
Nov	9	2.0	5.8	.1	;	7800.	7700.	
Dec	1	0.0		0.0	:	0.	0.	
Jan	:	21.1	8.3	1.7	:	132600.	130900.	
Feb	:	77.2	15.6	12.0	:	936000.	924000.	
Mar	:	30.9	9.6	2.9	:	226200.	223300.	
	:				:			
	 :				:			
Total	;	1026.4	24.7	253.7	:	19788600.	19534900.	

Huai Yai and Khlong Chaliang Lab

:			:	:	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: : Hu :	ai Yaı (78 Kn/2)	Khlone Chaliane Lat (77 Km2)
	:				:		
Apr	1	93.4	13.7	12.7	:	990600.	977900.
May	:	96.5	14.1	13.6	ŧ	1060800.	1047200.
Jun	:	175.9	24.4	42.9	:	3346200.	3303300.
Jul	•	173.1	23.0	48.4	:	3775200.	3726800.
Aug	:	77.0	15.6	12.0	*	936000.	924000.
Ser	t	161.3	31.5	50.8	:	3962400.	3911600.
0ct	:	114.2	25,4	29.0	:	2262000.	2233000.
Nov	:	24.1	8.7	2.0	;	156000.	154000.
Dec	‡	0.0		0.0	:	Q,	0.
Jan.	:	.7	5.7	0.0	;	0.	0.
Feb	:	0.0		0.0	:	0.	0.
Mar	:	39.2	10.7	4.1	;	319300.	315700.
	:				:		
	:	~~~~~~~			;		
Total	:	955.4	22.6	215.5	:	16309000.	16593500.

Huai Yai and Khlong Chaliang Lab

					:	Runoff (m3)		
Month	:	Rainfall (pm)	Runoff Coefficient (%)	· Runoff (#m)	: H	uai Yai (78 Km2)	Khlons Chalians Lai (77 Km2)	
	;				:			
19A	:	25.0	4.8	1.1	:	85800.	84700.	
May	:	217.3	29.8	64.7	.;	5046600.	4931900.	
Jun	:	143.2	20.2	28.9	ŕ	2254200.	2225300.	
Jul	:	64.5	13.9	8.9	:	694200.	685300.	
Aus	ć	349.9	50.9	177.5	:	13845000.	13667500.	
Sep	:	324.1	52.6	170.4	:	13291200.	13120800.	
0ct	:	81.8	21.2	17.3	:	1349400.	1332100.	
Nov	1	17.6	7.8	1.3	:	101400.	100100.	
Dec	:	0.0		0.0	:	0.	0.	
Jan	:	0.0		0.0	:	0.	0.	
Feb	;	10.4	6.9	.7	:	54600.	53900.	
Mar	:	48.4	11.8	5.7	:	444600.	438900.	
	:				;			
	:							
Total	:	1281.2	37.2	476.5	;	37167000.	36690500.	

Huai Yai and Khlong Chaliang Lab

	.			9	;	Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (sn)	4	Huai Yai (78 Km2)	Khlone Chaliane Lab (77 Km2)	
	:		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		:			
Apr	:	67.1	10.3	6.9	:	538200.	531300.	
May -	, :	85.4	12.6	10.7	;	834600.	823900.	
Jun .	, 1 ,	113.0	16.2	18.3	:	1427400.	1409100.	
Jel /	:	78.6	15.8	12.4	;	967200.	954800.	
Aus	:	127.5	22.1	28.1	:	2191800.	2163700.	
Sep	, i ,	283.8	43.1	133.9	:	10834200.	10695300.	
Oct.		15.1	12.5	1.8	;	140400.	138600.	
Nov	· í	0.0		0.0	:	0.	0.	
Bec ∙	, 0	0.0		0.0	:	0.	0.	
Jan	;	0.0		0.0	;	Ú.	0.	
Feb-	٠, :	37.1	10.4	3.8	:	296400.	292600.	
≕ Mar 1	:	- 2.8	5.9	.1	:	7800.	7700.	
	* ;	· · · · · · · · · · · · · · · · · · ·			:			
. ".	- :			,	:			
Total	- \$	815.4	27.1	221.0	:	17238000.	17017000.	

Year : 1956 Huai Yai and Khlong Chaliang Lab

	:		Du / (: Runoff (m3)			
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: Ho	uai Yai (78 Km2)	Khlons Chalians Lal (77 Km2)	
	:				:			
Apr	:	38.8	6.6	2.5	:	195000.	192500.	
May	1	152.7	21.4	32.6	:	2542800.	2510200.	
Jun	:	141.5	19.9	28.1	:	2191800.	2163700.	
Jul	:	219.7	34.1	74.9	:	5842200.	5767300.	
Aus	:	154.5	25.6	39.5	:	3081000.	3041500.	
Sep	:	291.5	48.4	141.0	:	10998000.	10857000.	
Oct	:	52.5	17.4	9.1	:	709800.	700700.	
Nov	1	2.5	5.9	.i		7300.	7700.	
Dec	:	0.0		0.0	:	0.	0.	
Jan	:	0.0		0.0	:	0.	0.	
Feb	1	12.9	7.2	.9	:	70200.	69300.	
Mar	:	125.3	21.8	27.3	:	2129400.	2102100.	
	:				:			
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	:	************	. O		:	**		
Total	:	1191.9	29.9	356.0	:	27768000.	27412000.	

Huai Yai and Khlong Chaliang Lab

Year	:	1957
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	:		Runoff Coefficient (%)	Runoff (mm)	:	Runoff (m3)		
Month	:	Rainfall (mm)			: - : Hu :	ai Yai (78 Km2)	Khlons Chalians Lad (77 Km2)	
	;	····			:			
Apr		74. i	11.2	8.2	:	639600.	631400.	
Hay	:	91.6	13.5	12.3	:	959400.	947100.	
Jun	:	144.2	20.3	29.2	;	2277600.	2248400.	
Jul	1	96,2	18.1	17.4		1357200.	1339300.	
Aus	1	215, 1	33.5	72.0	:	5616000.	5544000,	
Ser	:	227.1	40.0	90.8	;	7032400.	6991600.	
0ct	•	112.1	25.1	28.1	:	2191800.	2163700.	
Nov	;	0.0		0.0	;	0.	Û.	
Dec		0.0		0.0	:	0.	0.	
Jan	:	102.9	18.9	19.4	;	1513200.	1493800.	
Feb	:	23.6	8.6	2.0	:	156000.	154000.	
Mar	:	43.7	11.2	4.8	;	374400.	389600.	
	:				:			
	;				:	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
Total	:	1130.6	25.1	234.2	:	22167600.	21883400.	

Huai Yai and Khlong Chaliang Lab

	:		Runoff Coefficient (%)	· Runoff (nn)	:	Runoff (m3)			
Month	:	Rainfall (mm)			: - : Hu	uai Yai (78 Km2)	Khlone Chaliane Lat (.77 Km2)		
					:				
Apr	:	36.4	6.3	2.2	:	171600.	169400.		
May	;	86.3	12.8	11.0		858000.	847000.		
Jun	:	156.3	21.9	34.2	1	2667600.	2633400.		
Jul	:	255.7	33.8	99.2	:	7737600.	7638400.		
Aug	;	186.7	29.8	55.6	÷	4336800.	4231200.		
Sep	:	209.3	37.7	78.9		6154200.	6075300.		
Oct	:	55.4	17.8	9.8	:	764 4 00.	754600.		
Nov	;	0.0		0.0	:	0.	0.		
Dec	:	0.0		0.0	:	0.	0.		
Jan	:	0.0		0.0	:	0.	0.		
Feb	:	33.1	9.9	3.2	:	249600	246400.		
Mar		78.3	15.7	12.2	:	951600.	939400.		
	:				:				
	:		.,		:				
Total	;	1097.5	27.9	306.3	:	23391400.	23585100.		

Huai Yai and Khlong Chaliang Lab

	:		Runoff Coefficient (%)		:	Runoff (m3)			
Month	;	Rainfall (mm)		Runoff (mm)	:	Huai Yai (78 Km2)	Khlone Chaliane La (77 Km2)		
	:				:				
APr-		83.0	12.3	10.2	:	795600.	785400.		
May	. :	239.8	32.7	78.4	:	6115200.	6036800.		
Jun	, -	109.3	15.8	17.2	:	1341600.	1324400.		
Jul	- , 1-	220.4	34.2	75.3	;	5973400.	5798100.		
Aus `	ï	108.2	19.6	21.2	:	1653600.	1632400.		
Sep	•	442.3	68.0	300.7	;	23454600.	23153900.		
Oct 1	0-1	34.4	15.0	5.1	:	397800.	392700.		
Nov	•	23.4	8.6	2.0	1	156000.	154000.		
Dec	:	0.0		, 0.0	. :	0.	0.		
Jan -	:	3.0	- 6.0	. i	:	7800.	7700.		
Feb	:.	.3	5.6	0.0	:	0.	Û.		
Har	:	42.7	11.1	4.7	:	366600.	361900.		
	:_			- .	1				
	:				;	** ** ** ** ** ** ** ** ** ** ** ** **	, b 4 - 1		
Total	;	1306.8	39.4-	514.9	:	40162200.	39647300.		

Year	;	1960
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	:		Runoff	Runoff (mm)	:	Runoff (m3)			
Month	:	Rainfall (mma)	Coefficient (%)		: : H	uai Yai (78 Km2)	Khlons Chalians Lab (77 Km2)		
	;				:				
Aer	:	9.0	2.7	,2	:	15600.	15400.		
May	:	130.4	18.5	24.1	:	1879300.	1855700.		
Jun	:	63.7	9.8	6.2	:	483600.	477400.		
Jul	:	134.9	23.1	31.1	:	2425300.	2394700.		
Aug	:	187.3	29.9	56.0	:	4368000.	4312000.		
Ser	:	105.5	24.3	25.6	:	1996300.	1971200.		
0ct	:	109.3	24.8	27.1	:	2113800.	2086700.		
Nov	:	8.6	6.7	.5	:	39000.	38500.		
Dec	:	0.0		0.0	:	0.	0.		
Jan	:	8.1	6.6	.5	:	39000.	38500.		
Feb		23.0	8.6	1.9	å	149200.	146300.		
Mar	:	28.4	9.3	2.6	:	202800.	200200.		
	:				:				
	:				 1	********	***************************************		
Total	:	303.2	21.8	175.8	:	13712400.	13534600.		

Huai Yai and Khlong Chaliang Lab

	:			Runoff (mm)	:	Runoff (m3)		
Honth	:	Rainfall (mm)	Runoff Coefficient (%)		: - : Ho	uai Yai (78 Km2)	Khlone Chaliane Lab (77 K#2)	
	:				:			
APC	:	62.5	9.7	6.0	:	468000.	462000.	
May	:	197.6	27.2	53.7	:	4188600.	4134900.	
Jun	:	177.5	24.6	43.6	:	3400800.	3357200.	
Jg)	:	251.8	38.2	96.1	:	7495900.	7399700.	
Aug	:	243.1	37.1 ,	90.1	:	7027800.	6937700.	
Ser	:	152.9	30.4	46.4	:	3619200.	3572800.	
Oct	:	67.0	19.3	12.9	:	1006200.	993300.	
Nov	:	0.0		0.0	:	Û.	0.	
Dec	:	0.0		0.0	:	0.	0.	
Jan	:	6.6	6.4	.4	B	31200.	30900.	
Feb	:	0.0		0.0	:	0.	0.	
Mar	ŧ	21.0	8.3	1.7	;	132600.	130900.	
	:				:			
~~	:		ب ما کل باد بن کا شا که ان س کن <u>سست شا من</u> در دن ب		:		***************************************	
Total	:	1180.0	29.7	350.9	:	27370200.	27019300.	

Huai Yai and Khlong Chaliang Lab

	:		D (1	Runoff (mm)	:	Rund	off (m3)
Month	:	Rainfall (pm)	Runoff Coefficient (%)		: : }	luai Yai (* 78 Km2)	Khlone Chaliane Lat (77 Km2)
	 :				:		
Apr	:	124.6	17.7	22.0	:	1716000.	1694000.
May	1	124.3	17.7	22.0		1716000.	1694000.
Jun	;	61.7	9.6	5.9	·:	460200.	454300.
Jui	0 8	190.6	30.3	57.7	:	4500600.	4442900.
Aus	:	248.4	37.8	93.8	Í	7316400.	7222600.
Sep	4	276.3	46.4	128.2	;	999 9600.	9871400.
0ct	:	54.0	17.6	9.5	i	741000.	731500.°
Nov	ŝ	23.1	3.6	1.9	ŧ	148200.	146300.
Dec	:	2.1	5.8	. 1	:	7800.	7700.
Jan	;	0.0		0.0	:	0.	0.
Feb	:	7.4	6.5	. 4	:	31200.	30800.
Har	:	40.8	10.9	4.4	:	343200.	338800.
	:				:		
	:				:		
Total	:	1153.3	30.0	345.9	:	26930200.	26634300.

Huai Yai and Khlong Chaliang Lab

-	:	•	Runoff Coefficient (%)	Runoff (mm)	Runoff (m3)			
Month	:	Rainfall (mm)			: - : Ho	vai Yai (78 Km2)	Khlone Chaliane La (77 Km2)	
	 !				;		7	
Aer	_*t	142.4	20.0	28.4	:	2215200.	2186800.	
May	i	142.6	20.1	23.6	;	2230300.	2202200,	
Jun	, :	159.5	22.3	35.5	:	2769000.	2733500.	
Julii,		290.9	43.3	125.9	:	9820200.	9694300.	
Aus	, 1	343.0	~ 50 . 1	171.8	:	13400400.	13228600.	
Sep	• 1	276.9	46.5	128.7	:	10038600.	9909900.	
0ct	;	257.7	44.0	113.3	:	8837400.	8724100.	
Nov		63.5	13.8	8.7	:	678600.	669900.	
Dec 🕙		19.1	8.0	1.5	:	117000.	115500.	
Jan	:	7 -	5.7,	0.0	:	Û.	0.	
Feb	:	.2	5.6	0.0	:	0.	0.	
Mar	:	12.9	7.2		:	70200.	69300.	
			-	-	:			
-	:		-12		:			
Total	•	1709.4	37.6	643.3	:	50177400.	49534100.	

Year : 1964

	• •		5 - 11	Runoff (mm)	:	: Runoff (m3)			
Month	; ;	Rainfall (mm)	Runoff Coefficient (%)		: - : Ho	vai Yai (78 Km2)	Khlone Chaliane Lab (77 Km2)		
	;				·	. — — — — — — — — — — — — — — — — — — —			
Arc	:	47.9	7.8	3.7		288600.	284900.		
May	:	284.4	38.5	109.4	:	8533200.	8423800.		
Jun		157.0	21.9	34.3	:	2675400.	2641100.		
Jul	;	200.3	31.6	63,2	:	4929600.	4866400.		
Aus	:	282.5	42.2	119.2	:	9297600.	9178400,		
Ser	;	367.3	58.2	213.7	;	16663600.	16454900.		
0ct	:	123.4	26.6	32,8	:	2558400.	2525600.		
Nov	:	12.0	7.1	.8	;	62400.	61600.		
Dec	i	2.0	5.8	.1	:	7800.	7700,		
dan	:	0.0	-~	0.0	ţ	0.	0.		
Feb		25.8	8.9	2.2	:	171600.	169400.		
Mar	:	86.2	16.8	14.4	:	1123200.	1103800.		
	:				:				
Total	:	1588.8	37.4	593.8	:	46316400.	45722600.		

Huai Yai and Khlong Chaliang Lab

	:		n		:	Run	off (m3)
Month	:	Rainfall (ma)	Runoff Coefficient (%)	Runoff (mm)	; - : Hu :	uai Yai (78 Km2)	Khlons Chalians Lab (77 Km2)
	 :				:		
Arr	:	67.9	10.4	7.0	:	546000.	539000.
May	:	83.9	12.5	10.4	:	311200.	300800.
Jun	:	226.6	31.0	70.2	:	5475600.	5405400.
Jul	*	99.3	18.5	18.3	;	1427400.	1409100.
Aus	:	255.4	38.7	98.8	:	7706400.	7607600.
Ser	:	234.4	41.0	96.1	:	7495800.	7399700.
0ct	:	37.4	15.4	5.7	:	444600.	438900.
Nov	:	19.5	8.1	1.5	:	117000.	115500.
Dec	:	3.4	6.0	.2		15600.	15400.
Jan	•	9.4	6.3	.6	;	46800.	46200.
Feb	:	13.2	7.3	.9 `	:	70200.	69300.
Mar	:	14.0	7.4	1.0	:	78000.	77000.
	:				:		
	:				;		
Total	:	1064.4	29.2	310.7	:	24234600.	23923900.

