Table 5.10.2 Comparison of Alternative Plans Intersection: Rama IV/Kasemrat Rd. (No. 035)

natives (Recommended improvement)	Two-way, 4-lane new road New road Rama IV Widening	Construction Cost 30.1	Land Acq. Cost 6.4	Total 36.5	Maintenance Cost per annum	tion Period	Improvement 1.036 — 0.815 Effect Max. Queue length (EB) 550 m — 50 m Max. Stopped delay (EB) 7 min. — 40 sec.	Major Constraints Both sides of Rama IV Rd. already built-up.	Advantage/ Area of new road is mainly Disadvantage in open spaces. Land acquisition required for lane increase.
Alternatives	Improvement Plan	Construction Cos			tenance annum	Construction Period		Evaluation Major Constraint	Advantage/ Disadvantage

Table 5.10.3 Economic Analysis (in 10 years)

Item	Result
Initial Cost	36 Mil.B.
Cost in Present Value (C)	41 M11.B.
Benefit in Present Value (B)	105 Mil.B.
Net Benefit (B-C)	64 Mil.B.
в/с	2.58
IRR	0.272

The net benefit is reasonably high, the B/C is more than 2.0 and the IRR is 0.272. Therefore, the new road plan is recommended for the future execution.

5.11 Dindaeng - Asok Rd., No.900

(1) General description (Figure 5.11.1)

This intersection is located at the south end of Middle Ring Rd. (or Ratchadaphisek Rd.). The traffic volume coming here is not large compared with those of other intersections. One of the reasons is that an east road called Samsen Khlong-side Road has not been completed nor connected to east Ramkhamhaeng Road. If the road is connected to the east road, the traffic volume will increase significantly.

(2) Traffic flow

As Samsen Khlong-side road has not been completed, traffic volume to/from the road accounts only 300 - 500 PCU/hr. Therefore, traffic diagram shows almost like a 3 - leg intersection instead of actural 4 - leg one. (Figure 5.11.2). When the road is completed, the traffic flow will change drastically.

(3) Constraints (Figure 5.11.1)

Dindaeng - Asok road in the south has only 6 lanes at standard section in comparison with 8 lanes of Middle Ring road in the north. Along the Dindaeng - Asok road, concrete buildings are already built-up and the sidewalk has only about 3 m width. For the widening of the road from 6 - lane to 8 - lane, large land acquisition will be required.

A bridge at Khlong Samsen has also only 6 lanes. If the widening of the road is required, the bridge widening, the land acquisition and shifting of the sidewalk are also required. Around the bridge, building demolition will be small because the buildings are already set back around there.

(4) Degree of congestion (existing)

Existing conditions are summarized in Table 5.11.1.

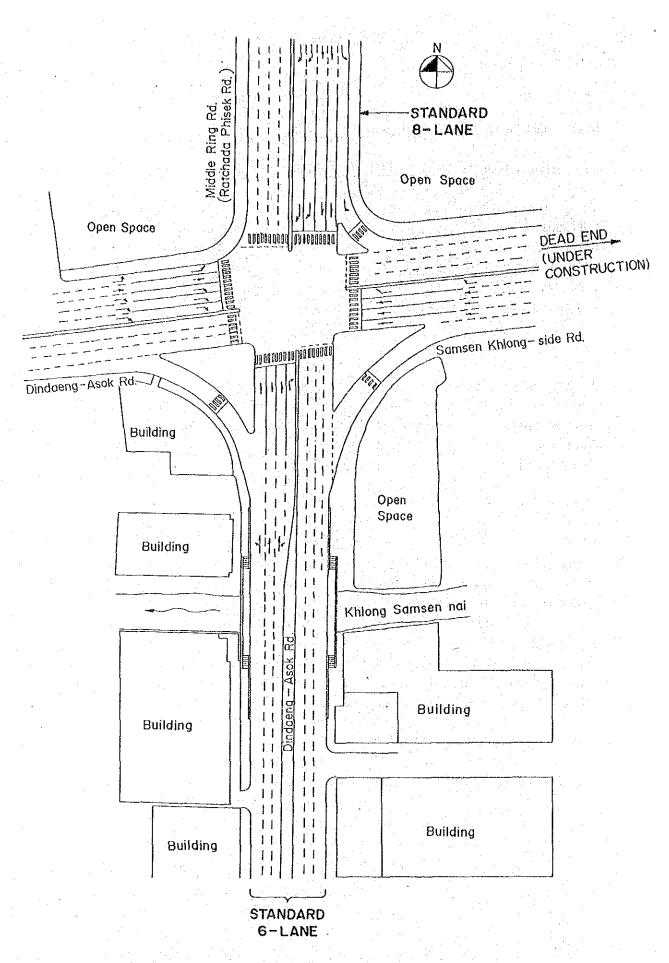


Figure 5.11.1 Dindaeng-Asok Intersection

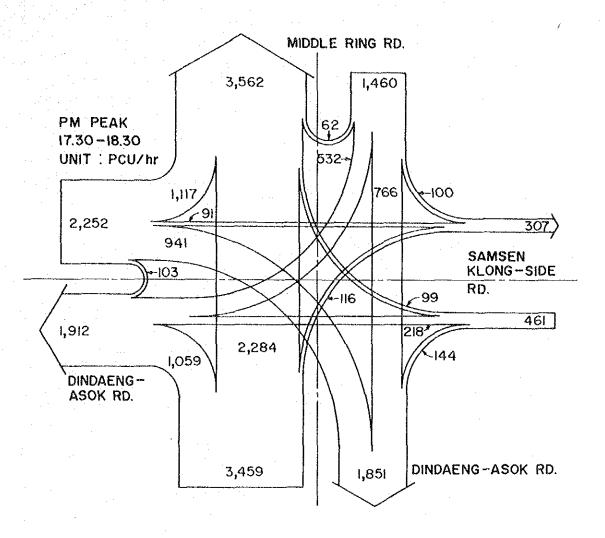


Figure 5.11.2 No. 900

Traffic Volume at Dindaeng—Asok Rd.

Table 5.11.1 Existing Condition of Intersection

Item	Characteristics
1) Morning peak hour	09:00 - 10:00
Max. Traffic	2,649 PCU/hr, north bound
2) Evening peak hour	17:00 - 18:30
Max. Traffic	3,562 PCU/hr, north bound
3) Saturation degree	
of intersection	
Morning	1.007
Evening	1.144
4) Queue length	
Morning	400 - 600m, 07:30 - 08:00
	south bound
•	400 - 600m, 07:45 - 08:00
	west - south bound
Evening	200 - 400m, 16:30 - 18:30
-	north bound and west - south,
	north bound
5) Stopped delay	
Evening	7 min. west - south, north bound
6) Cycle length	
Morning	291 sec. (irregular)
Evening	219 sec. (regular)

(5) Cycle time analysis

- Green time is properly arranged to each direction.
- Cycle length is long and irregularly controlled in the morning.

(6) Planning of improvement alternatives

1) Points of planning

As mentioned in General description of this intersection, the improvement plan at this location should be closely coordinated with that of Middle Ring Road. Furthemore, Samsen Khlong-side road, which is an east leg of the intersection, has been under construction. When the road is completed, traffic from east of Bangkok will come to the intersection and its traffic volume will increase significantly. The traffic condition will be much different from the existing one.

Therefore, the basic direction of the improvement plan at this intersection must be clarified in order to realize a balanced and integrated improvement of the road network concerned, based on a comprehensive and long term perspects in consideration of future traffic demand. The studies carried out here was confined to analyse the existing conditions and to present the effects on them in case of widening the road from 6-lane to 8-lane which is stretched on the adjacent segment of Middle Ring Road.

(7) Improvement effects

If south Dindaeng - Asok road was widened to 8 - lane, the saturation degree of the intersection will decrease from existing 1.144 to around 0.8. It is still a little congested.

5.12 Petburi Rd./Soi Asok, No.220

(1) General description (Figure 5.12.1)

This intersection is a 4-leg one and located at Petburi Rd. with a flyover above it. Crossing roads are Dindaeng-Asok Rd. and Soi Asok. Those two roads will form Middle Ring Rd. in future. The existing road width and the number of lanes from the north road are shown in Table 5.12.1 and Figure 5.12.1.

Road Name	Road Width	Number of Lanes
Middle Ring Rd.	40 m	8
Dindaeng-Asok Rd.	30 m	6,
Soi Asok	20 m	4
Middle Ring Rd. extension (Under construction)	40 m	8

Table 5.12.1 Road Width along Middle Ring Rd.

In the table, existing Dindaeng - Asok Rd. (30 m in width) and Soi Asok (20 m) are narrower than both sides of Middle Ring Rd. (40 m)

Middle Ring Rd. is a major road surrounding the Bangkok city. Therefore, the improvement of the intersection which is located on the future Middle Ring Rd. should be considered with that of Middle Ring Rd.

Then, for this intersection, at-grade improvement plan without land acquisition is proposed and analyzed Overall improvement of Middle Ring Rd. is expected to be studied in future.

(2) Traffic flow (Figure 5.12.2)

Petburi Rd. in the west is one way except a contra bus lane. Petburi Rd. in the east is dual way but its east bound traffic is relatively small (about 800 PCU/hr) due to the flyover of east bound. Traffic volume in the evening is analyzed here because its saturation ratio is bigger than that in the morning.

Queue length is 200 - 400 m between 16:30 and 18:00 on Petburi Rd.

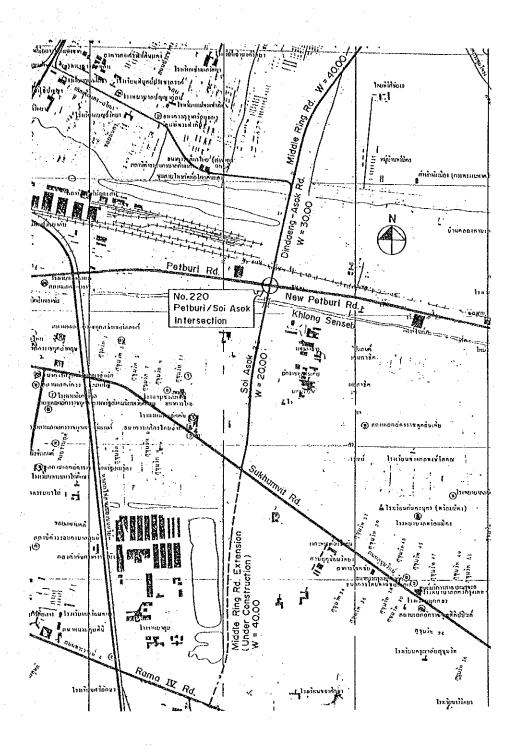


Figure 5.12.1 Petburi Rd./Soi Asok

(3) Constraints (Figure 5.12.3)

- At north west side of the intersection, commercial buildings are located. Therefore, road widening for left-turn is difficult.
- Traffic island in the north west of the intersection is disturbing the traffic flow of the left-turn.
- Soi Asok and the bridge above khlong Sen Sab has only 4 lanes.

(4) Degree of congestion (existing)

Existing conditions are summarized in Table 5.12.2.

