2.5 Proposed Facilities for the Feasibility Study

2.5.1 Drainage Facilities

The drainage system in each polder has been studied and recommended in the previous subsection. In this system, the allocation of discharge capacity of drainage facilities is planned as shown in Fig. C.34. Fig. C.36 shows how drainage facilities are hydraulicly effective on flood protection.

However, the degree of flood protection must be upgraded stage by stage. Existing drainage facilities are divided into three kinds i.e., primary, secondary and tertiary drainage facilities.

According to the results, primary drainage facilities are ranked as high, followed by secondary and tertiary drainage facilities.

Consequently, the implementation schedule is planned to start the construction of primary drainage facilities.

Taking into account of above facts, the drainage facilities as shown in Table C.20 to C.22 are proposed for the Feasibility Study. Improvement works of klongs are classified into five classes as follows.

Class I: new drain with retaining wall

Class II: widening + deepening + construction of retaining wall

Class III: deepening + construction of retaining wall
Class IV: no improvement (IV-2) or deepening (IV-1)

Class V: widening + deepening

The hydraulic studies are conducted on condition that the proposed drainage facilities for the Feasibility Study are constructed. Fig. C.36 shows the contour lines of maximum water level while Fig. C.37 shows the relationship between pump discharge and the storage capacity of klongs and retention areas. According to the results, there are some floodings in the east area of Phra Khanong Polder in which drainage facilities are not proposed.

Table C.20 Proposed Pumping Station

	Pump Capacity	Prop	osed Pump	Capacity (m ³ /s)
Pumping Sta	tion	in M/P	in F/S	Utilized Existing	To be newly Const.
Bang Khen	1. Bang Khen New	} 15	} 15	9	
and	2. Bang Khen Old	j J	J	12	-
Bang Sue	3. Bang Sue	50	36	36	_
Polder	4. Huay Kwang		3	0	3
	Sub-total	65	54	57	-
Phrak-	1. Phra Khanong	90	90	105	
hanong	2. Lao	-	9	0	9
Polder	3. Gig	3	_	-	-
	4. Kacha	6		_	
	5. Campus	-	1	1	
	Sub-total	99	100	106	9
Bang Na	1. Jek	6	3	6	_
Polder	2. Bang Oa	18	12	18	_
	3. Bang Na	21	15	15	-
	4. Bang Nang Chine	9	9	Ð	9
	5. Bang Lai	-	6	0	6
	6. Klet		9	0	9
	Sub-total	54	54	39	24
	Total	216	206	2 0 2	36

Table C.21 Proposed Drains

	Dimension	Proposed	Proposed c	losed drain
Location	n	Length in M/P (in) (m)	Size (m)	Length (m)
Bang Sue Polder	Sena Nichom Rd 59 Ratchada Ditch 44 K. Lat Yao 49-51		B1.9x1.9 2@B2x2 B2x3	2,000 100 70
	Sub-total			2,170
Phrak- hanong	Highway Ditch 113 From Ramkhamkaeng 118		2@B2x2	900
Polder	university to Saen Saep		ø 1.5	200
	Sub-Total			1,100
Bang Na	Cross of Bang-Na-Trad	· · · · · · · · · · · · · · · · · · ·		
	Rd 9		2@B3x3	175
Polder	Cross of High-way 5 Sukhumvit Soi 50 54 Connecting with		\$ 2.5B 2.5x2.5	35 700
	K. Bang 54 Jek and side ditch along Sukhumrit Rd. soi 50		B 2.5x2.5	150
	Sub-total			1,060
	Total	30,000		4,330

	Drainage	Block				Rivisods	verom in R	9			р	J pasonor	Jorks in F/S	Km)	
Polder	Area	No.	Klong	H	II	III	Jystem III	N N	Total	×	ı II	III	III IV	>	Total
Phra Khanong		78-85	K.Phra Khanong	,		3.7	3.9	ı	7.6	ŀ	1	3.7	3.9	ı	7.6
		76 77	K. Tan	ı	4.0	3.2	1	ı	3.6	ţ	0.4	3.2	ı	1	3.6
		63-76	K.Saen Saeb	I	8	3,8	7.4 (1.8+5.6)	I	13.0	1	1.8	. s	1.8	ı	7.4
			Sub-total	i	2.2	10.7	11.3	1	24.2	ı	2.2	10.7	5.7	ı	18.6
Bang Khen	Bang Khen	18-22	K.Bang Khen	1	ı	ı	3.9	6.5	10.4	ı	ı	ı	ı	ı	,
		1-10	K.Lat Phrao	1	i	ı	16.5	ı	16.5	ŧ	1	1	1	ı	,
		11-16	K.Bang Sue	ŧ	2.0	0.9	6.2	1	14.2	ı	8.0	2.80	3,44	07.0	7.44
and	pue	24-28	K.Prem Prachakon	ı	ı	1	7.2	1	7.2	ı	1		ı	1	ı
		45	K.Kasesart	ι	ı	1.75	ı	t	1.75	ı	t	1.75	ı	ı	1,75
		48.49.5(48.49.50 K.Lat Yao	1	ı	2.4	ı	1	2.40	1	1	2.15	i	ı	2,15
Bang Sue	Bang Sue	62	K. Toong	ı	ı	ı	1.35	ı	1.35	ŀ	ŀ	1	ı	ł	J
		57.58	K.Phya Wake	ı	ı	2.00	ı	1	2.00	ı	i	2.00	ı	ı	2.00
		60.61	K.Nam Kaem	1	1	1	2.4	1	2.4	F	ı	ı	ı	ı	,
		51-55	Rachada Ditch	5.20	ı	1	ŀ	ı	5.20	5.20	ı	1	ı	1	5.20
		29-37	H.W.Side Ditch	I	t	ı	5.6	t	5.6	ı	1	ı	1	ı	ı
			Sub-total	5.20	2.0	12.15	3.15	6.5	0.69	5.20	0.80	8.70	3.44	0,40	18.54
Phra Khanong	Huay Kwang	87-89	K.Lat Phrao	l	1	ı	79.5	0.80	5.47	t	Ì	1	4.67	0.80	5.47
		56, 123–125	K.Huay Kwang	1	ı	2.00	ı	4.00	6.00	I	1	1	i	ı	J
		132-134	K.Nasong	1	0.20	3.30	1	ı	3.50	ı	1	1	1	1	ı
		96-06	K.Sam Sen	1	ı	1.10	4.10	1.00	6.20	ı	1	1.10	4.10	1.00	6.20
		129-130	N.W.Side Ditch	ı	ı	1	2.7	1	2.70	1	ı	1	1	ı	ı
		126	K.Chuad Yai	ı	ŀ	1	1	2.00	2.00	1	ı	1	1	ı	1
		127	K.Sam Saen Noi	1	ı	1	ı	2.00	2.00	ı	ł	1	ı	t	1
		118-120	Chao Khun Sing	1	ı	ı	2.00	09.0	2.60	i	ı	1	2.00	09.0	2,60
		139	Yai Soon	į	ı	1	0.40	1.20	1.60	ı	ı	1	0,40	1.20	1.60
		137	Ku-Nam	ı	ŀ	1	0.40	ı	0,40	1	1	1	ı	ı	1
		122	Wat Tuk	1	ı	1.8		1.7	3.5	1	ı	ı	1	ı	1
		140	K. Bang Kapi	ı	ı	ı	1.0	1	1.0	ŧ	ı	1	ì	l	1
			Sub-total	ı	0.20	8.2	15.27	13.3	36.97	ı	ı	1.10-	11.17	3.60	15.87

			Table C.22	Proposed	pasc	Klong	Improvement	lent (2					(Unit;'km)	'km)	
00 €	Drainage	Block	K lone		R	Rivised S	System in F	F/S			l [sed	Works in	F/S	
rorger	Area	No.	Suoty	H	II			۵.	Total	H	II			م	Total
Phra Khanong	K. Chan	99-101	N.N	1	'	1		4.8	4.8		Ī	,	ŀ	0	0 -
		102 103	K.Bang Toei	I	í	1	3.9	ı	3.9	. 1	ı	. 1	0	1	0
		86	K.Bang Chala	1	t	1.3	ŧ	ŧ	1.3	1	I	0	ı	ı	0
		16	K.Phlu	١	1	ī	1	1.0	1.0	ŧ	1	ŧ	1	0	0
			Sub-total	ŀ	1	H.3	3.9	5.8	11.0	1	1	0	0	Ö	0
Phra Khanong	Lat Phrao	107	z.z	I	ı	1.2	1	I	1.2	ŀ	ı	0	1	ı	0
		109	Lat Pla Kao	i	ŧ	ı	ı	1.2	1.2	ı	ı	ı	t	0	0
		108	N.N	I	1	1	I	1.5	1.5	ı	ı	ţ	1	0	0
		110-114	Chan	ı	4.9	2.2	1	ı	7.1	ı	0	0	1	1	0
		115	Sua Noi	ı	ı	1.0	t	ı	1.0	i	ı	0	1	1	0
		116	Song Kla Tiam	l	ı	2.5		1	2.5	ı	ı	0	ŀ	I	0
		104-106	Ta Nang	ı	ı	1.2	4.2	ı	5.4	t	1	0	0	1	0
			Sub-total	ı	6.4	۳ ٠ 8	4.2	2.7	19.9						
Phra Khanong	Ramkha-	144	K.Chit	ı	ı	1.3	1	ı	1.3	ì	ı	1.3	1	ı	1.3
	mhaeng	145	K.Gig	j	ı	0.97	0.57	1	1.54	1	ı	0.97	0.57	ı	1.54
	(Hua Mark North)	148-150	K.Cacha	1	1	1.6	3.50	ı	5.10	1	Ì	1.6	2.8	1	4.4
	•	151-153	R.W Side Ditch	1	ı	4.0	1	ι	4.00	ı	ı	1	0	t	0
			Sub-total	1	1	7.87	4.07	ı	11.94	1	ı	3.87	3.37	ı	7.24
Phra Khanong	Hua Mark	156 157	Sakae	1	ı	2.8	ı	1	2.8	i	1	0	1	ı	0
	(South)	154	Z.Z	1	ı	ı	1	0.5	0.5	1	i	ı	ı	0	0
		155	N.N	ı	ı	1.6	ι	ı	1.6	ł	1	.0	1	ı	0
		161	Hua Mark	1	1	1.0	ı	ı	1.0	ı	1	0	ı	ı	0
			Sub-total	ŀ	1	5,4	t	0.5	5.9	1	ı	0	1	ı	0
Phra Khanong	Pattana Karn	141-143 158-160	N.N	1	ı	ı	3.0	5.5	8.5	ı	ı	1	0	0	0
			Sub-total	ı	1	I	3.0	5.5	8.5	1	1	1	0	0	0
***************************************						İ									1 1 1 1 1 1 1

Polder Draínage Area Bang Na Bang Na	age Block a No. 214 Na 215 208-214 165-171	Klong		Riv		Cvetem in 17/9				À	sed	Liorko in	F/S	
		Siloty				4						7		
			ĭ	II		'	Λ	Total	Н	II	III	IV	Λ	Total
	208-214	K, Bang Ao	,	f	1,0	1.1	ı	2.1	ı	ı	1.6	0.5	1	2.1
	165-171	K.Bang Nang Chine	ı	ſ	3.26	0.60	1	3.86	ı	ı	3.26	9.0	1	3.86
		K.Klet	1.2	ſ	5.0	ı	ı	6.2	1.2	ı	5.0	ı	ı	6.2
	172-177	K. Bang Na	ŀ	0.2	4.0	2.0	ı	6.2	ı	0.2	4.0	2.0	ı	6.2
	190-193	K.Bang Lai	ı	ı	3.0	0.5	1	3,5	ı	1	3.0	0.5	t	3.5
	194-197	K.Wang Bon	ŧ	ſ	2.6	1	ı	2.6	ì	ı	2.6	1	1	2.6
	206	K.Suan Aoi	1	ſ	ŀ	9.0	ı	9.0	ı	1	ı	9.0	i	0.6
	207	K.Kwang Lang	ı	ı	ı	1.0	1	1.0	ŀ	ı	ı	1.0	1	1.0
	178-184	Bang Na-Trad	1	ſ	I	5.95	1	5.95	1	ı	ı	ì	ı	ı
	187	Rd. K_1	ı	r	t	1.35		1.35	l	I	1	1.35	1	1.35
	186	и К2	1	ſ	Ŀ	9.0	ı	9.0	ı	l	t	9.0	1	9.0
	185	" K3	1	1	ı	9.0	ı	9.0	1	1	ı	9.0	t	9.0
	228-230	K.Bangchak(1)	ı	ſ	2.35	ı	ı	2,35	1	ι	2.35	1	t	2.35
	231-232	" (2)	1	ſ	1.75	ı	ı	1.75	1	1	1.75	1	ı	1.75
	225 227	K.Jek	1	ſ	ı	1.58	1	1.58	1	1	1	0	ŀ	0
	219	K.Bang Jek	ı	ı	1	1.0	i	1.0	1	t	ı	ı	ı	0
	220 216-218	N.N (soi50)	ı	ı	ı	2.75	ı	2.75	i	1	ι	ı	ı	0
	162-164	N.N (Branch of K.Klet)	i.	í	ı	3.10	t	3.10	1	I,	I	1	ı	0
	225	N.N (Branch of Bang Na Chine)	I	f	ı	0.6	1	9.0	1	1	1	0	t	0
		Sub-total	1.2	0.2 2	22.96	23.33	t.	47.69	1.2	0.2	23.56	7.75		32.71
Total			6.4	9.5 7	76.68	108.22 3	34.30	235.1	4.9	3.2	47.93	31.43	4.00	92.96

2.5.2 Hydraulic Study for Project Evaluation

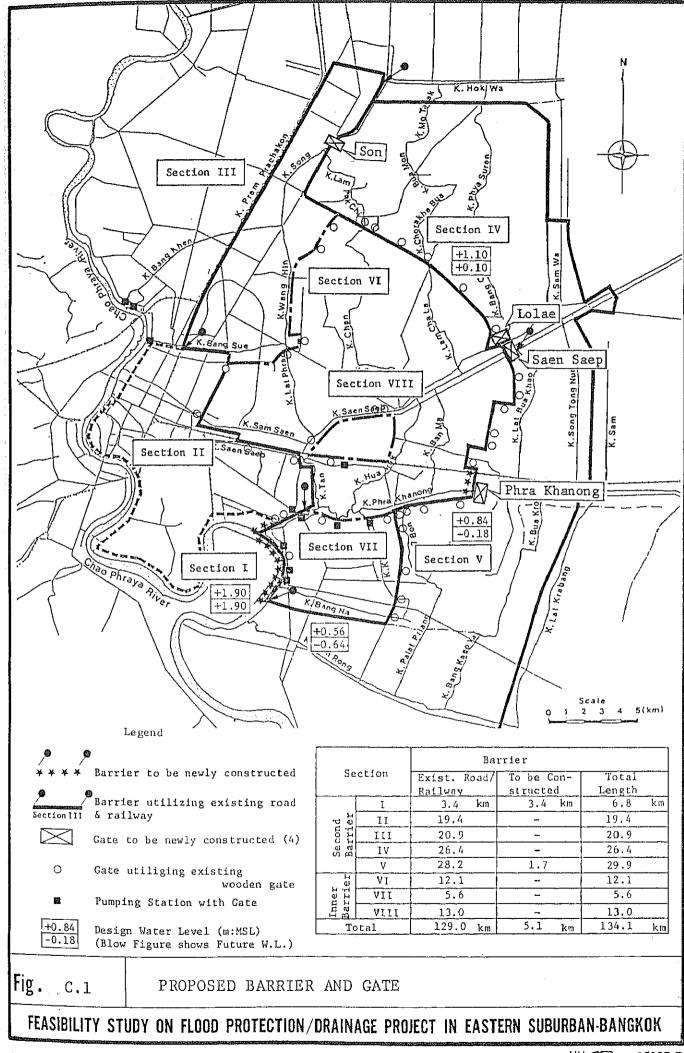
In this subsection, hydraulic effects have been studied based on Table C.23.