Huai Yai and Khlong Chaliang Lab

	:		Runoff Coefficient (%)	.Runoff (san)	:	: Runoff (m3)			
Month	:	Rainfall (ma)			: - : Ho	ai Yai - (78 Km2)	Khlong Chaliang La (77 Km2)		
	 ;				;				
Apr	:	45.1	7,4	3,3	;	257400.	² 254100.		
May	;	214.4	29.4	63.0	•	4914000.	4851000		
Jun	;	82.6	12.3	10.1		787800.	777700.		
Jul	;	268.7	40.4	108.5	:	3463000.	8354500.		
Aus	:	260.3	39.4	102.5	:	·7995000.	7892500.		
Sep		117.7	25.8	30.3	;	2363400.	2333100.		
0ct	:	106.7	24.4	26.0	:	2028000.	2002000.		
Nov	:	26.9	9,1	2.4	:	187200.	134800.		
Dec	:	10.4	6.9	.7	:	54600.	53900.		
Jan	;	.5	5.6	0.0	:	0.	0,		
Feb	;	5.1	6.2	.3	:	23400.	23100.		
Mar	:	,3	5.6	0.0	:	0.	Ũ, .		
,,_,	:	•-			:				
			,,		;				
Total	:	1138.7	30.5	347.1	:	27073800.	26726700.		

Huai Yai and Khlong Chaliang Lab

	:					Run	off (m3)
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: - : Hu :	ai Yai (78 K#2)	Khlone Chaliane Lat (77 Km2)
	 :				;		
Apr	ŧ	185.0	25,6	47.3	:	3689400.	3642100.
May	:	199.9	27.5	54.9	;	4282200.	4227300.
Jun		74.4	11.2	8.3	;	647400.	639100.
Jul	: *	151.9	25,3	38.4	:	2995200.	2956800.
Aug -	:	149.0	24.9	37.1	:	2893800.	2856700.
Sep		226.5	40.0	90.6	:	7066800.	6976200.
- Oct	· . (17.1	12.8	2.1	:	163800.	161700.
- Nov-		16.4	7.7	1.2	:	93600.	92400.
Dec	:	0.0	·	0.0	:	0.	0.
. 'Jan	. ;	. 0.0 - 1		0.0	:	Ģ.	٥,
Feb	:	10.2	6.9	.7	;	54600.	53900.
- Mar	5 t	18.3	7.9	1,4	:	109200.	107300.
	:	- ,			: 		
Total	:	1043.7	26.9	282.0	:	21996000.	21714000.

Year : 1968

Month	:		Runoff Coefficient (%)	Runoff (mm)	:	Runoff (m3)		
	: :	Rainfall (mm)			: - : Hu :	ai Yai (73 Km2)	Khlons Chalians Lat (77 Km2)	
	;				:	**************************************		
Arr		113.0	16.2	18.3	:	1427400.	1409100.	
May	:	182.3	25.2	45.9	:	3580200.	3534300.	
Jun	:	134.1	19.0	25.4	:	1981200.	1955800.	
Jul	•	215.4	33.5	72.1	:	5623800.	5551700.	
Aus	:	55.9	12.8	7.1	:	553800.	546700.	
Ser	:	173.9	33.8	60.4	\$	4711200.	4650800.	
0ct	:	62.1	18.6	11.5	:	897000.	885500.	
Nov	:	17.8	7.9	1.4	:	109200.	107800.	
Dec	:	0.0		0.0	:	0.	٥.	
Jan	1	66.9	14.2	9.4	:	733200.	723300.	
Feb	:	0.0		0.0	:	0,	0.	
Mar	:	24.7	8.8	2.1	:	163300.	161700.	
	:				:			
			* = = = 7 + 11		:			
Total	:	1051.1	24.1	253.6	:	19780800.	19527200.	

Huai Yai and Khlong Chaliang Lab

	:		Runoff Coefficient (%)	Runoff (pm)	:	: Runoff (m3)			
Month	•	Rainfall (mm)			: Ht	ai Yai (78 Km2)	Khlons Chalians Lat (77 Km2)		
,	:				;				
Arr	:	58.5	9.2	5.3	:	413400.	408100.		
Hay	:	92.0	13.5	12.4	:	967200.	954300.		
Jun	:	145.0	20.4	29.5	:	2301000.	2271500.		
Jul	;	123.6	21.6	26.6	:	2074800.	2048200.		
Aus	:	165.6	27.1	44.8	:	3494400.	3449600.		
Sep	:	280.7	47.0	131.9	:	10283200.	10156300.		
Oct	;	179.6	33,9	60.8	:	4742400.	4681600.		
Nov	:	.8	5.7	0.0	:	0.	٥.		
Dec	:	0.0		0.0	:	Û,	0.		
Jan	:	0.0		0.0	:	0.	ŷ.		
Feb	:	.2	5.6	0.0	:	Û.	0.		
Mar	;	60.1	13.4	8.0	;	624000.	616000.		
	:				:				
	:			,,,drannuad 1 4	;	·446445477			
Total	•	1106.1	28.9	319.3	:	24905400.	24586100.		

Year : 1970

	:		Runoff Coefficient (%)	. Runoff (mm)	:	: Runoff (m3)		
Month	; ; ;	Rainfall (mm)			: . : H:	uai Yai (78 Km2)	Khlone Chaliane Lat	
	:				:	Ţ	 	
APr	:	80.6	12.0	9.6	:	748800.	739200.	
May	:	142.8	20.1	28.7	:	2238600.	2209900.	
Jun	:	274.1	37.1	101.6	i	7924800.	7823200.	
Jul	:	213.4	33.3	71.0	;	5538000.	5467000.	
Aus	:	252.3	38.3	96.6	:	7534800.	7438200.	
Ser	:	231.8	40.7	94.3	;	7355400.	7261100.	
9ct	:	50.3	17.1	8.6	:	670500.	662200.	
Nov	:	3.0	6.0	.1	:	7800.	7700.	
Dec	8	13.0	7.3	.9	:	70200.	69300.	
Jan	:	.9	5.7	0.0	;	0.	0.	
Feb	:	24.7	8.8	2.1	:	163800.	161700.	
Mar	:	12.3	7.2	.8	:	62400.	61600.	
	f				:			
	:				:			
Total	:	1299.2	31.9	414.3	:	32315400.	31901100.	

Huai Yai and Khlong Chaliang Lab

	:		n			Runoff (m3)		
Month	:	Rainfall (mm)	Runoff Coefficient (%)	Runoff (mm)	: H	uai Yai (78 Km2)	Khlone Chaliane Lab (77 Km2)	
	;				:			
Apr	- 1	108.2	15.6	16.8	:	1310400.	1293600.	
May	.:	210.3	28.9	60.7	:	4734600.	4673900.	
Jun	, :	65.8	10.1	6.6	:	514800.	508200.	
Jul	. :	117.0	20.8	24.3	:	1895400.	1871100.	
Aus	•	180.5	29.0	52.3	:	4079400.	4027100.	
Sep	T. 12	123.0	26.5	32.5	:	2535000.	2502500.	
- Oct	. [-	27.7	14.2	3.9	:	304200.	300300.	
Nov		2.4	5.9	.1	:	7800.	7700.	
Dec		2.7-	5.9	1	:	7800.	7700.	
Jan		0.0		. 0.0	:	0.	0.	
Feb	' <u>'</u> :	30.2	9.5	2.8	:	218400.	215600.	
Han	, · ·	18.3	7.9	1.4	:	109200.	107800.	
	- ; -	; , , , , , , , , , , , , , , , , , , ,			:			
	:	,		*	;			
- Total	_ :	836.1	22.7	201.5	:	15717000.	15515500.	

Year : 1972

	:				:	: Runoff (m3)			
Month	;	Rainfall (mm)	Runoff Coefficient (%)	Runoff (am)	:	Huai Yai (78 Km2)	Khlone Chaliane Lat (77 Km2)		
	;		**************************************	· <i></i>	:				
Apr	:	63.3	9.8	6.2	:	483600.	477400.		
May	:	60.0	9.4	5.6	:	436800.	431200.		
Jun	:	228.3	31.2	71.2	1	5553600.	5482400.		
Jel	:	165.5	27.0	44.6	:	3478300.	3434200.		
euA	:	192.4	30.5	58.6	ŧ	4570800.	4512200.		
Sep	:	115.1	25.5	29.3	2	2235400.	2256100.		
Oct	:	109.4	24.8	27.1	:	2113800.	2096700.		
Nov	:	22.6	8.5	1.9	:	148200.	146300.		
Dec	:	3.3	6.0	.1	:	7800.	7700.		
Jan	į	0.0		0.0	•	0,	ů.		
Feb	:	6.1	6.4	.3	:	23400.	23100.		
Mar	:	65.0	14.0	9.1	:	709800.	700700.		
	:				:				
	:		*		:	- #1			
Total	:	1031.0	24.6	254.0	:	19812000.	19558000.		

Huai Yai and Khlong Chaliang Lab

	:		w		ſ	Runoff (m3)		
Month	;	Rainfall (mm)	Runoff Coefficient (%)	Runoff (na)	Ho	Jai Yai (78 Km2)	Khlons Chalians Lab (77 Km2)	
	;		, — , , , , , , , , , , , , , , , , , ,		:			
Apr	:	37.7	6.5	2.4	:	187200.	184800.	
Hav	\$	127.9	18.2	23.2	:	1309600.	1786400.	
ปบก	:	139.7	19.7	27.5	:	2145000.	2117500.	
Jul	:	155.8	25.8	40.1	:	3127800.	3087700.	
Aus	;	154.0	25,6	39.4	:	3073200.	3033800.	
Ser	:	331.5	53.6	177.6	:	13852800.	13675200.	
0ct	:	34.1	15.0	5, 1	:	397800.	392700.	
Nov	;	.2	5.6	0.0	:	0.	0.	
Dec	;	0.0		0.0	:	0.	0.	
Jan	:	.3	5, 6	0.ú	:	v.	ů.	
Feb			9.3	2.6	:	202800.	200200.	
nan	:	2.6	5.9	.1	:	7800.	7700.	
	:				:			
	 :		**************************************		:	* * *	· &co.c	
Total	:	1012.5	31.4	318.0	:	24804000.	24486000.	

Huai Yai and Khlong Chaliang Lab

Year : 1974

:					:	Run	off (m3)
Month	?	Rainfall (mm)		Runoff (ma)	: - : H:	Jai Yai (,78 Km2)	Khlons Chalians Lat (77 Km2)
	 ,		* ** *** *** ** ** ** ** ** ** ** ** **		:		* 1
Apr	;	57.9	9.1	5.2	:	405600.	400400.
May	:	79.2	11.8	9.3	.;	725400.	716100.
Jun	;	76.7	11.5	8.8	:	686400.	677600.
Jul	;	136.8	23.3	31.8	;	2480400.	2448600.
Aus	:	157.0	25.9	40.6	•	3166800.	3126200.
Sep		108.7	24.7	26.8	:	2090400.	2063600
0ct	:	135.3	28.1	38.0	:	2964000.	2926000.
Nov	:	11.5	7.1	.8	:	62400.	61600.
Dec	:	0.0		0.0	i	0.	Ú.
Jan	:	53.7	13.2	7.7	:	600600.	592900.
Feb	:	43.7	11.2	4.8	:	374400.	369600.
Mar	;	116.2	20.6	23.9	:	1864200.	1840300.
	:				;		:
	:	,, ,, 48 4 4 4 4 4 4 4 4 4 4 4 4			:		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
Total	:	981.7	20.1	197.7	:	15420600.	15222900.

Huai Yai and Khlong Chaliang Lab

Year : 1975

Month	:		Runoff Rainfall Coefficient (mm) (%)		4	Runoff (m3)	
	:			Runoff (mm)	: H	uai Yai (78 Km2)	Khlons Chalians Lab (77 Km2)
	, ;				:		
Apr	•	.1	1.6	0.0	:	0.	0.
May	- ;	152.0	21.3	32.3	:	2519400.	2437100.
Jun	- :	87.9	13.0	11,4	:	889200.	877800.
·Jul	:	113.9	21.0	24.9	:	1942200.	1917300.
euA	: `	213.3	33.3	71.0	:	5538000.	5467000.
Sep	:	137.2	34.9	65.3	•	5093400.	5028100.
- Oct	3	107.7	24.5	26.3	:	2051400.	2025100.
Nov	•	6.4	6.4	,4	;	31200.	30800.
Dec	? •	0.0		0.0	:	0.	0.
` Jan	< i	0.0		0.0	:	θ.	0.
Feb	. :	17.2	7.8	1.3	:	101400.	100100.
. Har	;	36.3	10.3	3.7	:	283600.	234900.
	:	 					
-	. :	:	•	`	:		
Total	:	927.0	25.5	235.6	:	18454800.	18218200.

Huai Yai and Khlong Chaliang Lab

Year : 1976

	:		Runoff		;	Runoff (m3)		
Month	:	: Rainfall Coefficient Runo	Runoff (mm)	: H	uai Yai (78 Km2)	Khlons Chalians Lab (77 Km2)		
	:				:			
Arr	:	37.4	6.4	2.3	:	179400.	177100.	
May	1	230.7	31.5	72.6	:	5662300.	5590200.	
Jun	:	110.3	15.9	17.5	:	1365000.	1347500.	
Jul	4	236.6	36.3	85.8	:	6592400.	6605600.	
Aus	:	300.1	44.5	133.5	:	10413000.	10279500.	
Ser	:	309.0	50.7	156.6	;	12214800.	12058200.	
0ct	:	173,9	33.1	57.5	:	4485000.	4427500.	
Nov	:	16.9	7.8	1.3	:	101400.	100100.	
Dec	:	0.0		0.0	:	0.	Ò.	
Jan	:	.2	5.6	0.0	:	Ů.	Û,	
Feb	:	0.0		0.0	:	0.	0.	
Nar	` !	3.2	6.0	.1	:	7300.	7700.	
	1				:			
	:				;			
Total	:	1418.3	37.2	527.2	:	41121600.	40594400.	

Huai Yai and Khlong Chaliang Lab

Year : 1977

	:		Dungff		:	Run	off (m3)
Month	# # #	Rainfall (mm)	Runoff 1 Goefficient (%)	Runoff (sm)	: Hu	ai Yai (78 Km2)	Khlone Chaliane La (77 Km2)
	:				:	~~~~**	
Arr	:	34.6	6.3	2.3	:	179400.	177100.
May	:	160.3	22.4	36.0	:	2808000.	2772000.
Jun	:	100.8	15.7	17.0	:	1326000.	1309000.
Jul	;	133.1	22.8	30.3	:	2363400.	2333100.
Aus	:	167.0	27.2	45.4	:	3541200.	3495800.
Ser	:	189.0	35.1	66.3	1	5171400.	5105100.
Oct		90.4	22.3	20.1	:	1567800.	1547700.
Nov	:	.3	5.6	0.0	:	Û.	0.
Dec	•	17.3	7.8	1.3	:	101400.	100100.
Jan	:	0.0		û.Û.	;	Û.	Ú.
Feb	:	2.3	5.9	.1	:	7800.	7700.
Mar	:	33.0	10.5	3.9	:	304200.	300300.
	:				:		:
	:				:		
Total	•	943.6	23.6	222.7	:	17370600.	17147900.