Table 5.12.2 Existing Condition of Intersection

Item	Characteristics
1) Morning peak hour	09:00 - 10:00
Max. Traffic	2,311 PCU/hr, west-south bound
2) Evening peak hour	17:00 - 18:00
Max. Traffic	1,955 PCU/hr, west-north bound
3) Saturation degree	0.897 (Evening)
of intersection	
4) Queue length	
Morning	500 - 600 m, 07:15 - 07:45
	south bound
	300 - 600 m, 07:15 - 07:45
	west bound
Evening	200 - 400 m, 16:30 - 18:00
	west bound
5) Stopped delay	
Evening	5 min. 30 sec. west bound
6) Cycle length	
Morning	300 sec.
Evening	120 sec.

Saturation degree is 0.897 and stopped delay is more than 5 min. The improvement of the intersection is required.

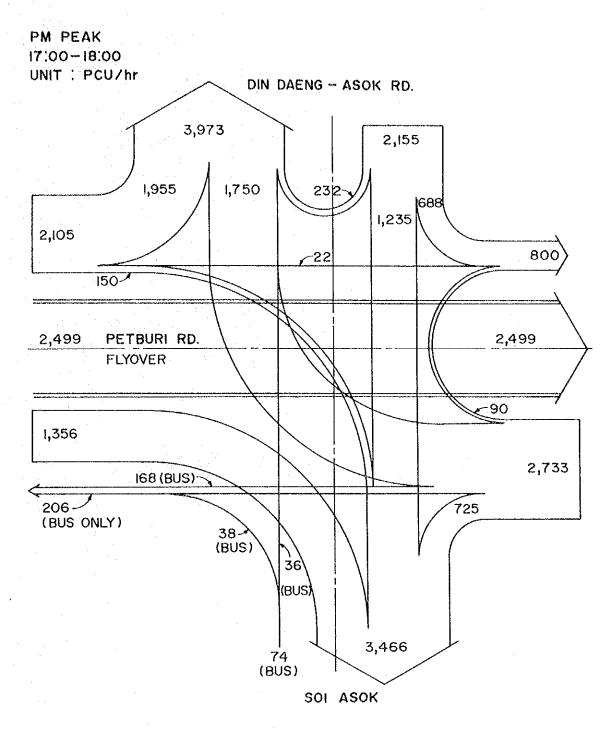


Figure 5.12.2 No. 220

Traffic Volume at Petburi Rd./Soi Asok

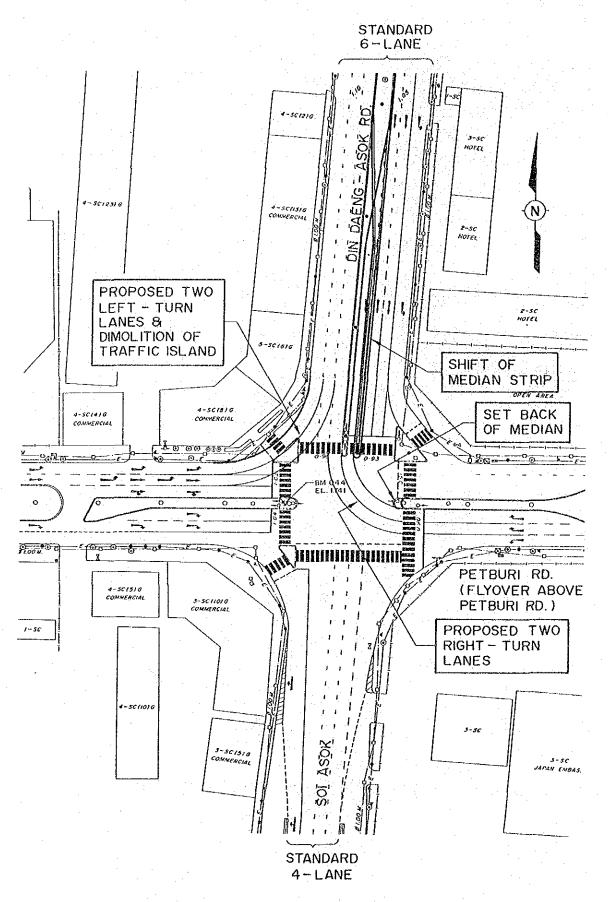


Figure 5.12.3 Proposed Improvement at Petburi/Soi Asok

(5) Cycle time analysis

- Green time is properly arranged to each direction.
- Cycle time in the morning is long, 300 sec.

(6) Planning of improvement alternatives

1) Points of planning

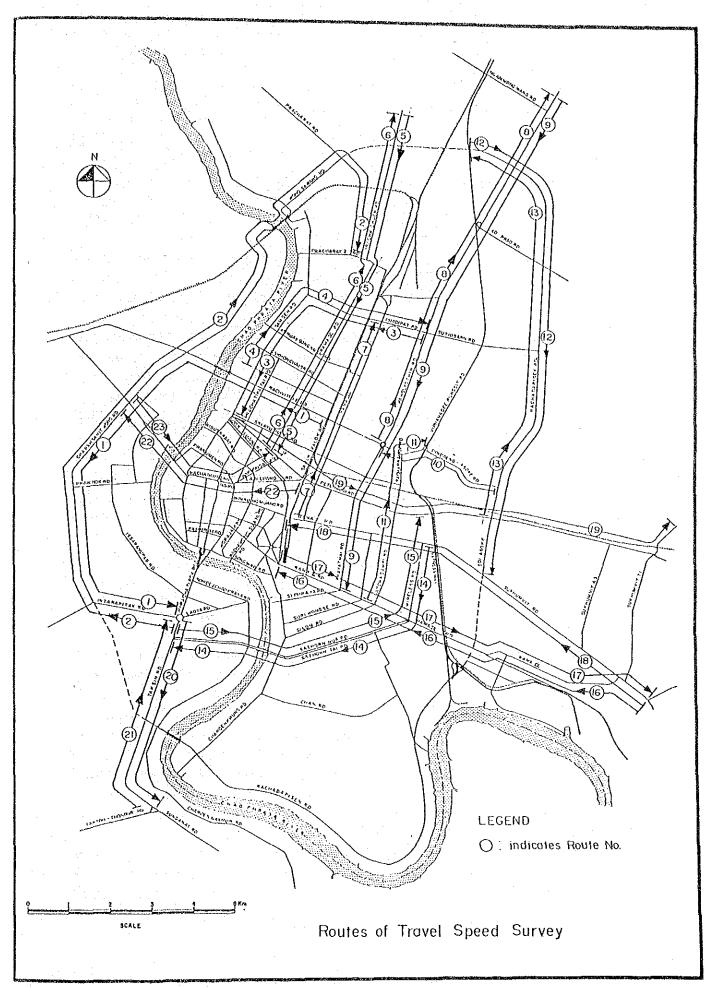
As described in general description, this intersection's improvement should be with that of Middle Ring Road. Therefore, at-grade improvement without land acquisition is planned in this section. A flyover is adready constructed above the intersection.

2) Improvement plan

The problem of this intersection is that two turning movements are interfered each other, left-turn from the west and right-turn from the east (Figure 5.12.2).

To accommodate the two large turning traffics, the west side lane of Dindaeng-Asok road is proposed to be increased from existing 3-lane to 4-lane at the intersection (Figure 5.12.3). For the widening, its median strip is to be shifted to east side. The east through lane is reduced from 3 to 2 but the capacity of traffic is enough. The Dindaeng-Asok road has 6 lanes in standard section. It has enough capacity for the present traffic volume from/to the intersection.

APPENDICES



Measurement of Travel Speed

(1) Equipments

Necessary equipments are as follows

1) Passenger car

A usual passenger car is needed as a sample car to measure the travel speed. Any type of car, a truck or a small bus, can be used as far as it is equipped with an accurate odometer and can carry at least one person (preferably two persons) in addition to the driver. However, a passenger car is preferable because it can be driven at the average speed of the circumstantial traffic more easily than other types of motor vehicle.

2) Wrist watch with a second hand

A wrist watch is used to measure the travel time. The watch has to have a second hand so that the travel time can be measured to the accuracy of seconds. A stop watch can be used instead a wrist watch. In this case, time (of the day) of the measurement should be recorded for the later reference.

(2) Measurement

- 1) Travel speeds should be measured in the section of roads on the both sides of the suspected traffic bottleneck.
- 2) Travel speed can be obtained by measuring the length of the measurement section, D and the time spent for travelling the measurement section, t.
- 3) The section of measurement of travel speed should be designed in such a manner that it has a reference point at one end and the suspected bottleneck on the other. When a suspected bottleneck can not be identified or measurement of travel speed is planned over a certain route of the road network, measurement section can be simply set to coincide with road segments between the intersections. The length of the measurement section should be less than 1.5km.
- 4) Measurement should be carried out during morning and/or evening peak hours so that the suspected bottleneck can be evaluated at its severest traffic congestion.

Fluctuation of Measured Travel Speed and Its Statistical Meaning

(1) Fluctuation of measured travel speed

Condition of traffic congestion in Bangkok greatly varies every day. Therefore, it is desirable to measure travel speed as many times as possible. In this study, however, limitation of time was also important factors to be considered in the planning of the travel speed survey. As the result, travel speeds were measured six times for morning and evening peak hours, respectively, on each route of survey.

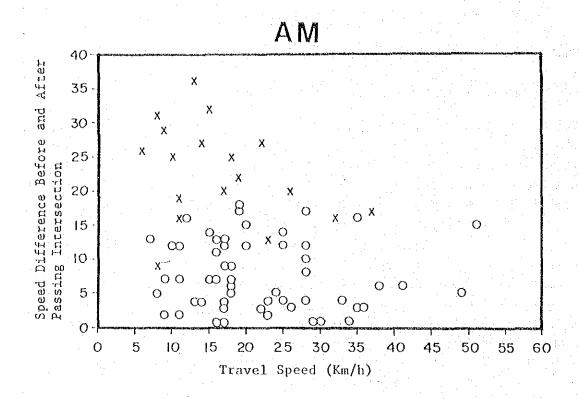
In the early stage of the travel speed, travel speeds on a certain route were intensively (approximately 30 times) measured.

As a result, 356 speed data were obtained. Assuming that these speed data are normally distributed, the standard deviation, was calculated to be $10.02 \, (\text{Km/h})$. Given this standard deviation, maximum deviation of the mean value of six samples (measured speeds) from the mean value of the population was estimated to be \pm 7.5 Km/h with confidence level of 85%. This implies that if travel speed are measured six times and the average value is calculated, the "true" average value is within the range of (average of six measurement) \pm 7.5 Km/h. (This expression may not be correct in a strictly statistical (or mathematical) sense. However, the authors' intention is to give the better picture of the situation.

(2) <u>Significance of difference of speed between</u> before and after passing intersections

As the measured speeds fluctuate significantly, speed increments (or decrements) should be examined to see if they are meaningful.

The result of statistical examination is shown in Figure 1. According to the figure, the boundary between "significant" and "non-significant" seems to be somewhere between 15 Km/h and 20 Km/h. Hence, if the difference of speeds between before and after passing an intersection is larger than 20 Km/h (or 15 Km/h), this difference can be said not to be due to "fluctuation".