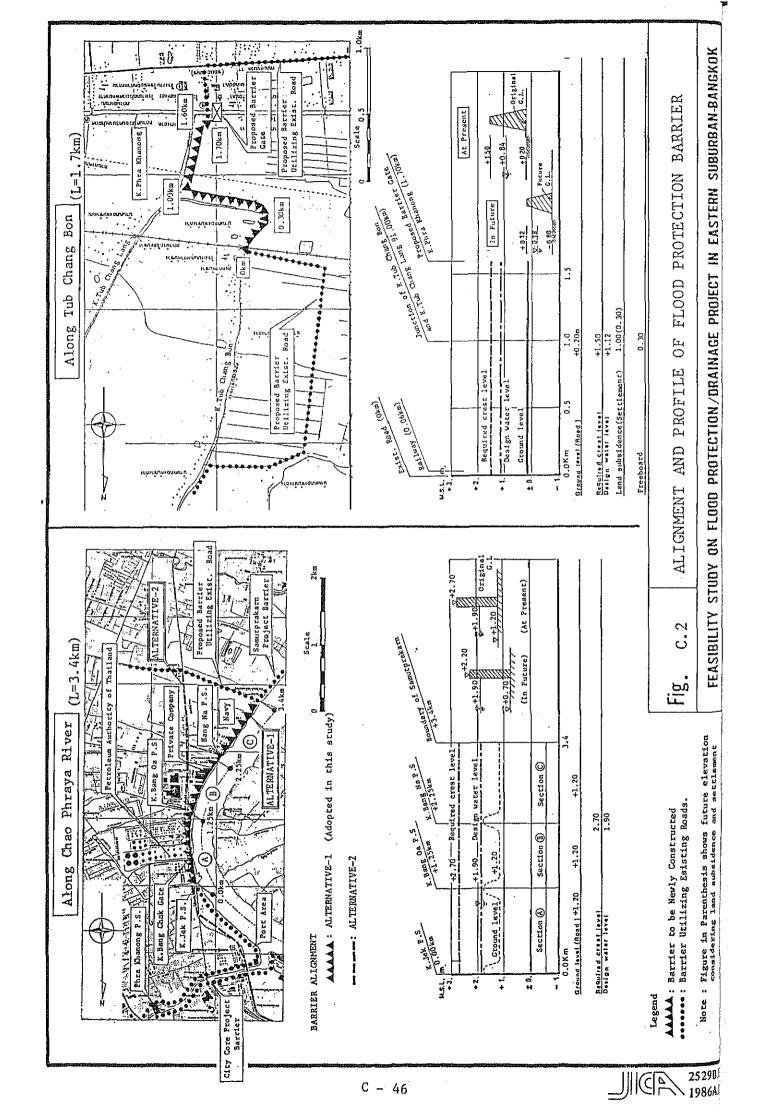
Table C.23 Calculated Case

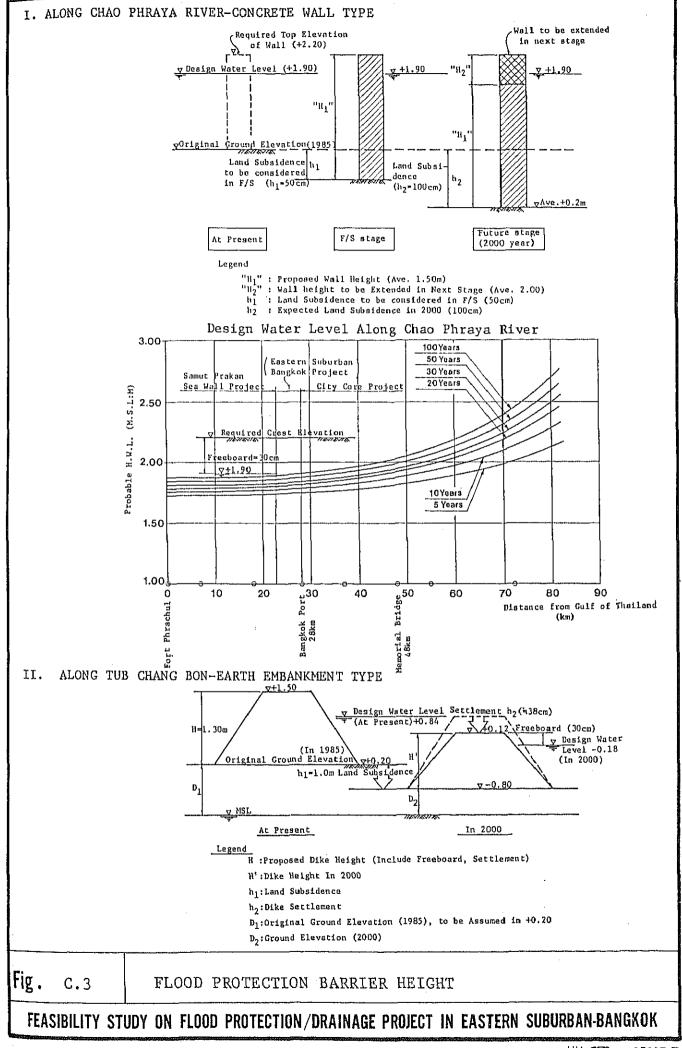
Case	Green Belt	Second Barrier	Land Use	F/S Facilities	Rainfall Frequency
1	0	0	1985	Х	
2	0	0	1985	0	- 2, 5, 10, 20 Years
3	0	O	2000	X	-
4	0	0	2000	0	·

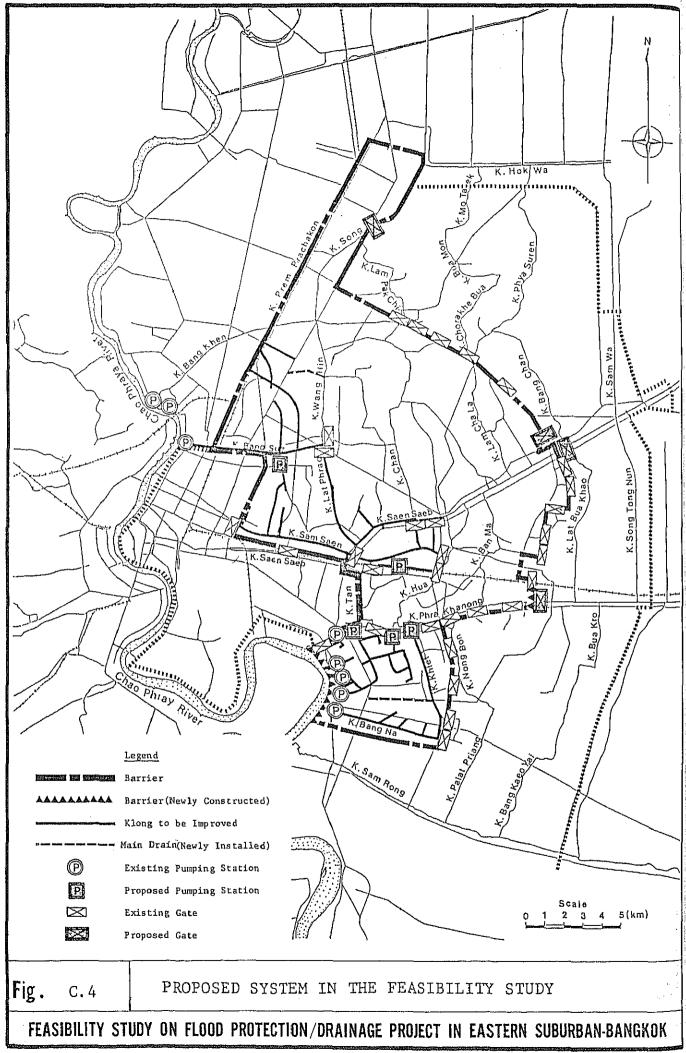
The results are shown in Figs. C.38 to C.45.

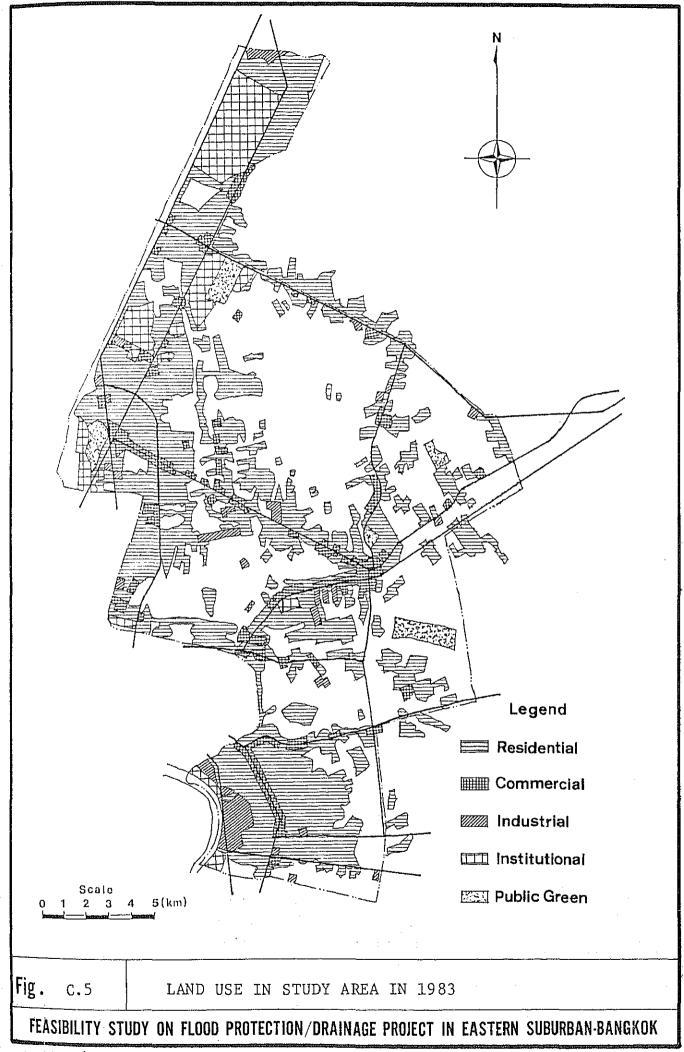
According to the results, flooding situation will be alleviated by about 10 cm around areas where proposed facilities are constructed. On the other hand, it is obvious that the proposed facilities will be little effective on areas where drainage facilities are not proposed in the Feasibility Study.