Table 4.2 Calculation Sheet of Operation Study

	SPILL OUT VOLUME	8 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		10 SPILL-OUT VOLUME	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
9 IRRIGATION WATER DEMAND	2934. 3977. 1686. 330. 2380. 7546. 12018. 5520.		9 IRRIGATION WATER DEMAND	2934. 3972. 4020. 1086. 330. 7880. 1998. 3288. 12018.	
	INFLOW OF R. PASAK	0. 0. 489. 7737. 508. 1159.28. 29990. 9708.		INFLOW OF R.PASAK	1619, 1410, 465, 596, 57213, 57213, 11620, 5730, 5736,
Reservoir	8 Suplementapy Water Supply	29377 79777 1685 1685 00 00 00 00		7 Suplementary Water Supply	1795 7795 17867 1787 1787 100 100 100
Yai	6 DOWNSTREAY REQUIREMENT	2557. 2557. 2557. 2557. 2557. 2557. 2557.		6 DOWNSTREAM REQUIREMENT	257 257 257 257 257 257 257 257
Huai Saduang	¥A TER SUPPLY	155. 155. 155. 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		WATER SUPPLY	155 150 150 00 00 150 150
(1)	S INFLOW OF HUAI S, YAI	0.00.00.00.00.00.00.00.00.00.00.00.00.0		5 IRFLOW OF HUAI S.YAI	211. 4511. 3840. 3715. 7056. 4736.
	7 PRECIPI- TATION	00 00 00 00 00 00 00 00 00 00 00 00 00		APRECIPI- TATION	28.4 2 1 1 2 2 2 3 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4
	2 SVAPORATION	8 9 1 1 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		2 EVAPORATION	143 134 76 136 101 113 157 150 150
** ; :* - ; :	CAPACITY	1356. 1356. 8990. 9094. 14040. 13505.		1 CAPACITY	11260. 8436. 43636. 2680. 2680. 6278. 9783. 11581. 14040. 13491.
1970	MONTH	######################################	1971	H T M T H	FEBRARA STORES OF THE STORES O

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Yai
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10 SPILL-OUT ' VOLUME		10 SP141-0UT VGLIHE	3282 2486 2486
9 IRRIGATION HATFR DEMAND	2934, 3972, 4020, 1686, 130, 2880, 9546, 1998, 12018, 5520,	9 IRRIGATION WATER DEMAND	2934, 3972, 4626, 1686, 330, 2880, 1998, 12614, 5520,
TYFEOW OF R.PASAK	1324. 459. 687. 10638. 1842. 55708. 55138. 25464.	INFLOW OF R. PASAK	1430 112 112 0 0 2225 29554 128120 168702 61288
7 SUPLFMENTAP1 WATER SUPPLY	1610. 8513. 4020. 999. 1927. 7704. 0.	7 SUPLEHENTAPY WATER SUPPLY	3860 3860 14870 1380 00 00 00
6 DOWNSTREAM REOUIREMENT	2557 2567 2667 2667 2667 2667 2667 2667	6 DOWNSTREAM REGUIREMENT	257. 257. 257. 257. 257. 257.
WATER Supply		WATER	155 165 165 166 166 166 166 166 166 166
S INFLOW OF HUAI S.YAI	710. 710. 710. 710. 7573. 2873. 7837. 713.	S INFLOW OF HUAL S, YAL	605 605 605 703 7131 5347 5347 5460 667
APRECIPI- TATION	484400WV84 	4 PRECIPI- 15110N	57. 21. 21. 12. 13.9. 17.7. 17.7.
2 3 EVAPORATION	2001 2001 2004 2004 2006 2006 2006	2 EVAPORATION	00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
NONTH . CAPACITY.	10560 6563 2775 2775 2167 6433 6433 6433 1790 1790 1025 10184	CAPACITY	8302- 4626- 544- 544- 11670- 11670- 14040- 14040- 14040- 14040- 14040-
MONTH	P N O N P L L X P R R R P C C C C C C C C C C C C C C C	1975 HINOH	FEB. TAR. TAR. JUN. JUL. SEP. SEP.

	SPILL-OUT Volume	00000000000
	9 IRRIGATION SPILL-OUT WATER DEMAND VOLUME	2032 3973 4020 1080 1080 100 1204 1204 1201 1201 1201 1201 1201
	THELOW OF R. PASAK	000000000000
	7 SUPLEMENTARY WATER SUPPLY	20020 10020 10020 10020 10020 1003 1003
	6 Downstreah Reguirement	22222222222222222222222222222222222222
	#ATER SUPPLY	66 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6
	5 	7,74
	, PRECIPI- TATION	
	EVAPORATION	##QQQQCQQ
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1978	НОИТН	A PARENT CONTRACTOR CO

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	0
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13 TOTAL SPILL-OUT VOLUME	840 00. 840 00. 448896. 1121. 00.	13 TOTAL SPILL-041 VOLUPE	0. 0. 7028. 16935. 6834. 37412. 0.
"12 PILL-OUT OL, AFTER GGULATED	6 0 0 0 0 0 4 4 4 5 7 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 IPILL-011 OL. AFTER	0. 0. 7026. 16792. 6725. 7816. 3715.
11 SHRPLUS S OR V DEFICIT R	1.3040. 1.0040. 1.0040. 1.0040. 1.0040.	11 SURPLUS S OR V DEFICIT R	-2332. -2509. -2579. 17121. 16722. 6725. 7816. 7818. -18809.
10 TOTAL DEMAUD	3885 3885 22865 1106 1106 2867 2862 1018 1018 1018	10 TOTAL DEMAND	3169. 3832. 3965. 1104. 7867. 7867. 8327. 5057.
y DDWNSTREAM REQUIRI→ HENT	\$C\$	o PASTREAM REOUIRE→ MENT	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$
R Water Supply	185. 185. 185. 00. 00. 180.	8 Water Do Supply	24.24.2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 24.2 2 2 2
7 16a710k 1ep req.	2152. 2913. 2948. 1236. 242. 7000. 1465. 2411. 8411. 6048.	7 RIGATION ATER REG.	2152. 2948. 1236. 212. 212. 7000. 1465. 4048.
6 NFLOW 1PR	0.00.00.00.00.00.00.00.00.00.00.00.00.0	6 INFLOW IN	837. 3123. 586. 5184. 18725. 19739. 10143. 2866.
5 PTRECT I POLUME	00000000000000000000000000000000000000	7. 1.1.1. 1.00.1 1.00	000000000000000000000000000000000000000
4 UISTED PACTIY	0. 0. 0. 0. 0. 18157. 74780. 24780. 2468.	4 4njuster Capacity S	18449. 15683. 15683. 11654. 14687. 24780. 24780. 24780. 24780. 17850.
FCIPI- AN ATION CA	6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 PRFCIP1- A1 TATION C	2000 C C C C C C C C C C C C C C C C C C
2 FVAPD- PRI RATION TJ	81 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	2 EVADO- PE RATION 1	108. 108. 108. 108. 108. 108.
	0.0 2600.0 2600.0 2600.0 14054.0 24740.0 19647.0 18527.0	1 APAG11Y	16118. 11686. 14617. 24780. 24780. 24780. 17490. 17497.
HONTH CAPACITY	SAFES. SAFES.	1954 MONTH C	М В В В В В В В В В В В В В В В В В В В

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13 TOTAL SPILL-OUT VOLUME	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 TOTAL SPILL-OUT VOLUME	88876 45270 50157
12 OL. AFTER EGULATED	0.00.00.00.00.00.00.00.00.00.00.00.00.0	12 PILL-OUT OL. AFTER EGULATED	45270 45270 29920
SURPLUS SI OR VO DEFICIT RI	1 3 3 4 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	11 SURPLUS S OR V OEFICIT R	.3169. .3574. .3063. .8180. .65199. .2068. .47937. .29095. .79095.
10 TOTAL DEMAND	2000 2000 2000 2000 2000 2000 2000 200	10 TOTAL DEMAND	3169. 3882. 1221. 7247. 7862. 7862. 3247. 3246.
9 WNSIREAN RECUIRF MFNI	#	9 30411 STPEAN REQUIRE— MFNT	7 X X X X X X X X X X X X X X X X X X X
B WATER NOV SUPPLY F	155. 155. 155. 150. 00. 00. 150.	8 WATER DOI Supply	1555 1555 1555 1550 1550 1550
7 116471014 1ER REG.	2152. 2913. 7948. 1236. 242. 7000. 1665. 4048. 6813.	7 116ATION TER RED.	2152. 2913. 2948. 1256. 2412. 7000. 1465. 2411. 8811.
INFI DU TRP	547. 19706. 19706. 199706. 99981. 5784. 258.	6 N1 LOW 188	258. 258. 907. 10401. 598146. 50764. 33166.
5 DIRECT 1 PILL~0UT VOLUME	00000000000000000000000000000000000000	5 018ECT 1 SP1LL-0UT VOLUME	000000000000000000000000000000000000000
4 (1.11) STFD APACITY SI	15326. 10096. 6245. 7800. 16794. 17985. 24.780. 24.762. 70741.	4 10.115.115.115.115.115.115.115.115.115.1	16948. 11717. 8091. 50028. 13702. 16407. 26777. 27714. 2677.
RECIPI- AL	0 29. 29. 17. 18. 17. 17. 10. 10.	3 RECIPI - AI TATION C.	500 500 500 500 500 500 500 500 500 500
EVAPO- PI RATION	25. 51. 75. 75. 75. 71. 889. 114.	2 EVAPO- PF RATION 1	28
PACITY	10158 2825 2825 1676 17883 24780 24780 15772 15772	A P A C 1 T Y	11779. 8143. 5028. 13507. 118073. 24,780. 24,780. 15567.
MONTH C	A H H A H H H H H H H H H H H H H H H H	1955 MONTH C	ATA ATA ALL ALB AN ANUTO PROCT.

13 TOTAL SPILL-0HT VOLUME	0.00.00.00.00.00.00.00.00.00.00.00.00.0	13 TOTAE SPILL-0117 VOLUME	13904.
12 SPILL-OUT VOL. AFTER REGULATED	00. 7.00. 7.00. 2.7.4.7. 2.7.89.1.	SPILL-OUT VOL. AFTER RFGULATER	100 13773 110774
11 SURPLUS OR DEFICIT	132255 13768 13965 13965 14589 14589 121611 12168 11017	11 SURPLUS OR DEFICIT	-3169. -3607. -2419. 1418. 4080. 1368. 11977. -9675.
10 TOTA1 DEMAND	3393 3393 3393 2323 2323 3323 3324 3325 3325 3325 332	10 101Ai DEMAHD	3169. 3842. 3765. 7721. 7721. 7747. 7746. 9675.
9 WWNSTREAM REQUIRE- MFNT	~~ @ & & & & & & & & & & & & & & & & & &	9 POWRSTRFAM REQUITE MENT	2
B Hater DO Supply	22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	B Water Do Supply	155.
7 Rigation Ater Reo.	20152 20183 1236 1236 1246 24112 4048	7 Rigation Ater Reg	2152. 2913. 2948. 1736. 242. 7112. 7112. 7465. 88113. 6048.
INFLOW TRE	954. 2801. 1964. 16486. 14468. 31137. 7567.	inflow IRI	225. 1546. 3639. 5184. 6415. 19400. 114020.
5 P1RECT SP1L-001		5 18 F C 1 P I L L - OU T V O L UME	\$ 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
A HISTED APACITY S	14287- 11982- 8148- 4164- 4728- 5614- 19235- 24780- 24780- 22780- 17419-	4 6 JUSTEB APACITY	16311 13070 9611 6965 12444 13490 74780 74780
3 RECIPI- A TATION C	2 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	5 RECIPI- A TATION C	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
EVAPO- RATION	EX 6 x k k c C C L K L K C C C C C C C C C C C C C C	2 EVAPO- P RATION	28 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
CAPACITY	12053 4184 4744 5786 5788 19153 74780 24780 24780 17515 1602	° C A P A C I T Y	13142. 9463. 6992. 8383. 12455. 13812. 24780. 24780. 14966.
MONTH	00000000000000000000000000000000000000	1959 MONTH-0	MAPARES CONTRACTOR CON

HINTH CAPACITY EVADO- PRECEPT - ADJUSTED DIRECT THE UND IRRIGATION WATER DOWNSTREM TOTAL STREPLAND TOTAL STREET TOTAL STRE	13 TOTAL SPIIL—0UT VOLUME	0.00.00.00.00.00.00.00.00.00.00.00.00.0		13 TOTAL SPILL-OUT VOLUME	0.00.00.00.00.00.00.00.00.00.00.00.00.0
The corrective capacity The correction The correcti	12 PILL-OUT OL, AFTE EGULATED	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		12 OL. AFTE EGULATEO	70.40 60 6" PV
NOWTH CARCITY EARD PRECIPION TAILON TA	11 21,05 38 1011	5169 53169 53655 53655 53179 50878 50878 50878 50878	•	11 URPLUS OR EFICIT	-3169. -3574. 57974. -5792. -5792. 18417. 18417. 13606.
HONTH CAPACITY EVADO— PRECIPI— ADJUSTED DIRECT NILUD IRRIGATION WATER DOUNGSTREAM RATION TAILOR CAPACITY SPILL-OUT NILUD IRRIGATION WATER DOUNGSTREAM RATION TAILOR CAPACITY SPILL-OUT NILUD IRRIGATION WATER DOUNGSTREAM RATE NISS. 10. 10. 10. 10. 10. 10. 10. 10. 10. 10.	10 0141 E4AN	444644444444444444444444444444444444444		10 TOTAL DEMAN	3169. 3832. 39832. 2225. 7104. 7862. 3246.
MONTH CAPACITY EVAPOR PRECIP - ADJUSTED DIRECT WILLOW IRRIGATION WATER REG. SUPPLY	≪ Ш	\$		ΑH	$\begin{array}{c} x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow x \Rightarrow x \Rightarrow x \Rightarrow x \\ x \vdash x \Rightarrow x \Rightarrow$
FORTH CAPACITY EVAPO- BRECIPL ADJUSTED DIRECT SHILLOUT BATER FEG. 1210. BA	A TER DC	~4wv ~0v0000000		7 L L L L L L L L L L L L L L L L L L L	
MONTH CAPACITY EVAPO- PRECIPI- ADJUSTED DIRECT NELUW II MAITON TATION CAPACITY SPILL-OUT NOLUME NOT NEED NEED NEED NEED NEED NEED NEED NEE	7 IGATION TER REQ	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		7 164710H TER RED	2152. 2913. 2913. 1246. 242. 7000. 1465. 6813.
HONTH CAPACITY EVADO- PRECIPI- ADJUSTED DIRECT NOLUNI VOLUNI VOLU	6 NFLOW IR	50 50 50 50 50 50 50 50 50 50 50 50 50 5		A NFLOW I	258. 9757. 1667. 1771. 31692. 7627. 7657. 25697.
MONTH CAPACITY EVAPO- PRECIPI- ADJUSTED PRECIPI-	5 17 - 0 0 1 17 - 0 0 1	C0C0CC0K-000		5 18EGT 11L-001 OLUNF	
MONTH CAPACITY EVAPO- PRECIPI- A RATION TATION C RATION TATION DAN 1858- 61- 84- 175- 175- 175- 175- 175- 175- 175- 175	4 JUSTED PACITY S	7.5 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		LUSTED I	12686. 5866. 11690. 110908. 11772. 26780. 26780.
JAN. 15284. 103. JAN. 15284. 103. HAR. 7303. 65. HAR. 7303. 65. HAR. 11359. 61. JUL. 24780. 177. JUL. 24780. 173. HOU. 15717. 77. JUL. 24780. 173. HOV. 13850. 103. DEC. 17757. 76. JAN. 85873. 48. HAR. 11665. 77. JUN. 24780. 173. NUC. 24780. 139.	3 FC1P1- A AT104 C	00-radecence		\$ FCTP1- A ATION C	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
JAN. 15284 JAN. 15284 JAN. 15284 JUL. 24780 JUN. 24780 JUL. 24780	7 VAPO- P Aliou	00000000000000000000000000000000000000		7 VAPO- P ATION	7.2.2.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.
M	APACII	5284 73594 73594 73594 7359 74780 7759 7757		1 APACITY	9517 5893. 11682. 11038. 11665. 74780. 24780. 24780.
	MONTH	0001 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1963		PERR PERR PERR PERR PERR PERR PERR PERR