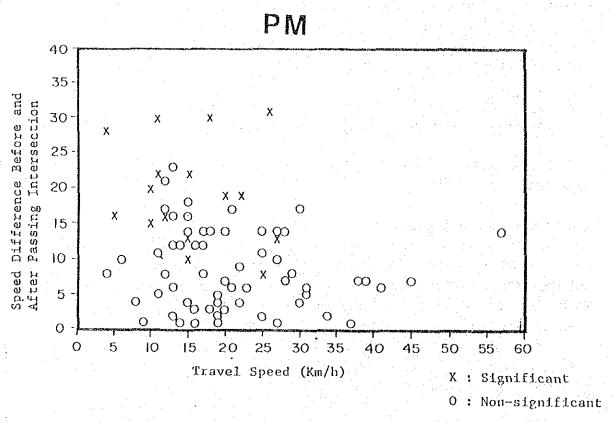


Figure 1 Significant Speed Difference

Correction for measurement section length

In this study, travel speed was measured between major intersections along the survey routes.

The distances of these intersection (hereinafter referred as "measurement section length") range from 0.15 km to 2.2 km as shown in Figure 1.

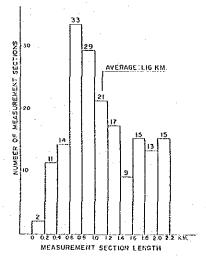


Figure 1 Histogram of Measurement Section Length

This variation of measurement section length give a problem to the proposed rating system, as illustrated in Figure 2.

Let us assume two measurement sections with same length of congestion. In the measurement section with short section length, average travel speed over the measurement section is mainly governed by the travel speed in the congested section, and is measured to be low. On the other hand, in the measurement section with long section length, the average travel speed is measured to be fairly high because of the high speed at non-congested section.

Thus, congestion in the long measurement section has possibility of not being picked up as bottlenecks.

This shortcoming can be compensated by the following procedure.

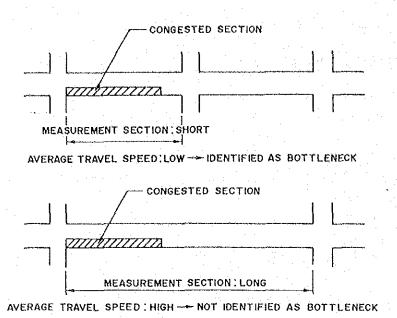


Figure 2 Effect of Difference of Measurement Section Length

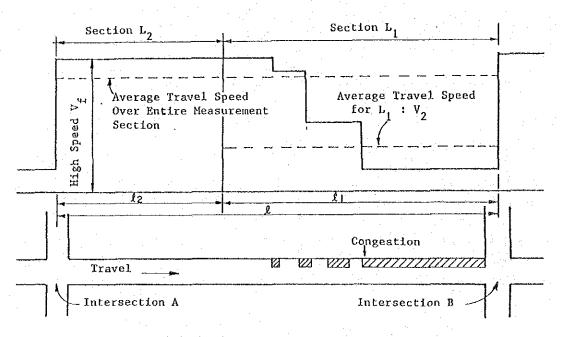


Figure 3 Schematic Curve of Travel Speed

Figure 3 shows the schematic curve of travel speed between intersections. After passing Intersection A, running speed of the vehicle increases to $V_{\rm f}$ which is usually equal to or close to the "free flow speed" defined in HCM, 1985. Because of this high speed, $V_{\rm f}$, the average travel speed is calculated to be relatively high, as shown in the figure. Average travel speed over the section $L_{\rm l}$ adjacent to the Intersection B, including congested section, can be calculated as follows,

$$V_{c} = \frac{\ell_{1}}{t_{L1}} \qquad \dots (1)$$

Where \mathbf{l}_1 is the length of the section including congested section and \mathbf{t}_{L1} is the time to travel \mathbf{l}_1 section.

Hence,

$$t_{L1} = t - t_{L2}$$

$$= t - \frac{2}{V_f}$$

$$= t - \frac{2 - 1}{V_f} \qquad(2)$$

where t is total travel time from Intersection A to Intersection B,

 t_{L2} is travel time for L section,

12 is length of L section and

V_r is high speed observed immediately after passing Intersection A.

Substituting Equation (2) into (1), we get

$$V_{\rm c} = \frac{\ell_1}{t - \frac{\ell_2}{V_{\rm f}}} \qquad \dots (3)$$

In this equation, t is already measured. l_1 can be chosen as such a way as to be close to average measurement section length (1.16 km). Only $V_{\rm f}$ is unknown. By determining $V_{\rm f}$ by some method, $V_{\rm C}$ can be calculated.

For this reason $V_{\rm f}$ of long measurement length (longer than 1.5 km) were measured.

 $V_{\rm f}$ was measured by recording time and distance at short intervals until the vehicle reach the tail of congested section. $V_{\rm f}$ thus measured are shown in Table 1 and 2, together with $V_{\rm c}$ values calculated by assuming l_1 = 1.0 km. Using these $V_{\rm c}$ values as $V_{\rm i}$ in the proposed rating method, additional three and four bottlenecks are identified for the morning and evening peak hour, respectively. Those additionally identified bottlenecks are shown in

Figure 4 with asterisk.

The Figure 4 also shows comparison of bottlenecks with police officer's opinion. If more than one officer identified the same congested intersections, they are round marked in the figure. It shows good conincidence with the result of the rating system.

The additional bottlenecks identified here are not included among the locations for preparing improvement plans.

On the contrary to above-described situation, the average travel speed is measured to be low where the measurement section length may be short, even if the congested section is not long. This occurs because the significance of congested section relative to the entire measurement section increases as the measurement section length becomes short, and the average travel speed calculated over the measurement section tends to be governed by the speed in the congested section rather than the high speed before entering the congested section.

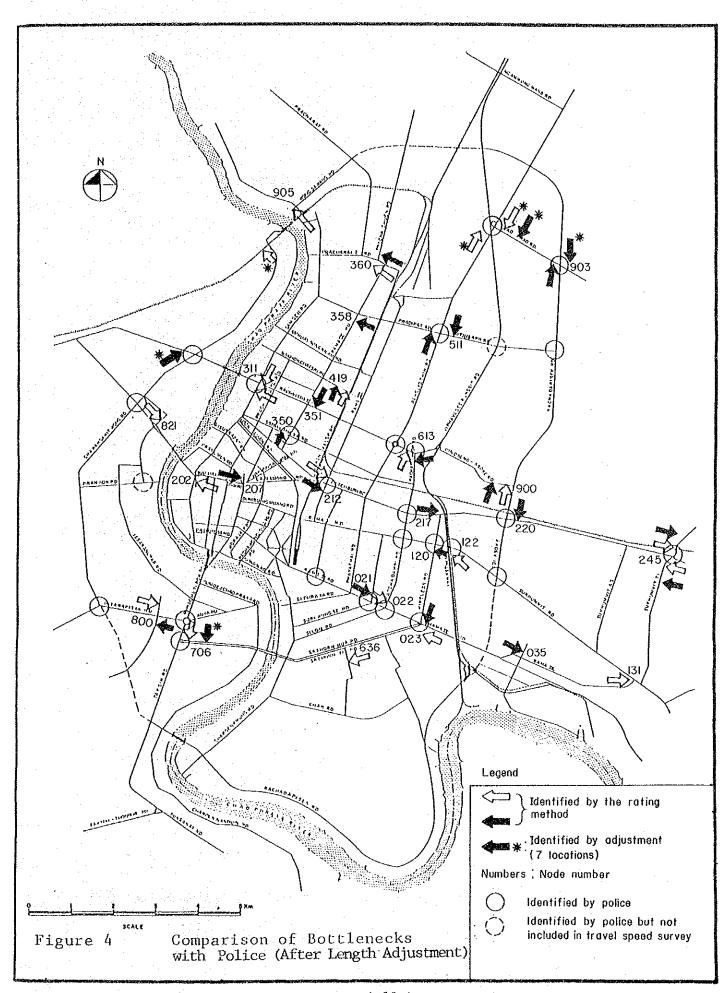
This problem, however, is rather easy to handle, compared to the problem seen in the long measurement section. If it is found that a certain bottleneck was identified because of the shortness of the measurement section length, the identified bottlenecks can be discarded.

Table 1 Vf and Vc in Long Measurement Sections (AM)

Rt. No.	Measure- ment Section Sta Sta.	Length (Km)	Vf (Km/h)	V (Km/h)	Average Travel Speed Over Enter Section V _A (Km/h)
1	3 - 4	1.66	49.1	32.6	38.8
	4 5	1.87	38.2	19.1	25.5
2	6 - 7	1.87	48.2	8.29	17.0
	7 - 8	3.68	53.3	6.3	20.2
	9 –10	1.78	55.5	60.8	58.2
5	1 - 2	2.25	49.7	32.6	42.0
	2 – 3	1.67	52.1	27.9	39.7
	3 – 4	1.82	53.3	28.6	37.6
6	7 – 8	1.82	42.7	49.2	48.9
	8 - 9	1.67	47.7	28.9	40.7
	9 –10	2.25	40.2	38.3	40.6
8	5 – 6	1.84	51.9	29.9	40.1
	6 - 7	1.89	37.6	37.5	39.7
9	3 - 4	1.89	45.0	8.28	20.4
	4 - 5	1.84	43.3	29.9	37.4
12	2 - 3	2.77	60.0	21.8	48.1
	3 - 4	1.71	60.9	39.2	48.2
	5 - 6	2.79	60.5	13.8	41.6
13	2 - 3	2.79	61.0	25.8	43,4
	: 4 - 5	1.71	57.0	23.5	34.9
	5 – 6	2.77	71.7	30.8	51.0
14	6 - 7	1.87	74.8	39.2	54.2
15	1 - 2	1.87	71.1	31.0	45.5
19	10-11	2.35	49.8	32.4	41.4
20	2 - 3	2.01	49.1	41.0	46.3
21	3 - 4	2.01	42.7	30.8	36.5
22	6 - 7	1.53	52.7	27.7	41.3
23	2 - 3	1.63	56.6	12.5	20.1

Table 2 Vf and Vc in Long Measurement Sections (PM)

Rt. No.	Measure- ment Section Sta Sta.	Length (Km)	Vf (Κm/h)	V _c (Km/h)	Average Travel Speed Over Enter Section, V _A (Km/h)
1	3 - 4	1.66	49.1	14.6	20.9
	4 ~ 5	1.87	38.2	19.9	27.0
2	6 - 7	1.87	48.2	10.2	17.9
·	7 - 8	3,68	53.3	12.9	32.3
	9 -10	1.78	55.5	31.1	39.2
5	1 - 2	2,25	49.7	38.5	44.6
	2 - 3	1.67	52.1	34.7	40.1
	3 - 4	1.82	53.3	35.1	42.1
6	7 – 8	1,82	42.7	30.8	37.0
	8 – 9	1.67	47.7	22.1	31.6
	9 –10	2.25	40.2	20.7	30.7
8	5 - 6	1.84	51.9	15.6	26.9
	6 - 7	1.89	37.6	32.4	36.5
9	3 - 4	1.89	45.0	11.8	22.4
	4 - 5	1,84	43.3	36.7	41.8
12	2 - 3	2.77	60.0	20.3	37.6
	3 - 4	1.71	60.9	30.0	44.4
	5 - 6	2.79	60.5	30.7	47.4
13	2 - 3	2.79	61.0	26.43	47.0
	4 - 5	1.71	57.0	13.9	26.3
	5 - 6	2.77	71.71	40.3	57.5
14	6 - 7	1.87	74.82	12.3	39.5
15	1 - 2	1.87	71.1	46.7	61.9
19	10-11	2.35	49.8	8,75	22.7
20	2 - 3	2.01	49.05	15.4	22.0
21	3 - 4	2.01	42.67	31.1	38.6
22	6 - 7	1.53	52.67	31.3	38.8
23	2 - 3	1,63	56.64	27.5	40.9



Reasons of "Un-selected" 17 intersections

Group D Characteristics: Improvement plan already exists or under study (5 locations).