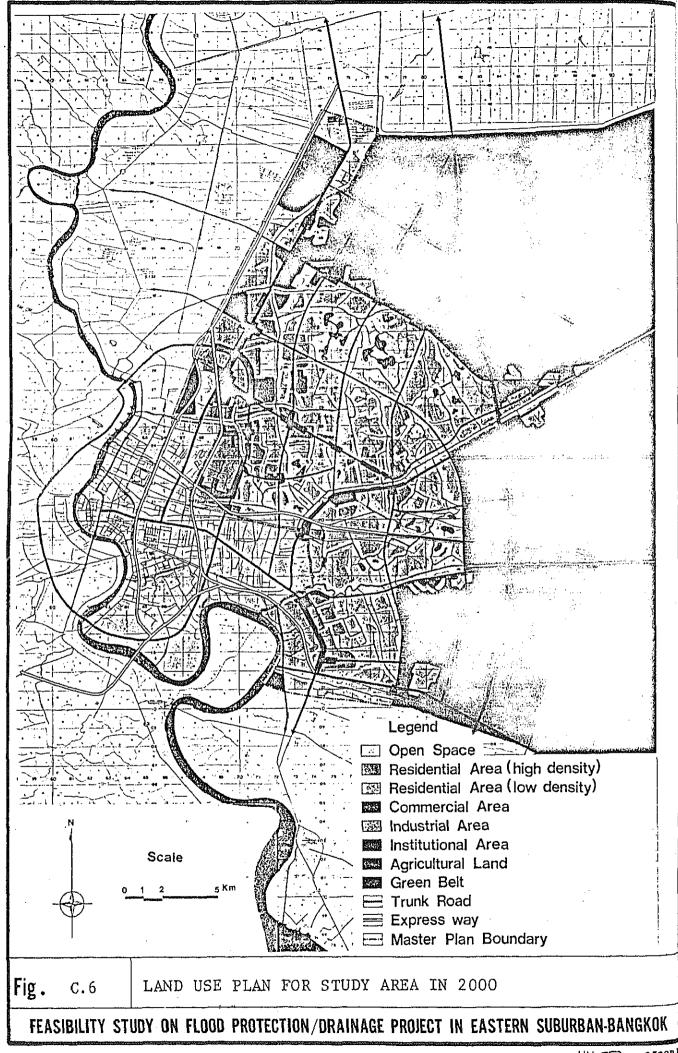


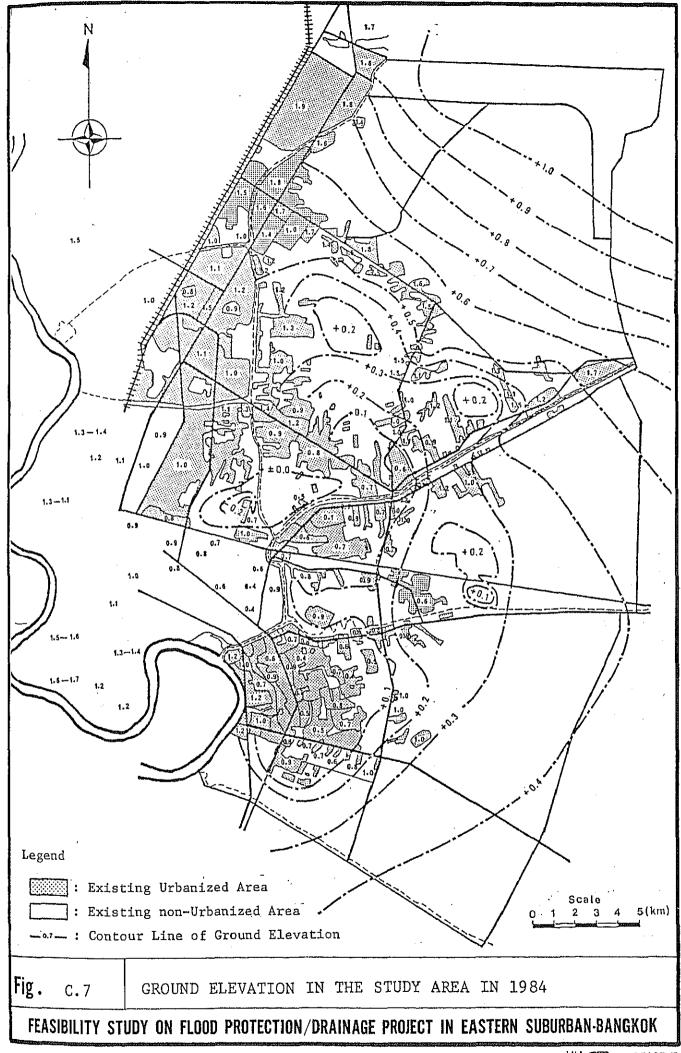


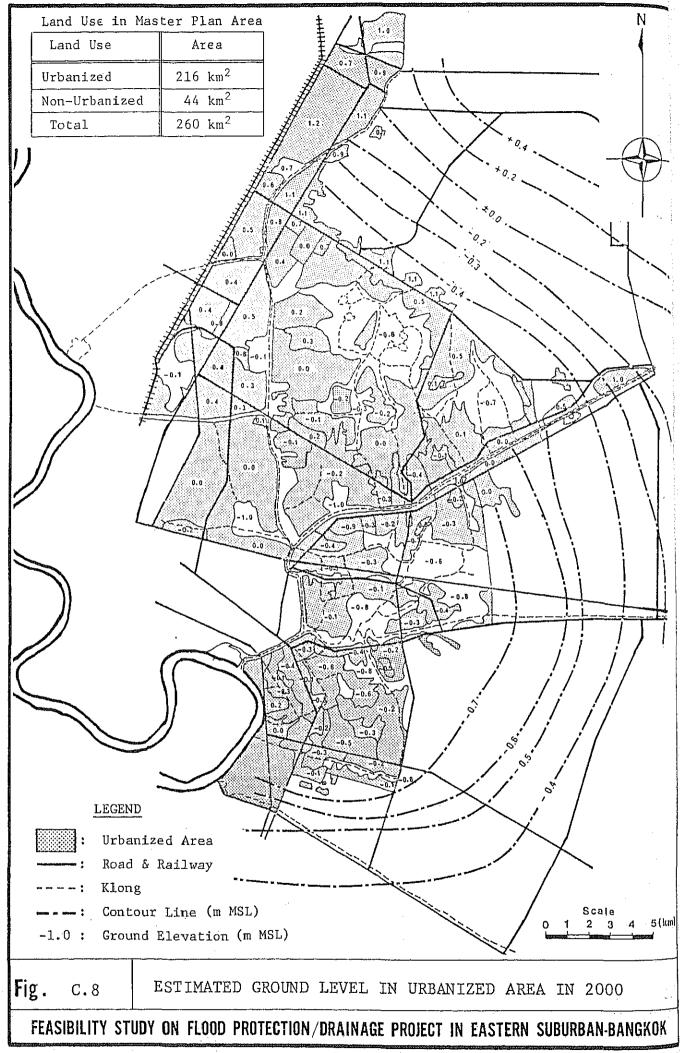


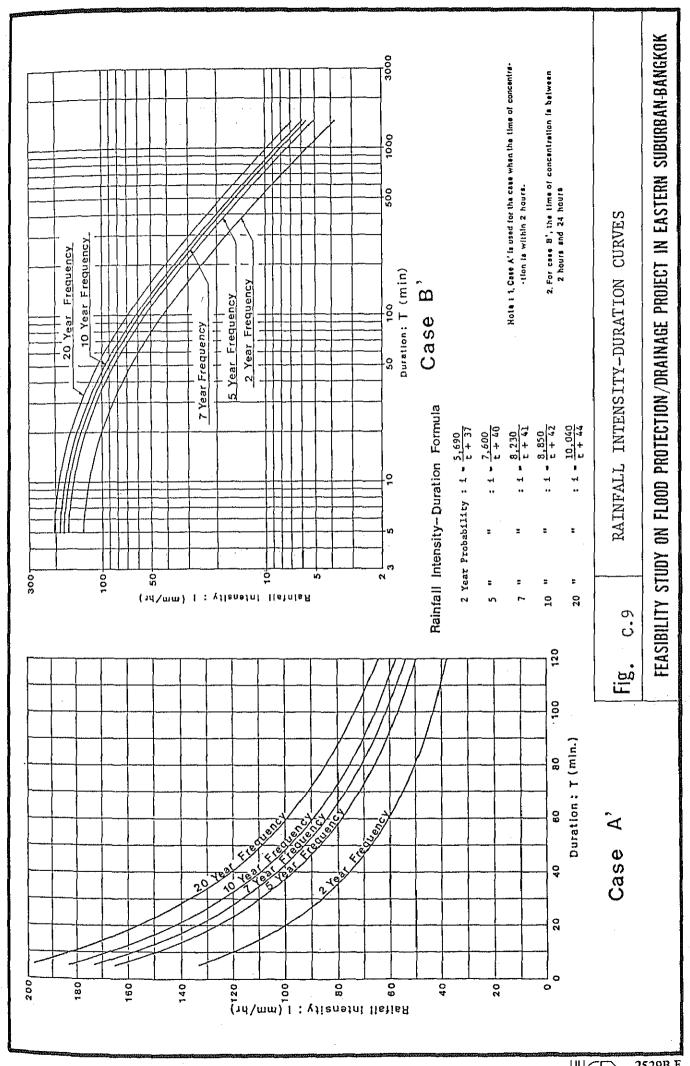








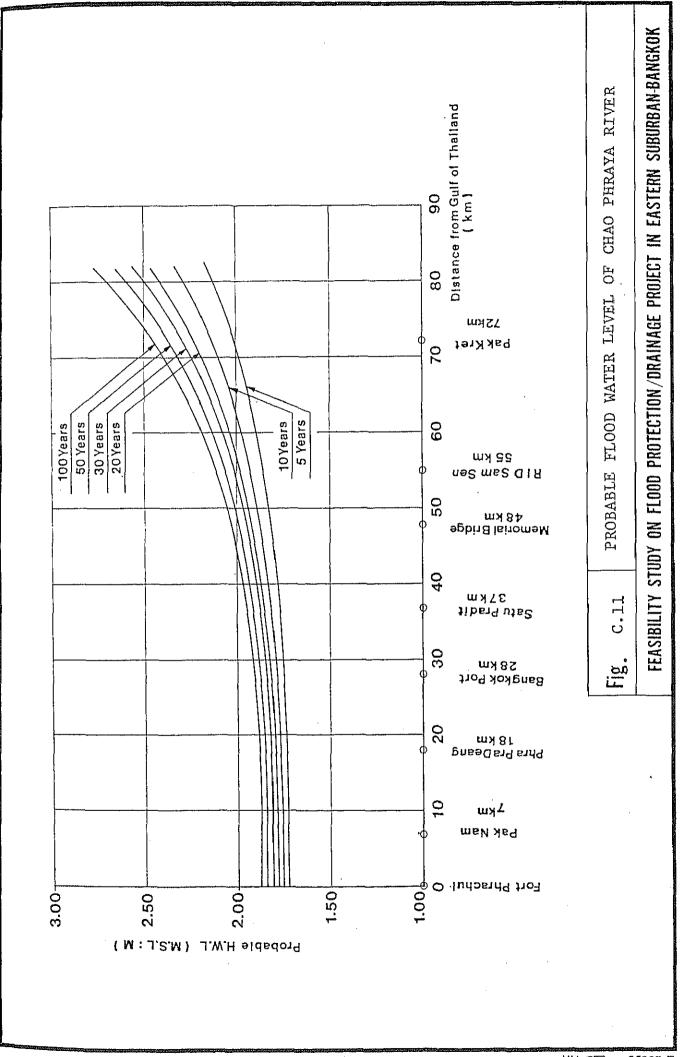


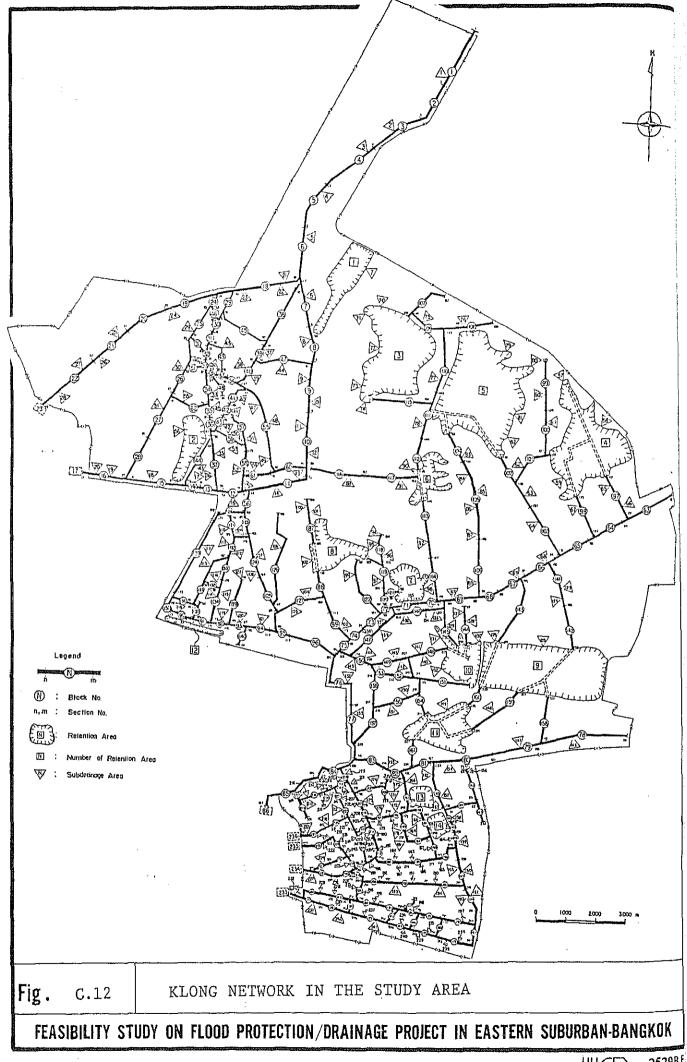


250 - 3months 200 Area in km² 150 100 50 Areal Reduction Factor 0.5

RAINFALL AREAL REDUCTION FACTOR C.10

Fig.





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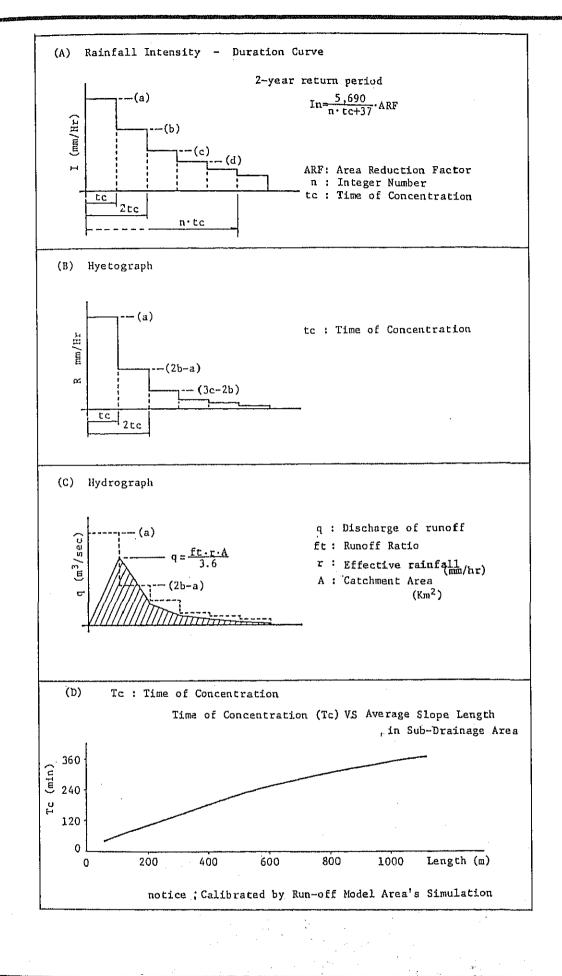


Fig. c.13

TRANSFORMATION OF RAINFALL INTO RUNOFF

Typical Dimensions of Existing Klongs

Ground Elevation : Max. 1.1 meter above MSL

DINENTION OF MODEL AREA : 8.8 square kilometers

: Urbanized Percentage is

Land Use

approximatery 83 %.