13 TOTAL SPILL-OUT VOLUME	27287. 1600. 1606. 5439. 15263. 12845.	13 1074 SPILL-0UT VOLUME	3530. 3530. 22321. 19039. 0.0
12 VOL. AFTER REGULATED	0.0 0.0 27287 1606. 5340. 15061. 29920. 12704.	12 SPILL-OUT VOL. AFTER REGULATED	3530. 3530. 18875. 18875.
SURPLUS S OR V DEFICIT R	13832 13832 13832 13832 15832 15840 12704 12704	11 SHRPLUS OR V	13169. 13579. 112834. 1112821. 10545. 178835. 198875.
10 TOTAL DEMAND	8882 8882 89882 8782 7862 8862 8083 1017	10 TOTAL DEMAND	3169. 38832. 38853. 1224. 1104. 2862. 3246. 9675. 5035.
O ENSTREAM REDUIRE- MENT	\$	9 Nownstream Reguire— Meht	8 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
8 WATER DO SUPPLY	150 150 150 150 150 150	B WATER I SUPPLY	155. 155. 150. 00. 150.
7 Rigation Aten Red.	2152. 2948. 1236. 747. 7000. 1445. 88113.	7 RRIGATION Water Reg.	2152. 2913. 2913. 1236. 242. 7000. 7000. 7411. 8813.
A IRI	0. 3255 42697 45697 17368 17368 33166 129	o INFLOUIF	998. 386. 12500. 13492. 3027. 29334. 22121.
S PIRECT SPILI-OUT VOLIME	00 00 00 00 00 00 00 00 00 00 00 00 00	5 DIRECT SPILL-OUT VOLUME	
A LUSTED A	20545. 17271. 15330. 9527. 10527. 24780. 24780. 24780. 24780. 24780.	APACITY	18522. 15289. 12367. 8709. 6505. 14765. 24770. 24770.
3 RECIPI- AL 1A110N C	2827 2850 2800 2800 2800	3 TATION C	29. 12. 12. 12. 12. 12. 13. 13. 13. 13. 13. 13.
EVAPO- PERATION	115. 105. 105. 127. 137. 137. 108.	2 EVAPÜ- P RATION	104. 201. 201. 201. 101. 100. 100. 100. 100
TAPACITY	17876. 13439. 9565. 10357. 24780. 24780. 24780. 24780. 19751.	1 6 a P a C 1 T Y	15354. 12655. 8788. 6488. 17701. 76780. 76780. 15042. 9927.
MOMTH C	DAM. REB. AAR. JUR. JUR. AUG. AUG.	1965 PORTH	A A A A A A A A A A A A A A A A A A A

MON TH	AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA	1967 NONTH	AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA
CAPACITY	AL 88 25	CAPACITY	0 24,80 36,80 42,73 40,73 72,66 59,72
2 FVAPO- RAT104	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2 E VAPO- RA I I 0%	0 6 6 6 7 7 8 6 8 7 6
S PRECIPITATION	60+00%V6##66	3 PRECIPI- TATION	0 d d d d d d h h d o c
4 ADJUSTE CAPACIT	8840 1755 1759 878 378	4 ADJUSTE CAPACIT	268 368 409 992 892
5 N NIRFCT Y SPILL-OUT YOLINE		5 0 018667 7 SPILL-0111 VOLUME	000000000000000000000000000000000000000
6 14FLOY		, 14FLG	00.000.000.000.000.000.000.000.000.000
IRRIG Wate	2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
ATTON GREG. 5	2152 2913. 2948. 1736. 242. 24112. 24112. 4048. 6048.	7 IA TION R REO.	20152 2013 2013 2012 2012 2013 2013 2013
8 Water Down Supply Ri		8 WATER DOWN: SUPPLY RE	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
9 DOWHSTREAM REQUIRE- MFNT	$\begin{array}{c} \alpha \\ \gamma \\$	9 HISTREAM REGULTRE— MENT	######################################
10 TOTAL CEMAND	33 33 33 33 33 33 33 33 33 33 33 33 33	10 TOTAL GEMAND	33838 38838 39682 79691 78687 78687 5687 5688
SURPLUS SORPLOS OR OR OFFICIT	13832 13832 11931 11931 15962 16962 16763 16763	11 SURPLUS OR DEFICIT	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
12 SPILL—AUT VOL, AFIER RFGULATED		12 SPILL-OUJ VOL. AFTER REGULATED	
13 TOTAL SPILL-OUT VOLUME		13 107AL SPILL-OUT VOLUME	

	3 L -011 PE	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 -04T -04T	
•	13 TOTAL SP1L1-047 VOLUPE	173 910 910 910 914 147	13 TOTAL R SPIEL-OUME VOLUME	8 8 8
	12 SPILL-OUT VOL, AFTER REGULATED	77305 9031- 85560- 53060- 53060- 5478- 1478-	12 SPILL-OUT VOL. AFTER REGULATED	3 × 5 × 5 × 5 × 5 × 5 × 5 × 5 × 5 × 5 ×
	11 SURPLUS OP DEFICIT	111 1288 1288 1288 1288 1288 1288 1288 1388 1	11 SURPLUS OR DEFICIT	1
	10 701AL Demand	3389 3389 39652 1104 3867 3678 3678 3678 3678 3678 3678 3678	10 Total Demand	3169. 3832. 2221. 11221. 29427. 32427. 10133.
	9 DOWNSTREAM REQUIRE— MENT	8	9 REBUIRE- MENT	2672 2672 2672 2672 2672 2672 2672 2672
	B WATER DOI SUPPLY	2422 2422 2422 2422 2422 2422 2422 242	8 WATER DO SUPPLY	155. 155. 155. 00. 00. 150.
	7 16ATION TER REG.	2152. 2913. 1236. 242. 7000. 1465. 6648.	7 RIGATION ATER REG•	2152. 2913. 2943. 1256. 7000. 1465. 4048.
	6 INFLOW IRR	0. 1127. 6086. 39284. 11978. 1422. 7631. 4411.	6 INFLOU IRR	837. 5989. 1288. 2378. 2378. 5688. 5693. 46915.
	5 PTRECI SPTIL-0UT VOLIPPE	66666777 106666	S DIRECT SPILL-OUT	
	Apacity Si	0.0 0.0 3905. 24780. 74780. 74777. 19572.	6 10 JUSTED APACITY	13191. 10796. 7012. 8982. 8982. 14641. 11245. 14764. 14147.
	3 FC101- A AT10H C	, , , , , , , , , , , , , , , , , , ,	RFCIPI A	22. 2. 41. 26. 26. 69. 63. 63. 63. 63.
	Z EVAPO- PP RATION 1	00. 141. 131. 100. 100.	2 FVAPO- P RATION	2
	1 APAG11Y	0. 3865. 74780. 24780. 24780. 74780. 19477. 13243.	1 Capacity	10859. 7028. 9036. 8049. 14677. 11737. 11737. 18216. 1353.
1968	MORTH C	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1969 HDNIH (MAPRAPER APPRAPER APP

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13 TOTAL SPILL-OUT VOLUME	0. 0. 0. 0. 0. 2.0819. 17.552. 17.552.	13 TOTAL SPILL-OUT	1454 00. 1454 1754 11053
12 SPILL-OUT VOL. AFTER REGULATED	0. 0. 0. 0. 11632. 70819. 17201.	12 SPILL-OUT VOL, AFTER	0.00.00.00.00.00.00.00.00.00.00.00.00.0
SURPLUS SORPLUS SORPLU	-3169. -3832. -4890. -15806. -23425. -2720. -17201. -2945.	11 SURPLUS OR DEFICIT	13126 13126 17265 1776 11776 11766 11760 10760 10760 10760 10760 10760 10760 10760 10760 10760 10760 1
10 TOTAL S DEMAND	3169. 3852. 1965. 7265. 7862. 7326. 3246. 5033.	10 TOTAL DEMAND	3159. 3852. 3965. 2221. 7947. 7377. 7377. 7377. 1017.
9 NSTREAM EQUIRE-	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9 DDURSTR7AM REQUIPE MENT	22 22 24 24 24 24 24 24 24 24 24 24 24 2
SUPPLY REC	1000 1000 1000 1000 1000 1000 1000	8 WATER DD SUPPLY	155. 150. 150. 150. 150.
RIGATION VATER REU.	2152 2913 2013 242 212 2000 1465 411 8813 6048	7 RIGATION ATEN REQ.	2152. 2913. 2948. 1256. 217. 217. 7000. 745. 7601.
6 INFLOW IRR	8855. 676. 5610. 5610. 7857. 79527. 79527. 6730. 6730.	AH LOUTHE	708- 1513- 1513- 1280- 12861- 6876- 78687- 14200- 5878- 5878-
5 PIRECT I SPILL-OUT	000000000000000000000000000000000000000	5 018FCT SP1LL-001	60000 CT
4. DJUSTED D APACITY'SP	12572. 9149. 5313. 10133. 10331. 17982. 18399. 24780. 24780. 21771.	6 DJUSTED APACITY	15986. 12757. 9279. 70579. 70579. 16720. 22,780. 24,780. 24,780. 74,780.
RECIPI - AU TATTON CA	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	\$ REC101- A TATION C	0. 24. 34. 156. 156. 310. 715.
2 . EVAPO- PR RATION T	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 EVAPO- P RATION	200 28. 56. 108. 118. 118.
1 APACITY	,	T CAPACITY	12815. 9033. 7127. 4877. 16667. 24780. 24780. 24780. 24780. 15769.
MOMTH C	A A B A B B B B B B B B B B B B B B B B	1971 MDMTH 0	1 A A B B B B B B B B B B B B B B B B B

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0. 0. 0. 0. 7806. 14854.

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13 TOTAL SPILL-OUT VOLUME	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 TOTAL SPILL-OUT VOLUME	12923. 16276. 20.
12 SPILL-OUT VOL, AFIER REGULATED	0. 0. 0. 12623. 4514. 0.	12 SPILL-OUT VOL. AFTER REGULATED	0.000000000000000000000000000000000000
SURPLUS OR DEFICIT	1 1 2 8 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	11 SURPLUS OR DEFICIT	-1140. -1803. -2613. -2271. -2271. -262. -262. -262. -261.
10 101AL DFMA4D	5169 38159 39159 1106 7296 7286 7286 7286 7286 7013	10 101al Demand	3169 3832 3965 7221 7104 7867 7327 7327 1017
9 104HSTRFAM RFQUIRE- MFNI	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9 PEAH REQUIRE→ HENT	8 x 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
8 WATER DO SUPPLY	44.44.44.44.44.44.44.44.44.44.44.44.44.	8 WATER DO SUPPLY	24 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
7 RIGATION ATER REG.	2452 29433 1236 2472 7000 1465 4043	7 Rrigation Water Reg.	2152 2913 2913 2013 2015 4011 4011
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	22 32 32 32 32 32 32 32 32 32 32 32 32 3	6 INFLOW I	2029. 2029. 1527. 4766. 7168. 13524. 17955. 17955. 77857.
5 101111-00T VOLUME	# d c c d d d d d d d d	5 DIRECT SPILL-DUT VOLUNE	60000000000000000000000000000000000000
4 ADJUSTED CAPACITY S	9292 6087 7277 1577 15132 15232 27857 24780 24781 17431	4 ADJUSTED GAPAEITY S	16372 15186 15586 10598 8 1657 16504 224780 23505 23505
3 RFC1P1- TAT10N	11. 13. 13. 12. 12. 12. 12.	3 PREC1P1- / TATION	50. 21. 21. 23.0. 25.0. 160.
EVAPO- P RATION	0 1 1 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 E VAPO- F RATION	91. 902. 905. 806. 728. 100. 116.
APACITY	6126. 2288. 695. 1175. 1200. 20015. 20731. 26780. 1761. 1641.	1 CAPAC11Y	15252. 13383. 10692. 12029. 15039. 24780. 24780. 25780. 25994.
MO%IH TH	######################################	1975 HTMOM	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4

12 13 SPILL-DUI TOTAL VOL, AFTER SPILL-GUT REGULATED VOLUPE

2392 2392 15354 17104 1305

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JAR. AAR. JUL. JUL. SED. OCT.

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13 Тота SPILL-OUT VGLINE	ó	•	•						Ċ	Ċ		o
12 SPILL-OUT VOL. AFIER REGULATED	0	· c							Ċ	Ċ	0	.0
TURPLUS SOURPLUS SOURPLUS SOUR SOUR SOUR SOUR SOUR SOUR SOUR SO	-5169.	-3832	-3965	-2221	-1104.	-2947	-7862.	-2327.	-3246	-9675	-5053	-1017
10 101al Demand	3169.	3832	1965	2-21.	1104	2962	7862.	2327.	\$246	9675	5033	1017.
9 WNSTPEAM REGULRE- MFNI	862.	779.	862.	835.	862.	835.	862	862	835.	862.	835.	862.
8 Water bol Supply (155.	140.	155.	150.	ċ	0	o	0°	ċ	•	150.	155.
/ IRPIGATION Water Red.	2152.	2915.	2948.	1236.	242	2112.	7000.	1465.	2411.	8813.	4048	• 0
6 14 LOW 148	0	å	•	ċ	ċ	.0	ċ	0	•	0	ċ	• 0
SPILL-OUT	0	0	.	· ·	ċ	• •	ċ	ċ	c	ċ	ċ.	c
ADJUSTED I	0	.0	ċ	o°	ċ	ċ	ċ	0	ċ	ċ	Ċ	.
RFCIPL- A Tation C	c	•	ċ	ċ	ď	•	ċ	c	0	c.	c	
EVAPO-'P RATION.	.	<u>.</u>	c.	c.	ċ	c.	c	0	c	c	c	Ċ
APACITY	0	•	ů	0	•	0	c	0	0	c	c ·	· c
JONER		F F 9	ZAB.	APR.	∀ Σ	JUN.	10 L	AUG.	SEP	٠ ا	> C	DFC.