- 12) Sathon/Taksin Rd. (No.706)
 The extension of Sathon Rd. to Middle Ring Rd. and Outer Ring Rd. has been planned by PWD. The study includes the improvement of the intersection.
- 13) Ratchawithi/Sam Sen Rd. (No.311)
 An existing 2-lane bridge at Khlong Sam Sen to the north of the intersection is one of the north of the intersection is one of the causes of traffic congestion. Widening of the bridge to 4-lane one is under planning by BMA.
- 14) Sathon Rd./Soi St. Louis (No.636)

 BMA has a plan of widening and realignment of Soi St. Louis, from existing 2-lane to 4-lane.
- 15) Phetchakasem/Thoet Thai Rd. (No.800)

 BMA has already planned to widen the existing bridge at Khlong Bangkok
 Yai from existing 4-lane to 6-lane. When the missing link of Middle
 Ring Rd. between Phetchakasem and Taksin Rd. is constructed, the
 traffic volume will decrease.
- 16) Wongsawang/Pibul Songkhram Rd. (No.905)

 The congestion is considered to be caused by the existing 2-lane Rama
 VI bridge. A new Rama VI bridge of 6-lane is now under detail design
 and the construction is expected to be completed in 1990.

Group E

Characteristics: The intersections are located beside Khlong with each arch-shaped bridge, along Rama V Rd. (3 locations).

- 17) Thahan/Rama V Rd. (No.358)
- 18) Ratchawithi/Rama V Rd. (No.351) and
- 19) Si Ayutthaya/Rama V Rd. (No.350)

 Benefit/Cost ratio is estimated small and policy coordination with DDS is required.

Group F

Characteristics: At the time of traffic survey, the construction was in progress at the site or near the site (2 locations).

- 20) New Arun Ammarin/Arun Ammarin Rd. (No.821) Widening of approach lanes by two were completed by BMA in February 1986.
- 21) Nakhonchaisi/Swankhalok Rd. (No.419)

 At the time of traffic survey, this intersection was used as a detour road for the construction of a bridge at Khlong Sam Sen in Rama VI Rd. The bridge was completed and the bottleneck was solved.

Group G

Characteristics: One of continuous intersections are selected for planning.

The others can be planned according to the typical plan (2 locations).

- 22) Rama IV/Suriwong Rd. (No.021) and
- 23) Rama IV/Silom Rd. (No.022)

 The Sathon Rd. intersection is selected for detail planning. The above two will be studied together because they are closely located, but excluded from the detail planning for the Sathon is selected as a typical.

Group H

Characteristics: Due to other various reasons, the intersections are not selected.

- 24) Lat Phrao/Middle Ring Rd. (No.903)

 Existing traffic volume is not so much compared with other intersections.
- 25) Ratchadamnoen Klang/Ratchadamnoen Nok Rd. (No.207)
 Existing 8-lane bridge seems to have not enough capacity for a big
 traffic volume. But the bridge is a historical one and needs longer
 study time due to its complexity.
- 26) Sukhumvit/Witthayu Rd. (No.120)
- 27) Sukhumvit/Expressway Ramp (No.122) and

28) Petburi/Witthayu Rd. (No.217)
The three intersections were located near the Expressway on and off ramps and SRT railway. Coordination with other agencies necessitates a longer study period.

Overhead, Profit and Tax as a percentage of Construction Cost

No.	Net Construction Cost (Baht)	OPT (%)
1	downwards 80,000	33.77
. 2	50,000 - 100,000	30.64
3	100,000 - 300,000	27.52
4	300,000 - 500,000	24.40
5	500,000 - 800,000	23.36
6	800,000 - 1,000,000	22.32
7	1,000,000 - 2,000,000	21.28
8	2,000,000 - 5,000,000	20.23
9	5,000,000 - 10,000,000	19.19
10	10,000,000 - 20,000,000	18.15
11	20,000,000 - 40,000,000	17,11
12	40,000,000 - 60,000,000	16.38
13	60,000,000 — 100,000,000	15.55
14	upwards 100,000,000	14.51

Source: Cost Estimation Section, Engineering Design Sub-Division, Design Division, Department of Public Works, BMA

Unit Construction Cost								
Item	Uni t	Cost (Baht)	Remarks					
Flyover								
- Main Span $50^{\text{m}} < L \le 60^{\text{m}}$	2 m	21,000	Continuous Box Girder					
$40 < L \le 50$	m ²	18,000	- ditto -					
30 < L ≤ 40	2 m	16,000	- ditto -					
$20 < L \leq 30$	m ²	8,000	Prestressed Concrete					
		to the state of th	Pretension Girder					
- Side Span	m ²	8,000	RC continuous slab					
			girder					
- Approach Structure	. 2 m	4,200						
Underpass	3	2,500	Facilities are included					
- Tunnel	* m * m	1,500	- ditto -					
- Depressed Structure	* m	2,200	- ditto -					
- Basin - Pedestrian underpass	m	150,000	4 m. wide					
- redestitan underpass		100,000						
Bridge	_m 2	8,000						
- River Crossing L \leq 25 $^{\text{m}}$	m ²	8,000						
- Pedestrian Bridge	m	55,000	3 m. wide					
Pavement	2	600	+ - 0 25 ^m					
- Carriageway	2	600	$t = 0.25^{m}$					
- Sidewalk	m l	300	Interrock surface					
Pedestrian Island	_2 m	50						
Demolition	2							
- Existing Flyover	m ² 2	1,100						
- Buidling	m	200						
- Wooden House	m ²	50						
- Pedestrian Bridge	m 3 2	2,000						
- Sidewalk	m ²	200)					
Traffic Signal	set	400,000						
rigitic Algust	300	200,000						

Note: *excavated Volume

Unit Rate of Land Acquisition Cost

Node No.	Name of Intersection	Name of Road	Unit Rate (Baht/m ²)	Land Use
202	Ratchadamnoen Klang/ Ratchadamnoen Nai Rd.	Rachini	10,000	Government Institution Area
613	Dindaeng/ Ratchaprarop Rd.	Dindaeng	7,500	Residential/ Commercial Area
511	Pradipat/ Phahon Yothin Rd.	Phahon Yothin	10,000	Commercial Area
212	Petburi/ Rama VI Rd.	Rama VI	10,000	Commercial Area
630	Pracharat II/ Pracha Chuen Rd.	Khan Khlong Prapa	2,000	Residential Area
131	Sukhumvit/ Rama IV Rd.	Sukhumvit (within 40 m from R/W)	8,750	Commercial Area
		Sukhumvit (beyond 40 m)	2,000	
245	New Petburi/ Ramkhamheng Rd.	Phatthanakan/ Ramkhamheng (within 40 m from R/W)	8,750	Commercial Area
		Phatthanakan/ Ramkhamheng (beyond 40 m)	2,000	
035	Rama IV/ Kasemrat Rd.	Rama IV (within 40 m from R/W)	7,000	Residential Area
		Rama IV (beyond 40 m)	2,000	

Unit Rate of Compensation Cost

Unit : Baht per sq.m.

1-2 Storey, wood, residential, poor condition	1,500
Brick/concrete, commercial or residential (multi family) good condition	3,000
Brick/concrete, commercial or residential (multi family) poor condition	2,000
Wood, commercial, industrial or warehouse, poor condition	1,500
Brick/concrete, commercial industrial or warehouse, good condition	3,000
Brick/concrete, commercial, industrial or warehouse, poor condition	2,000
1-2 Storey, concrete/brick, residential, poor condition	2,000

Land Acquisition Cost

Description		Unit Rate (Baht per sq.m)	Area (sq.m)	Acquisition Cost (Million Baht)
Ratchadamnoen Klang/ Ratchadamnoen Nai	A1t - 3	10,000	480	4.8
Dindaeng/Ratchaprarop Alt - I		7,500	330	2.5
Pradipat/Phahon Yothin	Alt - 1	10,000	1,780	17.8
Petburi/Rama VI	Alt - 1	10,000	2,180	21.8
Pracharat II/	Alt - 1	2,000	2,810	5.6
Pracha Chuen	Alt - 2	2,000	11,880	23.8
		8,750	3,360	29.4
Sukhumvit/Rama IV	Alt - 1	2,000	12,570	25.1
		TOTAL	15,930	54.5
		8,750	3,600	31.5
Í	Alt - 2	2,000	10,200	20.4
		TOTAL	13,800	51.9
		8,7 50	210	1.8
Petburi/Ramkhamheng	Alt - 1	4,500	430	1.9
		TOTAL	640	3.7
	•	8,750	180	1.6
	Alt - 2	4,500	550	2.5
	·	TOTAL	730	4.1
		7,000	2,100	14.7
Rama IV/Kasemrat		2,000	3,540	7.1
		TOTAL	5,640	21.8

Compensation Cost

Description		Wooden, residential bldg. (sq.m.)	Brick/concrete commercial or residential bldg. poor condition (sq.m.)	Brick/concrete commercial or residential bldg. good condition (sq.m.)	Total (Million Baht)
Unit rate (Baht per sq.m)	5q.m)	1,500	2,000	3,000	
Pradipat/Phahon Yothin	A1t - 1	ı	ì	43,000	129.0
Petburi/Rama VI	Alt - 1	I	ı	8,500	25.5
Pracharat II/	A1t - 1	4,220	130		6.6
Pracha Chuen	Alt - 2	5,700	650		6.6
Sukhumvit/Rama IV	Alt - 1	029	3,570	6,420	27.4
	Alt - 2.	330	5,130	14,520	54,4
Rama IV/Kasemrat		ı	-	1,850	5.6

Construction Quantity (1)

	Item	Unit	Alt 1	A1t 2	Alt 3
Rama IV/Siphraya -Sathon	Flyover Main span (L=60 m) Main span (L=50 m) Side span Approach structure	2 m2 m2 m2 m2	- - - -	6,800 - 6,400 1,770	6,800 1,240 9,260 1,620
	Pedestrian underpass	m	11 	160	160
	Pavement Carriageway	2 m 2	-	680	930
	Island	m T	-	7,000	9,570
	Demolition Sidewalk	2 m	_	260	260
	Traffic signal	set	-	4	6
/8u	Underpass Tunnel Depressed structure Basin	*m3 *m3 *m3	-	-	19,400 21,000 480
Ratchadamnoen Klang/ Ratchadamnoen Nai	Pedestrian underpass	m	=	35	25
	Pavement Carriageway Sidewalk	2 m ₂ m	1,900	1,560 -	3,250 380
	Island	2 m	2,150	2,700	2,960
	Demolition Sidewalk	_m 2	55	85	320
	Traffic signal	set	5	4	5
rop	Flyover Mainspan (L=50 m) Mainspan (L=40 m) Side span Approach structure	m2 m2 m2 m2 m2	- 640 1,920 620	960 - 1,600 760	- - -
Dindaeng/Ratchaprarop	Underpass Tunne1 Depressed structure Basin	*m3 *m3 *m3	3,080 17,280 400	3,080 17,280 400	-
	Pavement Carriageway Sidewalk Island	m2 m2 m2	2,280 410 4,730	1,820 590 4,810	-
	Demolition Sidewalk	2	410	870	•
	Traffic Signal	set	2	3	_