Dimension			TYT	Typical Section	
Маше	Length	Widch	Depth	Bocrom Elev.	Remarks
Klong Kacha	3.5 Km	8.0 H	1.5 =	E11.2 m	
Kong Gig	2.0	11.0	 82	E11.8	
Klong Sakae	0.3	5.5	2.3	E11.1	
Klong Lao	9.7	13.0	2.4	£11.5	
None Name	6.0	6.5	1.7	£11.2	
Klong Chic	1.6	t	l	1	
Total	9.0 Km	'	1	1	

Existing Facilities of Flood Protection for Model Area

Remarks	Submersible	Severage Pump		;		=	Booden Gate	1	T	=	r	=	£	
Operation	Kanual			Menual		Manuel	Manual	Manual	Manuel	Manual	Hanual	Hannal	Hanus	
Installed Month/Kear	1982			1981		1982	1881	1981	1981	1981	1981	1961	1981	
Size	\$24" x 2 Unic	\$20" × 1	\$14" x 2	\$14 x 1	\$15" × 1	¢12" × 1	H m W m leaf 2.50x2.00x2	H B W B	H m C m 2.10×2.00	-:	1 m V m 2.10×2.00	30x2.(3.00×2.00	
Name of Klong	K, XACHA			K. CIG		K. CHIT	K. KACHA	K. CIC	K. CHIT	K. C1G	NONE	K. 1.40	K. SAKAE	
Place	1			71		m	-	7	m	7	'n	٥	۲	
Facilities	úen _å						Gane							

1 Fig. (G), (H): Water Level Gauging Station
(A): Rain Gauge Station

LOCATION MAP OF MODEL AREA AND DIMENSIONS OF DRAINAGE FACILITIES FOR MODEL AREA

FEASIBILITY STUDY ON FLOOD PROTECTION/DRAINAGE PROJECT IN EASTERN SUBURBAN-BANGKOK

LOCATION MAP OF MODEL AREA

: Rain Gauge Station

WWW. : Topographic Divida

: Klong

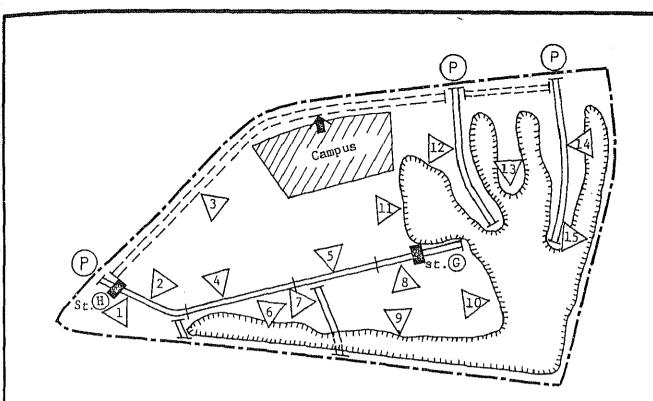
LEGEND

: Pumping Station

(E)

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ï



Legend

Klong

=== Pipe, Box culvert

Pumping Station

Sub-drainage Area

Retention Area

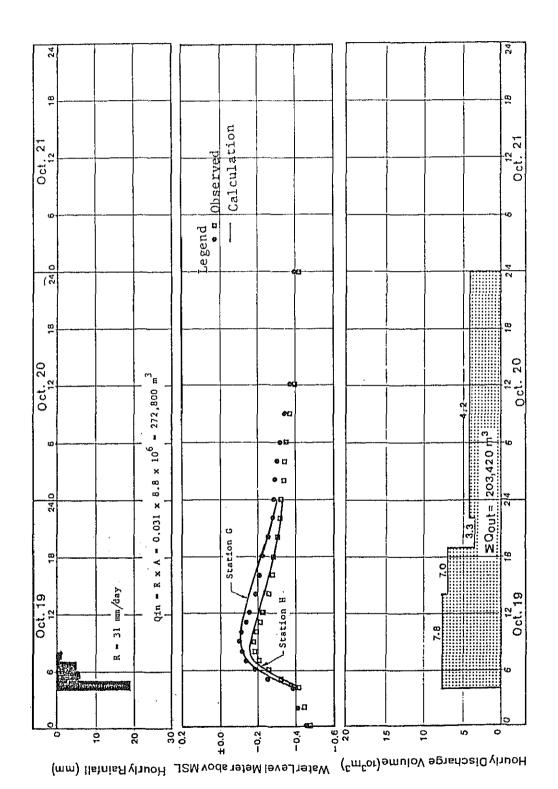
st. G Water Level Gauging Station

No.of Sub- Drainage Area	Area (km ²)
1	0.22
2	0.31
3	0.37
4	0.16
5	0.32
6	0.14
7	0.14
8	0.36
9	0.37
10	0.18
11	0,33
12	0.13
13	0.14
14	0.19
15	0.15
Campus	0.43
Retention Area	4.56
Total	9.00

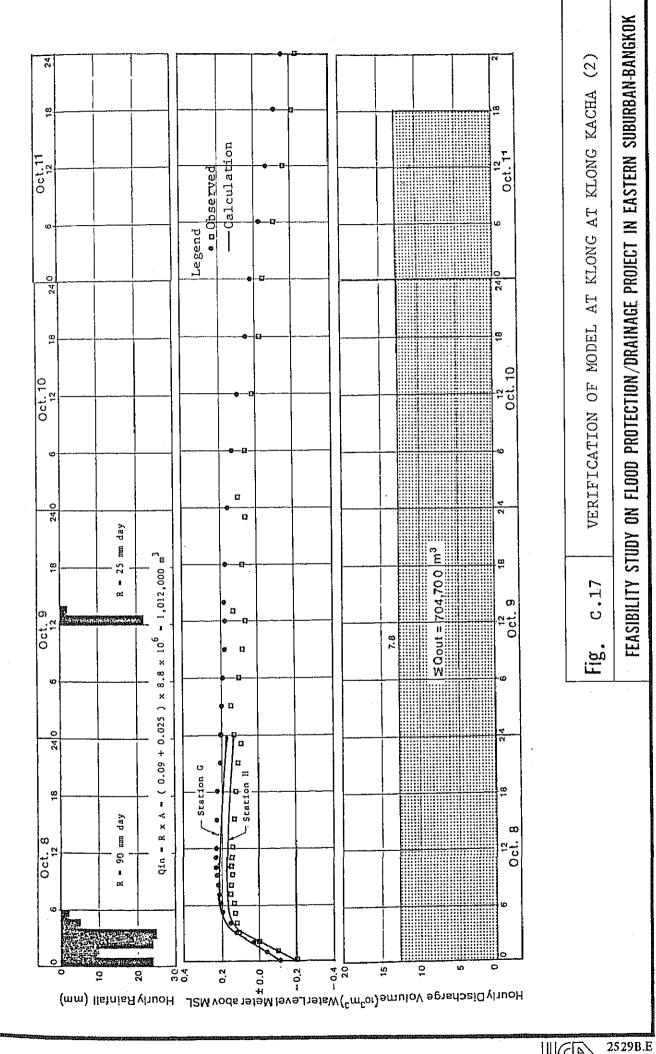
Fig. c.15

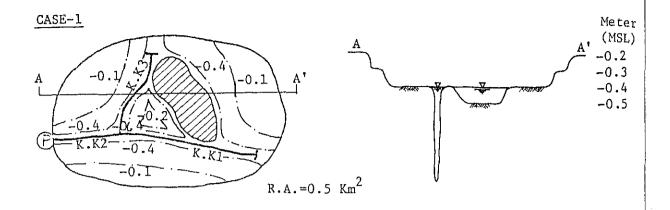
SCHEMATIC MODEL OF RUN OFF MODEL AREA

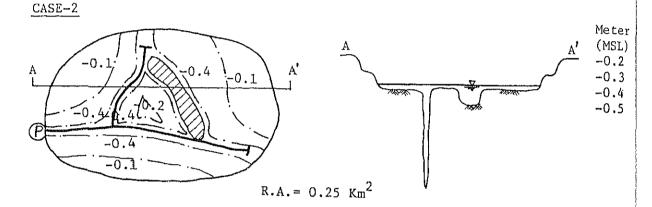
Fig.

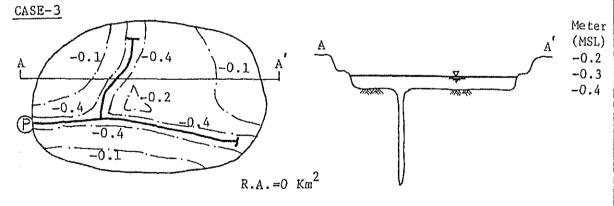


2529B^j 1986A^j









Legend:

: Retention Area (R.A.)

Dimension of Klongs

Name of Klong	Length (Km)	х	Width (m)
K. K1	0.9	х	12.0
K. K2	1.0	х	15.0
К. КЗ	0.8	х	11.0

Fig. c.18

EFFECT OF RETENTION AREA FOR ALLEVIATING FLOODING

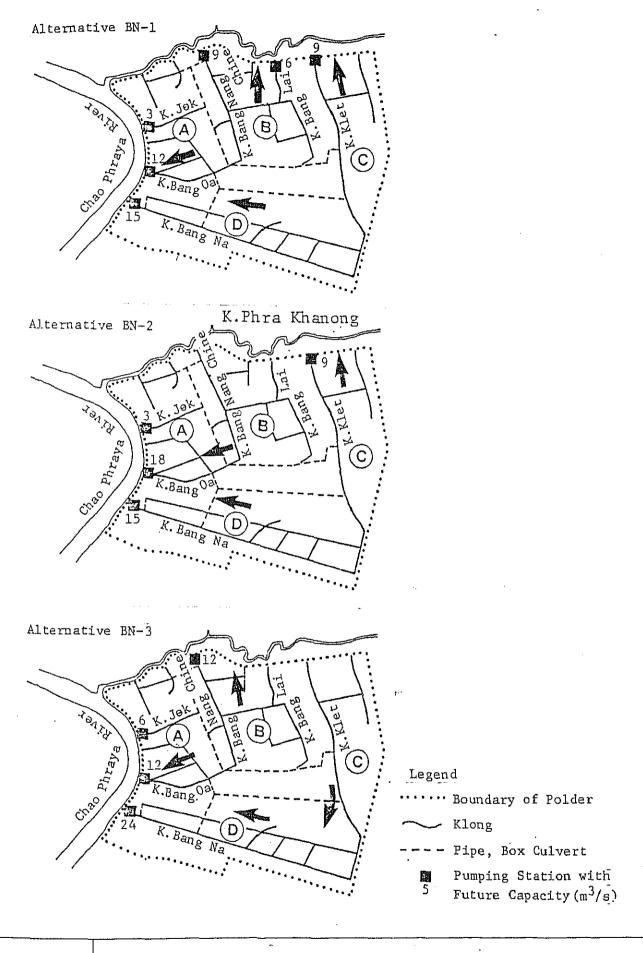
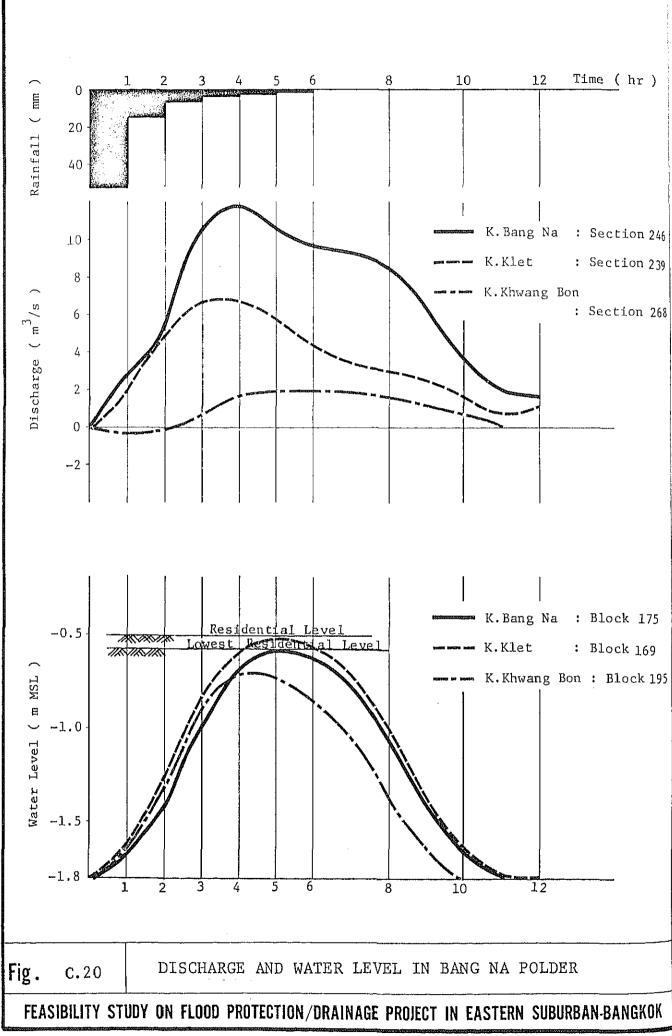
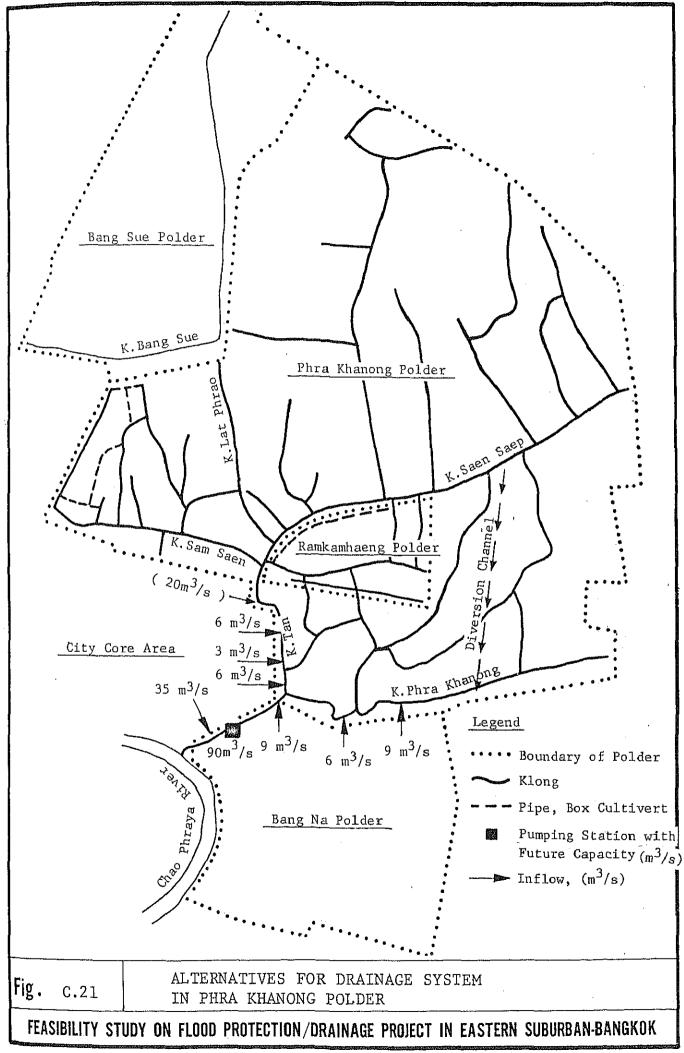
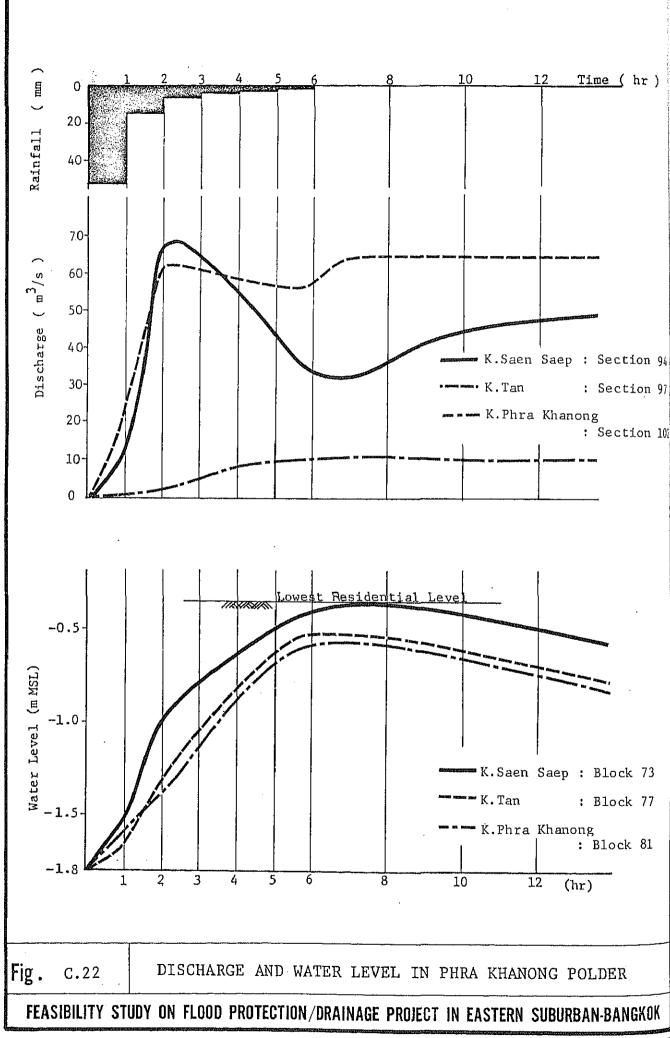
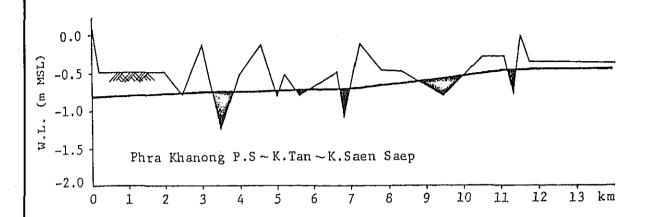