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13 TOTAL SPILL-OUT VOLUME	11	13 TOTAL SPILL-001 VOLUME	0.000 0.000 0.000 3229.000
SPILL-OUT VOL. AFTER REGULATED	1 5 1 9 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 SPILL-OUT VOL. AFTER REGIILATED	00000000000000000000000000000000000000
SURPLIS OR DEFICIT	1 886.7 1 886.7 2 886.7 2 888.7 2 888.7 2 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	11 SURPLUS OR DEFICIT F	1 600 2 7 633 2 7 633 2 7 8 3 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
10 101At Reykho	733 884 989 250 111 120 120 1728 1728 1728	10 TOTAL DEMAND	733. 886. 989. 550. 274. 11110. 703. 703.
9 DDWNSTRFAM Y REGULRF-	200. 200. 200. 200. 200. 200. 200.	9 DRVNSTREAM T RFOUTRE	209. 209. 209. 209. 209. 202. 202.
8 44TFR DO SUPPLY		8 Water Dr Supply	
7 1016a1108 1ater Reg.	5000 5000 5000 5000 5000 5000 5000 5000 6000	7 Rigatiou Aifr Rio	524. 695. 789. 789. 968. 1674. 756. 1526.
6 1 VFLOV 180	0.000000000000000000000000000000000000	6 1951 1951	153. 956. 926. 1061. 375. 936. 2262. 156.
\$ 019167 Volume	000000000000000000000000000000000000000	5 018FCT 1971L-045 VOLIMF	• • • • • • • • • • • • • • • • • • • •
6 AB 1445 1E th C	0.00.00.00.00.00.00.00.00.00.00.00.00.0	4 PJUSTED C APACITY SF	2956. 2595. 2595. 2054. 2054. 7042. 7090. 7120. 6208.
3 RFCIPI- AB Tation Ca	5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	F(191- A	25. 10. 20. 27. 27. 10. 10. 10.
LVAPU- PR PATIOS T	20000000000000000000000000000000000000	6 EV4PD- PP: RATION 1	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
1 Capacity	2.00 2.00 7.120 7.120 7.120 7.120 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.	1 APEC11Y	2387. 2650. 1637. 2861. 2861. 5102. 7030. 7170. 6266.
ноити с.	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	1953 MOUTH C	1 A A A A A A A A A A A A A A A A A A A

. -			E	
13 TOTAL SPILL-041 VOLUME	7 2 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9		13 TOTAL SPILL-GUT VOLUPE	2,3000000000000000000000000000000000000
12 \$011L-007 \$01, AF1FR REGULATED	0.00.00.00.00.00.00.00.00.00.00.00.00.0		12 SPILL-OUT VOL, AFTER RFGULATED	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
SURPLES SE OR VE	-733. -884. -660. -766. -1145. -1145. -1767. -1627.	•	11 SURPLUS S OR V DEFICIT R	-735. -8429. -1544. -1546. -1737. -1738. -2009.
10 TOTAL PENAND	2 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		10 TOTAL DEMAND	733. 884. 9884. 9884. 11110. 11810. 965. 31116.
oustream Fouther	209. 209. 209. 209. 209. 209. 209.		9 DDWHSTREAM F REQUIRE-	209. 209. 209. 209. 209. 209. 209. 209.
R 9 Water Downstream Supply Require-			8 Water Do Supply	000000000000000000000000000000000000000
THRIGATION SALER PEG.	524. 545. 348. 368. 568. 708. 501. 501. 501. 501.		7 RIGATION ATER REO.	524. 695. 780. 348. 55. 908. 1674. 756. 501. 1576.
4 441 80144	8.00 8.00 8.00 2.20 2.20 1.30 1.30 1.01		AFLOS IR	0. 55. 445. 558. 558. 1427. 1427. 1407. 1400.
5 PIRECT 1 SP111-0017	~ ••••••• •••••		5 D1PECT 1 SP1LL-001 V01UMF	
A PACITY OV	4313. 2813. 2812. 1910. 1966. 7108. 7108. 8178.		4 JUSTED PACITY	3559. 1766. 1166. 1169. 1731. 2015. 7045. 7045.
terret an. Atton tal	0 14. 110. 110. 275. 508. 78.		3 FCIPI AD ATION CA	64. 0. 4. 5. 11. 5. 5. 11. 7. 5. 11. 7. 5. 11. 7. 5. 5. 7. 5. 5. 7. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5.
EVAPO- PRE	2000 X C C C C C C C C C C C C C C C C C		2 EVAPO- PR PATION T	25. 27. 28. 28. 28. 28. 28. 28. 28.
	55 81. 75 81. 75 81. 75 81. 71 70. 71 70.		1 PAC1 TY	7627. 1766. 1266. 1174. 1756. 7048. 7169. 7169. 7169. 7169.
MODTH CA	A P P R R R R R R R R R R R R R R R R R	, , ,	, z	JAM. FEB. AAR. AAR. JUL. JUL. AUG. CEP. OCT.

13 TOTAL SFILL "OUT VOLUPE	0.00.00.00.00.00.00.00.00.00.00.00.00.0	13 TOTA! SPILL-GUT	0.000000000000000000000000000000000000
12 SPILL-OUT VOL. AFTER PFGULATED	0.00.00.00.00.00.00.00.00.00.00.00.00.0	12 SPILL-CUT VOL. AFTER REGULATED	0. 200. 230. 3990. 580. 580. 580. 680. 680.
11 SUPPLUS S OR V DEFICIT H	781. 18478. 18478. 1848. 1858. 1858. 1858. 1858.	11 SURPLUS 5 OEFICIT	-7557 -634. -846. -376. -376. -276. -1577.
10 TOTAL DEPAND	7555 9880 11110 7055 7055 7115	; 0 TO:AL DEMAND	735 885 989 989 11110 1110 1883 775 1775 1775 1775
9 NARSTPEAN RFQHJRE- MFNI	2002. 2002. 2002. 2002. 2002.	9 19 H S I R F A M R F R W I R E - 4 F W I	209. 189. 202. 202. 202. 209. 202.
8 Water Bos Supply	66666666666	8 WATER BO SUPPLY	00000000000
7 VRIGATION WATER RFP.	522. 243. 343. 343. 1674. 1674. 1526.	7 481647108 Waifp Reg.	524. 695. 348. 348. 1676. 7001. 701.
6 W 1861	1513 1513 1766 1772 1773 1775 1775 1775 1775 1775 1775 1775	#1 PFF07 18	2000 2000 2000 2000 2000 2000 2000 200
5 PIREC1 SPILL-OUT VOLHGI	27.	S PTRECT SPILL-60F VOLUNF	666666WVV066
Anjusten r Capacity Si	4730. 4730. 34078. 36078. 5205. 7120. 7120. 7001.	4 CJUSTER APACITY	2662. 12911. 1287. 1218. 1688. 7117. 7120. 7170. 7170. 7061.
3 FCIPI- AIION	5.7. 1.7. 1.7. 1.7. 1.7. 1.0. 1.0.	7 RECIPI- A TATION C	0 0 11 1 2 0 0 0 1 1 2 1 2 1 2 1 2 1 2 1
EVAPO- FVAPO- RAFION	2000 2000 2000 2000 2000 2000 2000 200	2 E VAPO- P RATION	. 2001 2001 2001 2001 2001 2001 2001 2001
APACITY	6911. 6011. 6031. 7181. 7181. 7180. 71	1 CAPACITY	1929. 1277. 1287. 1466. 7120. 7120. 7120. 7120. 7120.
HORTH C	PAPARA DULAPPRA DULAPRA DOCTA	1959 1959 1001 H	PERP PARRY PARRY VANY UNIV. UNIV. OCT.
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×.	13 TOTAL SP11L-OUT VOLUME	00000000000	13 TOTAL SPILL-3UT VOLUME	0.00.00.00.00.00.00.00.00.00.00.00.00.0
,	12 VOL, AFTER REGULATED	00000000000	12 SPILL-OUT VOL. AFIFR REGULATED	2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	SURPLUS SOR VORFICIT R	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	11 SURPLUS OR V	-694 -736 -736 -736 -736 -736 -736 -736 -736
	10 101Al DEMAND	733. 884. 9884. 5580. 7710. 703. 1728.	10 10141 DFKAND	733 884 989 550 274 1110 9883 703 703 209
	9 SOUNS TREAM RECULIRE- MENT	00000000000000000000000000000000000000	9 WNSTREAM PEUTRF	2000 2000 2000 2000 2000 2000 2000
	8 WATER BOUSUPPLY	00000000000	8 WATER DO'SUPPLY	0000000000000
	7 RIGATION ATER RFO.	524. 7895. 789. 348. 65. 908. 1674. 501. 756.	7 RIGATION ATER PEQ.	524. 695. 780. 348. 348. 1674. 756. 501. 2907.
	м в потыны 9	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	6 124 18 14	26 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	SPILL-OUT		5 DTRECT SPILL—0817 VOLUME	.00 .00 .00 .00 .00 .00 .00 .00
	4 DJIISTED APAGITY	2656 3708 316 316 3566 4966 6265 5703 5703	4 ADJUSTED CAPACITY :	3223 2502 1741 1741 943 7120 7120 7120 7120 7120 7120
	8 REC191- A TATION C		3 171104 (4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
	FV&PO. P PATION	2564 C116864	EVAPO-P	20077777777777777777777777777777777777
	ионти сарабиту	1732. 1924. 1607. 1607. 1510. 6254. 5263. 5263.	1 CAPACITY	7550 1767 955 861 7119 7120 7120 7120 7120 7120
1950	мом Ти	14 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1961 NONTH	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4

13 107AL SP1LL-007 VOLUME	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 TOTAL SPILL-011F VOLUME	68 19 - 6 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
12 12 1911—00T 191. AFTER REGULATED	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 SPILL—OUT VOL, AFTER REGULATED	6619. 12435. 9336. 5721.
11 SURPLUS S OR V DEFICIT R	-702. -884. -884. -1166. -650. -651. -7375. -1580.	SURPLUS SOR V	-733 -853 -646 1665 1766 7958 7356 -12435
10 TOTAL DEYAND	733 9884 550 1110 11883 705 716 1728 1728 1728	10 Total Demand	733. 8844. 989. 7110. 7110. 703. 703.
9 OWNSTREAH REDUIRF-	200. 200. 200. 200. 200. 200. 200.	9 OUNSTREAM PFOULPE- MEDI	2002. 2002. 2002. 2002. 2002. 2002. 2003.
8 Water Bo Supply	2000000000	8 WATER D SHPPLY	20000000000
Huai Yai PRIGATION VATEP HEG.	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7 Rigatiov Ater peq.	524. 595. 348. 348. 965. 1576. 501. 1576.
H 6 14 18 24 18	11. 12. 17. 16. 17. 16. 17. 16. 16. 16. 16. 16. 16. 16. 16	0 14FL0X 13	3444 22444 22444 22444 22444 22444 13460 13660 13660 13661 13661
5 5 10 1 1 1 - 0 11 1 10 1 1 1 - 0 11 1	6666669676 74	nirfet Spillant Volume	724 424 426 434 444 444 444 444 444 444 444 444 44
4 0.01157Ep APAC1TY 4	2408, 1777,	4 BJHSTEO APACITY	280 S. 70 4 6. 11 7 8. 5 31 . 5 70 3 . 7 1 7 0 .
RECIPI A	404 40 4 6 4 6 4 6 4 6 4 6 4 6 4 6 4 6 4	5 REC1P1- A TAT104 C	20. 40. 40. 40. 40. 40. 40. 40. 40.
2 FVAPA-P RAT104		2 EVAPú- P Ration	**************************************
CAPACITY	2206. 1292. 1580. 1580. 3023. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120.	CAPACITY	2073. 1196. 537. 7196. 7170. 7170. 7170. 7170.
1962 MONTH (2	1965 MOHTH	JARRA MARRA APRA JULA JULA SEP
	ıv − 54		

4036. 14036. 1408. 3151. 8520. 6240.

920. 920. 6436. 6941.

13 101AL SP11L-011T VOLUPE	3865. 7196. 1697. 10.	13 TOTAL SPILL-0UT VOLUME	
12 01. AFTER FGULATED	7 7 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	12 SPILL-OUT VOL, AFTER REGULATED	15 88 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
SURPLUS S OR V DEFICIT R	1.000 1.001 1.001 1.001 1.000 1.000 1.1000 1.1000 1.1000 1.1000 1.1000	11 Supplus OR OR	11 28 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
10 101A1 DEMAND	23.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10 Total Demand	7555 9884 9884 7110 1883 7055 7055 7006
9 115 T R S A A 16 G U J R E — 116 E U J	789. 789. 700. 700. 700. 700. 700. 700. 700. 70	9 00##8 1 R E A M R E Q U L R E — M E N I	2009. 2009. 2009. 2009. 2009. 2009.
8 WATER DOWN SUPPLY RE		8 WATER DO Supply	
7 1681 198 1FR RFG.	524. 595. 780. 368. 908. 1675. 756. 1576.	7 RIGATION ATER REQ.	524. 695. 780. 348. 968. 1674. 756. 796.
4 NFLOU 188	747 7628 7878 7878 7888 7888 7888	6 6 12 4 4	23. 23. 23. 27. 27. 27. 27. 27. 27. 27. 27. 27. 27
5 014FCT 3: 1011L-011T VOLUME	00 00 00 00 00 00 00 00 00	0186C1 1 591LL-QUT	
4 Abjusten Caparity Si	7485 4466 4467 7120 7120 5978 6388	njusten Apacity	6180. 3402. 2496. 1517. 4700. 7076. 7170. 7170. 7170. 7170.
3 CC191-	7 7 2 2 2 2 2 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4	SRECIPI A	20 20 10 10 10 10 10 10 10 10 10 10 10 10 10
rvapo- ne Ration I	727 727 727 727 727 727	2 EVAPO- P KATIOU	25. 27. 27. 27. 27. 27. 27.
APAE 11 K	798. 35. 35. 6641. 6301. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120. 7120.	4 P A C I	3467. 2541. 1505. 4651. 7120. 7120. 7120. 7120. 7170.
MONTH	AR MARALLANDONG ARAYAYAYAYAYAYAYAYAYAYAYAYAYAYAYAYAYAYA	1967 MONTH - C	JAR. 1AR. 1AR. 1ANT. JUH. SCP. 961.

13 TOTAL SPILL-OUT VOLUME	1786. 5658. 00. 00.	13 TOTAL SP1LL-OUT VOLUME	8781. 1705.
12 SPILL-OUT VOL. AFTER REGULATED	3 6 5 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 SPILL-OUT VOL. AFTER REGULATED	8281 000 000 000 000 000 000 000
SURPLUS SURPLU	1755 1880 1887 1887 1887 1887 1887 1887 1887	SURPLUS OR N	-884. -875. -137. 1197. 1197. 1798. -1728.
10 TOTAL DEF''40	733 8884 5984 11110 12884 7710 17710 17710 17710	10 TOJAL DEMAND	733 8862 9896 1110 1110 1110 1110 1110 1110 1110 11
9 DOWNSTREAM ' REGUIRE- PENT	0.000000000000000000000000000000000000	9 DOWNSTREAM RECUIRE- MENT	2002 2008 2008 2008 2008 2008 2008 2008
K WATER DO	00000000000	8 Water Do Supply	60000000000
7 Rigation Aier Red,	524. 7895. 7896. 348. 908. 1074. 756. 7907.	7 RRIGATION WATFR REG.	524. 695. 780. 348. 908. 1674. 756. 1526.
/M 1 Nefron 168	1,25. 1,25. 1,28. 1,28. 1,08. 1,08. 1,09.	6 INFLOW IRI	733. 164. 413. 967. 2301. 2045. 3694. 10288.
5 DIRECT SPII; -011T VOLUM	0000000000	5 DJRFFT SP11L-0011	
Anjuste Capacity s	2149. 1599. 281. 4203. 7209. 7209. 7209.	4 Abjusted Capacity S	2869 2899 1085 1165 1705 1705 1818 5816 7150 7150
2 PRC51P1- A TATION (0 5 6 7 7 7 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8	PRECIPI- A TATION C	20 20 20 20 20 20 20 20 20 20 20 20 20 2
FVAPOL P RATION	201 1400 2010 14	2 EVAPA- P PATION	さんさく サンチャルウルク 終ニアルベット・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・
1 CAPAC11Y	1417. 571. 6717. 4197. 5075. 7120. 6481. 7120. 7200.	1969 1 КОИТН САРАСІІТ	2940. 2016. 1160. 1008. 1898. 2897. 3101. 5664. 7120. 5304.
1904 HOUTH C	L JARRATAR A LARRA A L	1969	JARRA AARA JARA JARA BAGG PECT

13 101A. SP111-0UT VOLUPE	60.00.00.00.00.00.00.00.00.00.00.00.00.0		13 TOTAL SPILL-OUT VOLUME	9 9 9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
12 SPILL-OUT VOL. AFTER REGULATED	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		12 SPILL-OUT VOL, AFTER REGULATED	00000000000000000000000000000000000000
SURPLUS OR OR OFFICIT P	1333 1333 1305 1305 1305 1305 1305 1305	•	11 SURPLUS OR DEFICIT	1733, 1720, 1720, 1760, 1760, 1760, 1781, 1720,
10 TJTAL DE44MD	733. 989. 989. 1276. 11883. 705. 3716.		10 10tal Demand	2344. 9884. 9889. 18110. 18110. 7085. 7085.
9 DDWNSTPEAM Y REQUIRE-	2009. 2009. 2009. 2009. 2009. 2009.		9 DOWNSTREAM REDIIPE- NENT	00000000000000000000000000000000000000
A Water Doi Supply			B Rater do Supply	0000000000
7 Rigation Aifr Rfq.	524. 695. 740. 348. 908. 1676. 750. 1526.		7 RIGATION ATER RFO.	\$24. 695. 7805. 348. 65. 968. 1674. 754. 754. 7501.
fuelow 186	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		6 11 FLOW 181	164. 164. 1755. 1775. 1895. 1895. 1895. 1895. 1895.
5 01RFCT 1911L-001 VOLUME	6000 Lt ##		5 01REC7 P1LL-0Uf VOLUME	
A THE STAND OF SECTION	4964. 4173. 7256. 7261. 7170. 7120. 7120. 720.		APACITY S	2651. 1159. 731. 1002. 5450. 6853. 67170. 7055.
3 RECIPITATION CA			5 ECIPI- A ATION C	04/22/22/200
EVAPO- P RATION	, , , , , , , , , , , , , ,		7 EVAPO- PR RATION T	**************************************
APACITY	4231. 3289. 3089. 3080. 7170. 7170. 7120. 7120. 7180. 2885.		1 CAPACIIY	11779. 2557. 2557. 5557. 5557. 7170. 7170.
E TROO	PAN FEBR A PAN FEBR S S S S S S S S S S S S S S S S S S S	1971	моитн с	LAAR NABB NABB HAY JUNA JUNA SEP DCT

2828. 1616.