Construction Quantity (2)

	Item	Unit	Alt 1	Alt 2	Alt 3
	Flyover				
	Main span (L=50 m)	m2		990	1 040
	Side span	2		,	1,240
	Approach structure	m ₂		2,880 850	3,600
ij				630	1,060
Pradipat/Phahon Yothin	Pedestrian bridge	m.	110		<u></u> .
×	Pavement	,	•		
Ö	Carriageway	m ₂	1,590	450	450
ar ar	Sidewalk	m ²	1,780	_	
Ph	Tellond	2			
t t	Island	ın	~	2,750	3,410
О¹ Св	Demolition				
당	Building	m ₃ ²	43,000	-	-
11	Pedestrian bridge	m ₂	73	~	-
O.	Sidewalk	m m	1,590	-	
	Traffic signal	set	4	2	2
	Flyover	2	-		
	Main span (L=35 m)	m ₂		280	-
	Side span	m ₂	-	3,580	-
·	Approach structure	m ²	-	250	· ·-
Petburi/Rama VI	Pedestrian bridge	m	64		-
લુ	Pavement	2			[
. 6	Carriageway	m ₂	1,810	-	-
Œ,	Sidewalk	m	1,620		
Ĺ	Island	2	450	2,970	_
, pr	Island	m	430	2,510	
et l	Demolition	,			
1-14	Flyover	m ₃ ²	_	1,230	
·	Pedestrian bridge	m ₂	12	-	-
	Building	m ₂	8,370		-
	Sidewalk	m	1,030	_	
	Traffic signal	set	2	1	-
렸	Bridge	2	290	920	_
jage		1111	130	320	
] 3c	Pavement	2			
, id	Carriageway	m ₂	3,800	9,890	-
ra	Sidewalk	m	3,890	7,710	_
Pracharat II/Prachachuen	Island	2 m	_	1,870	-
🛱	Demolition				
မှ	Building	2 m	130	650	-
អូ	Wooden house	m ₂	4,220	5,700	
ਰੂ	Pedestrian bridge	¹¹¹ 3	19	19] -
l å		m		ļ	
ρ,	Traffic signal	set	4	3	1 -

Construction Quantity (3)

	· · · · · · · · · · · · · · · · · · ·				A STATE OF THE PARTY OF THE PAR
	Item	Unit	Alt 1	A1t 2	A1t 3
ΔI	Flyover Main span (L=34 m) Side span Approach structure	2 m2 m2 m2		-	580 5,420 950
Sukhumvi t/Rama	Pavement Carriageway Sidewalk Island	2 m2 m2	9,290 5,520 1,330	8,140 4,780 1,150	40 - 3,810
Sukhı	Demolition Sidewalk Traffic signal	m ²	220 4	340 4	4 0
ხდ	Flyover Main span (L=35 m) Side span Approach structure	2 m ₂ m ² m ²		320 2,160 900	
amkhamhen	Bridge River crossing Pedestrian bridge	2 m m	640 185	640 -	-
Petburi/Ramkhamheng	Pavement Carriageway Sidewalk Island	2 m2 m 2 m	2,520 1,040 920	1,270 790 3,080	-
ρ	Demolition Sidewalk Signal	2 m	2,050 4	970 5	-
mrat	Pavement Carriageway Sidewalk	2 m2 m2	4,580 1,670	-	-
Rama IV/Kasemrat	Island Demolition Building Sidewalk	m 2 m 2 m 2 m 2 m 2 m 2 m	1,850 460	-	
Petburi/ Ra Soi Asok	Traffic signal Pavement Carriageway Sidewalk	set 2 m2 m2	124 22		
Pet	Island	2	94	20 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	-

Construction Cost (1)

		# 64°3 ************************************	Unit:	1,000 Baht
	Item	Alt 1	Alt 2	Alt 3
	Flyover Main span (L=60 m) Main span (I=50 m)	Min.	142,800	142,800 22,320
ıya m	Side span Approach structure		51,200 7,420	74,080 6,800
IV/Siphraya - Sathon	Pedestrian underpass	·	24,000	24,000
IV/S	Pavement Carriageway	_	410	660
Rama	Island Demolition		350	480
PC4	Sidewalk		50	50
	Traffic signal	-	1,600	1,200
	TOTAL		227,830	272,290
	Underpass Tunnel Depressed structure Basin		 	48,500 .31,500 1,060
Klang/ Nai	Relocation of utilities	-		6,250
Ratchadamnoen Kl Ratchadamnoen Na	Pedestrian underpass Pavement Carriageway Sidewalk	1,140	5,250 940 -	3,750 1,950 110
chade chade	Island	110	130	150
Rat	Demolition Sidewalk	10	20	60
	Traffic signal	2,000	1,600	2,000
	TOTAL	3,260	7,940	95,330
rop	Flyover Main span (L=50 m) Main span (L=40 m) Side span Approach structure	7,680 15,360 2,600	17,280 12,800 3,190	**** *** ***
Dindaeng/Ratchaprarop	Underpass Tunnel Depressed structure	7,700 25,920	7,700 25,920	- -
idaeng/R	Basin Pavement Carriageway	1,370	1,090	-
Din	Sidewalk Island	180 240	180 240	-
	Demolition Sidewalk	80	170	_
	Traffic signal	800	1,200	-
	TOTAL	62,810	70,650	-

Construction Cost (2)

	Construct	ion Cost (2)	Unit:	1,000 Baht
- <u>1</u>	Item	Alt 1	Alt 2	Alt 3
Ç <u>.,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, </u>	Flyover Main span (L=50 m)		17,980	22,320
ដ	Side span Approach structure	-	23,040 3,570	28,800 4,450
Yothin	Pedestrian bridge Pavement	6,050	<u>-</u>	
ahon	Carriageway Sidewalk	950 530	270	270
at/Ph	Island Demolition	~	140	170
Pradipat/Phahon	Building Pedestrian bridge Sidewalk	8,600 510 320	_ ·	<u>.</u>
	Traffic signal	1,600	800	800
	TOTAL	18,560	45,640	56,810
	Flyover Main span (L=35 m) Side span Approach structure	- ·	2,240 28,640 1,050	- -
	Pedestrian bridge	350	<u>-</u>	-
ma VI	Pavement Carriageway Sidewalk	1,090 490	<u>-</u>	
tburi/Rama	Island	20	150	•
Petbu	Demolition Flyover Pedestrian bridge Building	20 1,670	1,350 - -	-
	Sidewalk Traffic signal	210 800	400	<u>-</u>
	TOTAL	4,650	33,830	-
	Bridge	2,320	7,360	· •
II/Prachachuen	Pavement Carriageway Sidewalk	2,280 1,160	5,930 2,310	
/Praci	Island Demolition	-	90	1944 - 19 <u>2</u> 1 1851 - 1951 - 1951 1851 - 1951
	Building Wooden house	30 210	130 290	-
Pracharat	Pedestrian bridge Traffic signal	1,600	40 1,200	• • • • • • • • • • • • • • • • • • •
Pr	TOTAL	7,640	17,350	_

Construction Cost (3)

		Unit	: 1,000 Baht
Item	Alt 1	Alt 2	A1t 3
Flyover Main span (L=34 m) Side span Approach structure	-		4,640 43,360 3,990
Pavement Carriageway Sidewalk Island Demolition	5,570 1,660	4,880 1,430	20
Island Demolition	70	60	190
Sidewalk	40	70	10
Traffic signal	1,600	1,600	
TOTAL	8,940	8,040	52,210
Flyover Main span (L=35 m) Side span Approach structure	-	2,560 17,280 3,780	- -
Bridge River crossing Pedestrian bridge Pavement Carriageway Sidewalk A D D D D D D D D D D D D D D D D D D	5,120 10,180	5,120	-
Pavement Carriageway Sidewalk	1,510 310	760 240	-
Demolition	50	150	-
Sidewalk Signal	410 1,600	190 2,000	-
TOTAL	19,180	32,080	-
Pavement Carriageway Sidewalk	2,750 500		-
Island	30	ma.	
Sidewalk Island Demolition Building Sidewalk Traffic Signal	370 90	<u>.</u> _	-
Traffic Signal	800	_	-
TOTAL	4,540		
Pavement Carriageway Sidewalk Sidewalk Sidewalk Sidewalk	74.4 6.6	- 	-
Island	4.7	-	-
TOTAL	86	-	-

Appendix 4.4.8

Vehicle Operating Costs (Baht/1000 kms)

Speed Kph	Motor- cycle	Car	Pick-Up	Taxi	Truck	Bus	Weighted average
5	860	3459	2518	1962	6590	9326	2680
10	791	3095	2252	1734	5356	7455	2357
15	685	2721	1938	1528	4692	6306	2052
20	617	2471	1753	1410	4293	5639	1861
25	568	2282	1621	1325	4067	5198	1724
30	533	2142	1525	1268	3878	4881	1624
35	513	2040	1473	1233	3770	4684	1559
40	495	1955	1433	1204	3684	4530	1505
45	482	1892	1404	1185	3664	4439	1468
50	475	1849	1390	1180	3632	4367	1445
55	470	1811	1374	1175	3627	4366	1426
60	465	1814	1364	1174	3635	4370	1424
65	468	1824	1364	1179	3676	4385	1430
70	468	1827	1362	1179	3720	4501	1436
75	472	1840	1372	1190	3807	4590	1450
80	480	1850	1392	1204	3951	4728	1470
85	486	1880	1418	1220			_
90	496	1910	1451	1242			
Vehicular Composition	0.250	0.341	0.218	0.133	0.024	0.034	1,000