Fig. C.19 ALTERNATIVES FOR DRAINAGE SYSTEM IN BANG NA POLDER









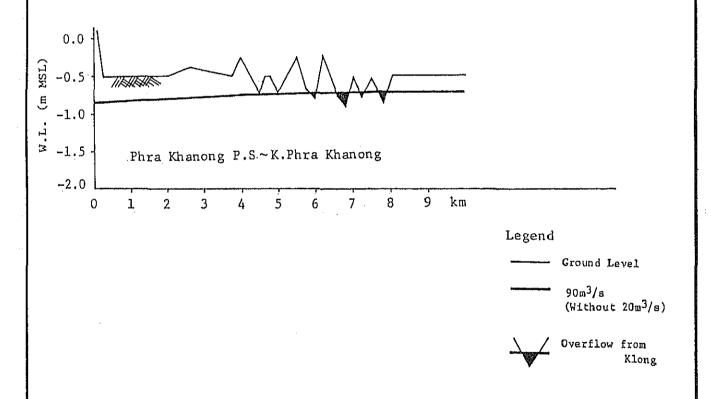
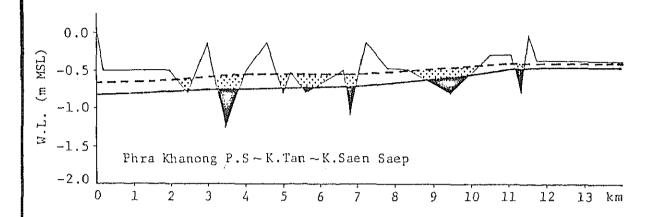
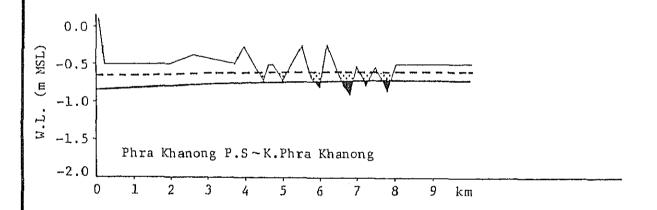


Fig. C.23 PROFILE OF MAXIMUM WATER LEVEL(PHRA KHANONG P.S; 90m 3/S)





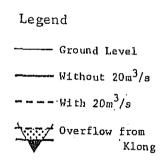
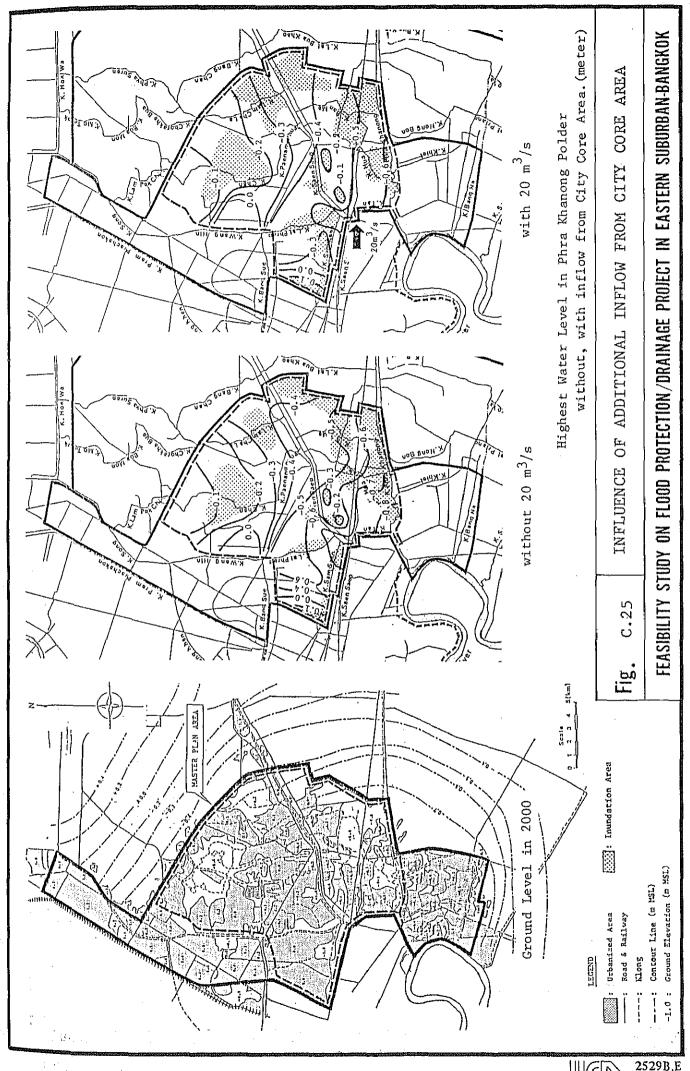
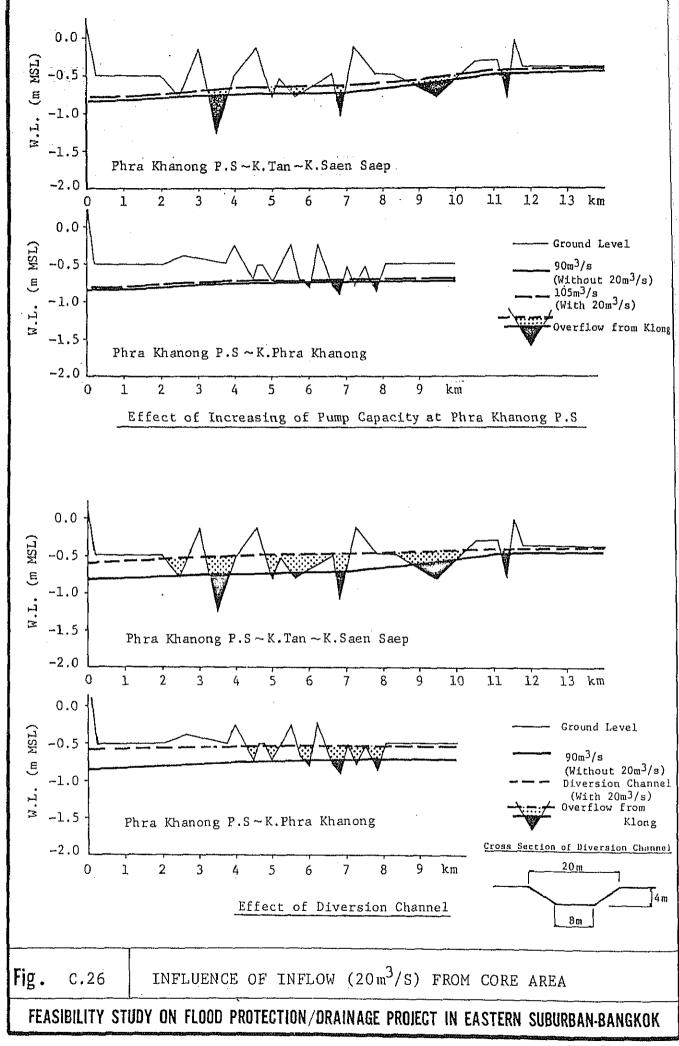
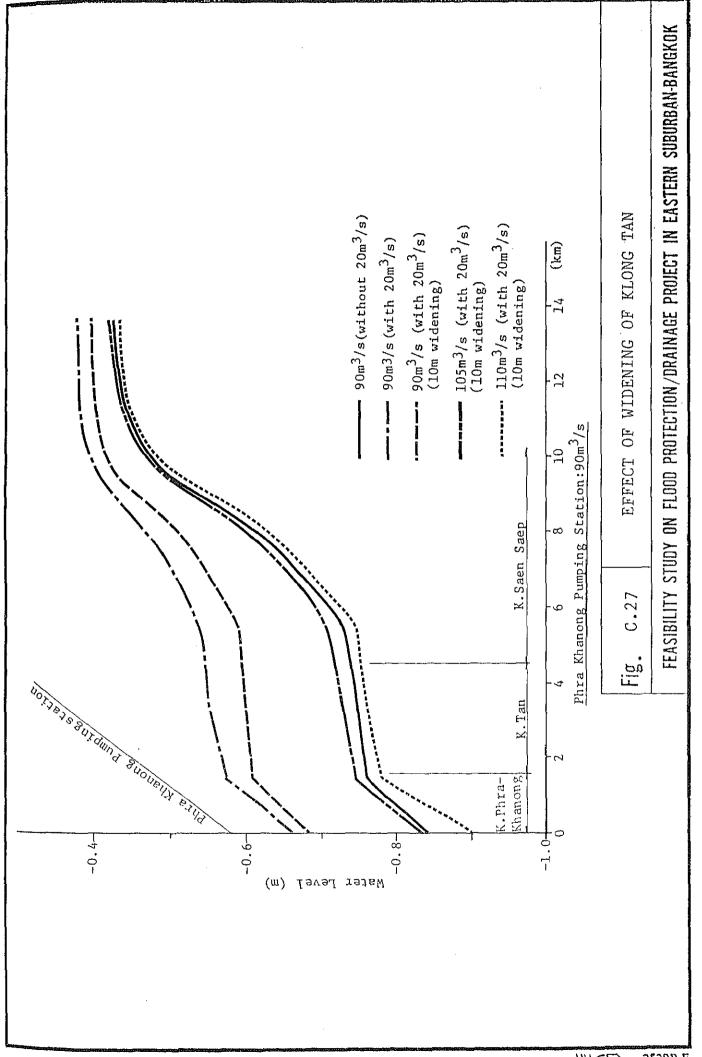