13 TOTAL SPILL-OUT VOLUME		13 TOTAL SPILL-OUT VOLUME	1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
12 SPILL-OUT VOL. AFTER RFGULATEN	60600606000	12 SPILL-OUT VOL. AFTER REGULATED	13660 43660 60 60 60 60 60 60
SURPLUS OR DEFICIT	1733 16811 19811 1522 1522 1387 1387	SURPLUS S OR V DEFICIT R	1132 8750 1550 1776 1776 1776 1776 1776 1776 1776 177
16 101AL DEMAND	2000 000 000 000 000 000 000 000 000 00	10 TOTAL DEMAND	255 9884 5584 1815 1
9 DUNSTREAM RFOUIRF- MENT	24222222222222222222222222222222222222	9 HASTREAM PEQUIRF- MENT	2000 2000 2000 2000 2000 2000 2000
R WATER DOI SUPPLY		B WATER DO'S SUPPLY	0000000000000
f CATION TER RFO.	522 7227 7227 7227 7227 7227 7227	7 IGATION TFR RED.	522 448 448 400 400 400 400 400 400
, 6 INFLOW TRRIC WATE	203. 408. 725. 686. 725. 5167. 5167. 5966.	6	601. 374. 1804. 2519. 889. 1889. 5093. 7051.
5 PIRECT SPILL-0017 VOLIE	00000000000	5 P1RECT SP1L1-AUIT VOLUME	
4 DJUSTED APAGITY	22 22 22 24 25 25 25 25 25 25 25 25 25 25 25 25 25	4 DJUSTED APACITY	21591. 1691. 2504. 37091. 3708. 3718. 7120. 7120. 5986.
3 PFCIPI- A TATION C	0400000000000000	3 1865191- A TATION C	77.2 4.4 6.7 0 77.0 6.7 0 77.0 6.7 0 77.0 6.7 0 77.0 0
EVAPD- P HATION	WV- WWWW	EVAPO- P PATION	######################################
cAPACITY	1558 861. 861. 788. 788. 788. 788. 788.	CAPACITY	2007. 1281. 2352. 1754. 52007. 7120. 7120. 7055.
н повети с	THA A PARA PARA PARA PARA PARA PARA PARA	1975 MONTH (JAN. MAR. MAR. JUN. JUN. AUG. ACT. DEC.

13 TOTAL SP1LL-0UT VOLUME	0.00.00.00.00.00.00.00.00.00.00.00.00.0	13 101AL SP11C-0UT VOLUME	0.000000000000000000000000000000000000
12 OL. AFTER IEGULATED	0.000000000000000000000000000000000000	12 SPILL-0117 VOL, AFTER REGULATED	0.000000000000000000000000000000000000
SURPLUS SOR	7333. 7783. 7783. 7371. 5390. 6460. 11512. 11512.	11 SURPLUS OFFICIT	1 1 2 4 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
10 Total Demand	25 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	10 Total Demand	733. 884. 989. 550. 550. 7110. 705. 7116.
9 DOWNSTREAM REQUIRF— MFbT	2009. 2009. 2009. 2009. 2009. 2009.	9 DOUKSTRFAM r REQUIRE— MENT	209. 209. 209. 209. 209. 209. 209.
B Water Dau Supply F	60666660000	8 WATER DO Supply	00000000000
7 16A T 1 0 H 1 E R R F 0 .	524. 780. 5480. 5481. 757. 757. 501.	7 Reigation Jater Rfo	524. 695. 780. 348. 65. 908. 756. 756. 501.
APLOW IRRI Wal	101 289 1789 1789 1789 17653 10693 10713 10713 101	6 INFLOW IRE	0 1779. 2 8079. 1526. 73563. 1578.
1865 1 116-011 011146	20 00 00 20 20 71 73 73	5 D1RFCT SP1LL-0117 VOLUME	600000000000000
A COLUSTER O	3973. 3205. 2387. 1587. 1587. 7170. 7170. 7170. 7170.	APJUSTEN CAPACITY S	5077 4286. 3559. 10041. 4787. 4737. 7130. 5773.
3 PRFC101- AD Talinu CA	2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	3 1A110H C	6 0 1 2 4 2 2 2 2 4 2 4 2 4 2 4 2 4 2 4 2 4
2 FVAPO- PR RATION 1	525. 101. 101. 101. 101. 101. 101.	2 FVAPO- P RATION	
1 CAPACITY	3241. 2421. 1687. 1590. 1590. 6953. 7120. 7120. 7120. 7120.	197? NOHTH CAPACITY	53 50 1 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
MONTH C	1	1977 40#TH	A P P P P P P P P P P P P P P P P P P P

	· -	~	^	4		æ	7	er.	6	10		12	13
# : E :	CAPACITY	FVAPOL	PRECIPI- TATION	CAPACITY CAPACITY	SPILL-OUT VOLINE	INFLOW	IRRIGATION Jaier Reg.	WATER DO Suppl	NUCSTREAP REGULBE— MENT	TOTAL Demand	SURPLITS OR V	SPILL-OUT VOL. AFTER REGULATED	TOTAL SPILL-OUT VOLUME
-													
JAN	<u>.</u> .	46	·	7. 3576									•
FEB.	1929.	07,	•	1. 2805									0
MAR.	-	3.6	. 13										ô
ADR.	64.5.	35											0
МΑΥ	459.	10	•	0. 633.			0. 65.	ċ	209	274.	-274.	0	o
700	0	v.	•								•		o
Jur.	ċ	Ó									•		0
4136		c	<u>ت</u>										0
SEP.		-											•
OCT.		6									•		0
×0∨.		c	•								٠		•
DEC.		c	ς,										0

Table 4.2 Calculation Sheet of Operation Study

(4) Khlong Chaliang Lab Reservoir

13 TOTAL SPILL-OUT VOLUME	0. 0. 0. 0. 0. 5614. 3467. 2425. 395.	13 TOTAL SPILL-OUT VOLUME	3 5 8. 3 5 8. 8 2 4 8 4. 3 2 9 8 4. 3 2 9 0 0. 1 5 8 7 4.
12 SPILL-OUT VOL. AFTER REGULATED	8 4 4 7 4 7 4 7 4 7 4 7 4 7 4 7 4 7 4 7	12 SPILL-OUT VOL. AFTER REGULATED	32624 32644 32644 32644 36644 3685 3685 3685
SURPLUS S OR V DEFICIT R	12886 13286 13286 13286 1328 1428 1428 1428 1428	SURPLUS OR OR OF ICIT B	1 632. 1 632. 2 64. 3 2 64. 3 2 64. 1 5 8 1.
TOTAL DEBAND	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10 Total Demand	288 3266 3266 3266 3266 3266 3266 3266 3
3 TER DOWNSTREAM PPLY RECUIRE- MENT	2006. 2006. 2006. 2006. 2006.	9 DOWNSTREAM REGUIRE- MENT	2006. 2006. 2006. 2006. 2006. 2006. 2006. 2006.
8 Water Bo	00000000000	8 WATER DO SUPPLY	
7 IRIGATION Jater Peo,	80. 1206. 1206. 130. 257. 116. 234.	7 2RIGATION JATER REG.	80. 106. 120. 53. 139. 257. 116. 257. 256.
4 186 148 148 148 148 148 148 148 148 148 148	0 0 3 54 3 974 7 8 1055	ه 1111ء 14	151. 924. 924. 928. 928. 1067. 927. 976. 5912.
5 0 f R E C T 5 P 1 L L — Q D T V D L UM E	00000° 710000°	DIRFCT SP1LL-AUT VALUME	-00000
L Capacity S Capacity S	0.0 1580 1586 1586 1572 1572 1502	4 Abjustp Capacity S	456. 1286. 1166. 1587. 1587. 1587. 1587. 1587.
PRECIPI-A TAITON C	000000000000000000000000000000000000000	1 1 таттой 1	11. 27. 30. 30. 55. 55. 57.
2 FVAPR- PI RATIRY	666688888	2 Е VAPO- Р RATIOH	64/WWWWWWW
NONTH CAPACITY	849.	1 CAPACITY	681. 1509. 1177. 1530. 1550. 1550. 1550. 1728.
HORTH (0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1955 MONTH	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7

13 101al 11.L-001 VOLUME	3431 1895 1895 13078 12920 676	13 TOTAL SPILL-OUT VOLUME	00000000000000000000000000000000000000
12 PILL-OUT DL. AFTER S EGULATED	0. 0. 0. 1886. 214. 13346. 12844. 676. 0.	12 SPILL-OUT VOL, AFIER REGULATED	0.00.00.00.00.00.00.00.00.00.00.00.00.0
11 SHRPLUS SP OR VO OLFICIT REL	-2286. -108. -168. -168. -1886. -1886. -1836. -234.	SURPLUS S OR V DEFICIT R	12386 12386 113. 278. 278. 104.02. 154.2. 174.2. 174.2.
10 10 TOTAL DEMAND	2886. 8262. 8263. 8394. 8394. 8483. 8483. 8483.	10 TOTAL DEMAND	286. 2962. 2962. 2966. 2966. 2966. 2966. 2966.
9 FRSTREAM AFMI AFMI	206. 206. 206. 206. 206. 206. 206. 206.	9 8 4 4 5 T R F A H R E Q II I R E M E N T	286 286 2066 2066 2066 2066 2066
B WATER DOW SUPPLY R	00000000000	8 WATER DO SUPPLY	000000000000
7 16AT1OH TER PEQ.	7200 1200 1200 130 130 130 130 130 130	7 RIGATION ATER REQ.	106. 120. 120. 130. 116. 116.
6 NFLOW 18R	0. 316. 4982. 725. 785. 13668. 1332. 100.	n PLOW IR	0. 439. 531. 8531. 1409. 1409. 10695.
5 1 RFC 1 1 1 LL-011 10 LUNE	00.00.00.00.00.00.00.00.00.00.00.00.00.	5 1 11 PECT 1 7 SPIIL-DUT VOLUME	000000000000
4 0.011STED D APACITY SP V	988. 878. 350. 350. 1520. 1520. 1520. 1506.	4 40JUSTED CAPACITY S	926. 382. 382. 486. 759. 1546. 1530. 1530. 1530.
3. 14EC1P12 40 7ATIOH CA	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 7 7 7 1 1 1 0 4	0-4/2/40 0-4/2/400 0-4/2/400
FVAPO- PH	22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2 EVAPO- P RATIOU	6411112888888888888888888888888888888888
Capacity	888. 3886. 1580. 1580. 1580. 1172.	1 CAPACITY	6888 8889 8889 1389 15880 15880 15880 8880 8880 8880
1954 МОИТН О	MARAN A L L L A PARRA A L L L L A PARRA A L L L L A PARRA A L L L A PARRA A L L L L L L L L L L L L L L L L L	1955	LARRA HARRA LARRA
	IV - 64		

13 TOTAL SPILL-OUT VOLUME	0 0 0 1834 5344 5742 10646 0	13 101AL SP1LL-0UT VOLUME	558. 558. 558. 772. 1919. 1262. 6761.
12 PILL-OUT OL, AFTER FGULATED	0 0 0 1 8 6 6 5 3 0 6 5 3 0 6 1 0 5 8 0 1 0 5 8 0	12 SPILL-OUT VOL, AFTER REGULATED	558. 358. 758. 1909. 877. 5272. 6715.
11 SURPLUS S OR V DFFICIT R	-286. -318. -318. -306. -320. -220. -220. -220. -220.	SURPLUS OR V	1286 1776 1776 1776 1776 1776 1776 1776 17
10 TOTAL DEMAND	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	10 TOTAL DEMAND	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
9 DOWNSTRFAH (PEQUIRF-	206. 206. 206. 206. 206. 206. 206.	9 DOWNSTREAM F REQUIPE-	206. 206. 206. 206. 206. 206. 206. 206.
8 WATER DOW Supply R		B WATER DO SUPPLY	
7 IGATION TER REQ.	80 170. 170. 153. 153. 170. 170. 234.	7 RIGATION ATER REG.	80. 120. 120. 130. 130. 116.
6 NF! OW IRR UA	293. 293. 193. 2164. 5767. 10357. 701. 8	6 INFLOW IRI	2107- 631- 631- 7248- 7248- 7346- 7346- 736- 736- 736- 736- 736- 736- 736- 73
5 N DIPECT 1 Y SPILL-OUT VOLUME		5 DIRECT POLINF VOLINF	020000 44
7, 111811 PACLT	512. 30. 30. 1530. 1530. 1530. 1516.	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	834. 526. 317. 1567. 1587. 1580. 1580. 1580.
S ECTPI - AD ATION CA		S S TATION C	0
2 EVAPO- PR RATION T	\$ 0 C % % % % % % % % % % % % % % % % % %	2 E V A P O - P R A T I O !!	72. 22. 23. 24. 25. 20.
1 CAPACITY	\$0. \$0. \$0. \$15.50. \$15.50. \$15.50. \$15.50. \$2.50.	Capacity	546. 1530. 1530. 1530. 1530. 1530. 1530. 841.
MONTH C	LAN SPER SPER SPER SPER SPER SPER SPER SPER	1957 HONTH	A A A B B B B B B B B B B B B B B B B B

13 TOTAL SPILL-GUT VOLUME	520. 360. 360. 380. 3890. 5839. 60.	13 TOTAL SPILL-OUT VOLUME	5858 5858 986 5375 1317 22990
12 PILL-OUT OL· AFTER REGULATED	520. 0. 340. 2294. 7175. 3959. 5798. 0.	12 PILL-OUT OL* AFTER EGULATED	5821. 9821. 984. 5335. 1310. 0.0
SURPLUS SI OR VC DEFICIT RI	1,208. - 1,208. - 2,40. - 2,00. - 1,00. - 1,00.	SURPLUS S OR V DEFICIT R	1286 6146 513 5821 5821 5335 1310 1289 1289
10 TOTAL OFMAND	0 P P P P P P P P P P P P P P P P P P P	10 TOTAL DEMAND	2000 2000 2000 2000 2000 2000 2000 200
9 WNSTREAM REQUIRE- MENT	2066. 2066. 2066. 2066. 2066. 2066. 2066.	9 VRSTREAN REGUIRE-	2006. 2006. 2006. 2006. 2006. 2006.
8 Water dow Supply R		8 Water Down Supply R	000000000000
7 IGATION TER RFQ.	25.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	7 116A 110H 17ER REQ.	200. 1206. 1206. 120. 130. 116. 116.
A NFLOW TRR	1696. 154. 150. 160. 160. 7033. 7281. 6075.	6 NF LOW IRR	246. 939. 785. 785. 1324. 1032. 23154. 395.
S DIRFCT 11 PILL-OUT VOLUME	600000 600000	5 DIRECT 1 SPILL-OUT VOLUME	00.00 00.00 00.00 00.00 00.00
4 JUSTED PACITY S	862 13505 13505 15505 15505 1517 1517	4 DJUSTED (APACITY ST	826. 826. 828. 1078. 1550. 1550. 1511. 1511.
3 . EG1P1- AD AT1ON GA	F	3 RFC1P1- A	0.48 448 44 44 44 44 44 44 44 44 44 44 44
EVAPO- PR RATION I		2 FVAPO- PI RATION	6-446644644644
APACITY	1550 1550 1550 1550 1550 1550 1550	CAPACITY	, 588 1588 1588 1588 1588 1288 7288 736
MONTH C	O S S A L L A A A B B B B B B B B B B B B B B	1959	H F F B F F B F F B F F B F F B F F B F F B F F F B F