Source: SSES Study, 1986

Remarks: Weighted Average; estimated by weighted average of vehicular

composition

Road Improvement Plan

Summary of Economic Evaluation

				1		ř.				·.									
	_						Eva	Evaluation Period: 10 Years	eriod:	10 Year	s			Eva	luation	Evaluation Period: 20 Years	20 Year		
;	,		Const. Initi-		У 5	, t	ř	Benefit (B)	۲۵	Eval	Evaluation		Cost		Benefit (B)	(3)	Eva	Evaluation	
No.	Ylan Lmp. No. No.	r. (Intersection)	Period al (Year) Cost		(per year)		Time Saving S	Time VOC Total Net Saving Saving Benefit Benefit (B-C)	Coral mefit B		B/C	IRR		Time	VOC Saving	Time VOC Total Net B/C. Saving Benefit (B-C)	Net Senefit (3-C)	3/C	IRR
٦ ٦	#5	Rama IV / Si Phraya - Sathon	2.	302.5	3.7	318.07 ;	771.51	318.07 771.51 68.66 840.17 522.10 2.64 0.235 336.49 1395.37 124.19 1519.56 1,183:07 4.52 0.269	0.17 5	22.10	2.64	0.235	336.49 I	395.37	124.19	95.6121	1183.07	4.52	0.269
다	m	Rama IV / Si Phraya - Sathon	74	361.7	3.9	377.09	912.86	377.09 912.86 81.24 994.10 617.01 2.64 0.233 396.51 1651.03 146.94 1797.97 1401.46 4.53 0.268	94.10 8	17.01	2.64	0.233	396.51	651.03	146.94	76-76/1	1,401.46	4.53	0.268
2-2	m	Rachadamnoen Klang / Nai	m	134.4	4.2	150.14	133.83	150.14 133.83 12.58 146.41 -3.74 0.98 0.031 171.05 260.75 24.51 285.26 114.21 1.67 0.107	17.91	-3.74	86.0	0,031	171.05	260.75	24.51	285.26	114.21	1.67	0.107
6. 6.	۲۱	Din Daeng / Rachaprarop	m	98.5	3.7	113.30	212.52	113.30 212.52 39.74 252.26 138.96 2.23 0.189 131.72 414.07 77.43 491.50 359.78 3.73 0.234	12.26 1	38.96	2.23	0.189	131.72	414.07	77.43	491.50	359.78	3.73	0-234
7-2	m	Pradipat / Phahon Yothin	2	76.7	1.1	81.64	168.40	81.64 168.40 64.50 232.90 151.25 2.85 0.255 87.12 304.57 116.65 421.22 334.10 4.83 0.287	12.90 1	51.25	2.85	0.255	87.12	304.57	116.65	421.22	334.10	4.83	0.287
۲۵ د	₹	Perburi / Rama VI	-1	58.2	6 0	64.60	75.34	64.60 75.34 25.77 101.11	11.11	36.51	1.57	0.139	69.08	128.10	43.81	36.51 1.57 0.139 69.08 128.10 43.81 171.91 102.83 2.49 0.186	102.83	2.49	0.186
i H	~ ~	Petburi / Rama VI	7	45.6	1.2	51.90	179.47	51.90 179.47 61.38 240.85 188.94 4.64 0.397 57.87 324.59 111.01 435.60 377.73 7.53 0.415	0.85	88.94	79.4	3.397	57.87	324.59	111.01	435.60	377.73	7.53	0.415
면 일 9	4	Pracharat II / Pracha Chuen		24.1	0.5	27.65	26.86	27.65 26.86 10.50 37.36	37.36	9.71	1.35	2,107	30.14	45.67	17.85	9.71 1.35 0.107 30.14 45.67 17.85 63.52 33.38 2.11 0.160	33.38	2.11	0-160
7-4	m 	Sukhumvic / Rama IV	7	70.4	1.5	77.96	156.17	77.96 156.17 20.30 176.47		98.51	2.26	3,207	85.42	282.45	36.71	98.51 2.26 0.207 85.42 282.45 36.71 319.16 233.74 3.74	233.74	3.74	0.245
. A	.H	Perburi / Ramkhamheng	н	30.5	0.1	37.61	288.87	37.61 288.87 121.04 409.91	19.91	72.30	0.90	126.0	42.59	491.16	205.80	372.30 10.90 0.951 42.59 491.16 205.80 696.96 654.3716.37 0.952	654.37	6.37	0.952
6-4	<u>д</u>	Rama IV / Kasemrat	н	36.5	9.0	40.76	73.27	40.76 73.27 32.10 105.37 64.60 2.58 0.272 43.75 124.59 54.57 179.16 135.41 4.09 0.299	5.37	64.60	2.58	3.272	43.75	124.59	54.57	179.16	135.41	60.7	0.299

Source: JICA Team Estimation

Remarks: 1. Costs and Benefits are calculated at 1986 price.

2. Rate of discount: 5% per annum.

3. Traffic flow and volume are set at 1986 level.

4. Improvement alternative number (Imp. Alt. No.) marked with (#) indicates the alternative whose economic evaluation is made for comparative purpose.

5. Weighted average time value: 22.1 Baht per vehicle-hour.

VOC stands for vehicle operating cost, comprising the costs of fuel, oil, tyres, depreciation, maintenance for parts and labour, interest and age depreciation.

7. PV: Present Val:

(2) Benefit/Cost Calculation

Plan No. : P-1 #(2)

Location : Rama IV/Surawongse-Silom

0.05 Bo (Mil.Baht/Y) = 136.49 Cost (Mil.Baht) = 302.50 Discount Rate =

Unit = Benefit & Cost : Baht in million

											· .					٠.								-
	172	-	-	ı	1	!	ı	i	1	, 1 ·	1	1	0.235	1	 • 1 	1	i	ı	1	1	1	1	1	0.269
	٦/ ل	2 /2	1	ı	ı	ı			ì	. #	Ļ	1	2.64	1	1	ı	ı	1	1	1	1	. 1	ı	4.52
	Net	Benefit	1	1	t,		ı	1)	ı	1		522.10	1		1	1	1	1	1		1	1	1183.07
		ΡV	0	00.00	23.8	17.9	2	6.90	01.8	7	92.38	7 . 5	840.17	3	9	0.9	2.3	ω ο΄	9.	2.5	59.55	6.7	0.4	1519.56
		Total	0	٠.	4.	5.4	5.4	6.4	7.9	4 9	136.49	9.4	1091.94		7.9	136.49	36.4	36.4	36.4	36.4	136.49	36.4	36.4	2456.86
Benefit	Gross	V.0.C	0	0	를		T.	٠	11.15	11.15	11.15	11.15	89.24	11.15	11.15	11.15	11.15	11.15	11.15	11.15	11.15	11.15	11.15	200.79
		T. Save	4		25.3	C/I	λ. Ω	25.3	25.3	125.34	125.34	125.34	1002.70	25.3	25.3	125.34	25.3	25.3	25.3	25.3	25.3	Ç	125.34	2256.07
		PV	51.	•	က်	4	. •	•	•	. •	2.50	. •	318.07	.2		2.06	တ္	ω,	7.	,		1.54	4.	336.49
		Total	51.2	7	3.7	.7	_	·	. 7	7	3.70	3.70	332.10	1.	1.	7.	سنة	~			3.70		1	369.10
Cost	Gross	м/о	0	0	1	·		1.	7.		3.70	7	29.60		1	-	1	7			3.70		7	09.99
		Const.	51.		•	•	00.0		•		0.00	00:00	302.50								00.0			302.50
	Year		r-4	7	m	ব	5	9	7	00	6	10	SUB	11	12	13	14	15	16	17	18	19	20	TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(3) Benefit/Cost Calculation

Plan No. : P-1 (3)

Location : Rama IV/Siphraya-Silom

0.05 Bo (Mil.Baht/Y) = 161.50 Cost (Mil.Baht) = 361.70 Discount Rate =

Unit = Benefit & Cost : Baht in million

	IRR		-	ı	:	ı	ı	ſ	<u> </u>	ı	ı	l	0.233	ı	·I	ı	ı	ļ	. 1	1	l	i	1	0.268
	٥/ لا	> />	1	í	ı		ı	1	1	1	1	ı	.2.64	1	i	ı	1	1	ı	1	ı	ı	ı	4.53
	Net	Benefit	1	ı	1	ı	1 2	1	ľ	1		1	617.01	1	1	1	ł	ı	1	ı	1	i	1	1401.46
		ΡV	0		9	139.51	32.	26.5	20.5	114.77	09:3	\circ	994.10	9.1	4.4	9.0	5.6	5.5	7.6	73.98	4.0	67.11	3.9	1797.97
		Total		0	61.5	161.50	61.5	61.5	61.5	61.5	61.5	61.5	1292.00	61.5	61.5	61.5	61.5	61.5	61.5	161.50	61.5	61.5	161.50	2906.99
Benefit	Gross	V.0.C	00.0	00.00	13.20	'n	13.20	•	7	ω,	13.20	ന	105.59	3.2	3.2	.2	3.2	3.2	3.2		3.2	13.20	3.2	237.57
		T. Save	•	00.00	4	148.30	48.3	48.3	48.3	148,30	48.3	48:3	1186.41	გ	α.	ຕຸ	ω,	ა ლ	ω 	148.30	ຜູ	8.3	4	2669.42
		₽V	80.8	2.2	S	3.37	N	0	o.	$\overline{}$	0	Ω.	377.09	ω.	2	Т.	٥.	o,	φ.	1.79	۲.	9	• 5	396.51
1°		Total	•	80.	•	3.90	•	•	٠	. •	•	•	392,90	•		•		•	•	3.90	•	•	•	431.90
Cos	Gross	М/О	٠			3.90	•			•		•	31.20	O)	O	o.	o.	σ	Q.	3.90	o,	ο.	ο.	70.20
		Const.	180.85	180,85	0.00	0.00	00.0	0.00	0.00	0.0	00.0	0.00	361.70							00.0			•	361.70
	Year		rd	7	ന	4	'n	9	7	00	on	10	SUB	T	12	133	14	1.5	16	17	18	19	20	TOTAL

Remarks are the same as Appendix 4.4.9 (1) Source; JICA Team Estimation

(4) Benefit/Cost Calculation

Plan No.: P-2 (3)

Location : Rachadamnoen Klang/Nai

Unit = Benefit & Cost : Baht in million 0.05 Bo (Mil.Baht/Y) = 27.90 Cost (Mil.Baht) = 134.40 Discount Rate =

	**************************************	IRR		1	ı	1	İ	ı	1	1	1	ı	ł	0.031	ı	1	1	ı	1	1	l	1	1		0.107
	٠	٦/ ٦	ì	ı	1	1	i	1	1	1.	!	1	1	0.98	1	I	ı		1	i,	1	t	l	1	1.67
			Benefit	1	1	i	1	. 1	1	i v	ı	1	1	-3.74	1	1	ì		1	1	1	1	ı	•	114.21
			ΡV	0	0	0.00	·~	2	∞	0	S	∞	7	146.41	7 1	6.3	J.	4 7	. •	3.4	2.7	2.1	1.5	o O	285.26
			Total		•	0	7.	6.7	27.90	7	6	7	7	195.27	7.9	7.9	7.9	7	27.90	6.	7	6	7.9	<u>~</u>	474.23
	Benefit	Gross	V.0.C			00.0	2.40		2.40	2.40	•	2.40	2.40	16.78		٠	•) b	2.40	•	•	•	•	٠	40.75
			T. Save			0.00	งา	S	. *	S	้	Ŋ	Ś	178.49	5.5	5.5	5.5	Ś	25.50	5.5	ιζ.	ίζ.	5.5	ις.	433.48
			PV	ું∞.	9	40.63	Q	4	2	~	O.	∞	·~	150.14	5	4	ć,	7	2.12	0	0	∞	7	1.6	171.05
	;¢		Total	4.8	ω	44.80	7	7	7	4	2		3	163.80	2	4	2		•	2		4		4.20	205.80
.	Cost	Gross	М/О	0	0	0.00	3	ď	2	d	N	?	4	29.40	3	?	7	3	4.20	7			.2	4.20	71.40
			. Const.	08.44	44.80	44.80	0.00	00.0	00.00	0.00	00.0	0.00	0.00	134.00	0.00				00.0			00.0			134.40
		Year		rI	7	<u></u>	4	5	9	7	ω	9	10	SUB	H	12	13	14	5	1:6	17	81	61	20	TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