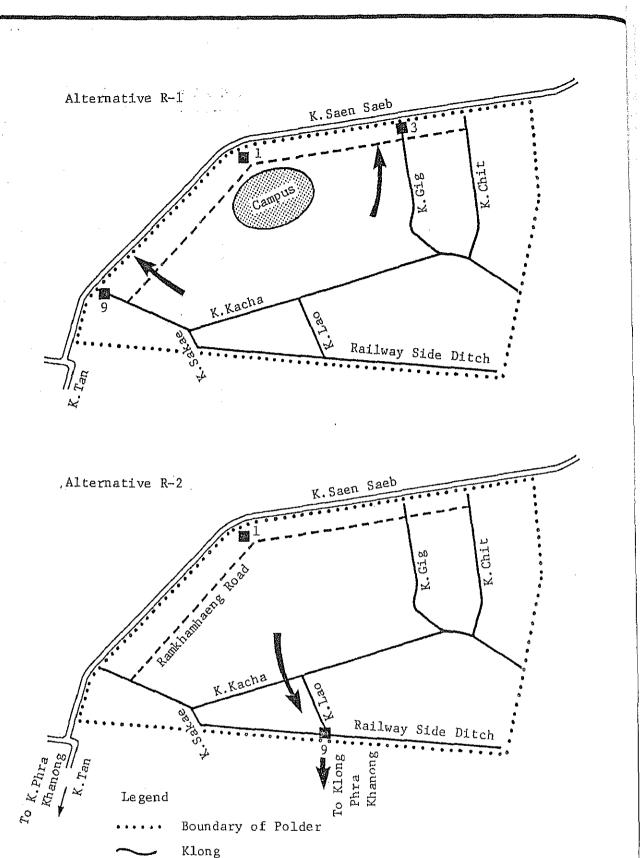
Fig. C.24

INFLUENCE OF INFLOW (20m³/S) FROM CORE AREA IN KLONGS PHRA KHANONG, TAN AND SAEN SAEP







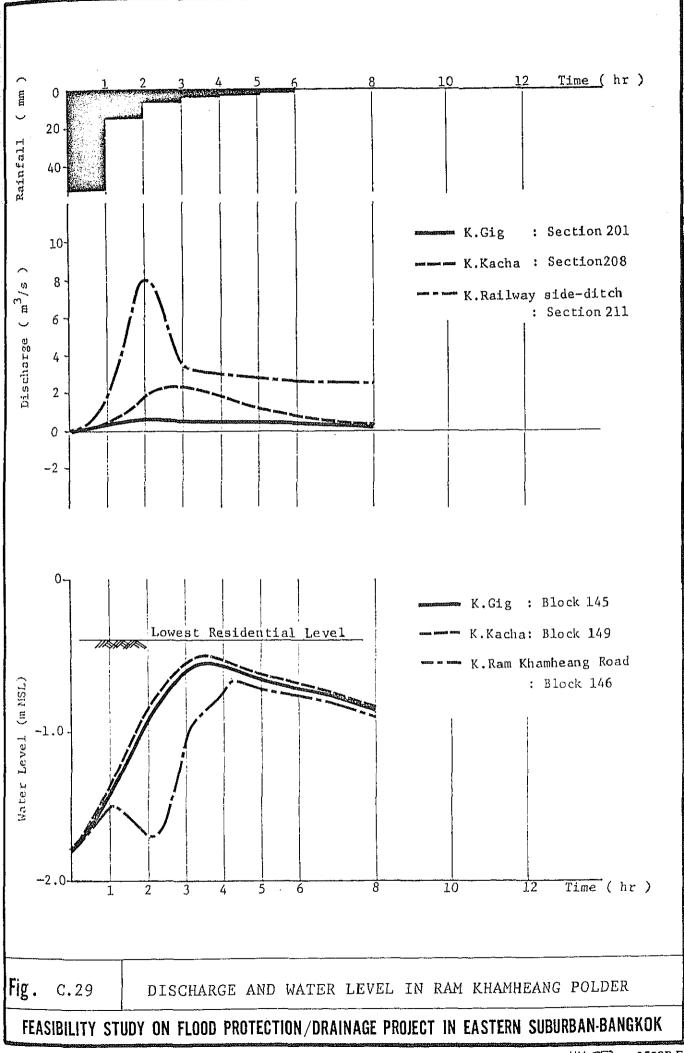


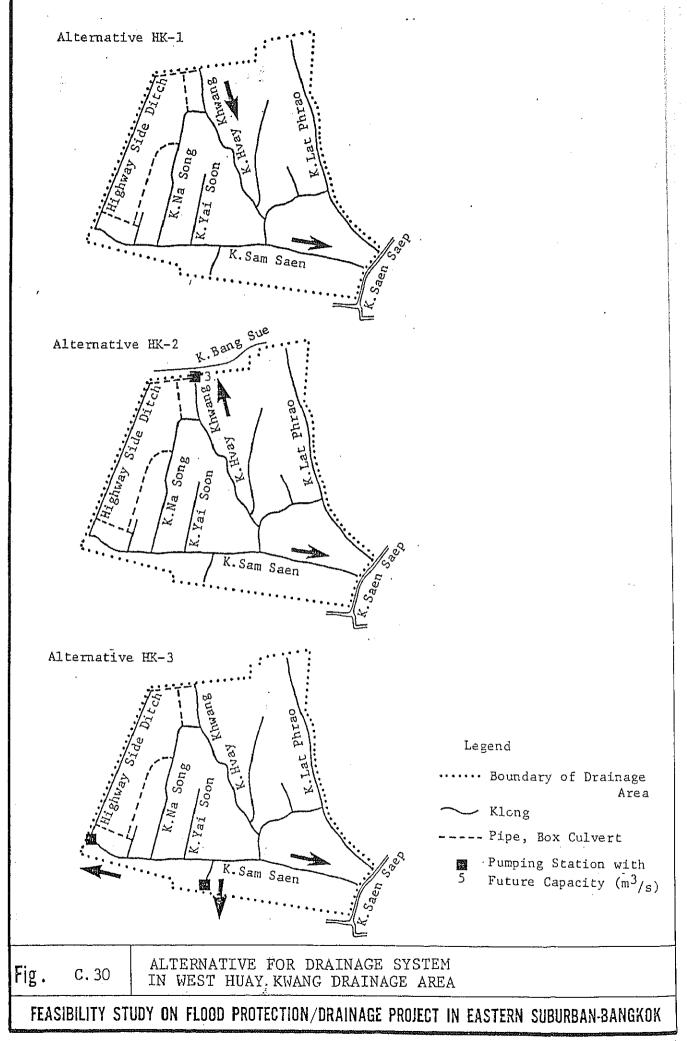
Pipe, Box Culvert

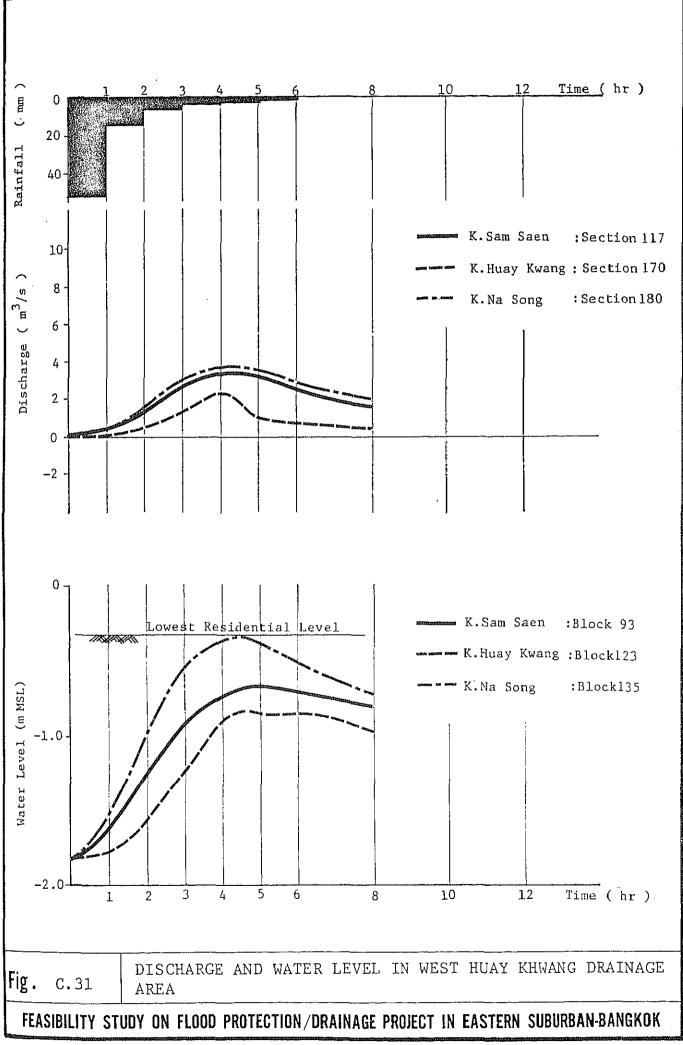
Pumping Station with Future Capacity, (m^3/s)