1960

13 TOTAL SPILL-OUT VOLUME	1100 11100 1700 1700 1700 1700 1700 170	13 TOTAL SPIIL-OUT VOLUME	29 29 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
12 SPILL-OUT VOL, AFTER REGULATED	111000 111000 114945 14940 16940 0	12 SPIEC-OUT VOL. AFTER RFGULATED	2 2 2 2 6 6 1 6 6
SURPLUS :	11 1	11 SURPLUS OR DEFICIT	1247. 1126. 1209. 3918. 3018. 3216. 1216.
10 TOTAL DEMAND	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10 TOTAL DEMAND	2866 27566 27566 27566 27566 27566 27566 27566
9 10VASTREAN REQUIRE— Ment	2006 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	9 UNSTREAH REGUIRE- MENT	206. 206. 206. 206. 206. 206. 206.
R Water Dou Supply F	60600000000	8 WATER DO SUPPLY	000000000000
7 16ation 1er reo.	80. 1206. 120. 139. 139. 116. 116. 234.	7 RIGATION ATER REO.	80. 120. 120. 150. 150. 175. 175. 175.
ANT LOS TRE	8. 362. 155. 1856. 477. 2395. 4312. 1971. 2087. 39.	75 841 MOJ JN E 9	39. 146. 200. 416. 418. 3357. 7400. 6938. 993.
5 018fc1 SP1LL-091	000000000000000000000000000000000000000	5 018FET SPILL-0011 VOLUNF	9.0 0.0 0.0 5.0 5.0 5.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6
APJUSTED (CAPACITY S	716. 132. 134. 165. 1530. 1530. 1530. 1530.	4 APJISTED GAPACITY *	866. 608. 451. 320. 537. 1550. 1530. 1521. 1947.
5 FEIPI- ATION	0 C C C C C C C C C C C C C C C C C C C	3 18 6 1 9 1 1 A 7 1 D 14	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
CVAPG- PHOR	20 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7 FVAPO- P RATIOH	17. 113. 10. 10. 10. 10. 10. 10. 10. 10.
i Capaciiť	438. 136. 170. 1530. 1530. 1530. 1100.	capacity	618. 562. 528. 1550. 1550. 1550. 1550. 1550.
NOWIH C	HEBB. HARR. LURY LULY COCT.	1961 MONTH	D

13 TOTAL SPILL-OUT VOLUME	0 0 14,56 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		13 TOTAL SPILL-OUT VOLUME	924 1993 2410 2410 9293 17987 9694 8172
12 SP1LL-OUT VOL. AFTER REGULATED	1456 00 1456 00 399 399 690 690 00		12 SPILL-OUT VOL. AFTER REGULATED	826. 1986. 2395. 9231. 17907. 9633.
SURPLUS OR CEFICIT	1255. 1471. 1471. 1980. 1980.		11 SURPLUS OR DEFICIT	286. 131. 1934. 1986. 2395. 9231. 9633. 8072.
10 TOTAL Demand	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		10 Total Demand	2000 2000 2000 2000 2000 2000 2000 200
9 BOWKSTRFAM T REDUIRE-	2000 2000 2000 2000 2000 2000 2000 200		9 WNSTRFAM REQUIRE— MENT	2006. 2006. 2006. 2006. 2006. 2006.
4 MATER DOI SUPPLY	6666666666		B Water DO Supply	000000000000
7 REGATION REG.	80. 106. 120. 139. 146. 234. 234.		7 Rigation Ater Req.	80. 120. 120. 139. 255. 116. 27. 266.
NETON 1861	31. 131. 1694. 454. 454. 7223. 7371. 746.		4 NFLOW 186	31. 539. 2187. 2284. 2284. 3829. 9910. 6724.
FIRECT SPILL-OUT	00000 11.000000000000000000000000000000		SPILL-NUT	0000× r 4000 000 000 000 000 000 000 000 000
4. 11.5 FE B. P. S. P. S	825 258. 258. 1586. 1514. 1530. 1514. 1514.		4 PJUSTER C	9880. 610. 610. 624. 1530. 1530. 1530. 1550.
3 601761- A 11708- C	-0-128444444444444444444444444444444444444		\$ RECIPI- AF TATION C/	0-m/4000x8/
EVAPO- PRI	25. 25. 25. 25. 25. 25. 25. 25. 25. 25.	,	2 EVAPO- PR RATION T	62727222222
7 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	22 20 20 20 20 20 20 20 20 20 20 20 20 2		1 APACITY	664 644 641 642 642 643 643 643 643 643 643 643 643 643 643
. , u	7	1963	MONTH C	FAN MARKER AND SOLUTION OF SOL

562. 5101. 5101. 7359. 7359.

562. 562. 5046. 7286. 7123.

286. 2885. 2885. 5066. 7286. 7123.

609. 476. 1255. 1550. 1550. 1550. 1530. 1299. 754.

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0. 0. 7727 2302 38903. 16178.

1286. 1297. 1257. 1257. 1257. 12503. 126178. 1377.

787. 587. 581. 1580. 1580. 1580. 1580.

1442 1448 1044 1014 1016

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13 TOTAL SP111-04T VOLUME	0. 3117. 430. 7627. 7628. 1353. 1353.	13 TOTAL SPILL—OUT VOLUME	200 2015. 2015. 2015. 2015. 2015. 67.55.
12 PILL-OUT OI, AFTER IEGULATED	3117. 7117. 7117. 7897. 7897. 7897. 1350.	12 12 12 12 15 15 15 15 15 15 15 15 15 15 15 15 15	2 2 2 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
SURPLUS S OR Y DEFICIT R	-240. -240. -240. -240. -240. -240.	SURPLUS S OR V OEFICIT R	1.2864 1.3264 1.3264 1.3389 1.300 1.400 1.400 1.400
10 101 pt 0 f f g k b	22 20 20 20 20 20 20 20 20 20 20 20 20 2	10 10 TOTAL DEMAND	2000 0000
9 WNSTREAM REGULAREN PFNT	22000000000000000000000000000000000000	9 ENSTREAM PEGUIRF- MENT	0 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
B Water Dou Supply	ccocccacco	8 WATER DOI SUPPLY	600000000000
7 IGATION ICR RFG.	80. 1206. 53. 53. 73. 74. 74. 74. 23.	/ RIGATION ATER RED.	80. 120. 120. 130. 137. 116. 777. 776.
6 18FLOW 18R	46. 69. 754. 754. 778. 778. 778. 778. 778. 778. 778. 77	6 1871 OU 188	25 25 25 25 25 25 25 25 25 25 25 25 25 2
5 01866 ¹ 0111 – 0111 voj inf		5 018fct P11t-OUT VOLURE	000000000000000000000000000000000000000
APACITY '	740. 740. 760. 11. 15. 15.50. 15.50. 15.50. 15.50.	6 64115769 APACITY 1	10000 11511 11511 11511 11511 11511 11500 11500 11500
SPRECIPI- A TATION C	/*************************************	3 REC P1- A TATION C	6444V 6464VWV66A46
FVAPO-	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	2 EVAPO- PI RATION	77- 78- 78- 78- 78- 78- 78- 78- 78- 78-
MOUTH CAPACITY	267. 267. 1550. 1550. 1550. 1550. 1550.	1 CAPACITY	780 1580 1580 1580 1580 1580 1680 1680 1680 1680
-	7 F F F F F F F F F F F F F F F F F F F	1967 1967 HORJH C	1

19443. 19443. 4045. 4045. 6056.

2958. 1624. 5128. 7168. 7168.

13 -T0TA1 SPILL-0UT VOLUME	7468. 77568. 700. 700. 700. 700. 700.	13 TOTAL SP1LL-0UT VOLUME	418 418 1418 1418 7240 7240
12 PILL-CUI OL, AFIER EGULATFD	0. 1468. 7484. 7116. 6984. 00.	12 PILL-OUT OL. AFTER EGULATED	4187. 1187. 1288. 3705. 2226.
SURPLUS SI OR VI DEFICIT R	1.08 2.992 1.088 2.098 2.088 2.010 1.088 1	11 SURPLUS S OR V DEFICIT R	1123 1123 1123 1123 123 123 123 123 123
10 TOTAL DEMAND	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	10 101al 0emand	6 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 DOWNSTREAM Y REQUIRE- MENT	2865 2000 2000 2000 2000 2000 2000 2000 20	9 WN STREAM REQUIRE-	08000000000000000000000000000000000000
8 WATER DOS SUPPLY F	00000000000	B WATER POI SUPPLY	000000000000
7 Rigation Ater Reu.	2001 10001 1	7 16A 110H 16R REO.	1006. 1706. 1106. 1106. 1106. 1106. 1106. 1106. 1106. 1106. 1106.
0 111 FLGW 148	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	6 HFLOW IRR	162. 1296. 6676. 508. 1871. 7503. 300.
5 818EST 1 8PILL-001 VOLUNE	00000000000000000000000000000000000000	5 01RECT 1 P1LL-0HT VOLUME	0000000000000
4 60 60 60 60 60 60 60 60 60 60 60 60 60	824. 527. 231. 1804. 1530. 1530. 1698.	4 PJUSTER APACITY SI	600 600 1286 1380 1380 1500 1500
3 14 - 14 - 14 - 14 - 14 - 14 - 14 - 14 -	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 PFC1P1- AC TATION D	0 m m m m m m m m m m m m m m m m m m m
EVAPO- P	4 L 4 L 4 L 4 L 4 L 4 L 4 L 4 L 4 L 4 L	7 EVAPN- PI RATION	
APACITY (APACITY	93839 118830 118830 118830 118830 118830 118830 118830 118830	CAPACITY	6413. 1972. 1973. 1530. 1530. 1530. 1157.
3 HI HOH	A M M M M M M M M M M M M M M M M M M M	1921 	
	75 TV = 72		

1972

13 707AL SPI11-0UT VOLUME	0. 0. 0. 0. 2994. 4223. 1439. 0.	13 TOTAL SP1LL-OUT VOLUME	734. 734. 734. 787. 787. 787. 775. 13476.
12 PILL-OUT OL, AFTER EGULATED	0. 4050. 6190. 1979. 1435.	12 SPILL-OUT VOL. AFTER REGULATED	734. 734. 1774. 2625. 13398. 0.
11 SUPPLUS S OR V DEFICIT R	1286 1276. 2226. 2226. 2243. 2439. 12888.	11 SURPLUS OR DEFICIT	286 - 269 - 375 - 1570 - 1770 - 1770
10 Total Demand	24 6 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	10 TOTAL DEMAND	23 24 26 26 26 26 26 26 26 26 26 26 26 26 26
9 WNSTREAM REGUIRE— MERT	206. 206. 206. 206. 206. 206. 206.	9 IOWNS TREAM REQUIRE— MENT	2006. 2006. 2006. 2006. 2006. 2006. 2006.
5 WATER DOW Supply R		8 WATER DO SUPPLY	0000000000
/ IGATION IFR RED.	80 1206 1206 130 116 116 234 234	7 11GATION 17FR RFD.	80. 106. 120. 53. 139. 116. 77. 234.
6 NFLOW IRRI	216. 216. 437. 431. 5487. 3434. 4512. 7256. 146.	6 INFLOW IRR	701. 701. 185. 1786. 2718. 3034. 3034. 3034.
5 018ECT 18 SP1LL-AUT VOLUME	900000 KWE 400	\$ P186CT 1 P7UHC V0LUHC	20. 20. 77. 78.
4 11571 D PACITY	489 199 120 120 1230 1530 1530 1930	A PACITY SE	980. 680. 761. 761. 1530. 1530. 1531.
3 FCIPI- AD ATION CA	6-66W0W0W0	3 RFC1P1 - AC TATION C.	0 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 -
2 EVAPO- PRI RATION I	54 W C & C & C & C & C & C & C & C & C & C	2 EVAPO- PR RATIOH 1	19. 17. 17. 17. 17. 17. 15.
1 APAC11Y	202. 123. 224. 436. 1530. 1530. 1530. 1220.	1 1 APACI I Y	694. 410. 780. 693. 1530. 1530. 1530. 1530. 1757.
MONTH CA	A A A A A A A A A A A A A A A A A A A	1975 HOWTH C	A A A A A A A A A A A A A A A A A A A

13 TOTAL SPILL-OUT VOLUME	0. 0. 1445. 17826. 1796. 2786.	13 TOTAL SP11 L-0UT VOLUME	0 1246. 1978. 531. 1463. 5185. 4785.
12 PILL-OUT OL. AFTER FGULATED	0. 0. 0. 1445. 1787. 1787. 2274.	12 PILL-OUT OL. AFTER EGULATED	1246. 1246. 1978. 1454. 1454. 1751. 1373.
11 SURPLUS S OR V OFFICIT R	-286. -192. -1918. -1918. -1918. -1918. -1918. -1918. -1918.	11 SURPLUS S OR V DEFICIT R	307 15 178. 15 178. 178531. 18889. 17871. 17871. 17871.
10 TOTAL Demand	286. 275. 275. 275. 277. 277. 277. 206.	10 10 10 10 10 10 10 10 10 10 10 10 10 1	2986 3262 3262 3266 3316 3316 332 332 532 532
9 NWNSTREAM REQUIPE-	2002 2006 2006 2006 2006 2006 2006	ON STREAM REGULTRE	7.000000000000000000000000000000000000
B Water Dau Supply F		8 WATER DOI SUPPLY	000000000000
7 RIGATION ATEP RFO.	80. 120. 53. 150. 157. 1757. 176. 234.	7 RIGATION ATER REO.	80. 1206. 1206. 120. 130. 130. 130. 140. 177.
NFLOW IRI	200. 200. 200. 216. 2176. 2006.	6 1NFLOW JR	593. 1840. 248. 1878. 1417. 5028. 217.
5 PIRFCT SPILL—OUT VOLUME	000000000000	5 D1REC1 P1LL-0HT V0LUMF	00000000000000000000000000000000000000
4 Apjuster Capacity S	2000 2000 2000 2000 2000 2000 2000 200	4 PJUSTED APACITY S	898 1785 1785 1785 1783 1580 1580 1580 1580
3 REC1P1- TATION	000000000000000000000000000000000000000	3 RECIPI-A TATION C	LLW WYAR. LLCCWYAR. LLCCWYAR.
2 EVAPO- P RATION	14464444444444444444444444444444444444	7 F V A P O - P R 4 T 1 D N	- VW W W W W W W W W W W W W W W W W W W
APACITY.	276. 179. 0. 0. 0. 0. 0. 15. 15. 15. 15. 15. 15. 15. 15. 15. 15) CAPACITY	1206. 1206. 1279. 1550. 1550. 1550. 150.
, `.Σ., `.	A A A A A A A A A A A A A A A A A A A	1975 HINOH H	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