5) Benefit/Cost Calculation

Plan No. : P-3 (2)

Location : Dindaeng/Rachaprarop

Bo (Mil.Baht/Y) = 48.06 Cost (Mil.Baht) = 98.50 Discount Rate =

	.	-		·										: .											
		IRR			1	ţ	1	1	ı	ł	1	1	-	0.189	1	ı	ŧ	1	ı	1	ı	ı	ł	ı	0.234
1		B/C		ţ	1	1	1	1	1	ŧ	ı	1	i	2.23		ı	ı	1	i	1	ı	ı	1	1	3.73
ייים אייים איי		Net	Benefit	1	ı	1	1	ı	1	1	ı	ı	l	138.96	1	1	1	ı	1	ı	ı	I	ł	ı	359.78
יייייייייייייייייייייייייייייייייייייי			∆ di		00.00	0	41.52	5.5	φ.	8	4.1	2.5	0.9	252.26	9.5	د. د	6.7	25.49	4.2	3.1	2.0	0.0	و. ف	19.02	491.50
77.0			Total		00.00		∞					48.06	0	336.45				48.06	•					•	817.10
[.]	Benefit	Gross	V.0.C	0.00	00.0			,	7.57	7.57	7.57	7.57	7.57	53.00	7.57	7.57		7.57					7.57	7.57	128.73
70.10 U.S. 1.10 C.S. 1.10			I. Save	0.00	00.0	0.00	65.05	67.07	40.49	40.49	•	65.05		283.45	40.49	4	0	40.49	\ ,	4.	7	67.07	65.05	7	688,37
(1000000000)			PV	2.8	31.27	9.7	7	0	ο,	7.	9	Ŋ	n	113.30	. 2	~~	0	1.96	∞	. 7	7	ò	₹.	٠,4	131.72
3800 00.	t)		Total	7	32.83	ď		•		•				124.40	•	•	•	3.70	٠	•	•	•		•	161.40
) - -	Cost	Gross	0/M		0.00	•	•	•	•	•		•	•	25.90				3.70			۲.	7	۲.	7.	62.90
(- (- 1 - 1 - 1	Const.	2.	32.83	ci.	•		•	•	•	•	•	98.50	•		•	00.00		•			•		98.50
		Year		щ	7	ന	4	ιΩ	9		ω	თ	10	SUB	11	12	13	14	15	16	17	18	19	20	TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(6) Benefit/Cost Calculation (Road Improvement Plan)

Plan No. : P-4 (3)

Location : Pradipat/Phahon Yothin

Unit = Benefit & Cost : Baht in million 0.05 Cost (Mil.Baht) = 76.70 Discount Rate = Bo (Mil.Baht/Y) = 37.84

		IRR	ı		ı	- 1	1	ı	ı	1	ı	1	0.255	1	1	ı	ł		ı	i			1	0.287
		B/C		.	·		1	1	į	1	ı	1	2.85	1	1	1	ı	ı	1	ł	. 1	1	ı	4.83
:	N q	Benefit			•	ı	.	1	1	1	1	l	151.25	1	. I	1	1	ŧ	1	1			*	334.10
		PV	0	0.00	. ~	2.6	1.1	29.65	8.2	8	Ω		232.90	3.2	7.	़	0.0	19.11	7	7.3	6.5		4	421.22
		Total	•		~	1		37.84	٧.	~	~	/	302.69	7.8	7	7.8	7.8	37.84	7.8	7		7.8		681.04
Benefit	Gross	V.O.C	0	0	4	7.0	0.4	10.48	0.4	7.0	4.0	7.0	83.83	7	7.0	0.4	0.4	10.48	7.0	7	7	4	0.4	188.60
		T. Save			۲.	7	_	27.36	7	7		7	218.86	7.3	7.3	7.3	7.3	27.36	7.3	7:3	7.3	.3	7.3	492.44
		PV	•	Ġ.	•		•	0.86	•		•	•	81.64		_		-	0.56			-			87.12
Ļ		Total	· Ω	က		∹.	П	1.10	Η.	П	Ц	Н.	85.50		٦		-!	1.10	Н,	4	Н	Ч.	Τ,	96.50
SOO	Gross	М/0	0	0	7	Н	r-H	1.10	덕.	-	, , , ,	۲.	8.80	Н.	٦	۲	ᅼ	1.10	۲.	r!		Н.	۲.	19.80
		Const.		œ.	•		•	0.00	•	•		•	76.70		2.0			0.0		•		. •	•	76.70
	Year		-1	7	<u>m</u>	4	Ŋ	•	7	ω	O)	10	SUB	러	12	F7	7.	7	16	17	18	.T	20	TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(7) Benefit/Cost Calculation (Road Improvement Plan)

Plan No. : P-5 #(1)

Location : Patburi/Rams VI (at grade)

0.05 Bo (Mil.Baht/Y) = 14.22 Cost (Mil.Baht) = 58.20 Discount Rate =

		Cos	ų.			Benefit					
Year		Gross				Gross					,
	Const.	м/о	Total	PV	T. Save	V.0.C	Total	ΡV	Benefit	Э/С 	LKK
Н				- 5	0	0.		0	ı	-	1
C1				∞	9.0	φ.	4	3.5	1	1	· I
ო	•			∞	0.6	9	4	2.9	1	ı	1
4	00.0	0.90	0.00	0.78	10.60	3.62	14.22	12.29	ł	. 1	ı
ĽΛ	•			۲.	9.0	9	4	1.7	i	ı	l
o	•			7.	0.6	9	4	H H	ì	1	1
7				9	9.0	9	4.	0.6	1	ı	l
∞				9	9.0	9	4.	0.1	1	1	1
თ	•			å	9.0	ø	t	ø.	ı	1	1
10	00.0		•	Ϋ́	0.6	9	4.	۲.	ı	l'	1
SUB	58.20	8.10	66.30	64.60	95.40	32.62	128.02	101.11	36.51	1.57	0.139
단단				3	9.0	9	4.2	1.		١	ı
12				ι	0.6	9	4.2	ς	ı	ı	l
13	00.00	06.0	06.0	0.50	10.60	3.62	14.22	7.92	- 1	ı	1
1,4	•	E		4.	0.6	Ó	4.2	δ.	ı	ŧ	 I
15				4	9.0	9	4.2	۲.	I,	1	t
16				4.	9.0	9	4.2	∞	I	ı	ì
17	•		•	4.	0.6	9.	4.2	ň	1	ı	l
87	•	•	•	ω,	9.0	9	4.2	4	ı	1	1
6				٣.	9.0	9	4.2	9	ı	ì	l
20	•			٠,	9.0	9	4.2	•	ı	1	ı
TOTAL	58.20	17.10	75.30	80.69	201.39	68.88	270.27	171.91	102.83	2.49	0.186
		***************************************				**************************************					

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(8) Benefit/Cost Calculation

Plan No. : P-5 (2)

Location : Petburi/Rams Vi (Flyover)

Unit = Benefit & Cost : Baht in million Bo (Mil.Baht/Y) = 39.13 Cost (Mil.Baht) = 45.60 Discount Rate = 0.05

Year Gross Gross Const. 0/M Total PV 22.80 0.00 22.80 22.7 80 0.00 22.80			Benefit					
Gonst. 0/M Total P 22.80 0.00 22.80 2							_	•
Const. 0/M Total P 22.80 0.00 22.80 2			Gross		l.			
22.80 0.00 22.80 2	j	T. Save	V.0.C	Total	PV	Benefit	B/C	IRR
22 80 0 0 0 22 80 2	ω		٠	- *		ı		1
	7	•	•		•	1	I,	. 1
0.00 1.20 1.20	0	တိ	. •	σ	'n	1	ı	ı
0.00 1.20 1.20	1.04	29.16	9.97	39.13	33.80	t	i	1
0.00 1.20 1.20	ം	9,	•	φ	7	1	ı	ı
0.00 1.20 1.20	σ,	6	•	Ġ	0	Ţ	ı	1
0.00 1.20 1.20	O	ď	•	σ,	σ	1.	1.	ı
0.00 1.20 1.20	ω.	9	•	9	1	1.	1	1
0.00 1.20 1.20	œ	σ	•	a)	Ś	1	l	1
0.00 1.20 1.20	7	σv		Ö	'n	ı	Ĺ.	1
sub 45.60 9.60 55.20 51.	1.90	233.25	75.27	313.02	240.85	188.94	4.64	0.397
0.00 1.20 1.20	7	9	6	9 1	0.7	-	1	1
0.00 1.20 1.20		9.1	ي	9	2.8	ı	1	. 1
0.00 1.20 1.20	Ó	9	9	9	1.7	1	ı	ı
4 0.00 1.20 1.20 0.	0.64	29.16	9.97	39.13	20.75	l	ı	1
0.00 1.20 1.20	S	9	φ.	<u>о</u>	9.7	1	1	ì
0.00 1.20 1.20	I/I	9.1	Q.	٦. و	8	1	1	I
0.00 1.20 1.20	Ŋ.	9	o,	9	7	ı	1	1
0.00 1.20 1.20	Š	9.1	Q.	<u>о</u>	7.0	1	1	. I
0.00 1.20 1.20	υ	9	O,	9	6.2	1	1	1
0.00 1.20 1.20	7	9.1	σ.	9	5.4	_	ı	1
TOTAL 45.60 21.60 67.20 57	7.87	524.81	179.48	704.29	435.60	377.73	7.53	0.415

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(9) Benefit/Cost Calculation (Road Improvement Plan)

Plan No. : P-6 (1)

Location : Precharat II/Prachachuen

Unit = Benefit & Cost : Baht in million Bo (Mil.Baht/Y) = 5.26 Cost (Mil.Baht) = 24.10 Discount Rate =

		Cost	L.			Benefit					
Vear		Gross				Gross			Net		(
	Const.	ж/о	Total	PΩ	T. Save	V.0.C	Total	₽V	Benefit	B/C	뒦
H		0	-	۰	0	0		٥.	, I	-1	1
7		N	Λ	٠	_	4.	•	O	1	l	1
ന		S	'n	•	1	4.	•	7	ı	1	1
4		V)	ς	•	7	4.		κ	1	ı	ı
Ŋ	0.00	05.0	0.50	0.41	3.78	1.48	5.26	4.32		ļ	ı
9		ູ	'n.	•	. 7	4	•	r-i	ı	ı	ł
<u></u>		S	S		7	1.48	•	9	1	ı	. [
∞		Ŋ	Ŋ	•	<u>.</u> .			r:	ı	ŧ	1
σ.		'n,	3	•	7	1.48		3	ı	ì	· I
10		3	5		7	•	5.26	3	ľ	1	1
SUB	24.10	4.50	28.60	27.65	34.01	13.30	47.31	37.36	9.71	1.35	0.107
근		5.	δ.	3	7	7	2.	.2	1	ı	ı
12		υ)	Ŋ	7	7	1.48	5	0	1	ı	ı
13	00.00	0.50	0.50	0.28	3.78		5.26	2.93	ı	ı	1
14		Ϋ́	3	7.	<u></u>	4.	7	.7	ı	ı	ı
15		ιÜ	2	Ġ	7	1.48	2	9.	ı	1	ı
16		Ŋ	v.	7	۲.	4.	7.	ı,	ı	ı	1
17		'n	'n	4	7	4	7	~₹	1	ı	ì
18		ζ.	S		7	•	7	2	ı	ı 1	1
13		3	Ŋ	7,	۲.	4.	7	1-4		1	1
20	0.00	3	3	.7	·-	7.	.2	۰.	_	-	1
TOTAL	24.10	9.50	33.60	30.14	71.80	28.07	99.87	63.52	33.38	2.11	0.160