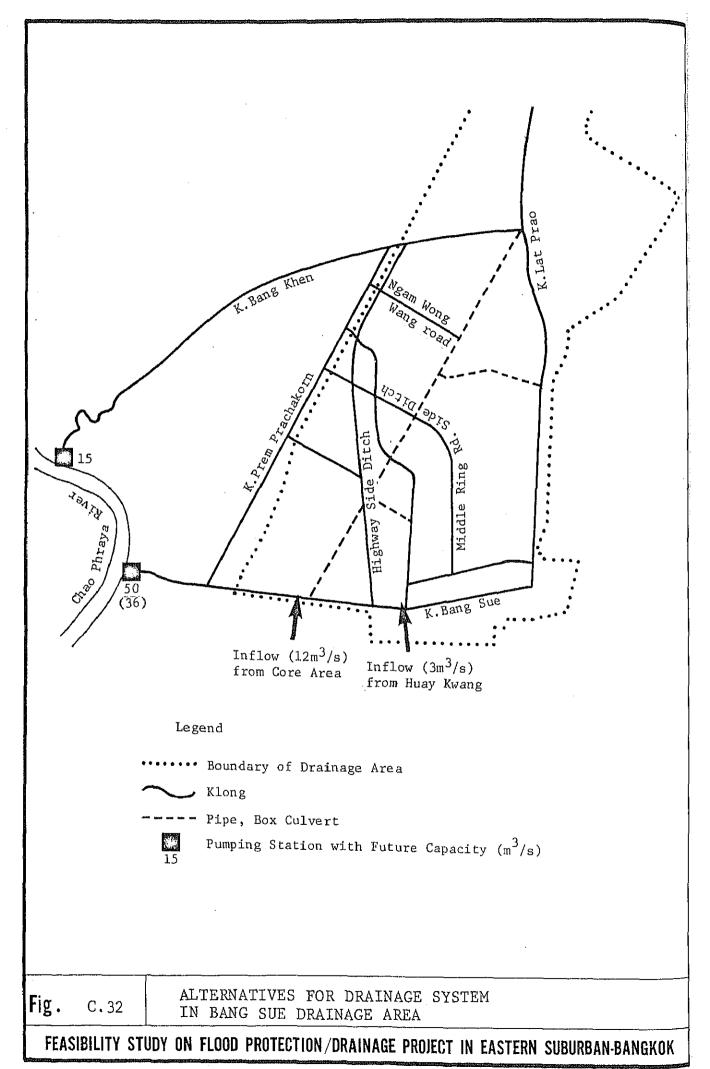
Fig. c.28

ALTERNATIVES FOR DRAINAGE SYSTEM IN RAMKHAMHAENG POLDER

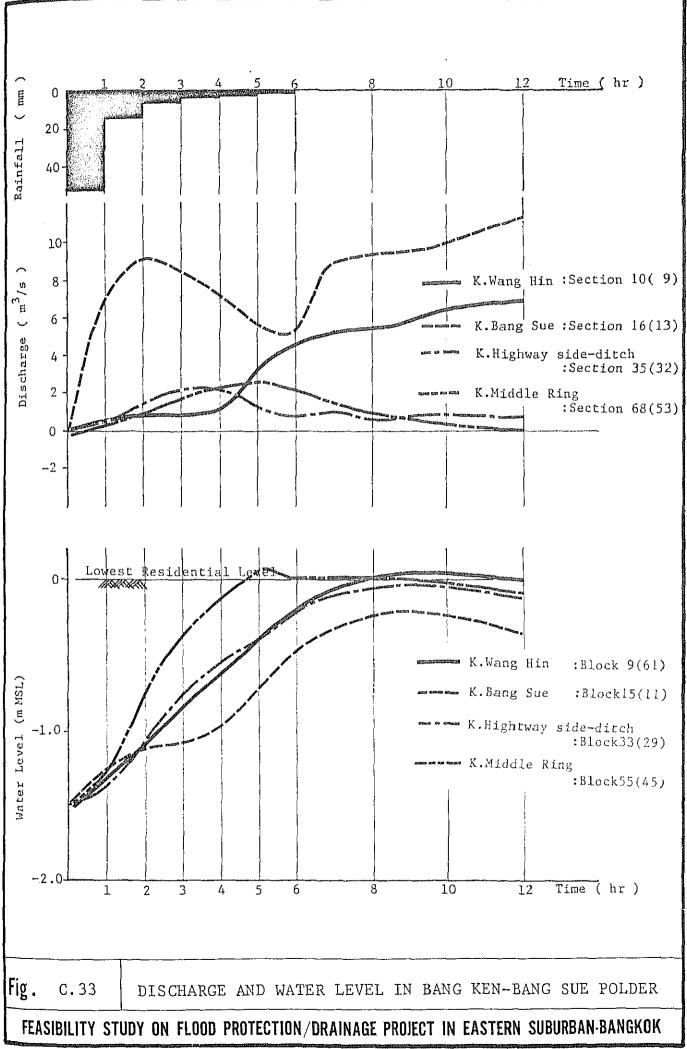


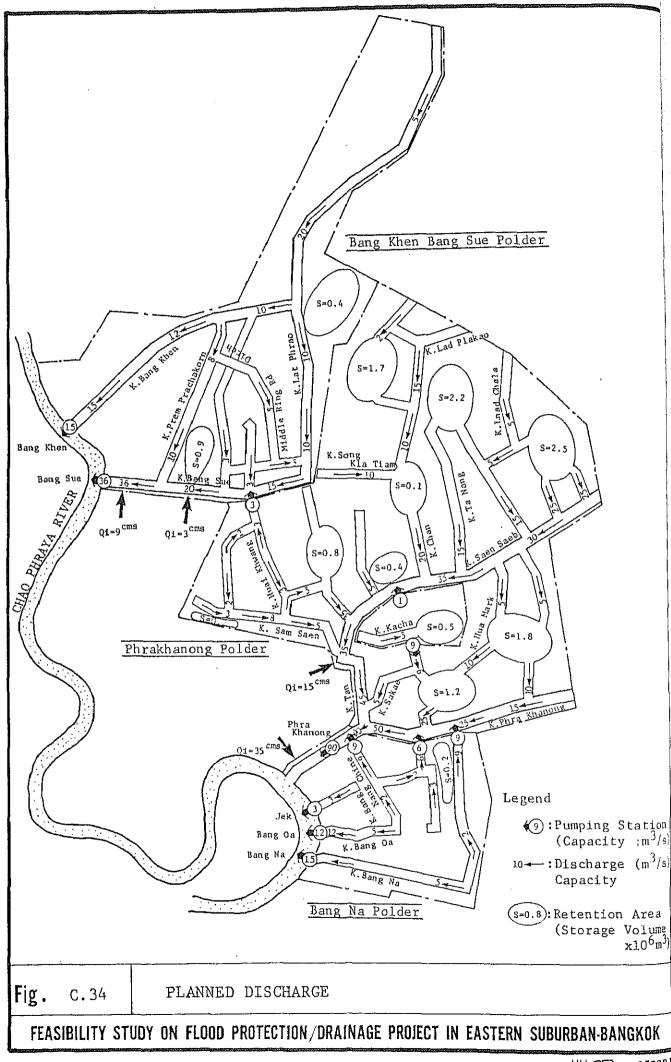


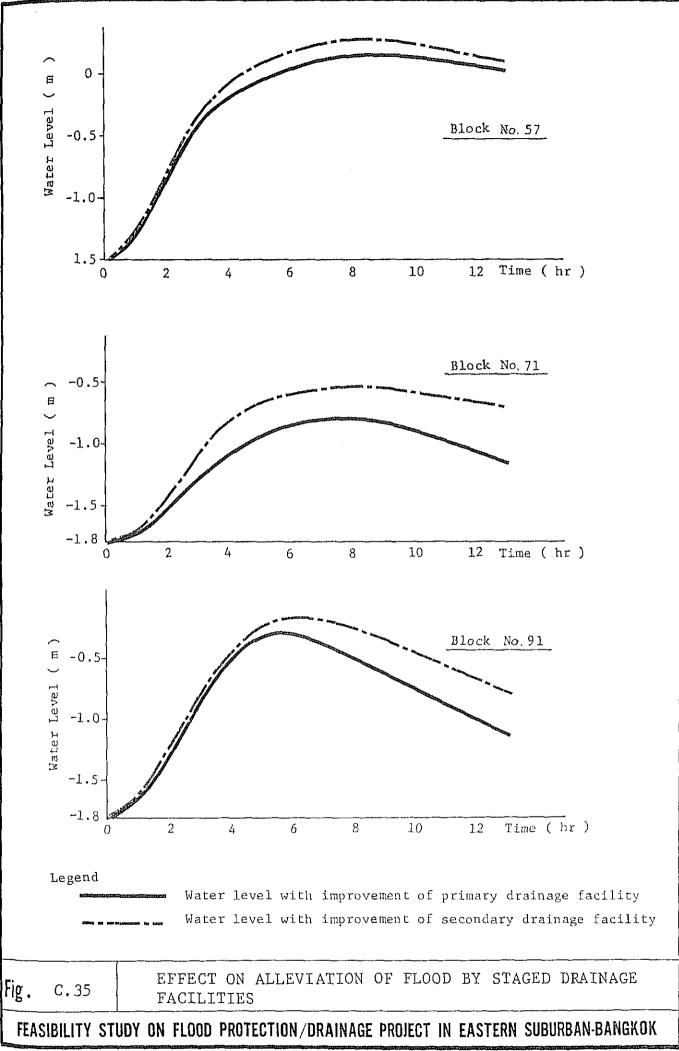


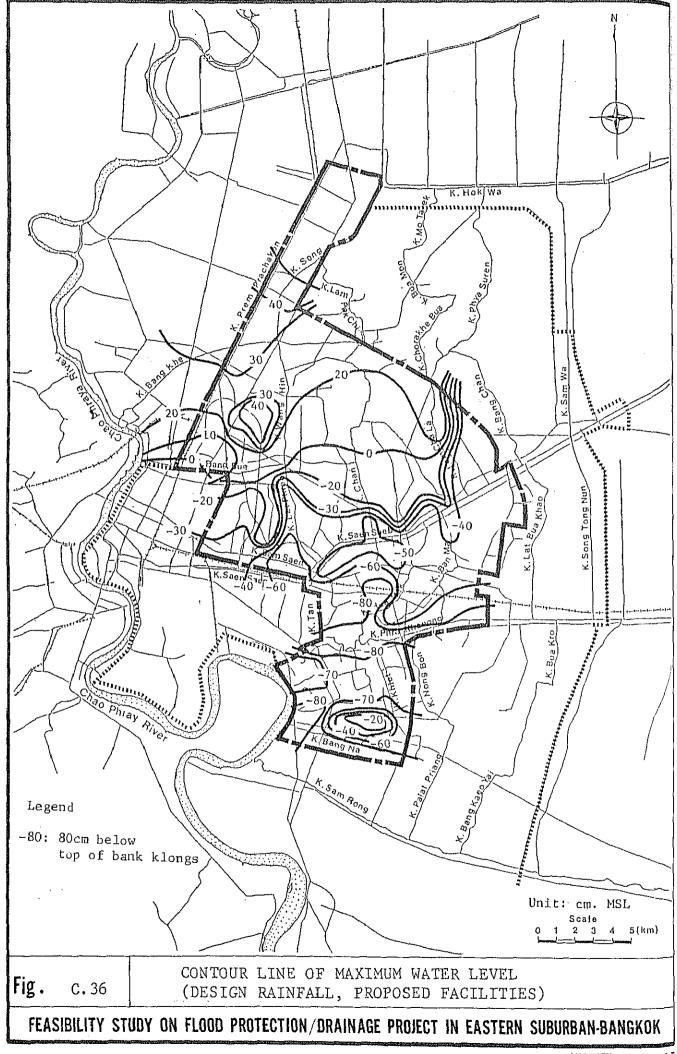


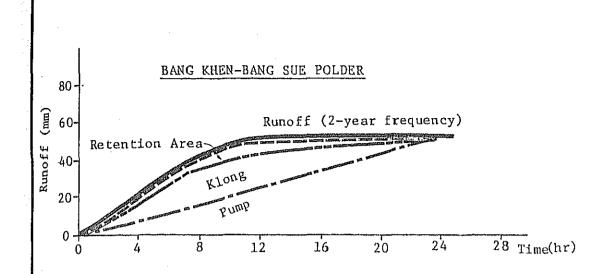
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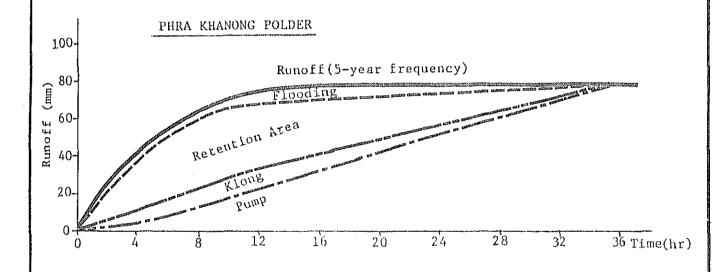












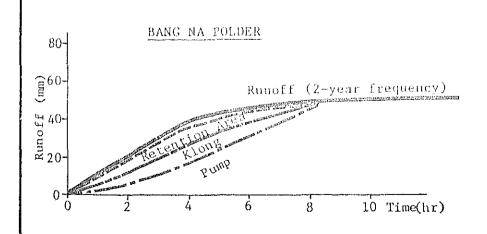
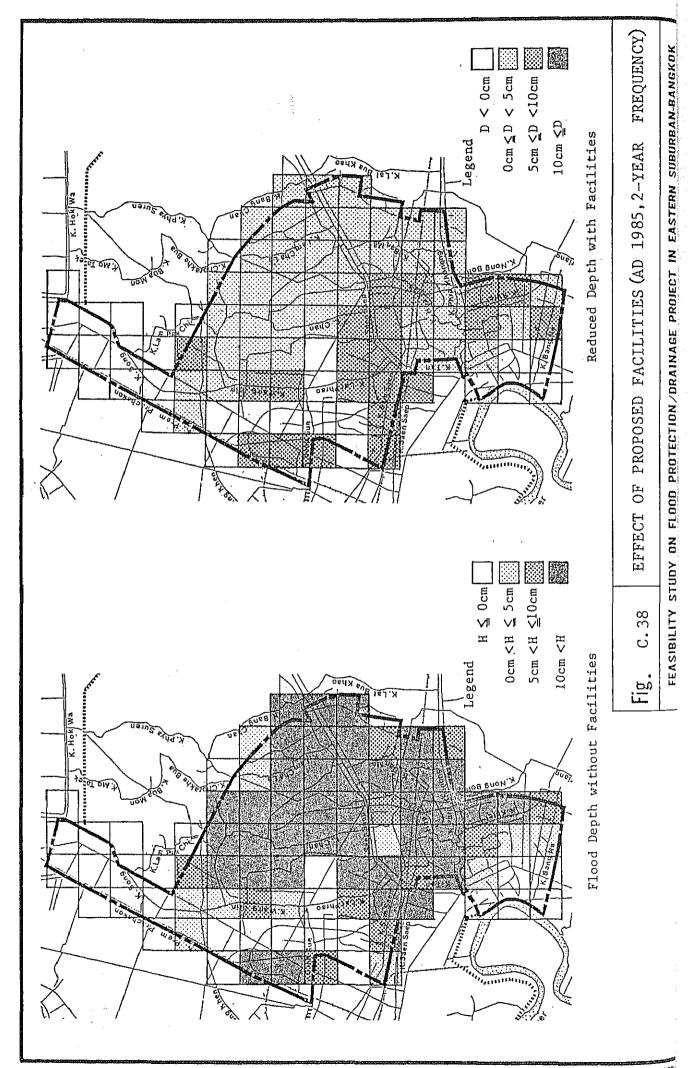
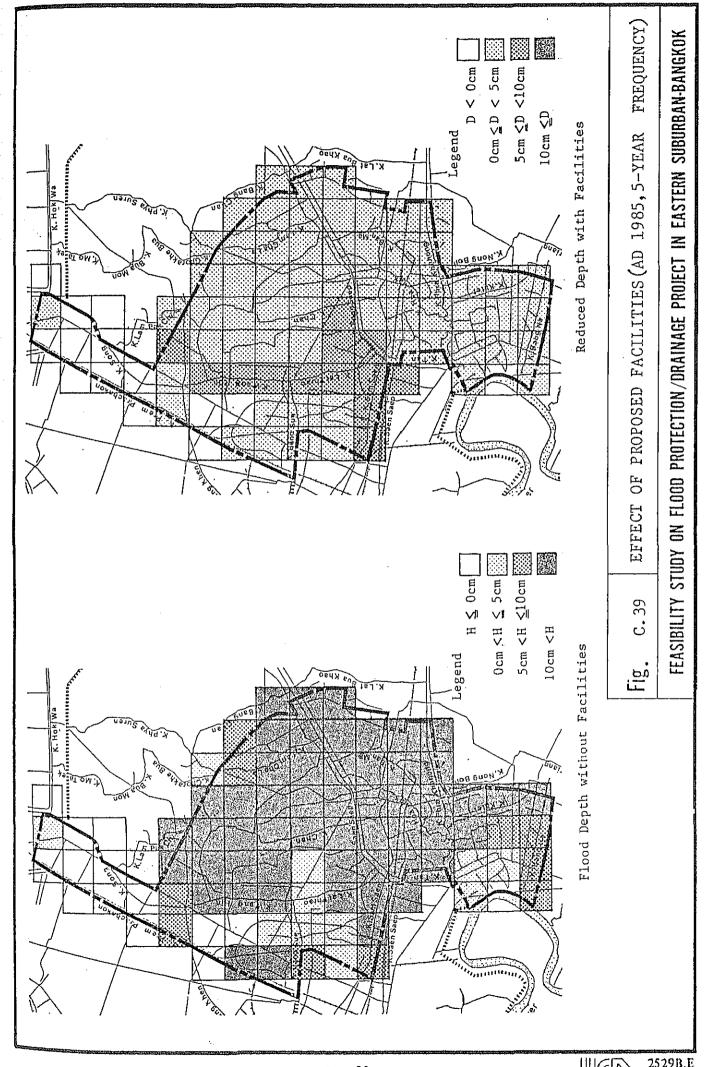
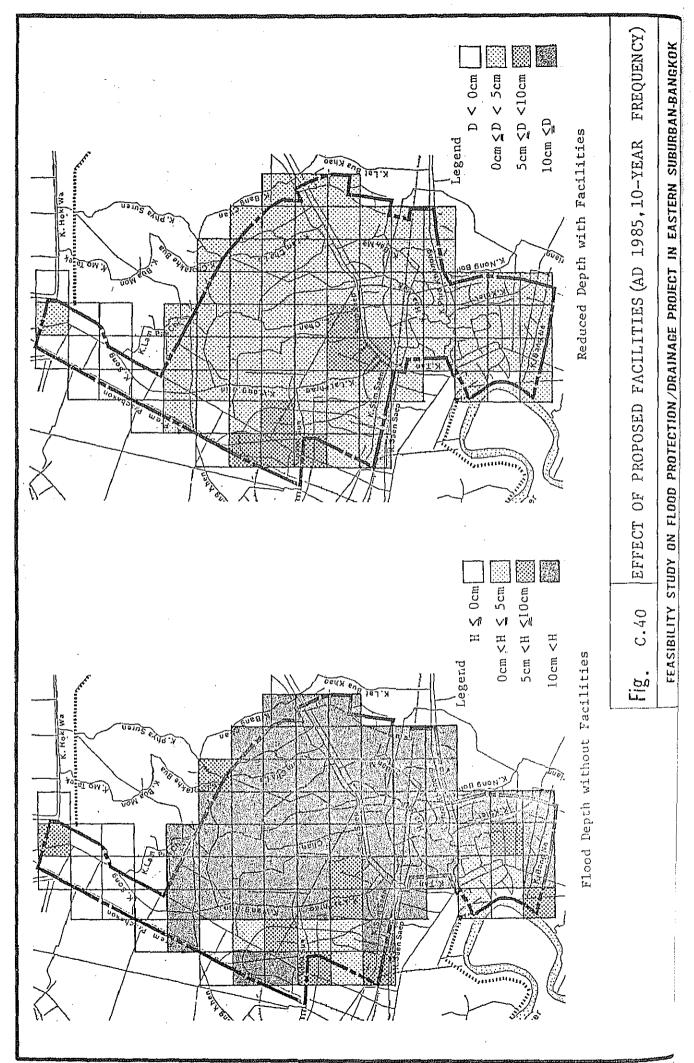