3 L -00T ME	000000000000000000000000000000000000000	3 L L - OU T IME	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
TOTAL SPILL- VOLUN	41 0011 0011 0011	13 10141 10141 10161	← ← W 4 ○ → ® ← ® ®
12 SPILL-OUT VOL. AFTER REGULATED	0 0 0 0 0 10864 10084 9958 11781 3778	12 SPILL-OUT VOL. AFIER REGULATED	1026 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
11 SURPLUS OR DEFICIT	1288 1792 1792 1792 1787 1787 1787 1787 1787 1787	SURPLUS OR DEFICIT	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
10 TOTAL DEMAND	2000 000 000 000 000 000 000 000 000 00	10 TOTAL DE4AND	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
9 OUNSTRFAM RFOUIRE MENI	200. 200. 200. 200. 200. 200. 200. 200.	9 OWNSTREAM RFQUJRF- HENT	2006. 2006. 2006. 2006. 2006. 2006. 2006.
R WATER DO Supply	00000000000	8 WATER D SUPPLY	6669666666
7 RIGATION ATFR REG.	2000 1200 1200 1300 1300 1300 1300 1300	7 RIGATION ATER REO.	80. 120. 130. 130. 116. 234. 234.
6 INFLOW IRI	100. 285. 177. 5590. 1348. 6607. 10058.	6 1 1 F L OW 1 R	0.00.00.00.00.00.00.00.00.00.00.00.00.0
S DIRECT ISPILLOUT	00000000000000000000000000000000000000	5 01RECT SP1LL-011T VOLKME	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
APACITY S	856 8560 8560 8711 1570 1580 1580 1580 1590	4 1000 1000 1000 1000 1000 1000 1000 10	926. 626. 526. 1570. 1530. 1530. 1541. 1656.
3 FCIPI- A ATION C	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 RECIPI- AL TATION C.	60000440000
2 EVAPO- PR BATIOU T	17. 110. 12. 28. 28. 29.	2 בעעס- P מאנדמ	8.4. 8.4. 8.4. 8.4. 8.4. 8.4. 8.4. 8.4.
i APACITY	570. 367. 319. 235. 1530. 1530. 1530. 1177.	CAPACITY	\$33. \$35. \$55. \$15.00. \$15.50. \$15.50. \$15.50. \$15.50.
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13 TOTAL SPILL-OUT VOLUME	
12 SPILL_DUT VOL. AFTER REGULATED	000000000000
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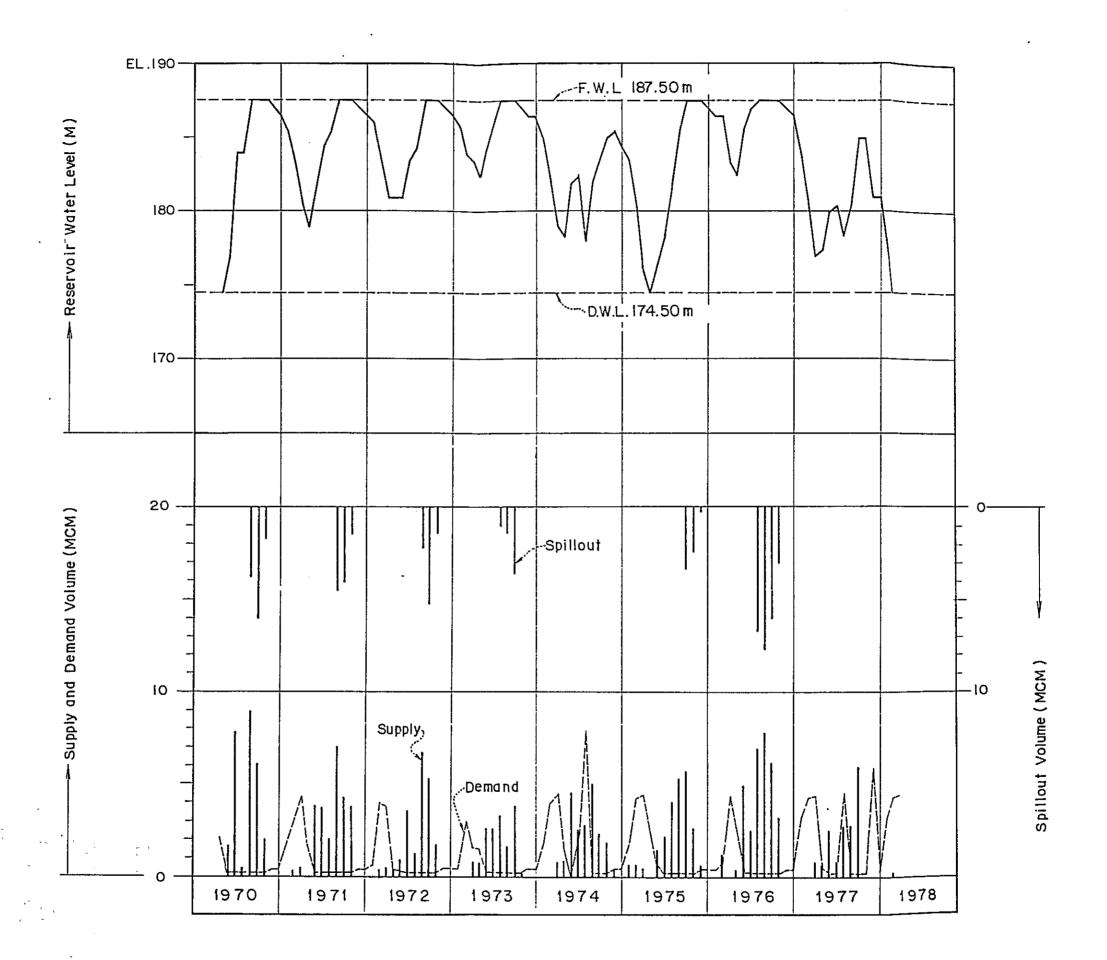


Fig. 4.1 OPERATION STUDY RESULT
(1) Huai Saduang Yai Reservoir

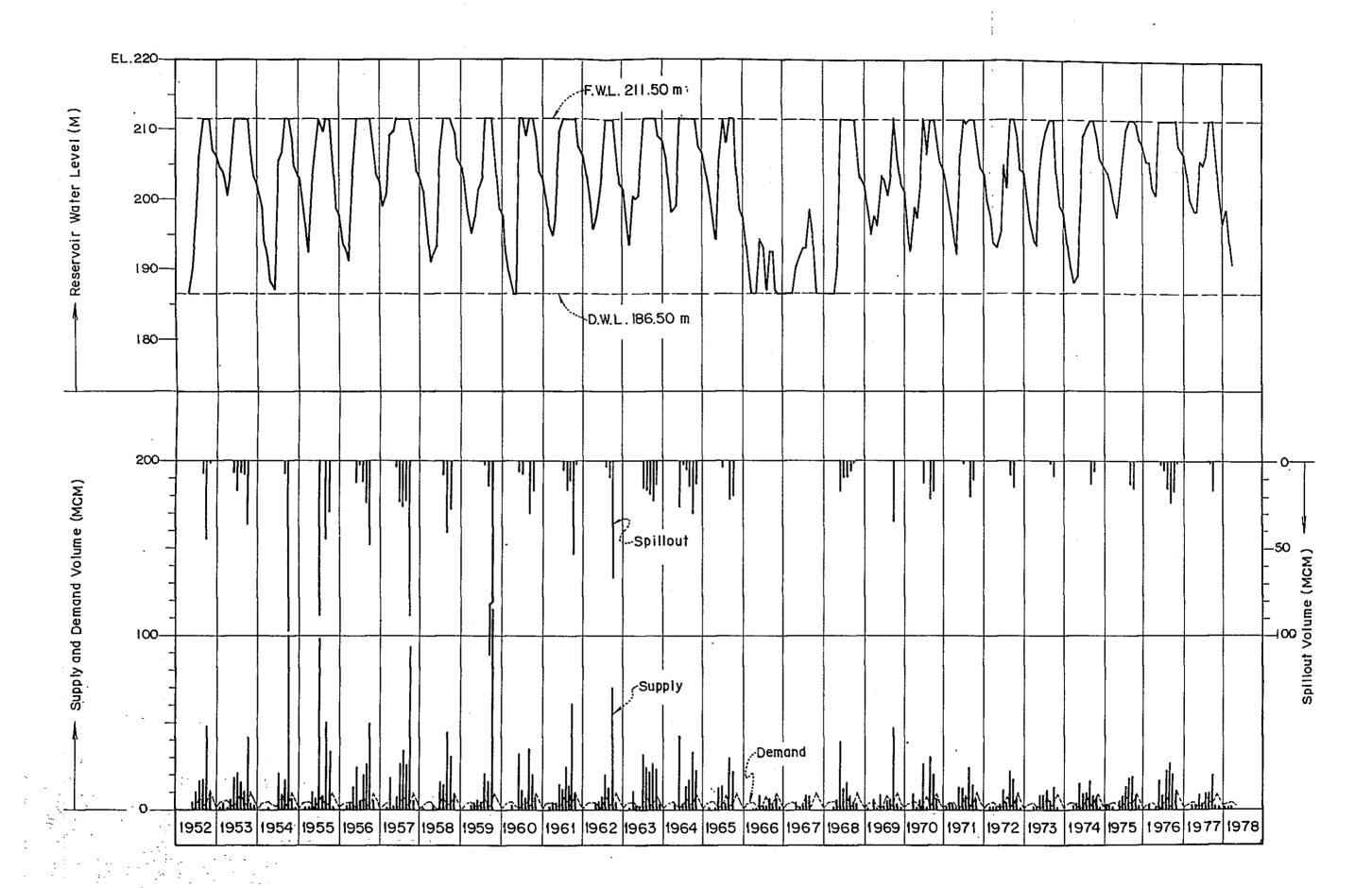


Fig. 4.1 OPERATION STUDY RESULT

(2) Hual Khon Kaen Reservoir

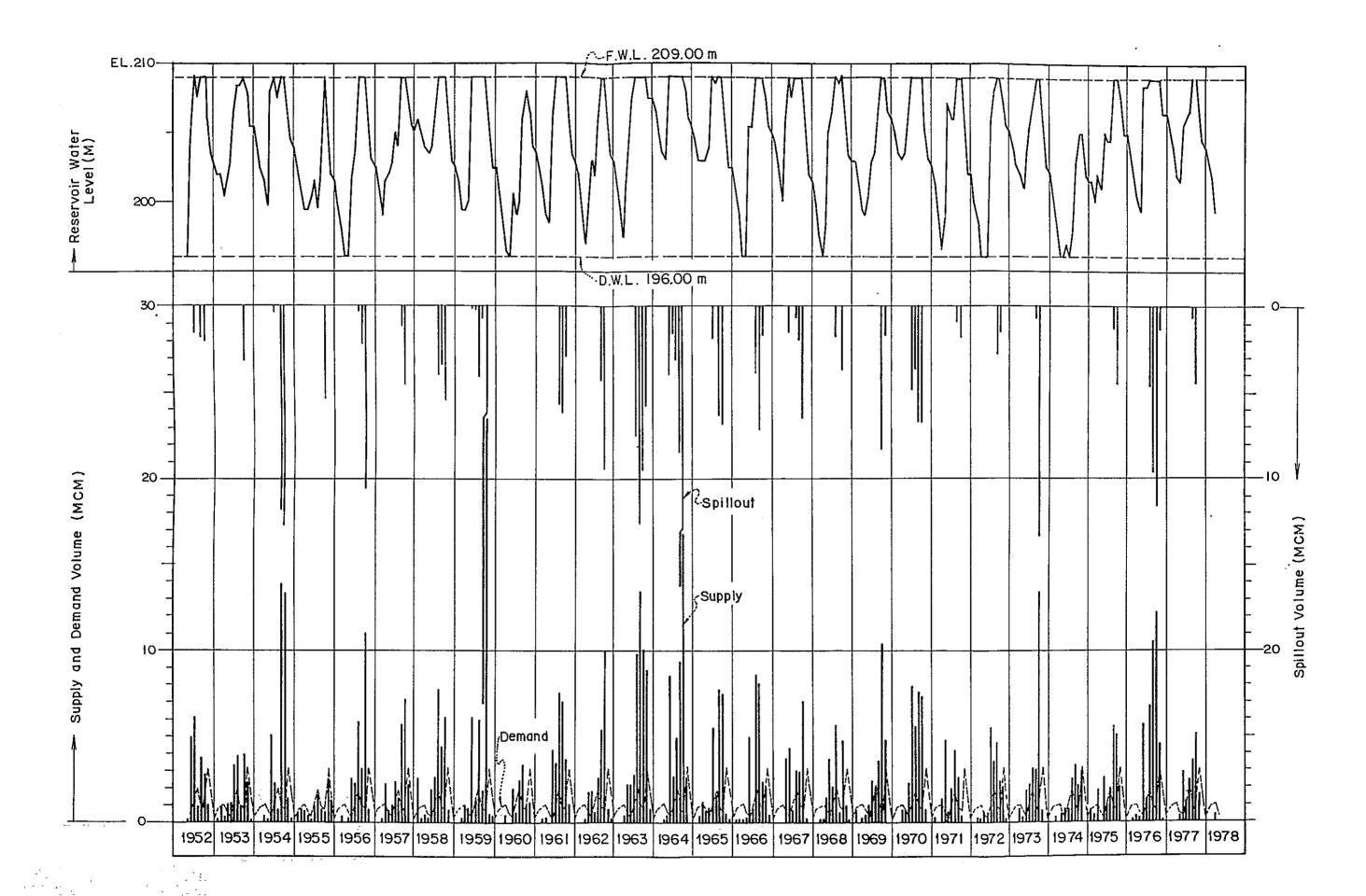


Fig. 4.1 OPERATION STUDY RESULT

(3) <u>Huai Yai Reservoir</u>

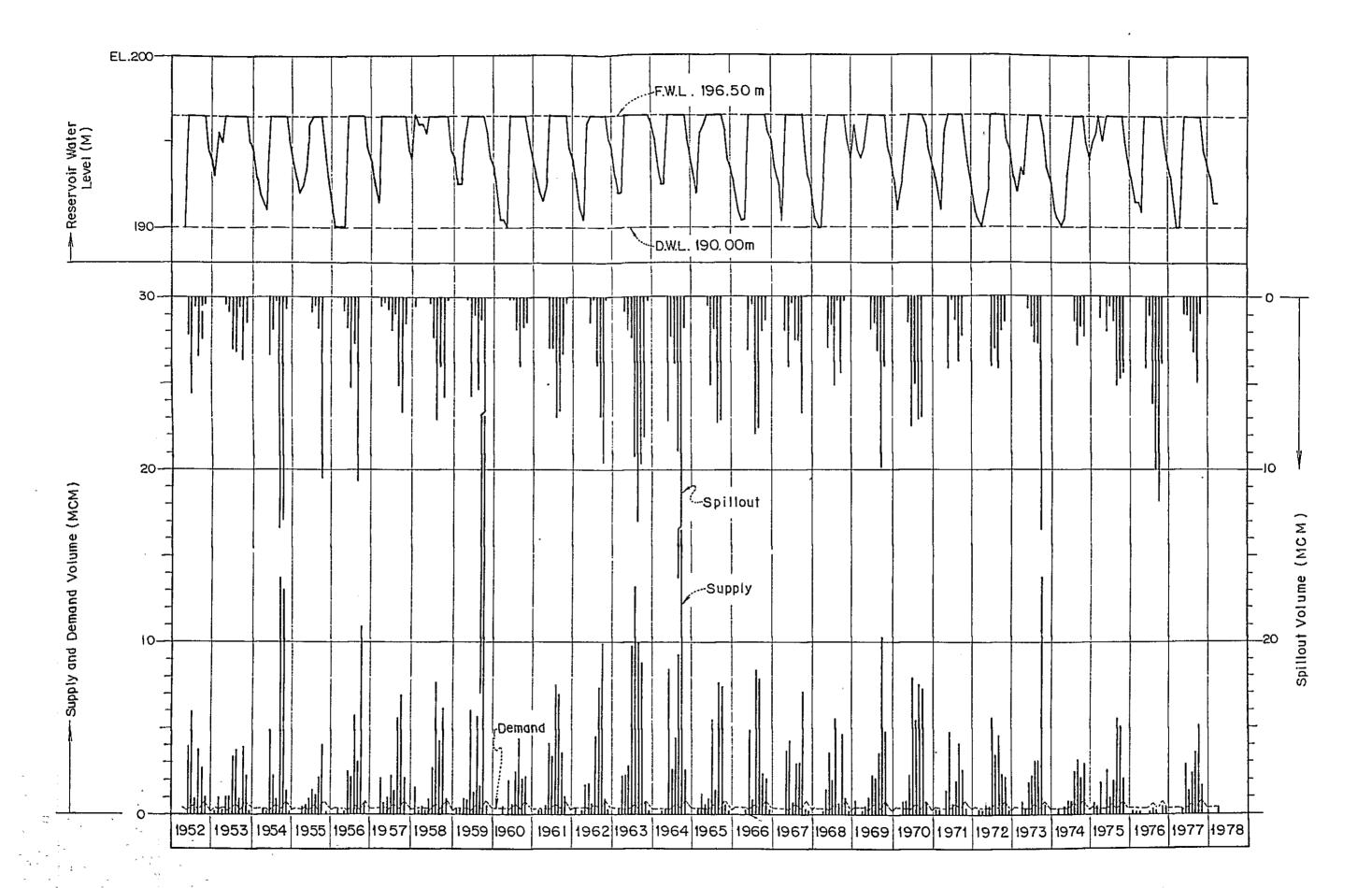


Fig. 4. 1 OPERATION STUDY RESULT

(4) Khlong Challang Lab Reservoir