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(10) Benefit/Cost Calculation (Road Improvement Plan)

Plan No. : P-7 (3)

Location : Sukhumvit/Rama IV

Unit - Benefit & Cost : Baht in million Bo (Mil.Baht/Y) = 28.67 Cost (Mil.Baht) = 70.40 Discount Rate = 0.05

-													-			<u>.</u>					-				
٠.			IRR		. I	. 1	. 1	1	. 1	1	1	. 1	1	0.207	1	•	ľ	ı	1	1		ι	1	l	0.245
			B/C	1	ı		ı	1	1		ŀ		ı	2.26	*	ł	- 1	. 1	-1	ı	. 1	J	. 1	. 1	3.74
		Net	Benefit		ı	1	ı	1	1	ı	1	ı	1	98.51		ı	1	1	1	1	l	1	•	1	233.74
			₽V			်ပ	4	ัต	2	·i	ં	ď	18.48	176.47	7.6	6.7	15.96	.7	7.7	3.7	ં∺ સ	2.5	6		319.16
			Total			8	8.6	8.6	28.67	8.7	8.7	8.7	8.7	229.35	o o	8	28.67	8	8.6	8.6	8.6	8.6			516.03
	Benefit	Gross	V.0.C	•		. •	. •	•	3.30		•		3.30	26.39	(1)	٠.	3.30	.ო	U.	Ċ.	n	ന	ന	(r)	59.36
			I. Save			υ.	ď,	5.	25,37	5.3			Ŋ	202.96	5.3	5.3	25.37	5.3	5.3	5.3	5.3	5.3	ď	5.3	456.67
			ΡV	C.	3.5	ι.	٣,	7	1.18	ᅼ	۰,	0	φ.	96*22	0	∞	0.84	∞	7	~	ø	ø	6	'n	85.42
	ינ		Total	7	5.2	S	N.	Š	1.50	S.	S	S	5	82.40	•	•	1.50	•	1.50	•	•	٠	•		97.40
-	Cost	Gross	м/о	00.0	•		11.0	•	1.50				• 1	12.00	Ś	ð	1.50	'n	J.	J	Ŋ,	'n	5	5	27.00
			Const.	35.20	Ś		•				•			70.40			0.00								70.40
		Year		H	7	<u>π</u>	7	ιΛ	9	<u></u>	œ	6	10	SUB	딤	12	13	77	7	16	17	18	61	20	TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(11) Benefit/Cost Calculation (Road Improvement Plan)

Plan No. : P-8 (1)

Location : Patburi/Ramkhamheng

Bo (Mil.Baht/Y) = 57.67 Cost (Mil.Baht) = 30.50 Discount Rate = 0.05

		LKK	ı	1	l	1	1	1.	1	1	1	ı.	0.951	i	1	1	ł	ı	1	ı	1	ı	1	0.952
	·	B/C	-	ı	1		1	ı	l,	1	ı	ł	10.90	-	1	1	1	1	ı	ı	ı	1	ı	16.37
	Net	Benefit	1	***	1	1	1	1	ı	1	I	1	372.30	ı	1	1	1	1	ı	1		ı	i	654.37
		₽V		4	2	•	~	45.19	. •	. •	9,	•	409.91	5.4	3.7	2.1	0.5	29.13	7.7	6.4	5.4	3.9	۲,	96*969
		Total		~	7	۲.	~	~	Γ,		۲.		519.03	7.6	7.6	7.6	7.6	57.67	7.6	7.6	7.6	7.6	7.6	1095.73
Benefit	Gross	V.0.C	0	7.0	7.0	7.0	7.0	17.03	7.0	7.0	7.0	17.03	153.26	7	7.0	0	7.0	17.03	7.0	7.0	7.0	7.0	7.0	323.55
		T. Save	00.00	40.64	•	40.64		79.07	•	79.07	•	79.07	365.77		•			40.64	•	•		40.04	40.64	772.18
		PV						0.78					37.61	٠.		•		0.51		•	•	0.42	•	42.59
3.t		Total		•				1.00	•		•	•	39.50	•	•	1.00	•	1.00	•	•	•		•	49.50
Cost	Gross	0/M		•	•			1.00	•	•			9.00	•	•	•	•	1,00	•	•	•		1.00	19.00
	·	Const.						0.0					30.50					0.00						30.50
	Year		Н	7	m	4	Ŋ	9 .	7	ω	o,	10	SUB		12	53	14	15	16	17	18			TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

(12) Benefit/Cost Calculation

Plan No. : P-9 (1)

Location : Rama IV/Kasemrat

Unit = Benefit & Cost : Baht in million Discount Rate = 0.05 Cost (Mil.Baht) = 36.50Bo (Mil.Baht/Y) = 14.82

	سسمع	-				****									مسنو	-						-			
			IRR		1	ı	I	1	ı	ı	. 1	1	1	0.272	,	ı	ł	1	1	1	1	ı	: . (ı	0.299
			B/C		ı	1	ı	1	1		1	i	ı	2.58	1	i	ı	1	1	1	I.	ı	ì	ı	4.09
		Net	Benefit		1	ì	J	ſ	ı	1	•	. 1	ţ	09.49		. 1		ì	1		1	1	1 2 1	1	135.41
			Δd	19	7	13.45	2.8	2.2	1.6	11.06	0.5	0.0	5	105.37	9.10	ွှ	8.25	ω	4.		6.79	4		5.87	179.16
			Total		4	14.82	4	4	4	4	4	4.	4.	133.42	14.82	7	14.82	4	14.82	4	4	7	4.	4.	281.86
	Benefit	Gross	V.0.C		4.51		, v	ŧΩ	'n	2	Ġ	, v	4.51	40.64	3	4.51	4.51			'n		Ŋ			85.79
			T. Save	0	0.3	ö	0.3	10.31	0.3	e. 0	10.31	10.31	10.31	92.78	10.31	10.31	10.31	0.3	10.31		ണ്ട	٤,	,	0.3	195.87
														21,				<u> </u>					<i></i>		
			PV	'n	S	0.54	ů	4	7.	4	4.	4	œ.	40.76	ω.	w	0.33	ų.	0.3	7	4	7	4	4	43.75
***************************************	řŤ		Total	Ŋ	9	09.0	Ġ	9	9	9.	ø	6	9.	41.90	9	9	09.0	9	9	9	ø,	0	φ.	Ŷ	47.90
	Cost	Gross	м/о	0	9	0.60	ø	9	9	9.	9	S	9	5.40	9	9.	09.0	9	9	9	9	9.	ô.	9	11.40
			Const.	ൃ	0	0.00	0	0	0	0	0	0	0.	36.50	. •	•	0.00	•			•	•			36.50
		Year		Н	7	ო	7	Ŋ	٥	7	00	σι	10	SUB	디	12	13	14	-21	16	17	8	6 6	20	TOTAL

Source; JICA Team Estimation Remarks are the same as Appendix 4.4.9 (1)

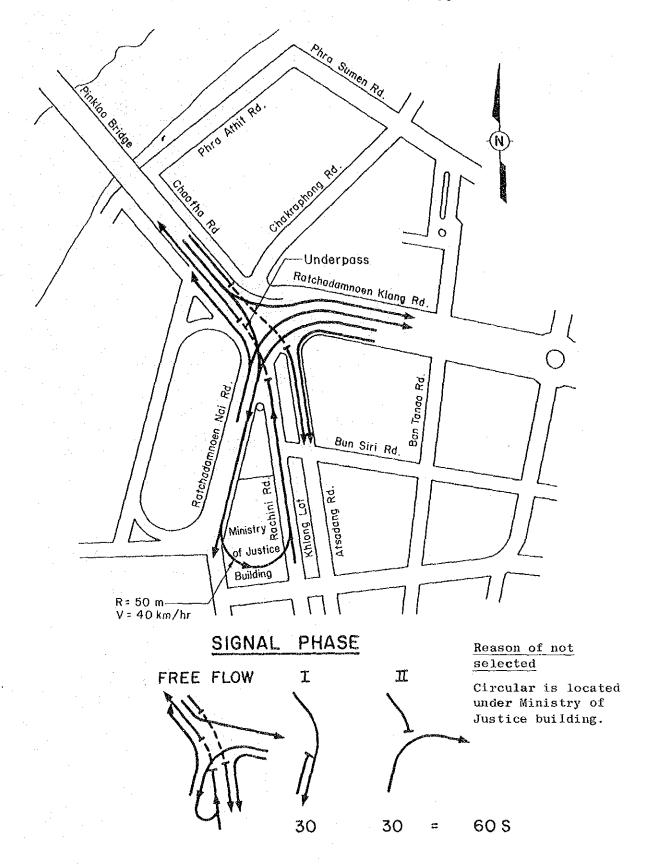


Figure 1 Ratchadamnoen Klang/Ratchadamnoen Nai Rd. Intersection, (Studied but not selected measure)

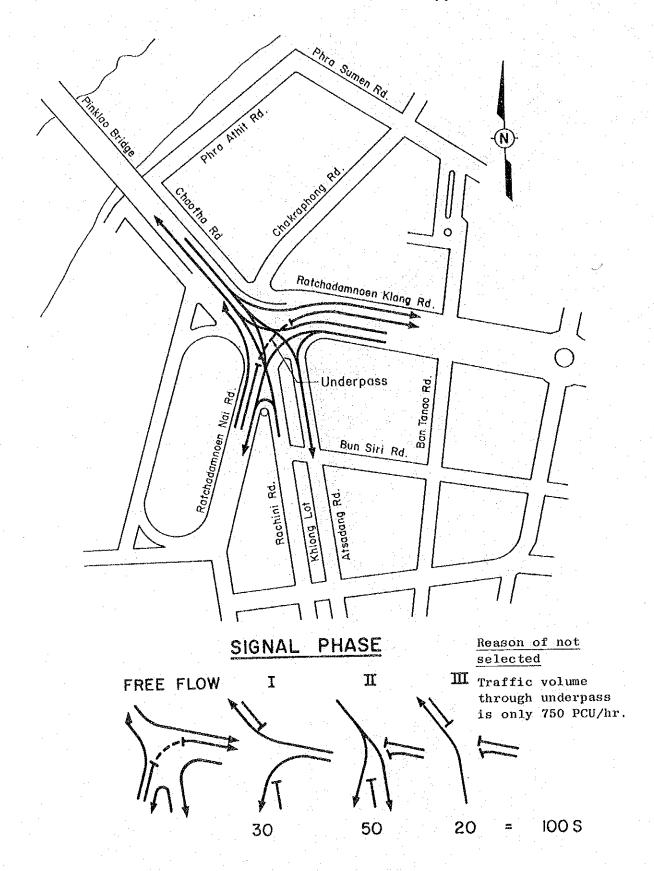


Figure 2 Ratchadamnoen Klang/Ratchadamnoen Nai Rd. Intersection, (Studied but not selected measure)

A-40