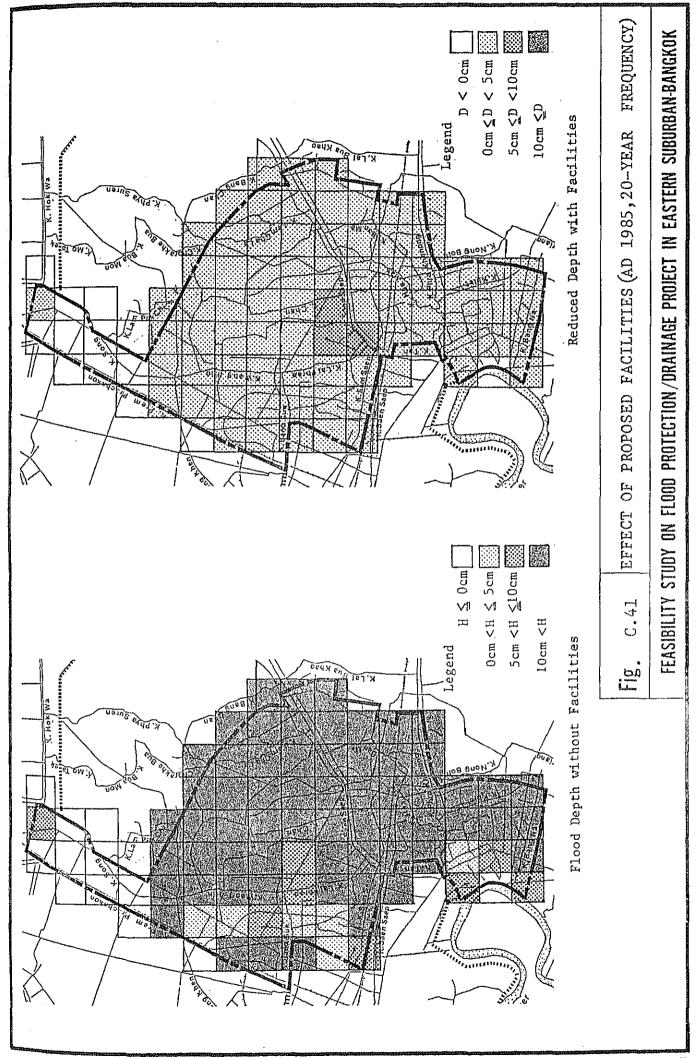
Fig. C.37 RELATIONSHIP BETWEEN PUMP DISCHARGE AND STORAGE CAPACITY

FEASIBILITY STUDY ON FLOOD PROTECTION/DRAINAGE PROJECT IN EASTERN SUBURBAN-BANGKOK

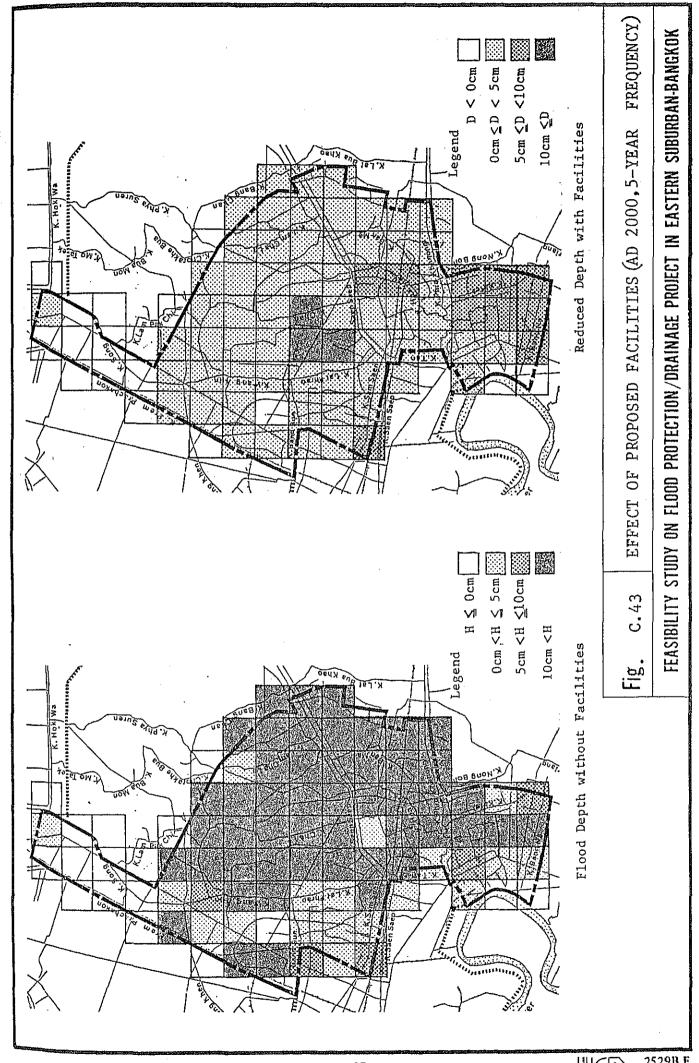


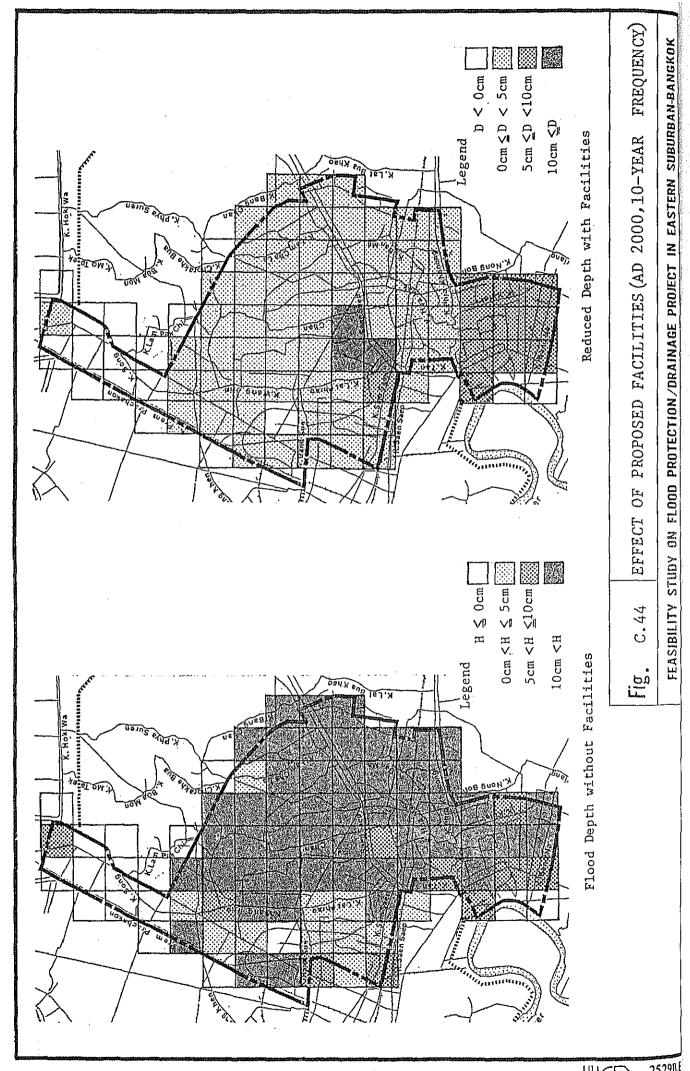


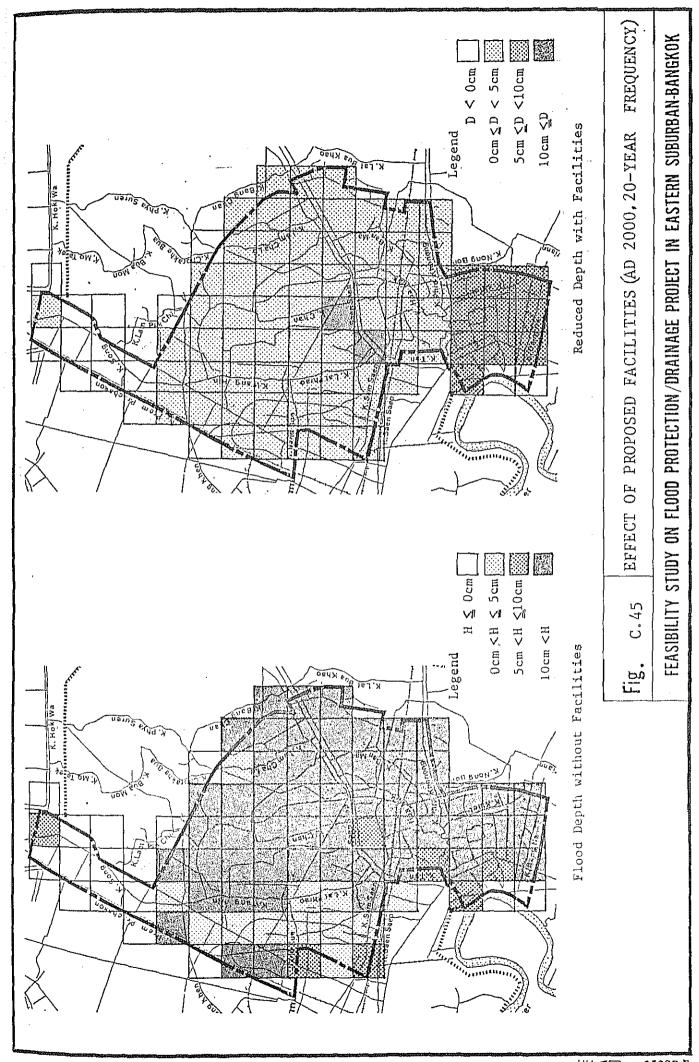


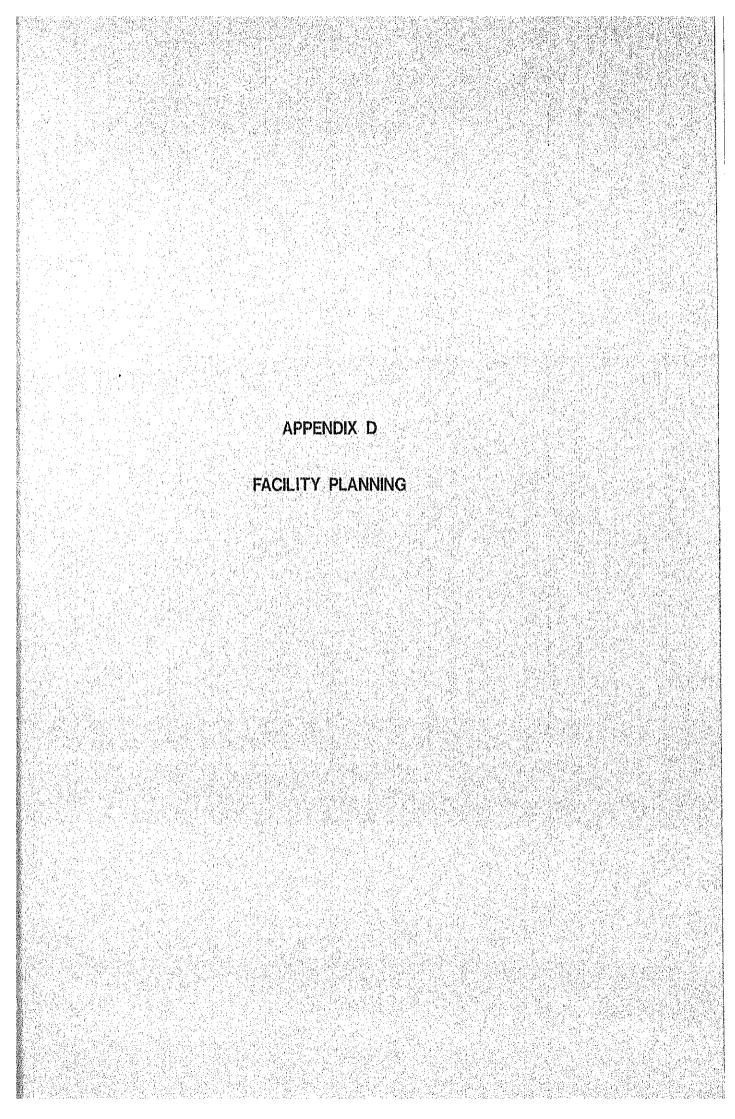












APPENDIX D FACILITY PLANNING

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APPENDIX D FACILITY PLANNING

1. FLOOD PROTECTION BARRIER

1.1 Along the Chao Phraya River

The 3.4 km length barrier is divided into 3 sections from A to C depending on the land use conditions along the river as shown in Fig. C.2. The existing facilities located along the river bank for each section are as follows:

Section A: Quay wall of National Petroleum Authority, concrete

panel wall type,

length of 1.25 km

Section B: Retaining wall of private companies, concrete panel

wall type, length of 1.00 km

Section C: Retaining wall of Navy, concrete panel wall type,

length of 1.15 km

Considering the existing conditions of structure and land use, two types (concrete wall and earth embankment) are envisaged as the structural alternatives for 3.4 km long barrier. Under the consideration of utilizing the existing retaining wall as much as possible and the existing land use, the concrete wall type is adopted in this project as shown in Fig.D.3.

For the flood barrier of the proposed concrete wall type, cut-off wall is planned to protect subsoil failure generated by the seepage due to the water level difference of in and out. Owing to the utilization of local materials, concrete sheet pile is adopted for the cut-off wall and the length of the pile is 7 m from the analysis of seepage. The cement mortar is planned to be grauted in joint of the each piles in order to make tight cut-off wall as shown in Fig.D.2. The structural dimensions are as follows:

Table D.1 Proposed Barrier Along Chao Phraya River

Barrier Type : Concrete wall with concrete sheet pile support

Length: 3.4 km Height: 1.5 m

1.2 Along Klong Tub Chang Bon

The east bank area of the Klong Tub Chang Bon of 1.7 km, where the barrier is proposed, is not urbanized at present and there remains swampy area. In the short distance of this section, there is the Klong Phra Khanong where several houses are directly faced to the klong. Considering the present site conditions, three types of barrier are envisaged as the structural alternatives as shown in Fig.D.3.

Alternative 1) Earth Embankment Type

Alternative 2) Concrete Wall Type

Alternative 3) Concrete Panel Wall Type

In case of Alternative 1 and 2, the barrier are constructed keeping away from the existing klong bank for the safety for bank slope failure.

Alternative 3) is constructed by vertical retaining wall, therefore, its construction cost is higher than others. Amongst the three types of the barrier structure, the earth embankment type is adopted in this project considering the existing land use conditions, economical and safety points of view. Top width of 5 meters of embankment is adopted for traffic use for the maintenance of the barrier and the klong as shown in Fig. D.2.